BRIDGE 36 -BRIDGE 22-1142 1210 <u>1211</u> City (\*) Pond 1122 Dam North 1142 Travis 1121 1121 1120 B 1128 EC. VICINITY MAP

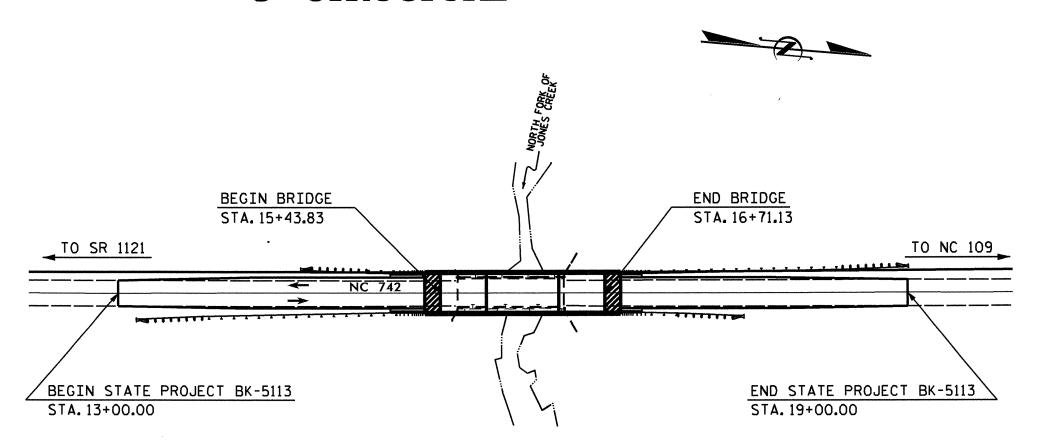
STRUCTURES

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

## ANSON COUNTY

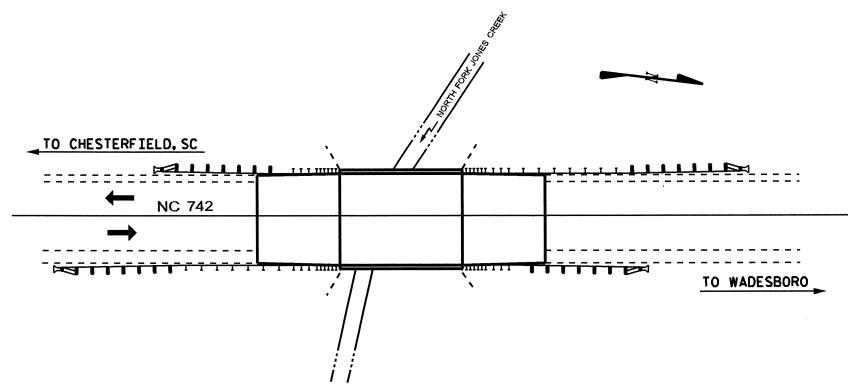
LOCATION: BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK 2.8 MILES SOUTH OF JUNCTION OF NC 109

TYPE OF WORK: GRADING, DRAINAGE, PAVING, & STRUCTURE



LOCATION: BRIDGE #36 ON NC 742 OVER NORTH FORK JONES CREEK 0.1 MILES SOUTH OF JUNCTION OF NC 109

TYPE OF WORK: BRIDGE SUPERSTRUCTURE REPLACEMENT



#### PROJECT LENGTHS

= 0.090 MILES LENGTH ROADWAY STATE PROJECT BK-5113 = 0.024 MILES LENGTH STRUCTURES STATE PROJECT BK-5113 TOTAL LENGTH STATE PROJECT BK-5113 = 0.114 MILES

LENGTH ROADWAY STATE PROJECT 17BP.10.H.2 0.009 MILES LENGTH STRUCTURES STATE PROJECT 17BP.10.H.2 = 0.007 MILES TOTAL LENGTH STATE PROJECT 17BP.10.H.2 = 0.016 MILES for the

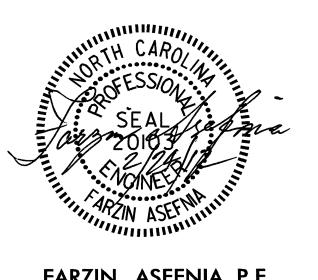
Prepared in the Office of:

STRUCTURES MANAGEMENT UNIT

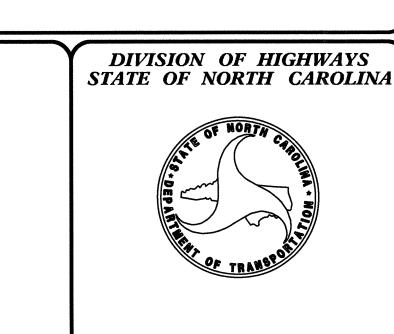
NCDOT STRUCTURES MANAGEMENT UNIT 2012 STANDARD SPECIFICATIONS

LETTING DATE: MARCH 22, 2012

ERIC NELSON, P.E. PROJECT ENGINEER



PROJECT DESIGN ENGINEER



STATE PROJECT REFERENCE NO.

BK-5113

ANSON COUNTY #22

STATE PROJECT REFERENCE NO.

ANSON COUNTY #36

P. A. PROJ. NO.

**25** 

DESCRIPTION P.E.

CONST

SHEET TOTAL SHEETS

P.E. CONST

N.C.

N.C.

42564.1.1

42564.1.3

17BP.10.H.2

17BP.10.H.2

STATE HIGHWAY DESIGN ENGINEER

FARZIN ASEFNIA, P.E.

45

000

BRIDGE 22 PROJE 1142 1210 <u>1211</u> City 1122 <u>P</u>ond Dam 1142 TIP **Travis** 1121 <u>1121</u> 1120 1128 **VICINITY MAP STRUCTURES** 

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

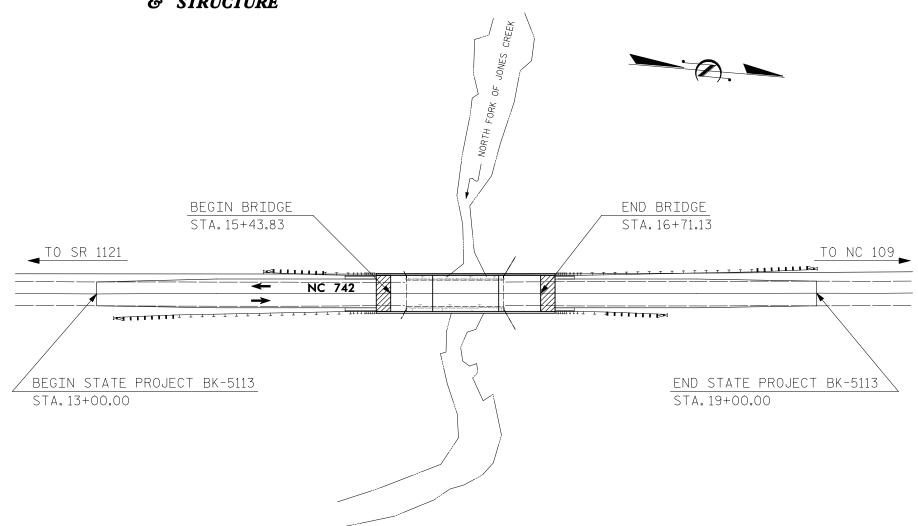
### ANSON COUNTY

N.C. 25BK-5113 ANSON COUNTY #22 DESCRIPTION 42564.1.1 42564.1.3 CONST

LOCATION: BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK

2.8 MILES SOUTH OF JUNCTION OF NC 109

TYPE OF WORK: GRADING, DRAINAGE, PAVING, & STRUCTURE



#### PROJECT LENGTH

LENGTH ROADWAY STATE PROJECT BK-5113 LENGTH STRUCTURES STATE PROJECT BK-5113 = 0.024 MILES TOTAL LENGTH STATE PROJECT BK-5113

= 0.090 MILES

= 0.114 MILES

NCDOT STRUCTURES MANAGEMENT UNIT

2012 STANDARD SPECIFICATIONS

Prepared in the Office of:

Florence & Hutcheson

LETTING DATE: MARCH 22, 2012 ERIC NELSON, P.E.

FARZIN ASEFNIA, P.E.

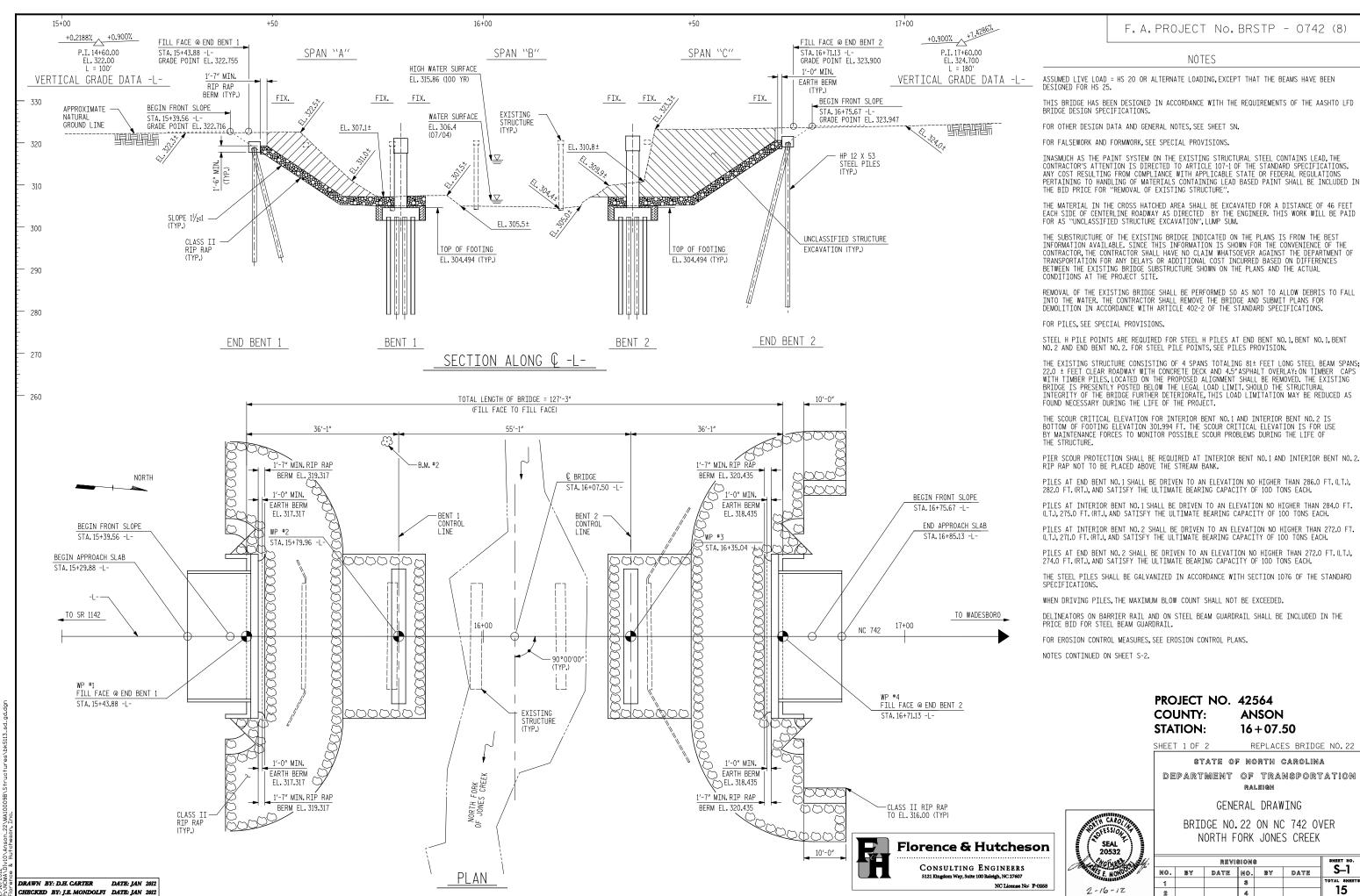


DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA



STATE HIGHWAY DESIGN ENGINEER

ONTR



THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LFD

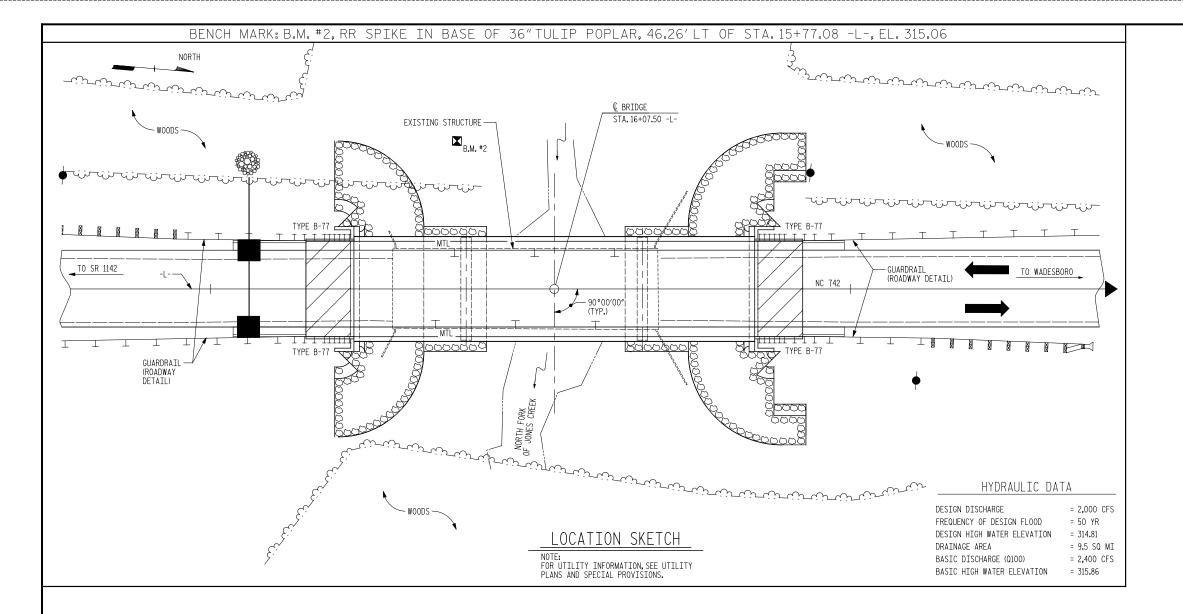
PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN

THE MATERIAL IN THE CROSS HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 46 FEET EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID

PIER SCOUR PROTECTION SHALL BE REQUIRED AT INTERIOR BENT NO.1 AND INTERIOR BENT NO.2

DEPARTMENT OF TRANSPORTATION

S-1 15



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	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION	UNCLASSIFIED STRUUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL (BRIDGE)	SPIRAL COLUMN REINFORCING STEEL	GAL	12 X 53 VANIZED EL PILES	STEEL PILE POINTS	CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES CO	"X 1'-9" STRESSED NCRETE CORED SLABS
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YD.	LUMP SUM	LB.	LB.	NO.	LIN.FT.	NO.	LIN.FT.	TON	SQ. YD.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE											250				33	1,375
END BENT 1				12.6		2,145		6	204	6		300	333			
BENT 1				42.1		6,463	584	16	368	16						
BENT 2				42.4		6,504	606	16	496	16						
END BENT 2				12.6		2,145		6	276	6		332	369			
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	109.7	LUMP SUM	17,257	1,190	44	1,344	44	250	632	702	LUMP SUM	33	1,375



SEAL 20532 2-16-12

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY.

NOTES (CONT.)

