

Traffic Capacity Analysis

U-4700 US 321 Widening

Catawba, Burke, and Caldwell Counties, North Carolina

Prepared For:

North Carolina Department of Transportation



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**Capacity Analysis Report
US 321 Widening
(NCDOT STIP U-4700)**

**An Analysis of 2040 No-Build Alternative and 2040 Build
Alternatives**

For:

North Carolina Department of Transportation

By:

AECOM Technical Services of North Carolina

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1.0 INTRODUCTION

This report summarizes the capacity analysis findings for North Carolina Department of Transportation (NCDOT) State Transportation Improvement Program (STIP) Project U-4700. Project U-4700 consists of widening of US 321 for 14 miles in Catawba, Burke, and Caldwell Counties. The project limits have been revised and now begin at US 70 in Hickory and end at Southwest Boulevard in Lenoir. Several major intersections along the study corridor will be considered for improvements and upgrades. In the vicinity of the project, US 321 serves as a primary north-south route connecting several cities and towns to each other and to Interstate 40.

The analysis area is shown below in the **Vicinity Map**. The intersection list for this analysis is as follows:

- 1. US 321 at 13th Street SW
- 2. US 321 at Main Avenue Drive NW
- 3. US 321 at 2nd Avenue NW
- 4. US 321 at 7th Avenue NW
- 5. US 321 at 9th Avenue NW
- 6. US 321 at Clement Boulevard
- 7. US 321 at 13th Avenue Drive NW
- 8. US 321 at 15th Avenue NW/14th Avenue Circle NW
- 9. US 321 at Grace Chapel Road
- 10. US 321 at Alex Lee Blvd
- 11. US 321 at Pooveys Grove Church Road
- 12. US 321 at US 321 A/River Bend Drive
- 13. US 321 at Pinewood Road
- 14. US 321 at N. Highland Avenue
- 16. US 321 at Lower Cedar Valley Rd
- 17. US 321 at Quarry Estates Rd
- 18. US 321 at Pine Mountain Rd
- 19. US 321 at CCCTI South Entrance
- 20. US 321 at CCCTI Main Entrance
- XX. US 321 at CCCTI North Entrance
- 21. US 321 at Mount Herman Road
- 25. 12th Avenue NW at Old Lenoir Road
- 33. 1st Street SW Connector at 2nd Avenue SW
- 33. 2nd Avenue SW at 14th Street SW
- 34. 2nd Avenue SW at 13th Street SW
- 35. 1st Avenue SW at 17th Avenue SW
- 36. 1st Avenue SW at 1st Street SW Connector
- 38. 1st Avenue SW at 13th Street SW
- 37. 1st Avenue SW at 14th Street SW
- 44. 12th Street Drive at Old Lenoir Road
- 441. Clement Blvd at 12th Street Drive
- 50. Pinewood Road at N. Highland Avenue
- 2. Pinewood at Dudley Shoals
- 7. US 321 at Dudley Shoals

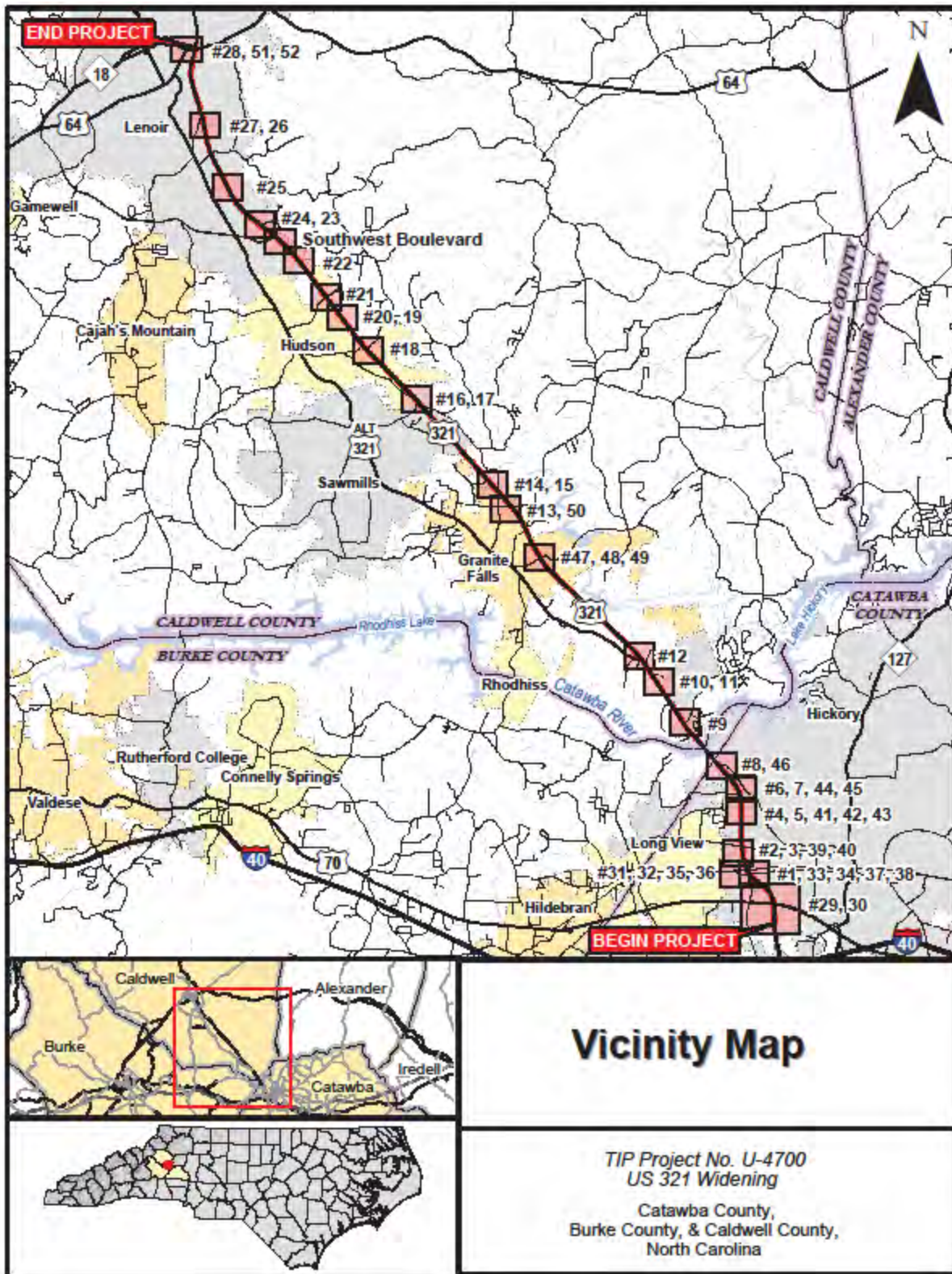


Figure 1

2.0 SCOPE

This report includes an evaluation of the following alternatives:

- 2040 No-Build Alternative
- 2040 Build Alternative
 - 13th Street
 - One-Way Split Partial Cloverleaf Interchange
 - Partial Cloverleaf Interchange
 - Two-Way Partial Cloverleaf Interchange
 - Two-Way Split Partial Cloverleaf Interchange
 - Clement Boulevard
 - Partial Cloverleaf Interchange
 - Superstreet
 - Grace Chapel Road
 - Superstreet
 - Reverse Superstreet
 - Dudley Shoals Road
 - At-Grade Intersection
 - Freeway Ramp
 - Lower Cedar Valley Road & Pine Mountain Road
 - Mount Herman Road & Caldwell Community College and Technical Institute
 - MDI Simulation Analysis

Using the traffic forecast information provided by the NCDOT (dated February 2017) and located in **Appendix A**, a Synchro, SimTraffic, and Highway Capacity Software capacity analysis was performed. Per the current scope of the project, the analysis included no-build conditions as well as build conditions with recommendations on future improvements.

3.0 EXISTING CONDITIONS

US 321 is a four-lane divided highway that is a primary north-south corridor providing connections between Hickory and Lenoir. It carries 42,100 vehicles per day south of US 70, dropping to 25,000 vehicles per day north of SR 1933 (Southwest Blvd) and peaks at 43,200 vehicles across the Catawba River. It has a posted speed limit of 55 miles per hour (mph) south of US 70, 45 mph through Hickory, NC, and 55 mph north of the Catawba River. Approximately 6-13% of this traffic is heavy vehicles.

13th St SW is a four-lane divided east-west roadway providing access to businesses. It carries 3,800 vehicles per day west of US 321, and 4,500 vehicles per day east of US 321, with a posted speed limit of 35 mph. Approximately 9% of this traffic is heavy vehicles.

SR 1371 (Clement Blvd) is a four-lane undivided east-west roadway providing access to businesses and residences in northwest Hickory. It carries 4,500 vehicles per day west of US 321, and 13,100 vehicles per day east of US 321, with a posted speed limit of 35 mph. Approximately 3-5% of this traffic is heavy vehicles.

SR 1751 (Grace Chapel Rd) is two-lane undivided an east-west roadway providing access to residential communities. It carries 7,000 vehicles per day with a posted speed limit of 45 mph. Approximately 4% of this traffic is heavy vehicles.

SR 1102 (Dudley Shoals Rd) is a two-lane undivided east-west roadway providing access to residential communities. It carries 3,200 vehicles per day, with a posted speed limit of 35 mph. Approximately 8% of this traffic is heavy vehicles.

SR 1108 (Lower Cedar Valley Rd) is a two-lane undivided north-south roadway providing access to residential communities. It carries 7,300 vehicles per day west of US 321, and 3,100 vehicles per day east of US 321, with a posted speed limit of 45 mph. Approximately 4-6% of this traffic is heavy vehicles.

SR 1809 (Pine Mountain Rd) is a two-lane undivided east-west roadway providing access to residential communities. It carries 6,900 vehicles per day west of US 321, and 7,000 vehicles per day east of US 321, with a posted speed limit of 35 mph. Approximately 4% of this traffic is heavy vehicles.

SR 1160 (Mount Herman Rd) is a two-lane undivided east-west roadway providing access to residential communities and is within the vicinity of Caldwell Community College. It carries 8,600 vehicles per day west of US 321, and 6,900 vehicles per day east of US 321, with a posted speed limit of 35 mph. Approximately 3% of this traffic is heavy vehicles.

4.0 METHOD OF ANALYSIS

4.1 Traffic Volume Assumptions

The traffic forecast used for this project was conducted and furnished to AECOM by NCDOT (dated February 2017), as shown in **Appendix A**. Using this forecast and NCDOT's Intersection Analysis Utility (IAU) tool, AM and PM peak hour volumes were developed for all of the alternatives being evaluated. The NCDOT 2016-2025 STIP was reviewed, and no fiscally constrained projects within or adjacent to the project study area are assumed to be in place. The IAU spreadsheets for all scenarios are located in **Appendix B**.

4.2 Capacity Analysis

The highway capacity analyses performed were based on methodologies from the *Highway Capacity Manual (HCM 2010)*. Traffic modeling software used in the capacity analyses were *Synchro 9.1* and *SimTraffic 9.1, (Build 910, Rev 24)*, and *Highway Capacity Software (HCS) 2010 Version 6.80*. The analyses were conducted in accordance with the latest NCDOT Congestion Management Unit's *Capacity Analysis Guidelines for TIP projects*, dated July 2015.

The traffic carrying ability of a roadway is described by levels of service (LOS) that range from LOS A to LOS F. **Table 1** defines the traffic flow conditions and approximate driver comfort level at each level of service for signalized and unsignalized intersections, as well as basic freeway segments and ramp merges/diverges. Note that the delays associated with LOS for signalized intersections are different from those associated with unsignalized intersections. *HCM 2010* explains that drivers perceive that a signalized intersection is designed to carry higher traffic volumes, and therefore expect to experience greater delays at signalized intersections. A signalized intersection is described by a single LOS. Unsignalized intersections are assigned a LOS for each minor movement. Basic freeway segments and ramps are described by density (passenger cars per mile per lane (pc/mi/ln)) or volume-to-capacity (v/c) ratios.

Table 1 – Level of Service (LOS) Index					
LOS	Traffic Flow Conditions	Delay (sec) Signalized Intersections	Delay (sec) Unsignalized Intersections	Basic Freeway Segment Density (pc/mi/ln)	Ramp Merge/Diverge Density (pc/mi/ln)
A	Progression is extremely favorable and most vehicles do not stop at all.	≤ 10	≤ 10	≤ 11	≤ 10
B	Good progression, some delay.	> 10 - 20	> 10 - 15	> 11 - 18	> 10 - 20
C	Fair progression, higher delay.	> 20 - 35	> 15 - 25	> 18 - 26	> 20 - 28
D	Unfavorable progression, congestion becomes apparent.	> 35 - 55	> 25 - 35	> 26 - 35	> 28 - 35
E	Poor progression, substantial delay.	> 55 - 80	> 35 - 50	> 35 - 45	> 35, and v/c < 1.0
F	Poor progression, extreme delay.	> 80	> 50	> 45, or v/c > 1.0	v/c > 1.0

Source: HCM 2010

For the queue analysis, simulation runs (using SimTraffic) were performed to analyze the queuing and blocking problems. The queue reported is the greater of the SimTraffic maximum queue and the Synchro 95th percentile queue.

5.0 FINDINGS

5.1 2040 No-Build Alternative

A capacity analysis was performed for 2040 No-Build Alternative AM and PM peak periods for all of the study intersections and freeway network. The analyses were based on signal design plans and congestion management guidelines from NCDOT (confirmed by observations in the field). Levels of service are summarized in **Table 2**, while traffic volumes are shown in **Figures 1A, 1B, and 1C**. Levels of service and lane configurations are shown in **Figure 2A, 2B, and 2C**, and detailed Synchro and SimTraffic reports are located in **Appendix C**. This analysis was performed to establish a baseline for comparison of travel times between US 70 and US 321 ALT, and not the entire project network.

Table 2 – 2040 No-Build Alternative Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
1	US 321 at 13th Street SW	C	C
	Eastbound Left	F	F
	Eastbound Left / Through / Right	E	E
	Westbound Left	E	E
	Westbound Through	E	E
	Westbound Right	A	A
	Northbound Left	E	D
	Northbound Through	C	C
	Northbound Right	A	A
	Southbound Left	F	E
	Southbound Through	A	A
	Southbound Right	A	A
2	US 321 at Main Avenue Drive NW	-	-
	Northbound Left	D	C
	Southbound Left	C	C
	Eastbound Left / Through / Right	F	F
	Westbound Left / Through / Right	F	F

*As per the HCM 2010, V/C ration in excess of 1.0 is LOS F.

Table 2 (Continued) - 2040 No-Build Alternative Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
3	US 321 at 2nd Avenue NW	E	E
	Eastbound Left	E	E
	Eastbound Through / Right	F	F
	Westbound Left / Through / Right	F	F
	Northbound Left	F	F
	Northbound Through	C	F*
	Northbound Right	A	A
	Southbound Left	E	E
	Southbound Through / Right	F*	C
4	US 321 at 7th Avenue NW	B	C
	Eastbound Left / Through / Right	E	E
	Westbound Left	E	E
	Westbound Through / Right	F	D
	Northbound Left	E	E
	Northbound Through	A	C
	Northbound Right	A	B
	Southbound Left	E	E
	Southbound Through / Right	B	A
5	US 321 at 9th Avenue NW	B	C
	Eastbound Left	F	F
	Eastbound Through / Right	E	D
	Westbound Left	F	D
	Westbound Through / Right	F	D
	Northbound Left	E	E
	Northbound Through / Right	A	C
	Southbound Left	F	E
	Southbound Through / Right	A	A

*As per the HCM 2010, V/C ration in excess of 1.0 is LOS F.

Table 2 (Continued) - 2040 No-Build Alternative Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
6	US 321 at Clement Boulevard	E	E
	Eastbound Left	F	E
	Eastbound Left / Through	F	F
	Eastbound Right	E	D
	Westbound Left	F	F
	Westbound Left / Through	F	F
	Westbound Right	F	D
	Northbound Left	E	D
	Northbound Through	F*	F*
	Northbound Right	B	A
	Southbound Left	D	F
	Southbound Through / Right	B	B
7	US 321 at 13th Avenue Drive NW	-	-
	Eastbound Right	F	C
	Northbound Left	D	C
8	US 321 at 15th Avenue NW & 14th Avenue Circle NW	C	F
	Eastbound Left	E	E
	Eastbound Through	E	E
	Eastbound Right	D	D
	Westbound Left / Through	E	E
	Westbound Right	D	F
	Northbound Left	F	E
	Northbound Through	B	F
	Northbound Right	B	A
	Southbound Left	F	E
	Southbound Through	B	B
	Southbound Right	A	A

*As per the HCM 2010, V/C ration in excess of 1.0 is LOS F.

Table 2 (Continued) - 2040 No-Build Alternative Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
9	US 321 at Grace Chapel Road	C	B
	Westbound Left	F	E
	Westbound Right	E	E
	Northbound U-Turn	F	F
	Northbound Through	A	F*
	Northbound Right	A	A
	Southbound Left	E	E
	Southbound Through	C	A
10	US 321 at Alex Lee Boulevard	B	A
	Eastbound Left / Through	E	E
	Eastbound Right	D	D
	Westbound Left / Through	F	E
	Westbound Right	D	D
	Northbound Left	E	D
	Northbound Through	C	A
	Northbound Right	B	A
	Southbound Left	E	E
	Southbound Through / Right	A	A
11	US 321 at Pooveys Grove Church Rd	-	-
	Westbound Right	C	D
12	US 321 at US 321A & River Bend Dr	D	D
	Eastbound Left	E	D
	Eastbound Through	F	F
	Eastbound Right	A	A
	Westbound Left	F	E
	Westbound Through	D	D
	Westbound Right	C	C
	Northbound Left	F	E
	Northbound Through	C	C
	Northbound Right	B	A
	Southbound Left	E	E
	Southbound Through	D	C
	Southbound Right	A	A

Movements with zero delay were omitted

*As per the HCM 2010, V/C ration in excess of 1.0 is LOS F.

Eight (8) out of the twelve (12) intersections operate at LOS D or better in both peak hours. The intersections of US 321 at 13th Ave Dr NW, and US 321 at 15th Ave NW/20th Ave NW operate at LOS F in one peak hour, and LOS C in one peak hour. The intersections of US 321 at 2nd Ave NW, and US 321 at Clement Blvd operate at LOS E in both peak hours.

The average travel time per vehicle in the simulation is 7 minutes in the AM peak period and 7.2 minutes in the PM peak period. The average travel distance per vehicle is 2.1 miles with an average speed of 21 mph in the AM and 20 mph in the PM.

5.2 2040 Build Alternative

5.2.1 13th Street

5.2.1.1 One-Way Split Partial Cloverleaf Interchange

A capacity analysis was performed for 2040 Build Alternative – 13th Street – One-Way Split Partial Cloverleaf Interchange AM and PM peak periods for all of the study intersections and freeway network. Intersection levels of service are summarized in **Table 3A**, HCS results are summarized in **Table 3B**, and queue reports are summarized in **Table 3C**. Traffic volumes are shown in **Figures 3A** and **3B**. Levels of service and lane configurations are shown in **Figures 4A** and **4B**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix D**.

Table 3A – 2040 Build Alternative – 13th Street One-Way Split Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
10	15th St SW / 15th St Connector at 1st Ave SW	-	-
	Northbound Left / Through	B	B
	Southbound Through	C	C
	Southbound Right	B	B
32	15th St SW at 1st Ave SW Connector / 2nd Ave SW	-	-
	Northbound Through / Right	B	B
	Southbound Left / Through	B	B
33	14th St SW at 2nd Ave SW	-	-
	Northbound Through / Right	B	B
	Southbound Left / Through	C	C
34	2nd Ave SW at 13th St SW	-	-
	Northbound Through	B	B
	Northbound Right	B	B
	Southbound Left / Through	B	B
35	2nd Ave SW at 13th St SW		
	Eastbound Left / Through / Right		
	Westbound Left / Through / Right		
	Northbound Left		
	Northbound Through / Right		
	Southbound Left / Through / Right		

Table 3A (Continued) – 2040 Build Alternative – 13th Street One-Way Split Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
37	14th St SW at 1st Ave SW	-	-
	Northbound Left / Through	C	C
	Southbound Through / Right	B	C
38	13th St SW at 1st Ave SW	-	-
	Northbound Left / Through	B	B
	Southbound Through / Right	B	B

Movements with zero delay were omitted

All six (6) intersections operate at LOS C or better in both peak hours.

Table 3B – 2040 Build Alternative – 13th Street One-Way Split Partial Cloverleaf Interchange HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges and Diverges			
201	US 321 NB – Ramp to 13th St	C	C
202	US 321 SB – Ramp from 13th St	B	B
203	US 321 NB – Ramp from 13th St	B	C
204	US 321 SB – Ramp to 13th St	C	B

All four (4) ramps operate at LOS C or better in both peak hours.

Table 3C – 2040 Build Alternative – 13th Street One-Way Split Partial Cloverleaf Interchange Queuing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
10	15th St SW / 15th St Connector at 1st Ave SW	SBR	53	56	100
34	2nd Ave SW at 13th St SW	NBR	27	49	100

None of the intersections exhibit “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.1.2 Partial Cloverleaf Interchange

A capacity analysis was performed for 2040 Build Alternative – 13th Street – Partial Cloverleaf Interchange AM and PM peak periods for all of the study intersections and freeway network. Intersection levels of service are summarized in **Table 4A**, HCS results are summarized in **Table 4B**, and queue reports are summarized in **Table 4C**. Traffic volumes are shown in **Figures 5A** and **5B**. Levels of service and lane configurations are shown in **Figures 6A** and **6B**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix E**.

Table 4A – 2040 Build Alternative – 13th Street Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
1	13th St SW at US 321 On/Off Ramp	-	-
	Westbound Left / Right	B	B
	Southbound Left / Through	A	A
	Southbound Through	A	A
2	13th St SW at US 321 SB Off Ramp	-	-
	Eastbound Left / Right	B	B
	Northbound Left / Through	A	A
	Northbound Through	A	A
10	15th St SW at 1st Ave SW	-	-
	Northbound Left	A	B
32	15th St SW at 1st Ave SW Connector / 2nd Ave SW	-	-
	Northbound Through / Right	B	A
	Southbound Left / Through	B	B
33	14th St SW at 2nd Ave SW	-	-
	Northbound Through / Right	B	A
	Southbound Left / Through	B	B
34	2nd Ave SW at 13th St SW	-	-
	Northbound Through	B	B
	Northbound Right	B	B
	Southbound Left	B	B
	Southbound Through	B	C

Table 4A (Continued) - 2040 Build Alternative – 13th Street Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
35	17th Ave SW at 1st Ave SW	B	B
	Eastbound Left / Through / Right	A	A
	Westbound Left / Through / Right	A	B
	Northbound Left	B	B
	Northbound Through / Right	B	B
	Southbound Left / Through / Right	C	C
37	14th St SW at 1st Ave SW	-	-
	Northbound Left / Through	C	C
	Southbound Through / Right	B	C
38	13th St SW at 1st Ave SW	-	-
	Northbound Left / Through	B	C
	Southbound Through / Right	B	B

Movements with zero delay were omitted

All nine (9) intersections operate at LOS C or better in both peak hours.

Intersection 35 was added to only this scenario to determine if it would function acceptably. A traffic count was taken May 30-31, 2017. The traffic forecast was used to calculate the growth factor resulting in 1.146 for 2040. The intersection functions well with a V/C ratio of less than 0.6. See **Appendix B** for the traffic count and growth factor calculations.

Table 4B – 2040 Build Alternative – 13th Street Partial Cloverleaf Interchange HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges and Diverges			
201	US 321 NB – Ramp to 13th St	B	C
202	US 321 SB – Ramp from 13th St	B	B
203	US 321 NB – Ramp from 13th St	B	B
204	US 321 SB – Ramp to 13th St	C	B

All four (4) ramps operate at LOS C or better in both peak hours.

5.2.1.3 Two-Way Partial Cloverleaf Interchange

A capacity analysis was performed for 2040 Build Alternative – 13th Street – Two-Way Partial Cloverleaf Interchange AM and PM peak periods for all of the study intersections and freeway network. Intersection levels of service are summarized in **Table 5A**, HCS results are summarized in **Table 5B**, and queues are summarized in **Table 5C**. Traffic volumes are shown in **Figures 7A** and **7B**. Levels of service and lane configurations are shown in **Figures 8A** and **8B**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix F**.

Table 5A – 2040 Build Alternative – 13th Street Two-Way Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
1	13th St SW at US 321 On/Off Ramp	-	-
	Westbound Left / Right	B	B
	Southbound Left	A	A
2	13th St SW at US 321 SB Off Ramp	-	-
	Eastbound Left / Right	B	B
	Northbound Left	A	A
32	15th St SW at 1st St SW Connector / 2nd Ave SW	-	-
	Westbound Left	A	A
	Northbound Through / Right	A	A
	Southbound Left / Through	A	A
34	2nd Ave SW at 13th St SW	-	-
	Northbound Right	B	B
	Southbound Left / Through	B	B
35	12th St SW at 2nd Ave SW / 2nd Ave	-	-
	Northbound Through / Right	B	B
	Southbound Left / Through	B	B
36	15th St SW at 1st Ave SW	-	-
	Westbound Left / Through	A	A
	Northbound Left / Right	B	B
37	1st Ave SW at 14th St SW	-	-
	Eastbound Left	A	A
	Southbound Right	B	B

Table 5A (Continued) - 2040 Build Alternative – 13th Street Two-Way Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
38	1st Ave SW at Driveway	-	-
	Southbound Right	B	B
39	12th St SW at 1st Ave SW / 1st Ave	-	-
	Northbound Left / Through	B	B
	Southbound Through / Right	B	B

Movements with zero delay were omitted

All nine (9) intersections operate at LOS B or better in both peak hours.

Table 5B – 2040 Build Alternative – 13th Street Two-Way Partial Cloverleaf Interchange HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges and Diverges			
201	US 321 NB – Ramp to 13th St	C	C
202	US 321 SB – Ramp from 13th St	C	B
203	US 321 NB – Ramp from 13th St	B	C
204	US 321 SB – Ramp to 13th St	C	C

All four (4) ramps operate at LOS C or better in both peak hours.

Table 5C – 2040 Build Alternative – 13th Street Two-Way Partial Cloverleaf Interchange Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
1	13th St SW at US 321 On/Off Ramp	SBL	31	31	100

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.1.4 Two-Way Split Partial Cloverleaf Interchange

A capacity analysis was performed for 2040 Build Alternative – 13th Street – Two-Way Split Partial Cloverleaf Interchange AM and PM peak periods for all of the study intersections and freeway network. Intersection levels of service are summarized in **Table 6A**, and HCS results are summarized in **Table 6B**. Traffic volumes are shown in **Figures 9A** and **9B**. Levels of service and lane configurations are shown in **Figures 10A** and **10B**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix G**.

Table 6A – 2040 Build Alternative – 13th Street Two-Way Split Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
32	15th St SW at 1st St SW Connector / 2nd Ave SW	-	-
	Eastbound Left / Through / Right	A	A
	Westbound Left / Through / Right	A	A
	Northbound Left / Through / Right	B	B
	Southbound Left / Through / Right	C	C
33	US 321 SB Off Ramp at 2nd Ave SW	-	-
	Westbound Left / Through	A	A
	Northbound Left / Right	C	C
34	13th St SW at 2nd Ave SW	-	-
	Eastbound Left / Through / Right	A	A
	Westbound Left / Through / Right	A	A
	Northbound Left / Through / Right	F	D
	Southbound Left / Through / Right	C	B
36	15th St SW at 1st Ave SW	-	-
	Westbound Left / Through	A	A
	Northbound Left / Right	B	B
37	14th St SW at 1st Ave SW	-	-
	Eastbound Left / Through / Right	A	A
	Westbound Left / Through / Right	A	A
	Northbound Left / Through / Right	B	B
	Southbound Left / Through / Right	B	B

Table 6A (Continued) - 2040 Build Alternative – 13th Street Two-Way Split Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
38	13th St SW / Driveway at 1st Ave SW	-	-
	Eastbound Left / Through / Right	A	A
	Westbound Left / Through / Right	A	A
	Northbound Left / Through / Right	B	B
	Southbound Left / Through / Right	B	B

Movements with zero delay were omitted

Five (5) of the six (6) intersections operate at LOS C or better in both peak hours. The intersection of 13th St SW at 2nd Ave SW operates at LOS F in the AM peak hour, and LOS D in the PM peak hour, stemming from the minor side street turning movement. There is available capacity for this movement being less than 0.85. If a signal were added, it would have an LOS of B. However, a traffic signal may not be warranted by the projected future year volumes and thus was not included in the analysis.

Table 6B – 2040 Build Alternative – 13th Street Two-Way Split Partial Cloverleaf Interchange HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges and Diverges			
201	US 321 NB – Ramp to 13th St	C	C
202	US 321 SB – Ramp from 13th St	C	B
203	US 321 NB – Ramp from 13th St	B	C
204	US 321 SB – Ramp to 13th St	C	C

All four (4) ramps operate at LOS C or better in both peak hours.

5.2.2 Clement Boulevard

5.2.2.1 Partial Cloverleaf Interchange

A capacity analysis was performed for 2040 Build Alternative – Clement Boulevard – Partial Cloverleaf Interchange AM and PM peak periods for all of the study intersections and freeway network. Intersection levels of service are summarized in **Table 7A**, HCS results are summarized in **Table 7B**, and queue reports are summarized in **Table 7C**. Traffic volumes are shown in **Figure 11**. Levels of service and lane configurations are shown in **Figure 12**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix H**.

Table 7A – 2040 Build Alternative – Clement Boulevard Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
1	US 321 SB Ramps at Clement Blvd	A	A
	Eastbound Through	B	B
	Eastbound Right	A	A
	Westbound Left	B	B
	Westbound Through	A	A
	Northbound Left	D	D
	Northbound Right	A	A
2	US 321 NB Ramps at Clement Blvd	B	C
	Eastbound Left	D	E
	Eastbound Through	B	C
	Westbound Through	C	C
	Westbound Right	A	B
	Southbound Left	B	B
	Southbound Right	A	A
3	Clement Blvd / Old Lenoir Rd at 12th Ave	B	B
	Eastbound Left	A	B
	Eastbound Through	A	A
	Westbound Through / Right	D	C
	Southbound Left	C	C
	Southbound Right	A	B

Table 7A (Continued) – 2040 Build Alternative – Clement Boulevard Partial Cloverleaf Interchange Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
4	12th Ave at 12th St Dr	C	B
	Eastbound Through / Right	E	D
	Westbound Left	C	B
	Westbound Through	A	A
	Northbound Left	D	D
	Northbound Right	A	A

Movements with zero delay were omitted

All four (4) intersections operate at LOS C or better in both peak hours.

Table 7B – 2040 Build Alternative – Clement Boulevard Partial Cloverleaf Interchange HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges and Diverges			
201	US 321 NB – Ramp to Clement Blvd	B	C
202	US 321 SB – Ramp from Clement Blvd	C	B
203	US 321 NB – Ramp from Clement Blvd	C	D
204	US 321 SB – Ramp to Clement Blvd	D	C

All four (4) ramps operate at LOS D or better in both peak hours.

Table 7C – 2040 Build Alternative – Clement Boulevard Partial Cloverleaf Interchange Queuing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
1	US 321 SB Ramps at Clement Blvd	EBR	53	52	250
		WBL	246	232	400
		NBL	150	129	Continuous Lane
2	US 321 NB Ramps at Clement Blvd	EBL	93	128	250
		WBR	221	799	Continuous Lane
		SBL	197	244	250
		SBR	133	48	Continuous Lane
3	Clement Blvd / Old Lenoir Rd at 12th Ave	EBL	371	297	250/ Continuous Lane
		SBL	274	225	175
		SBR	400	397	Continuous Lane
4	12th Ave at 12th St Dr	WBL	700	367	600
		NBL	112	180	100
		NBR	155	290	Continuous Lane

One queue for Old Lenoir Rd exceeds the proposed storage lengths by approximately 125 feet. The southbound left exceeds the storage length, additional storage is not recommended as it would require a wider roadway.

The Queues for 12th St Dr do exceed the proposed storage lengths. Westbound left storage is proposed as 600 feet, additional storage would require further widening and would not be recommended. The southbound left exceeds the storage length, additional storage is not recommended as would require a wider roadway to be constructed.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues with the exceptions noted above.

5.2.2.2 Superstreet

A capacity analysis was performed for 2040 Build Alternative – Clement Boulevard – Superstreet AM and PM peak periods for all of the study intersections. Intersection levels of service are summarized in **Table 8A**, and queue reports are summarized in **Table 8B**. Traffic volumes are shown in **Figure 13**. Levels of service and lane configurations are shown in **Figure 14**, and detailed Synchro and SimTraffic reports are located in **Appendix I**.

Table 8A – 2040 Build Alternative – Clement Boulevard Superstreet Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
25	12th St Dr NW / Old Lenoir Rd at Clement Blvd	C	B
	Eastbound Left	B	A
	Eastbound Through / Right	B	A
	Westbound Left	E	D
	Westbound Through	B	A
	Westbound Right	B	A
	Northbound Left	B	C
	Northbound Through	B	C
	Northbound Right	B	C
	Southbound Left	D	D
	Southbound Through / Right	C	C
43	SB US 321 at 7th Ave NW / 7th Ave NW Left-Over	B	C
	Eastbound Right	D	D
	Northwestbound Left	D	B
	Southbound Through	A	C
	Southbound Right	A	A
44	NB US 321 at 7th Ave NW U-Turn	B	C
	Southeastbound Left	D	D
	Northbound Through	A	C

Table 8A (Continued) - 2040 Build Alternative – Clement Boulevard Superstreet Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
61	NB US 321 at Clement Blvd / Clement Blvd Left-Over	B	B
	Westbound Right	C	C
	Southeastbound Left	C	D
	Northbound Through	B	A
	Northbound Right	A	A
62	SB US 321 at 13th Ave Dr NW / Clement Blvd U-Turn	B	A
	Eastbound Right	A	A
	Northwestbound Left	D	D
	Southbound Through / Right	B	A
63	SB US 321 at Clement Blvd / Clement Blvd Left-Over	C	C
	Eastbound Right	E	D
	Northwestbound U-Turn	D	D
	Northwestbound Left	D	D
	Southbound Through	B	B
	Southbound Right	A	A
64	NB US 321 at 9th Ave NW / Clement Blvd U-Turn	C	C
	Westbound Right	D	E
	Southeastbound Left	C	D
	Northbound Through / Right	B	B
441	Old Lenoir Rd / 12th Ave NW	A	B
	Eastbound Left	B	C
	Eastbound Through	A	A
	Westbound Through / Right	B	B
	Southbound Left	D	D
	Southbound Right	A	A

Movements with zero delay were omitted

All eight (8) intersections operate at LOS C or better in both peak hours.

Table 8B –2040 Build Alternative – Clement Boulevard Superstreet Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
25	12th St Dr NW / Old Lenoir Rd at Clement Blvd	EBL	113	74	175
		WBL	250	102	150
		WBR	131	151	125
		NBL	26	88	125
		SBL	367	156	Continuous Lane
		SBTR	135	65	450
43	SB US 321 at 7th Ave NW / 7th Ave NW Left-Over	EBR	252	541	Continuous Lane
		NWL	234	91	225
		SBR	157	111	250
44	NB US 321 at 7th Ave NW U-Turn	SEL	137	418	425
61	NB US 321 at Clement Blvd / Clement Blvd Left-Over	WBR	346	200	250/ Continuous Lane
		NBR	54	114	Continuous Lane
		SEL	175	282	350
62	SB US 321 at 13th Ave Dr NW / Clement Blvd U-Turn	NWL	365	134	Continuous Lane
63	SB US 321 at Clement Blvd / Clement Blvd Left-Over	EBR	191	169	Continuous Lane
		NWL	173	137	450
		SBR	156	116	Continuous Lane
64	NB US 321 at 9th Ave NW / Clement Blvd U-Turn	WBR	273	287	250/ Continuous Lane
		NBTR	324	268	375
		SEL	230	267	275
441	Old Lenoir Rd / 12th Ave NW	EBL	199	299	200
		SBL	144	163	Continuous Lane
		SBR	104	258	300

Two queues for 12th St Dr at Clement Blvd exceed the existing storage lengths. The westbound left and right queues exceed the storage by approximately 50 and 75 feet respectively. Additional storage would require realignment of the existing roadway and is not recommended for only an extra 75 feet.

One queue for Old Lenoir Rd at 12th Ave exceeds the existing storage lengths. The eastbound left queue exceeds the storage by approximately 100 feet. Additional storage would require realignment of the existing roadway and is not recommended for only an extra 100 feet.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues with the exceptions noted above.

5.2.3 Grace Chapel Road

5.2.3.1 Superstreet

A capacity analysis was performed for 2040 Build Alternative – Grace Chapel Road – Superstreet AM and PM peak periods for all of the study intersections. Intersection levels of service are summarized in **Table 9A**, and queue reports are summarized in **Table 9B**. Traffic volumes are shown in **Figure 15**. Levels of service and lane configurations are shown in **Figure 16**, and detailed Synchro and SimTraffic reports are located in **Appendix J**.

Table 9A – 2040 Build Alternative – Grace Chapel Road Superstreet Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
91	US 321 at Grace Chapel Rd	B	B
	Westbound Right	D	D
	Northbound Through	B	A
	Northbound Right	A	A
	Southeastbound Left	C	D
92	SB US 321 at Grace Chapel Rd U-Turn	B	B
	Northwestbound Left	D	C
	Southbound Through	B	A
93	NB US 321 at Grace Chapel Rd U-Turn	-	-
	Southeastbound Left	C	C

Movements with zero delay were omitted

All three (3) intersections operate at LOS D or better in both peak hours.

Table 9B – 2040 Build Alternative – Grace Chapel Road Superstreet Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
91	US 321 at Grace Chapel Rd	WBR	478	226	300/ Continuous Lane
		SEL	122	53	150
92	SB US 321 at Grace Chapel Rd U-Turn	NWL	144	133	200
93	NB US 321 at Grace Chapel Rd U-Turn	SEL	28	29	200

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.3.2 Reverse Superstreet

A capacity analysis was performed for 2040 Build Alternative – Grace Chapel Road – Reverse Superstreet AM and PM peak periods for all of the study intersections. Intersection levels of service are summarized in **Table 10A**, and queue reports are summarized in **Table 10B**. Traffic volumes are shown in **Figure 17**. Levels of service and lane configurations are shown in **Figure 18**, and detailed Synchro and SimTraffic reports are located in **Appendix K**.

Table 10A – 2040 Build Alternative – Grace Chapel Road Reverse Superstreet Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
91	US 321 at Grace Chapel Rd	B	A
	Westbound Left	D	E
	Westbound Right	D	D
	Northbound Through	A	A
	Northbound Right	A	A
	Southbound Through	B	A
92	SB US 321 at Grace Chapel Rd U-Turn	-	-
	Northwestbound Left	C	B
93	NB US 321 at Grace Chapel Rd U-Turn	-	-
	Eastbound Left	C	D

Movements with zero delay were omitted

All three (3) intersections operate at LOS D or better in both peak hours.

Table 10B – 2040 Build Alternative – Grace Chapel Road Reverse Superstreet Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
91	US 321 at Grace Chapel Rd	WBL	272	273	300
		WBR	106	46	Continuous Lane
		NBR	-	-	400
92	SB US 321 at Grace Chapel Rd U-Turn	NWL	22	23	200
93	NB US 321 at Grace Chapel Rd U-Turn	EBL	45	67	200

None of the intersections present “excessive” queueing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.4 Dudley Shoals Road

5.2.4.1 At-Grade Intersection

A capacity analysis was performed for 2040 Build Alternative – Dudley Shoals Road – At-Grade Intersection AM and PM peak periods for all of the study intersections. Intersection levels of service are summarized in **Table 11A**, and queue reports are summarized in **Table 11B**. Traffic volumes are shown in **Figures 19A** and **19B**. Levels of service and lane configurations are shown in **Figures 20A** and **20B**, and detailed Synchro and SimTraffic reports are located in **Appendix L**.

Table 11A – 2040 Build Alternative – Dudley Shoals Road At-Grade Intersection Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
1	Dudley Shoals Rd at US 321 SB On-Ramp	-	-
	Eastbound Left / Through	A	A
2	Dudley Shoals Rd at Pinewood Rd	-	-
	Eastbound Left / Right	C	C
	Northbound Left / Through	A	A
7	US 321 SB at Dudley Shoals Rd On-Ramp	-	-
	Eastbound Right	F	C
131	NB US 321 at Pinewood Rd / Pinewood Rd Left-Over	C	B
	Westbound Right	D	D
	Northbound Through	B	B
	Northbound Right	B	A
	Southeastbound U-Turn	D	D
	Southeastbound Left	B	C
132	SB US 321 at Pinewood Rd U-Turn	A	A
	Northwestbound Left	E	D
	Southbound Through	A	A
133	SB US 321 at Pinewood Rd / Pinewood Rd Left-Over	B	B
	Eastbound Right	D	D
	Northwestbound Left	D	C
	Southbound Through	A	A
	Southbound Right	A	A

Table 11A (Continued) – 2040 Build Alternative – Dudley Shoals Road At-Grade Intersection Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
134	NB US 321 at Pinewood Rd U-Turn	B	C
	Eastbound Left	D	E
	Northbound Through	A	C
141	NB US 321 at Highland Ave / Highland Ave Left-Over	A	A
	Westbound Right	D	D
	Northbound Through / Right	A	A
	Southeastbound Left	D	D
142	SB US 321 at Highland Ave U-Turn	B	A
	Northwestbound Left	D	C
	Southbound Through	B	A
143	SB US 321 at Highland Ave / Highland Ave Left-Over	C	A
	Eastbound Right	E	D
	Northwestbound Left	C	D
	Southbound Through / Right	C	A

Movements with zero delay were omitted

Nine (9) of the ten (10) intersections operate at LOS C or better in both peak hours. The intersection of US 321 Southbound at the Dudley Shoals Road On-Ramp operates at LOS F in the AM peak hour, and LOS C in the PM peak hour, stemming from the minor side street turning movement.

Table 11B – 2040 Build Alternative – Dudley Shoals Road At-Grade Intersection Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
131	NB US 321 at Pinewood Rd / Pinewood Rd Left-Over	WBR	159	139	200
		NBR	62	28	Continuous Lane
		SEL	176	156	225

Table 11B (Continued) – 2040 Build Alternative – Dudley Shoals Road At-Grade Intersection Queuing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
132	SB US 321 at Pinewood Rd U-Turn	NWL	144	139	200
133	SB US 321 at Pinewood Rd / Pinewood Rd Left-Over	EBR	299	256	300/ Continuous Lane
		NWL	160	188	275
		SBR	15	12	Continuous Lane
134	NB US 321 at Pinewood Rd U-Turn	EBL	156	265	300
141	NB US 321 at Highland Ave / Highland Ave Left-Over	WBR	188	72	Continuous Lane
		SEL	45	44	100
142	SB US 321 at Highland Ave U-Turn	NWL	114	51	175
143	SB US 321 at Highland Ave / Highland Ave Left-Over	EBR	265	84	Continuous Lane
		NWL	25	94	100

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.4.2 Freeway Ramp

An analysis using a free flow ramp was also conducted at Dudley Shoals for 2040 Build Alternative. HCS results are summarized in **Table 12**, and traffic volumes are shown in **Figures 19A** and **19B**. Levels of service and lane configurations are shown in **Figure 21**, and detailed HCS reports are located in **Appendix M**.

Table 12 – 2040 Build Alternative – Dudley Shoals Road Freeway Ramp HCS Level of Service			
#	Segment	Level of Service or v/c Ratio	
		AM Peak	PM Peak
Freeway Merges			
204	US 321 SB – Ramp from Dudley Shoals Rd	C	B

The ramp operates at LOS C or better in both peak hours. This performs substantially better than stop control.

5.2.5 Lower Cedar Valley Road & Pine Mountain Road

A capacity analysis was performed for 2040 Build Alternative – Lower Cedar Valley Road & Pine Mountain Road AM and PM peak periods for all of the study intersections. Intersection levels of service are summarized in **Table 13A**, and queue reports are summarized in **Table 13B**. Traffic volumes are shown in **Figures 22A** and **22B**. Levels of service and lane configurations are shown in **Figure 23A** and **23B**, and detailed Synchro and SimTraffic reports are located in **Appendix N**.

Table 13A – 2040 Build Alternative – Lower Cedar Valley Road & Pine Mountain Road Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
161	NB US 321 at Lower Cedar Valley Rd / Lower Cedar Valley Rd Left-Over	B	B
	Westbound Right	D	D
	Northbound Through / Right	B	A
	Southeastbound U-Turn / Left	D	D
162	SB US 321 at Lower Cedar Valley Rd U-Turn	B	A
	Westbound Left	E	D
	Southbound Through	A	A
163	SB US 321 at Lower Cedar Valley Rd / Lower Cedar Valley Rd Left-Over	A	B
	Eastbound Right	D	D
	Northwestbound Left	D	C
	Southbound Through	A	A
	Southbound Right	A	A
164	NB US 321 at Lower Cedar Valley Rd U-Turn	A	C
	Eastbound Left	D	D
	Northbound Through	A	C
171	NB US 321 at Quarry Estates Rd	-	-
	Westbound Right	A	B
172	SB US 321 at Quarry Estates Rd	-	-
	Eastbound Right	B	A

Table 13A (Continued) – 2040 Build Alternative – Lower Cedar Valley Road & Pine Mountain Road Intersection Level of Service

#	Intersection	Level of Service	
		AM Peak	PM Peak
181	NB US 321 at Pine Mountain Rd / Pine Mountain Rd Left-Over	B	B
	Westbound Right	D	D
	Northbound Through	A	A
	Northbound Right	A	A
	Southeastbound Left	C	D
182	SB US 321 at Pine Mountain Rd U-Turn	C	B
	Westbound Left	D	C
	Southbound Through	C	B
183	SB US 321 at Pine Mountain Rd / Pine Mountain Rd Left-Over	B	B
	Eastbound Right	D	D
	Northwestbound Left	D	C
	Southbound Through	A	A
184	NB US 321 at Pine Mountain Rd U-Turn	A	B
	Eastbound Left	C	E
	Northbound Through	A	A
185	Pine Mountain Rd at Ramp to Pine Mountain Road	-	-
	Southbound Right	A	B

Movements with zero delay were omitted

All eleven (11) intersections operate at LOS C or better in both peak hours. One U-turn movement at Lower Cedar Valley and Pine Mountain Rd operate at LOS E in one peak hour, however the capacity is less than 0.85.

Table 13B – 2040 Build Alternative – Lower Cedar Valley Road & Pine Mountain Road Queuing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
161	NB US 321 at Lower Cedar Valley Rd / Lower Cedar Valley Rd Left-Over	WBR	203	131	150/ Continuous Lane
		SEL	125	154	225
163	SB US 321 at Lower Cedar Valley Rd / Lower Cedar Valley Rd Left-Over	EBR	249	285	250/ Continuous Lane
		NWL	278	136	300
		SBR	116	52	Continuous Lane
164	NB US 321 at Lower Cedar Valley Rd U-Turn	EBL	142	247	350
181	NB US 321 at Pine Mountain Rd / Pine Mountain Rd Left-Over	WBR	282	304	400/ Continuous Lane
		NBR	56	56	225
		SEL	181	159	275
182	SB US 321 at Pine Mountain Rd U-Turn	WBL	247	199	350
183	SB US 321 at Pine Mountain Rd / Pine Mountain Rd Left-Over	EBR	255	194	200/ Continuous Lane
		NWL	116	138	225
184	NB US 321 at Pine Mountain Rd U-Turn	EBL	182	202	250

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

5.2.6 Mount Herman Road & Caldwell Community College and Technical Institute

A capacity analysis will be performed for 2040 Build Alternative – Mount Herman Road & Caldwell Community College and Technical Institute (CCCTI) AM and PM peak periods for all of the study intersections. At present, a plan is being developed that will alter the internal circulation and the driveways at CCCTI. Once this plan is complete, the analysis along US 321 can be completed.

5.2.7 MDI Simulation Analysis

A capacity analysis was performed for 2040 Build Alternative – to determine travel time reductions for implementing a superstreet for Merchants Distributors Incorporated (MDI). Intersection levels of service are summarized in **Table 15A**, and queue reports are summarized in **Table 15B**. Traffic volumes are shown in **Figure 24A, 24B, 24C, 24D, and 24E**. Levels of service and lane configurations are shown in **Figure 25A, 25B, 25C, 25D, and 25E**, and detailed Synchro, SimTraffic, and HCS reports are located in **Appendix P**.

Table 15A – 2040 Build Alternative – MDI Simulation Analysis Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
11	US 321 at 13th St SW	-	-
	Eastbound Right	F	D
	Westbound Right	F	F
21	SB US 321 at Main Ave Dr NW	-	-
	Eastbound Right	D	D
31	NB US 321 at 2nd Ave NW / 2nd Ave NW Left-Over	B	B
	Westbound Right	D	D
	Northbound Through	A	A
	Northbound Right	A	A
	Southeastbound Left	D	C
32	SB US 321 at 2nd Ave NW U-Turn	B	A
	Northwestbound Left	D	D
	Southbound Through	A	A
33	SB US 321 at 2nd Ave NW / 2nd Ave NW Left-Over	B	B
	Eastbound Right	D	D
	Northwestbound Left	D	C
	Southbound Through	A	A
	Southbound Right	A	A
34	NB US 321 at 2nd Ave NW U-Turn	B	D
	Northbound Through	B	C
	Southeastbound Left	D	E

Table 15A (Continued) - 2040 Build Alternative – MDI Simulation Analysis Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
43	SB US 321 at 7th Ave NW / 7th Ave NW Left-Over	B	B
	Eastbound Right	D	D
	Northwestbound Left	D	B
	Southbound Through	A	B
	Southbound Right	A	A
44	SB US 321 at 7th Ave NW U-Turn	A	B
	Northbound Through	A	A
	Southeastbound Left	C	D
61	NB US 321 at Clement Blvd / Clement Blvd Left-Over	B	C
	Westbound Right	C	D
	Northbound Through	A	B
	Northbound Right	B	A
	Southeastbound Left	B	C
62	SB US 321 at 13th Ave Dr NW / Clement Blvd U-Turn	B	B
	Eastbound Right	A	A
	Westbound Left	D	D
	Southbound Through / Right	B	A
63	SB US 321 at Clement Blvd / Clement Blvd Left-Over	B	B
	Eastbound Right	E	D
	Northwestbound U-Turn	D	D
	Northwestbound Left	D	D
	Southbound Through / Right	A	B
64	NB US 321 at 9th Ave NW / Clement Blvd U-Turn	B	C
	Westbound Right	D	D
	Northbound Through / Right	B	B
	Southeastbound Left / U-Turn	D	D
81	NB US 321 at 15th Ave NW	A	A
	Westbound Right	C	D
	Northbound Through / Right	A	A
	Southeastbound Left	D	D

Table 15A (Continued) – 2040 Build Alternative – MDI Simulation Analysis Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
82	SB US 321 at 15th Avenue NW U-Turn	-	-
	Westbound Left	D	C
83	SB US 321 at 15th Avenue NW / 20th Avenue NW	-	-
	Eastbound Right	D	C
	Northwestbound Left	F	F
84	NB US 321 at 15th Avenue NW U-Turn	-	-
	Eastbound Left	C	D
91	US 321 at Grace Chapel Rd	B	A
	Westbound Left	D	D
	Westbound Right	C	D
	Northbound Through	A	A
	Northbound Right	A	A
	Southbound Through	B	A
92	SB US 321 at Grace Chapel U-Turn	-	-
	Westbound Left	C	B
93	NB US 321 at Grace Chapel U-Turn	-	-
	Eastbound Left	C	D
101	NB US 321 at Alex Lee Blvd / Alex Lee Left-Over	A	A
	Westbound Right	D	D
	Northbound Through	A	A
	Northbound Right	A	A
	Southeastbound Left	C	D
102	SB US 321 at Alex Lee Blvd U-Turn	-	-
	Northwestbound Left	F	E
103	SB US 321 at Alex Lee Blvd / Alex Lee Left-Over	-	-
	Eastbound Right	D	C
	Southbound Right	-	-
	Northwestbound Left	F	F
104	NB US 321 at Alex Lee Blvd U-Turn	-	-
	Eastbound Left	B	C

Table 15A (Continued) - 2040 Build Alternative – MDI Simulation Analysis Intersection Level of Service			
#	Intersection	Level of Service	
		AM Peak	PM Peak
121	NB US321 at River Bend Dr / River Bend Dr Left-Over	B	C
	Westbound Right	C	E
	Northbound Through	B	C
	Northbound Right	A	A
	Southeastbound Left	B	C
122	SB US 321 and River Bend Dr U-Turn	B	B
	Westbound Left	C	C
	Southbound Through	B	A
123	SB US 321 at US 321A / US 321A Left-Over	B	A
	Eastbound Right	E	D
	Northwestbound Left	C	D
	Southbound Through	B	A
	Southbound Right	A	A
124	NB US 321 at US 321A U-Turn	-	-
	Eastbound Left	D	F

22 of the 26 intersections operate at LOS D or better in both peak hours. The intersections at LOS E or F are all unsignalized intersections with volumes that would not warrant a traffic signal.

The Southbound U-turn for MDI was not signalized in the analysis due to the inadequate volume to warrant a traffic signal (<100 Vehicles per hour). It may however be necessary to install a signal for safety reasons due to the large number of trucks making the U-turn, and to provide gaps in traffic during peak hours so that the trucks are able to make the U-turn in a reasonable time frame.

Table 15B – 2040 Build Alternative – MDI Simulation Analysis Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
31	NB US 321 at 2nd Ave NW / 2nd Ave NW Left-Over	WBR	202	228	Continuous
		NBR	96	69	Continuous
		SEL	143	115	400
32	SB US 321 at 2nd Ave NW U-Turn	NWL	206	229	450
33	SB US 321 at 2nd Ave NW / 2nd Ave NW Left-Over	EBR	232	251	Continuous
		NWL	224	116	650
		SBR	117	124	Continuous
34	NB US 321 at 2nd Ave NW U-Turn	SEL	227	325	350
43	SB US 321 at 7th Ave NW / 7th Ave NW Left-Over	EBR	208	340	Continuous
		NWL	195	78	200
		SBR	102	72	Continuous
44	SB US 321 at 7th Ave NW U-Turn	SEL	71	177	350
61	NB US 321 at Clement Blvd / Clement Blvd Left-Over	WBR	351	473	Continuous
		NBR	133	137	475
		SEL	238	244	525
62	SB US 321 at 13th Ave Dr NW / Clement Blvd U-Turn	EBR	8	-	Continuous
		NWL	184	128	350
63	SB US 321 at Clement Blvd / Clement Blvd Left-Over	EBR	191	210	Continuous
		NWL	76	78	475
64	NB US 321 at 9th Ave NW / Clement Blvd U-Turn	WBR	167	149	Continuous
		NBTR	244	460	375
		SEL	161	295	300
81	NB US 321 at 15th Ave NW	WBR	93	162	Continuous
		SEL	161	85	625
82	SB US 321 at 15th Ave NW U-Turn	NWL	53	46	250
83	SB US 321 at 15th Ave NW / 20th Avenue NW	EBR	106	79	Continuous
		NWL	103	52	300
84	NB US 321 at 15th Ave NW U-Turn	SEL	41	78	300

Table 15B (Continued) – 2040 Build Alternative – MDI Simulation Analysis Queueing and Recommended Storage					
#	Intersection	Movement	Queue (feet)		Recommended Storage (feet)
			AM Peak	PM Peak	
91	US 321 at Grace Chapel Rd	WBL	246	171	500/Continuous
		WBR	58	45	500/Continuous
		NBR	-	-	550
92	SB US 321 at Grace Chapel Rd U-Turn	NWL	21	21	250
93	NB US 321 at Grace Chapel Rd U-Turn	SEL	47	80	450
101	NB US 321 at Alex Lee Blvd / Alex Lee Left-Over	WBR	186	146	Continuous
		NBR	60	71	Continuous
		SEL	46	79	475
102	SB US 321 at Alex Lee Blvd U-Turn	NWL	375	77	400
103	SB US 321 at Alex Lee Blvd / Alex Lee Left-Over	EBR	22	60	Continuous
		SBR	2	-	175
		NWL	116	22	350
104	NB US 321 at Alex Lee Blvd U-Turn	SEL	26	31	300
121	NB US321 at River Bend Dr / River Bend Dr Left-Over	WBR	334	414	Continuous
		NBR	173	178	Continuous
		SEL	102	391	575
122	SB US 321 and River Bend Dr U-Turn	WBL	511	326	575
123	SB US 321 at US 321A / US 321A Left-Over	EBR	233	219	Continuous
		NWL	133	133	600
		SBR	80	104	Continuous
124	Northbound US 321 at US 321A U-Turn	EBL	234	342	300

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

The average travel time per vehicle in the simulation is 6.4 minutes in the AM and PM peak periods. The average travel distance per vehicle is 3 miles with an average speed of 29 mph in the AM and 28 mph in the PM. This is a 18% reduction in travel time and 39% increase in travel speed.

Table 15C – 2040 Network Delay Analysis (No Build vs. Superstreet Build)

	AM Peak			PM Peak		
	No Build	Superstreet Build	% Change	No Build	Superstreet Build	% Change
Vehicles Exited (veh / hr)	9,280	7,590	-18.21%	9,152	7,567	-17.32%
Vehicles Entered (veh / hr)	9,480	7,743	-18.32%	9,449	7,823	-17.21%
Travel Distance (mi)	19,809	23,314	17.69%	19,394	23,078	19.00%
Travel Time (hr)	1,089.5	819.5	-24.78%	1,121.9	823.7	-26.58%
Average Speed (mph)	21	29	38.10%	20	28	40.00%
Total Delay (hr)	652.6	296.8	-54.52%	562.3	303.0	-46.11%
Total Stops	19,136	16,664	-12.92%	23,477	16,346	-30.37%
Fuel Usage (gal)	806.5	843.0	4.53%	792.8	833.9	5.18%
Per Veh. Distance (mi)	2.1	3.0	44.00%	2.1	3.0	43.83%
Per Veh. Time (min)	7.0	6.4	-7.97%	7.2	6.4	-11.26%
Per Veh. Delay (min)	3.0	2.1	-29.34%	3.3	2.2	-34.54%
Per Veh. Stops	1.9	2.0	6.45%	2.3	1.9	-16.09%
Per Veh. Fuel (gal)	0.09	0.11	27.89%	0.09	0.11	27.13%

The total number of vehicles drops 18% for the build condition, however, the overall travel distance goes up 18% as well. Travel time drops significantly as does total delay. Travel Speed also goes up significantly. Total stops decrease as well.

The change in the number of vehicles is likely the result of the superstreet configuration that enables more options to balance flows instead of making vehicles disappear and appear at points due to imbalances. The change in the travel distance is partly the result of the relocation of traffic from 15th Ave NW to Clement Blvd, which extends the distance some cars travel on the network by about ½ mile, as well as the extra distance that some left turners must travel to utilize the proposed U-turns.

Distance per vehicle rises by 44%, with travel time dropping by 10%. Delay time per vehicle drops by 32% and stops per vehicle drop by 5%. Fuel usage per vehicle climbs by 27%, but it must be remembered that vehicles are traveling much farther than previously.

Overall, travel speed goes up, and travel time and delay go down.

6.0 CONCLUSIONS & RECOMMENDATIONS

Sections 6.1 through 6.2.7 summarize the findings and recommendations of the capacity and queuing analyses.

6.1 2040 No-Build Alternative

Eight (8) out of the twelve (12) intersections operate at LOS D or better in both peak hours. The intersections of US 321 at 13th Ave Dr NW, and US 321 at 15th Ave NW/20th Ave NW operate at LOS F in one peak hour, and LOS C in one peak hour. The intersections of US 321 at 2nd Ave NW, and US 321 at Clement Blvd operate at LOS E in both peak hours.

The average travel time per vehicle in the simulation is 7 minutes in the AM peak period and 7.2 minutes in the PM peak period. The average travel distance per vehicle is 2.1 miles with an average speed of 21 mph in the AM and 20 mph in the PM.

6.2 2040 Build Alternative

6.2.1 13th Street

6.2.1.1 One-Way Split Partial Cloverleaf Interchange

All five (5) intersections operate at LOS C or better in both peak hours.

All four (4) ramps operate at LOS C or better in both peak hours.

None of the intersections present "excessive" queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.1.2 Partial Cloverleaf Interchange

All nine (9) intersections operate at LOS C or better in both peak hours.

All four (4) ramps operate at LOS C or better in both peak hours.

None of the intersections present "excessive" queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.1.3 Two-Way Partial Cloverleaf Interchange

All nine (9) intersections operate at LOS B or better in both peak hours.

All four (4) ramps operate at LOS C or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.1.4 Two-Way Split Partial Cloverleaf Interchange

Five (5) of the six (6) intersections operate at LOS C or better in both peak hours. The intersection of 13th St SW at 2nd Ave SW operates at LOS F in the AM peak hour, and LOS D in the PM peak hour, stemming from the minor side street turning movement. From the projected traffic volumes, it is not expected to warrant a traffic signal.

All four (4) ramps operate at LOS C or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

Of the four alternatives analyzed, all four would satisfy the projected traffic demand and the first three operate acceptably at all locations analyzed.

6.2.2 Clement Boulevard

6.2.2.1 Partial Cloverleaf Interchange

All four (4) intersections operate at LOS C or better in both peak hours.

All four (4) ramps operate at LOS D or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.2.2 Superstreet

All eight (8) intersections operate at LOS C or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

The interchange option would concentrate traffic east heading into town onto Clement Blvd and provide for a grade separation of the railroad. The superstreet would split the traffic to 15th Ave NW and Clement Blvd as it is at present for traffic heading east into town and keep the three closely spaced traffic signals and the rail crossing within 900ft. The superstreet would leave much of the recent construction along Clement Blvd while the interchange would require replacement. Either option would provide the needed capacity and operate acceptably for the 2040 projected traffic volumes.

6.2.3 Grace Chapel Road

6.2.3.1 Superstreet

All three (3) intersections operate at LOS D or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.3.2 Reverse Superstreet

All three (3) intersections operate at LOS D or better in both peak hours.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

Both alternatives would function acceptably. The superstreet would require hundreds more U-turns on US 321 in the peak hour than the reverse superstreet.

6.2.4 Dudley Shoals Road

6.2.4.1 At-Grade Intersection

Nine (9) of the ten (10) intersections operate at LOS C or better in both peak hours. The intersection of US 321 Southbound at the Dudley Shoals Road On-Ramp operates at LOS F in the AM peak hour, and LOS C in the PM peak hour, stemming from the minor side street turning movement.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.4.2 Freeway Ramp

The ramp operates at LOS C or better in both peak hours.

Both alternatives are expected to provide relief to the Pinewood Rd intersection. The free flow ramp is expected to perform better than a stop controlled intersection at Dudley Shoals Rd. It, however, would be necessary to provide sufficient merge distance along US 321.

6.2.5 Lower Cedar Valley Road & Pine Mountain Road

Nine (9) of the eleven (11) intersections operate at LOS C or better in both peak hours. The intersections of US 321 Northbound at Quarry Estates Road, and US 321 Southbound at Quarry Estates Road operate at LOS E in one peak hour, and LOS D in one peak hour, stemming from the minor side street turning movement at locations that would not warrant a traffic signal.

None of the intersections present “excessive” queuing issues, defined by turning queue lengths of 200 feet in excess of the available storage bay. In general, the provided storage lengths adequately handle the queues.

6.2.6 Mount Herman Road & Caldwell Community College and Technical Institute

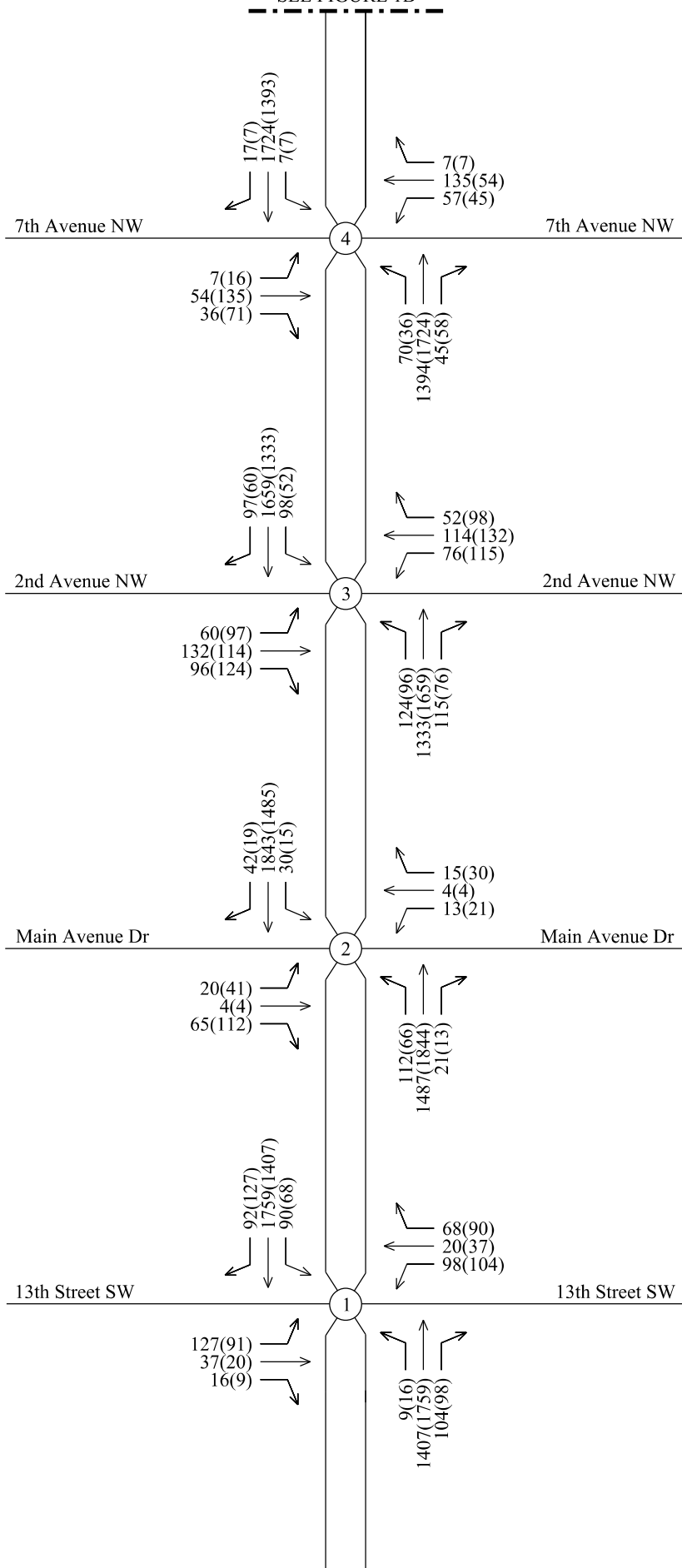
At present, a plan is being developed that will alter the internal circulation and the driveways at CCCTI. Once this plan is complete, the analysis along US 321 can be completed.

6.2.7 MDI Simulation Analysis

The average travel time per vehicle in the simulation is 6.4 minutes in the AM and PM peak periods. The average travel distance per vehicle is 3 miles with an average speed of 29 mph in the AM and 28 mph in the PM. This is a 18% reduction in travel time and 39% increase in travel speed.

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MATCH LINE A
SEE FIGURE 1B

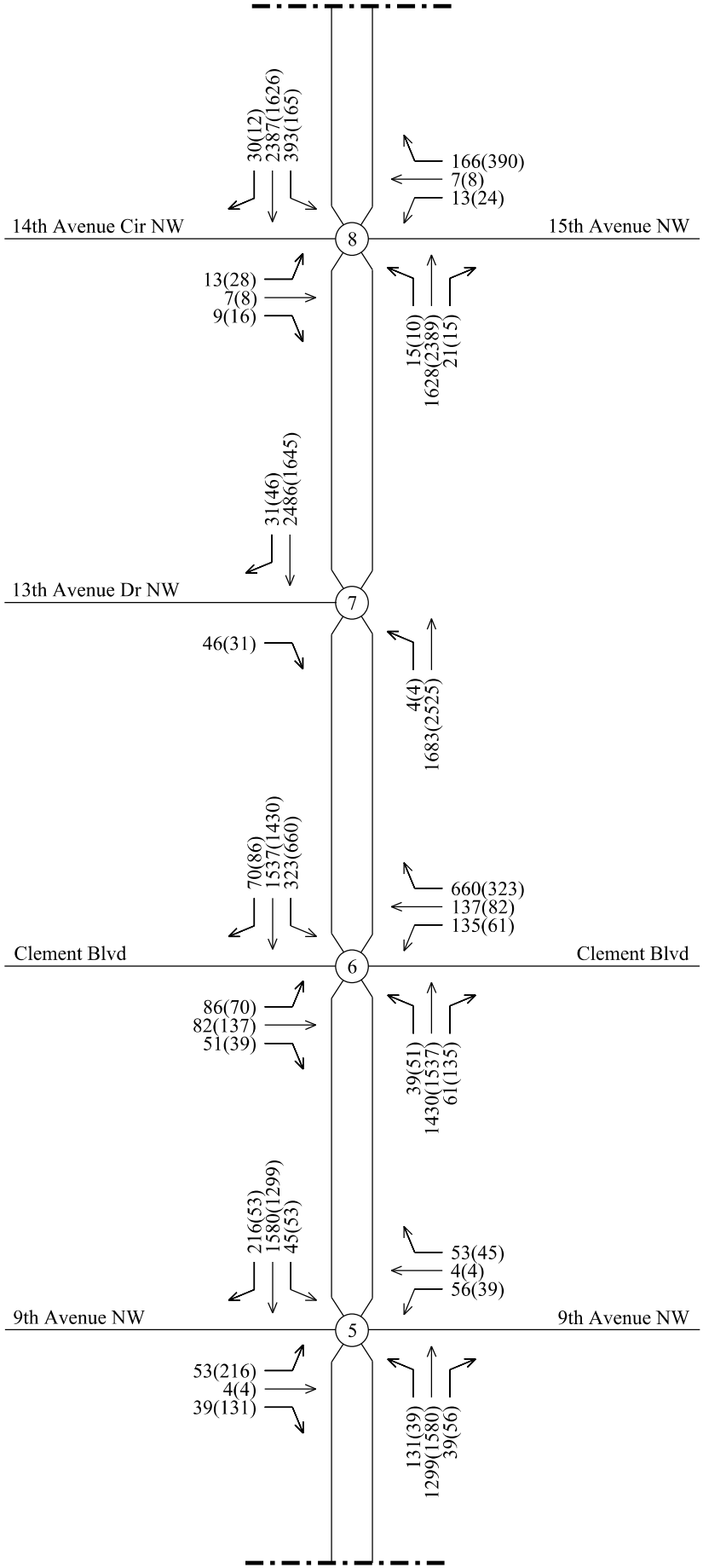


2040 No Build
Peak Hour Volumes
Figure 1A

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



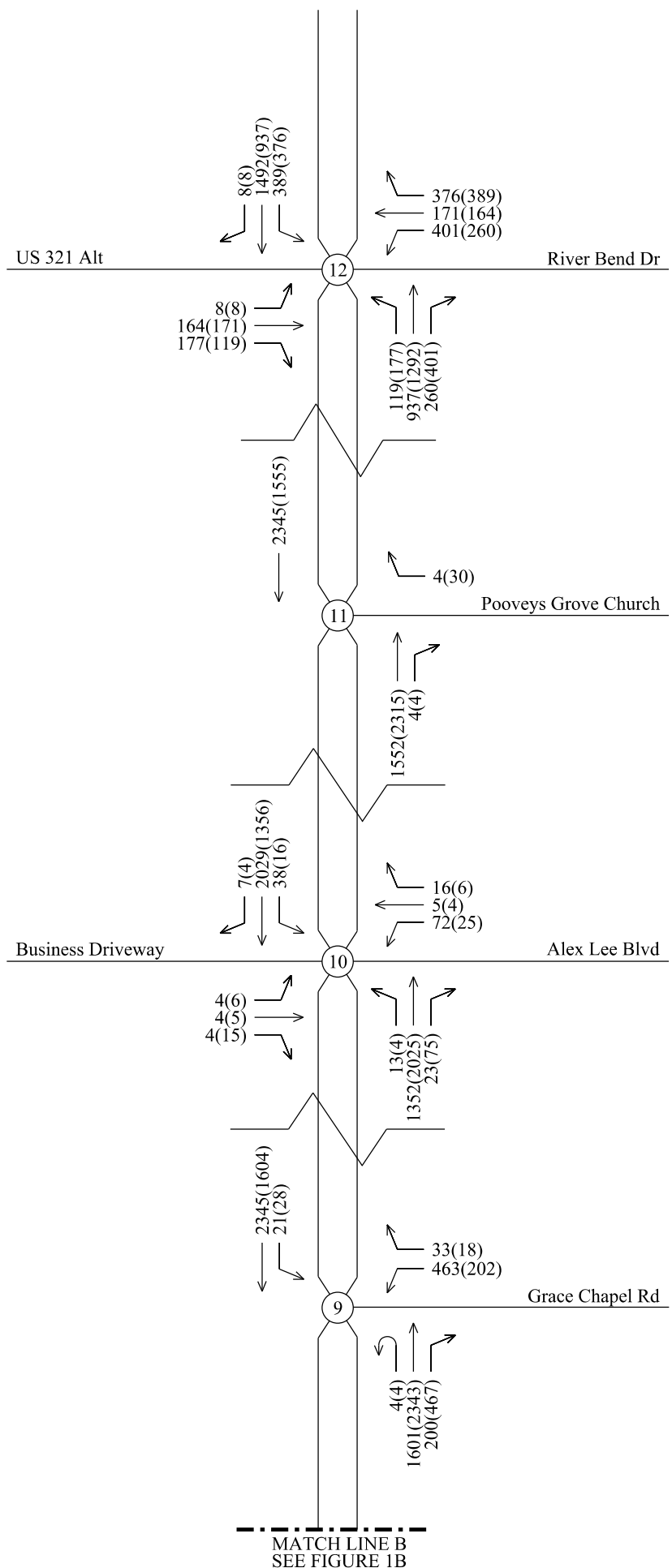
MATCH LINE B
SEE FIGURE 1C



2040 No Build
Peak Hour Volumes
Figure 1B

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

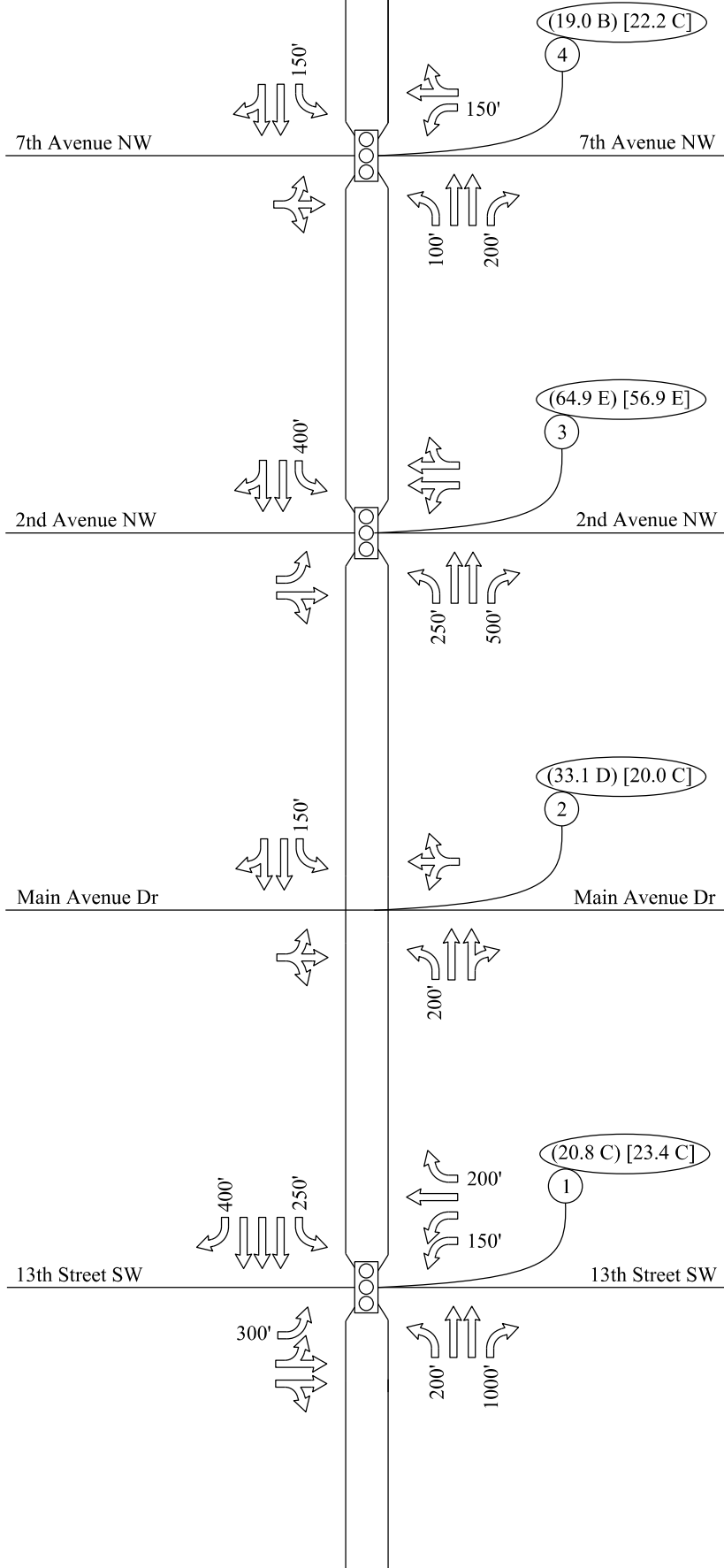




2040 No Build
Peak Hour Volumes
Figure 1C

← Turning Movement
⊙## Intersection Number
AM Peak Hour Volumes
(###) PM Peak Hour Volumes

MATCH LINE C
SEE FIGURE 2B

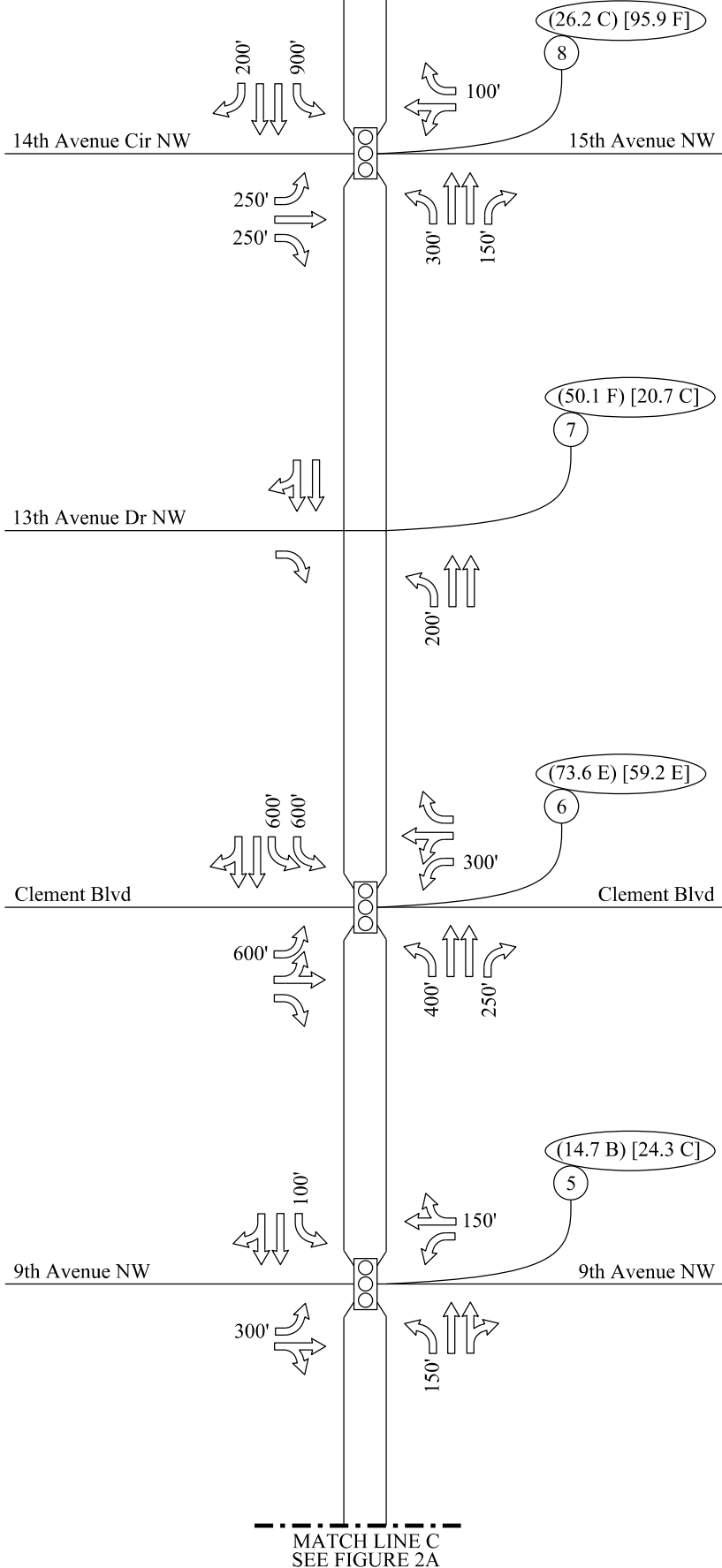


2040 No Build
LOS and Laneage
Figure 2A

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage



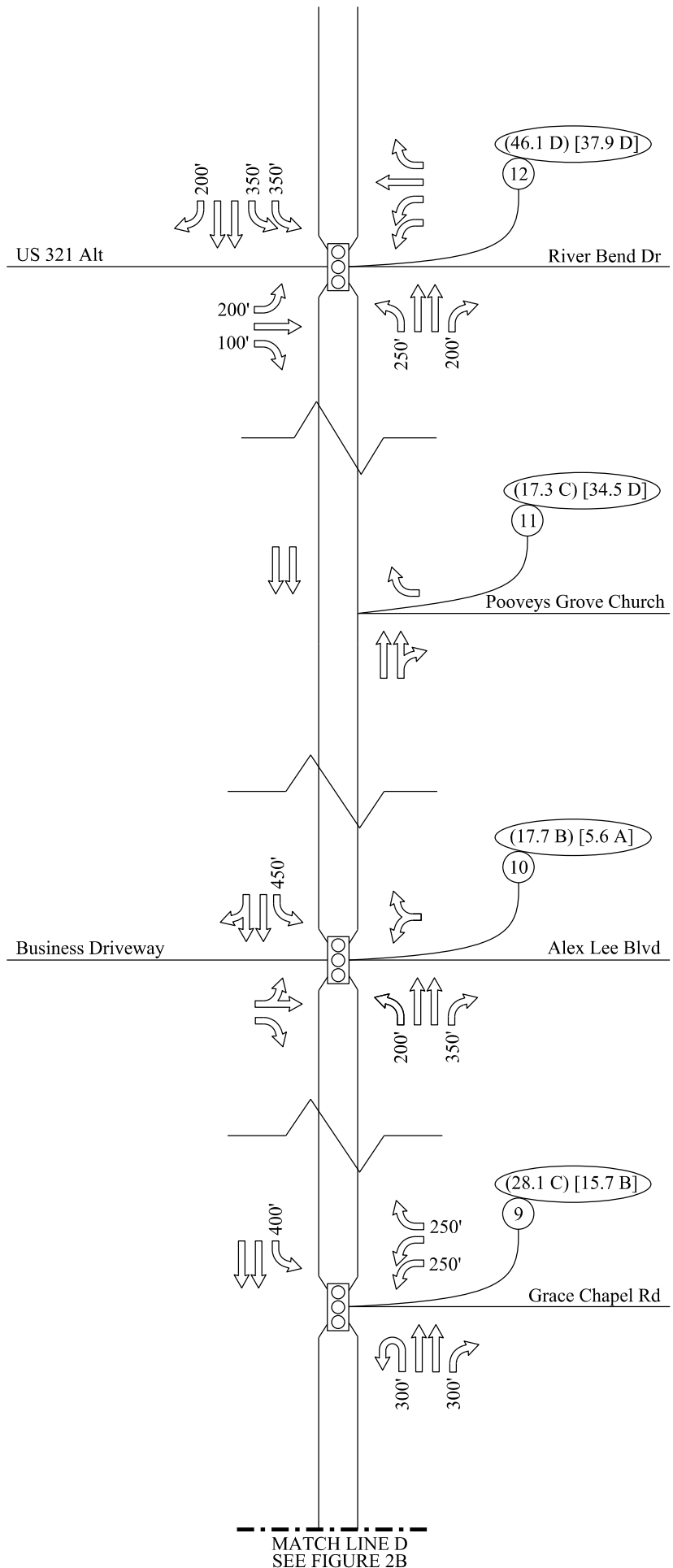
MATCH LINE D
SEE FIGURE 2C



2040 No Build
LOS and Laneage
Figure 2B

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage

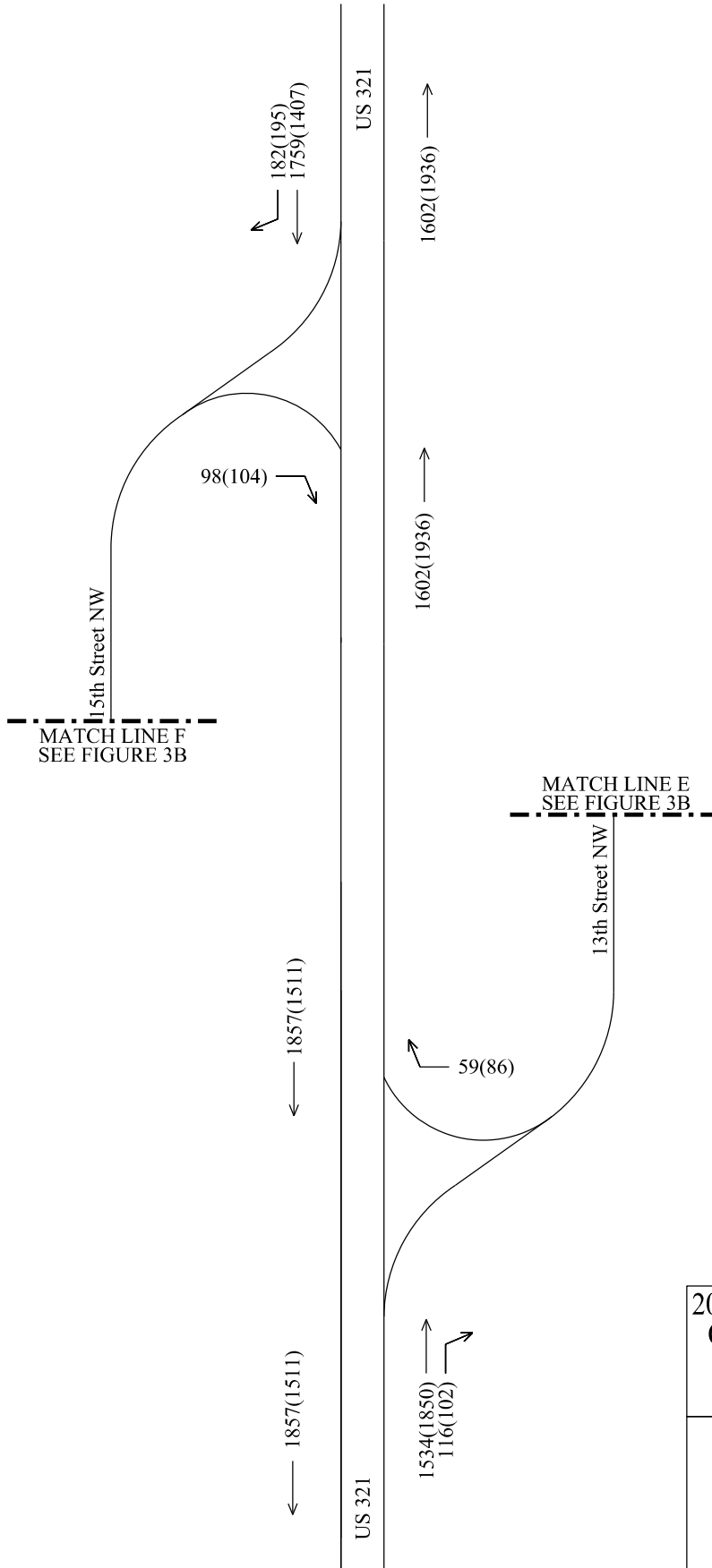




2040 No Build
LOS and Laneage
Figure 2C

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage



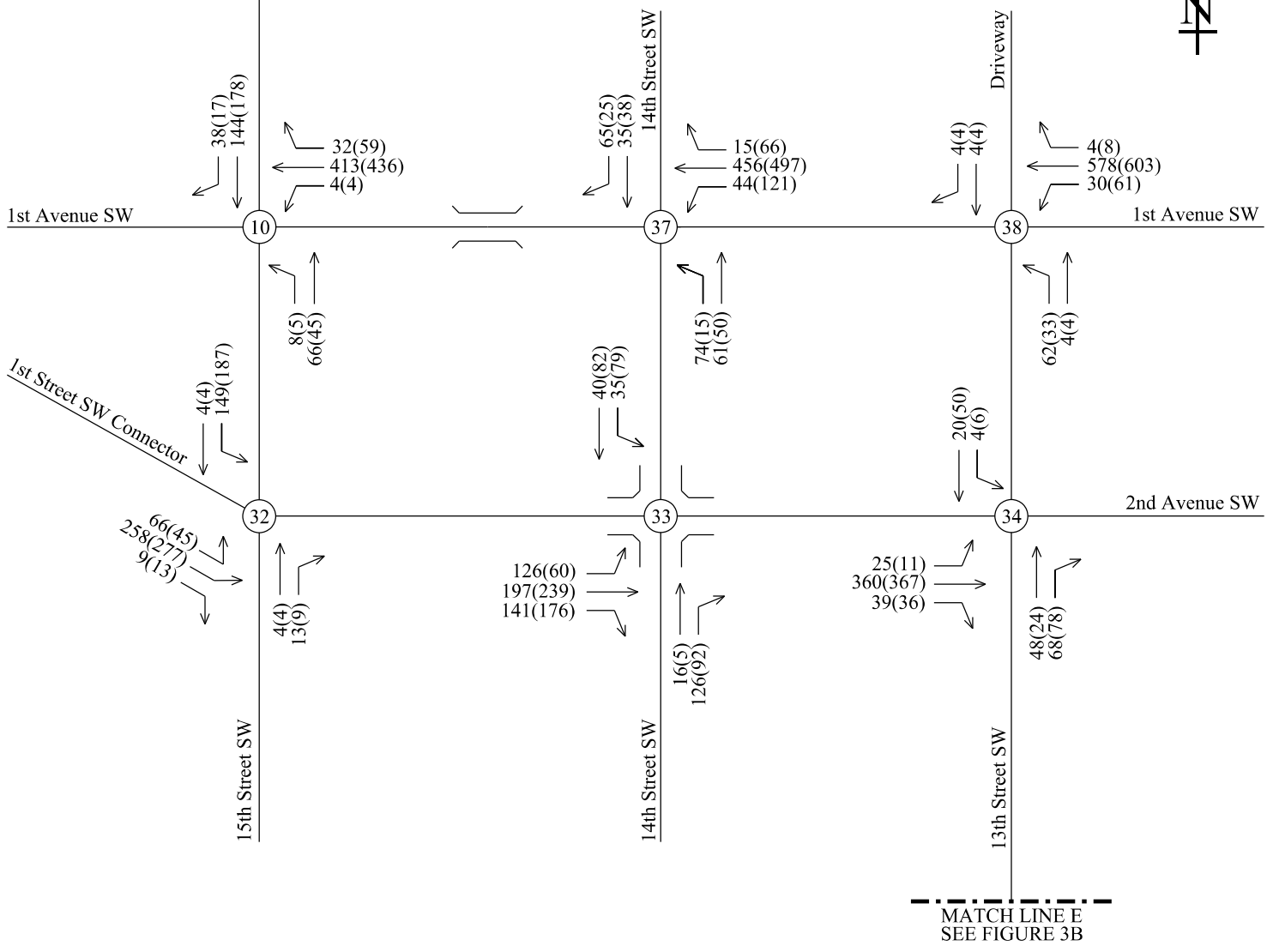


2040 Build - 13th Street Interchange
One-Way Split Partial Cloverleaf
Peak Hour Volumes
Figure 3A

- ← Turning Movement
- ⊕ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



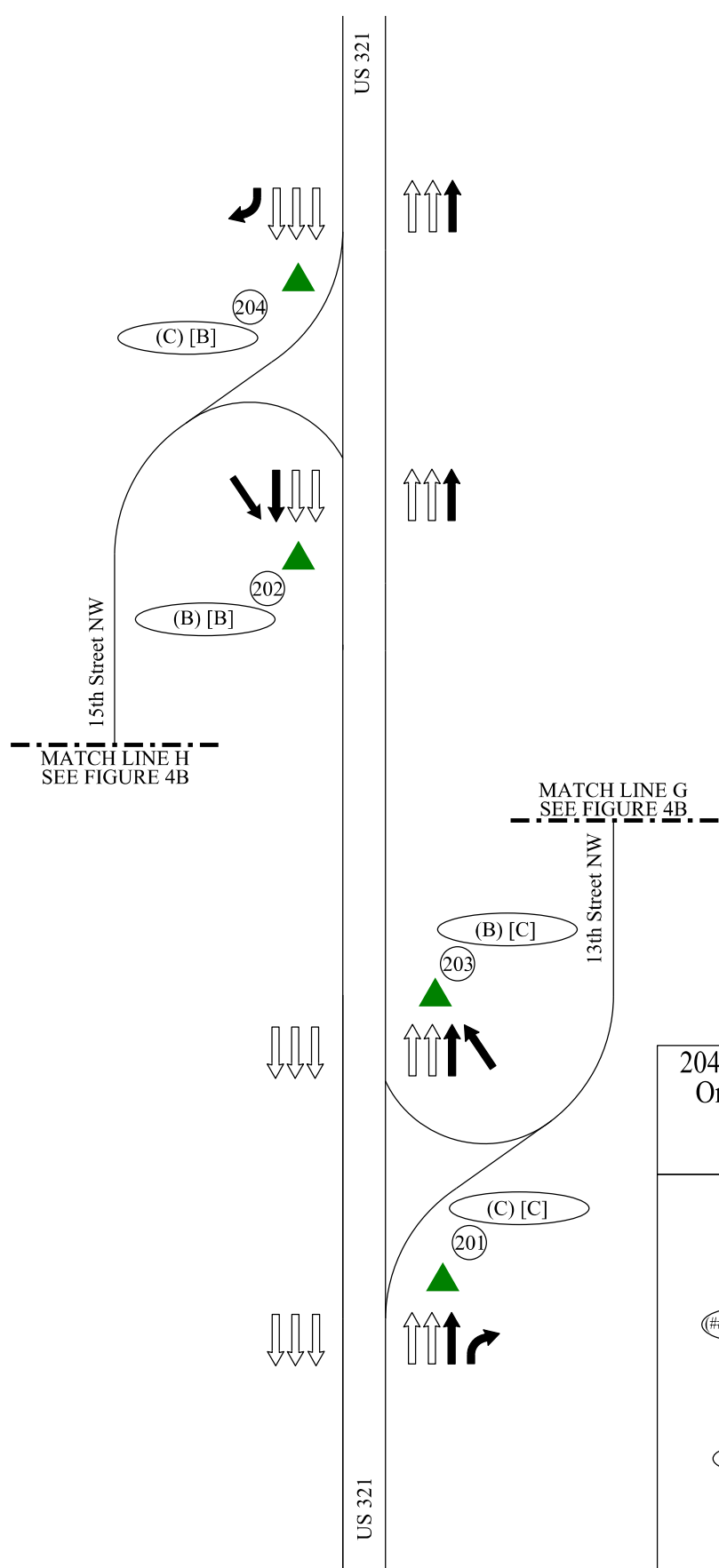
MATCH LINE F
SEE FIGURE 3A



2040 Build - 13th Street Interchange
One-Way Split Partial Cloverleaf
Peak Hour Volumes
Figure 3B

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



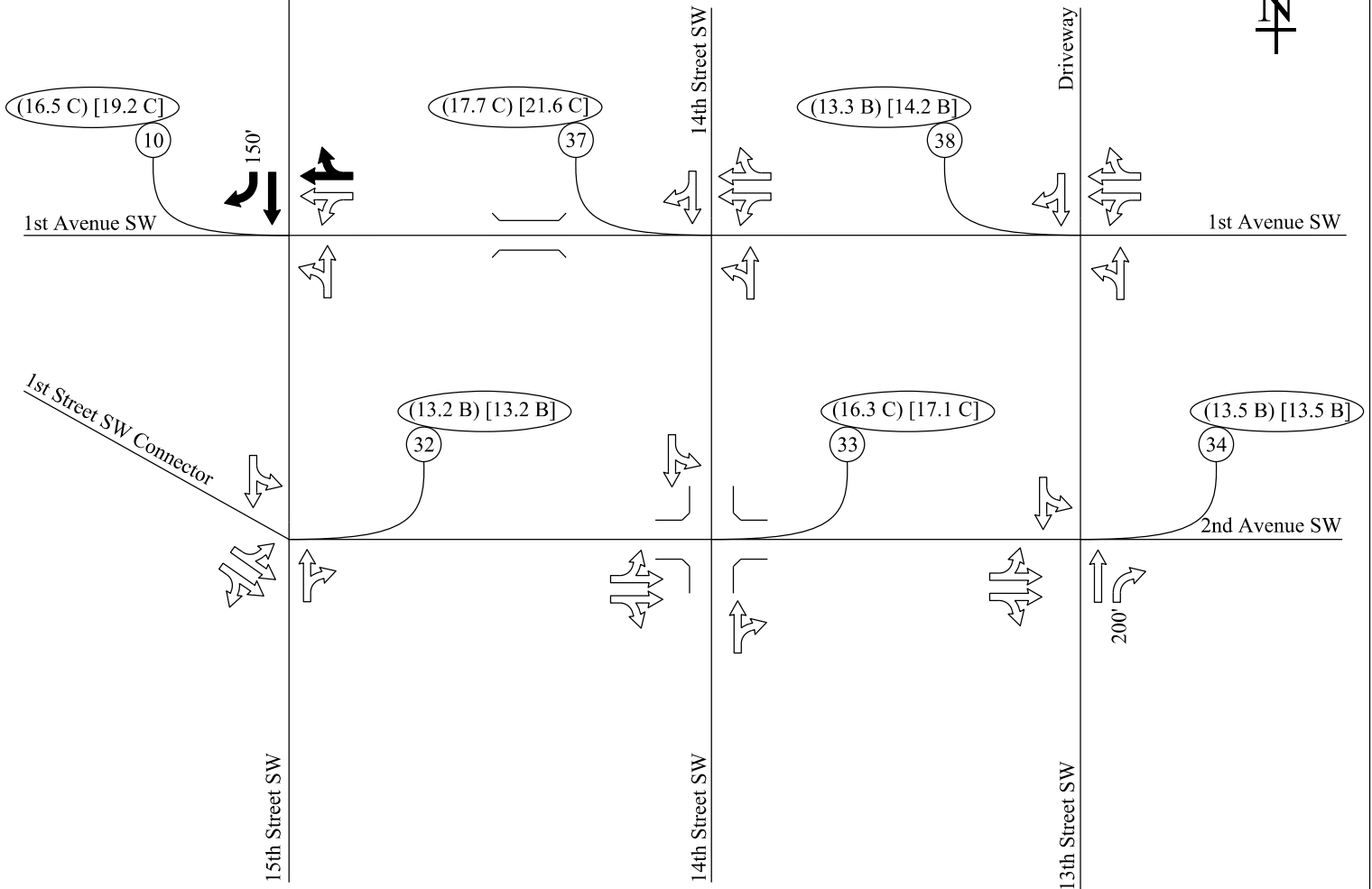


2040 Build - 13th Street Interchange
One-Way Split Partial Cloverleaf
LOS and Laneage
Figure 4A

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage



MATCH LINE H
SEE FIGURE 4A

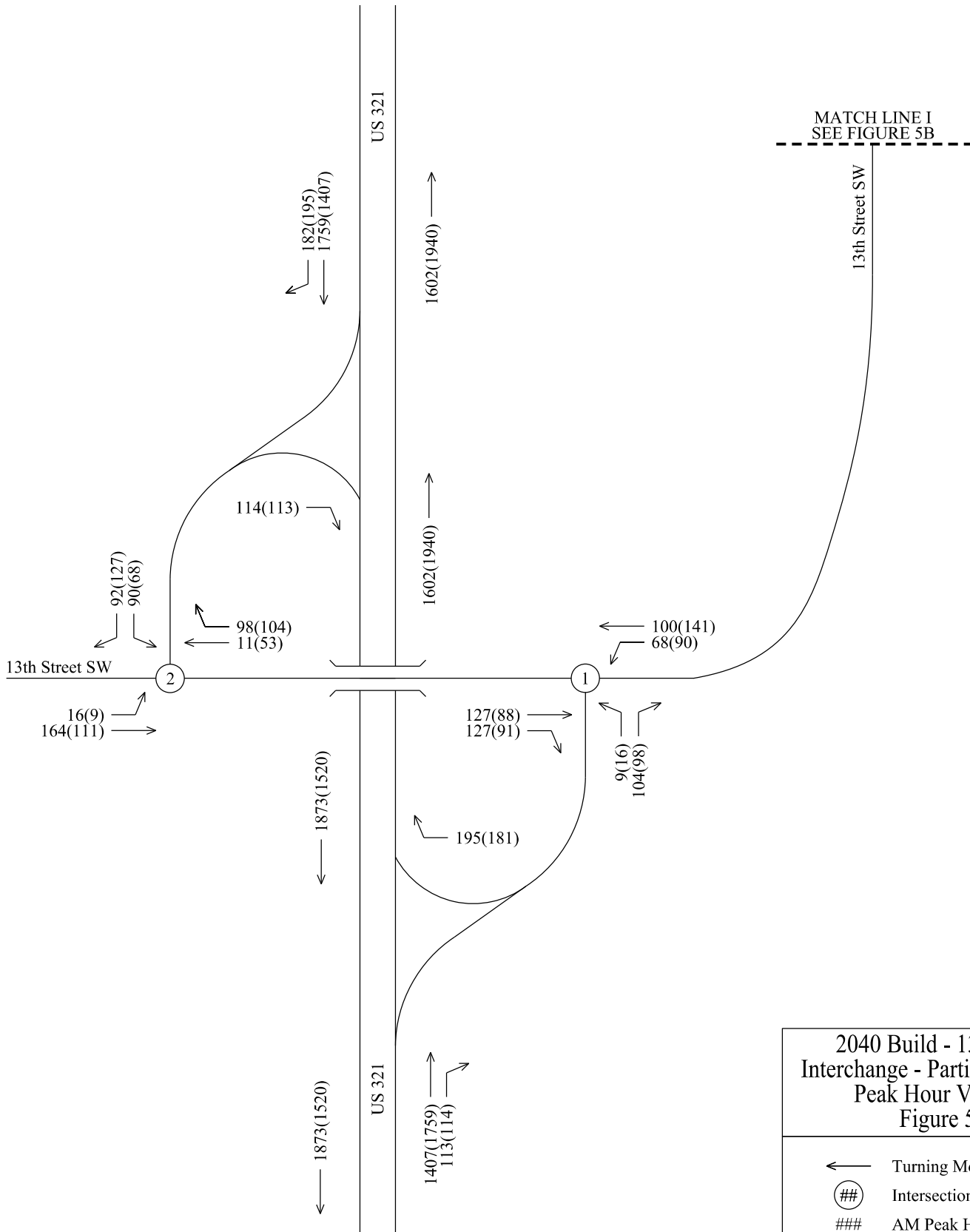


MATCH LINE G
SEE FIGURE 4A

2040 Build - 13th Street Interchange
One-Way Split Partial Cloverleaf
LOS and Laneage
Figure 4B

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage

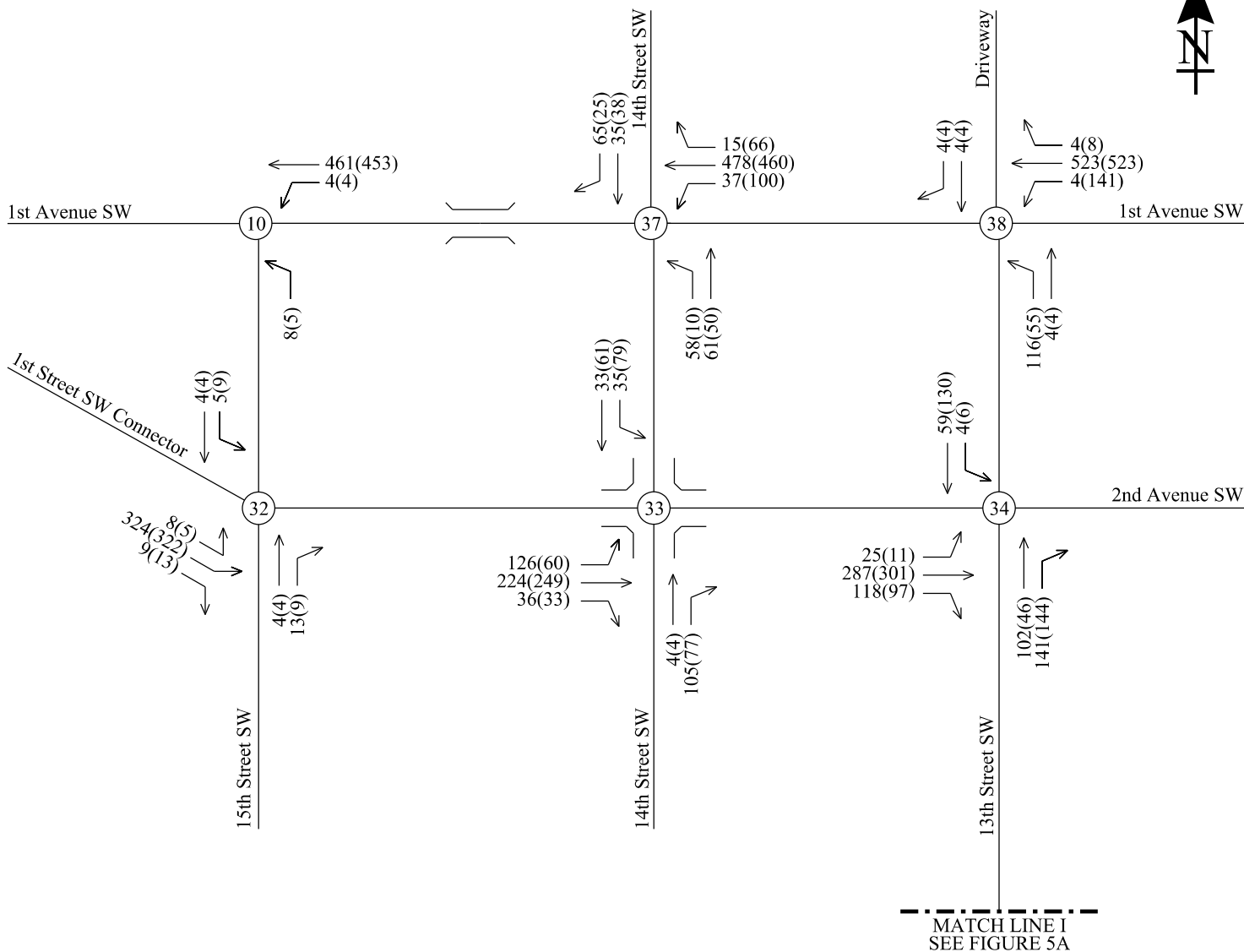




2040 Build - 13th Street
Interchange - Partial Cloverleaf
Peak Hour Volumes
Figure 5A

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

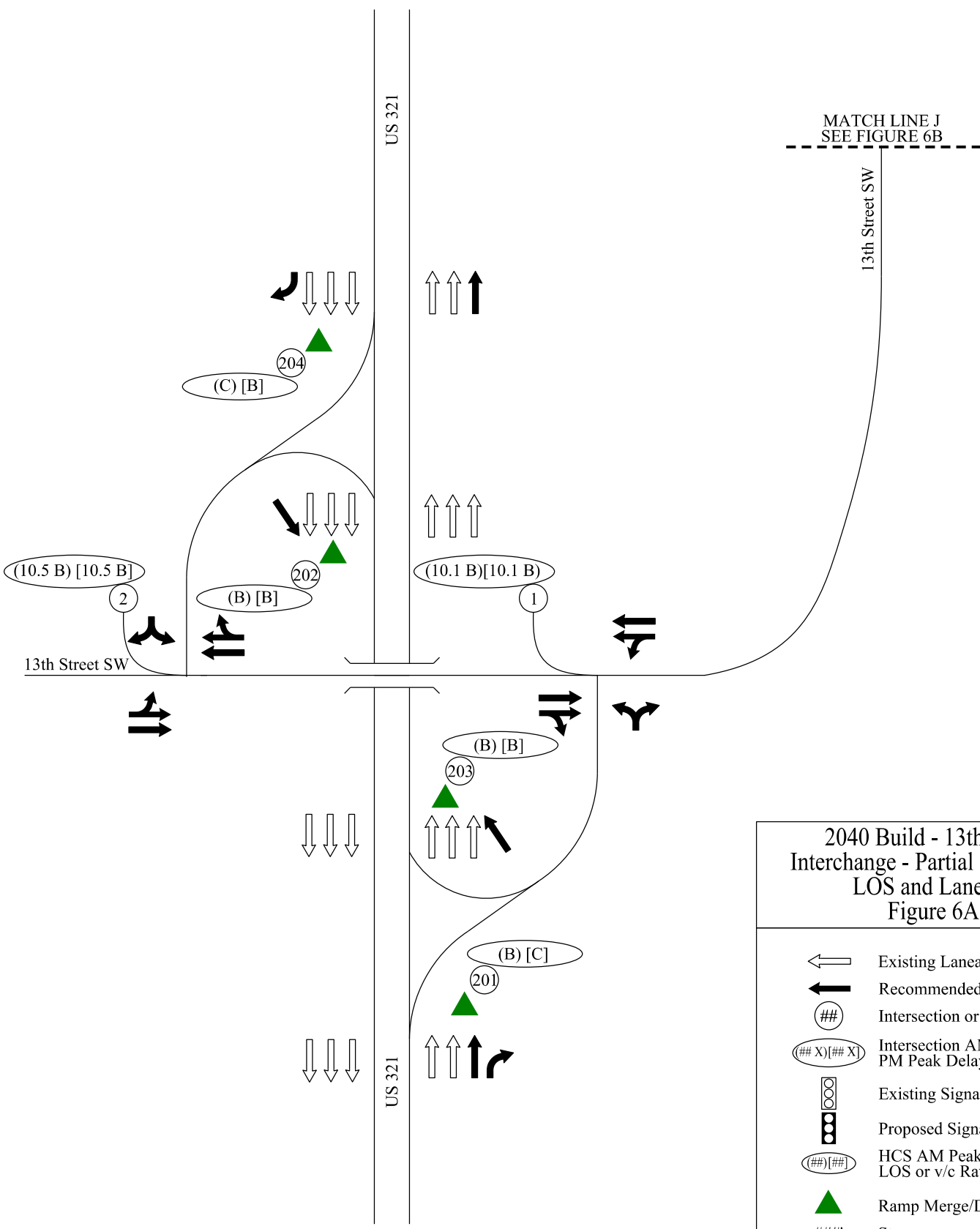




2040 Build - 13th Street Interchange - Partial Cloverleaf Peak Hour Volumes
Figure 5B

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

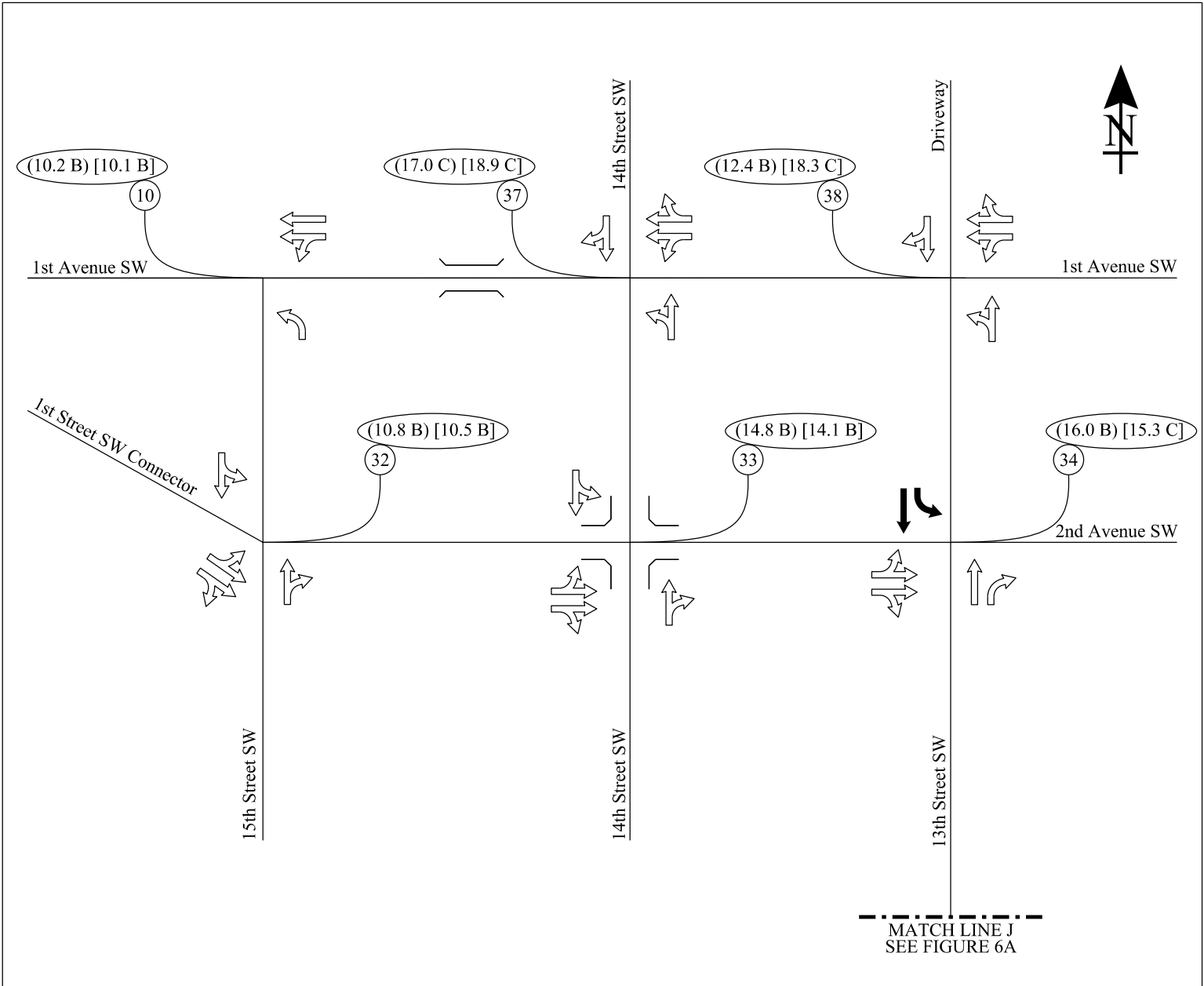













2040 Build - 13th Street Interchange - Partial Cloverleaf LOS and Laneage Figure 6A

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage

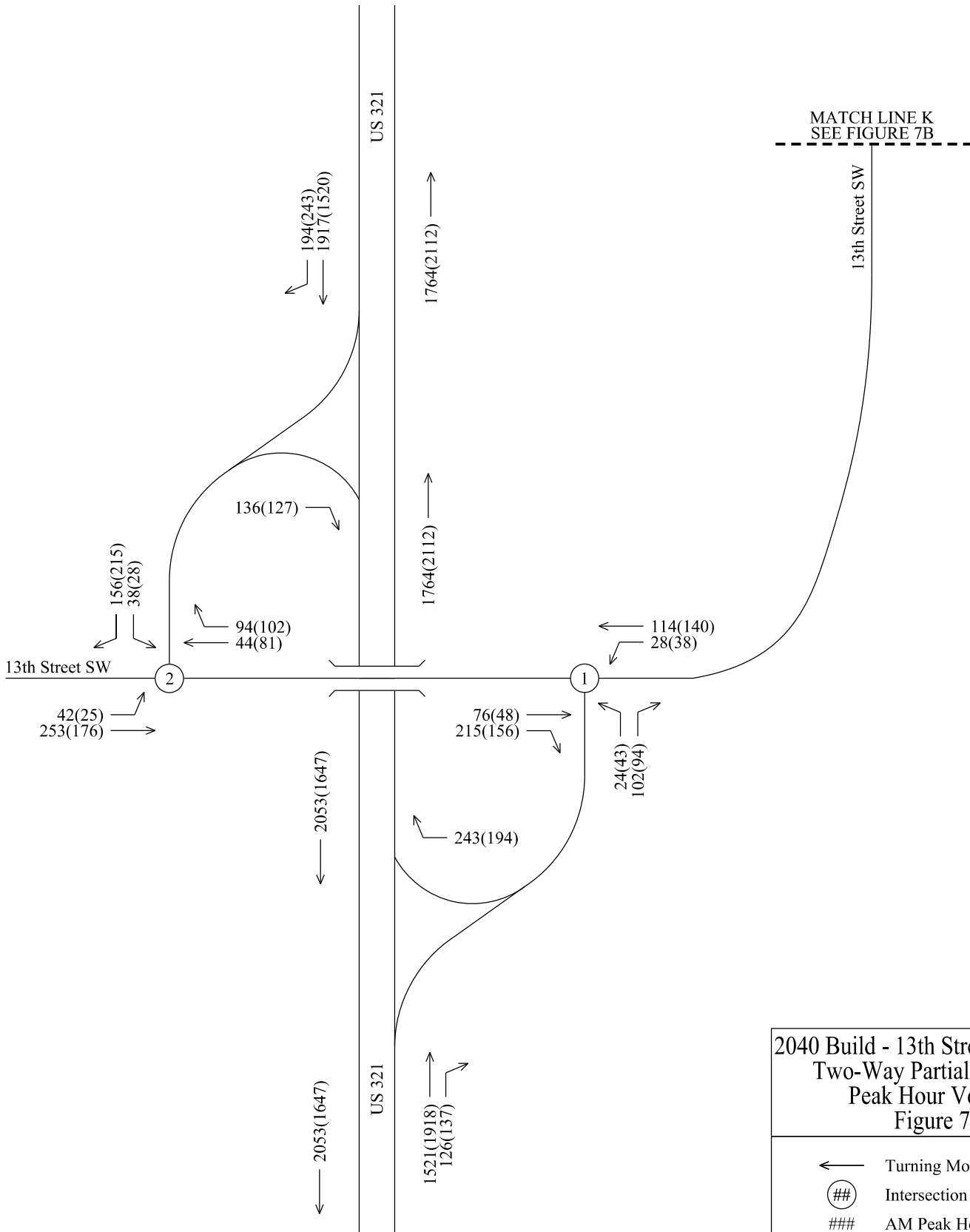




**2040 Build - 13th Street
Interchange - Partial Cloverleaf
LOS and Laneage
Figure 6B**

-  Existing Laneage
-  Recommended Laneage
-  Intersection or HCS Number
-  Intersection AM Peak and PM Peak Delay and LOS
-  Existing Signal
-  Proposed Signal
-  HCS AM Peak and PM Peak LOS or v/c Ratio
-  Ramp Merge/Diverge
-  Storage

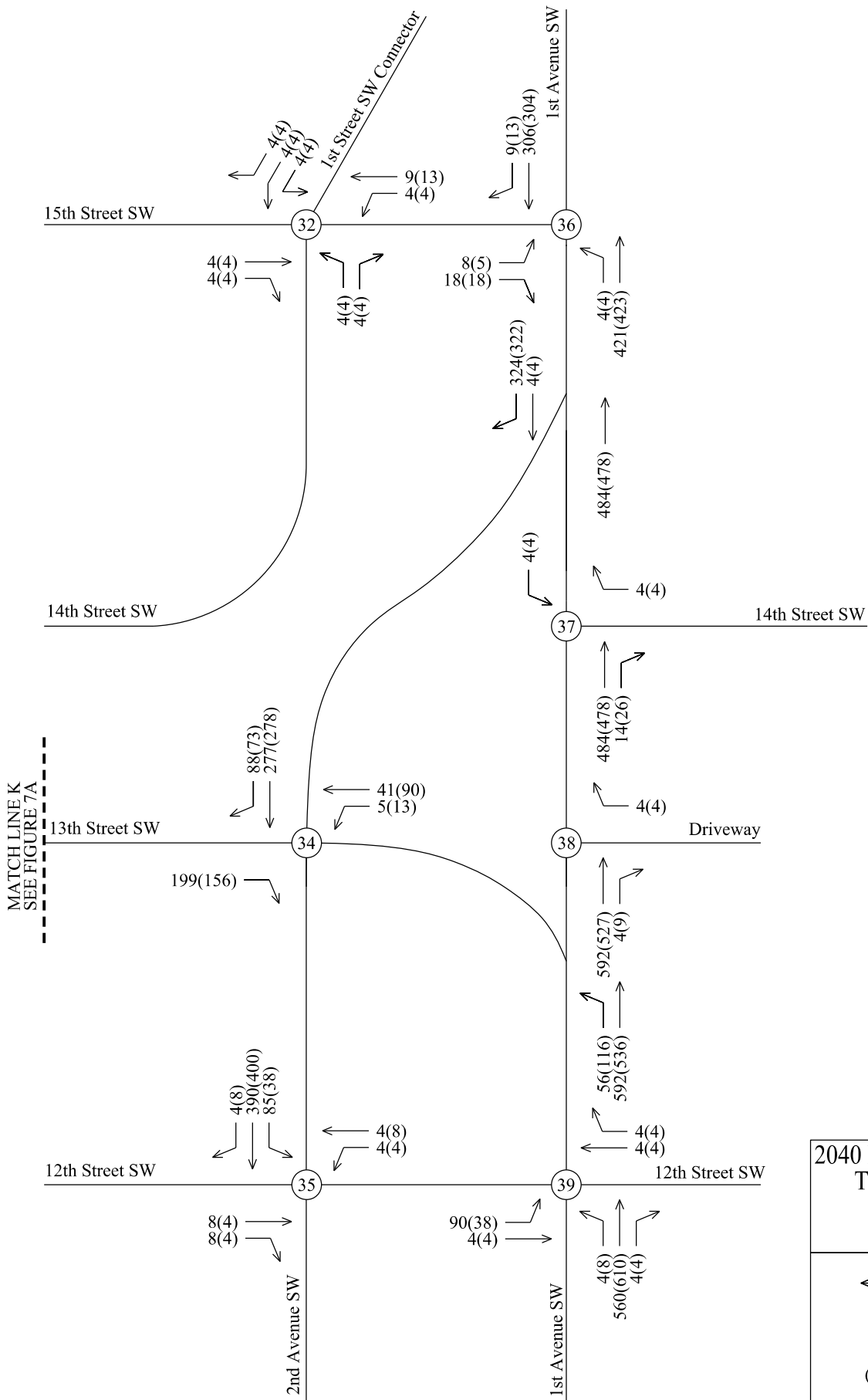




2040 Build - 13th Street Interchange
Two-Way Partial Cloverleaf
Peak Hour Volumes
Figure 7A

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



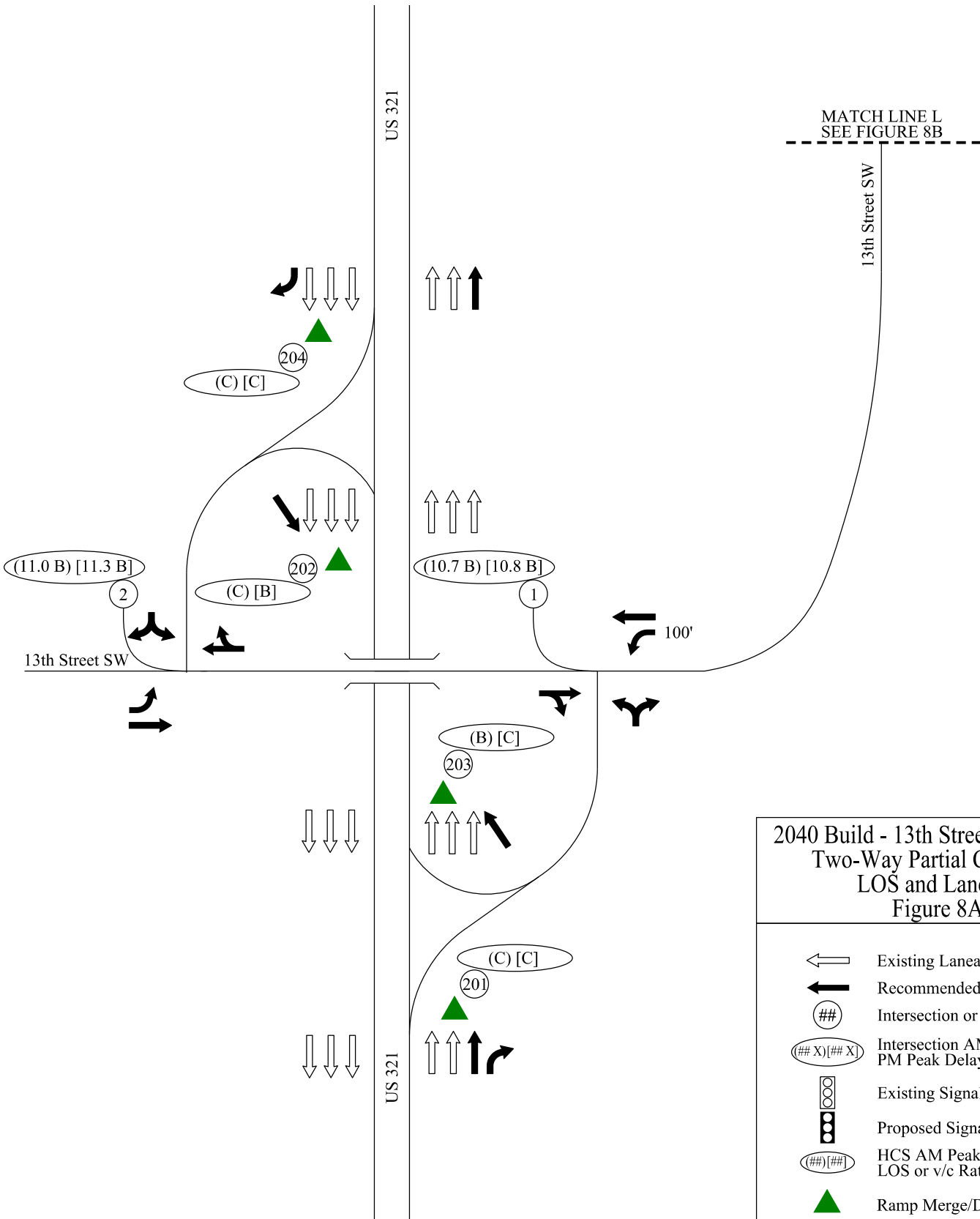


MATCH LINE K
SEE FIGURE 7A










2040 Build - 13th Street Interchange
Two-Way Partial Cloverleaf
Peak Hour Volumes
Figure 7B

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

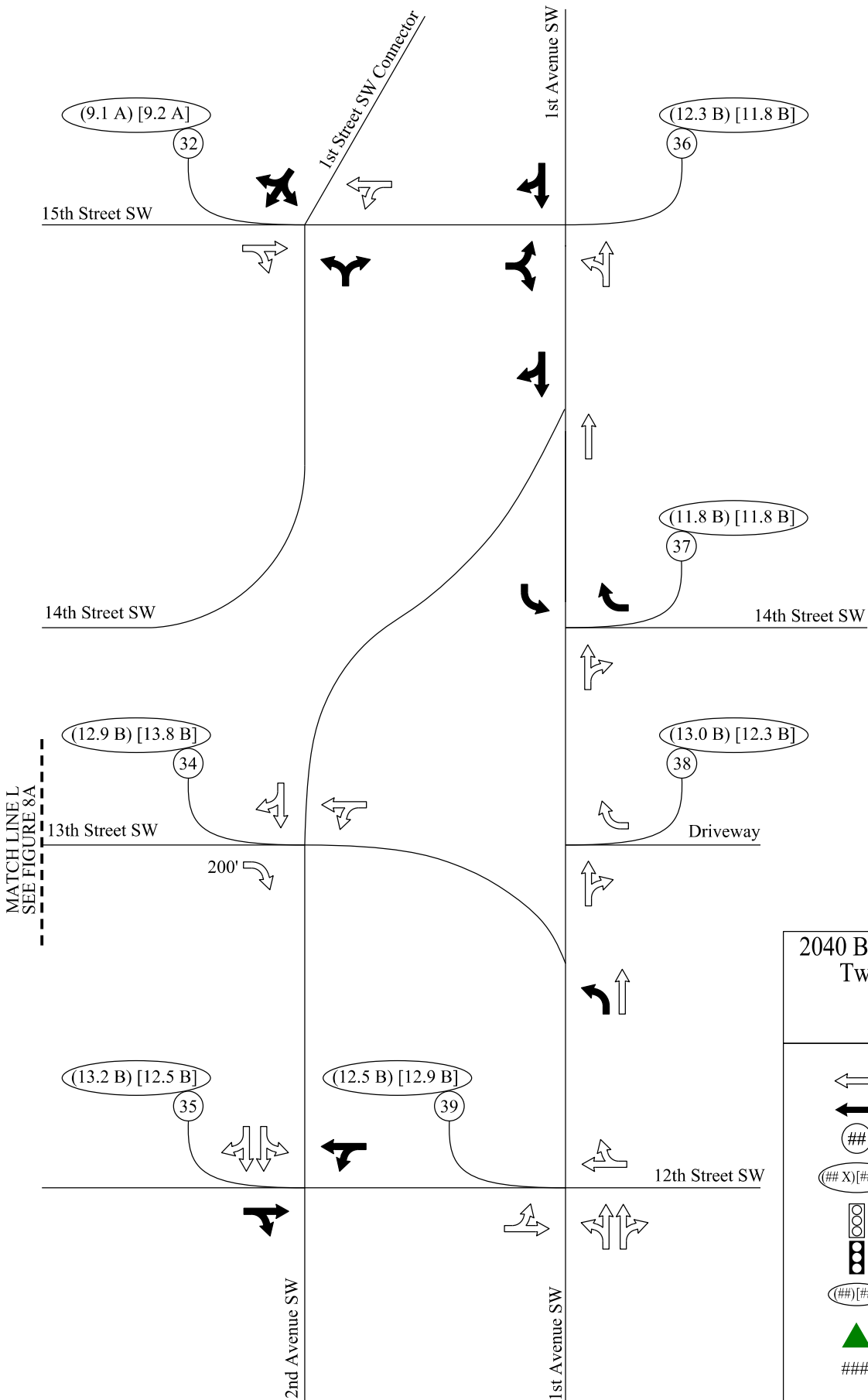




2040 Build - 13th Street Interchange
Two-Way Partial Cloverleaf
LOS and Laneage
Figure 8A

-  Existing Laneage
-  Recommended Laneage
-  Intersection or HCS Number
-  Intersection AM Peak and PM Peak Delay and LOS
-  Existing Signal
-  Proposed Signal
-  HCS AM Peak and PM Peak LOS or v/c Ratio
-  Ramp Merge/Diverge
-  Storage

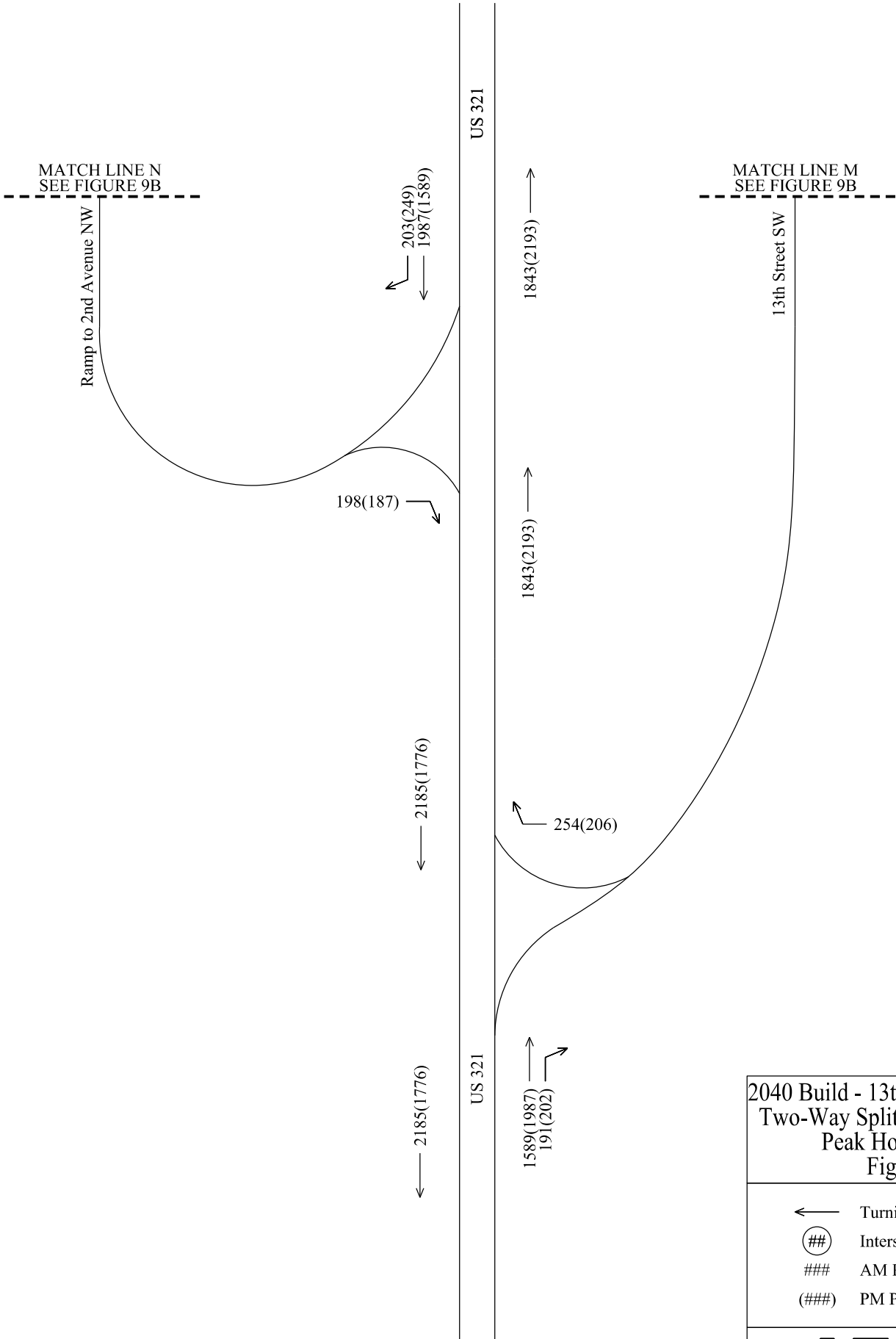




2040 Build - 13th Street Interchange
Two-Way Partial Cloverleaf
LOS and Laneage
Figure 8B

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage

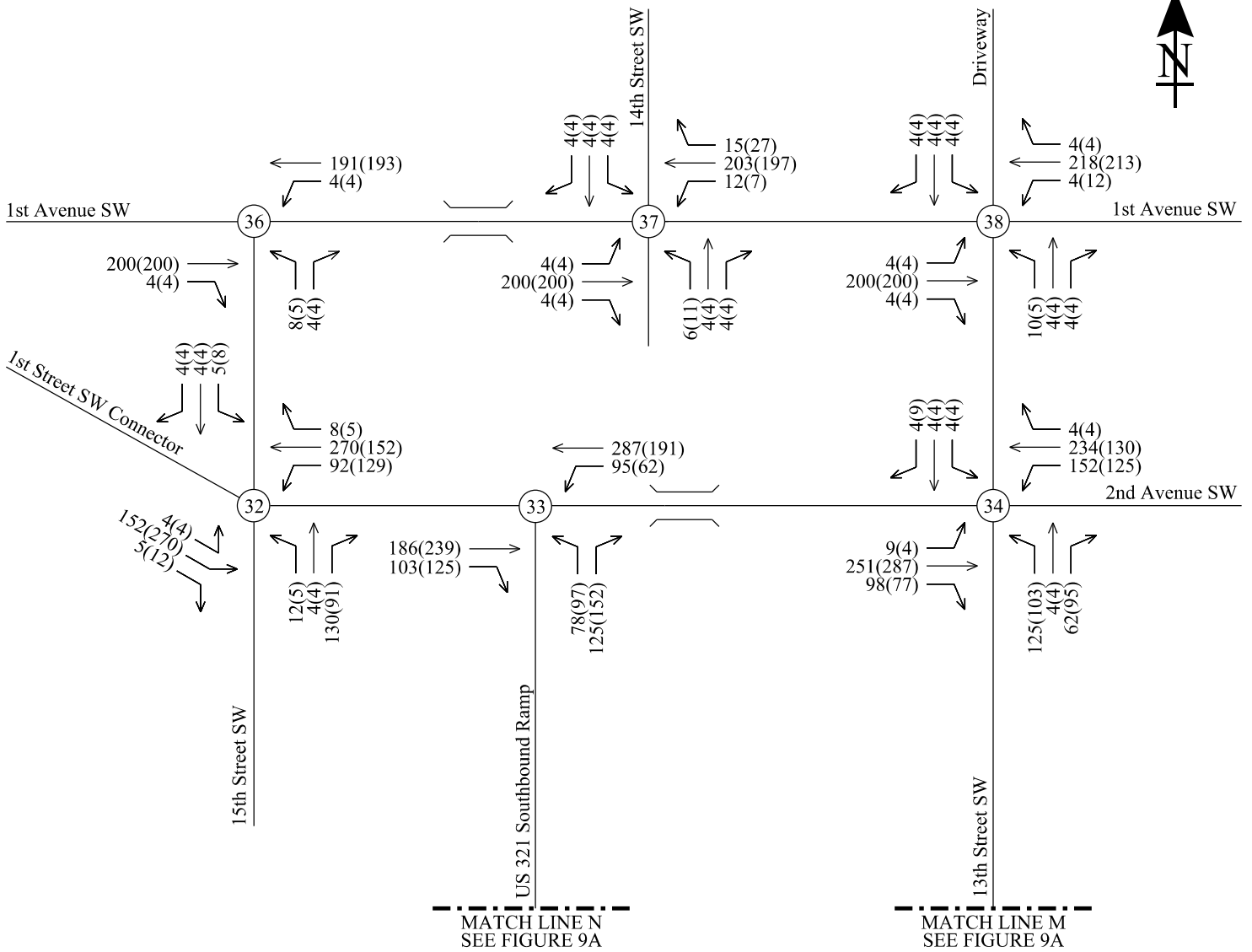




2040 Build - 13th Street Interchange
Two-Way Split Partial Cloverleaf
Peak Hour Volumes
Figure 9A

- ← Turning Movement
- Ⓝ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

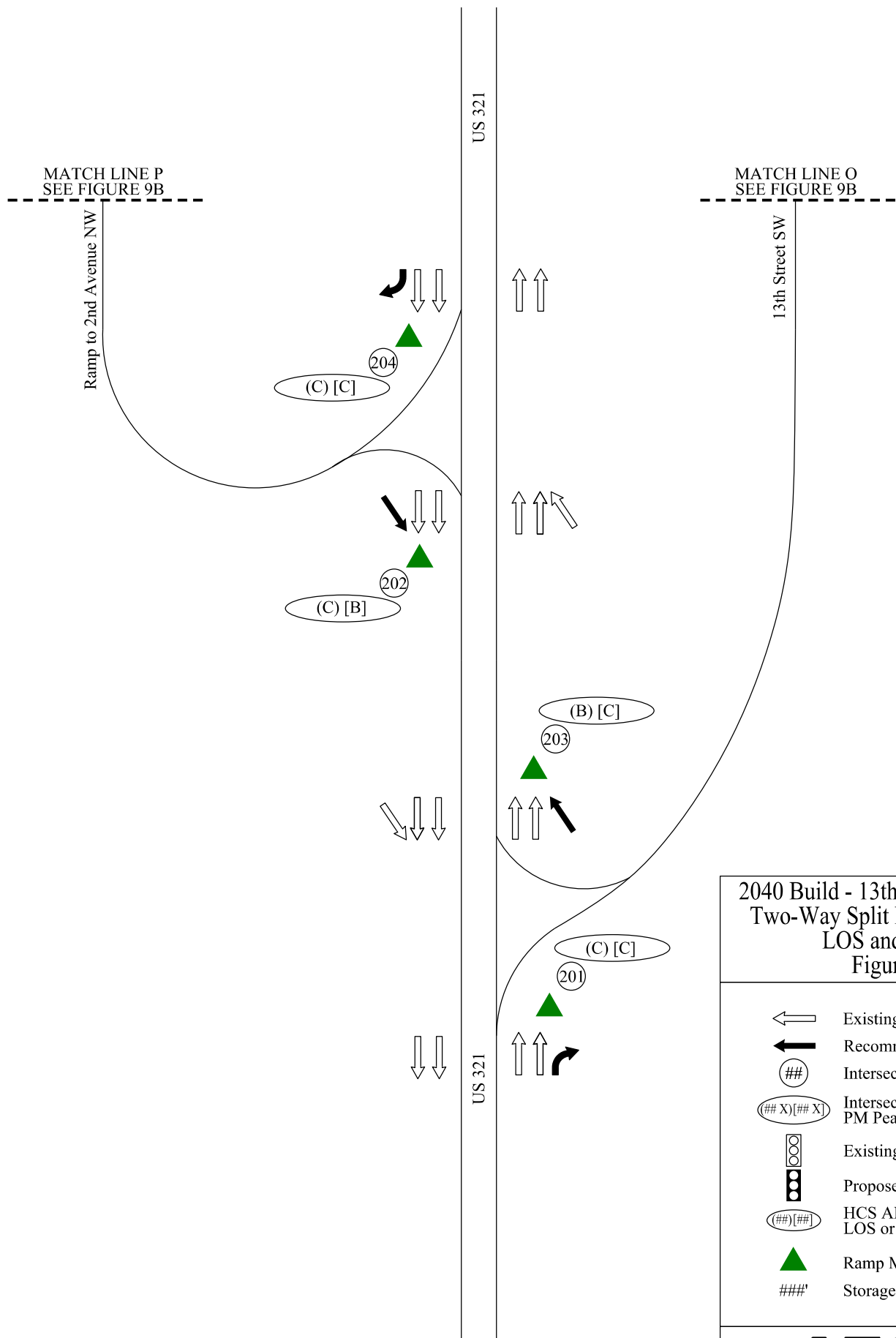




2040 Build - 13th Street Interchange
Two-Way Split Partial Cloverleaf
Peak Hour Volumes
Figure 9B

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

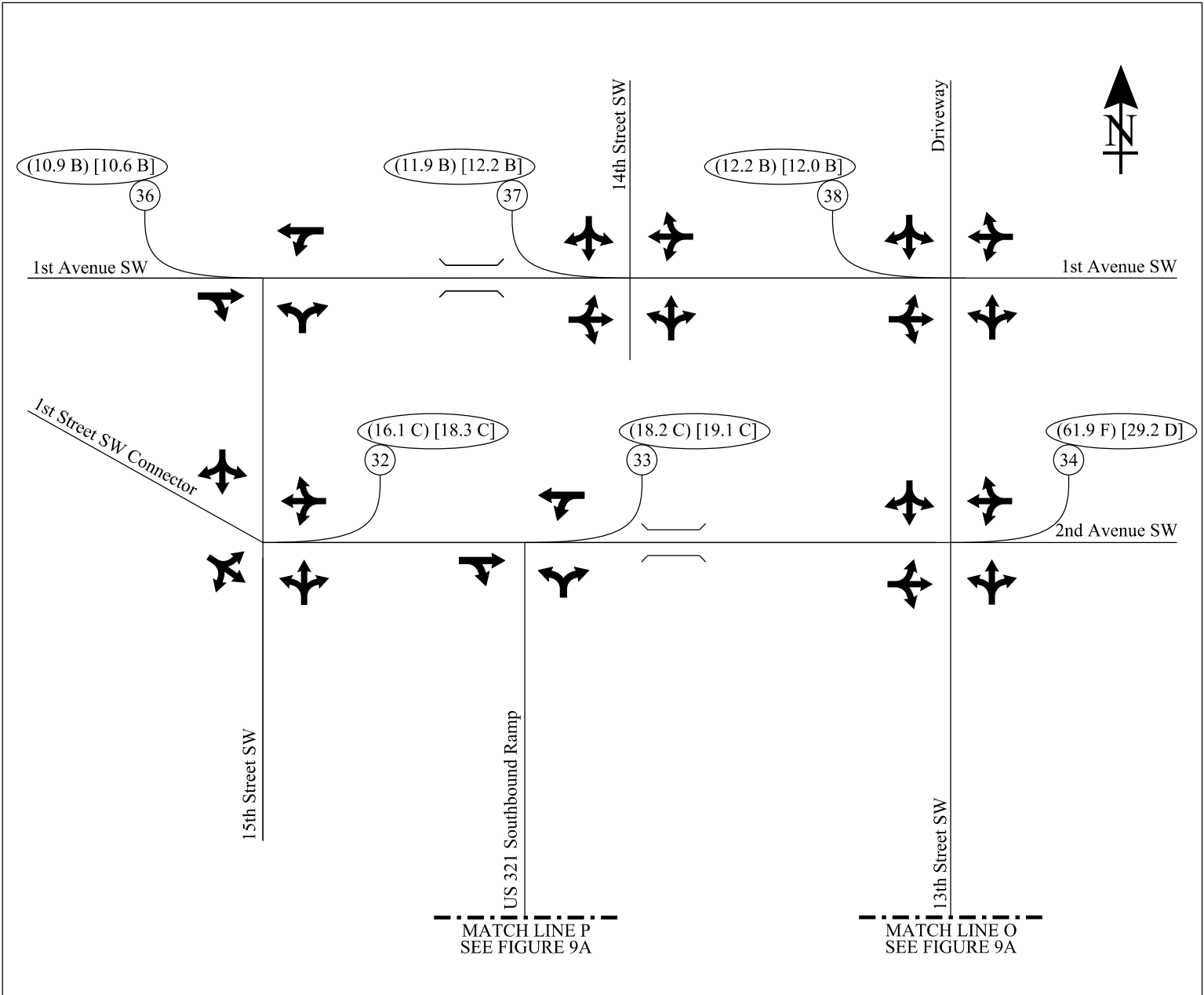




2040 Build - 13th Street Interchange
Two-Way Split Partial Cloverleaf
LOS and Laneage
Figure 10A

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage

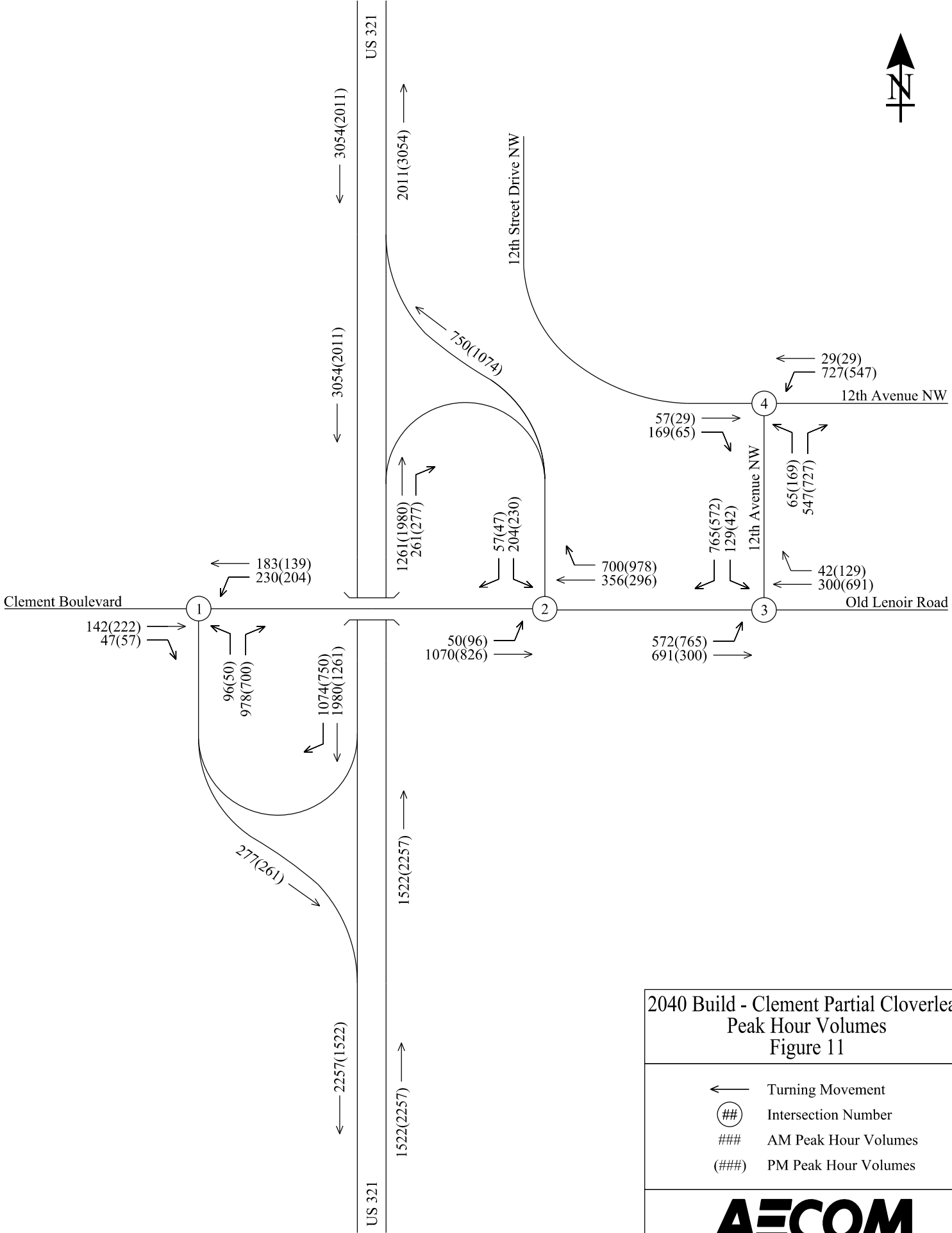




2040 Build - 13th Street Interchange
Two-Way Split Partial Cloverleaf
LOS and Laneage
Figure 10B

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage

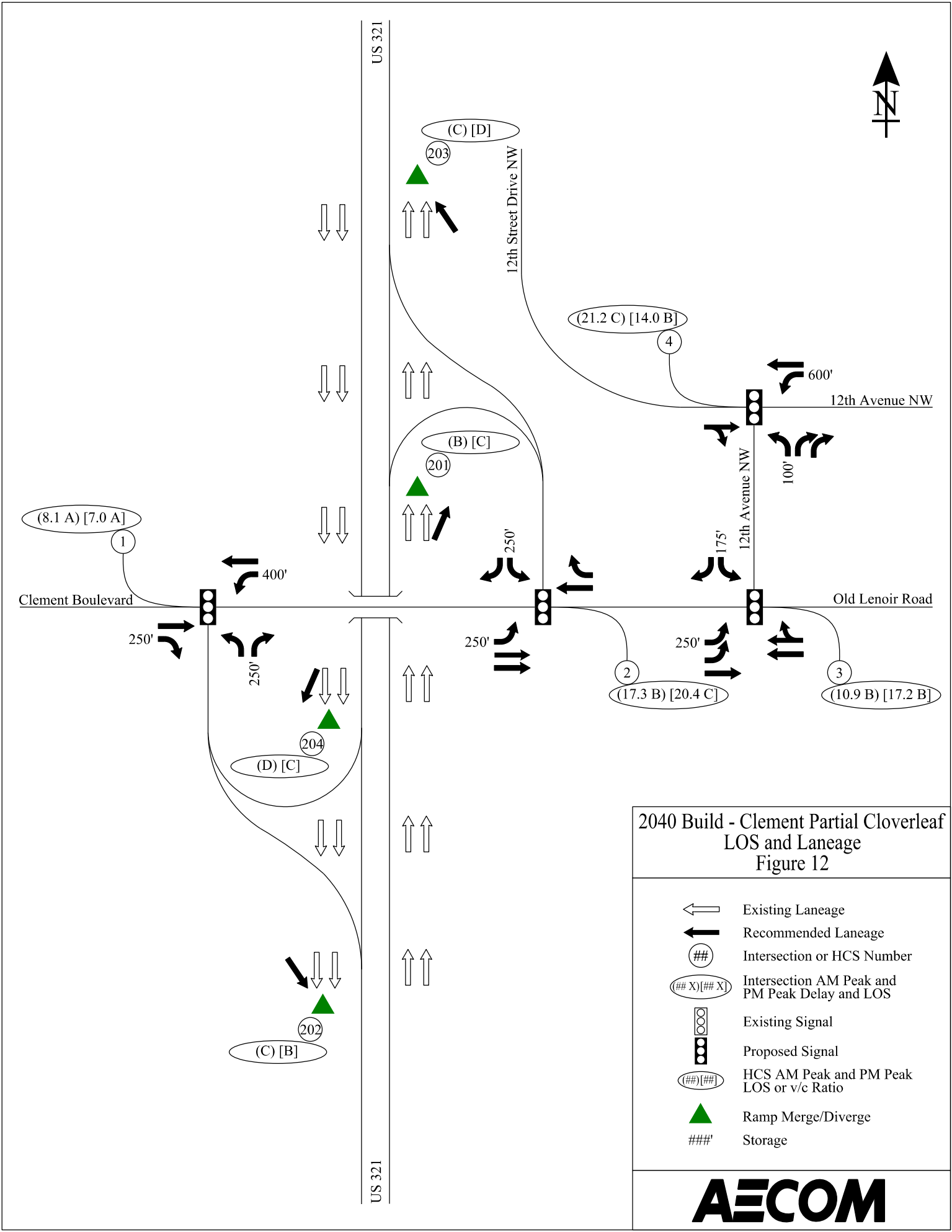




2040 Build - Clement Partial Cloverleaf
 Peak Hour Volumes
 Figure 11

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

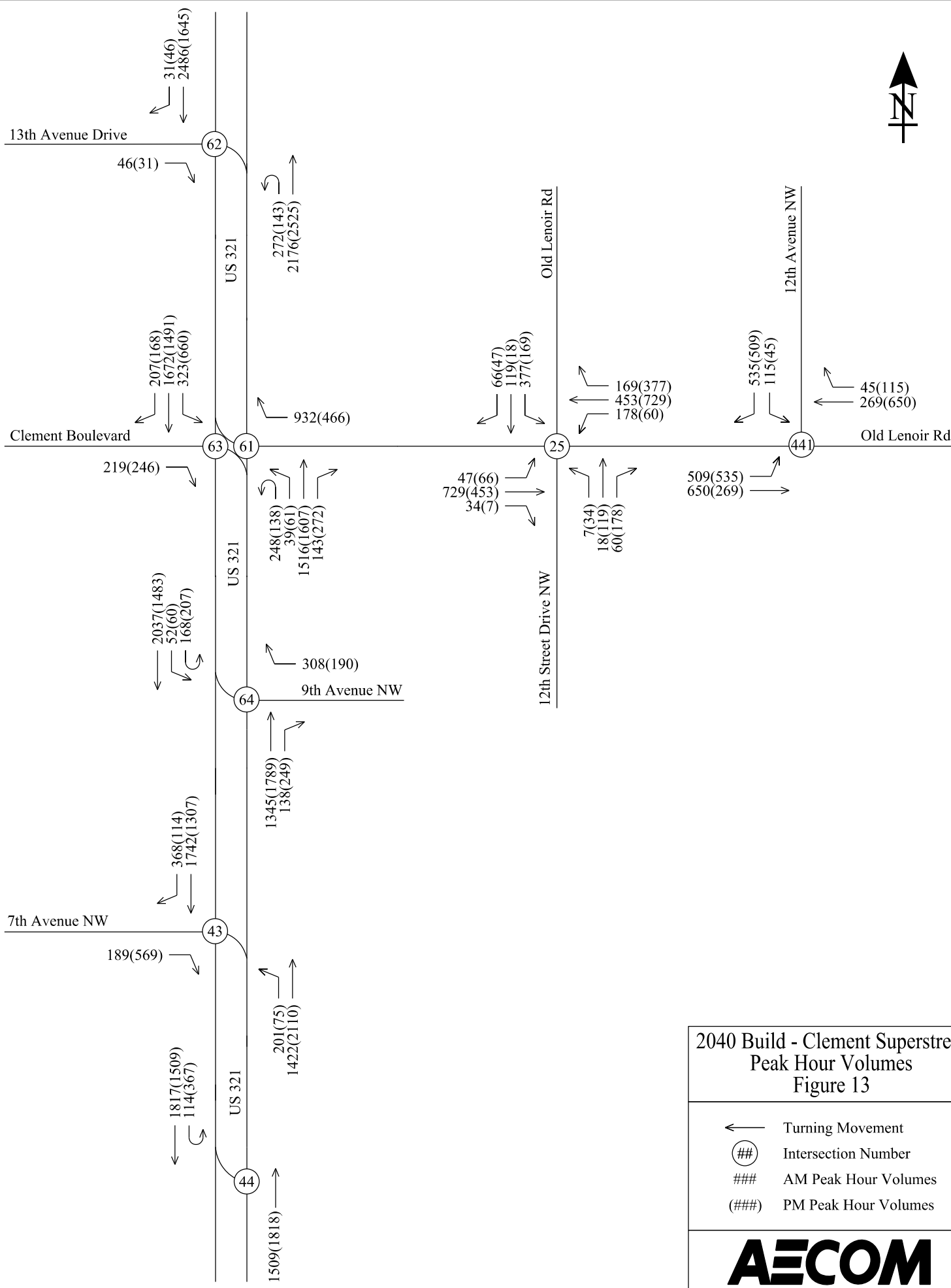




2040 Build - Clement Partial Cloverleaf
LOS and Laneage
Figure 12

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage





13th Avenue Drive

Clement Boulevard

7th Avenue NW

62

63

64

43

44

US 321

US 321

US 321

Old Lenoir Rd

12th Street Drive NW

25

12th Avenue NW

441

Old Lenoir Rd

46(31)

