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NOTES

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING, EXCEPT THAT CORED SLAB UNITS HAVE BEEN DESIGNED FOR HS25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE MATERIAL SHOWN IN THE CROSS HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

SEE SPECIAL PROVISIONS FOR: SUBMITTAL OF WORKING DRAWINGS FALSEWORK AND FORMWORK CRANE SAFETY GROUT FOR STRUCTURES PRESTRESSED CONCRETE MEMBERS.

	RIP RAP CLASS II	FILTER FABRIC FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	127	142
END BENT 2	94	104

EXISTING BRID
(TO BE REMO
24.2' CLEAR ROADWAY, 3 SPANS SUPERSTRUCTURE: TIMBER SUBSTRUCTURE: TIMBER

BM #1: 78.57'LT. STA.12+68.94 -L-, EL.842.78



DRIVE PILES AT END BENTS 1 & 2 TO A REQUIRED BEARING CAPACITY OF 120 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENTS 1 & 2 IS 60 TONS PER PILE.

DRILLED PIERS AT BENT 1 ARE DESIGNED FOR BOTH SKIN FRICTION AND END BEARING. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 20 TSF.

DRILLED PIERS AT BENT 2 ARE DESIGNED FOR BOTH SKIN FRICTION AND END BEARING. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 35 TSF (LEFT) AND 20 TSF (RIGHT).

DRILLED PIERS AT BENTS 1 & 2 ARE DESIGNED FOR AN APPLIED LOAD OF 154 TONS EACH AT THE TOP OF THE COLUMN.

INSTALL DRILLED PIERS AT BENT 1 TO AN ELEVATION NO HIGHER THAN 827 FEET AND SATISFY THE REQUIRED END BEARING CAPACITY.

INSTALL DRILLED PIERS AT BENT 2 TO AN ELEVATION NO HIGHER THAN 830 FEET AND SATISFY THE REQUIRED END BEARING CAPACITY.

THE SCOUR CRITICAL ELEVATION FOR BENT 1 IS 832 FEET (LEFT) AND 831.5 FEET (RIGHT). SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

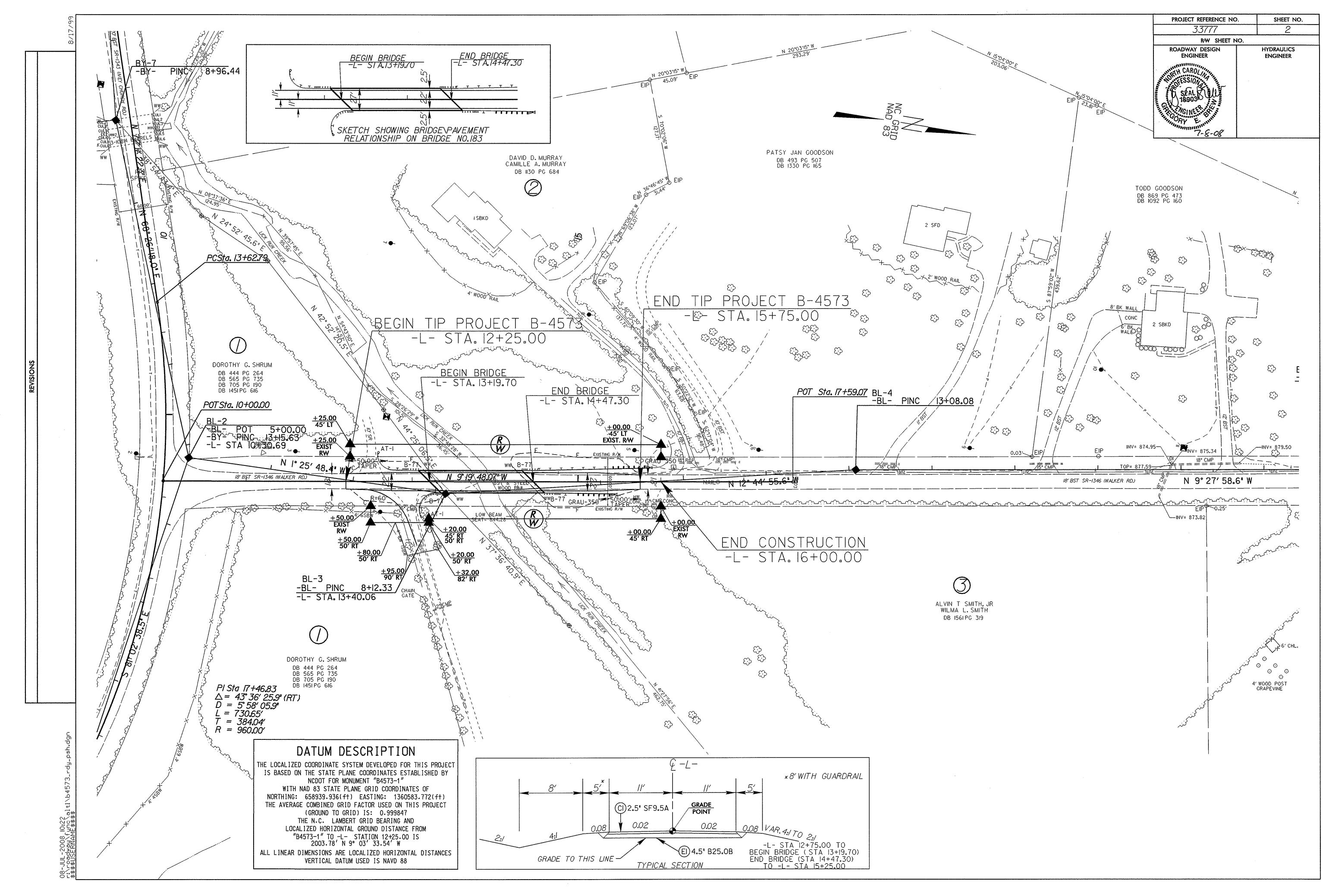
THE SCOUR CRITICAL ELEVATION FOR BENT 2 IS 835 FEET (LEFT) AND 834.5 FEET (RIGHT). SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

FOR DRILLED PIERS, SEE DRILLED PIERS SPECIAL PROVISION.

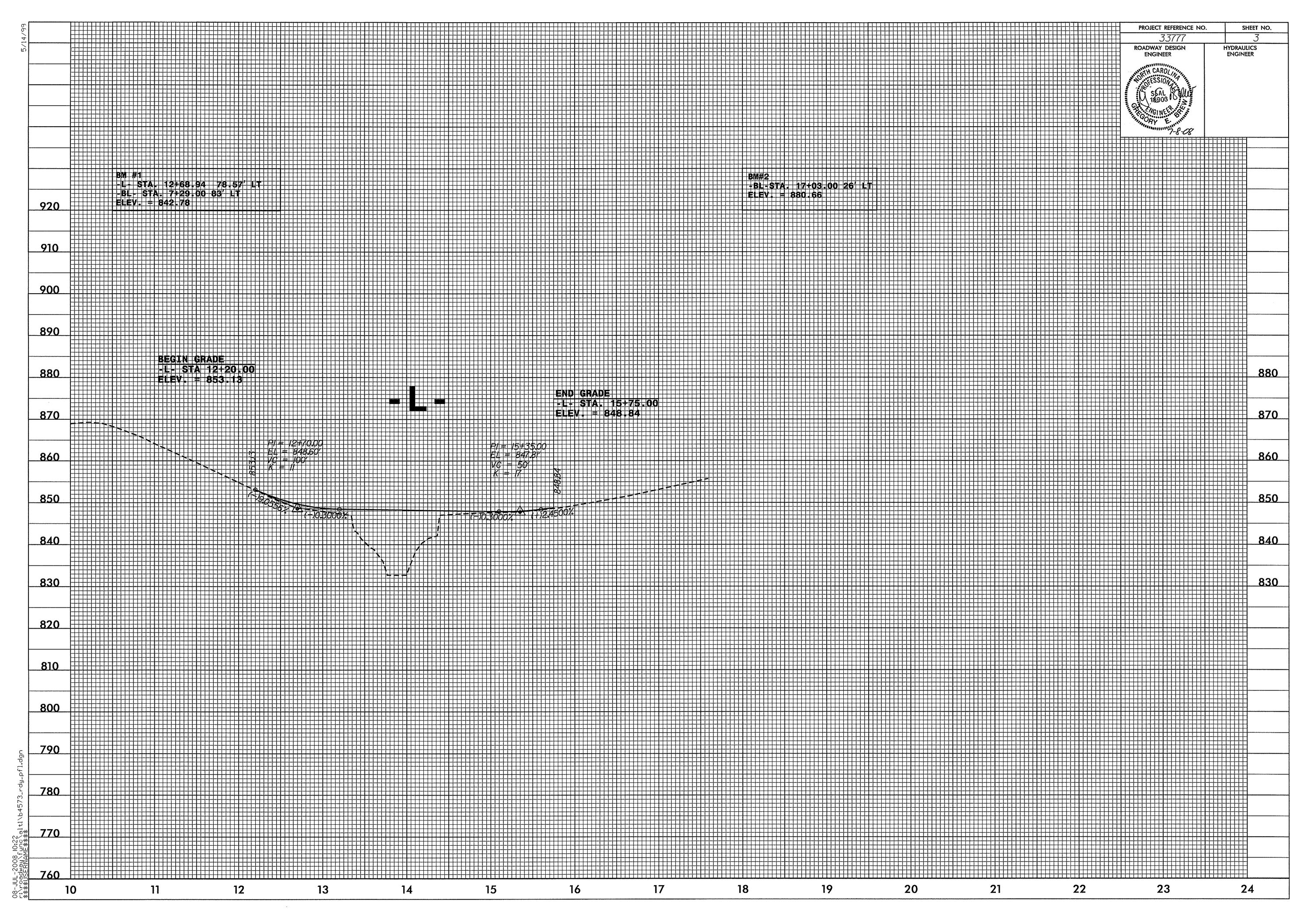
SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS.

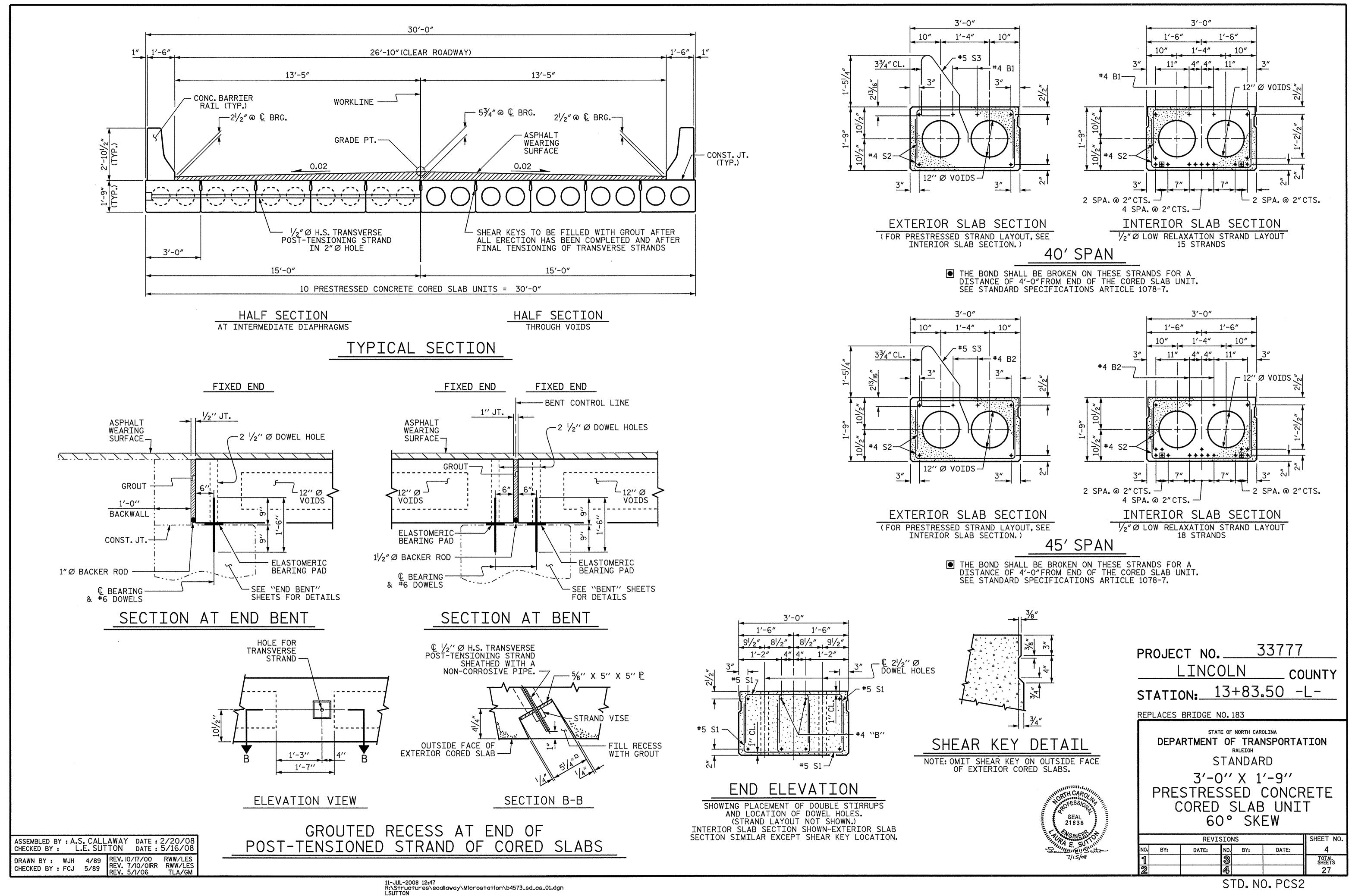
CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. SEE CROSSHOLE SONIC LOGGING SPECIAL PROVISION.

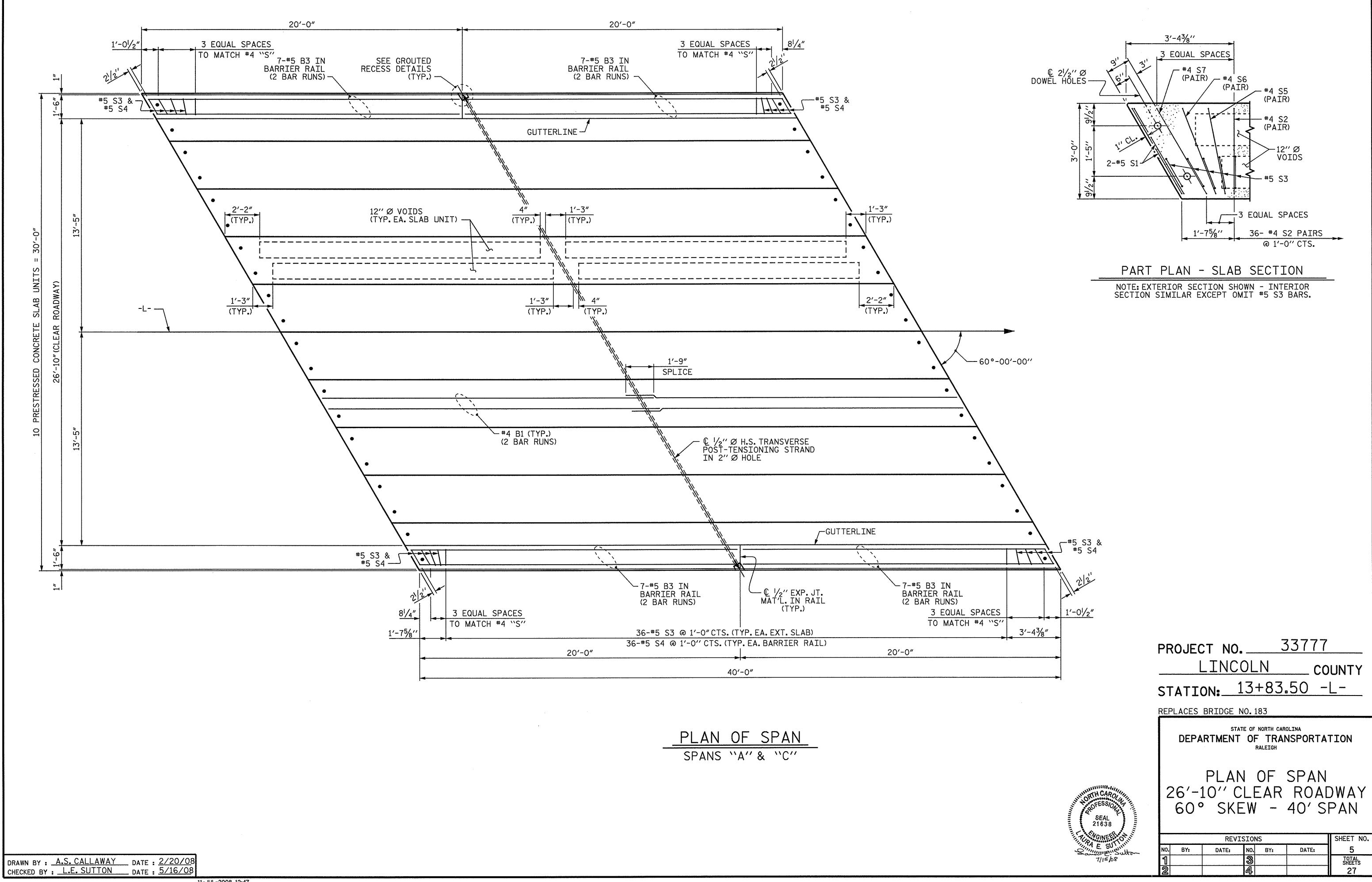
E NO. 183 ED) 35'-3'', 35'-0'', 35'-3'') DECK ON I-BEAMS	STATIO	T NO. LINCO DN: 1 BRIDGE NO	<u>0LN</u> .3+83	CO	UNTY
OVALUE SEAL 21638	G	RTMENT ENERA FOR E LICK F	RALEIGH	NSPORTA RAWIN OVER EEK ON TWEEN	IG
A CALLER SUTTINI	NO. BY:	REVIS	IONS NO. BY:	DATE:	SHEET NO.
y Sameling Sutton 9/29/08	1		3 4		TOTAL SHEETS 27