PROJECT NO. 42564 COUNTY: **ANSON** STATION: 16 + 07.50

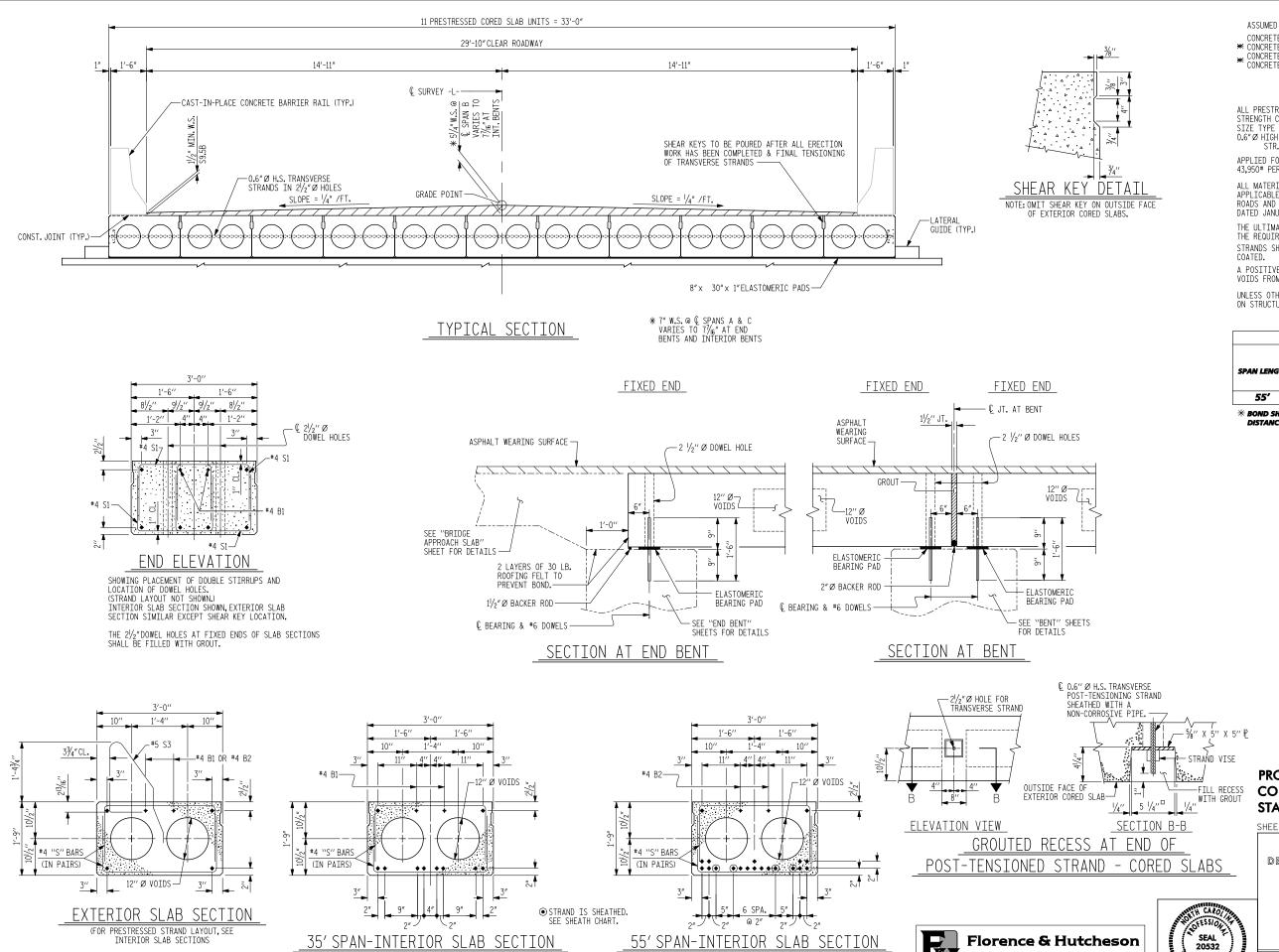
SHEET 2 OF 2

DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK

STATE OF NORTH CAROLINA

\*неет но. S—2 REVISIONS NO. BY DATE NO. BY DATE 15

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012



GENERAL NOTES

ASSUMED LIVE LOAD = HS25 OR ALTERNATE LOADING.

CONCRETE: f'c = 5000 psi, 35' SPAN ONLY

\*\* CONCRETE: f'c \_ = 4000 psi, 35' SPAN ONLY

\*\* CONCRETE: f'c = 7000 psi, 55' SPAN ONLY

\*\* CONCRETE: f'c \_ = 7000 psi, 55' SPAN ONLY

\*\* COMPRESSIVE STRENGTH @ TRANSFER OF STRESSING FORCE.)

ALL PRESTRESS STRANDS SHALL BE 7 WIRE, LOW RELAXATION, HIGH STRENGTH CABLES IN ACCORDANCE WITH THE SPECIFICATIONS. AREA 0.217 □" ULTIMATE STR. 58,600# PER CABLE

APPLIED FORCE 43,950# PER CABLE

ALL MATERIAL AND WORKMANSHIP SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES OF THE NC DEPARTMENT OF TRANSPORTATION DATED JANUARY 2012 AND WITH THE SPECIAL PROVISIONS.

THE ULTIMATE STRENGTH OF THE CORED SLAB UNIT MUST MEET THE REQUIREMENTS OF THE APPLICABLE AASHTO SPECIFICATIONS. STRANDS SHALL BE CUT FLUSH WITH ENDS OF SLABS AND EPOXY

A POSITIVE HOLD DOWN SYSTEM MUST BE EMPLOYED TO PREVENT VOIDS FROM RISING.

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{7}{4}\%$  .

	SHEATH CHAI	R <b>T</b>
SPAN LENGTH	NUMBER OF SHEATHED STRANDS PER EXTERIOR SLAB SECTIONS	NUMBER OF SHEATHED STRANDS PER INTERIOR SLAB SECTIONS
<i>55′</i>	<b>*4@2</b> "	<b>*4@2</b> "

**\* BOND SHALL BE BROKEN ON THESE STRANDS FOR A** DISTANCE OF 6 FEET FROM THE END OF THE SLAB

> PROJECT NO. 42564 **COUNTY: ANSON STATION:** 16 + 07.50

SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

3'-0" X 1'-9" PRESTRESSED CORED SLAB UNIT 29'-10" CLEAR ROADWAY - 90° SKEW

S-3 REVISIONS NO. BY DATE NO. BY DATE 15

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

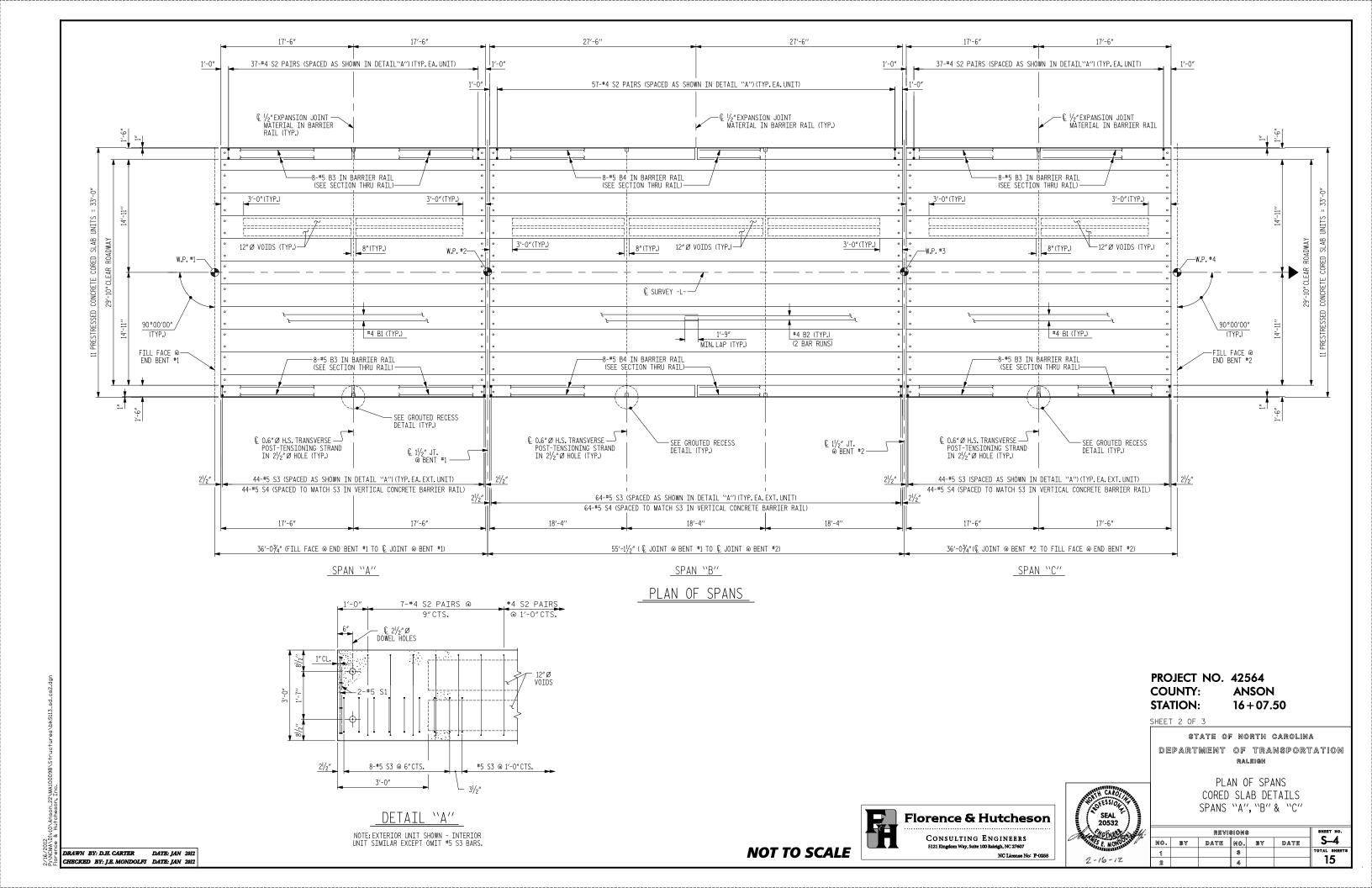
10 - 0.6" Ø H.S. STRANDS

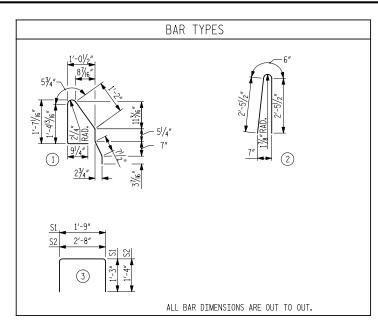
20 - 0.6" Ø H.S. STRANDS INTERIOR SLAB SECTIONS

**NOT TO SCALE** 

CONSULTING ENGINEERS

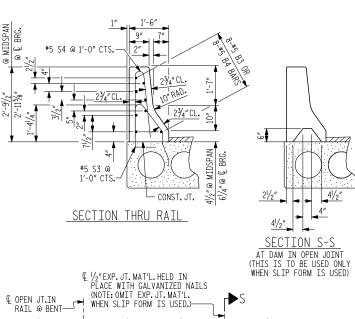
2-16-12

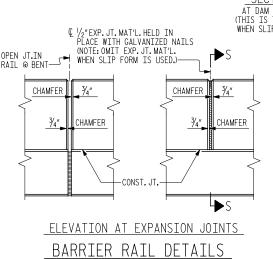




DEAD LOAD DEFLECTION AND CAMBER									
	SPANS "A" & "C"	SPAN "B"							
	3'-0" x 1'-9"	3'-0" x 1'-9"							
	0.6″Ø L.R. STRAND	0.6″Ø L.R. STRAND							
CAMBER (SLAB ALONE IN PLACE)	0.522"	2.595"							
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	0.090″ ↓	0.426″ ↓							
FINAL CAMBER	0.432"	2.169"							

\*\* INCLUDES FUTURE WEARING SURFACE





DRAWN BY: D.H. CARTER DATE: JAN 2012

CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

BILL OF MATERIAL FOR ONE 35'-O" INTERIOR CORED SLAB SECTION									
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
B1	4	#4	STR	18'-3''	49				
S1	8	#5	3	4'-3''	35				
S2	74	#4	3	5′-4′′	264				
REINF0	RCING:	STEEL		LBS	348				
5000 P	.S.I. CO	NCRETE		C. Y.	. 4.7				
0.6"Ø	L.R. STR	ANDS		NO.	. 10				

		MATER OR CC		FOR ( SLAB		35′-0″ TION
BAR	NO.	SIZE	TYPE	LENG	TH	WEIGHT
B1	4	#4	STR	18′-3	11	49
S1	8	#5	3	4'-3	"	35
S2	74	#4	3	5′-4	"	264
* S3	44	#5	1	5′-2	"	237
REINFO	RCING:	STEEL			LBS.	348
<b></b> ₩EP0X	Y COATE	D REINF	ORCING	STEEL	LBS.	237
5000 F	.S.I. CO	NCRETE			C. Y.	4.7
0.6"Ø	L.R. STR	ANDS			NO.	10

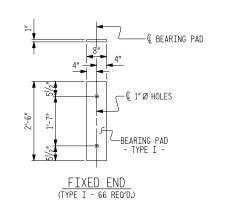
BIL				FOR ONE SLAB SEC			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
B2	4	#4	STR	28'-3"	75		
S1	8	#5	3	4'-3''	35		
S2	114	#4	3	5′-4′′	406		
REINFO	RCING	STEEL		LBS.	516		
7000 F	.S.I. CO	NCRETE		C. Y.	. 7.4		
0.6"Ø	L.R. STR	ANDS		NO. 20			

r T
16
45
.4
20

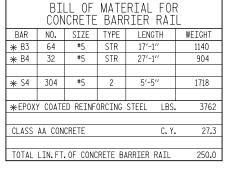
CORED SLABS REQUIRED										
NUMBER LENGTH TOTAL LENGTH					TOTAL LENGTH					
	SPAN A	SPAN B	SPAN C	SPAN A	SPAN B	SPAN C				
EXTERIOR C.S.	2	2	2	35'-0"	55'-0"	35'-0"	250'-0"			
INTERIOR C.S.	9	9	9	35'-0"	55'-0"	35'-0"	1125′-0″			

SUMMARY FOR EXTERIOR CORED SLAB SECTIONS										
					TOTAL					
		SPAN "A"	SPAN "B"	SPAN "C"	TOTAL					
REINFORCING STEEL	LBS.	696	1032	696	2424					
*EPOXY COATED REINFORCING STEEL	LBS.	474	690	474	1638					
5000 P.S.I. CONCRETE	C. Y.	9.4		9.4	18.8					
7000 P.S.I. CONCRETE (SPAN B)	C. Y.		14.8		14.8					
0.6" Ø L.R. STRANDS	NO.	20	40	20	80					
•		·	·							

SUMMARY FOR	INTERI	OR CORED	SLAB SE	ECTIONS		
					TOTAL	
		SPAN "A"	SPAN "B"	SPAN "C"	TOTAL	
REINFORCING STEEL	LBS.	3132	4644	3132	10908	
5000 P.S.I. CONCRETE	C. Y.	42.3		42.3	84.6	
7000 P.S.I. CONCRETE (SPAN B)	C. Y.		66.6		66.8	
0.6"Ø L.R. STRANDS	NO.	90	180	90	360	



ELASTOMERIC BEARING DETAILS



GRADE 270 S	TRANDS
	0.6"Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2 \slash\hspace{-0.05cm} \%$  DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4000 PSI FOR 35'SPANS AND 5000 PSI FOR

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, \( \frac{1}{2}'' \) IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS

TRANSVERSE POST-TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE MINIMUM AND MAXIMUM HEIGHTS OF THE BARRIER RAIL ARE SHOWN. THE HEIGHT OF THE BARRIER RAIL VARIES WHILE THE TOP OF THE RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE.