31(46)
2486(1645)

207(168)
1672(1491)
323(660)

219(246)

2037(1483)
52(60)
168(207)

189(569)

1817(1509)
114(367)

272(143)
2176(2525)

932(466)

248(138)
39(61)
1516(1607)
143(272)

308(190)

1345(1789)
138(249)

201(75)
1422(2110)

1509(1818)

66(47)
119(18)
377(169)

169(377)
453(729)
178(60)

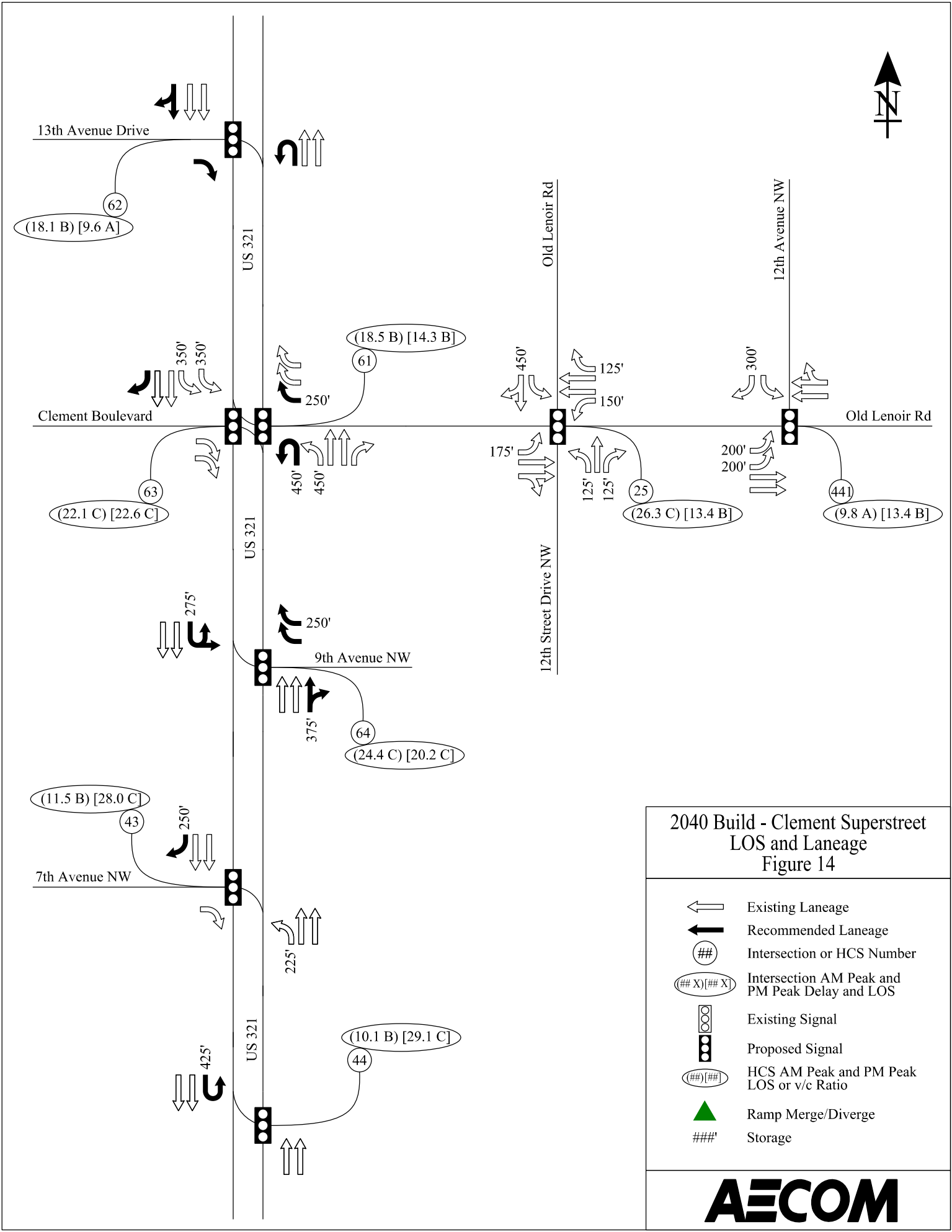
535(509)
115(45)

45(115)
269(650)

47(66)
729(453)
34(7)

7(34)
18(119)
60(178)

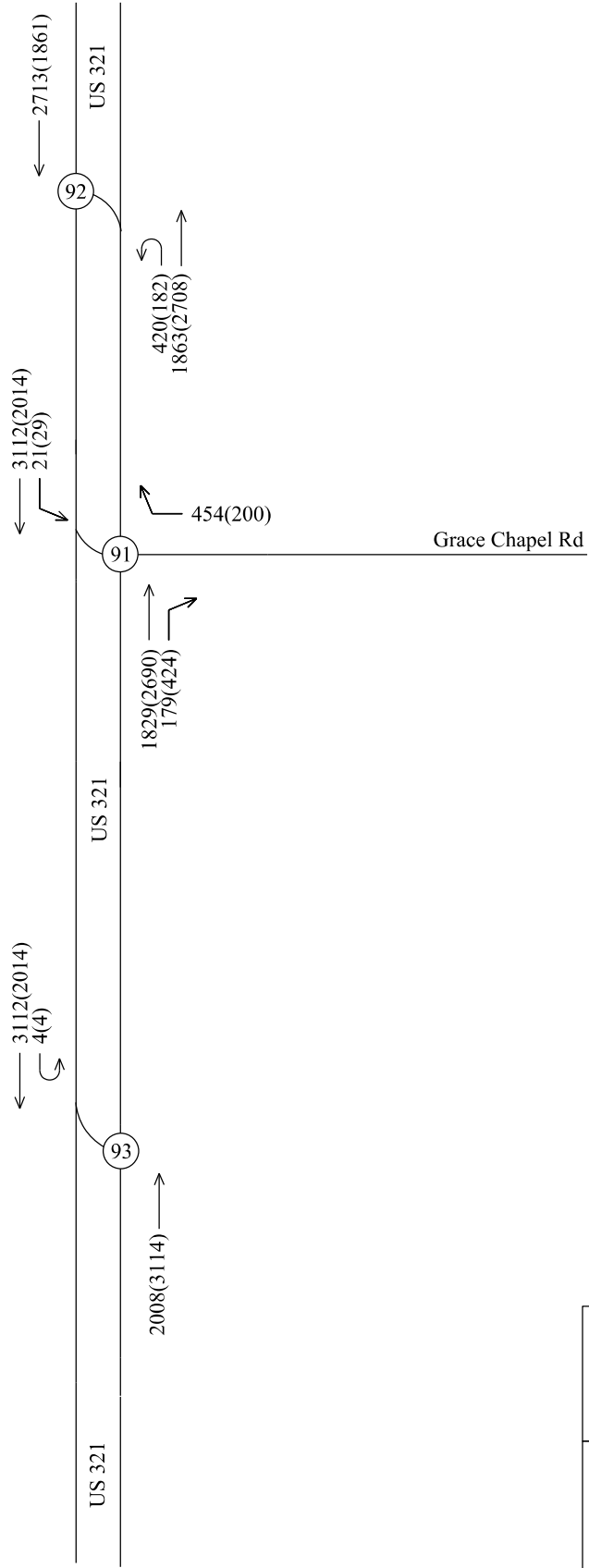
509(535)
650(269)



2040 Build - Clement Superstreet
LOS and Laneage
Figure 14

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage

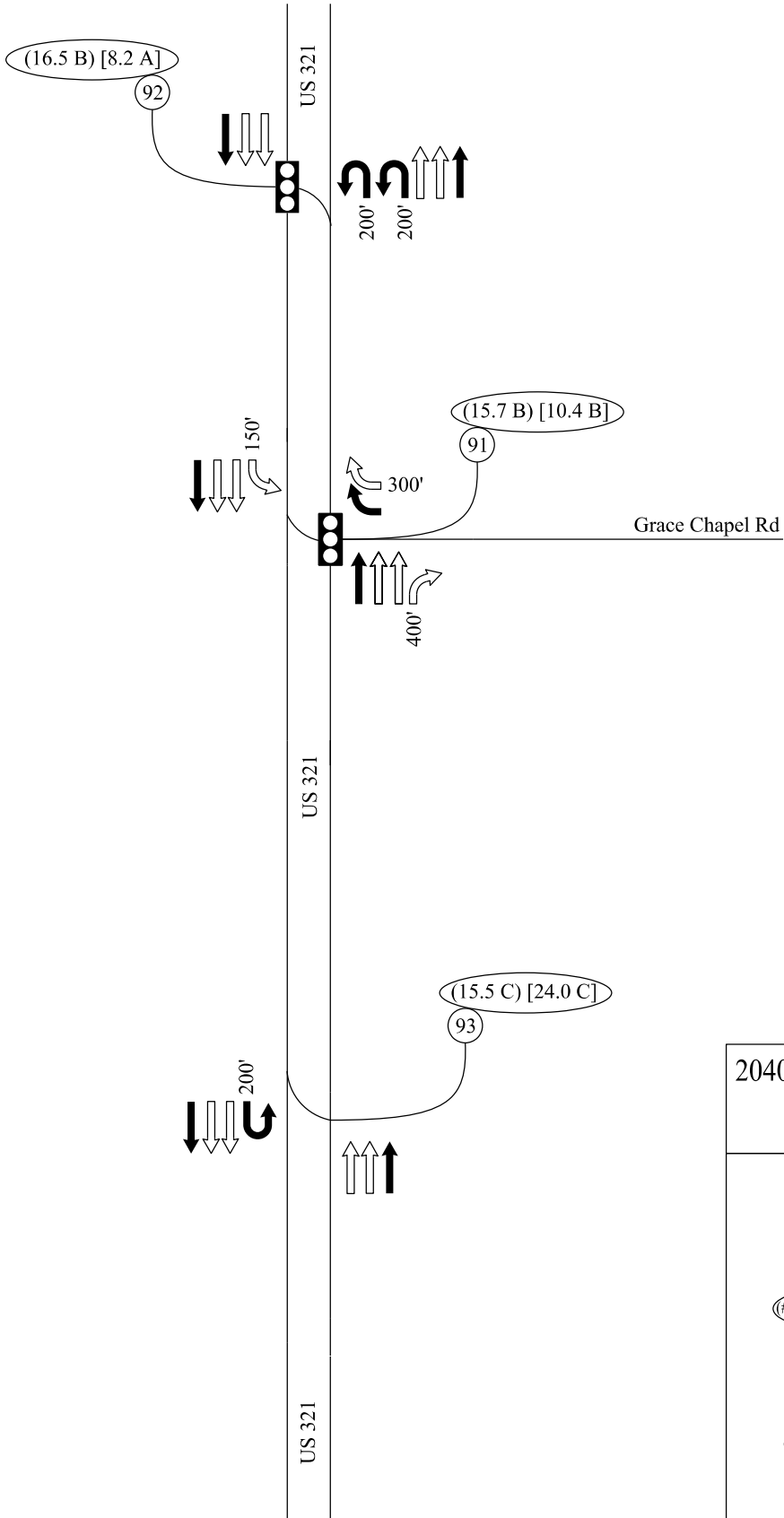




2040 Build - Grace Chapel
Superstreet
Peak Hour Volumes
Figure 15

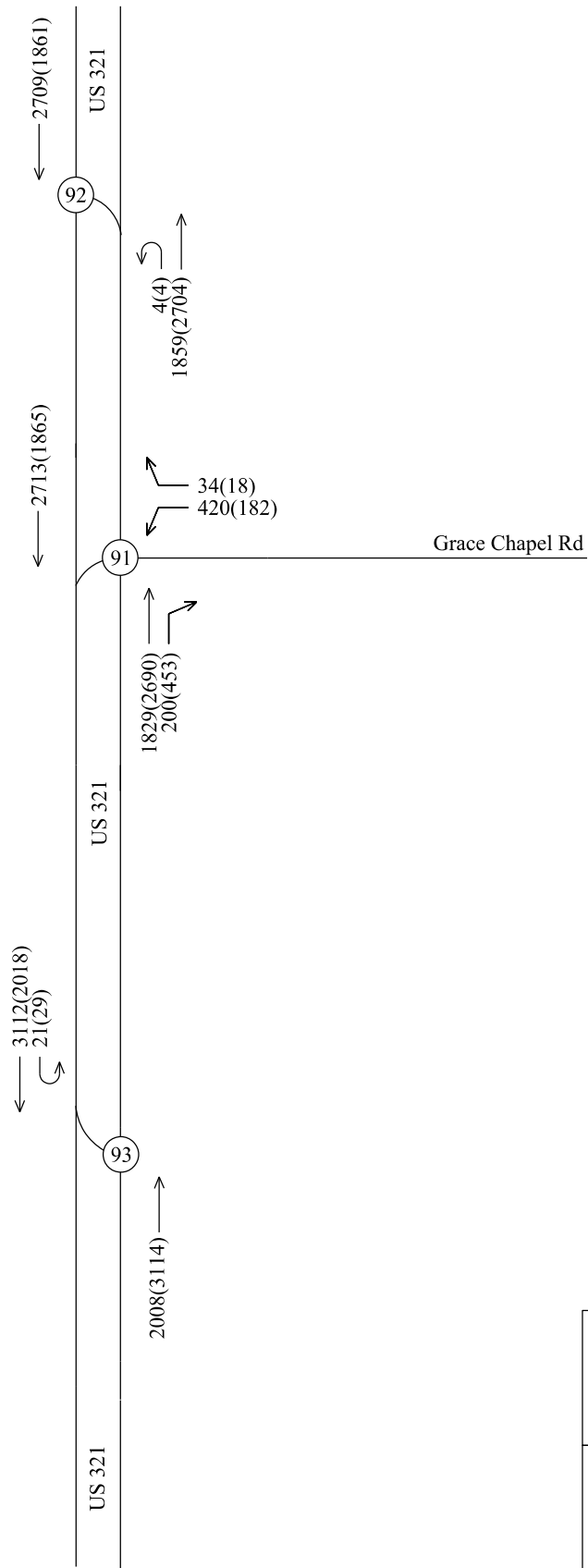
- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes





2040 Build - Grace Chapel Superstreet
LOS and Laneage
Figure 16

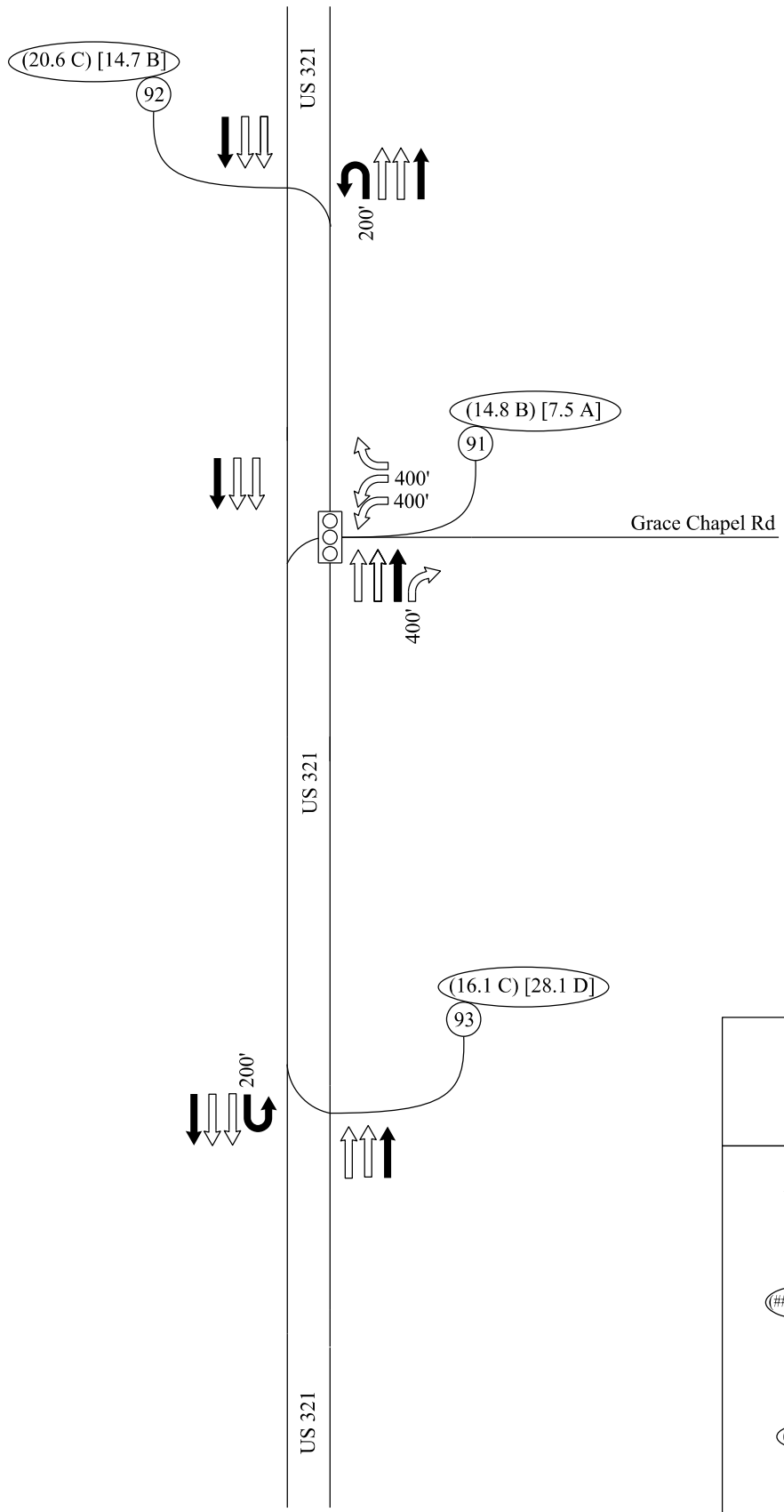
- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage



2040 Build - Grace Chapel
Reverse Superstreet
Peak Hour Volumes
Figure 17

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



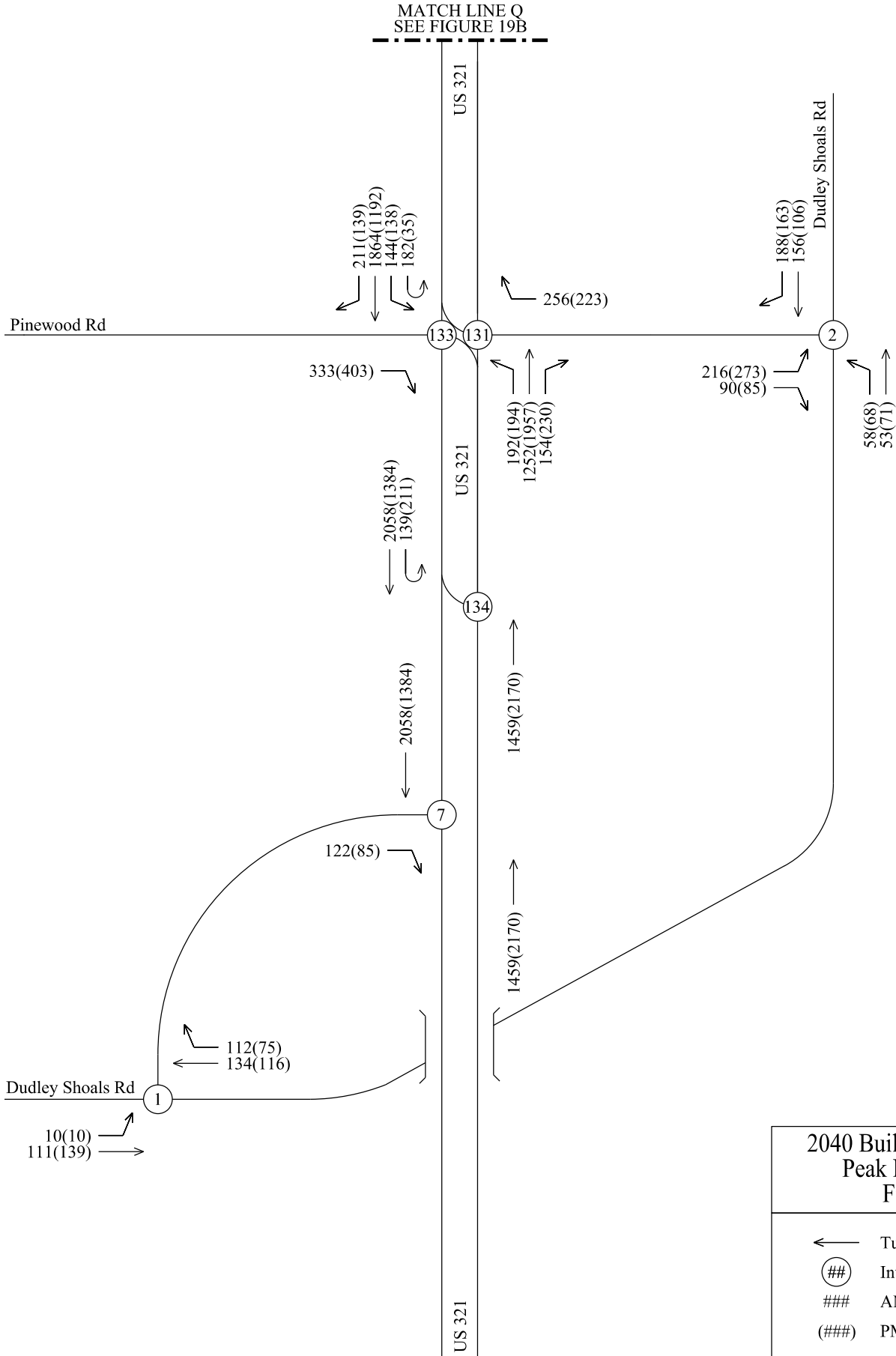


**2040 Build - Grace Chapel
Reverse Superstreet
LOS and Laneage
Figure 18**

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage



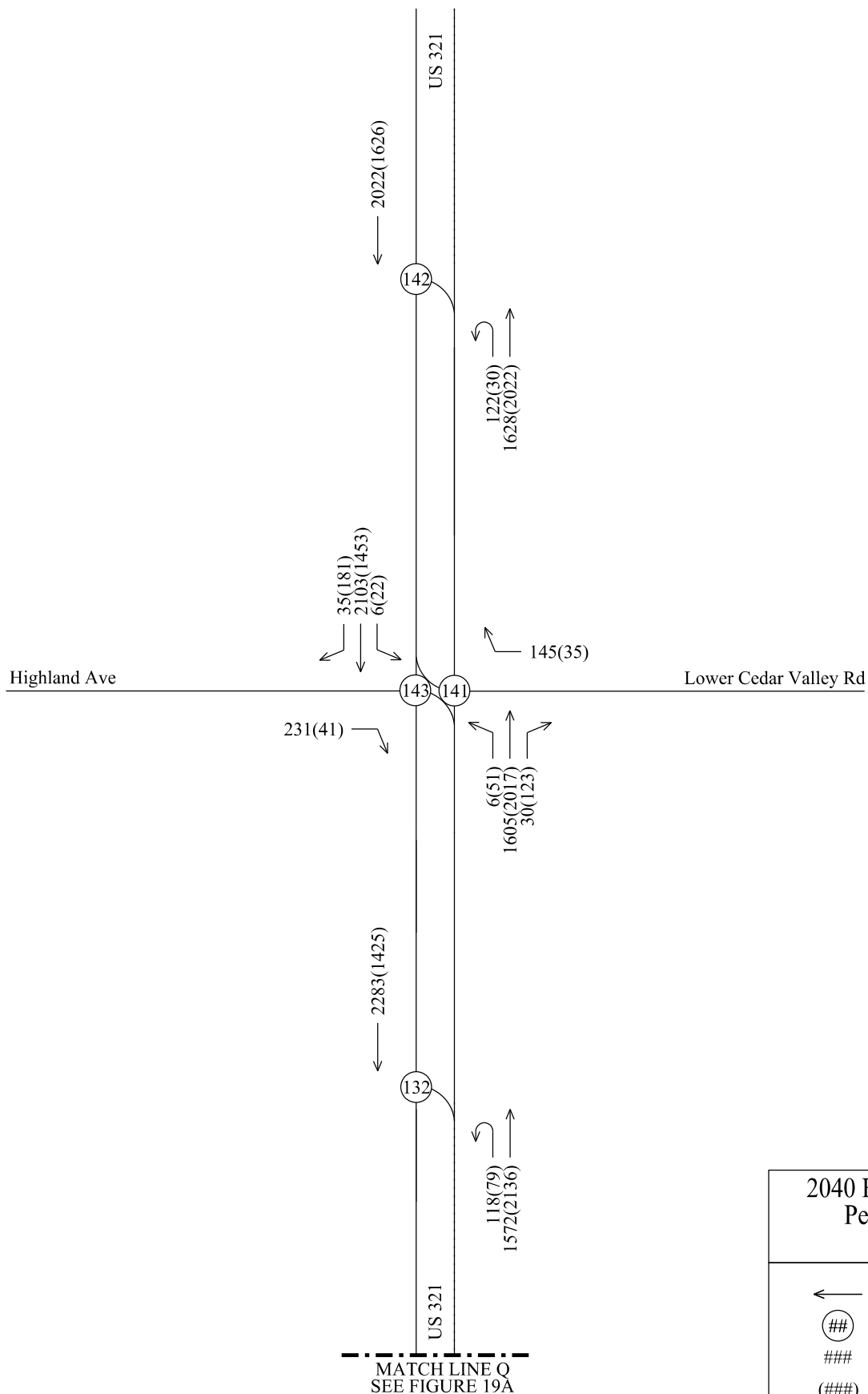
MATCH LINE Q
SEE FIGURE 19B



2040 Build - Dudley Shoals
Peak Hour Volumes
Figure 19A

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



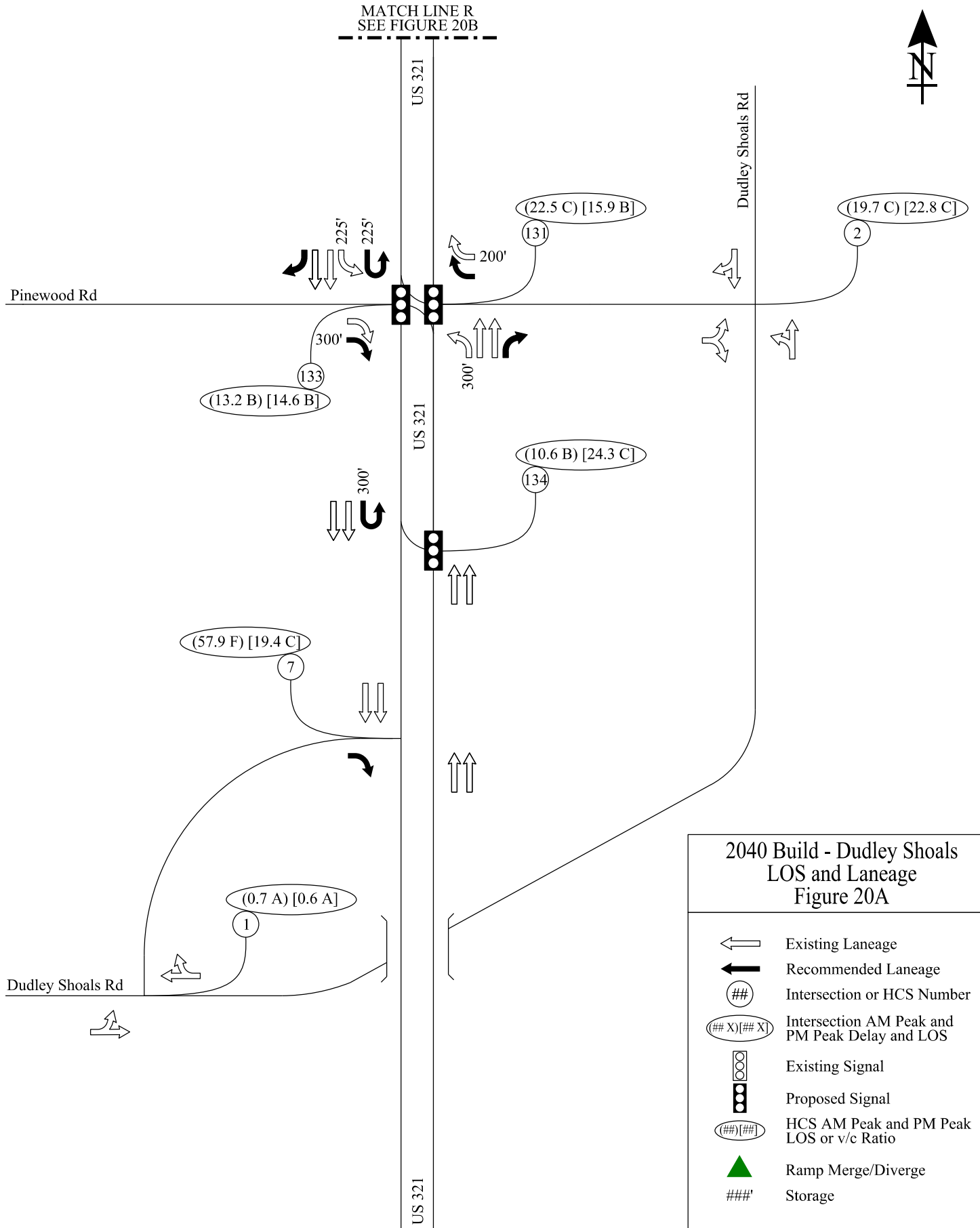


2040 Build - Dudley Shoals
Peak Hour Volumes
Figure 19B

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

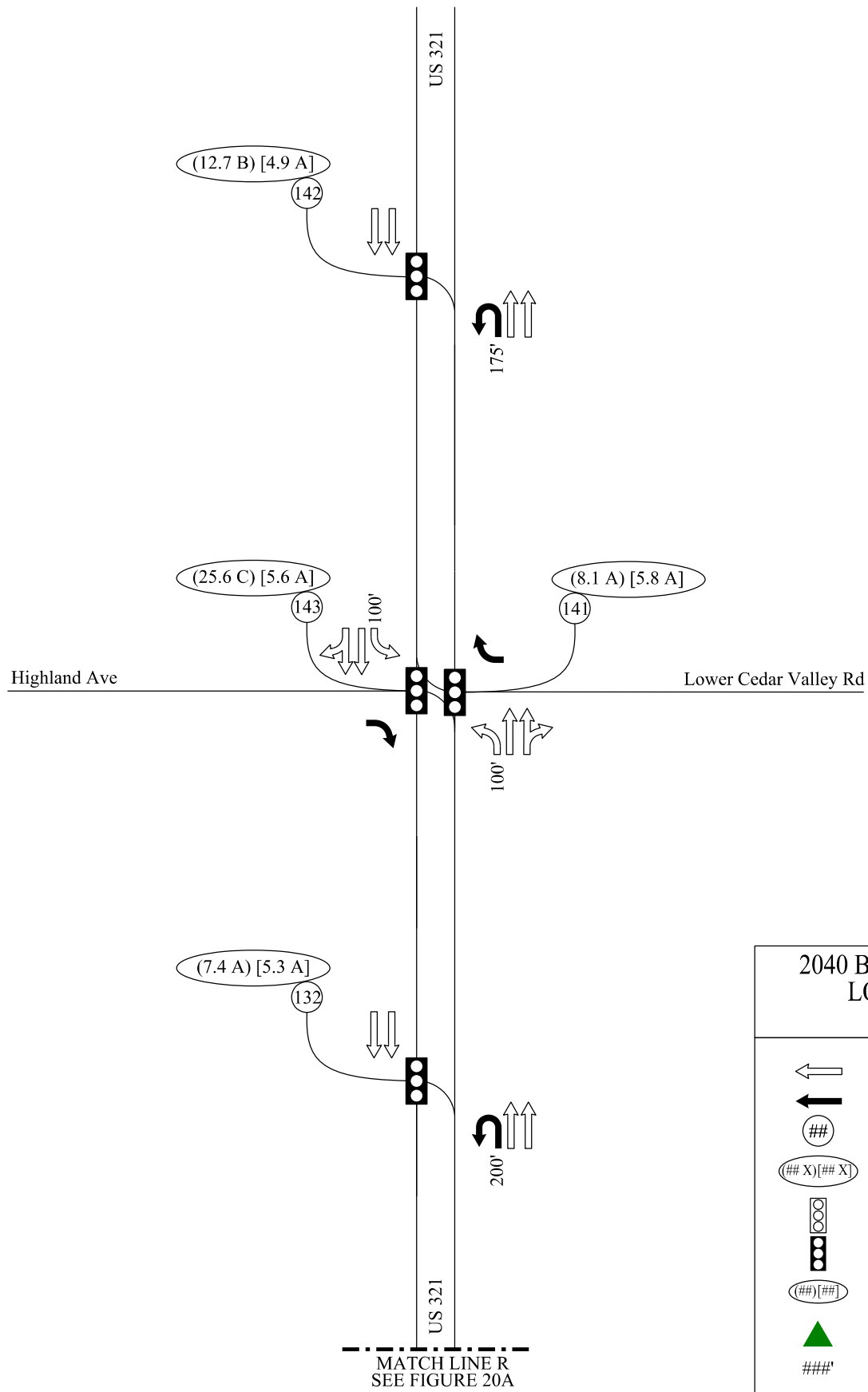


MATCH LINE R
SEE FIGURE 20B












2040 Build - Dudley Shoals
LOS and Laneage
Figure 20A

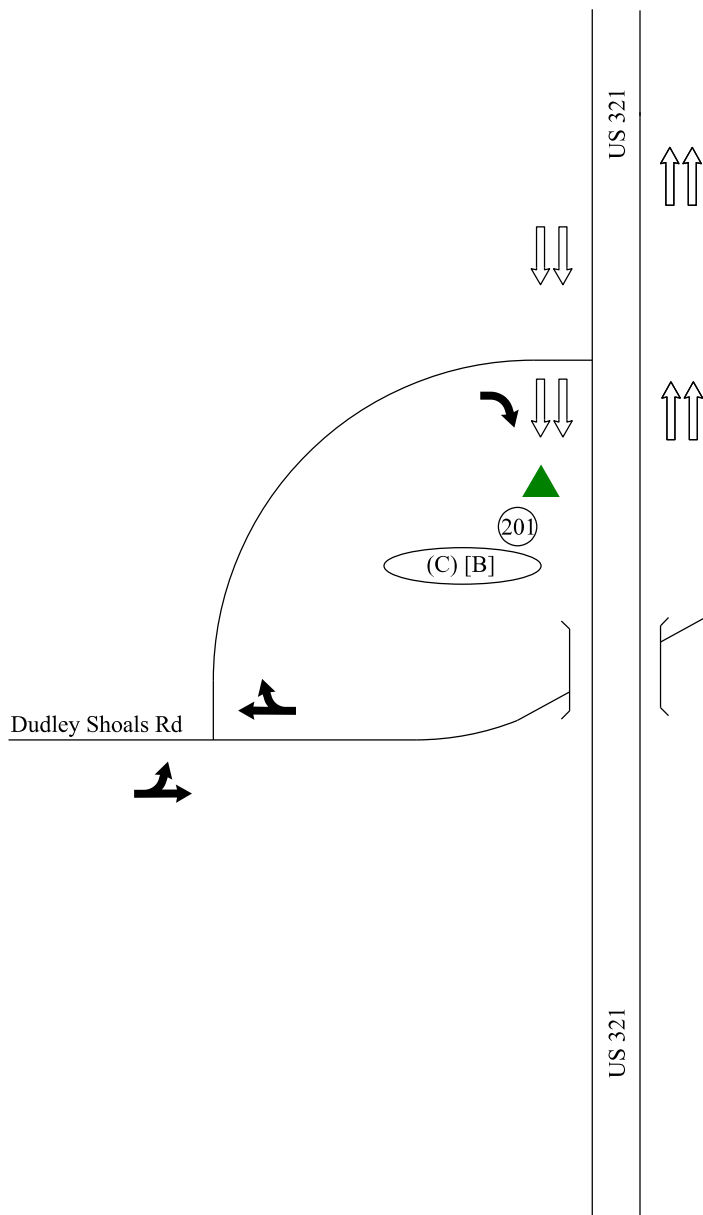
- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage






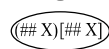





2040 Build - Dudley Shoals
LOS and Laneage
Figure 20B

-  Existing Laneage
-  Recommended Laneage
-  Intersection or HCS Number
-  Intersection AM Peak and PM Peak Delay and LOS
-  Existing Signal
-  Proposed Signal
-  HCS AM Peak and PM Peak LOS or v/c Ratio
-  Ramp Merge/Diverge
-  Storage



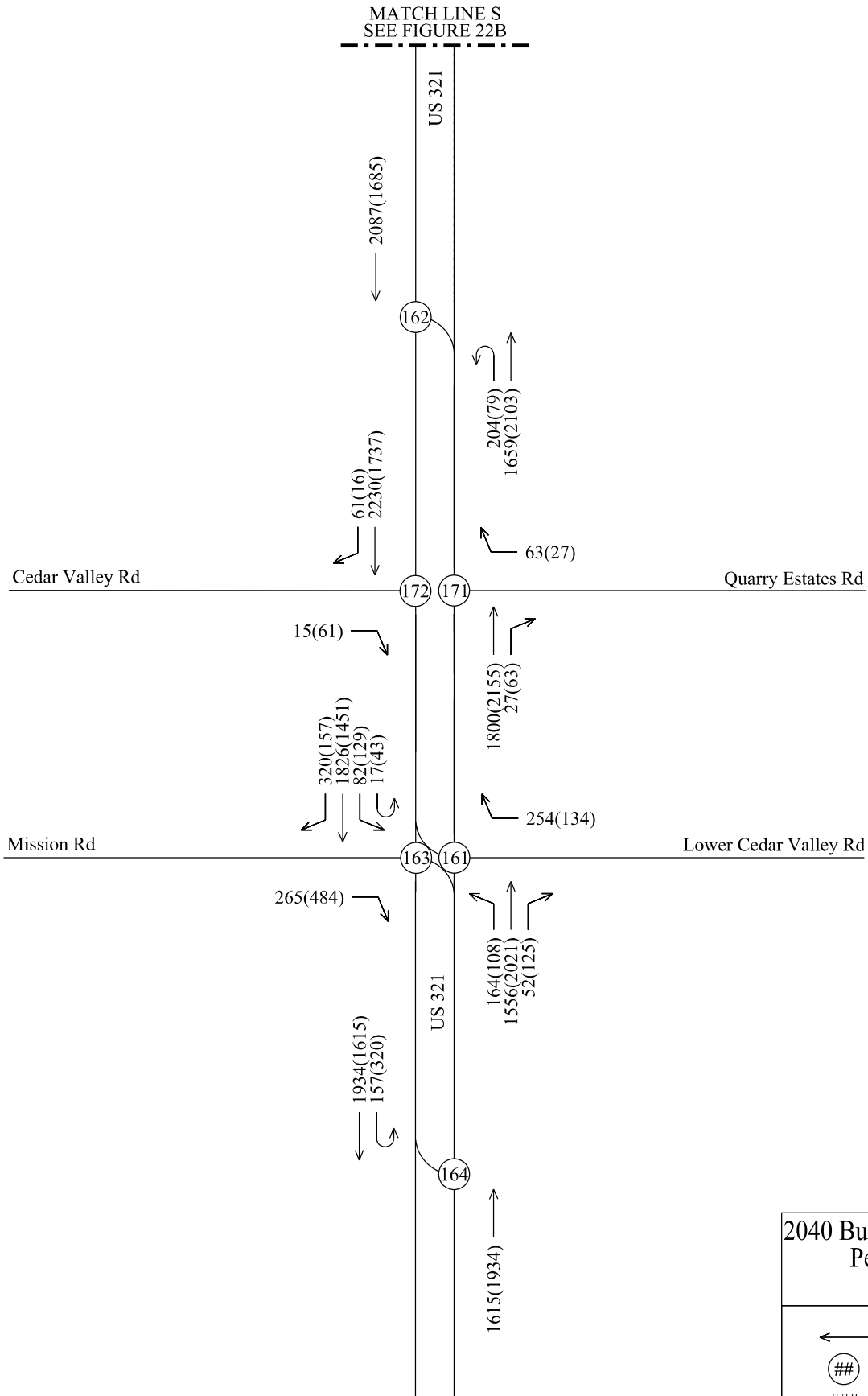


2040 Build - Dudley Shoals
Freeway Ramp
LOS and Laneage
Figure 21

-  Existing Laneage
-  Recommended Laneage
-  Intersection or HCS Number
-  Intersection AM Peak and PM Peak Delay and LOS
-  Existing Signal
-  Proposed Signal
-  HCS AM Peak and PM Peak LOS or v/c Ratio
-  Ramp Merge/Diverge
-  Storage



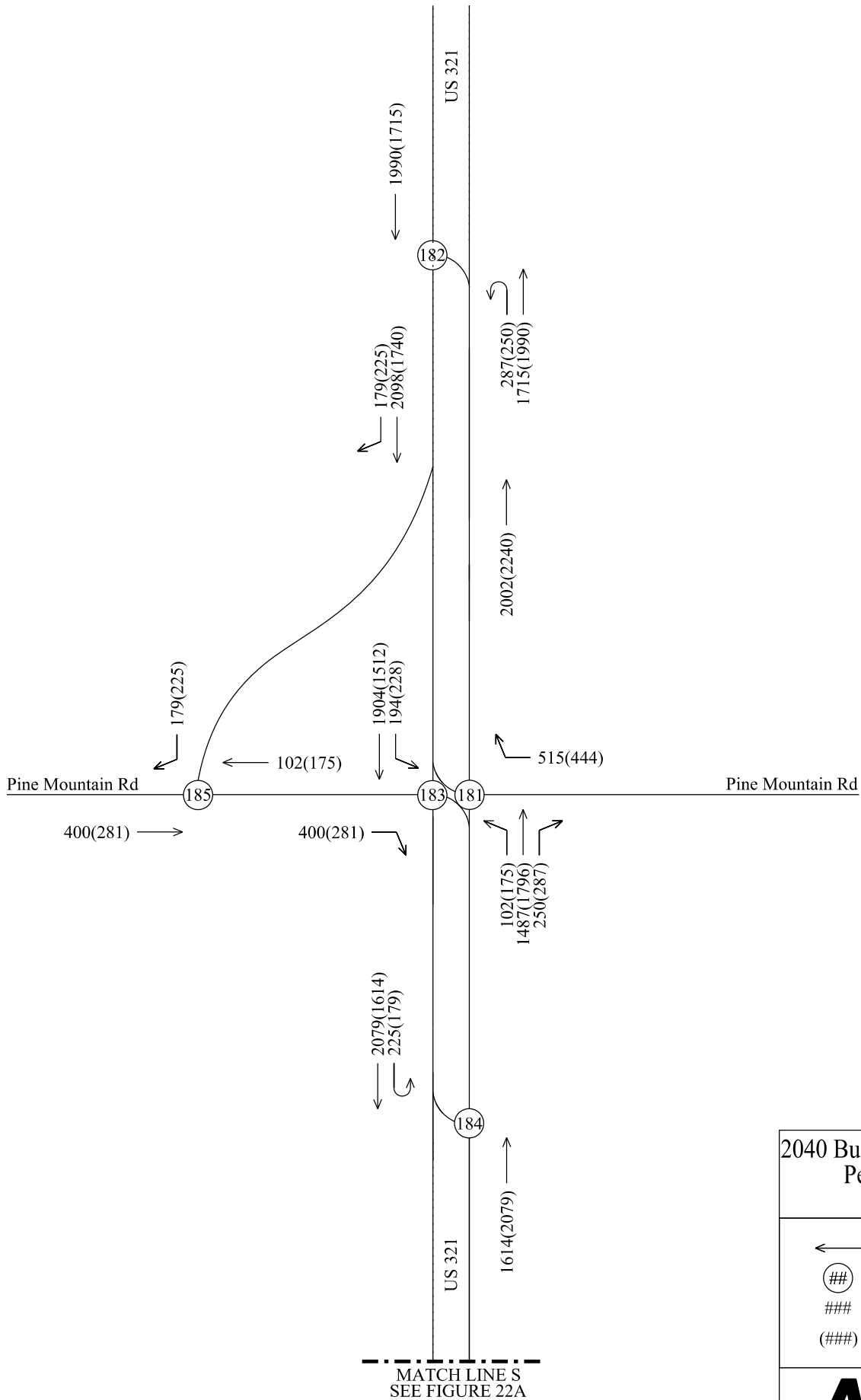
MATCH LINE S
SEE FIGURE 22B



2040 Build - Lower Cedar Valley
Peak Hour Volumes
Figure 22A

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



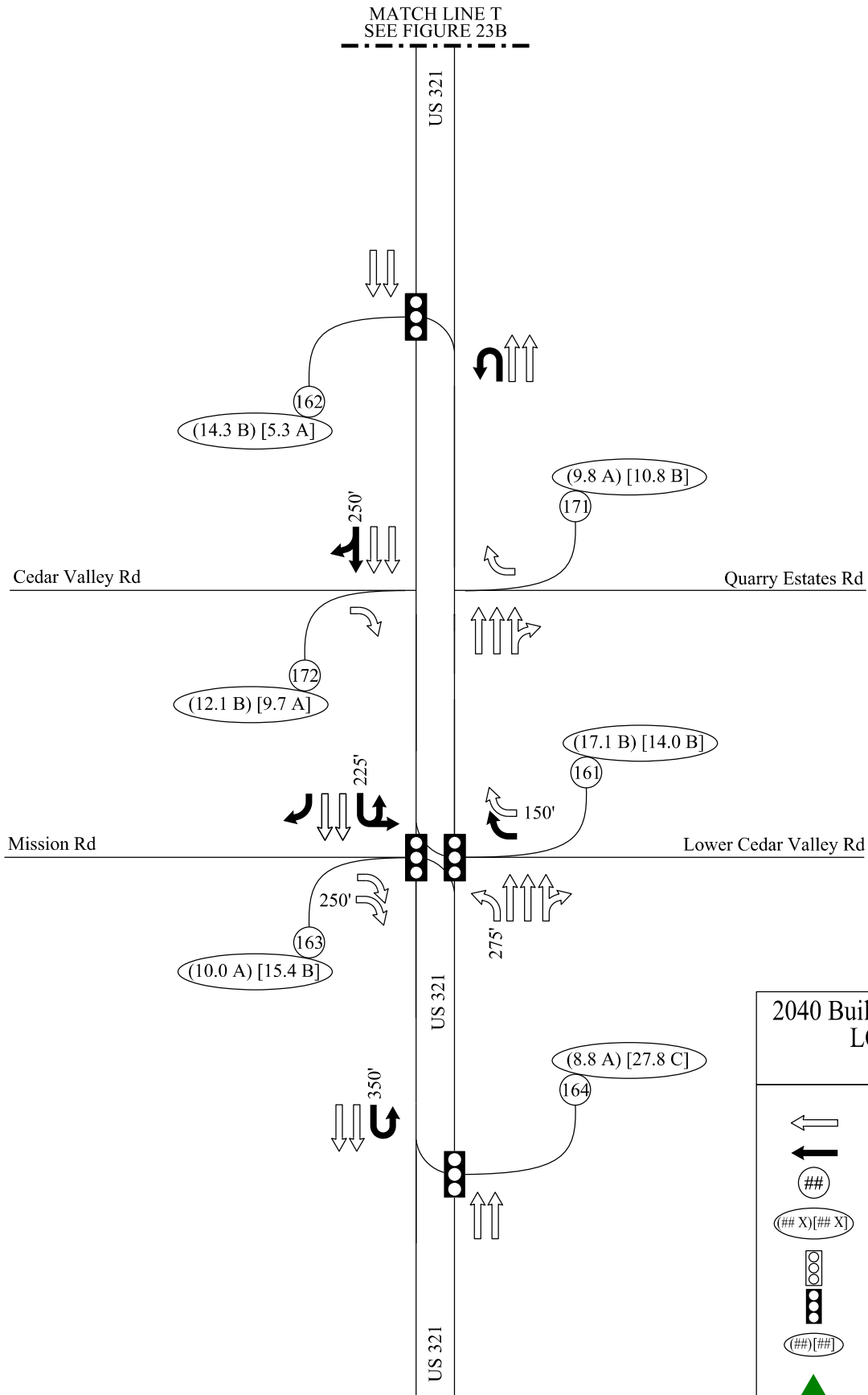


2040 Build - Lower Cedar Valley
Peak Hour Volumes
Figure 22B

- ← Turning Movement
- ⊙ Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



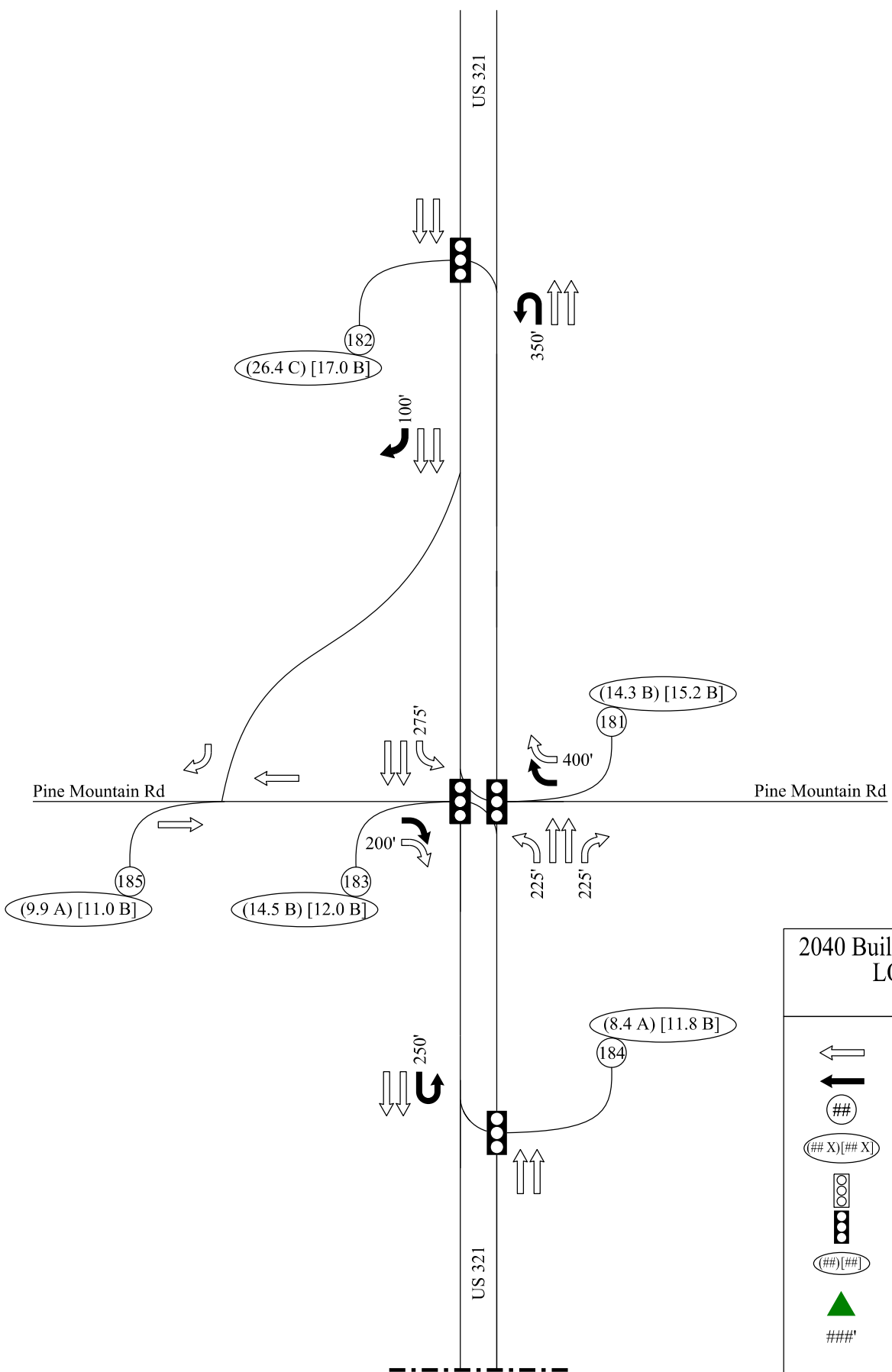
MATCH LINE T
SEE FIGURE 23B



2040 Build - Lower Cedar Valley
LOS and Laneage
Figure 23A

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage





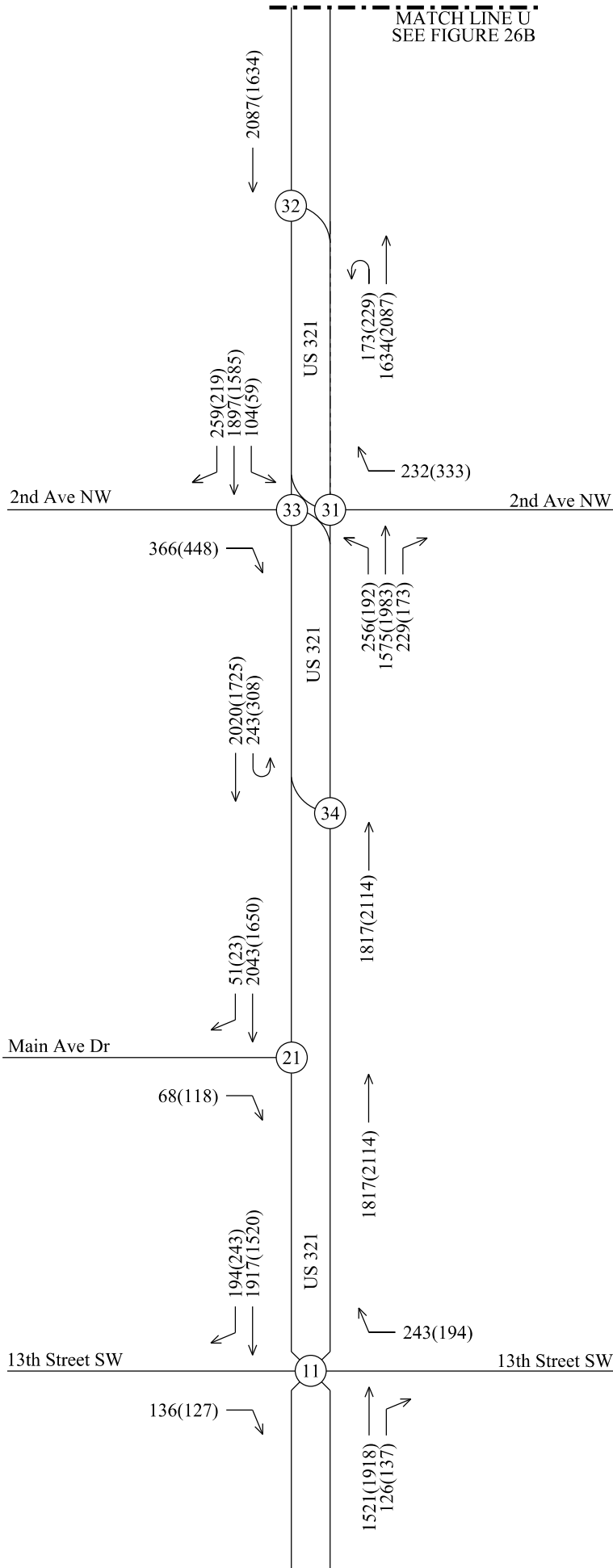
2040 Build - Lower Cedar Valley
LOS and Laneage
Figure 23B

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage

MATCH LINE T
SEE FIGURE 23A



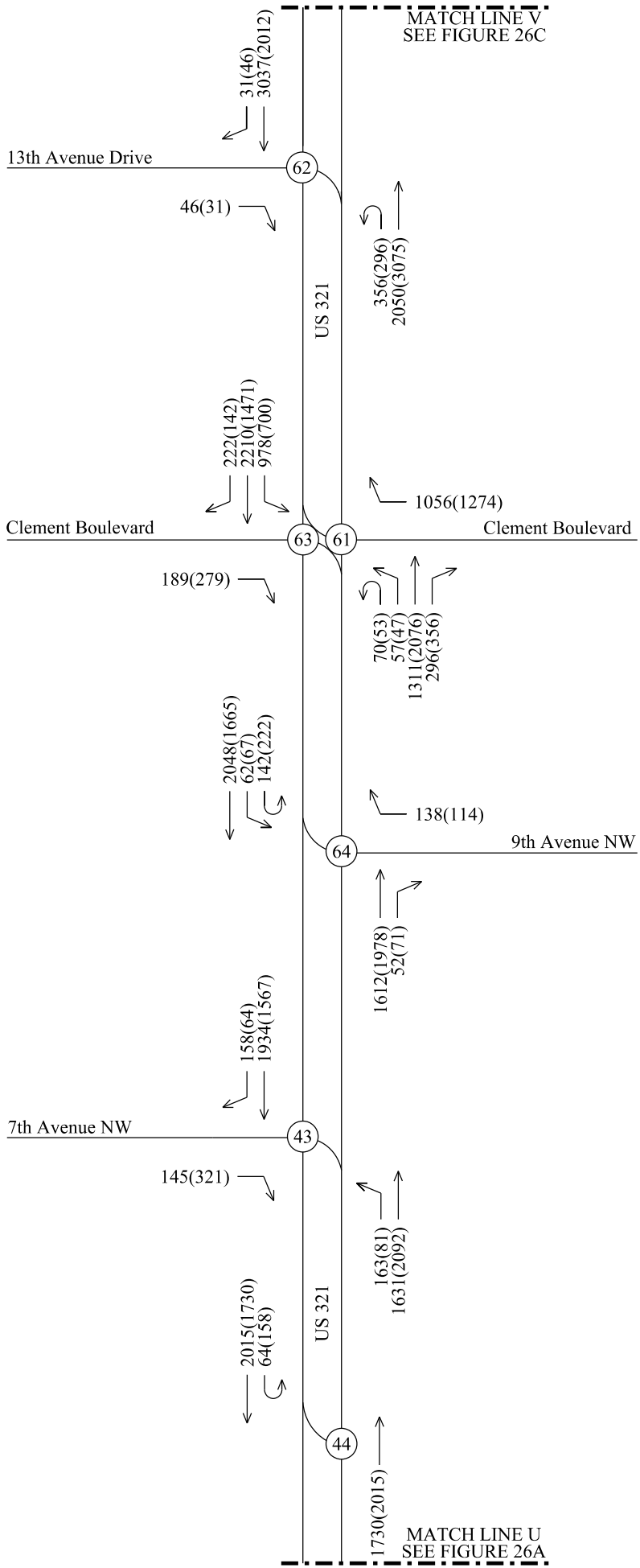
MATCH LINE U
SEE FIGURE 26B



2040 Build - MDI Simulation
Peak Hour Volumes
Figure 26A

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



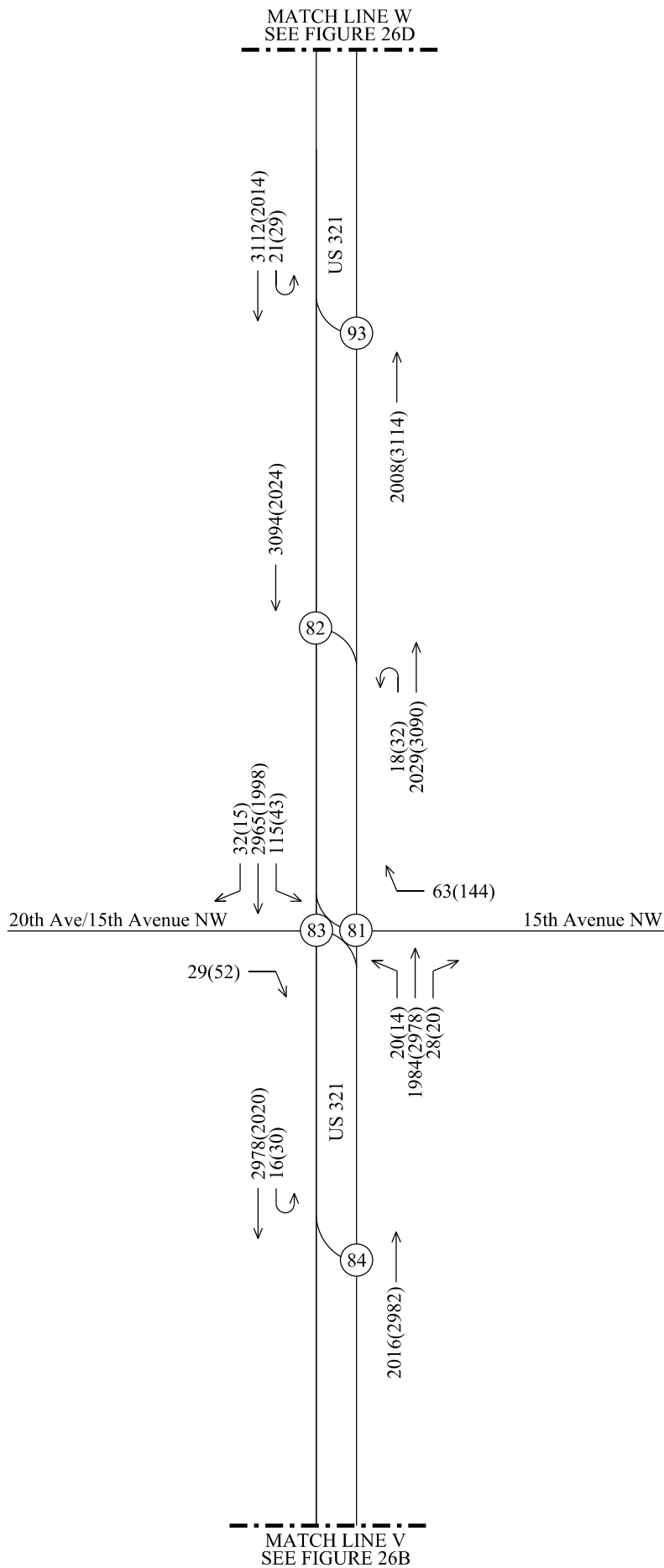


2040 Build - MDI Simulation
Peak Hour Volumes
Figure 26B

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



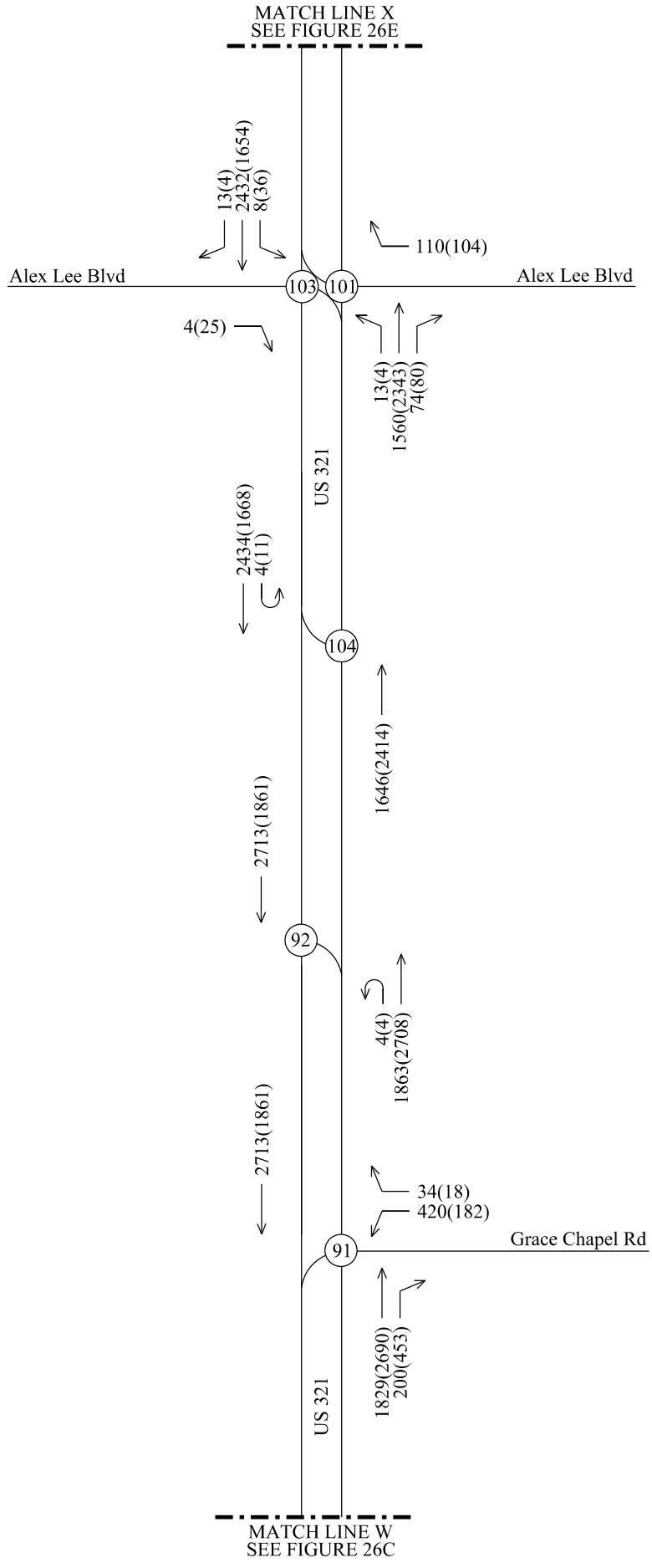
MATCH LINE W
SEE FIGURE 26D



2040 Build - MDI Simulation
Peak Hour Volumes
Figure 26C

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes

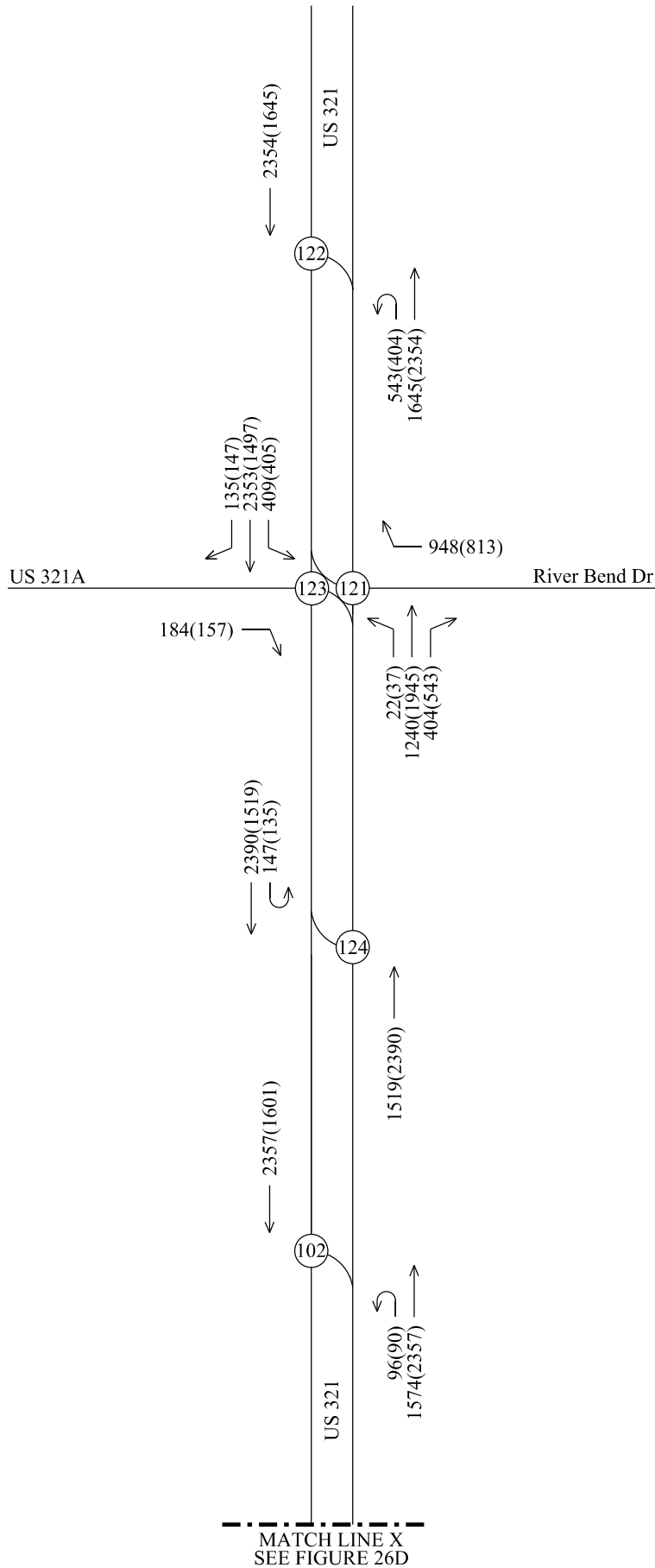




2040 Build - MDI Simulation
Peak Hour Volumes
Figure 26D

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



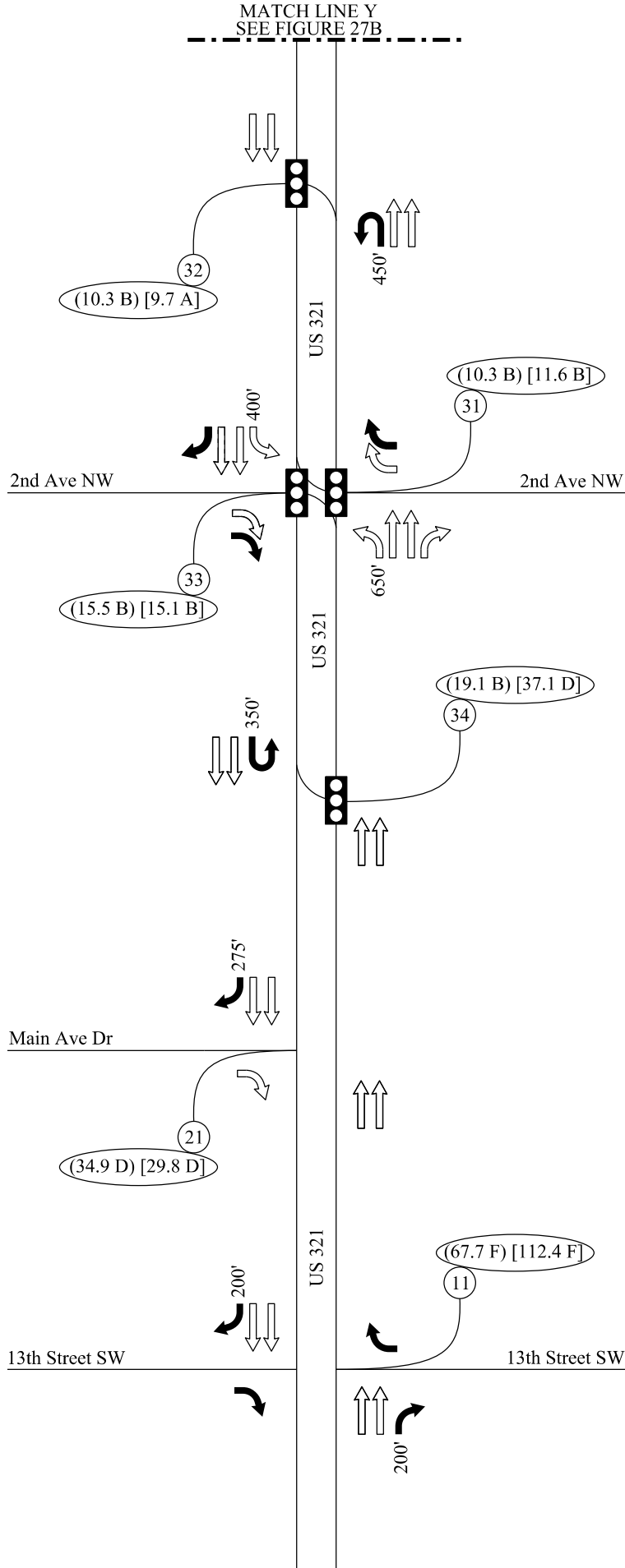


2040 Build - MDI Simulation
Peak Hour Volumes
Figure 26E

- ← Turning Movement
- ⊙## Intersection Number
- ### AM Peak Hour Volumes
- (###) PM Peak Hour Volumes



MATCH LINE Y
SEE FIGURE 27B



2040 Build - MDI Simulation
LOS and Laneage
Figure 27A

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage



MATCH LINE Z
SEE FIGURE 27C



13th Avenue Drive

(18.7 B) [11.3 B]

US 321



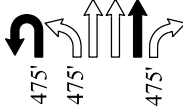
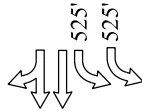
Clement Boulevard

(13.7 B) [16.5 B]

(15.5 B) [26.3 C]

(61)

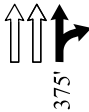
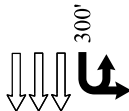
Clement Boulevard



(17.7 B) [22.5 C]

(64)

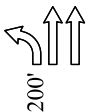
9th Avenue NW



7th Avenue NW

(10.0 B) [16.6 B]

US 321



(4.2 A) [10.1 B]

(44)



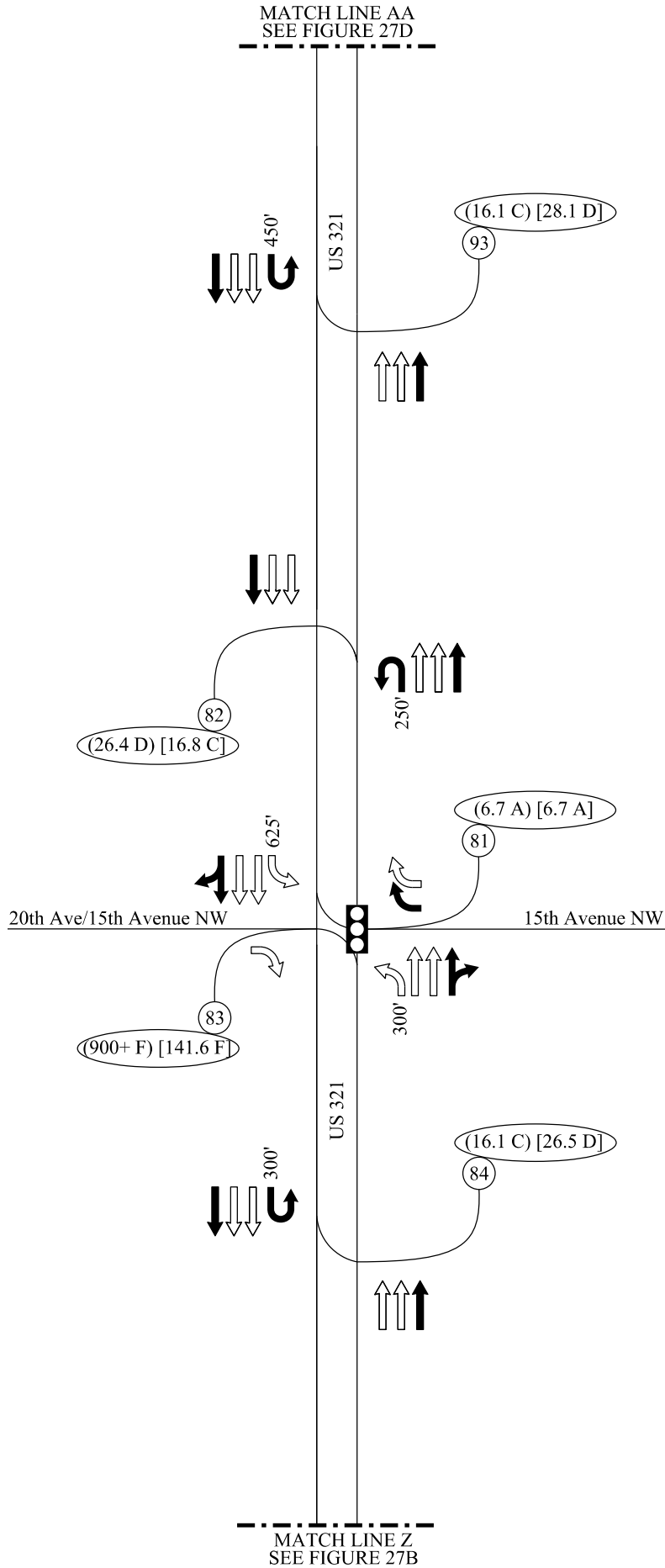
MATCH LINE Y
SEE FIGURE 27A

2040 Build - MDI Simulation
LOS and Laneage
Figure 27B

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage



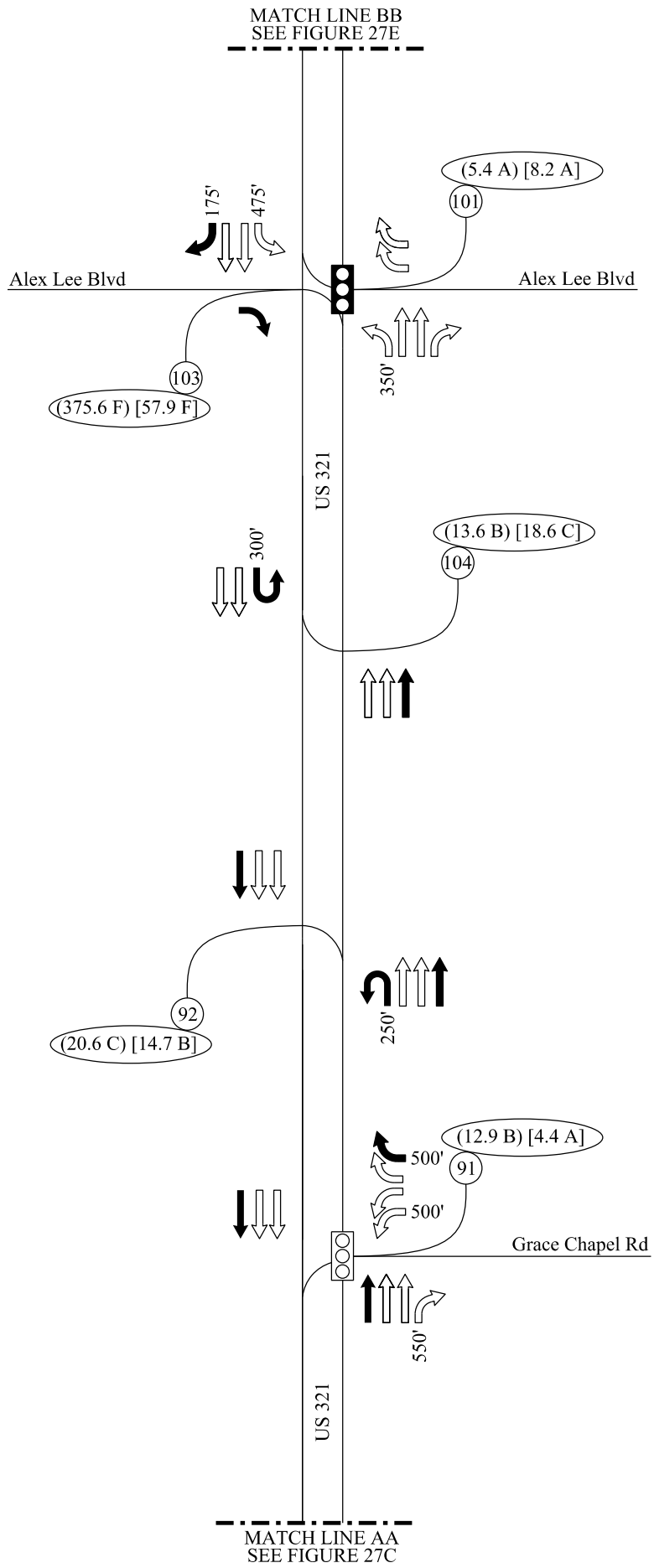
MATCH LINE AA
SEE FIGURE 27D



2040 Build - MDI Simulation
LOS and Laneage
Figure 27C

- Existing Laneage
- Recommended Laneage
- Intersection or HCS Number
- Intersection AM Peak and PM Peak Delay and LOS
- Existing Signal
- Proposed Signal
- HCS AM Peak and PM Peak LOS or v/c Ratio
- Ramp Merge/Diverge
- Storage

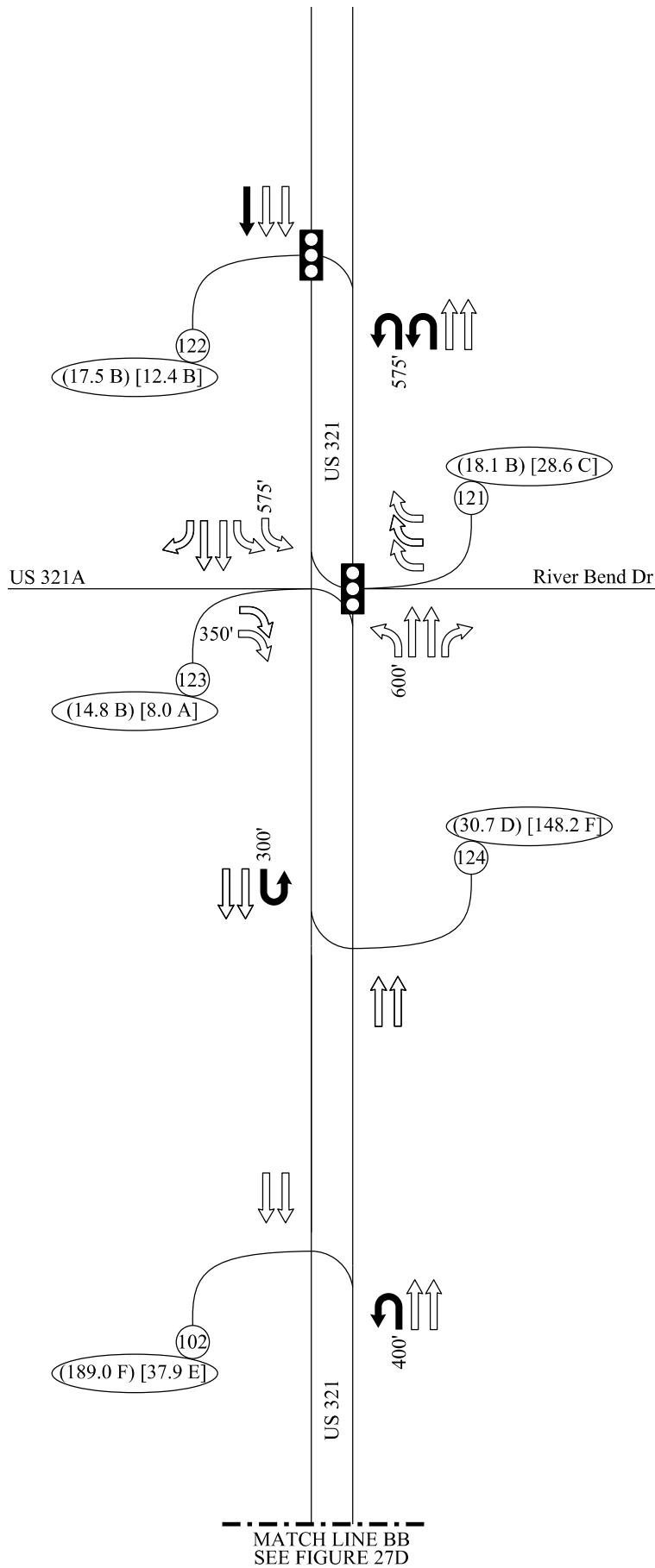




**2040 Build - MDI Simulation
LOS and Laneage
Figure 27D**

	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage





2040 Build - MDI Simulation LOS and Laneage Figure 27E	
	Existing Laneage
	Recommended Laneage
	Intersection or HCS Number
	Intersection AM Peak and PM Peak Delay and LOS
	Existing Signal
	Proposed Signal
	HCS AM Peak and PM Peak LOS or v/c Ratio
	Ramp Merge/Diverge
	Storage



APPENDIX A
U-4700 Traffic Forecast

February 28, 2017

Memorandum To: Gene Tarascio
Project Development Engineer
Project Development and Environmental Analysis

■
421 Fayetteville St.
Suite 600
Raleigh, North Carolina
27601

From: Taylor Honeycutt, PE
Kimley-Horn and Associates, Inc.

Subject: Traffic Forecast for U-4700 (US 321 widening) in Caldwell, Catawba and Burke Counties (Phase I)

Please find attached the Phase I and Phase II 2016 traffic estimates and 2040 traffic forecasts for U-4700 in Caldwell, Catawba and Burke Counties which concerns the widening of US 321 from 4-lane divided to 6-lane divided from Southwest Boulevard in Caldwell County to US 70 in Catawba County. The following scenarios are provided:

- Base Year 2016 No-Build
- Base Year 2016 Build 1 (Interchange Configuration 1 at 13th Street)
- Base Year 2016 Build 2 (Interchange Configuration 2 at 13th Street)
- Future Year 2040 No-Build
- Future Year 2040 Build 1 (Interchange Configuration 1 at 13th Street)
- Future Year 2040 Build 2 (Interchange Configuration 2 at 13th Street)

Brian Wert, Dean Ledbetter, and Paul Schroeder of NCDOT along with John Marshall of the Western Piedmont COG were contacted during the development of this forecast. Teresa Gresham of Kimley-Horn was also consulted regarding forecast assumptions and alternatives.

This forecast is being completed in two phases. Phase I consisted of only the intersections and surrounding area of the following:

US 321 @ Grace Chapel Road
US 321 @ Falls Avenue
US 321 @ Clement Boulevard
US 321 @ 13th Street SW

Phase I was completed and accepted on December 21, 2016. The remaining intersections are provided in the following Phase II forecast. Phase I intersections are included for reference.

Certain assumptions were made in the development of the forecast:

Fiscal Constraint. Within the Metropolitan Planning Organization (MPO) area, future forecasts are based on projects included in the Financial Plan for the 2040 Greater Hickory Long Range Transportation Plan (LRTP). This information is included in the official version of the Greater Hickory Regional Model.



Future Conditions and Development Activity. The forecast was developed using output from the Greater Hickory Regional Model. Assumptions about future development activity and changes in the distribution of population and employment in the forecast study area are implicit in the model.

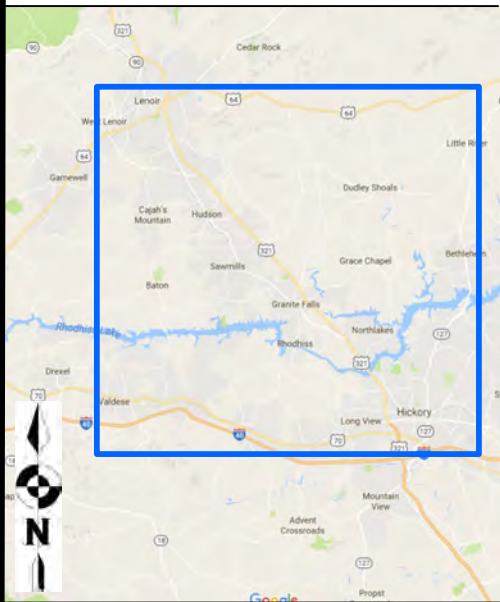
Forecast Methodology. Base year 2016 estimates and Horizon Year 2040 forecasts provided in the attached forecast were developed using a method under which observed traffic data as well as 2010 and 2040 model output were considered.

To determine any intermediate years, straight-line interpolation may be used. AADT volumes may be extrapolated for up to two years immediately following 2040. If it is determined that any of these assumptions have become inconsistent with the project and surrounding area activity, please request updated projections at this location.

This forecast was reviewed and approved by the Transportation Planning Branch on 02/24/2017.

Cc: Keith Dixon, Transportation Planning Branch
Terry Arellano, PE, Transportation Planning Branch
James Dunlop, PE, Congestion Management Section
Clark Morrison, PhD, PE, Pavement Management Unit
Glenn Mumford, PE, Roadway Design Unit
Anil Panicker, Division Planning Engineer
Dean Ledbetter, Division Planning Engineer
John Marshall, MPO Manager, Greater Hickory MPO





2016

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 1 OF 4

LEGEND

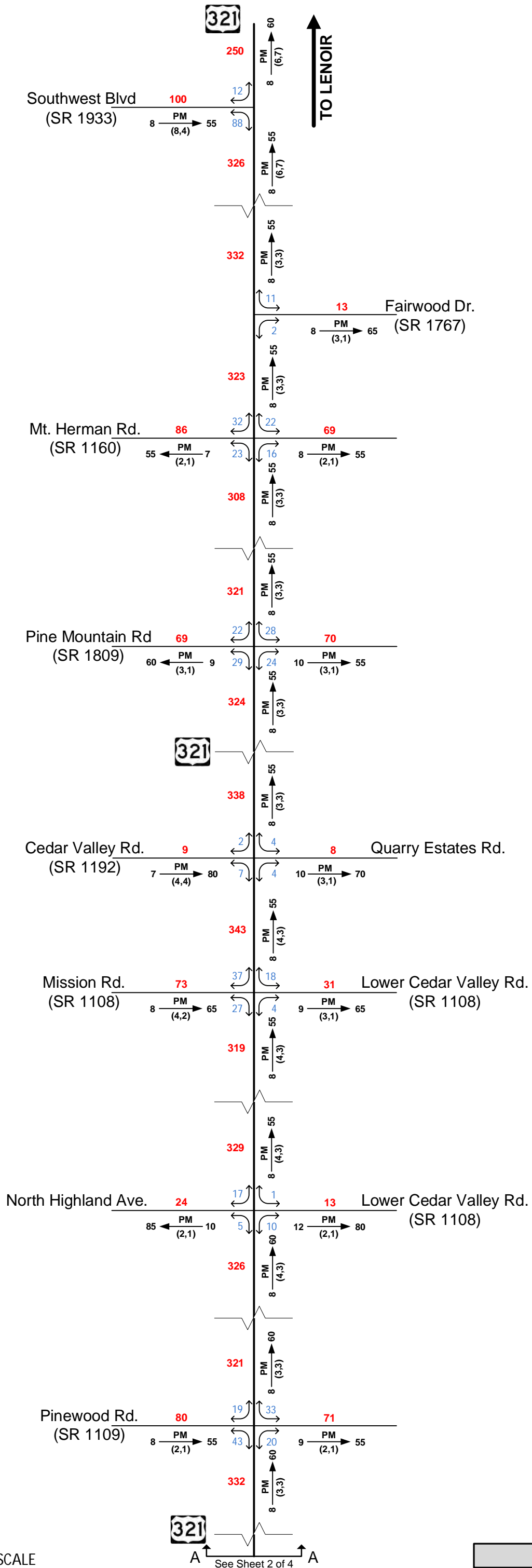
- ### No. of Vehicles Per Day (VPD) in 100s
- 1- Less than 50 VPD
- X Movement Prohibited
- Proposed Roadway

- DHV $\xrightarrow{\text{PM}}$ D
- (d, t)
- DHV Design Hourly Volume (%) = K30
- PM PM Peak Period
- D Peak Hour Directional Split (%)
- \rightarrow Indicates Direction of D
- (d, t) Duals, TT-STs (%)

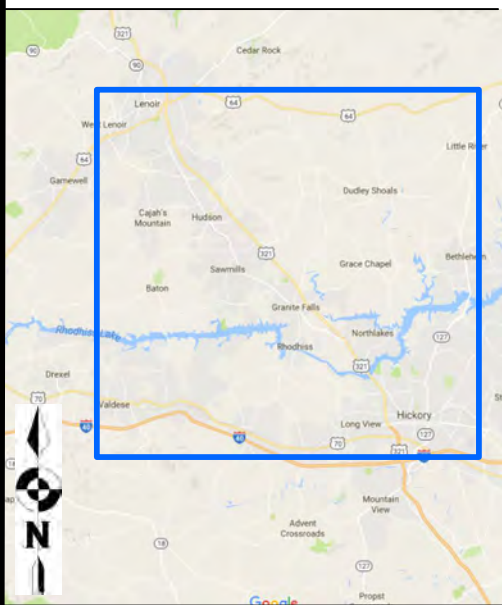
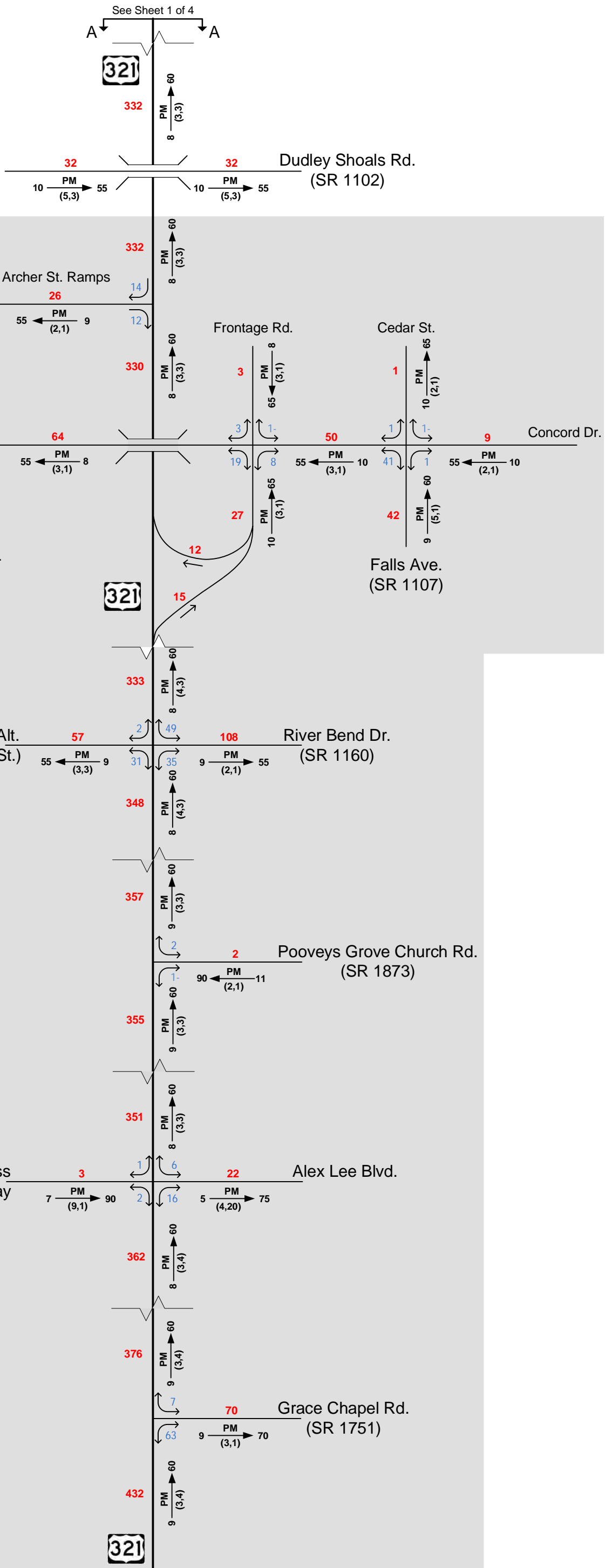
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COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	
PROJECT: US 321 Widening and Intersection Improvements	



NOT TO SCALE



Phase 1 Intersections Previously Submitted



2016

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 2 OF 4

LEGEND

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COUNTY: Caldwell, Catawba DIVISION: 11,12

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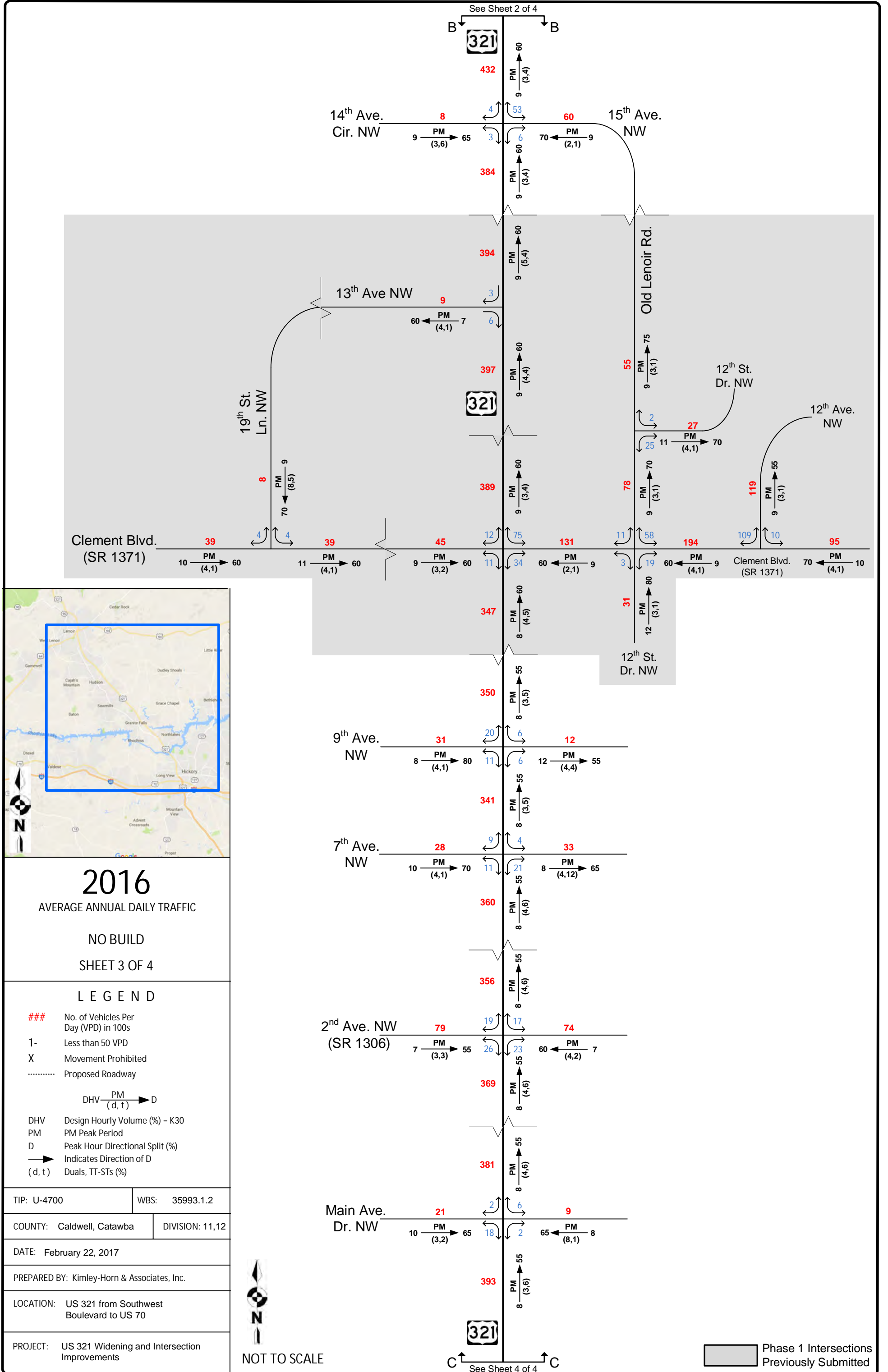
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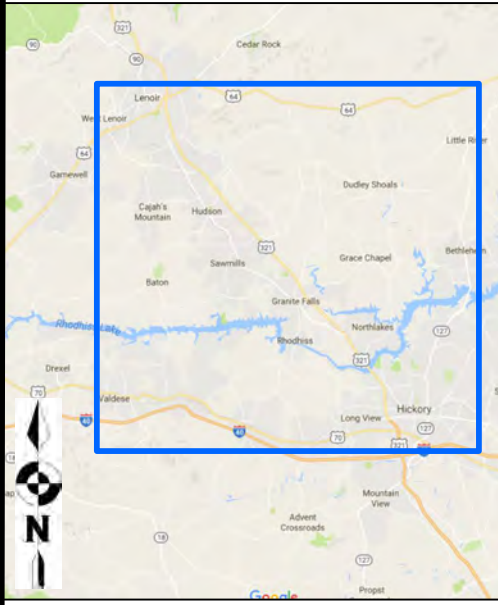
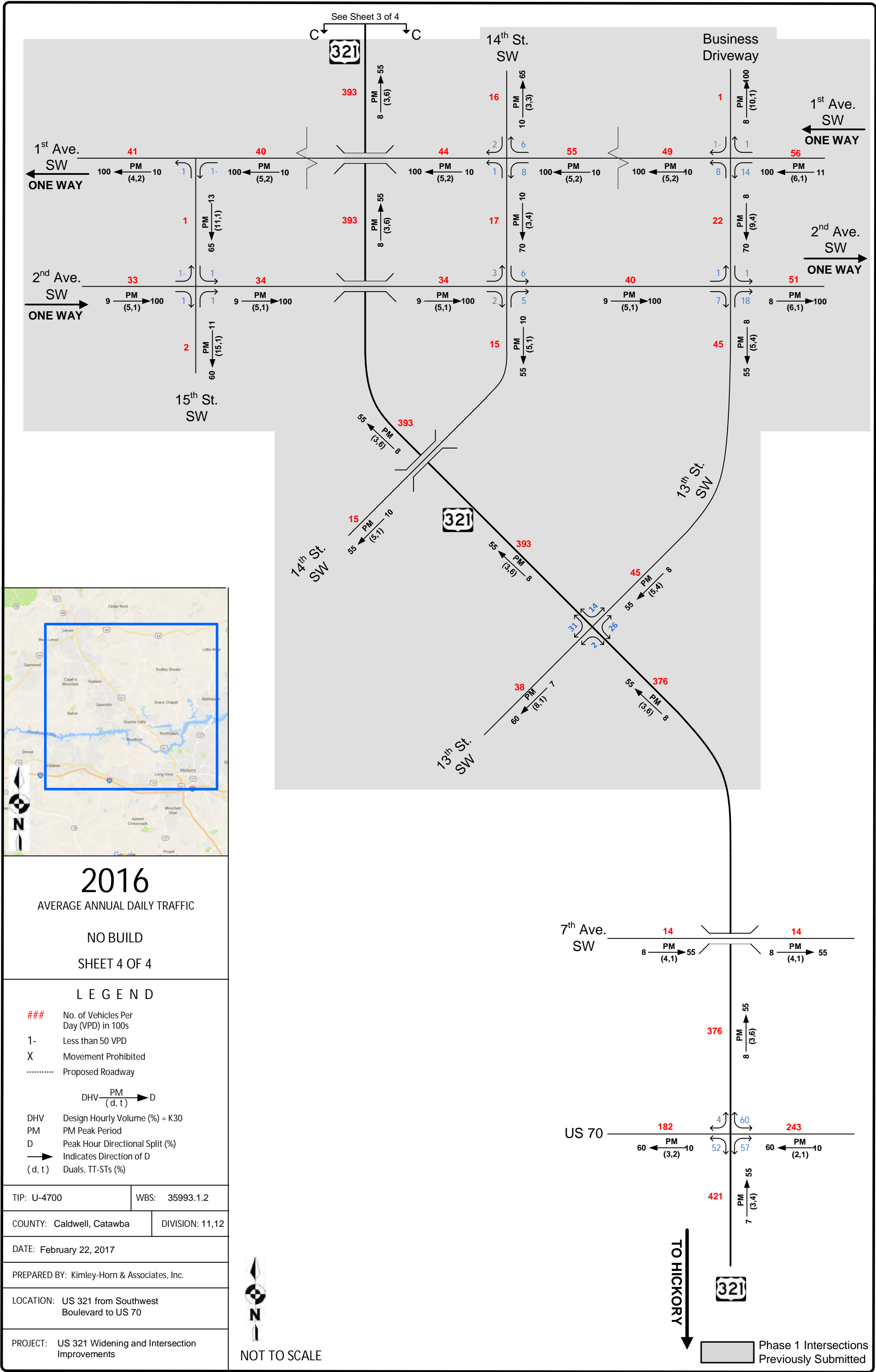
PROJECT: US 321 Widening and Intersection Improvements



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Phase 1 Intersections Previously Submitted





2016

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 4 OF 4

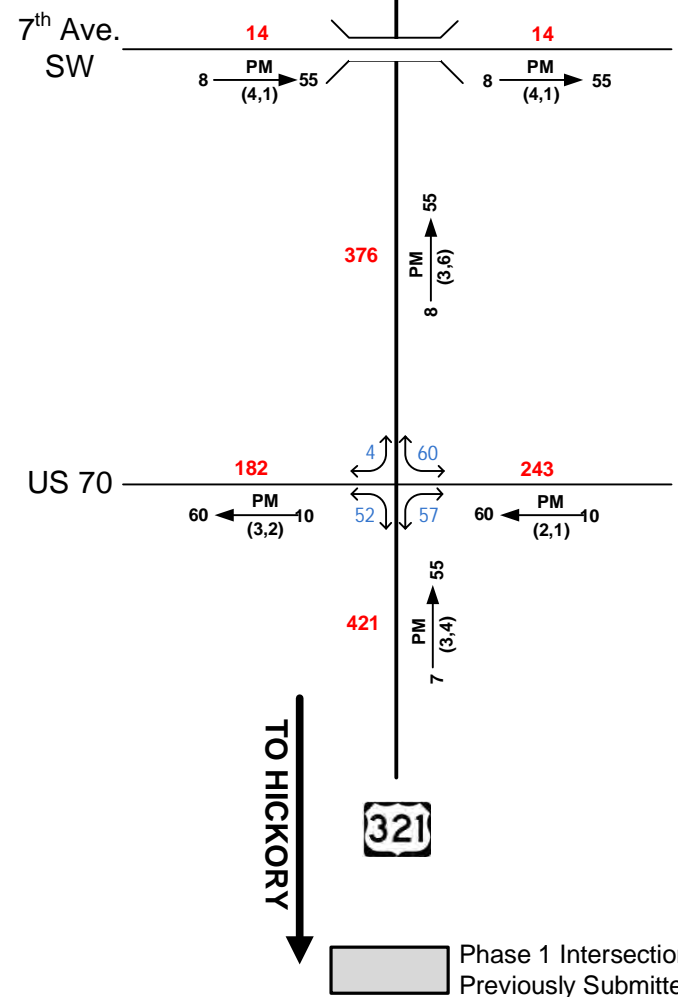
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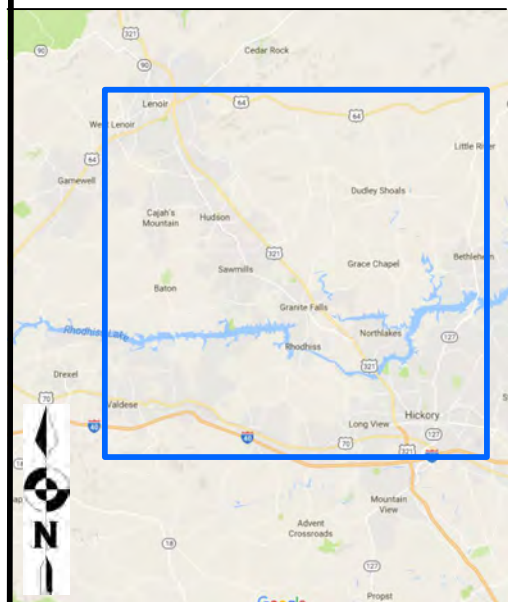


NOT TO SCALE



TO HICKORY

Phase 1 Intersections Previously Submitted



2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 1 OF 4

LEGEND

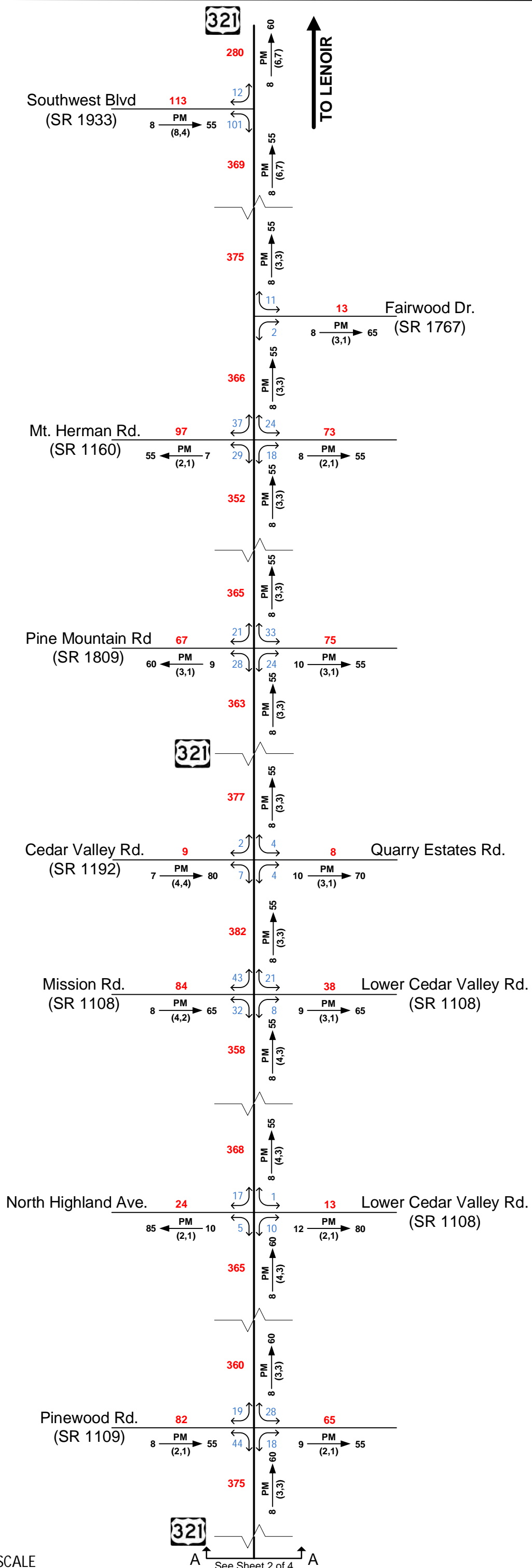
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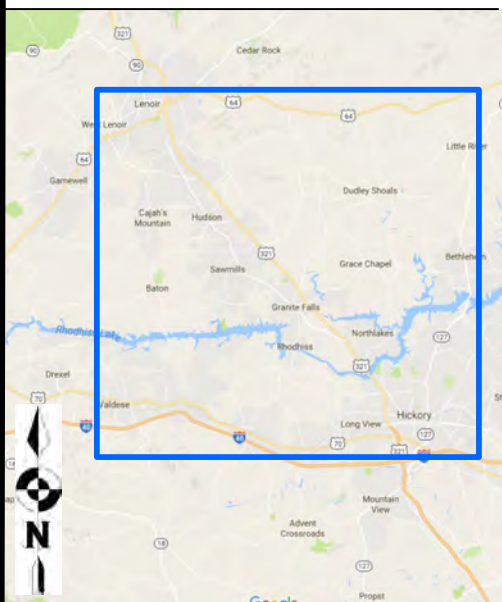
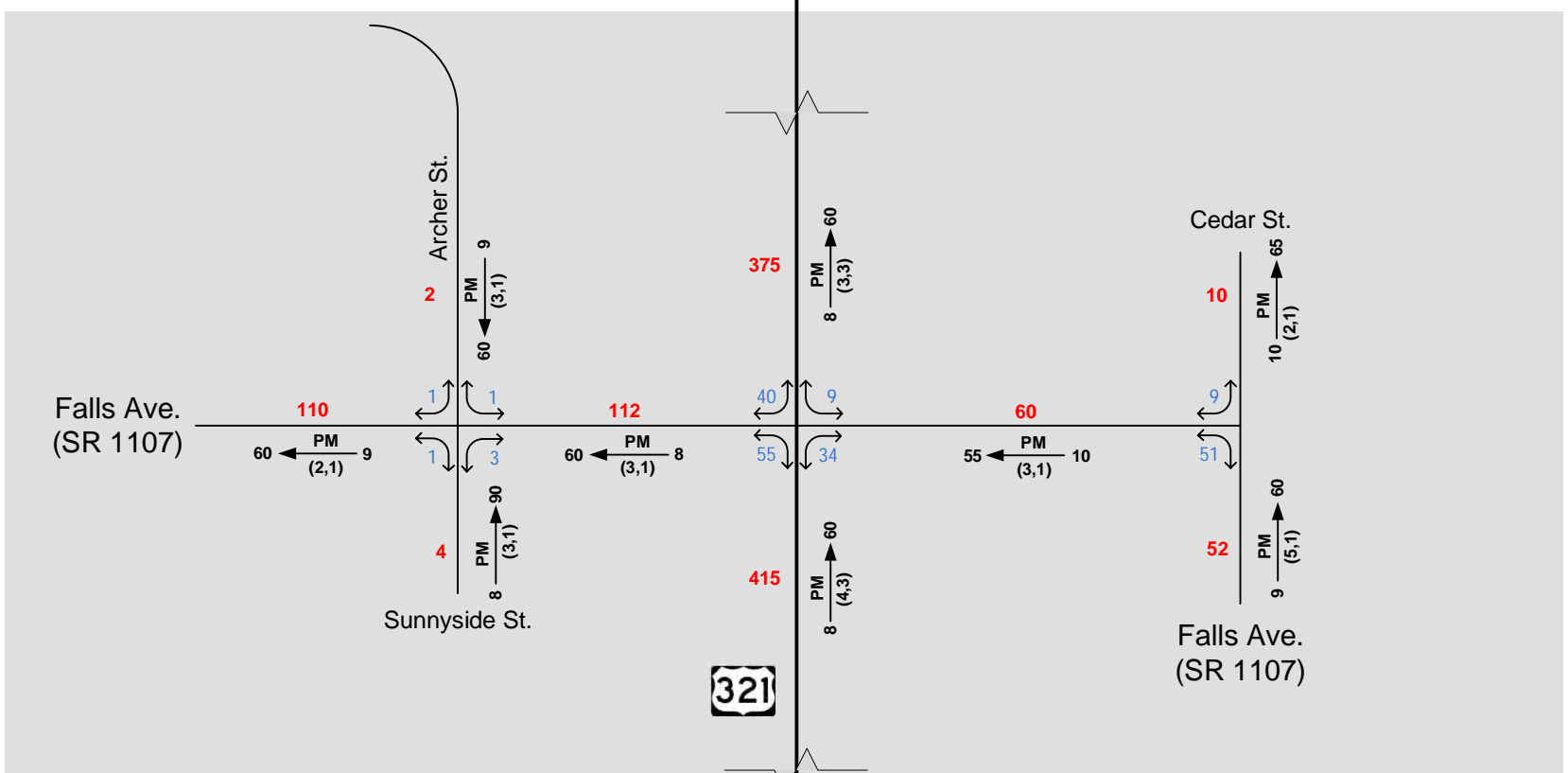
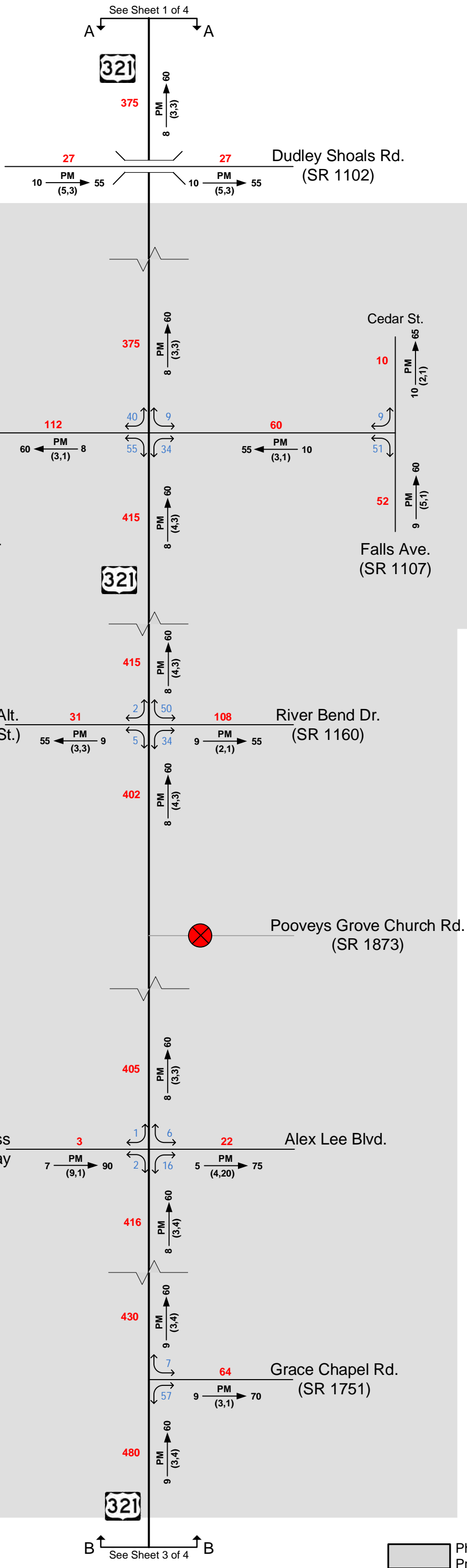
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2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 2 OF 4

LEGEND

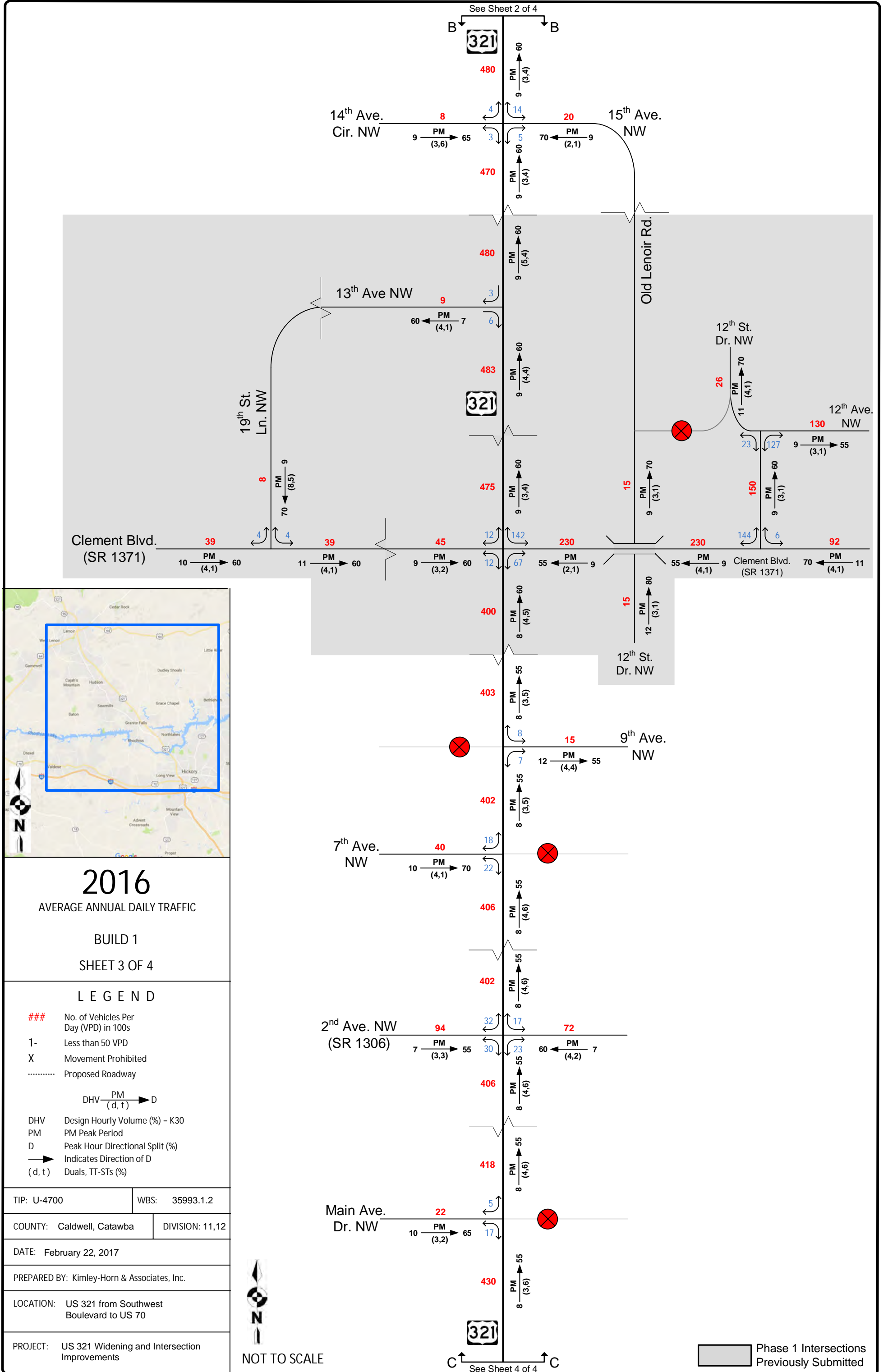
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Phase 1 Intersections Previously Submitted



See Sheet 2 of 4

B B

321

321

See Sheet 4 of 4

C C

14th Ave.
Cir. NW

15th Ave.
NW

13th Ave NW

Old Lenoir Rd.

12th St.
Dr. NW

12th Ave.
NW

Clement Blvd.
(SR 1371)

12th St.
Dr. NW

9th Ave.
NW

7th Ave.
NW

2nd Ave. NW
(SR 1306)

Main Ave.
Dr. NW

2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 3 OF 4

LEGEND

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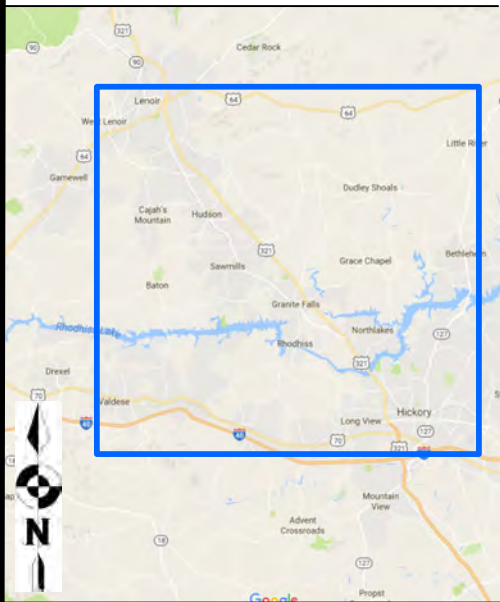
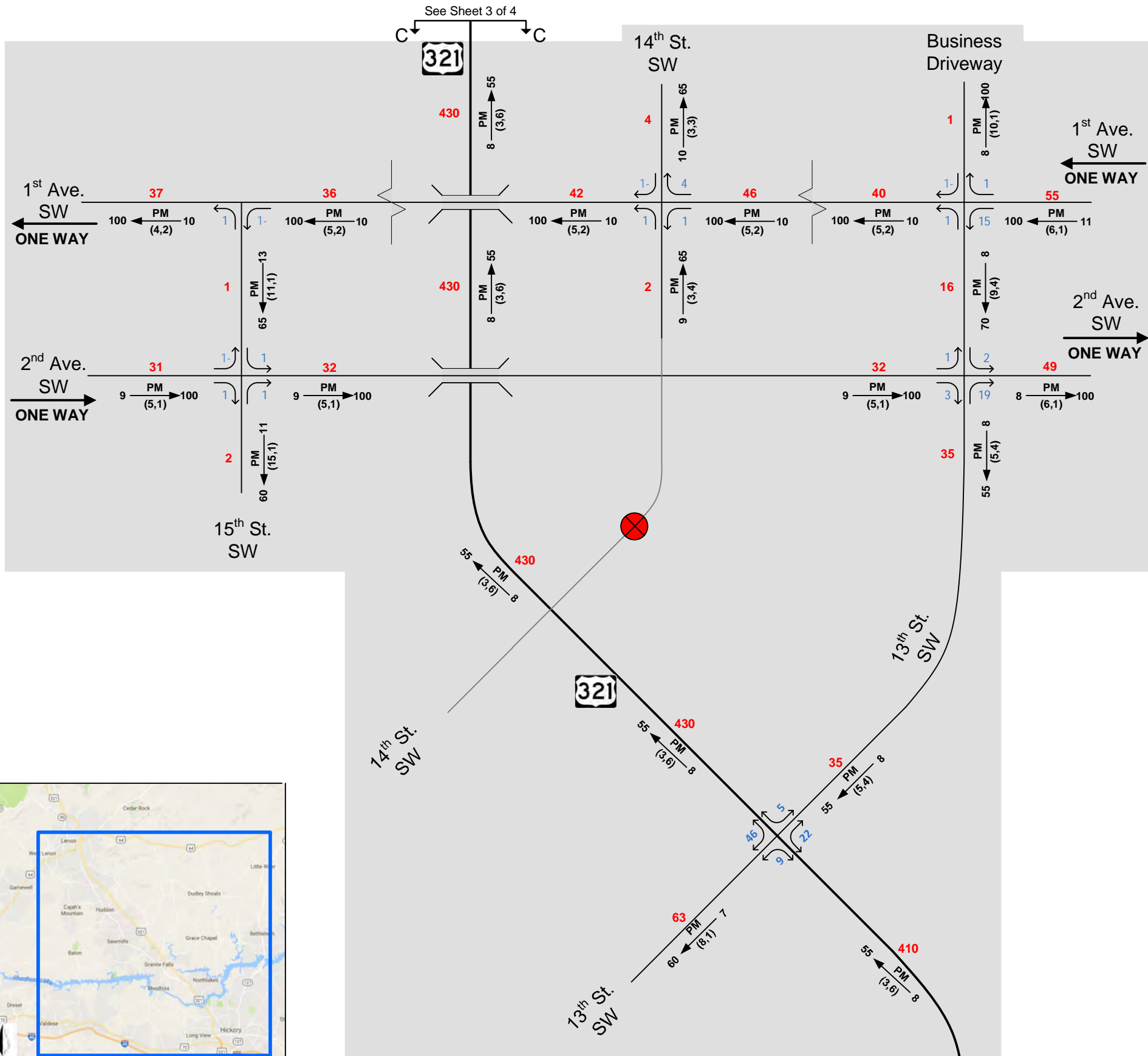
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Phase 1 Intersections
Previously Submitted



2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 4 OF 4

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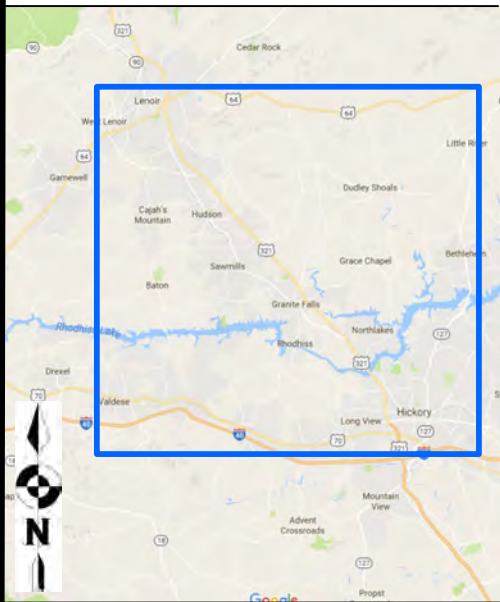
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PROJECT: US 321 Widening and Intersection Improvements



NOT TO SCALE



2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 1 OF 4

LEGEND

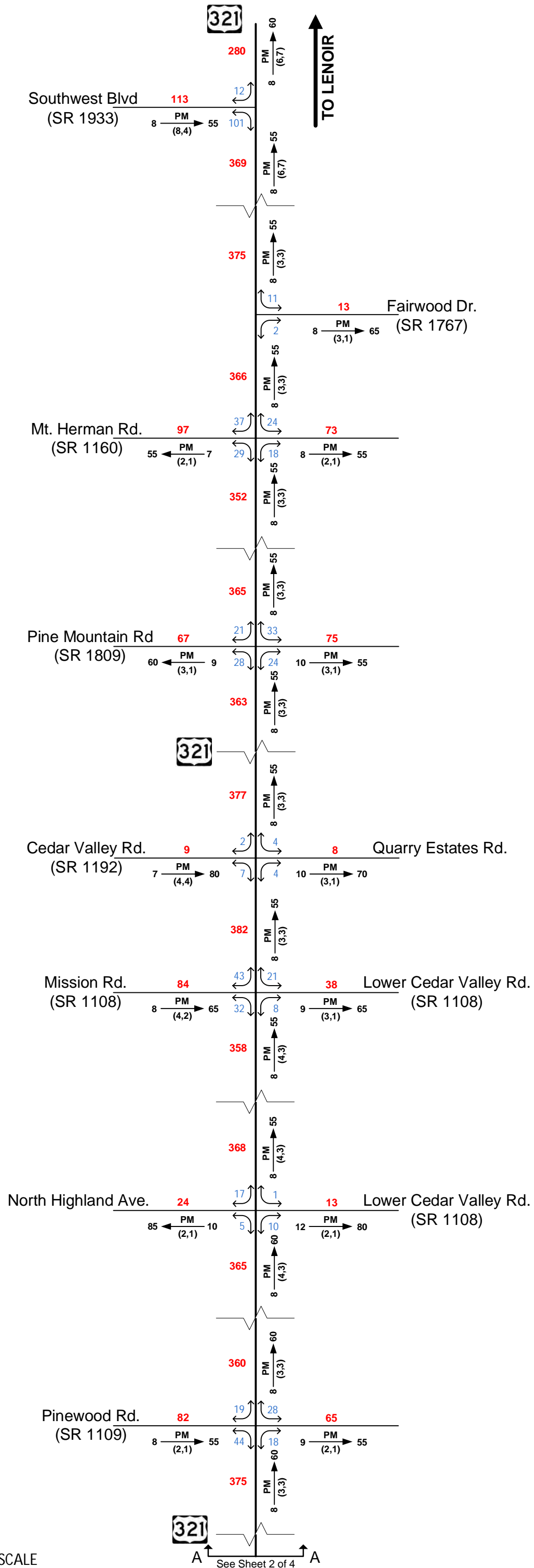
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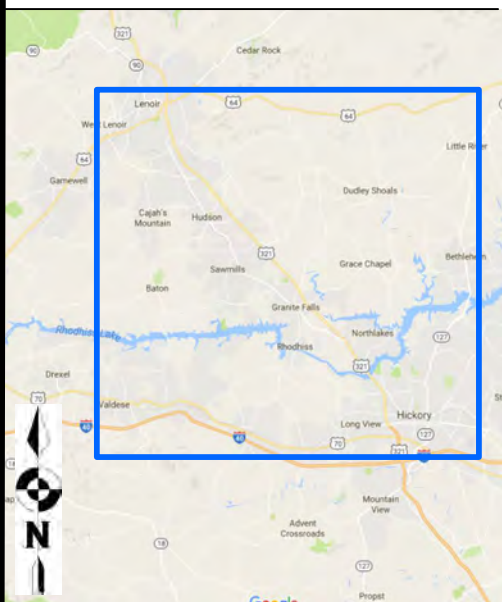
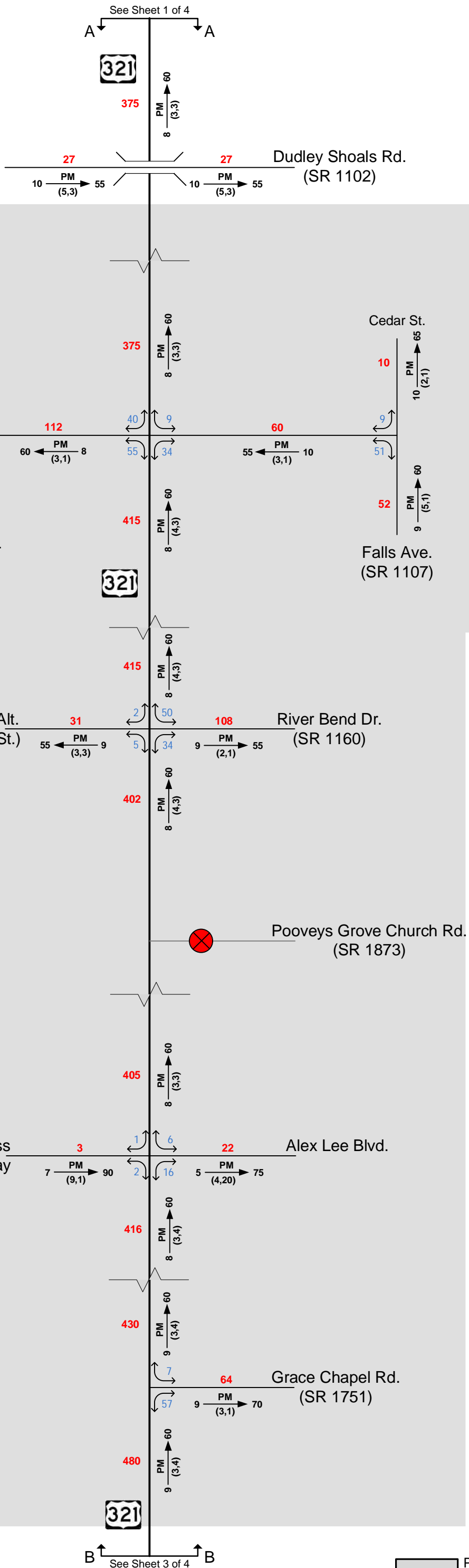
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NOT TO SCALE





2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 2 OF 4

LEGEND

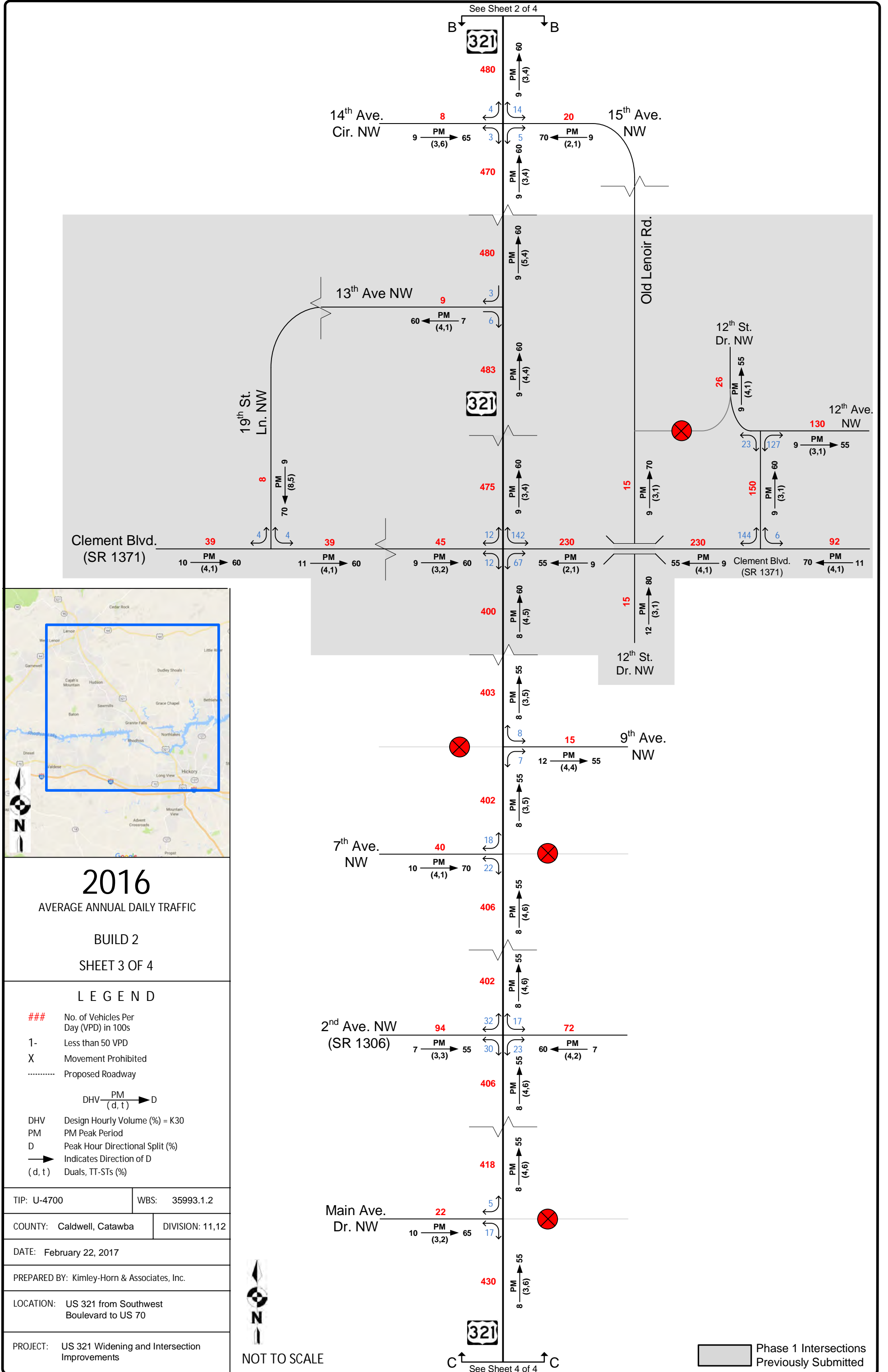
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NOT TO SCALE

Phase 1 Intersections Previously Submitted



See Sheet 2 of 4

B B

321

480

470

480

483

475

400

403

402

406

402

406

418

430

321

C C

See Sheet 4 of 4

14th Ave.
Cir. NW

15th Ave.
NW

13th Ave NW

Old Lenoir Rd.