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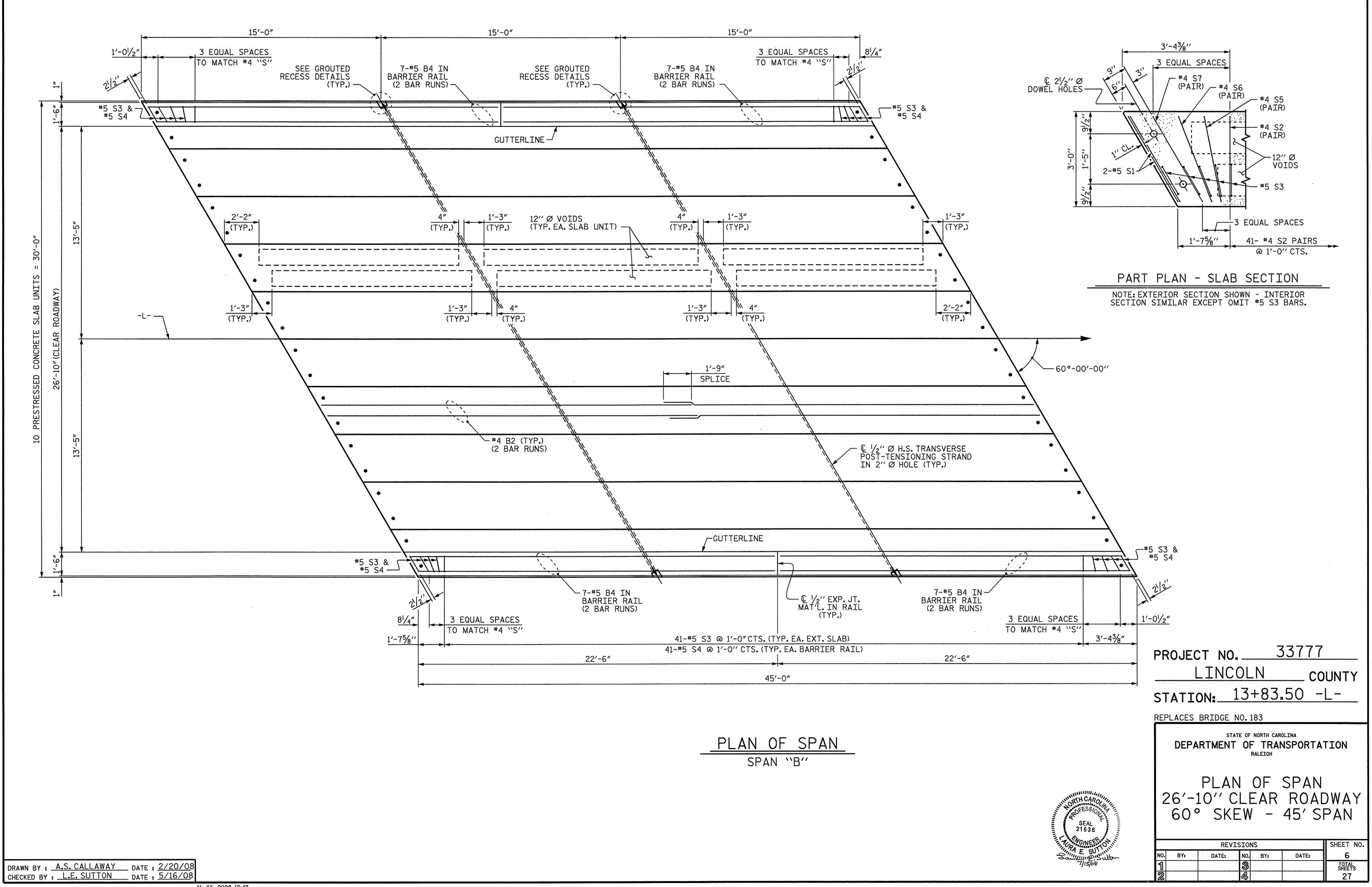


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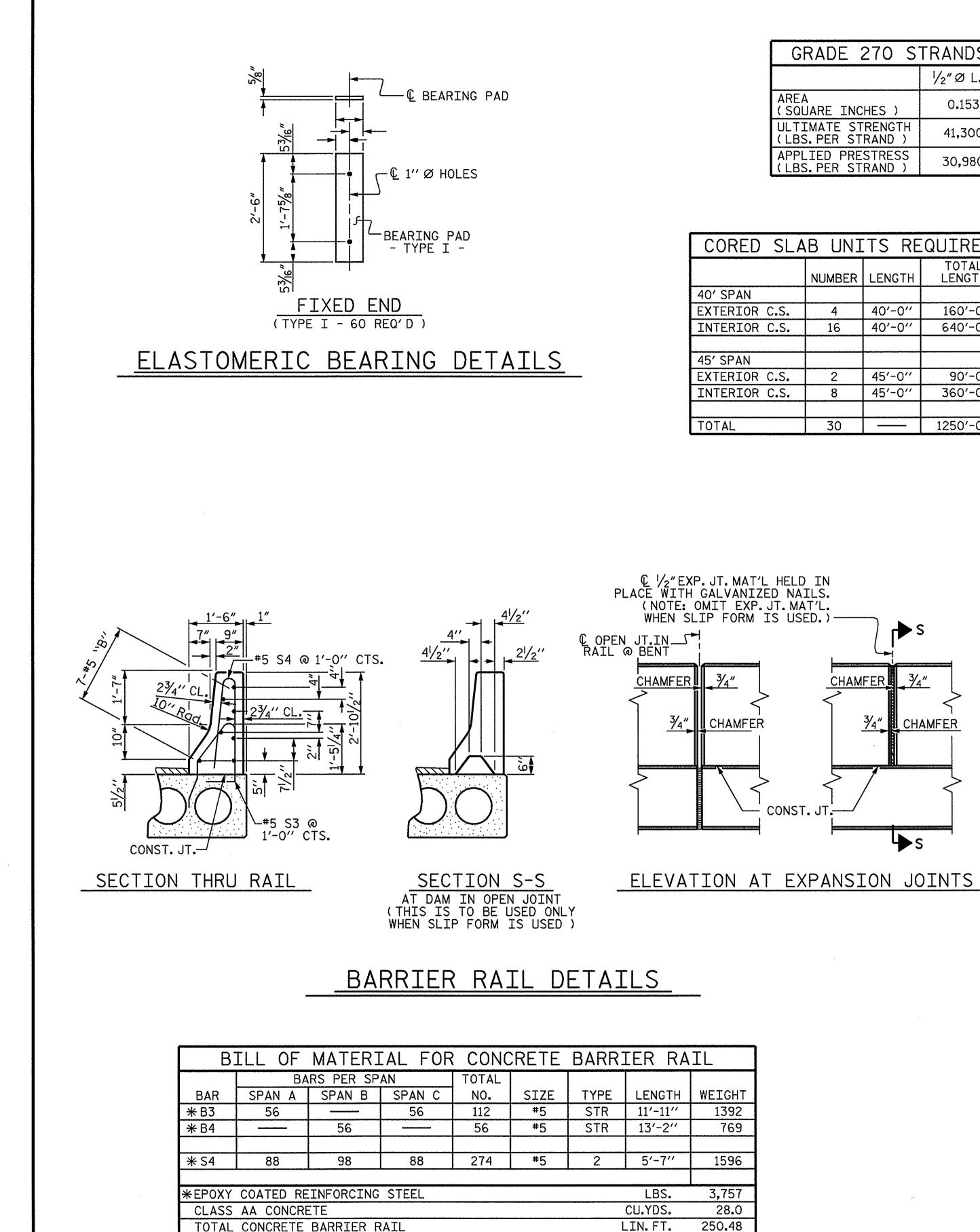
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ด้วิจึงที่มีมาตรงของสังษณะสายในสุขรับการให้เป็นสมัยในสมัยวันสังค์ไหว้านที่ได้มีสามารงสามารงการการการการการการก -



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ASSEMBLED BY : A.S. CALL CHECKED BY : L.E. SUT		2/20/08 5/16/08
	REV. 7/10/01 REV. 5/7/03RRR REV. 5/1/06	RWW/LES RWW/JTE TLA/GM

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BAR TYPES

GRADE 270 STRANDS			
	1∕2″Ø L.R.		
AREA (SQUARE INCHES)	0.153		
ULTIMATE STRENGTH (LBS.PER STRAND)	41,300		
APPLIED PRESTRESS (LBS.PER STRAND)	30,980		

SLAB UNITS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH		
C.S.	4	40'-0''	160'-0''		
C.S.	16	40'-0''	640'-0''		
C.S.	2	45'-0''	90'-0''		
C.S.	8	45'-0''	360'-0''		
	30		1250'-0''		

ALL BAR DIMENSIONS ARE OUT TO OUT.							
		ALL BA	L OF		IAL FO		
	(DNE 4	O' COF	RED SLA	B SECT	ION	
				EXTERIC	DR UNIT	INTERIC	DR UNIT
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	4	#4	STR	20'-9''	55	20'-9''	55
				A1 77/1	76	AI 711	70
<u>\$1</u>	8 72	#5 #4	<u>3</u>	<u>4'-3''</u> 5'-4''	35 257	<u>4'-3''</u> 5'-4''	<u> </u>
S2 * S3	44	# <u>4</u> #5	1	5'-4''	245	<u> </u>	
<u>↑ 35</u> S5	4	#4	3	5'-5''	14	5'-5''	14
<u>56</u>	4	#4	3	5'-7''	15	5'-7''	15
S7	4	#4	3	5'-9''	15	5'-9''	15
	RCING S	STEEL		LBS.	391	LBS.	391
*EPOXY	COATED	STEEL		LBS.	245		·····
	S.I. CO			CU. YDS.	5.7	CU. YDS.	5.7
	.R. STRA			NO.	15	NO.	15
		BTI	L OF	MATER	TAL FO	R	
	(ONE 4					
				EXTERIC		INTERIC	DR UNIT
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B2	4	#4	STR	23'-3''	62	23'-3''	62
S1	8	#5	3	4'-3''	35	4'-3''	35
S2	82	#4	3	5'-4''	292	5'-4''	292
<u>* S3</u>	49	#5 #4	1	5'-4'' 5'-5''	273	<u> </u>	1 /
S5 S6	4	#4	<u> </u>	5'-5''	<u>14</u> 15	5'-5''	<u> </u>
56 S7	4	#4	3	5'-9''	15	5'-9''	15
		·····					
REINFO	RCING S	STEEL		LBS.	433	LBS.	433
*EPOXY					077		
	P.S.I. CO	Ale		LBS. CU. YDS.	<u> </u>	CU. YDS.	6.4
				NO.	18	NO.	18
				• • • • •			<u>+</u> ~

DEAD LOAD DEFLECTION AND CAMBER				
	3'-0" × 1'-9" 1/2" Ø L.R. STRAND			
	40' SPAN	45' SPAN		
CAMBER (SLAB ALONE IN PLACE)	0.942"	1.389''		
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	0.134"	0.217''		
FINAL CAMBER	0.808''	1.172″		
** INCLUDES FUTURE WEARING SURFACE.				

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT. DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

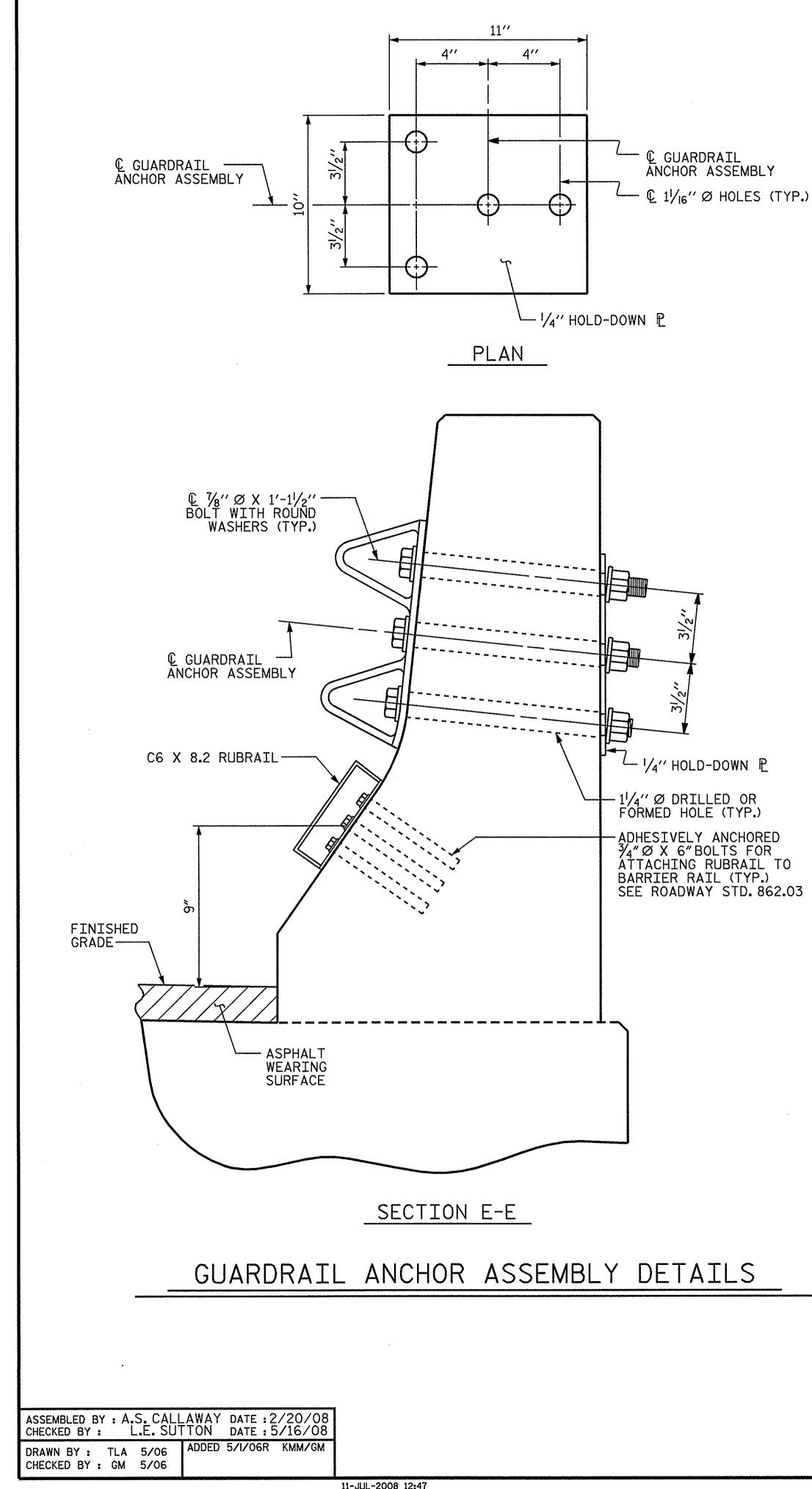
APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4000 PSI.

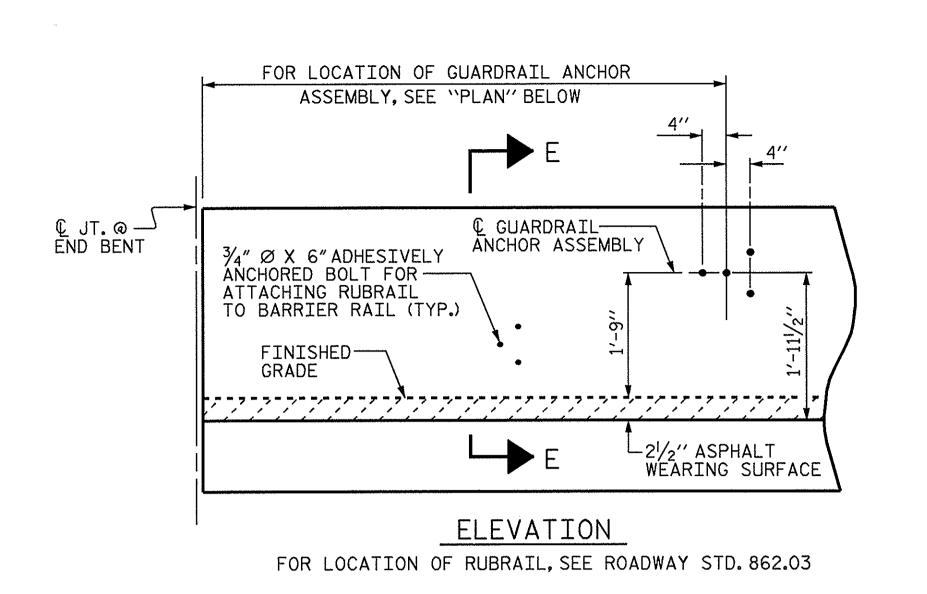
PROJECT NO. <u>33777</u> LINCOLN COUNTY					
STATION: 13+83.50 -L-					
REPLACES BRIDGE NO.183					
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 60° SKEW					
REVISIONS SHEET NO.					
NO. BY: DATE: NO. BY: DATE: 7 1 3 TOTAL SHEETS 2 4 27					
STD. NO. PCS3					

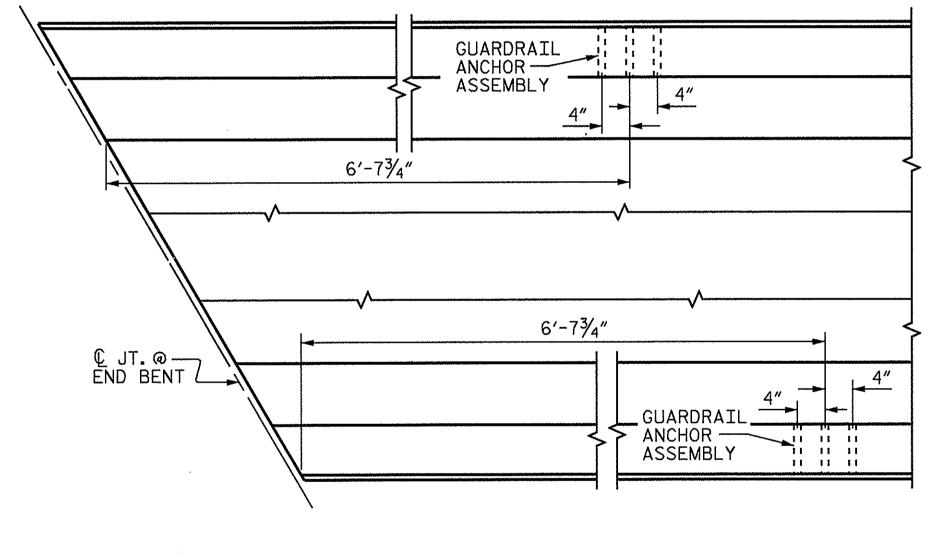




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PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

4 -BOLTS.