PROJECT NO. 42564 COUNTY: **ANSON STATION:** 16 + 07.50

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

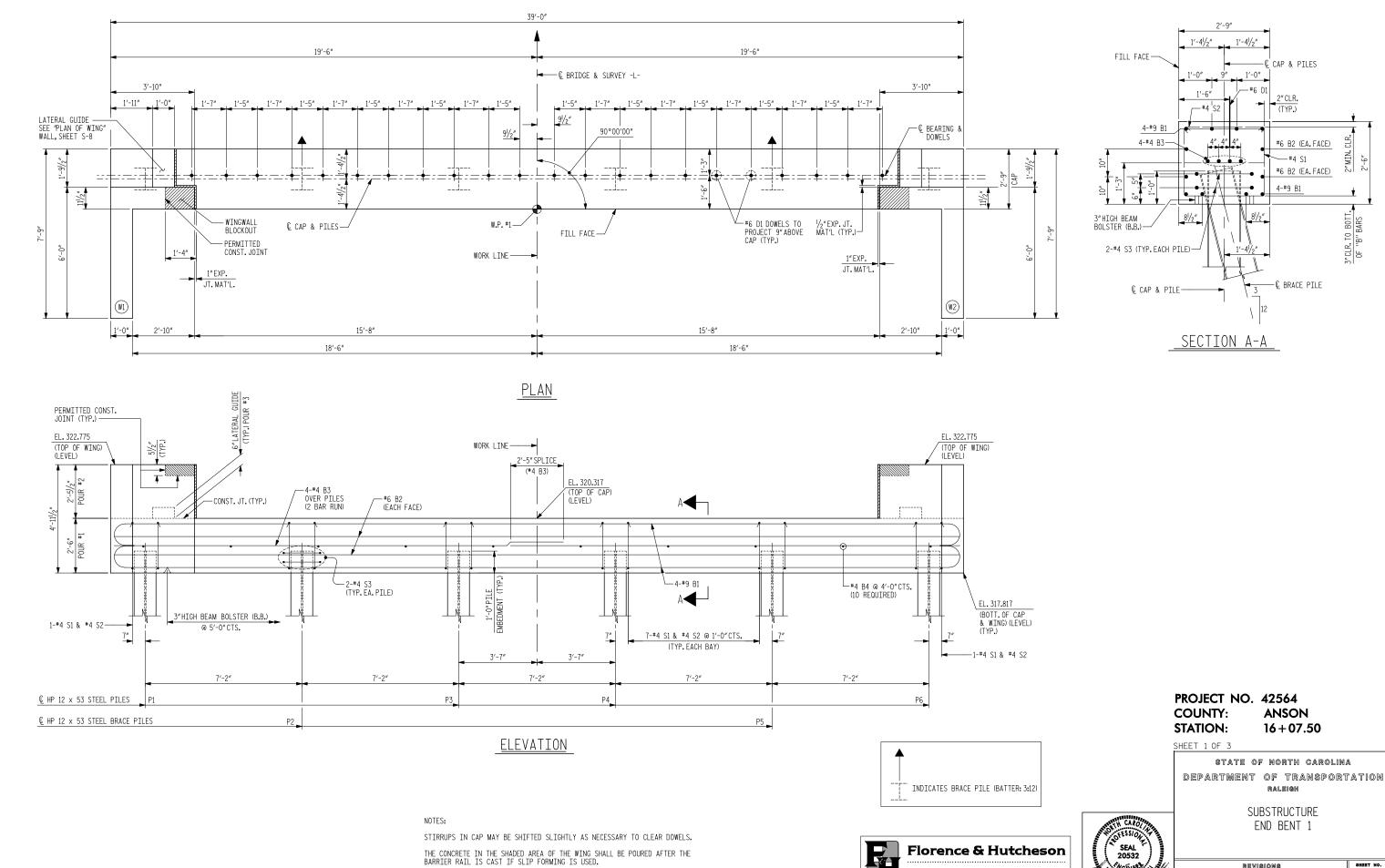
BILL OF MATERIALS



CONSULTING ENGINEERS

SHEET NO.			BIONS	REVI		
S-5	DATE	BY	NO.	DATE	BY	o.
TOTAL SHEETS			3			]
15			4			2

**NOT TO SCALE** 



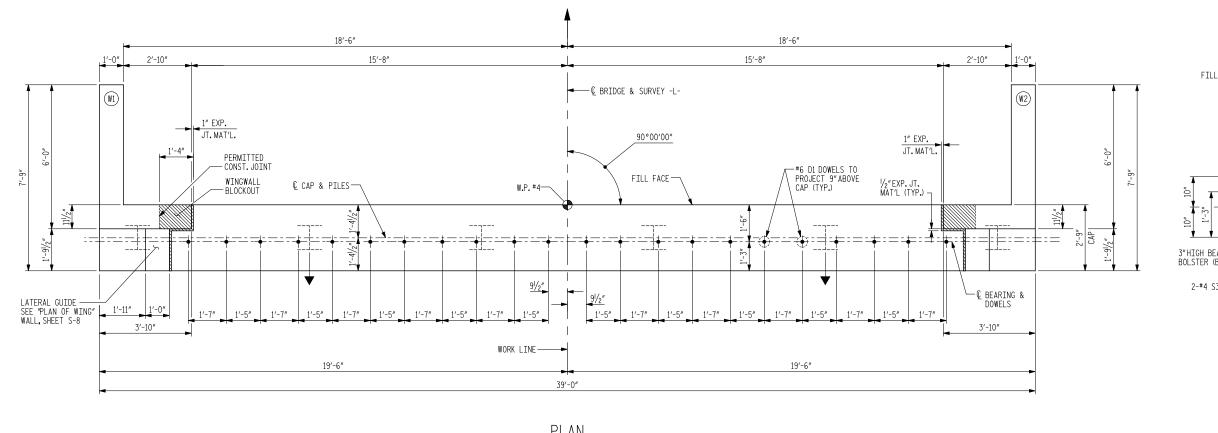
DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

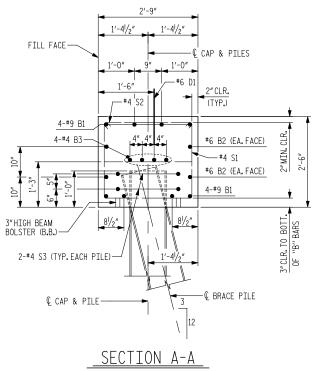
THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDES IF APPROVED BY THE ENGINEER. **NOT TO SCALE** 

CONSULTING ENGINEERS

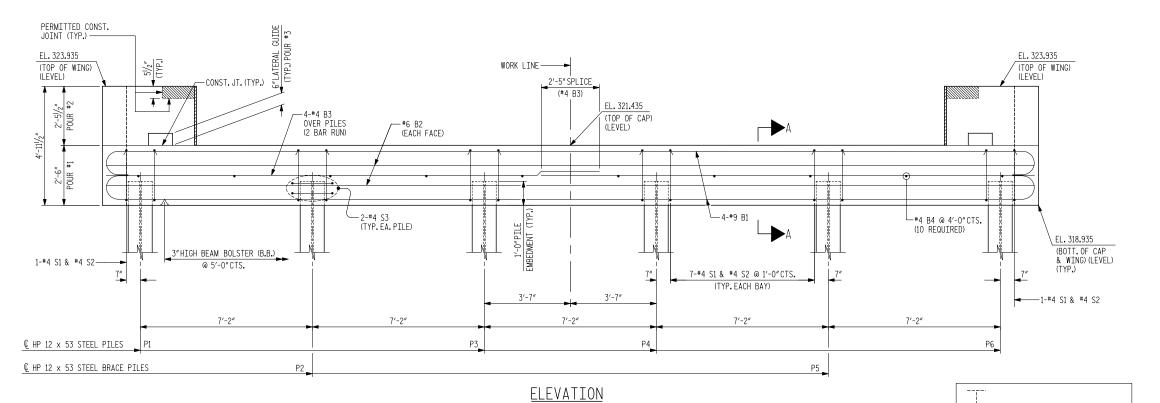
2-16-12

REVISIONS S-6 NO. BY DATE NO. BY DATE 15





#### PLAN



PROJECT NO. 42564 COUNTY: **ANSON** STATION: 16 + 07.50

SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH



INDICATES BRACE PILE (BATTER: 3:12)

Florence & Hutcheson

CONSULTING ENGINEERS

SUBSTRUCTURE END BENT 2

> REVISIONS S-7 NO. BY DATE NO. BY DATE 15

STIRRUPS IN CAP MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR DOWELS.

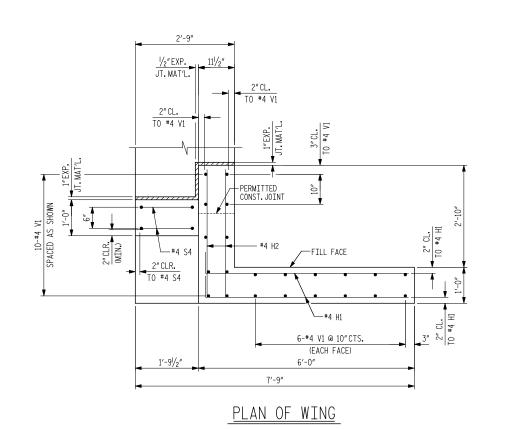
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

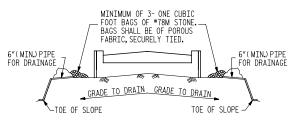
THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDES IF APPROVED BY THE ENGINEER.

**NOT TO SCALE** 

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

NOTES:



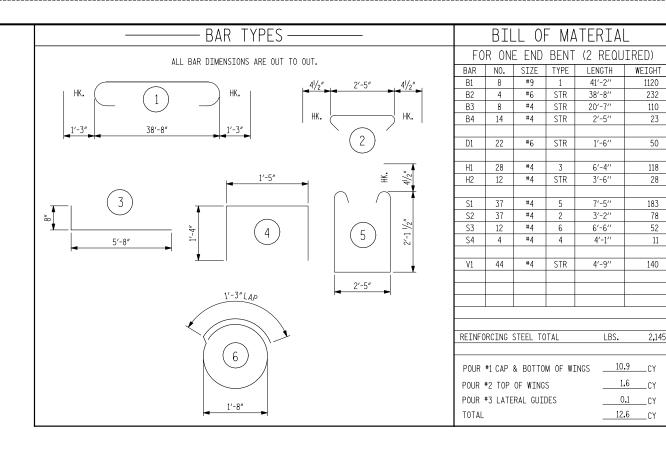


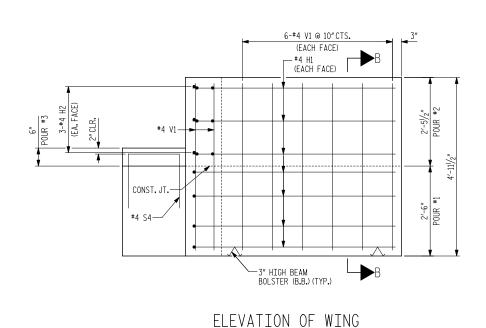
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

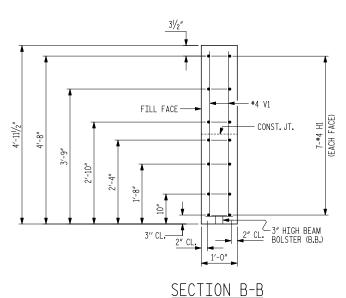
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

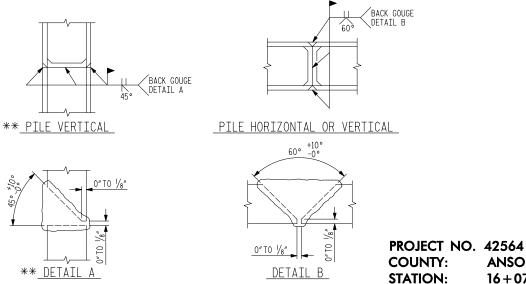
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT









PILE SPLICE DETAILS

\*\* POSITION OF PILE DURING WELDING

16 + 07.50STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

**ANSON** 



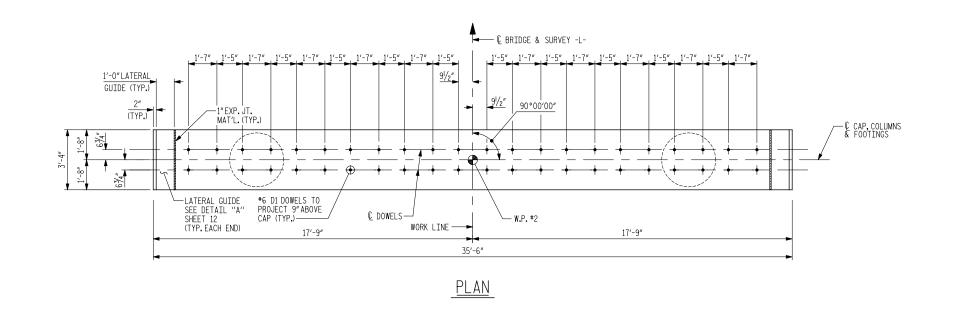


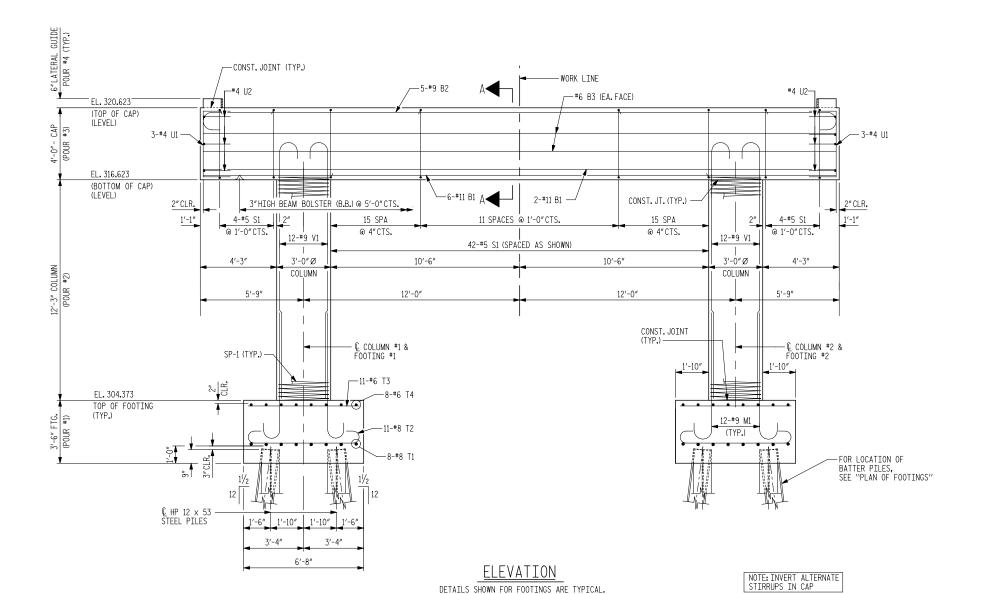
Sl	JBSTRI	JCTURE
END	BENT	DETAILS

REVISIONS S-8 NO. BY DATE NO. BY DATE 15

**NOT TO SCALE** 

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012



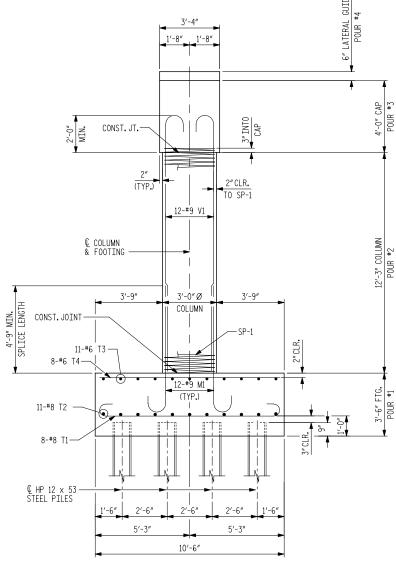


NOTES:

STIRRUPS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR DOWELS.
HOOKS ON "Y" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR SECTION A-A, SEE SHEET S-11.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDES IF APPROVED BY THE ENGINEER.  $\,$ 



END ELEVATION

PROJECT NO. 42564
COUNTY: ANSON
STATION: 16+07.50

HEET 1 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE BENT #1



Florence & Hutcheson

CONSULTING ENGINEERS

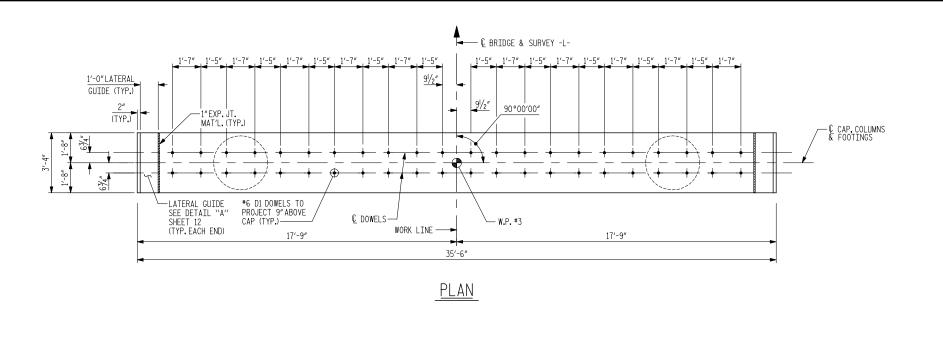
SUBSTR BEN SEAL 20532

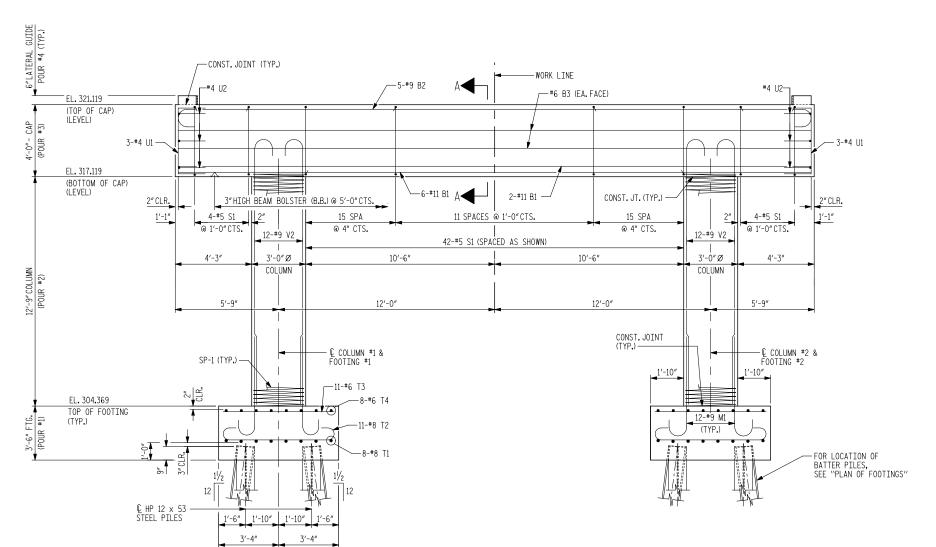
| REVISIONS | SAME NO. | BY | DATE | SAME NO. | SAME NO

**NOT TO SCALE** 

NOT TO

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012





<u>ELEVAT</u>ION

DETAILS SHOWN FOR FOOTINGS ARE TYPICAL.

6'-8"

DRAWN BY: D.H. CARTER DATE: JAN 2012

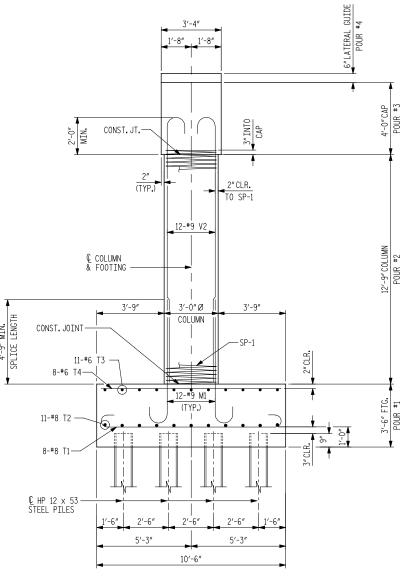
CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

NOTES:

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FOR SECTION A-A, SEE SHEET S-11.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDES IF APPROVED BY THE ENGINEER.