12th St.
Dr. NW

12th Ave.
NW

Clement Blvd.
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12th St.
Dr. NW

9th Ave.
NW

7th Ave.
NW

2nd Ave. NW
(SR 1306)

Main Ave.
Dr. NW

2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 3 OF 4

LEGEND

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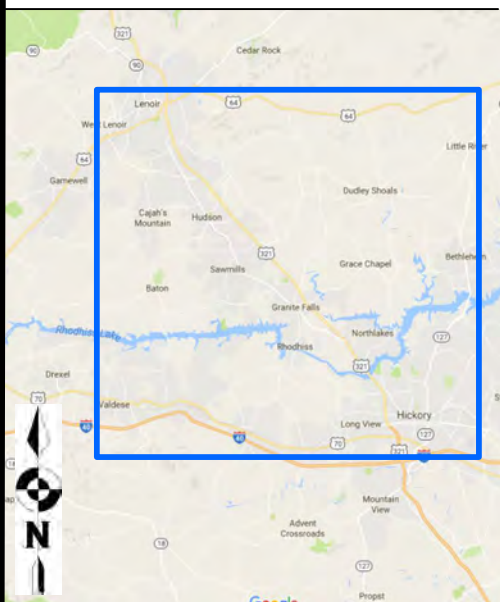
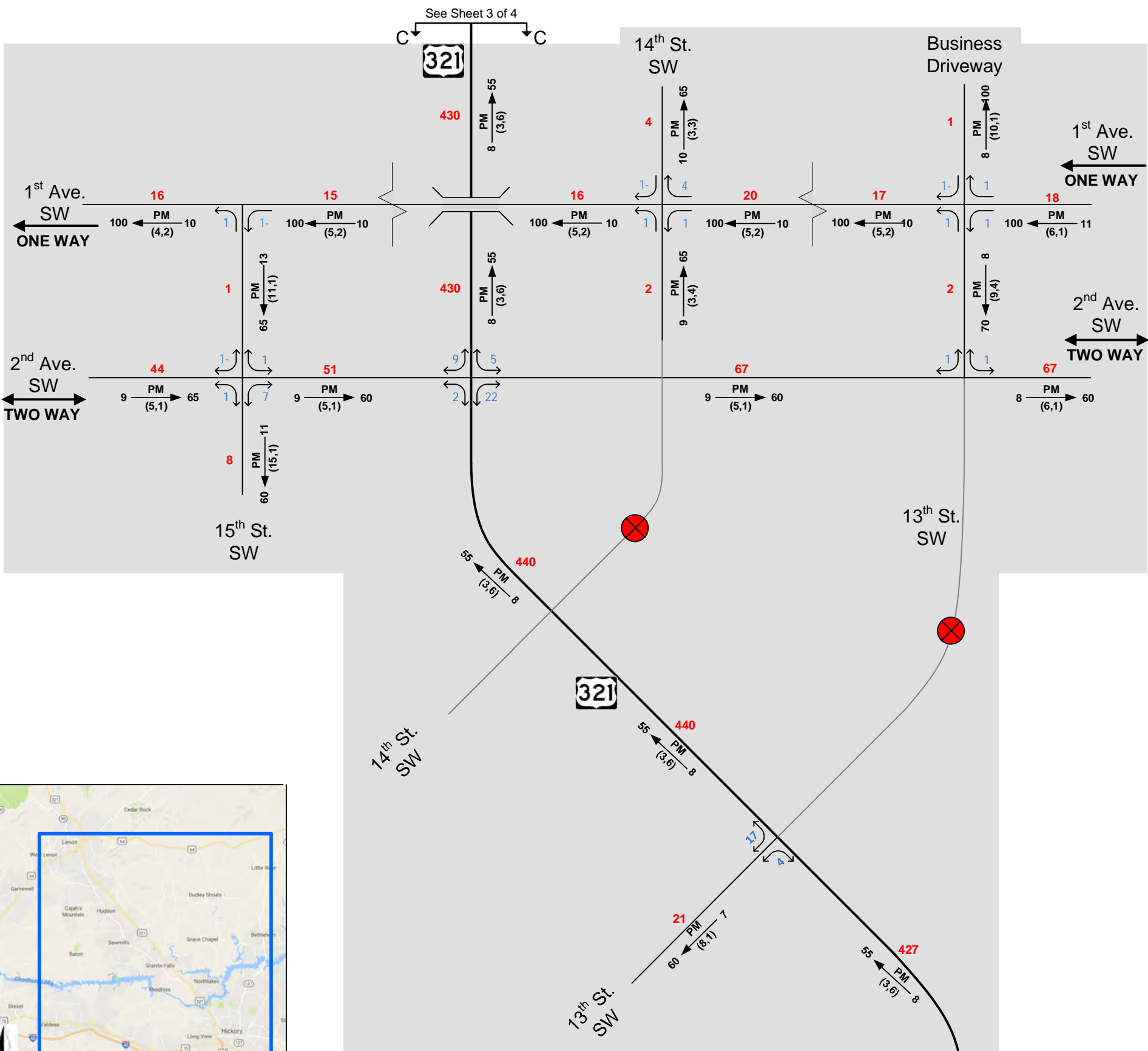
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PROJECT: US 321 Widening and Intersection Improvements



NOT TO SCALE

Phase 1 Intersections
Previously Submitted



2016

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 4 OF 4

LEGEND

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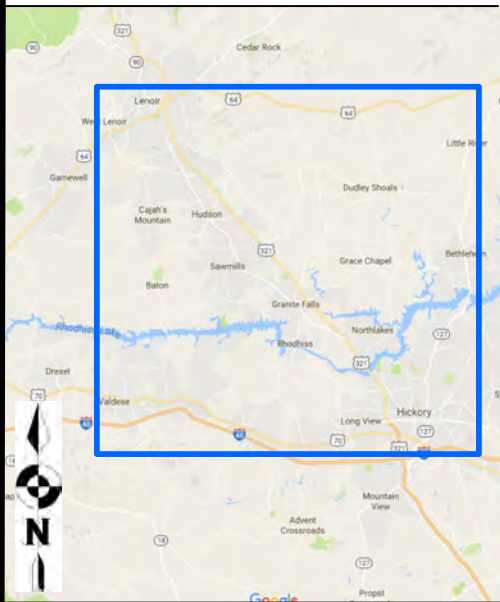
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NOT TO SCALE



2040

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 1 OF 4

LEGEND

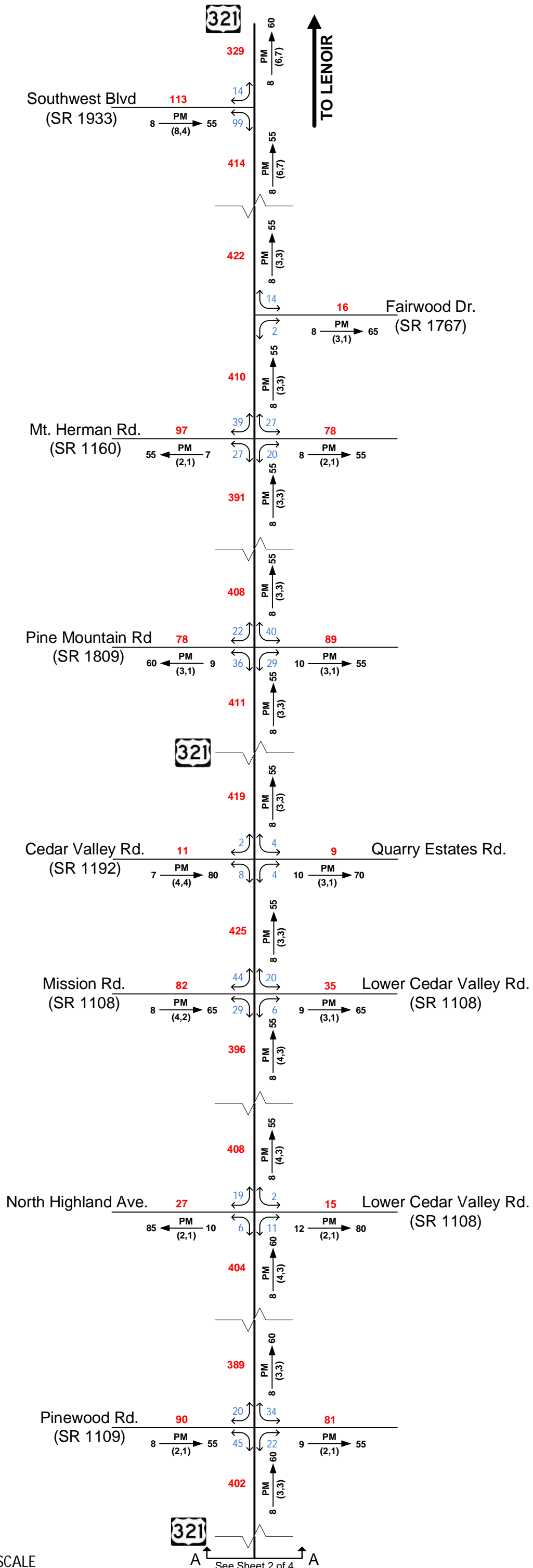
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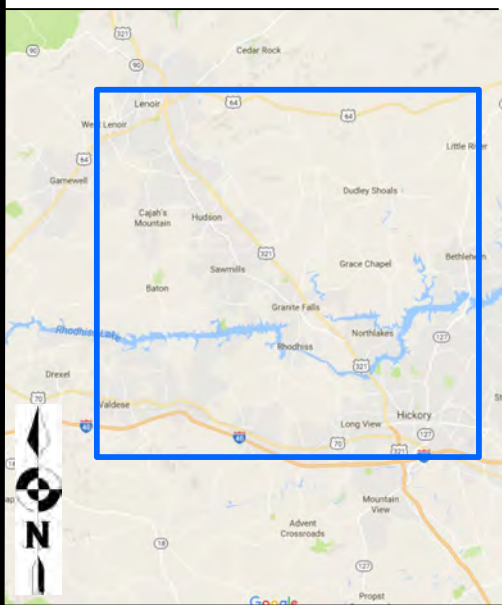
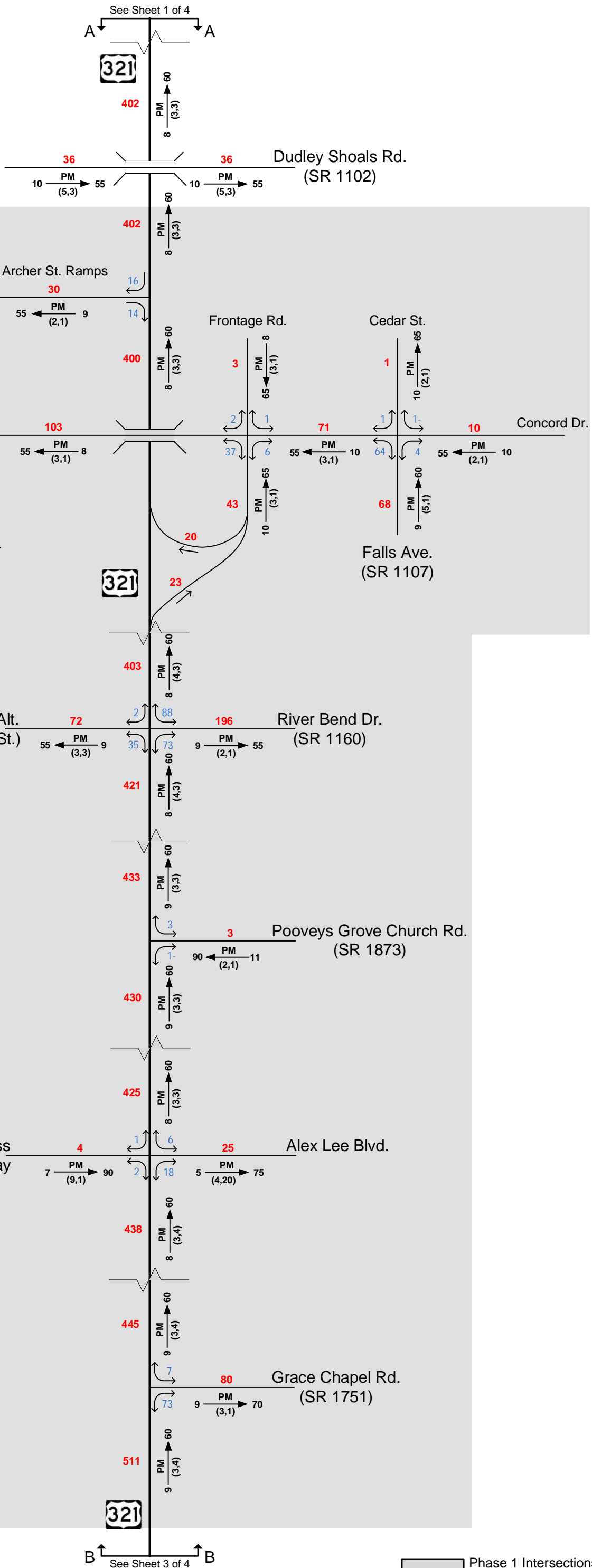
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NOT TO SCALE





2040

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 2 OF 4

LEGEND

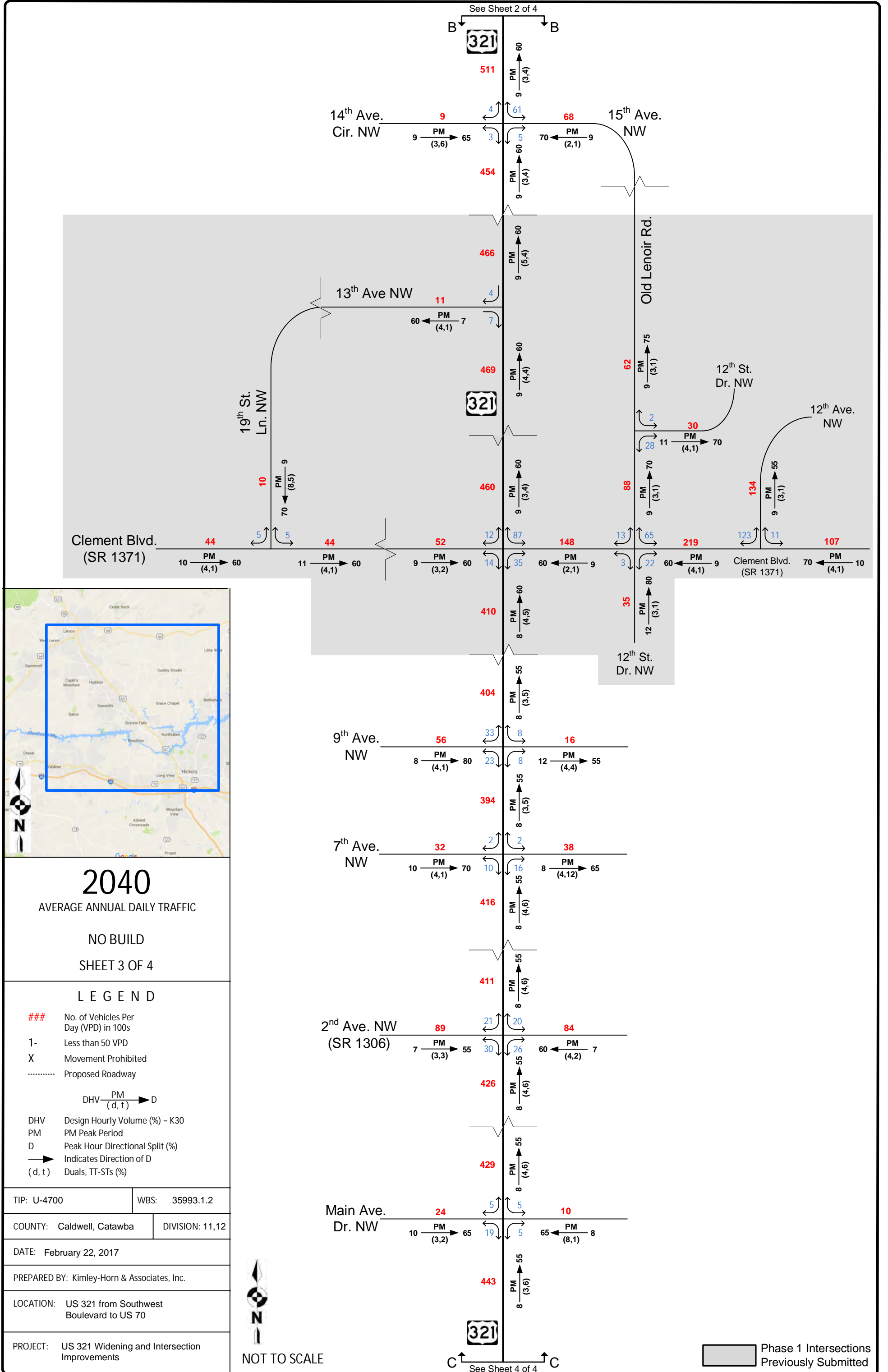
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NOT TO SCALE

Phase 1 Intersections Previously Submitted



2040

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 3 OF 4

LEGEND

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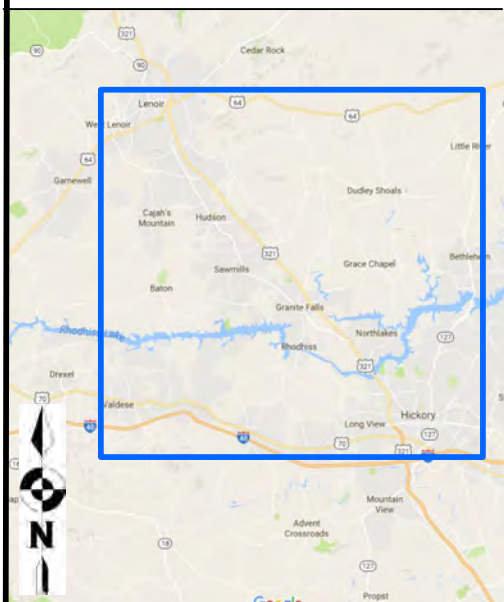
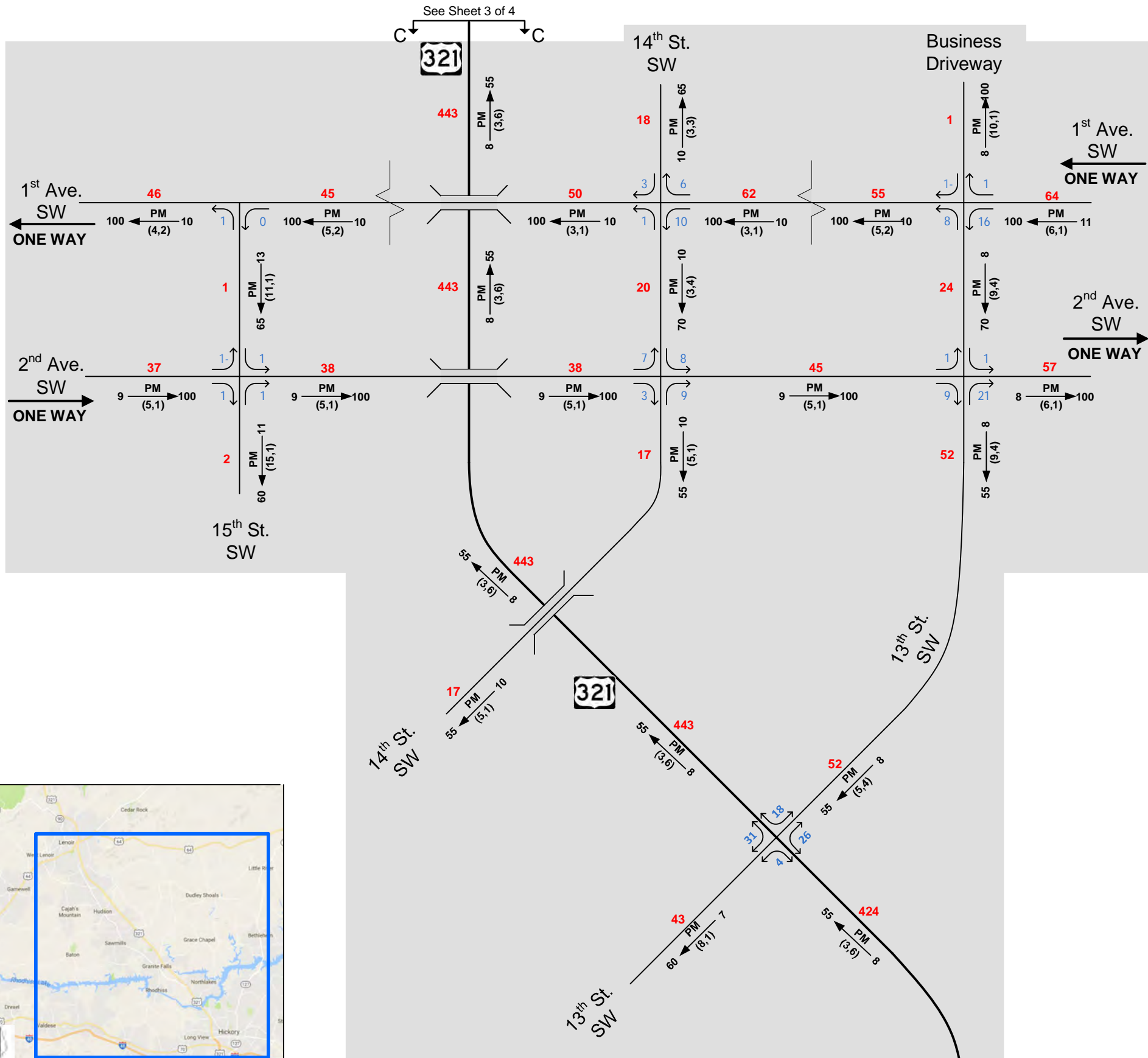
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NOT TO SCALE

Phase 1 Intersections Previously Submitted

See Sheet 3 of 4



2040

AVERAGE ANNUAL DAILY TRAFFIC

NO BUILD

SHEET 4 OF 4

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COUNTY: Caldwell, Catawba DIVISION: 11,12

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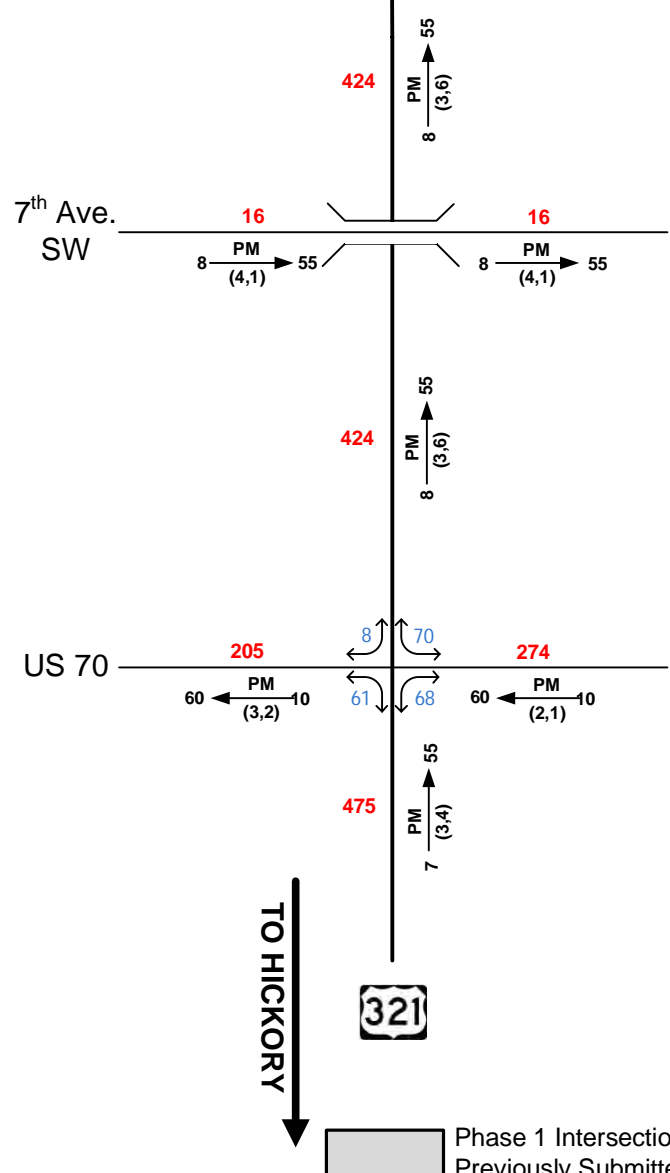
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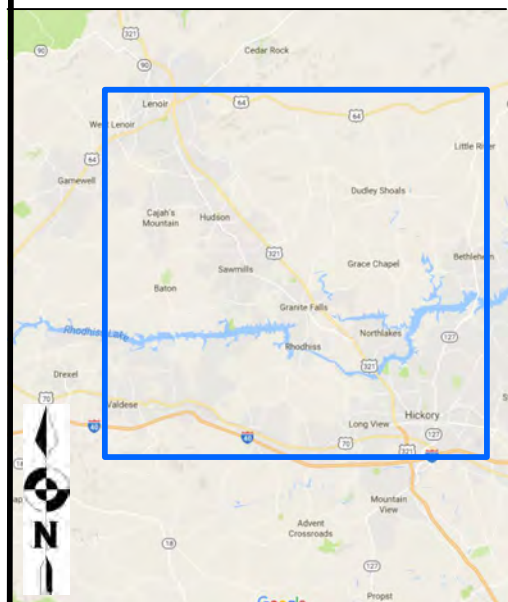


NOT TO SCALE



TO HICKORY

Phase 1 Intersections Previously Submitted



2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 1 OF 4

LEGEND

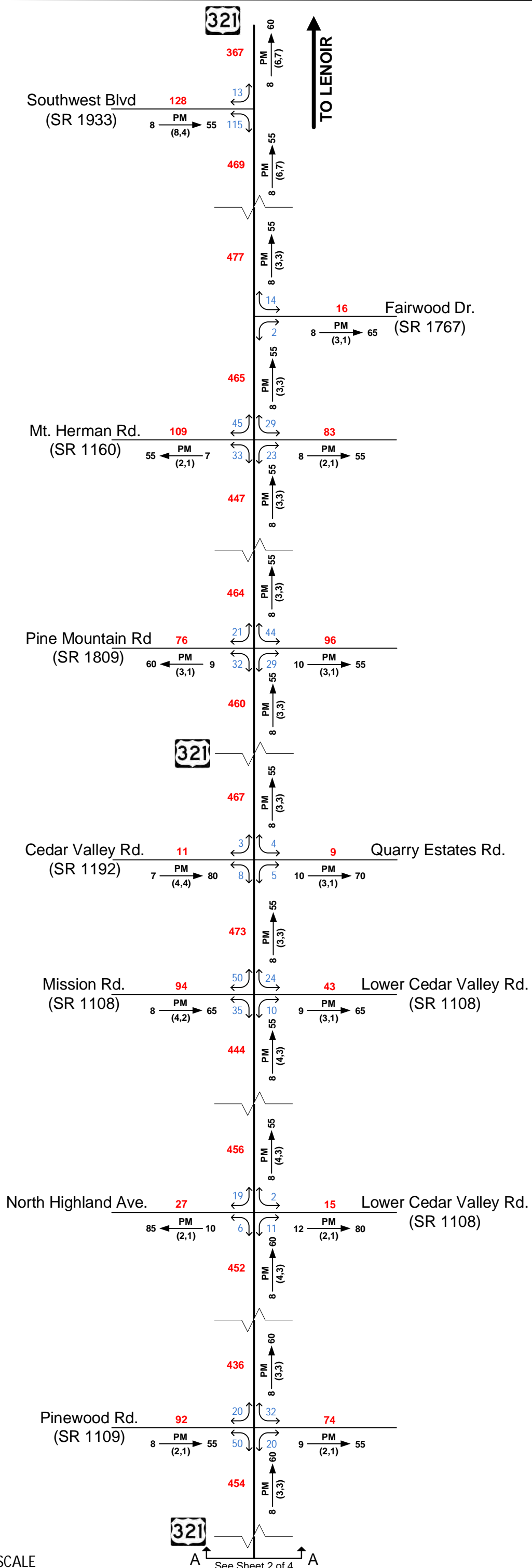
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- 1- Less than 50 VPD
- X Movement Prohibited
- Proposed Roadway

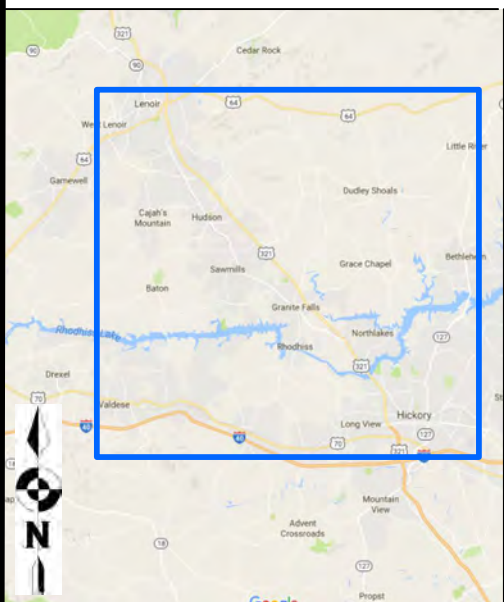
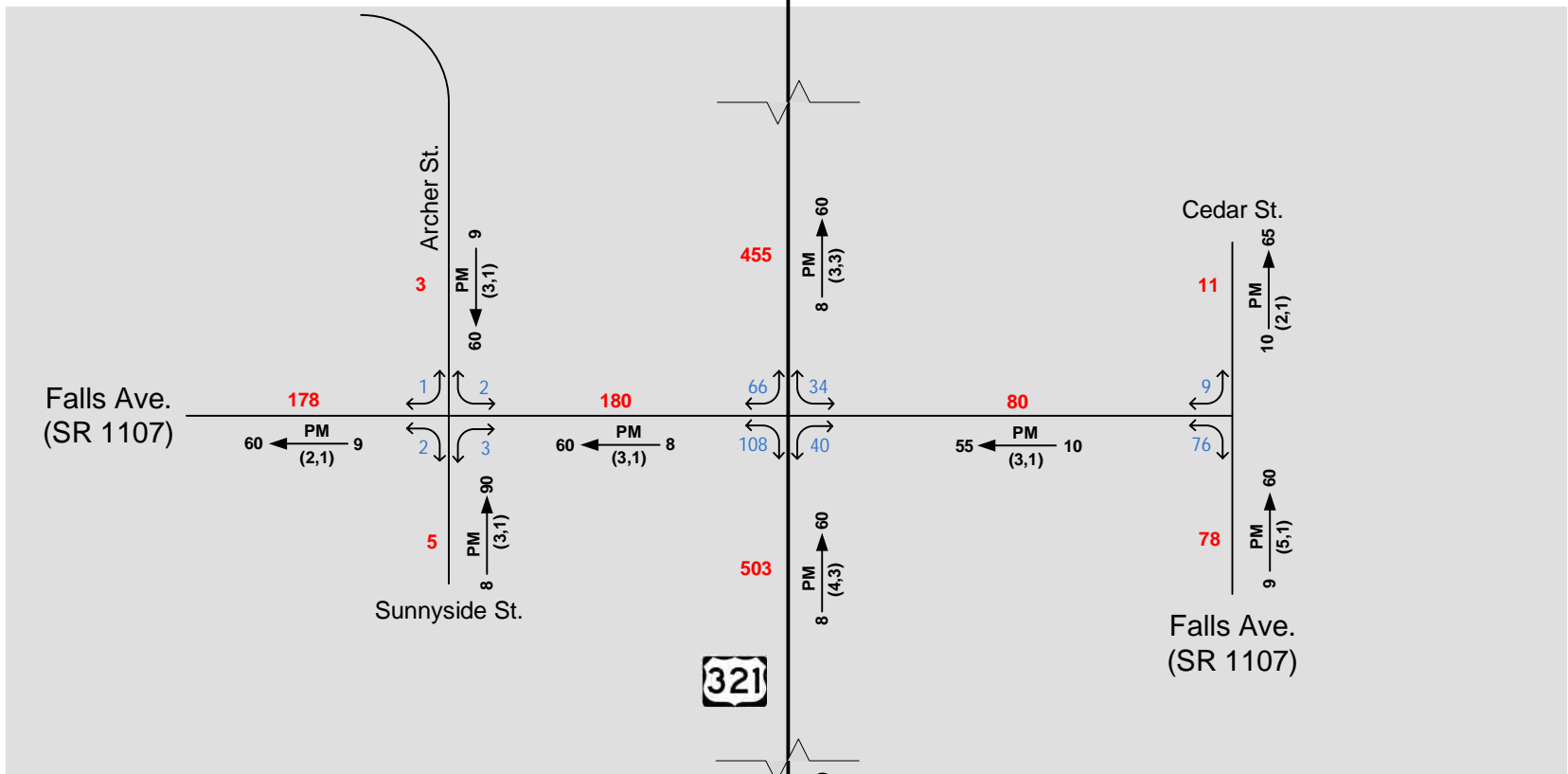
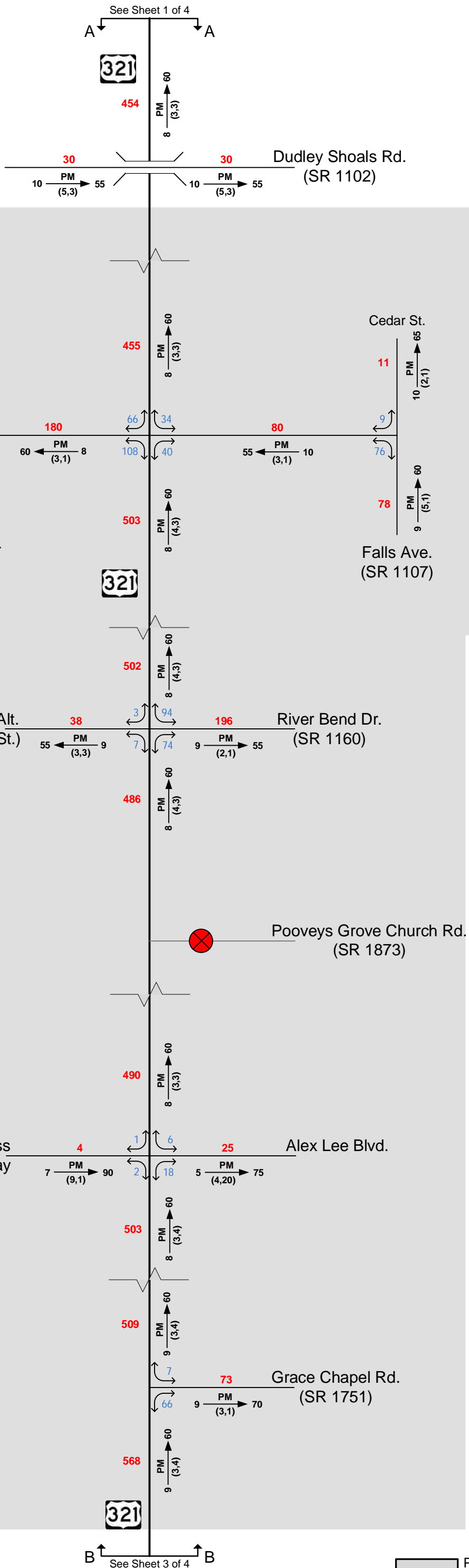
- DHV $\xrightarrow{\text{PM}} \text{D}$
(d, t)
- DHV Design Hourly Volume (%) = K30
- PM PM Peak Period
- D Peak Hour Directional Split (%)
- \rightarrow Indicates Direction of D
- (d, t) Duals, TT-STs (%)

TIP: U-4700	WBS: 35993.1.2
COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	
PROJECT: US 321 Widening and Intersection Improvements	



NOT TO SCALE





2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 2 OF 4

LEGEND

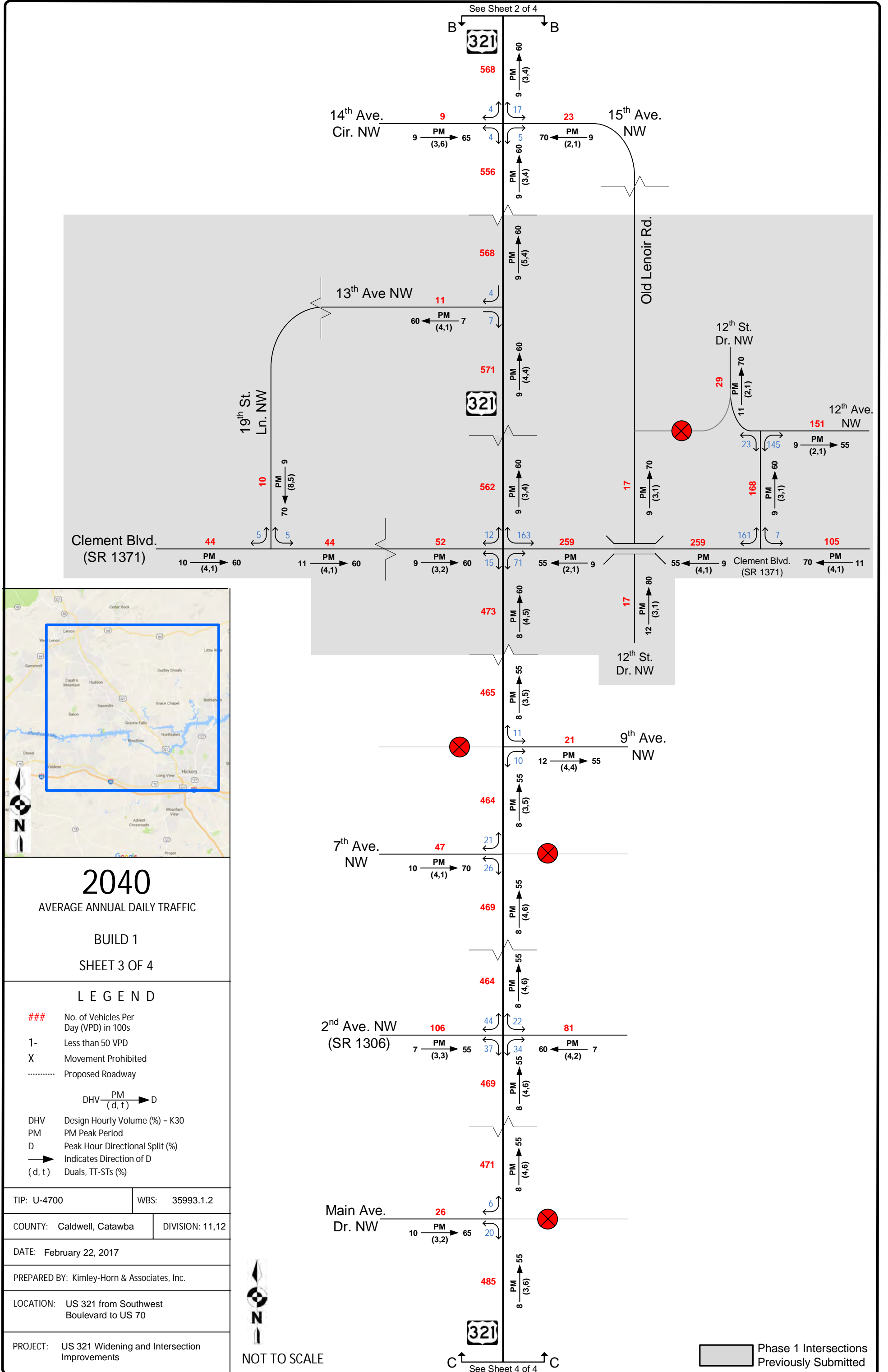
- ### No. of Vehicles Per Day (VPD) in 100s
- 1- Less than 50 VPD
- X Movement Prohibited
- Proposed Roadway
- DHV $\xrightarrow{\text{PM}} \text{D}$
DHV Design Hourly Volume (%) = K30
 PM PM Peak Period
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 (d, t) Duals, TT-STs (%)

TIP: U-4700	WBS: 35993.1.2
COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	



NOT TO SCALE

Phase 1 Intersections Previously Submitted



See Sheet 2 of 4

B B

321

568

556

568

571

562

473

465

464

469

464

469

471

485

321

C C

See Sheet 4 of 4

14th Ave.
Cir. NW

15th Ave.
NW

13th Ave NW

12th St.
Dr. NW

19th St.
Ln. NW

12th Ave.
NW

Clement Blvd.
(SR 1371)

Clement Blvd.
(SR 1371)

12th St.
Dr. NW

9th Ave.
NW

7th Ave.
NW

2nd Ave. NW
(SR 1306)

Main Ave.
Dr. NW

2040
AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1
SHEET 3 OF 4

LEGEND

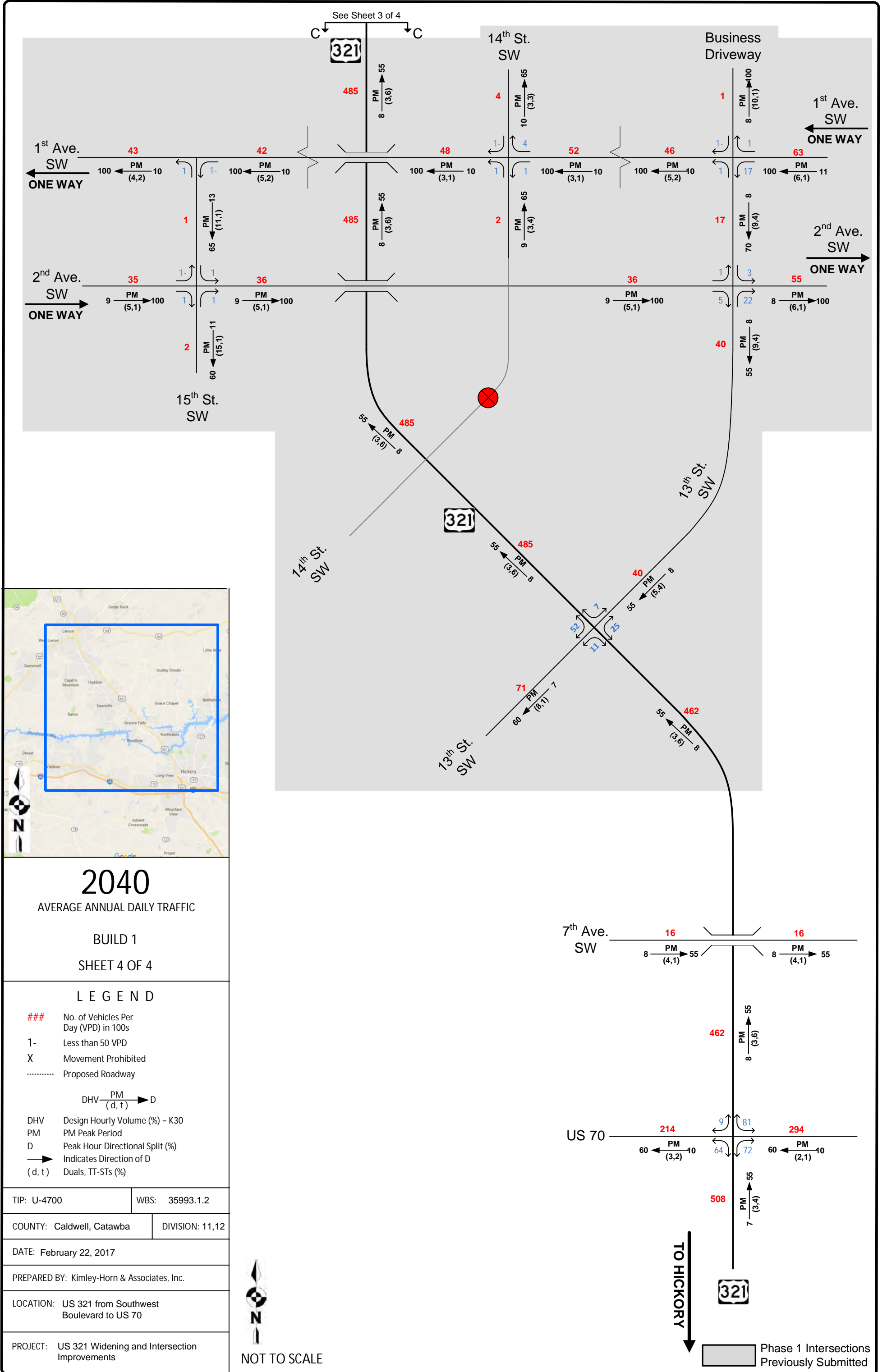
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- 1- Less than 50 VPD
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- Proposed Roadway
- DHV $\frac{PM}{(d, t)} \rightarrow D$
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TIP: U-4700	WBS: 35993.1.2
COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	
PROJECT: US 321 Widening and Intersection Improvements	



NOT TO SCALE

Phase 1 Intersections
Previously Submitted



2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 1

SHEET 4 OF 4

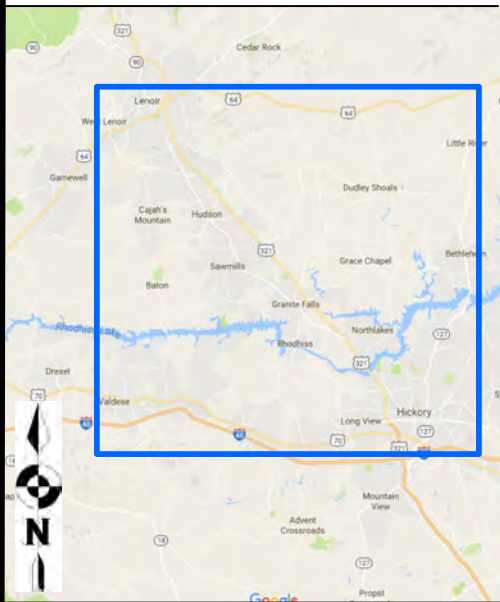
LEGEND

- ### No. of Vehicles Per Day (VPD) in 100s
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COUNTY: Caldwell, Catawba	DIVISION: 11,12
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PROJECT: US 321 Widening and Intersection Improvements	



NOT TO SCALE



2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 1 OF 4

LEGEND

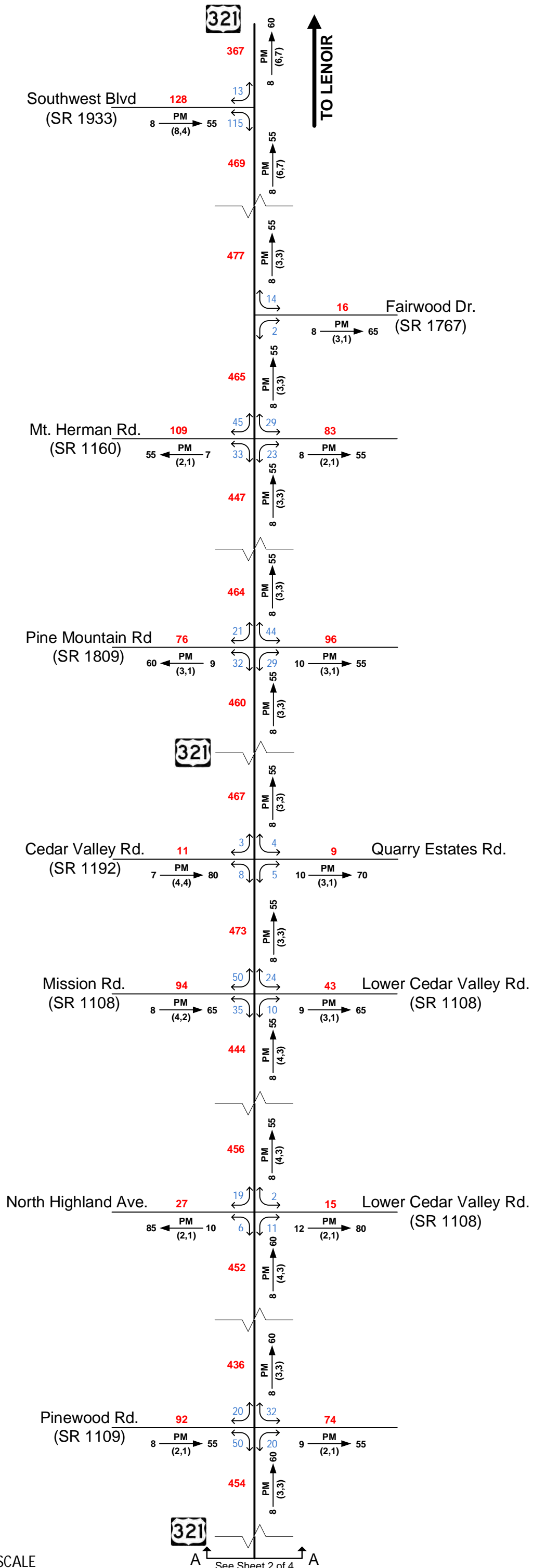
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- X Movement Prohibited
- Proposed Roadway

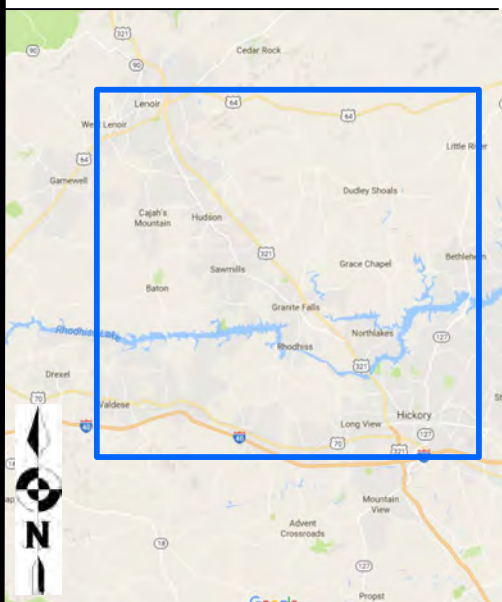
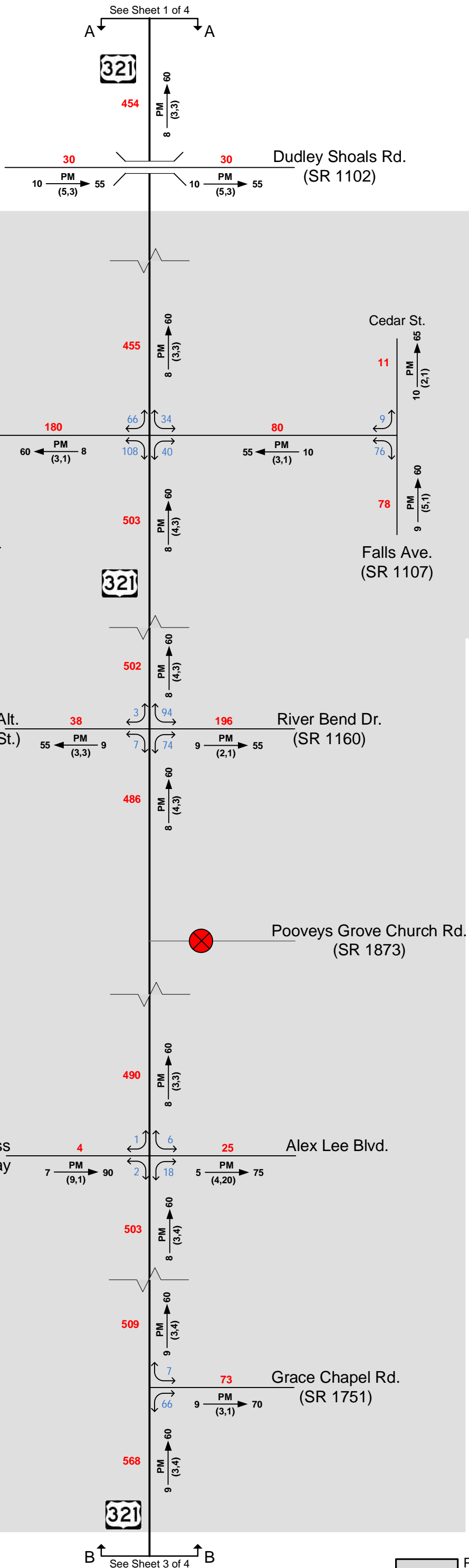
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COUNTY: Caldwell, Catawba	DIVISION: 11,12
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NOT TO SCALE





2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 2 OF 4

LEGEND

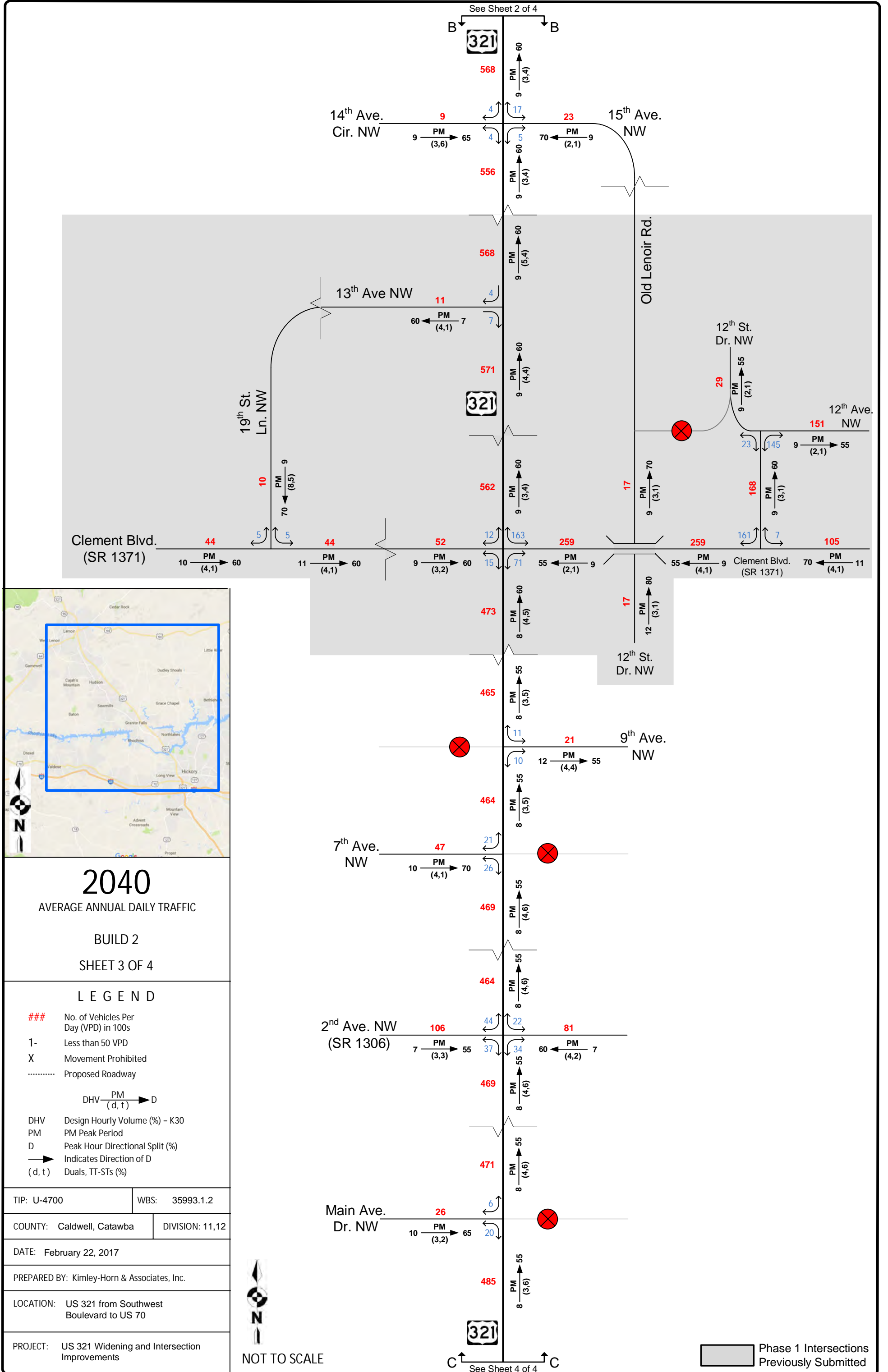
- ### No. of Vehicles Per Day (VPD) in 100s
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TIP: U-4700	WBS: 35993.1.2
COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	
PROJECT: US 321 Widening and Intersection Improvements	



NOT TO SCALE

Phase 1 Intersections Previously Submitted



See Sheet 2 of 4

B B

321

568

14th Ave.
Cir. NW

15th Ave.
NW

13th Ave NW

Old Lenoir Rd.

12th St.
Dr. NW

19th St.
Ln. NW

12th Ave.
NW

Clement Blvd.
(SR 1371)

12th St.
Dr. NW

9th Ave.
NW

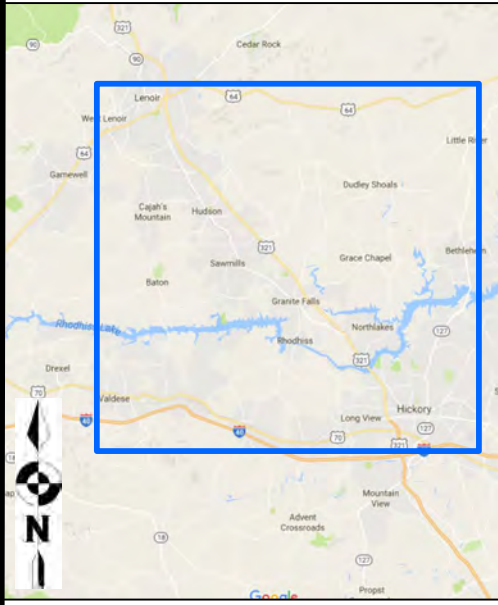
7th Ave.
NW

2nd Ave. NW
(SR 1306)

Main Ave.
Dr. NW

321

See Sheet 4 of 4



2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 3 OF 4

LEGEND

- ### No. of Vehicles Per Day (VPD) in 100s
- 1- Less than 50 VPD
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- DHV $\frac{PM}{(d, t)} \rightarrow D$
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TIP: U-4700 WBS: 35993.1.2

COUNTY: Caldwell, Catawba DIVISION: 11,12

DATE: February 22, 2017

PREPARED BY: Kimley-Horn & Associates, Inc.

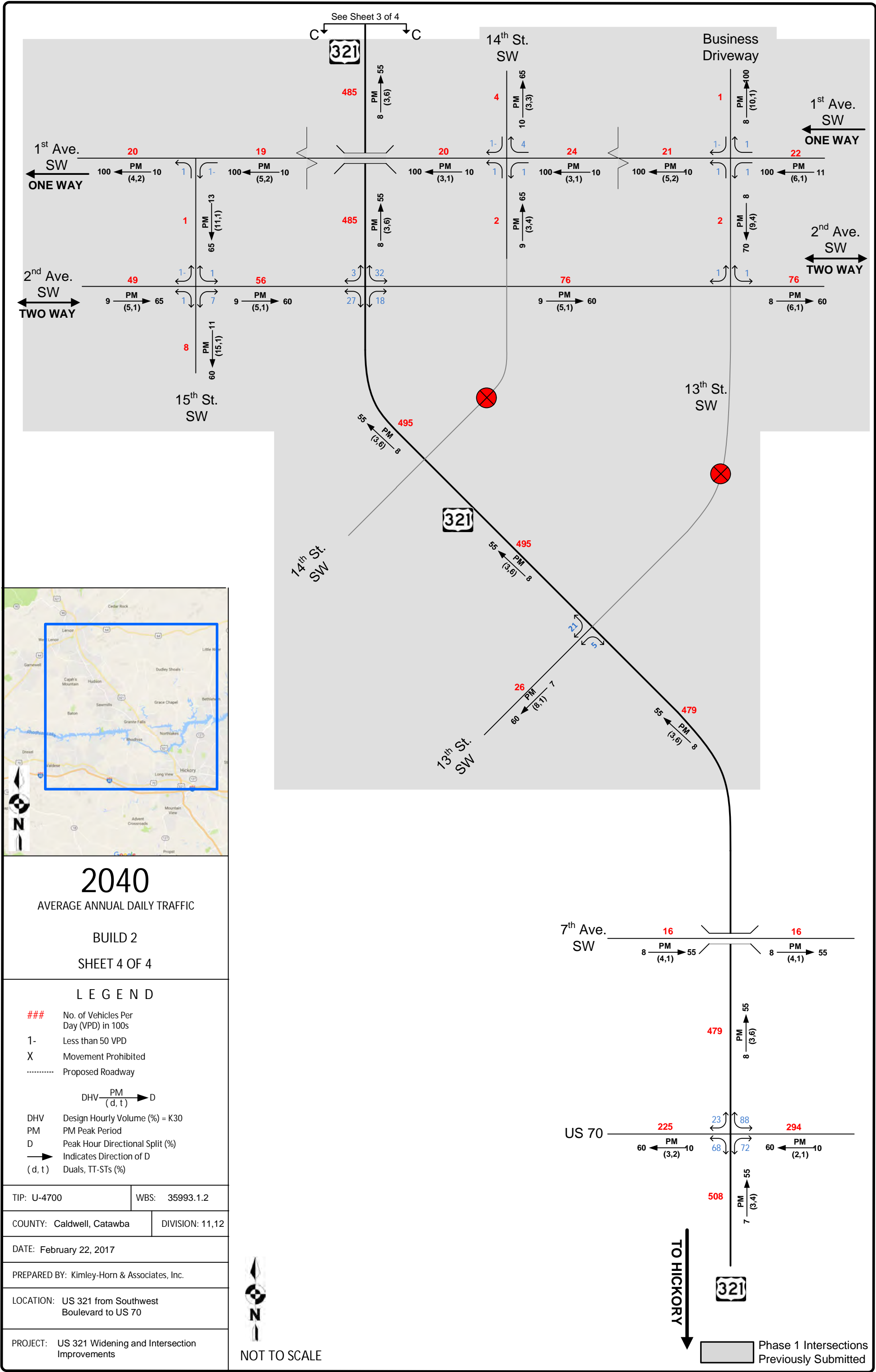
LOCATION: US 321 from Southwest Boulevard to US 70

PROJECT: US 321 Widening and Intersection Improvements



NOT TO SCALE

Phase 1 Intersections Previously Submitted



2040

AVERAGE ANNUAL DAILY TRAFFIC

BUILD 2

SHEET 4 OF 4

LEGEND

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TIP: U-4700	WBS: 35993.1.2
COUNTY: Caldwell, Catawba	DIVISION: 11,12
DATE: February 22, 2017	
PREPARED BY: Kimley-Horn & Associates, Inc.	
LOCATION: US 321 from Southwest Boulevard to US 70	



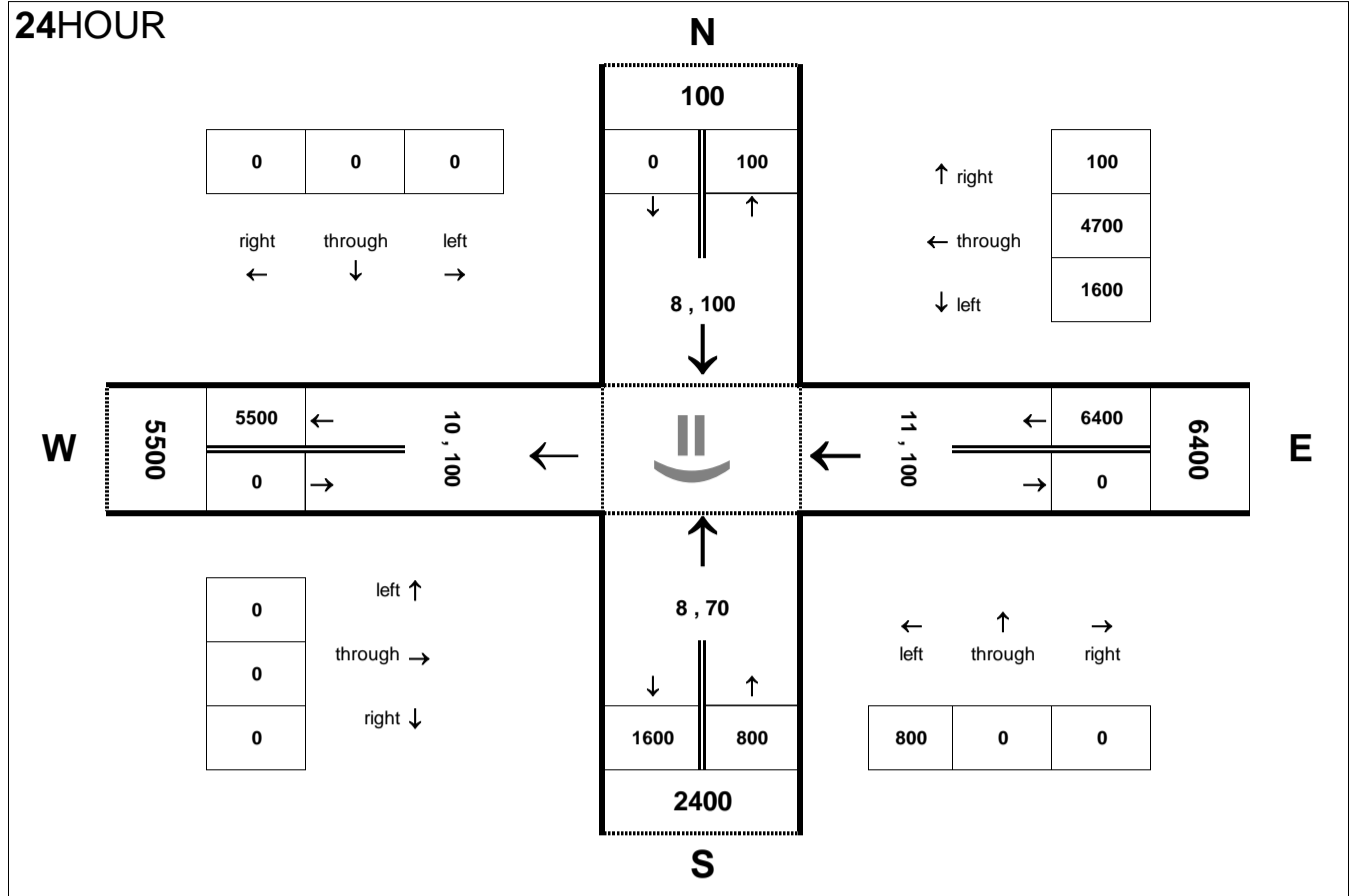
NOT TO SCALE

TO HICKORY

Phase 1 Intersections Previously Submitted

APPENDIX B
Peak Hour Traffic Volume Breakouts

24HOUR



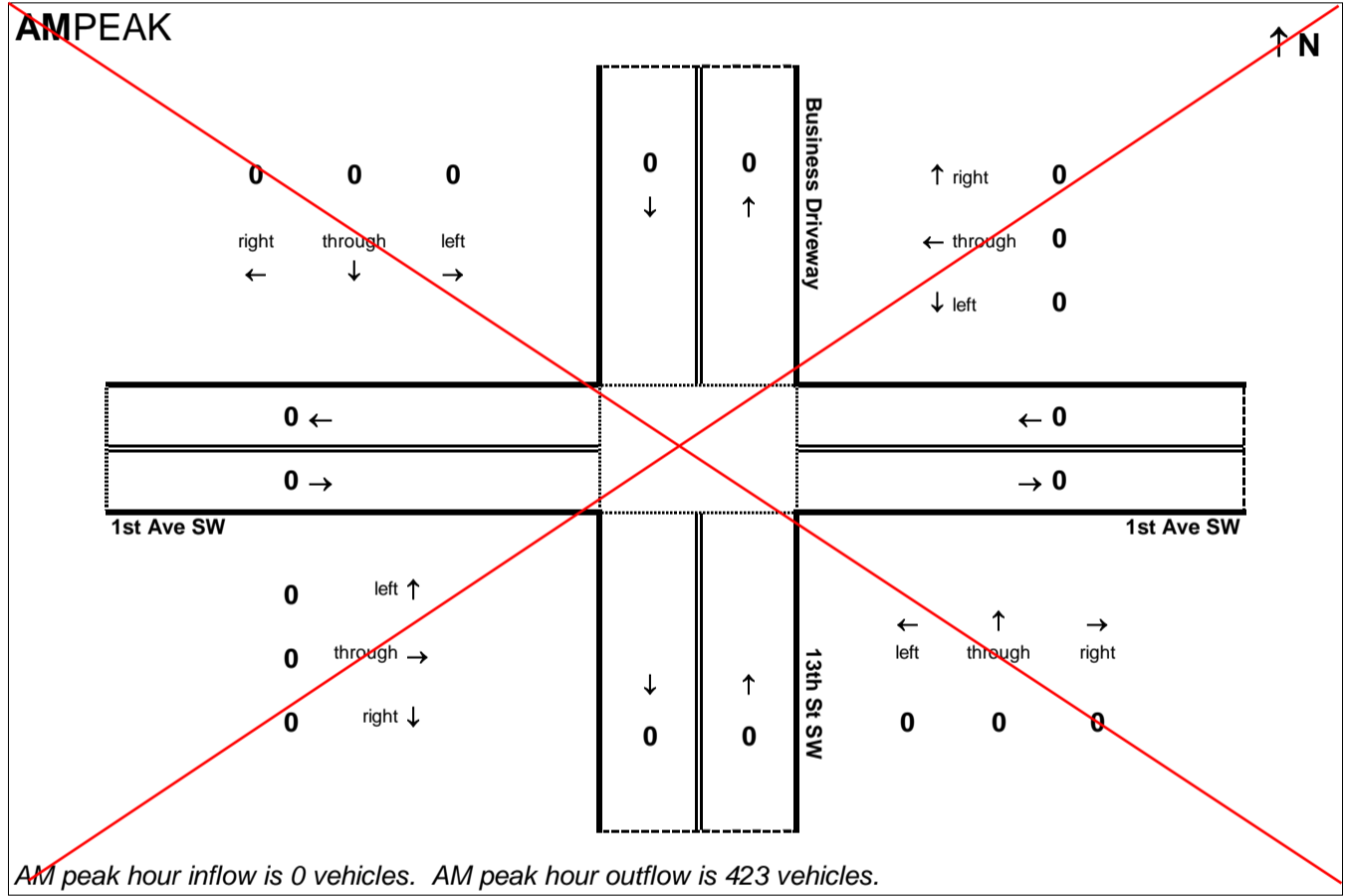
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

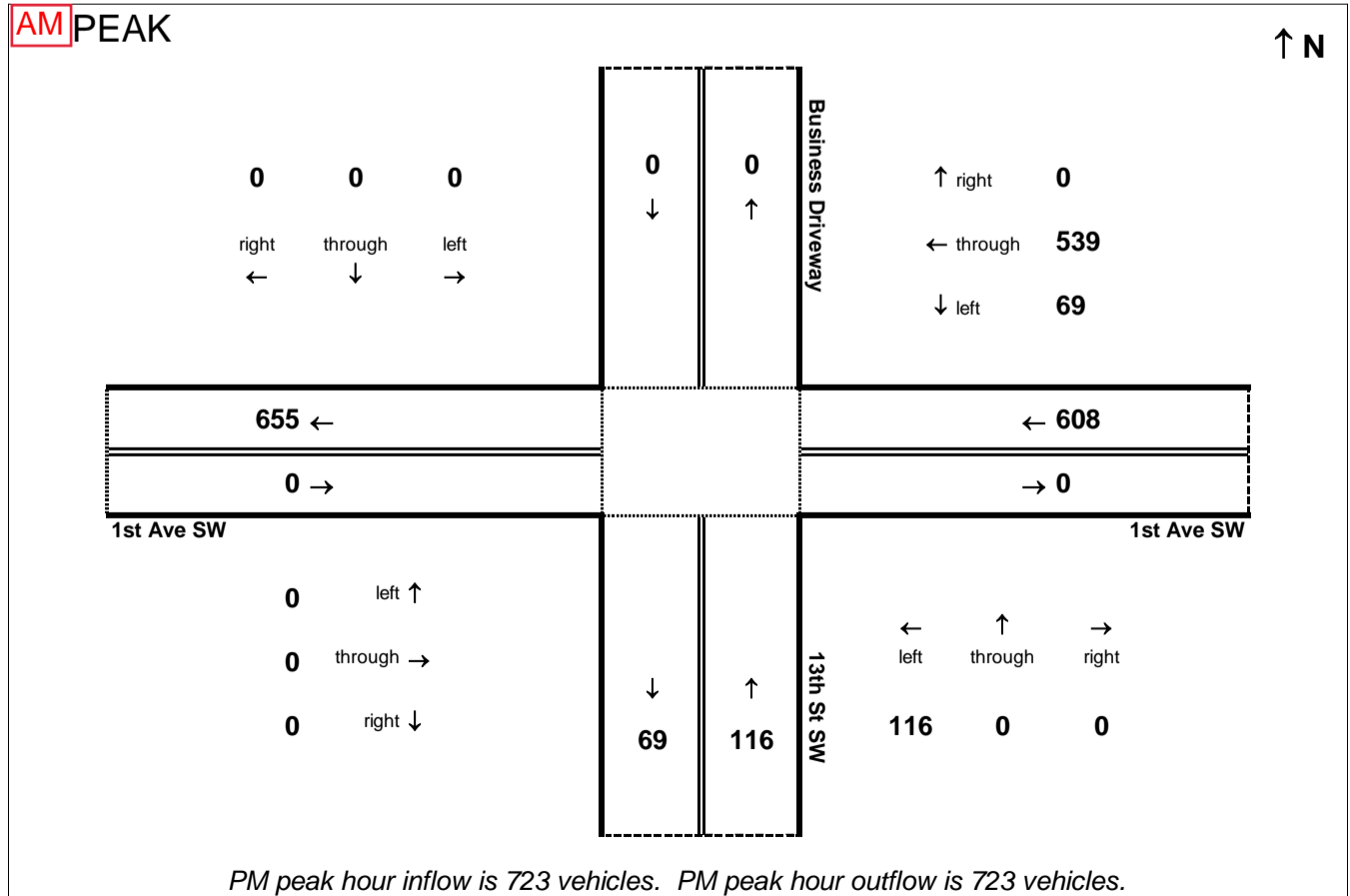
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 423 vehicles.

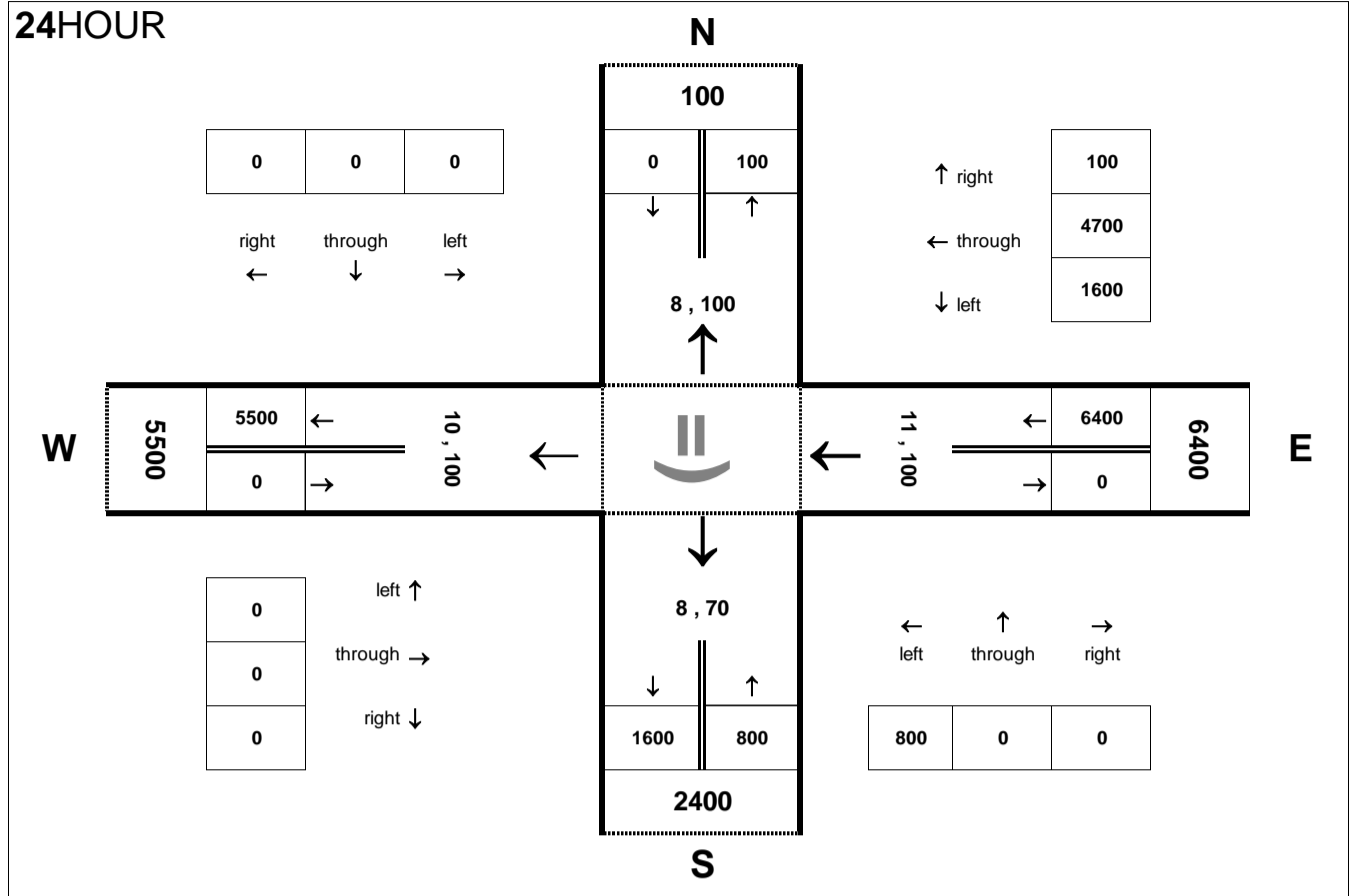
PM PEAK



PM peak hour inflow is 723 vehicles. PM peak hour outflow is 723 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



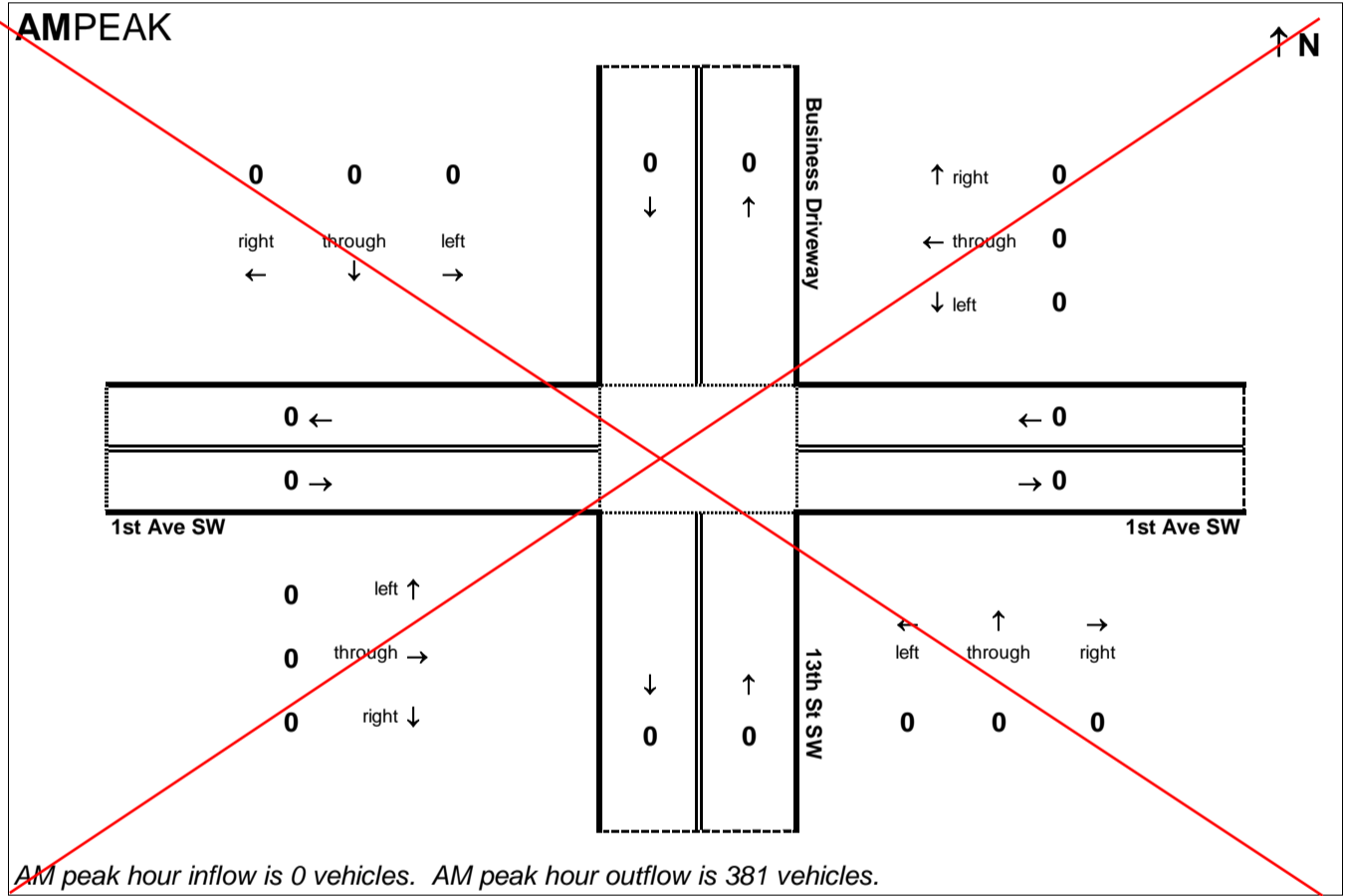
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Intersection of 1st Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

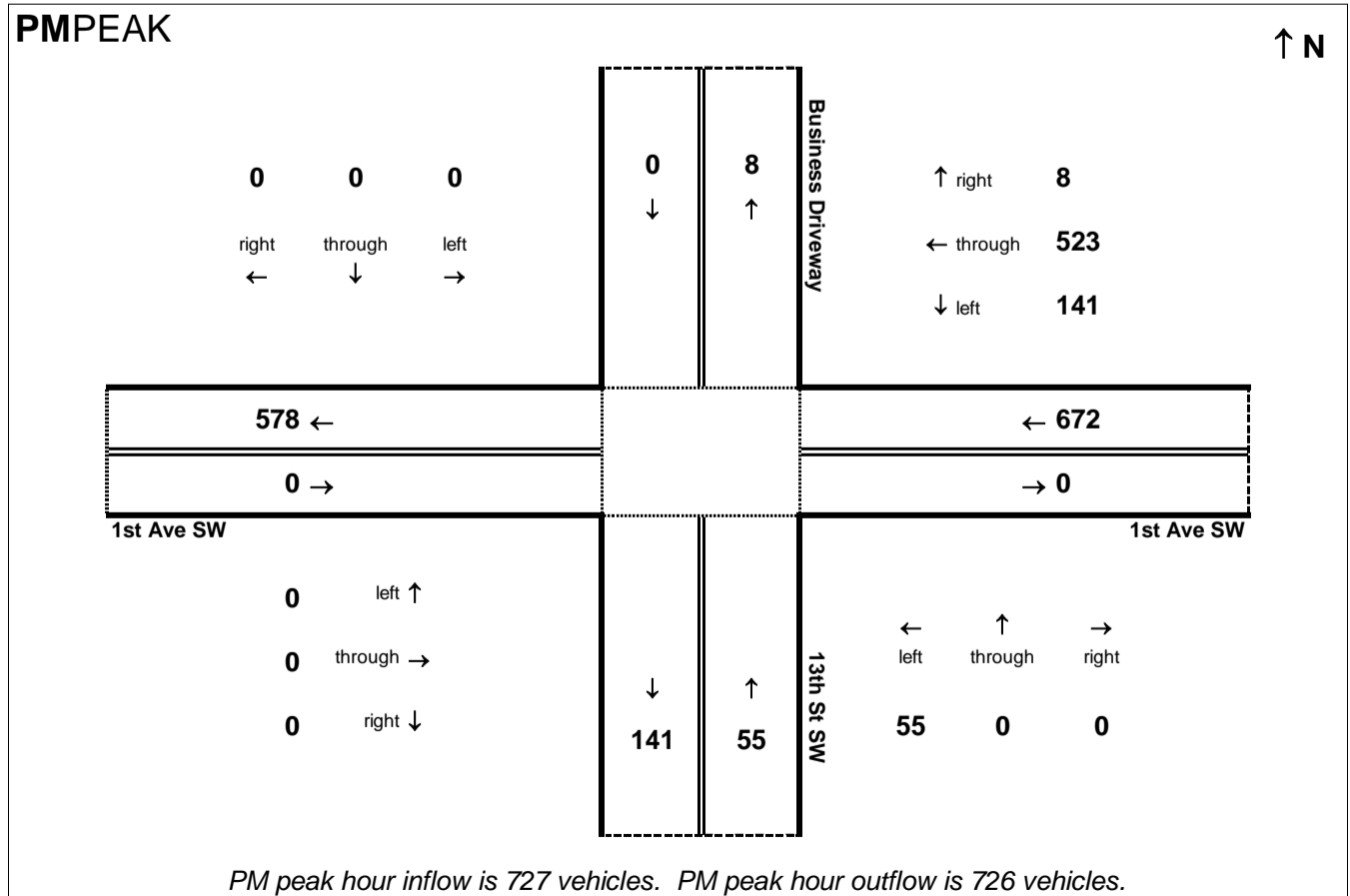
Traffic Data Year:
2040 No Build

Project:
U-4700

AMPEAK

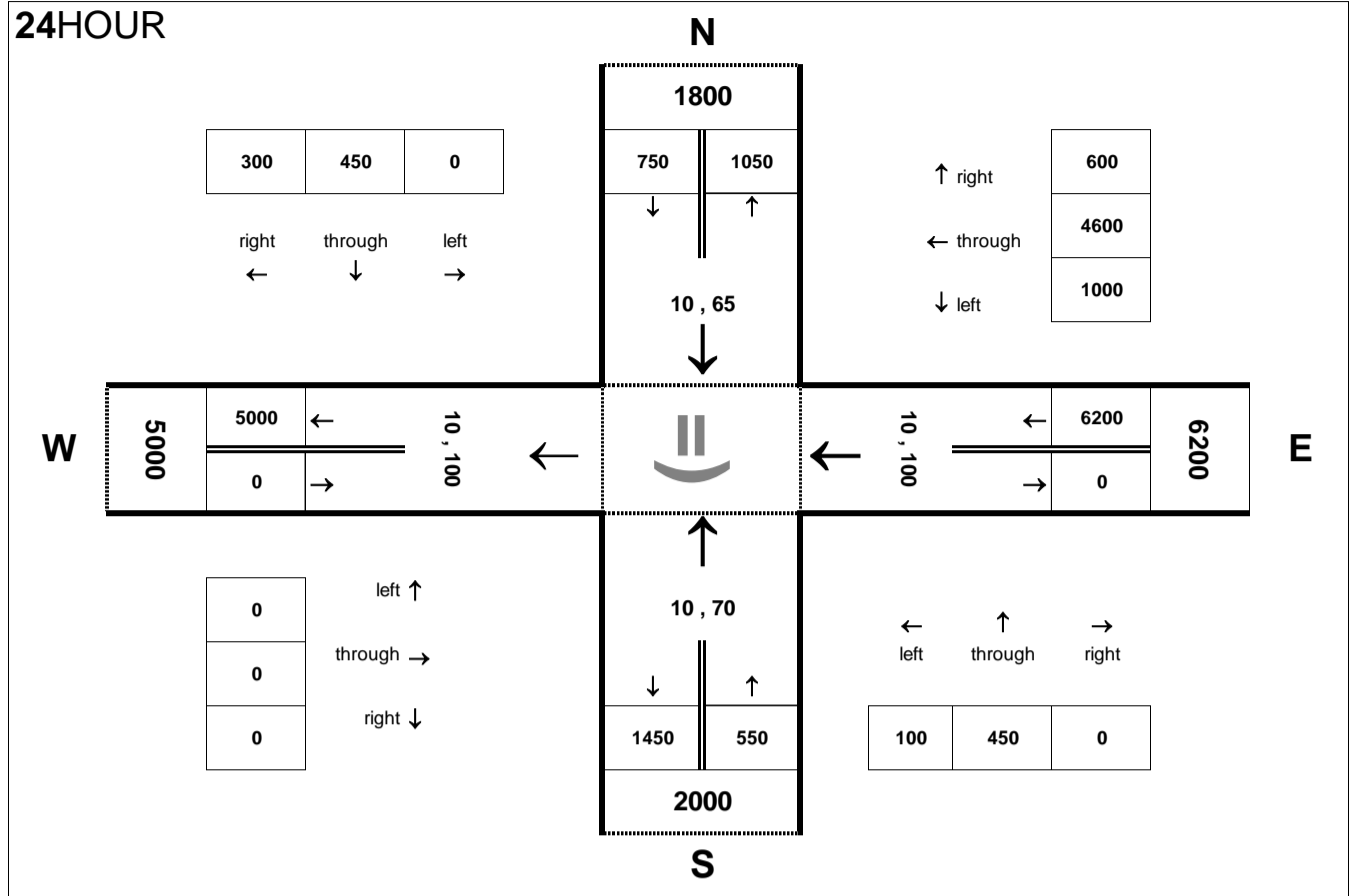


PMPEAK



PM peak hour inflow is 727 vehicles. PM peak hour outflow is 726 vehicles.

24HOUR



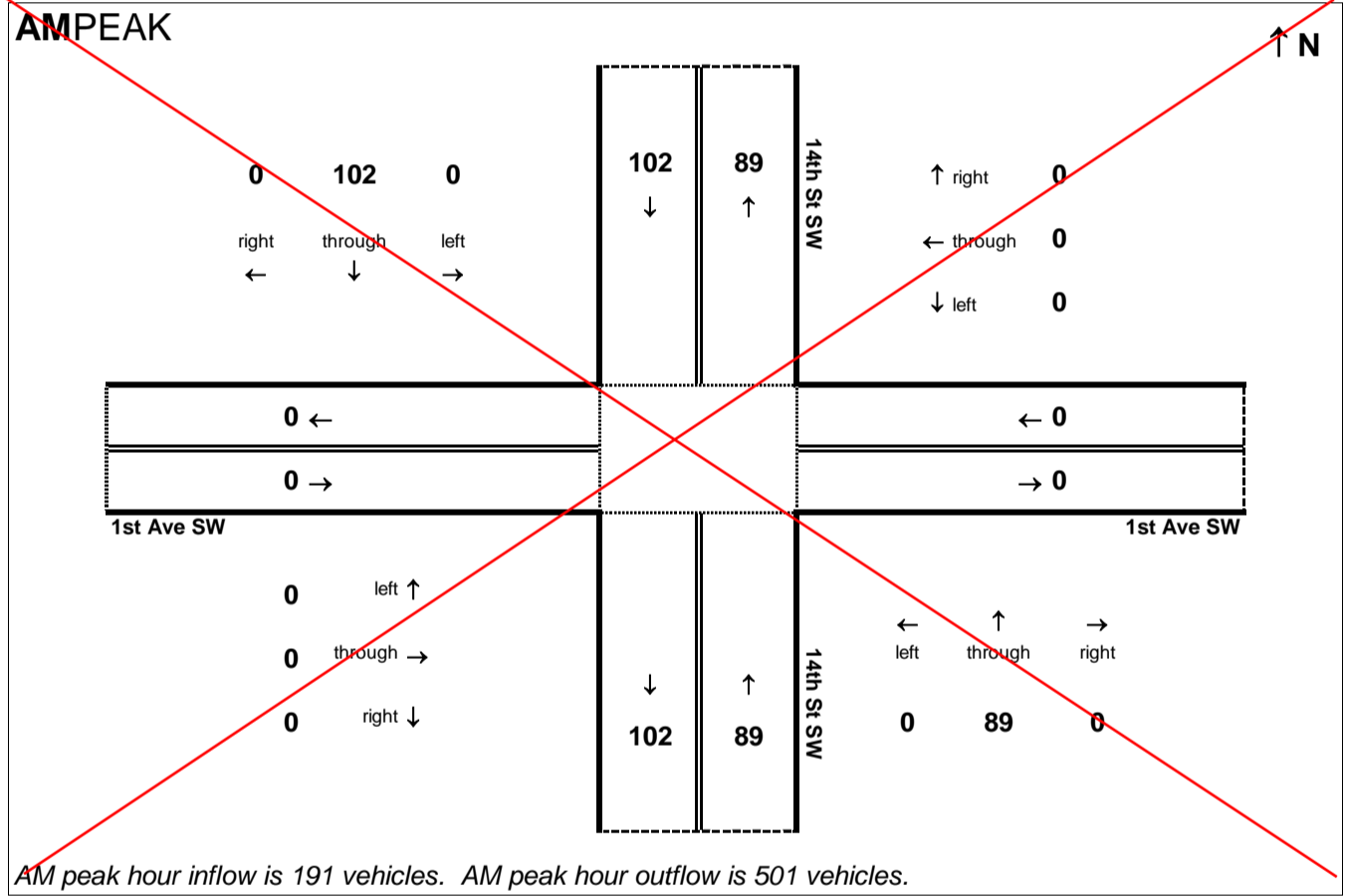
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

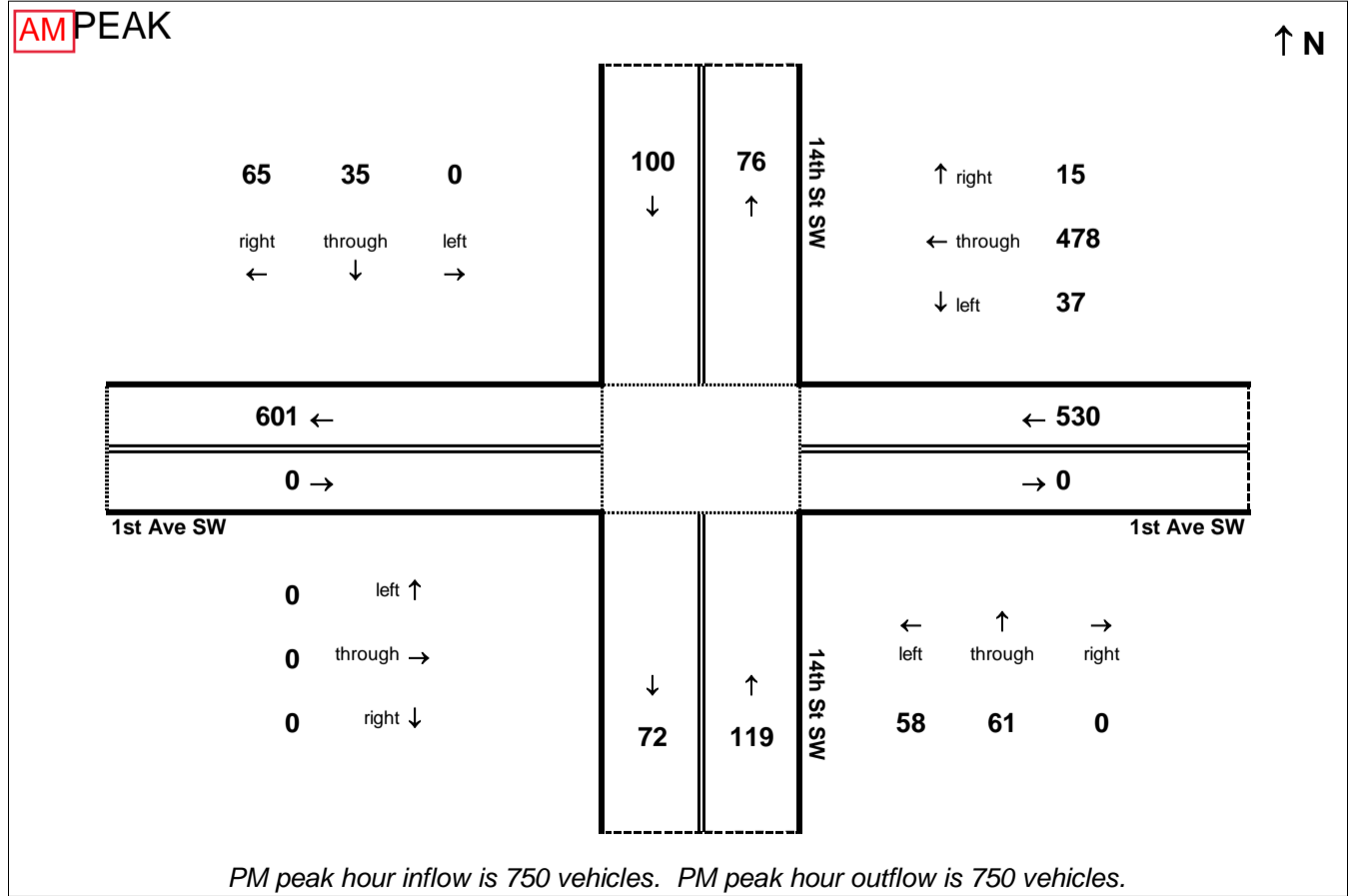
Traffic Data Year:
2040 No Build

Project:
U-4700

~~**AMPEAK**~~

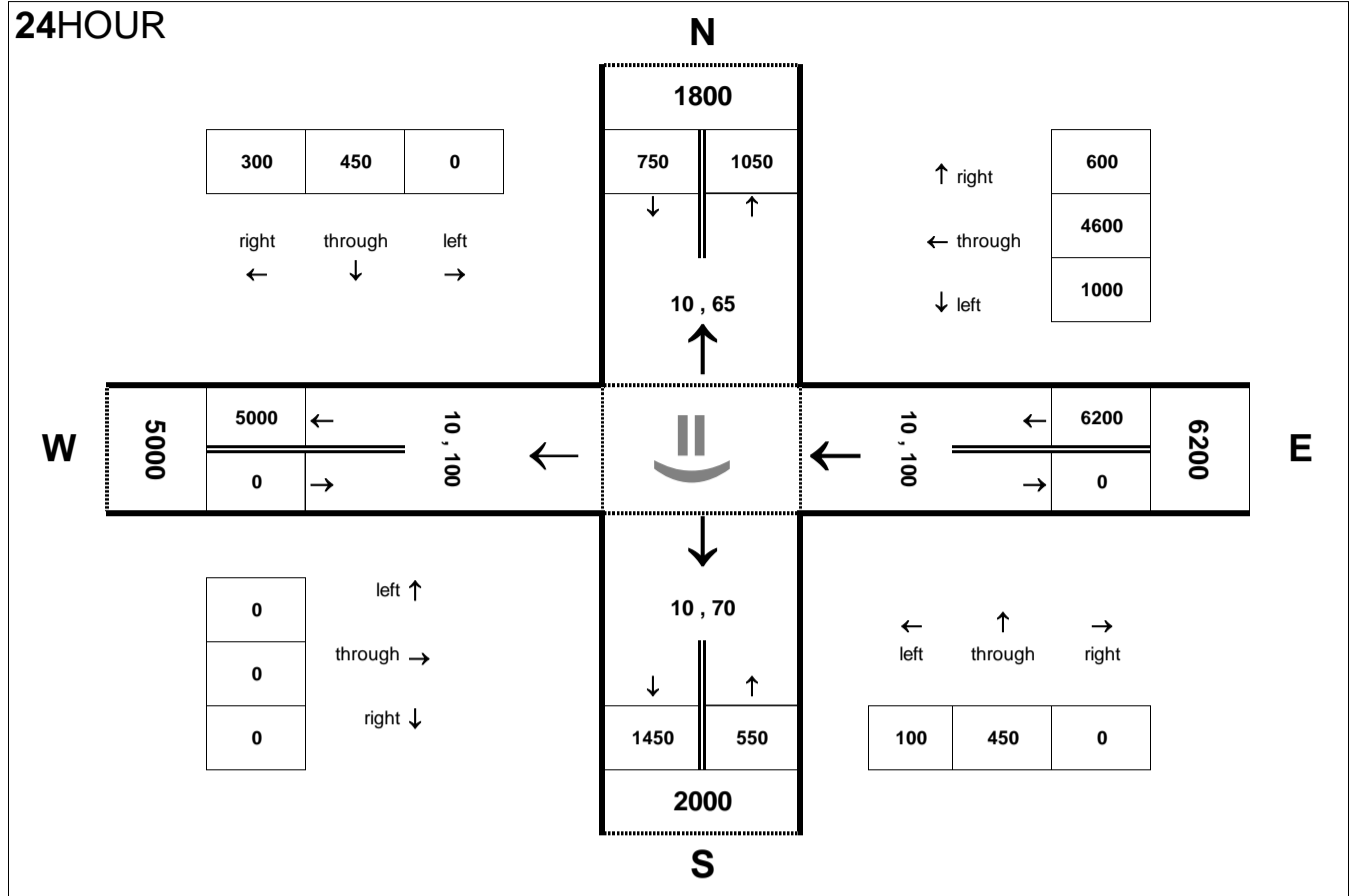


PM PEAK



Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



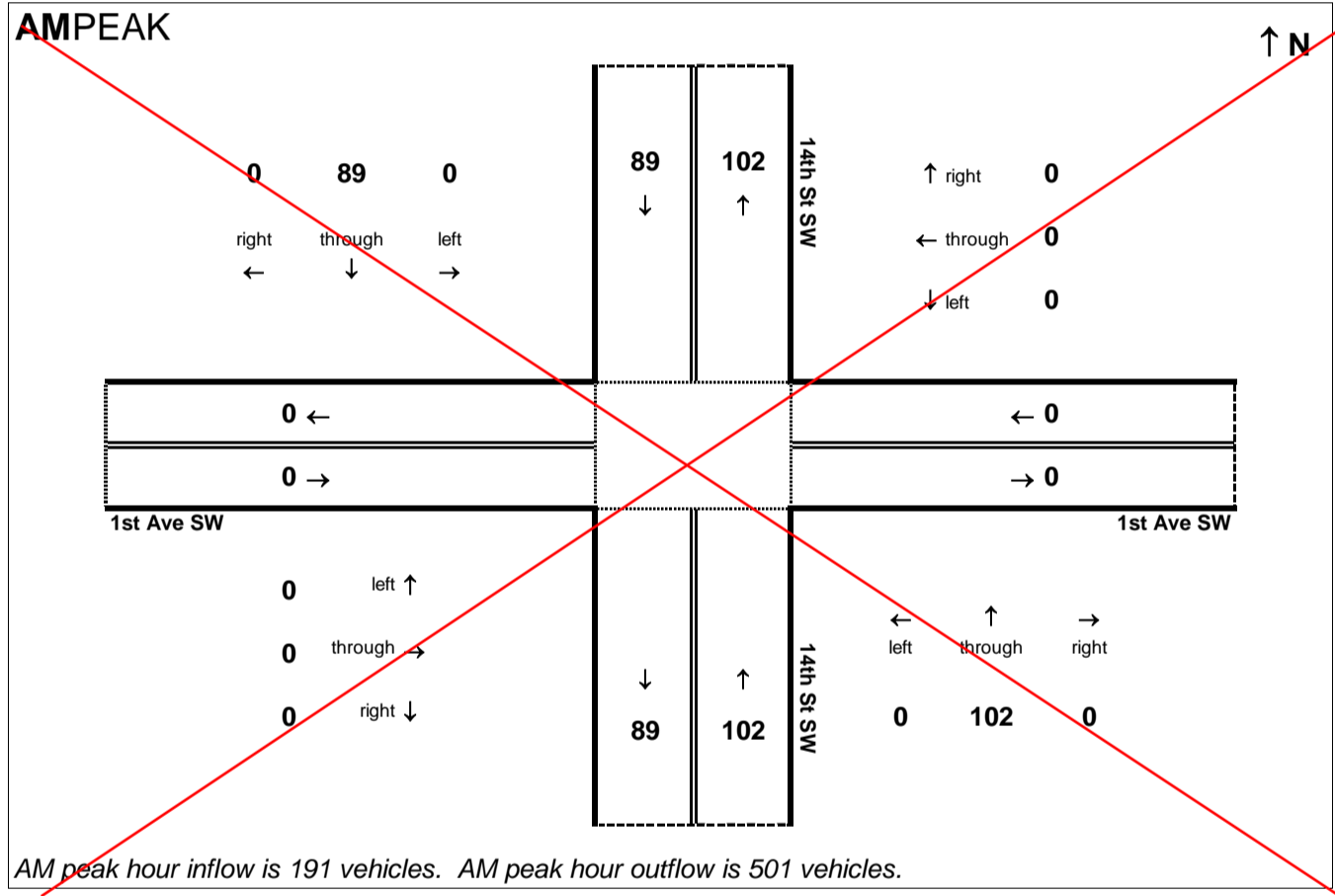
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

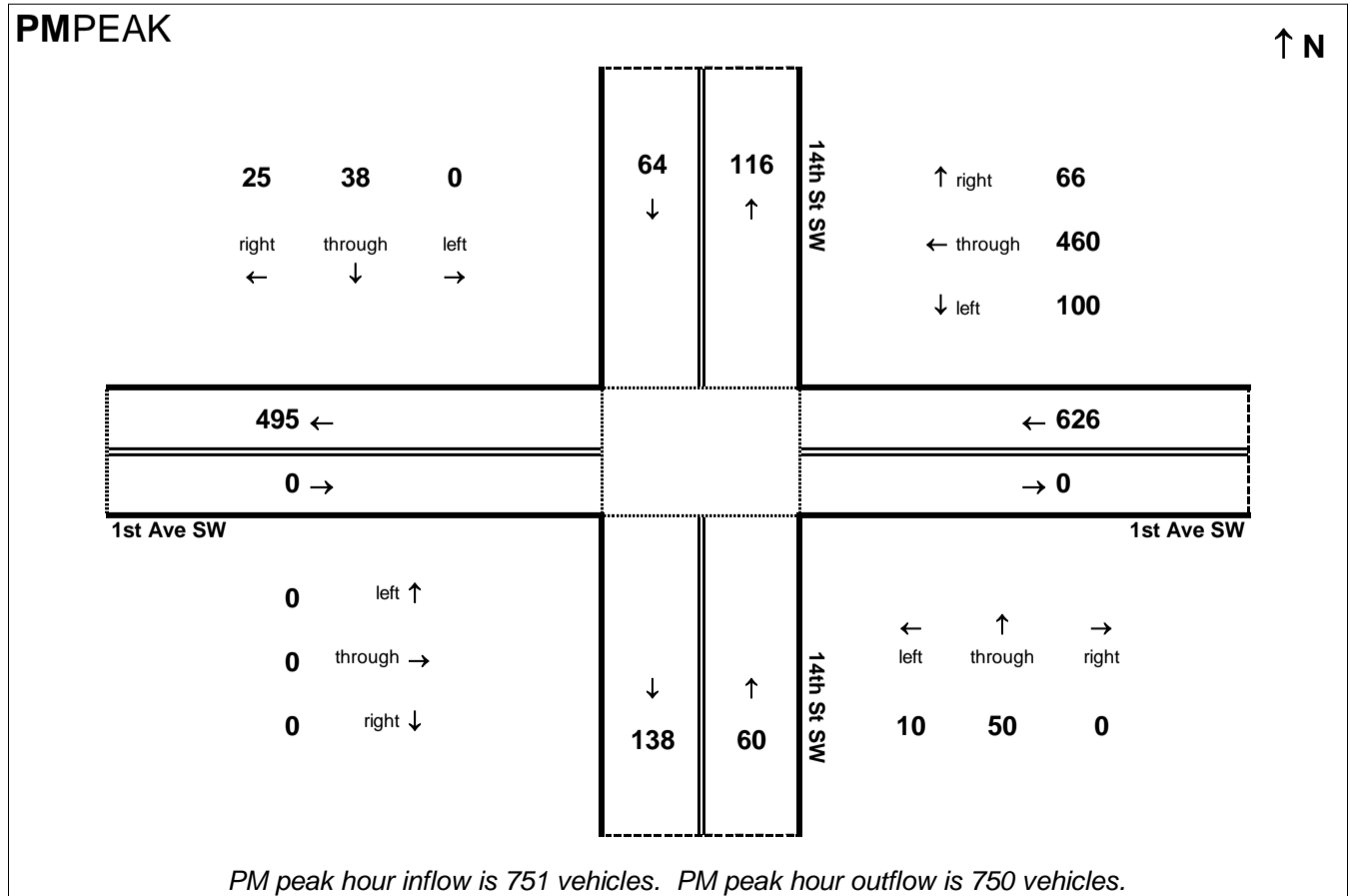
Project:
U-4700

AMPEAK



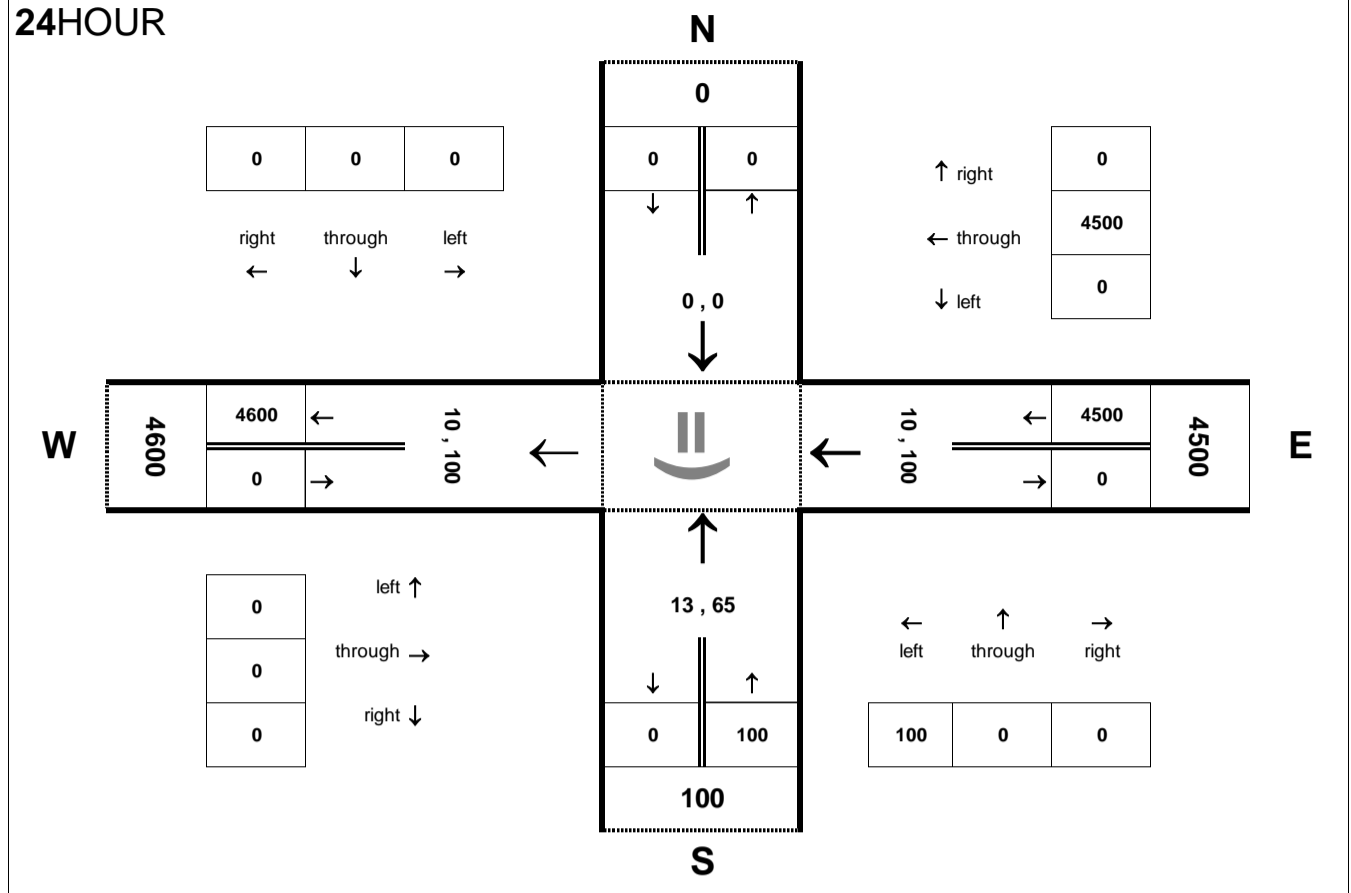
AM peak hour inflow is 191 vehicles. AM peak hour outflow is 501 vehicles.

PMPEAK



PM peak hour inflow is 751 vehicles. PM peak hour outflow is 750 vehicles.

24HOUR



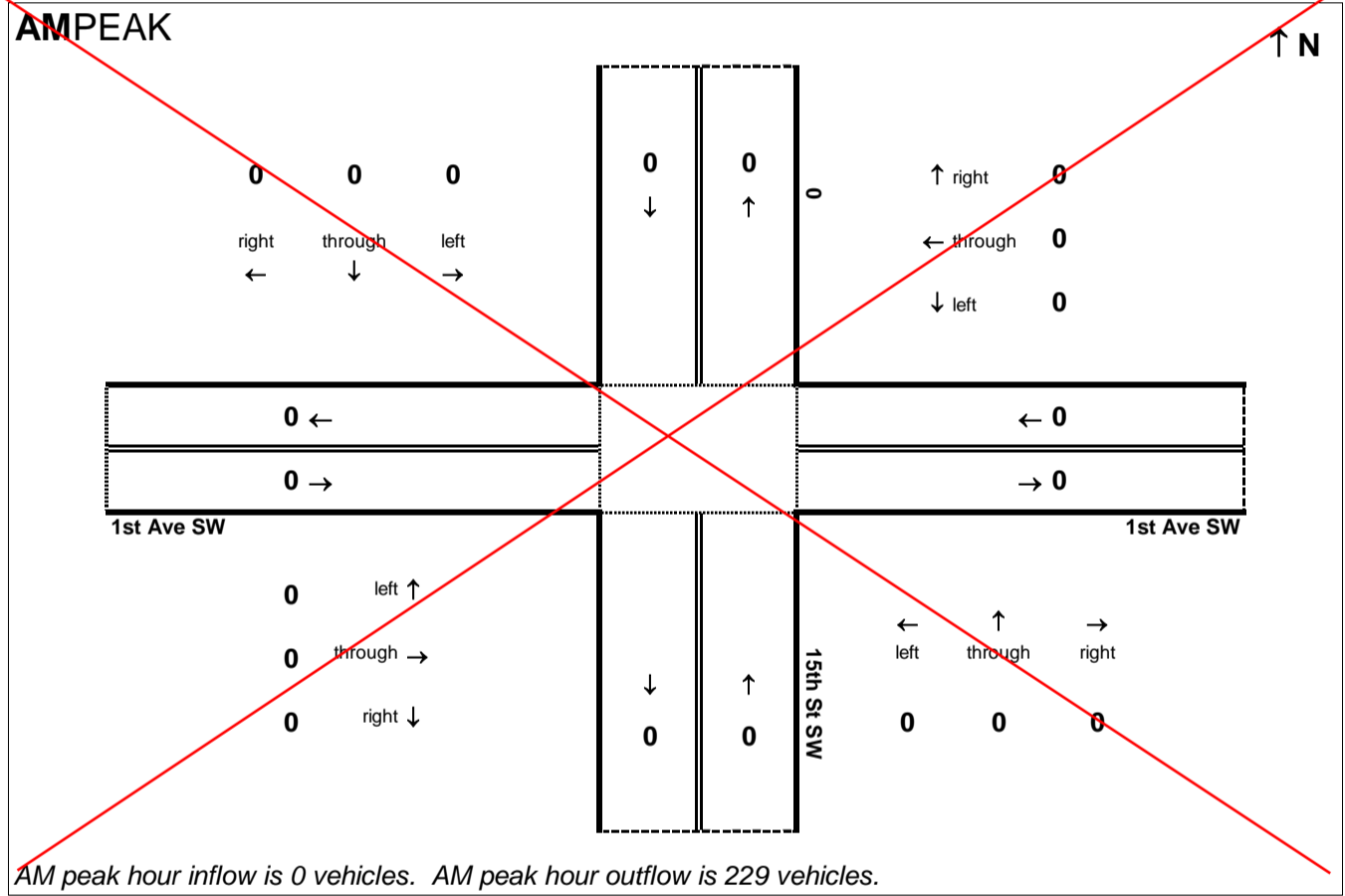
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

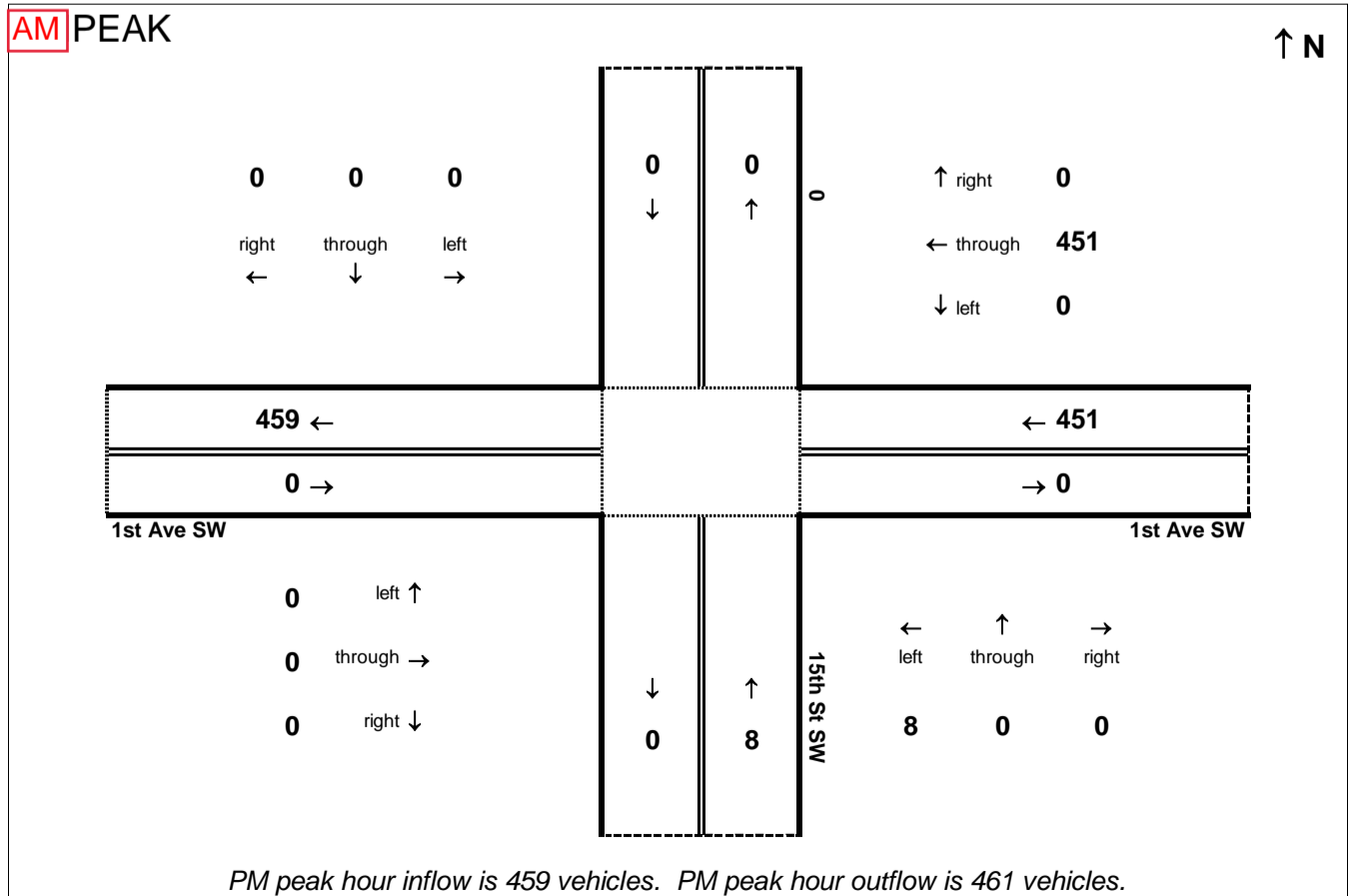
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 229 vehicles.

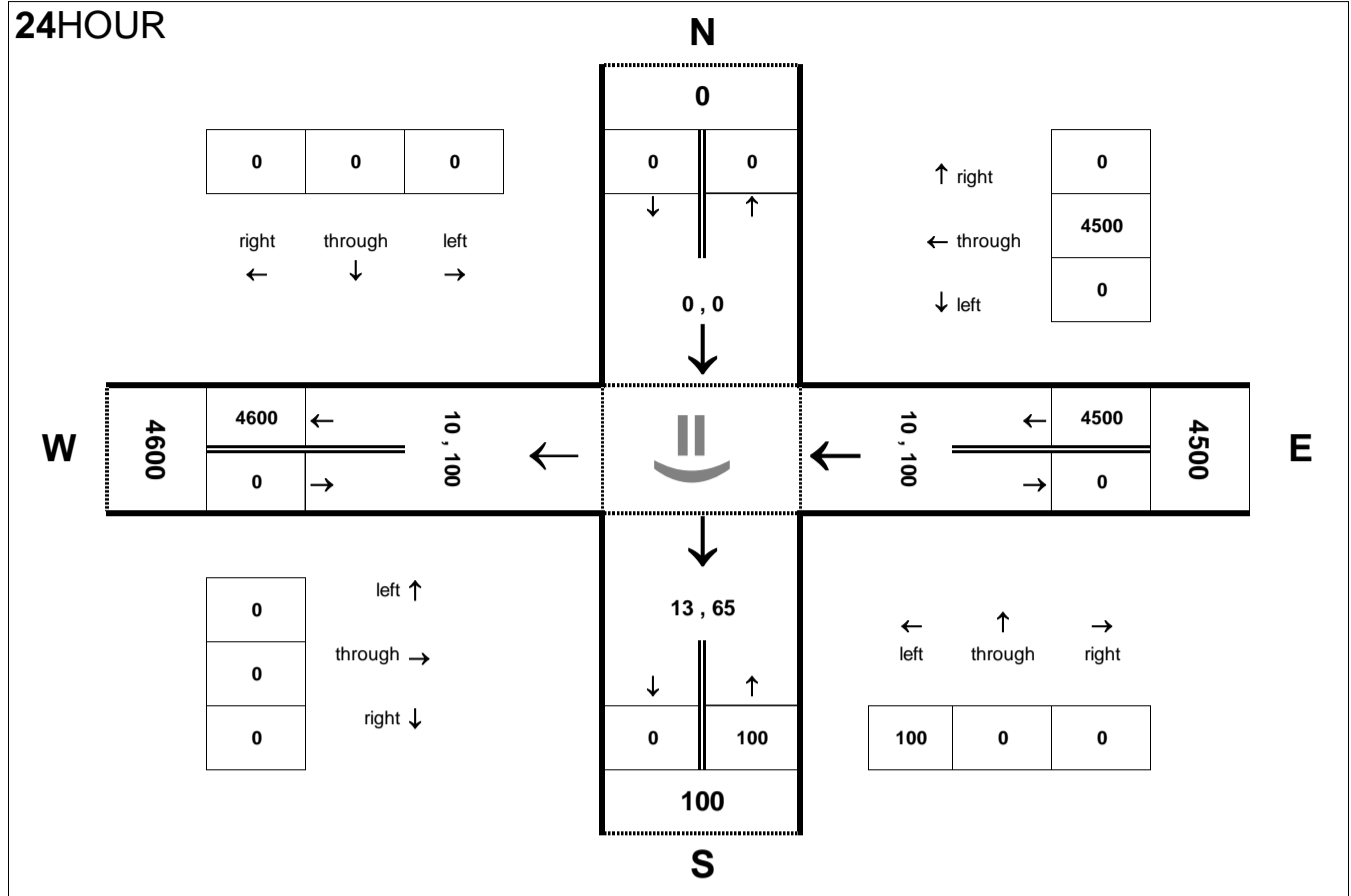
AM PEAK



PM peak hour inflow is 459 vehicles. PM peak hour outflow is 461 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



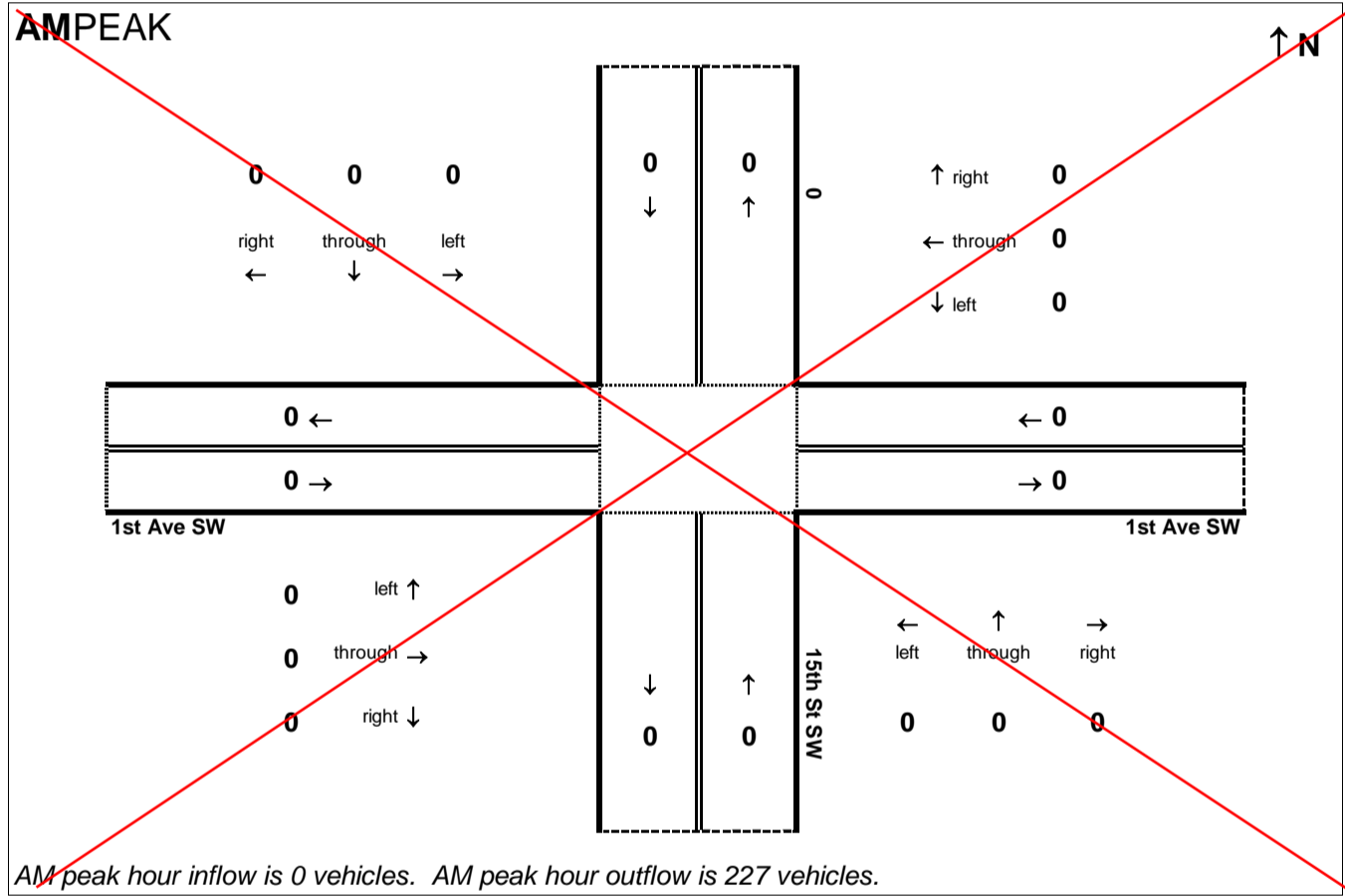
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

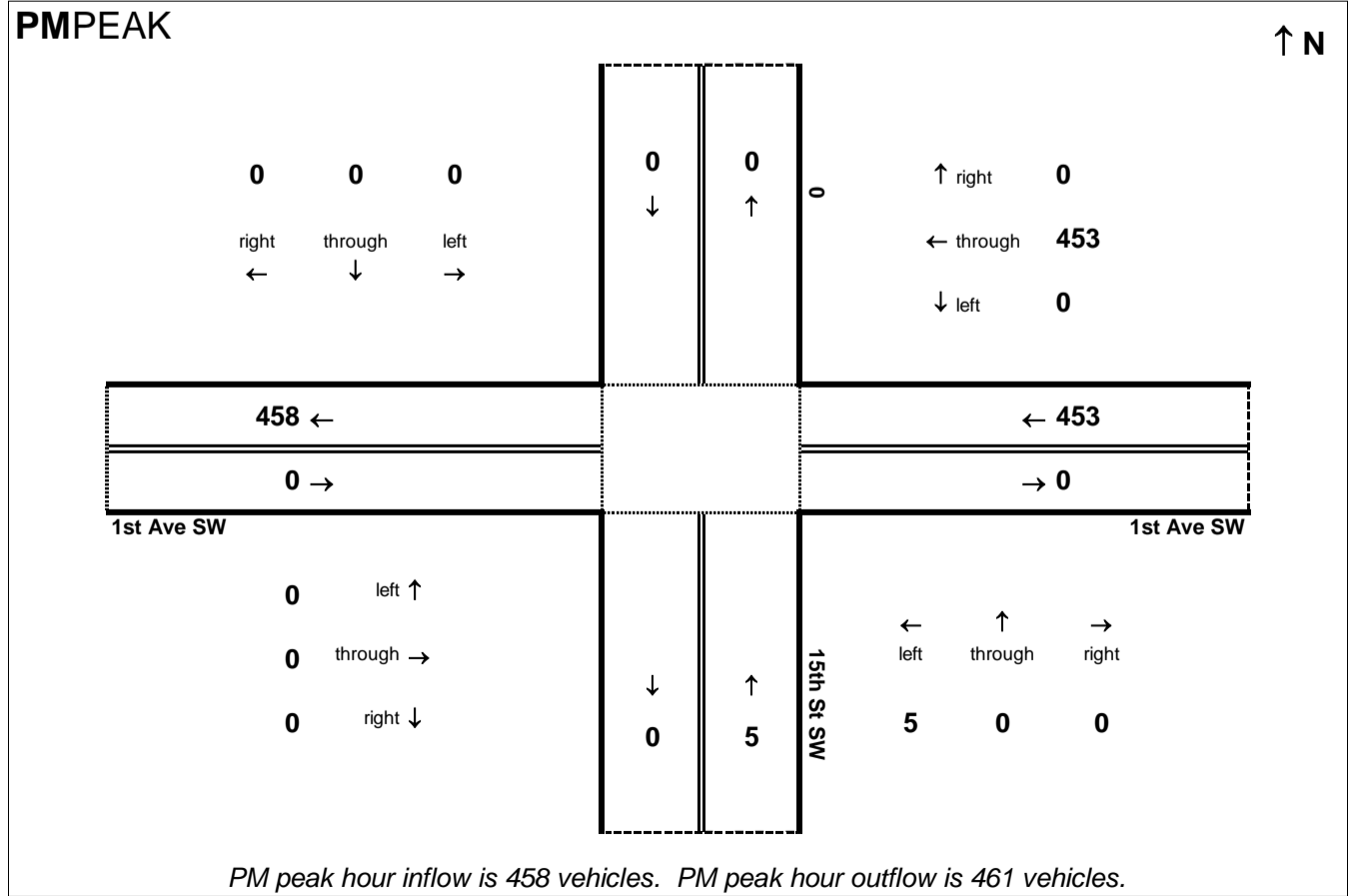
Project:
U-4700

AMPEAK

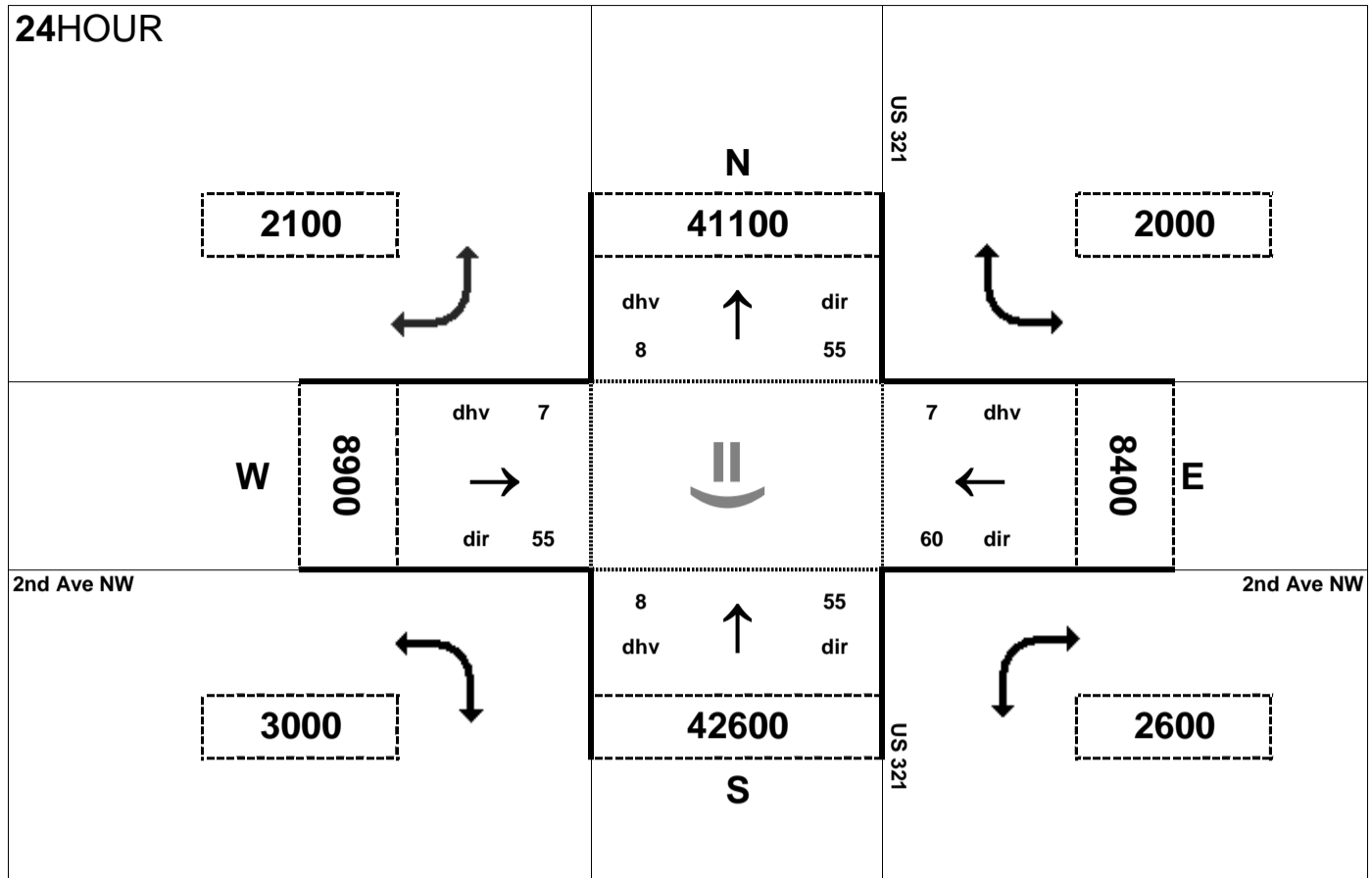


AM peak hour inflow is 0 vehicles. AM peak hour outflow is 227 vehicles.

PMPEAK



PM peak hour inflow is 458 vehicles. PM peak hour outflow is 461 vehicles.

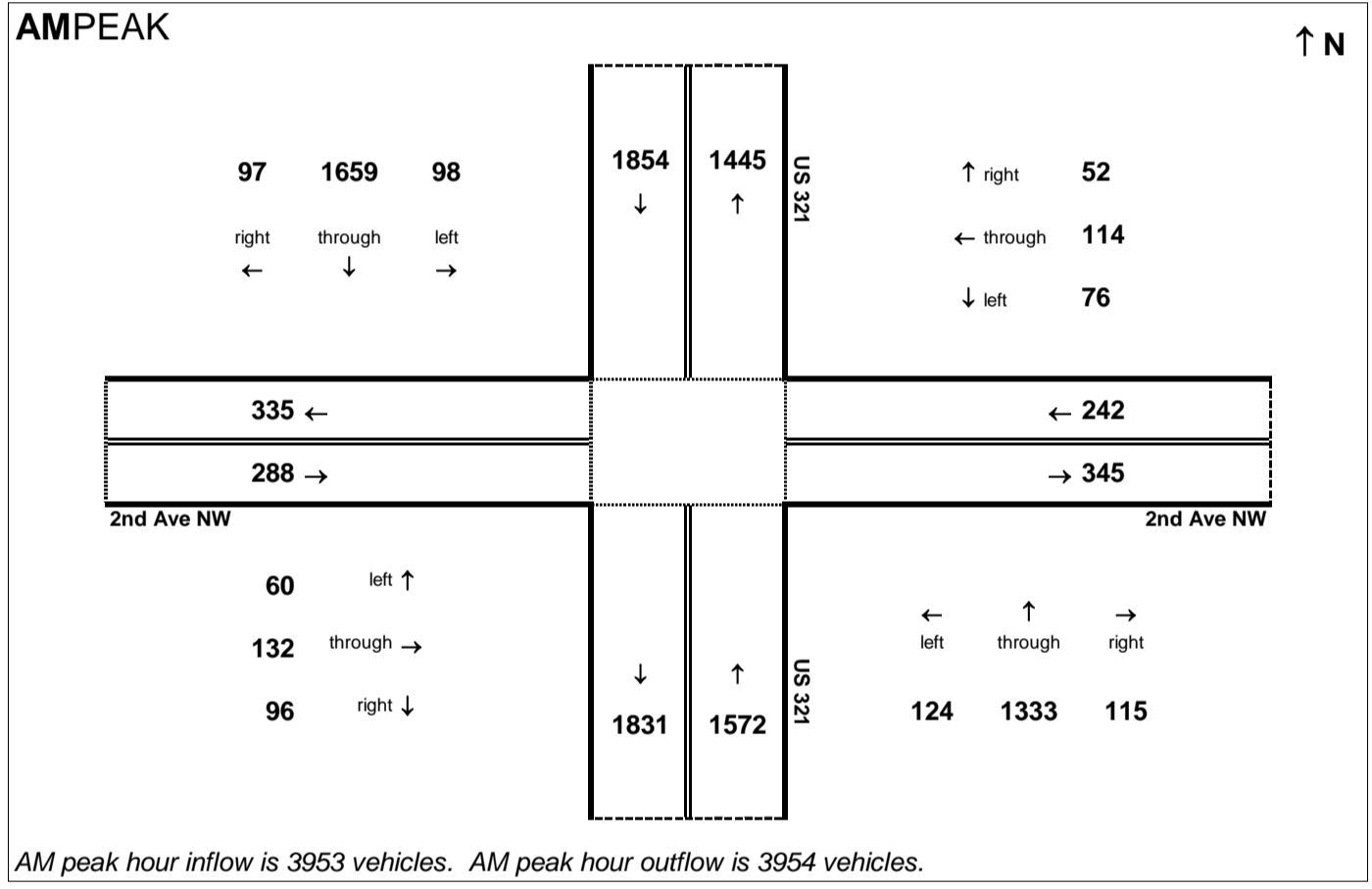


Peak Hour Volume Breakouts Report:
 Intersection of US 321 and 2nd Ave NW (SR 1306)

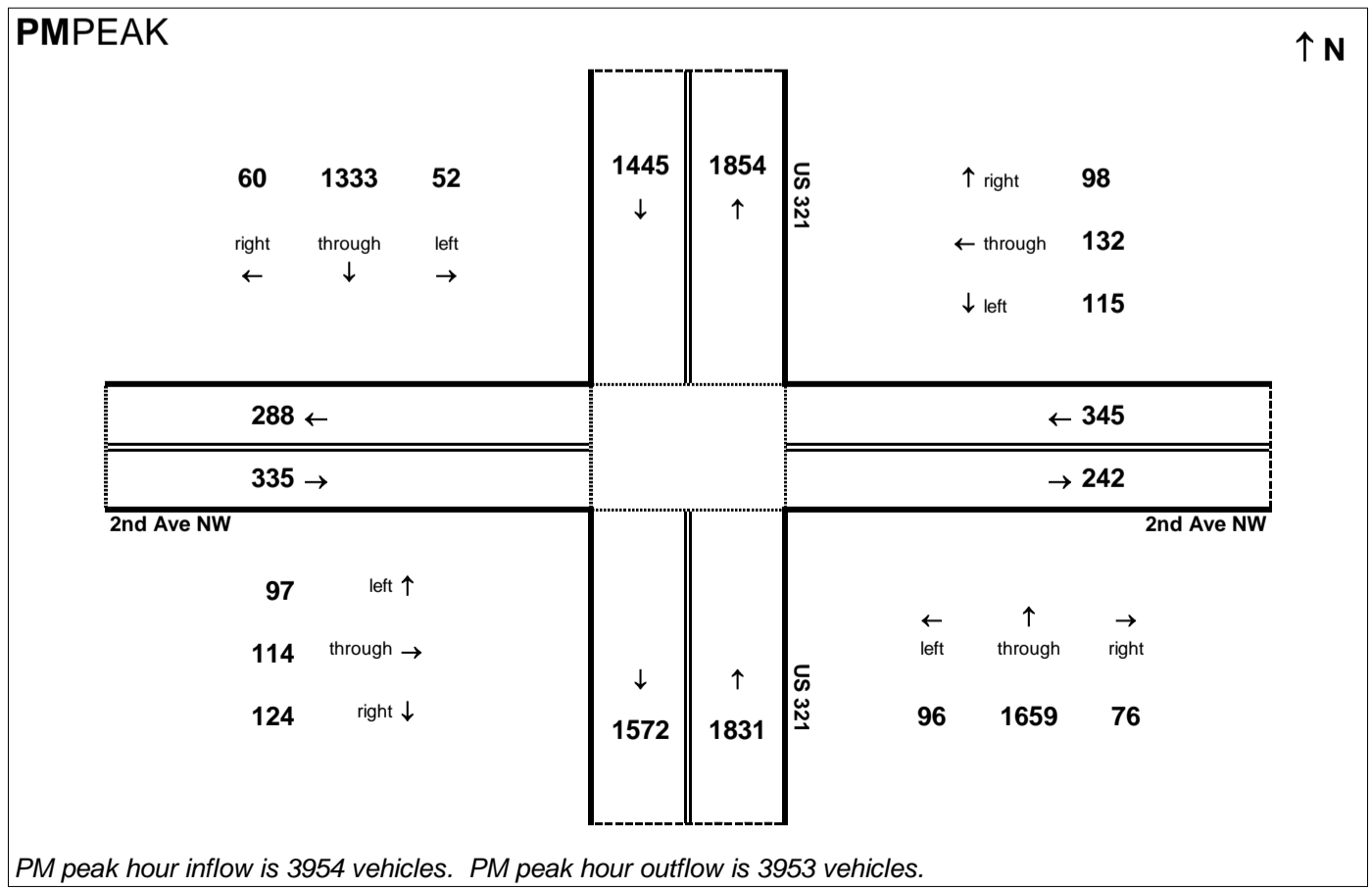
Traffic Forecast Release Date:
 February-17

Traffic Data Year:
 2040 No Build

Project:
 U-4700

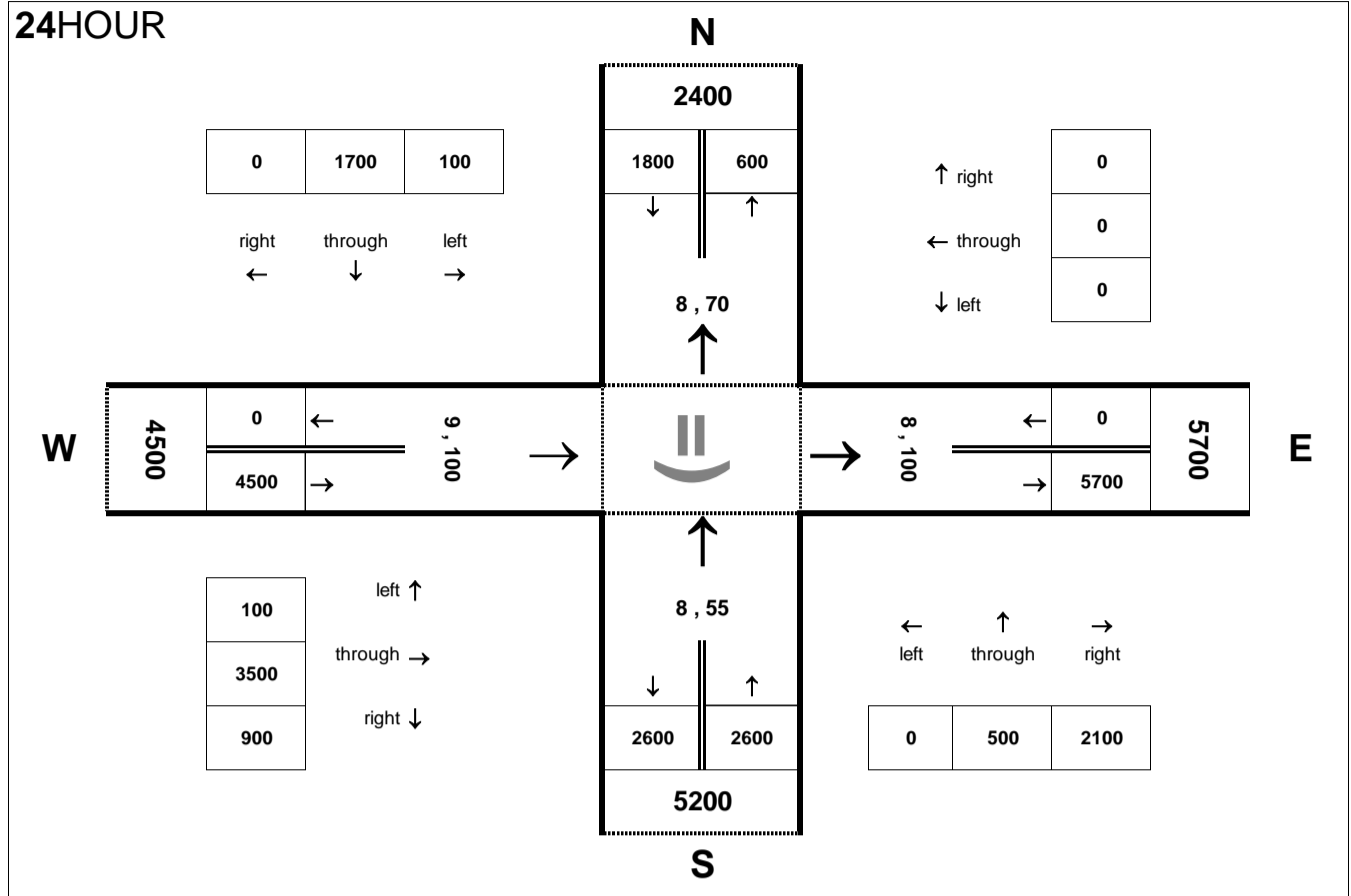


AM peak hour inflow is 3953 vehicles. AM peak hour outflow is 3954 vehicles.



PM peak hour inflow is 3954 vehicles. PM peak hour outflow is 3953 vehicles.