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 4 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

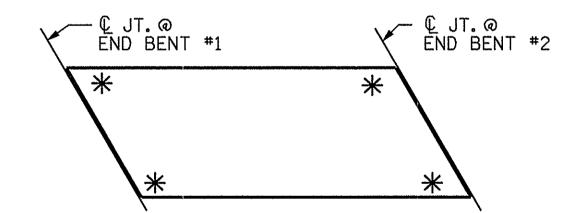
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE $\frac{3}{4}$ " Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

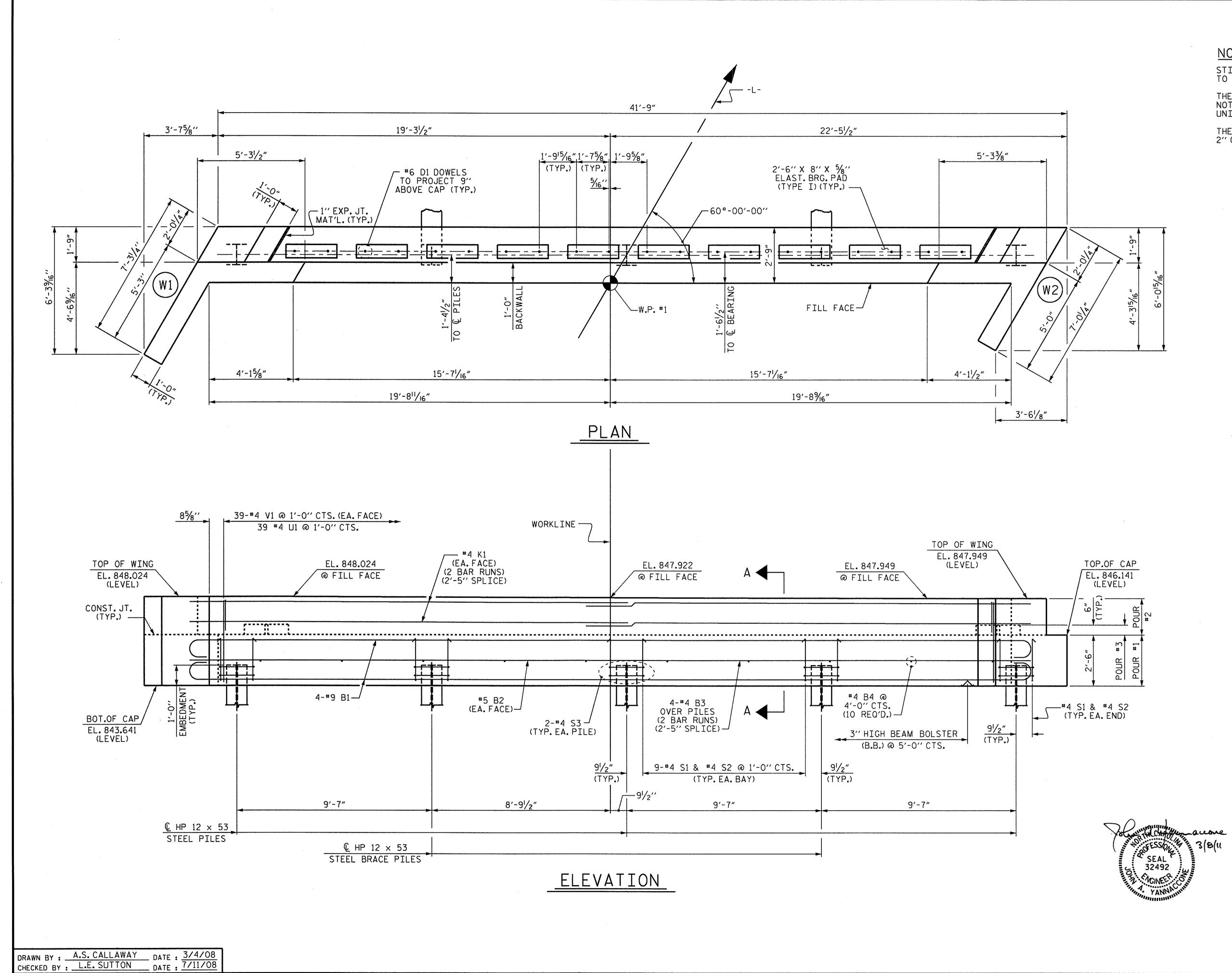


SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

	PROJEC	T NO.		33777		
	L	INC	OLN	CO	UNTY	
	STATION: 13+83.50 -L-					
	REPLACES	BRIDGE	NO. 183			
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
	STANDARD					
				NCHOF		
NUMBER OFESSION	FC	R BA	ARRIE	IR RA	IL	
SEAL 21638						
TINGINEER		REVI	SIONS		SHEET NO.	
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7/15/08	1	· · · · · · · · · · · · · · · · · · ·	3 4		TOTAL SHEETS 27	
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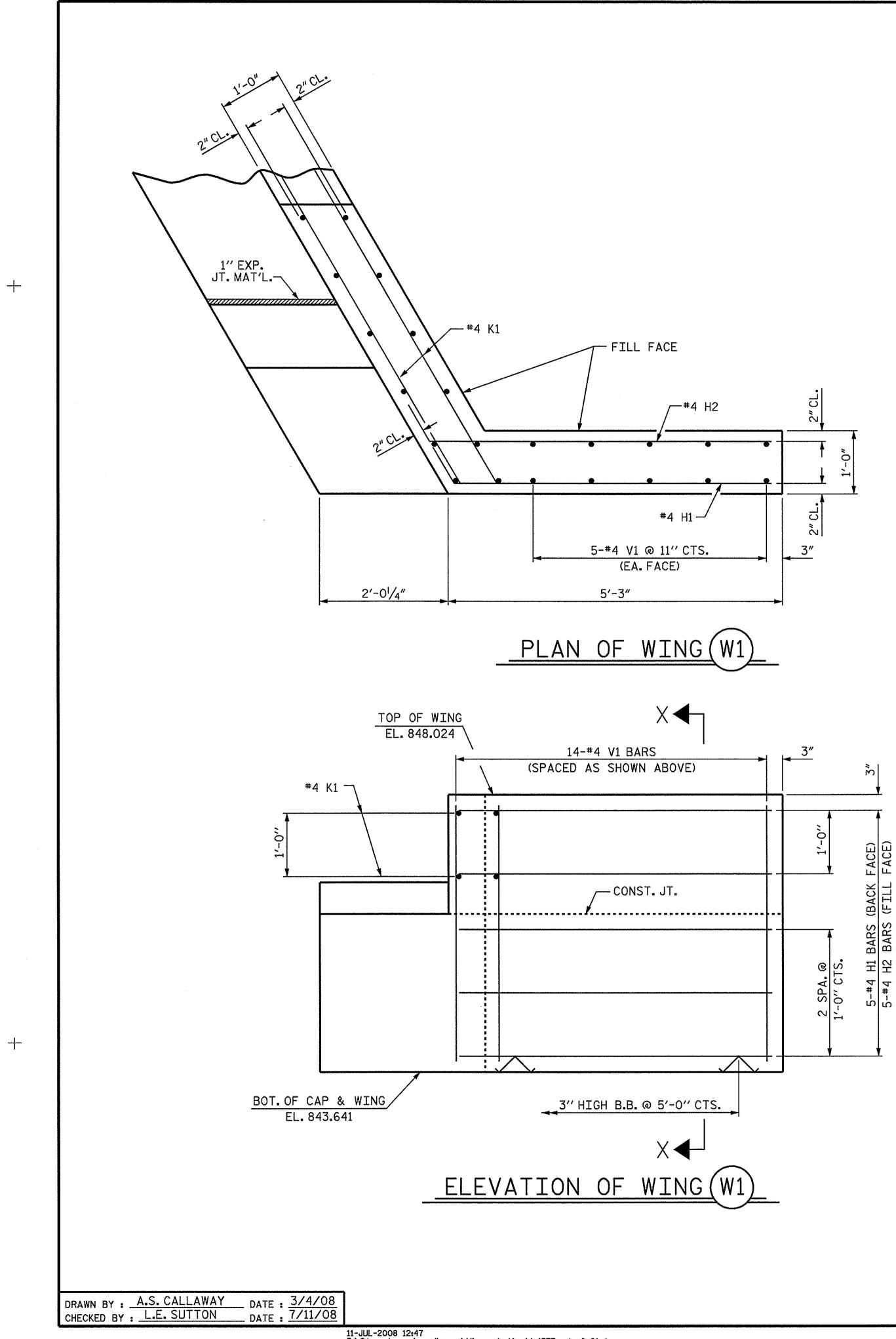
NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

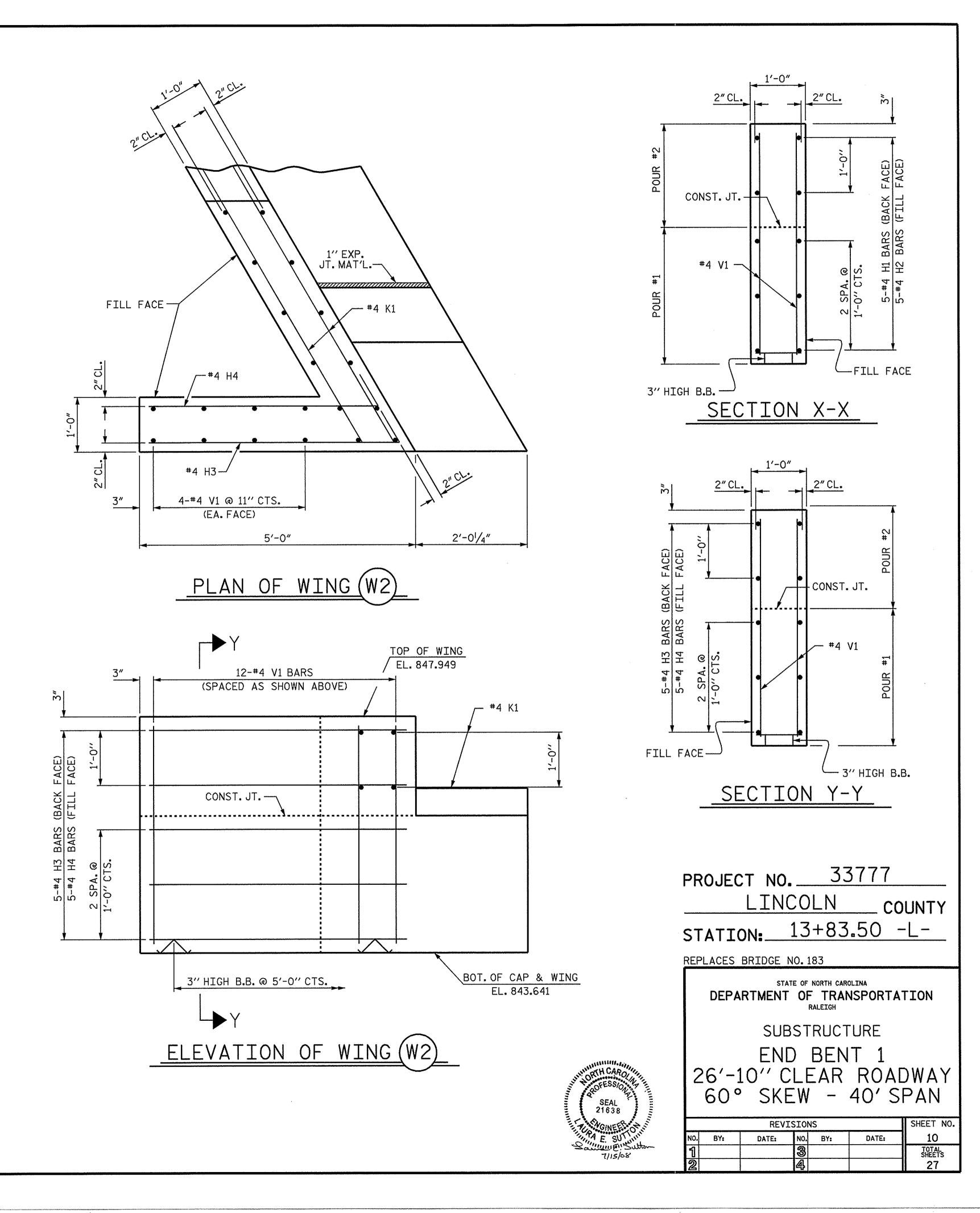
THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

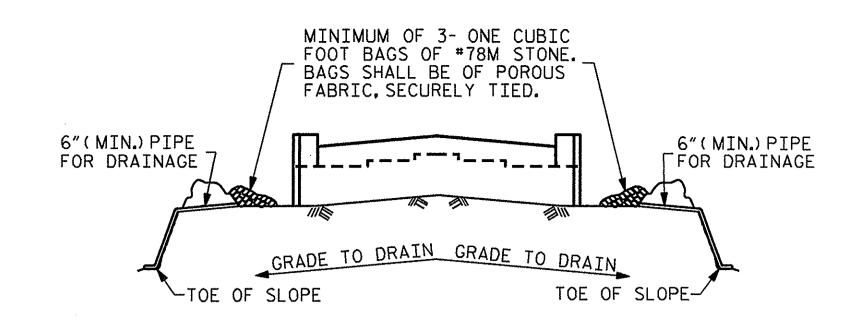
THE #4 VI BARS IN THE BACKWALL SHALL BE PLACED 2" CLEAR FROM THE TOP OF THE BACKWALL.

PROJECT NO. <u>33777</u> LINCOLN COUNTY					
STATION: 13+83.50 -L-					
REPLACES BRIDGE NO.183					
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE END BENT 1 26'-10'' CLEAR ROADWAY 60° SKEW - 40' SPAN					
REVISIONS SHEET NO NO. BY: DATE: NO. BY: DATE: 9					
Inc. DATE NO. DATE J 1 3 TOTAL SHEETS 2 4 27					



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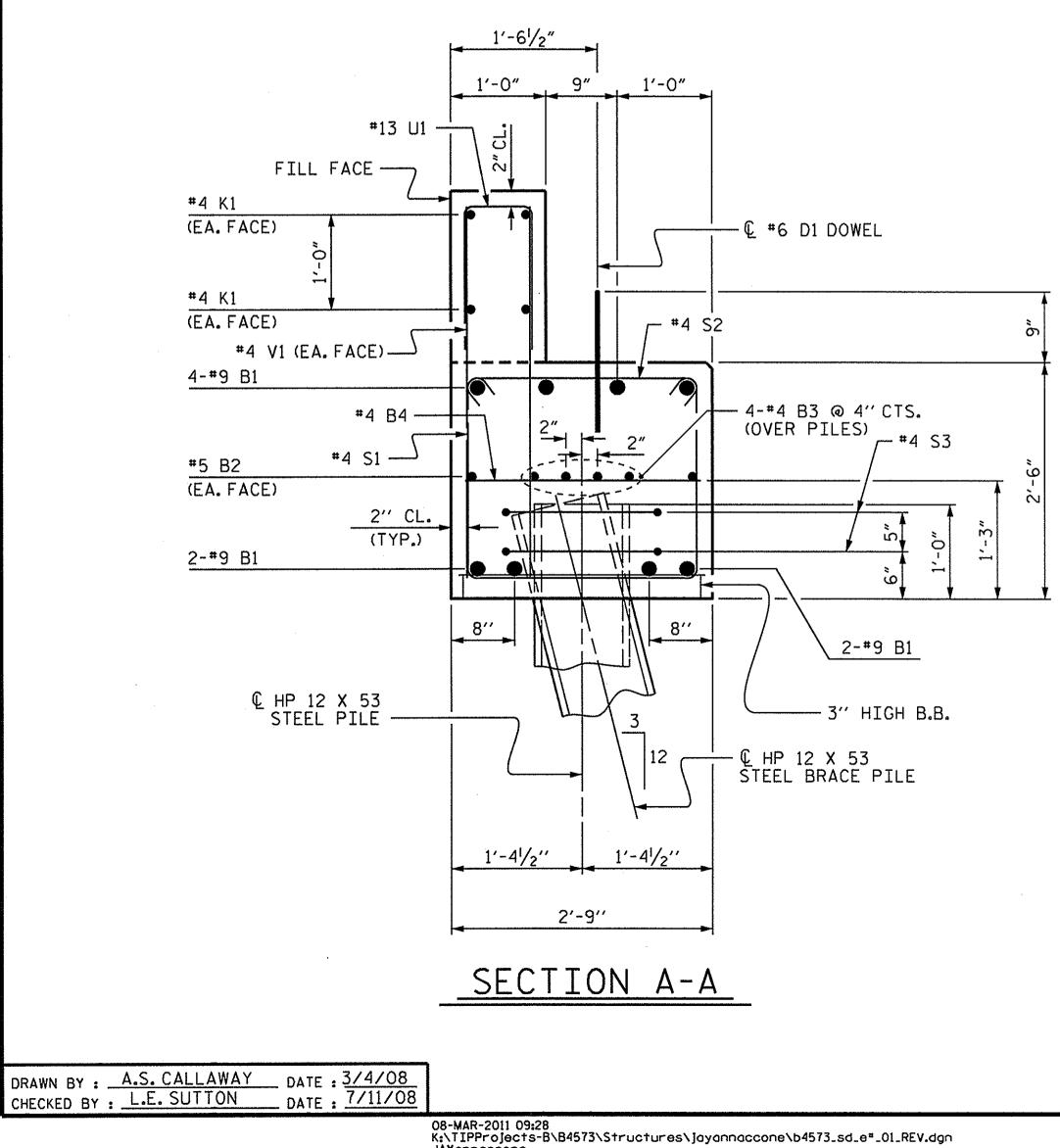
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