#### END ELEVATION

PROJECT NO. 42564 COUNTY: **ANSON STATION:** 16 + 07.50

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SEAL 20532

2-16-12

Florence & Hutcheson

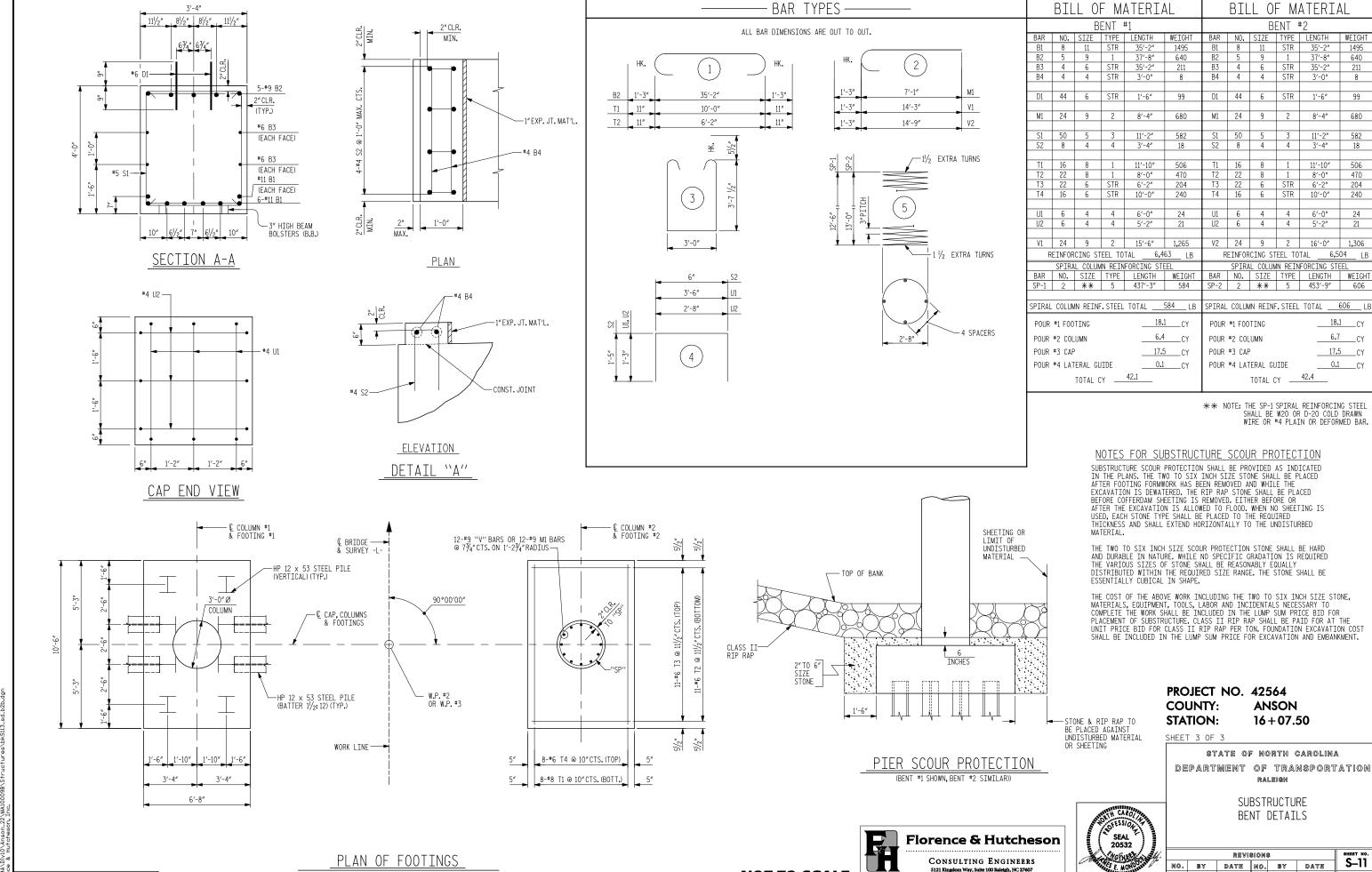
CONSULTING ENGINEERS

SUBSTRUCTURE BENT #2

REVISIONS S-10 NO. BY DATE NO. BY DATE 15

**NOT TO SCALE** 

NOTE: INVERT ALTERNATE STIRRUPS IN CAP



DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

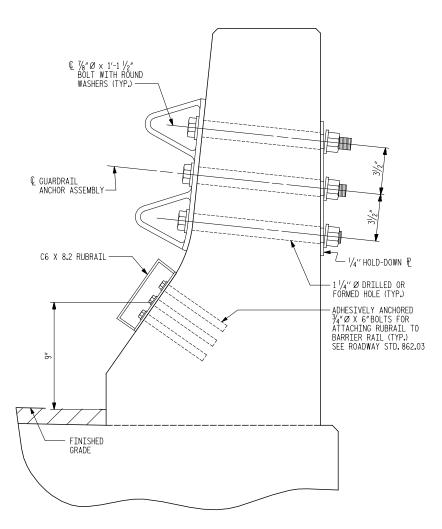
DETAILS SHOWN FOR FOOTINGS AND COLUMNS ARE TYPICAL.

**NOT TO SCALE** 



2-16-12

15





DRAWN BY: D.H. CARTER DATE: JAN 2012

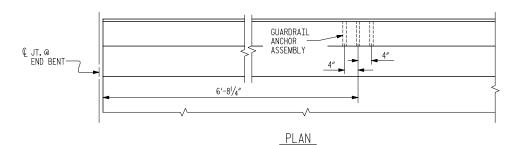
CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

#### GUARDRAIL ANCHOR ASSEMBLY DETAILS

FOR CONCRETE BARRIER RAIL ONLY
(FOR LOCATION OF GUARDRAIL ANCHOR ASSEMBLY, SEE THIS SHEET)

FOR LOCATION OF GUARDRAIL ANCHOR ASSEMBLY, SEE "PLAN" BELOW © JT. @ \_\_\_\_\_ ¾″ Ø X 6″ADHESIVELY ANCHORED BOLT FOR TO BARRIER RAIL (TYP.) -FRONT FACE OF CONCRETE BARRIER RAIL ─FINISHED GRADE ELEVATION

FOR LOCATION OF RUBRAIL, SEE ROADWAY STD. 862.03



LOCATION OF ANCHORS FOR GUARDRAIL

FOR CONCRETE BARRIER RAIL ONLY END BENT #1 SHOWN, END BENT #2 SIMILAR

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$  HOLD DOWN PLATE AND  $\frac{1}{4}$  BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

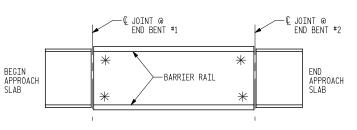
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF CONCRETE BARRIER RAIL OR COCNCRETE END POSTS, FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1  $I_4^{\prime\prime}$  Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE "%" Ø X 6"BOLTS WITH WASHERS, LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE ¾" Ø BOLT IS 12 KIPS, FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



#### SKETCH SHOWING POINTS OF ATTACHMENTS

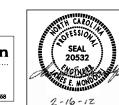
\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 42564 COUNTY: **ANSON STATION:** 16 + 07.50

REPLACES BRIDGE NO. 22

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

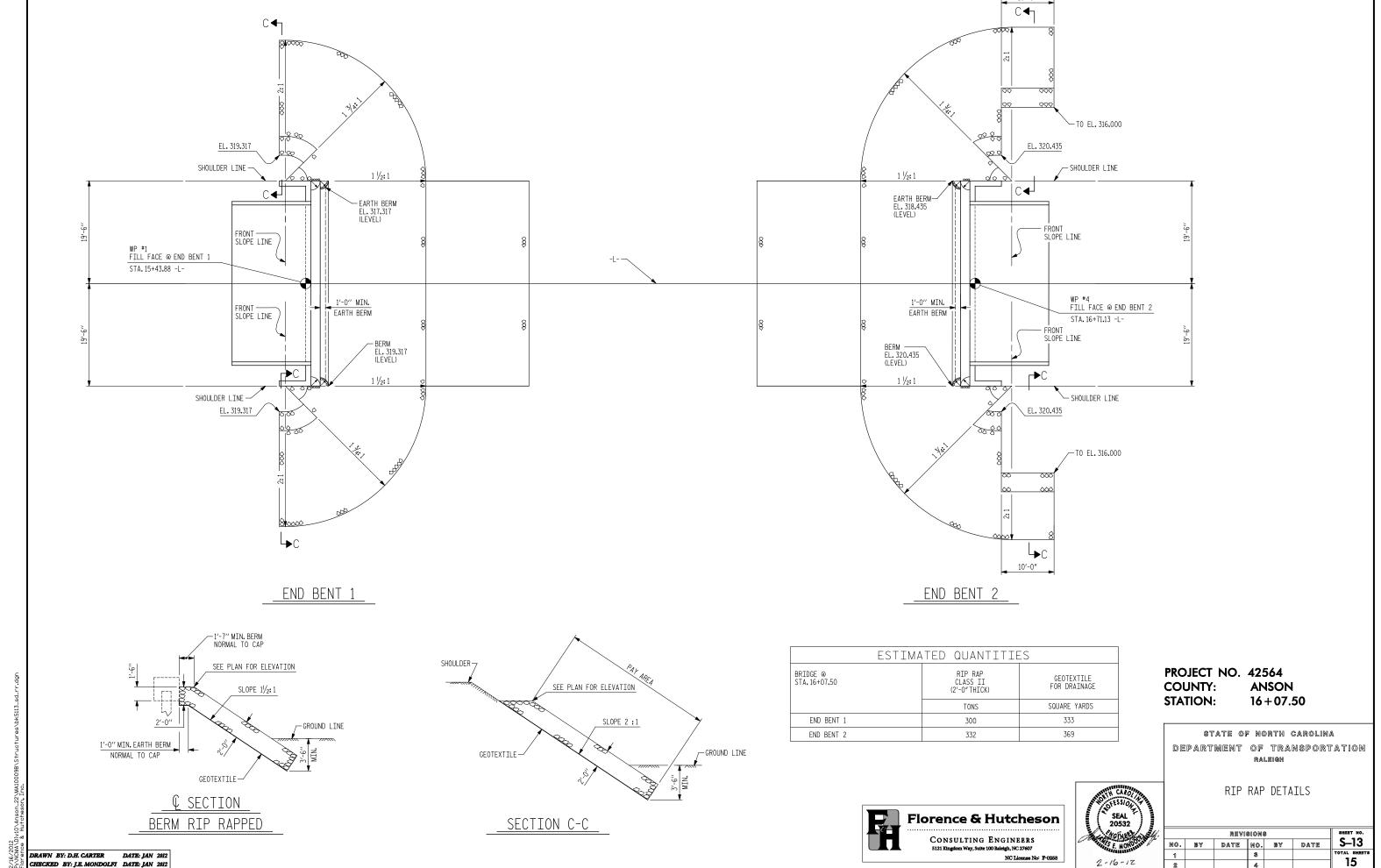
SUPERSTRUCTURE GUARDRAIL ANCHORAGE DETAILS

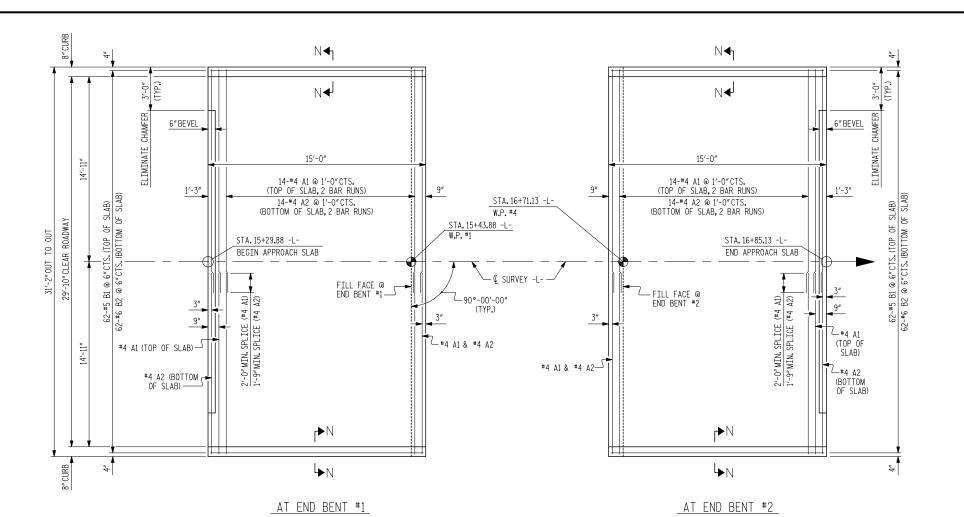


REVISIONS S-12 NO. BY DATE NO. BY DATE 15

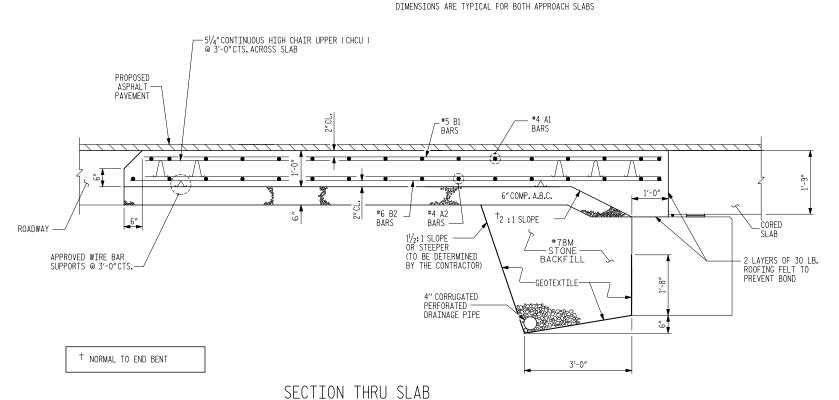
**NOT TO SCALE** 

Florence & Hutcheson CONSULTING ENGINEERS





PLAN OF APPROACH SLABS



FOR BRIDGE APPROACH FILL INCLUDING FABRIC. 4" Ø DRAINAGE PIPE. AND #78M STONE BACKFILL, SEE ROADWAY STANDARD DRAWINGS.

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO INSTALLATION OF CORED SLAB.

FABRIC SHALL BE TYPE 1 GEOTEXTILE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

\*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF END BENT CAP FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

#78 STONE BACKFILL, 4" Ø DRAINAGE PIPE AND FABRIC SHALL BE INCLUDED IN THE LUMP SUM PRICE BID FOR APPROACH SLABS.

FOR THE 4"Ø DRAINAGE PIPE OUTLET(S), SEE "BRIDGE APPROACH FILLS", ROADWAY STANDARD DRAWING 422.11.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL

THE 6"COMP.A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE OF EACH EDGE OF THE APPROACH SLAB.