24HOUR



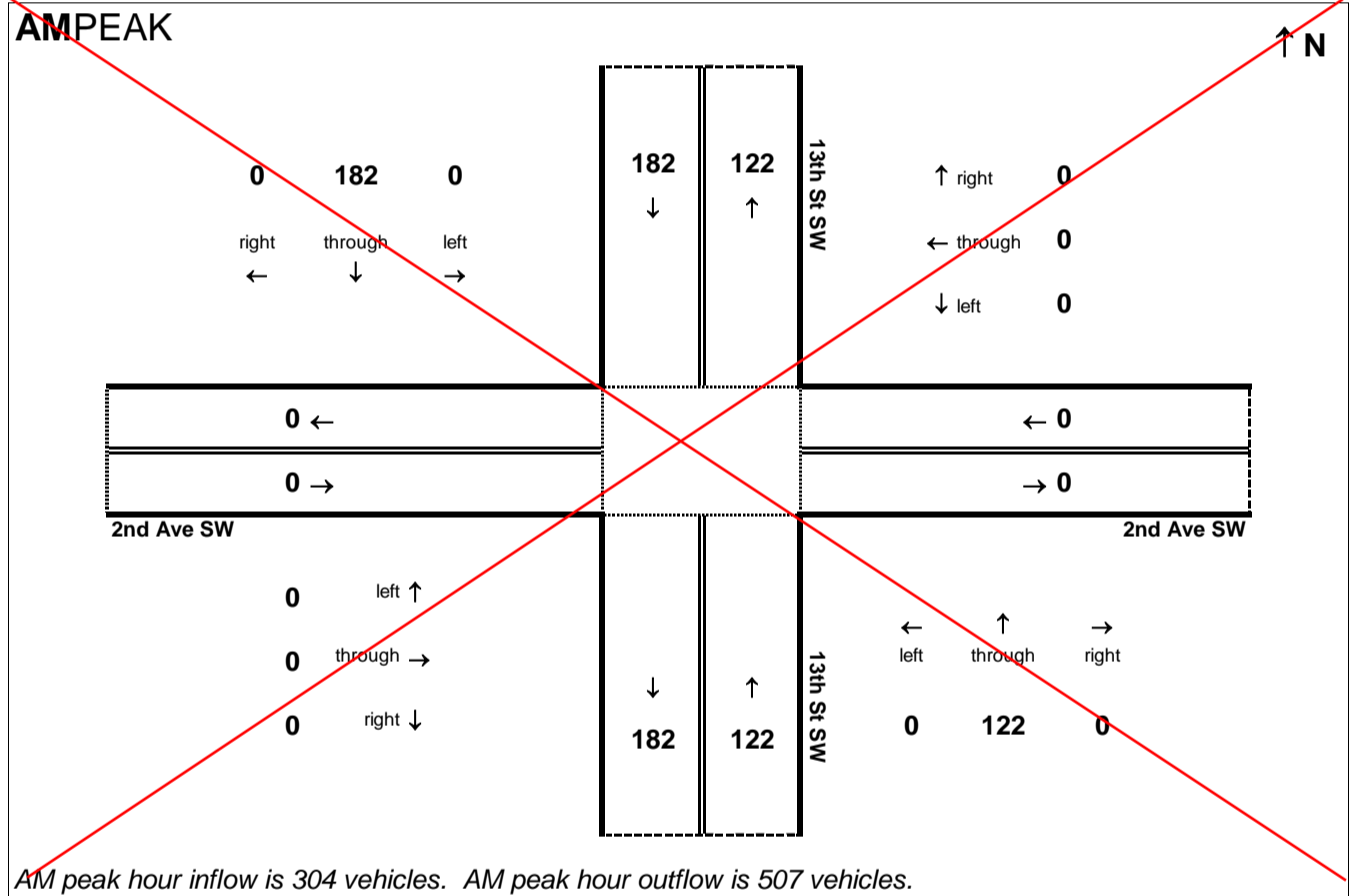
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

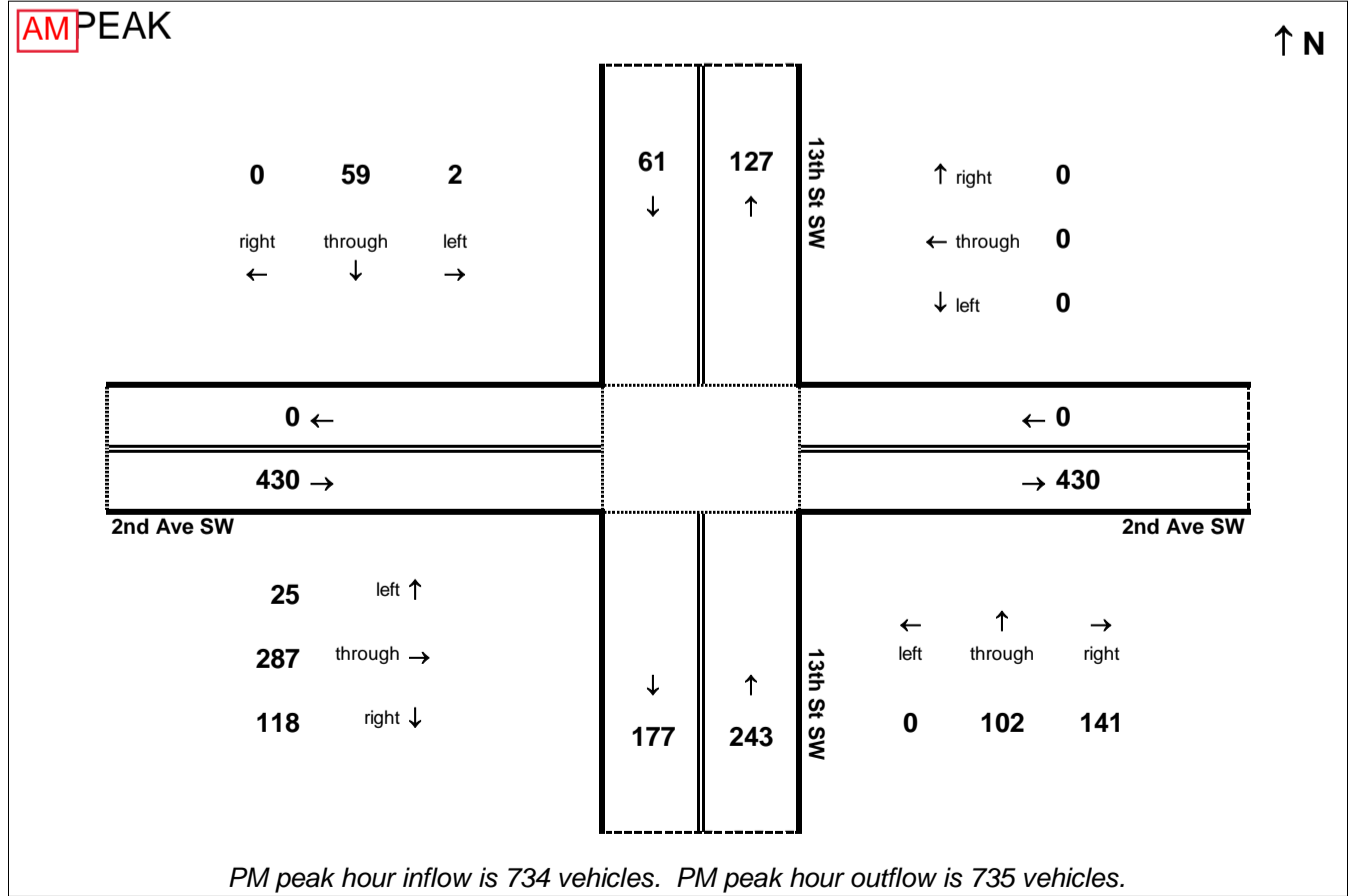
Traffic Data Year:
2040 No Build

Project:
U-4700

~~**AM PEAK**~~

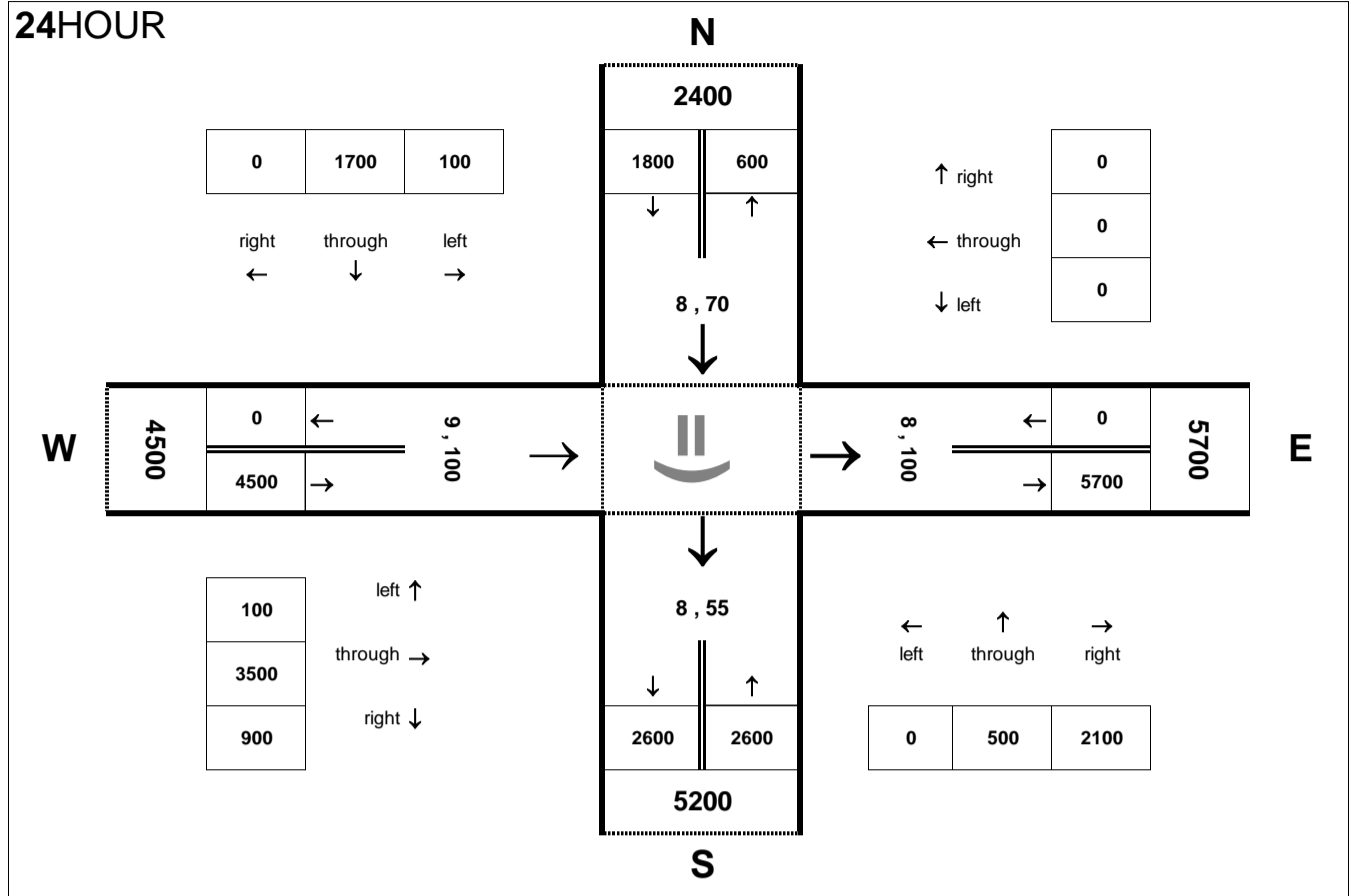


AM PEAK



Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



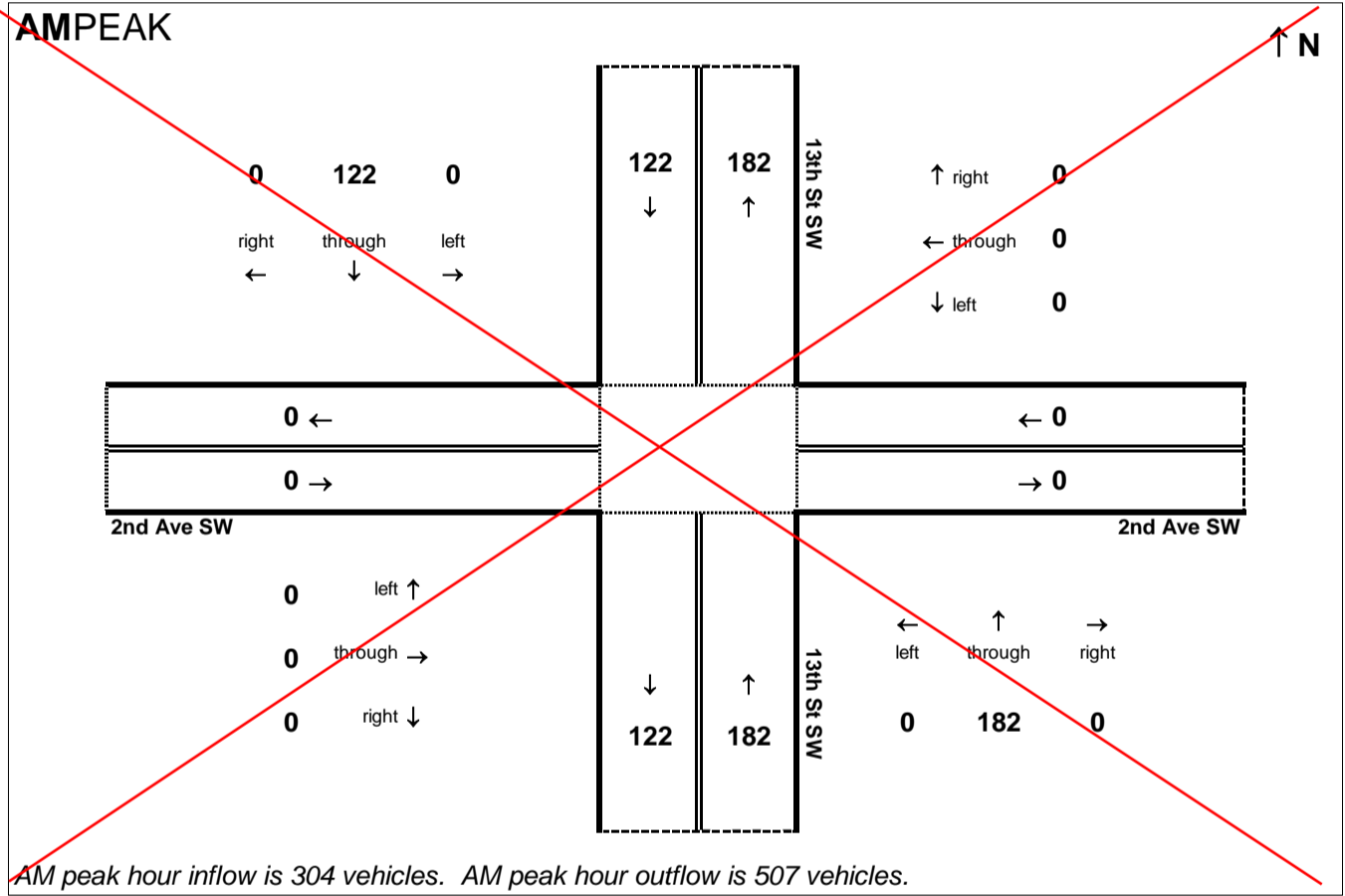
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

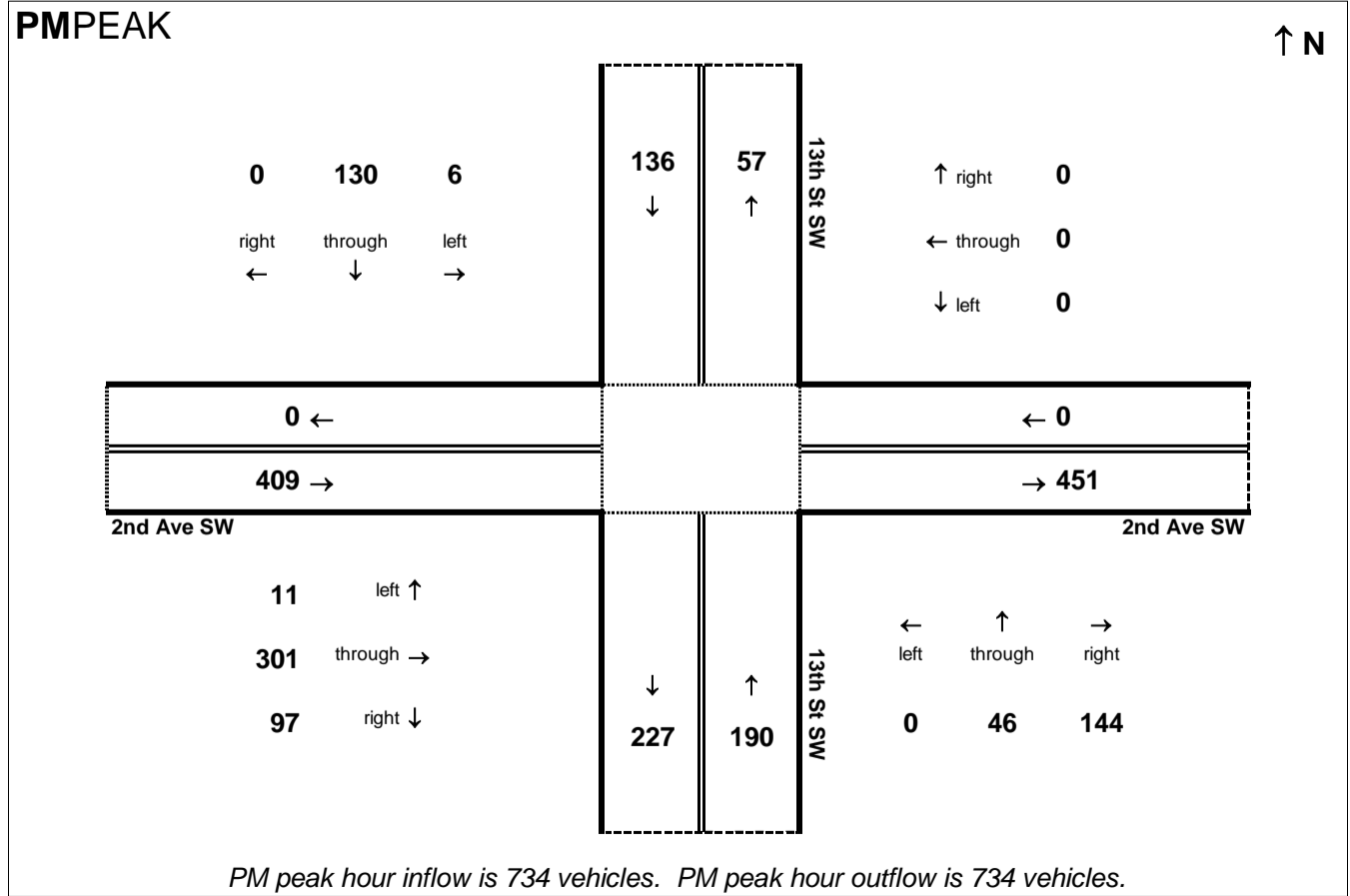
Project:
U-4700

AMPEAK



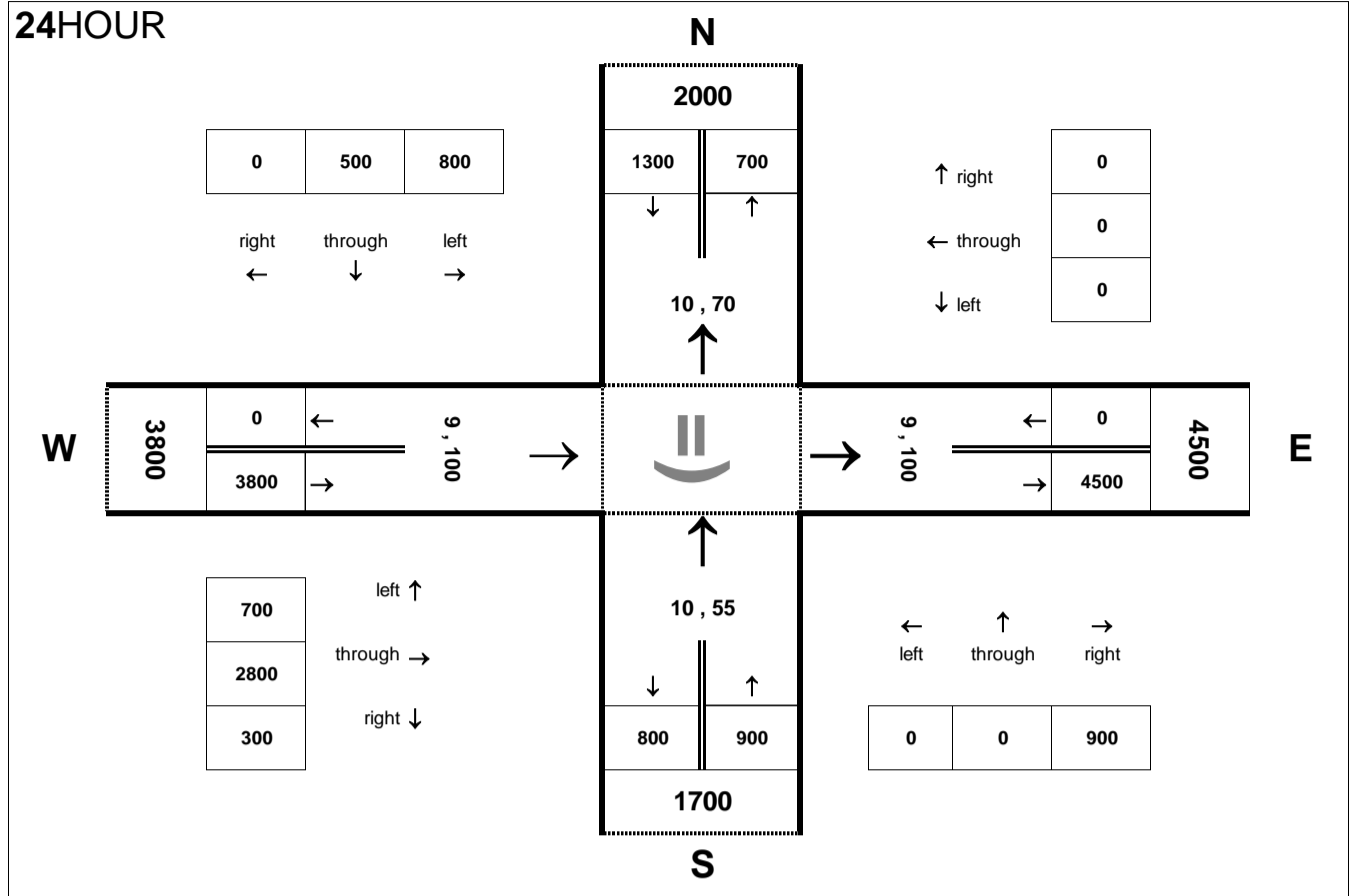
AM peak hour inflow is 304 vehicles. AM peak hour outflow is 507 vehicles.

PMPEAK



PM peak hour inflow is 734 vehicles. PM peak hour outflow is 734 vehicles.

24HOUR



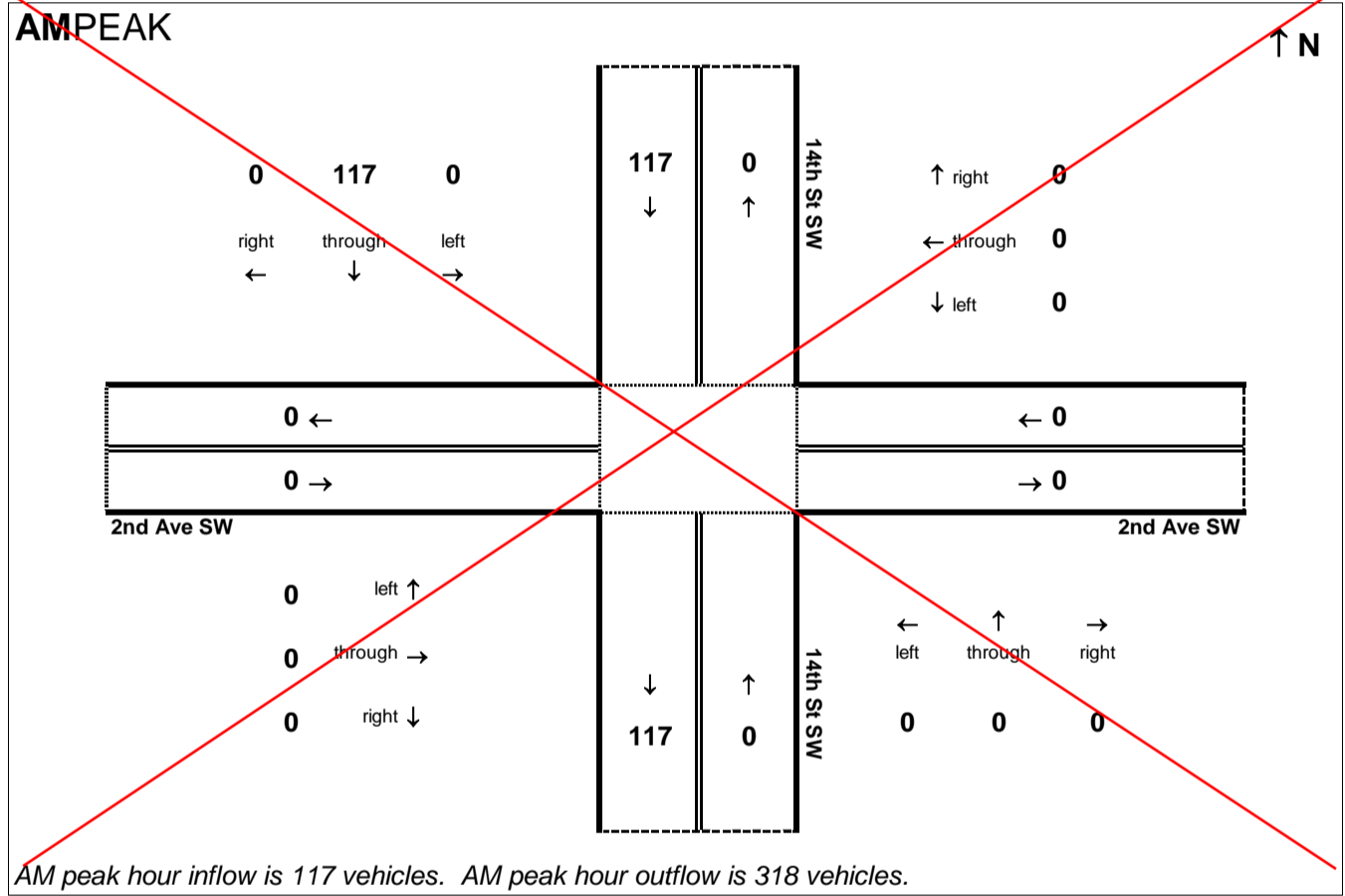
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

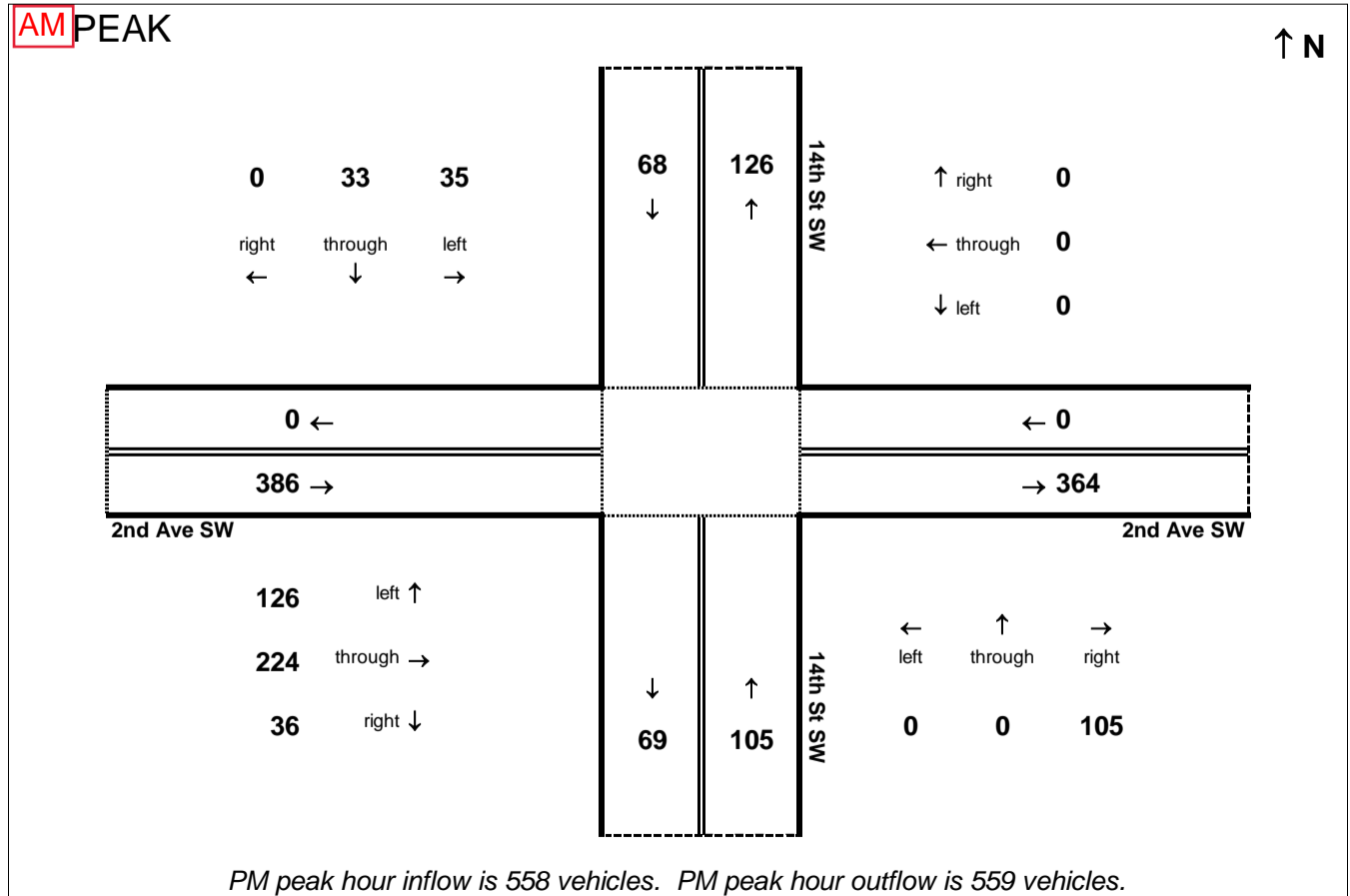
Project:
U-4700

AM PEAK



AM peak hour inflow is 117 vehicles. AM peak hour outflow is 318 vehicles.

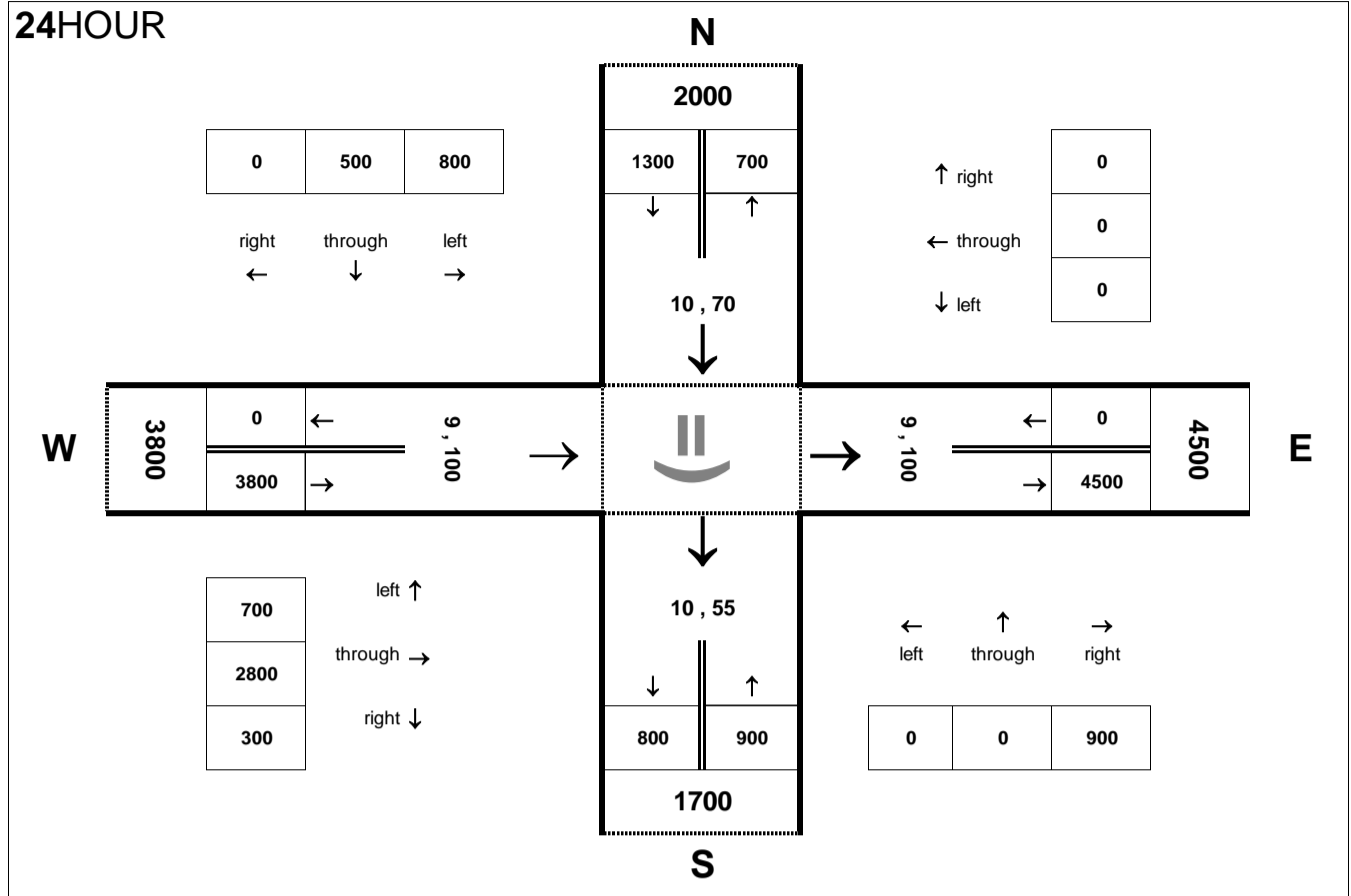
PM PEAK



PM peak hour inflow is 558 vehicles. PM peak hour outflow is 559 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



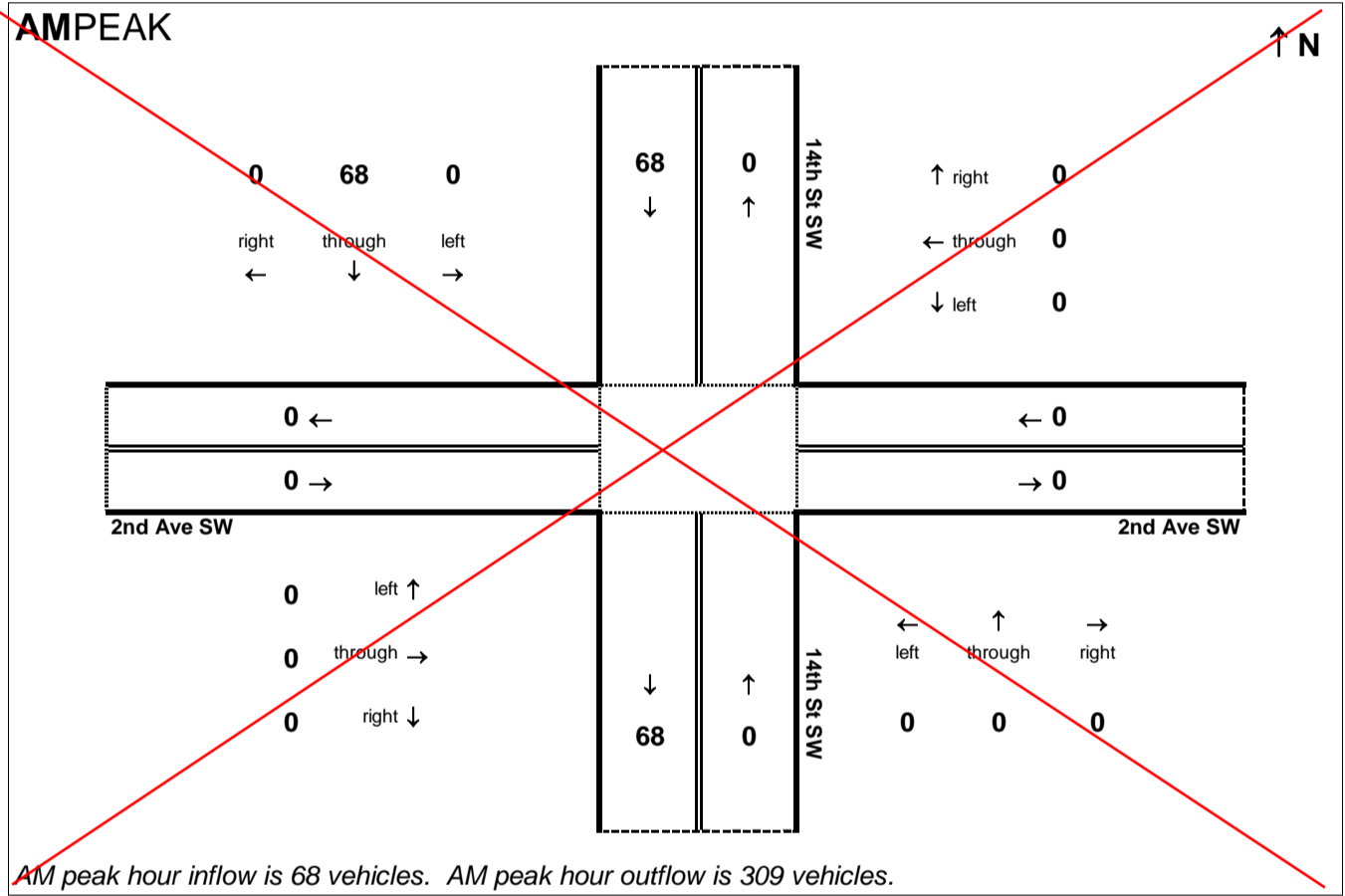
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

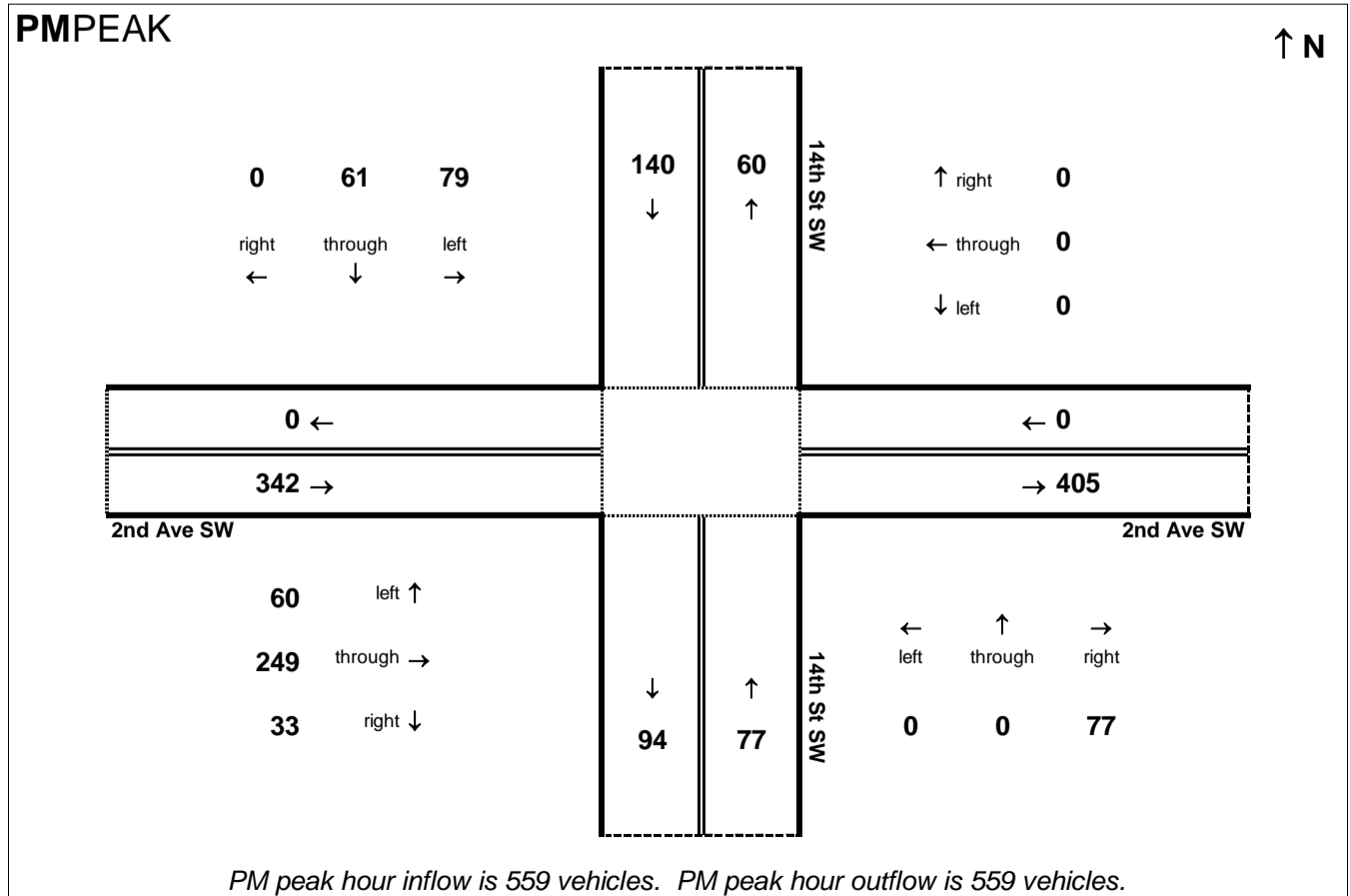
Traffic Data Year:
2040 No Build

Project:
U-4700

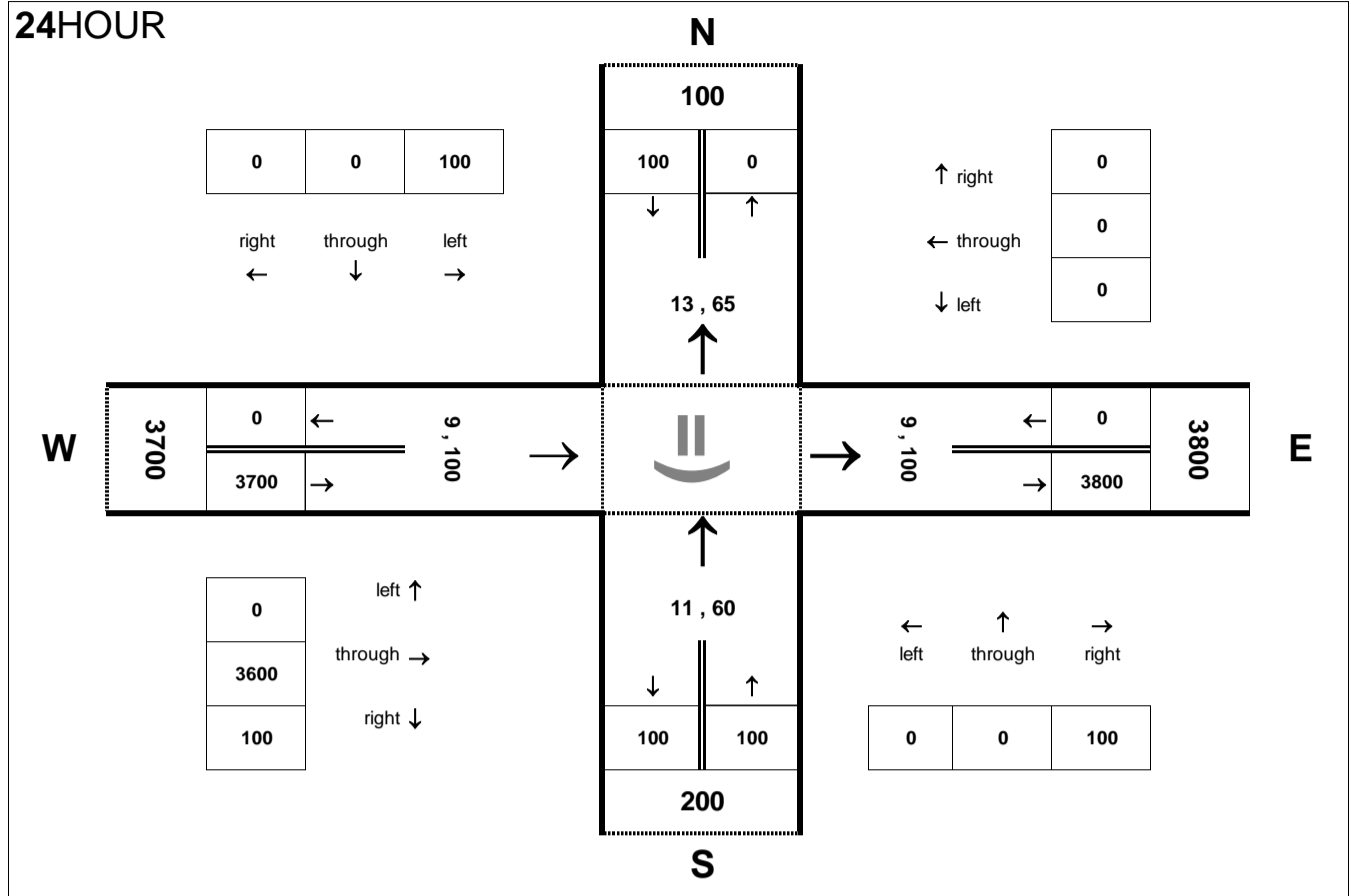
AMPEAK



PMPEAK



24HOUR



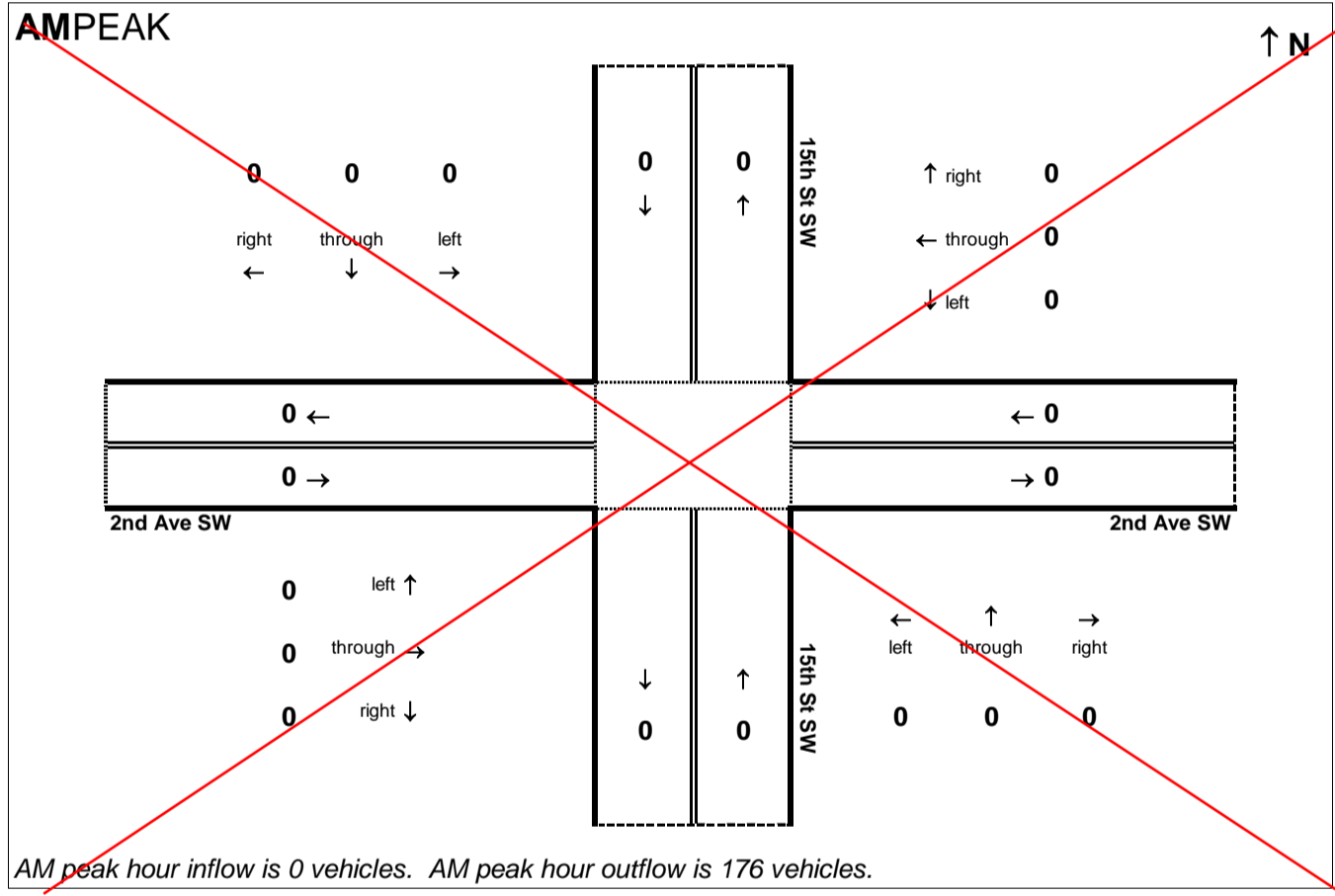
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

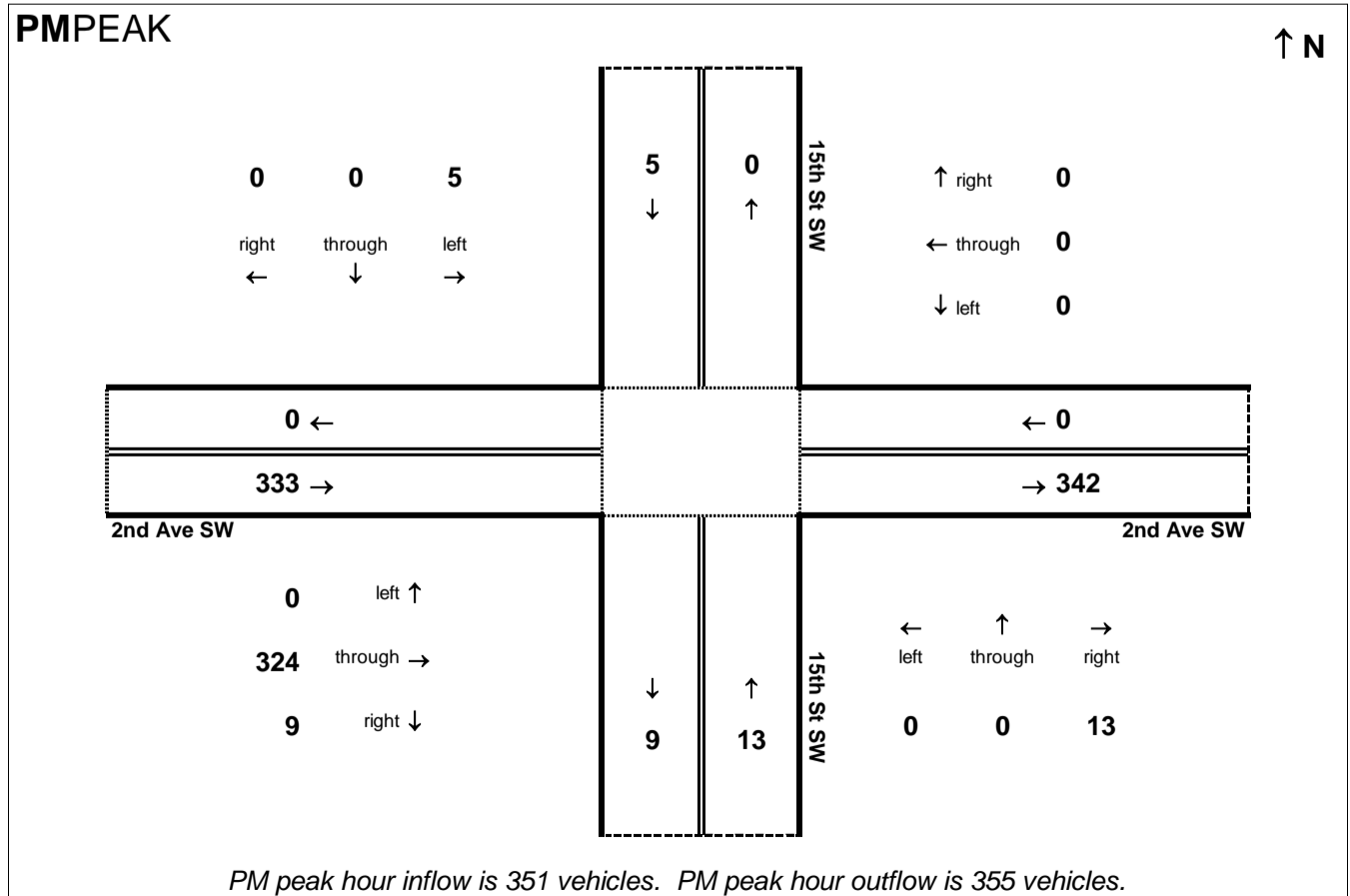
Project:
U-4700

AMPEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 176 vehicles.

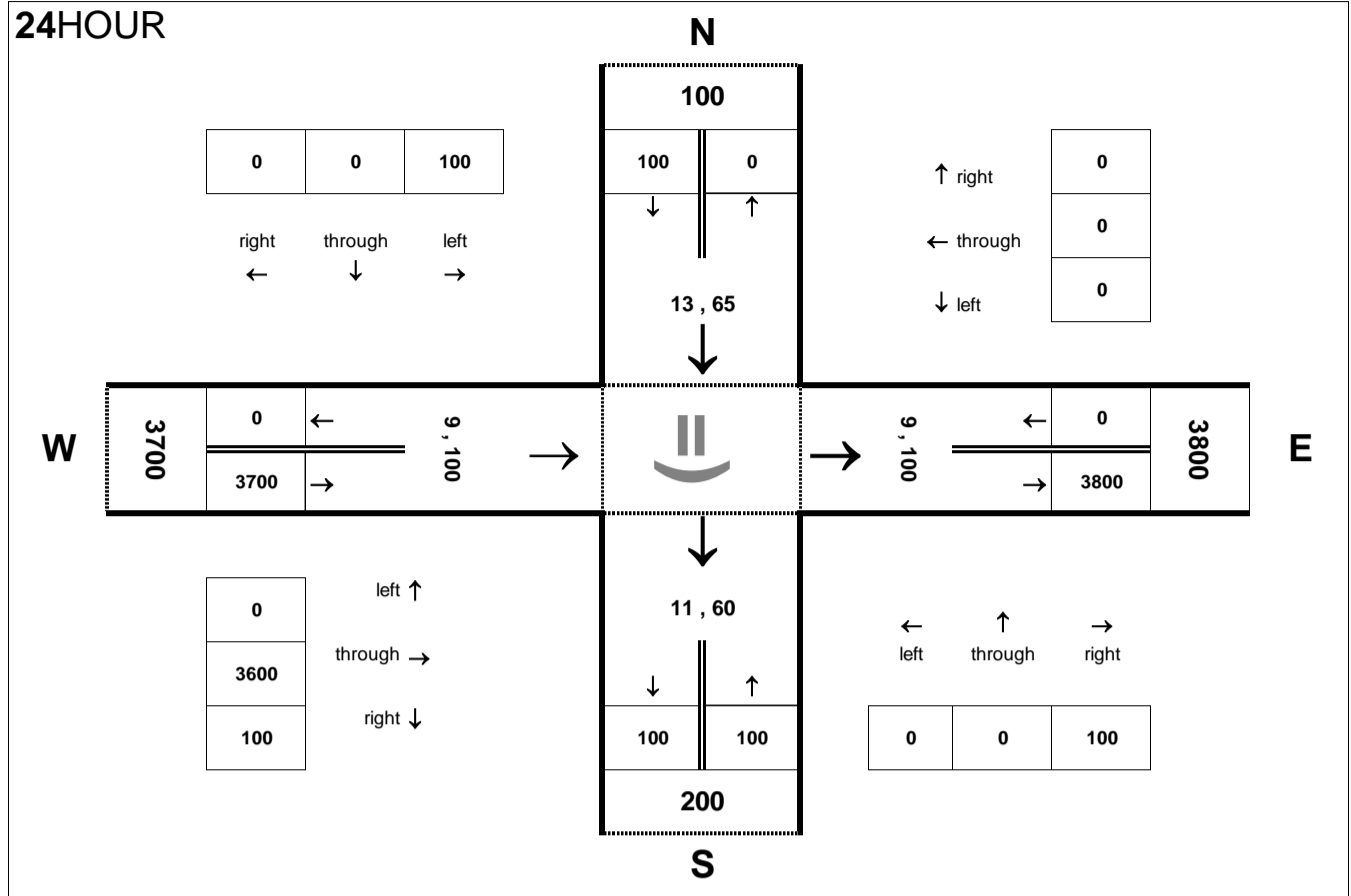
PMPEAK



PM peak hour inflow is 351 vehicles. PM peak hour outflow is 355 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



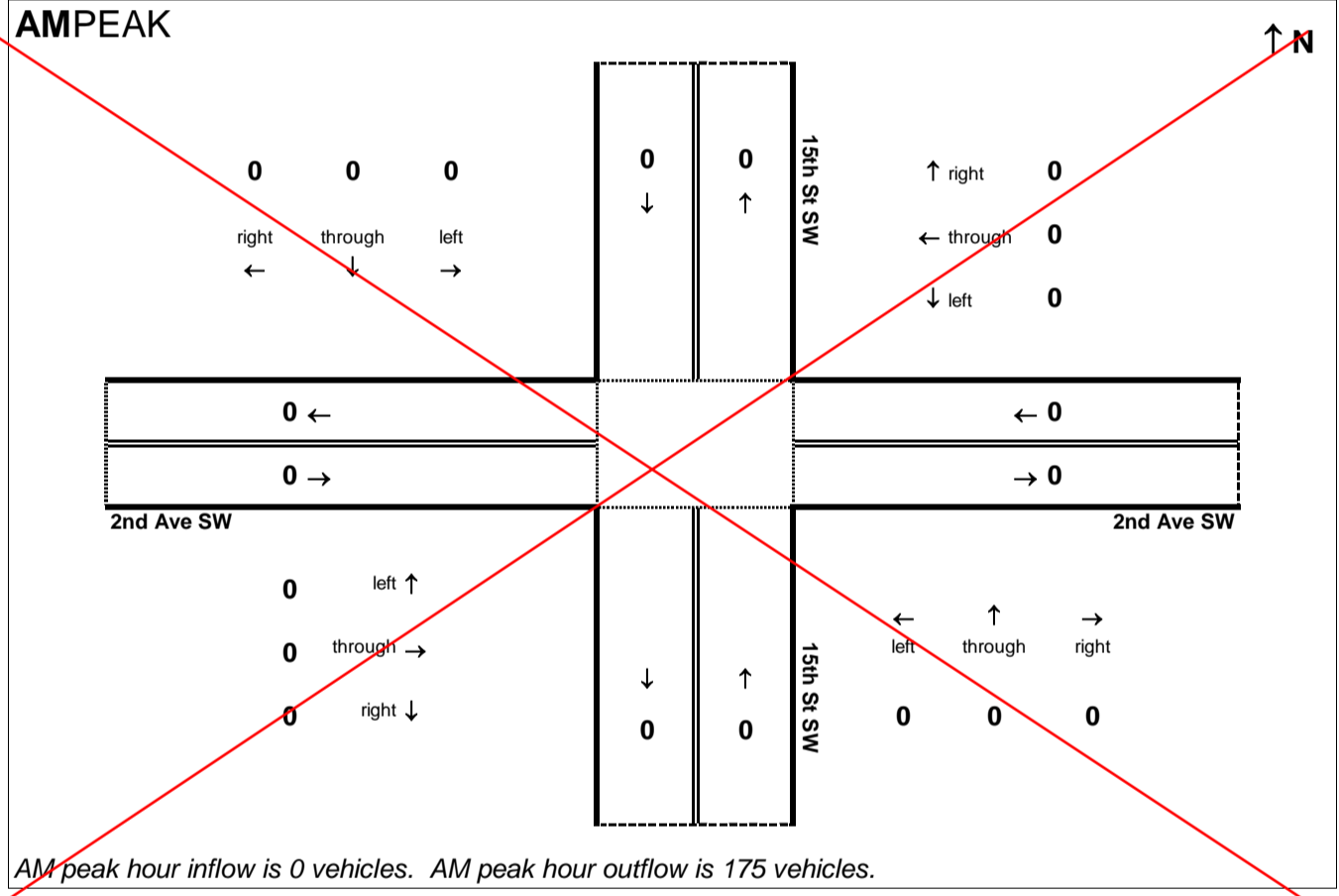
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

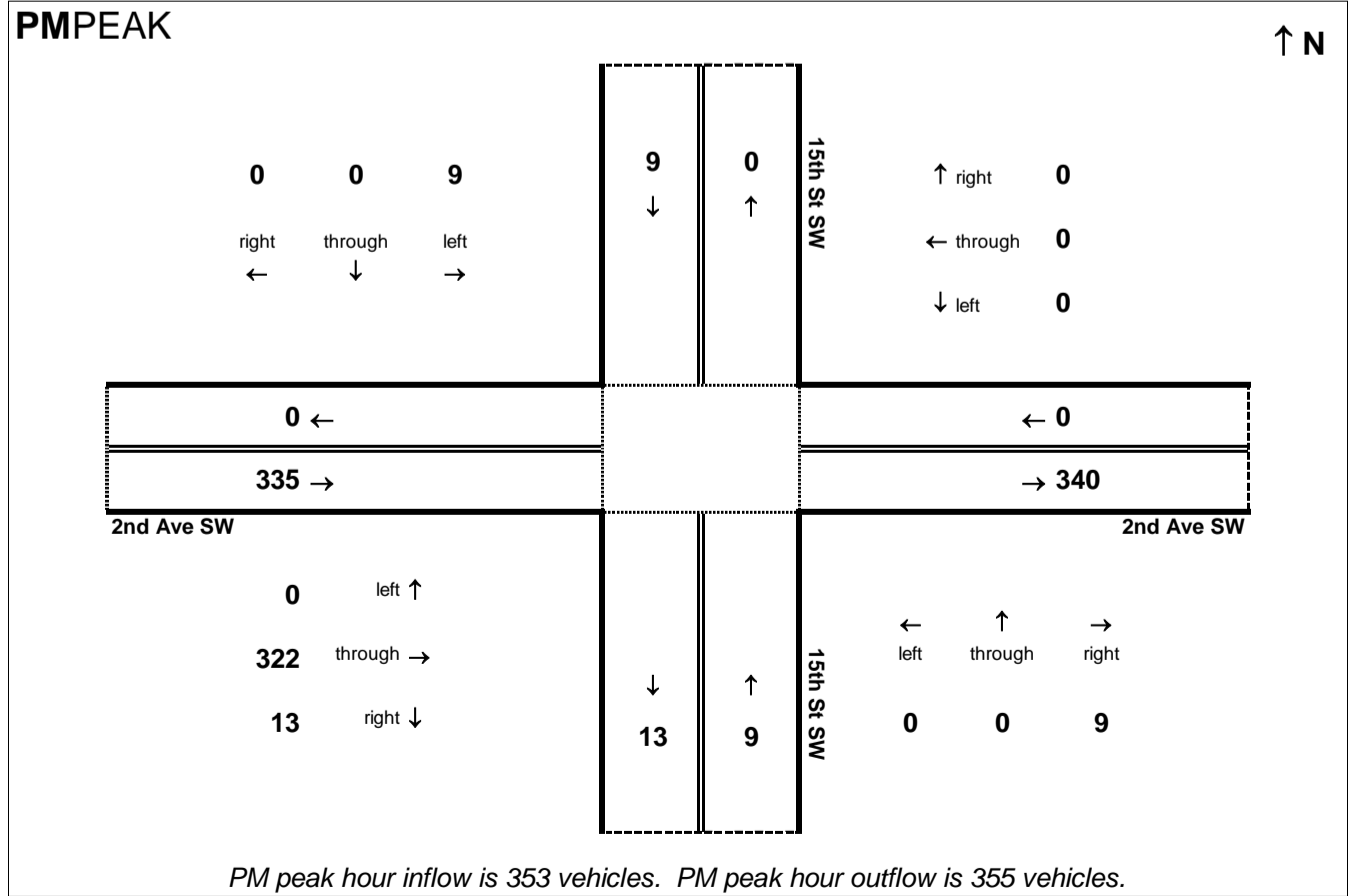
Traffic Data Year:
2040 No Build

Project:
U-4700

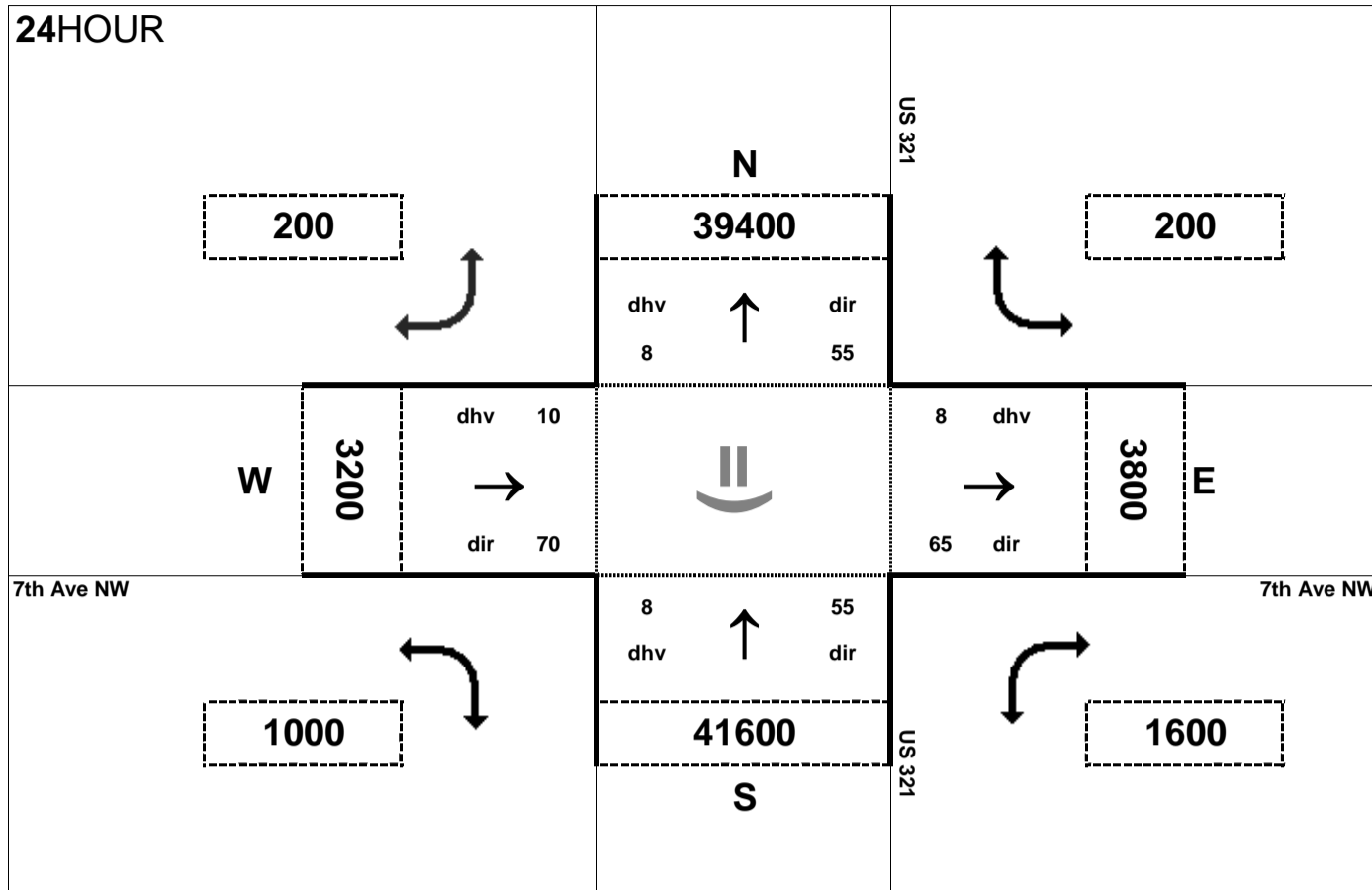
AMPEAK



PMPEAK



24HOUR



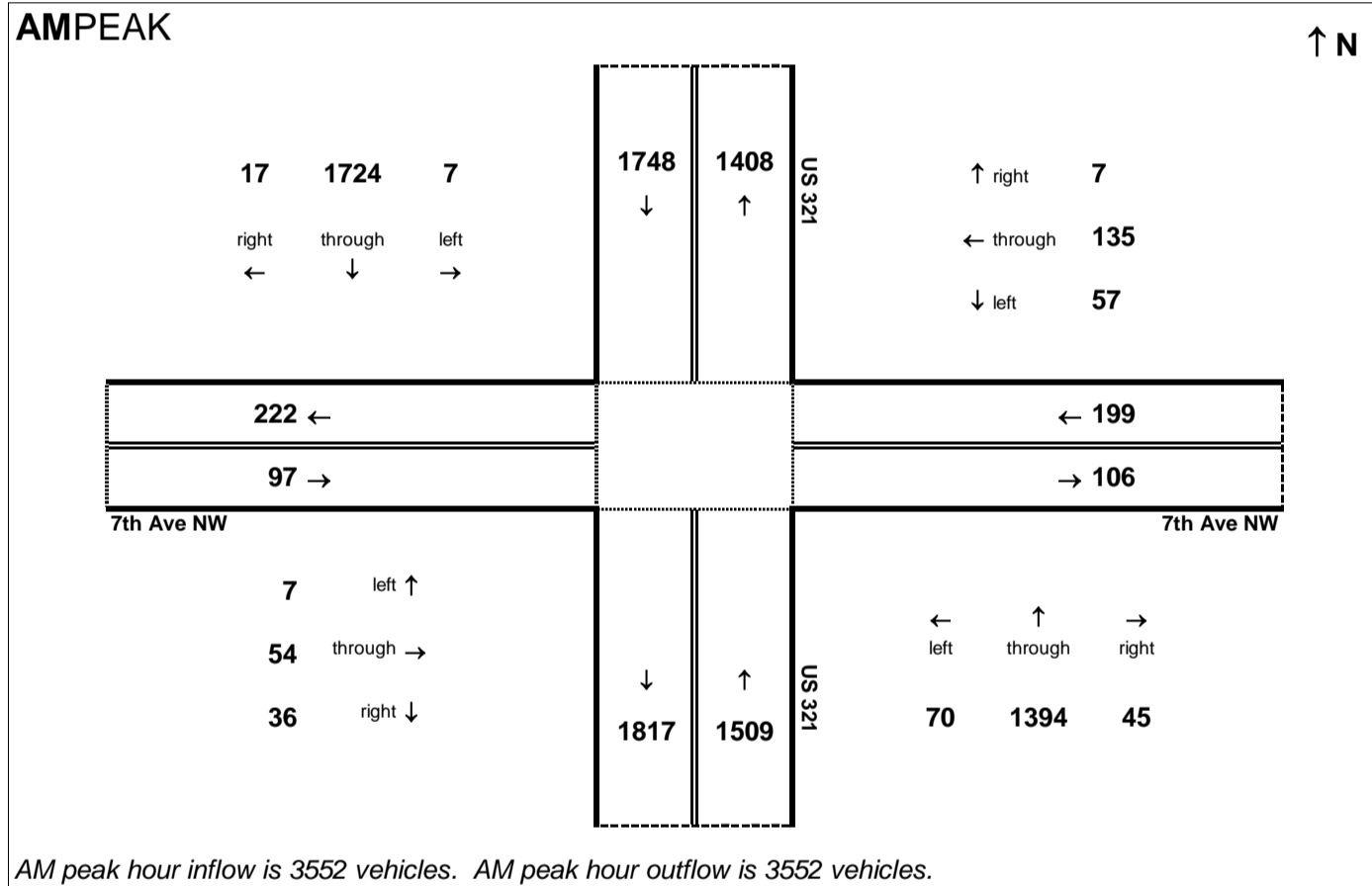
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 7th Ave NW

Traffic Forecast Release Date:
February-17

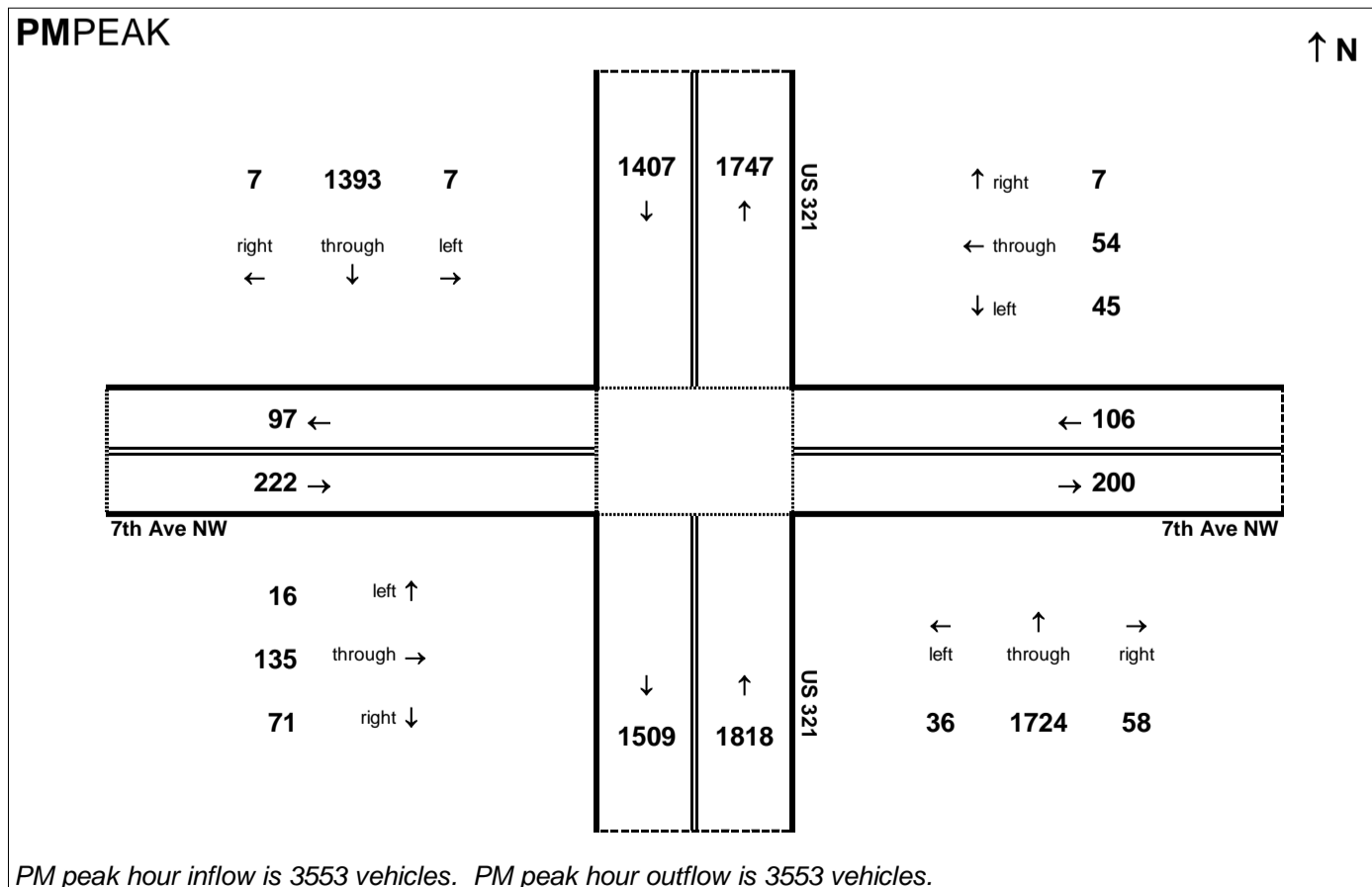
Traffic Data Year:
2040 No Build

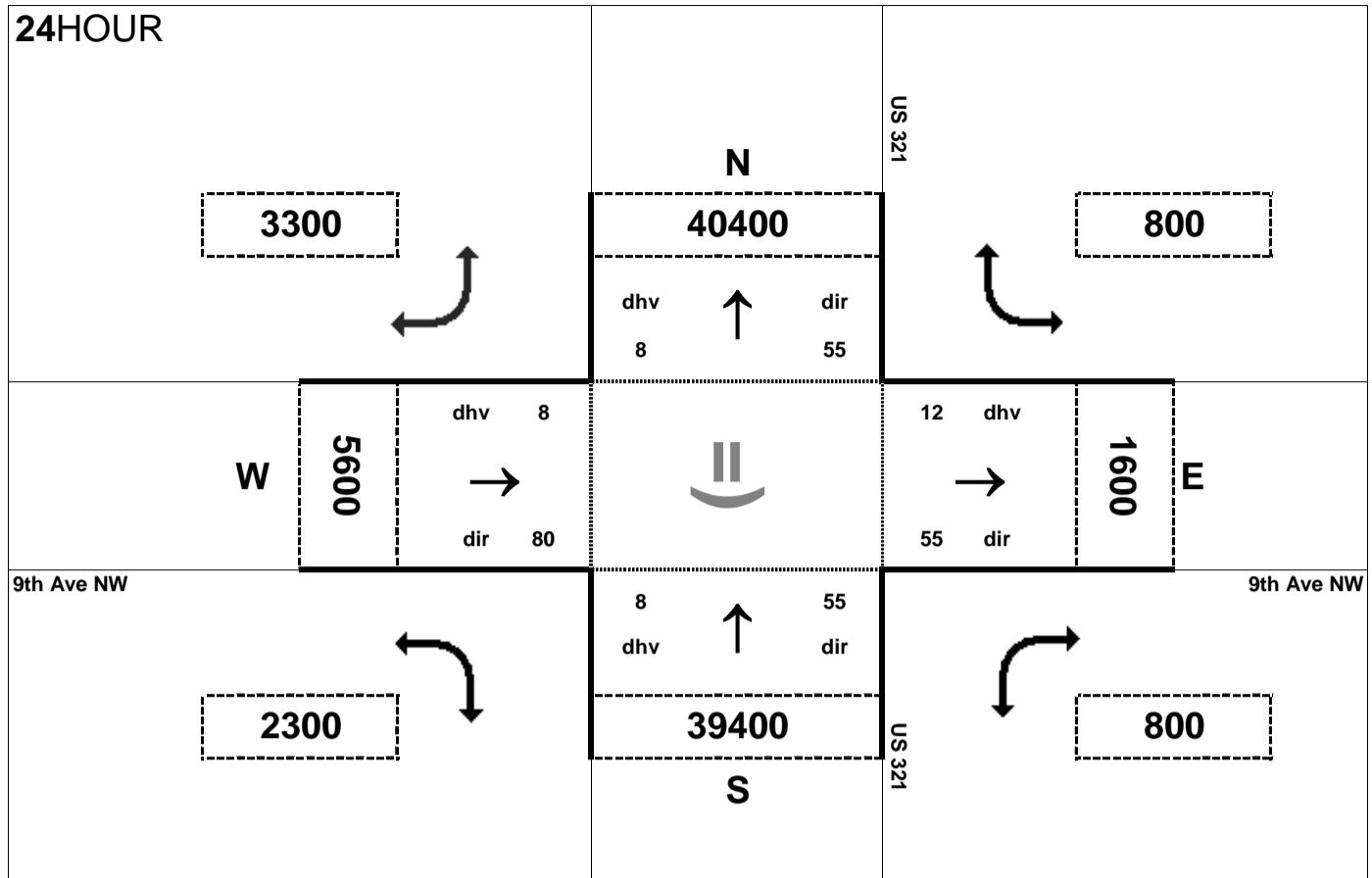
Project:
U-4700

AMPEAK



PMPEAK



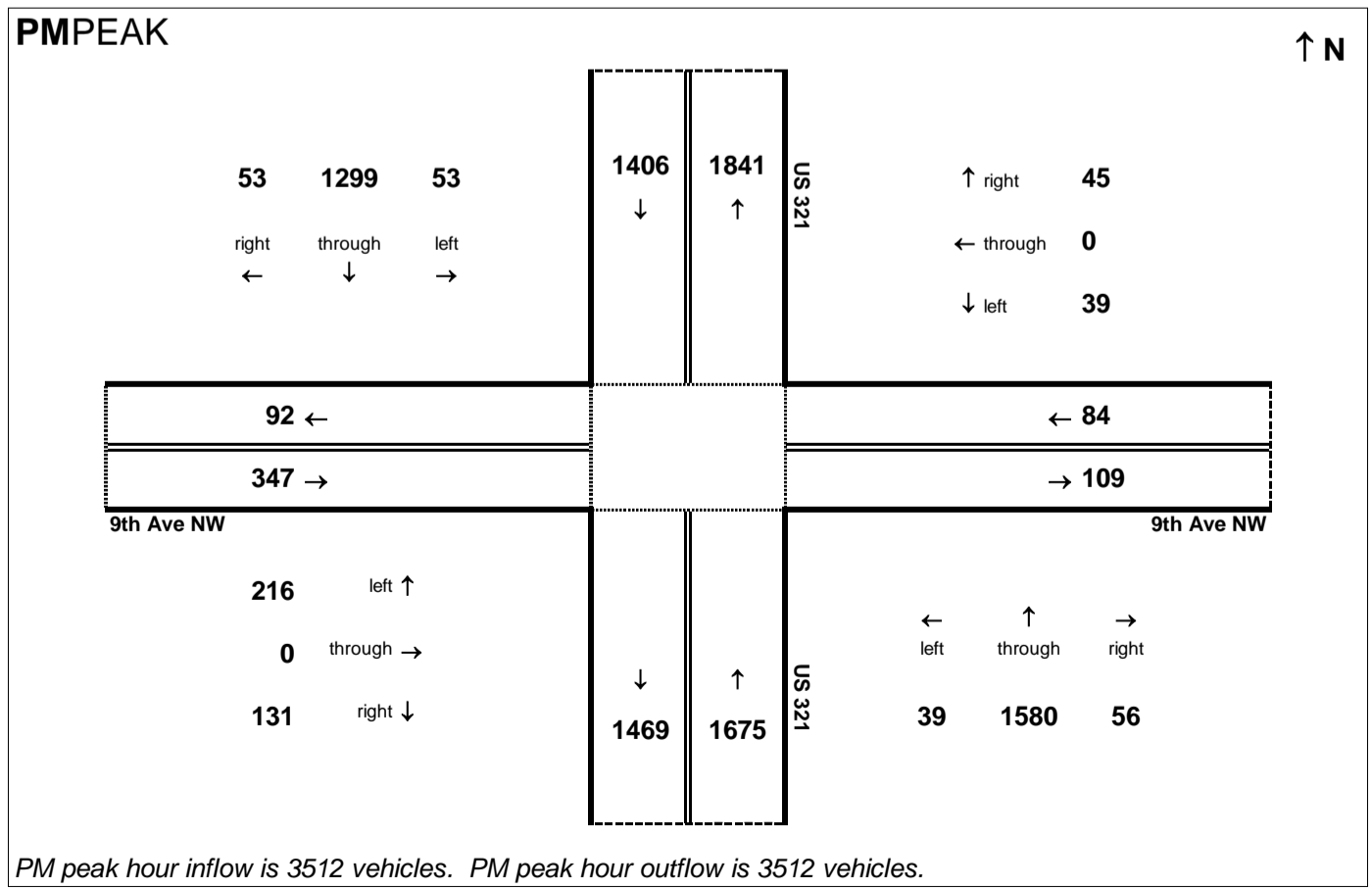
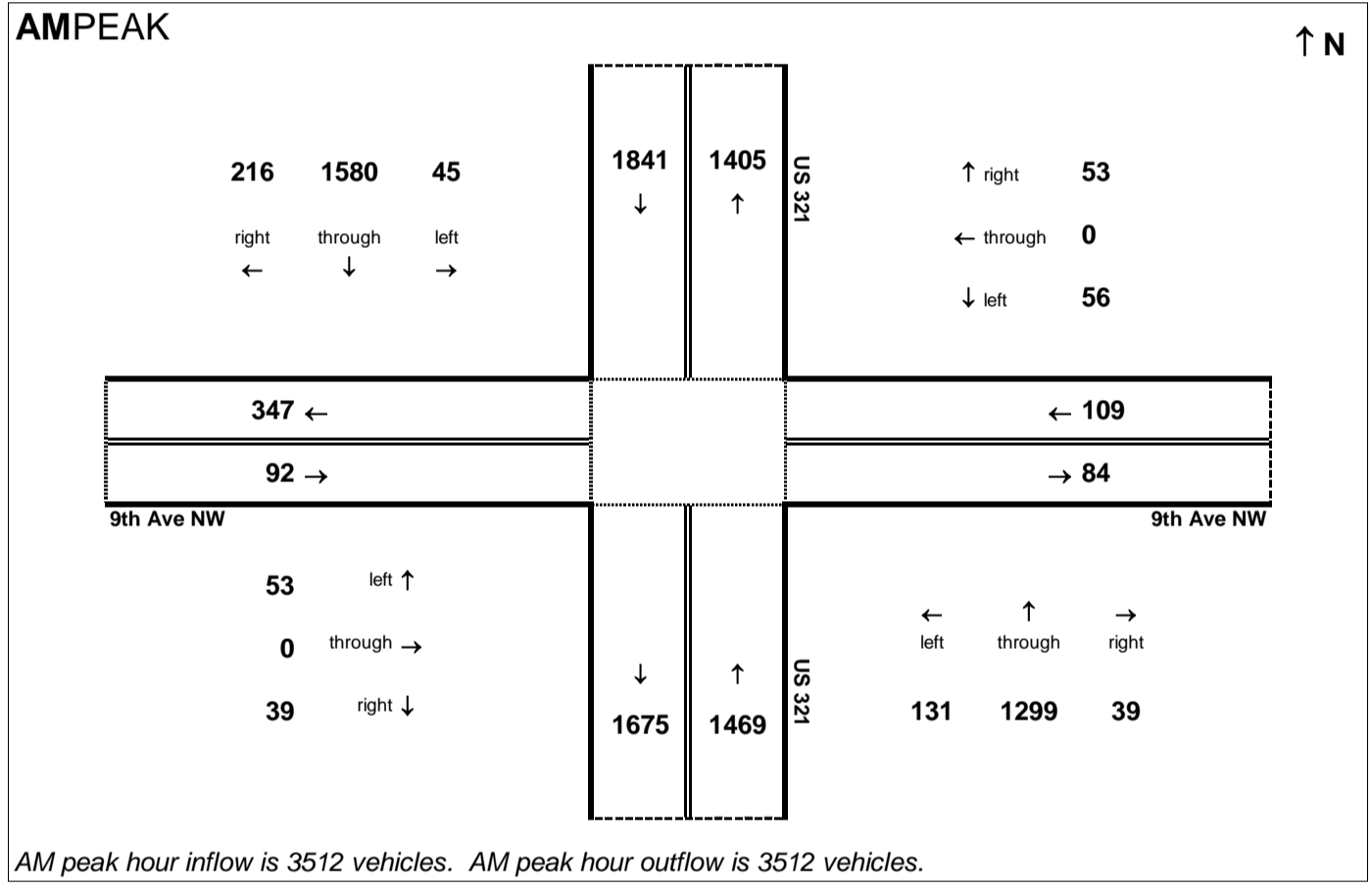


Peak Hour Volume Breakouts Report:
Intersection of US 321 and 9th Ave NW

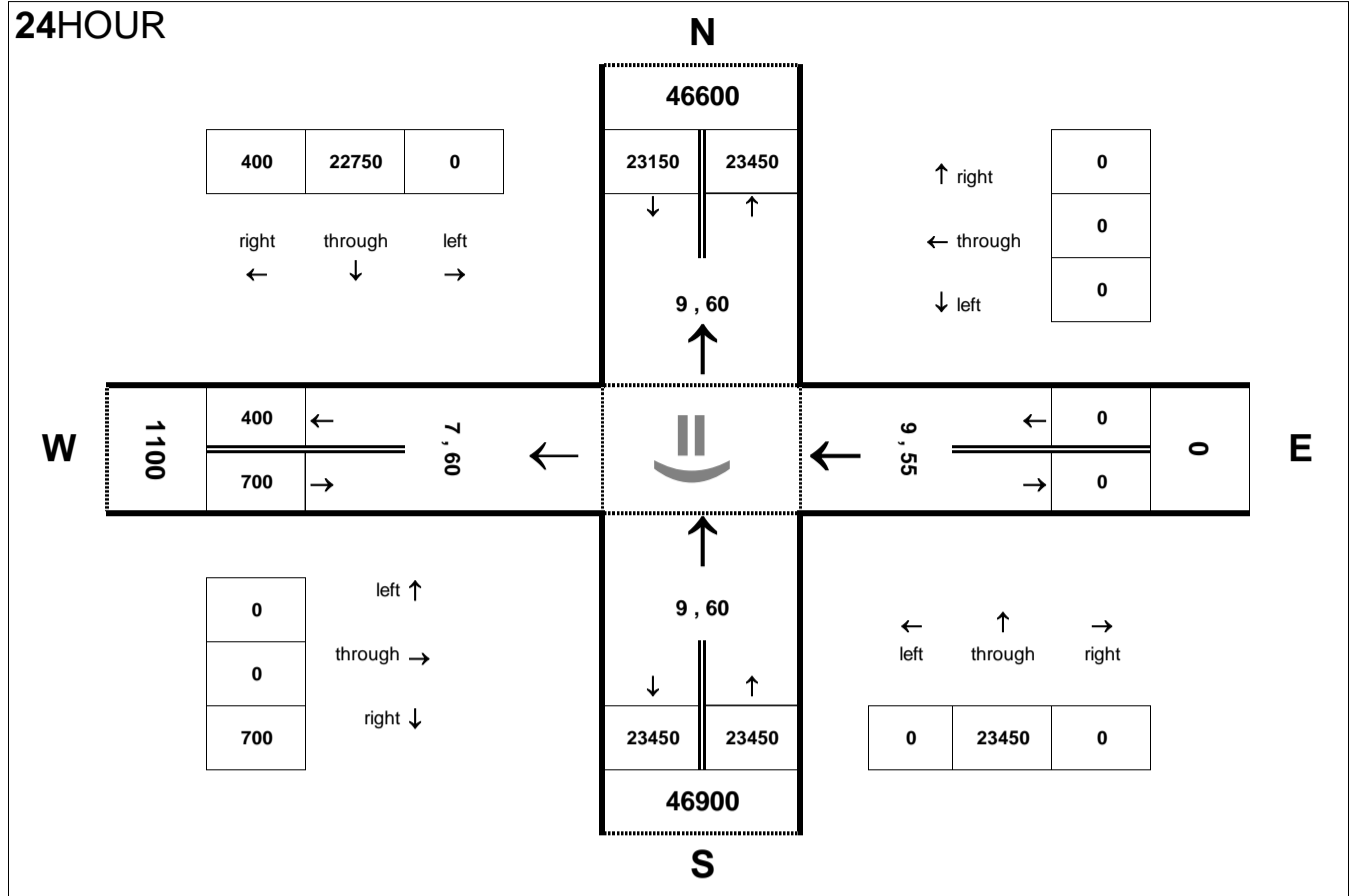
Traffic Forecast Release Date:
February-17

Traffic Data Year:
2040 No Build

Project:
U-4700



24HOUR



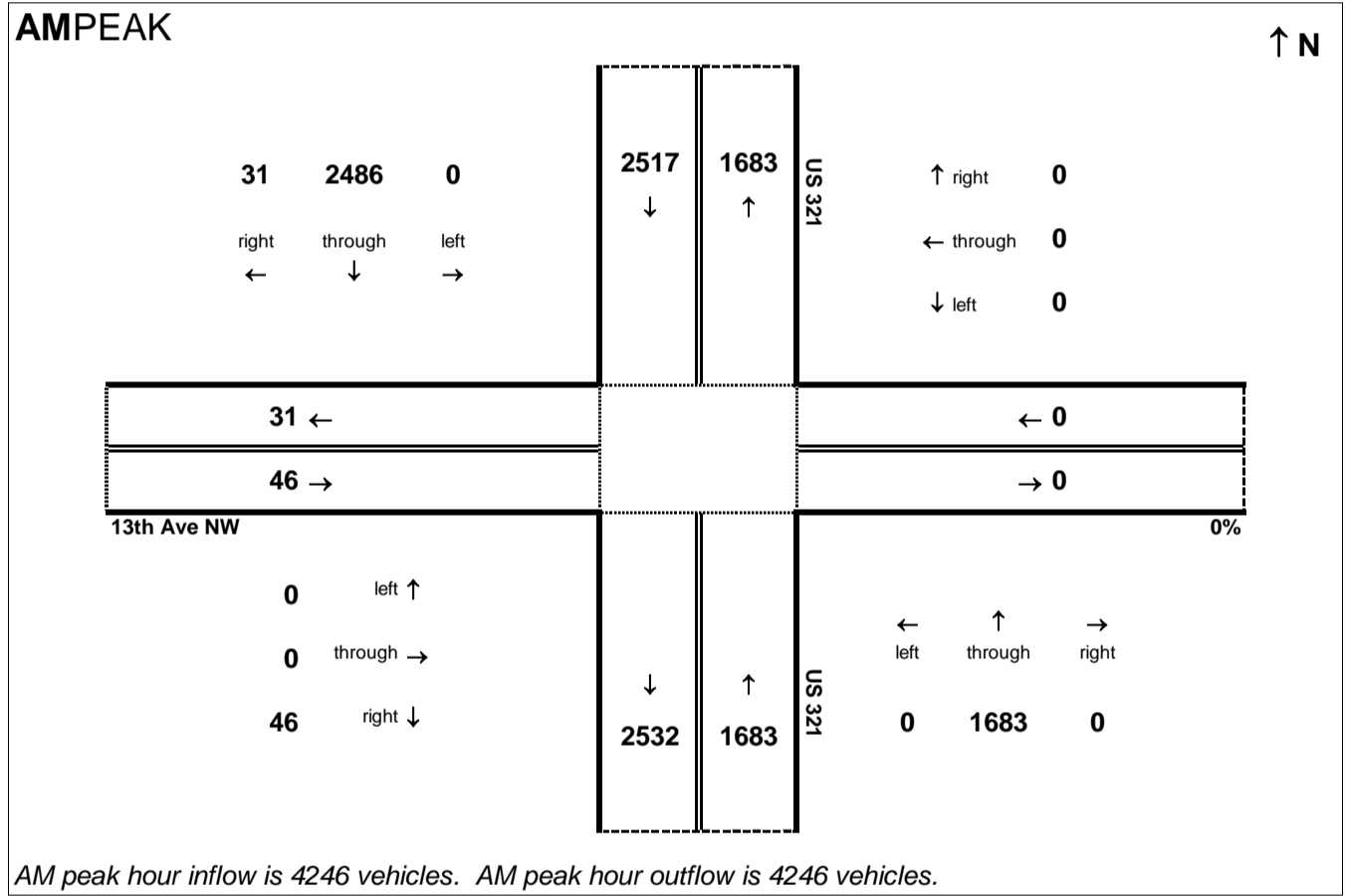
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 13th Ave NW

Traffic Forecast Release Date:
February-17

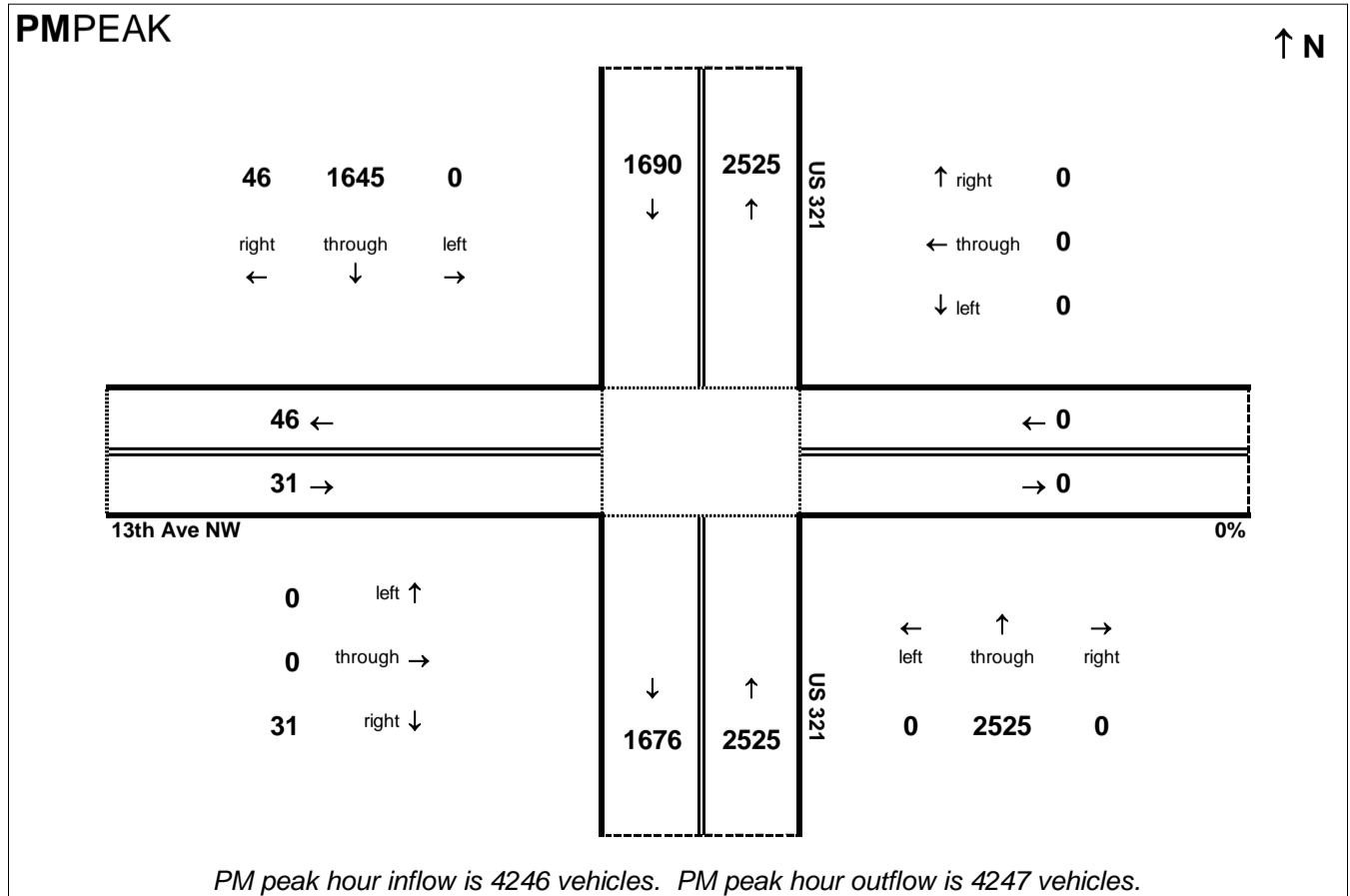
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2040 No Build

Project:
U-4700

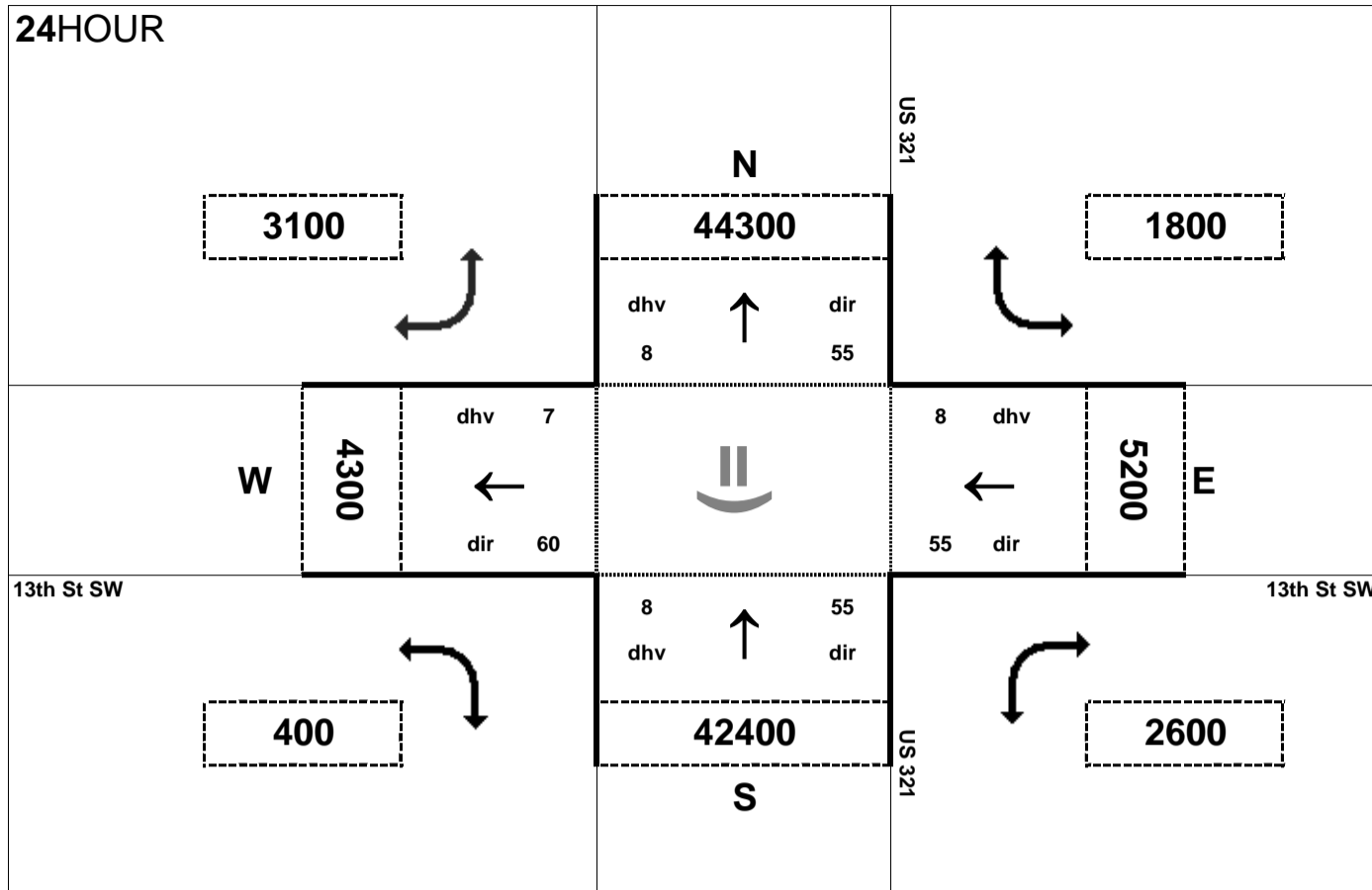
AMPEAK



PMPEAK



24HOUR



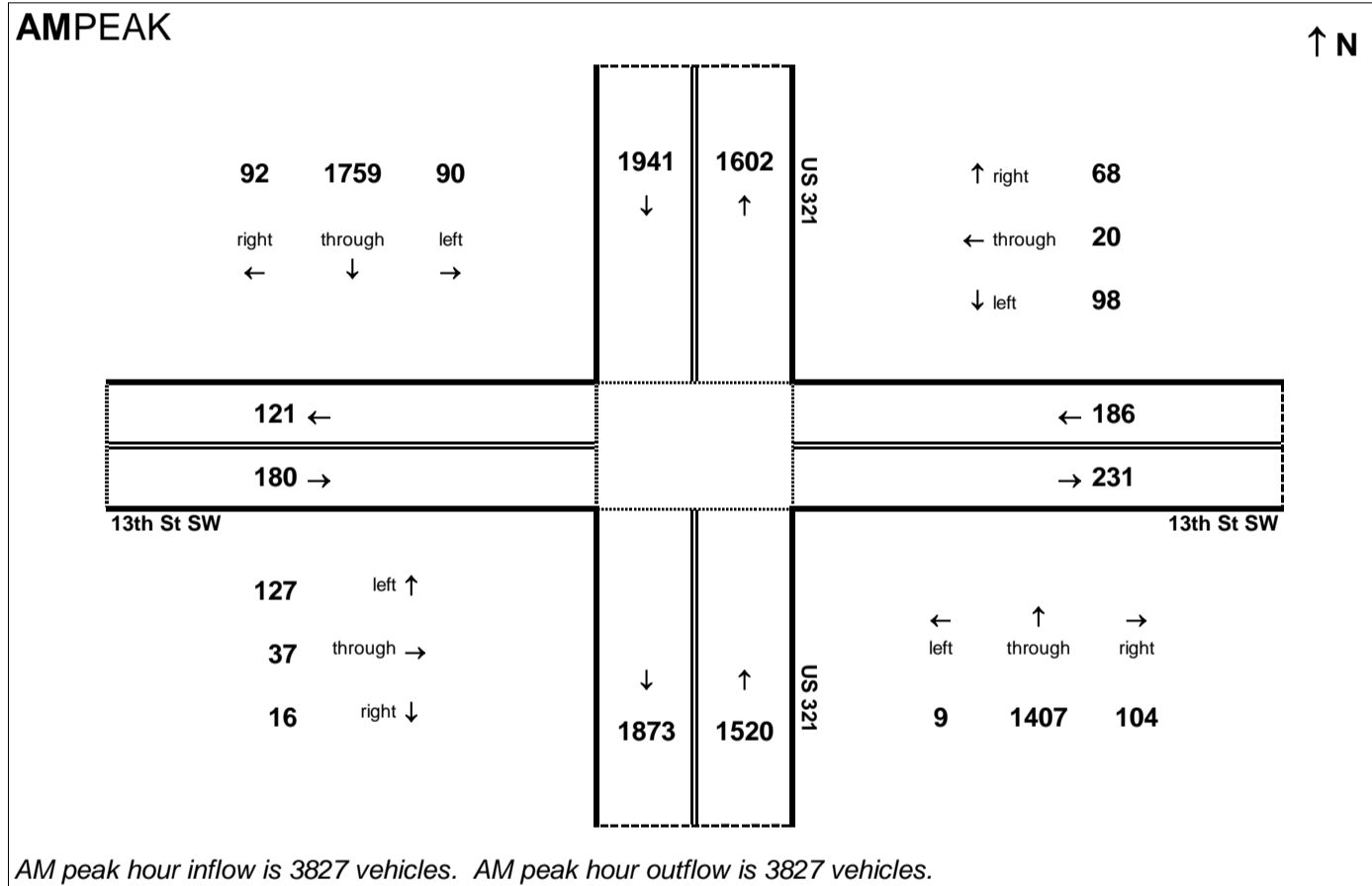
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 13th St SW

Traffic Forecast Release Date:
December-16

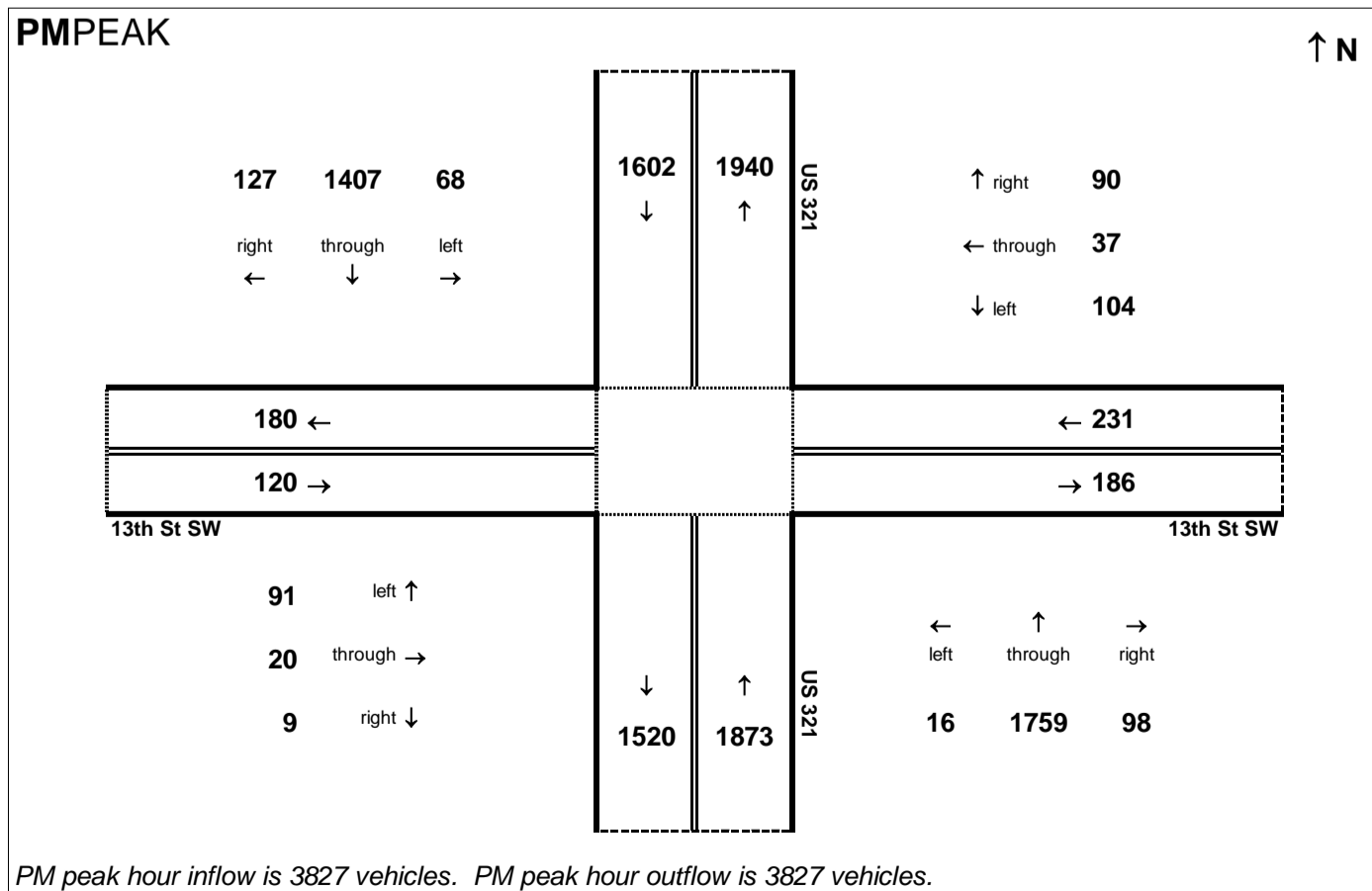
Traffic Data Year:
2040 No Build

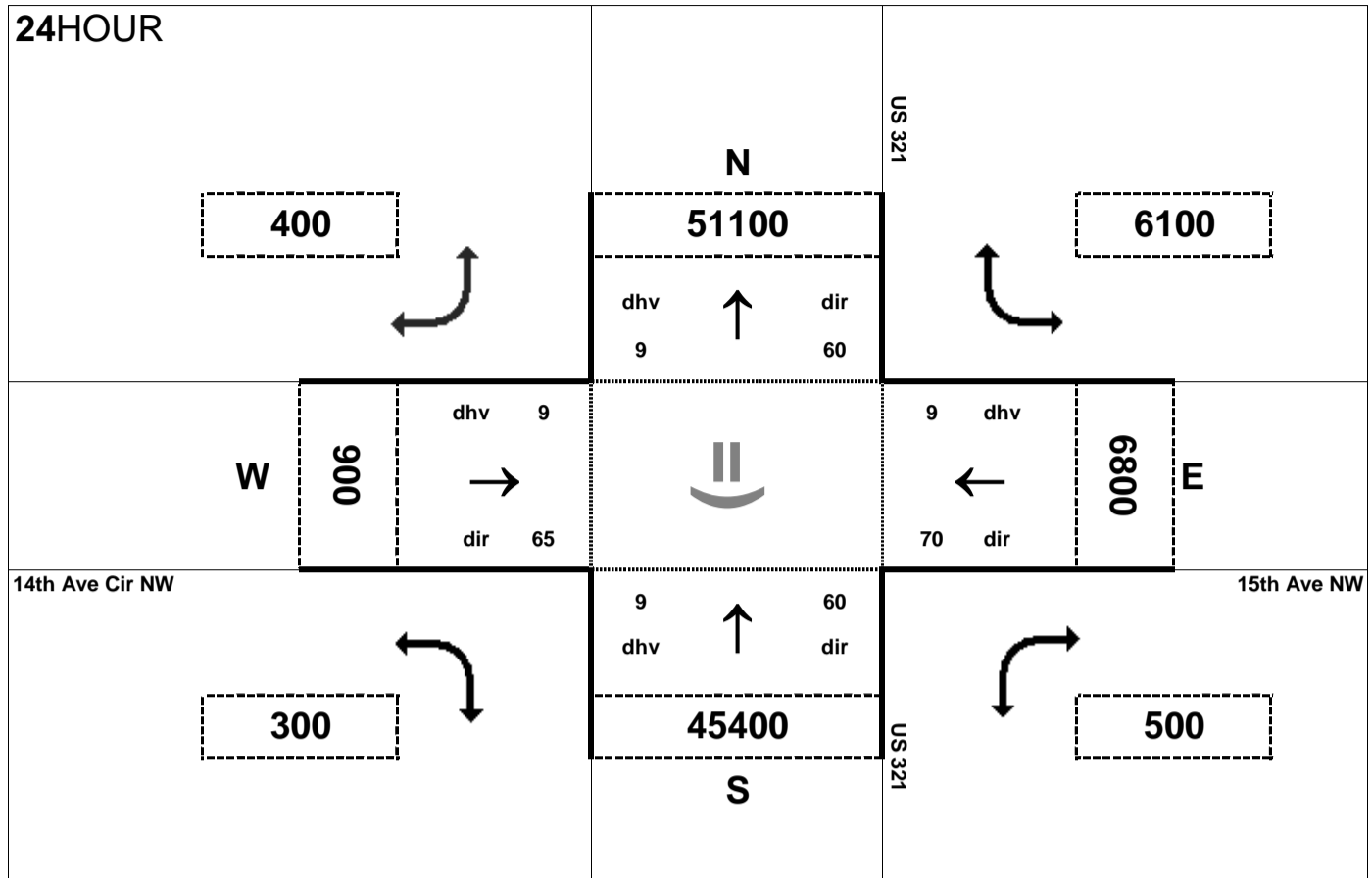
Project:
U-4700

AMPEAK



PMPEAK



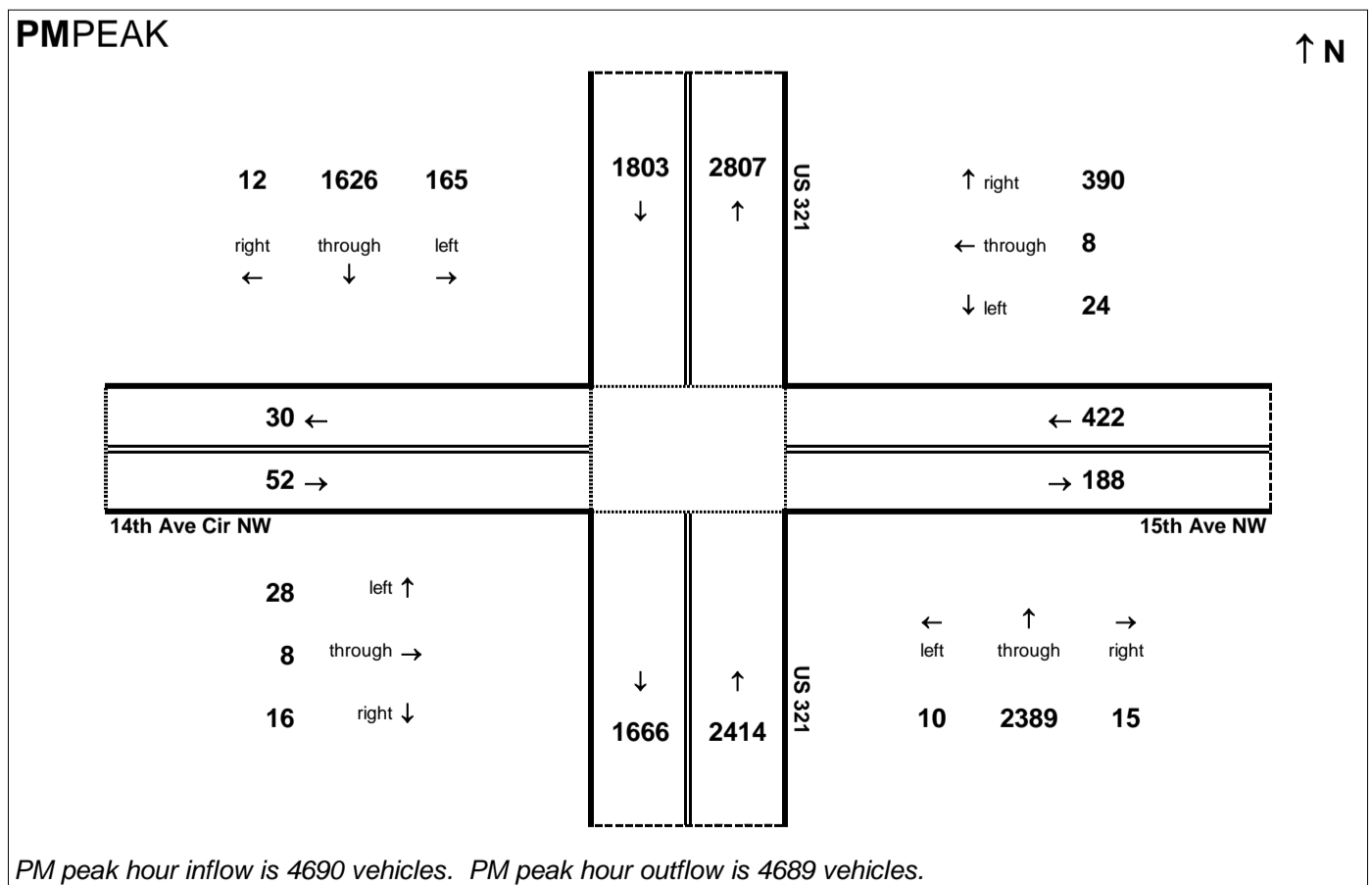
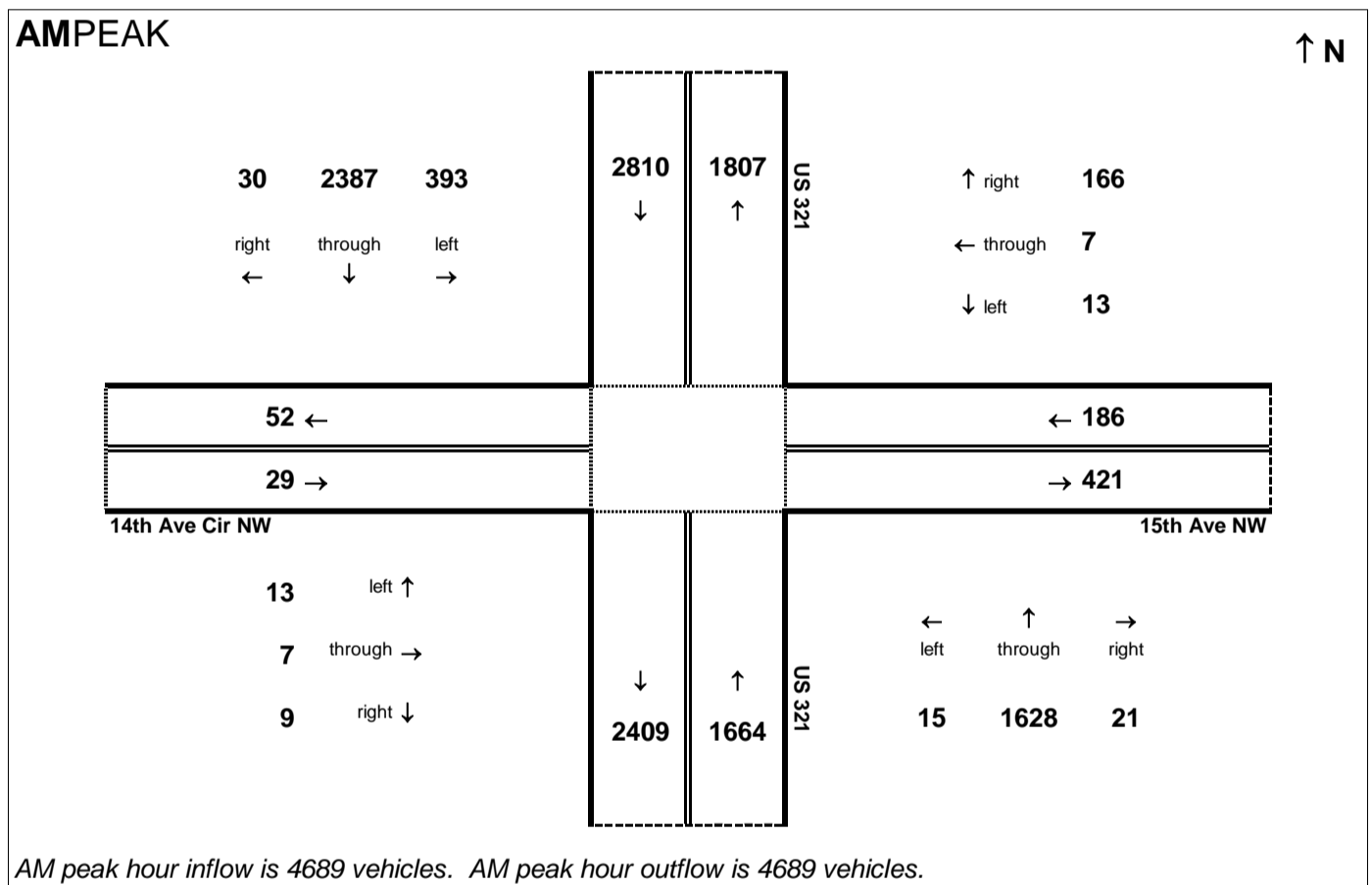


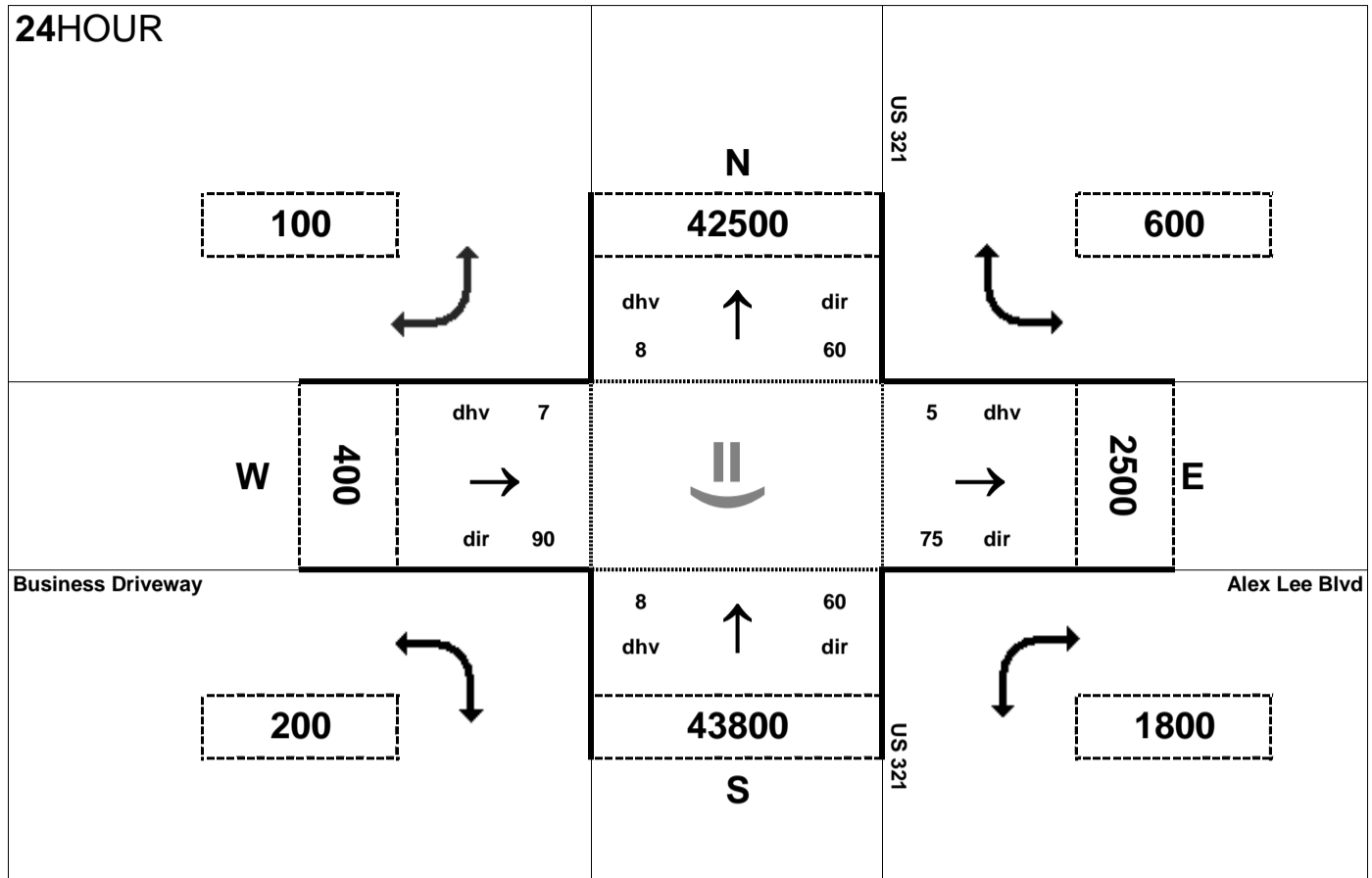
Peak Hour Volume Breakouts Report:
 Intersection of US 321 and 15th Ave NW / 14th Ave Cir NW

Traffic Forecast Release Date:
 February-17

Traffic Data Year:
 2040 No Build

Project:
 U-4700



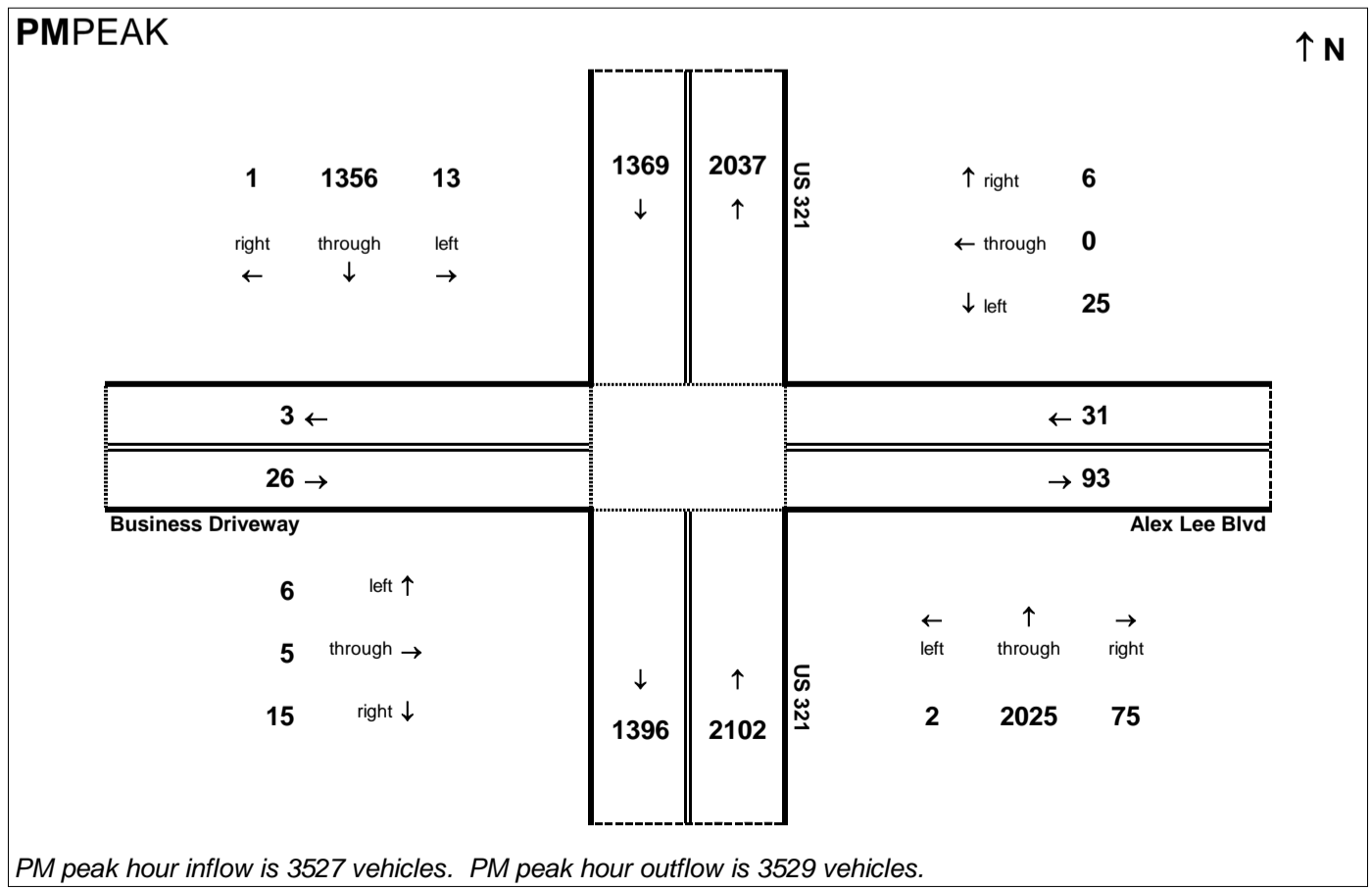
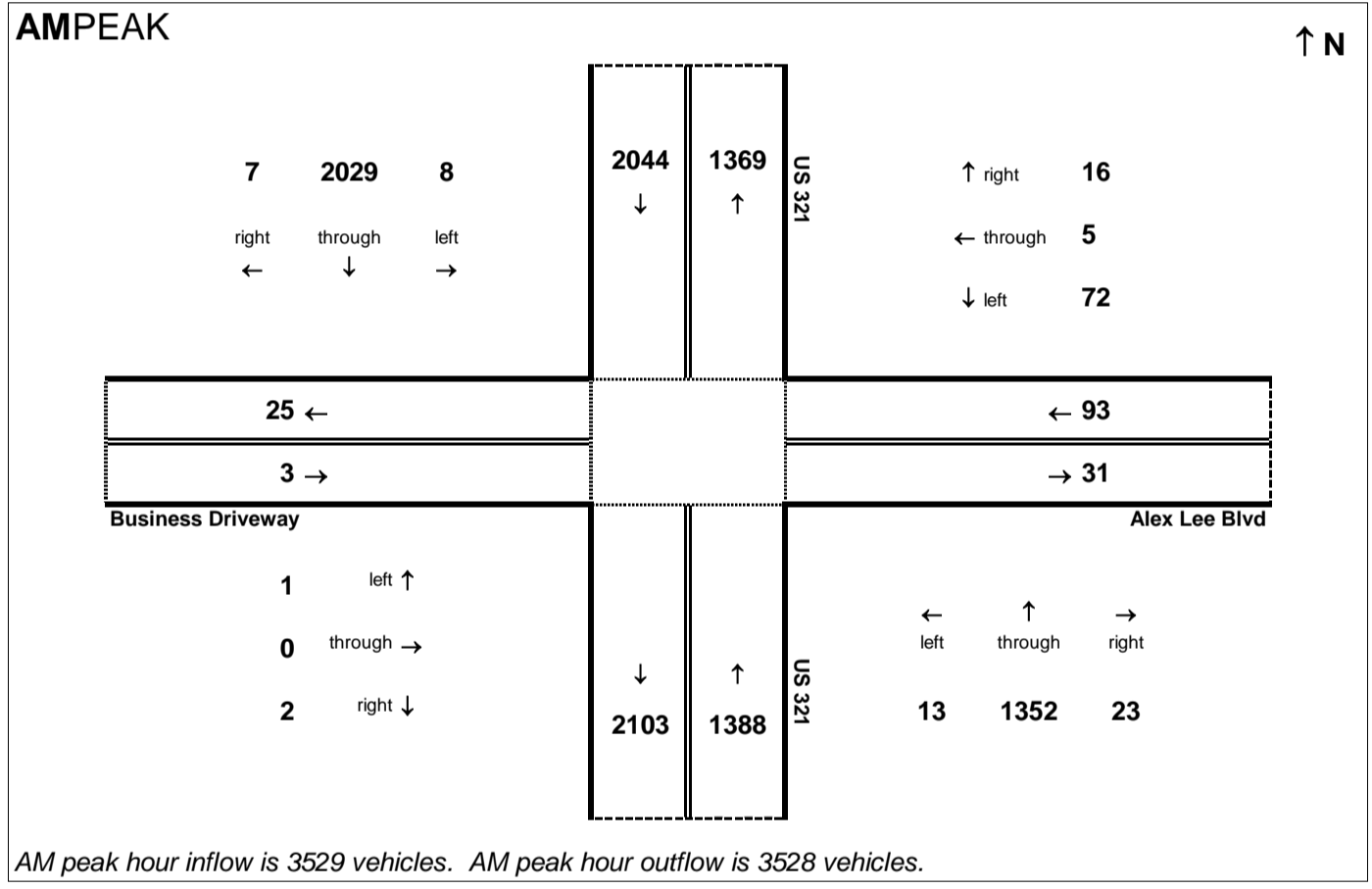


Peak Hour Volume Breakouts Report:
Intersection of US 321 and Alex Lee Blvd

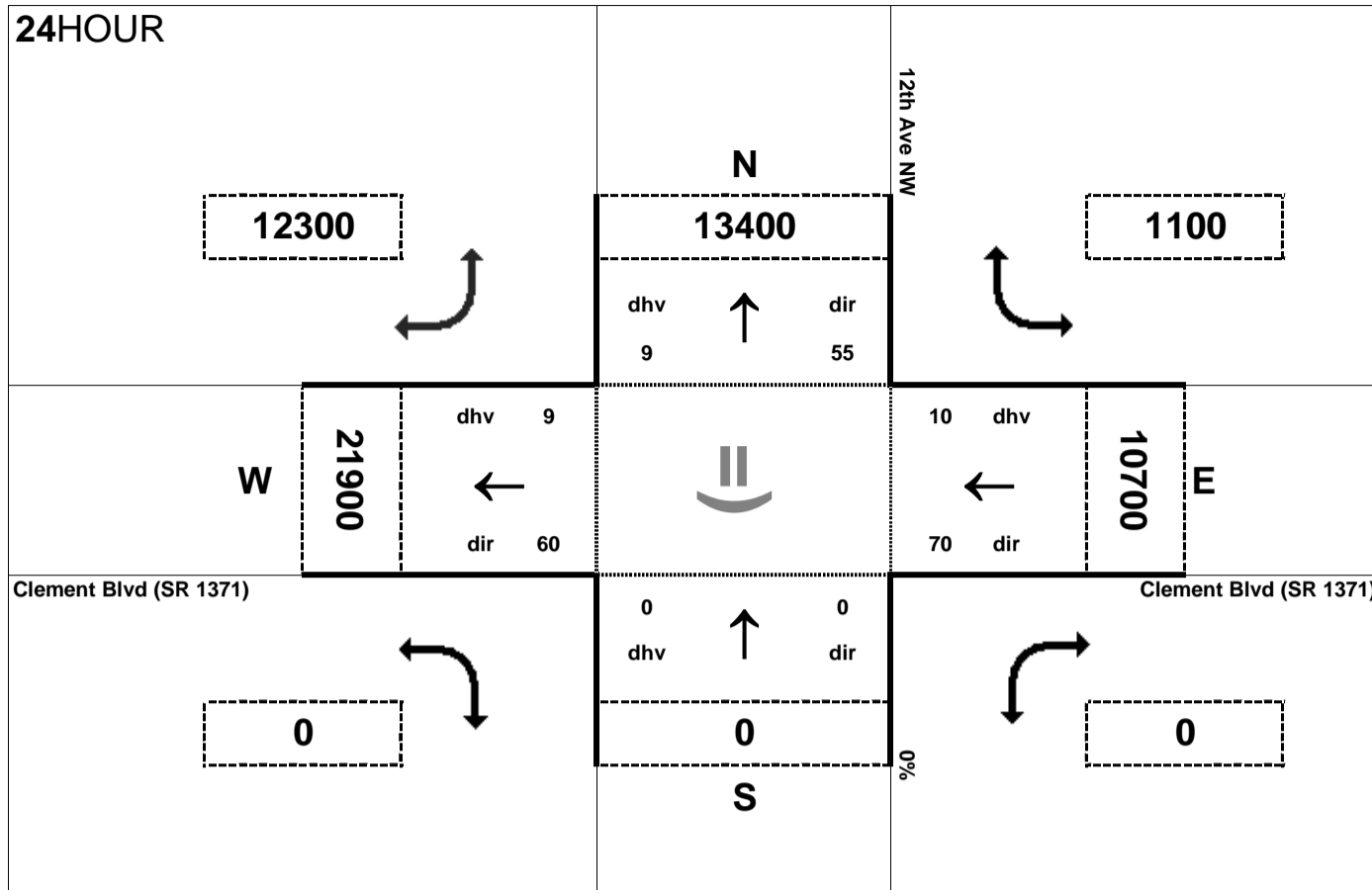
Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

Project:
U-4700



24HOUR



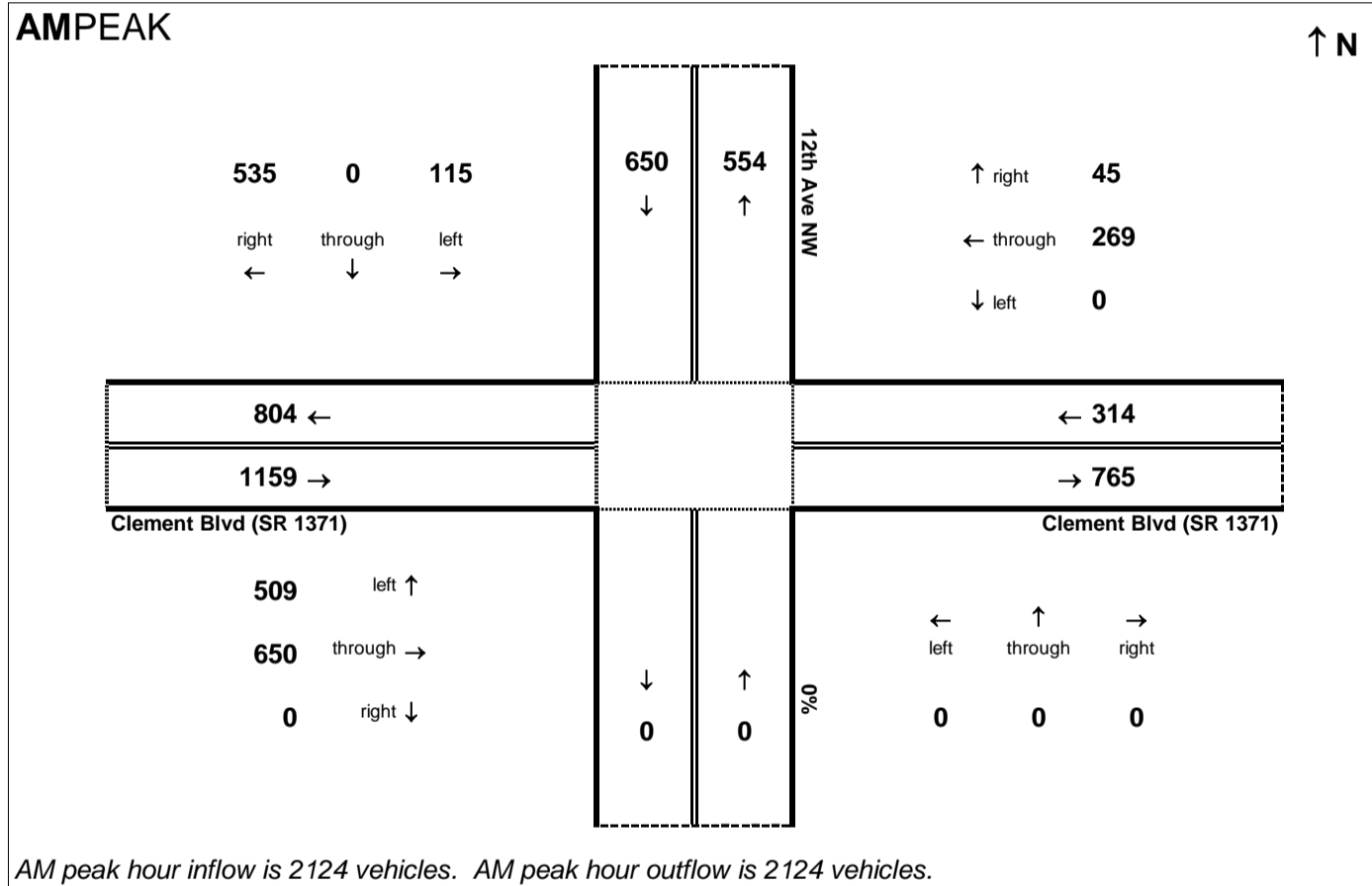
Peak Hour Volume Breakouts Report:
Intersection of Clement Blvd and 12th Ave NW

Traffic Forecast Release Date:
December-16

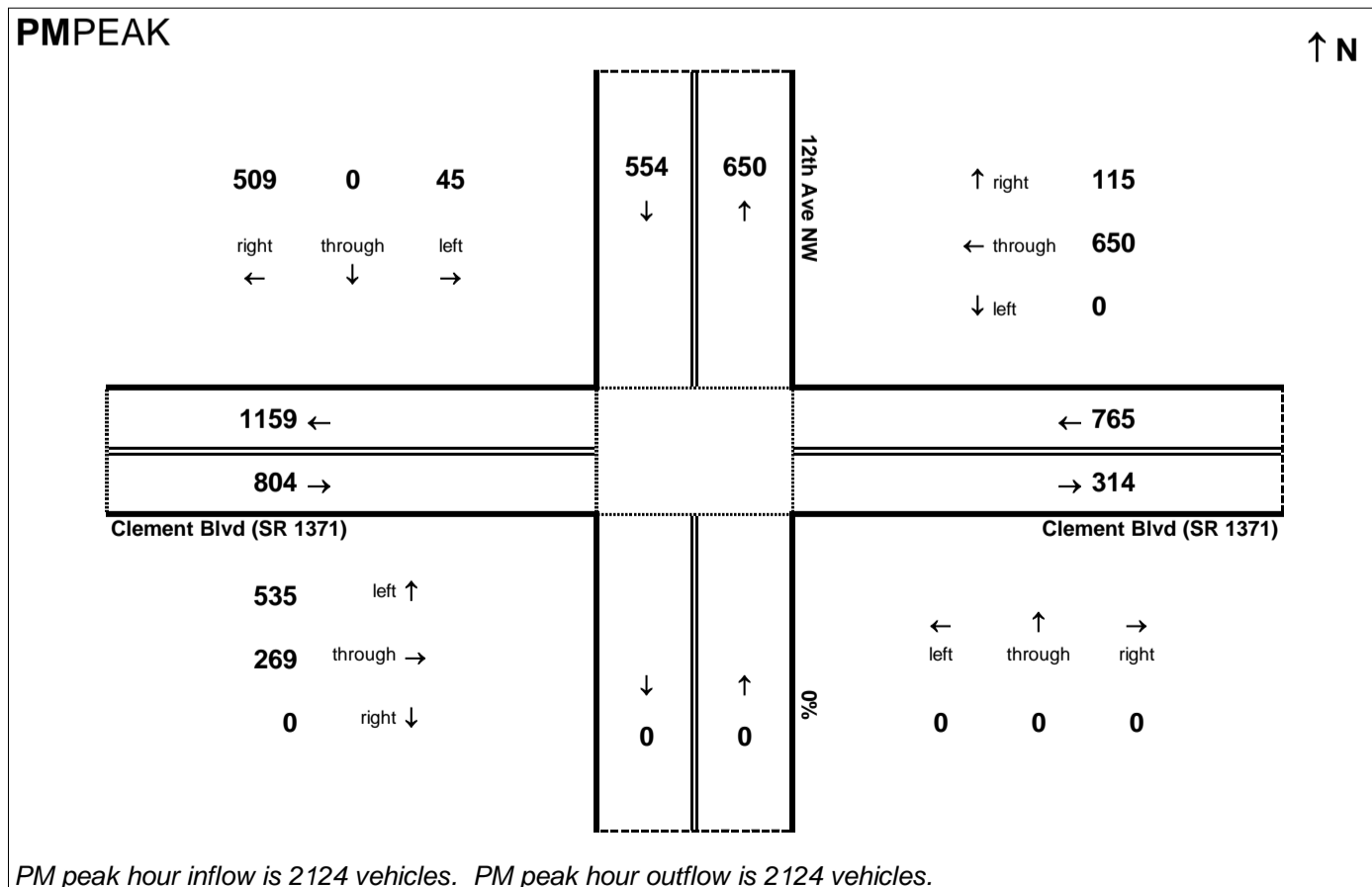
Traffic Data Year:
2040 No Build

Project:
U-4700

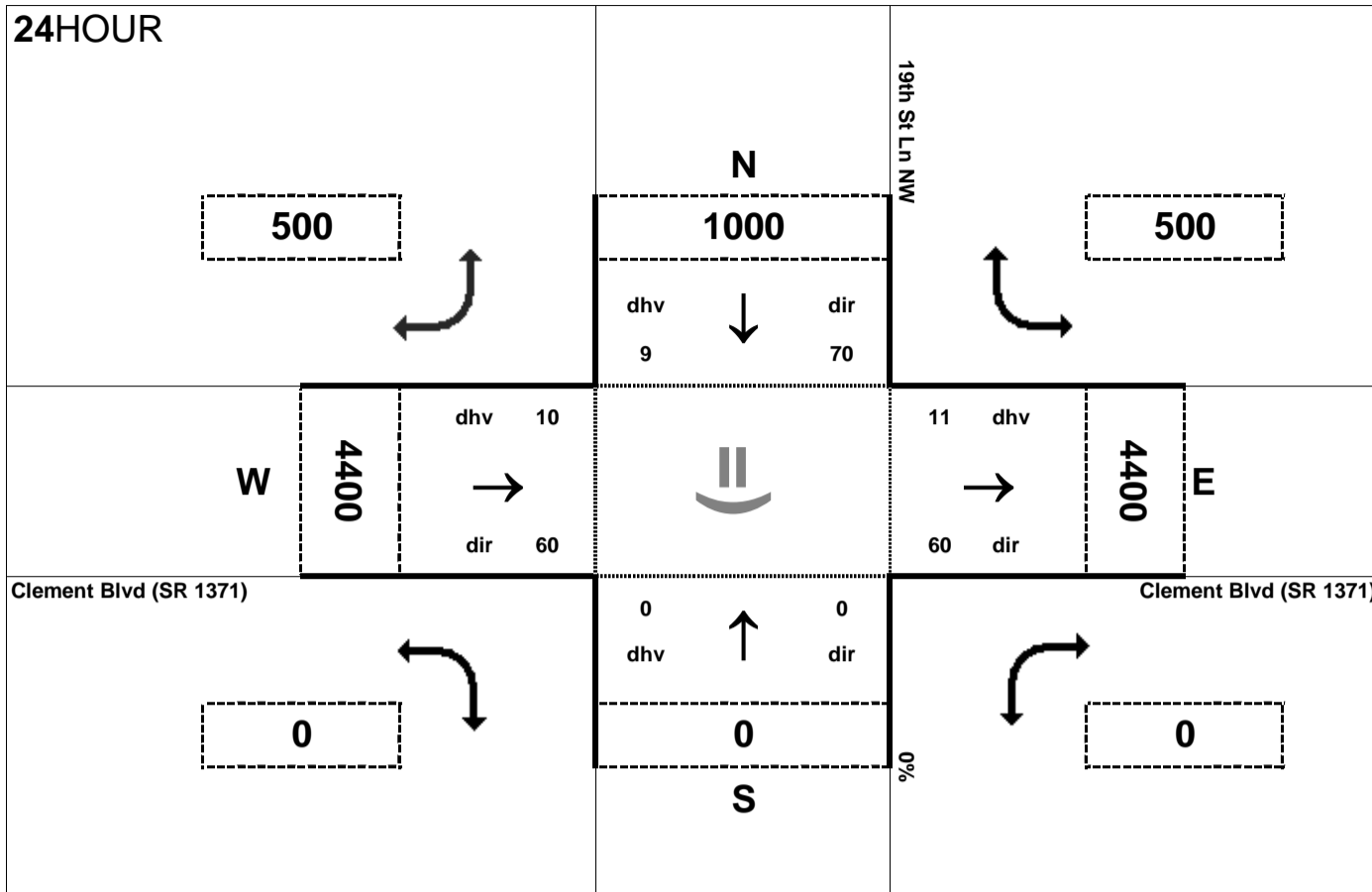
AMPEAK



PMPEAK



24HOUR



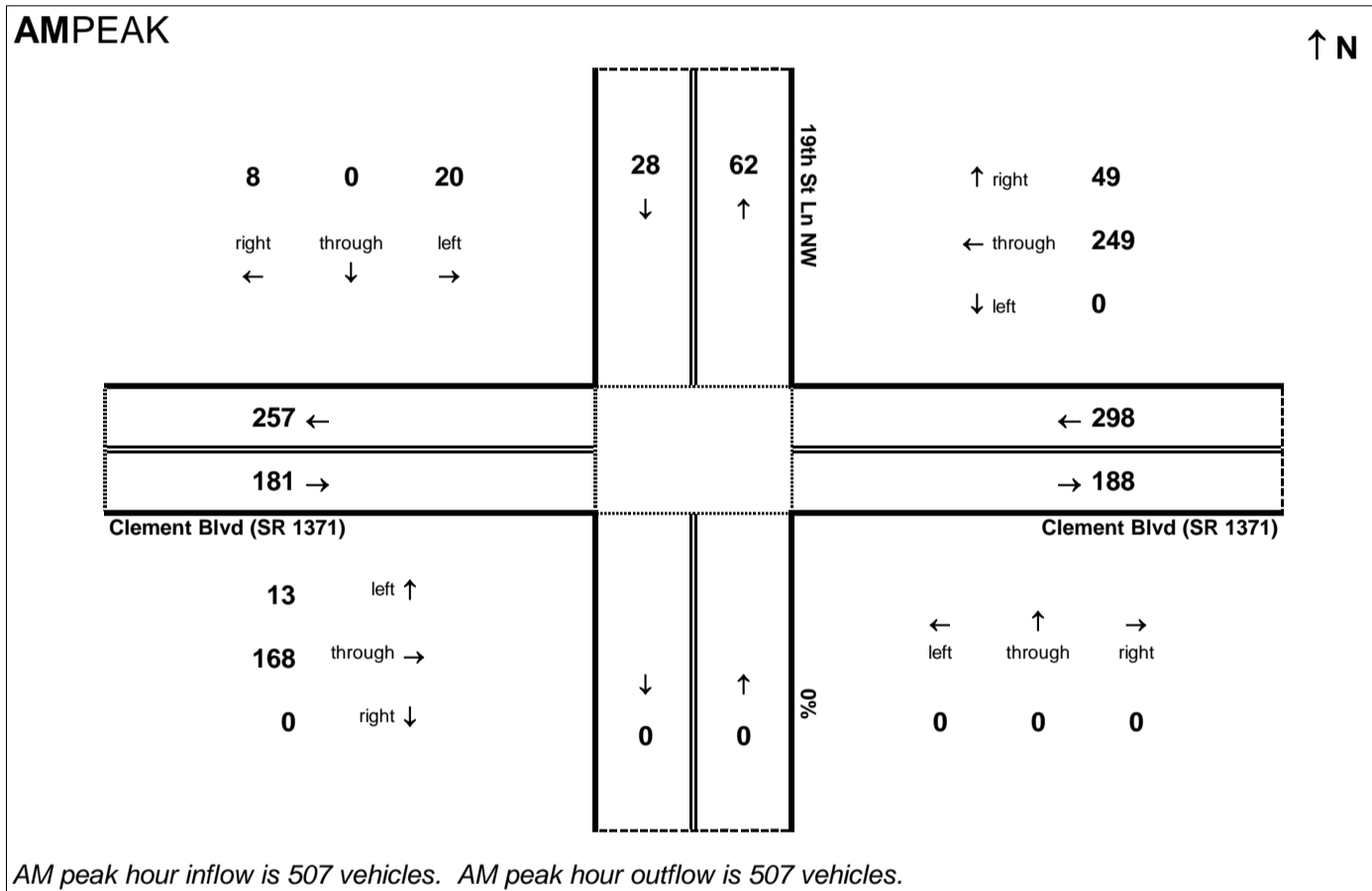
Peak Hour Volume Breakouts Report:
Intersection of Clement Blvd and 19th St Ln NW

Traffic Forecast Release Date:
December-16

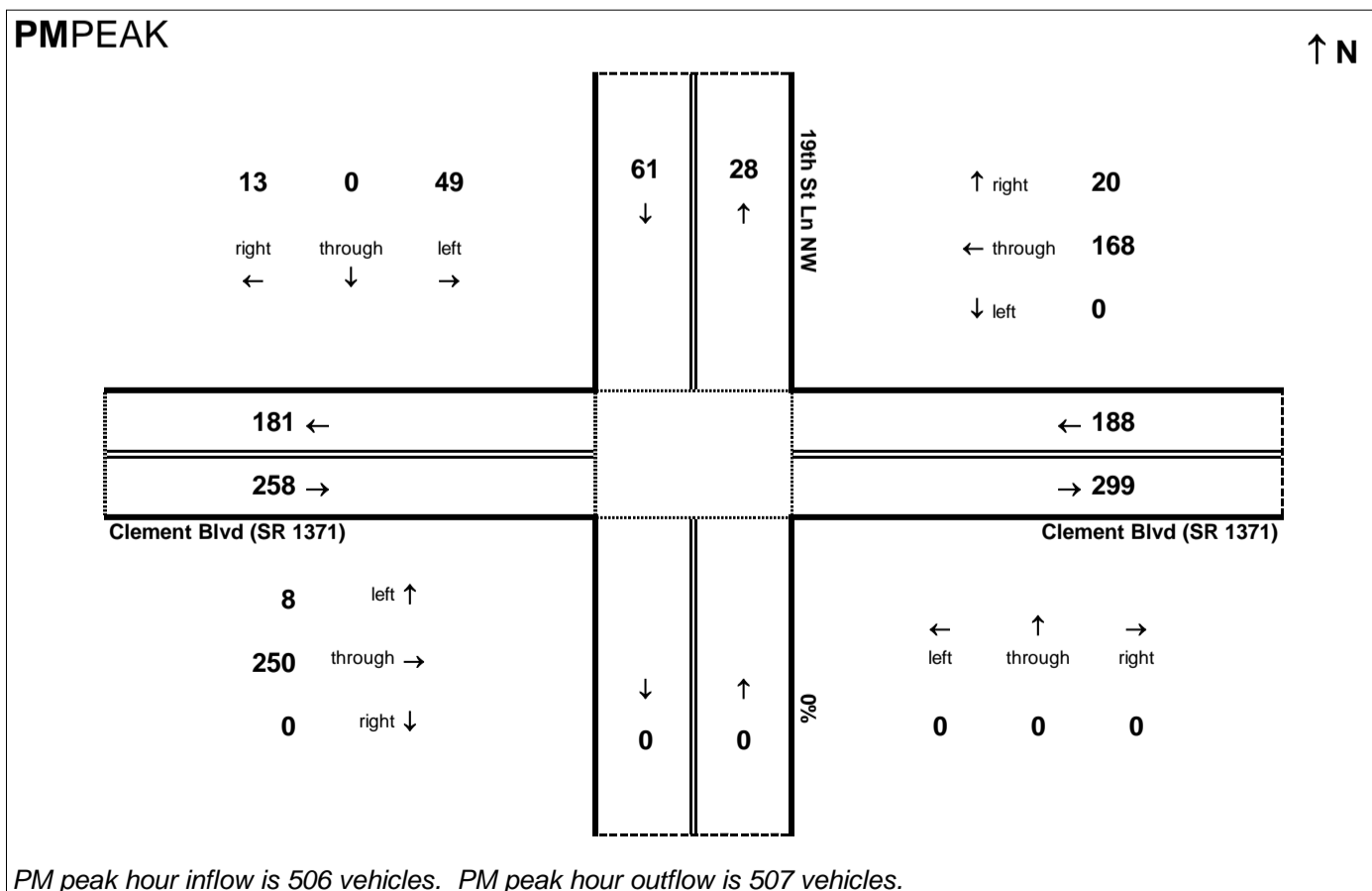
Traffic Data Year:
2040 No Build

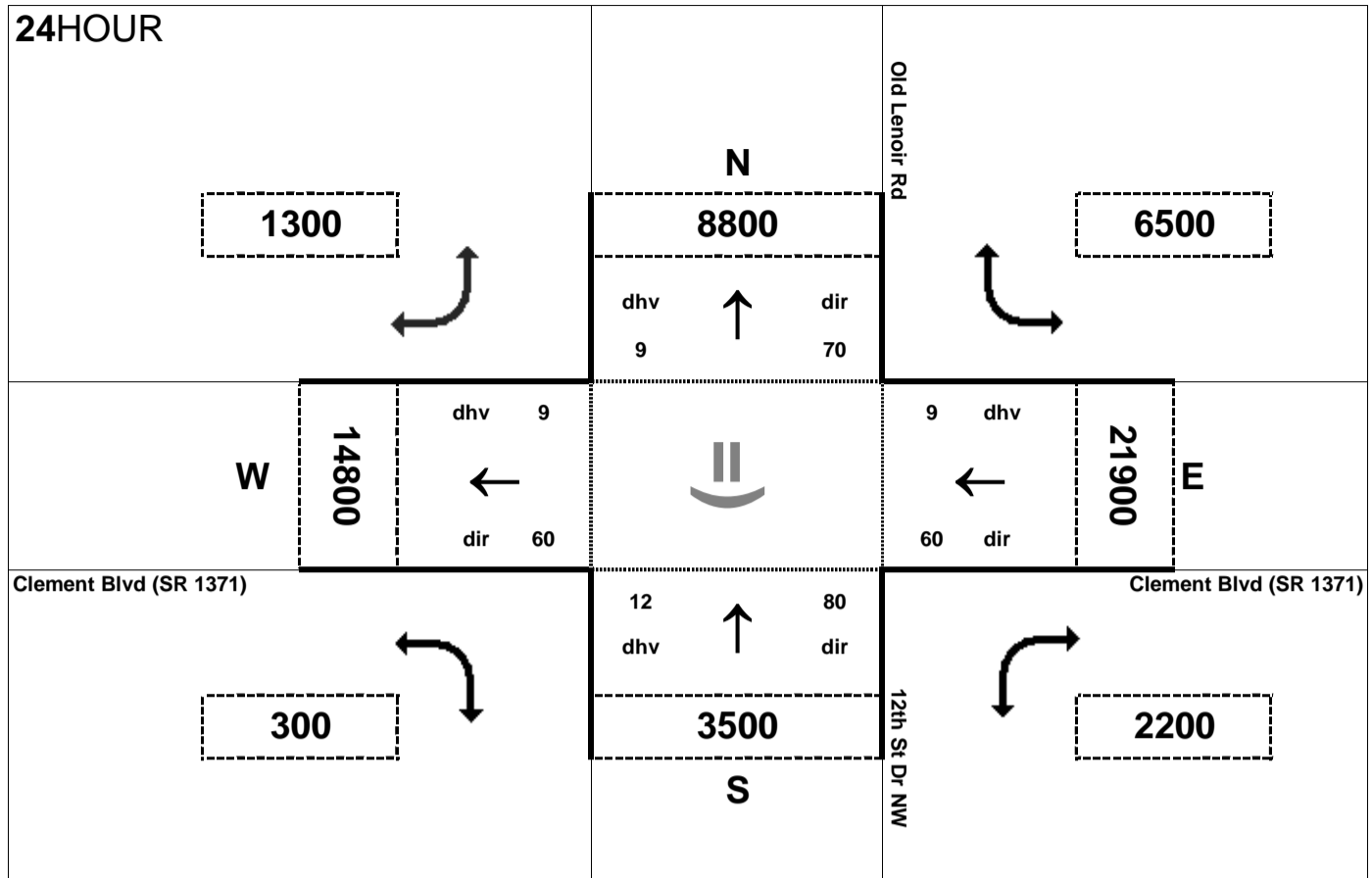
Project:
U-4700

AMPEAK



PMPEAK



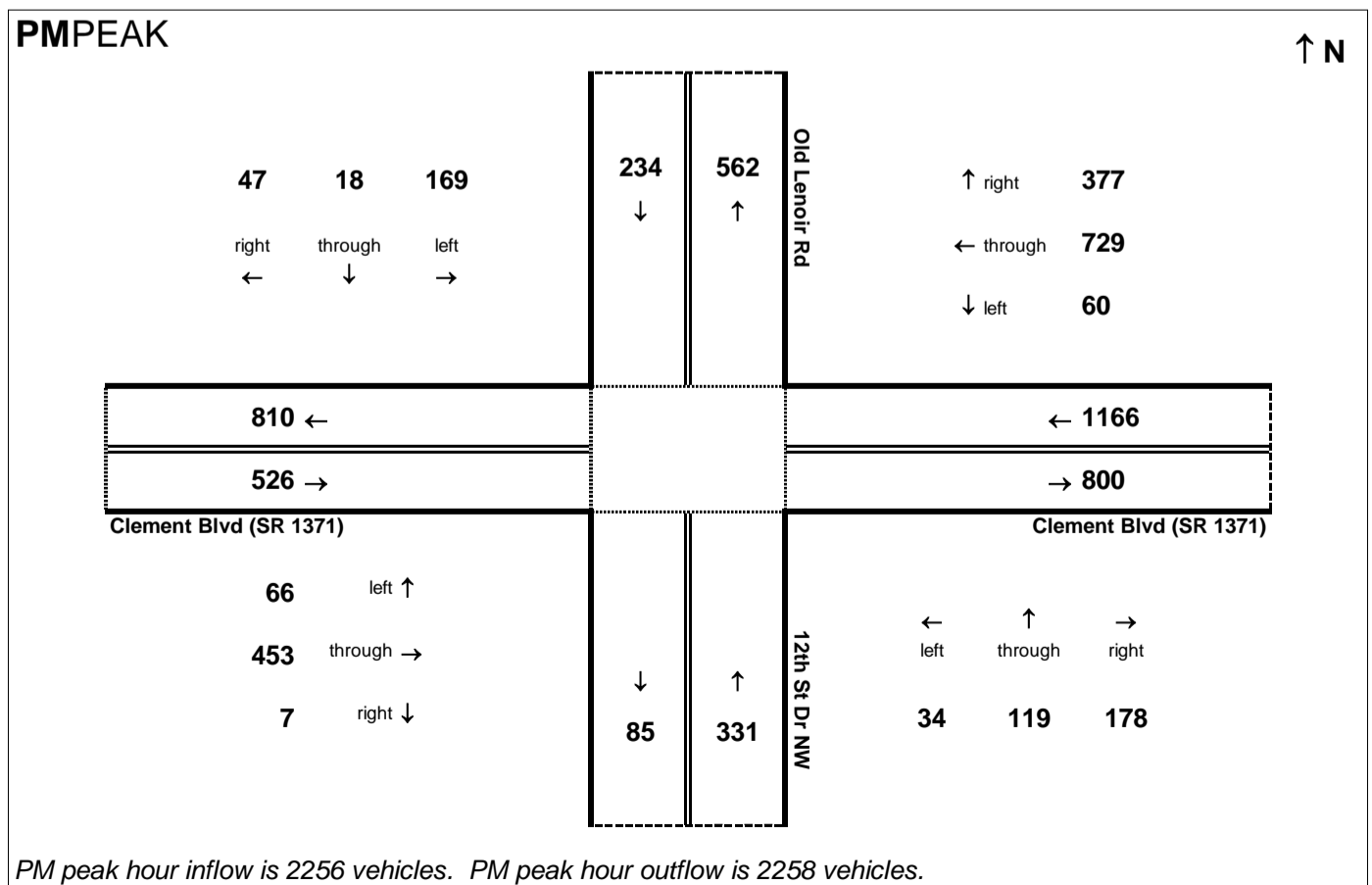
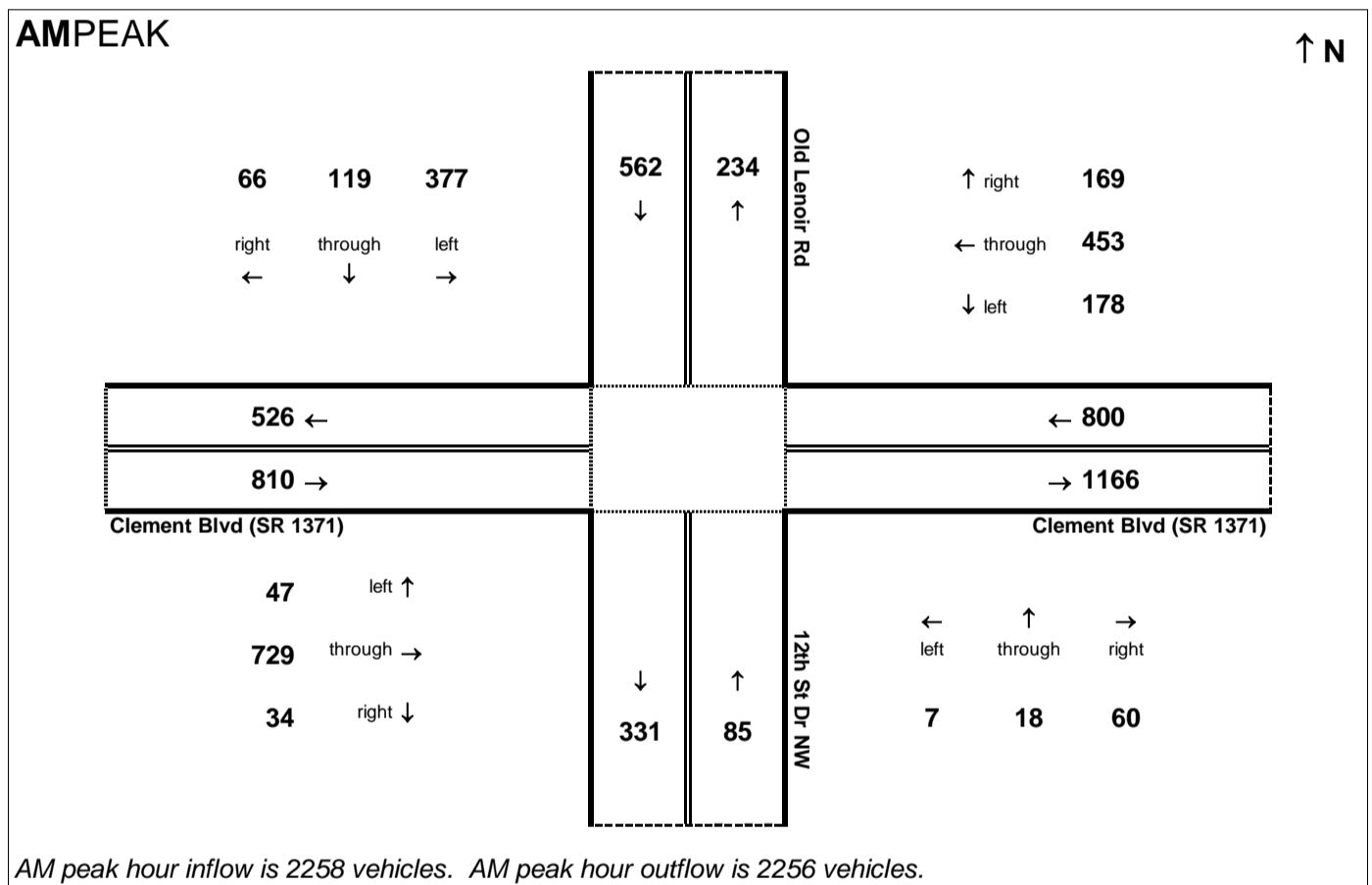