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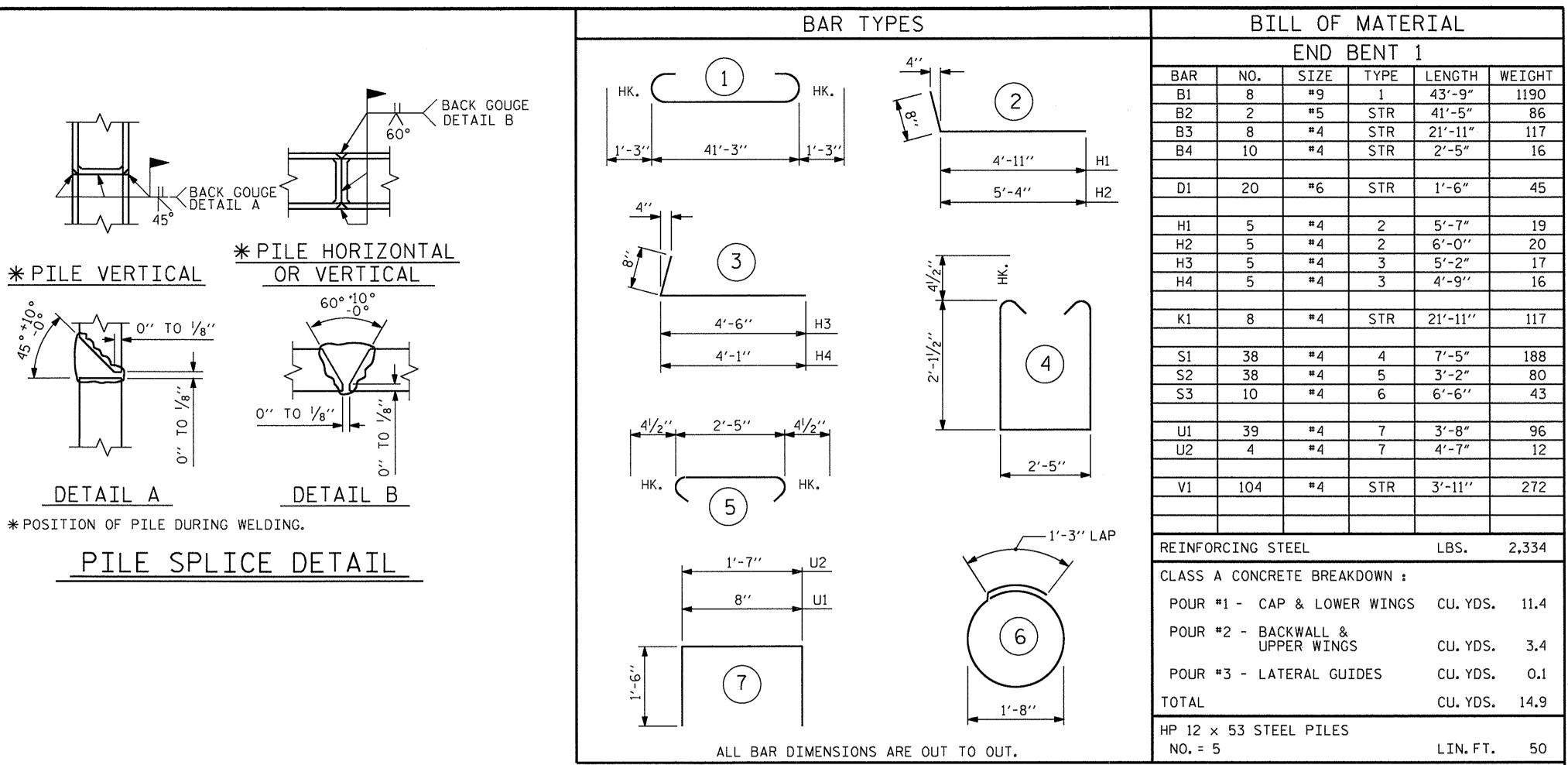
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

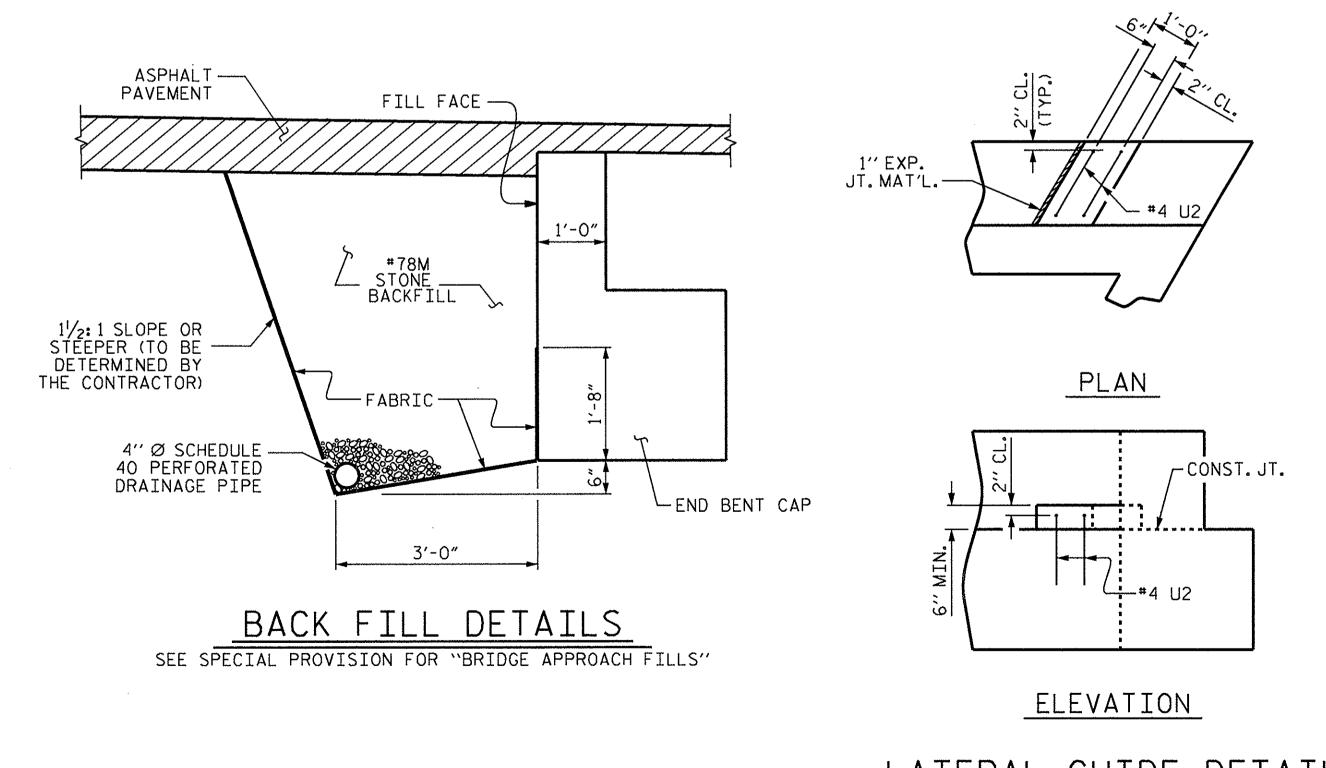
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

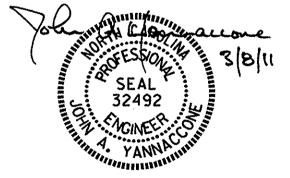


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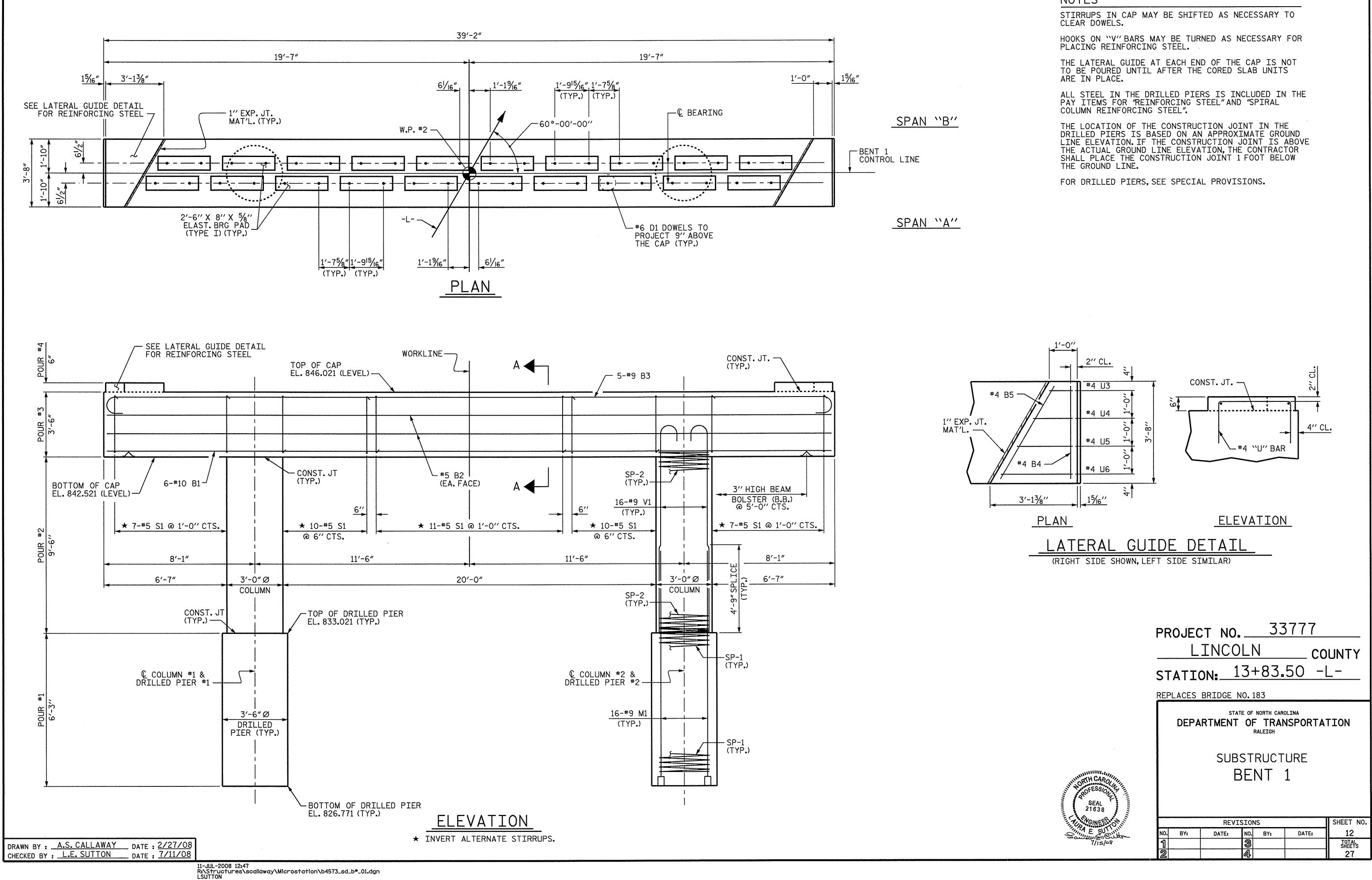


(RIGHT SIDE SHOWN, LEFT SIDE SIMILAR)



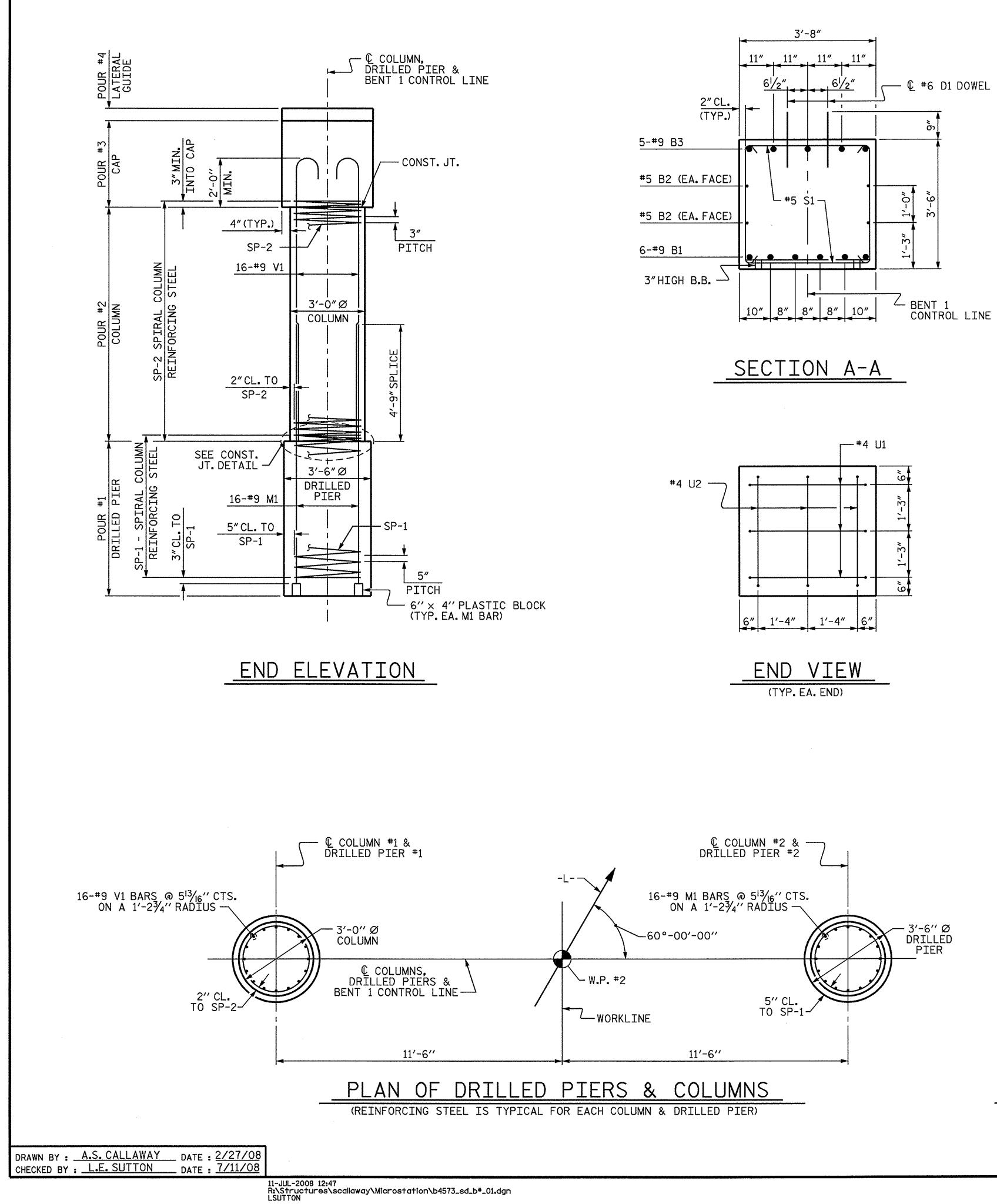
ATERAL GUIDE DETAIL

PROJECT NO. <u>33777</u> LINCOLN co	UNTY
STATION: 13+83.50 -	
REPLACES BRIDGE NO.183	
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTA RALEIGH	TION
SUBSTRUCTURE END BENT 1 26'-10'' CLEAR ROAD 60° SKEW - 40' S	
REVISIONS	SHEET NO.
NO. BY: DATE: NO. BY: DATE: 1 3 4	11 TOTAL SHEETS 27