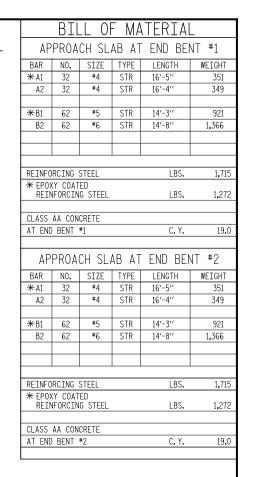
THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP.A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB, THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

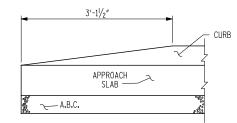
FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE CORED SLAB UNIT"

THE JOINT AT THE END BENT SHALL BE GROUTED AS SOON AS PRACTICAL AFTER THE CONSTRUCTION OF THE APPROACH SLABS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS



PROJECT NO. 42564 **COUNTY: ANSON STATION:** 16 + 07.50

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

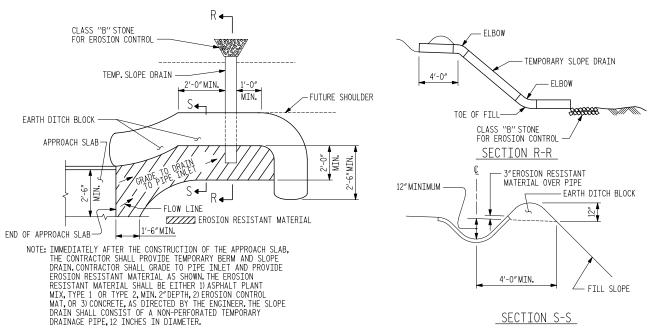
> APPROACH SLAB 29'-10" CLEAR ROADWAY 90° SKEW



**NOT TO SCALE** 

DRAWN BY: D.H. CARTER DATE: JAN 2012 CHECKED BY: J.E. MONDOLFI DATE: JAN 2012

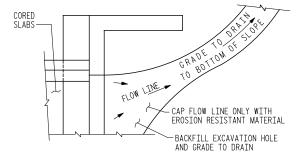
STD. NO. BAS13



#### PLAN VIEW

#### TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY
AFTER THE BACKFILLING OF THE END BENT EXCAVATION,
GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE
EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING
OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION
AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE
CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS
PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. 42564
COUNTY: ANSON
STATION: 16+07.50

SHEET 2 OF 2

SEAL 20532

2-16-12

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

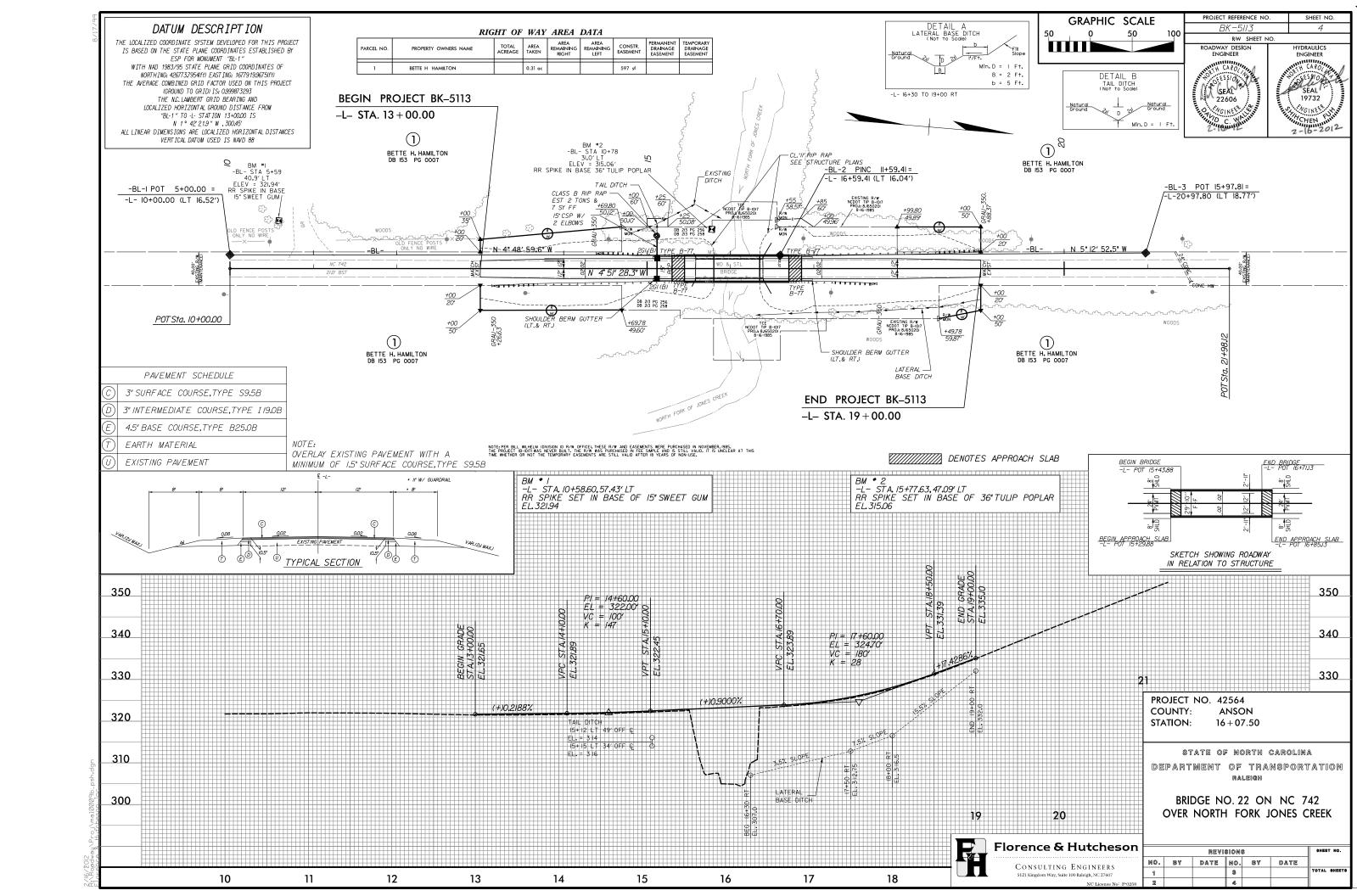
BRIDGE APPROACH SLAB DETAILS

Florence & Hutcheson

Consulting Engineers

S121 Kingdom Way, Suite 100 Raleigh, NC 27607

NC License No: F-0258



## EROSION CONTROL PLAN

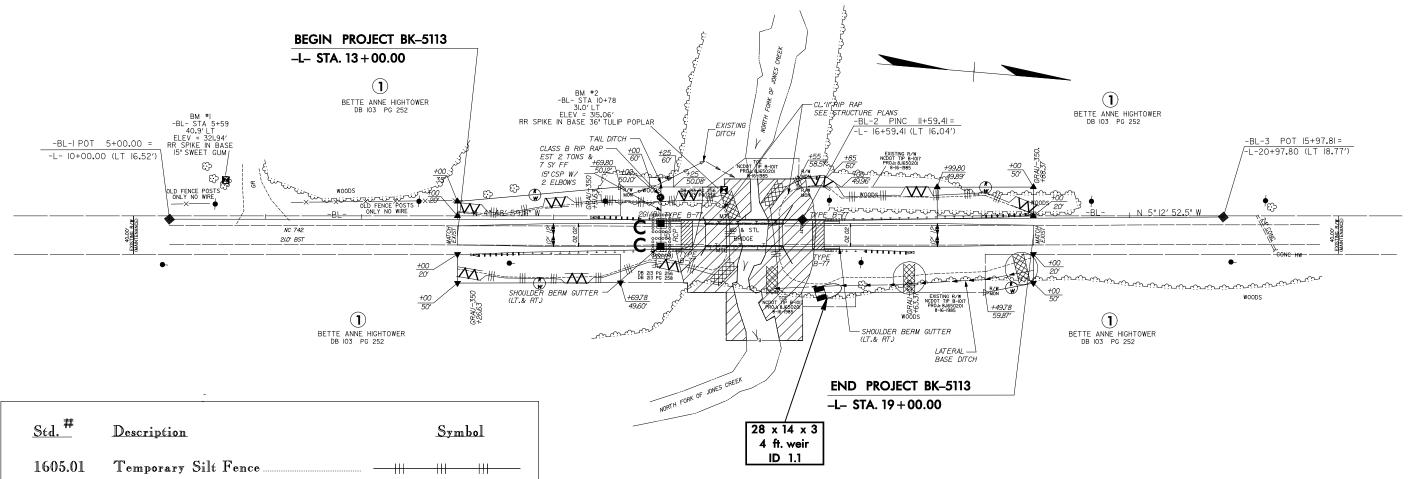
PROJECT REFERENCE NO.

BK-5//3

RW SHEET NO.

ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER



Special Sediment Control Fence 1606.01 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains. 1630.06 Special Stilling Basins 1632.03 Rock Inlet Sediment Trap Type C 1633.01Temporary Rock Silt Check Type-A Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM) 1634.02Temporary Rock Sediment Dam Type-B

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY
WITH THE REGULATIONS SET FORTH BY THE
NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011
ISSUED BY THE NORTH CARCLINA DEPARTMENT OF ENVIRONMENT AND
NATURAL RESOURCES DIVISION OF WATER QUALITY.

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH.N.C.

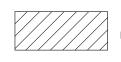
2012 STANDARD SPECIFICATIONS

#### 2012 STANDARD DRAWINGS

1604.01 Railroad Erosion Control Detail
1605.01 Temporary Sit Peace
1606.01 Special Sediment Control Fence
1607.01 Gravel Construction Entrance
1622.01 Temporary Berns and Slope Drains
1630.02 Sit Basin Type B
1630.03 Temporary Sit Ditch
1630.04 Silling Basin
1630.05 Temporary Diversion

1632.01 Rock Index Sediment Trup Type A
1632.02 Rock Index Sediment Trup Type B
1632.03 Rock Index Sediment Trup Type C
1633.01 Temporary Rock Silt Check Type A
1633.02 Temporary Rock Silt Check Type A
1634.02 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1634.03 Rock Pipe Index Sediment Trup Type A
1635.02 Rock Pipe Index Sediment Trup Type A
1645.01 Temporary Rock Sediment Trup Type B
1644.01 Coir Fibre Baffle

Special Stilling Basin 1645.01 Temporary Stream Crossing Matting Installation



ENVIRONMENTALLY SENSITIVE AREA
PLEASE SEE HIGH QUALITY WATERS NOTE

PROJECT NO. 42564
COUNTY: ANSON
STATION: 16+07.50

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

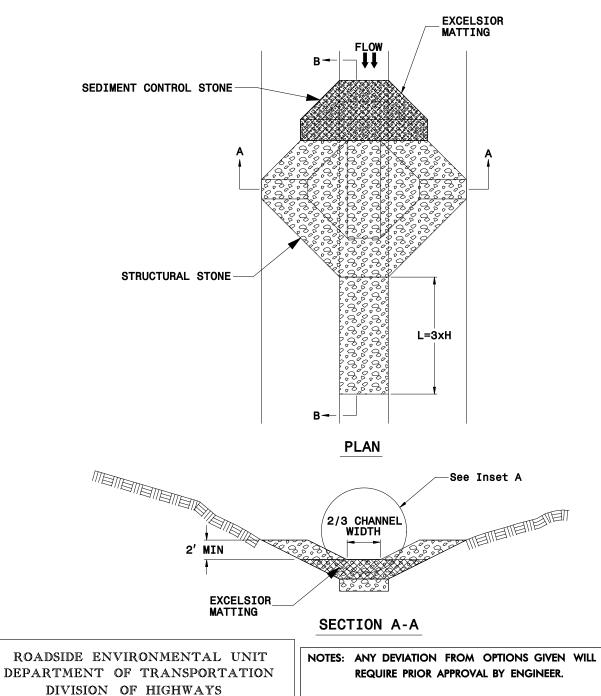
BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK

SHEET NO.	revisions					
1	DATE	BY	NO.	DATE	BY	NO.
TOTAL SHEETS			3			1
4			4			2

## EROSION CONTROL PLAN

PROJECT REFERENCE NO	PROJECT REFERENCE NO.						
BK-5113	EC-2						
RW SHEET N							
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER					
LINGHALLE		ENGINEER					

## TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)



RALRIGH, N.C.

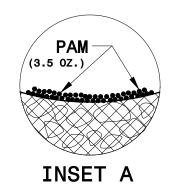
2006 STANDARD SPECIFICATIONS

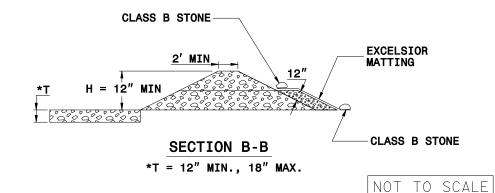
NOTES

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION. AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 3.5 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.





PROJECT NO. 42564 COUNTY: ANSON STATION: 16+07.50 **REPLACES BRIDGE NO. 22** 

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK

SHEET NO.	REVISIONS						
2	DATE	BY	NO.	DATE	BY	NO.	
TOTAL SHEETS			3			1	
4			46			2	

ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

PROJECT REFERENCE NO.SHEET NO.DK - 5/13FC - 3

#### DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

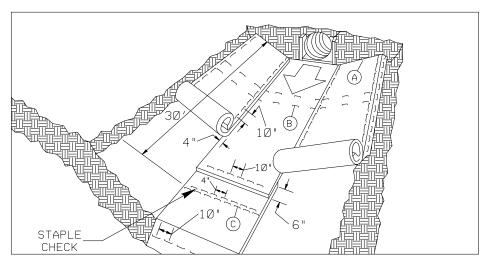
## SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

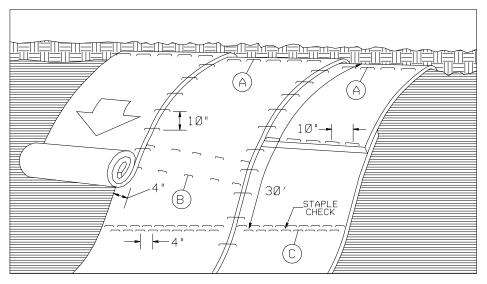
## EROSION CONTROL PLAN

PROJECT REFERENCE NO	), SHEET NO.
BK-5113	EC-4
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

#### MATTING INSTALLATION DETAIL



**MATTING IN DITCHES** 



**MATTING ON SLOPES** 

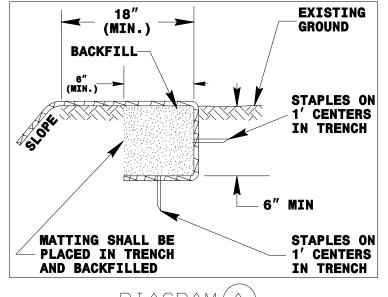


DIAGRAM (A

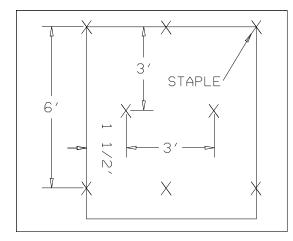


DIAGRAM (B)

NOT TO SCALE

# STAPLE CHECK PATTERN

PROJECT NO. 42564 COUNTY: **ANSON** STATION: 16 + 07.50**REPLACES BRIDGE NO. 22** 

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BRIDGE NO. 22 ON NC 742 OVER NORTH FORK JONES CREEK

REVISIONS 4 NO. BY DATE NO. BY DATE

NOTES:

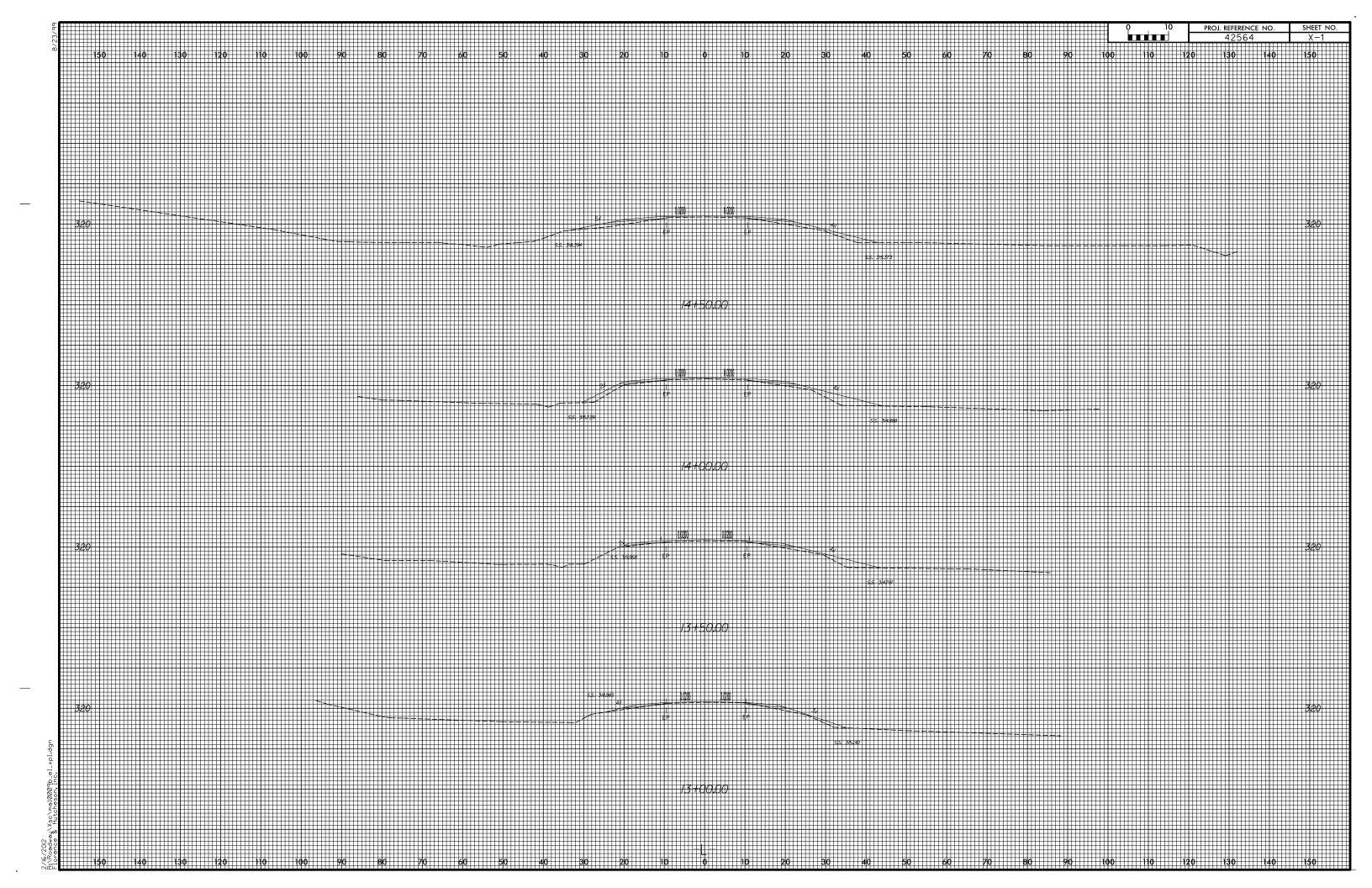
THIS DETAIL APPLIES TO STRAW, EXCELSIOR, AND PERMANENT SOIL REINFORCEMENT MAT (PSRM) INSTALLATION. STAPLES SHALL BE NO. 11 GAUGE STEEL WIRE FORMED INTO A "U" SHAPE WITH A MINIMUM THROAT WIDTH OF 1 INCH AND NOT LESS THAN 6 INCHES IN LENGTH.