Peak Hour Volume Breakouts Report:
 Intersection of Clement Blvd and Old Lenoir Rd

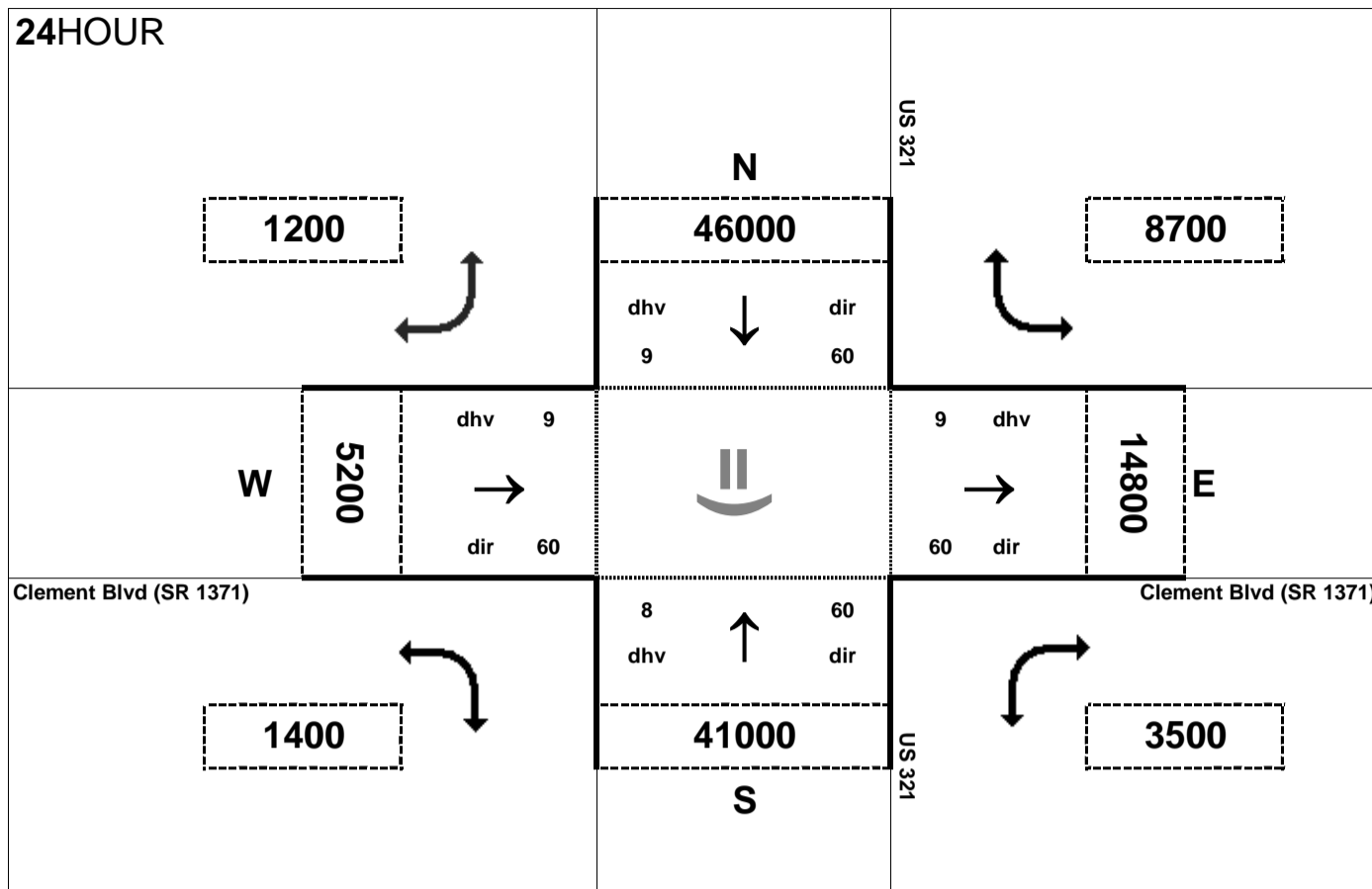
Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 No Build

Project:
 U-4700



24HOUR



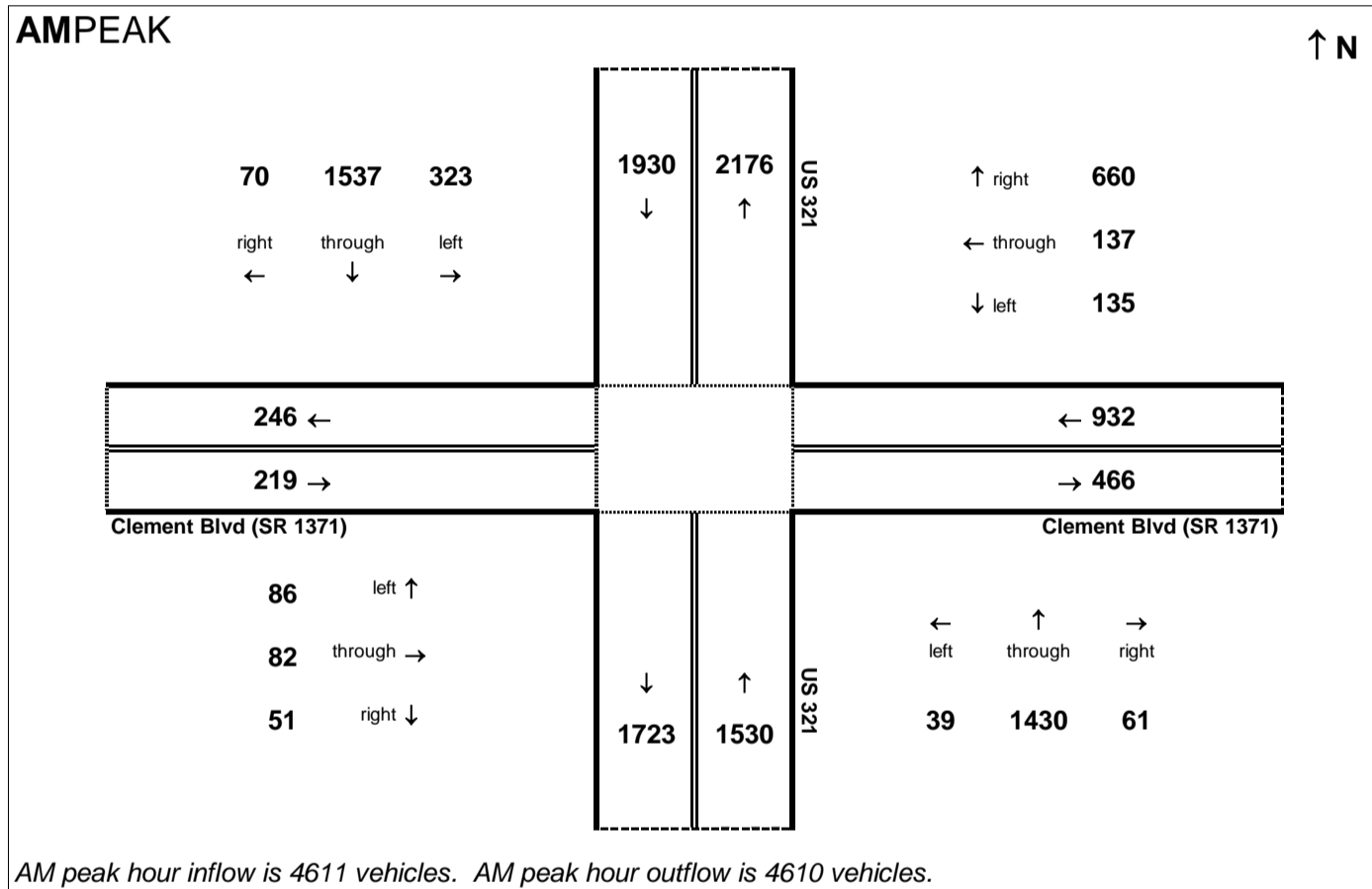
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Clement Blvd

Traffic Forecast Release Date:
December-16

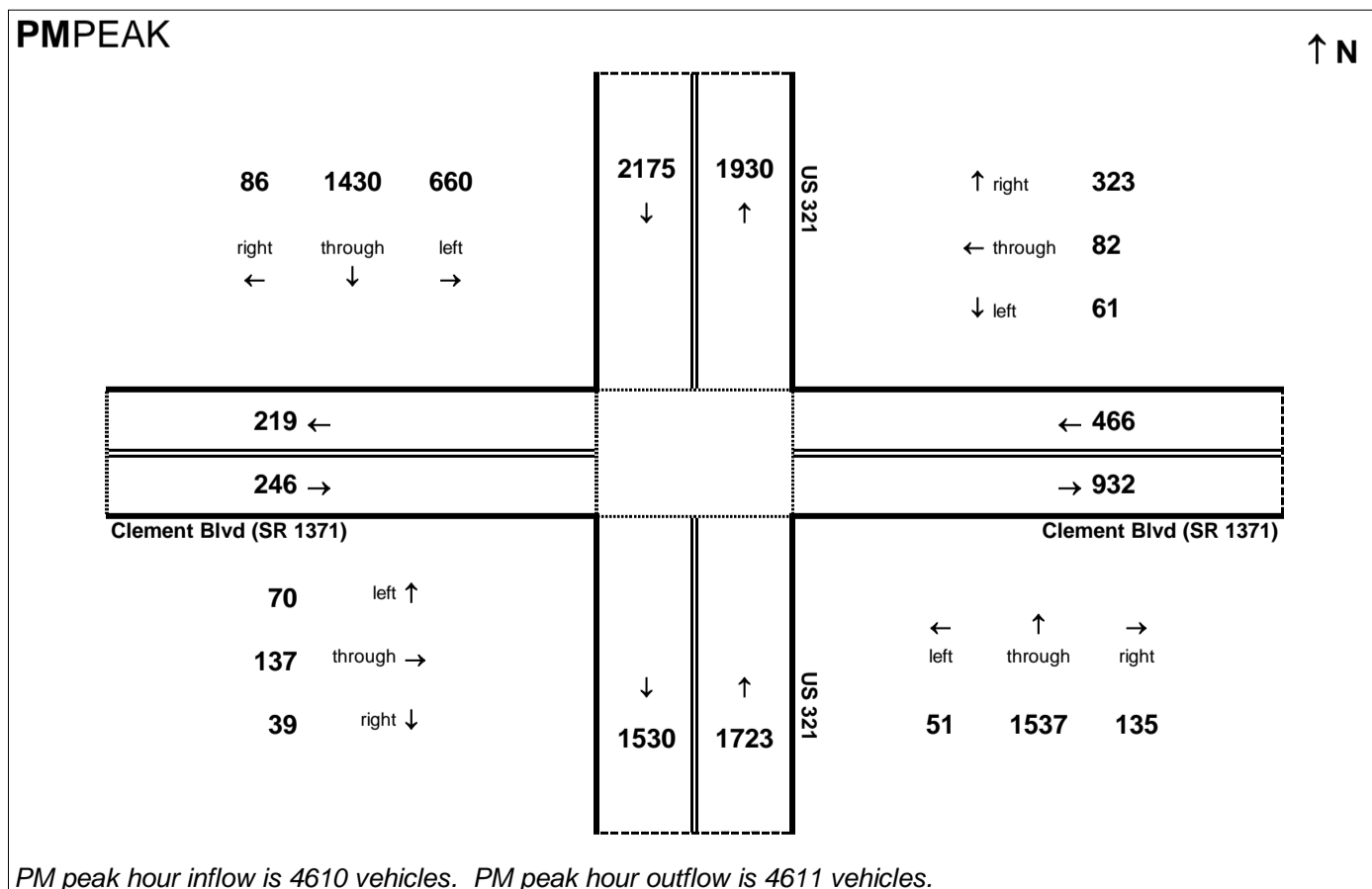
Traffic Data Year:
2040 No Build

Project:
U-4700

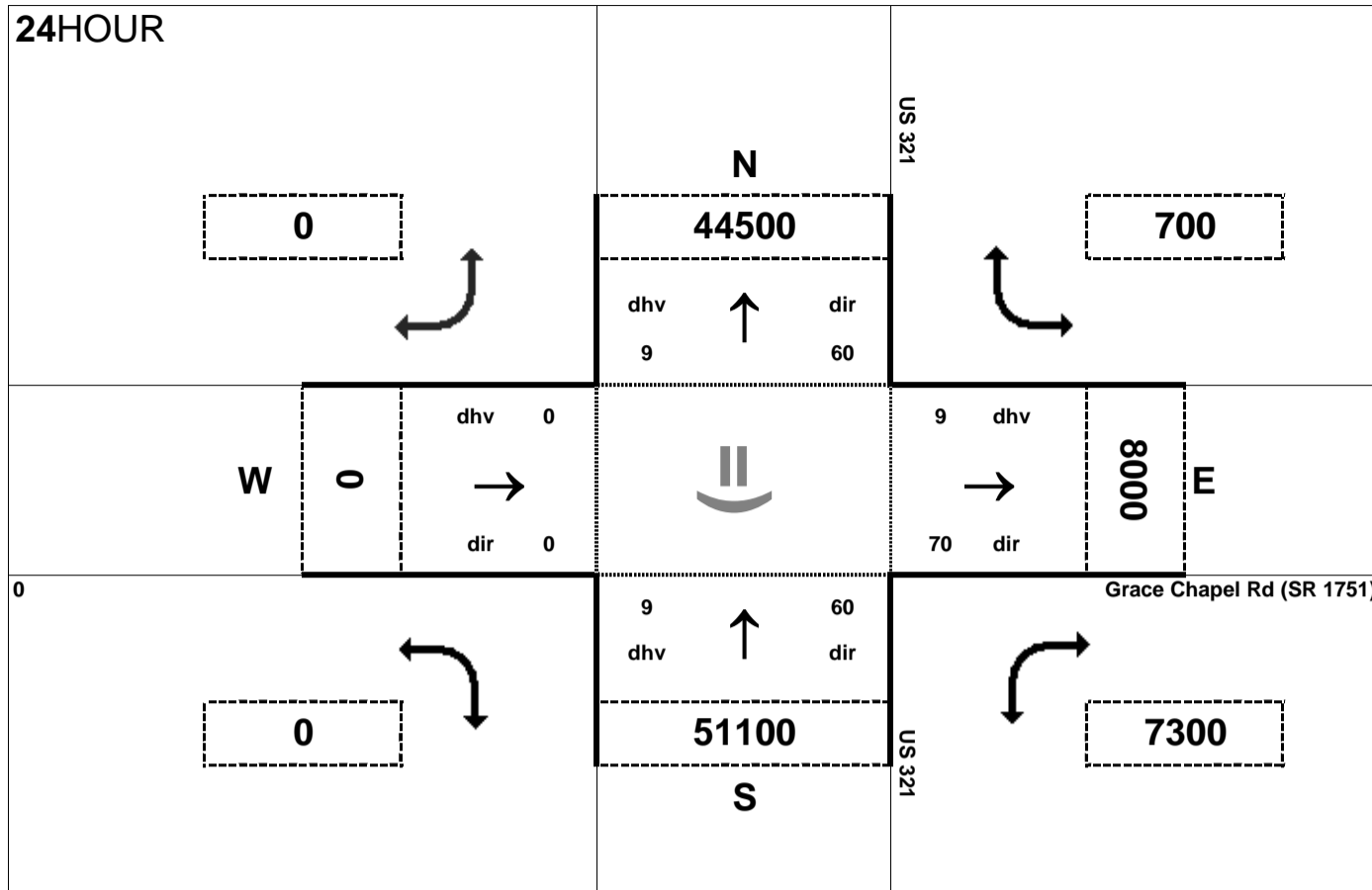
AMPEAK



PMPEAK



24HOUR



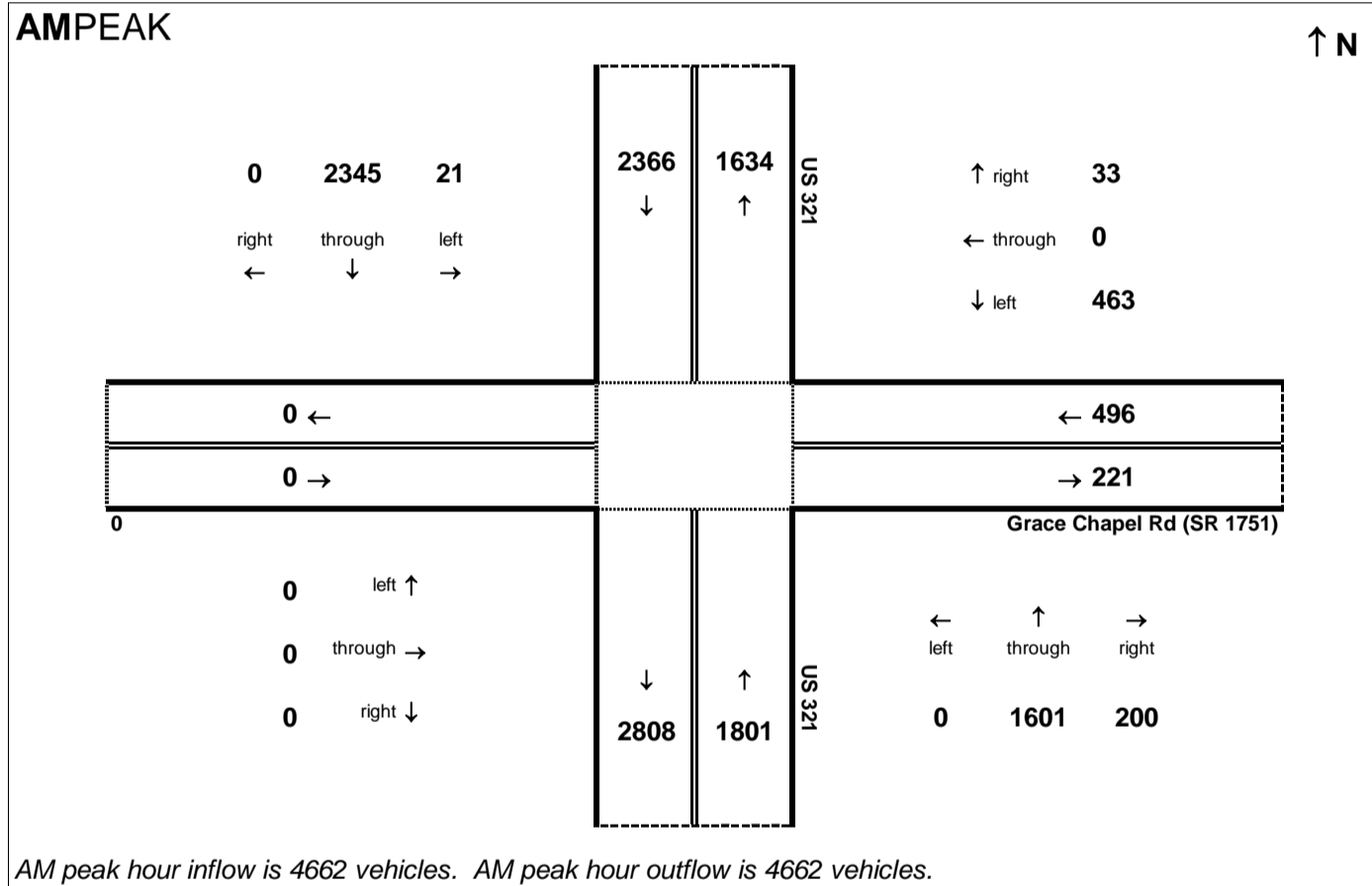
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Grace Chapel Rd

Traffic Forecast Release Date:
December-16

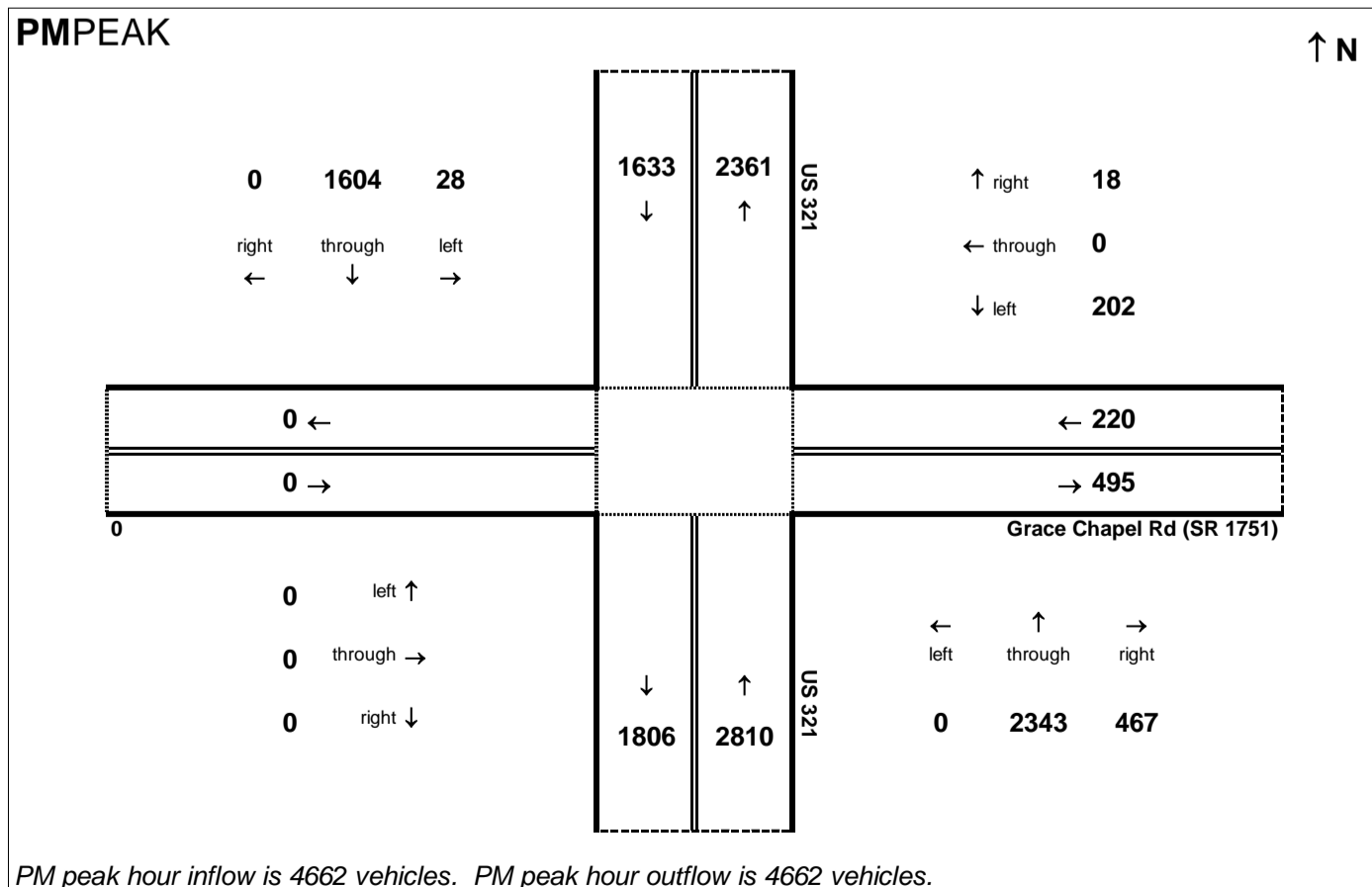
Traffic Data Year:
2040 No Build

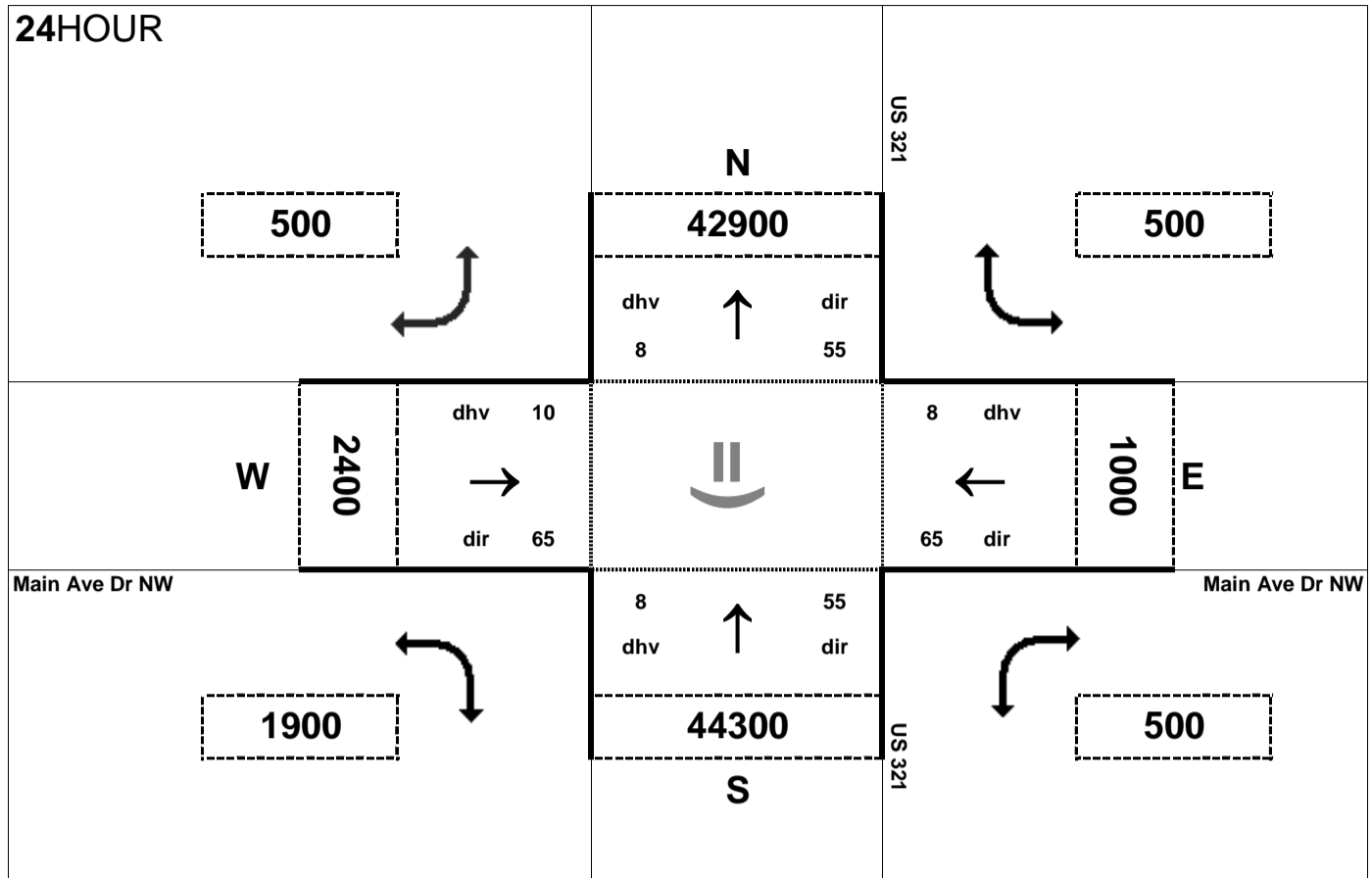
Project:
U-4700

AMPEAK



PMPEAK



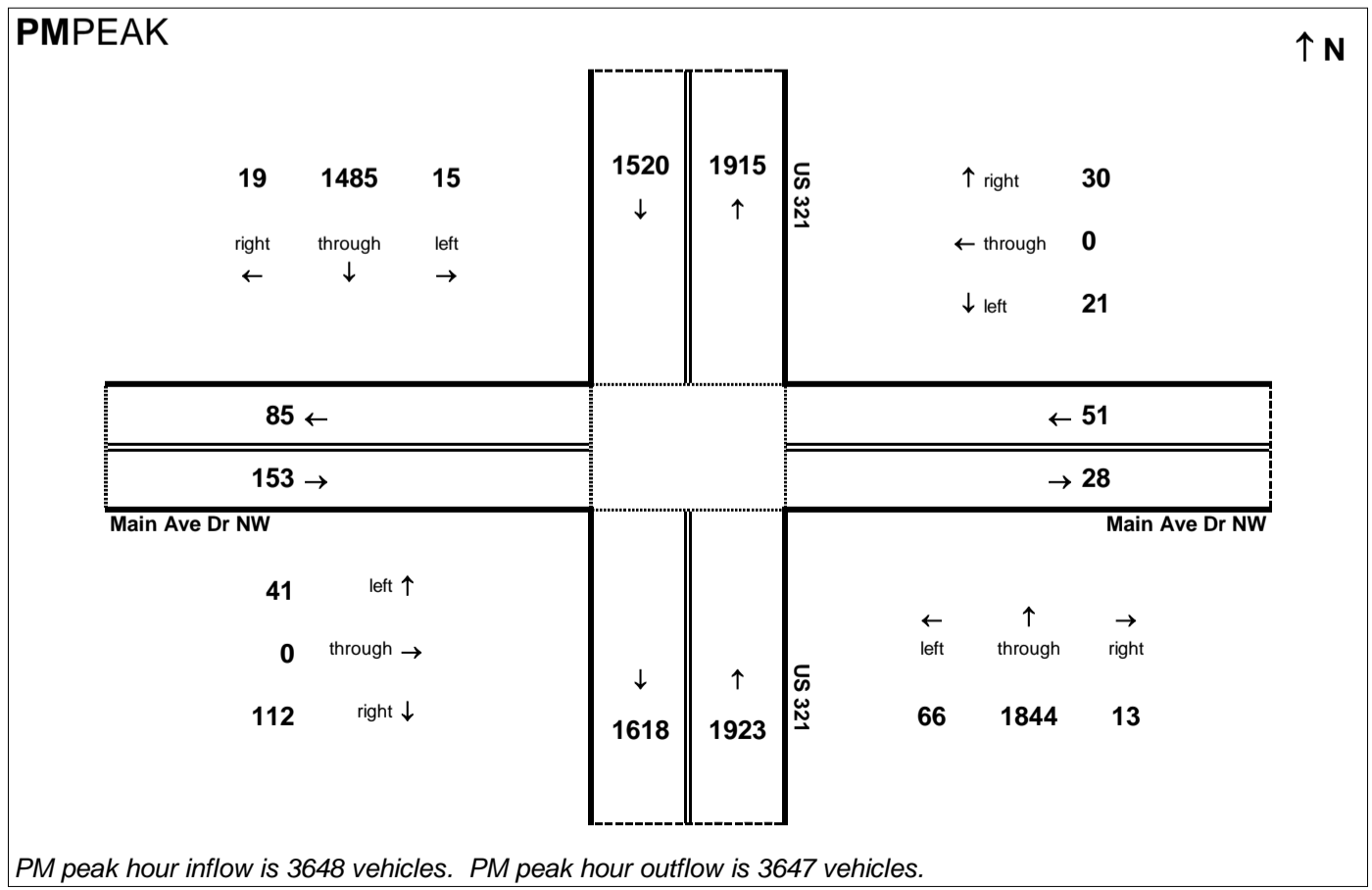
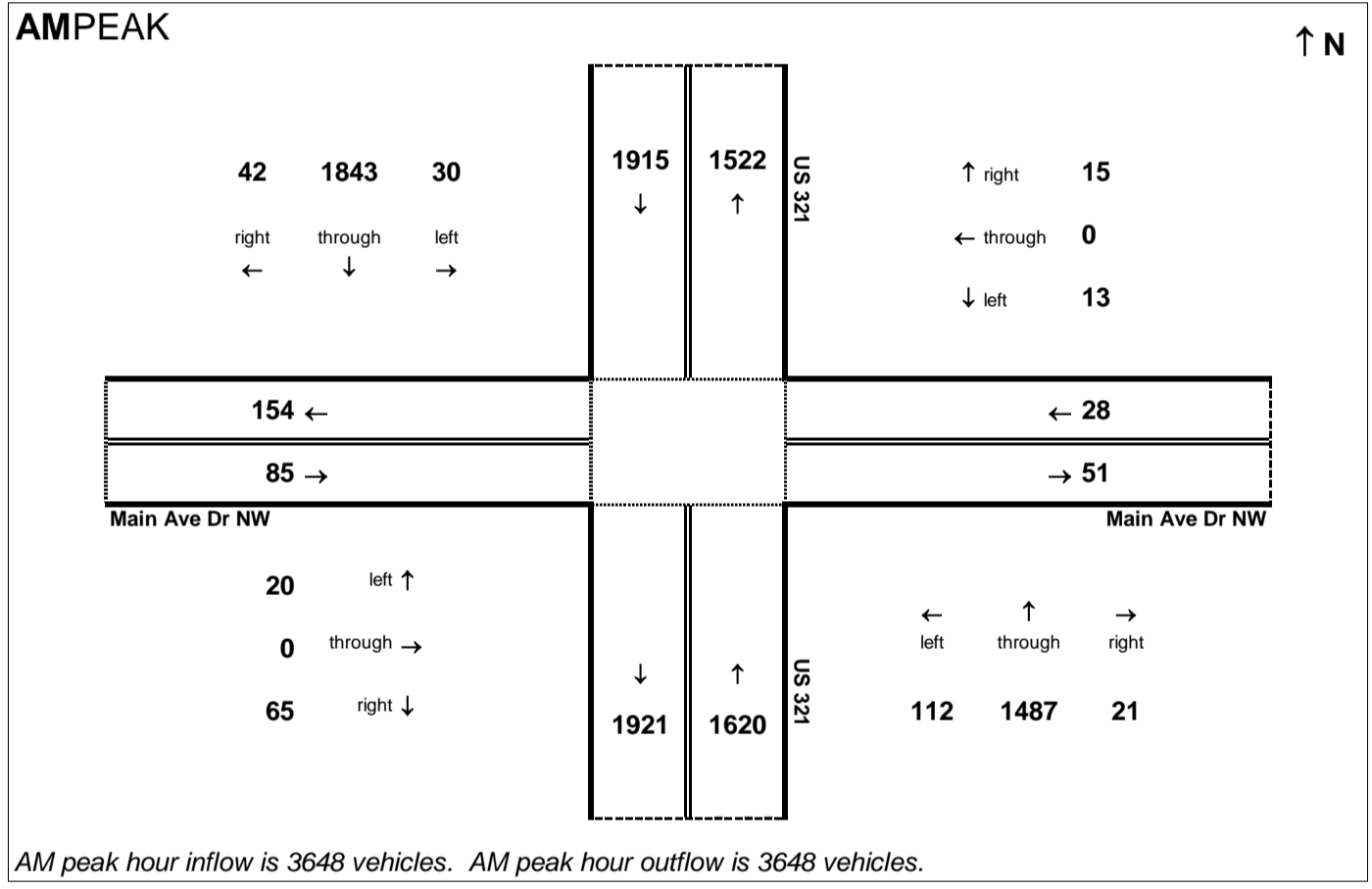


Peak Hour Volume Breakouts Report:
Intersection of US 321 and Main Ave Dr NW

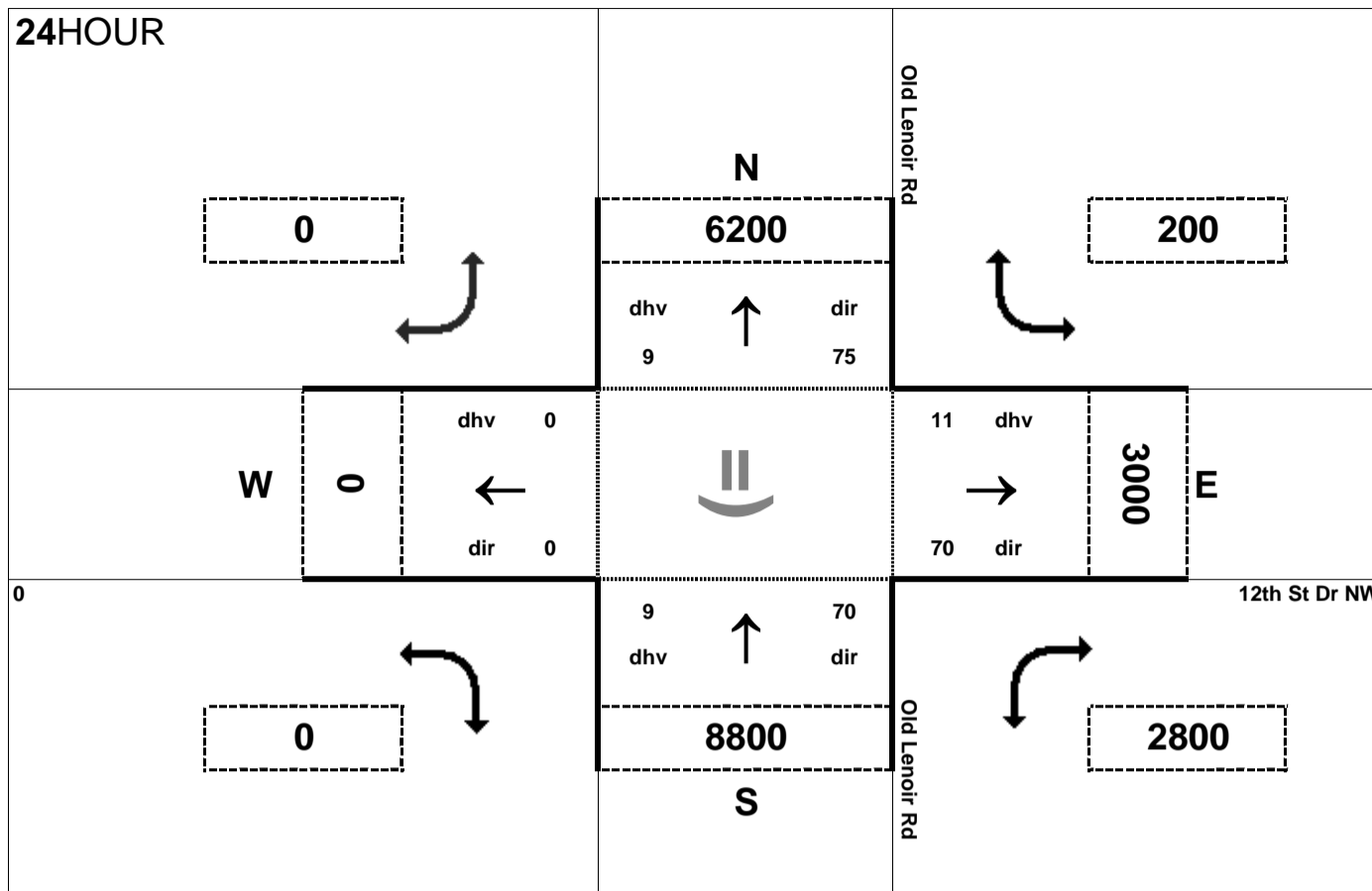
Traffic Forecast Release Date:
February-17

Traffic Data Year:
2040 No Build

Project:
U-4700



24HOUR



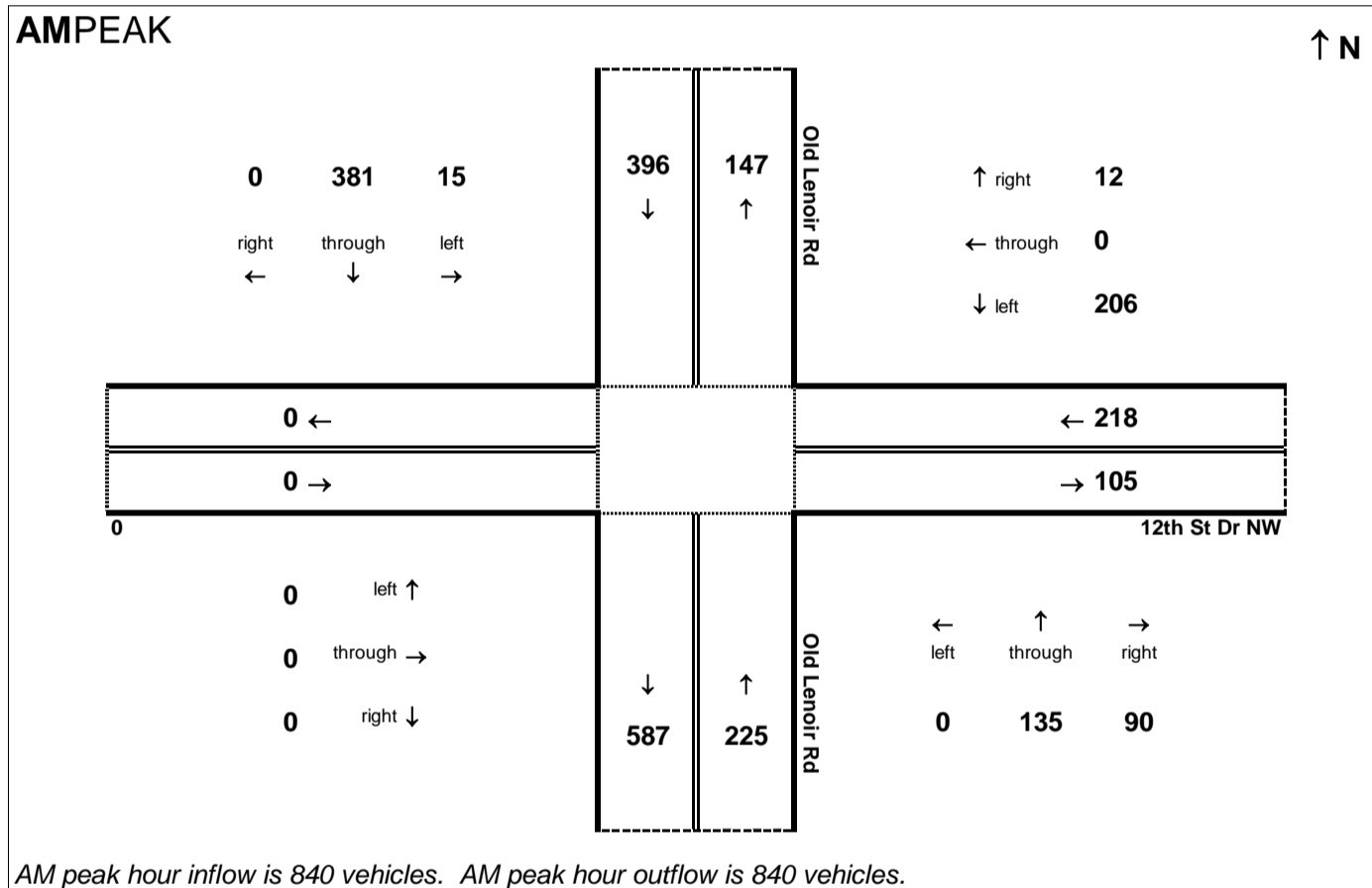
Peak Hour Volume Breakouts Report:
Intersection of Old Lenoir Rd and 12th St Dr NW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 No Build

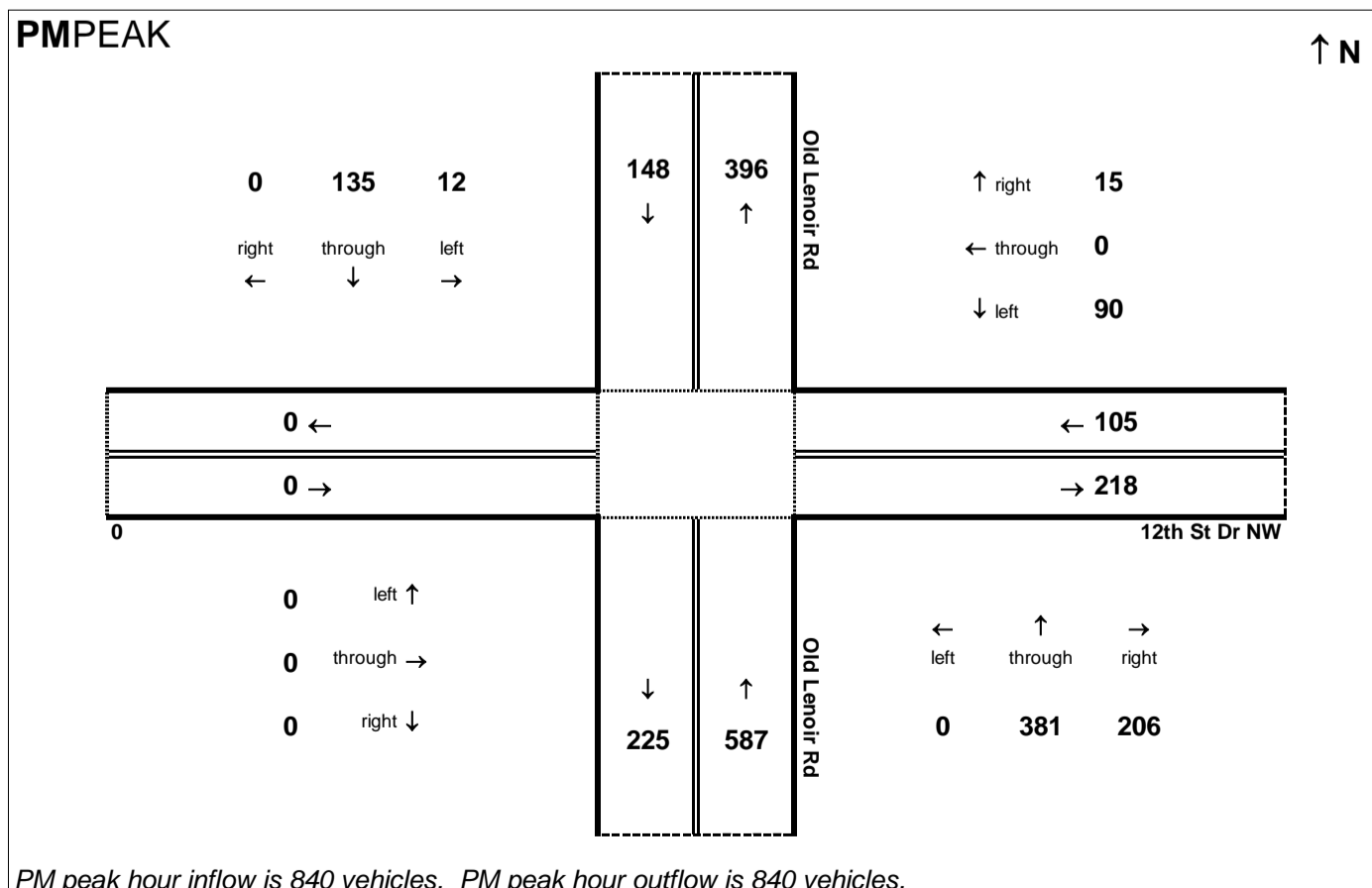
Project:
U-4700

AMPEAK



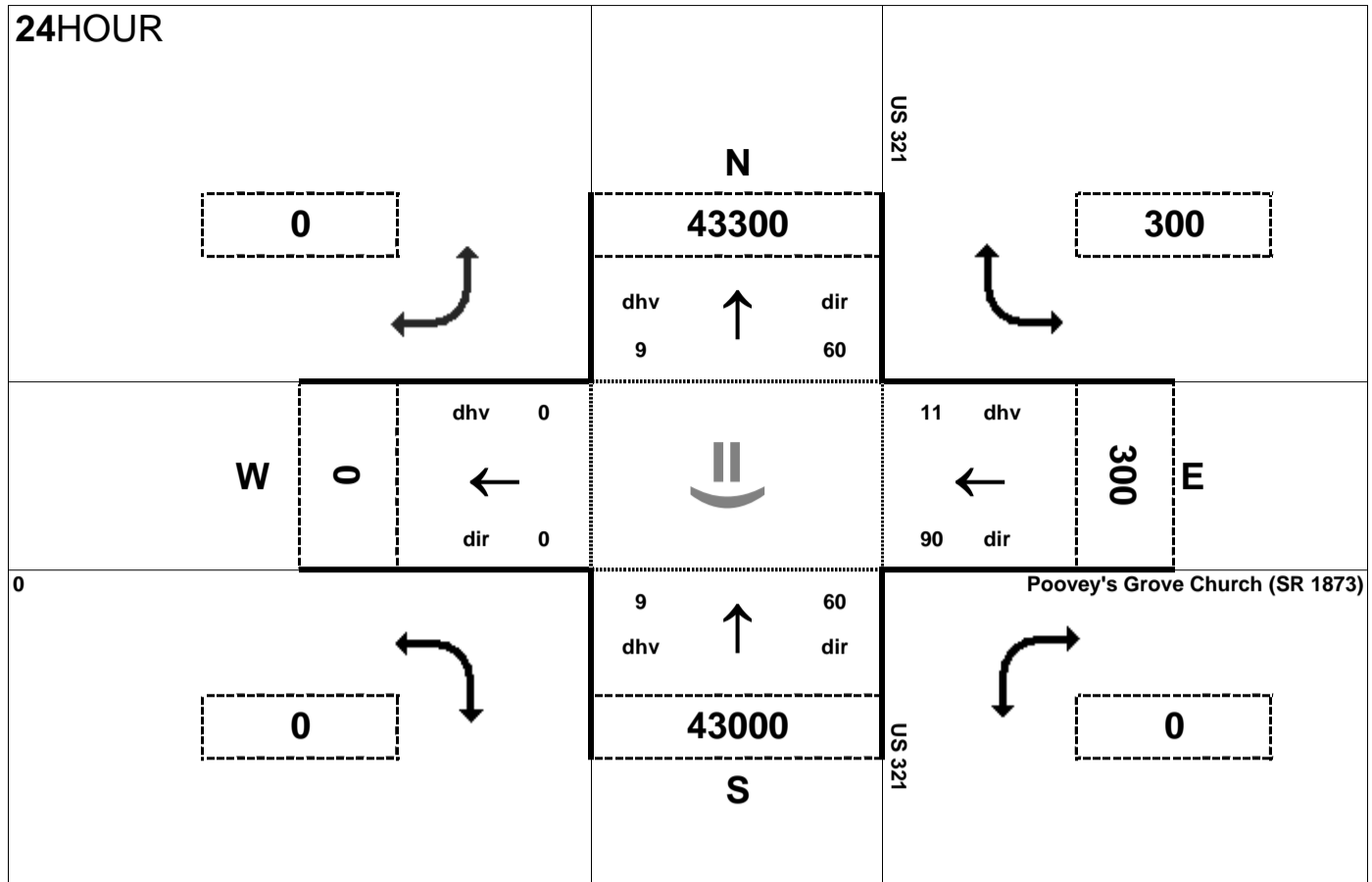
AM peak hour inflow is 840 vehicles. AM peak hour outflow is 840 vehicles.

PMPEAK



PM peak hour inflow is 840 vehicles. PM peak hour outflow is 840 vehicles.

24HOUR



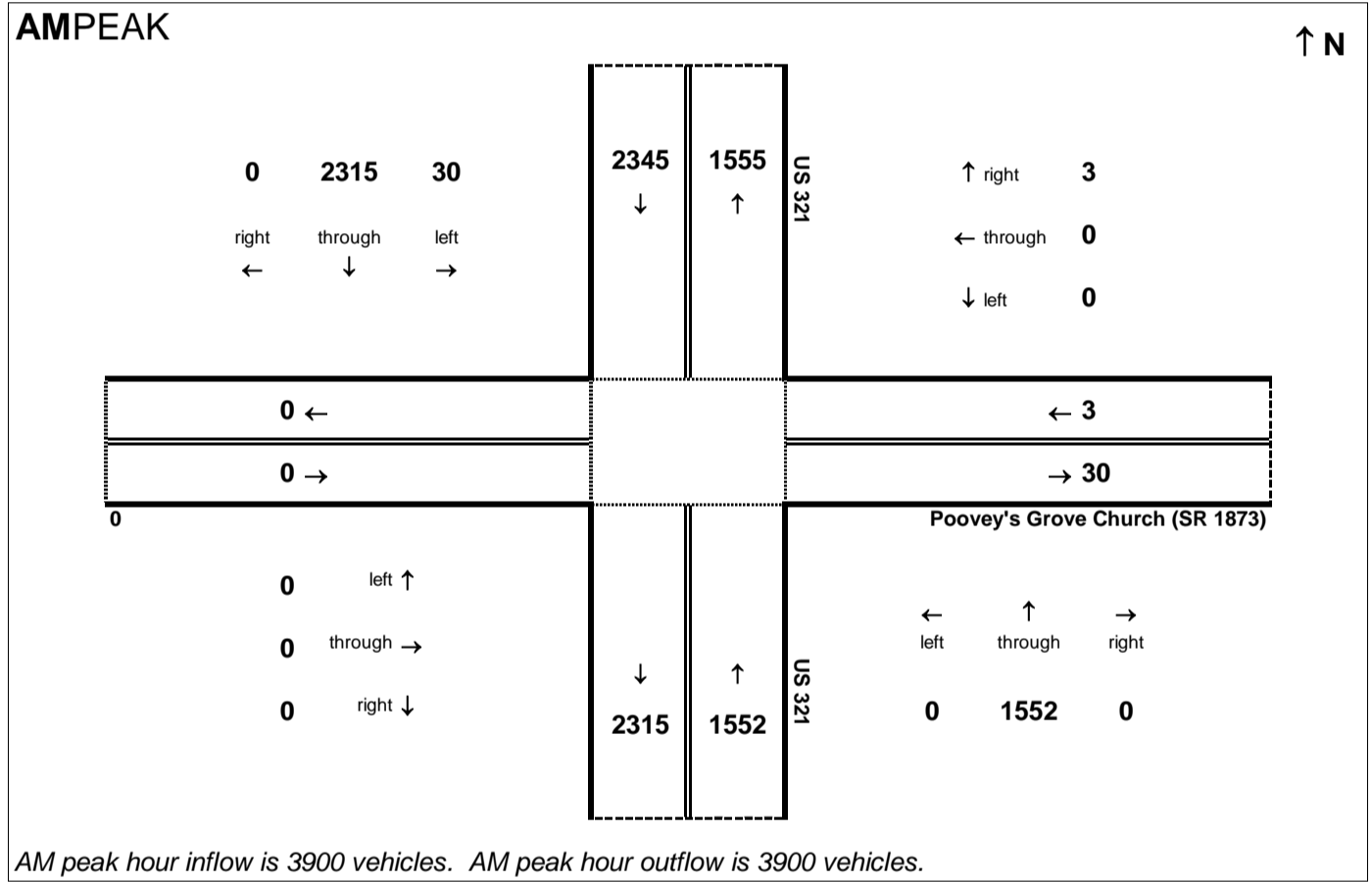
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Poovey's Grove Church

Traffic Forecast Release Date:
December-16

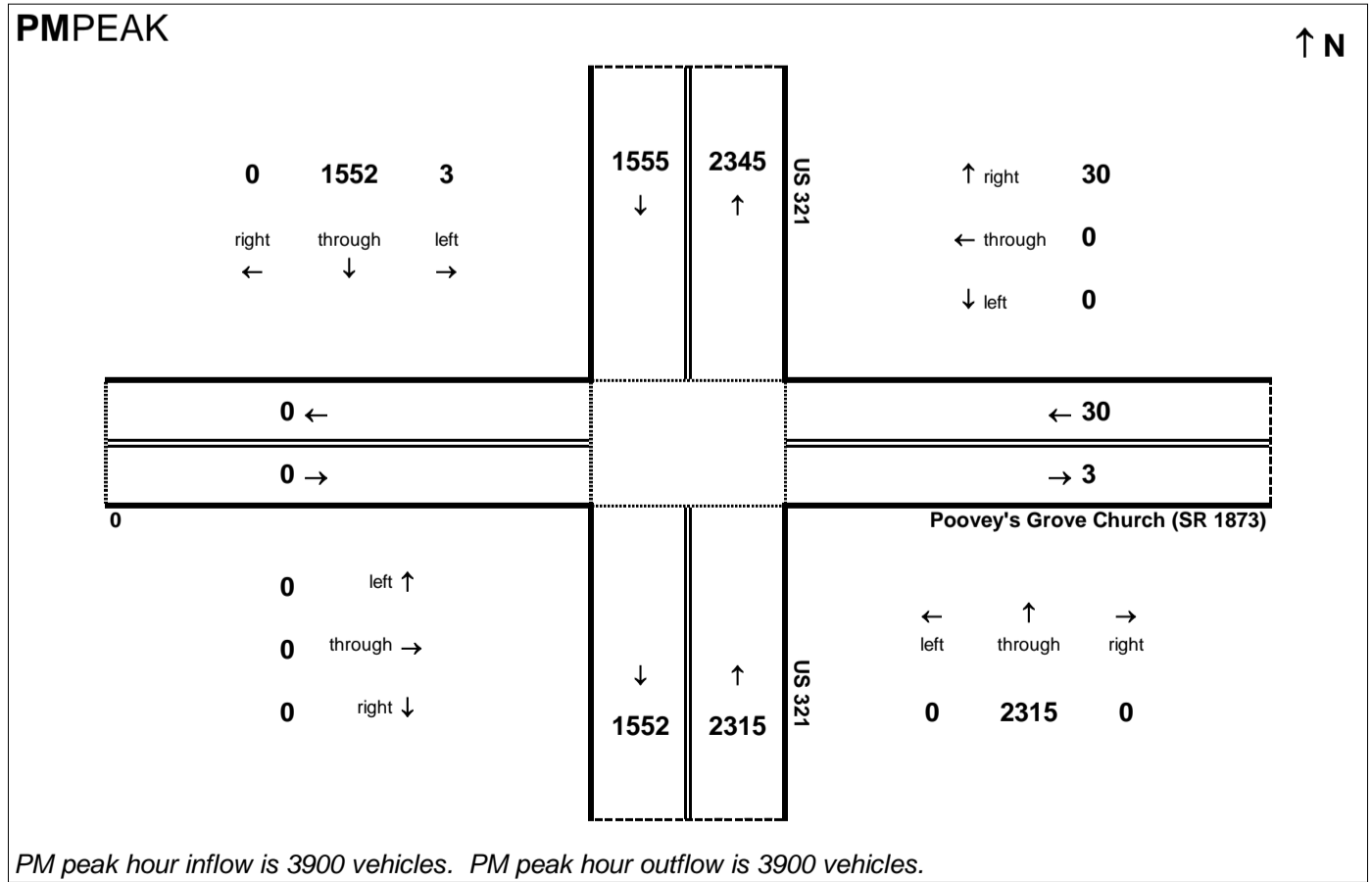
Traffic Data Year:
2040 No Build

Project:
U-4700

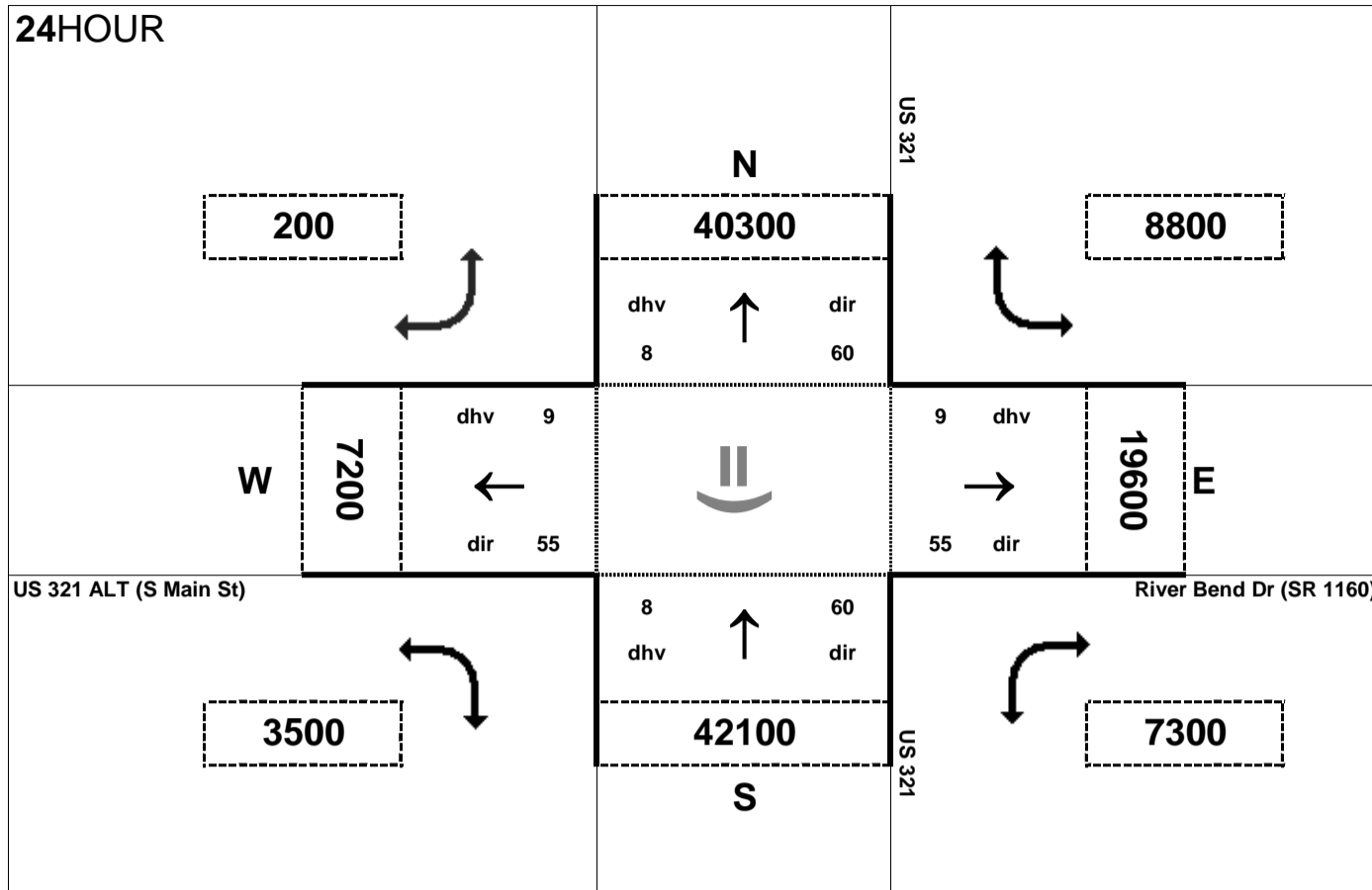
AMPEAK



PMPEAK



24HOUR



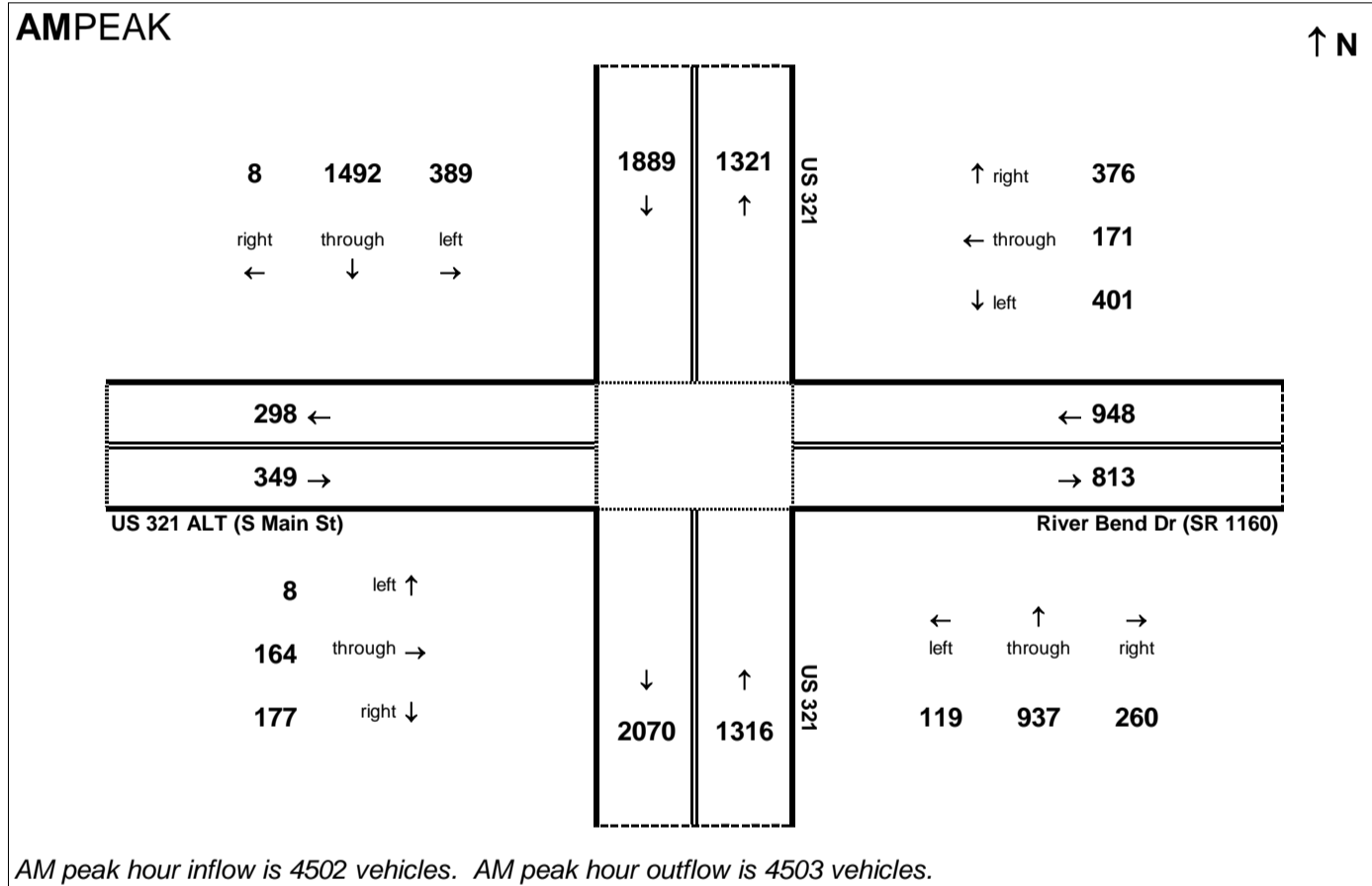
Peak Hour Volume Breakouts Report:
Intersection of US 321 and US 321 ALT

Traffic Forecast Release Date:
December-16

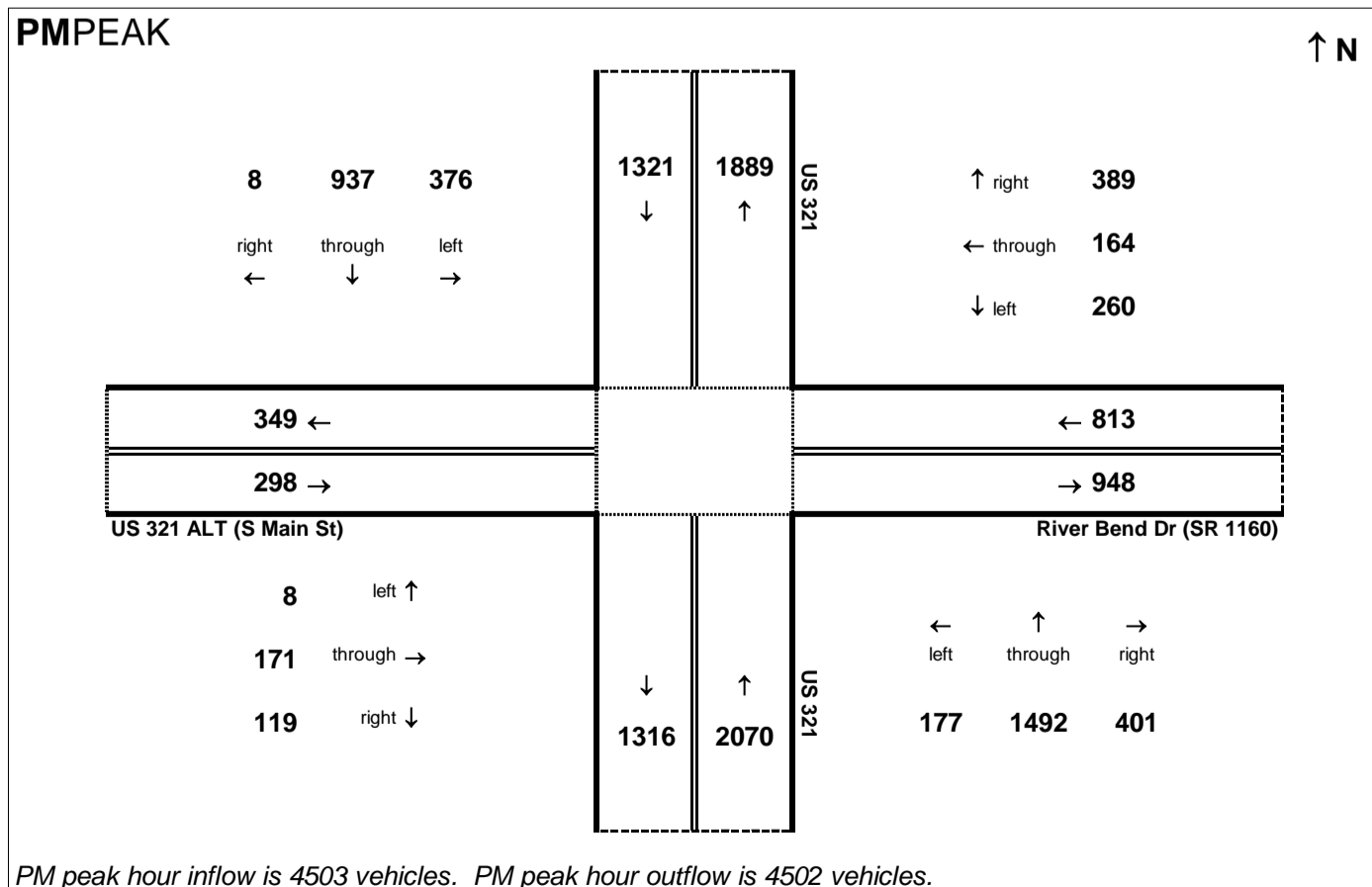
Traffic Data Year:
2040 No Build

Project:
U-4700

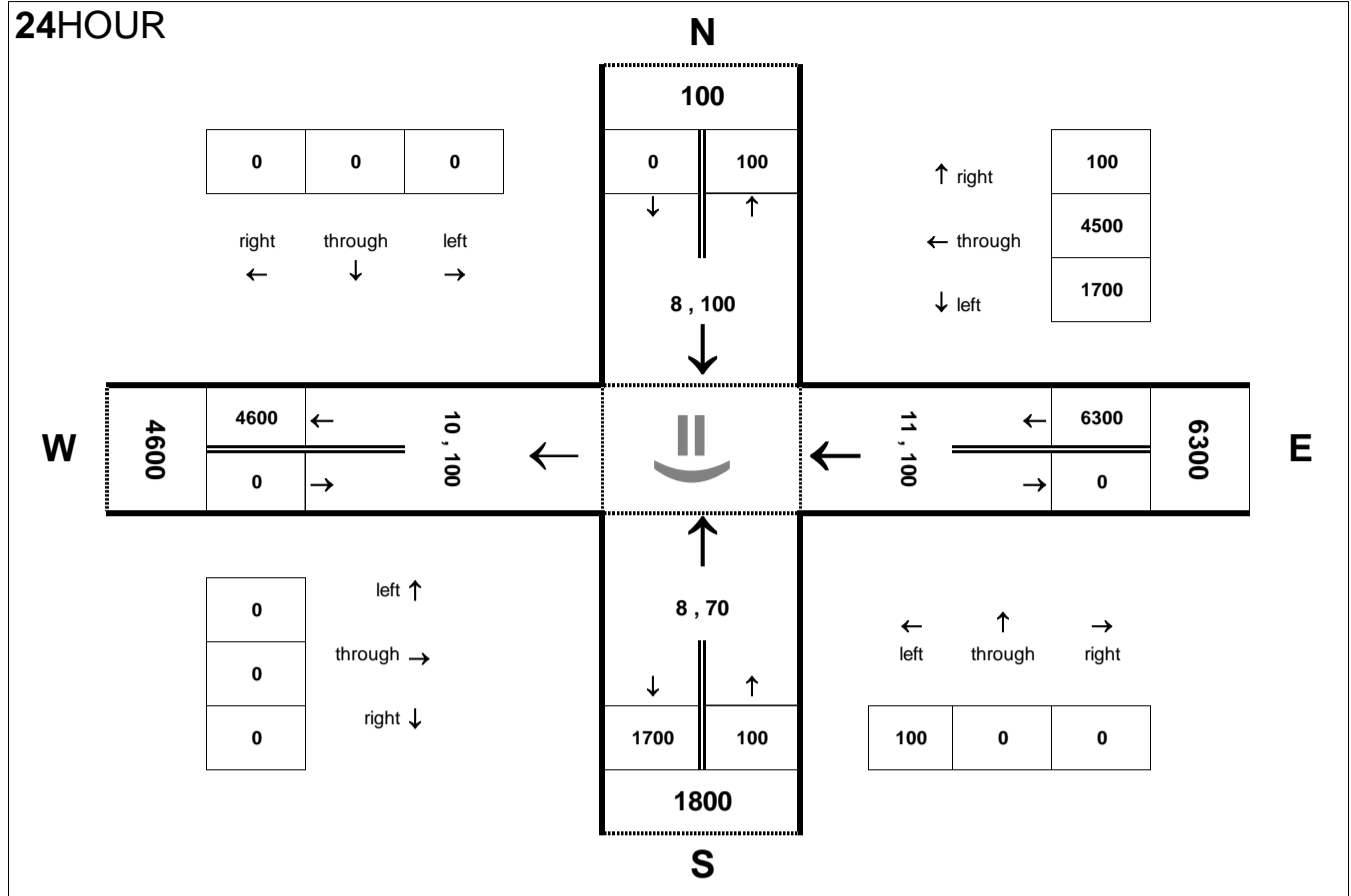
AMPEAK



PMPEAK



24HOUR



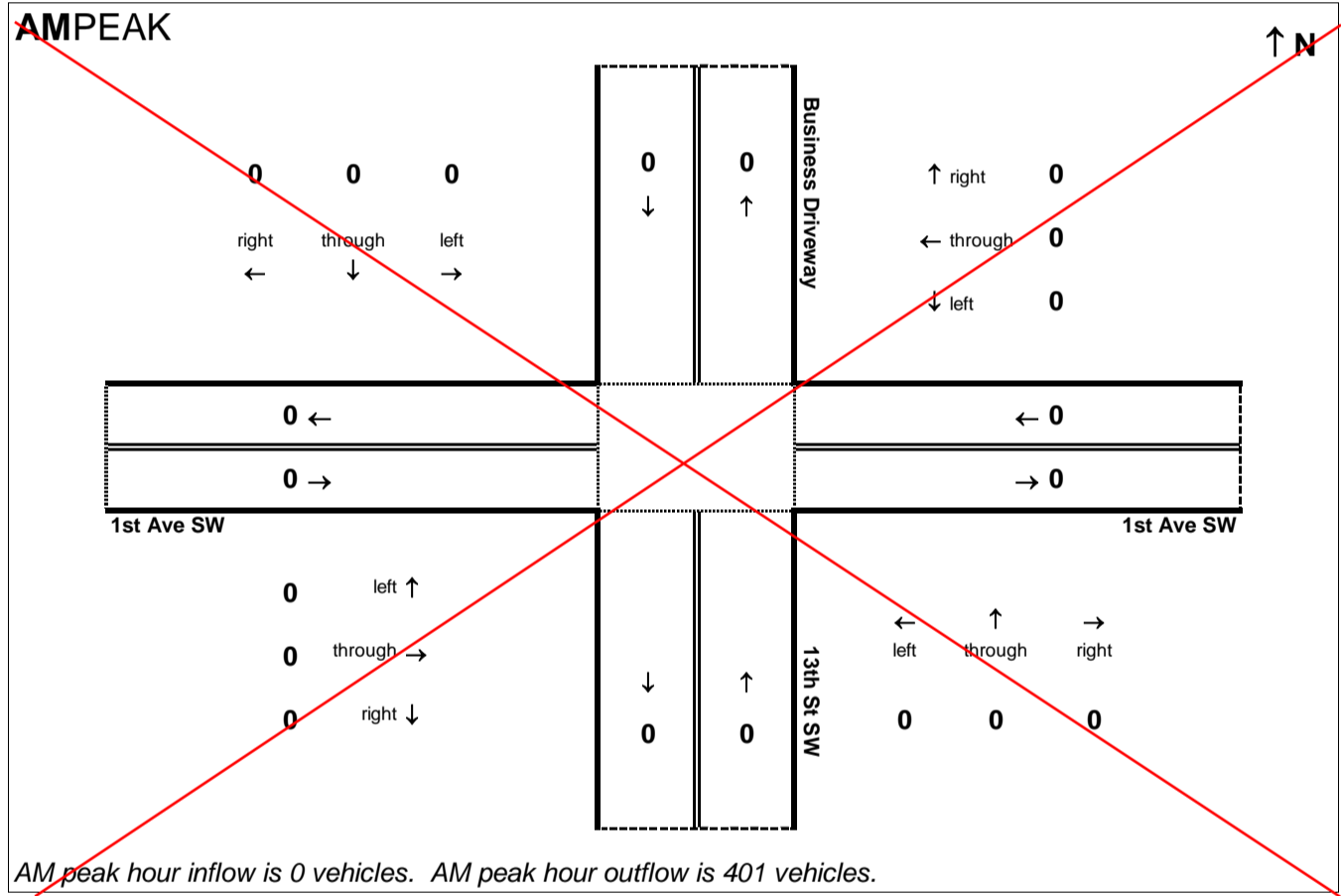
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

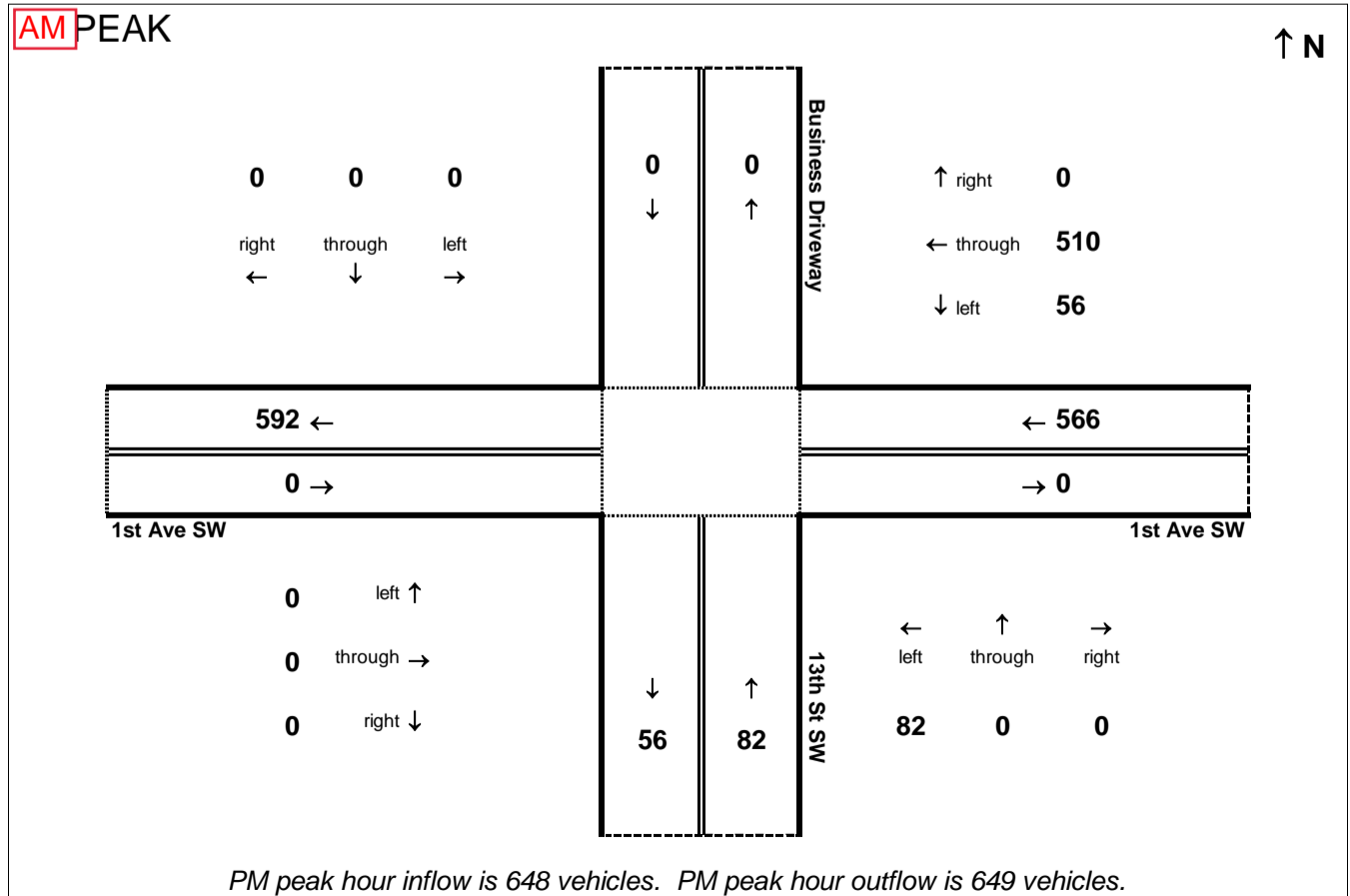
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 401 vehicles.

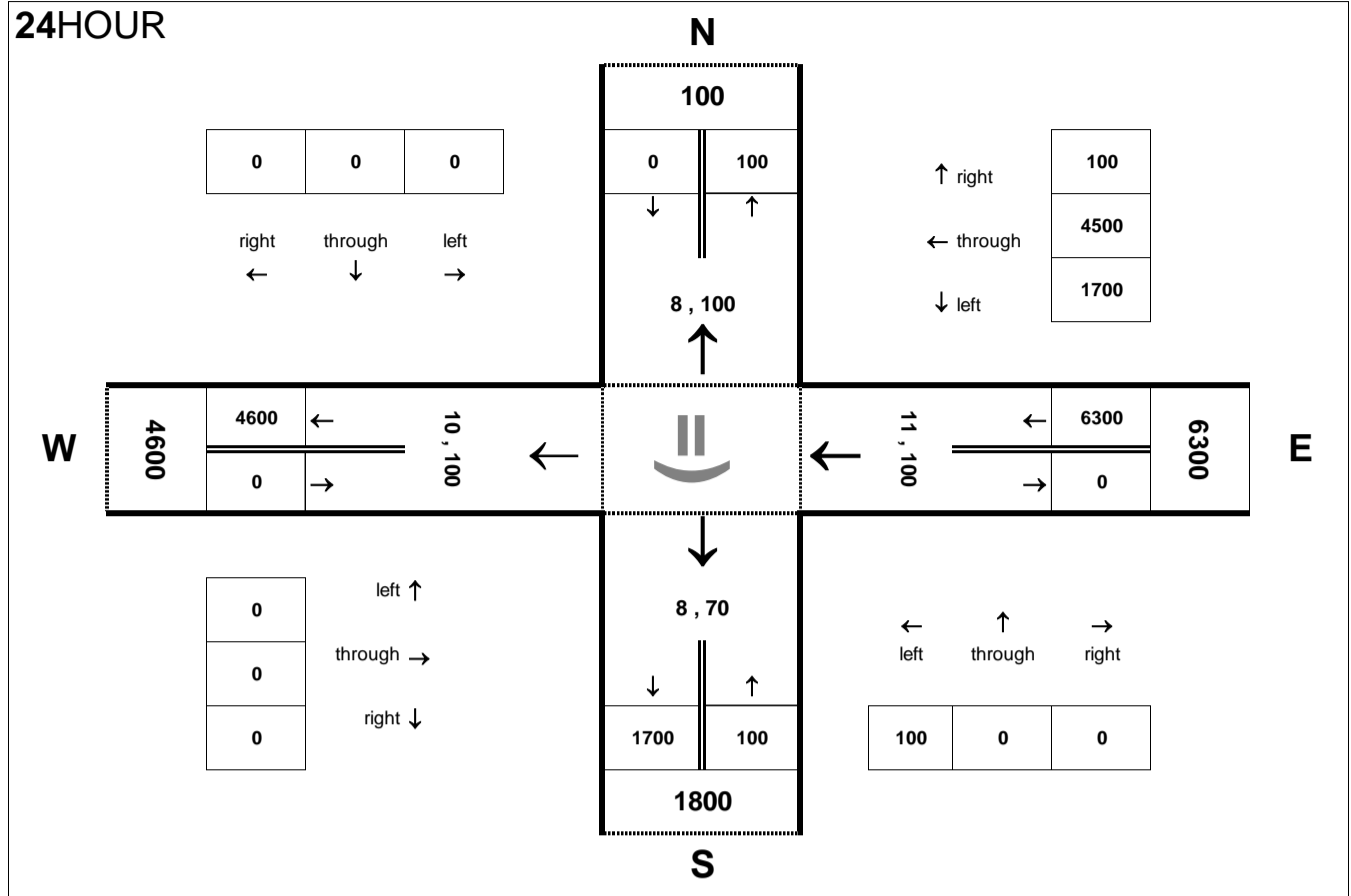
PM PEAK



PM peak hour inflow is 648 vehicles. PM peak hour outflow is 649 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



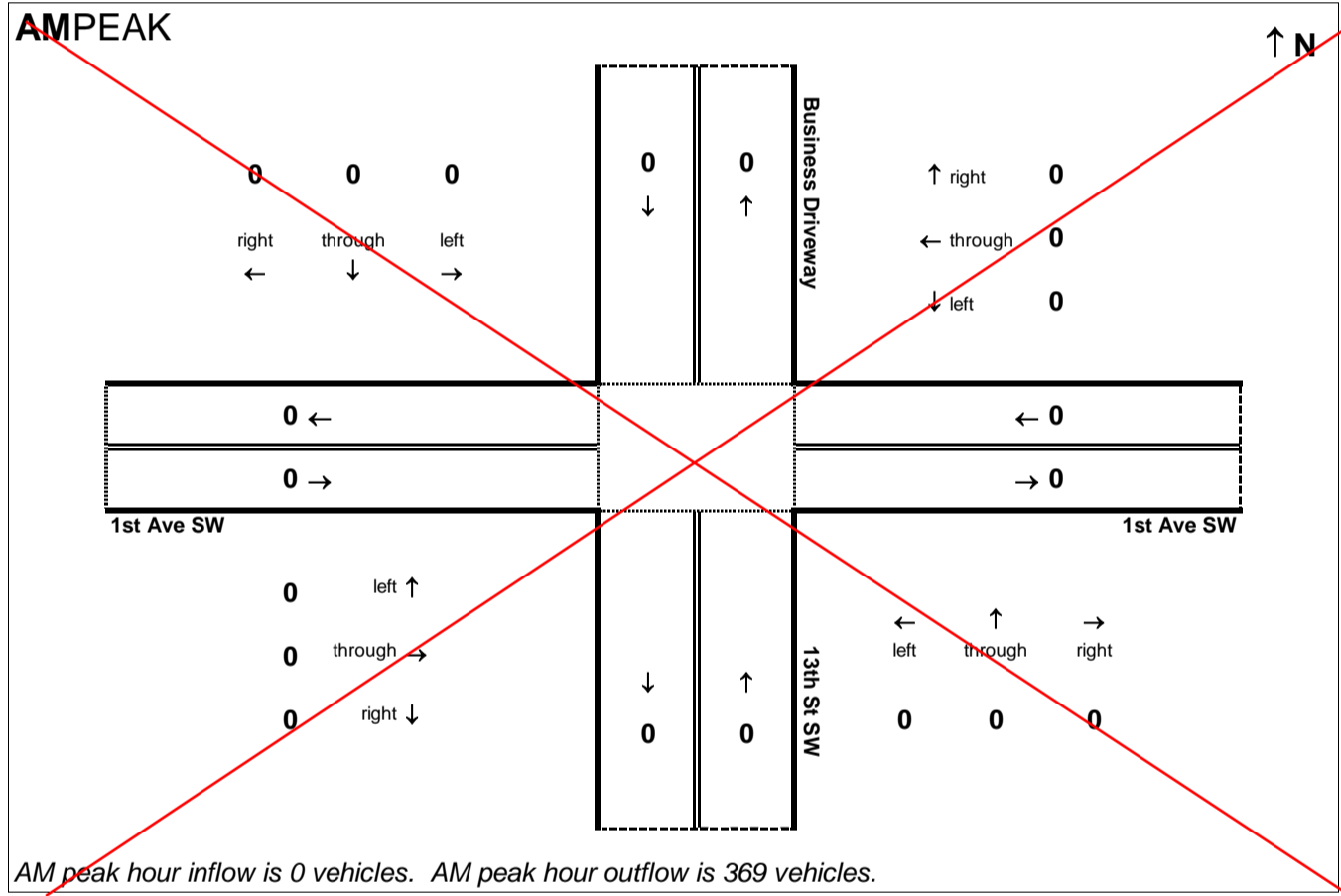
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

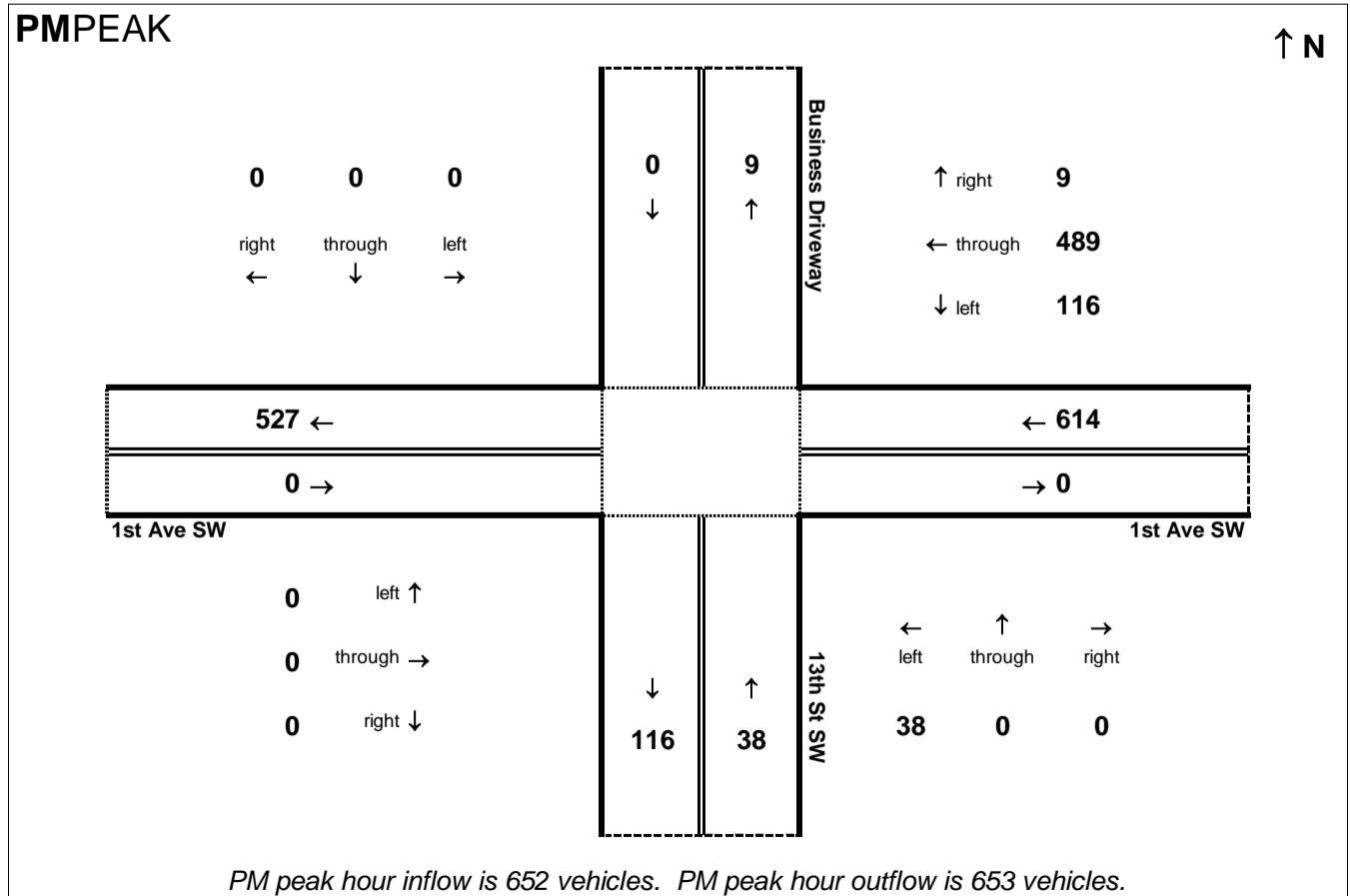
Project:
U-4700

AMPEAK



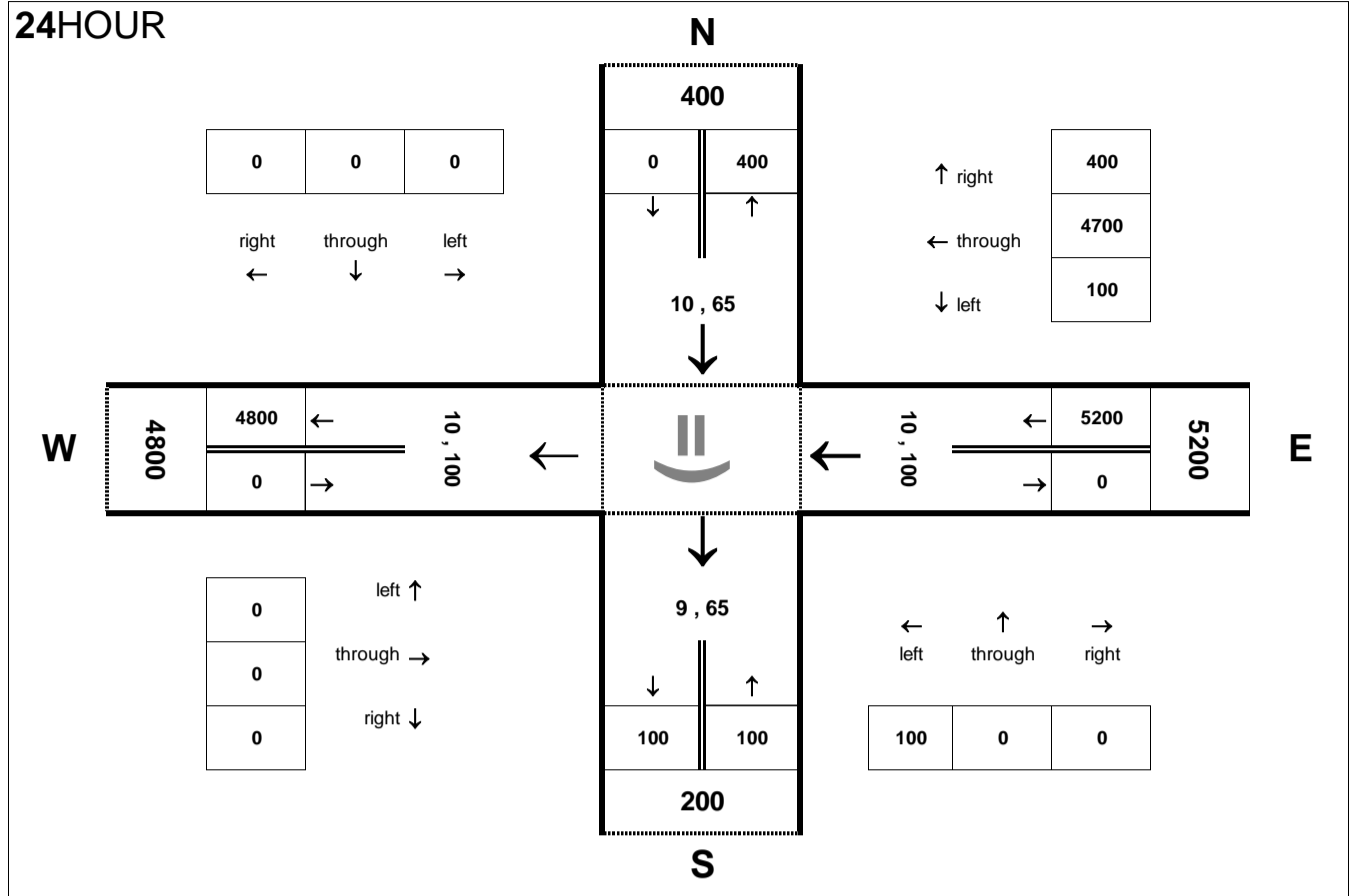
AM peak hour inflow is 0 vehicles. AM peak hour outflow is 369 vehicles.

PMPEAK



PM peak hour inflow is 652 vehicles. PM peak hour outflow is 653 vehicles.

24HOUR



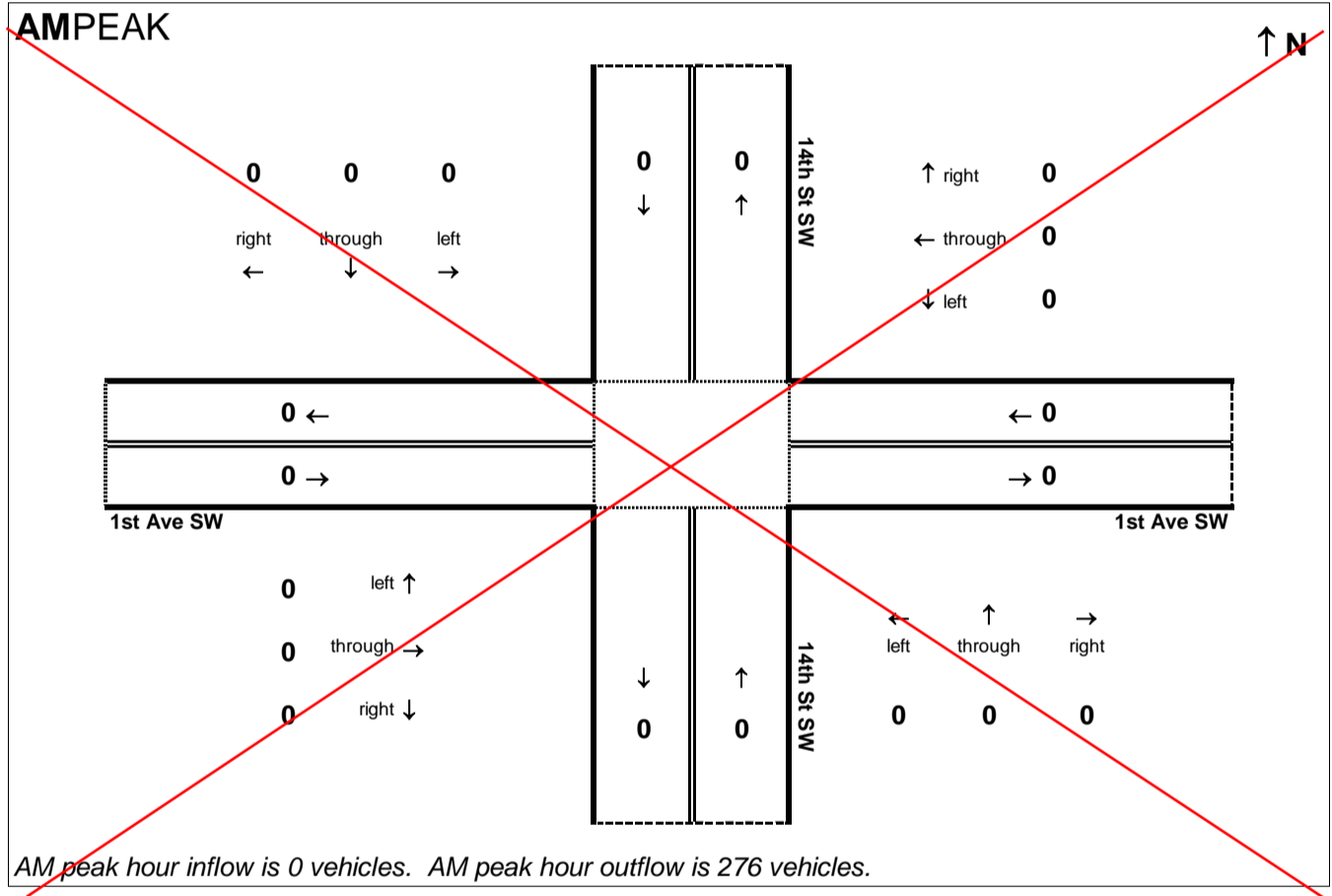
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

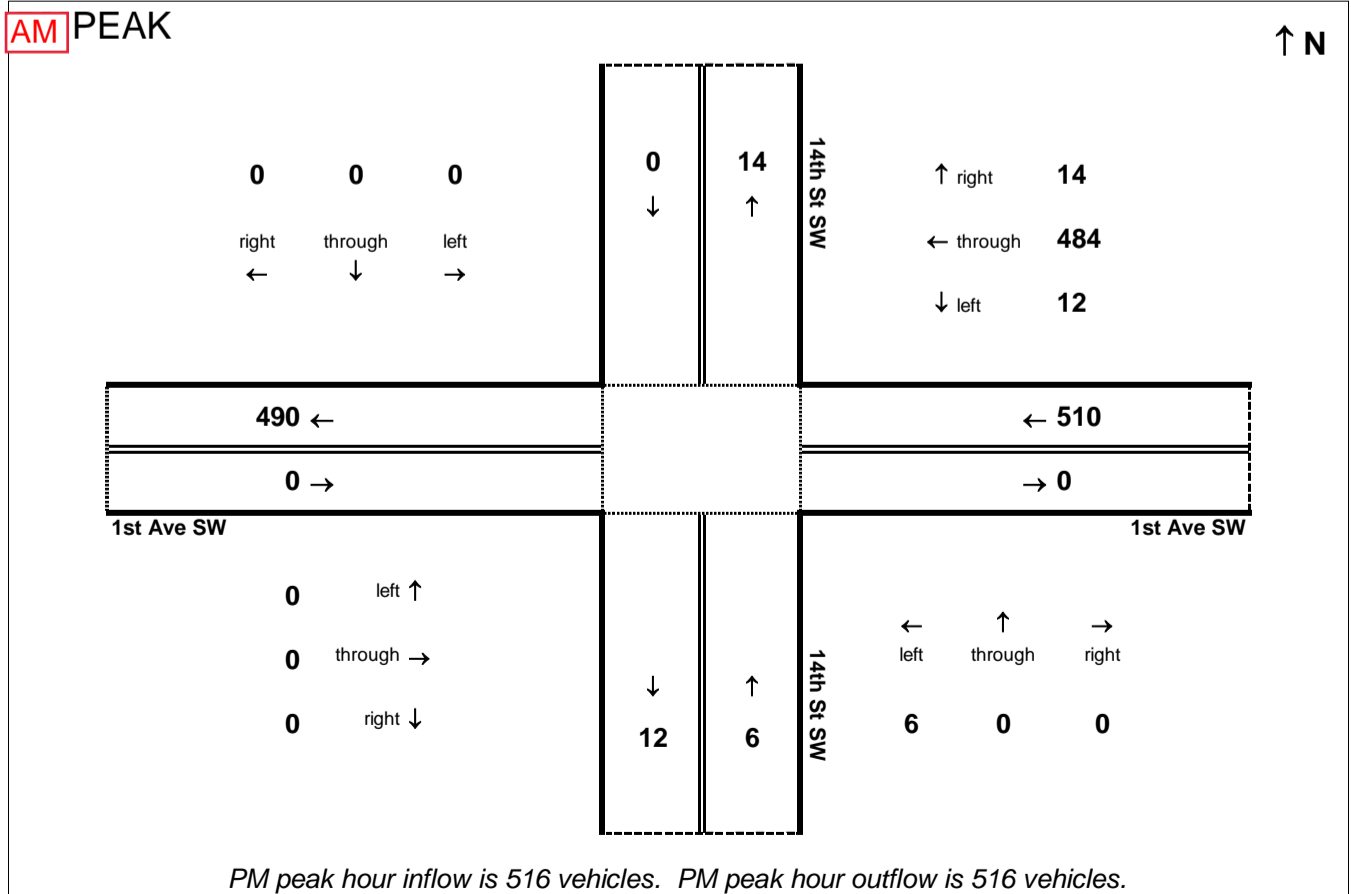
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 276 vehicles.

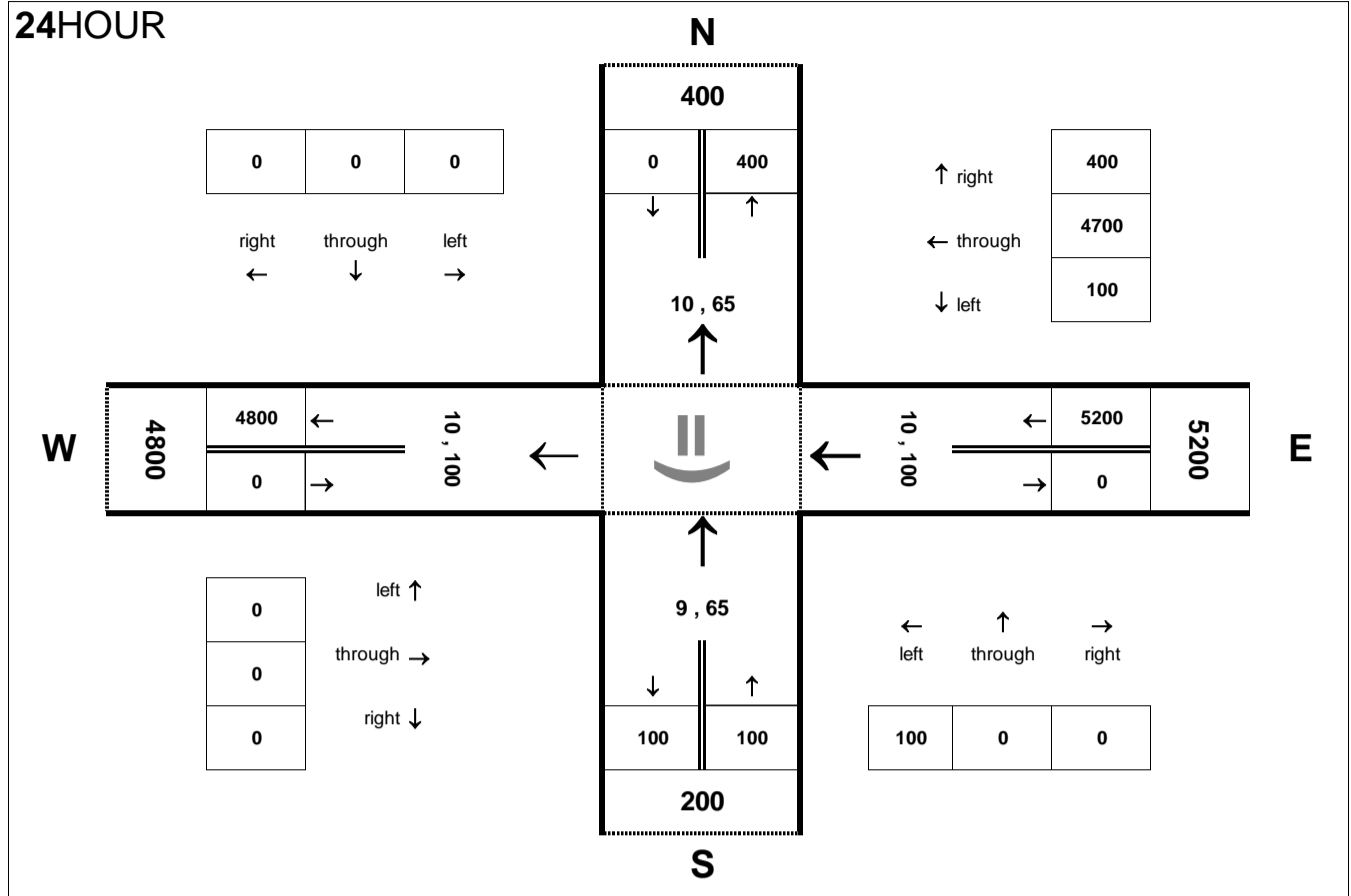
AM PEAK



PM peak hour inflow is 516 vehicles. PM peak hour outflow is 516 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



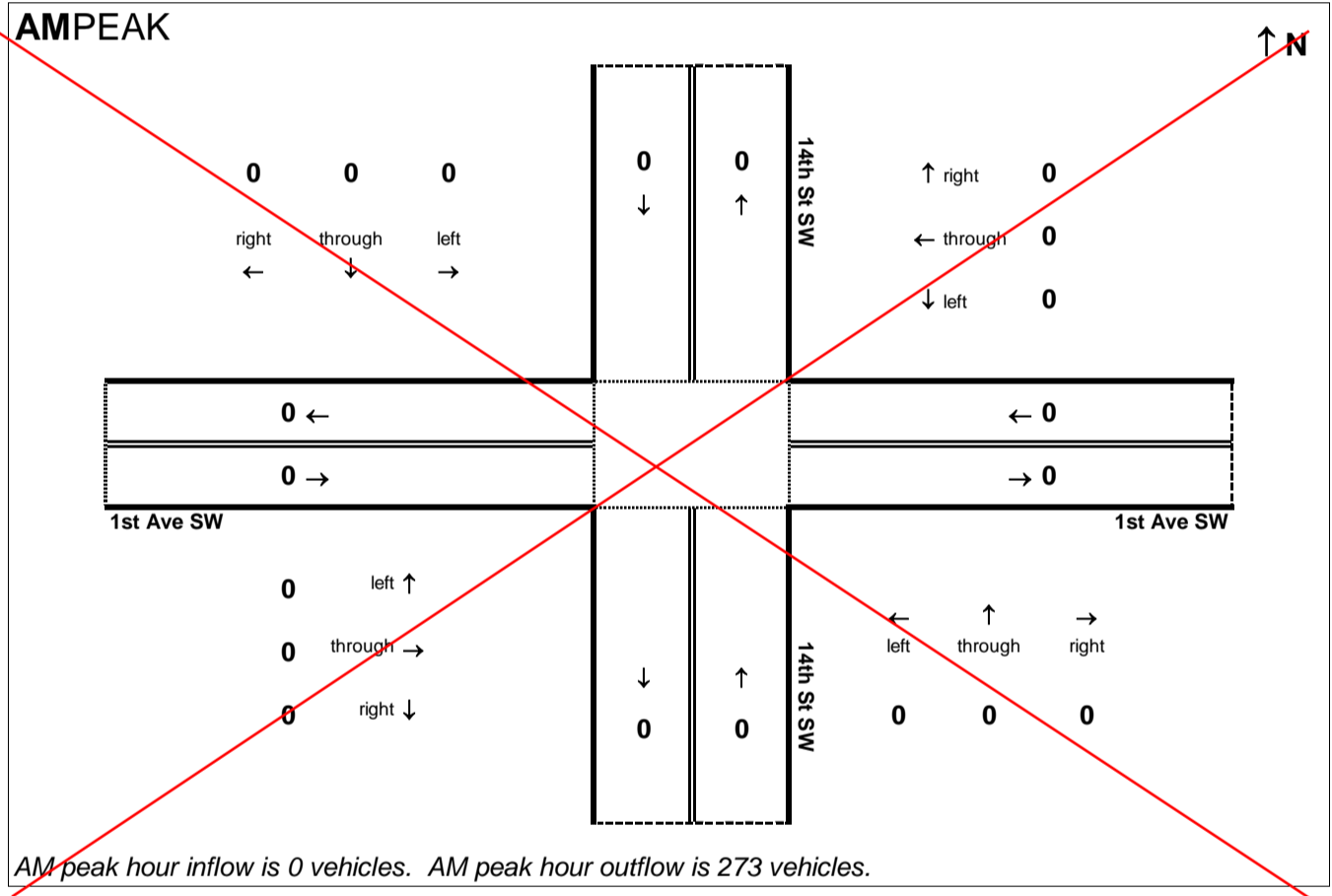
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

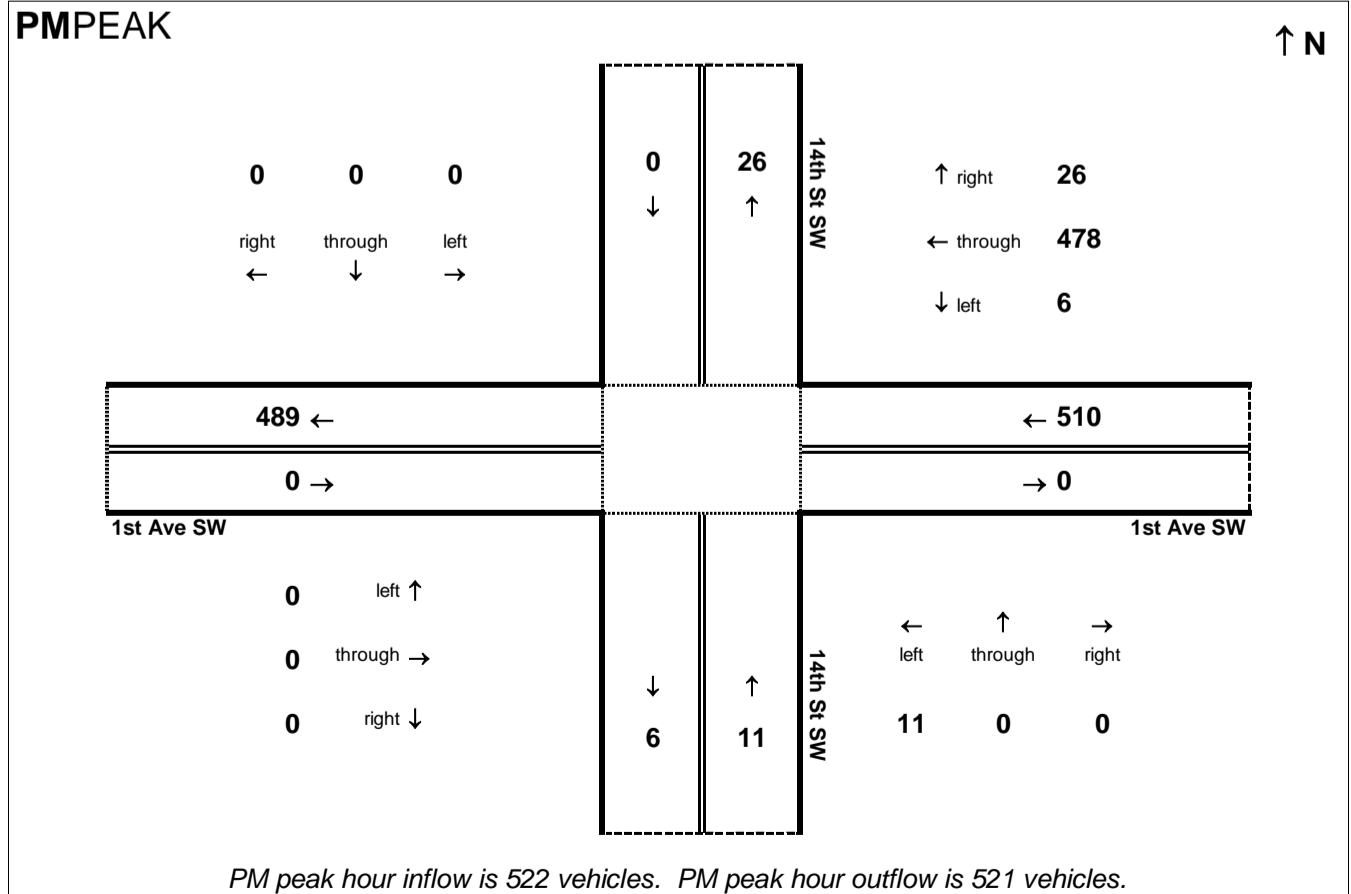
Traffic Data Year:
2040 Build 1

Project:
U-4700

AMPEAK

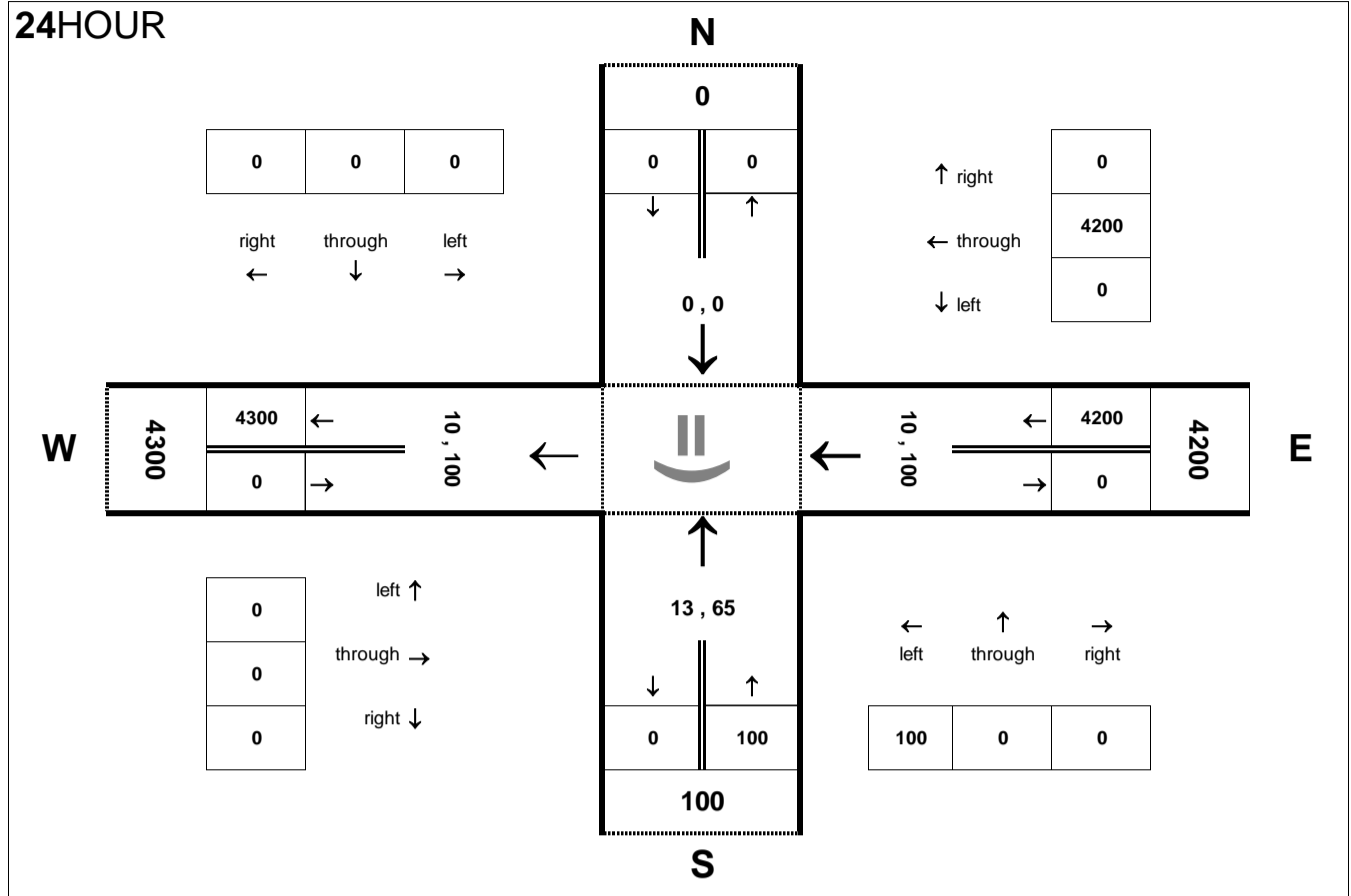


PMPEAK



PM peak hour inflow is 522 vehicles. PM peak hour outflow is 521 vehicles.

24HOUR



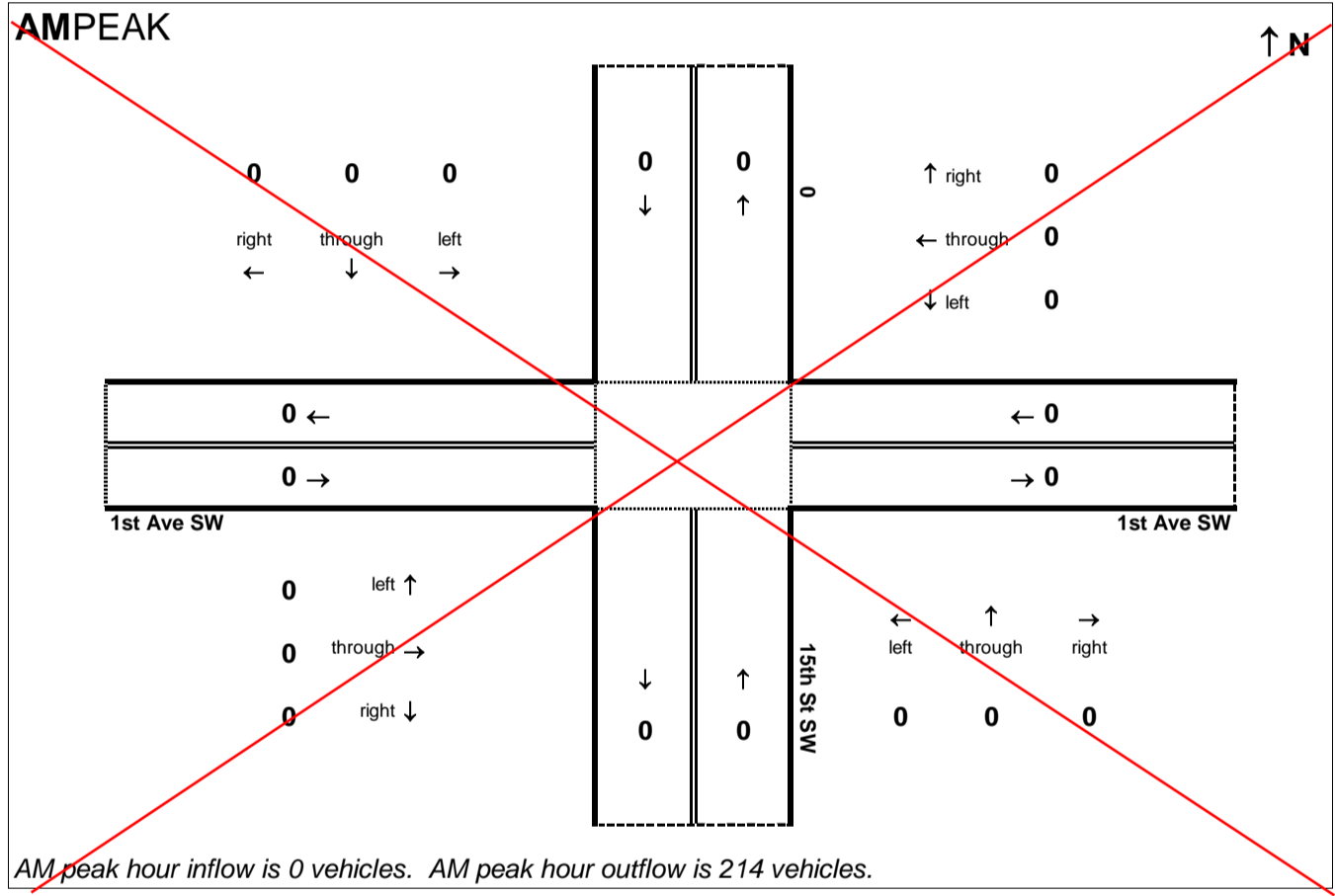
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

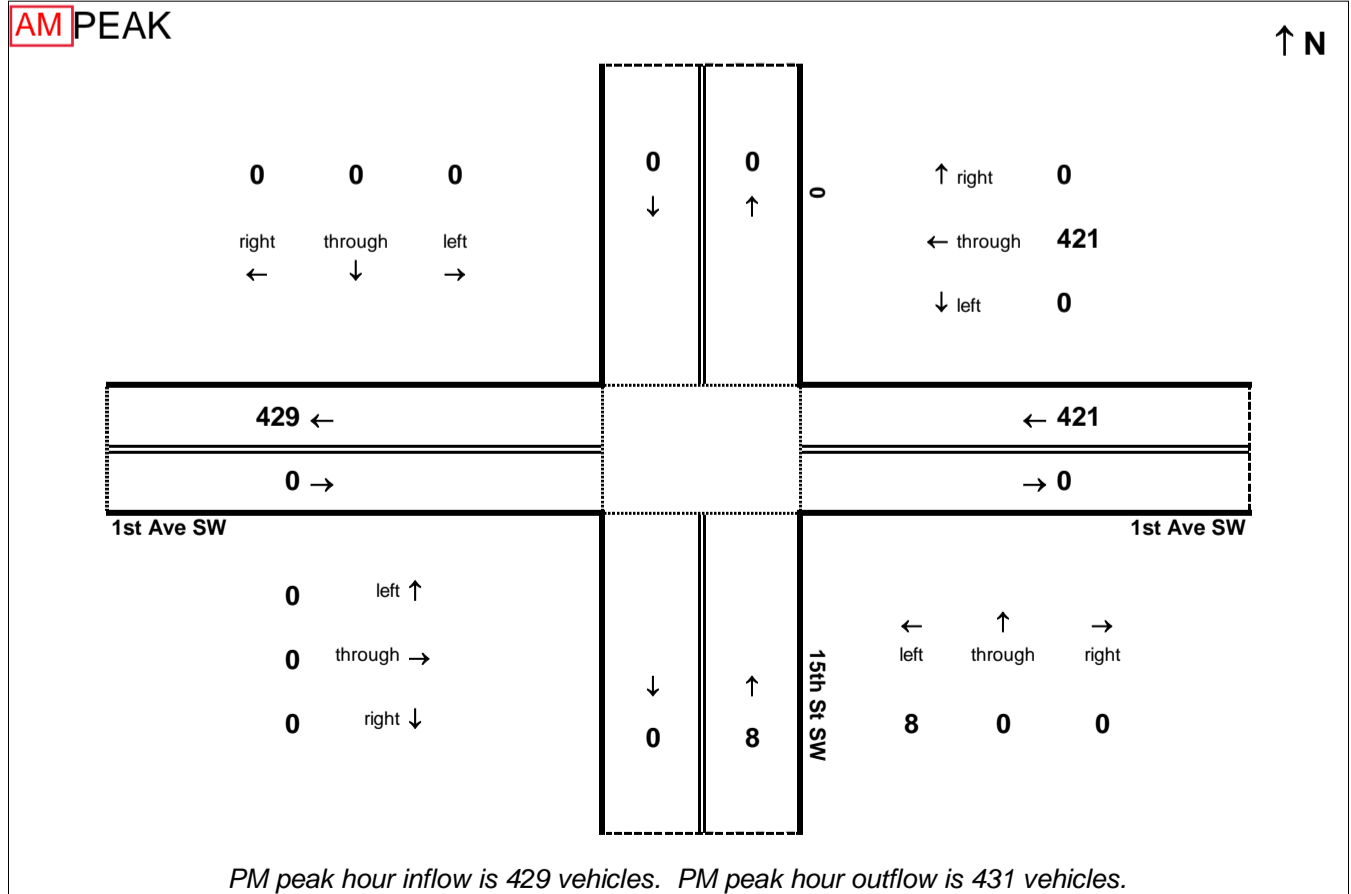
Project:
U-4700

~~**AM PEAK**~~



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 214 vehicles.

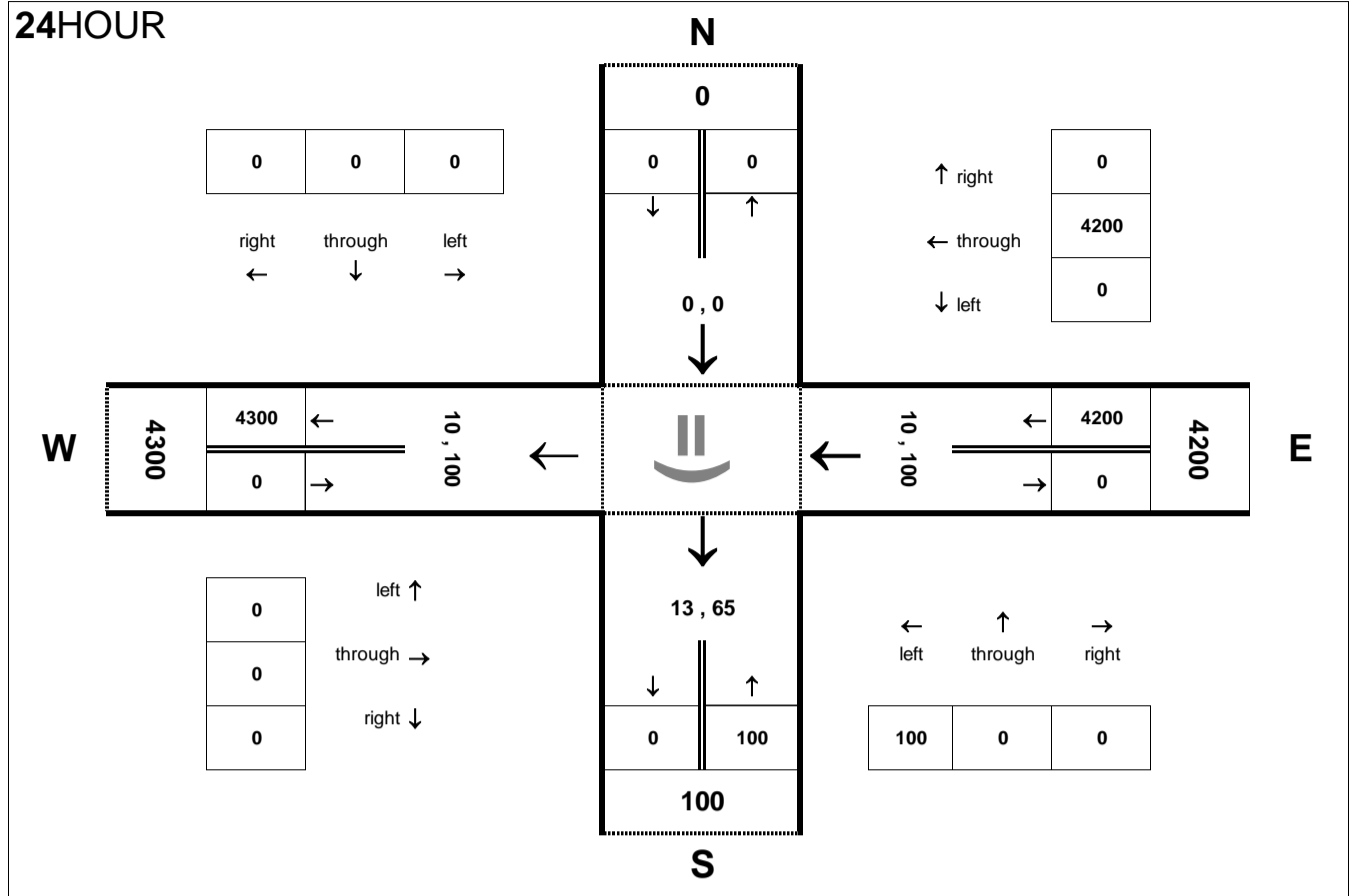
AM PEAK



PM peak hour inflow is 429 vehicles. PM peak hour outflow is 431 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



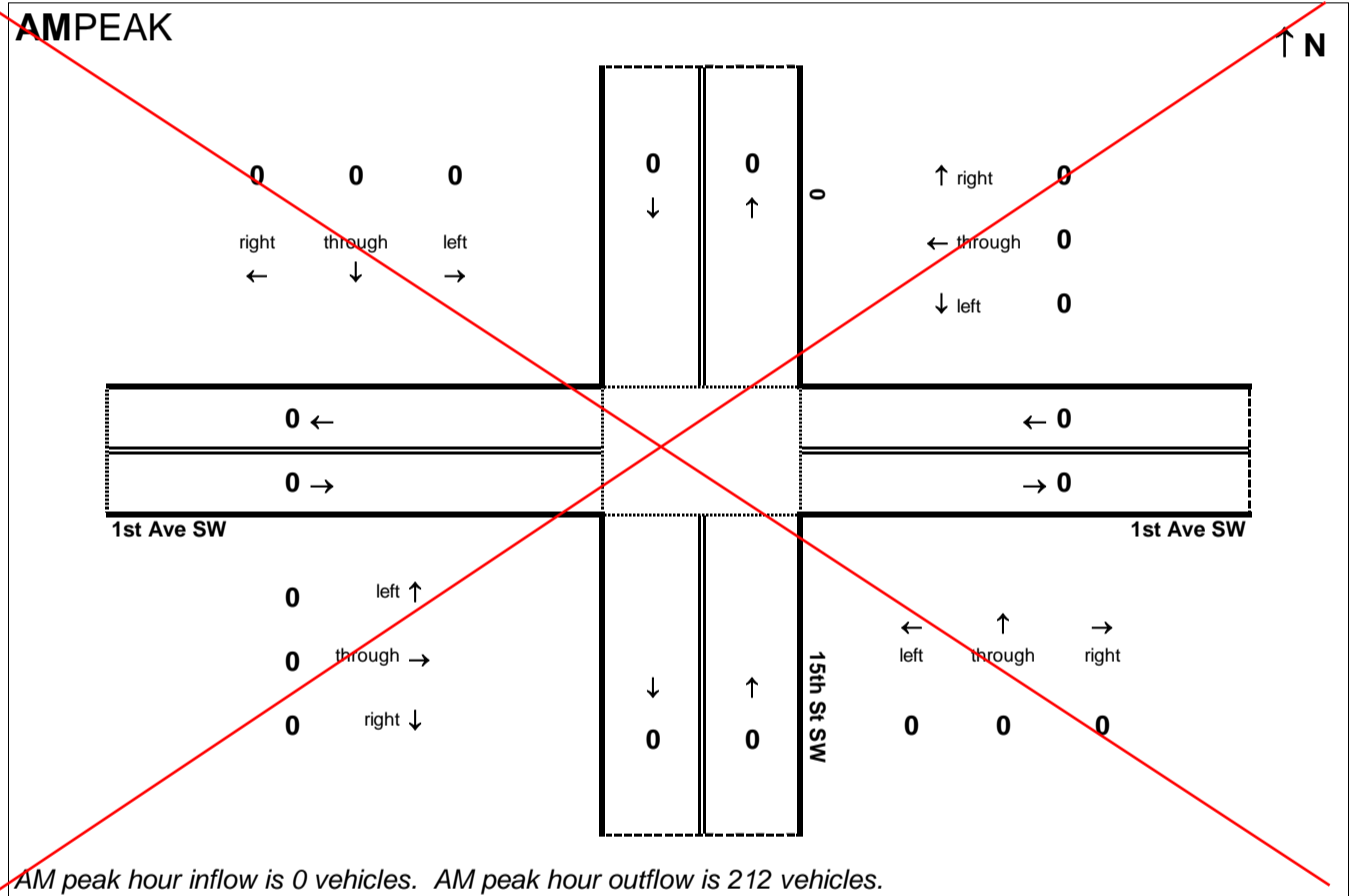
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

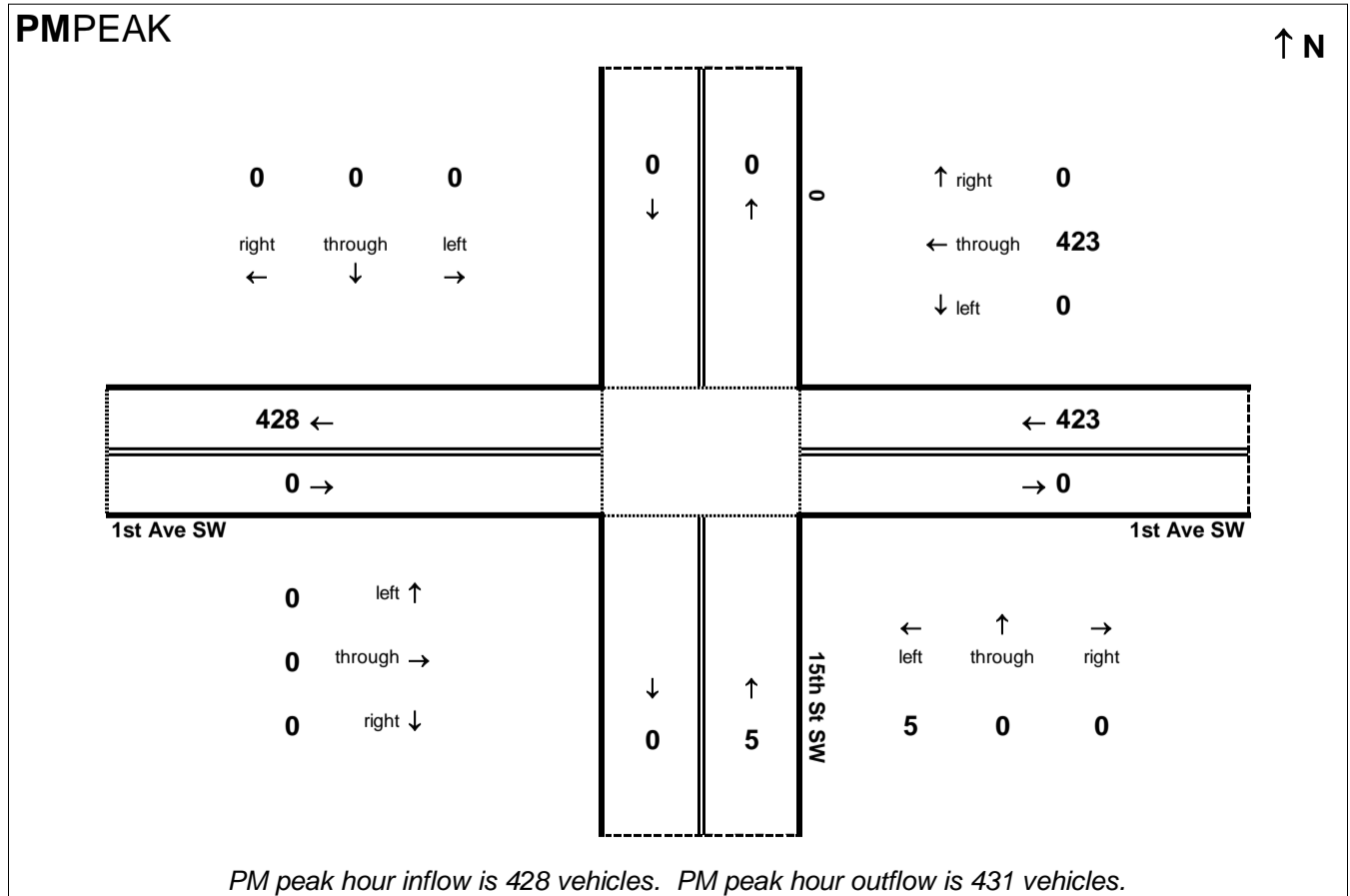
Project:
U-4700

AMPEAK

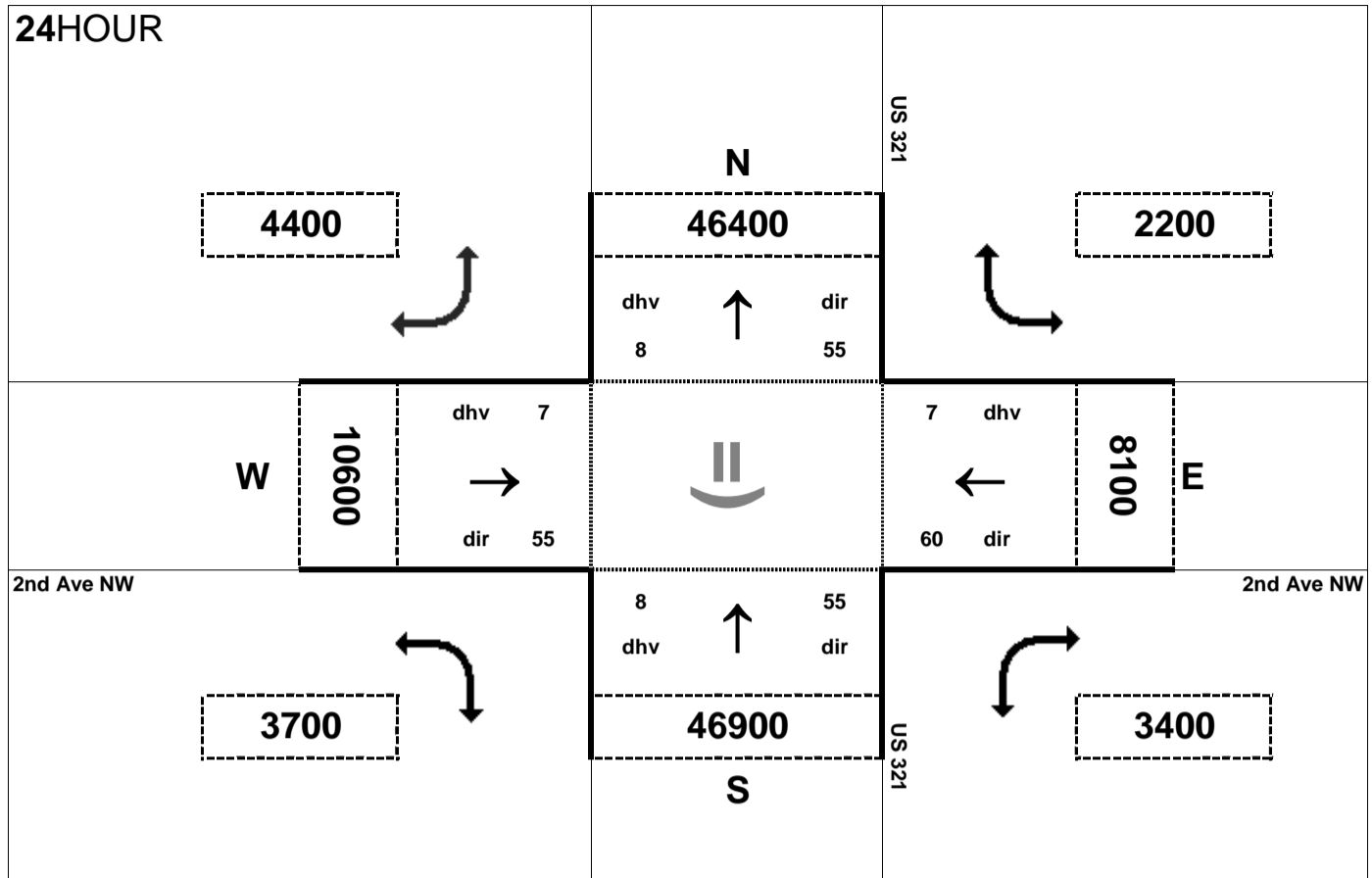


AM peak hour inflow is 0 vehicles. AM peak hour outflow is 212 vehicles.

PMPEAK



PM peak hour inflow is 428 vehicles. PM peak hour outflow is 431 vehicles.