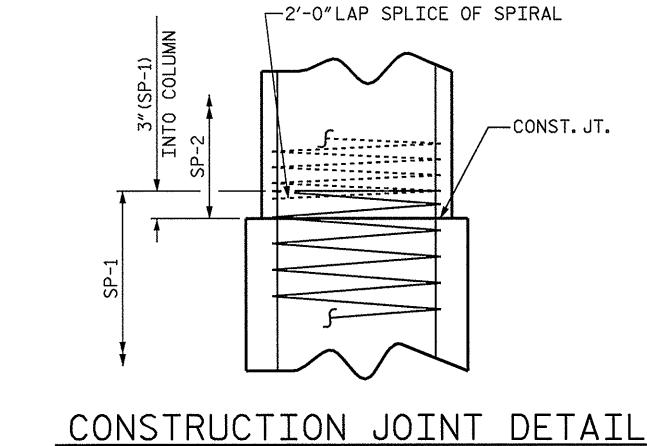


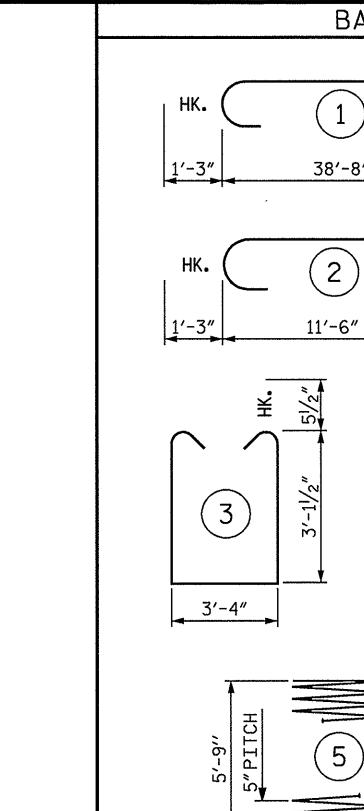
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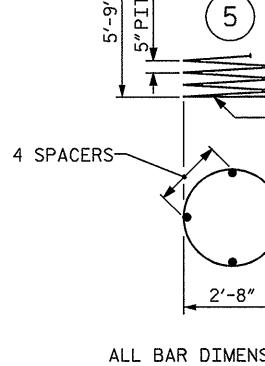
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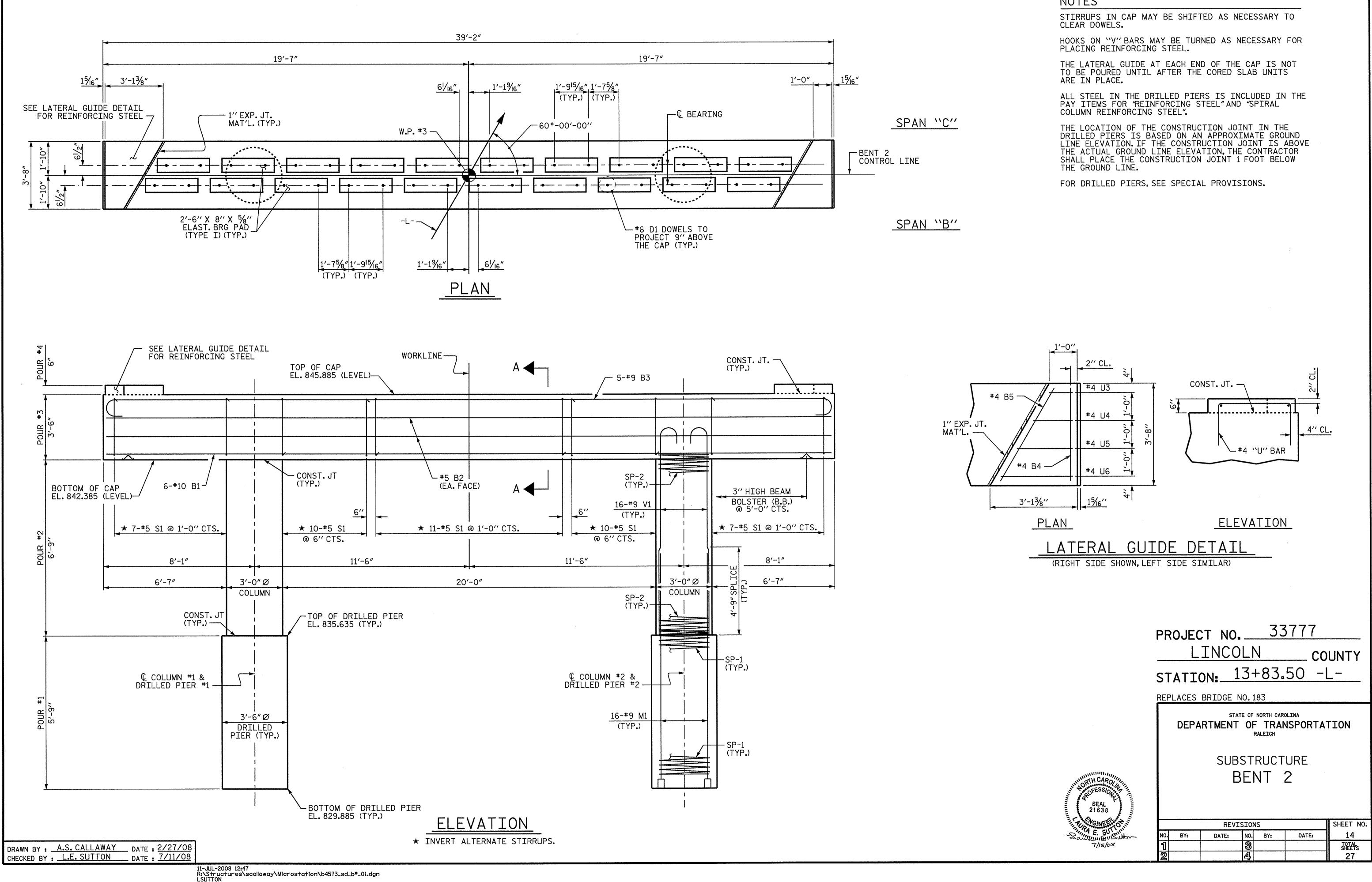


BAR B1 B2	BI		ENT :	TERIAL	
B1				1	
B1	I = N(f)	SIZE	TYPE	LENGTH	WEIGH
for the second se	NO. 6	<u>312E</u> #9	STR	38'-10''	1003
	4	#5	STR	38'-10''	1603
B3	5	<u> </u>	1	41'-2''	700
B4	2	#4	STR	3'-4''	4
B5	2	#4	STR	3'-10"	5
		·			
D1	40	#6	STR	1'-6''	90
M1	32	#9	STR	10'-6''	1142
S1	45	#5	3	10'-6''	493
U1			4		25
U2	6	#4	4	6'-0''	24
<u>U3</u>	2	#4	4	3'-7''	5
<u>U4</u>	2	#4	4	4'-2''	6
			4		6
<u>U4</u>	2	#4	4	5'-4''	7
		<u> </u>		10/ 01	
V1	52	#Y	2	12'-9''	1387
	<u> </u>		L		
REINF	ORCING	STEEL		LBS.	5,059
SP-1	2	*	5	131'-8''	275
_ SP-2	2	**	6	338'-4''	452
REINF	ORCING	STEEL		LBS.	727
CLASS	A CON	CRETE			
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	D1 M1 S1 U1 U2 U3 U4 U3 U4 V1 REINF SP-1 SP-2 SPIRA REINF SP-2 SPIRA REINF CLASS POUF POUF TOTAL DRILL POUF TOTAL DRILL POUF 3'-6"(s NOT 3 CSL T * T S W ***T S W	D1 40 M1 32 S1 45 U1 6 U2 6 U3 2 U4 2 U3 2 U4 2 U3 2 U4 2 U4 2 SP-2 2 SPIRAL COLUR REINFORCING SP-1 2 SP-2 2 SPIRAL COLUR REINFORCING CLASS A CON POUR #3 - C POUR #3 - C POUR #3 - C POUR #4 - L TOTAL DRILLED PIEF POUR #1 - D 3'-6"Ø DRILL NOT IN SOIL 3'-6"Ø DRILL NOT IN SOIL CSL TUBES * THE SP-1 SHALL BE WIRE OR **THE SP-2 SHALL BE WIRE OR **THE SP-2 SHALL BE WIRE OR	D1 40 #6 M1 32 #9 S1 45 #5 U1 6 #4 U2 6 #4 U3 2 #4 U4 2 #4	D1 40 #6 STR M1 32 #9 STR S1 45 #5 3 U1 6 #4 4 U2 6 #4 4 U2 6 #4 4 U3 2 #4 4 U3 2 #4 4 U4 2 #4 4 V1 32 #9 2 EXEMPTION: STEEL SP-1 2 * 5 SP-2 2 ** 6 SP-2 2 ** 6 SP-2 2 ** 6 SP-1 2 * 5 SP-2 2 ** 6 SP-2 2 ** 6 SP-2 2 ** 6 SP-1 2 * 5 SP-2 2 ** 6 SP-2 2	D1 40 #6 STR 1'-6'' M1 32 #9 STR 10'-6'' S1 45 #5 3 10'-6'' U2 6 #4 4 6'-2'' U2 6 #4 4 6'-0'' U3 2 #4 4 3'-7'' U4 2 #4 4 4'-2'' U3 2 #4 4 4'-2'' U4 2 #4 4 4'-9'' U4 2 #4 4 4'-9'' U4 2 #4 4 4'-9'' U4 2 #4 4 5'-4'' V1 32 #9 2 12'-9'' REINFORCING STEEL LBS. ES CL.YDS. SPIRAL COLUMN REINFORCING STEEL LBS. CLASS A CONCRETE POUR #3 - CAP CU.YDS. POUR #1 - DRILLED PIER CONCRETE POUR #1 - DRILLED PIERS LIN.FT.

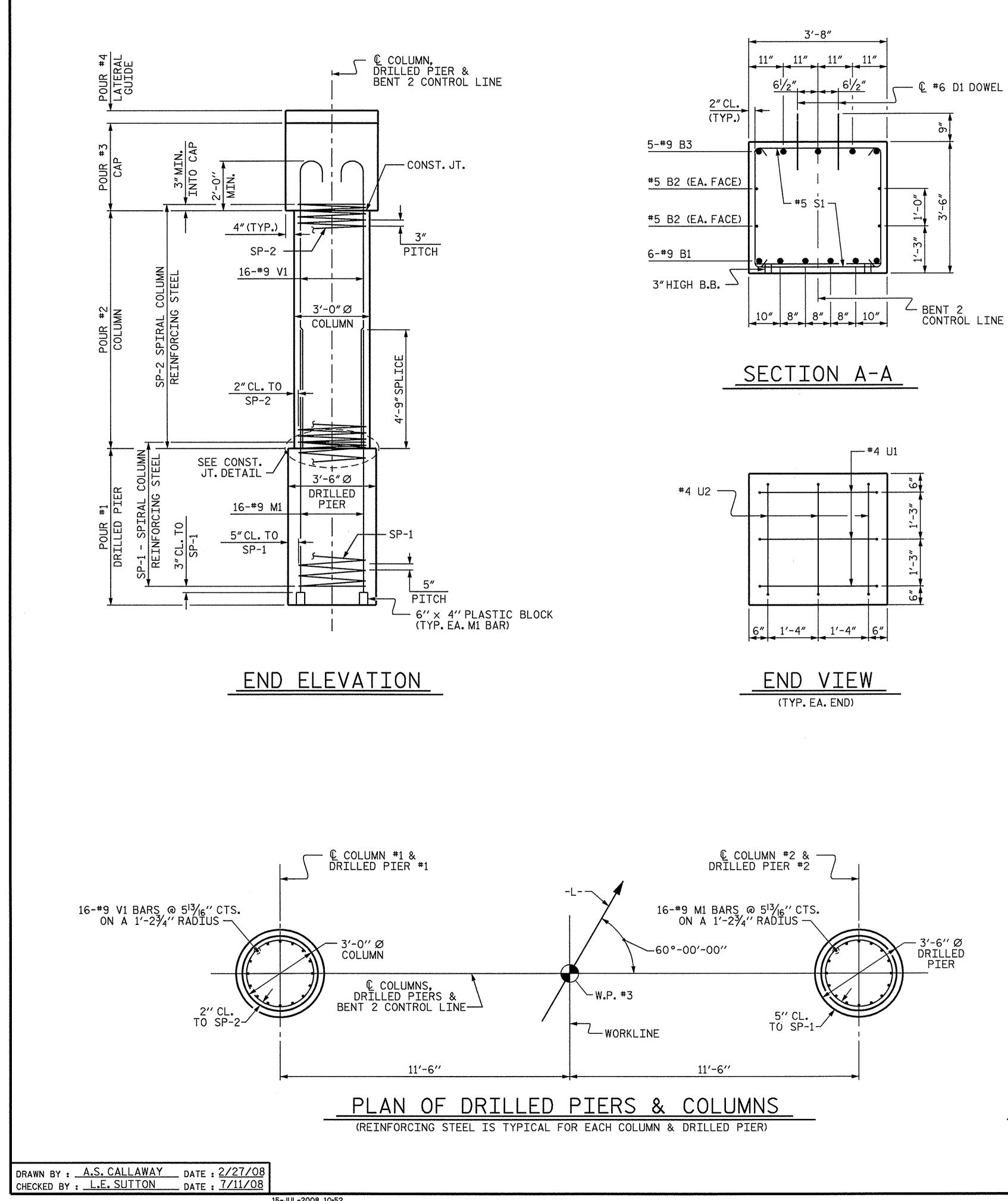
BENT 1

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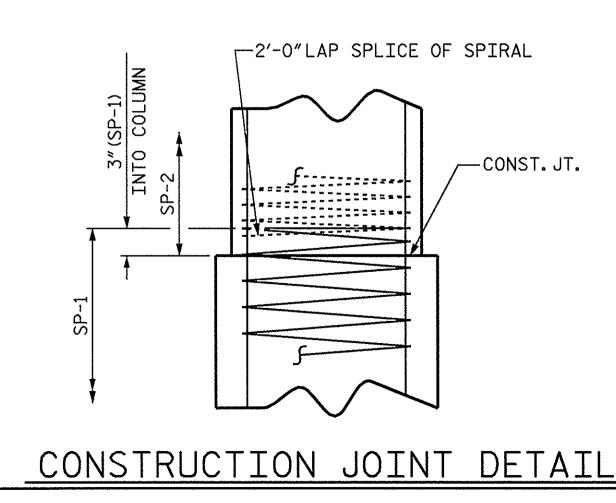


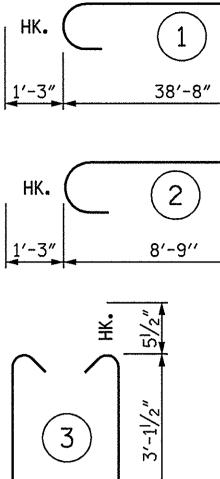


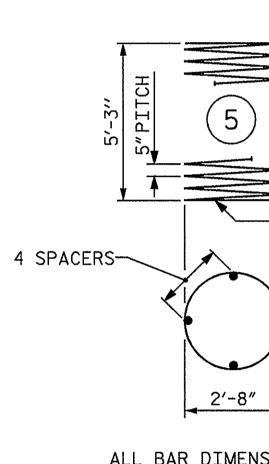
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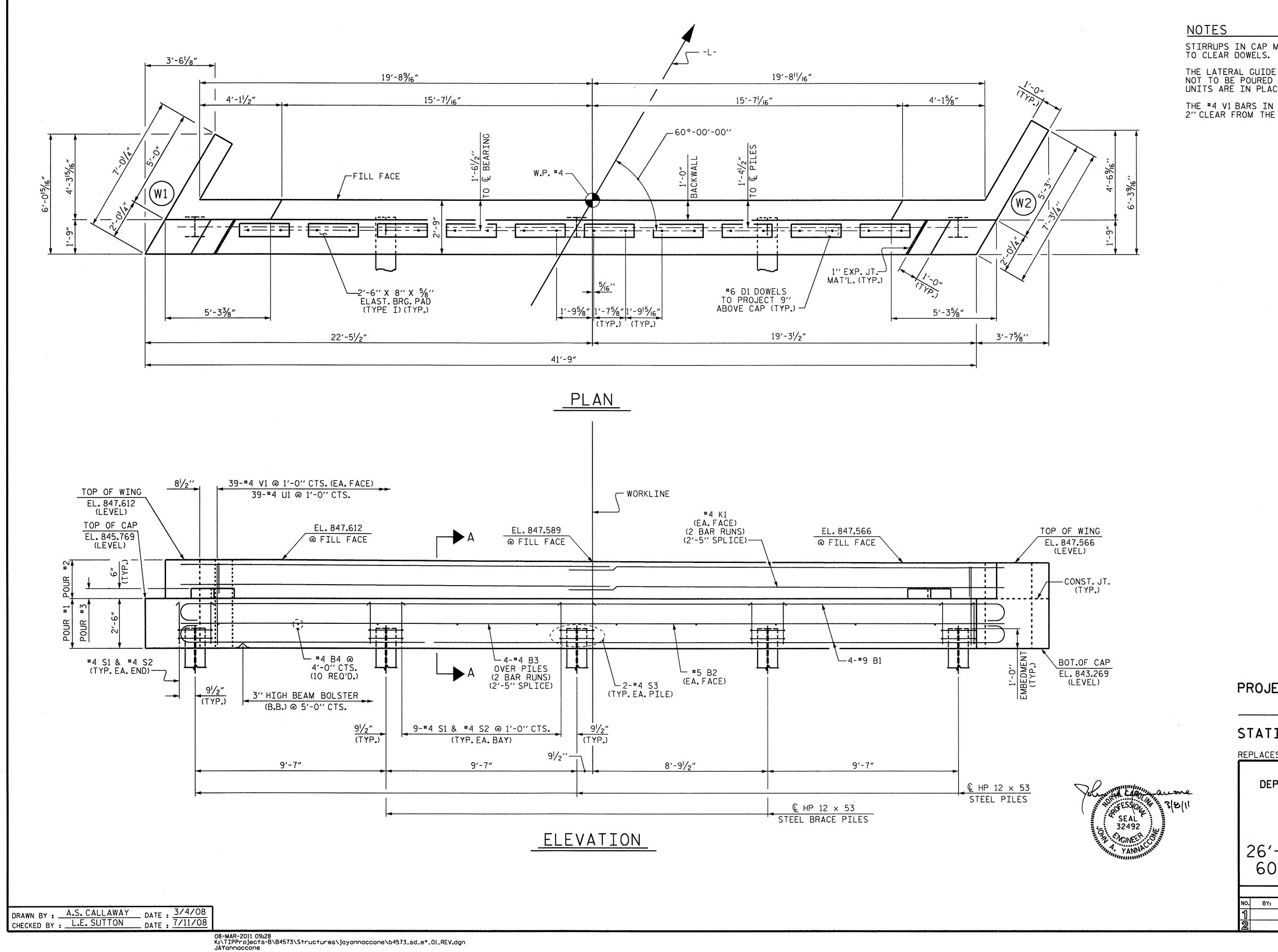
3'-4″