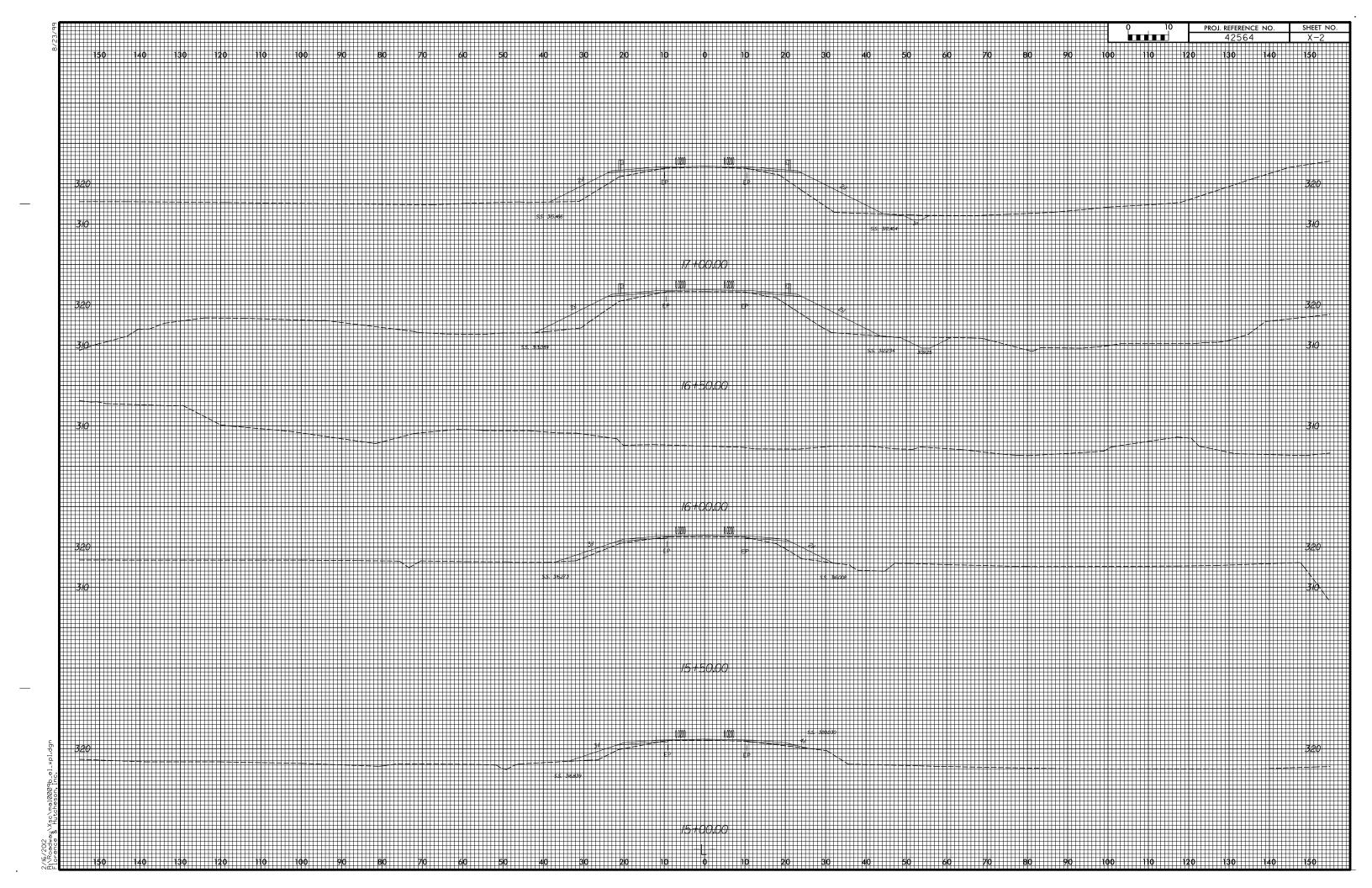
ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

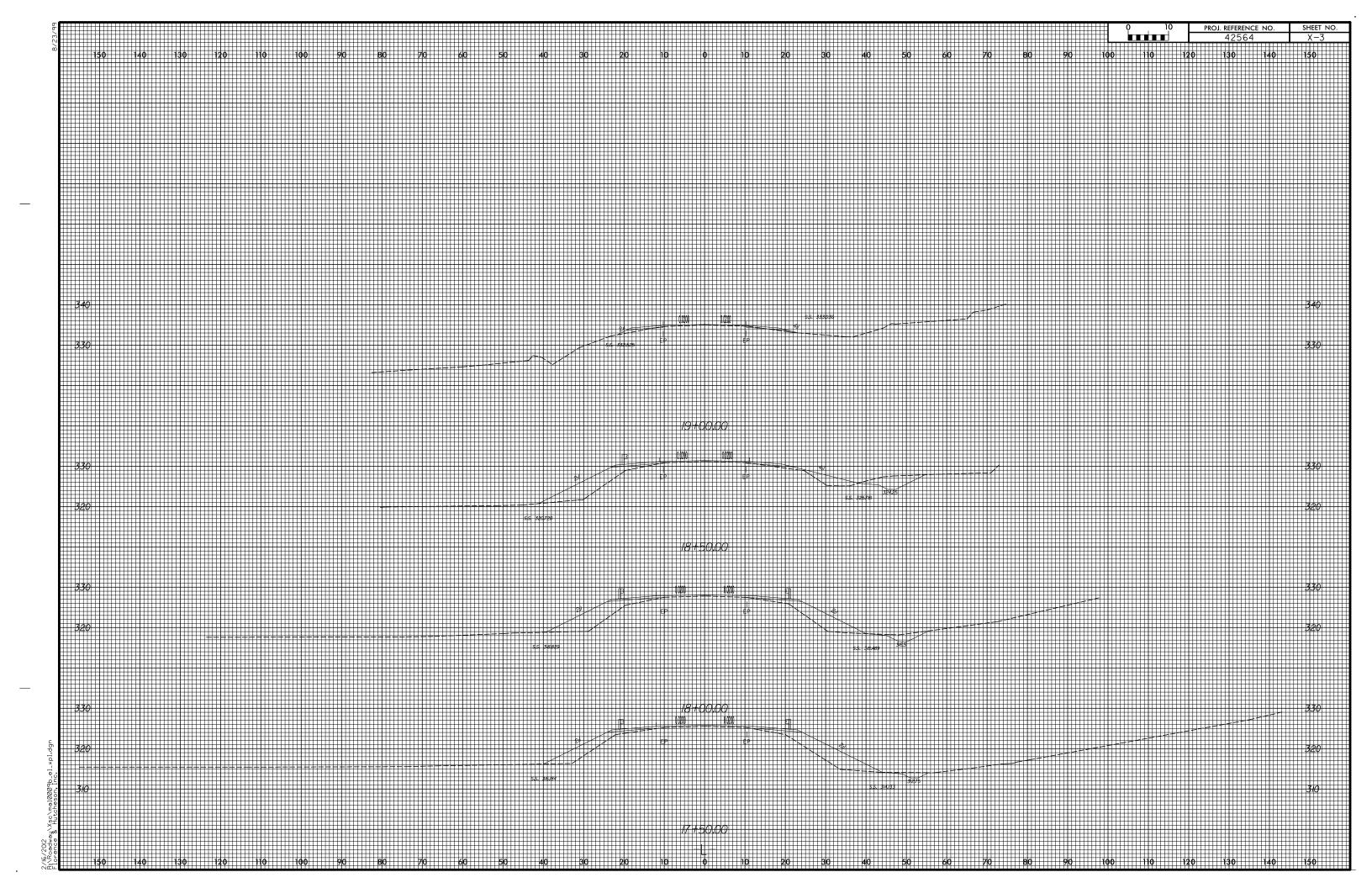
2006 STANDARD SPECIFICATIONS

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

> ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.







# 7BP. **PROJECT:**

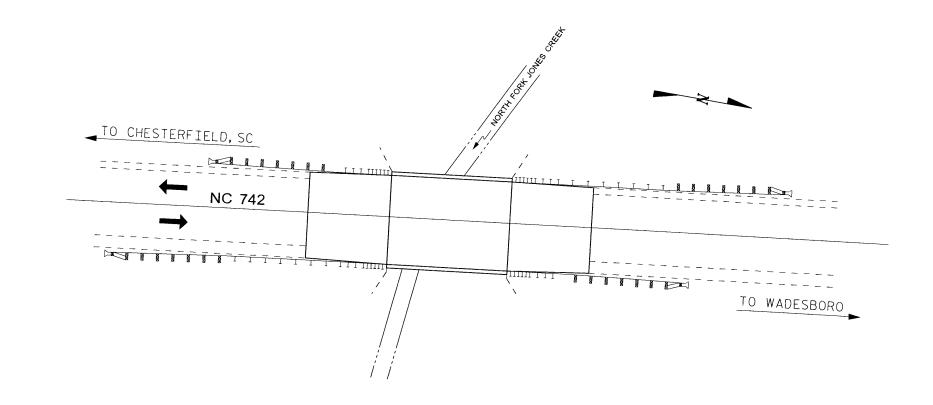
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

### ANSON COUNTY

N.C. 1 STATE PROJ. NO. F. A. PROJ. NO. 17BP.10.H.2 PE 17BP.10.H.2 CONST.

LOCATION: BRIDGE #36 ON NC 742 OVER NORTH FORK JONES CREEK 0.1 MILES SOUTH OF JUNCTION OF NC 109

TYPE OF WORK: BRIDGE SUPERSTRUCTURE REPLACEMENT



Church **PROJECT** LOCATION-<u>1140</u> <u>1138</u> VICINITY MAP

#### INDEX OF SHEETS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
1 THRU S-10	STRUCTURE PLANS

SN STANDARD NOTES

PROJECT LENGTH

PREPARED IN THE OFFICE OF: MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER: P-0671

FOR THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: *N.A*.

LETTING DATE: MARCH 22, 2012 MORRIS ISRAELNAIM, PE

RICK NELSON, PE



FARZIN ASEFNIA, PE



#### GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING (SUPERSTRUCTURE ONLY).

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

THE BRIDGE SUPERSTRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING SUPERSTRUCTURE CONSISTING OF 1 SPAN @ 37'-1" WITH A BITUMINUOUS CONCRETE FILLED STEEL PLANK DECK ON STEEL I-BEAMS ON REINFORCED CONCRETE ABUTMENTS, AND LOCATED AT THE SITE OF THE PROPOSED BRIDGE REHABILITION SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY POSTED BELOW LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE STRUCTURE FURTHER DETERIORATE, THE LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT

REMOVAL OF THE EXISTING BRIDGE SUPERSTRUCTURE SHALL BE PERFORMED SO AS TO NOT ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE SUPERSTRUCTURE AND SUBMIT PLANS IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

PROPERTIES AND DIMENSIONS OF THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAVILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON THE DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

FOR FORMWORK AND FALSEWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

PLANS DEVELOPED BASED ON FIELD MEASUREMENTS. NO SURVEY DATA AVAILABLE. CONTRACTOR MUST VERIFY FIELD CONDITIONS.

FOR TRAFFIC CONTROL PLANS, SEE STRUCTURE PLANS FOR BRIDGE  $\pm 22$ , PROJECT NO. 42564.1.3 .

PROJECT NO. WBS 17BP.10.H.2 ANSON COUNTY

N/A STATION: 36

BRIDGE NO .:

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

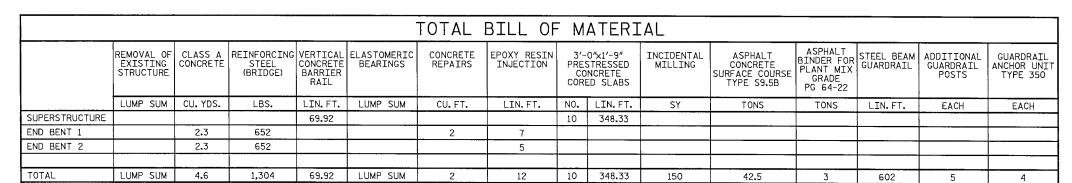
FOR BRIDGE #36 ON NC HWY 742 OVER NORTH FORK JONES CREEK

MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-067

2/02/2012

SFAL

REVISIONS SHEET NO S-I NO. BY: DATE: TOTAL SHEETS

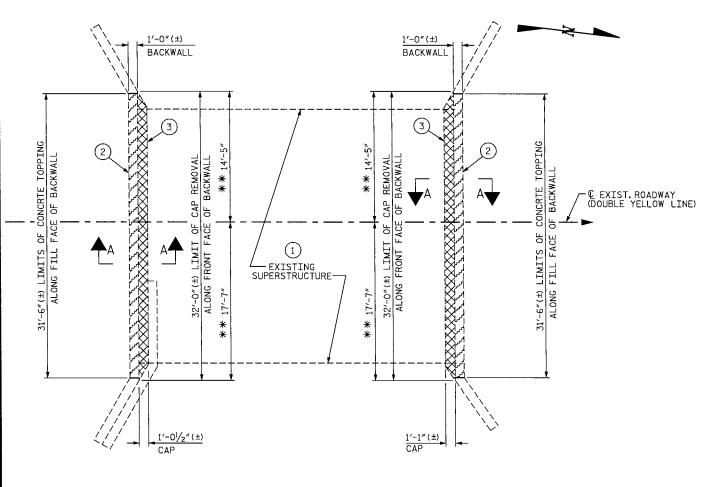


ABUTMENT 2

ABUTMENT 1

**PROFILE** 

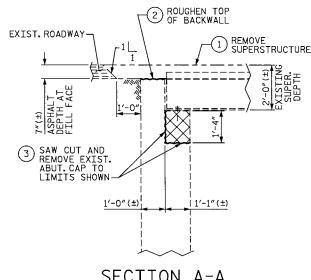
DRAWN BY : B.E. ATKINSON DATE : 10/11 CHECKED BY : M. ISRAELNAIM DATE : 11/11

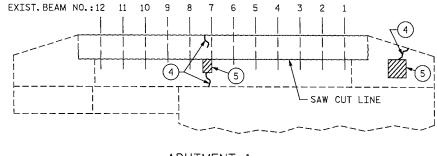


ABUTMENT 2

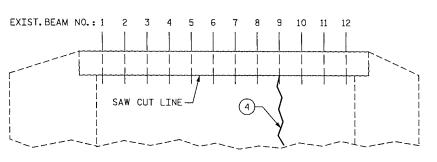
#### **DEMOLITION PLAN**

- (1) REMOVE EXISTING SUPERSTRUCTURE.
- ROUGHEN TOP OF BACKWALL TO PREPARE SURFACE FOR CONCRETE TOPPING.
- 3 SAW CUT ABUTMENT CAP FROM FRONT FACE OF BACKWALL TO FRONT FACE OF ABUTMENT CAP TO DEPTH OF 1'-4".
- \*\* FIELD VERIFY DISTANCE ALONG FRONT FACE OF BACKWALL





ABUTMENT 1 (LOOKING SOUTH)



ABUTMENT 2

#### REPAIR PLAN

- 4 INJECT EPOXY RESIN IN CRACK ALONG FRONT FACE OF BACKWALL, CAP AND WINGWALL AS SHOWN.
- 5 CLEAN AND PATCH SPALLED AND CRACKED SURFACE AREA ALONG FRONT FACE OF CAP AND WINGWALL AS SHOWN.

NOTES

DIMENSIONS SHOWN IN THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE AND ARE APPROXIMATE.

REMOVE PORTIONS OF EXISTING STRUCTURE IN ACCORDANCE WITH SECTION 402 OF THE STANDARD SPECIFICATIONS EXCEPT AS NOTED HEREIN.

THE CONTRACTOR SHALL EXERCISE CARE TO ENSURE THAT EXISTING STRUCTURAL ELEMENTS THAT ARE TO REMAIN IN PLACE ARE UNDAMAGED BY DEMOLITION ACTIVITIES. ALL DAMAGE TO EXISTING STRUCTURAL ELEMENTS THAT ARE TO REMAIN SHALL BE REPAIRED BY THE CONTRACTOR TO THE SATISFACTION OF THE ENGINEER AT NO COST TO THE DEPARTMENT.

ALL REINFORCING STEEL EXPOSED IN THE ABUTMENT DURING SAW CUT ACTIVITIES SHALL BE CUT FLUSH AT THE SAW CUT LINE AND DISCARDED.