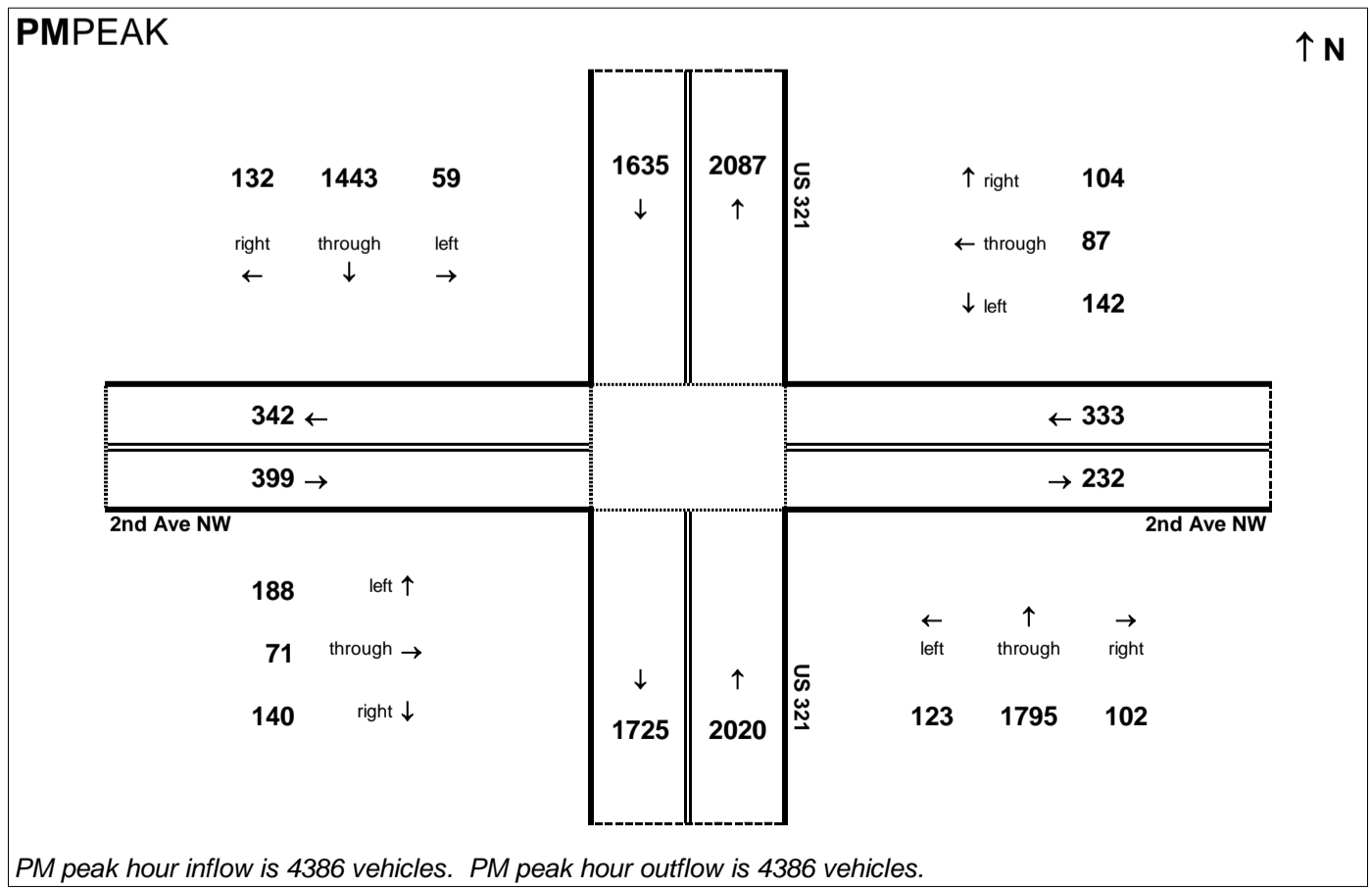
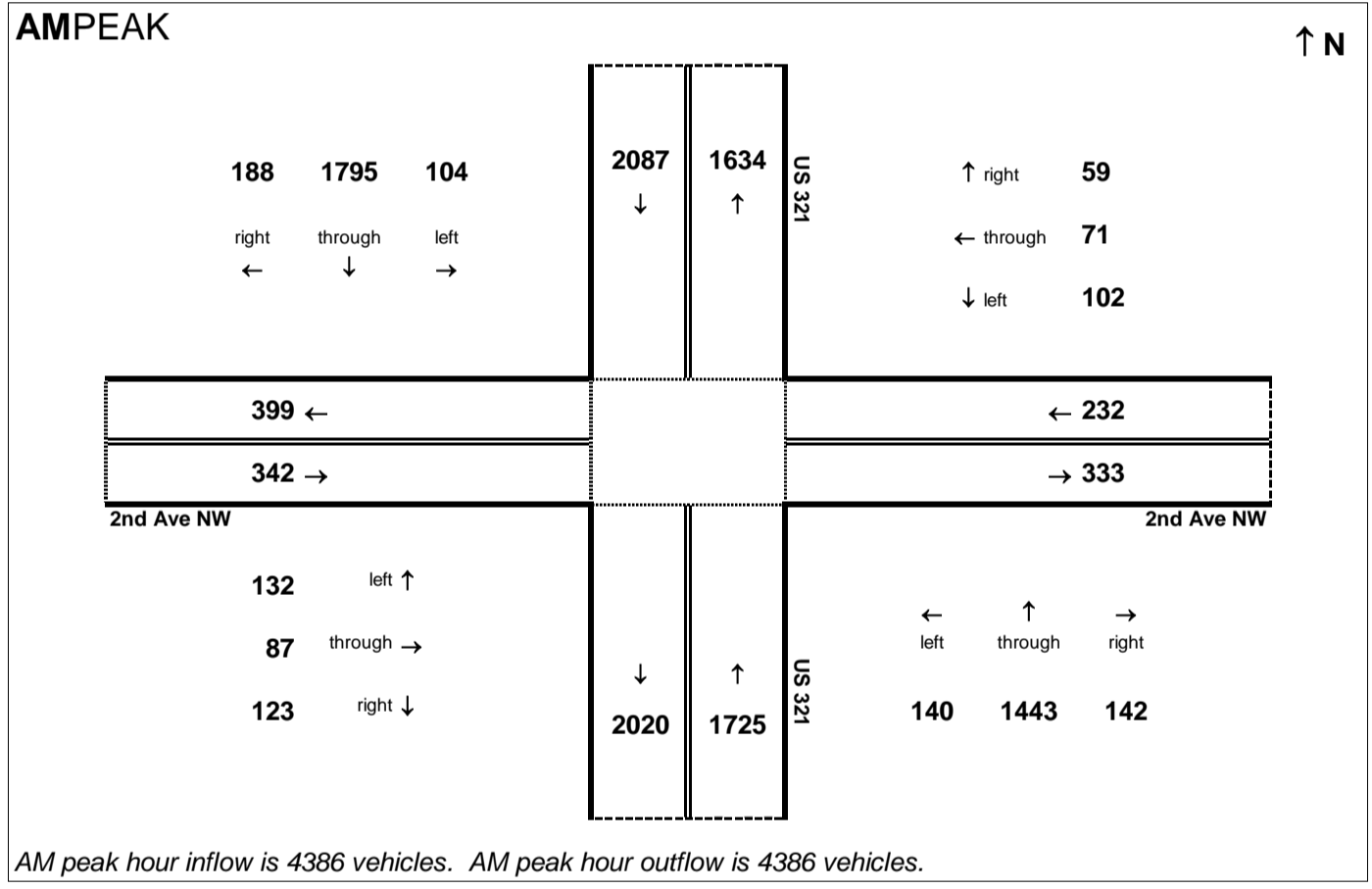


Peak Hour Volume Breakouts Report:
 Intersection of US 321 and 2nd Ave NW (SR 1306)

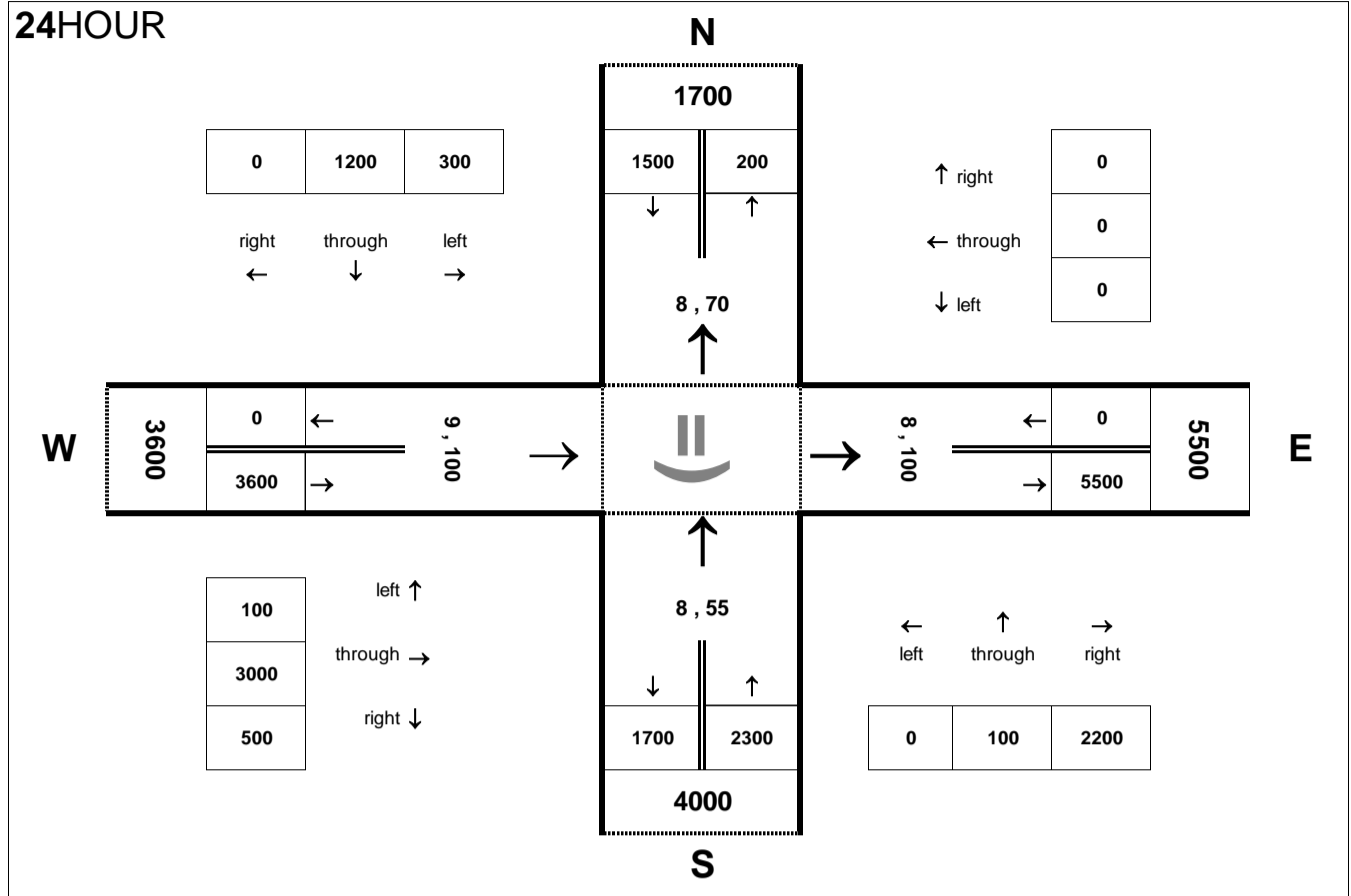
Traffic Forecast Release Date:
 February-17

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



24HOUR



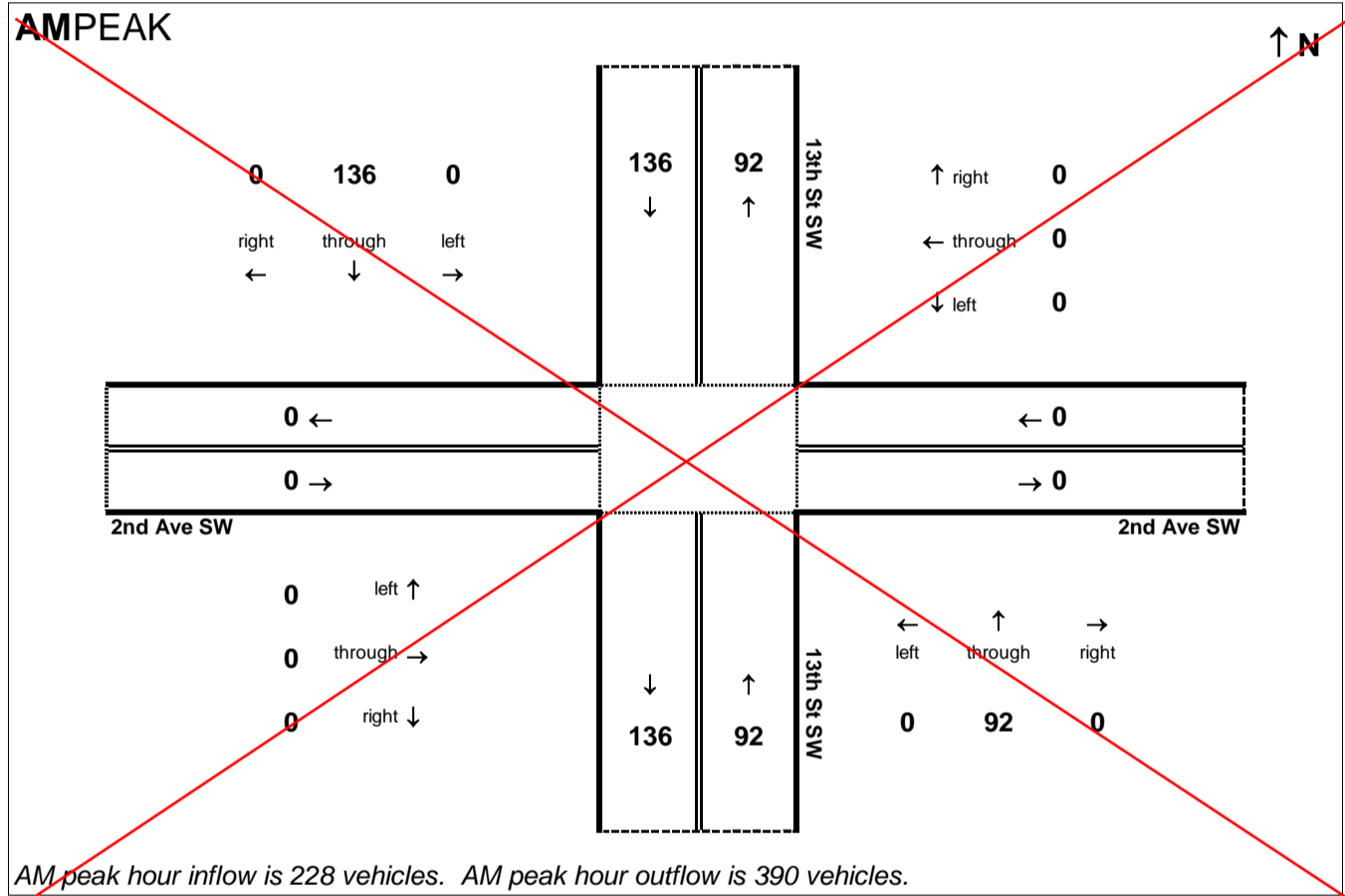
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

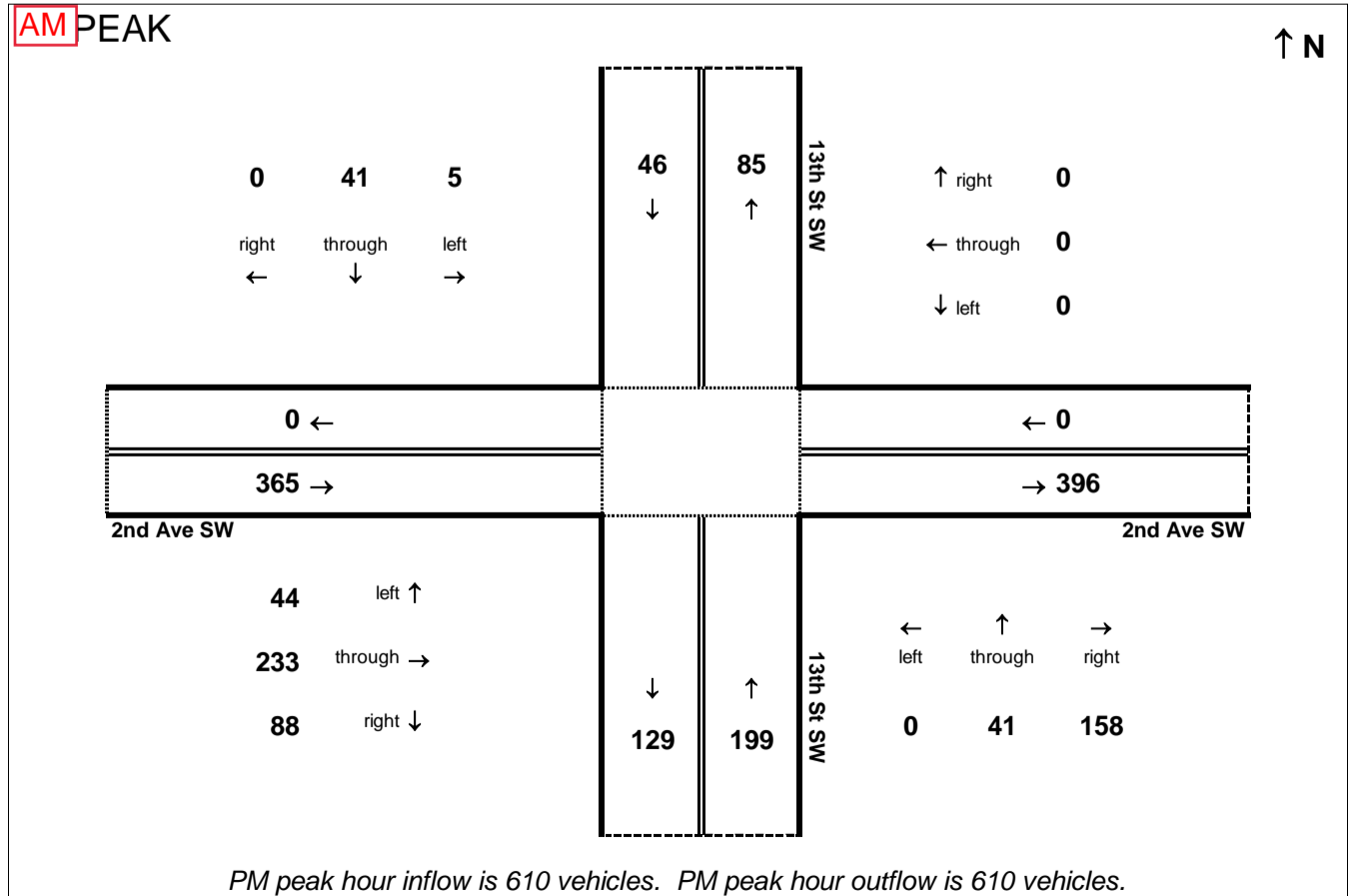
Traffic Data Year:
2040 Build 1

Project:
U-4700

AM PEAK

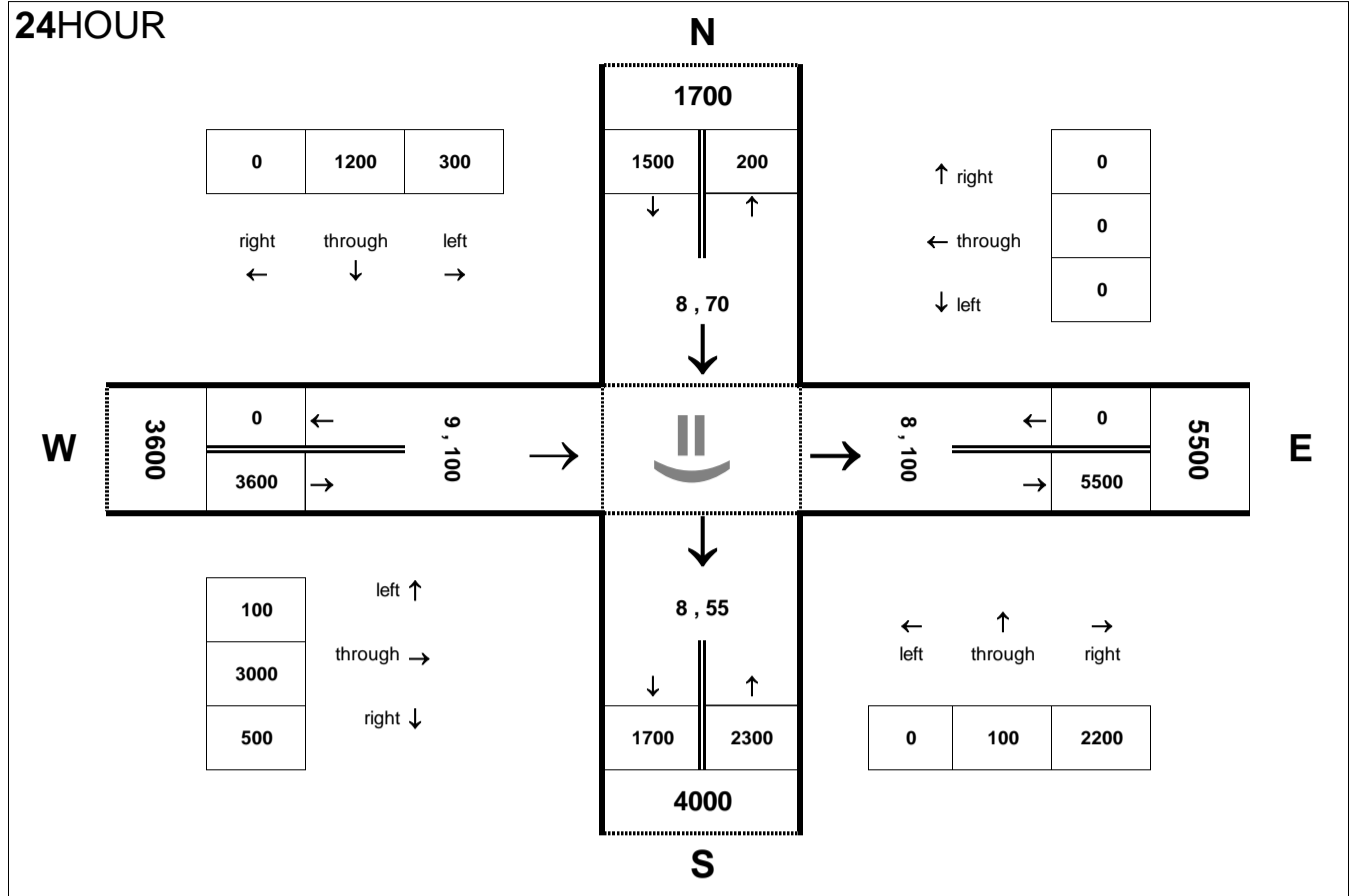


PM PEAK



Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



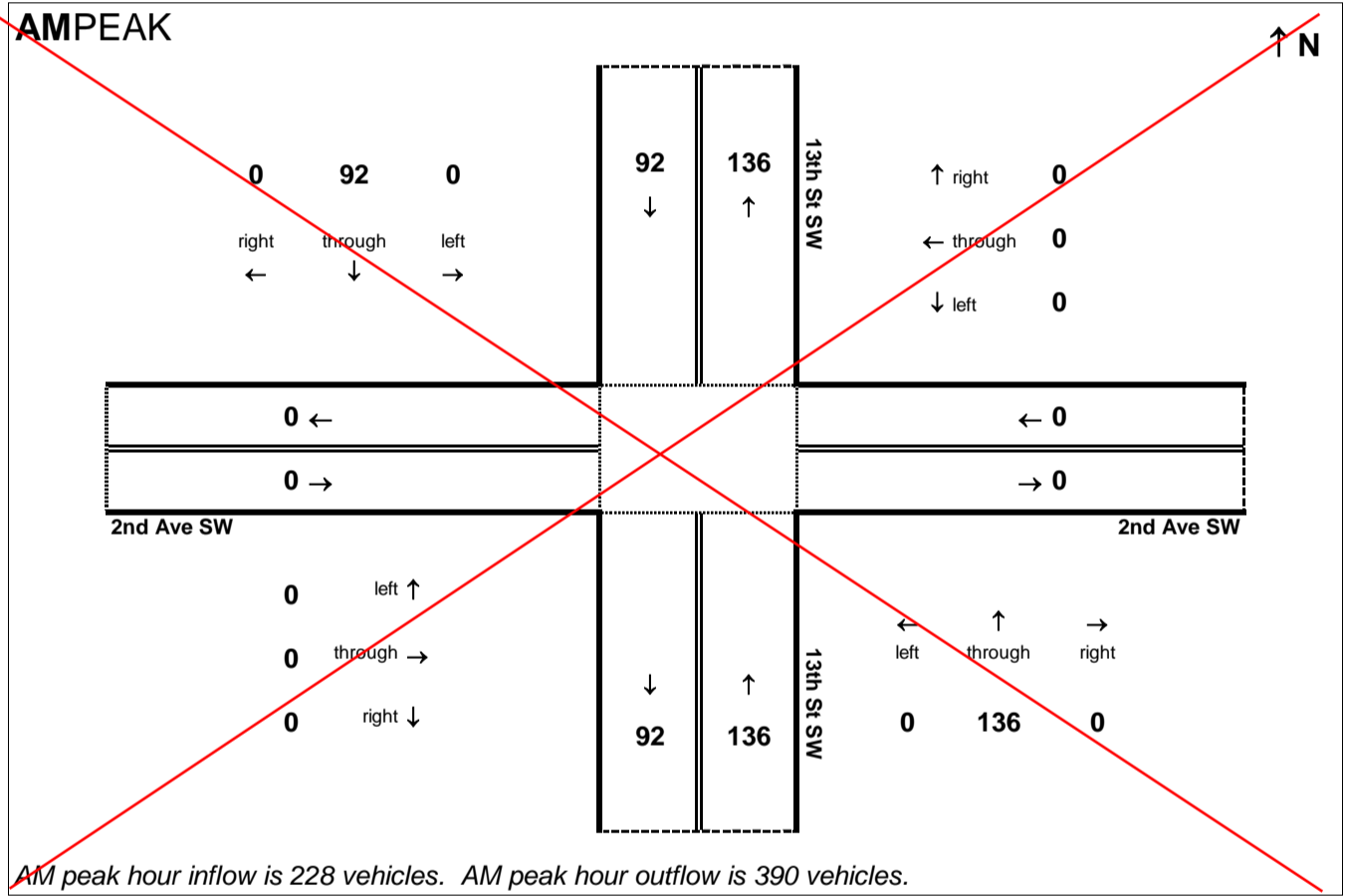
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

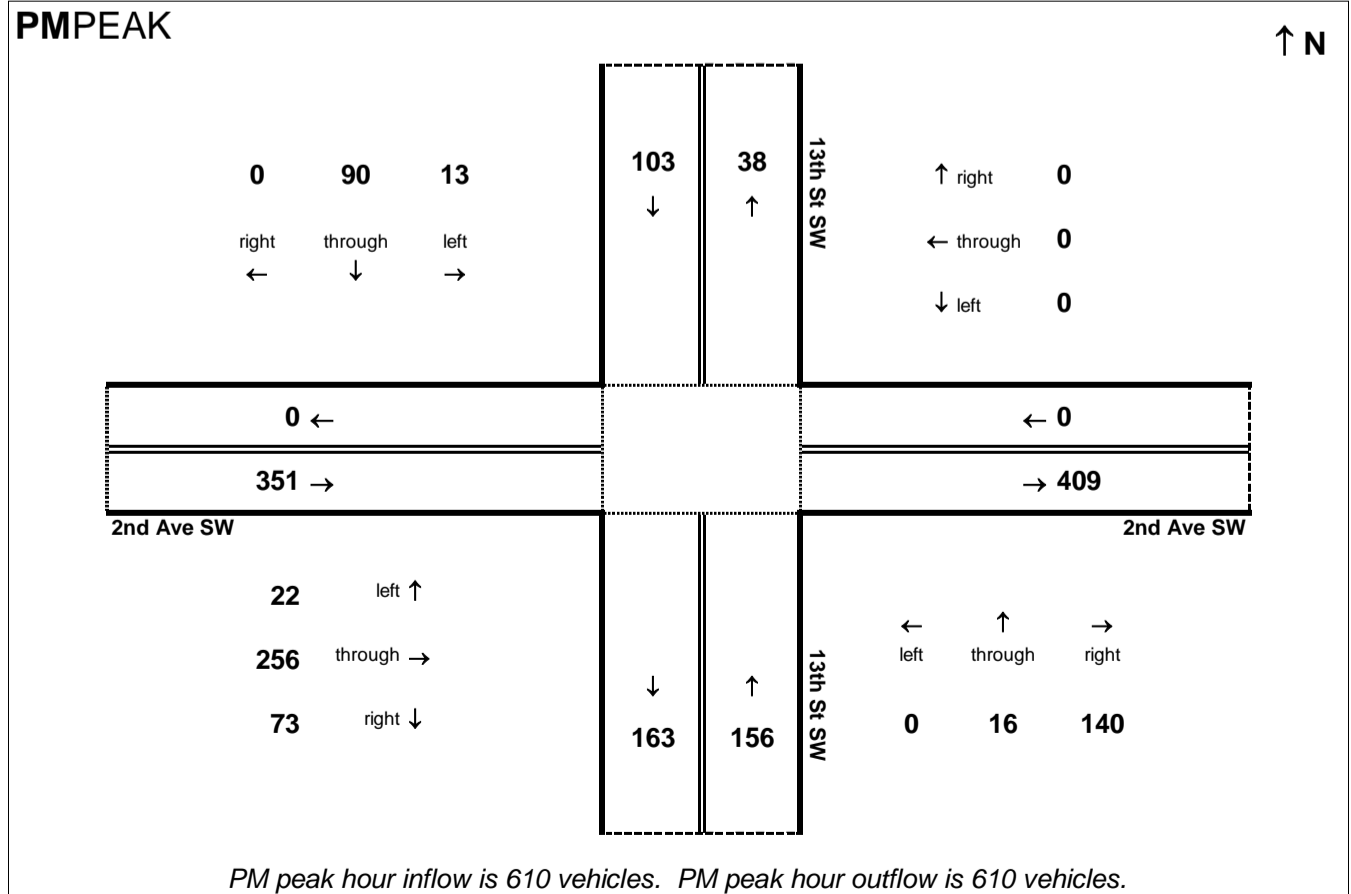
Project:
U-4700

~~**AMPEAK**~~



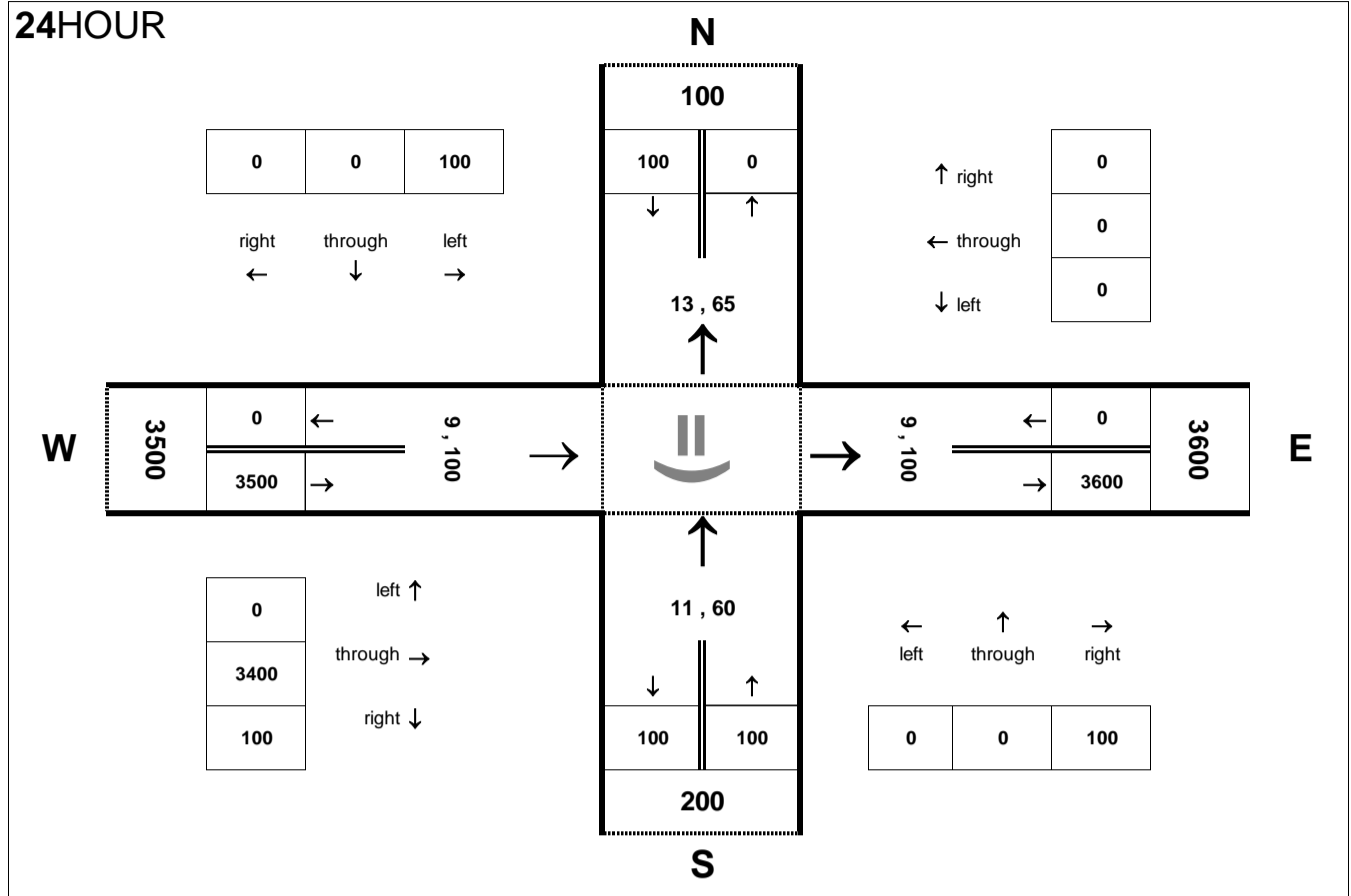
~~AM peak hour inflow is 228 vehicles. AM peak hour outflow is 390 vehicles.~~

PMPEAK



PM peak hour inflow is 610 vehicles. PM peak hour outflow is 610 vehicles.

24HOUR



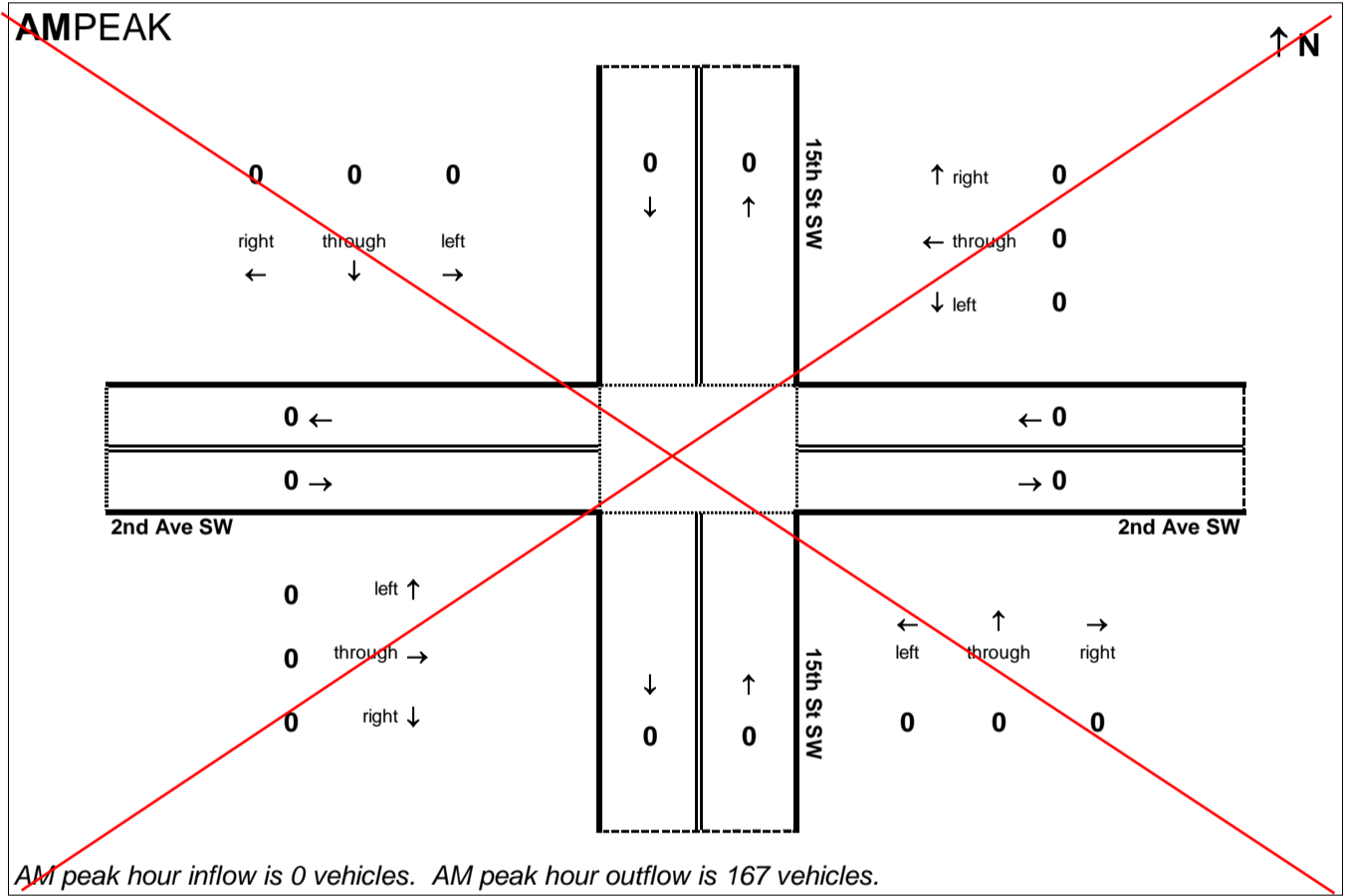
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

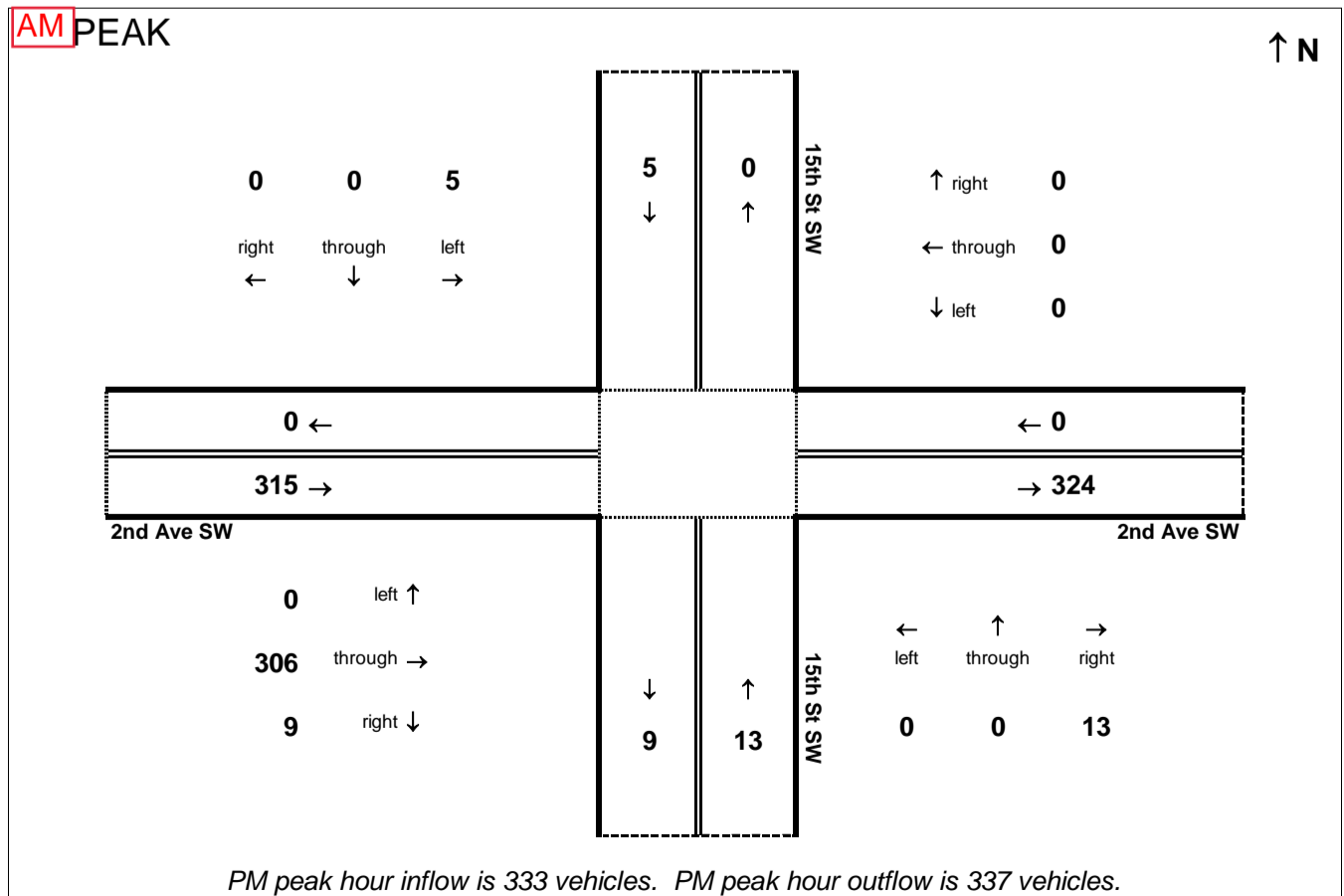
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 167 vehicles.

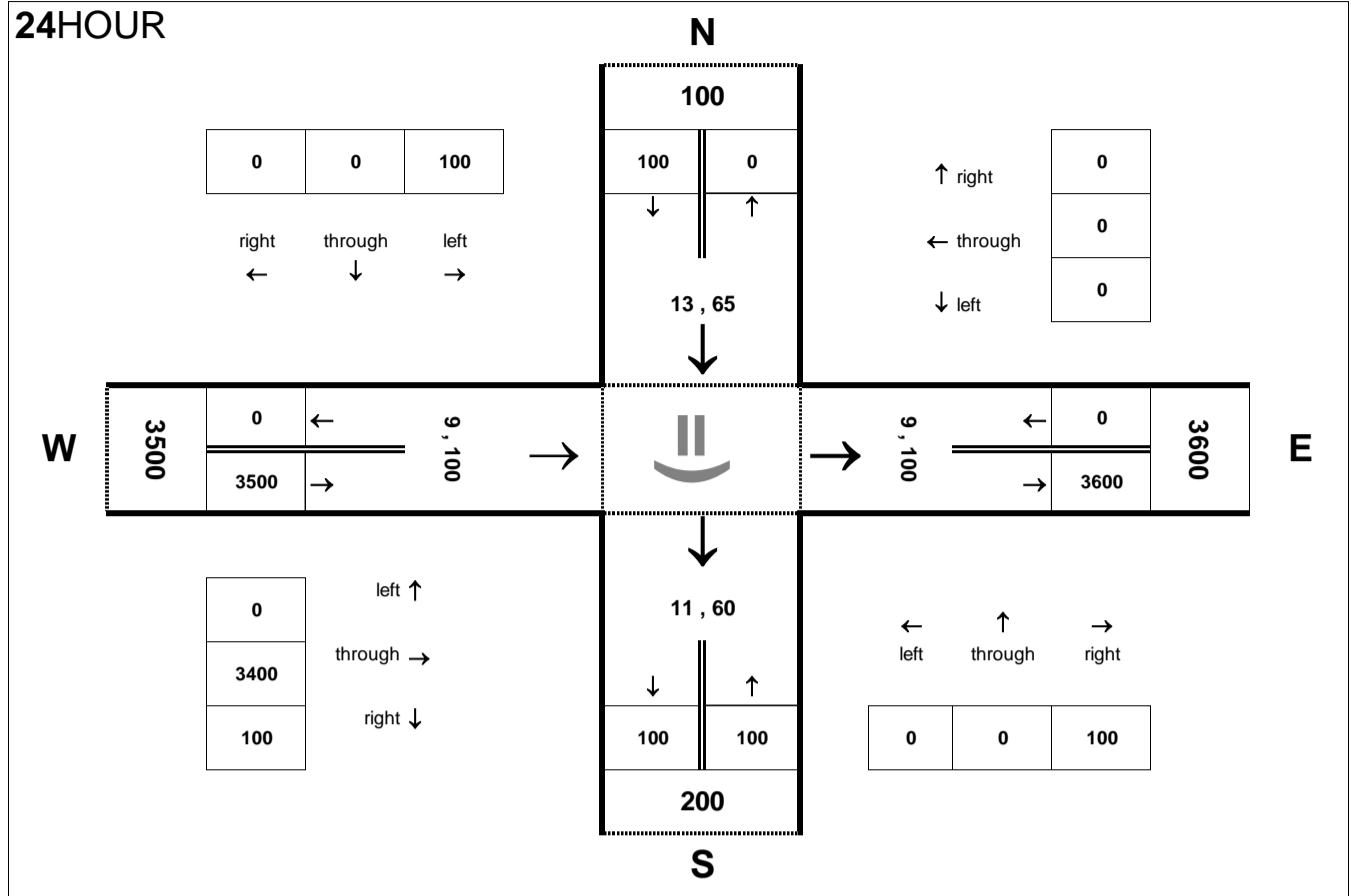
PM PEAK



PM peak hour inflow is 333 vehicles. PM peak hour outflow is 337 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



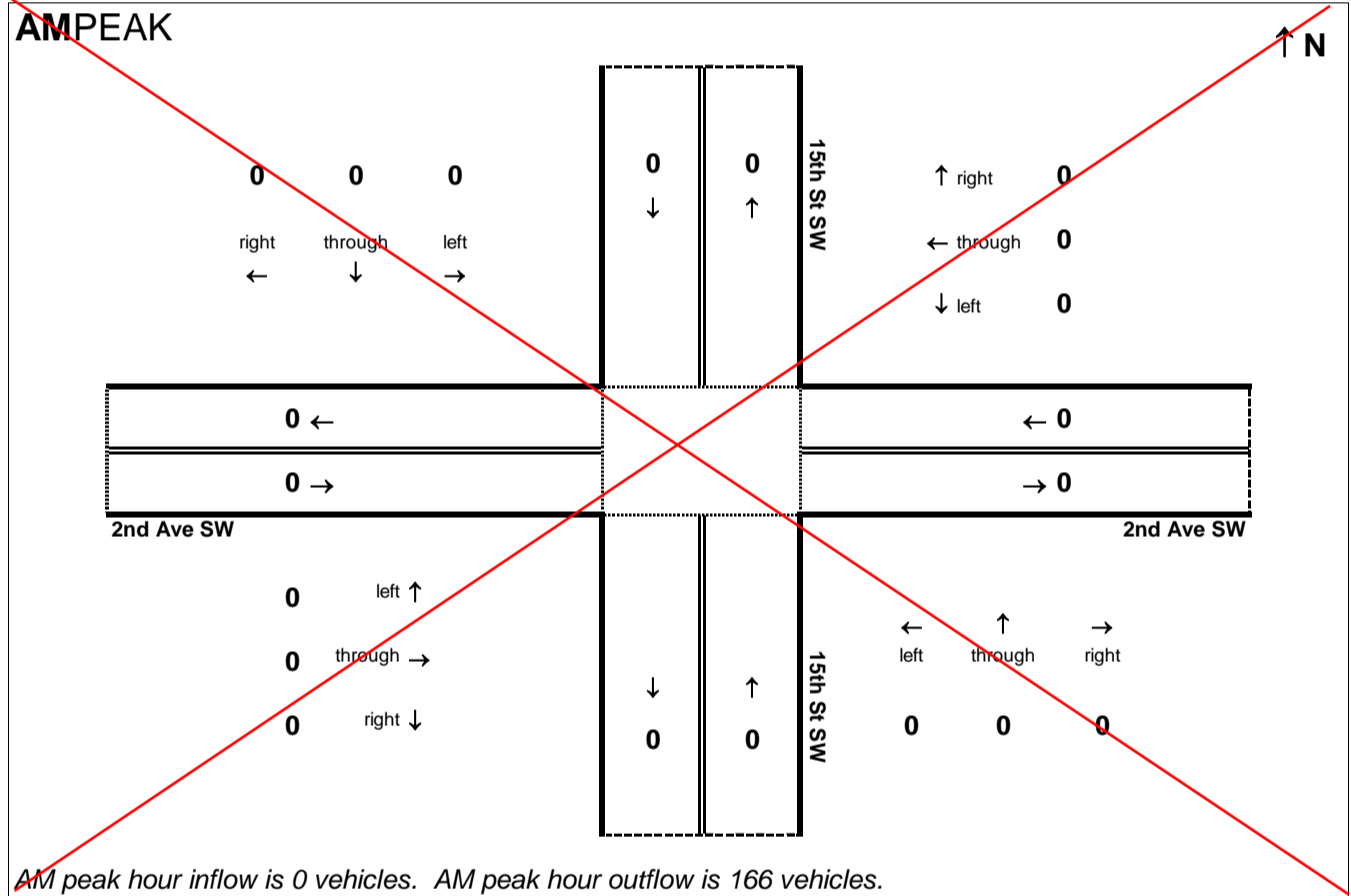
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

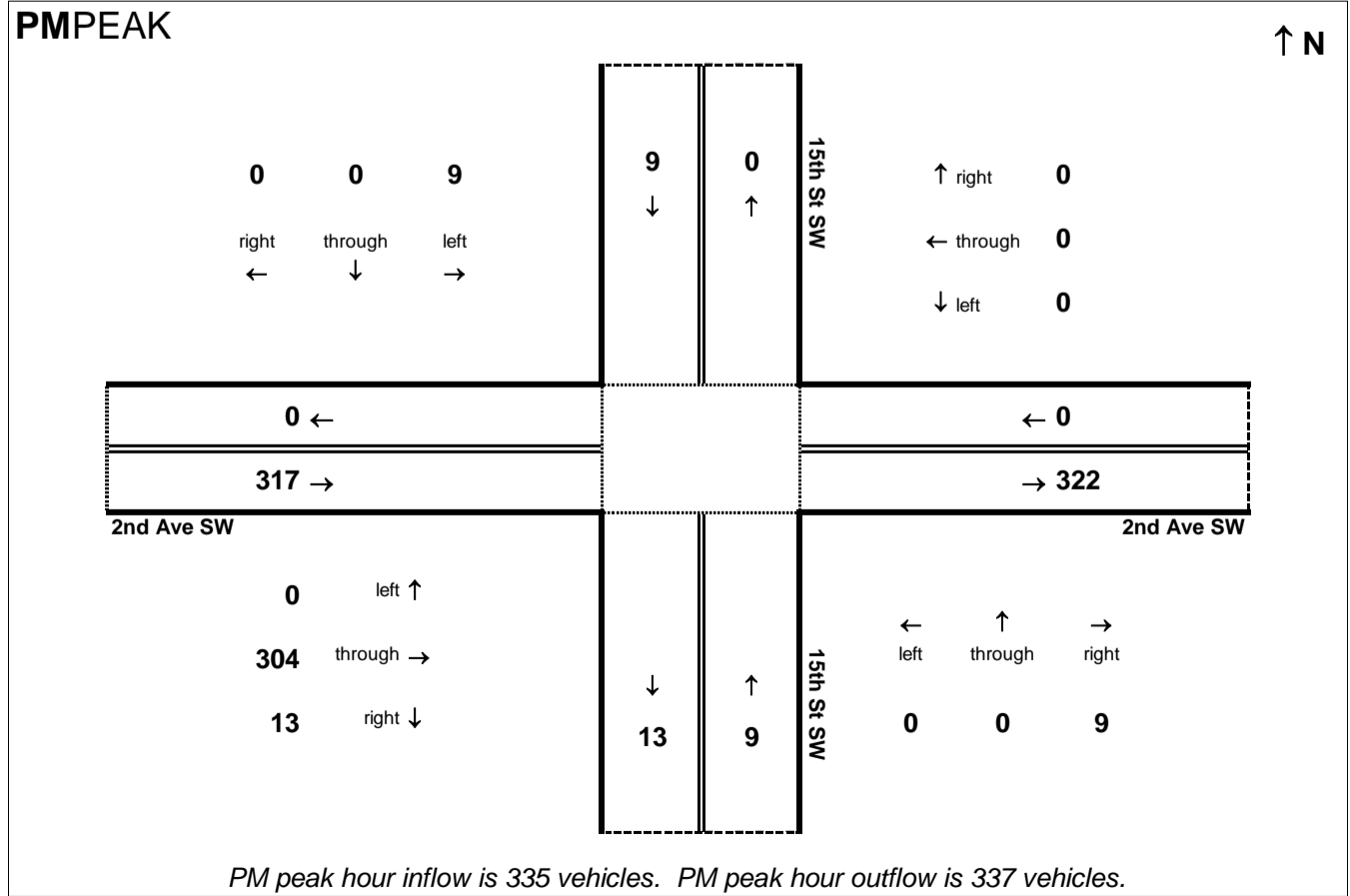
Traffic Data Year:
2040 Build 1

Project:
U-4700

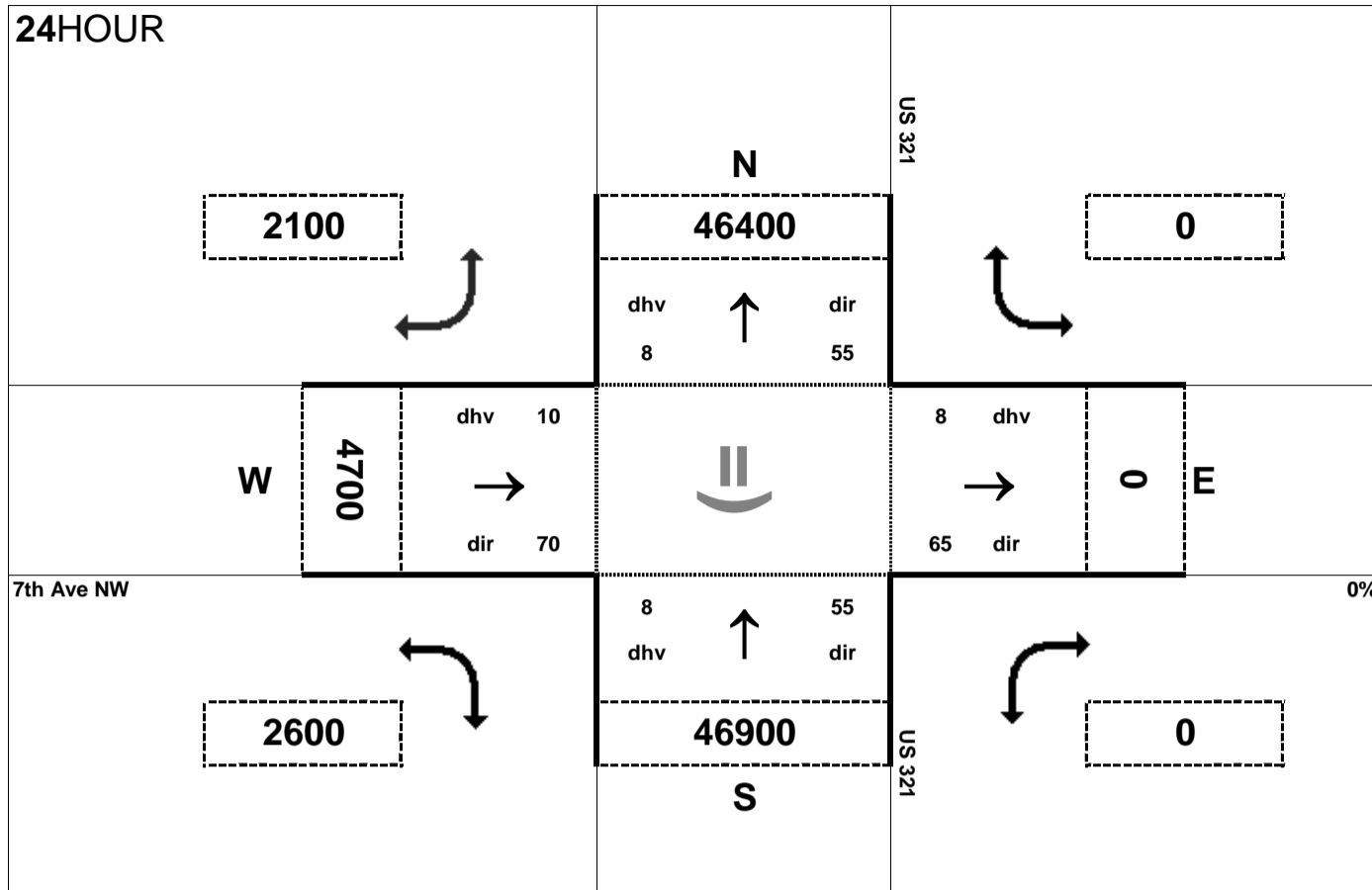
~~**AMPEAK**~~



PMPEAK



24HOUR



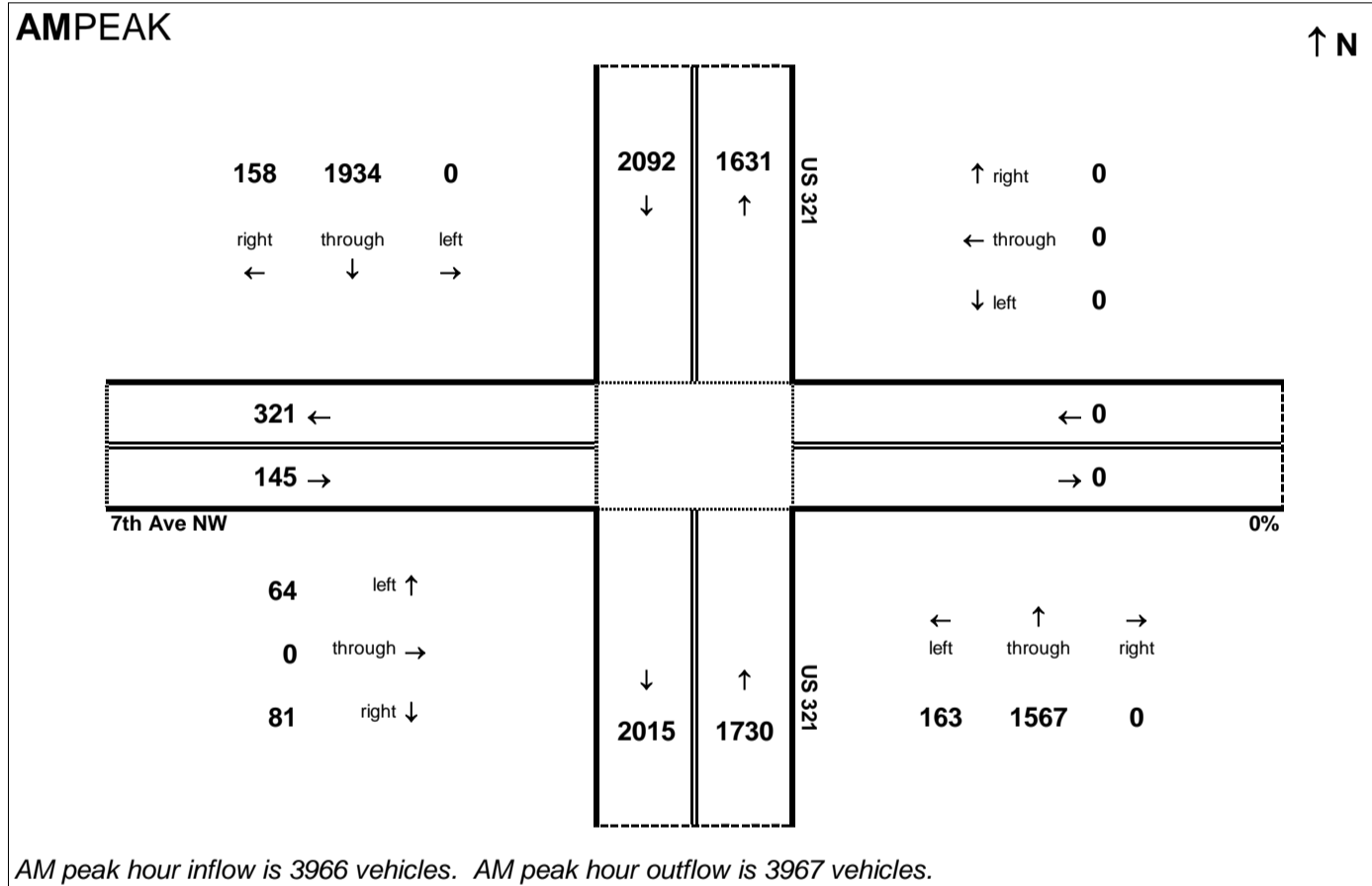
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 7th Ave NW

Traffic Forecast Release Date:
February-17

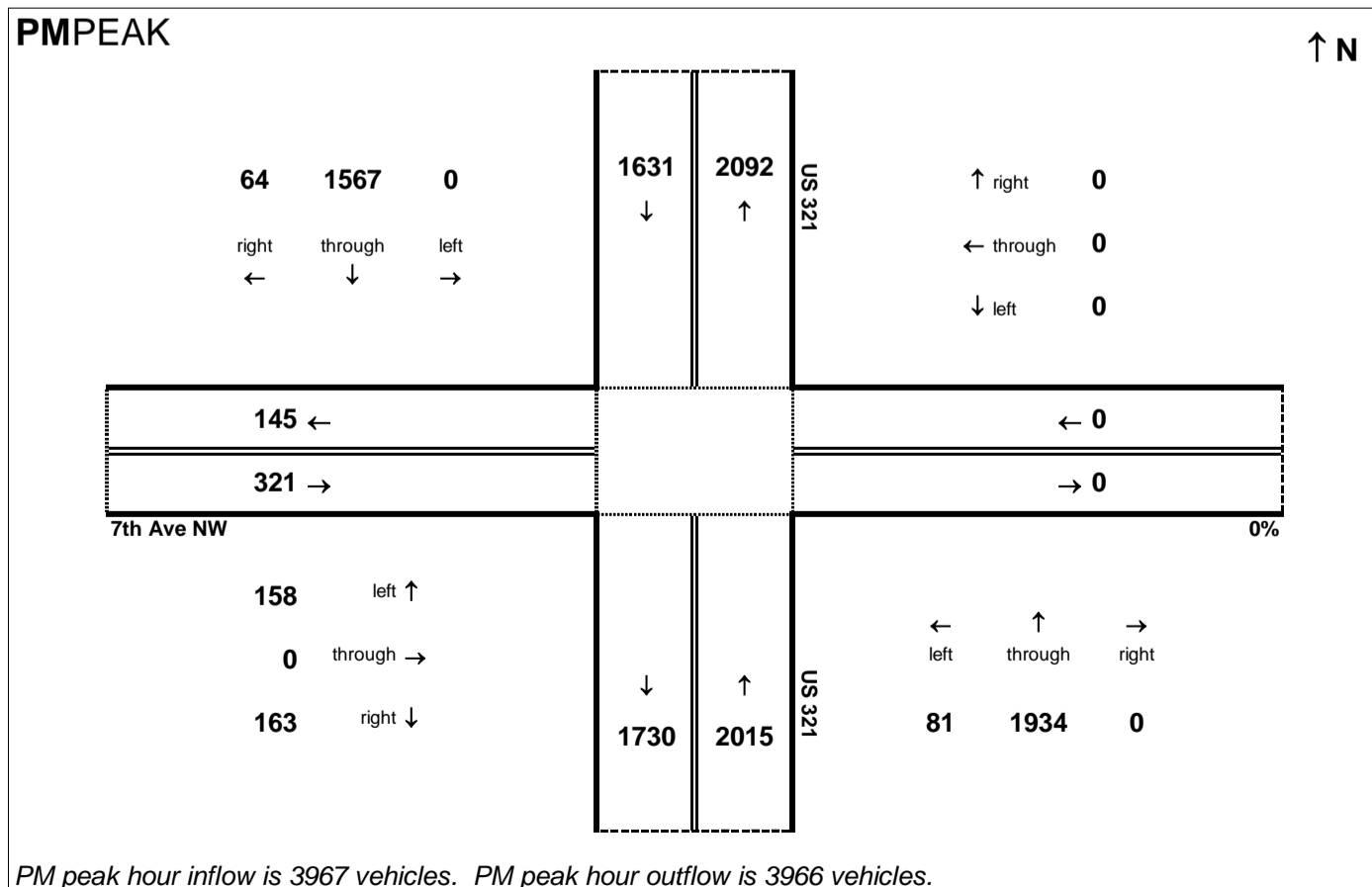
Traffic Data Year:
2040 Build 1

Project:
U-4700

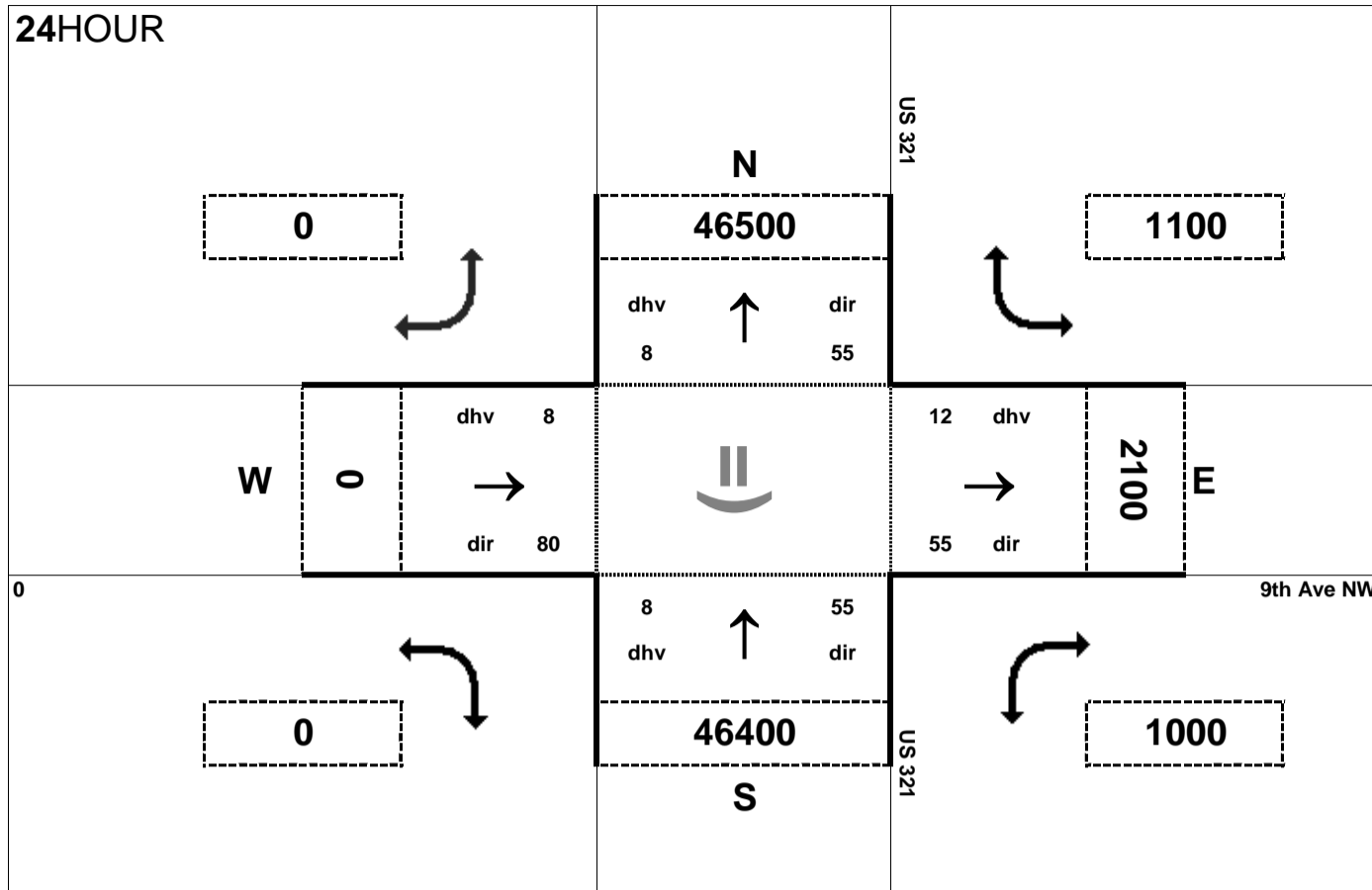
AMPEAK



PMPEAK



24HOUR



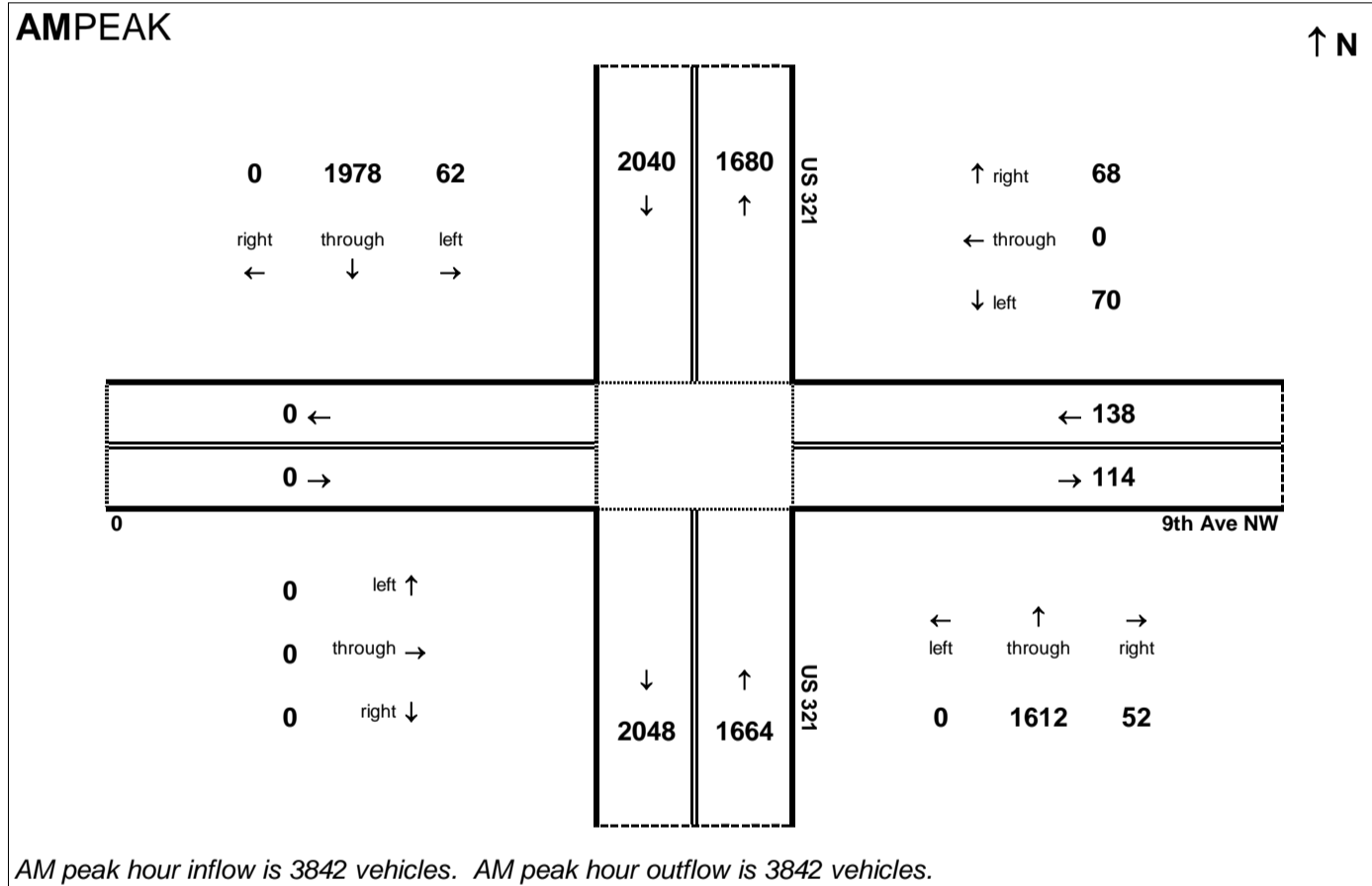
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 9th Ave NW

Traffic Forecast Release Date:
February-17

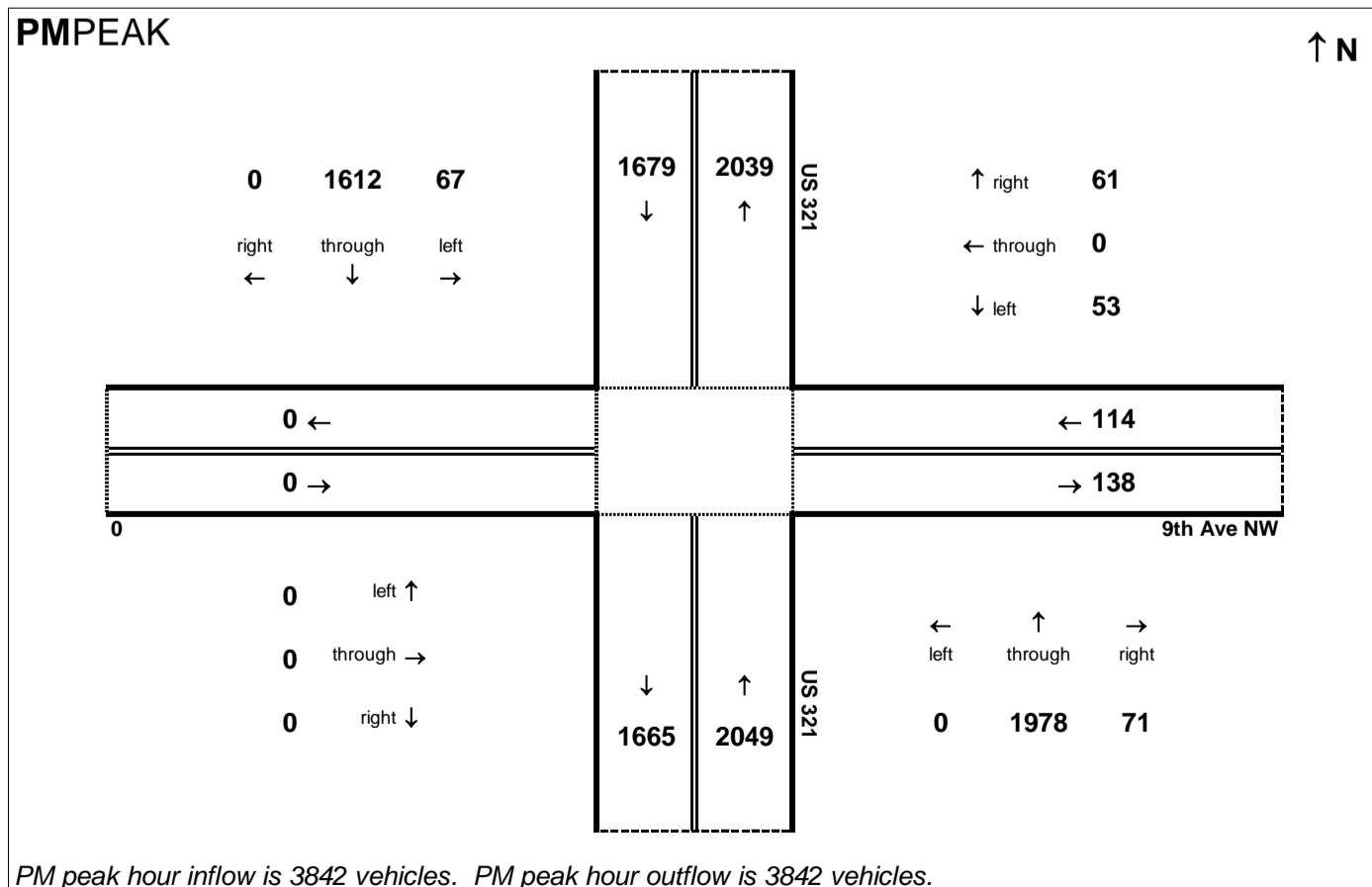
Traffic Data Year:
2040 Build 1

Project:
U-4700

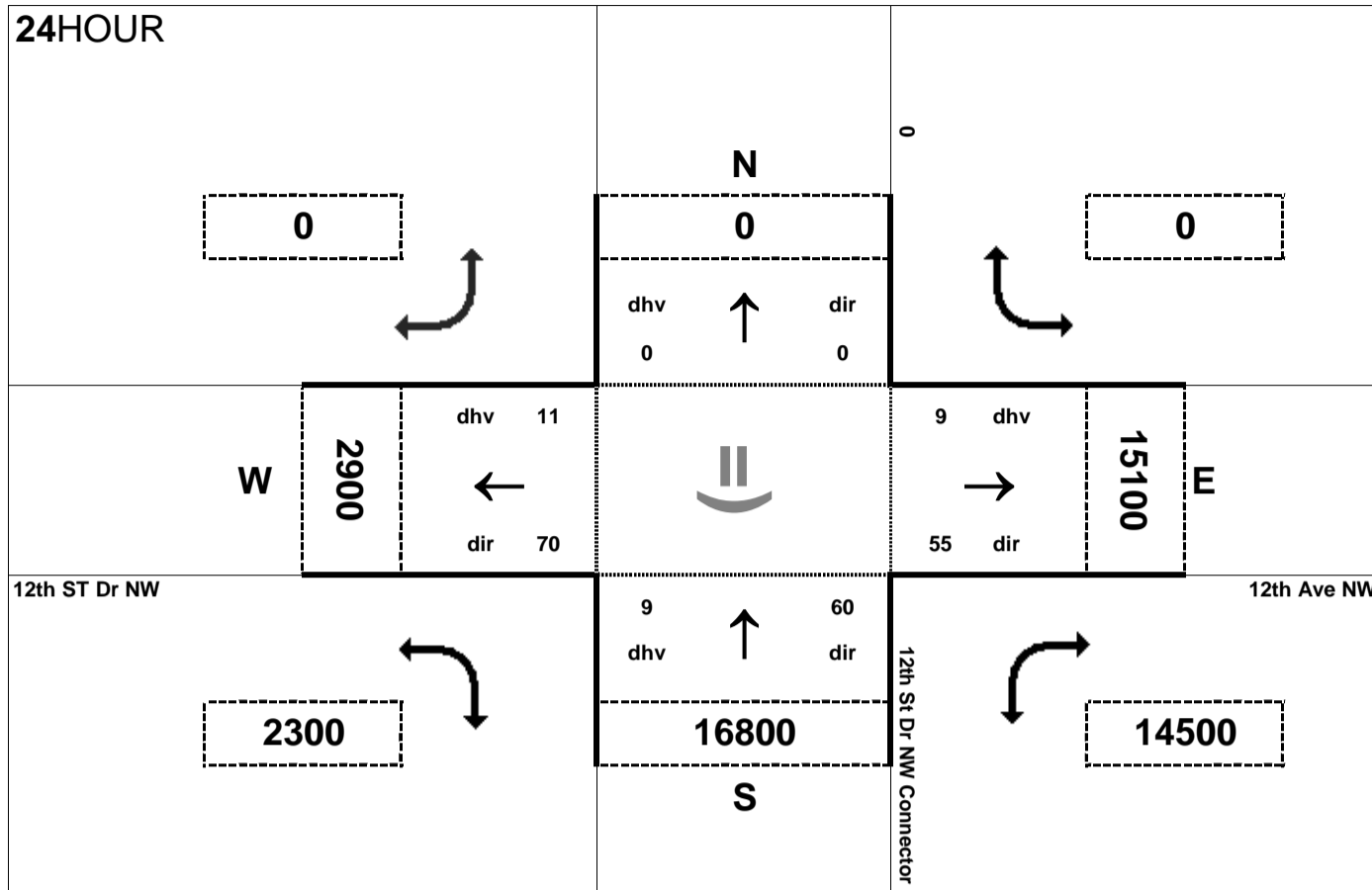
AMPEAK



PMPEAK



24HOUR



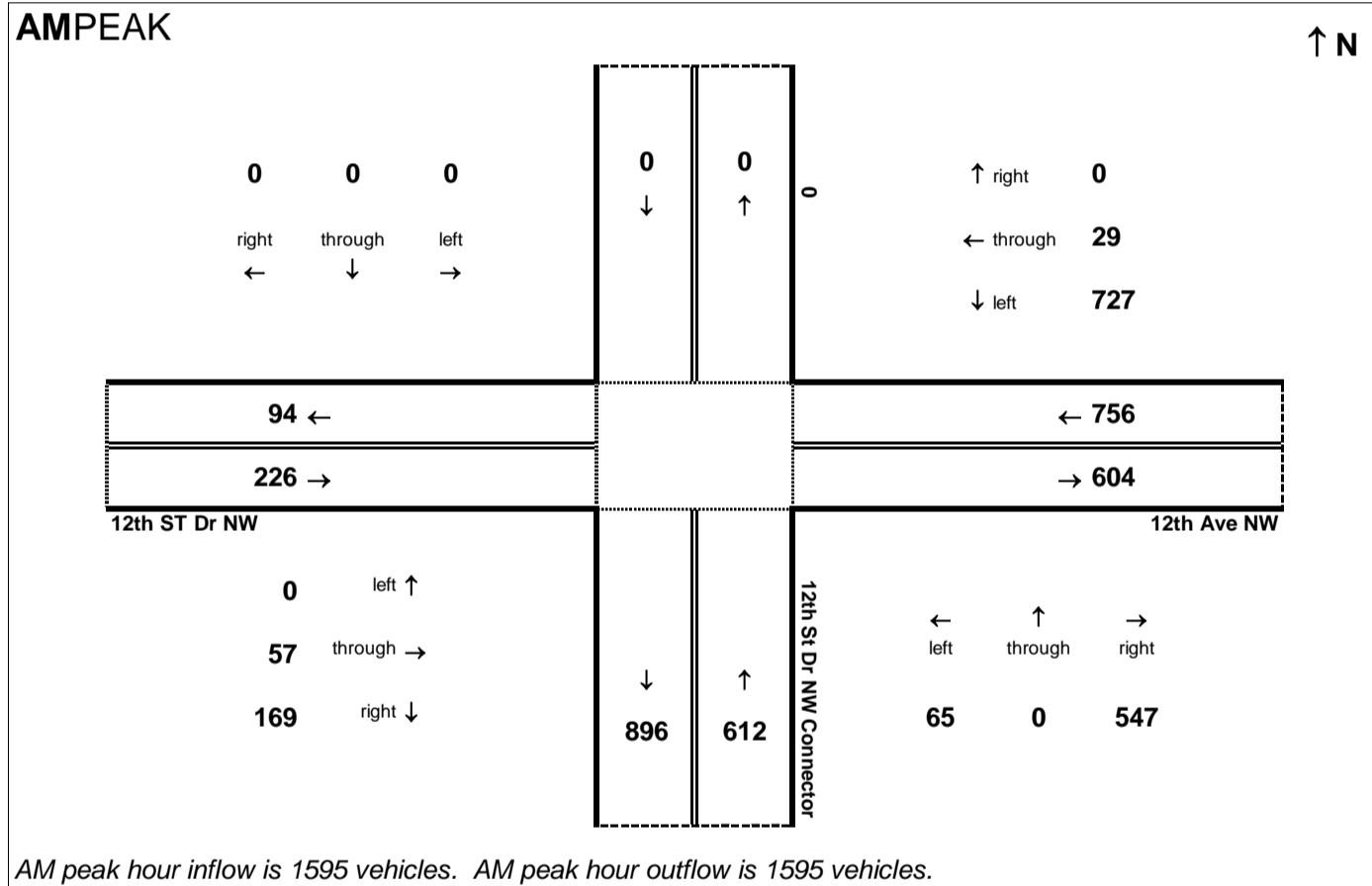
Peak Hour Volume Breakouts Report:
Intersection of 12th Ave NW at 12th St Dr NW

Traffic Forecast Release Date:
December-16

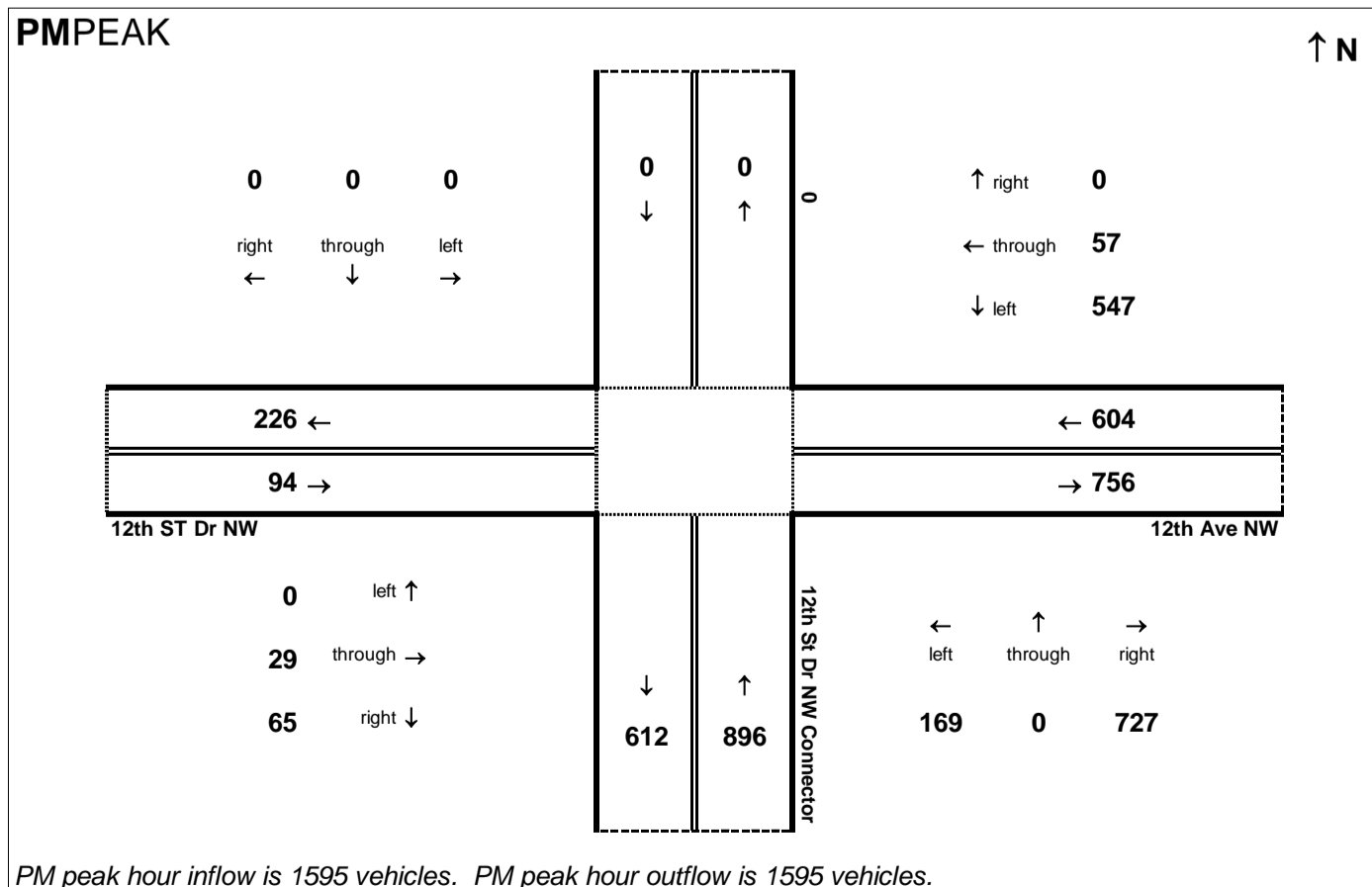
Traffic Data Year:
2040 Build 1

Project:
U-4700

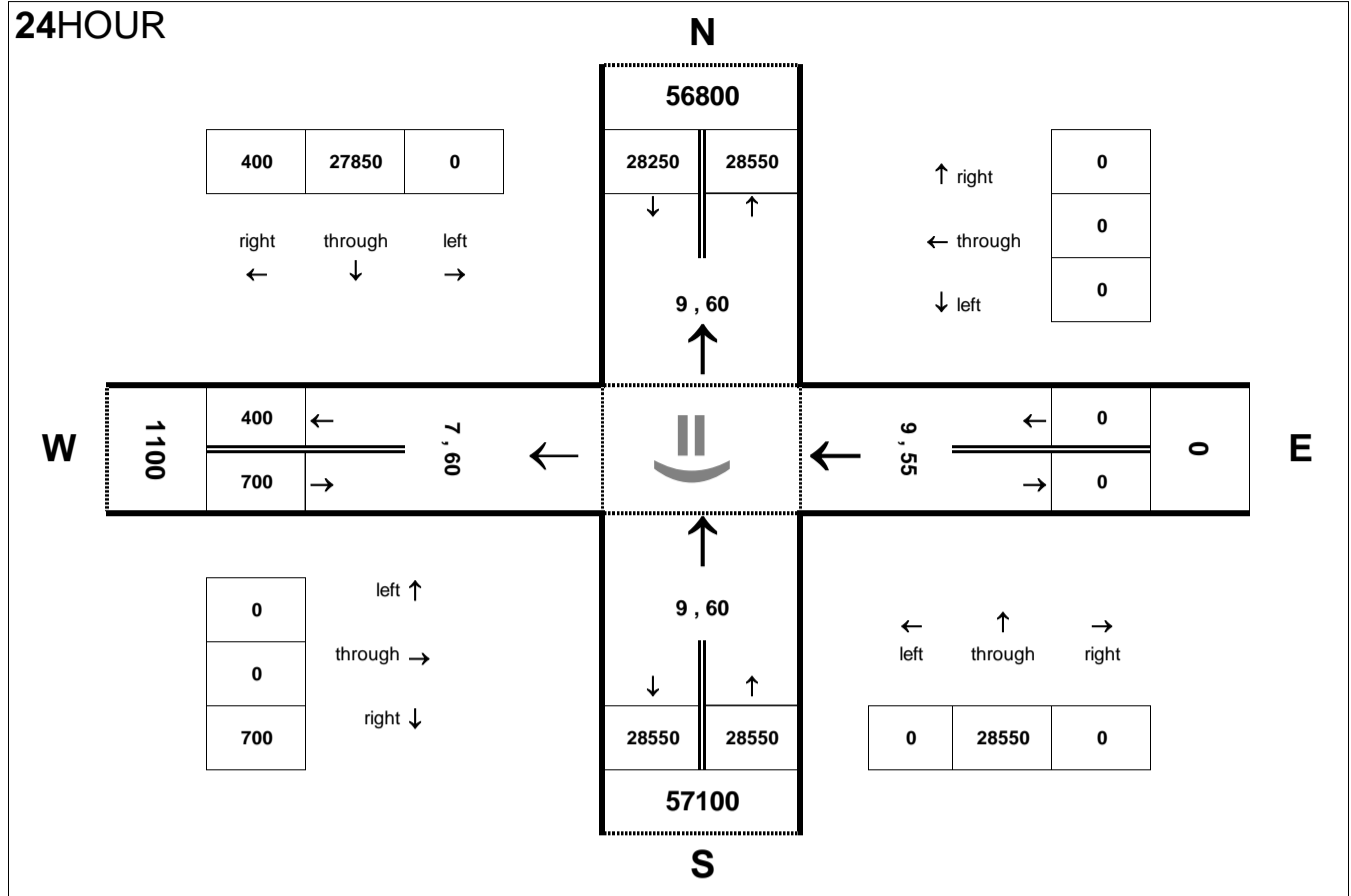
AMPEAK



PMPEAK



24HOUR



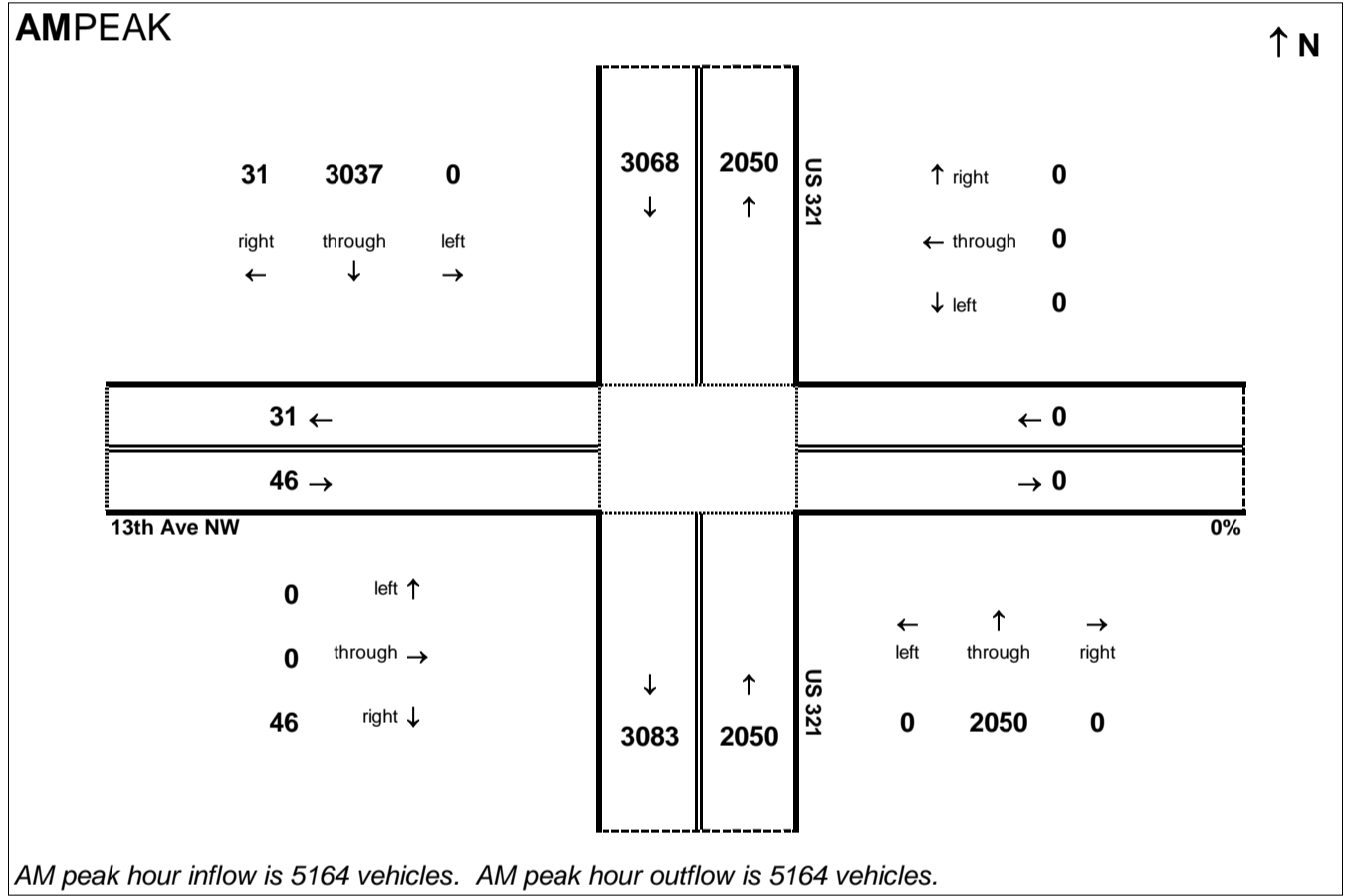
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 13th Ave NW

Traffic Forecast Release Date:
February-17

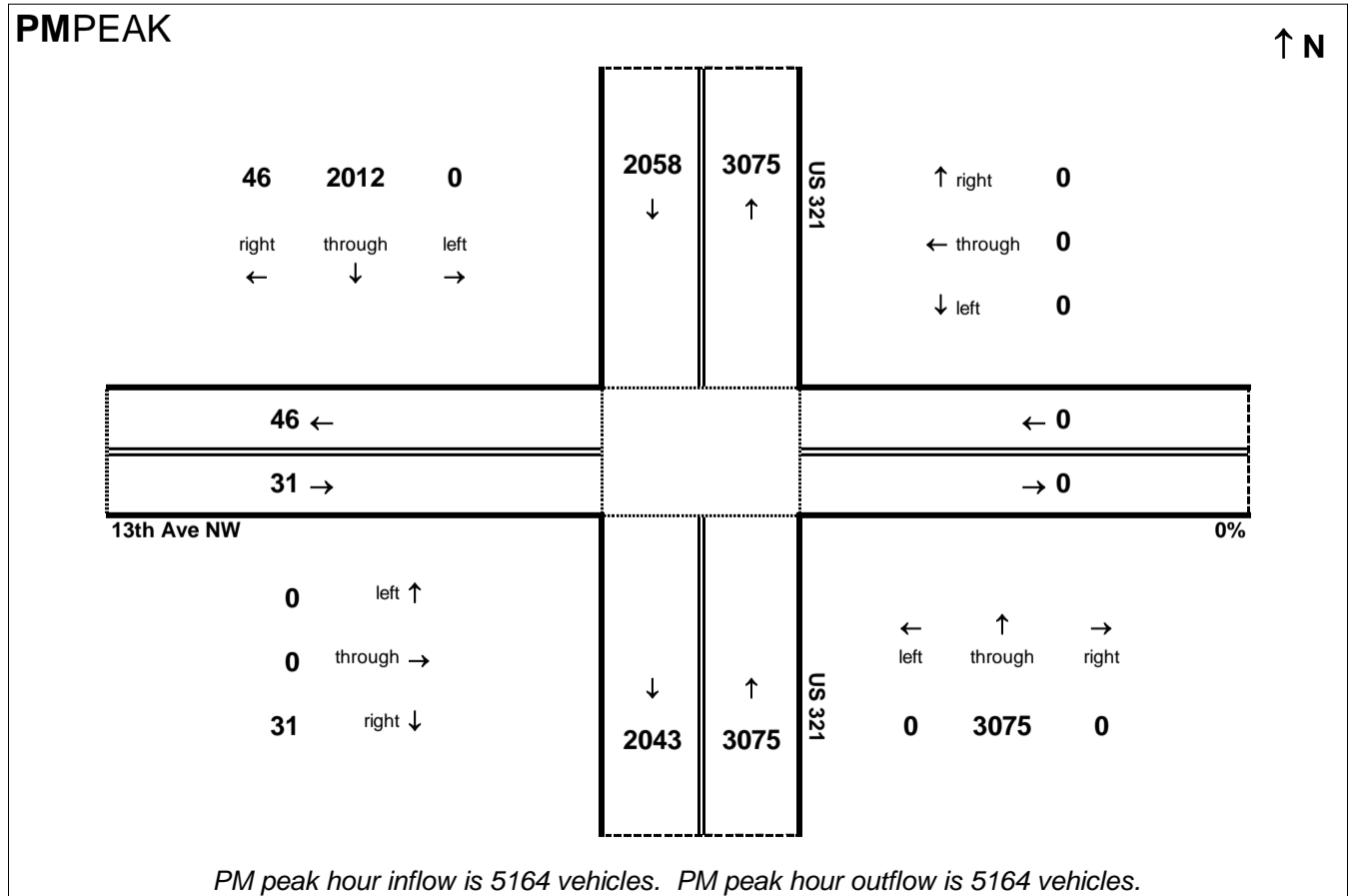
Traffic Data Year:
2040 Build 1

Project:
U-4700

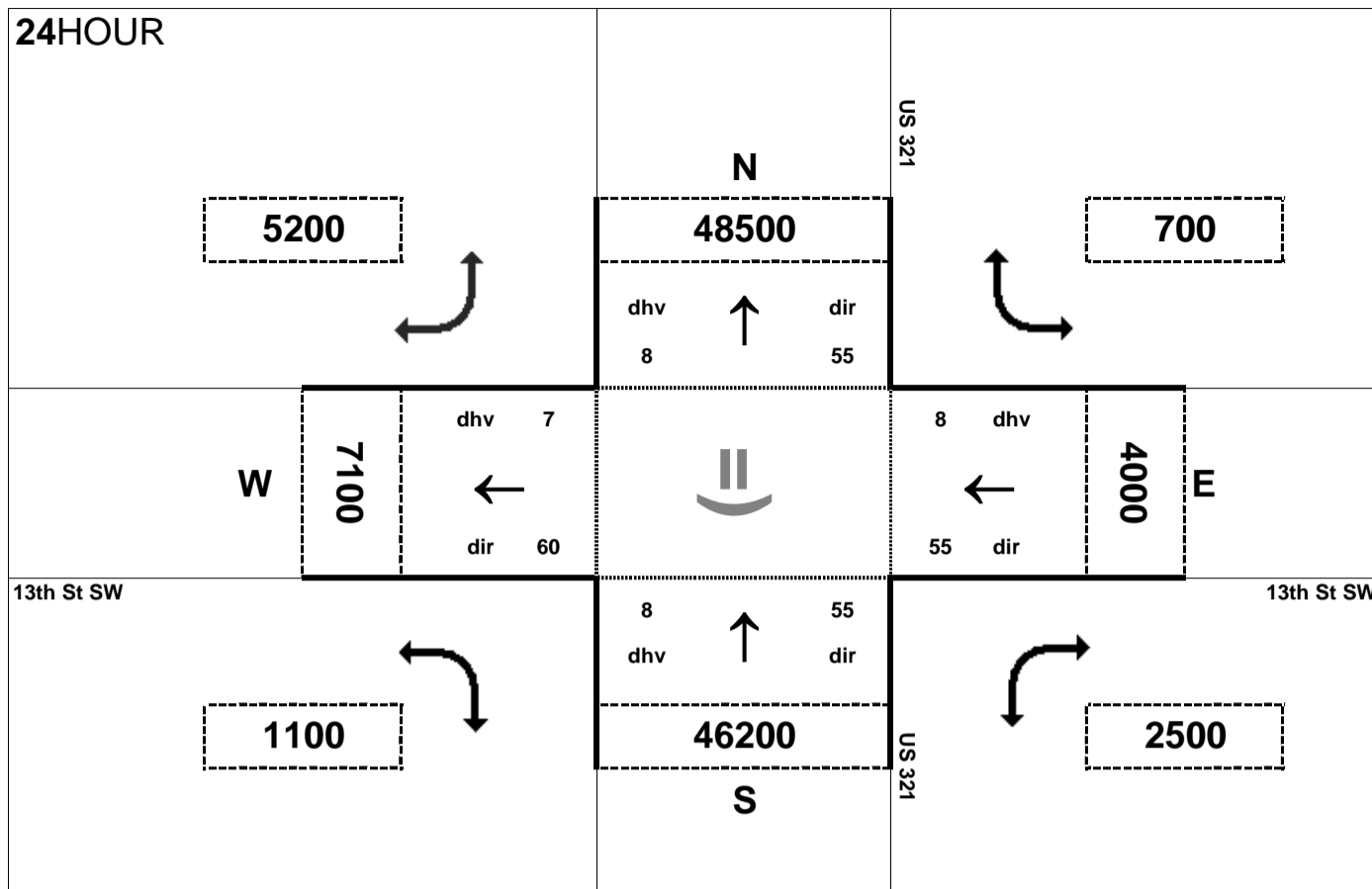
AMPEAK



PMPEAK



24HOUR



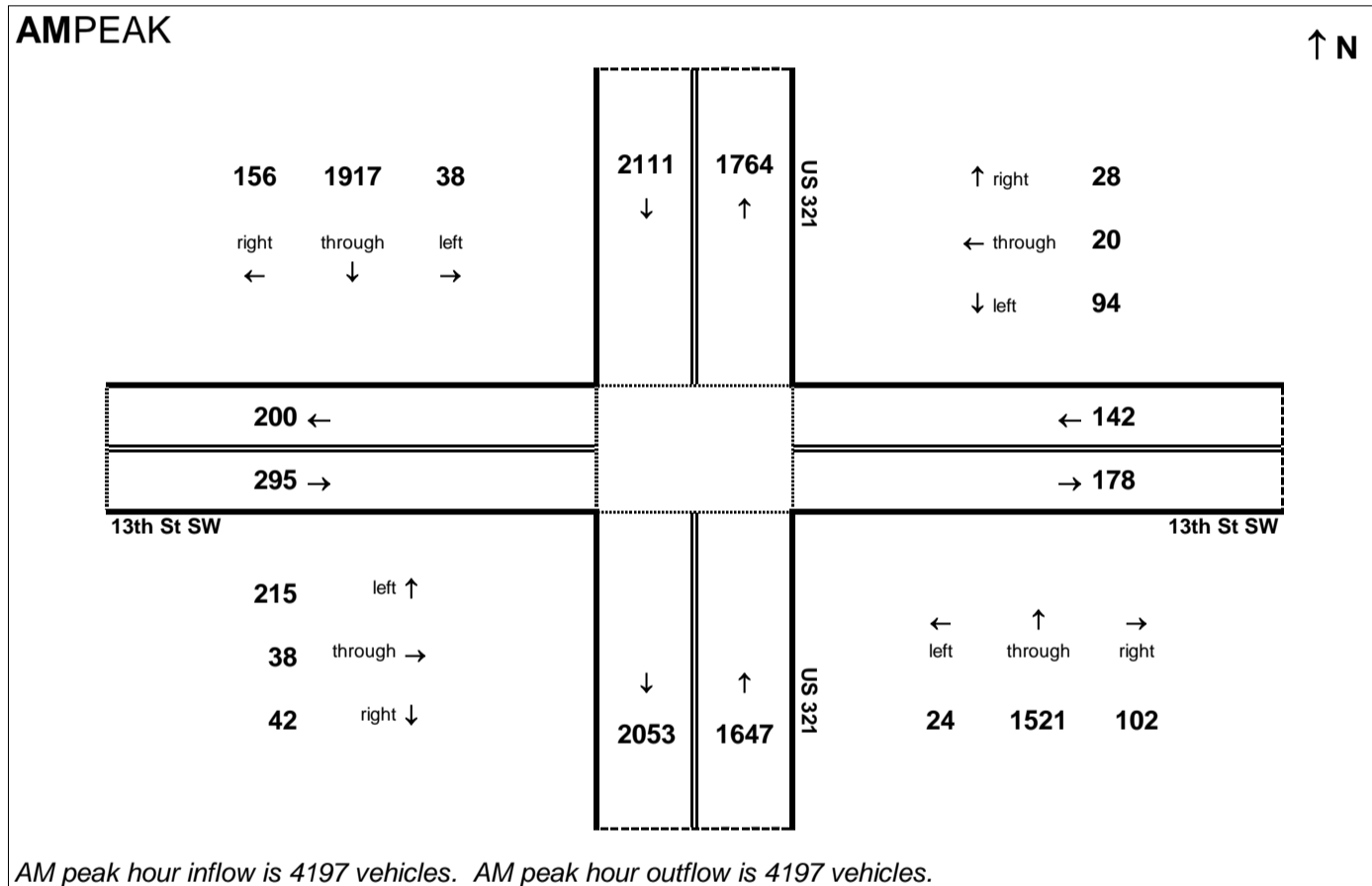
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 13th St SW

Traffic Forecast Release Date:
December-16

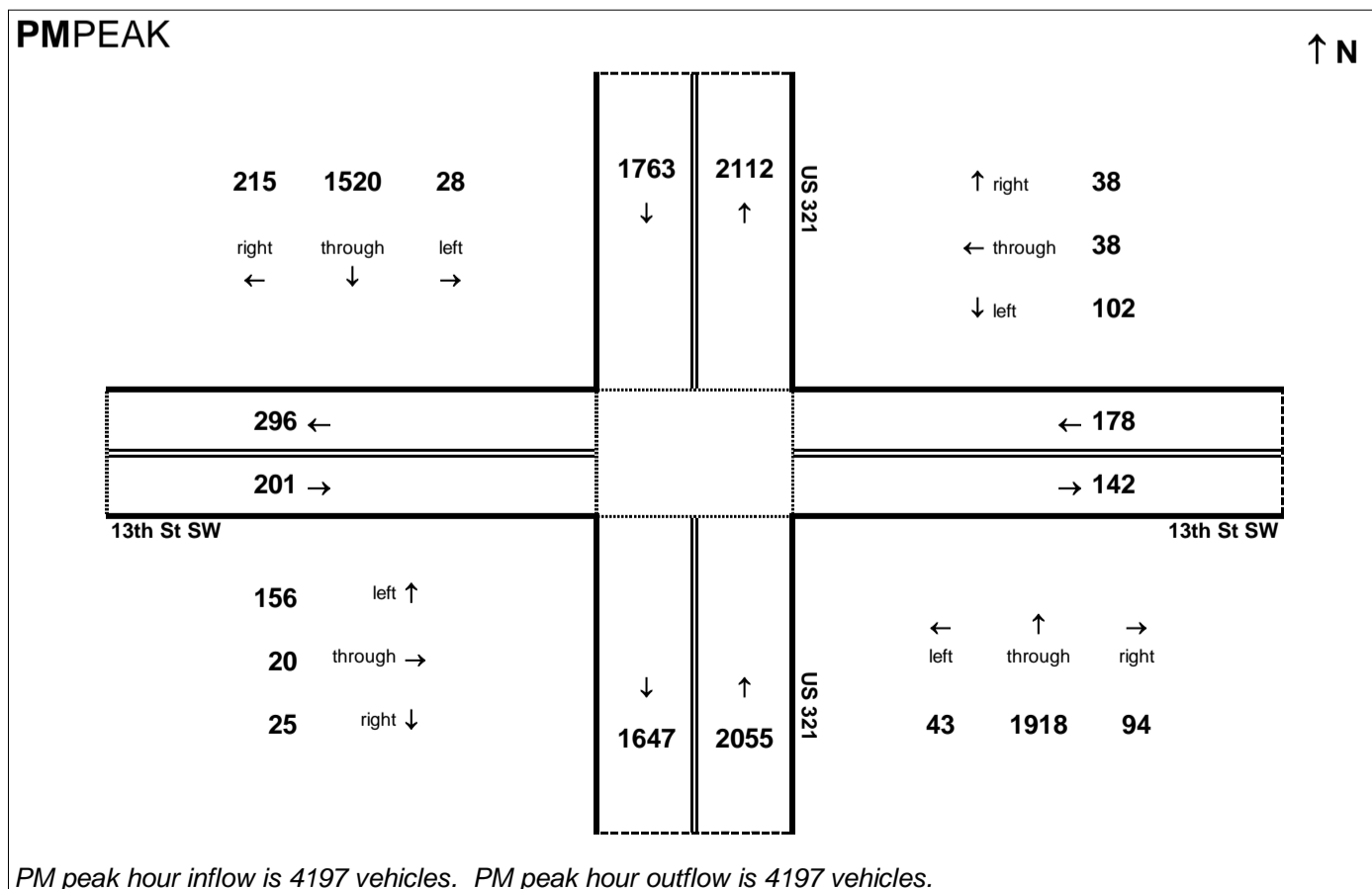
Traffic Data Year:
2040 Build 1

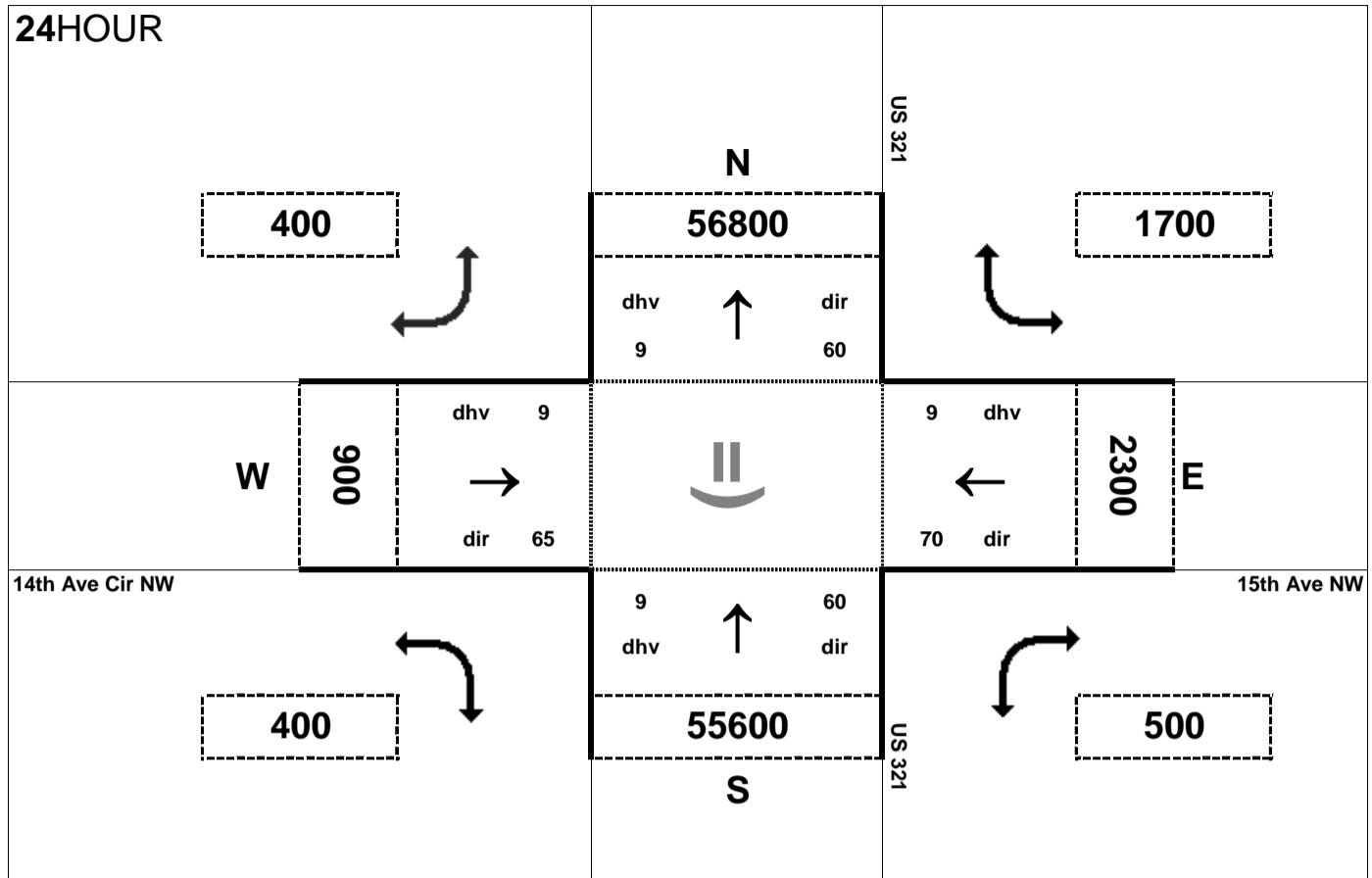
Project:
U-4700

AMPEAK



PMPEAK



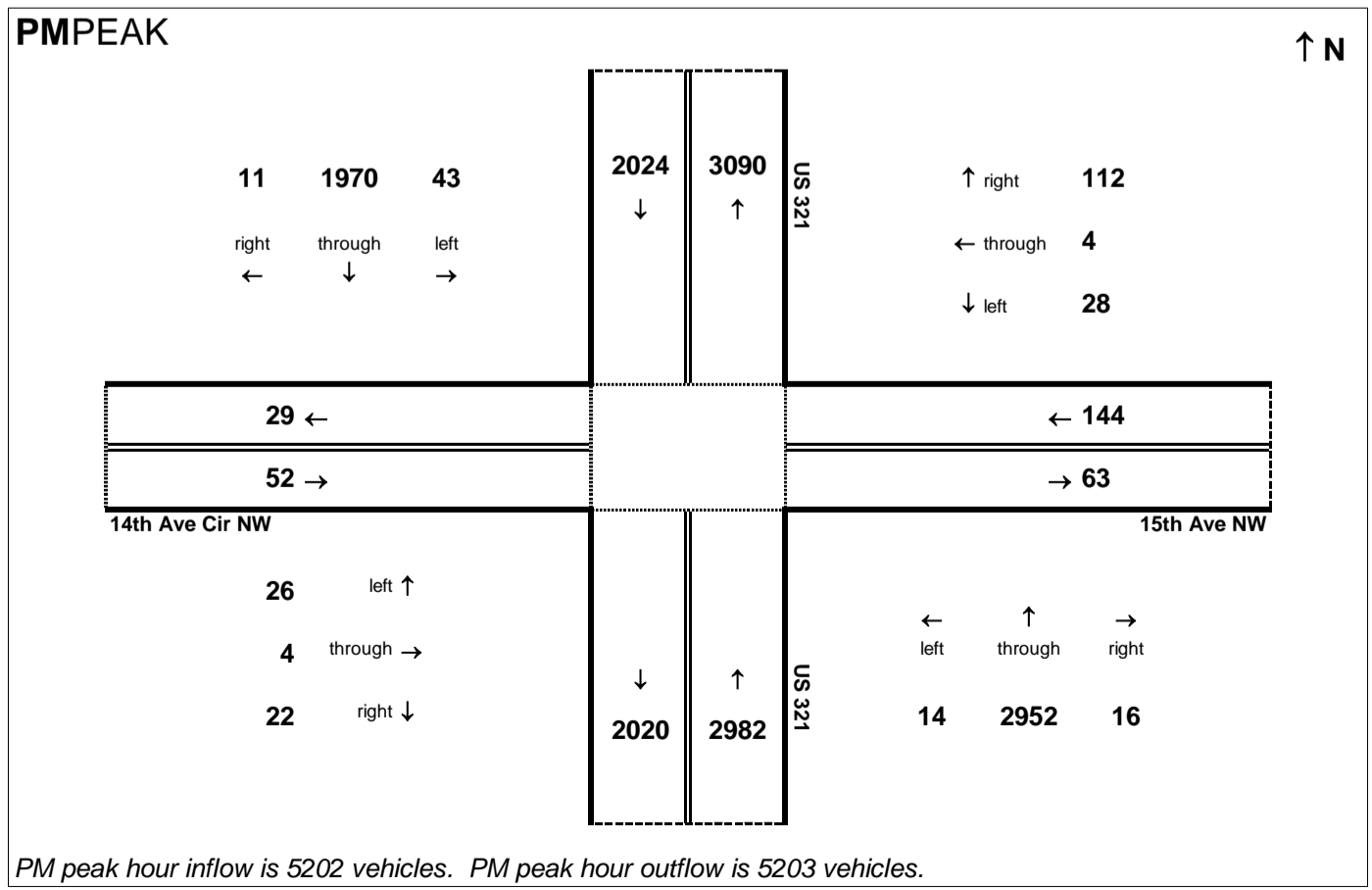
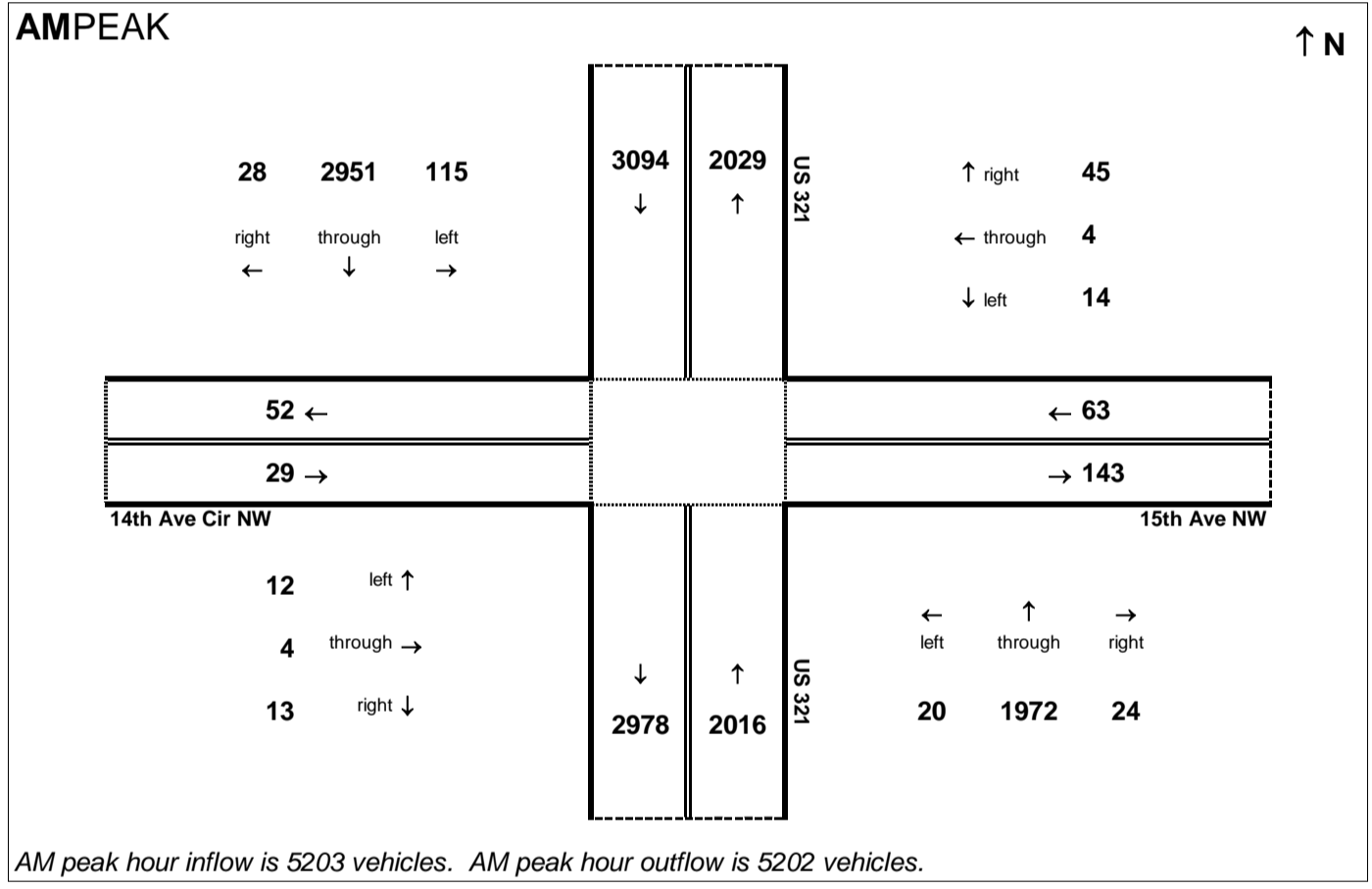


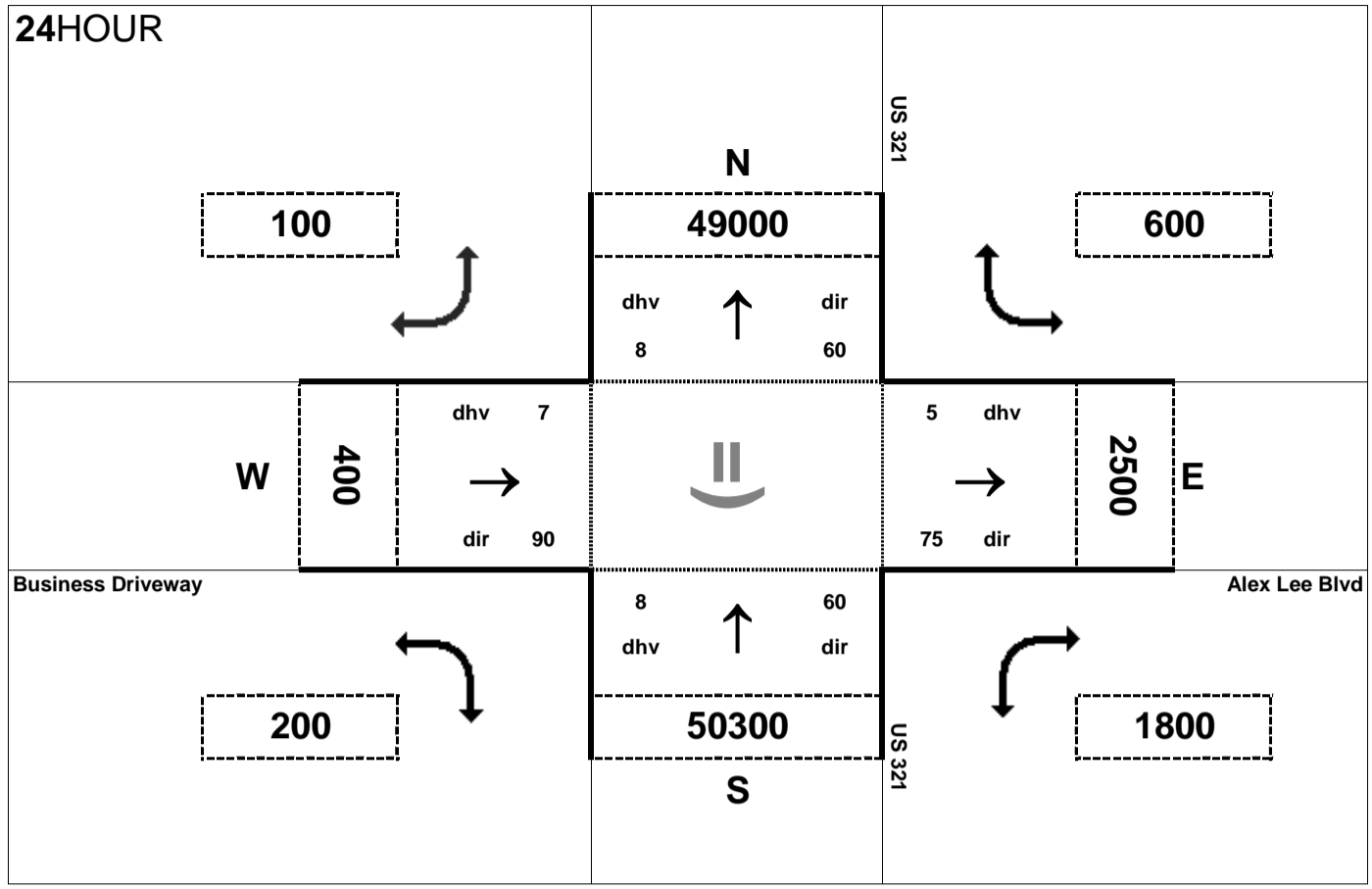
Peak Hour Volume Breakouts Report:
 Intersection of US 321 and 15th Ave NW / 14th Ave Cir NW

Traffic Forecast Release Date:
 February-17

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



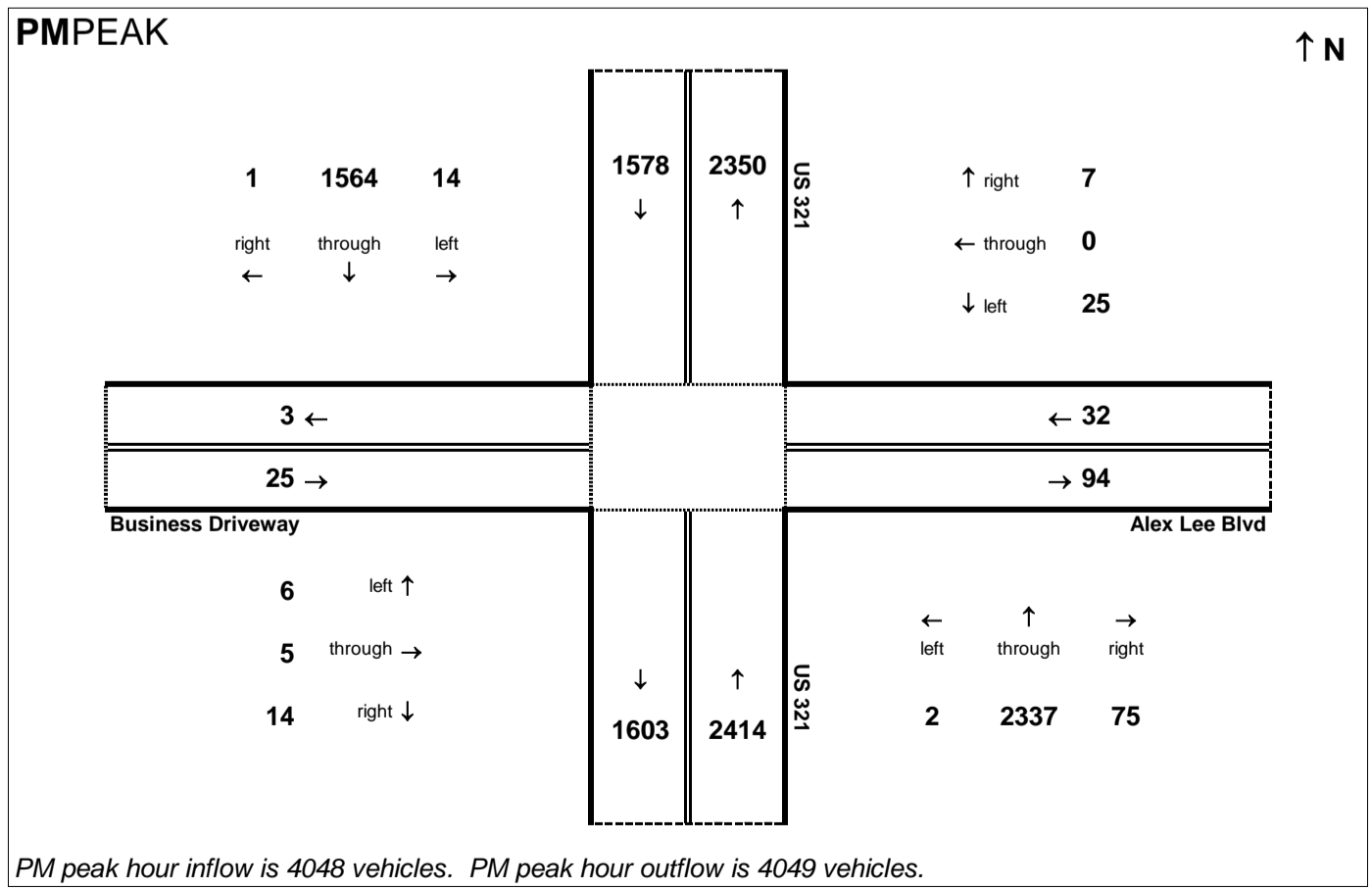
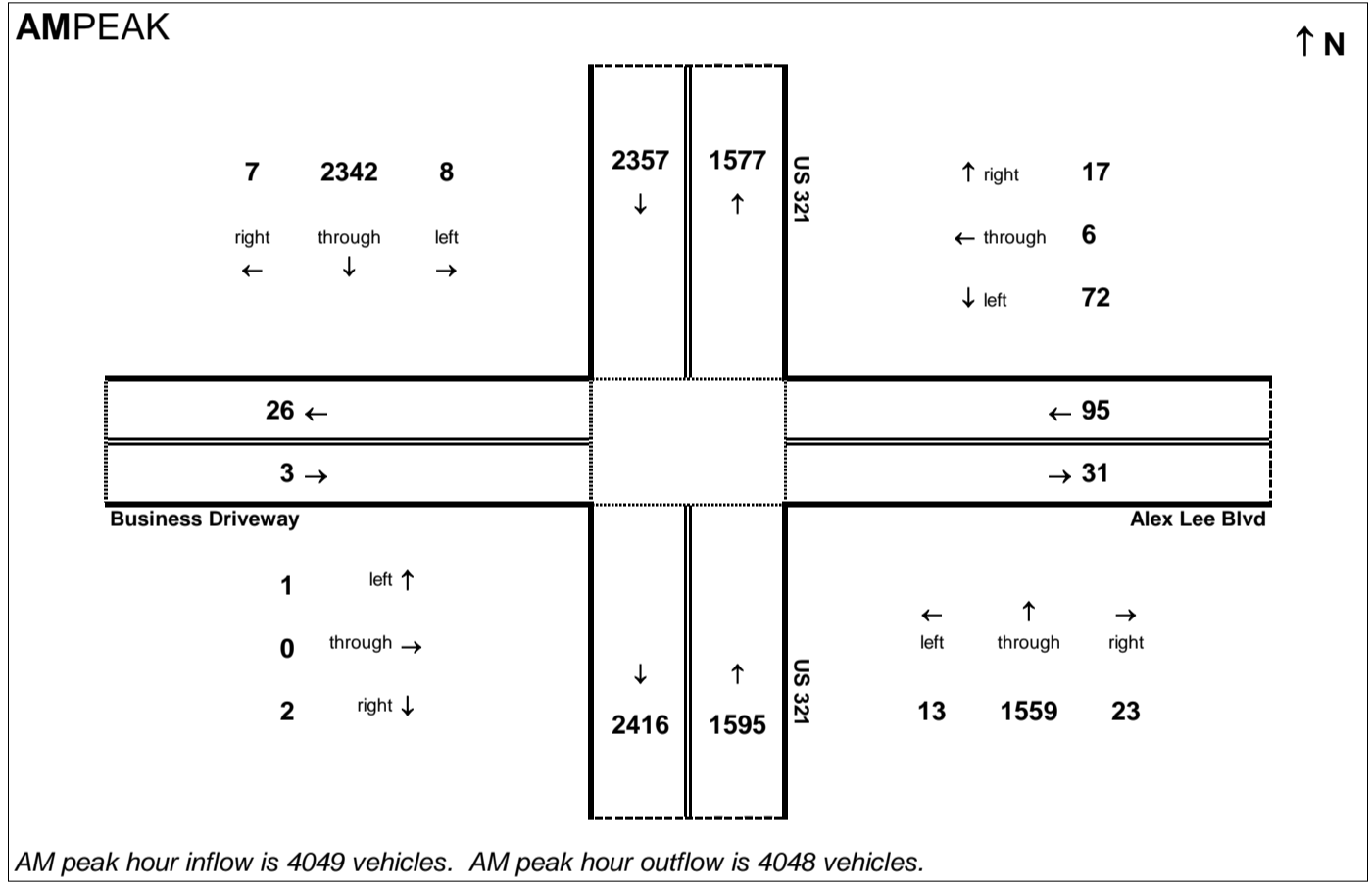


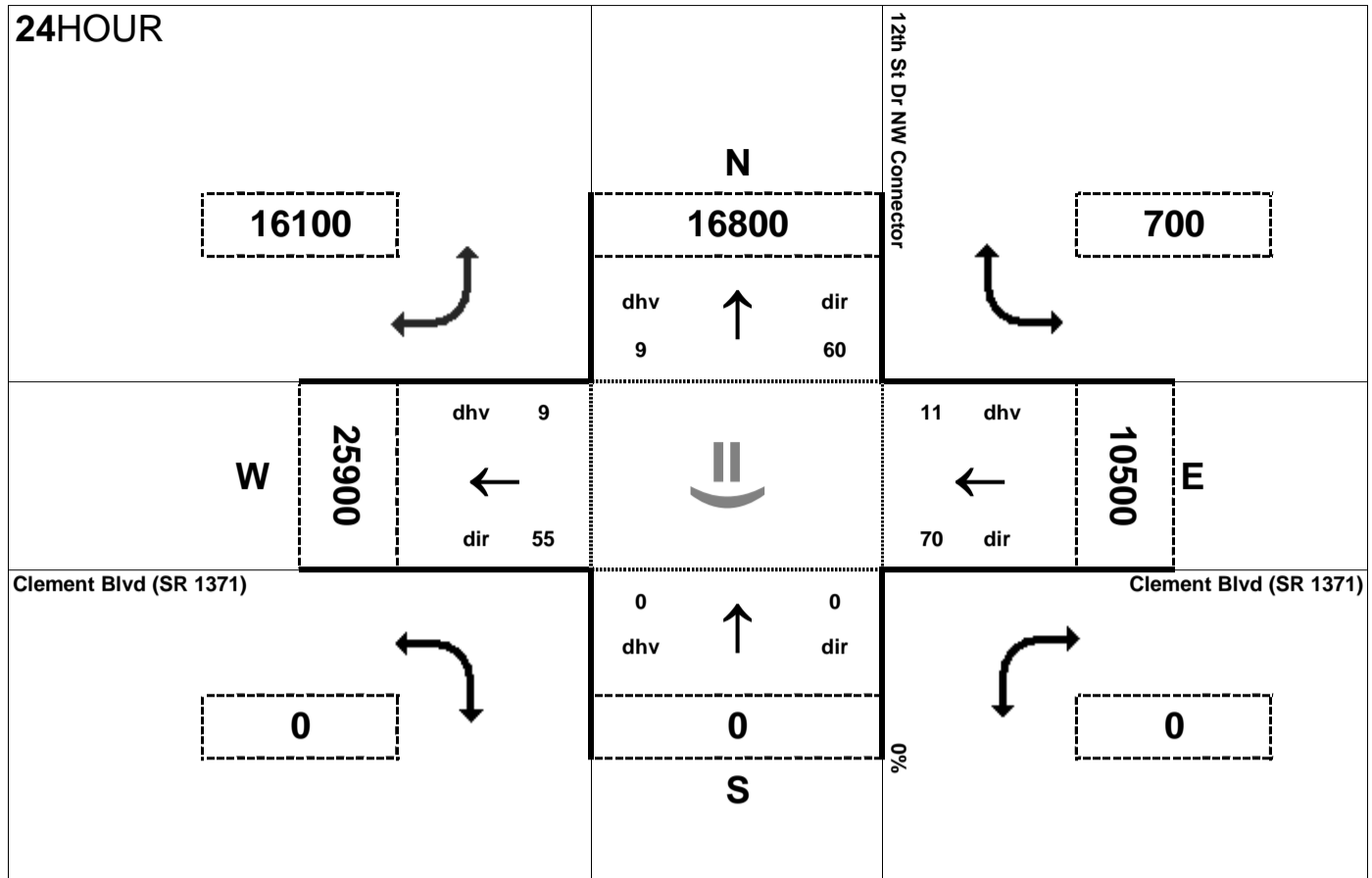
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Alex Lee Blvd

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

Project:
U-4700



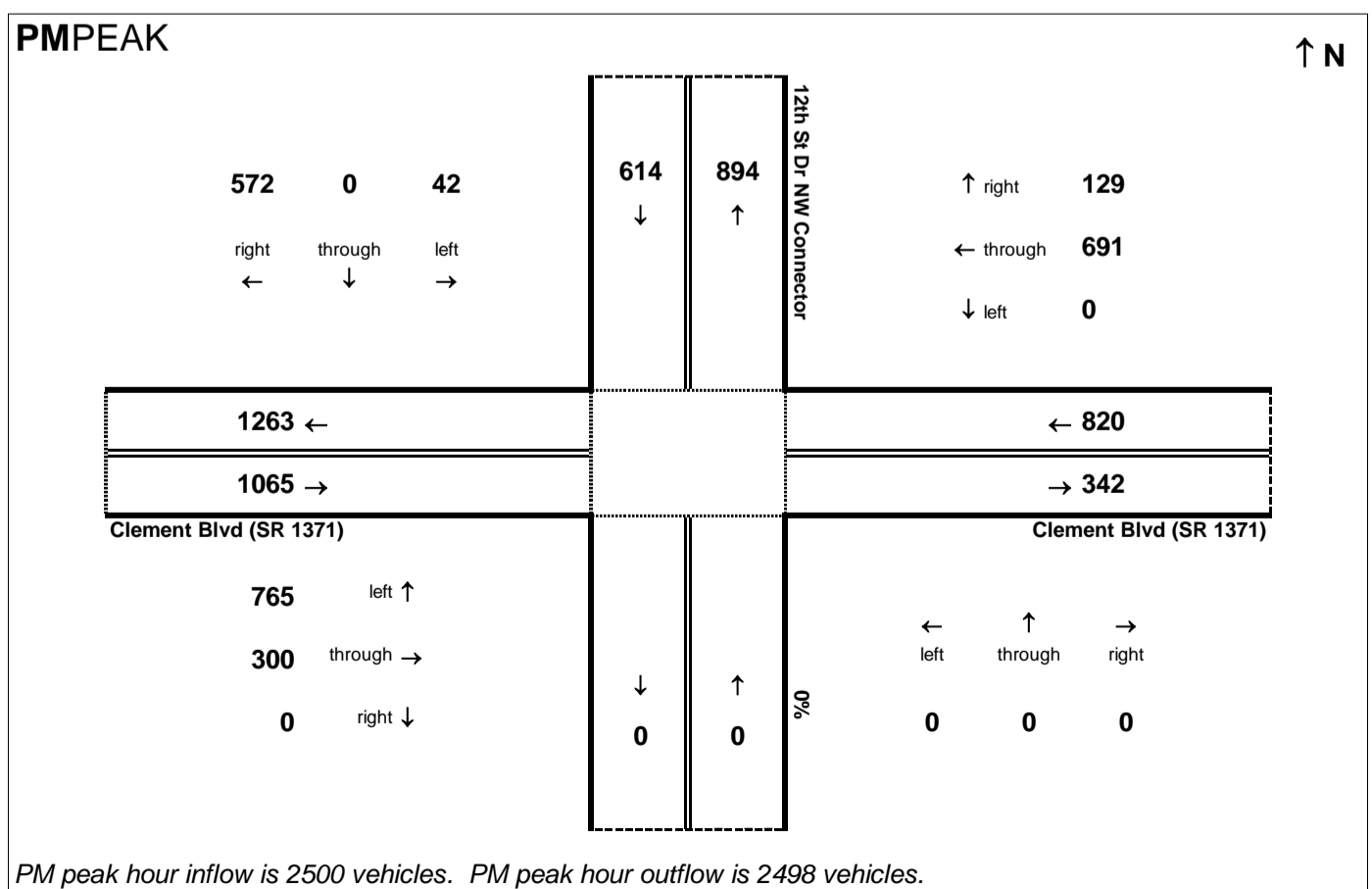
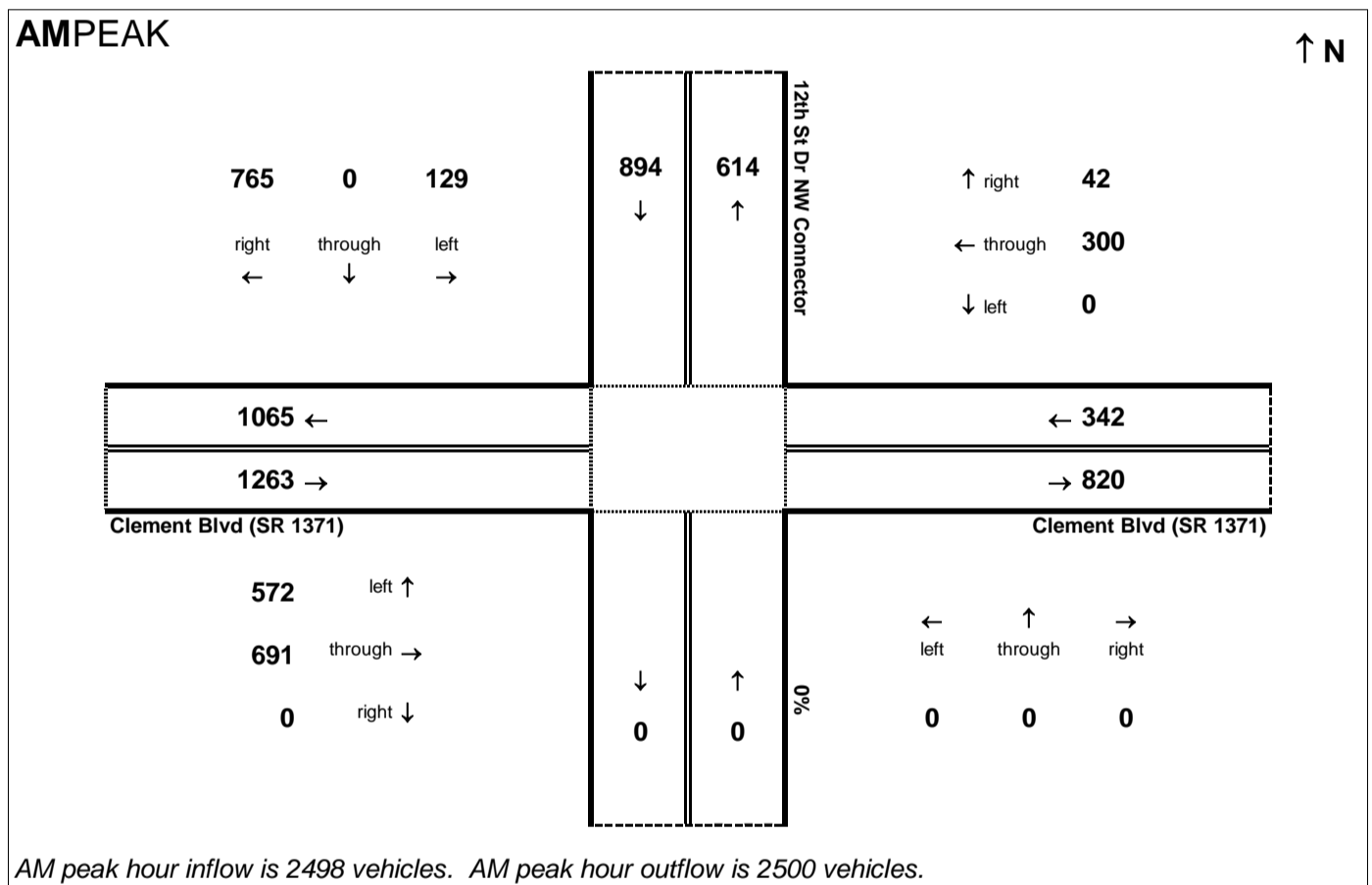


Peak Hour Volume Breakouts Report:
 Intersection of Clement Blvd at 12th St Dr NW
 Connector

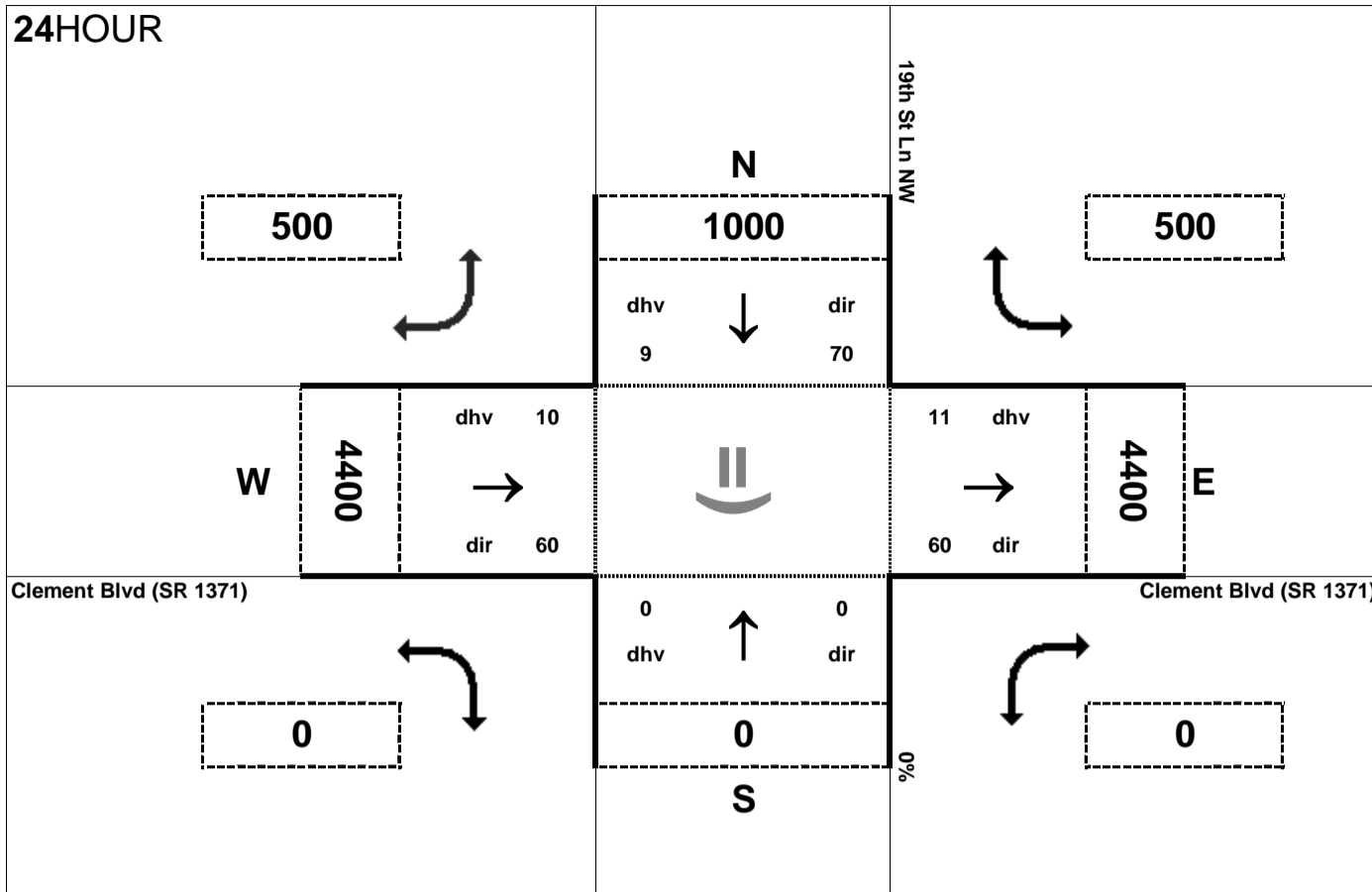
Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



24HOUR



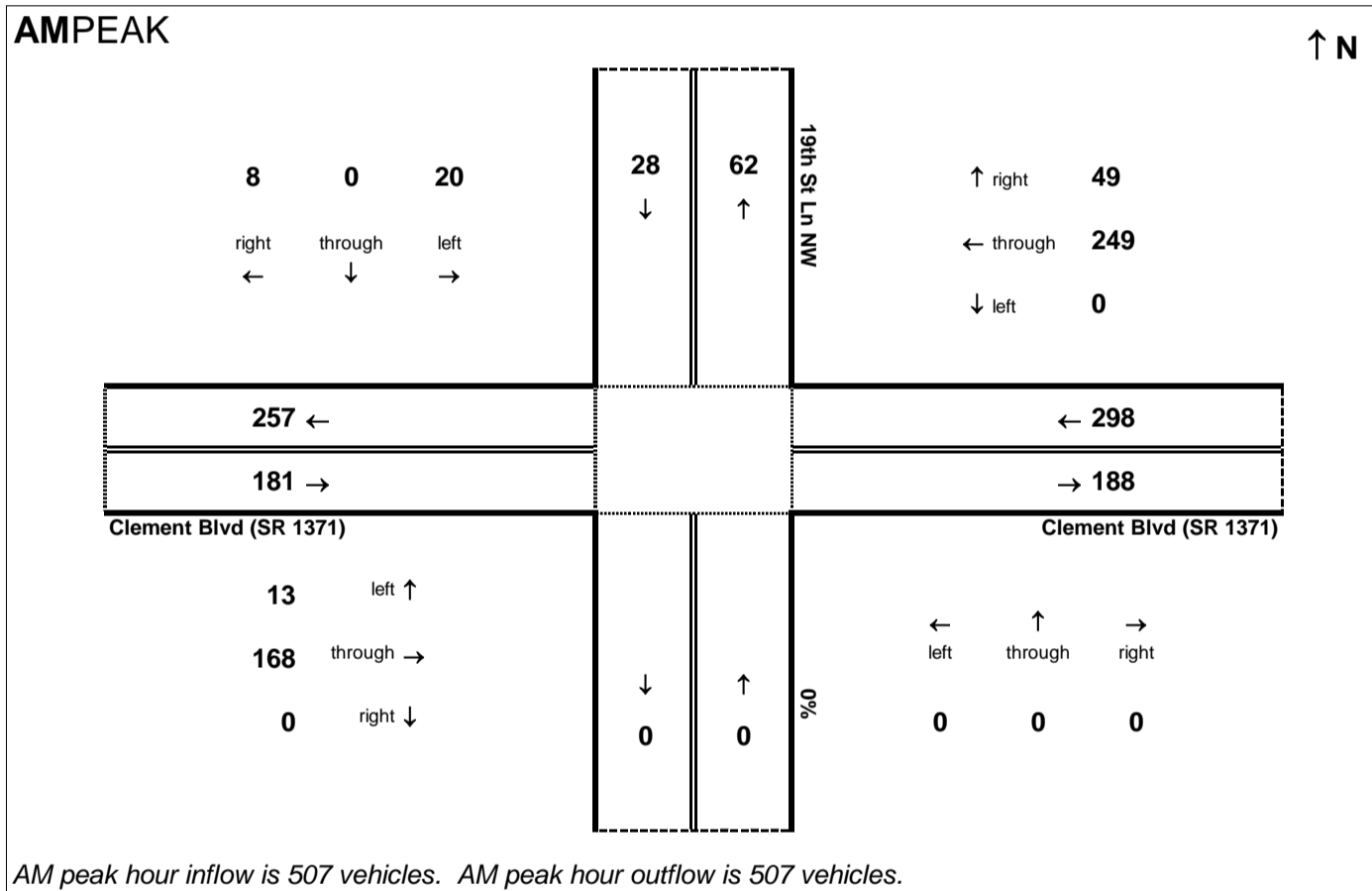
Peak Hour Volume Breakouts Report:
Intersection of Clement Blvd and 19th St Ln NW