ALL BAR DIMENS

BAR TYPES		BI	LL OF	- MA	TERIAL	
			BI	ENT 2	2	
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
1)) HK.	B1	6	#9	SIR	38'-10''	1003
	B2	4	#5	STR	38'-10''	162
·-8″ <u>1</u> ′-3″	B3	5	#9	1	41'-2''	700
	B4	2	#4	STR	3'-4''	4
	B5	2	#4	STR	3'-10''	5
<u>U6 2'-4"</u>	D1	40	#6	STR	1'-6''	90
U5 1'-9"		70	#0	CTD.	10/ 0//	1000
	M1	32	#9	STR	10'-0''	1088
	S1	45	#5	3	10'-6''	493
<u>U3</u> 7″				<u>J</u>	10 -0	33
U2 3'-0"		6	#4	4	6'-2''	25
U1 3'-2"	U2	6	#4	4	6'-0''	24
	U3	2	#4	4	3'-7''	5
	U4	2	#4	4	4'-2''	6
	U3	2	#4	4	4'-9''	6
	U4	2	#4	4	5'-4''	7
×9-,1,-0,						
· ·	V1	32	#9	2	10'-0''	1088
	REINF	ORCING	STEEL		LBS.	4,706
	SP-1	2	*	5	123'-5''	257
1 1/2 EXTRA TURNS	SP-2	2	**	6	247'-7''	331
0, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,		L COLUI ORCING			LBS.	588
	CLASS	A CON	CRETE			
	POUR	#2 - (OLUMNS	5	CU. YDS	. 3.5
	POUR	#3 - (CAP		CU. YDS	. 18.6
1 1/2 EXTRA TURNS			ATERAL	GUIDE	S CU. YDS	
4 SPACERS —	TOTAL	·			CU. YDS	. 22.4
	DRILL	ED PIE	R QUAN	TITIES:		
γ $\langle \prime \rangle$			R CONCE			
• • •			RILLED		CU. YDS	. 4.1
		Ø DRILL	ED PIE		LIN.FT	
-8"	3'-6" &		ED PIE	RS		
ACNETANC ADD OUT TO OUT					LIN. FT	
MENSIONS ARE OUT TO OUT.	CSL T	URF2			LLN. FI	. 66.00
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	PROJEC	CT NO. _INC(33777	
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A LINE A	NO. BY:	REVIS	NO. BY:	DATE:	SHEET NO
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STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY

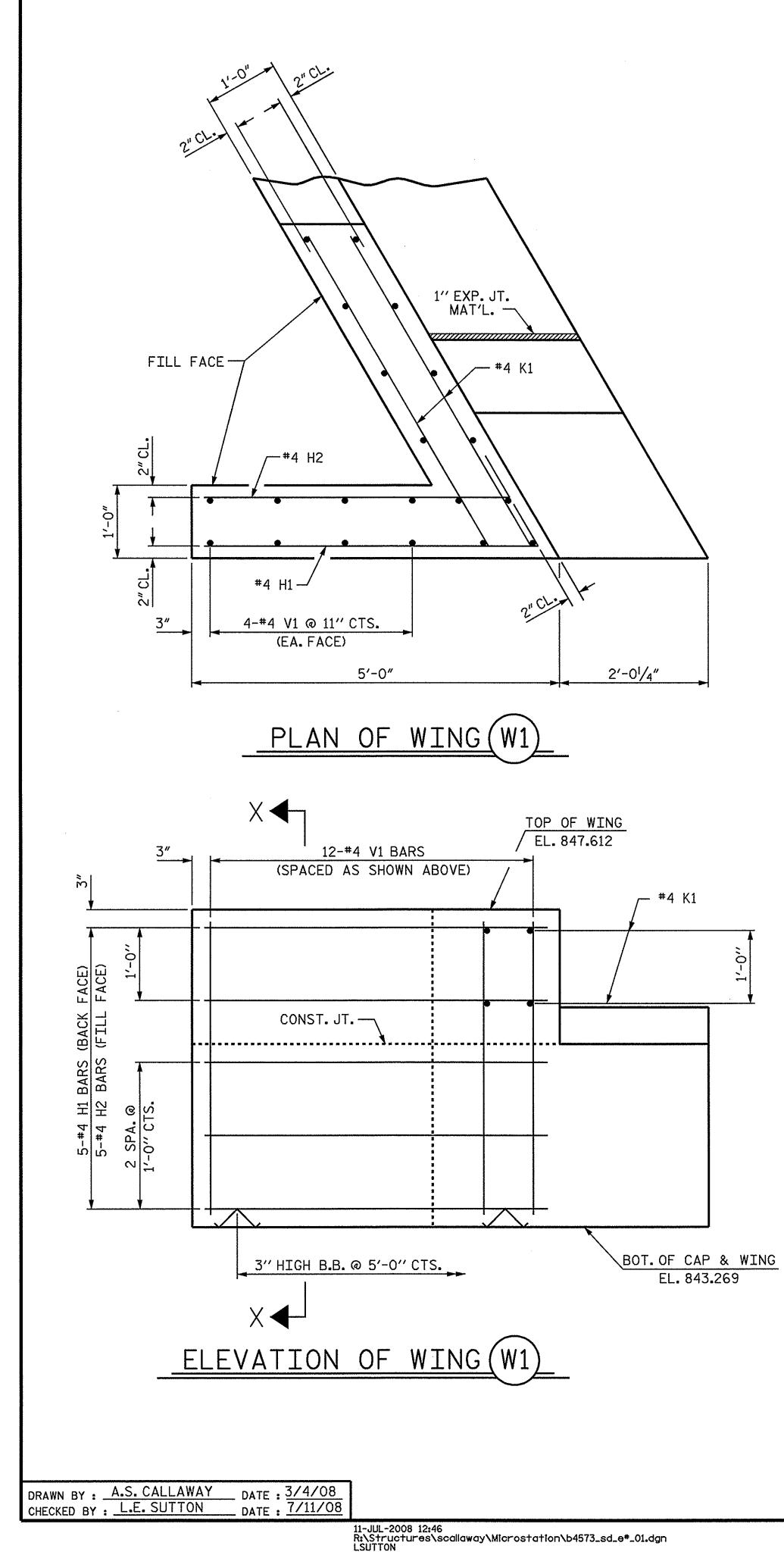
THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE #4 V1 BARS IN THE BACKWALL SHALL BE PLACED 2" CLEAR FROM THE TOP OF THE BACKWALL.

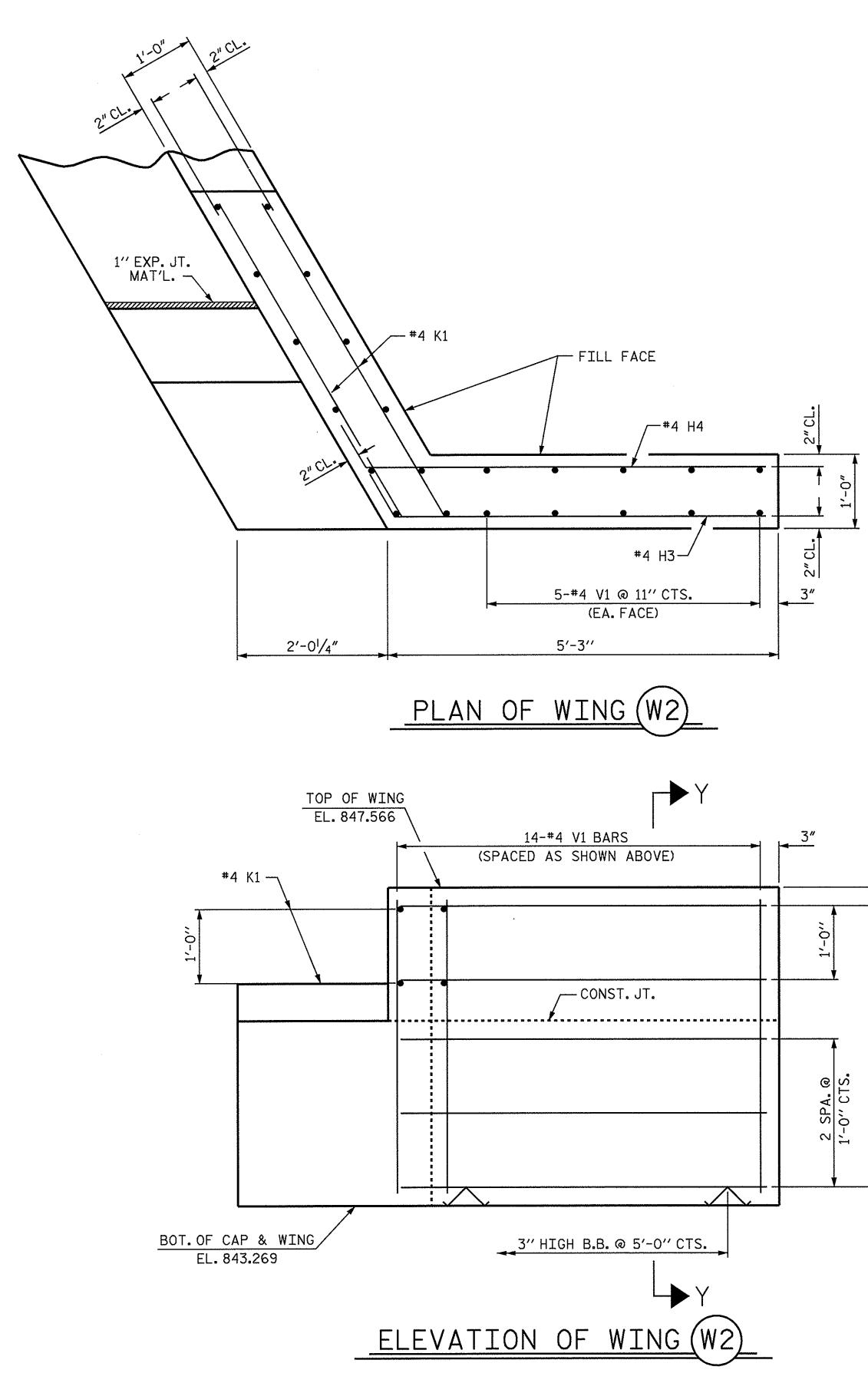
PROJECT NO. <u>33777</u> <u>LINCOLN</u> COUNTY STATION: <u>13+83.50</u> -L-
REPLACES BRIDGE NO. 183
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SUBSTRUCTURE END BENT 2
26'-10" CLEAR ROADWAY 60° SKEW - 40' SPAN
REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: 16
1 3 TOTAL SHEETS 2 4 27

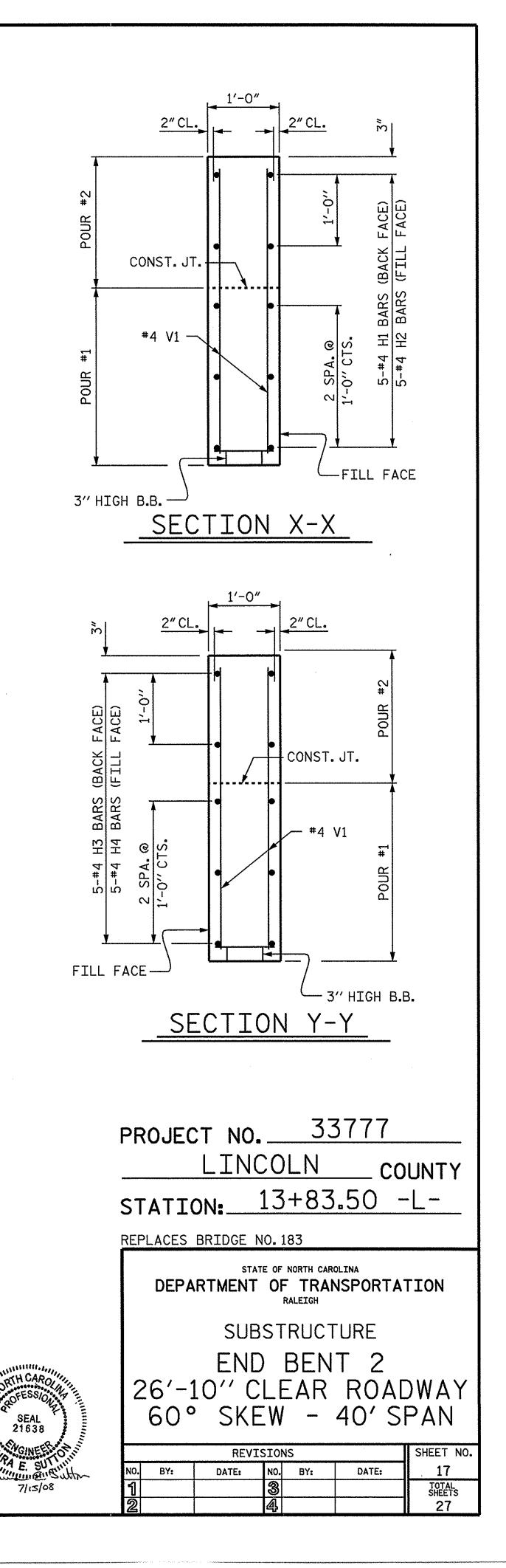
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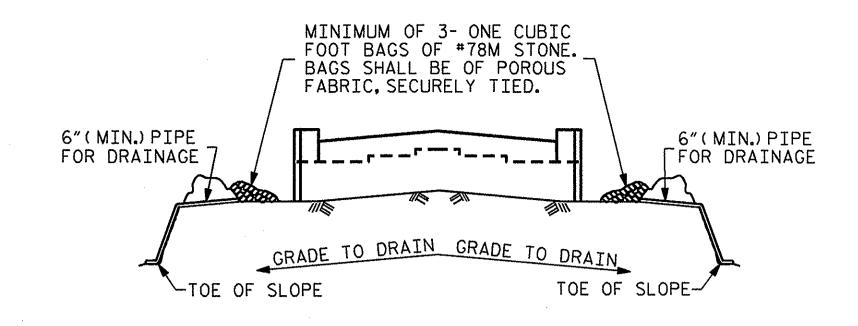


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5-#4 H3 BARS (BACK FACE) 5-#4 H4 BARS (FILL FACE)



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

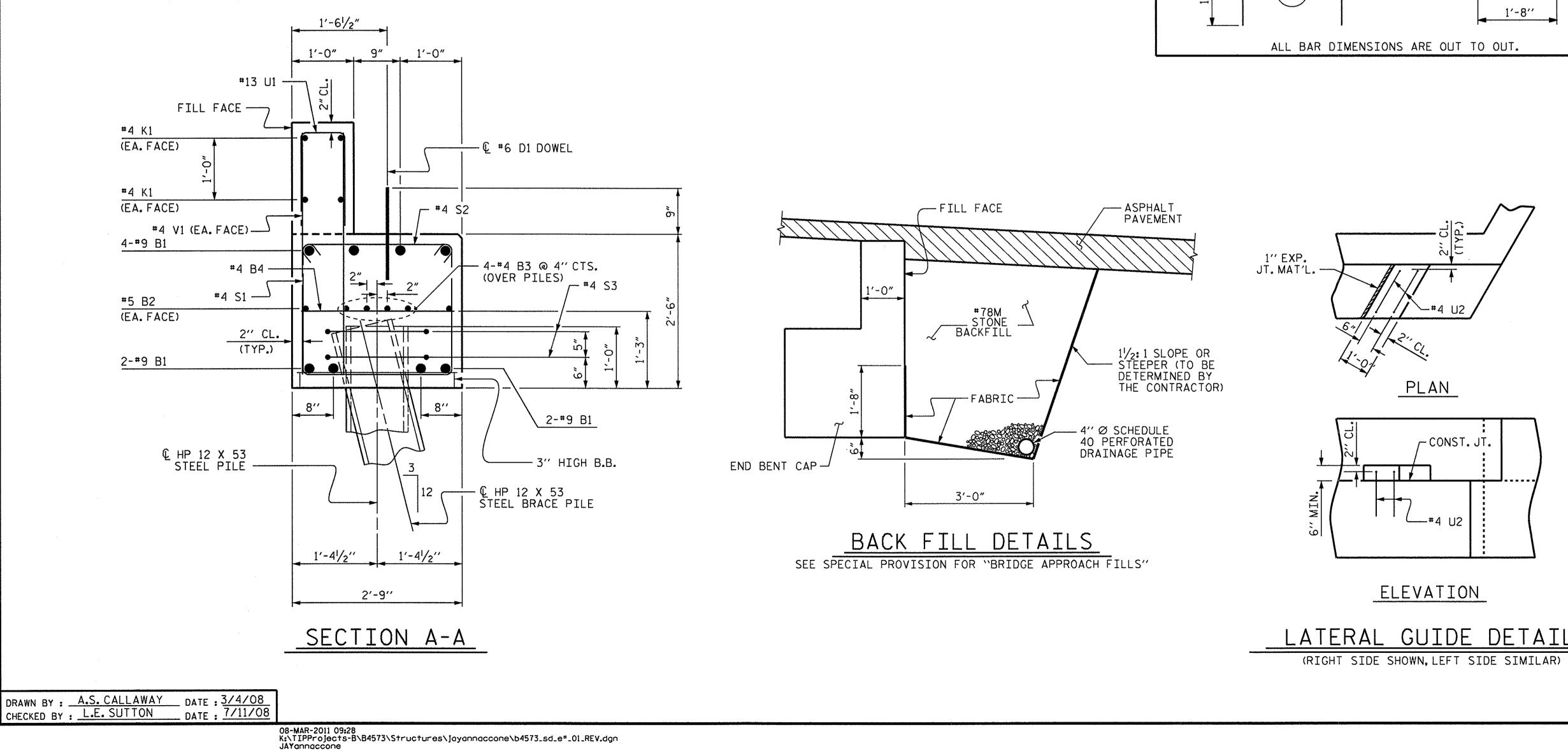
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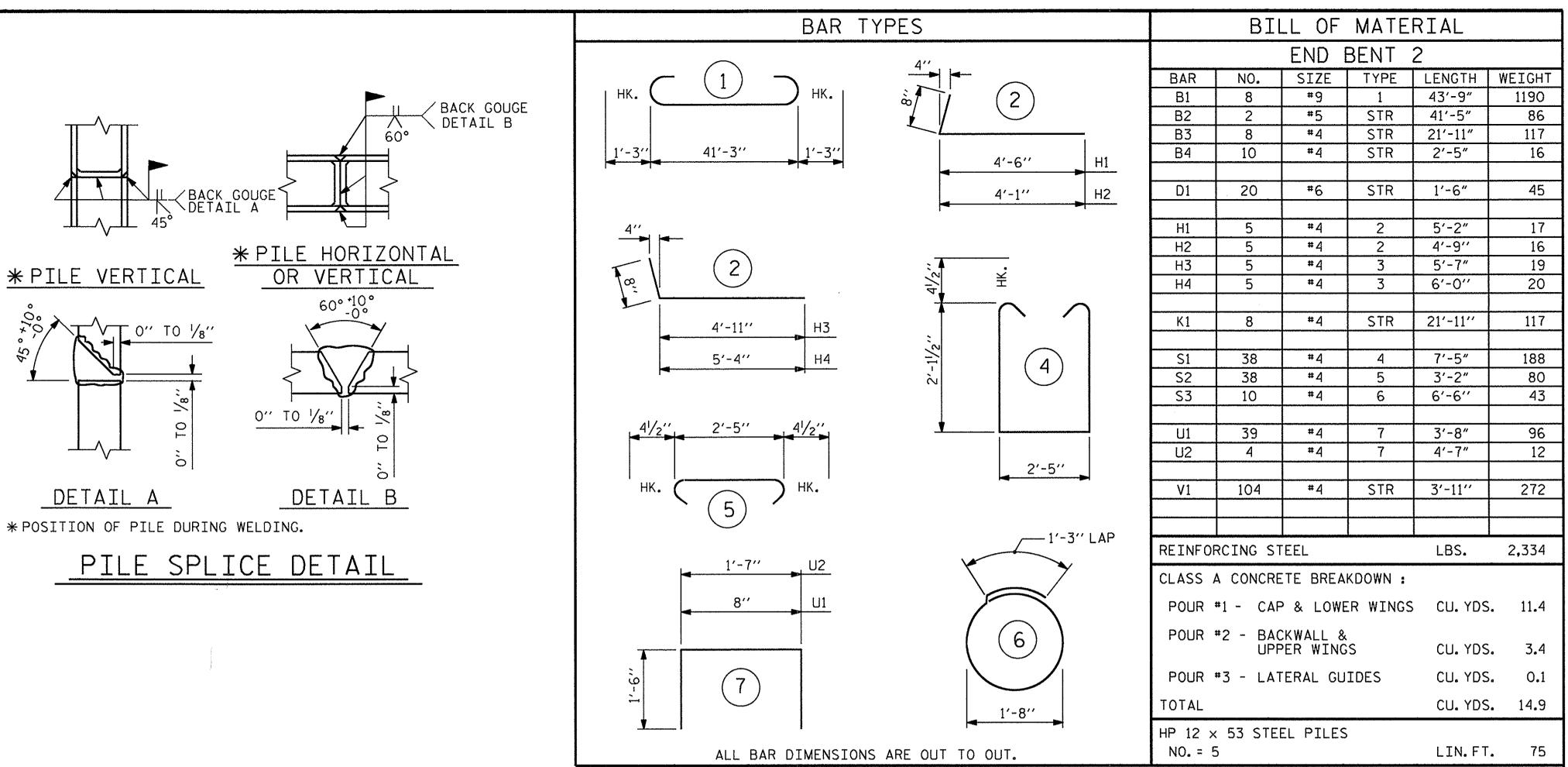
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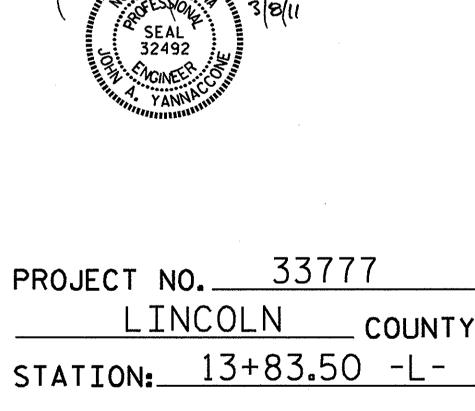
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT





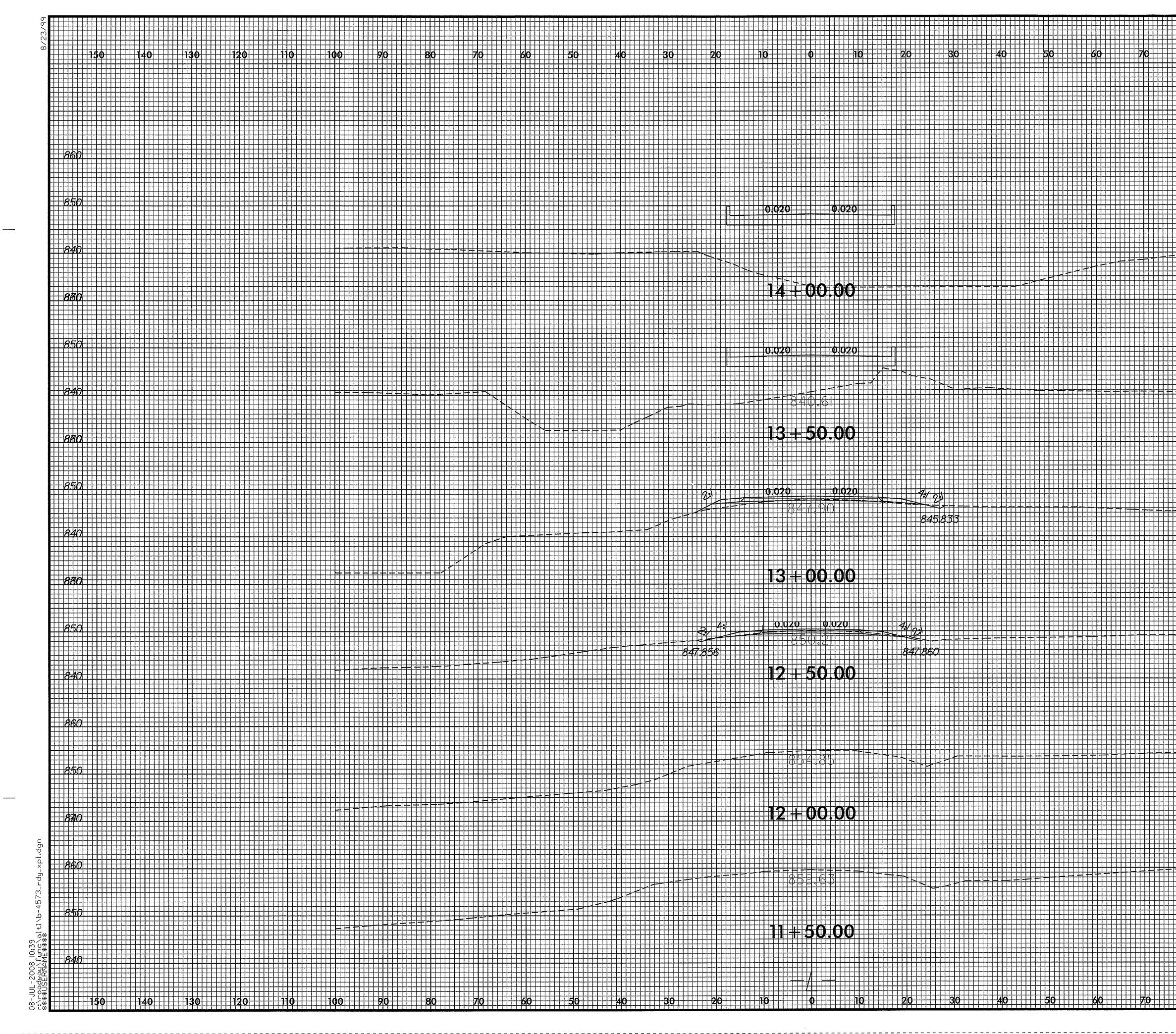


REPLACES BRIDGE NO. 183

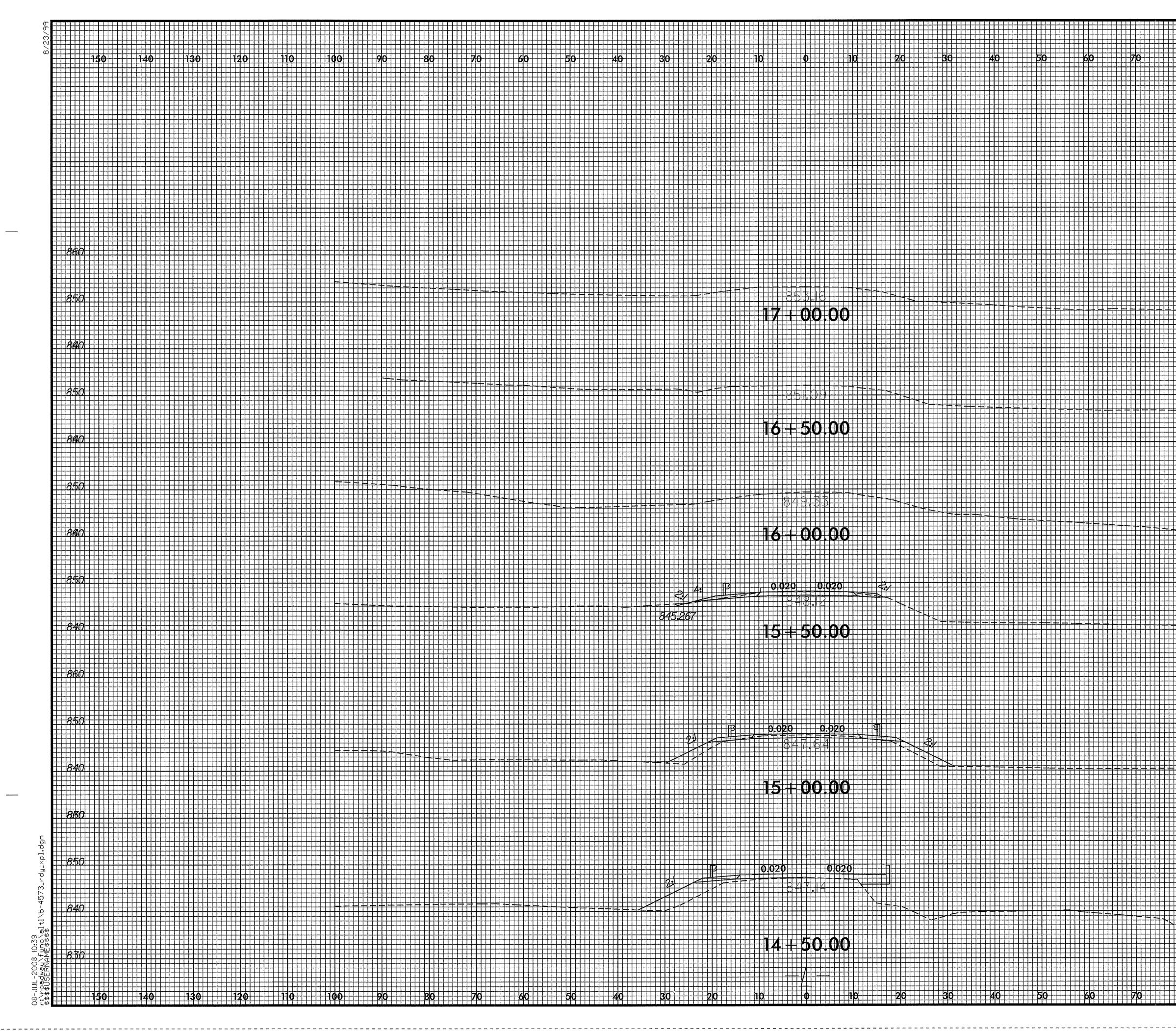
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE END BENT 2 26'-10" CLEAR ROADWAY 60° SKEW - 40' SPAN

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DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN	375 LBS. PER SQ. IN.
OF TIMBER	JID LOS. PER SU. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2006 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION. HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

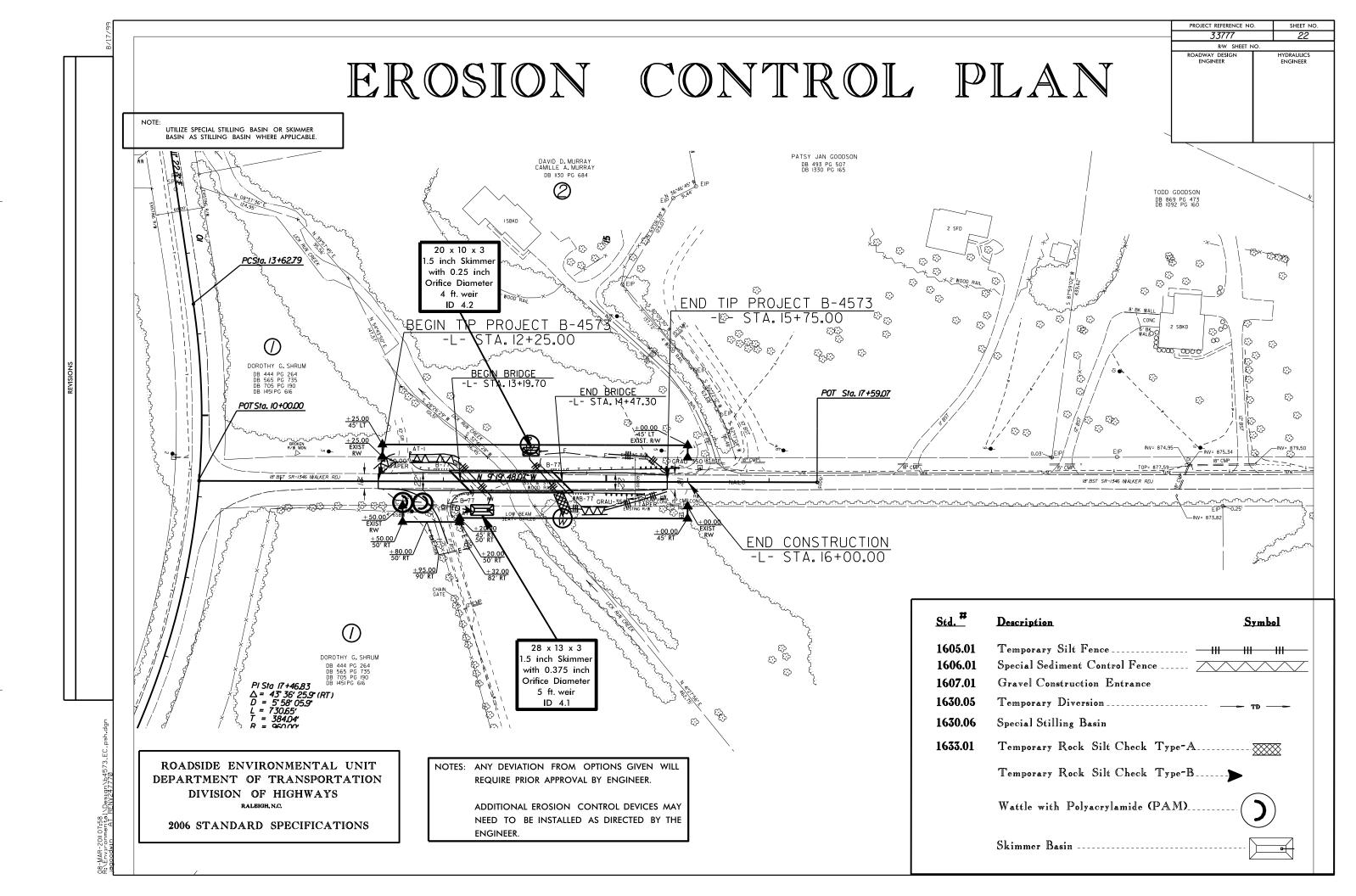
METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

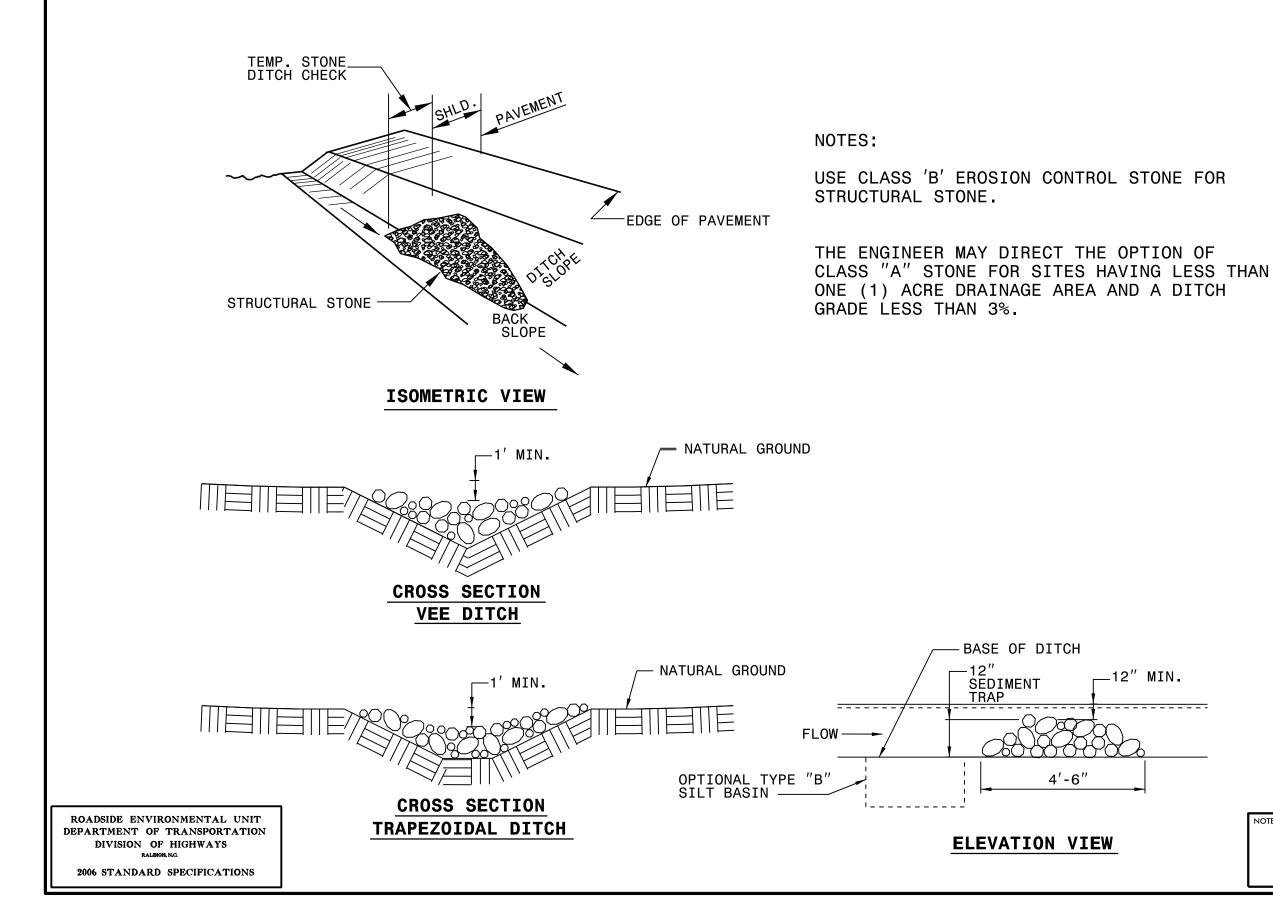
GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON. AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