2/02/2012

PROJECT NO. WBS 17BP.10.H.2 ANSON COUNTY N/A STATION: 36 BRIDGE NO .:

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
> RALEIGH

EXISTING BRIDGE DEMOLITION AND REPAIR **PLAN** 



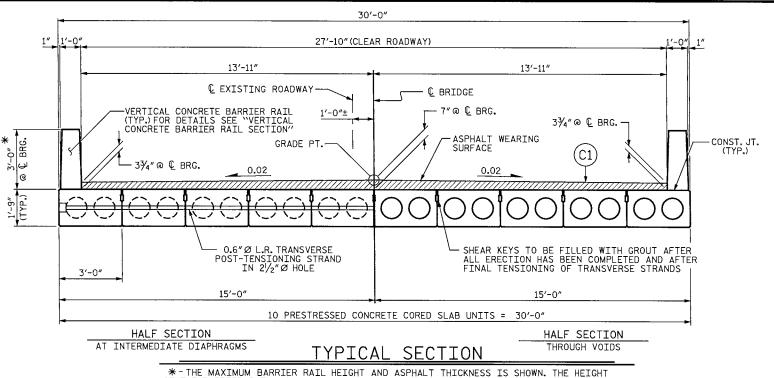
1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

		SHEET NO.				
٥.	BY:	DATE	NO.	BYs	DATE:	S-2
			3			TOTAL SHEETS
2			4			ll ll

DRAWN BY : B.E. ATKINSON DATE : 10/11 CHECKED BY : M. ISRAELNAIM DATE : 11/11

ABUTMENT 1

SECTION A-A



\*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE, FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

FIXED END C JT. AT ABUTMENT 11/2" JT. ASPHALT WEARING -21/2" Ø DOWEL HOLE SURFACE 12″Ø-VOIDS GROUT EXISTING ABUTMENT 2"Ø BACKER ROD BEARING PAD -SEE "ABUTMENT" SHEETS FOR DETAILS & #6 DOWELS

PROP. ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1"DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.

1'-6"  $9^{1/2}$   $9^{1/2}$   $8^{1/2}$ 4" 4" 1'-2" 

#### END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

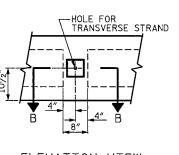


02/02/2012 90° SKEW

REVISIONS DATE: NO. BY: (919) 851-6606 FIRM PE NUMBER : P-0671

MI ENGINEERING 1011 SHAUB DRIVE, SUITE 10 RALEIGH, NC 27606

SECTION AT ABUTMENT



51/4" WITH GROUT OUTSIDE FACE OF EXTERIOR

ELEVATION VIEW

POST-TENSIONED STRAND OF CORED SLABS

+

DRAWN BY . B.E. LANNING \_\_ DATE : \_\_10/11 CHECKED BY : B.E. ATKINSON \_\_ DATE : 10/11

€ 0.6″Ø L.R. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A NON-CORROSIVE PIPE.

STRÅND VISE

GROUTED RECESS AT END OF

SECTION B-B

DEBONDING LEGEND

3'-0"

1'-4"

4" 4"

10"

2 SPA. -@ 2"CTS.

1'-6"

└ 4 SPA.

STRAND LAYOUT FOR 34'-10"UNIT

(9 STRANDS REQUIRED)

INTERIOR SLAB SECTION

0.6" Ø LOW RELAXATION

10"

Līṣ̣́ ¤ VOIDS 첫

- 2 SPA

@ 2"CTS. @ 2"CTS.

PROJECT NO. WBS 17BP.10.H.2

1'-4"

12″Ø VOIDS →

EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-0"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

OPTIONAL FULL LENGTH DEBONDED STRANDS.
THESE STRANDS ARE NOT REQUIRED. IF THE
FABRICATOR CHOOSES TO INCLUDE THESE STRANDS
IN THE CORED SLAB UNIT, THE STRANDS SHALL
BE DEBONDED FOR THE FULL LENGTH OF THE UNIT
AT NO ADDITIONAL COST. SEE STANDARD
SPECIFICATIONS ABITCLE 1078-7

SPECIFICATIONS, ARTICLE 1078-7.

3%" CL.

10"

ANSON COUNTY N/A STATION:

36 BRIDGE NO .:

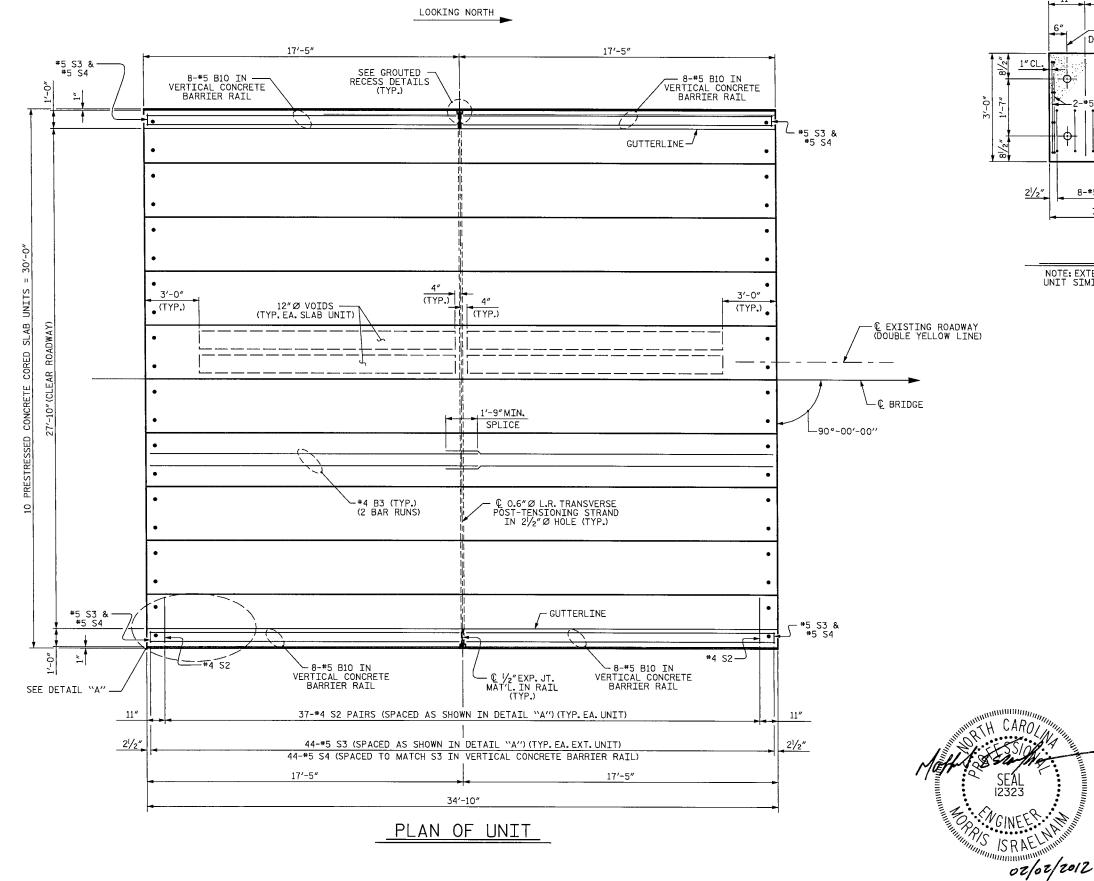
SHEET 1 OF 3

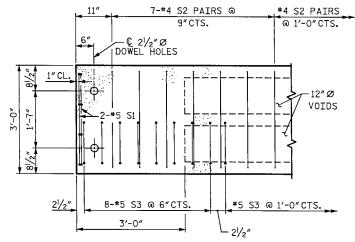
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT

SHEET NO.

S-3





DETAIL "A"

NOTE: EXTERIOR UNIT SHOWN - INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S3 BARS.

PROJECT NO. WBS 17BP.10.H.2

ANSON COUNTY

STATION: N/A

BRIDGE NO.: 36

SHEET 2 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

PLAN OF 34'-10"UNIT 27'-10" CLEAR ROADWAY 90° SKEW

MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

	REVI	SION	\$		SHEET NO.
BY:	DATE:	NO.	BY≥	DATE:	S-4
		3			TOTAL SHEETS
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DRAWN BY : B.E. LANNING DATE : 10/11
CHECKED BY : B.E. ATKINSON DATE : 10/11

#### ELASTOMERIC BEARING DETAILS

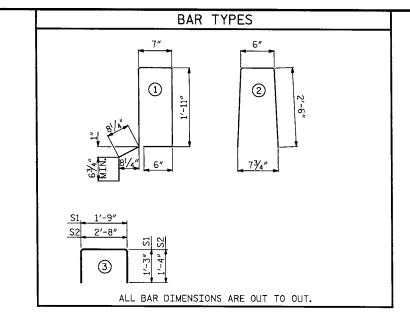
(TYPE I - 20 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

GRADE 270 STRANDS					
	0.6"Ø L.R.				
AREA (SQUARE INCHES)	0.217				
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600				
APPLIED PRESTRESS (LBS.PER STRAND)	43,950				

CORED	SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH					
34'-10" UNIT								
EXTERIOR C.S.	2	34'-10"	69'-8"					
INTERIOR C.S.	8	34'-10"	278'-8"					
TOTAL	10		348'-4"					

CONCRETE RELE	ASE STRENGTH
UNIT	PSI
34'-10"UNITS	4000



GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
27'-10" CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
	NORMAL CROWN SECTION	
34'-10" UNITS	3 <sup>3</sup> / <sub>8</sub> "	2'-11 <sup>5</sup> /8"

1'-0"

BI	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARF	RIER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL_NO.	SIZE	TYPE	LENGTH	WEIGHT
	34′-10″ UNIT					
<b>*</b> B10	32	32	#5	STR	17′-0″	567
* S4	88	88	#5	2	5′-6″	505
* EPOX	Y COATED REINFORCING STEEL		l	LBS.	<u></u>	1072
CLASS	AA CONCRETE			CU.YDS.		7.1
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		69.92

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
34'-10" CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1/2"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	1/8"
FINAL CAMBER	3/8″ ♦

34'-10" CORED SLAB UNIT	0.6" Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1/2"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	1/8"
FINAL CAMBER	3/8"

<u> </u>												
BILL OF MATERIAL FOR ONE												
					AB UNI							
ļ				EXTERT	OR UNIT	INTERIO	OR UNIT					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT					
B3	4	#4	STR	18'-3"	49	18'-3"	49					
		-										
S1	8	#5	3	4'-3"	35	4'-3"	35					
S2	74	#4	3	5'-4"	264	5′-4″	264					
* S3	44	#5	1	6'-2"	283							
REINF	ORCING :	STEEL	LB:	5.	348		348					
* EP0>	XY COATE	D										
REIN	<b>NFORCING</b>	STEEL	LB:	5.	283							
5000	P.S.I. CO	NCRETE	CU. YDS	5.	5.1		5.1					
0.6"Ø	L.R. STR	ANDS	No	) <b>.</b>	9	-	9					
			-			-						

**NOTES** 

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $21\!/_2\text{"}\varnothing$  DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE 2"  $\varnothing$  BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER, SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEFT IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

PROJECT NO. WBS 17BP.10.H.2 ANSON COUNTY N/A STATION: 36 BRIDGE NO .: SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
90° SKEW



MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

02/02/2012

CAROLIN

ISRAEL MA

SFAL

REVISIONS SHEET NO NO. BY: S-5 BY: DATE DATE: TOTAL SHEETS

@ € BRG. @ MIDSPAN	1" 10" 1" 2"CL. MIN.	
3'-0"  VARIES (SEE "GUTTERLINE ASPHALT" THICKNESS & RAII HETGHT" TABLE)	23/8"CL. 8-7/2 8-7	CHAMFER  CHAMFER  CONST. JT.  CHAMFER  CHAMFER

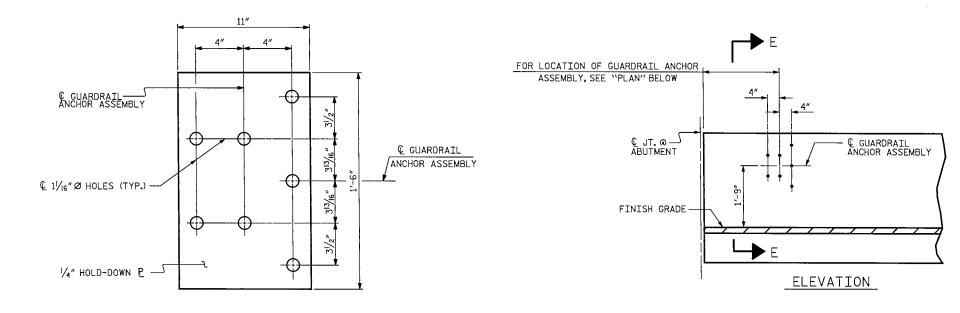
VERTICAL CONCRETE BARRIER RAIL SECTION

#5 S3 (SEE "PLAN OF UNIT" FOR SPACING)

DRAWN BY : B.E. LANNING \_ DATE : \_\_10/11 CHECKED BY . B.E. ATKINSON 

CONST. JT. -

+



© 1/8" Ø X 1'-2" BOLT WITH ROUND WASHERS (TYP.)

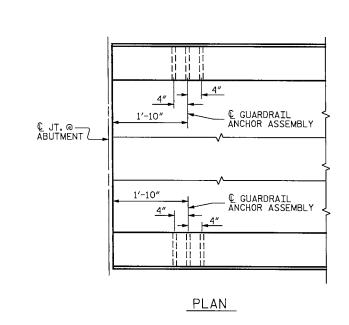
-⊈ GUARDRAIL ANCHOR ASSEMBLY

·11/4" Ø HOLE (TYP.)

PLAN

SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS



#### LOCATION OF ANCHORS FOR GUARDRAIL

ABUTMENT #1 SHOWN, ABUTMENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $^1\!\!/_4$  "HOLD DOWN PLATE AND 7 -  $^1\!\!/_8$  " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $7_8\%$  GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

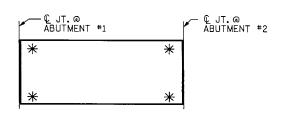
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4 % Holes shall be formed or drilled with a core bit. Impact tools will not be permitted. Any concrete damaged by this work shall be repaired to the satisfaction of the engineer.



#### SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. WBS 17BP.10.H.2 ANSON COUNTY N/A STATION: 36 BRIDGE NO .: .

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL



1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

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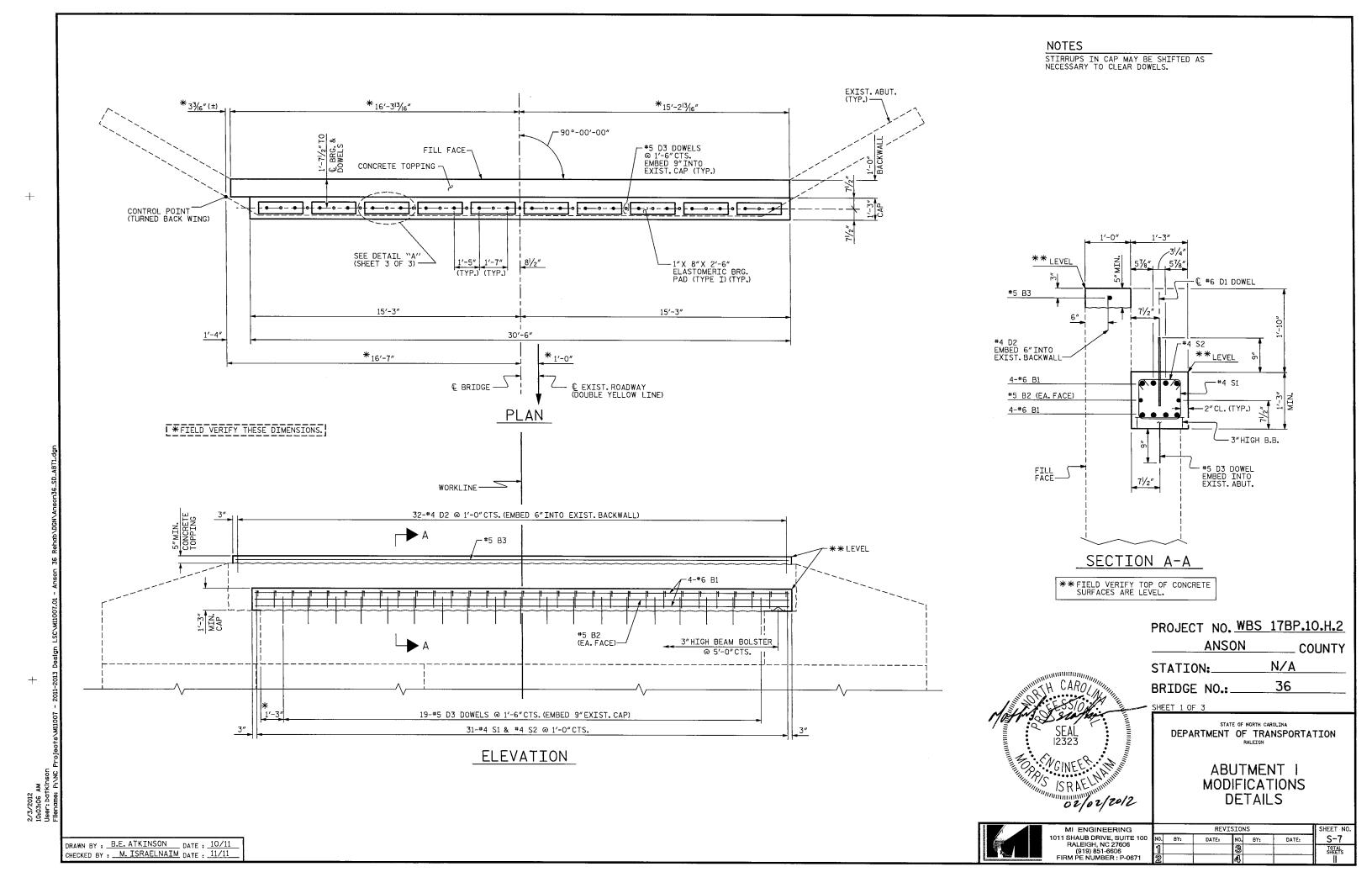
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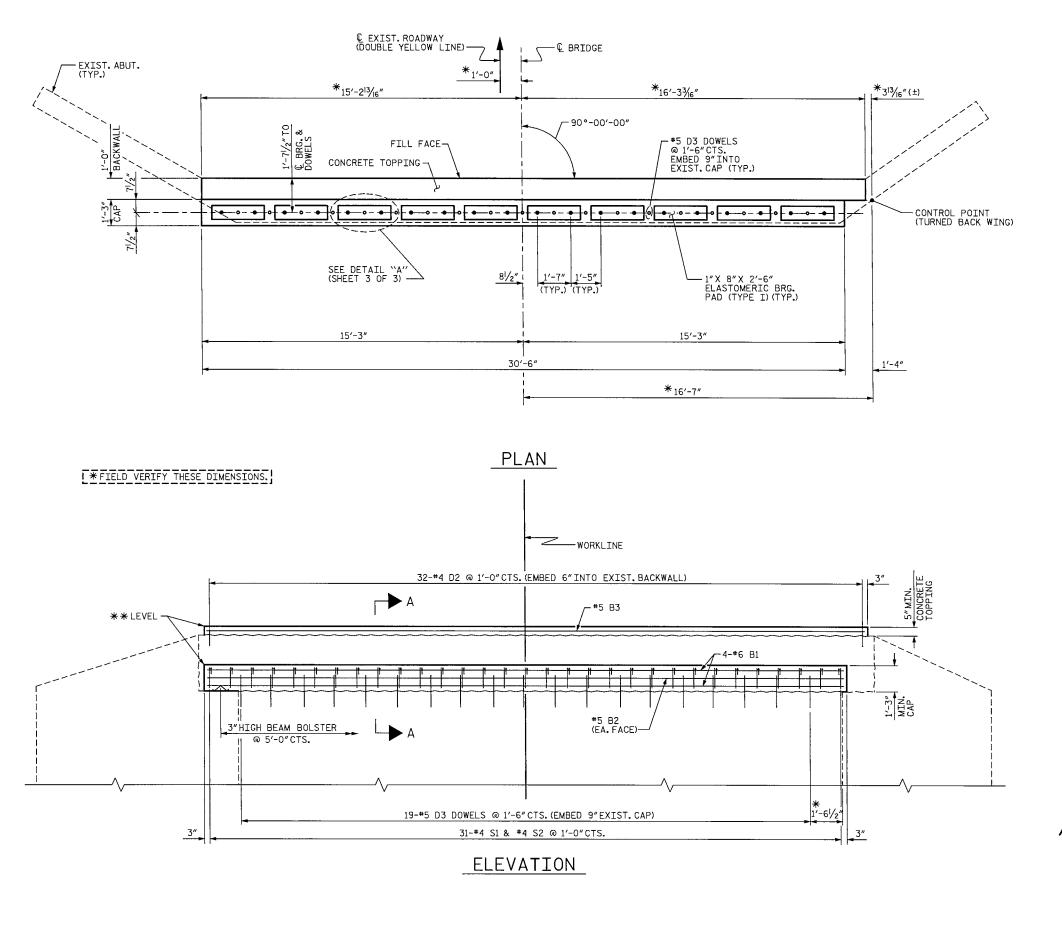
ASSEMBLED BY : B.E. LANNING CHECKED BY : B.E. ATKINSON DATE : 10/11 DATE : 10/11

1/4" HOLD-DOWN P-

02/02/2012

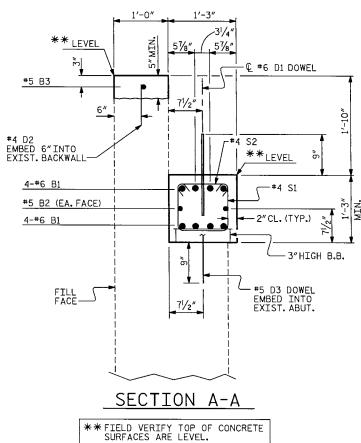
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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.



PROJECT NO. WBS 17BP.10.H.2

ANSON COUNTY
STATION: N/A

BRIDGE NO.: 36

SHEET 2 OF 3

STATE OF NORTH CAROLLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

ABUTMENT 2 MODIFICATIONS DETAILS

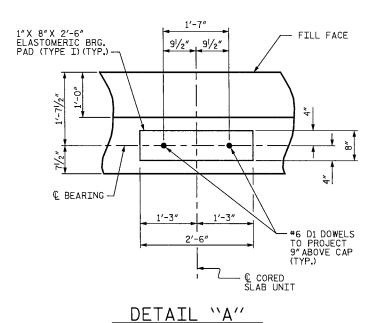


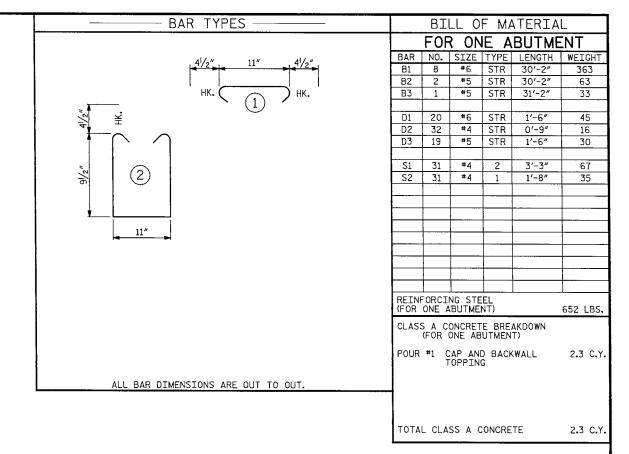
MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER: P-0671

02/02/2012

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DRAWN BY : B.E. ATKINSON DATE : 10/11
CHECKED BY : M. ISRAELNAIM DATE : 11/11





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SEAL 12323

SEAL 12323

SEAL 12323

PROJECT NO. WBS 17BP.10.H.2

ANSON COUNTY

STATION: N/A

BRIDGE NO.: 36

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

ABUTMENT DETAILS & BILL OF MATERIAL



MI ENGINEERING 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

		REV	ISION	s		SHEET NO.
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DRAWN BY : B.E. ATKINSON DATE : 10/11
CHECKED BY : M. ISRAELNAIM DATE : 11/11

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										STRE	NGTH	I LIM	IT ST	ATE				SE	RVICE	III	LIMIT	STA	TE	
									MOMENT					SHEAR						MOMENT				
LEVEL		VEHIOLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN (f†)	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN (ft)	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN (ft)	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.032		1.75	0.28	1.36	34.8′	EL	16.9	0.561	1.03	34.8′	EL	1.7	0.80	0.28	1.05	34.8′	EL	16.9	
DESIGN LOAD RATING		HL-93(0pr)	N/A		1.338		1.35	0.28	1.77	34.8′	EL	16.9	0.561	1.34	34.8′	EL	1.7	N/A						
		HS-20(Inv)	36.000	2	1.189	42.810	1.75	0.28	1.79	34.8′	EL	13.5	0.561	1.19	34.8′	EL	1.7	0.80	0.28	1.39	34.8′	EL	16.9	
IVATINO		HS-20(0pr)	36.000		1.542	55.494	1.35	0.28	2.32	34.8′	EL	13.5	0.561	1.54	34.8′	EL	1.7	N/A						
		SNSH	13.500		2,400	32.402	1.4	0.28	3.89	34.8′	EL	16.9	0.561	3.06	34.8′	EL	1.7	0.80	0.28	2.40	34.8′	EL	16.9	
		SNGARBS2	20.000		2.052	41.044	1.4	0.28	3.29	34.8′	EL	13.5	0.561	2.32	34.8′	EL	1.7	0.80	0.28	2.05	34.8′	EL	13.5	
		SNAGRIS2	22,000		2.053	45.174	1.4	0.28	3,26	34.8′	EL	13.5	0.561	2.21	34.8′	EL	1.7	0.80	0.28	2.05	34.8′	EL	13.5	
		SNCOTTS3	27.250		1.202	32.744	1.4	0.28	1.95	34.8′	EL	16.9	0.561	1.54	34.8′	EL	1.7	0.80	0.28	1.20	34.8′	EL	16.9	
	SV	SNAGGRS4	34.925		1.111	38.816	1.4	0.28	1.80	34.8′	EL	16.9	0.561	1.38	34.8′	EL	1.7	0.80	0.28	1.11	34.8′	EL	16.9	
į		SNS5A	35.550		1.079	38,354	1.4	0.28	1.75	34.8′	EL	16.9	0.561	1.46	34.8′	EL	1.7	0.80	0.28	1.08	34.8′	EL	16.9	
		SNS6A	39.950		1.041	41.601	1.4	0.28	1.69	34.8′	EL	16.9	0.561	1.37	34.8′	EL	1.7	0.80	0.28	1.04	34.8′	EL	16.9	
LEGAL		SNS7B	42.000	3	1,000	42.000	1.4	0.28	1.61	34.8′	EL	16.9	0.561	1.40	34.8′	EL	1.7	0.80	0.28	1.00	34.8′	EL	16.9	
LOAD RATING	ļ	TNAGRIT3	33.000		1.286	42,439	1.4	0.28	2.08	34.8′	EL	16.9	0.561	1.60	34.8′	EL	1.7	0.80	0.28	1.29	34.8′	EL	16.9	
RATING		TNT4A	33.075		1.285	42.512	1.4	0.28	2.08	34.8′	EL	16.9	0.561	1.51	34.8′	EL	1.7	0.80	0.28	1.29	34.8′	EL	16.9	
1		TNT6A	41.600		1.126	46.840	1.4	0.28	1.82	34.8′	EL	16.9	0.561	1.48	34.8′	EL	1.7	0.80	0.28	1.13	34.8′	EL	16.9	
	ST	TNT7A	42.000		1.163	48,833	1.4	0.28	1.89	34.8′	EL	16.9	0.561	1.37	34.8′	EL	1.7	0.80	0.28	1.16	34.8′	EL	16.9	
	=	TNT7B	42.000		1,144	48.061	1.4	0.28	1.85	34.8′	EL	16.9	0.561	1,33	34.8′	EL	1.7	0.80	0.28	1.14	34.8′	EL	16.9	
		TNAGRIT4	43.000		1.158	49.810	1.4	0.28	1.86	34.8′	EL	13.5	0.561	1,28	34.8′	EL	1.7	0.80	0.28	1.16	34.8′	EL	16.9	
		TNAGT5A	45,000		1.068	48.071	1.4	0.28	1.73	34.8′	EL	16.9	0.561	1.35	34.8′	EL	1.7	0.80	0.28	1.07	34.8′	EL	16.9	
<u> </u>		TNAGT5B	45.000		1.031	46.373	1.4	0.28	1.67	34.8′	EL	16.9	0.561	1.21	34.8′	EL	1.7	0.80	0.28	1.03	34.8′	EL	16.9	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\text{DW}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. WBS 17BP.10.H.2

COUNTY

S-I0

**ANSON** 

N/A STATION:\_

BRIDGE NO .: \_\_

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

LRFR SUMMARY FOR 34'-10"CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

02/02/2012 REVISIONS 1011 SHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671 DATE: NO. BY:

LRFR SUMMARY FOR SPAN 'A'

DRAWN BY : B.E. LANNING DATE : 10/11
CHECKED BY : B.E. ATKINSON DATE : 10/11

---- A.A.S.H.T.O. (CURRENT) **SPECIFICATIONS** 

---- SEE PLANS LIVE LOAD

IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O.

STRESS IN EXTREME FIBER OF

STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN.

RETNEORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SQ. IN.

CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN.

---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS. PER SQ. IN.

COMPRESSION PERPENDICULAR TO GRAIN

OF TIMBER - - - -375 LBS. PER SQ. IN.

EQUIVALENT FLUID PRESSURE OF EARTH - - - - -30 LBS. PER CU. FT.

(MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS, AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

#### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.
ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. ACTUAL BEAM CAMBER.

ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR AFTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" SHEAR STUDS FOR THE %4" STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" STUDS FOR 4 - 3/4" STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7.8" STUDS ALONG THE BEAM AS SHOWN FOR 3/4" STUDS BASED ON THE RATIO OF 3 - 7/8" STUDS STUDS FOR 4 - 3/4" STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING, CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

**ENGLISH** 

JANUARY, 1990

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## SEAL 15876 Mehrt 41 yehr

#### GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FEILD CONDITIONS OR RESULT IN DUPLICATE, OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING OR REMOVAL OF DEVICES, AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER.

#### TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

#### SIGNING

- B) STATE FORCES WILL BE RESPONSIBLE FOR PROVIDING PERMANENT SIGNING.
- C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING
  TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN THE ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

 ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

#### TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

#### PAVEMENT MARKINGS AND MARKERS

G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME	MARKING	MARKERS
1. NC 742	THERMOPLASTIC	PERMANENT RAISED

H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

#### MISCELLANEOUS

 TRAFFIC CONTROL PLANS AND QUANTITIES ARE APPLICABLE FOR THE CONSTRUCTION OF BOTH BRIDGE NO. 22 AND BRIDGE NO. 36.

DAY TIEL

#### ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS"-ROADWAY DESIGN UNIT-N.C. DEPARTMENT OF TRANSPORTATION-RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	TITLE
1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1135.01	CONES
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES & OFFSETS
1205.02	PAVEMENT MARKINGS - 2 LANE & MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	PAVEMENT MARKER SPACING
1251.01	RAISED PAVEMENT MARKERS
1261.01	GUARDRAIL & BARRIER DELINEATOR SPACING
1261.02	GUARDRAIL & BARRIER DELINEATOR TYPES
1262.01	GUARDRAIL END DELINEATION

#### PHASE I

- STEP 1: USING ROADWAY STANDARD DRAWING NUMBER 1101.04, SHEET 1 OF 1, INSTALL ALL DETOUR SIGNING KEEPING SIGNS COVERED (SEE SHEET 22).
- STEP 2: USING ROADWAY STANDARD DRAWING NUMBER 1101.03, SHEET 1 OF 9, CLOSE NC 742 TO TRAFFIC, UNCOVER ALL DETOUR SIGNING AND SHIFT TRAFFIC TO DETOUR (SEE SHEET 22).
- STEP 3: DISMANTLE AND REMOVE EXISTING BRIDGES NUMBER 22 & 36.

DAY TTEM

- STEP 4: COMPLETE CONSTRUCTION OF PROPOSED STRUCTURES, APPROACH ROADWAY TIE-INS, AND ASSOCIATED ITEMS.
- STEP 5: PLACE FINAL PAVEMENT MARKINGS (THERMOPLASTIC) AND MARKERS (PERMANENT RAISED) ON NC 742.
- STEP 6: USING ROADWAY STANDARD DRAWING NUMBER 1101.04, SHEET 1 OF 1, REMOVE ALL DETOUR SIGNING, ALL TRAFFIC CONTROL DEVICES AND OPEN NC 742 TO TRAFFIC.

PROJECT NO. 42564 COUNTY: ANSON STATION: 16+07.50

PROJECT NO. 17BP.10.H.2 COUNTY: ANSON STATION: N/A

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGN

BRIDGE NO.22 AND BRIDGE NO.36 ON NC 742 OVER NORTH FORK JONES CREEK

REVISIONS ONES THEN THE NO. BY DATE

FINAL PAV'T MARKING SCHEDULE

DECORPORA

PROJECT NO.: 42564 PROJECT NO.: 17BP.10.H.2

SIMBUL	L DESCRIPTION		PAT TIEM	STMDUL	- DESCRIPTION		LAI TIEM
		BREAKDOWN				BREAKDOWN	
		PAVEMENT MARKING	LINES			PAVEMENT MARKING LINES	
TA	WHITE EDGELINE	1200 LF	THERMOPLASTIC (4", 90mils)	TA	WHITE EDGELINE	174 LF	THERMOPLASTIC (4", 90mils)
TI	YELLOW DOUBLE CENTER LINE	1200 LF	THERMOPLASTIC (4", 120mils)	TI	YELLOW DOUBLE CENTER LINE	174 LF	THERMOPLASTIC (4", 120mils)
		PAVEMENT MARK	ERS			PAVEMENT MARKERS	
MA	YELLOW AND YELLOW	9 EA	PERMANENT RAISED	MA	YELLOW AND YELLOW	3 EA	PERMANENT RAISED



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eigh, NC 27607 1 NC License No: F-0258 2

NO. BY DATE NO. BY DATE