Traffic Forecast Release Date:
December-16

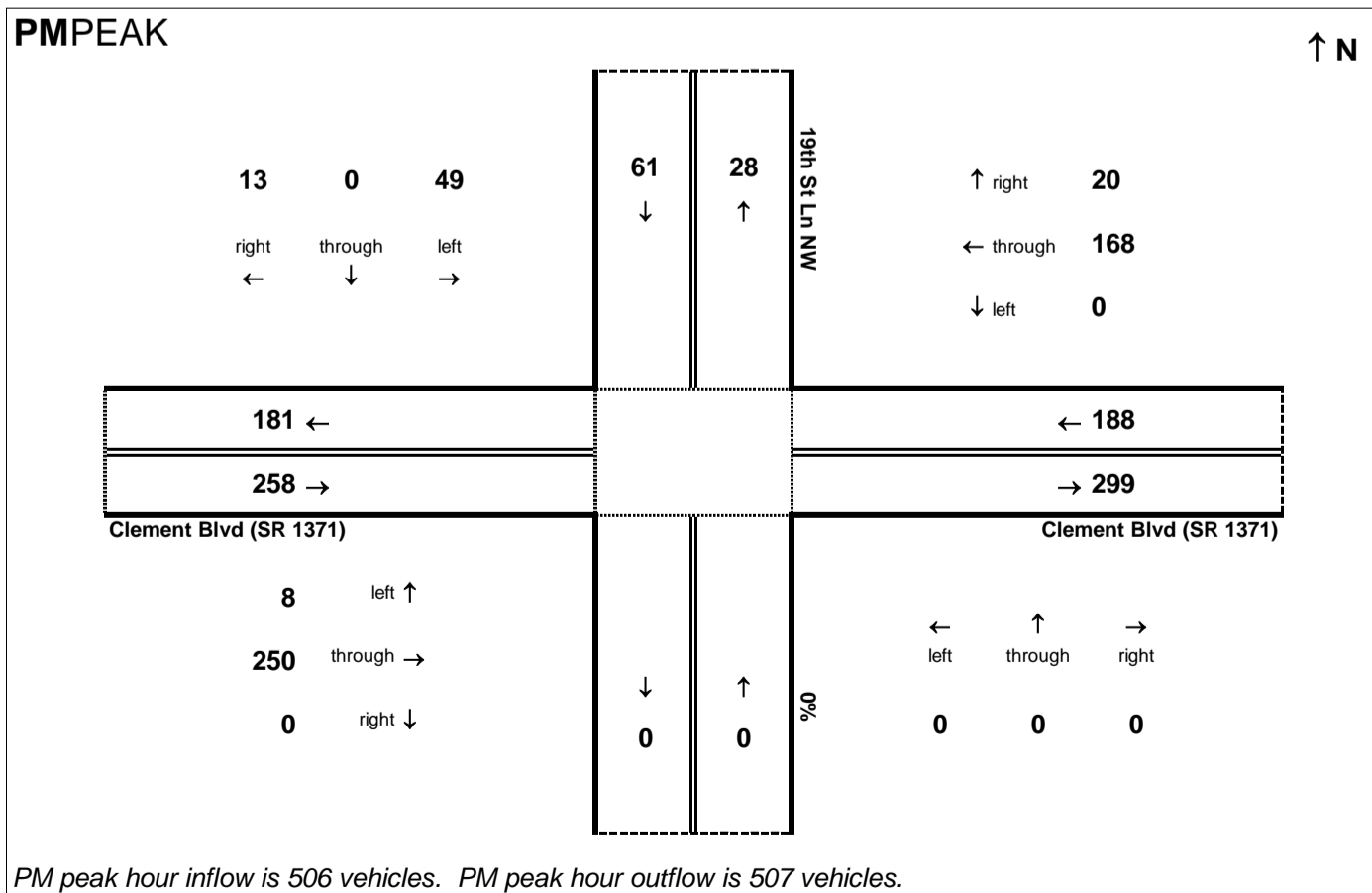
Traffic Data Year:
2040 Build 1

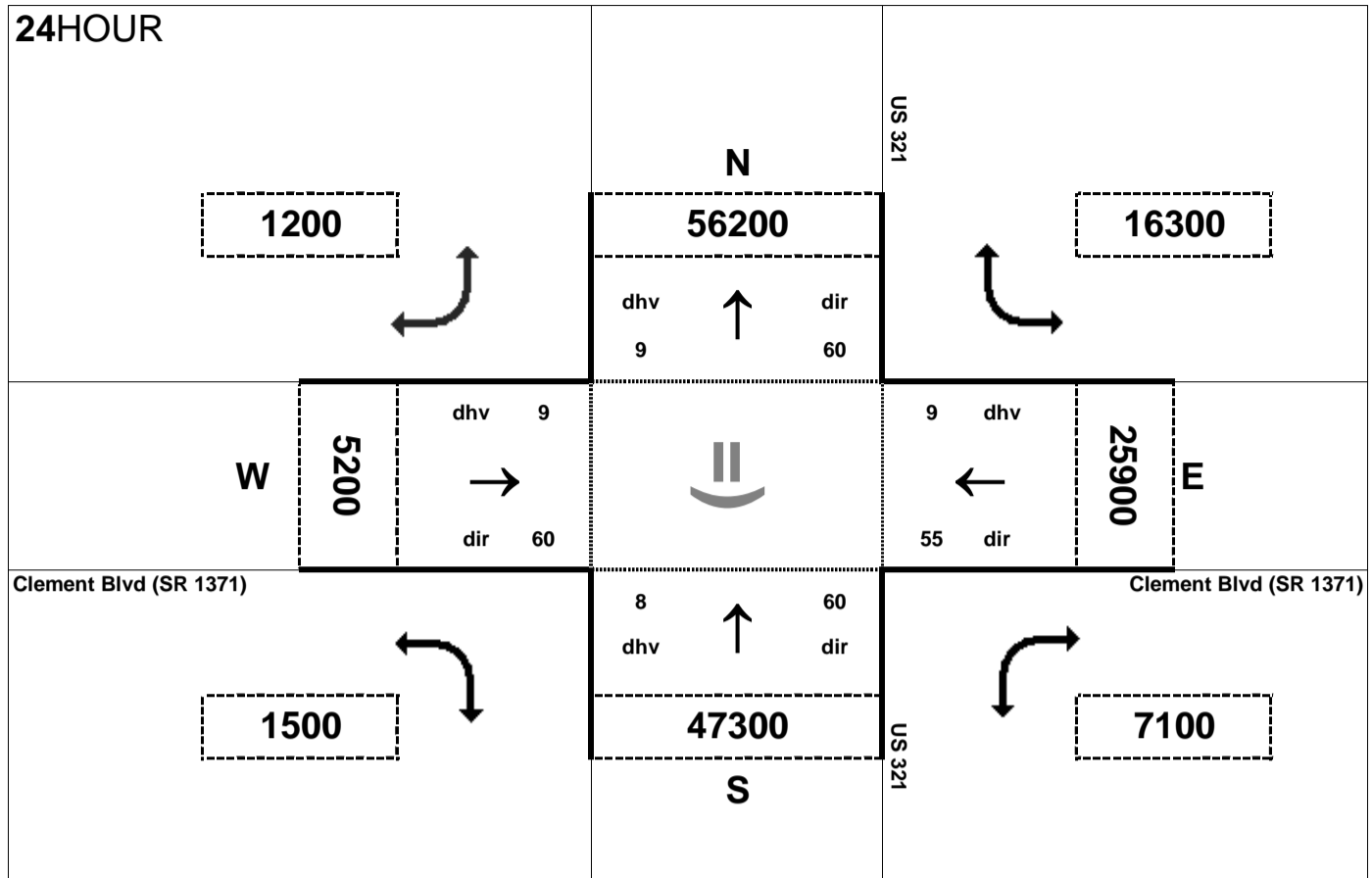
Project:
U-4700

AMPEAK



PMPEAK



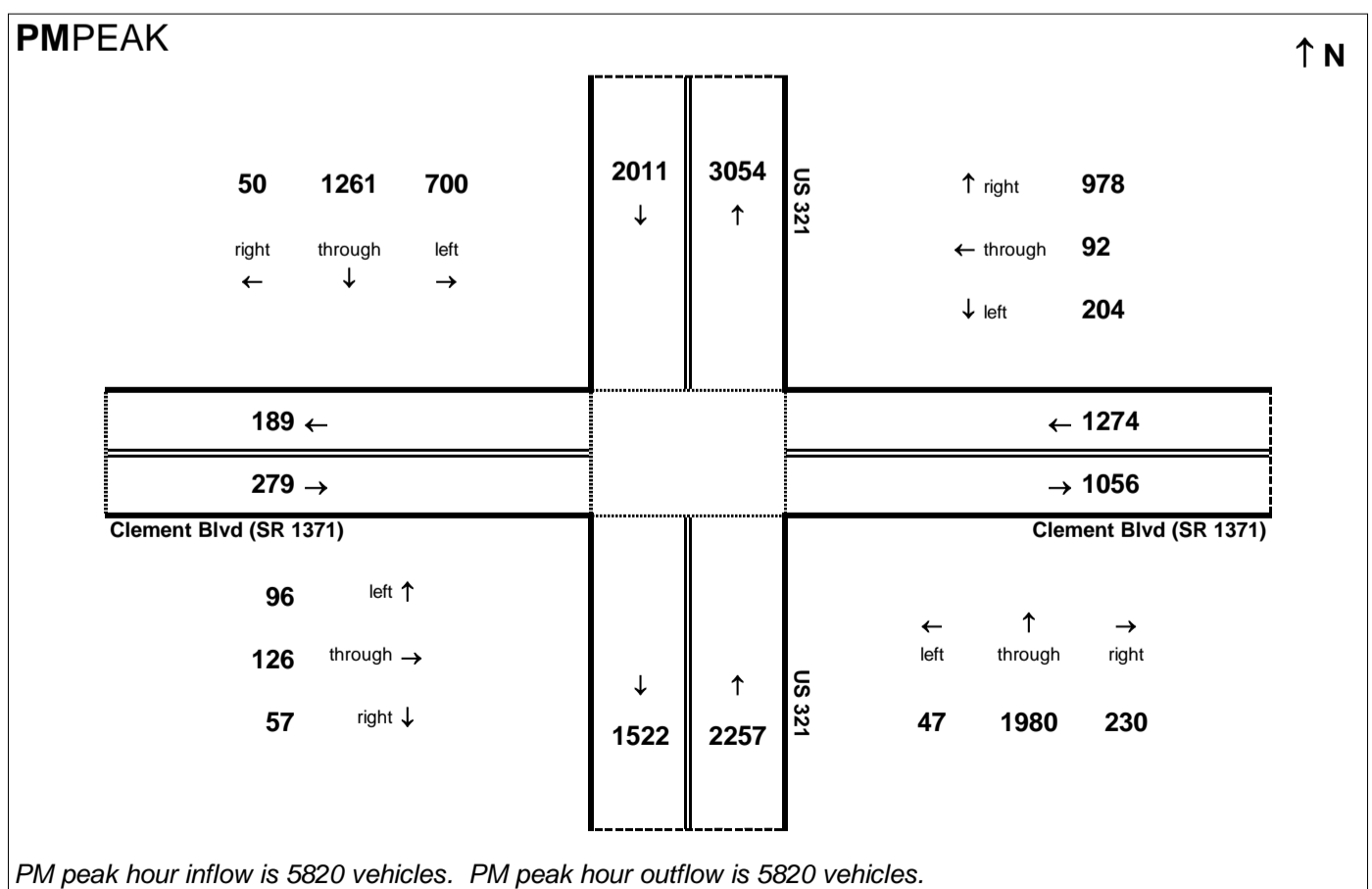
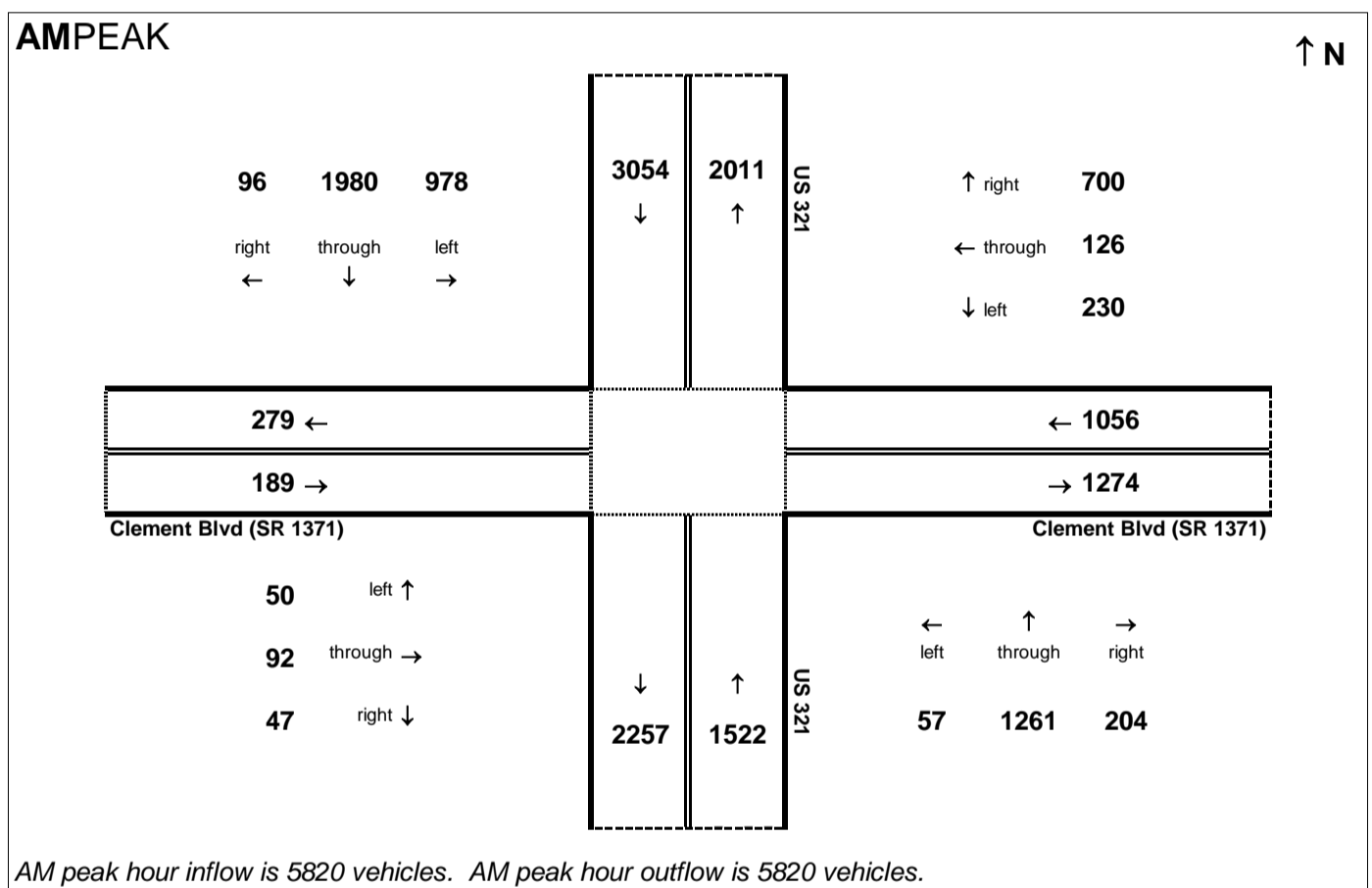


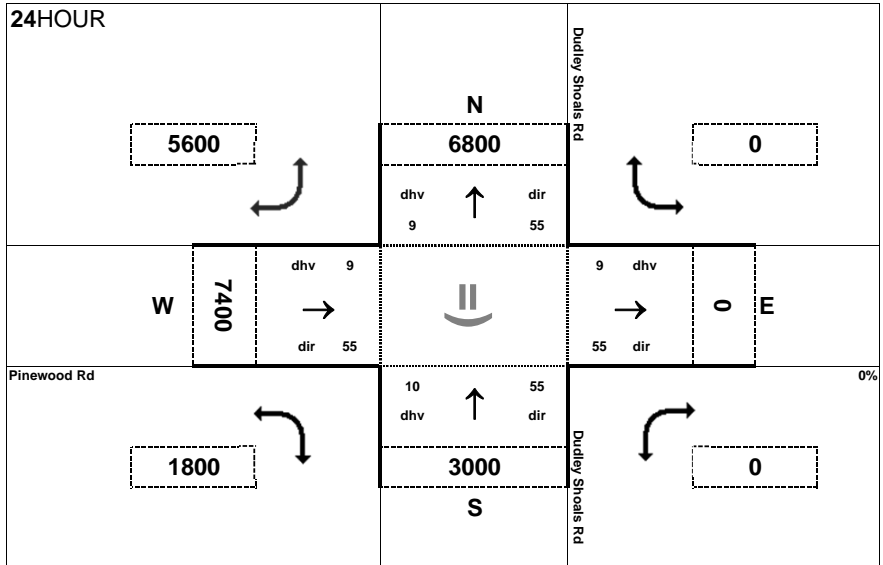
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Clement Blvd

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

Project:
U-4700



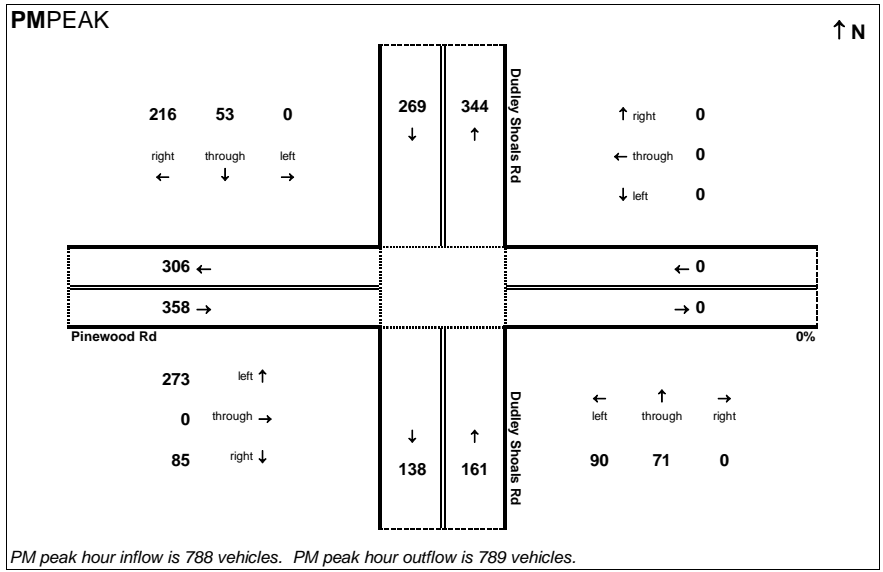
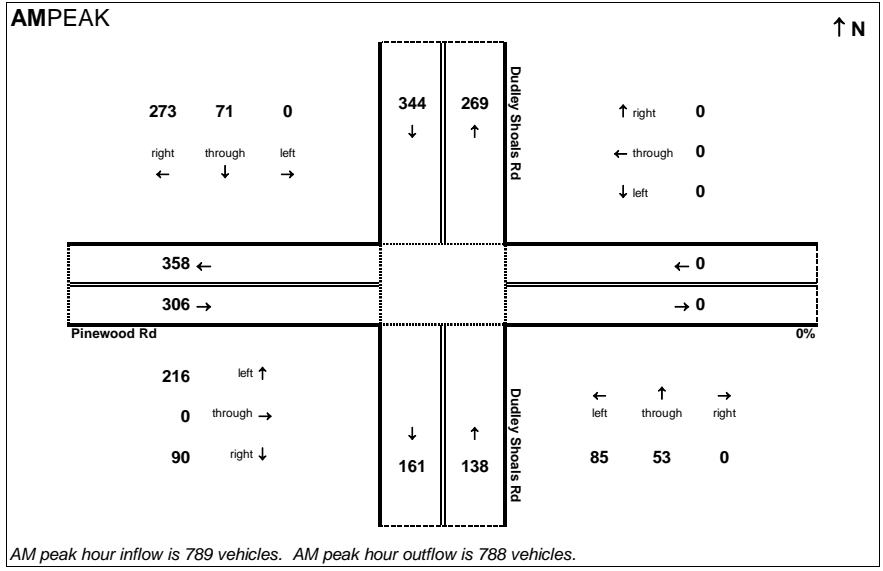


Peak Hour Volume Breakouts Report:
Intersection of Dudley Shoals Rd and Pinewood Rd

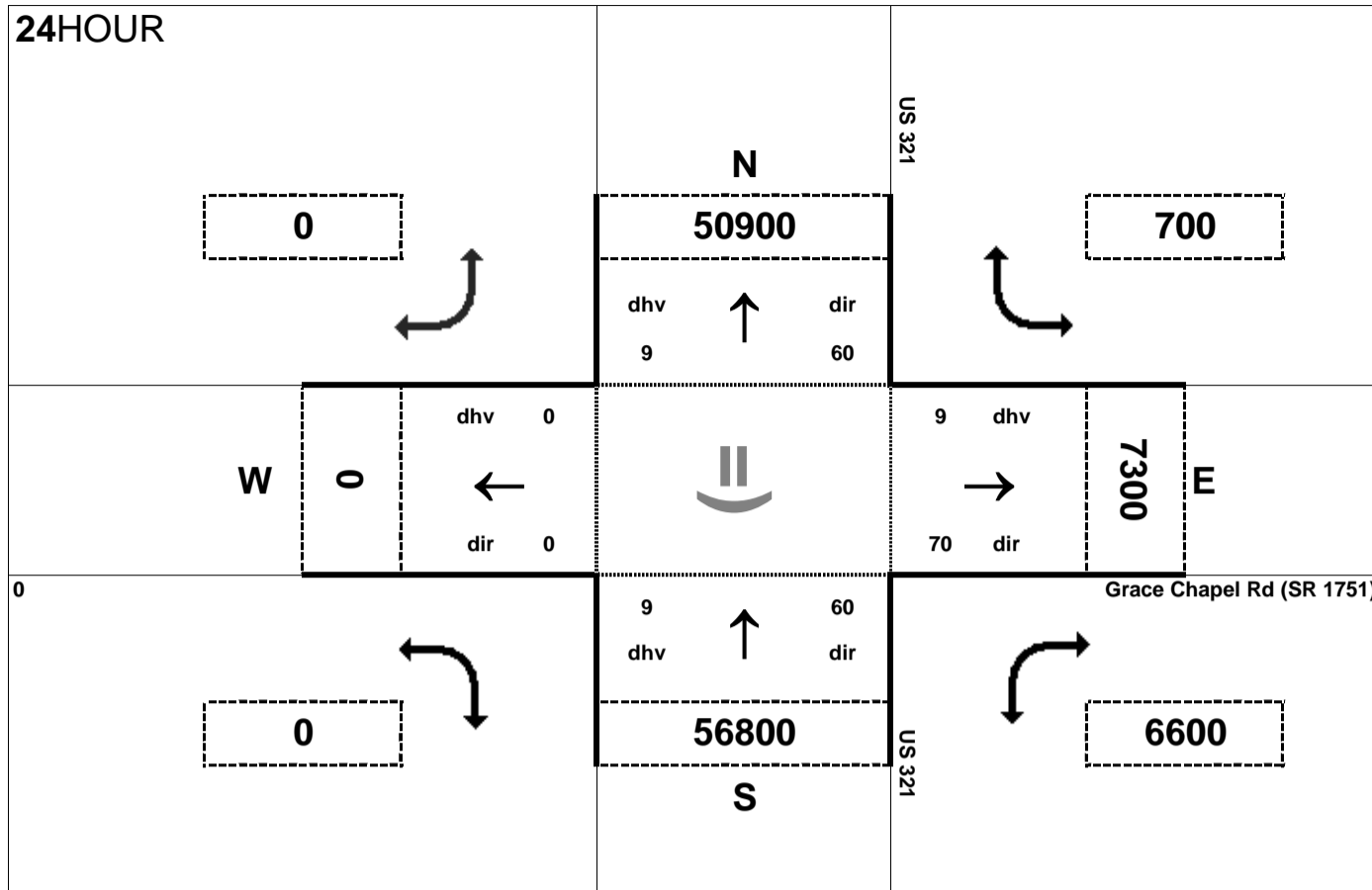
Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

Project:
U-4700



24HOUR



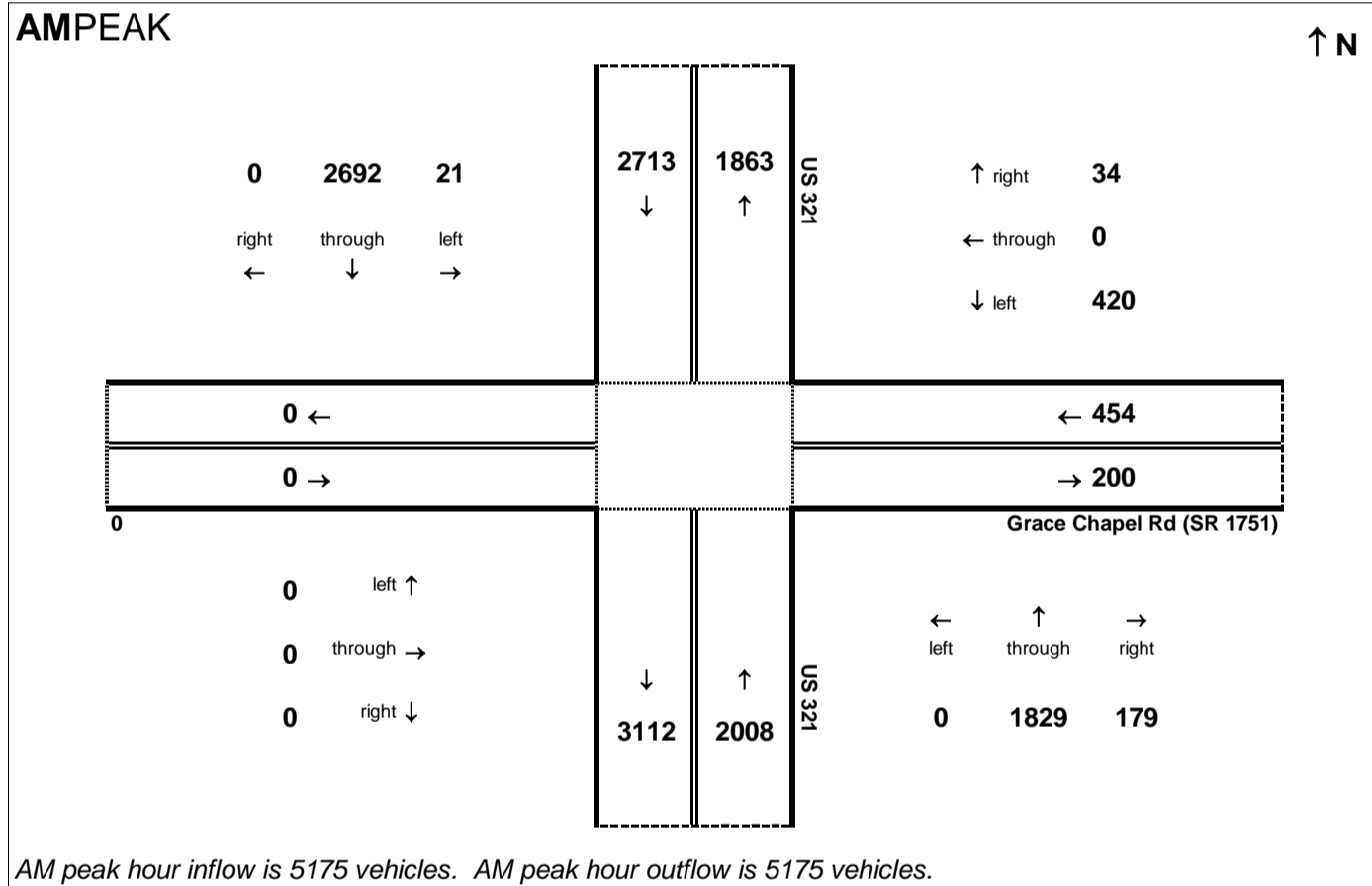
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Grace Chapel Rd

Traffic Forecast Release Date:
December-16

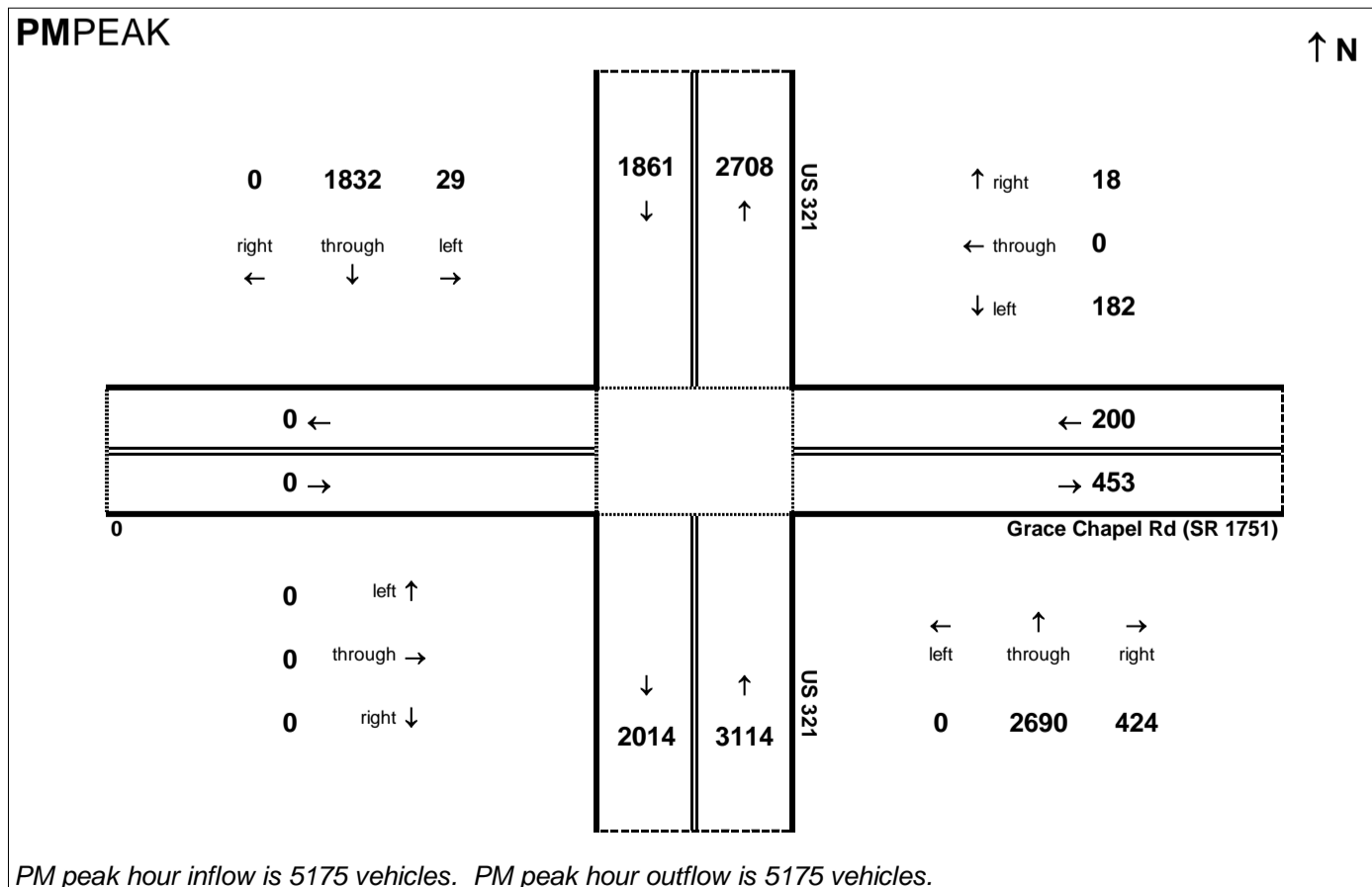
Traffic Data Year:
2040 Build 1

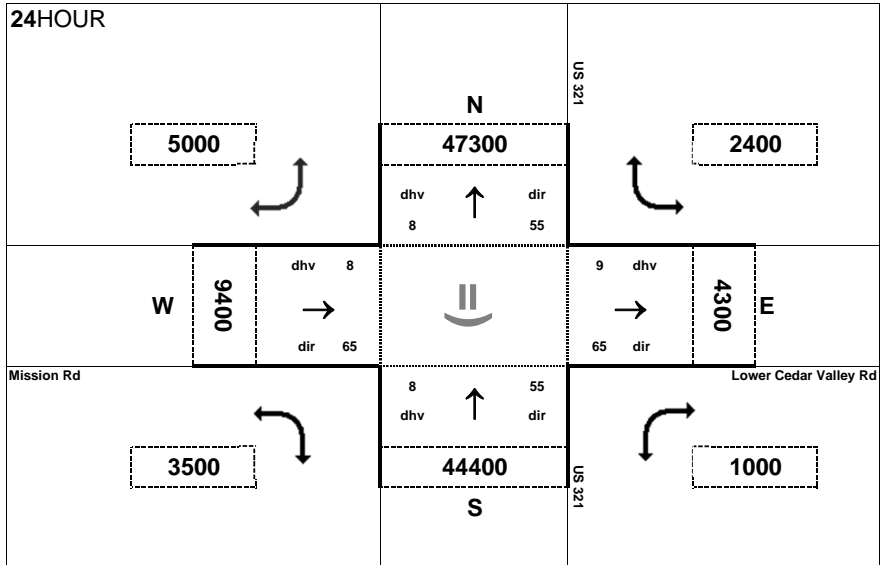
Project:
U-4700

AMPEAK



PMPEAK



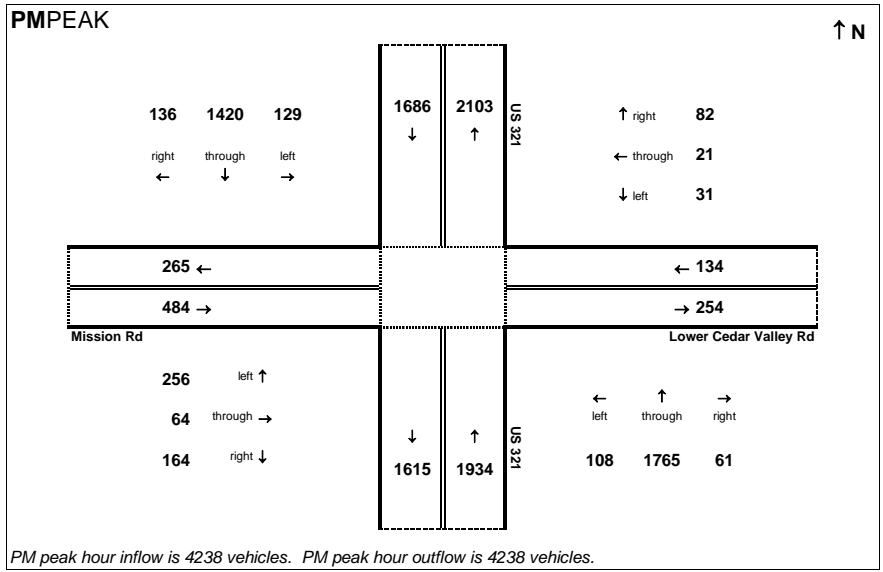
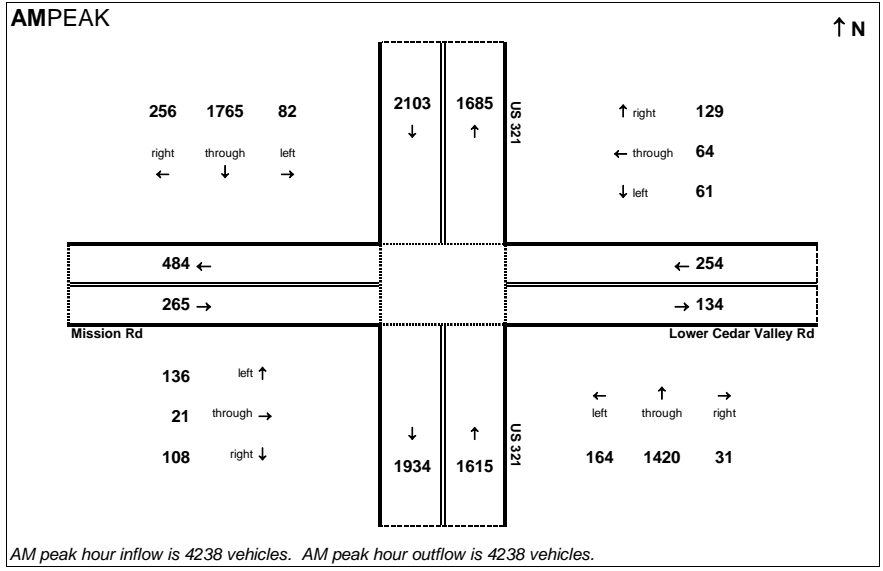


Peak Hour Volume Breakouts Report:
 Intersection of US 321 and Lower Cedar Valley Rd / Mission Rd

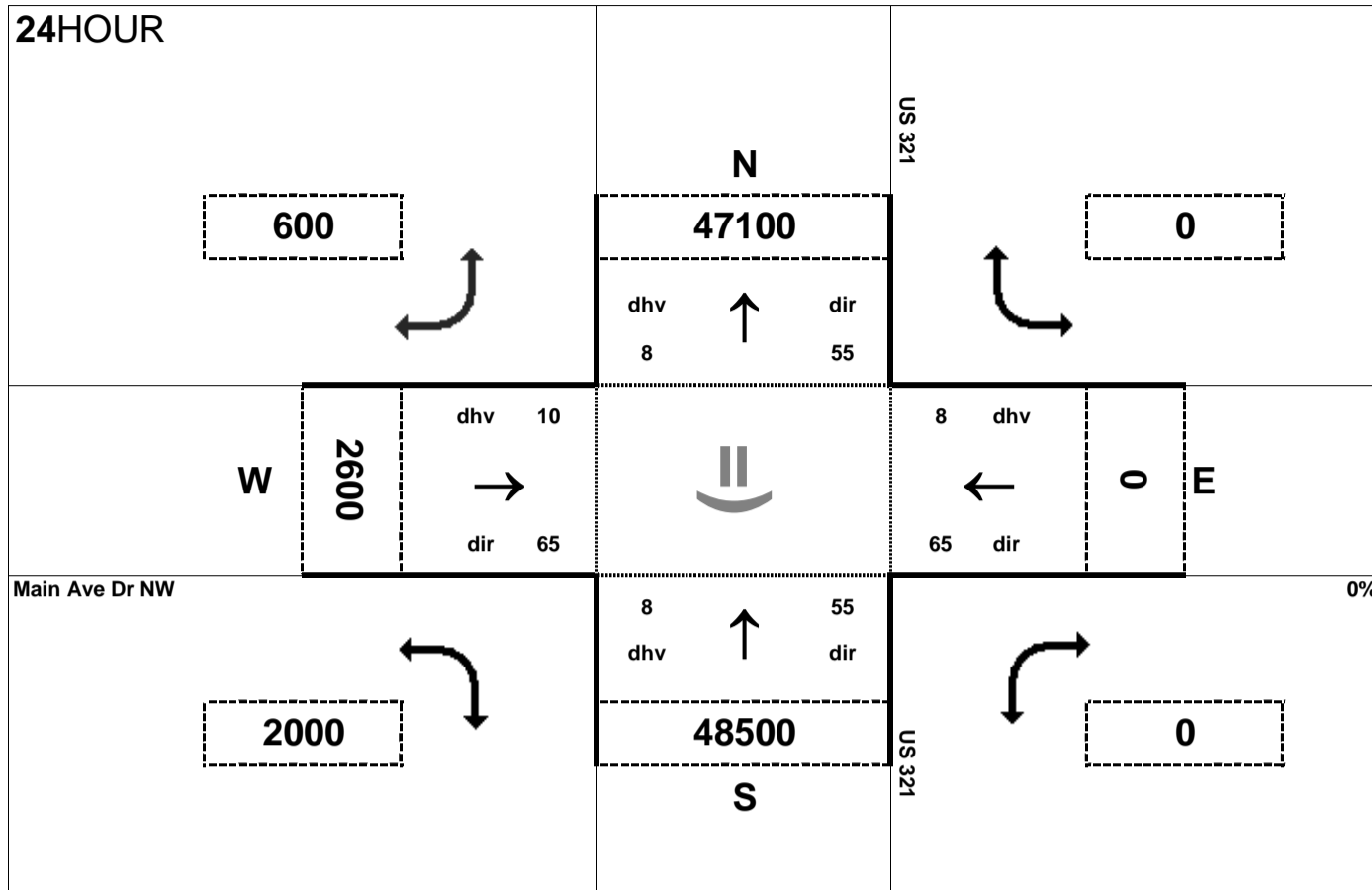
Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



24HOUR



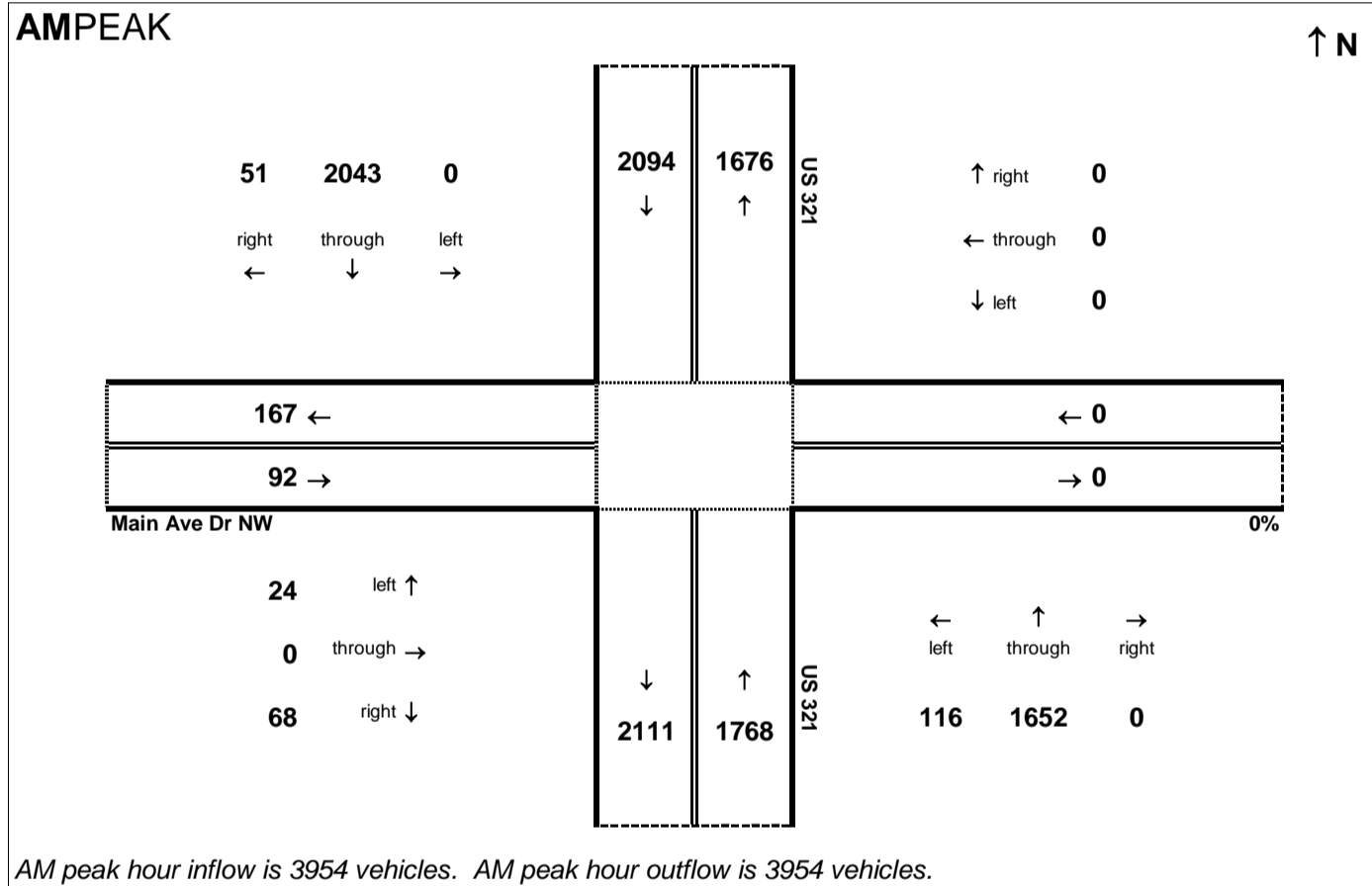
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Main Ave Dr NW

Traffic Forecast Release Date:
February-17

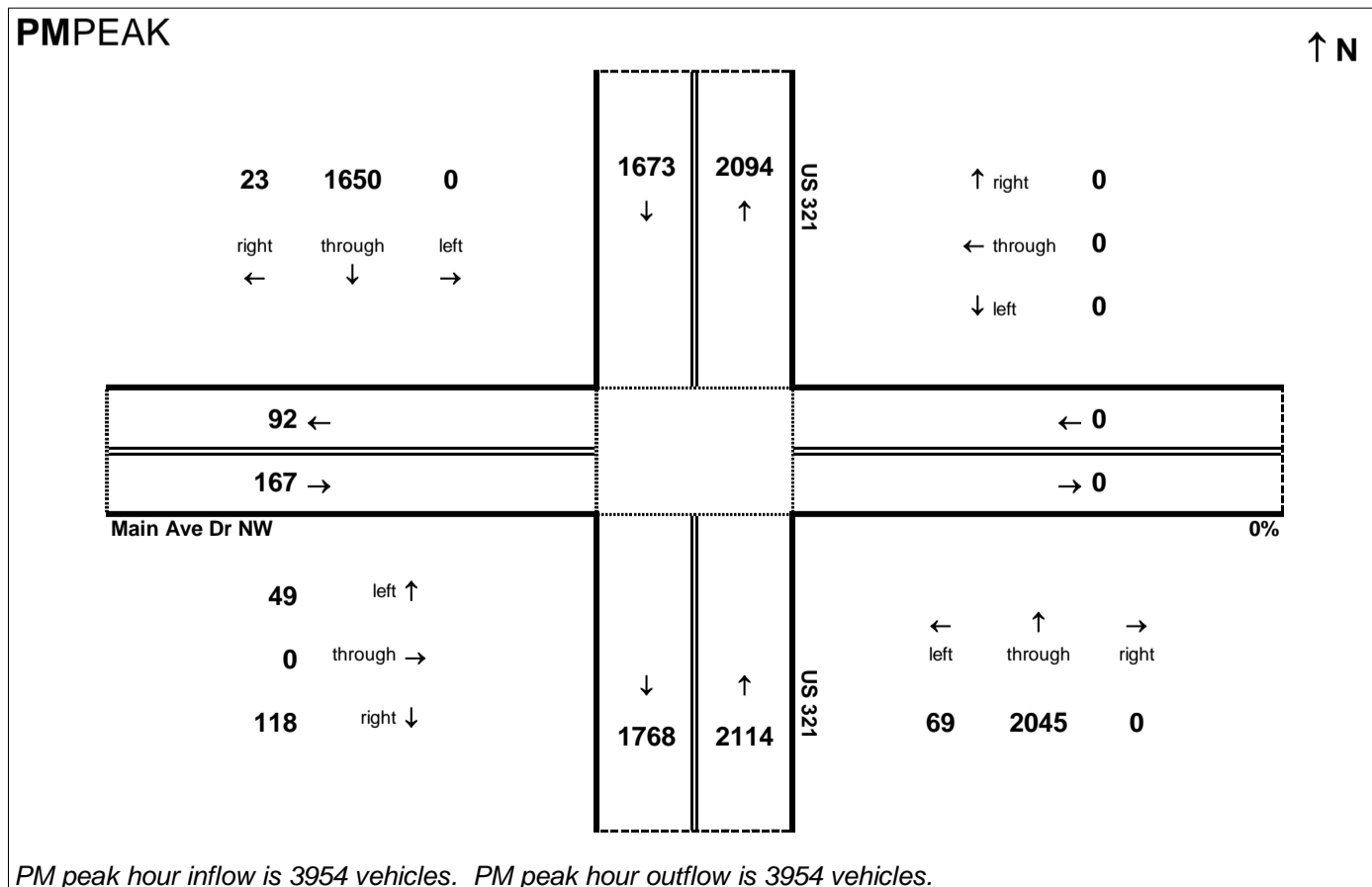
Traffic Data Year:
2040 Build 1

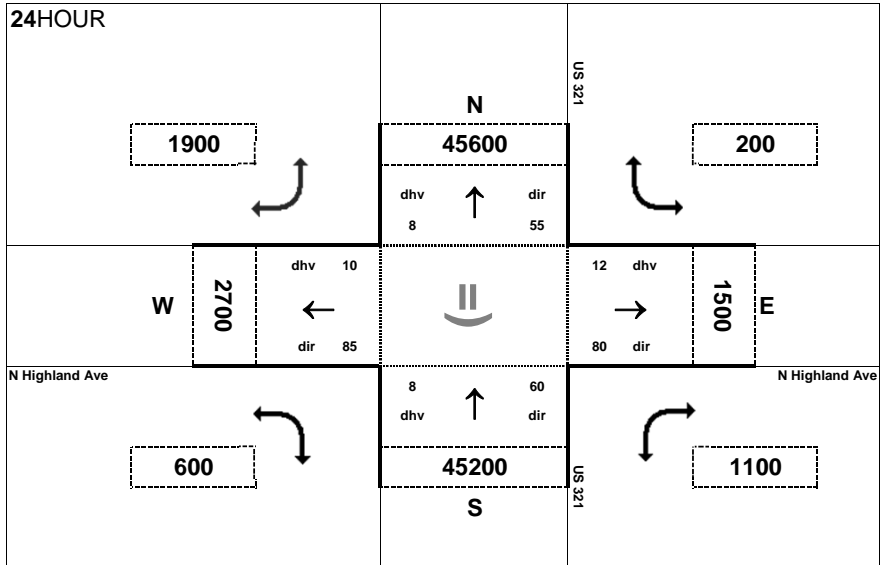
Project:
U-4700

AMPEAK



PMPEAK



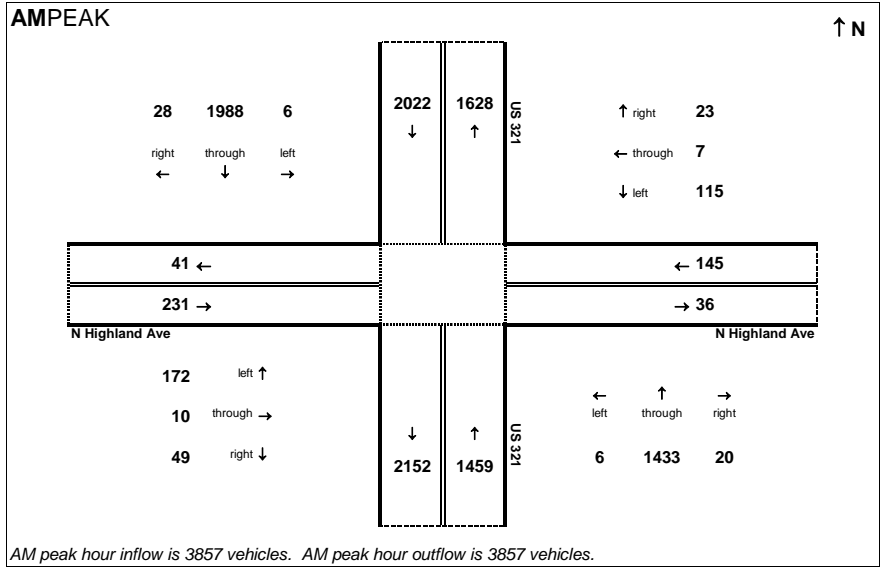


Peak Hour Volume Breakouts Report:
Intersection of US 321 and North Highland Ave

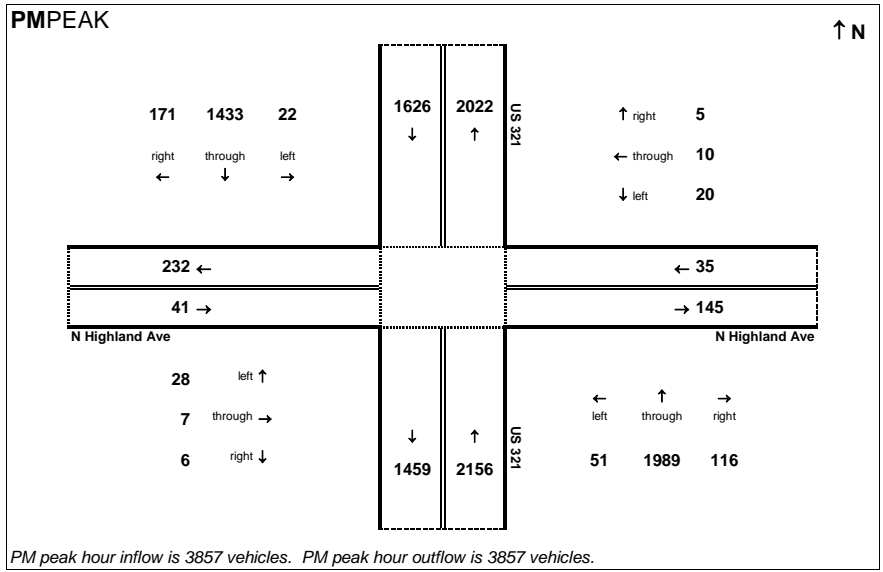
Traffic Forecast Release Date:
February-17

Traffic Data Year:
2040 Build 1

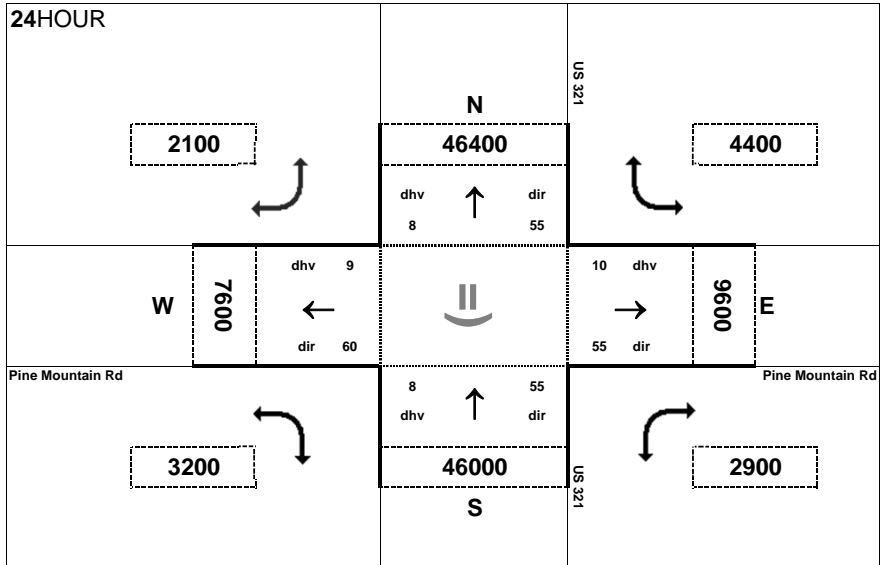
Project:
U-4700



AM peak hour inflow is 3857 vehicles. AM peak hour outflow is 3857 vehicles.



PM peak hour inflow is 3857 vehicles. PM peak hour outflow is 3857 vehicles.

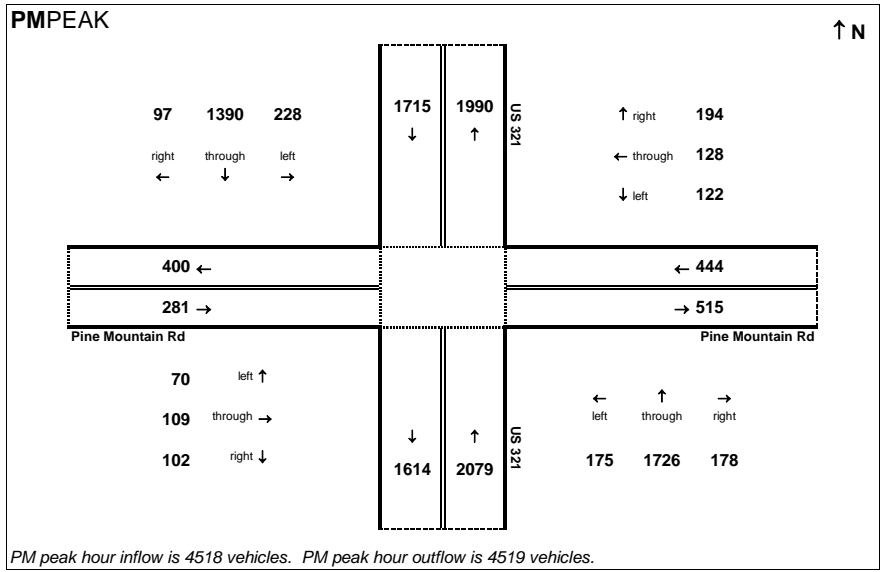
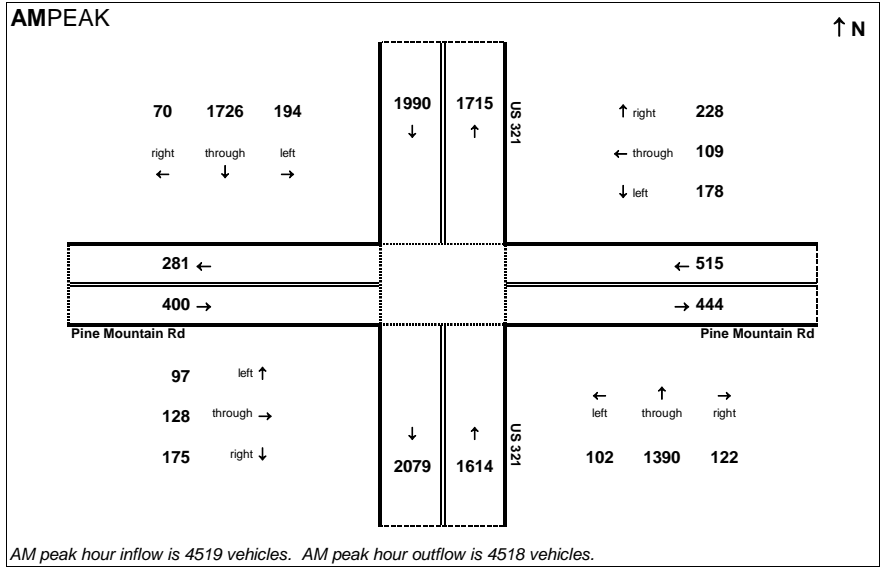


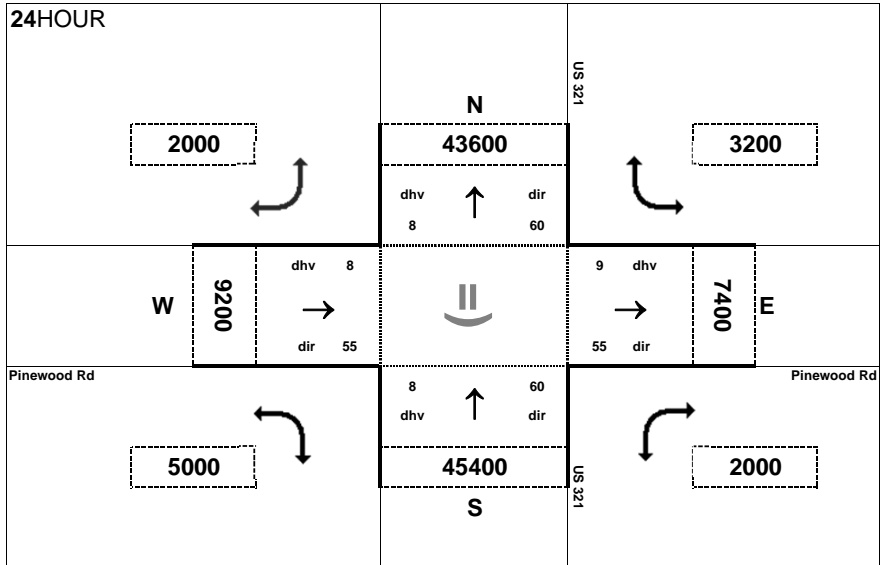
Peak Hour Volume Breakouts Report:
 Intersection of US 321 and Pine Mountain Rd

Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



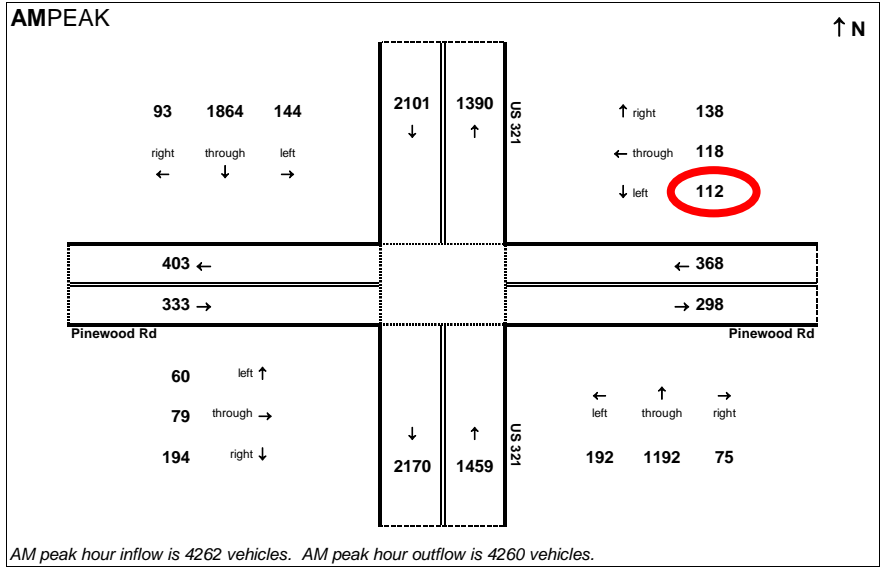


Peak Hour Volume Breakouts Report:
Intersection of US 321 and Pinewood Rd

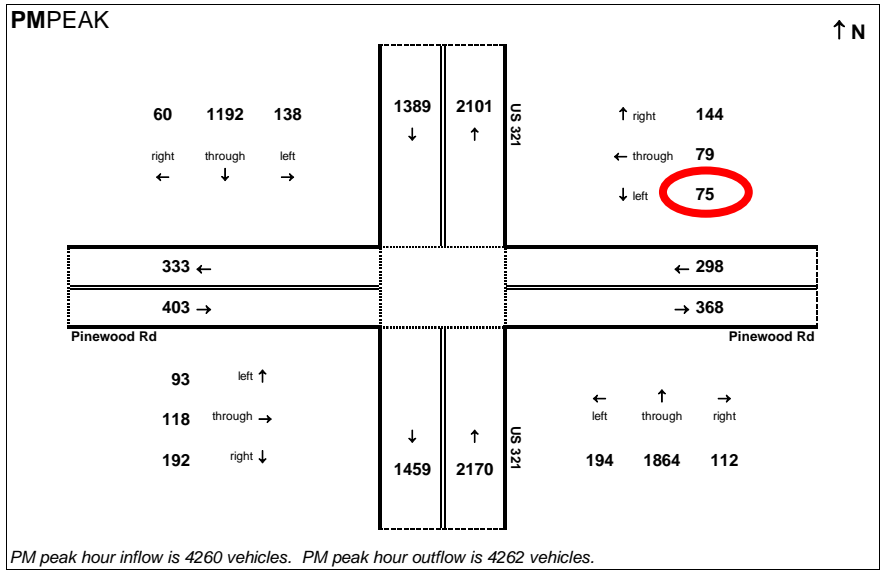
Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

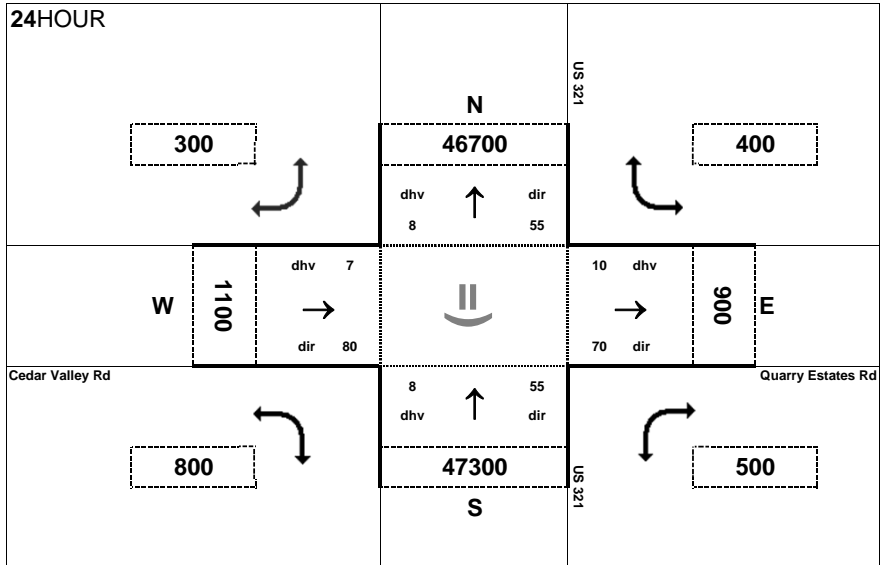
Project:
U-4700



AM peak hour inflow is 4262 vehicles. AM peak hour outflow is 4260 vehicles.



PM peak hour inflow is 4260 vehicles. PM peak hour outflow is 4262 vehicles.

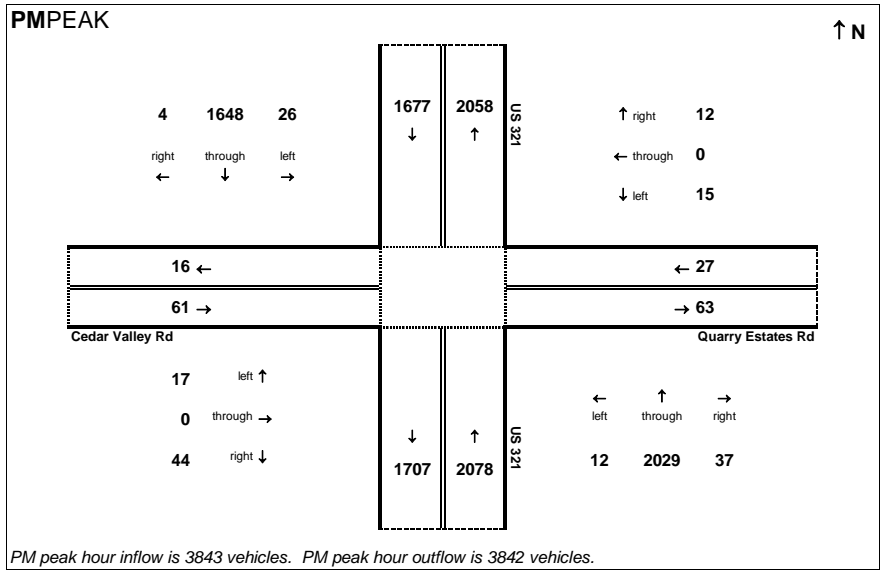
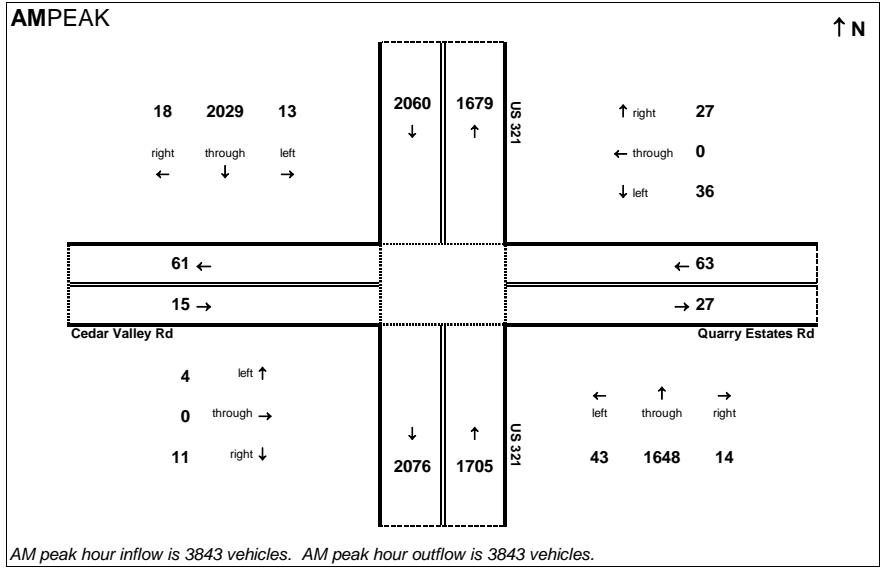


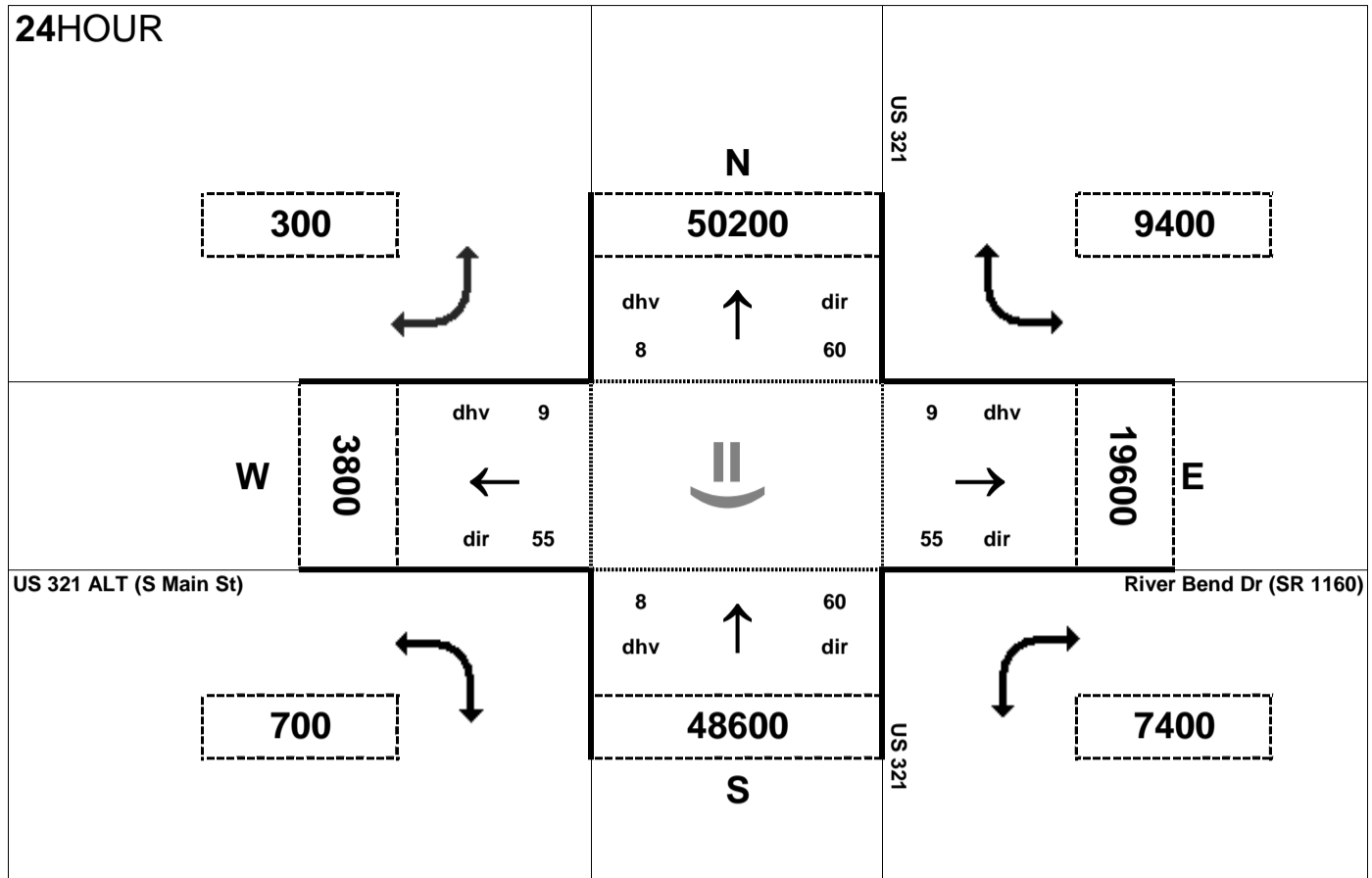
Peak Hour Volume Breakouts Report:
 Intersection of US 321 and Quarry Estates Rd /
 Cedar Valley Rd

Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 Build 1

Project:
 U-4700



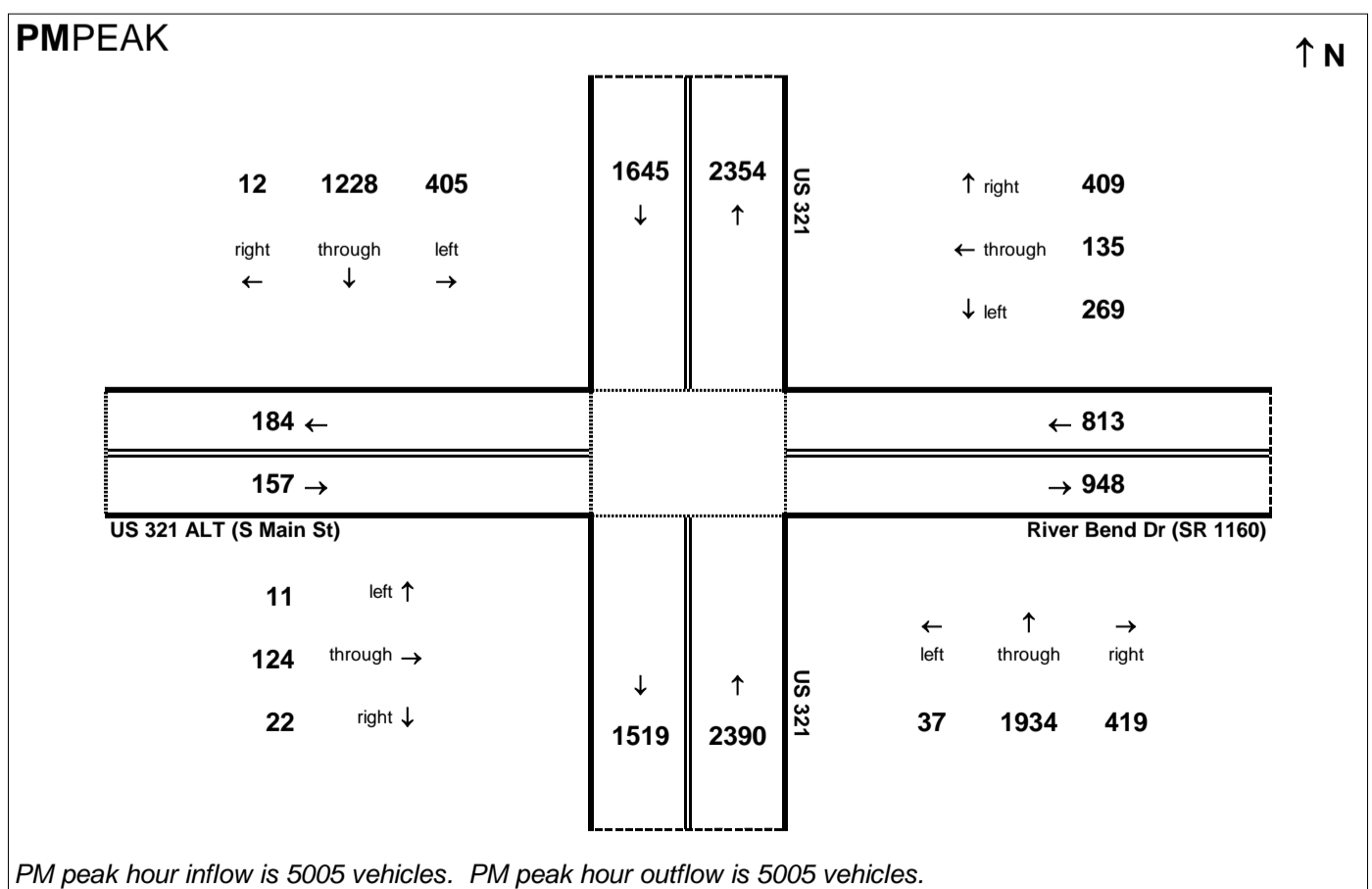
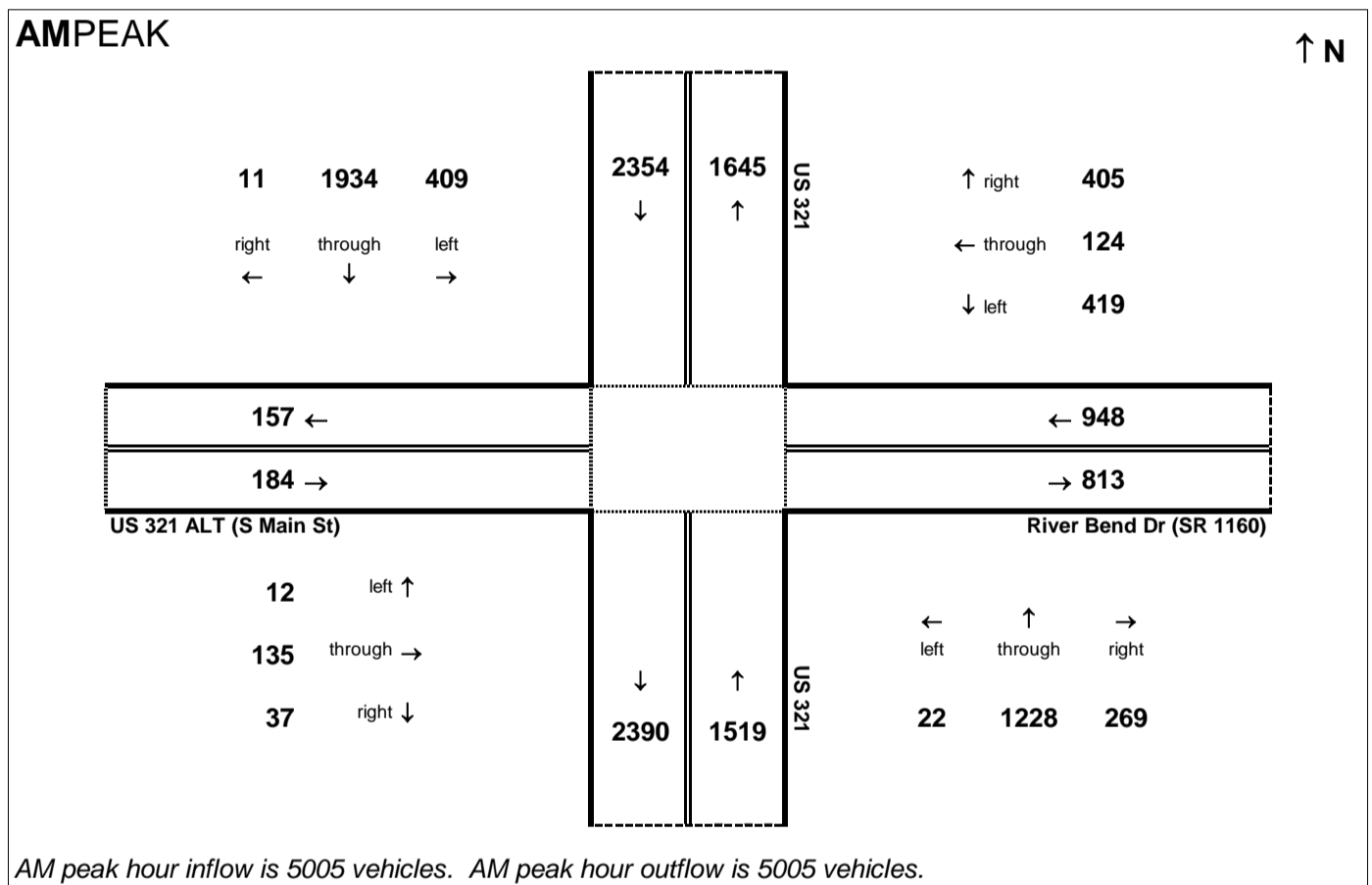


Peak Hour Volume Breakouts Report:
Intersection of US 321 and US 321 ALT

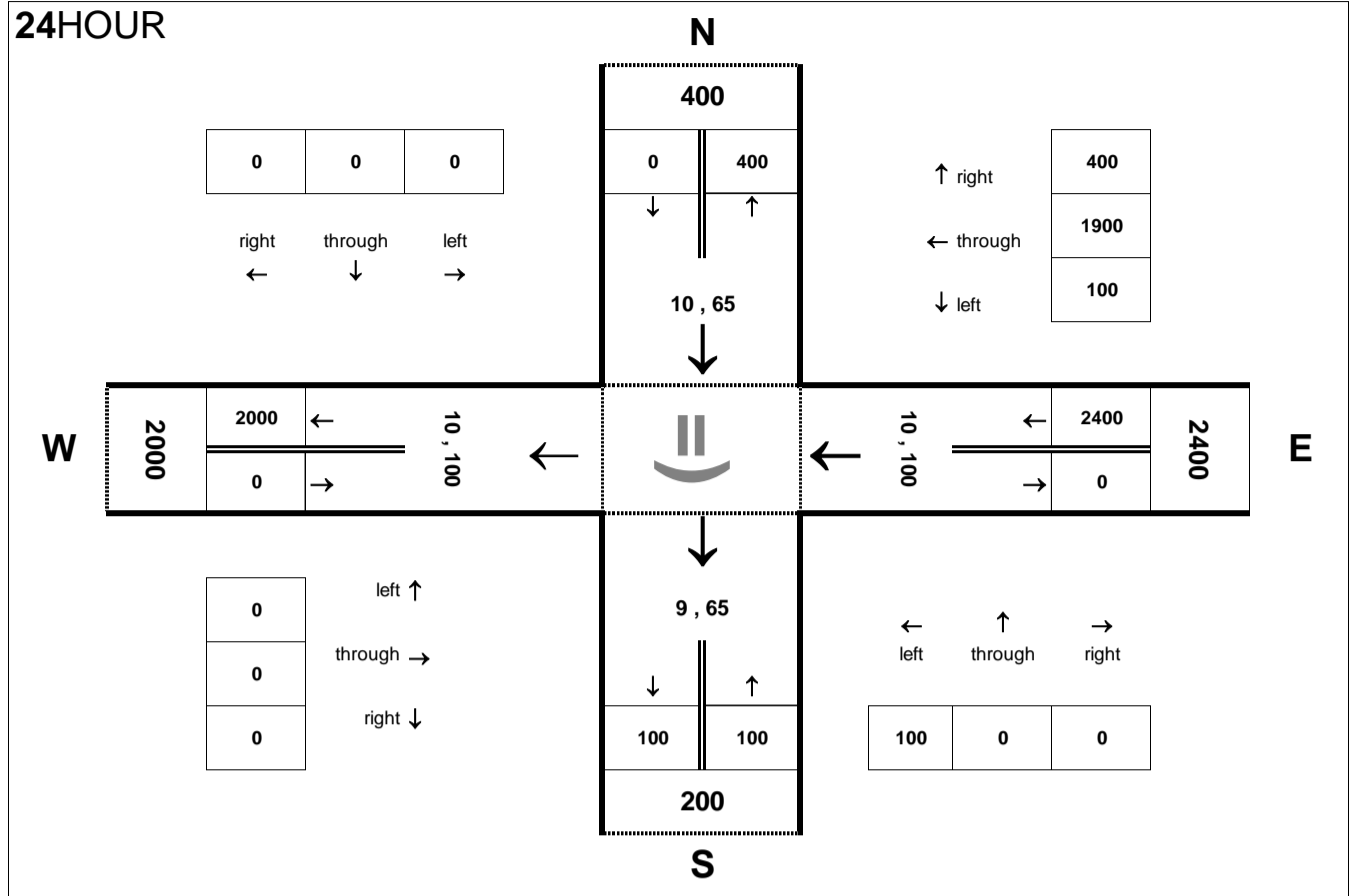
Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 1

Project:
U-4700



24HOUR



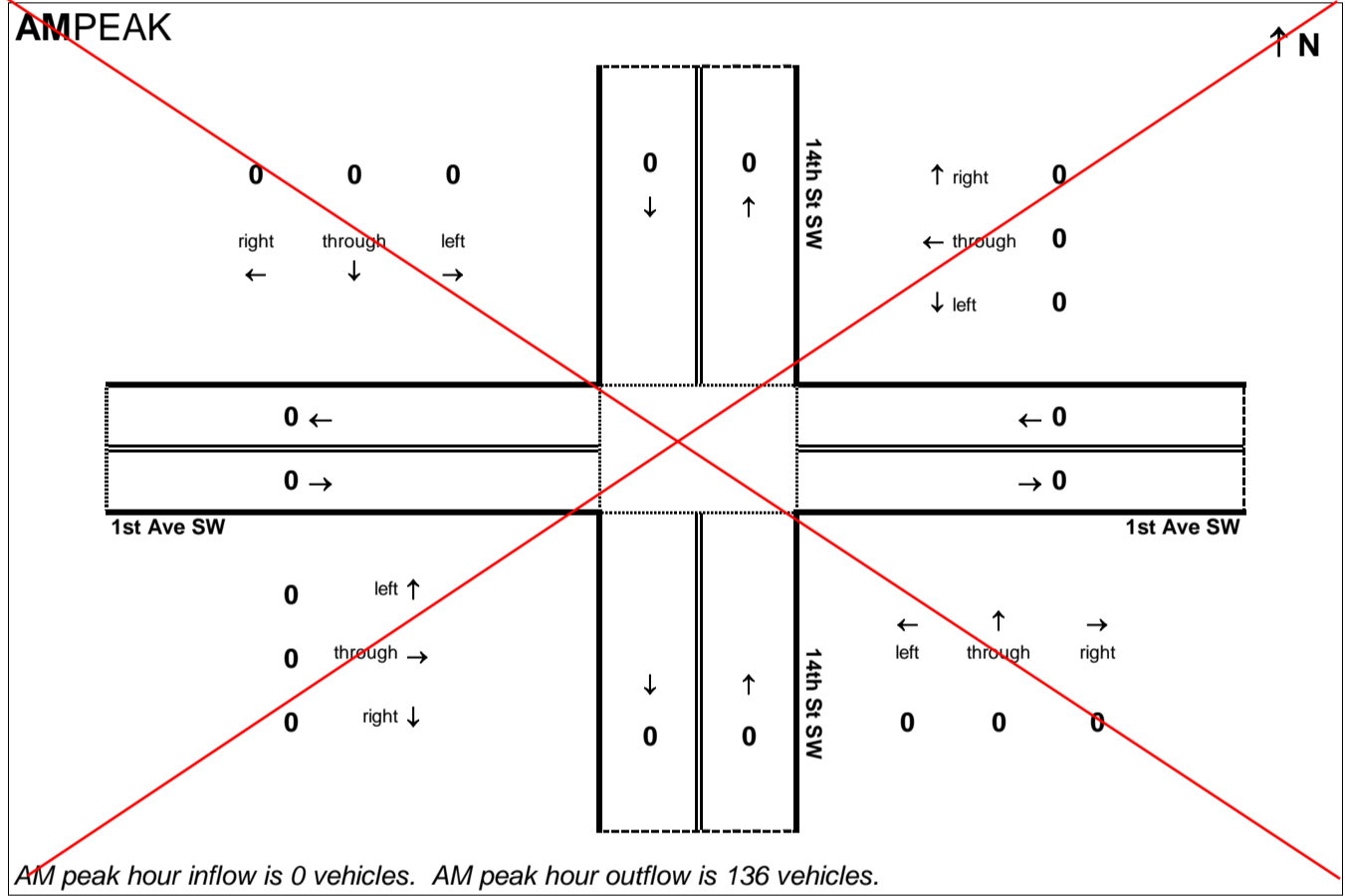
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 2

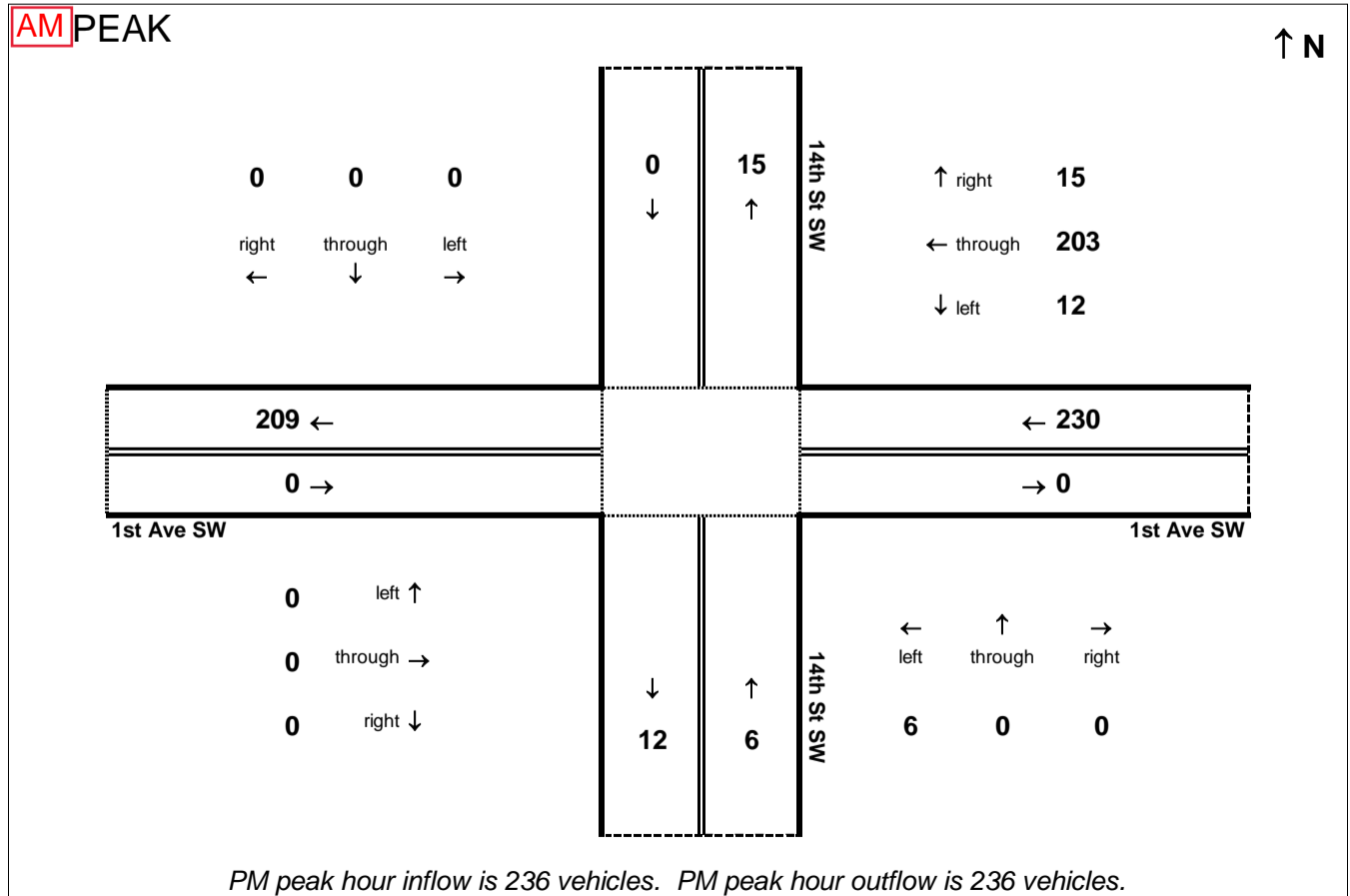
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 136 vehicles.

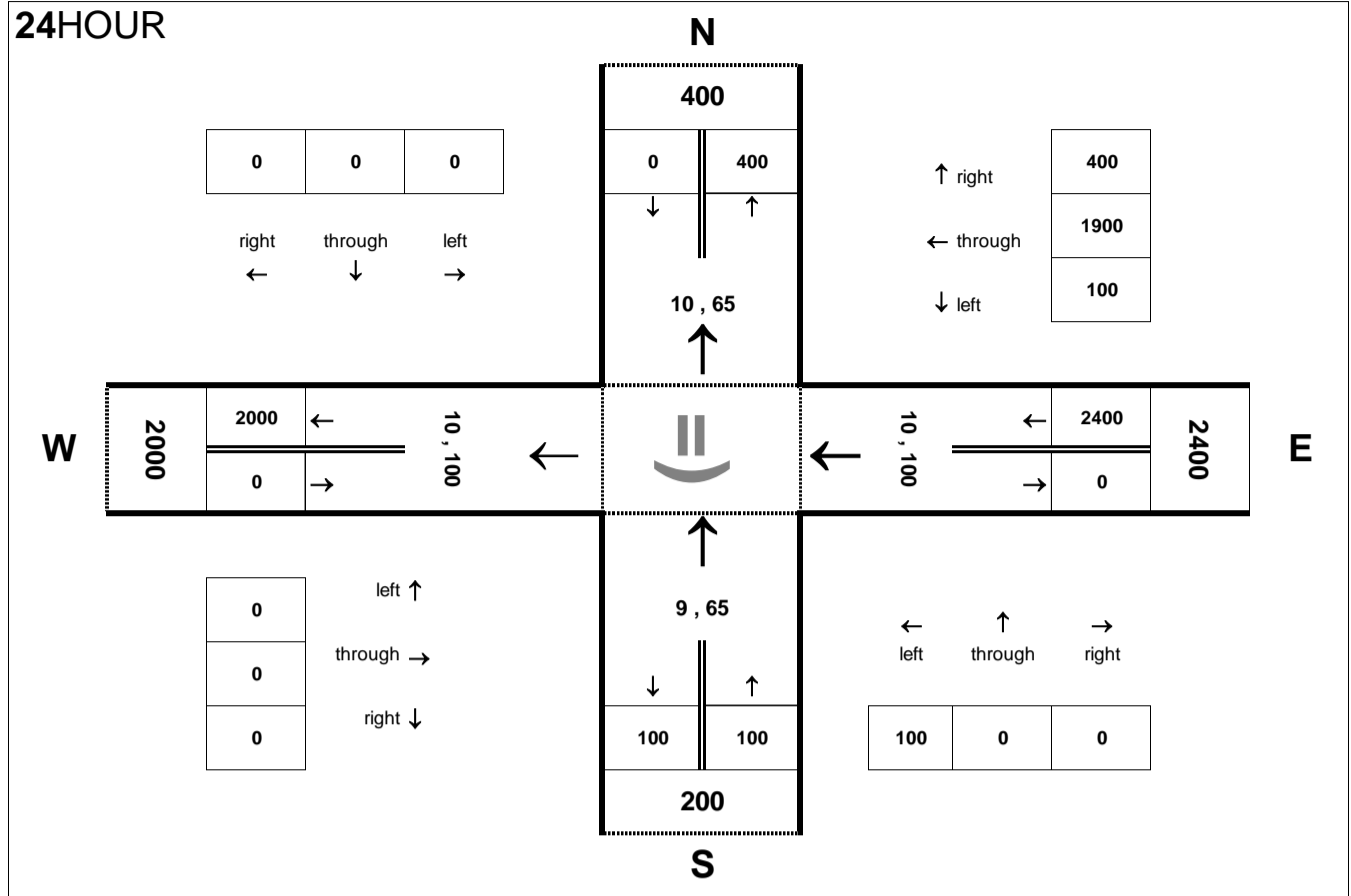
PM PEAK



PM peak hour inflow is 236 vehicles. PM peak hour outflow is 236 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directional for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



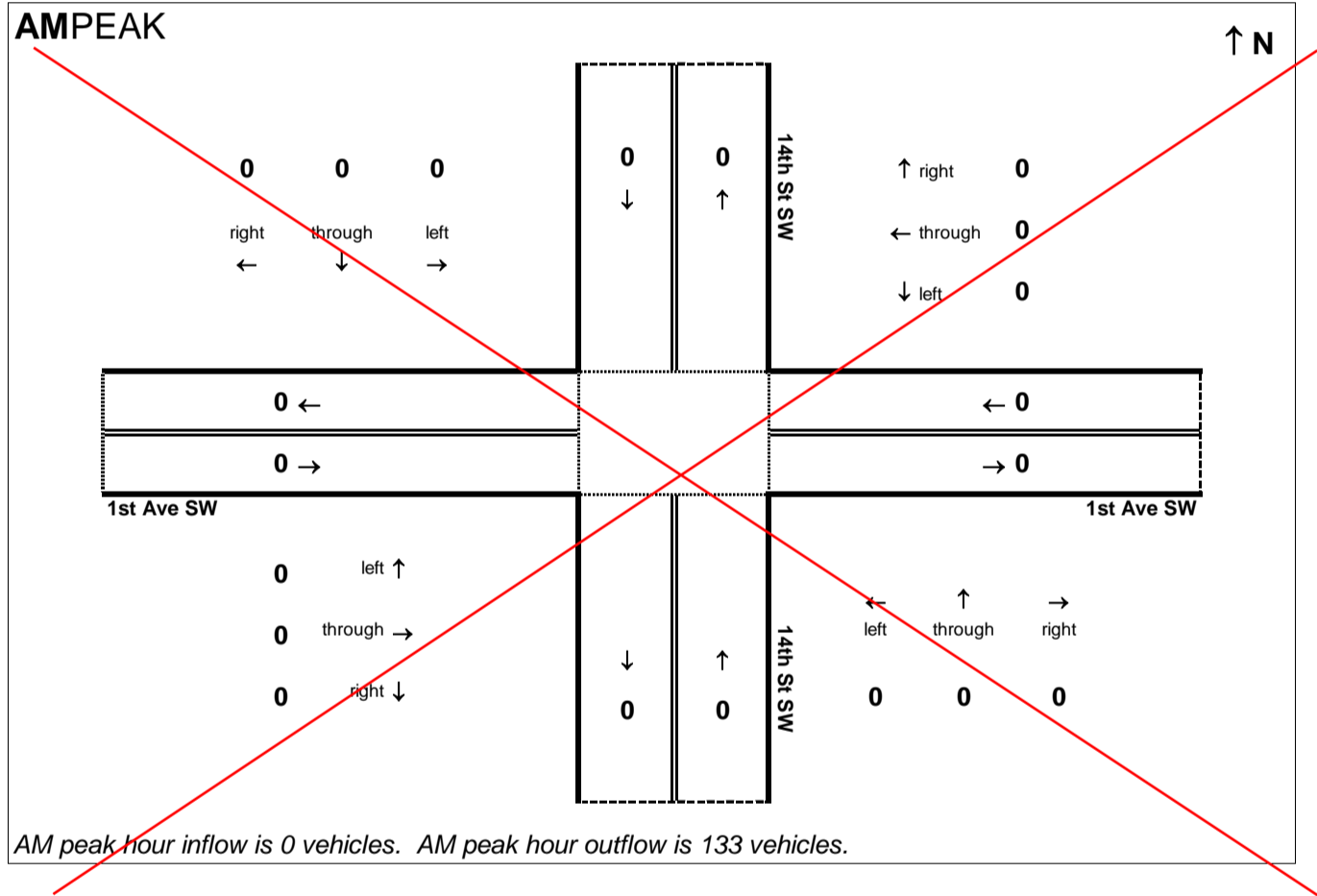
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 14th St SW

Traffic Forecast Release Date:
December-16

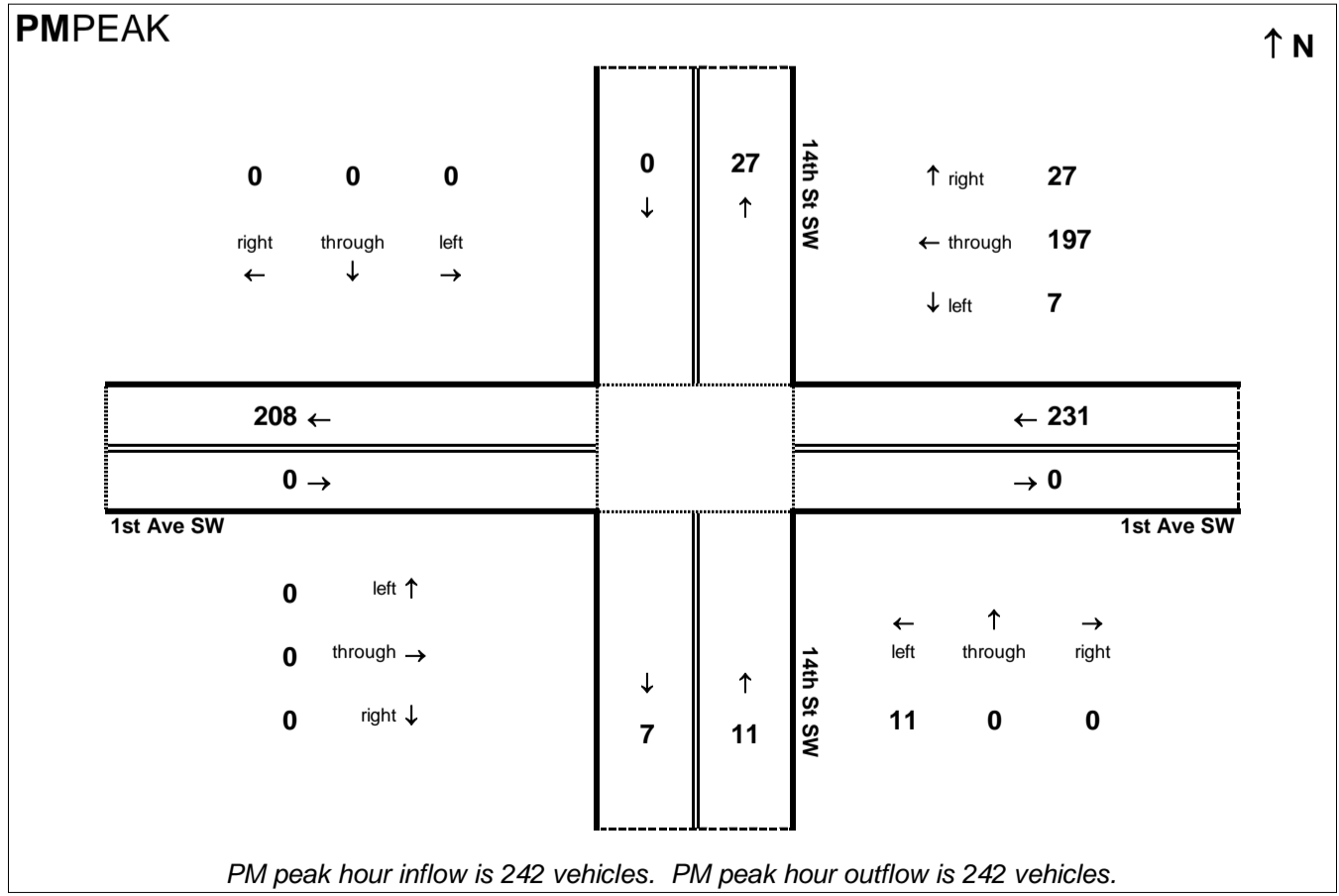
Traffic Data Year:
2040 Build 2

Project:
U-4700

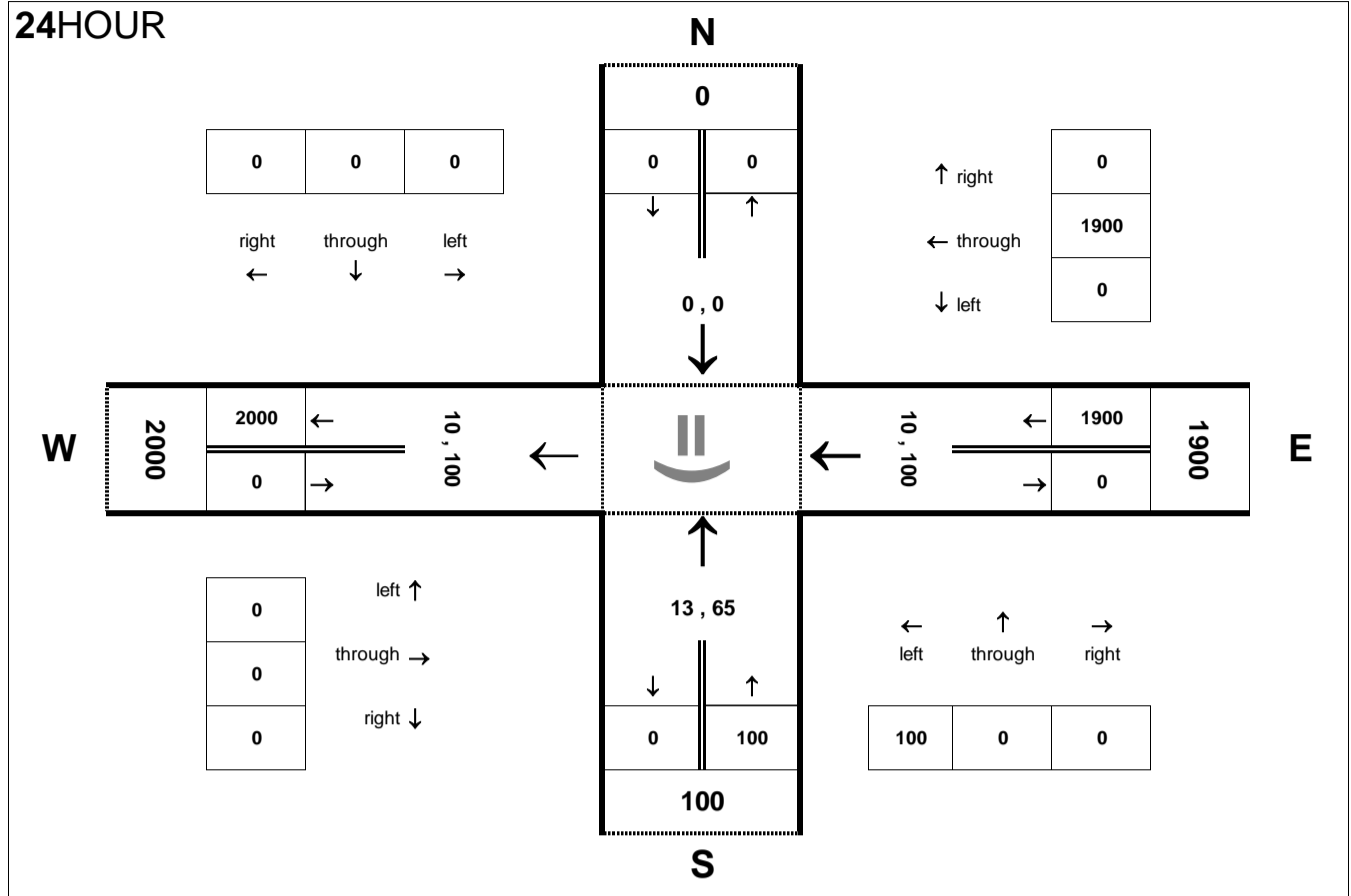
AMPEAK



PMPEAK



24HOUR



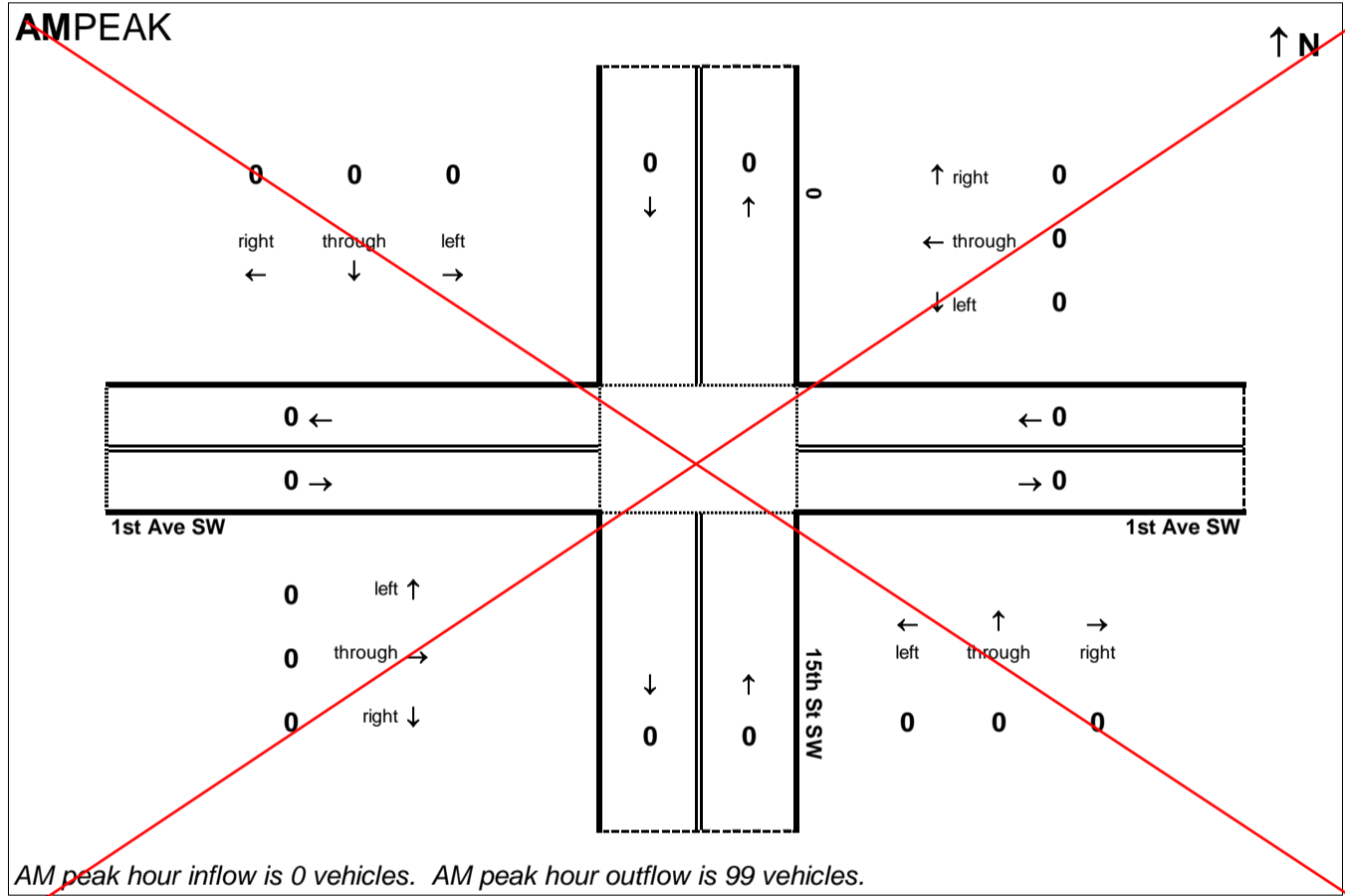
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 2

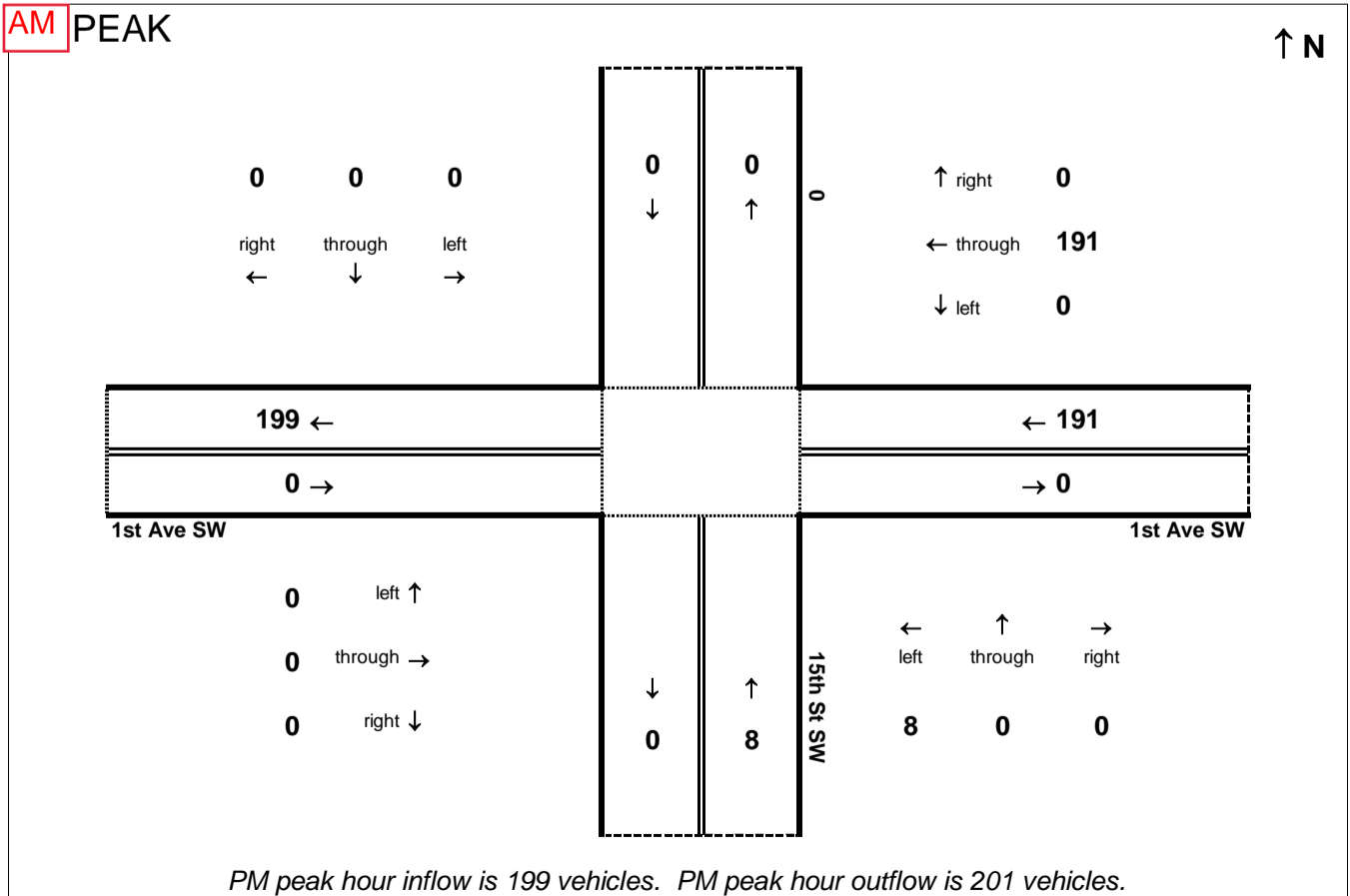
Project:
U-4700

AM PEAK



AM peak hour inflow is 0 vehicles. AM peak hour outflow is 99 vehicles.

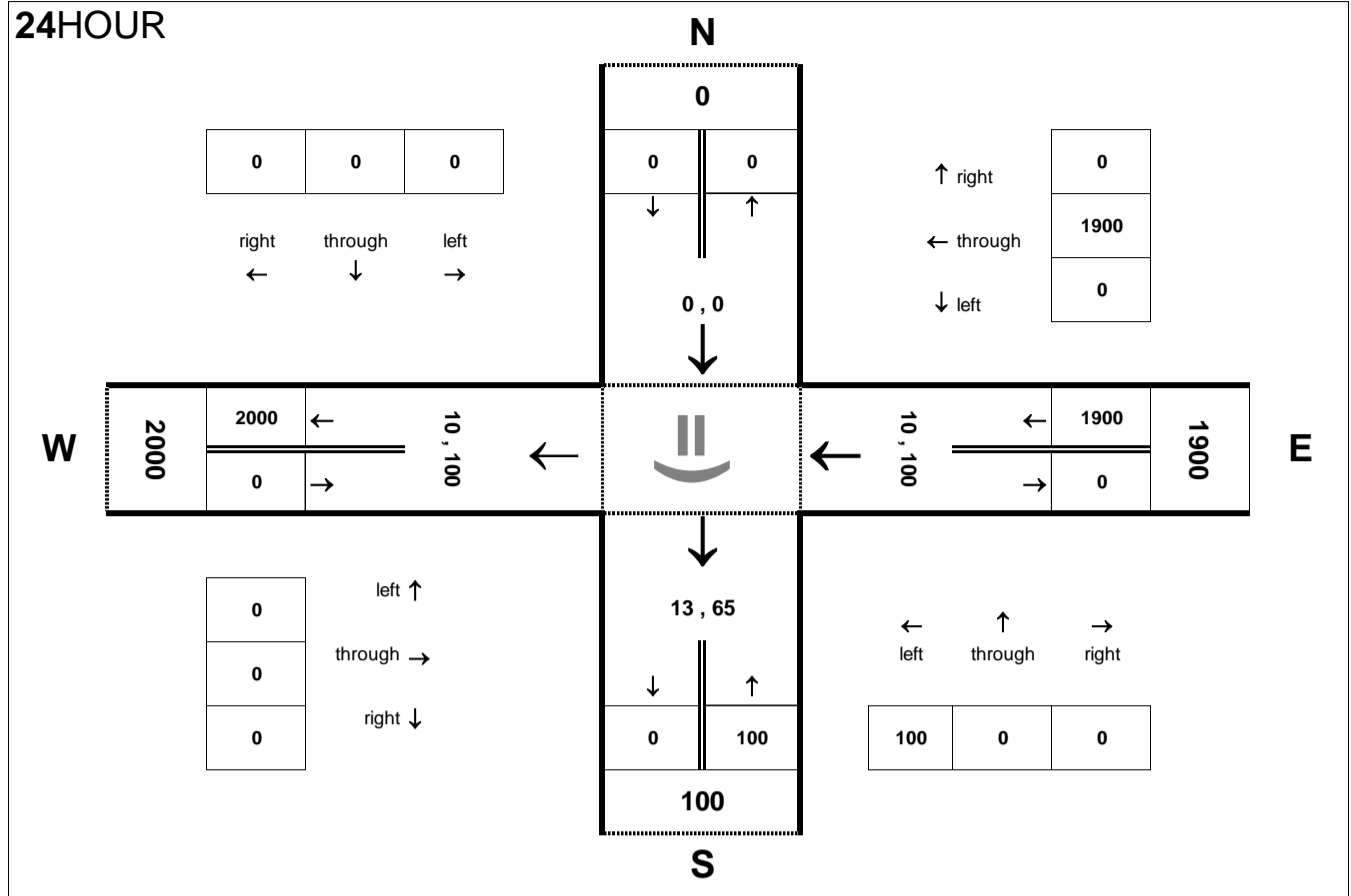
AM PEAK



PM peak hour inflow is 199 vehicles. PM peak hour outflow is 201 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



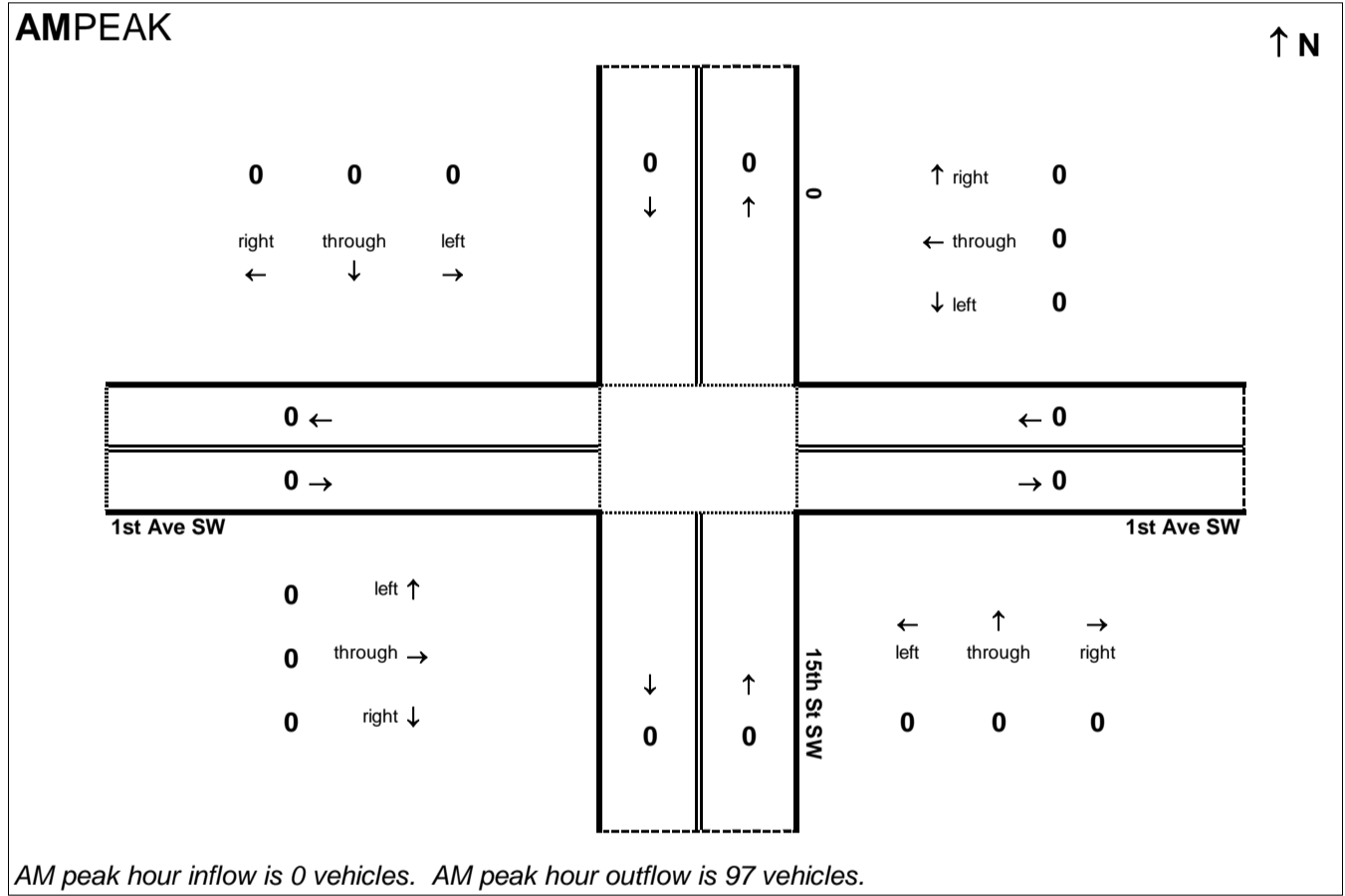
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

Traffic Data Year:
2040 Build 2

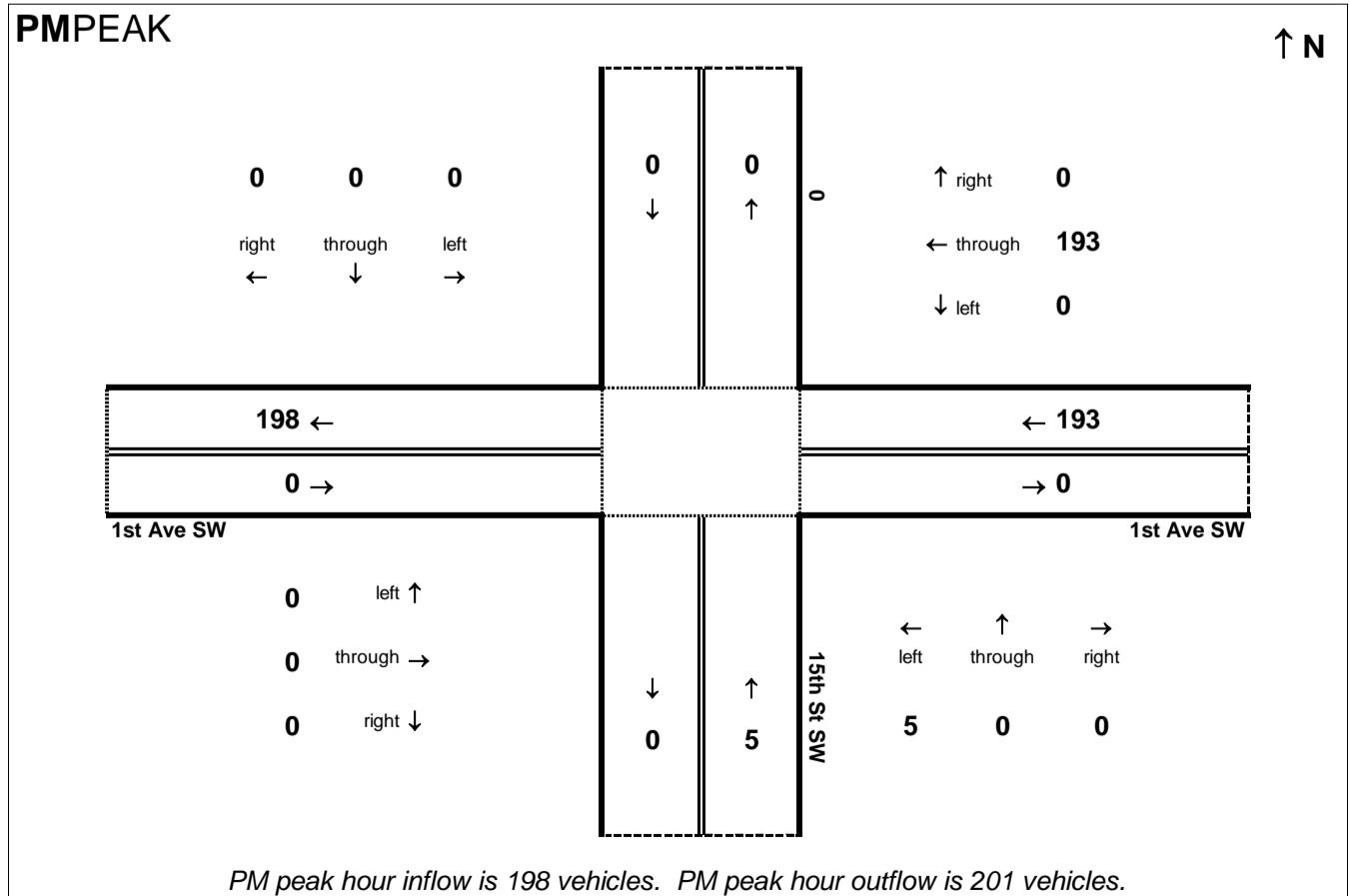
Project:
U-4700

AMPEAK



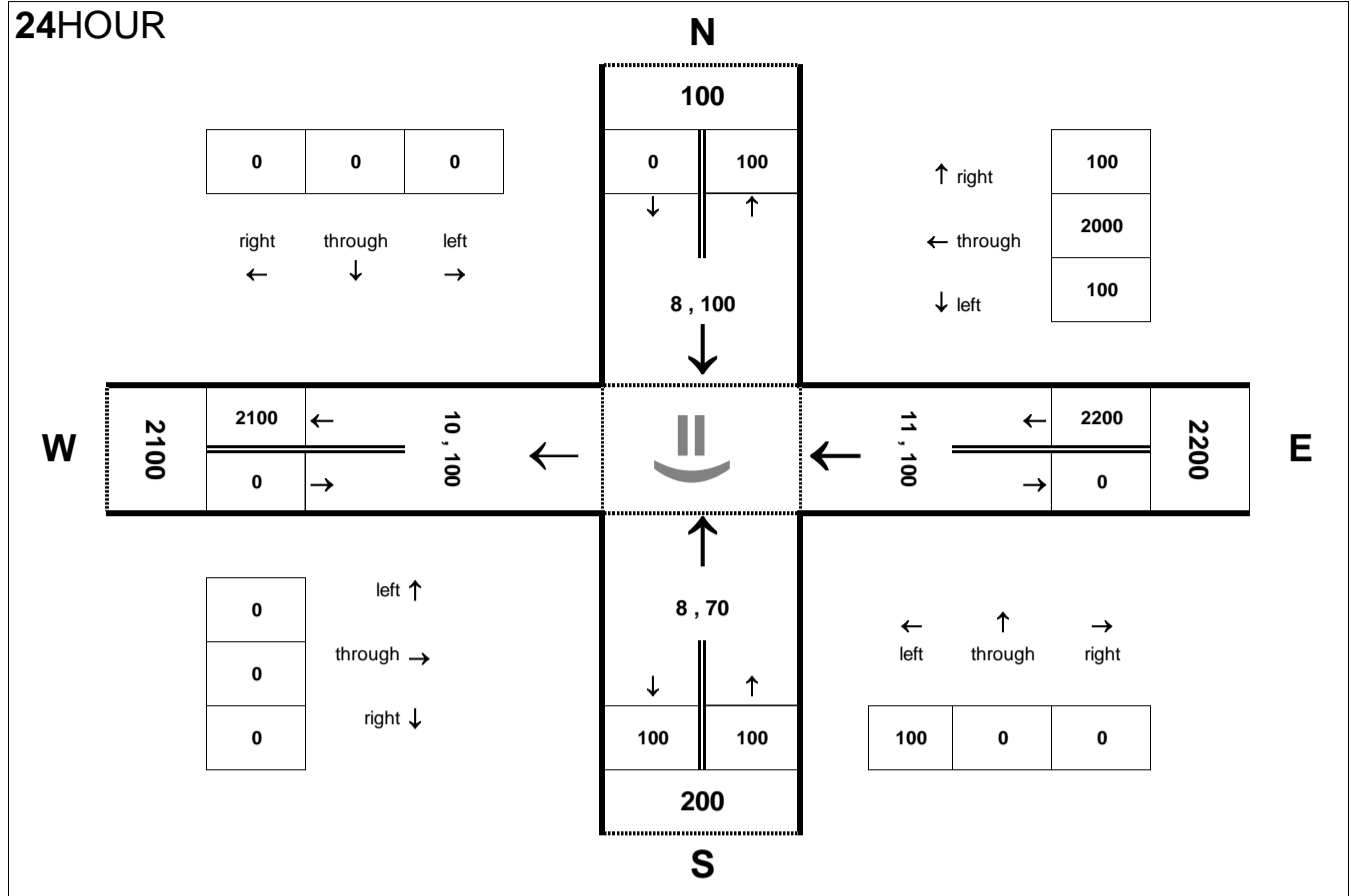
AM peak hour inflow is 0 vehicles. AM peak hour outflow is 97 vehicles.

PMPEAK



PM peak hour inflow is 198 vehicles. PM peak hour outflow is 201 vehicles.

24HOUR



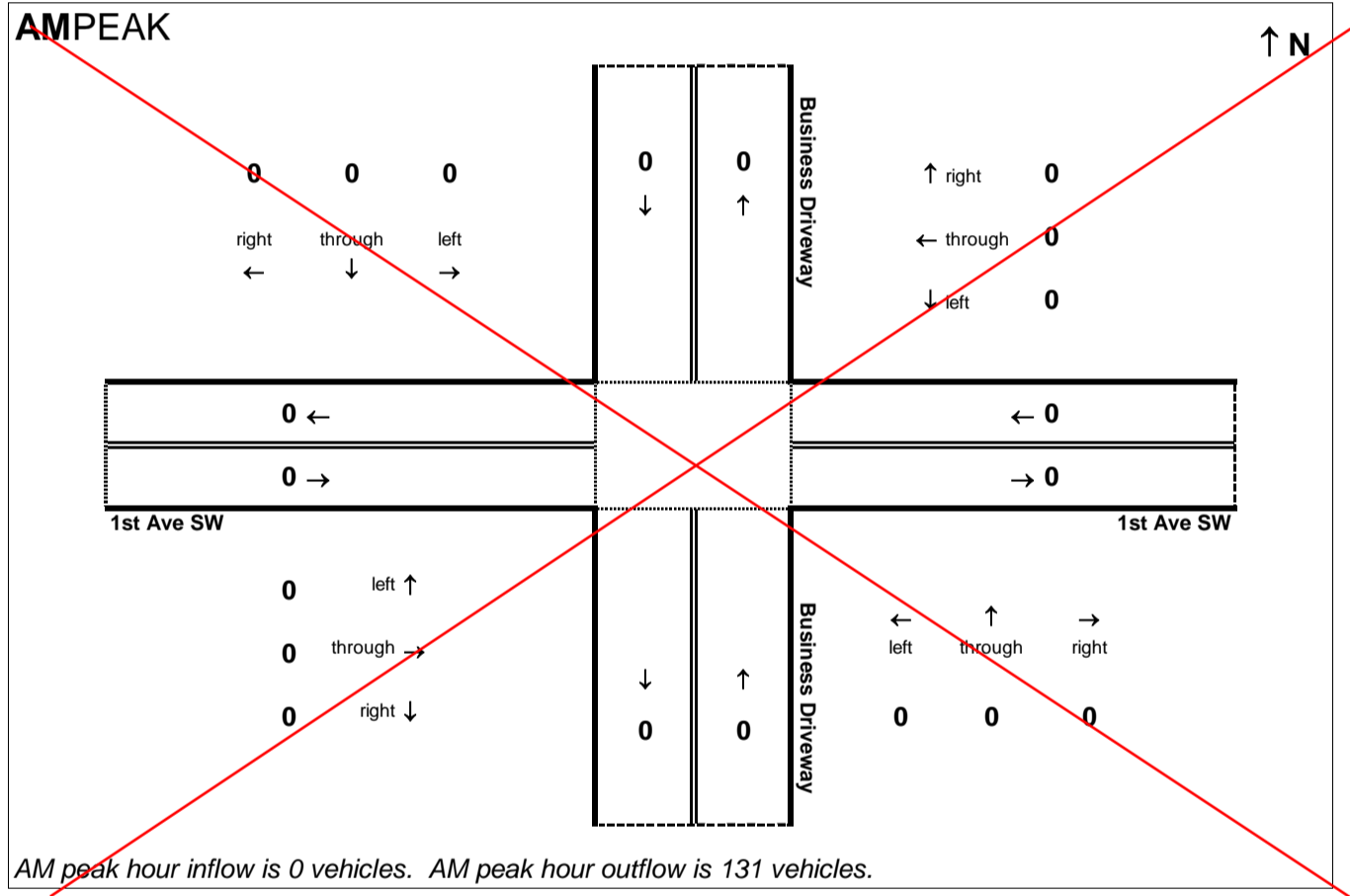
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and Business Driveway

Traffic Forecast Release Date:
December-16

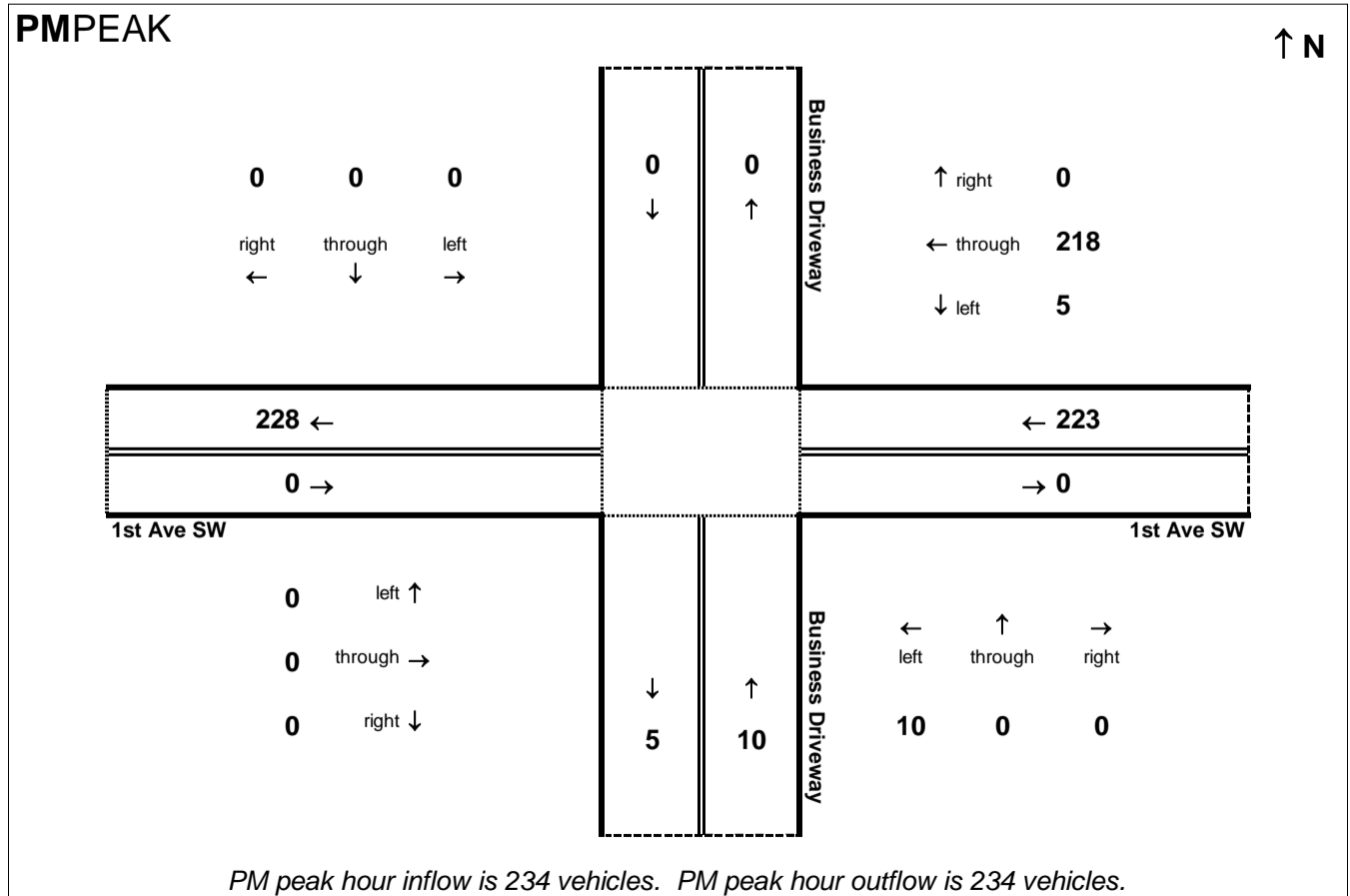
Traffic Data Year:
2040 Build 2

Project:
U-4700

AMPEAK



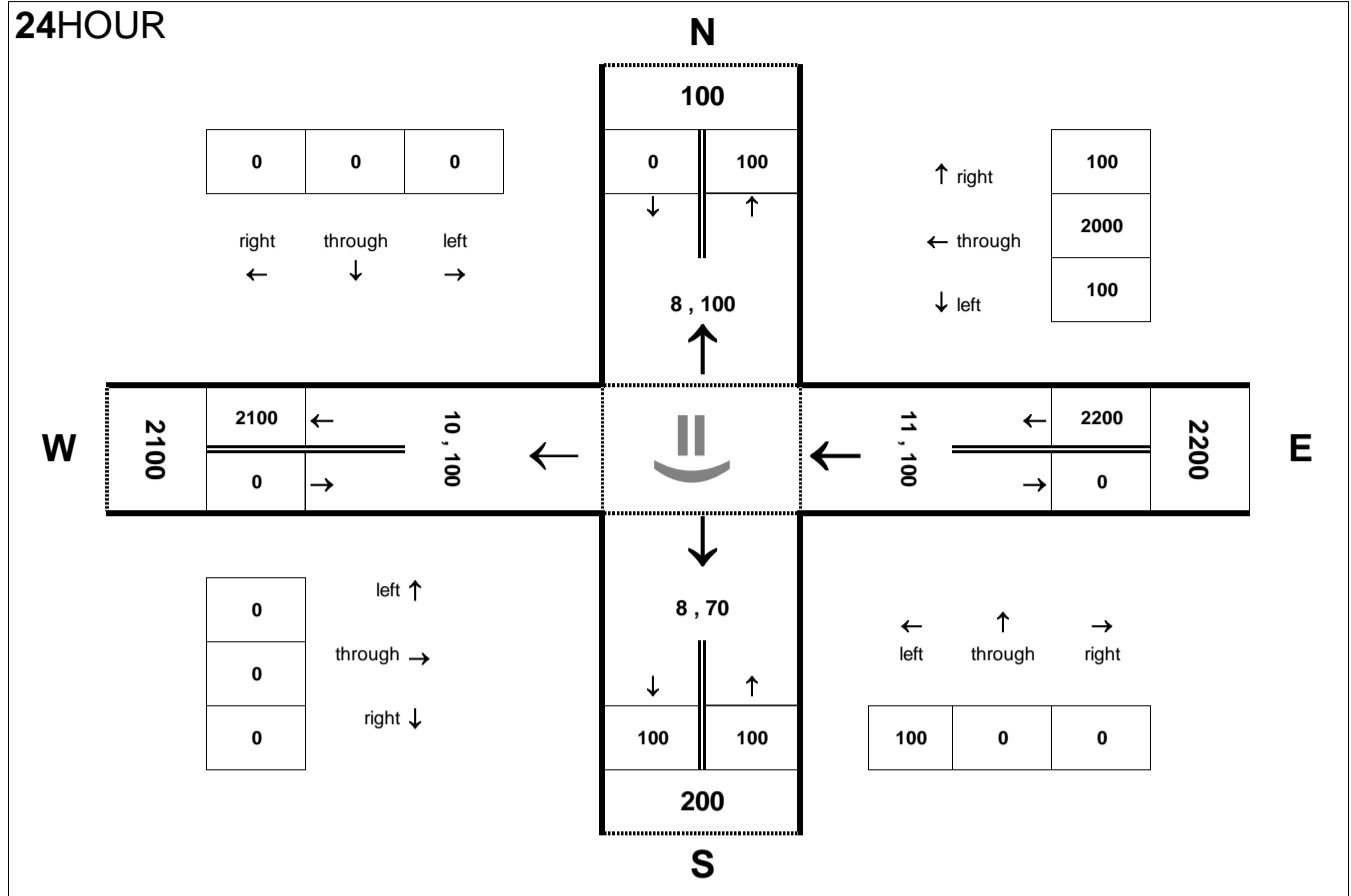
PMPEAK



PM peak hour inflow is 234 vehicles. PM peak hour outflow is 234 vehicles.

Note: In order to use the spreadsheet program, and use the 100% directional in the AM on the one-way street(s), the input directionals for the two way moves were reversed, and the pm output data was used for the am.