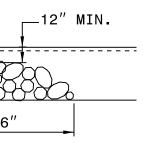




TEMPORARY ROCK SILT CHECK TYPE 'B' DETAIL

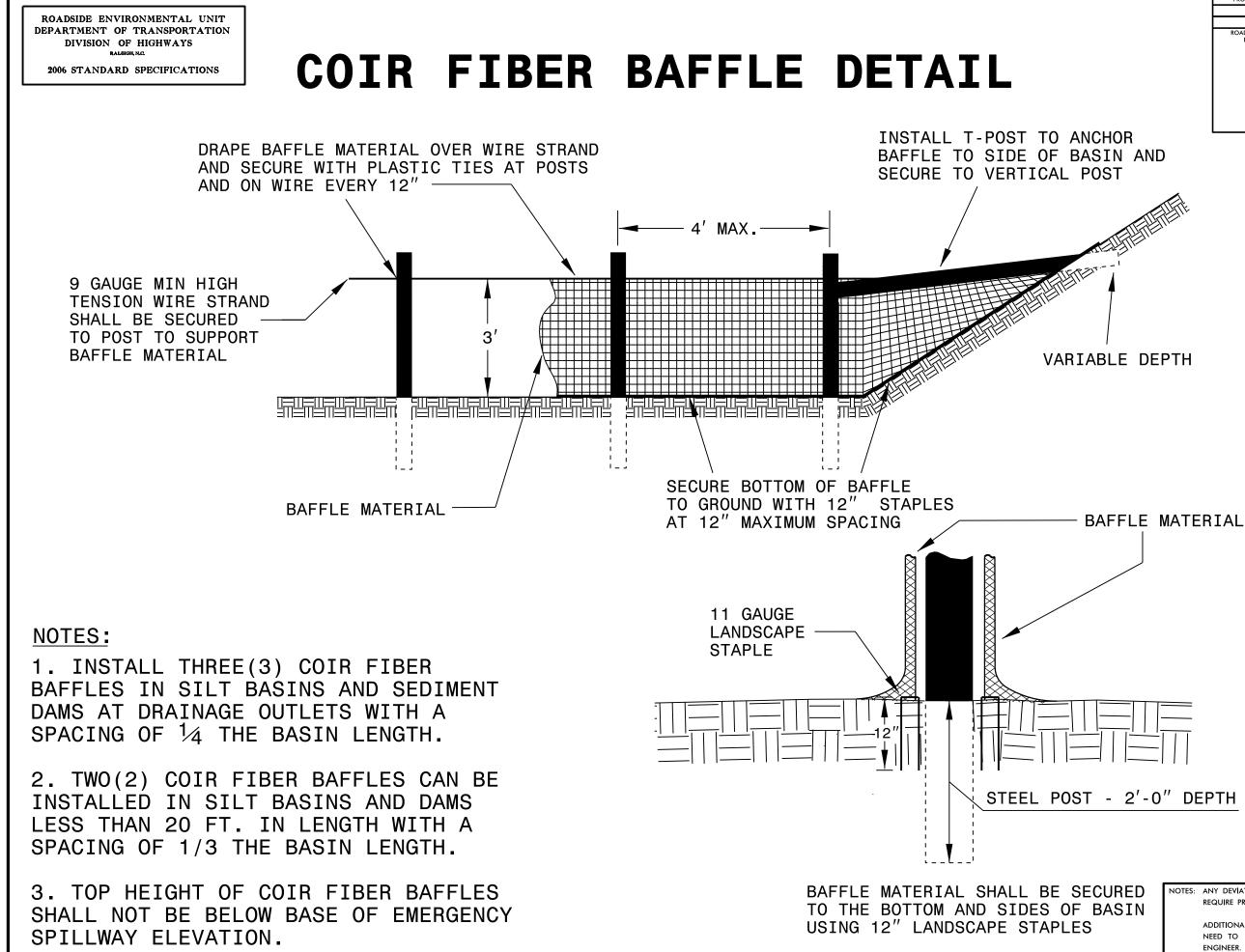


PROJECT REFERENCE NO	
33777	23
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAUUICS ENGINEER



NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER

> ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THI ENGINEER



PROJECT REFERENCE NC	D. SHEET NO.
33777	24
R/W SHEET N	۹۵.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

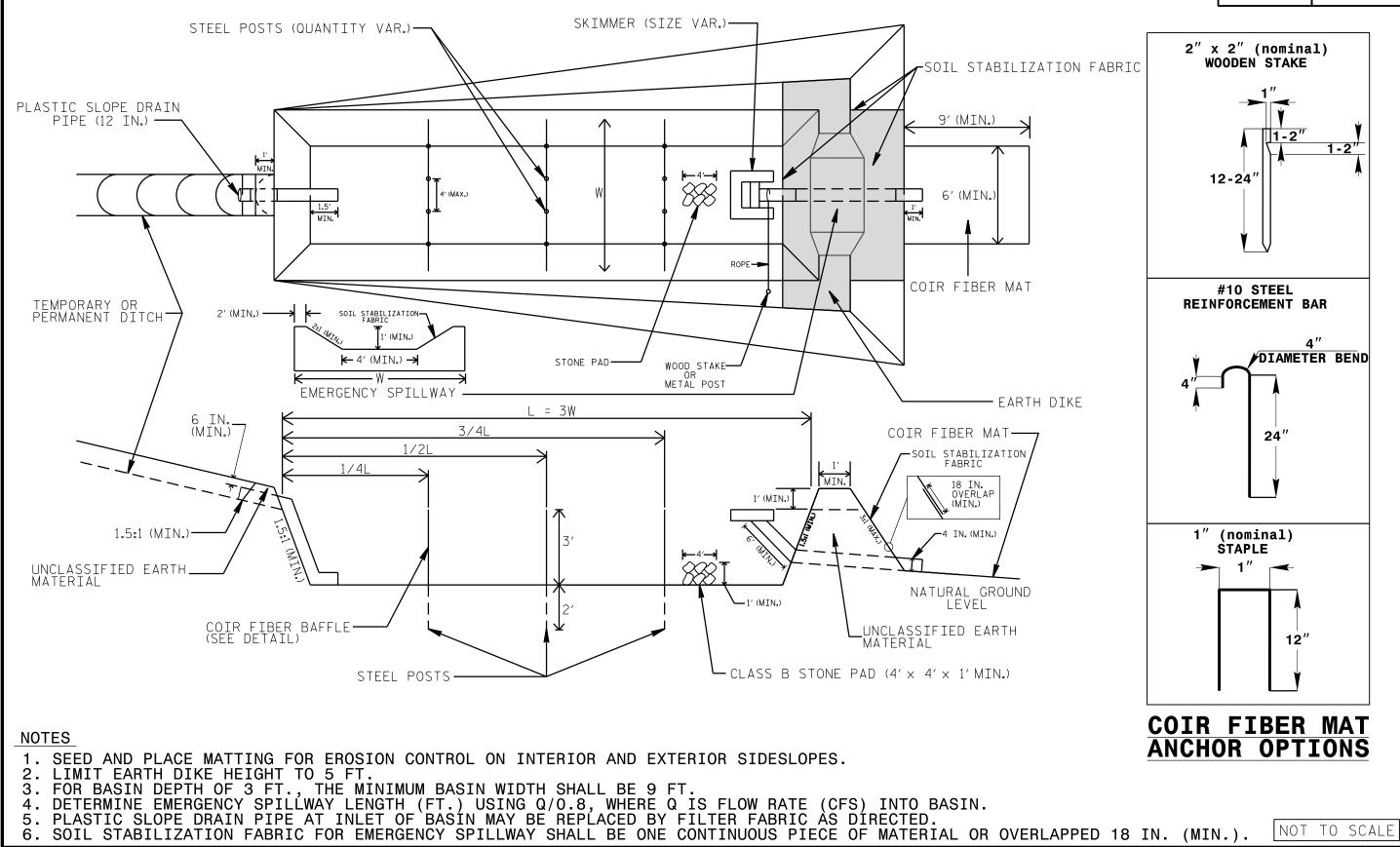
NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER

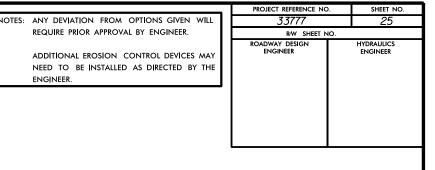
> ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER

SKIMMER BASIN WITH BAFFLES DETAIL

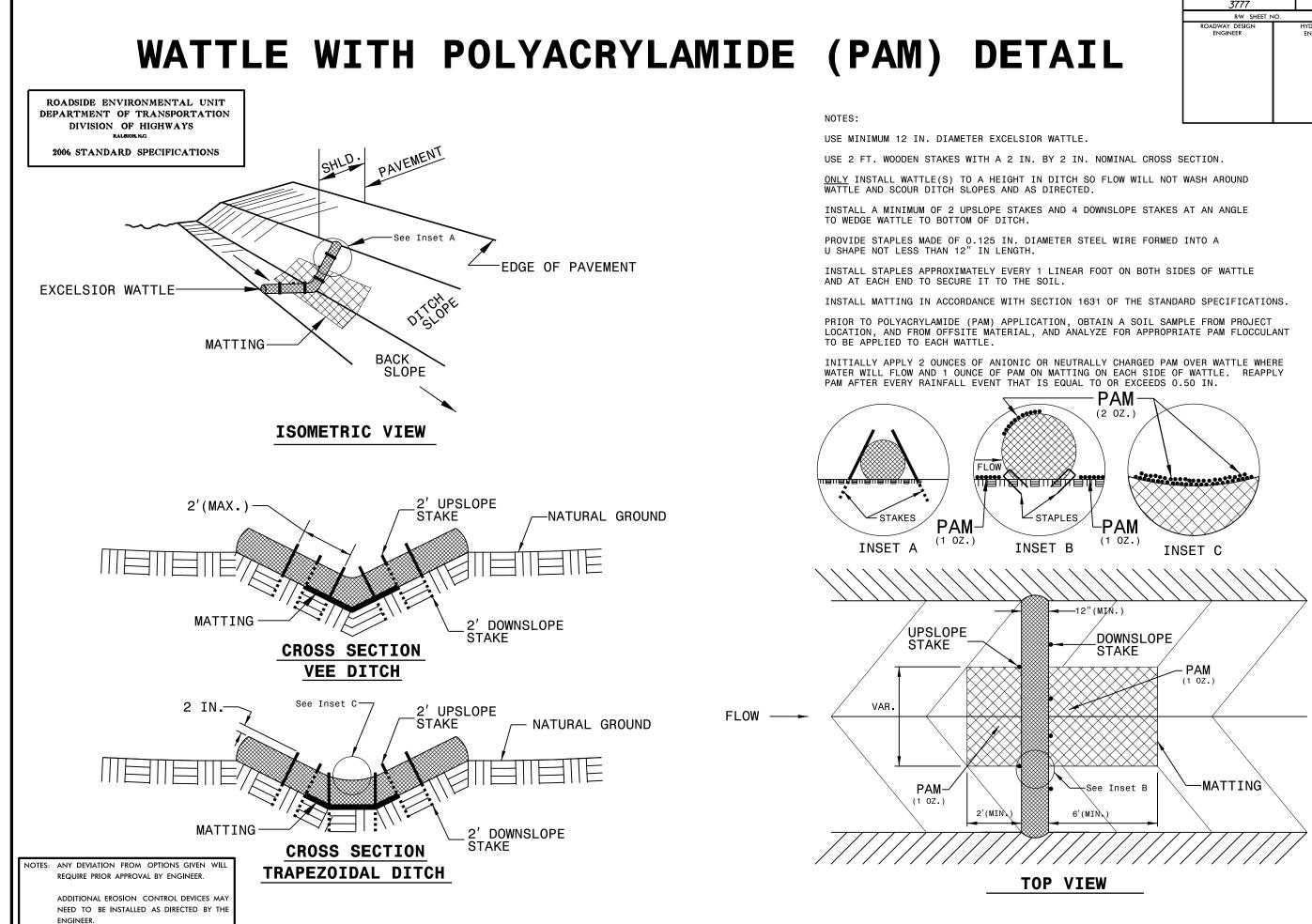
ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

2006 STANDARD SPECIFICATIONS





NOT TO SCALE

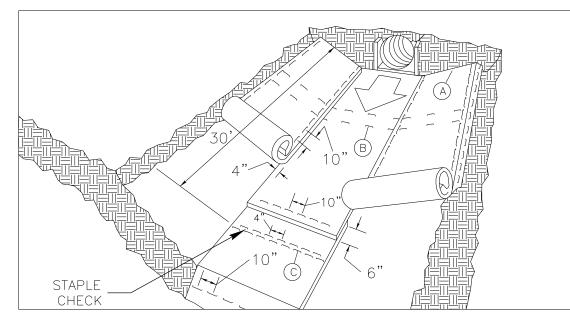


PROJECT REFERENCE NO	. SHEET NO.
3777	26
R/W SHEET N	IO.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

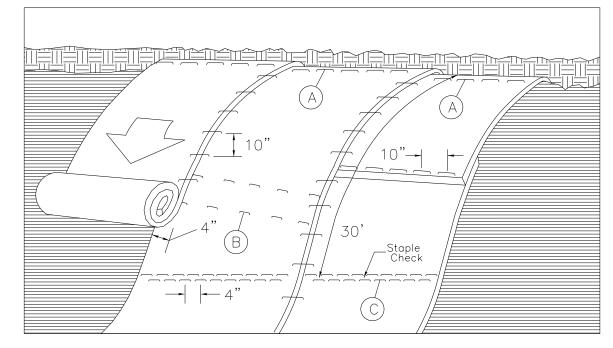
ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS RALBIGH, N.C.

2006 STANDARD SPECIFICATIONS

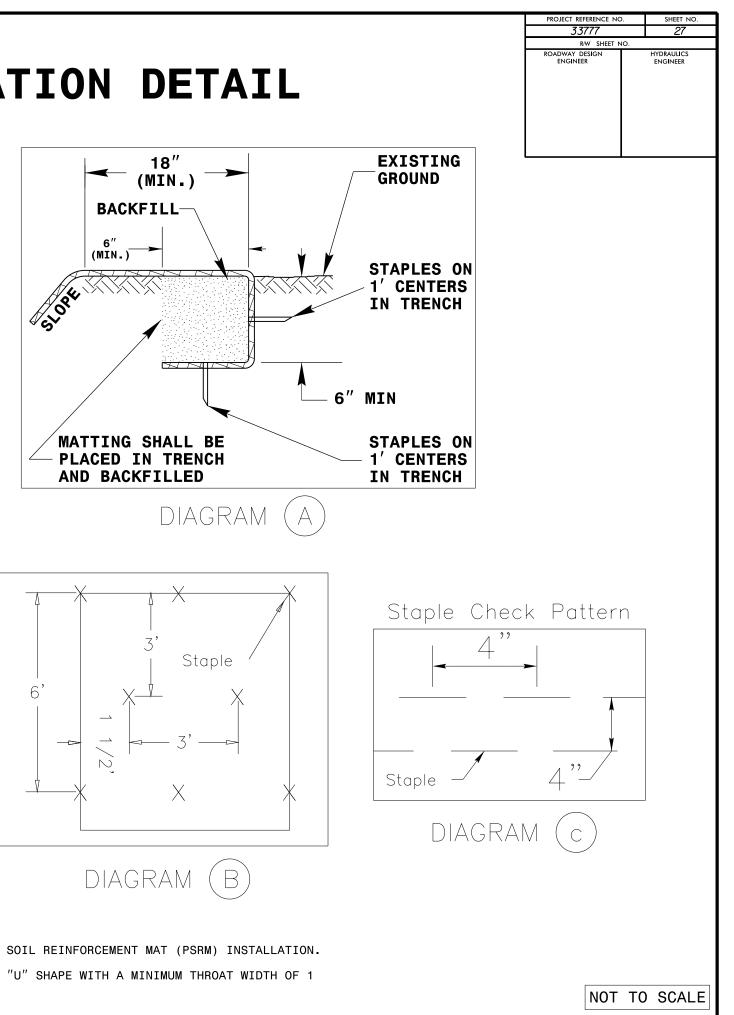
MATTING INSTALLATION DETAIL



MATTING IN DITCHES



6" (MIN.) SION MATTING SHALL BE - PLACED IN TRENCH AND BACKFILLED DIAGRAM



MATTING ON SLOPES



NOTES:

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

> ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER

THIS DETAIL APPLIES TO STRAW, EXCELSIOR, AND PERMANENT SOIL REINFORCEMENT MAT (PSRM) INSTALLATION.

STAPLES SHALL BE NO. 11 GAUGE STEEL WIRE FORMED INTO A "U" SHAPE WITH A MINIMUM THROAT WIDTH OF 1 INCH AND NOT LESS THAN 6 INCHES IN LENGTH.