24HOUR



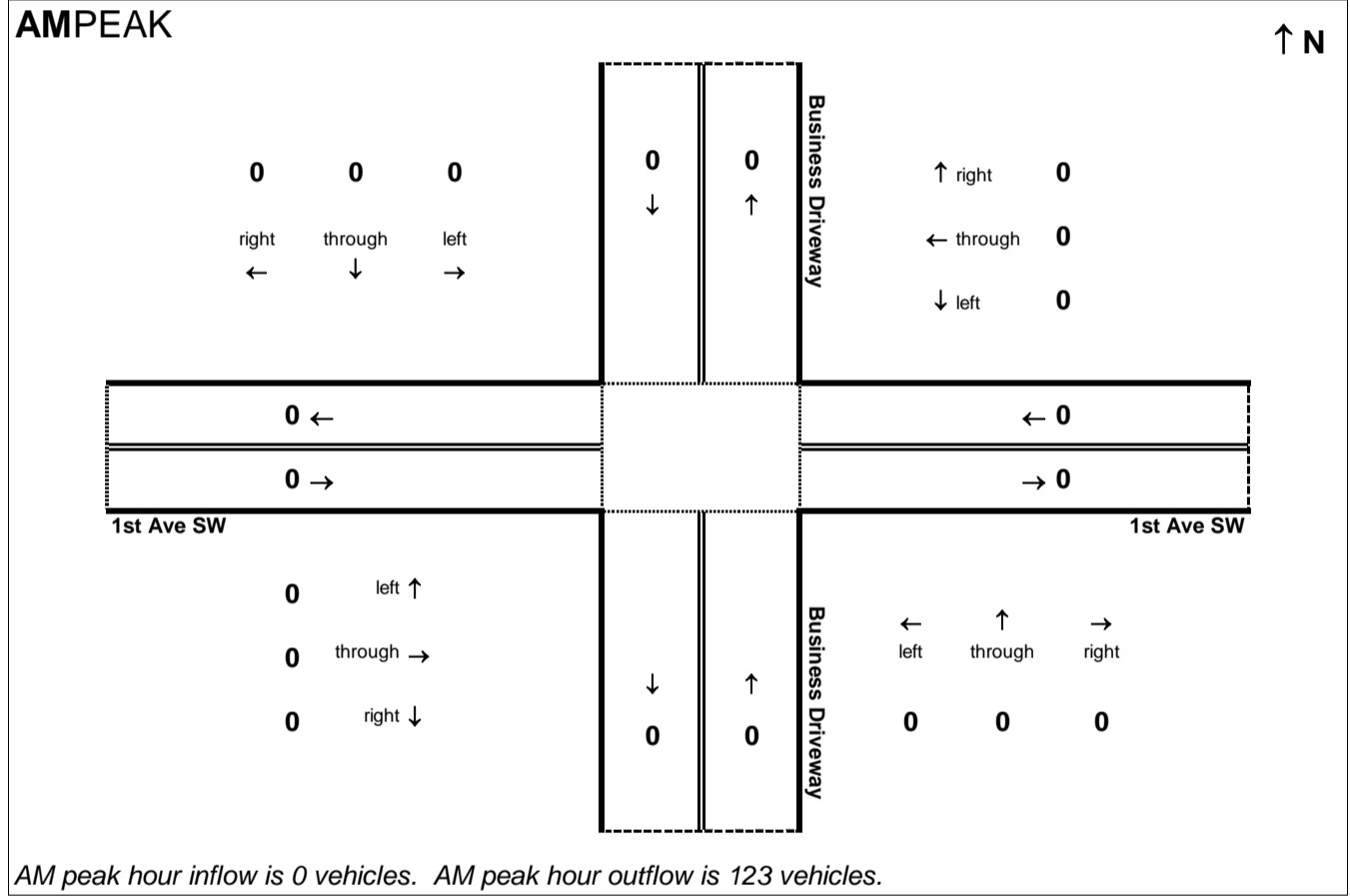
Peak Hour Volume Breakouts Report:
Intersection of 1st Ave SW and Business Driveway

Traffic Forecast Release Date:
December-16

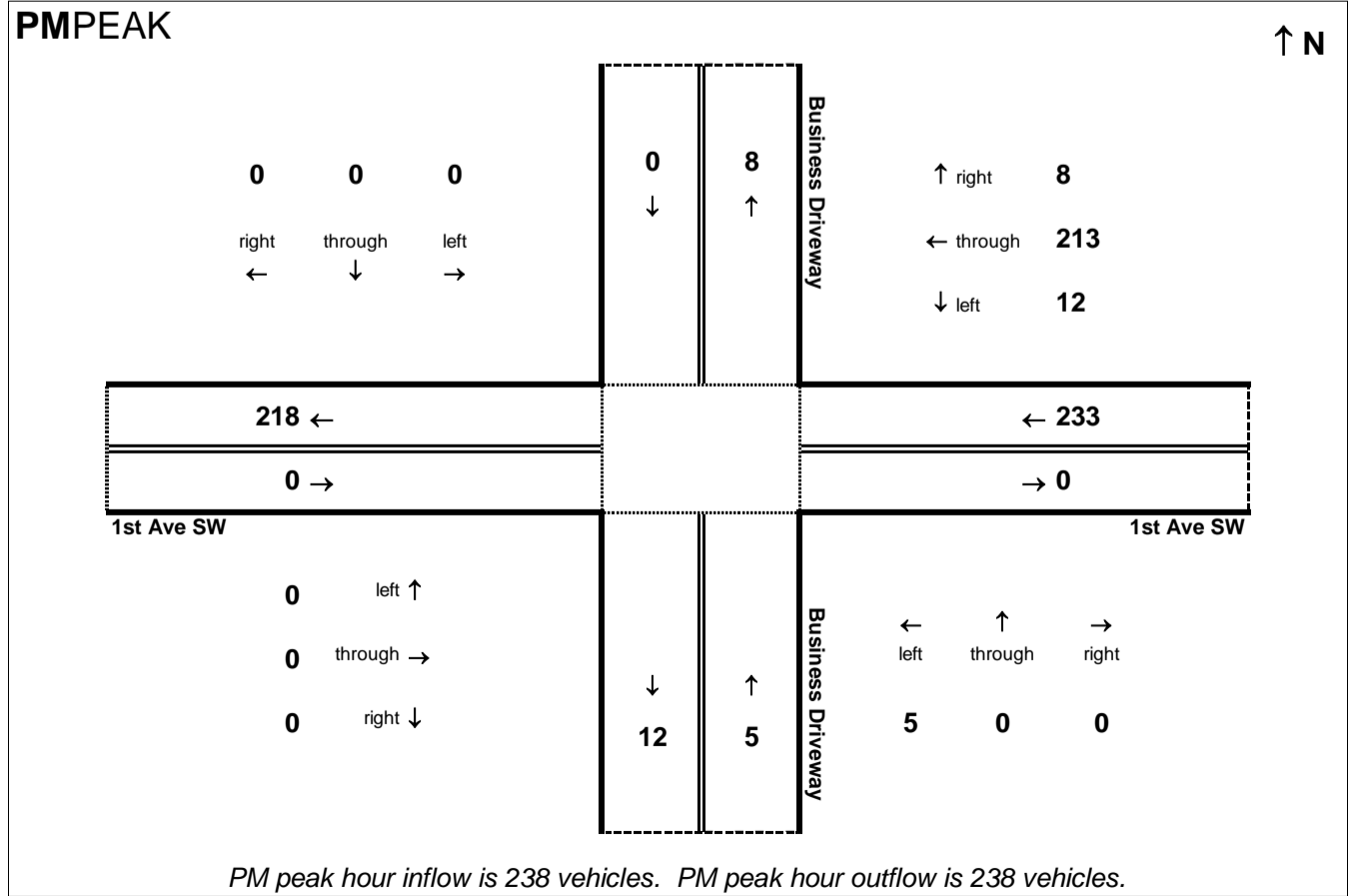
Traffic Data Year:
2040 Build 2

Project:
U-4700

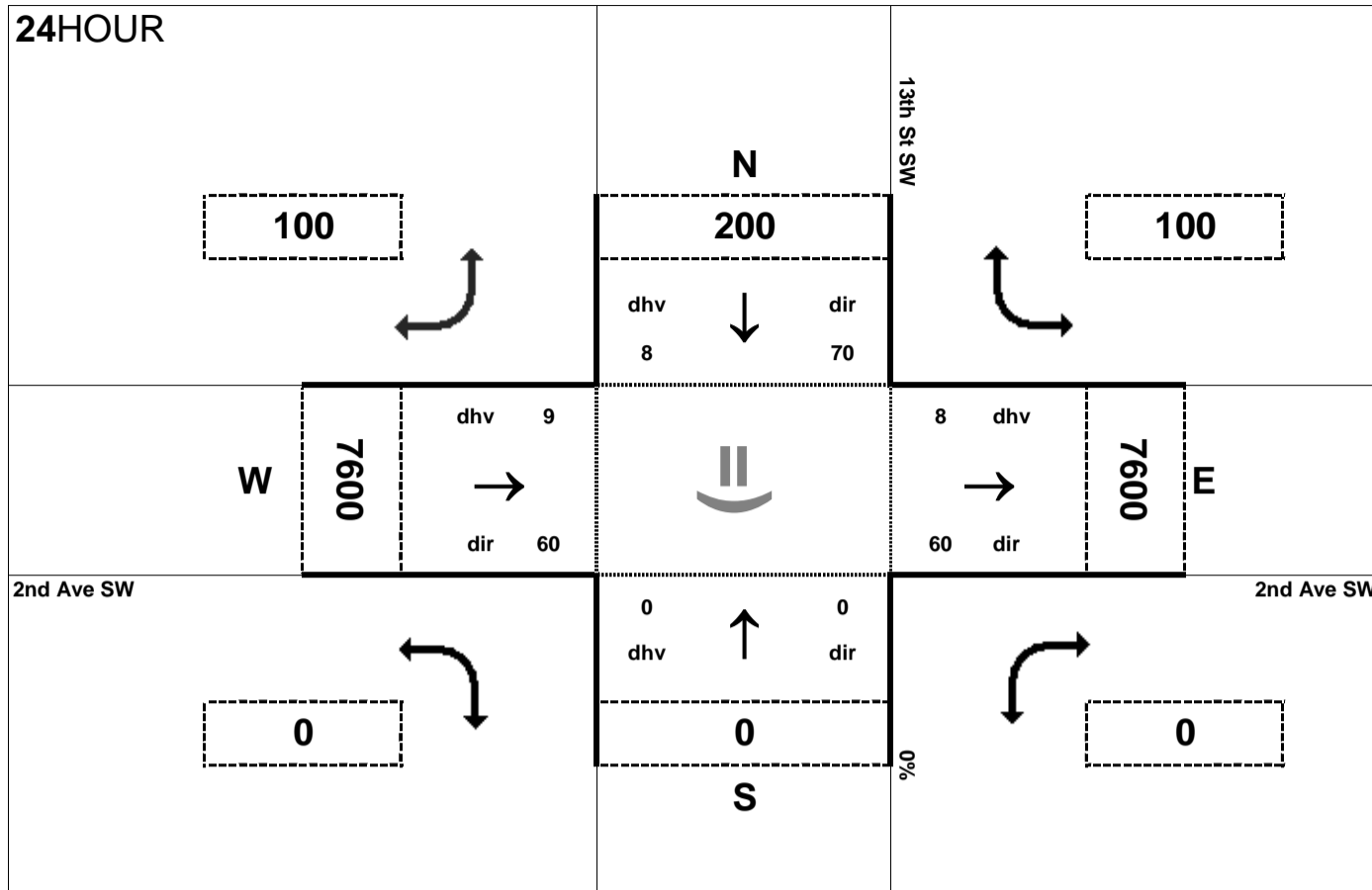
AMPEAK



PMPEAK



24HOUR



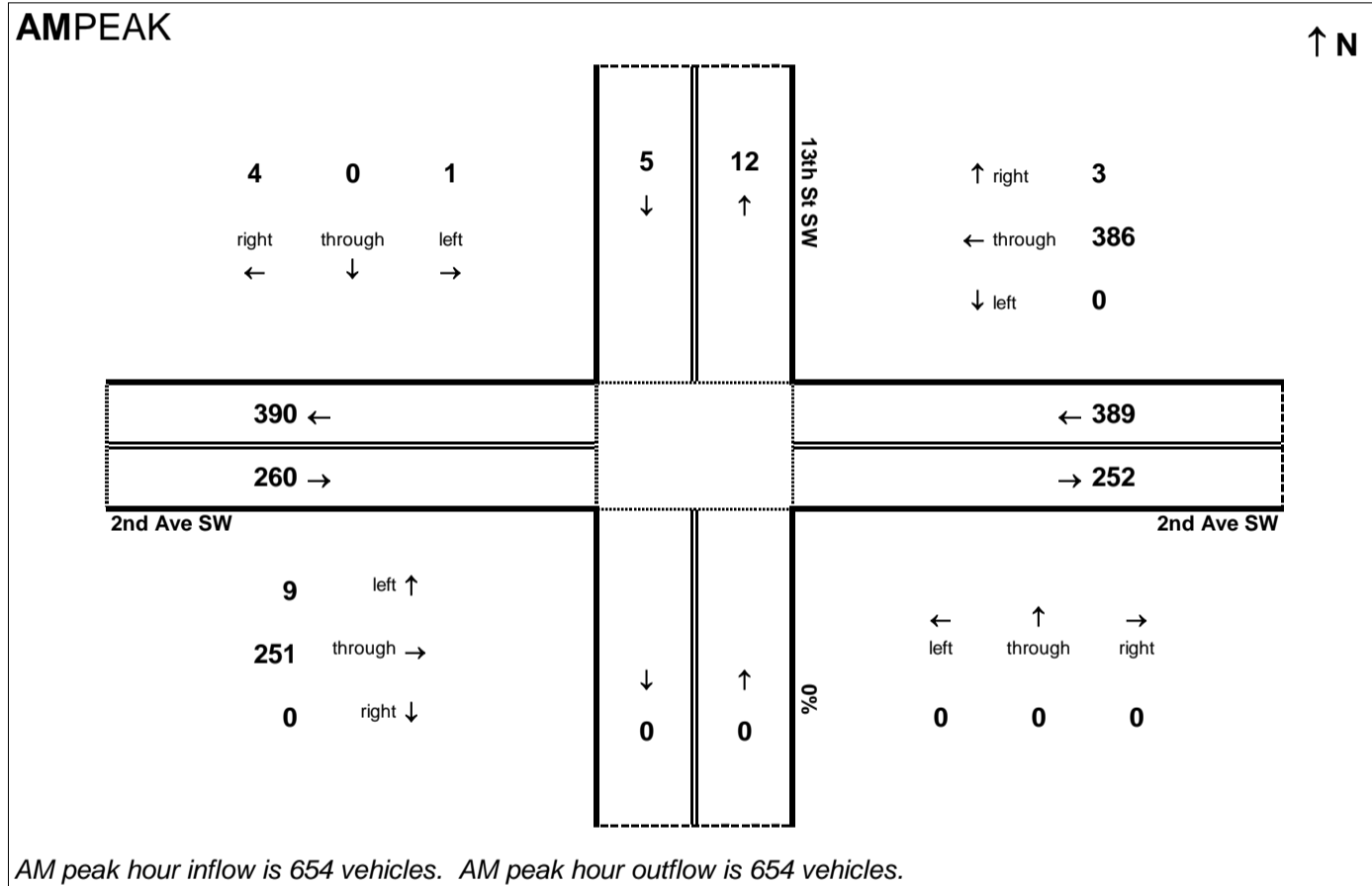
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 13th St SW

Traffic Forecast Release Date:
December-16

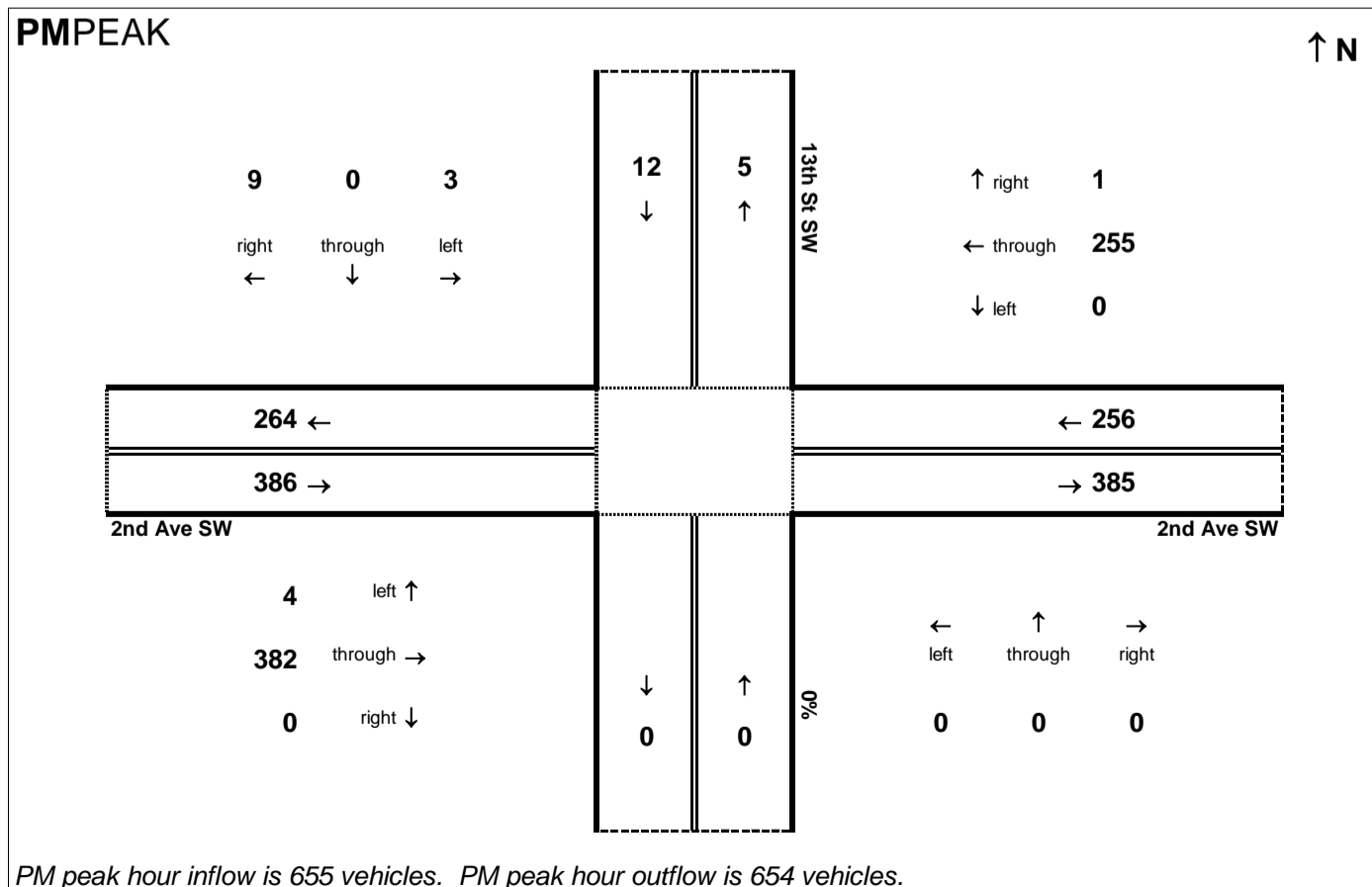
Traffic Data Year:
2040 Build 2

Project:
U-4700

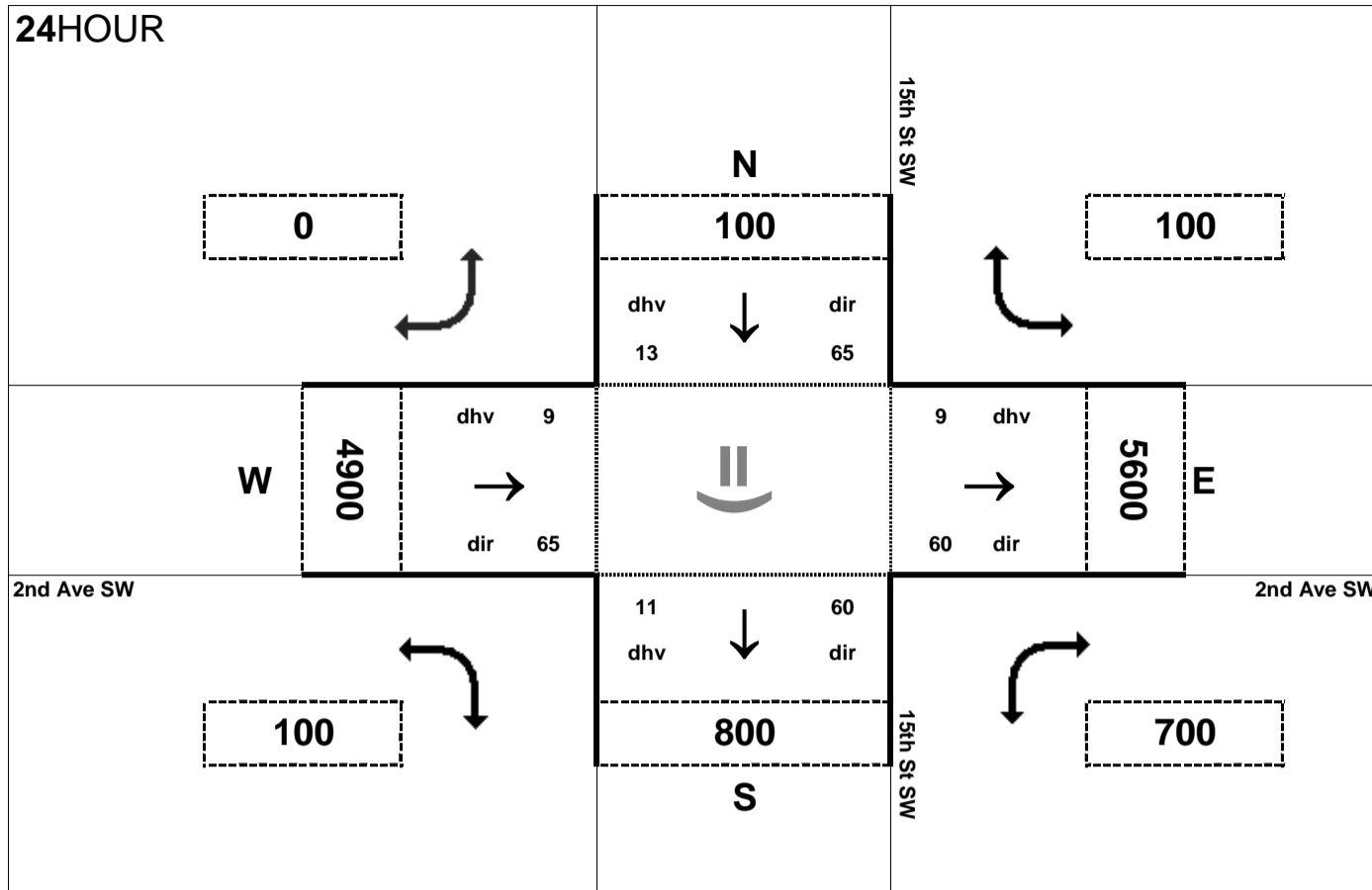
AMPEAK



PMPEAK



24HOUR



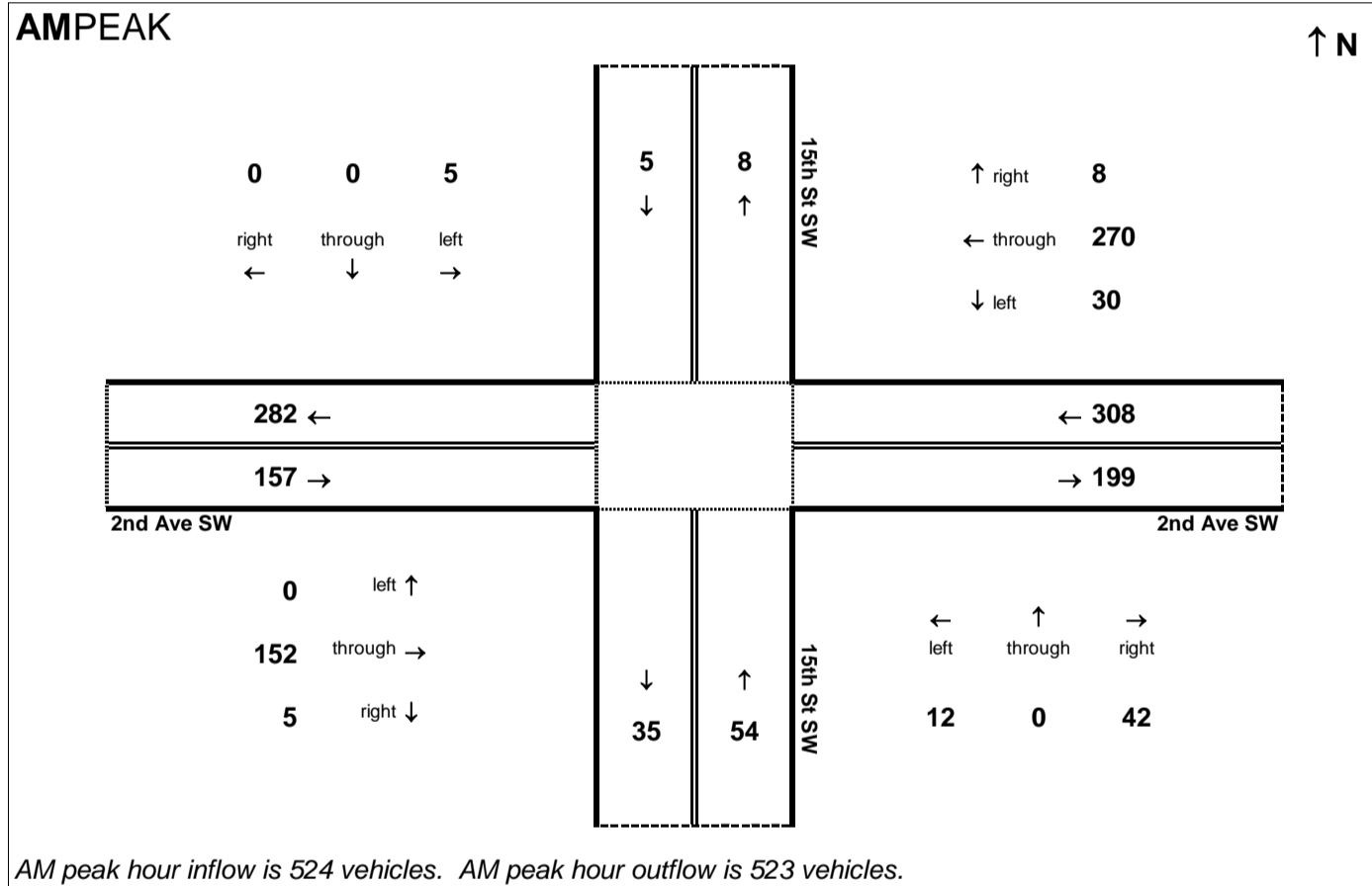
Peak Hour Volume Breakouts Report:
Intersection of 2nd Ave SW and 15th St SW

Traffic Forecast Release Date:
December-16

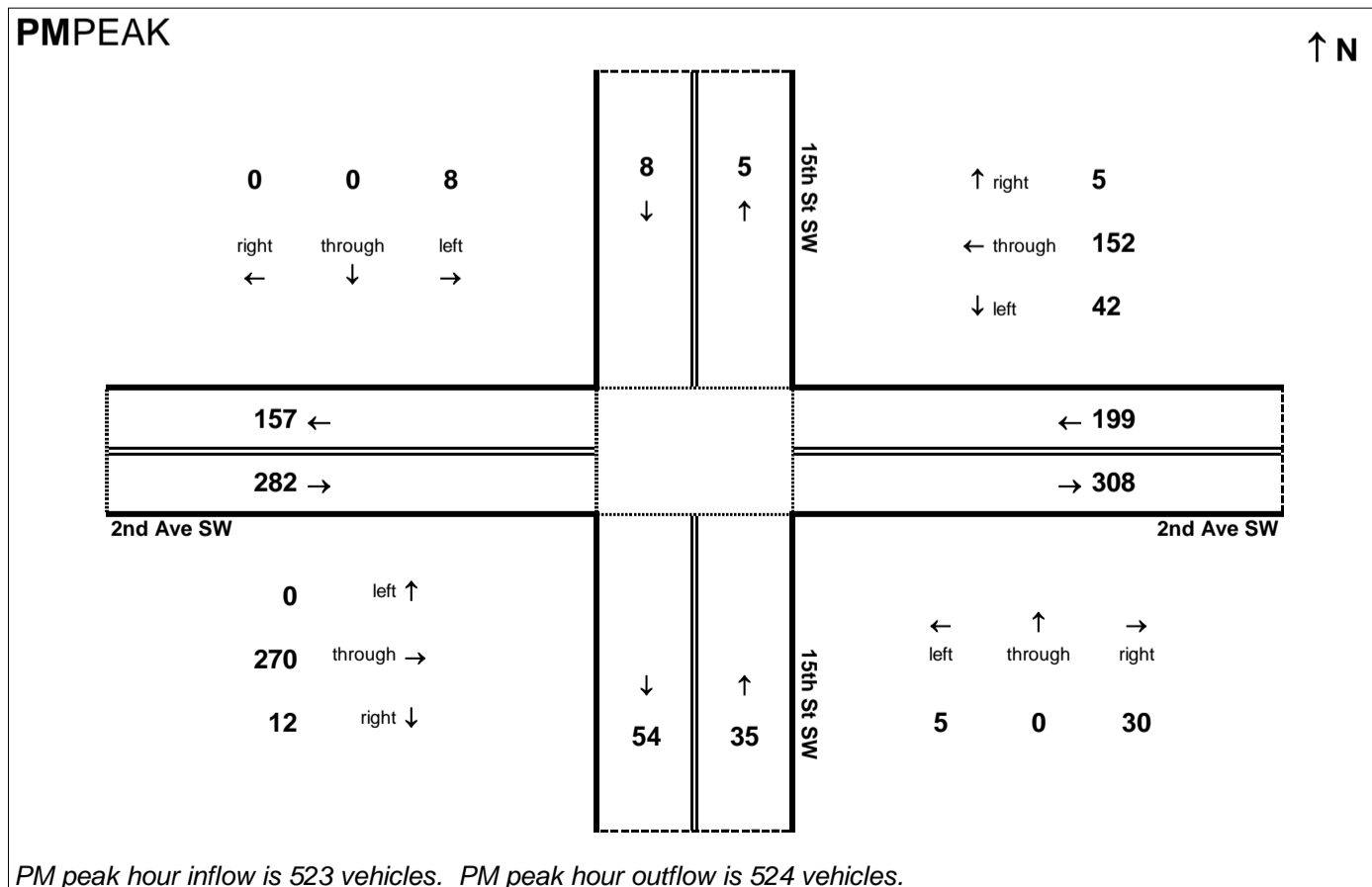
Traffic Data Year:
2040 Build 2

Project:
U-4700

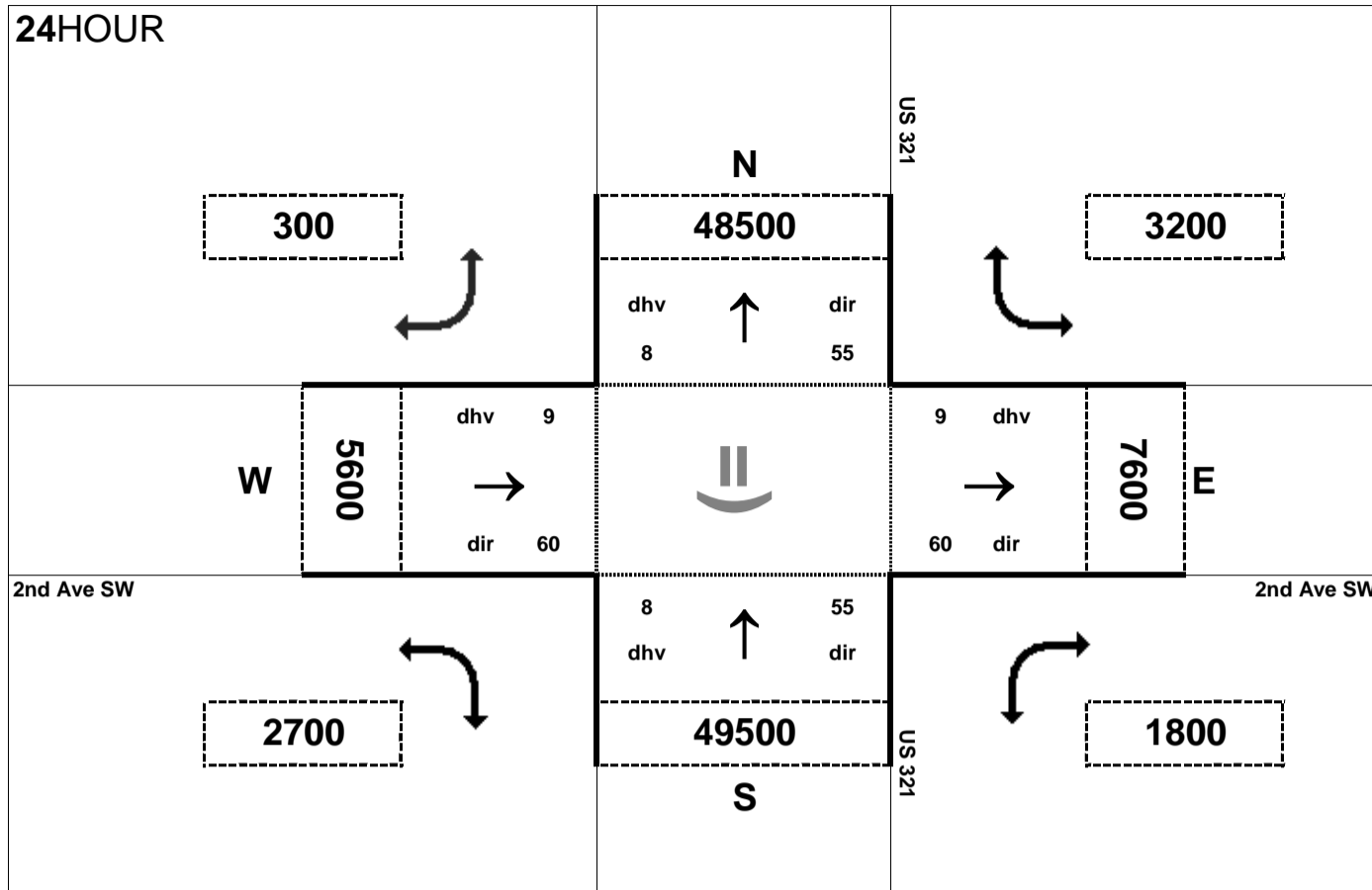
AMPEAK



PMPEAK



24HOUR



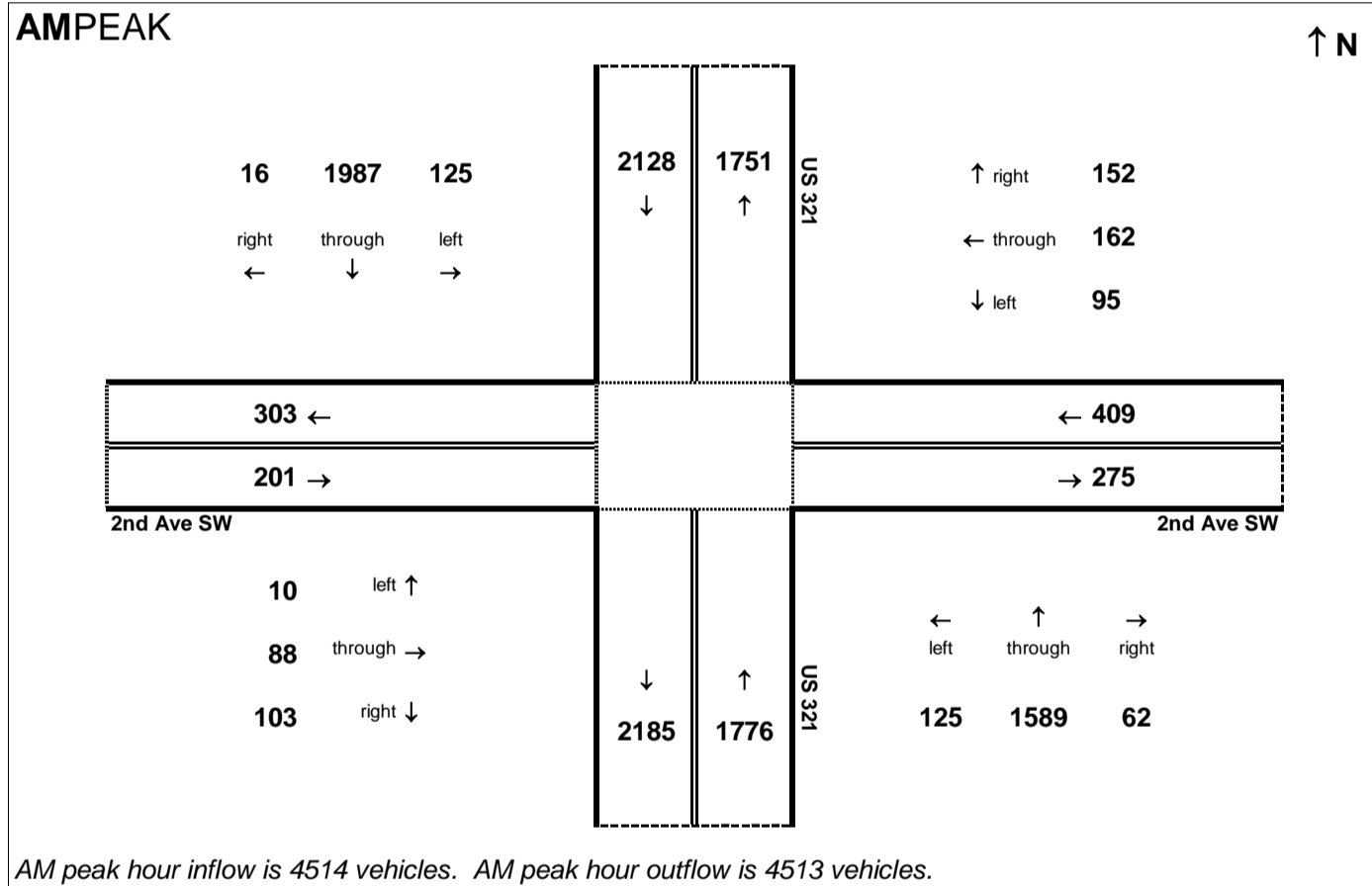
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 2nd Ave SW

Traffic Forecast Release Date:
December-16

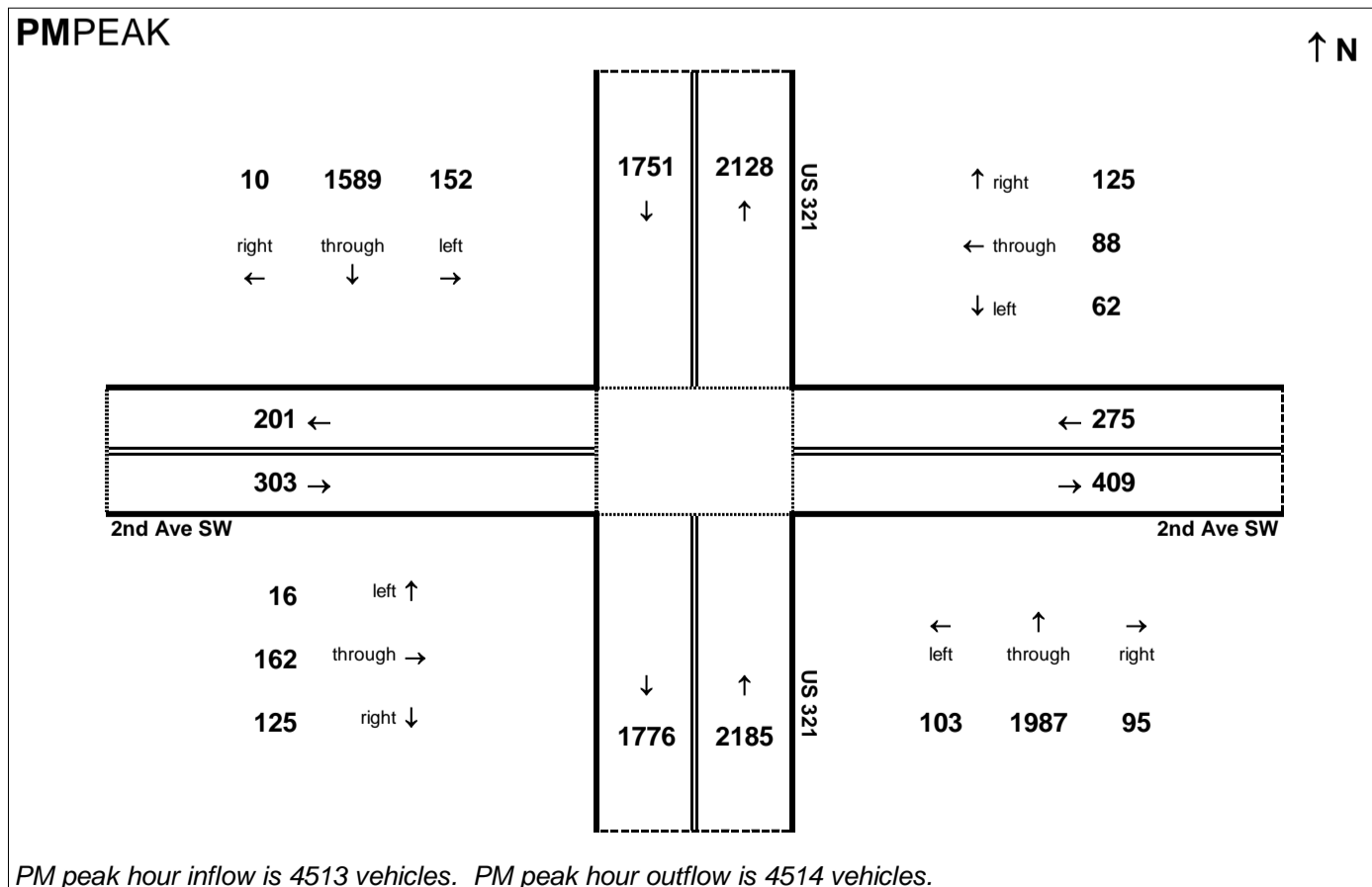
Traffic Data Year:
2040 Build 2

Project:
U-4700

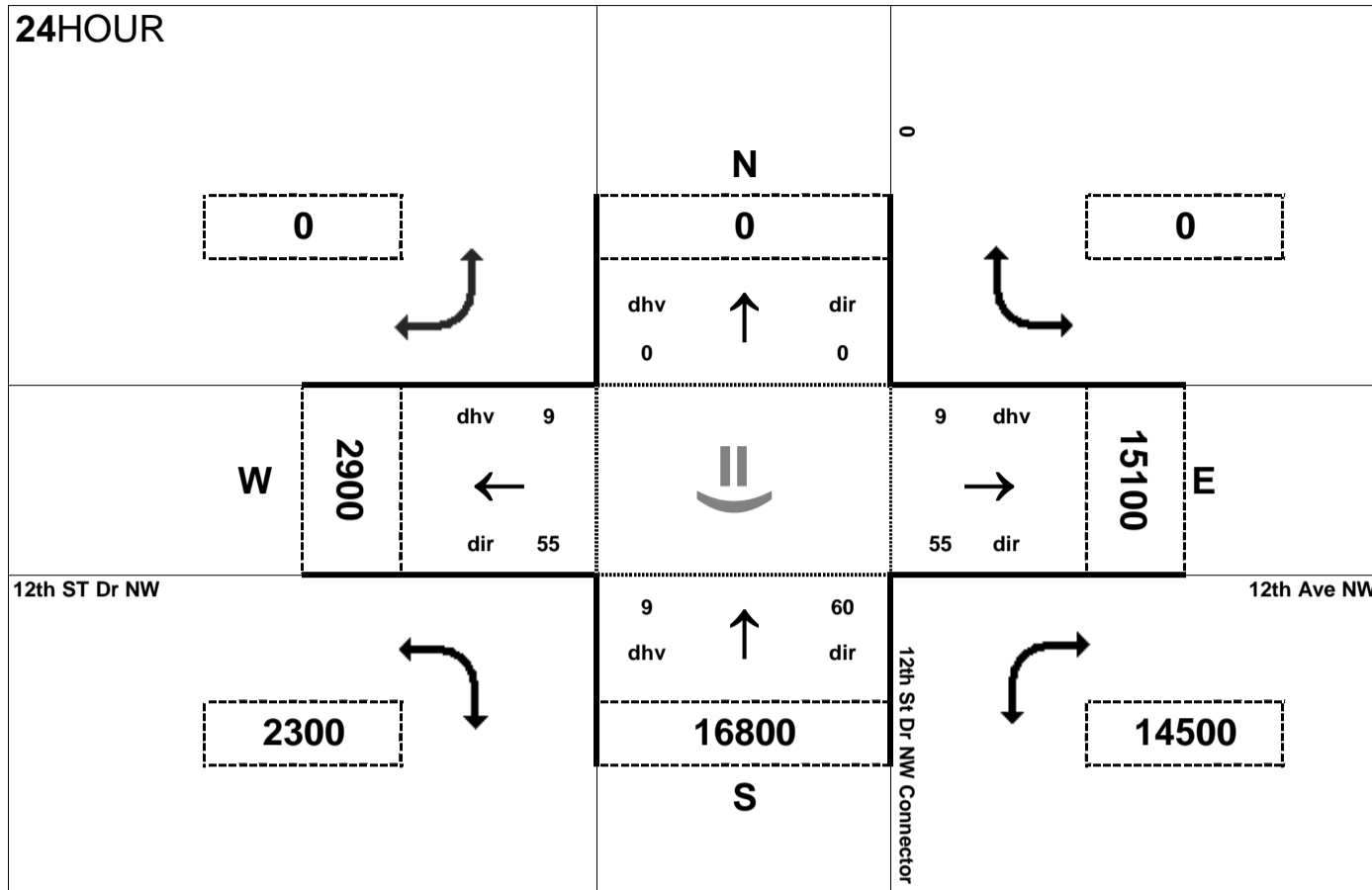
AMPEAK



PMPEAK



24HOUR



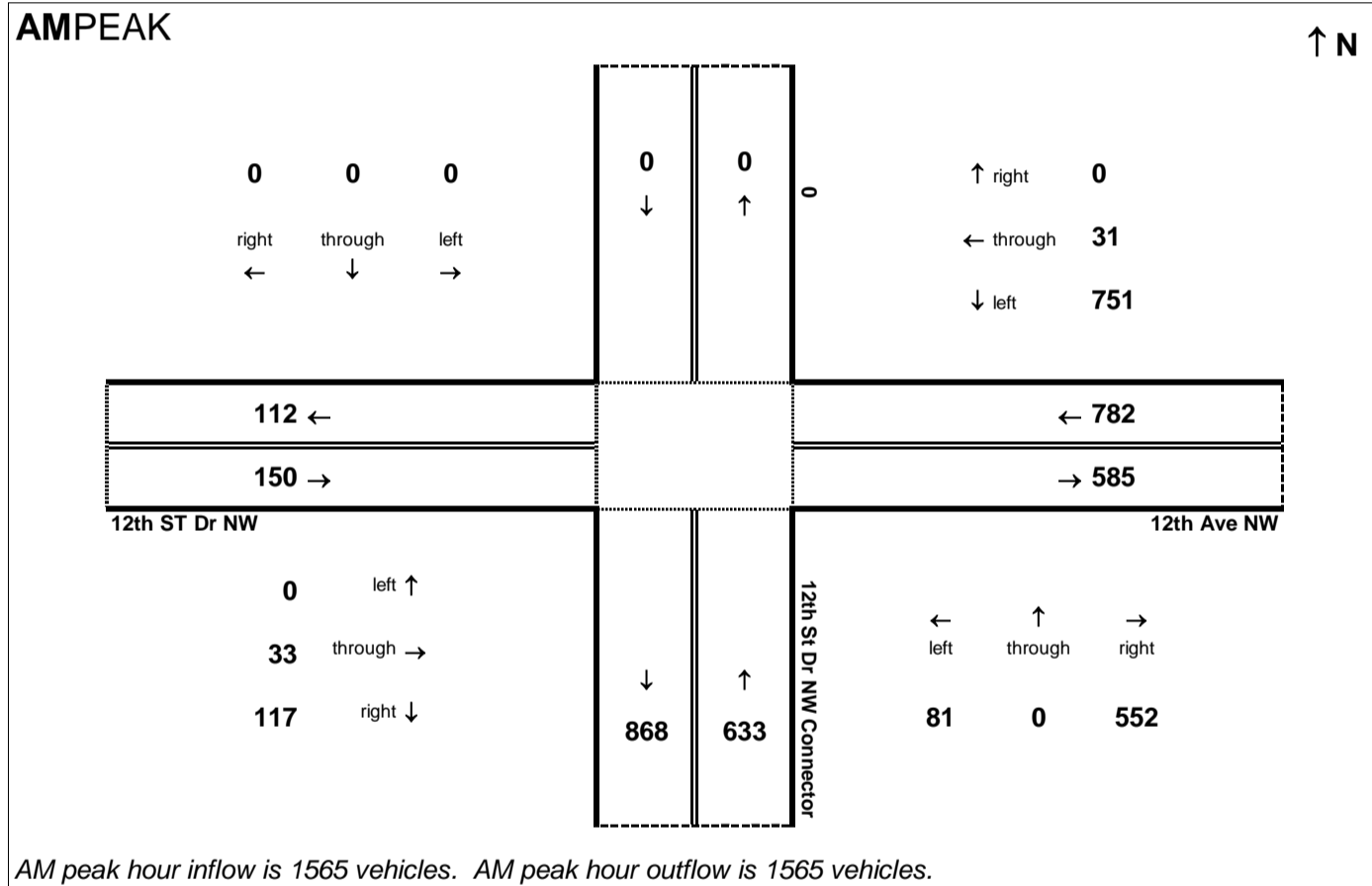
Peak Hour Volume Breakouts Report:
Intersection of 12th Ave NW at 12th St Dr NW

Traffic Forecast Release Date:
December-16

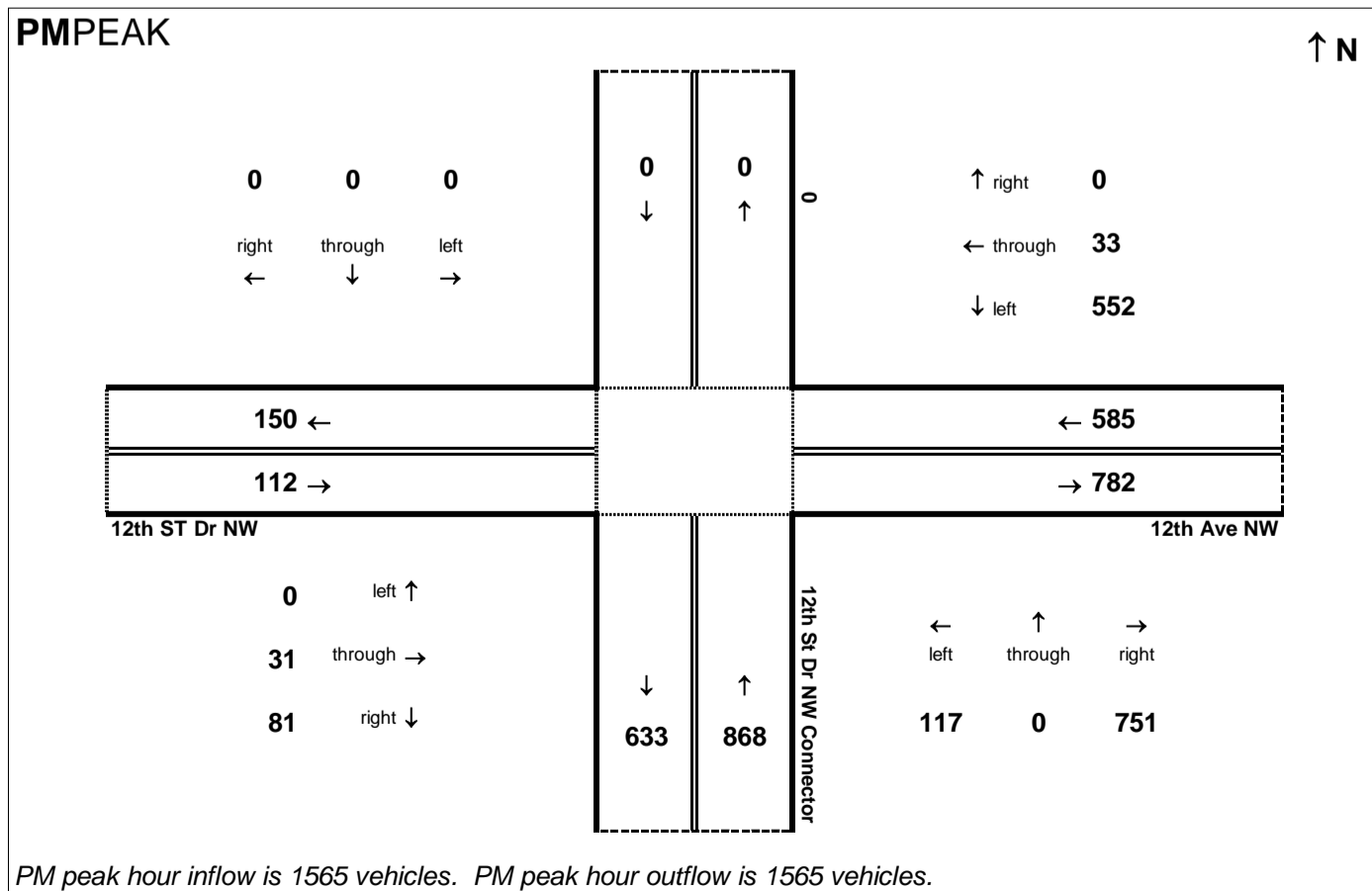
Traffic Data Year:
2040 Build 2

Project:
U-4700

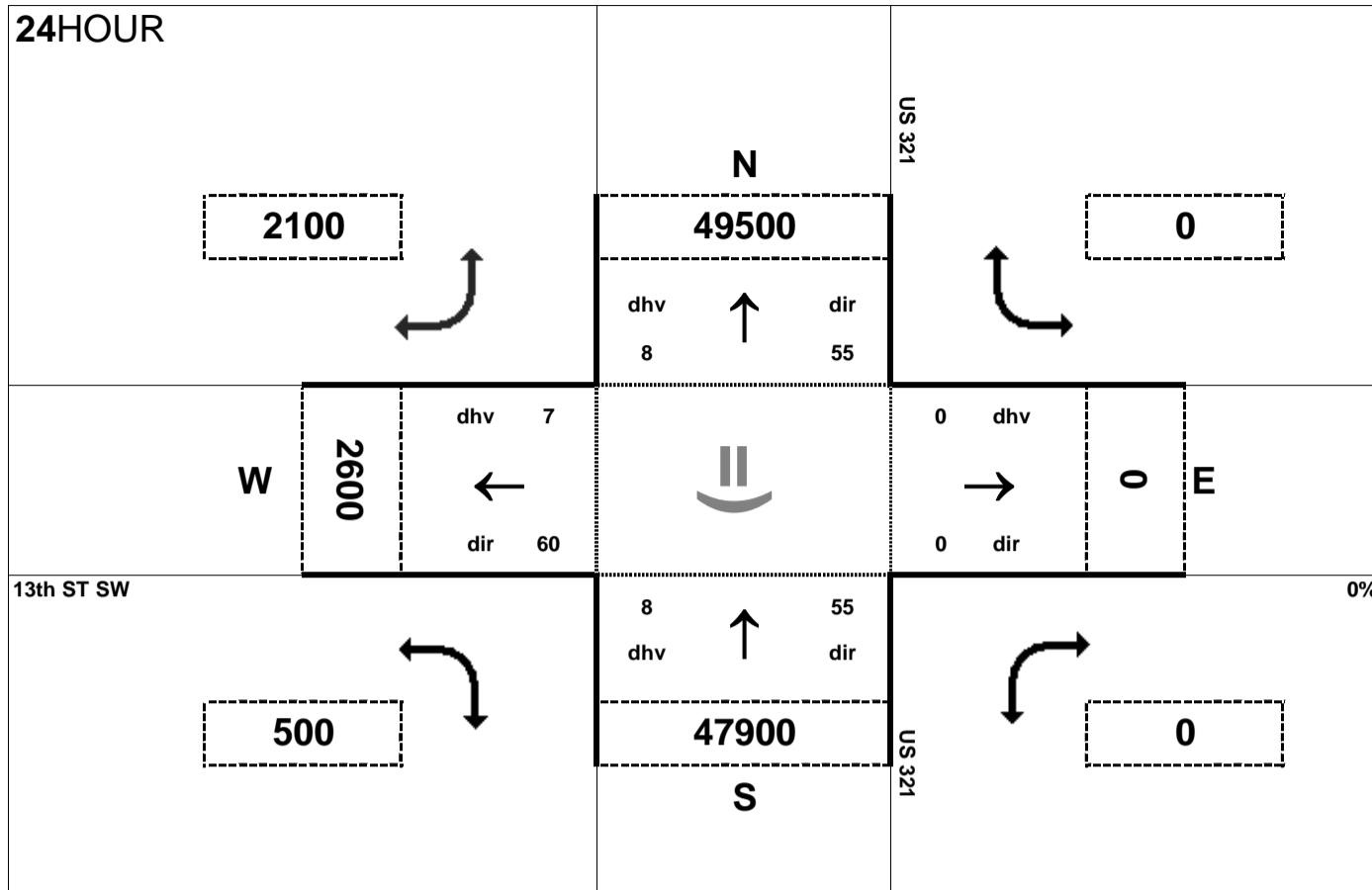
AMPEAK



PMPEAK



24HOUR



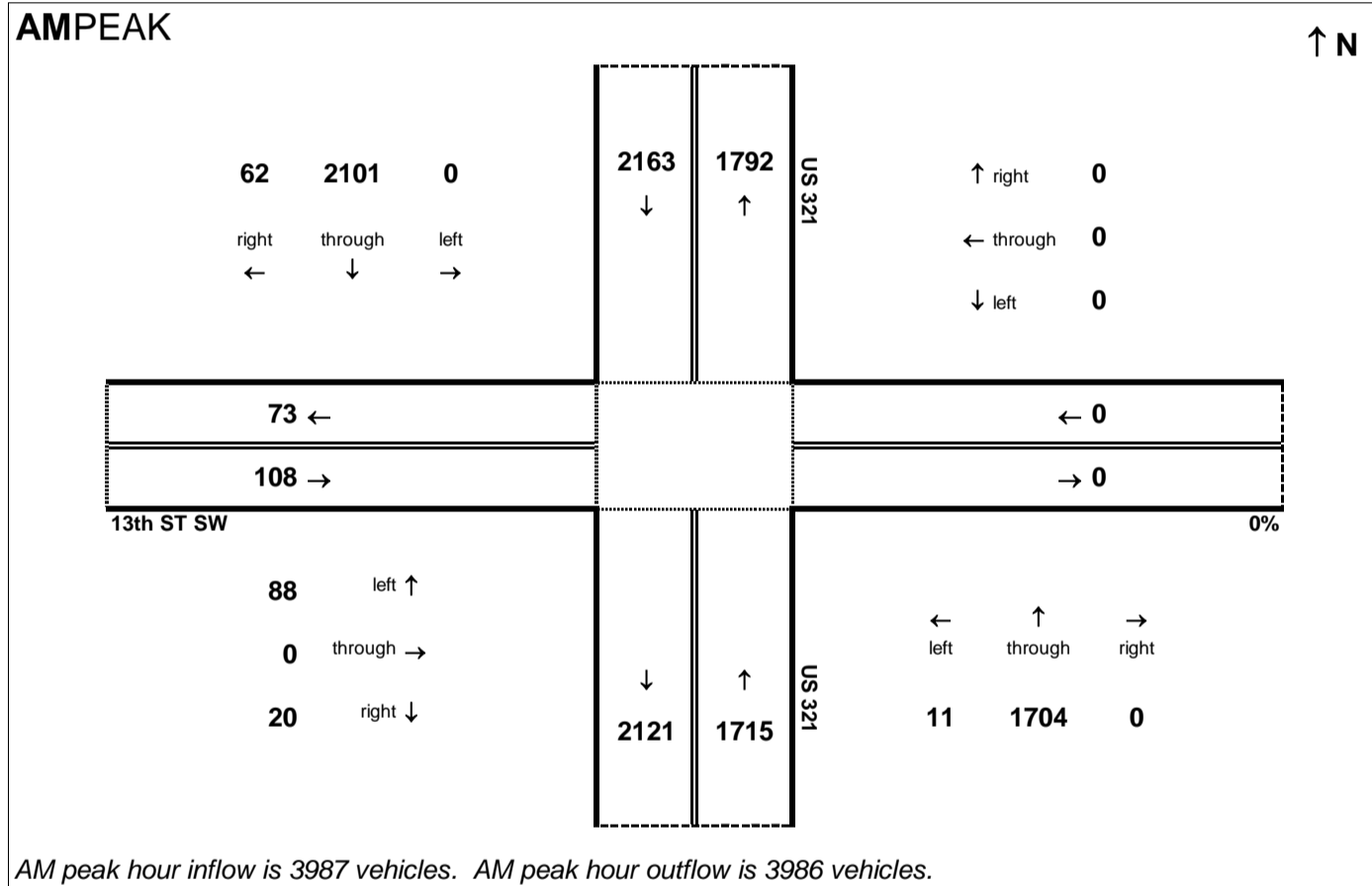
Peak Hour Volume Breakouts Report:
Intersection of US 321 and 13th St SW

Traffic Forecast Release Date:
December-16

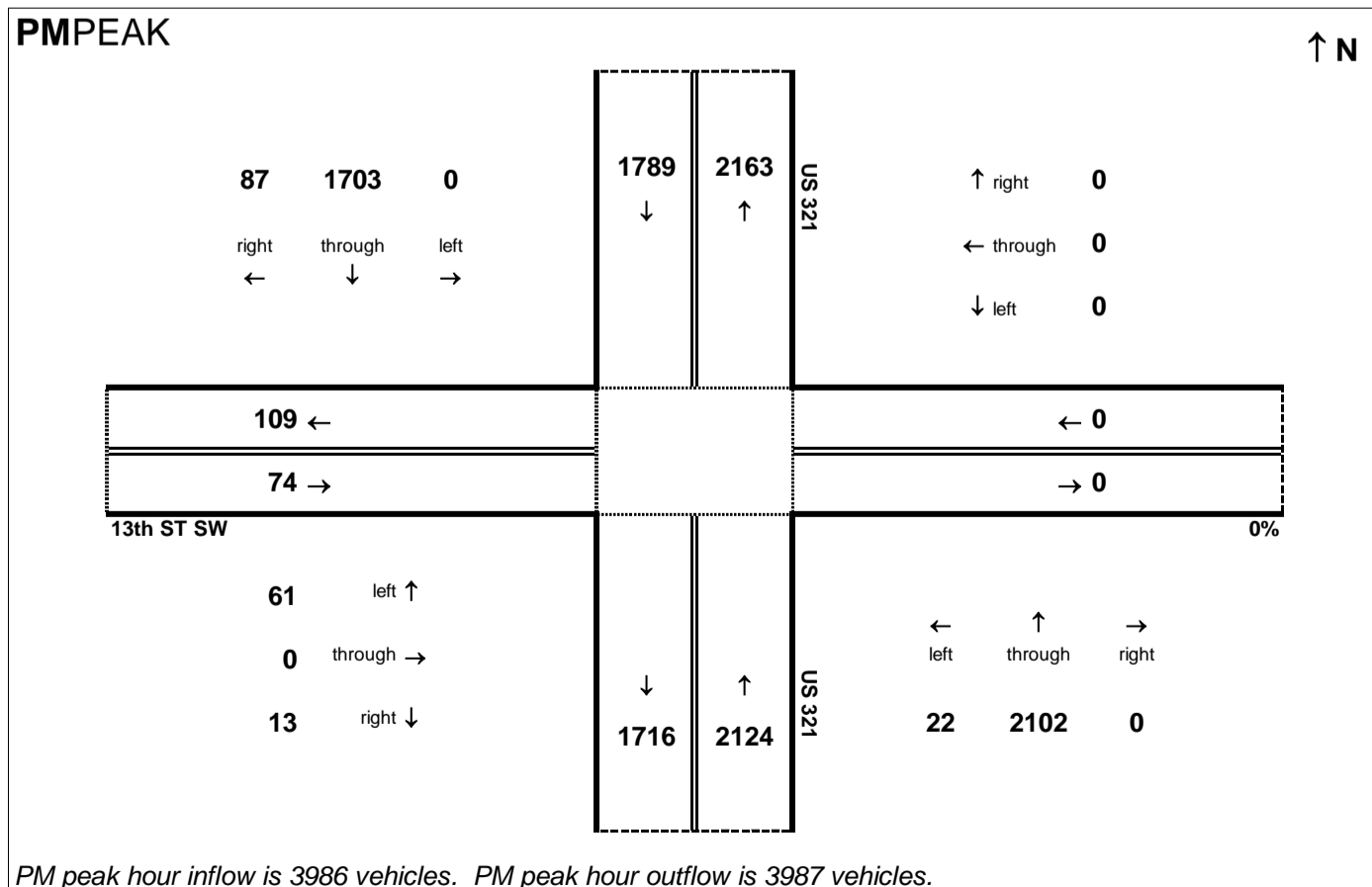
Traffic Data Year:
2040 Build 2

Project:
U-4700

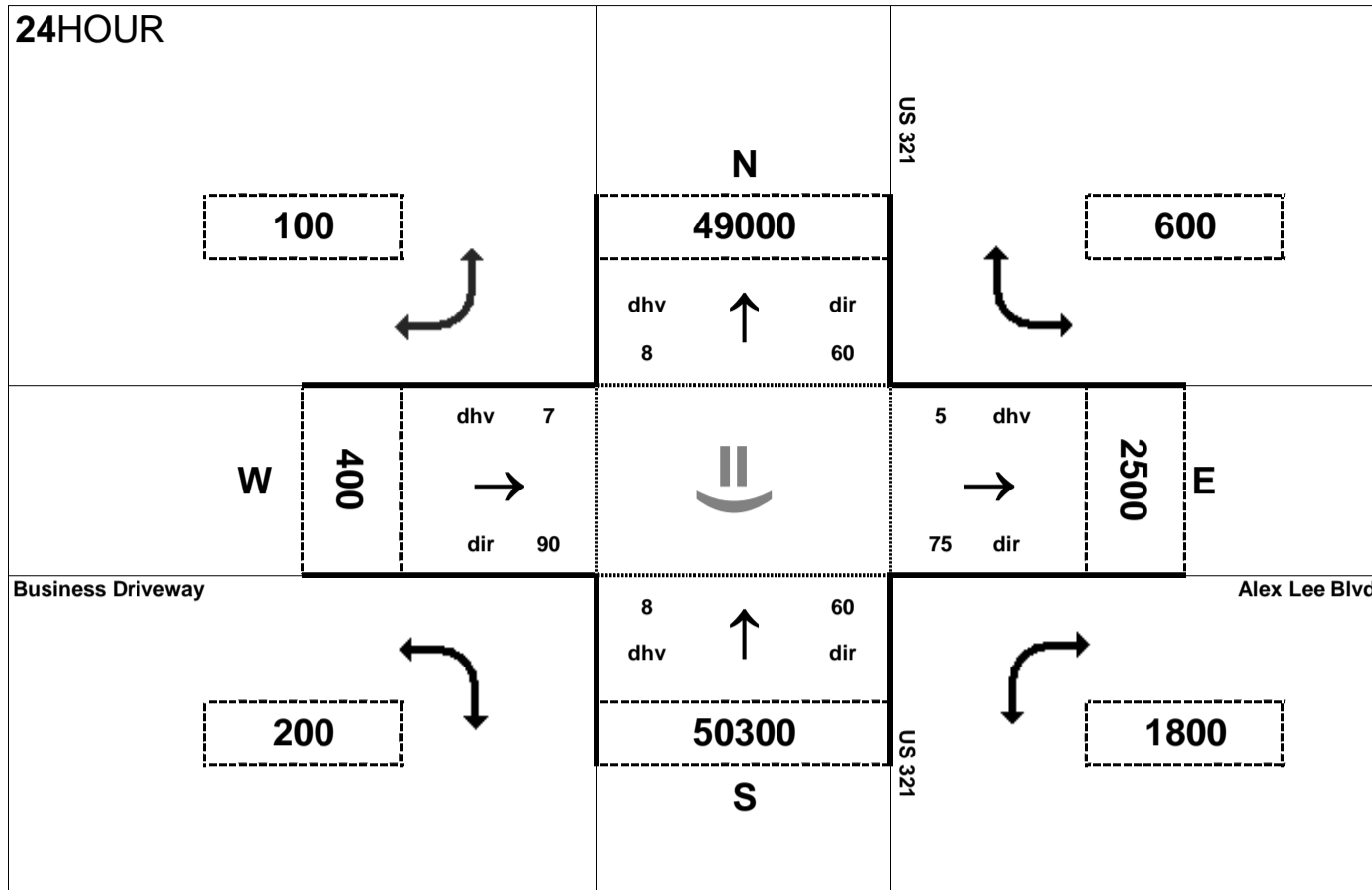
AMPEAK



PMPEAK



24HOUR



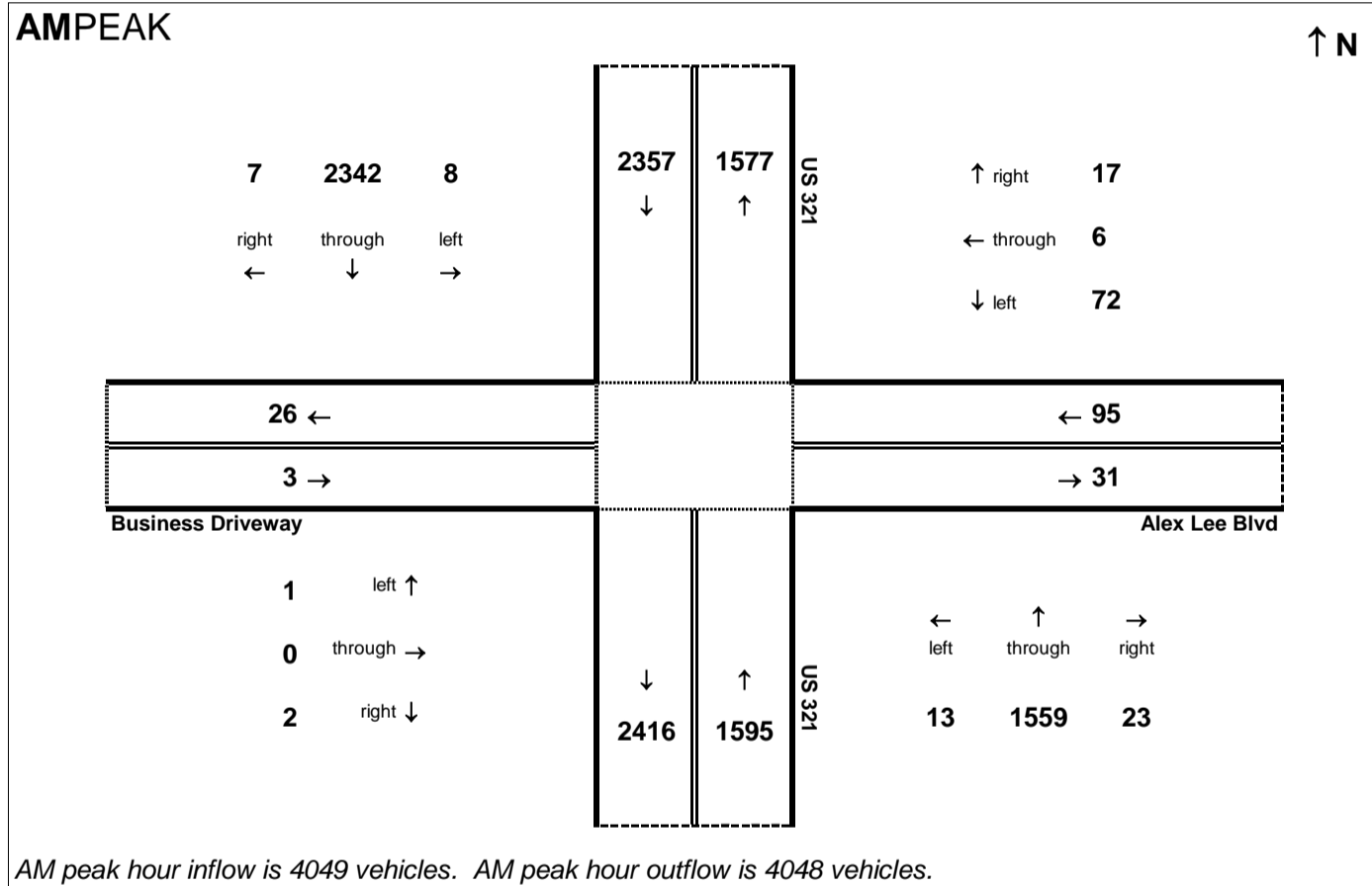
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Alex Lee Blvd

Traffic Forecast Release Date:
December-16

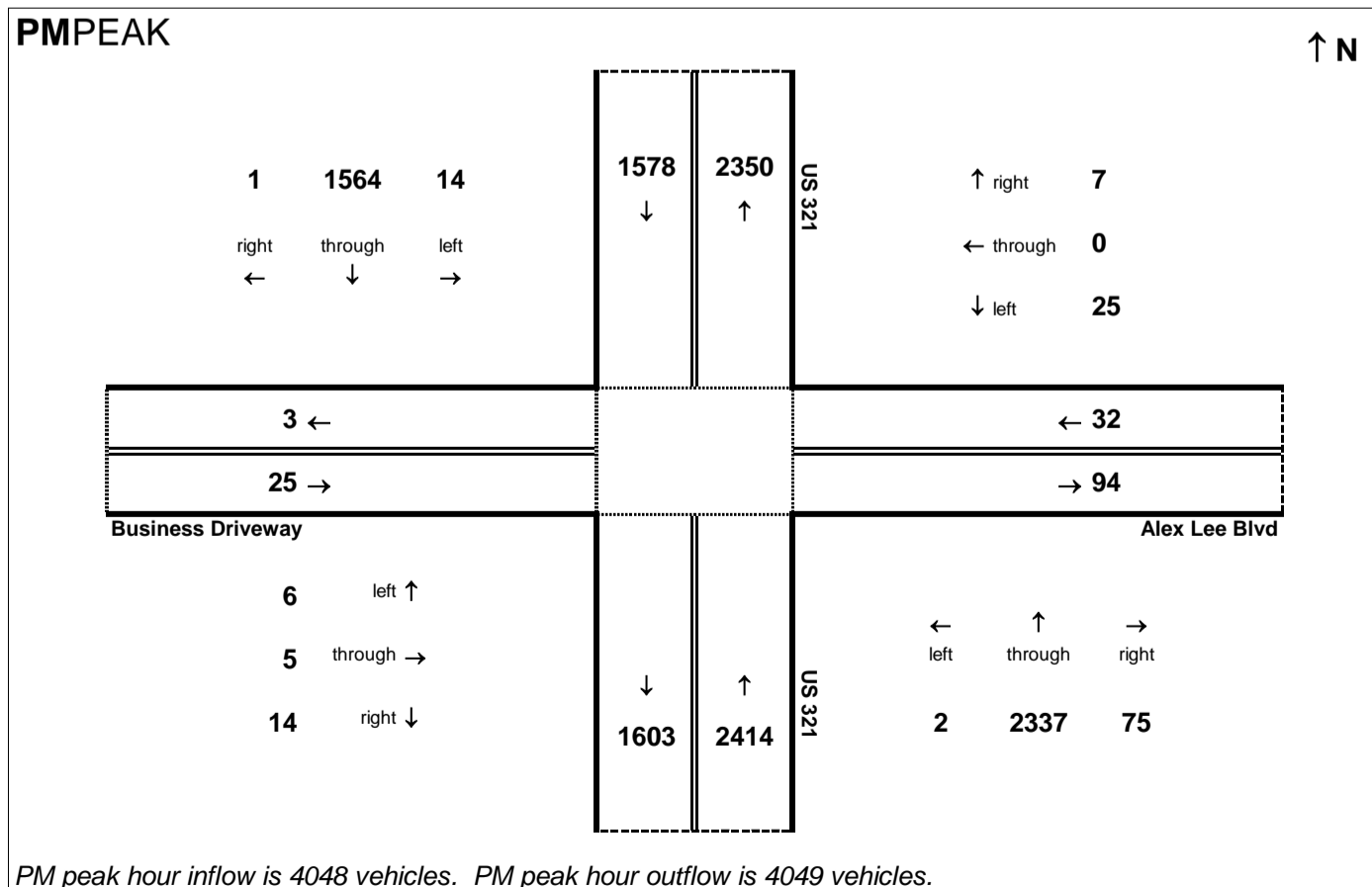
Traffic Data Year:
2040 Build 2

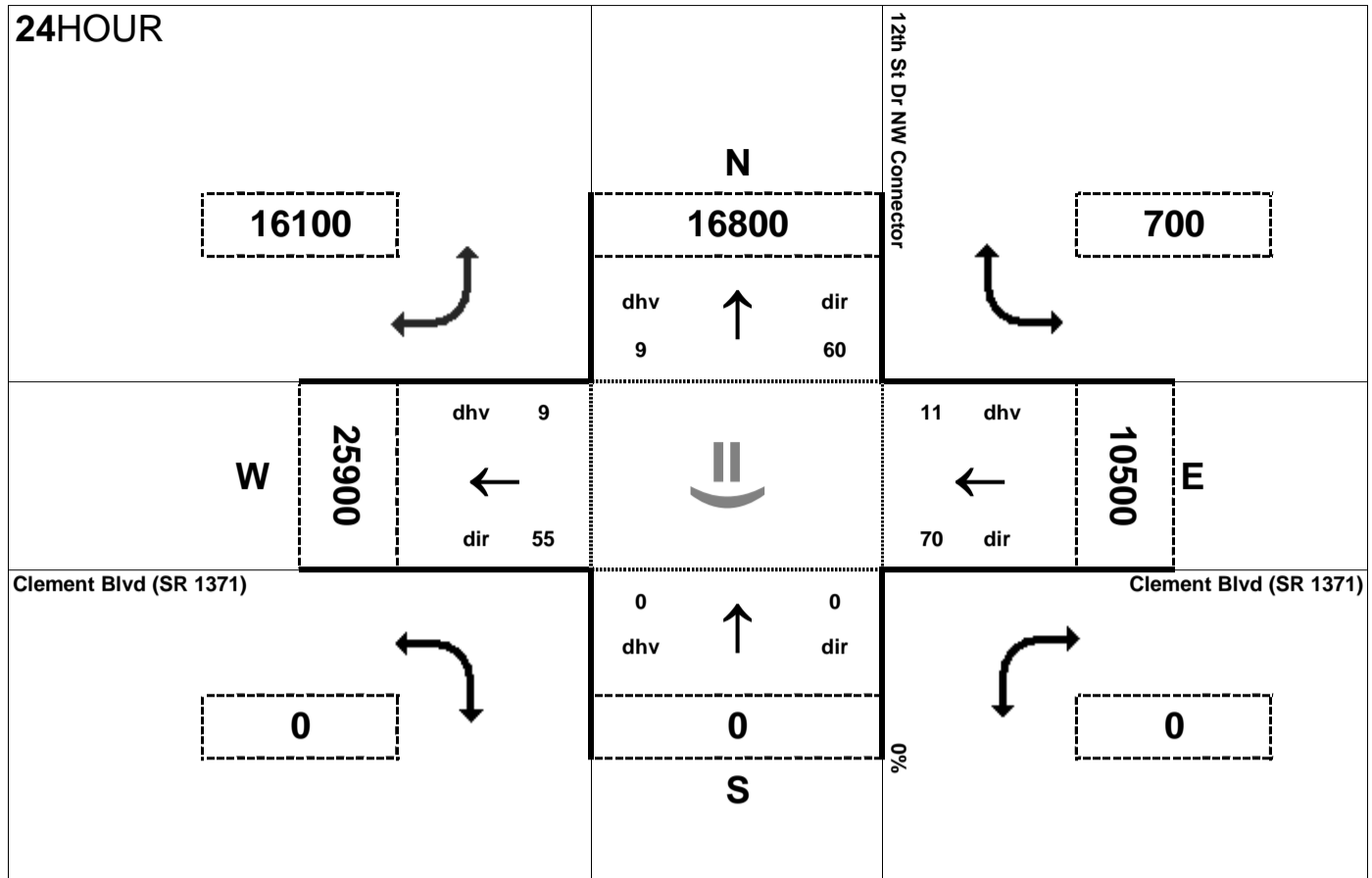
Project:
U-4700

AMPEAK



PMPEAK



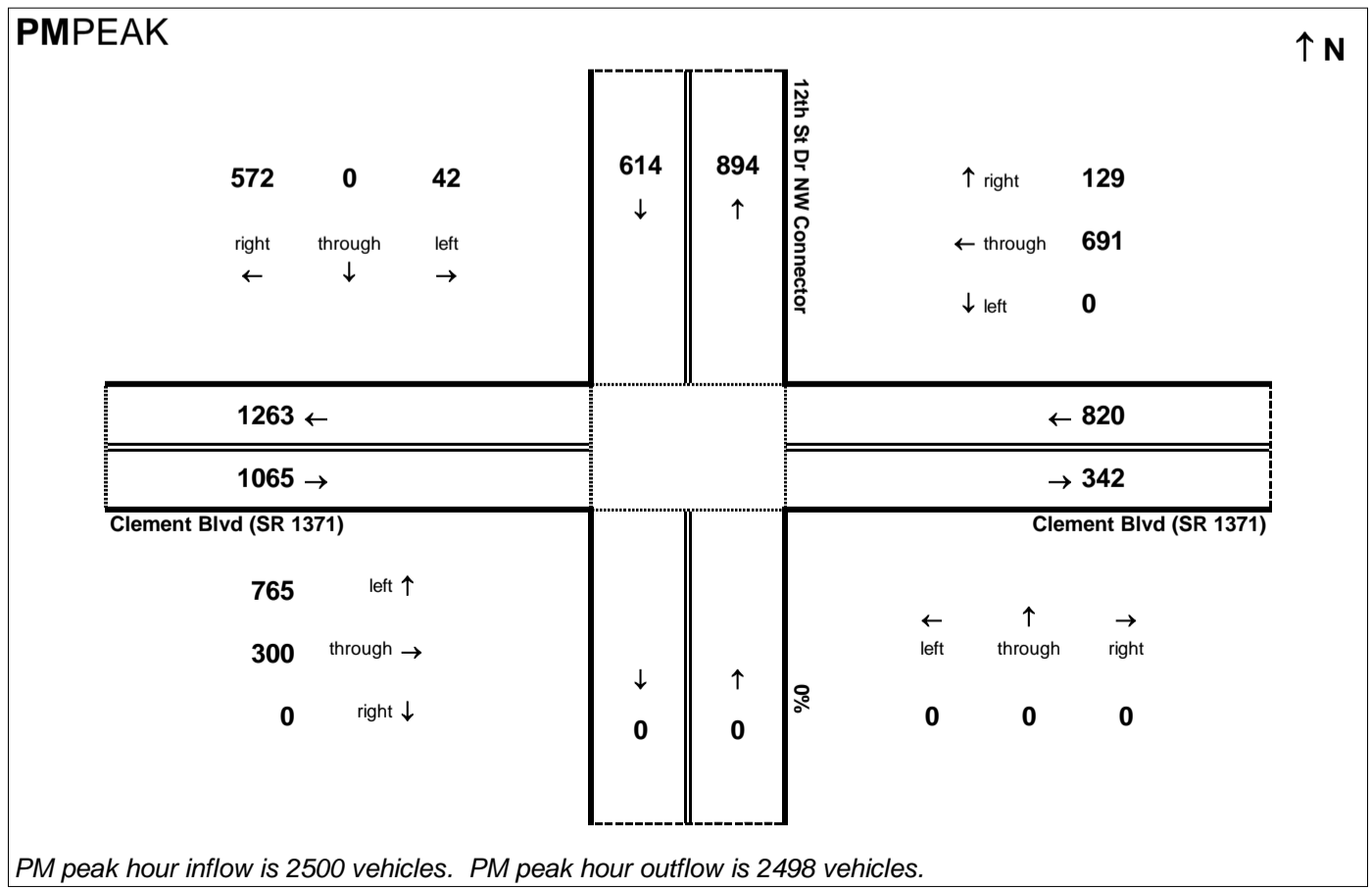
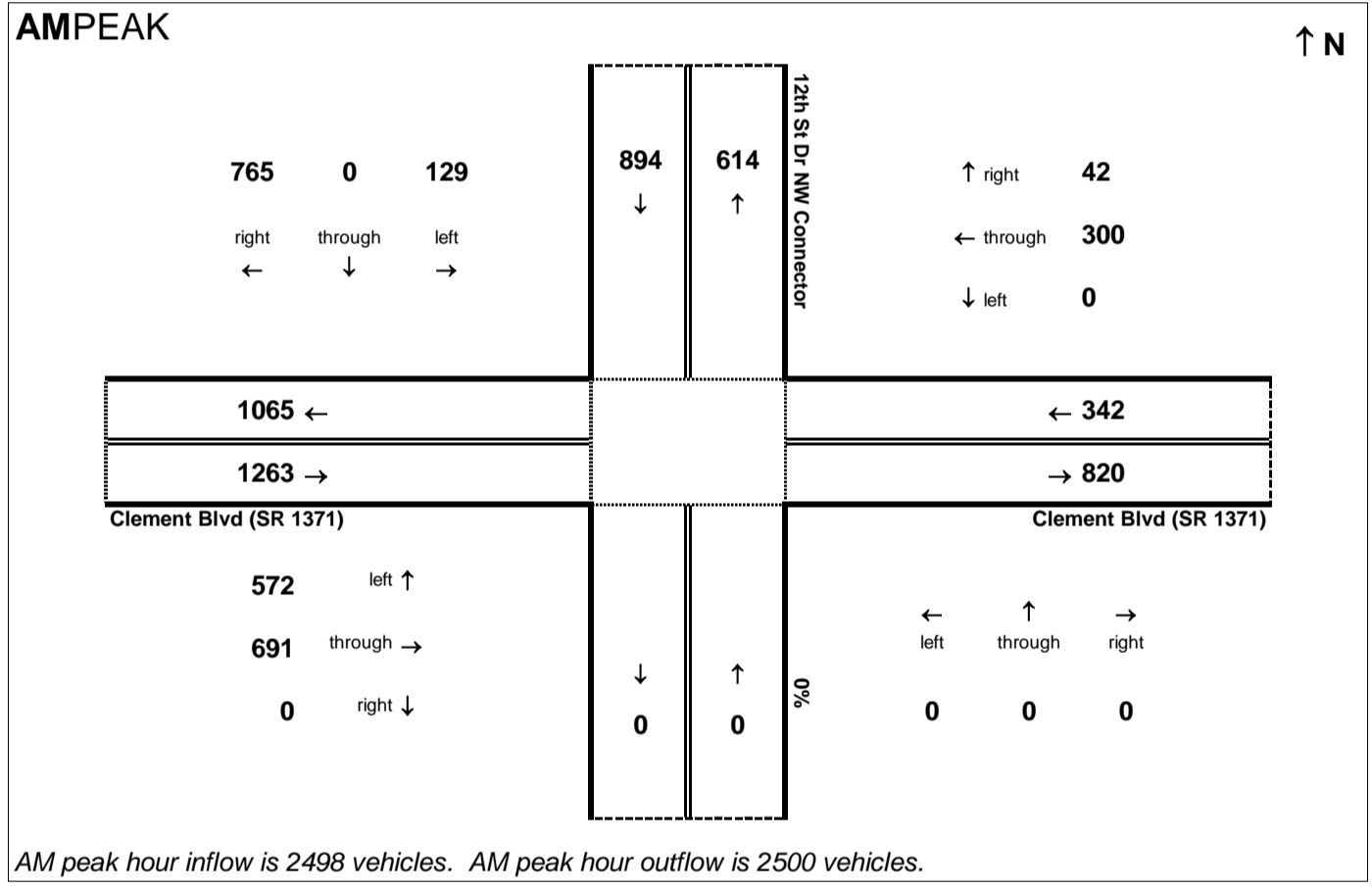


Peak Hour Volume Breakouts Report:
 Intersection of Clement Blvd at 12th St Dr NW Connector

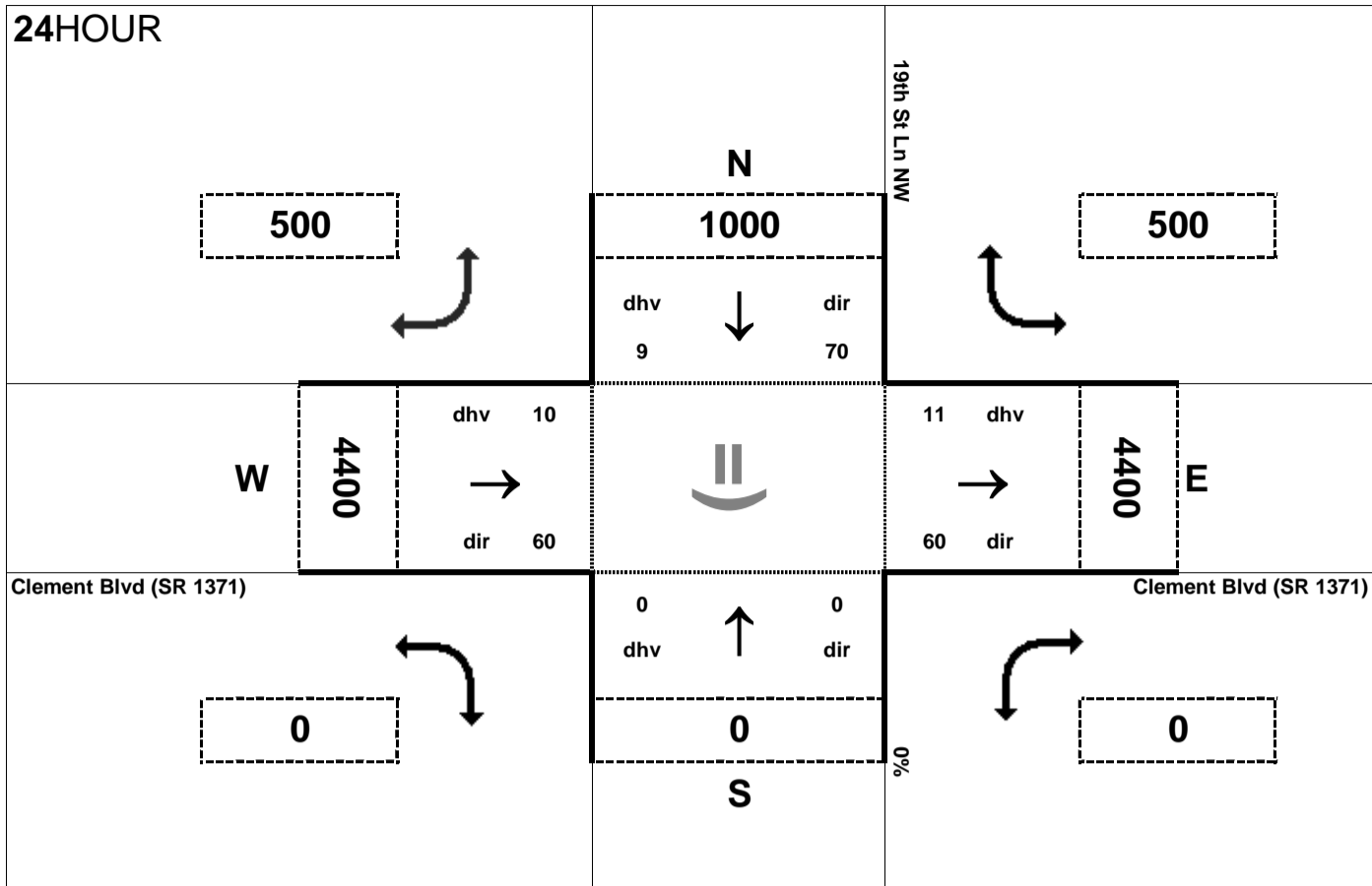
Traffic Forecast Release Date:
 December-16

Traffic Data Year:
 2040 Build 2

Project:
 U-4700



24HOUR



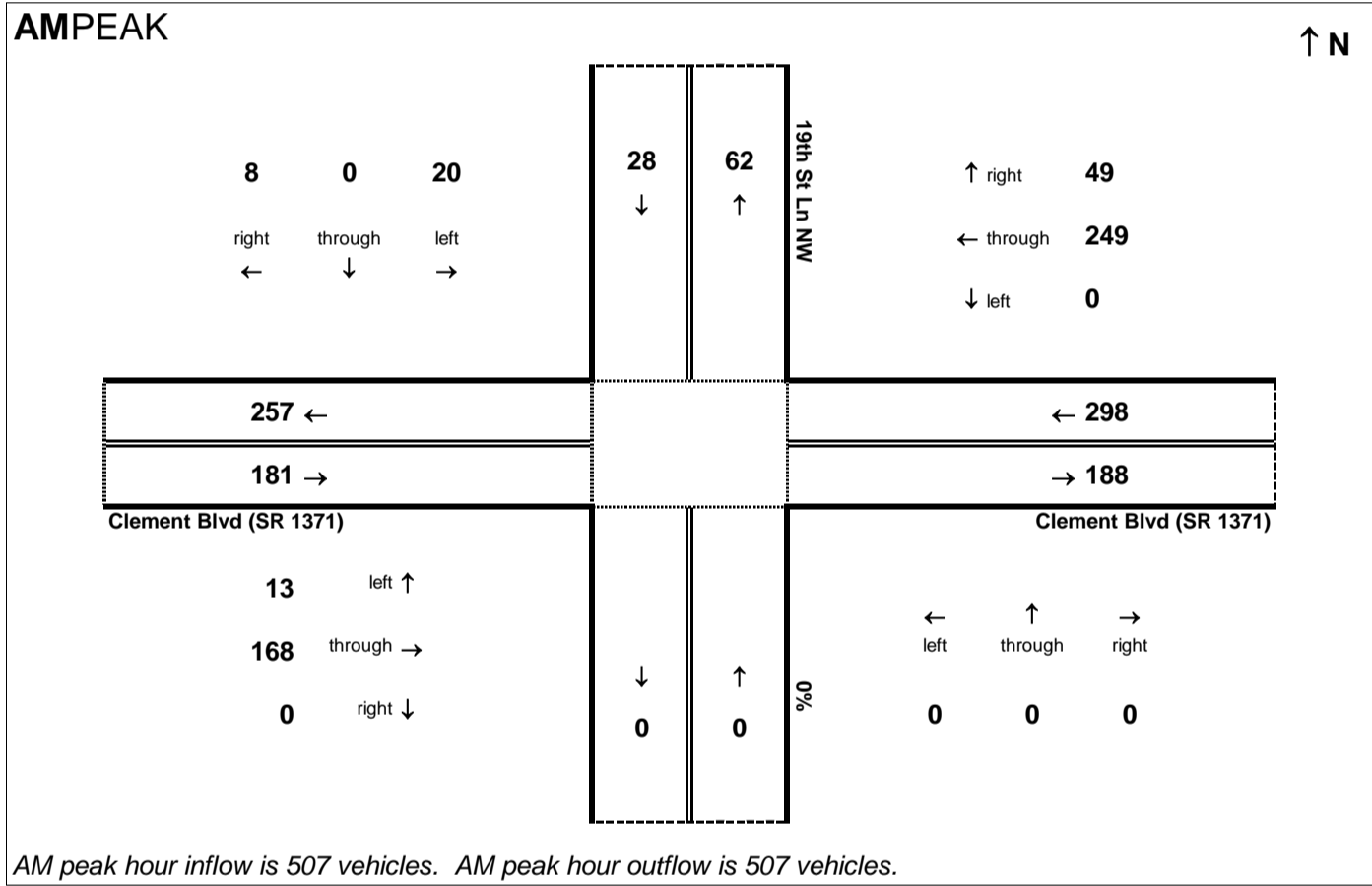
Peak Hour Volume Breakouts Report:
Intersection of Clement Blvd and 19th St Ln NW

Traffic Forecast Release Date:
December-16

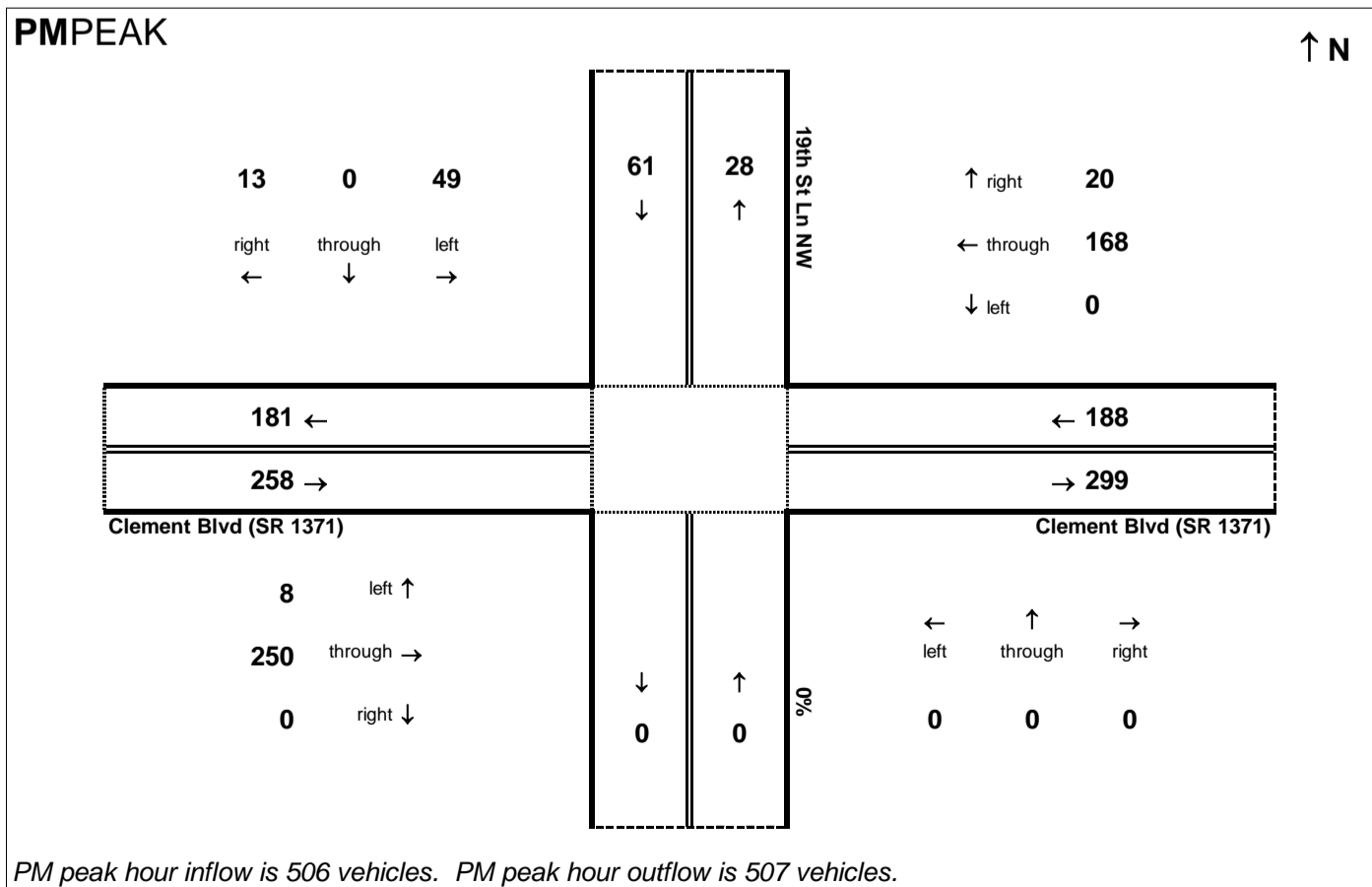
Traffic Data Year:
2040 Build 2

Project:
U-4700

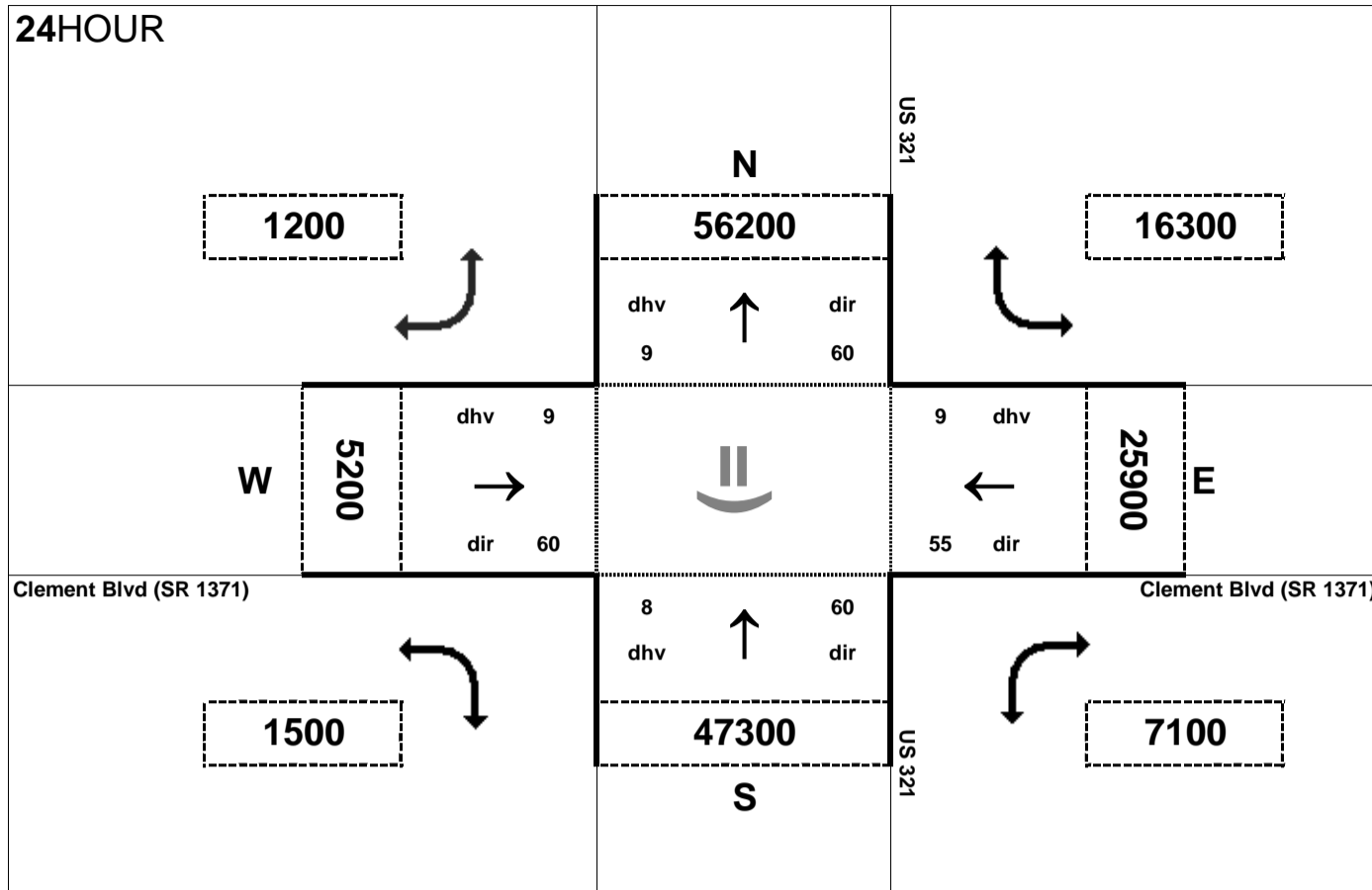
AMPEAK



PMPEAK



24HOUR



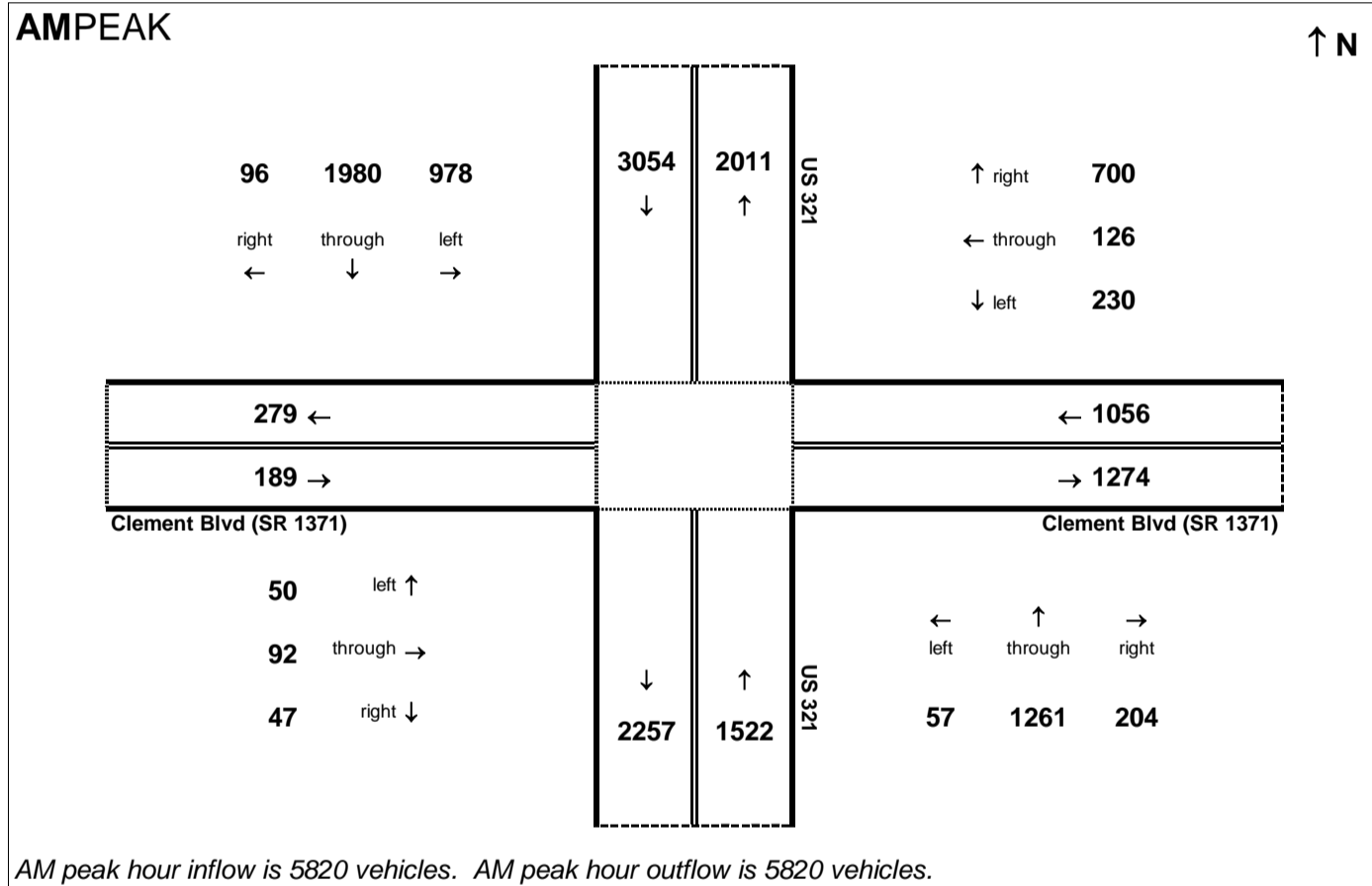
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Clement Blvd

Traffic Forecast Release Date:
December-16

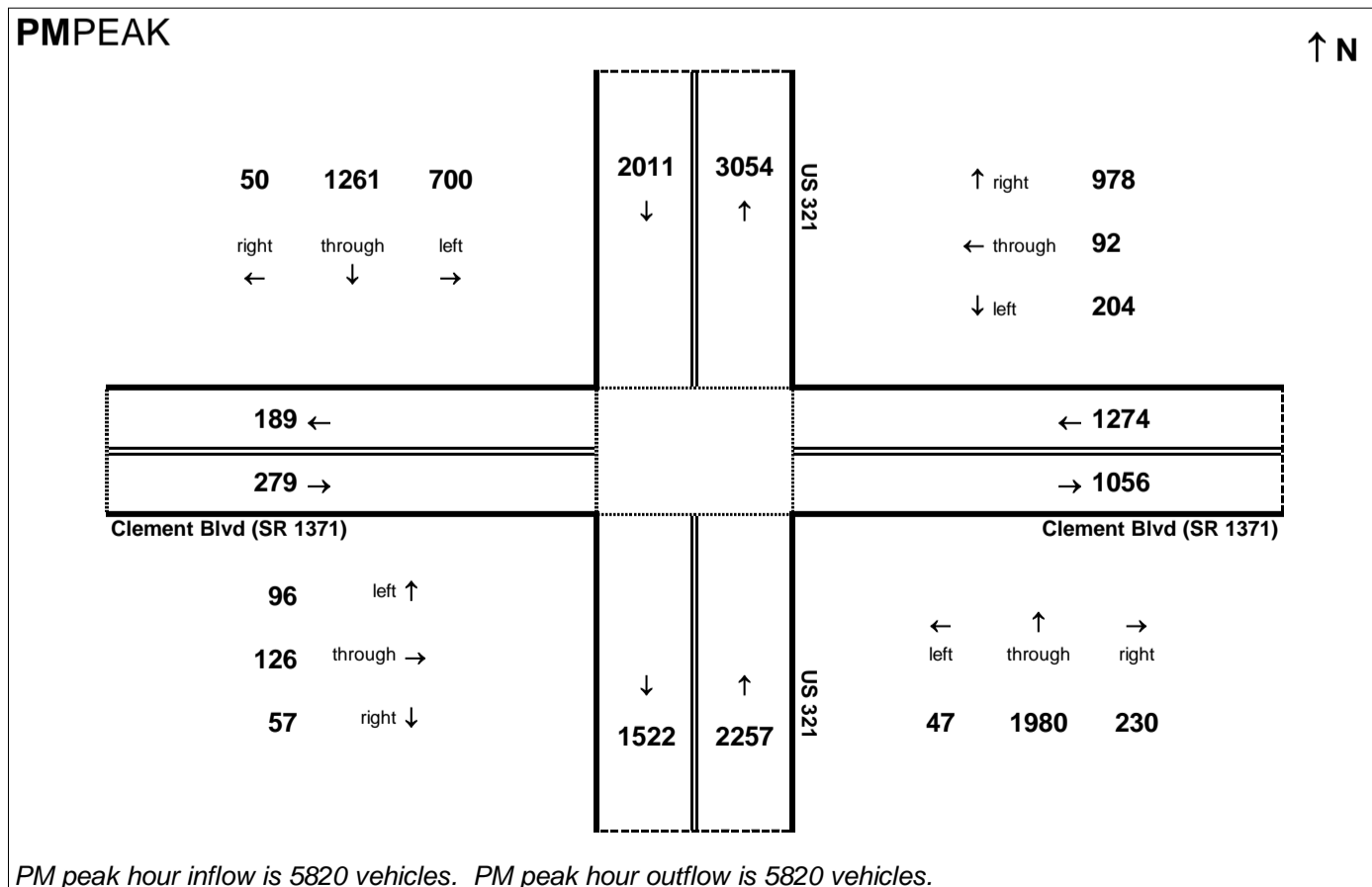
Traffic Data Year:
2040 Build 2

Project:
U-4700

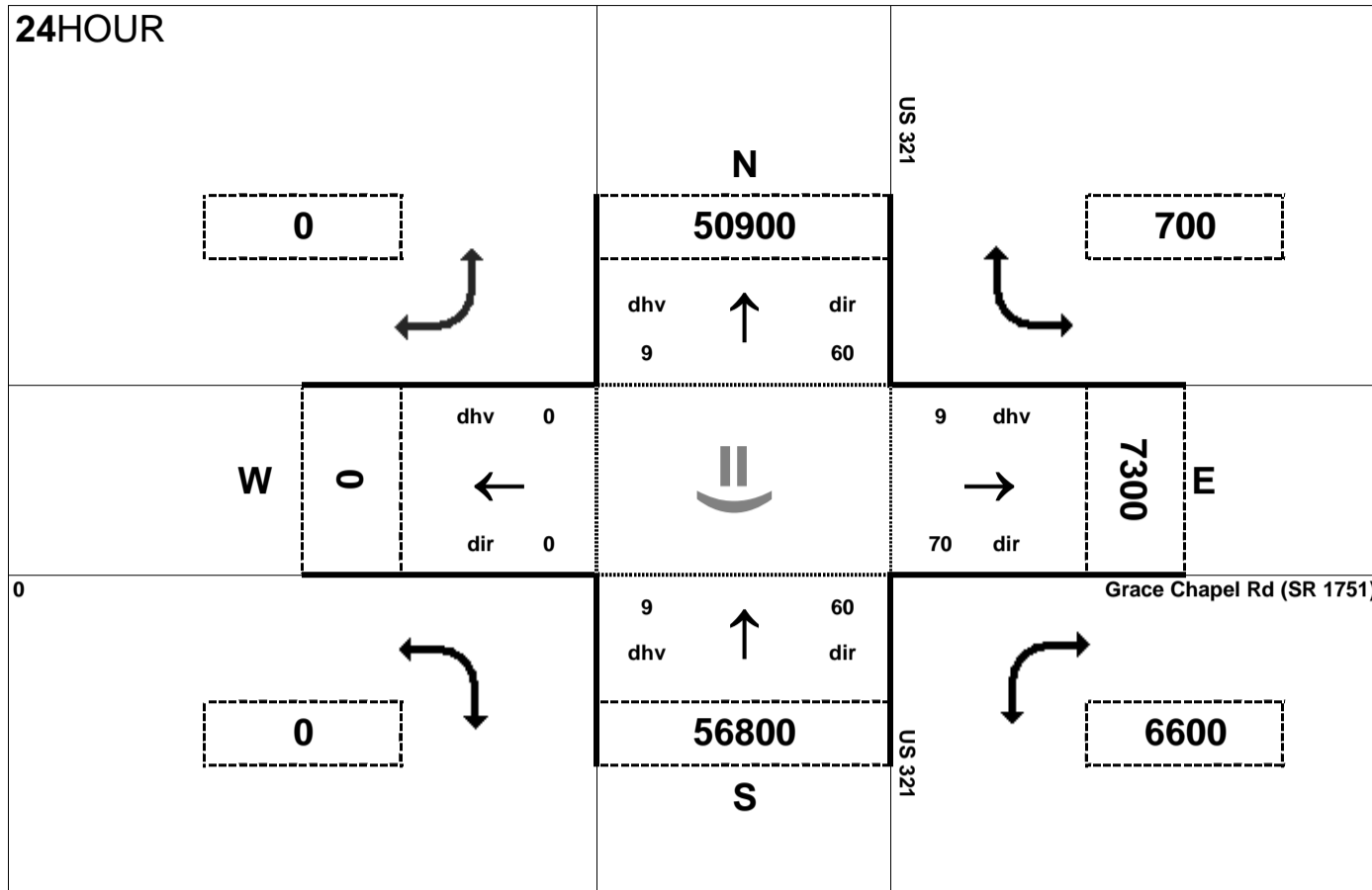
AMPEAK



PMPEAK



24HOUR



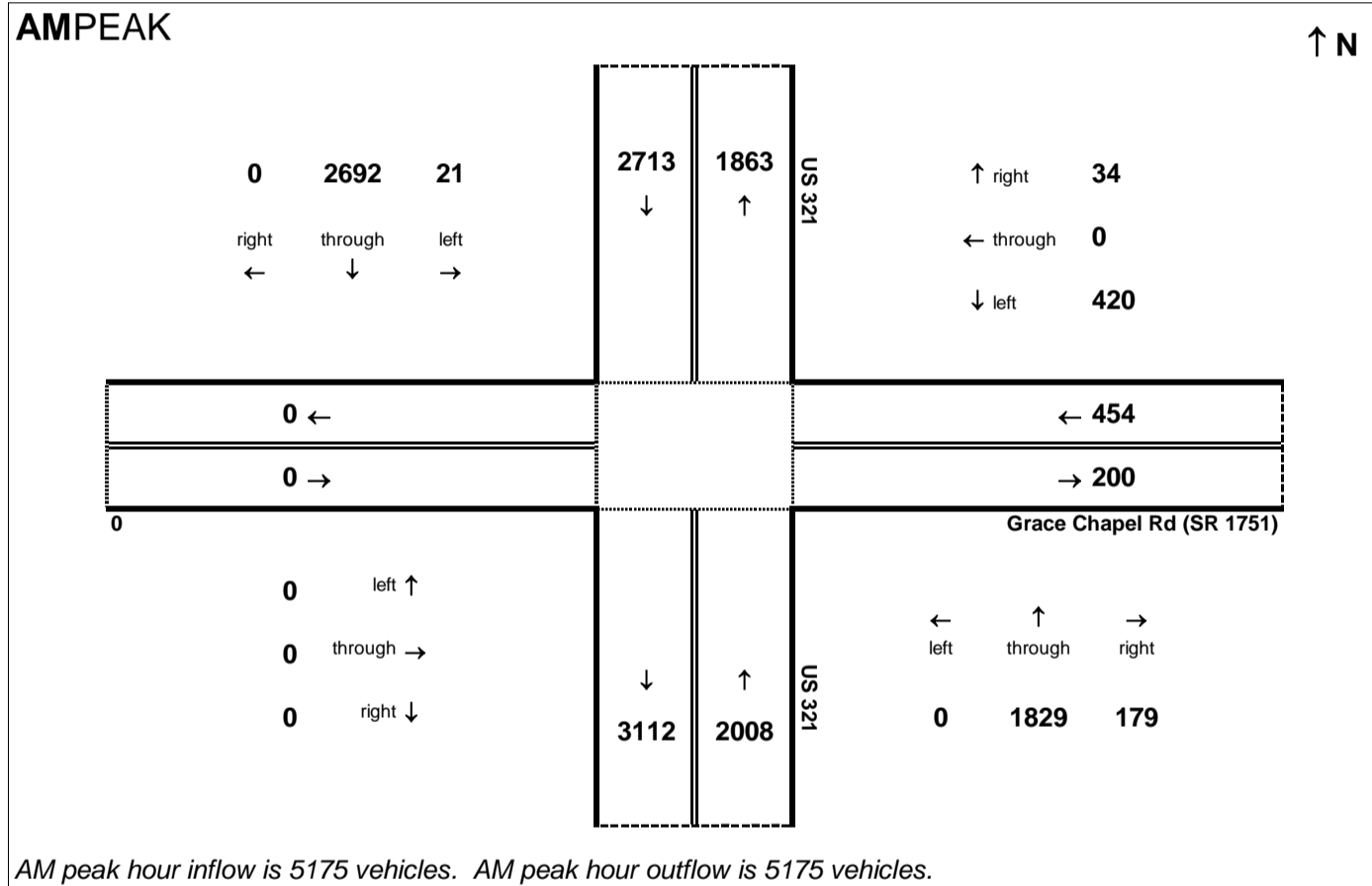
Peak Hour Volume Breakouts Report:
Intersection of US 321 and Grace Chapel Rd

Traffic Forecast Release Date:
December-16

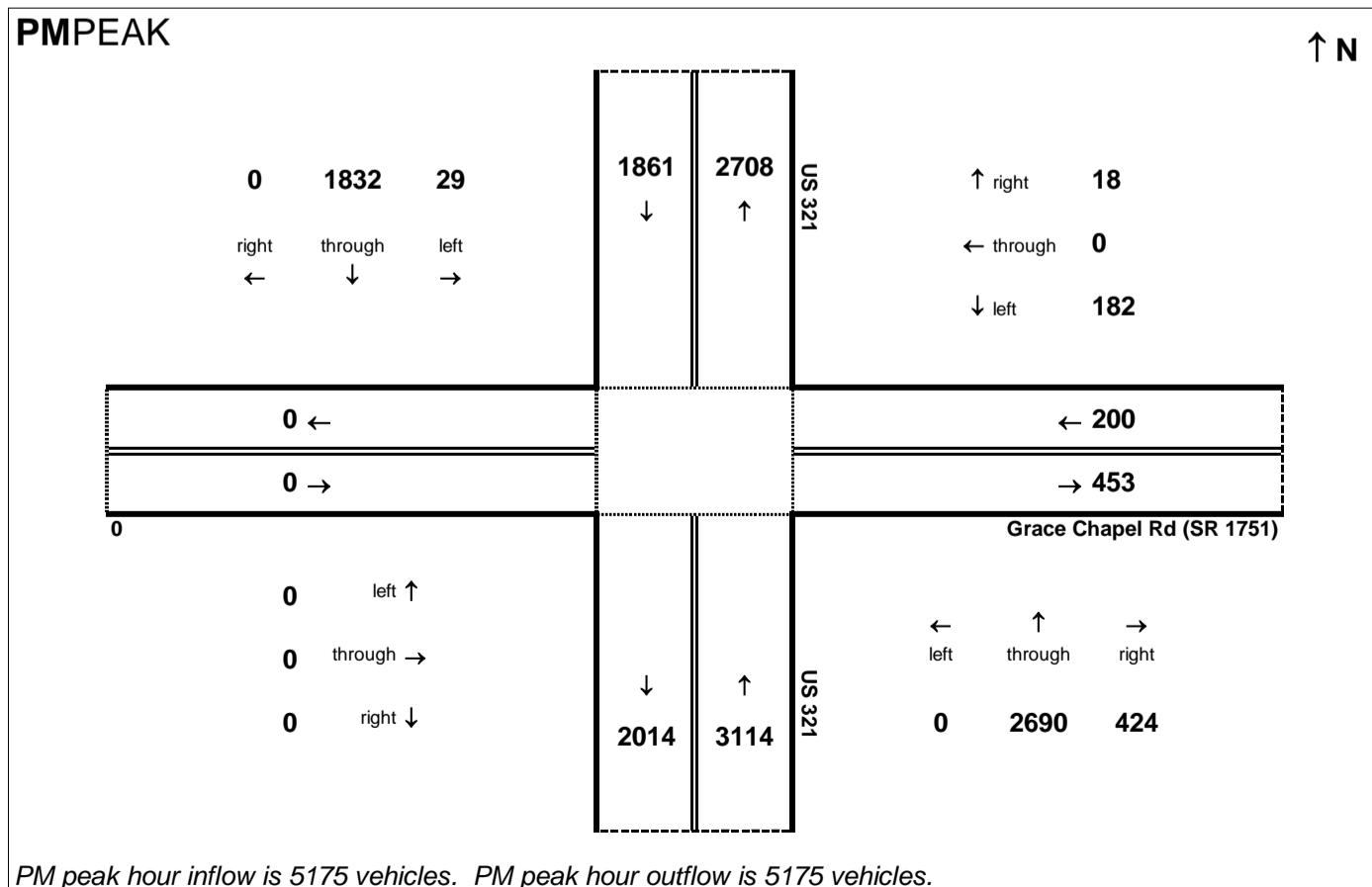
Traffic Data Year:
2040 Build 2

Project:
U-4700

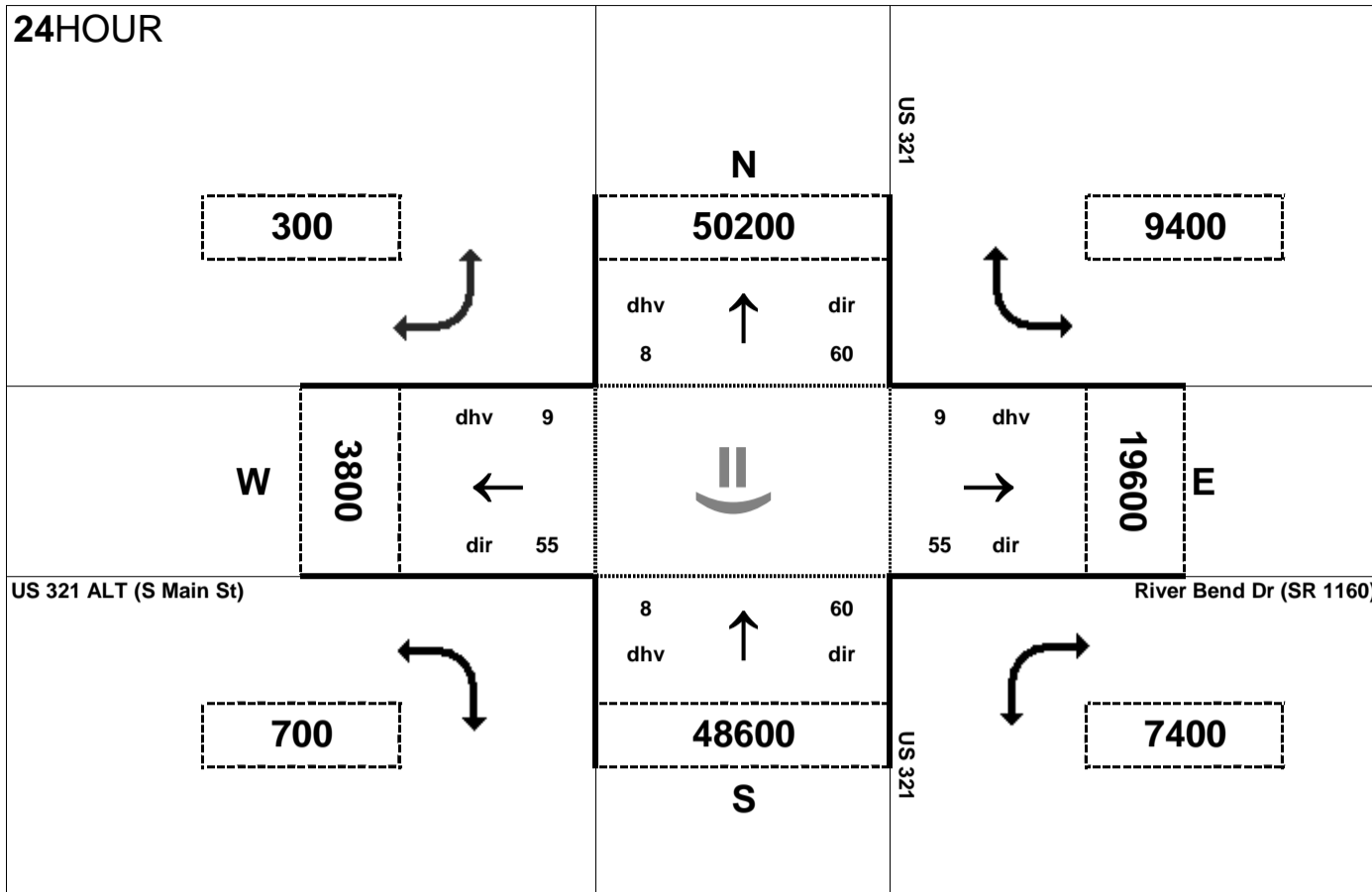
AMPEAK



PMPEAK



24HOUR



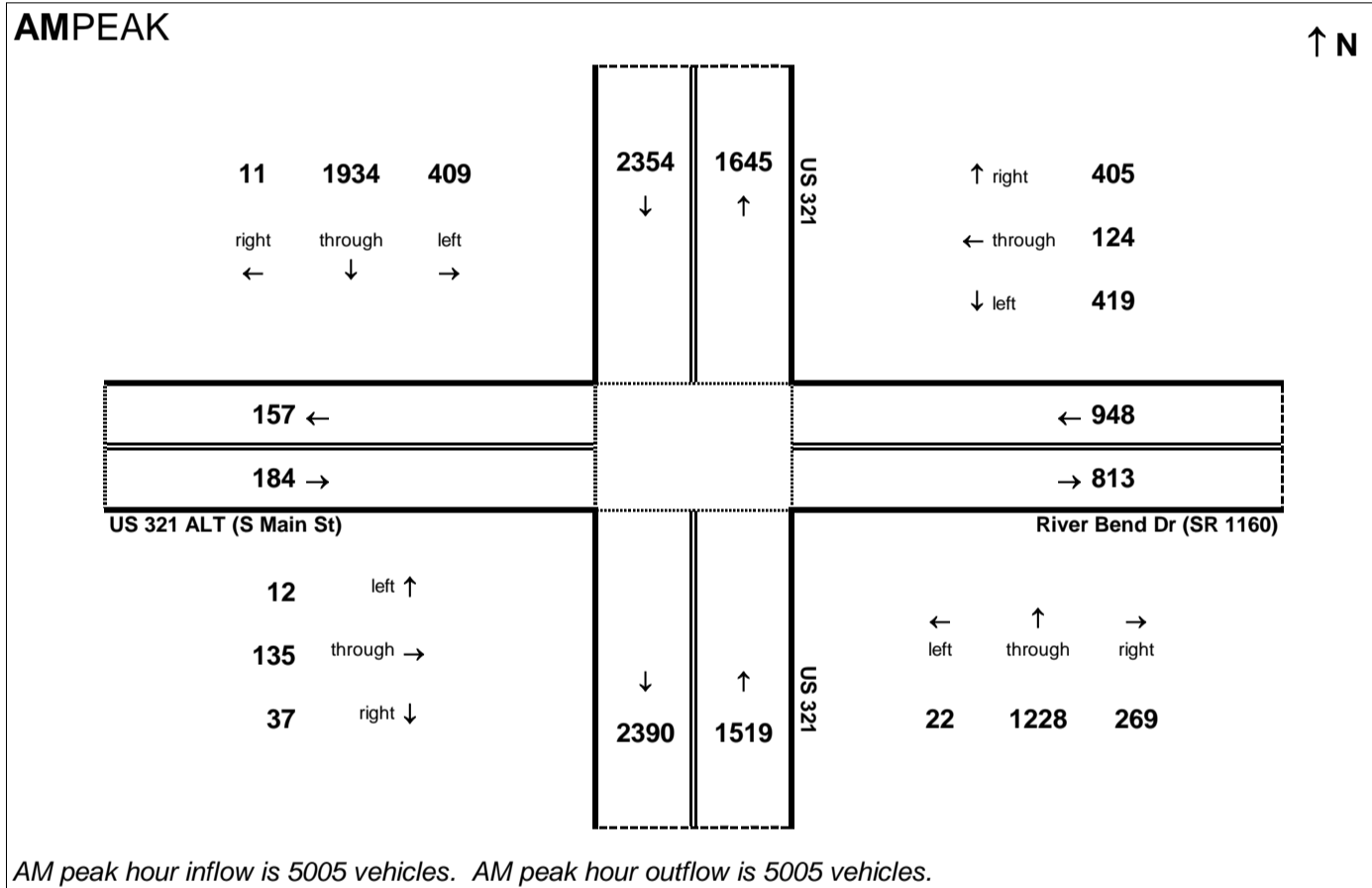
Peak Hour Volume Breakouts Report:
Intersection of US 321 and US 321 ALT

Traffic Forecast Release Date:
December-16

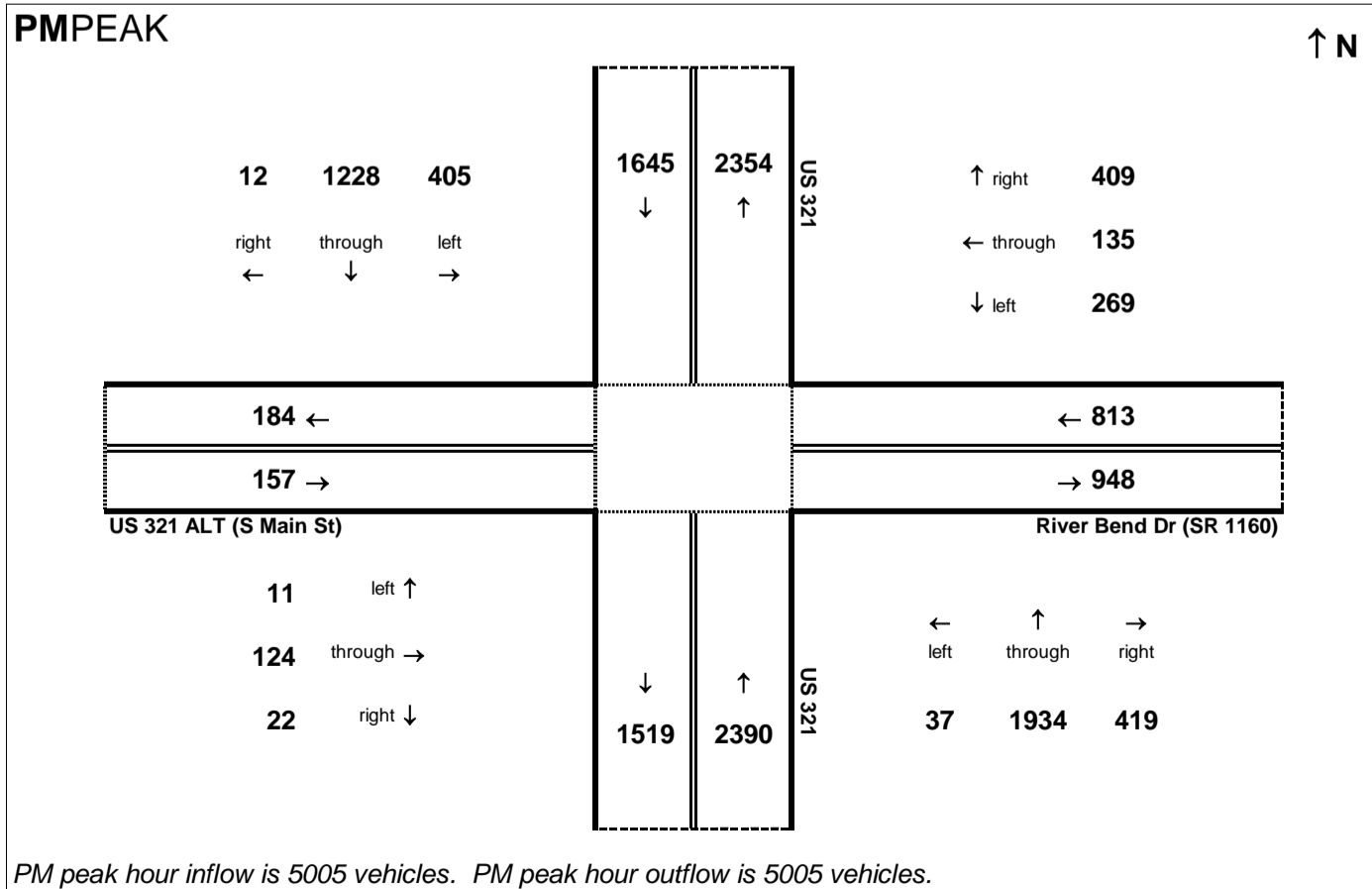
Traffic Data Year:
2040 Build 2

Project:
U-4700

AMPEAK



PMPEAK



Count Type: Turning Movement (No-Classification) - Intersection

NCDOT Count # 17-06132

Duration: 13 Hours

Milepost Begin: N/A

Catawba County

Volume (ADT): 8,406

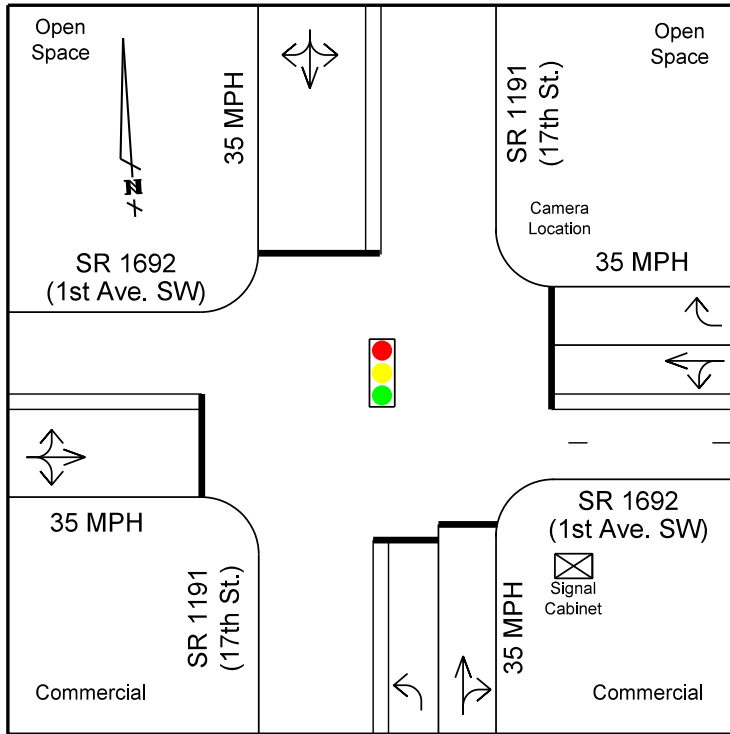
Milepost End: N/A

Hickory

05/30/2017 12:00 PM to 7:00 PM, Cloudy, 68 F Low, 83 F High, Trace Precipitation, Sky cover 0.9

Division 12

05/31/2017 6:00 AM to 12:00 PM, Cloudy, 63 F Low, 85 F High, Trace Precipitation, Sky cover 0.7



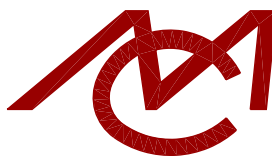
VICINITY MAP - N.T.S.

Comments:

1. Counted by: Walter Baker
2. Data processor: Jeff Hoppes
3. Method Used: CountCam 100
Petra Pro software.
4. Equipment Operating Properly: Yes.
5. School in session: Yes.
6. Area lighting present: Yes.
7. Construction present: No.
8. Disabled pedestrians: No.
9. Pedestrian movements: Yes.
10. Break times: N/A
11. Traffic flow disruption: No.
12. Traffic control: Signal.
13. Signal Inventory number: 12-0690
14. Distance to nearest stop sign less than 300 ft?
NB: N SB: N EB: N WB: N
15. Distance to nearest traffic signal less than 300 ft?
NB: N SB: N EB: N WB: N
16. Distance to nearest at-grade RR crossing less than 200 ft?
NB: N SB: N EB: N WB: N

Vendor: 60041
 Purchase Order: 4300292232
 Function Code: 1P10

Intersection of SR 1692
 (1st Avenue SW) and SR 1191
 (17th Street)



Mattern & Craig

CONSULTING ENGINEERS • SURVEYORS
 FIRM LICENSE No. C-1154
 12 BROAD STREET
 ASHEVILLE, NORTH CAROLINA 28801
 (828) 254-2201
 FAX (828) 254-4562

Comm. No. 2796

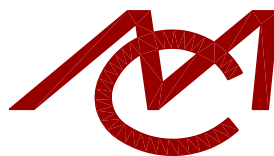


Northbound Approach looking North



Northbound Approach looking South

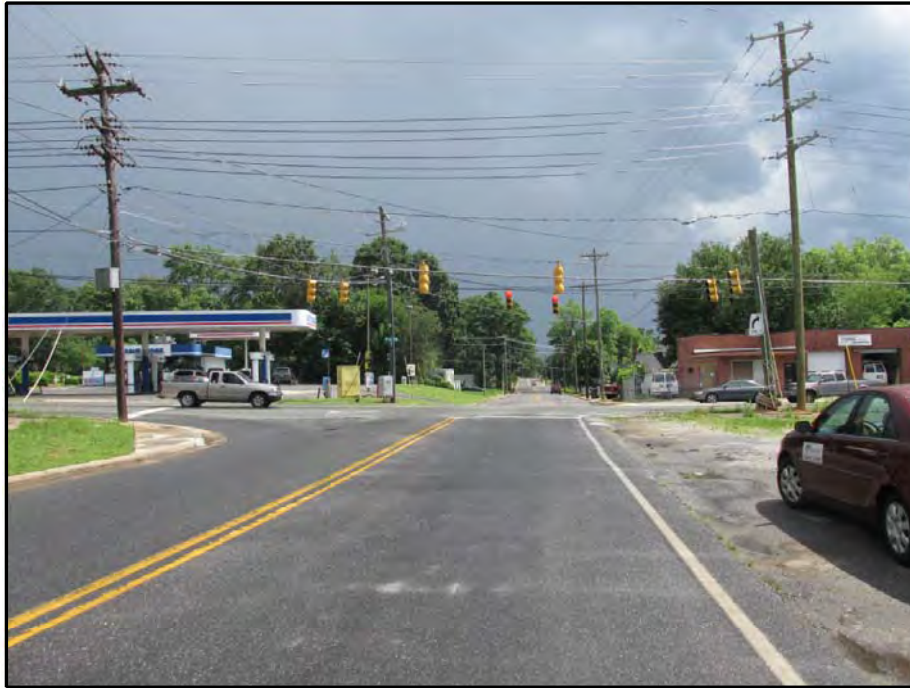
Intersection of SR 1692
(1st Avenue SW) and SR 1191
(17th Street)



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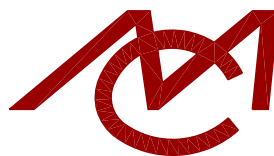


Southbound Approach looking South



Southbound Approach looking North

Intersection of SR 1692
(1st Avenue SW) and SR 1191
(17th Street)



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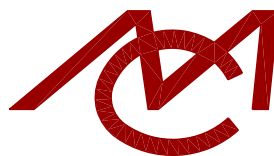


Eastbound Approach looking East



Eastbound Approach looking West

Intersection of SR 1692
(1st Avenue SW) and SR 1191
(17th Street)



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Comm. No. 2796

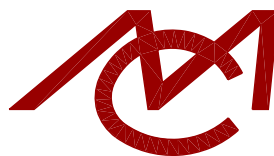


Westbound Approach looking West



Westbound Approach looking East

Intersection of SR 1692
(1st Avenue SW) and SR 1191
(17th Street)



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Comm. No. 2796

Mattern & Craig, Inc.

12 Broad St.

Asheville, NC, 28801

Phone: (828) 254-2201

Fax: (828) 254-4562

File Name : 1706132

Site Code : 17-06132

Start Date : 6/1/2017

Page No : 1

Groups Printed- PV - Pedestrians

Start Time	17th Street Southbound					1st Avenue SW Westbound					17th Street Northbound					1st Avenue SW Eastbound					Exclu. Total	Inclu. Total	Int. Total
	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total			
06:00 AM	2	9	4	2	15	6	7	1	0	14	0	3	2	0	5	0	23	5	0	28	2	62	64
06:15 AM	3	8	2	0	13	2	11	2	0	15	1	6	2	0	9	1	17	2	0	20	0	57	57
06:30 AM	5	9	8	0	22	7	21	8	1	36	1	6	0	2	7	2	43	7	0	52	3	117	120
06:45 AM	4	15	13	0	32	10	44	7	0	61	3	7	1	0	11	1	23	4	0	28	0	132	132
Total	14	41	27	2	82	25	83	18	1	126	5	22	5	2	32	4	106	18	0	128	5	368	373
07:00 AM	6	11	4	0	21	10	19	6	0	35	1	10	1	1	12	0	26	2	0	28	1	96	97
07:15 AM	4	10	5	0	19	8	27	7	0	42	1	13	0	0	14	4	29	0	1	33	1	108	109
07:30 AM	4	17	8	0	29	11	30	7	0	48	1	9	5	0	15	6	47	8	0	61	0	153	153
07:45 AM	8	12	9	0	29	5	33	6	0	44	3	18	5	1	26	4	44	5	0	53	1	152	153
Total	22	50	26	0	98	34	109	26	0	169	6	50	11	2	67	14	146	15	1	175	3	509	512
08:00 AM	5	5	7	0	17	16	38	2	0	56	1	22	1	0	24	4	36	4	0	44	0	141	141
08:15 AM	9	9	11	0	29	13	23	6	0	42	4	12	3	0	19	2	30	5	0	37	0	127	127
08:30 AM	5	12	6	0	23	13	29	4	0	46	2	20	1	0	23	1	27	8	0	36	0	128	128
08:45 AM	5	9	6	0	20	9	20	8	0	37	3	12	0	0	15	3	27	6	0	36	0	108	108
Total	24	35	30	0	89	51	110	20	0	181	10	66	5	0	81	10	120	23	0	153	0	504	504
09:00 AM	5	16	8	0	29	27	33	1	0	61	3	14	5	0	22	3	32	6	0	41	0	153	153
09:15 AM	10	8	14	0	32	13	19	3	0	35	2	4	6	0	12	2	43	8	0	53	0	132	132
09:30 AM	8	11	10	0	29	13	27	7	0	47	2	14	3	0	19	3	36	9	0	48	0	143	143
09:45 AM	4	11	9	0	24	15	42	13	0	70	1	13	5	0	19	3	30	10	0	43	0	156	156
Total	27	46	41	0	114	68	121	24	0	213	8	45	19	0	72	11	141	33	0	185	0	584	584
10:00 AM	5	9	7	0	21	16	39	7	0	62	2	13	3	0	18	1	29	10	0	40	0	141	141
10:15 AM	8	8	4	1	20	8	27	7	0	42	6	8	3	0	17	2	33	3	0	38	1	117	118
10:30 AM	6	11	24	0	41	10	39	4	0	53	3	8	2	0	13	2	28	12	0	42	0	149	149
10:45 AM	6	9	7	0	22	12	53	2	0	67	4	18	2	0	24	3	43	3	0	49	0	162	162
Total	25	37	42	1	104	46	158	20	0	224	15	47	10	0	72	8	133	28	0	169	1	569	570
11:00 AM	6	4	7	0	17	12	41	4	0	57	4	8	4	0	16	2	34	10	0	46	0	136	136
11:15 AM	11	9	10	0	30	11	34	2	0	47	3	9	1	0	13	3	26	11	0	40	0	130	130
11:30 AM	8	14	6	0	28	10	50	8	0	68	3	15	1	1	19	4	41	9	0	54	1	169	170
11:45 AM	8	7	9	1	24	12	23	7	0	42	4	14	2	1	20	4	20	6	0	30	2	116	118
Total	33	34	32	1	99	45	148	21	0	214	14	46	8	2	68	13	121	36	0	170	3	551	554
12:00 PM	14	14	14	0	42	16	48	7	0	71	5	22	4	0	31	6	36	6	0	48	0	192	192
12:15 PM	5	17	5	2	27	18	42	11	0	71	9	13	5	1	27	3	38	6	1	47	4	172	176
12:30 PM	13	16	5	0	34	10	37	12	0	59	3	17	8	0	28	2	48	15	0	65	0	186	186
12:45 PM	12	19	12	0	43	22	48	2	0	72	1	18	4	0	23	6	44	17	0	67	0	205	205
Total	44	66	36	2	146	66	175	32	0	273	18	70	21	1	109	17	166	44	1	227	4	755	759
01:00 PM	15	12	7	0	34	11	46	5	0	62	1	12	7	0	20	8	43	17	0	68	0	184	184
01:15 PM	14	14	7	0	35	16	32	7	0	55	3	9	1	0	13	6	41	11	0	58	0	161	161
01:30 PM	3	15	8	0	26	8	42	10	0	60	5	18	5	0	28	5	38	16	0	59	0	173	173
01:45 PM	7	13	9	0	29	15	56	7	0	78	1	10	4	0	15	4	46	18	0	68	0	190	190
Total	39	54	31	0	124	50	176	29	0	255	10	49	17	0	76	23	168	62	0	253	0	708	708
02:00 PM	6	10	6	0	22	14	38	5	0	57	2	19	1	1	22	8	36	7	0	51	1	152	153
02:15 PM	11	11	9	0	31	11	44	7	0	62	3	9	4	0	16	3	45	12	0	60	0	169	169
02:30 PM	7	8	8	0	23	22	50	7	0	79	4	12	2	0	18	5	59	9	0	73	0	193	193
02:45 PM	7	14	10	0	31	25	57	6	0	88	1	13	3	0	17	4	46	4	0	54	0	190	190
Total	31	43	33	0	107	72	189	25	0	286	10	53	10	1	73	20	186	32	0	238	1	704	705

Mattern & Craig, Inc.

12 Broad St.

Asheville, NC, 28801

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Fax: (828) 254-4562

File Name : 1706132

Site Code : 17-06132

Start Date : 6/1/2017

Page No : 2

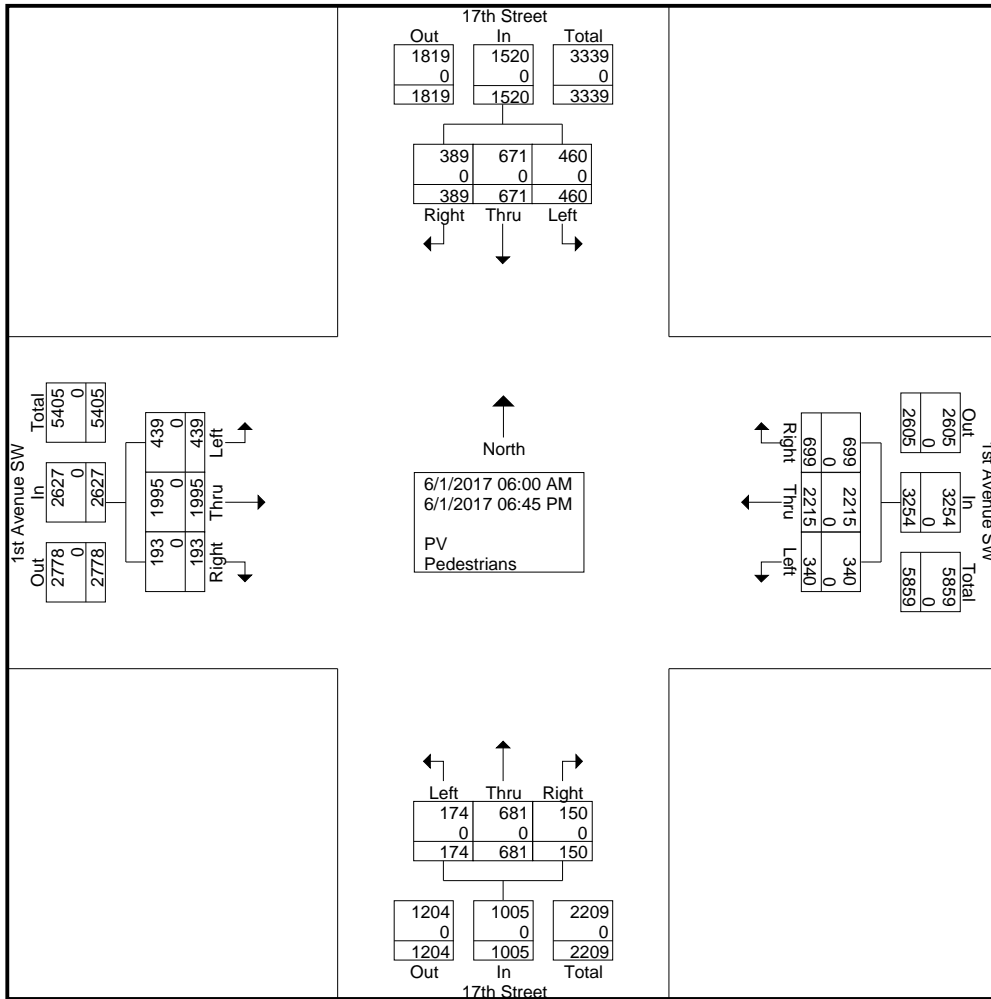
Groups Printed- PV - Pedestrians

Start Time	17th Street Southbound					1st Avenue SW Westbound					17th Street Northbound					1st Avenue SW Eastbound					Exclu. Total	Inclu. Total	Int. Total
	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total			
03:00 PM	8	11	15	2	34	16	44	9	0	69	6	14	1	0	21	4	48	6	0	58	2	182	184
03:15 PM	6	10	8	0	24	22	69	6	0	97	5	13	8	0	26	3	45	11	0	59	0	206	206
03:30 PM	10	11	14	0	35	13	64	12	0	89	5	12	3	0	20	7	69	8	0	84	0	228	228
03:45 PM	9	21	8	0	38	16	72	7	0	95	2	23	6	0	31	9	43	15	0	67	0	231	231
Total	33	53	45	2	131	67	249	34	0	350	18	62	18	0	98	23	205	40	0	268	2	847	849
04:00 PM	10	21	15	0	46	23	63	12	0	98	5	14	1	0	20	4	43	9	0	56	0	220	220
04:15 PM	4	10	11	2	25	15	60	7	0	82	3	12	5	0	20	8	51	14	1	73	3	200	203
04:30 PM	12	28	19	1	59	25	75	8	0	108	8	19	8	2	35	6	69	10	0	85	3	287	290
04:45 PM	7	26	8	2	41	14	79	7	0	100	2	21	2	0	25	1	37	8	0	46	2	212	214
Total	33	85	53	5	171	77	277	34	0	388	18	66	16	2	100	19	200	41	1	260	8	919	927
05:00 PM	8	22	6	0	36	15	95	12	0	122	2	14	5	1	21	7	56	10	0	73	1	252	253
05:15 PM	10	22	11	0	43	11	59	8	0	78	2	14	3	0	19	2	68	9	0	79	0	219	219
05:30 PM	11	23	12	2	46	16	58	5	1	79	4	18	5	1	27	2	30	10	0	42	4	194	198
05:45 PM	8	21	10	0	39	17	45	10	0	72	2	17	5	1	24	4	37	14	0	55	1	190	191
Total	37	88	39	2	164	59	257	35	1	351	10	63	18	3	91	15	191	43	0	249	6	855	861
06:00 PM	6	18	11	0	35	12	42	7	0	61	4	13	1	0	18	5	31	5	0	41	0	155	155
06:15 PM	5	3	7	1	15	8	49	3	0	60	2	10	3	2	15	4	24	4	0	32	3	122	125
06:30 PM	5	13	6	0	24	8	41	6	0	55	1	7	8	0	16	4	27	4	0	35	0	130	130
06:45 PM	11	5	1	0	17	11	31	6	0	48	1	12	4	0	17	3	30	11	0	44	0	126	126
Total	27	39	25	1	91	39	163	22	0	224	8	42	16	2	66	16	112	24	0	152	3	533	536
Grand Total	389	671	460	16	1520	699	2215	340	2	3254	150	681	174	15	1005	193	1995	439	3	2627	36	8406	8442
Apprch %	25.6	44.1	30.3			21.5	68.1	10.4			14.9	67.8	17.3			7.3	75.9	16.7					
Total %	4.6	8	5.5		18.1	8.3	26.4	4		38.7	1.8	8.1	2.1		12	2.3	23.7	5.2		31.3	0.4	99.6	
PV	389	671	460		1520	699	2215	340		3254	150	681	174		1005	193	1995	439		2627	0	0	8406
% PV	100	100	100	0	99	100	100	100	0	99.9	100	100	100	0	98.5	100	100	100	0	99.9	0	0	99.6
Pedestrians	0	0	0		16	0	0	0		2	0	0	0		15	0	0	0		3	0	0	36
% Pedestrians	0	0	0	100	1	0	0	0	100	0.1	0	0	0	100	1.5	0	0	0	100	0.1	0	0	0.4

Mattern & Craig, Inc.

12 Broad St.
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File Name : 1706132
 Site Code : 17-06132
 Start Date : 6/1/2017
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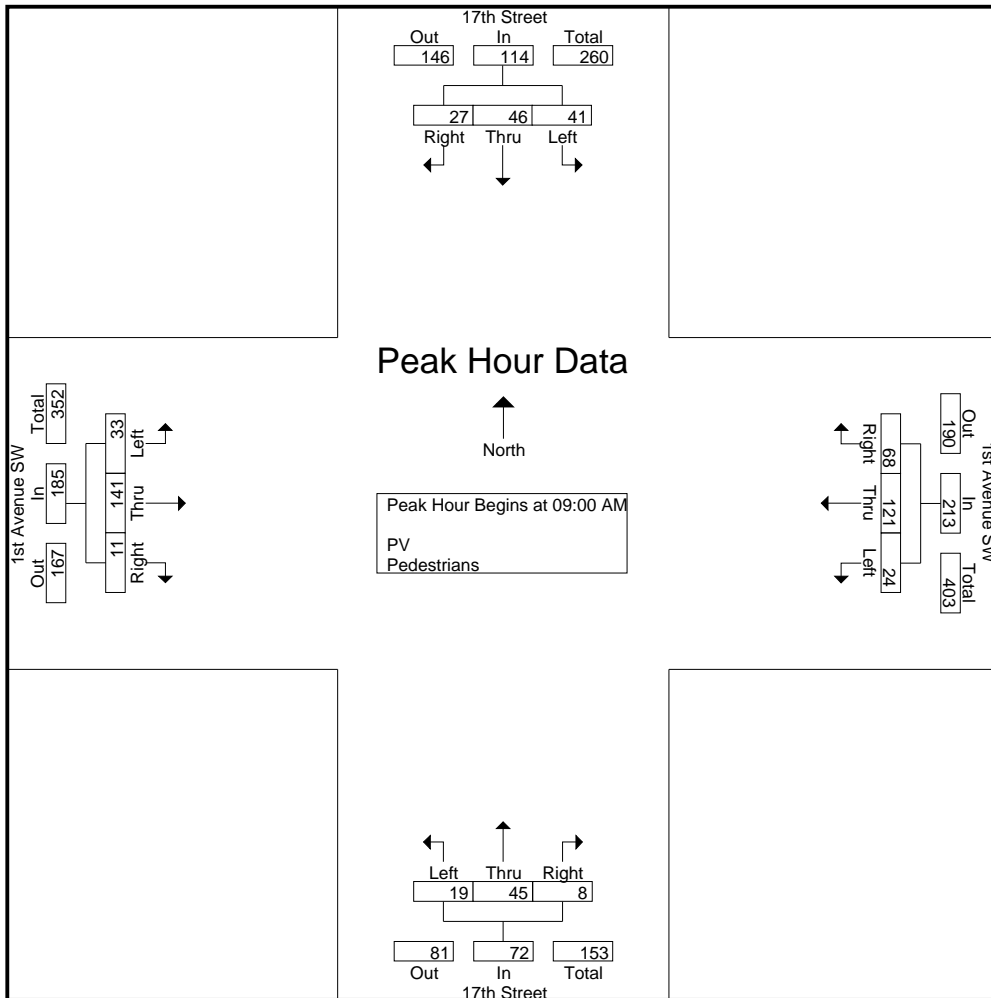
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File Name : 1706132
 Site Code : 17-06132
 Start Date : 6/1/2017
 Page No : 4

Start Time	17th Street Southbound				1st Avenue SW Westbound				17th Street Northbound				1st Avenue SW Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
09:00 AM	5	16	8	29	27	33	1	61	3	14	5	22	3	32	6	41	153
09:15 AM	10	8	14	32	13	19	3	35	2	4	6	12	2	43	8	53	132
09:30 AM	8	11	10	29	13	27	7	47	2	14	3	19	3	36	9	48	143
09:45 AM	4	11	9	24	15	42	13	70	1	13	5	19	3	30	10	43	156
Total Volume	27	46	41	114	68	121	24	213	8	45	19	72	11	141	33	185	584
% App. Total	23.7	40.4	36		31.9	56.8	11.3		11.1	62.5	26.4		5.9	76.2	17.8		
PHF	.675	.719	.732	.891	.630	.720	.462	.761	.667	.804	.792	.818	.917	.820	.825	.873	.936

Peak Hour Analysis From 06:00 AM to 09:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 09:00 AM

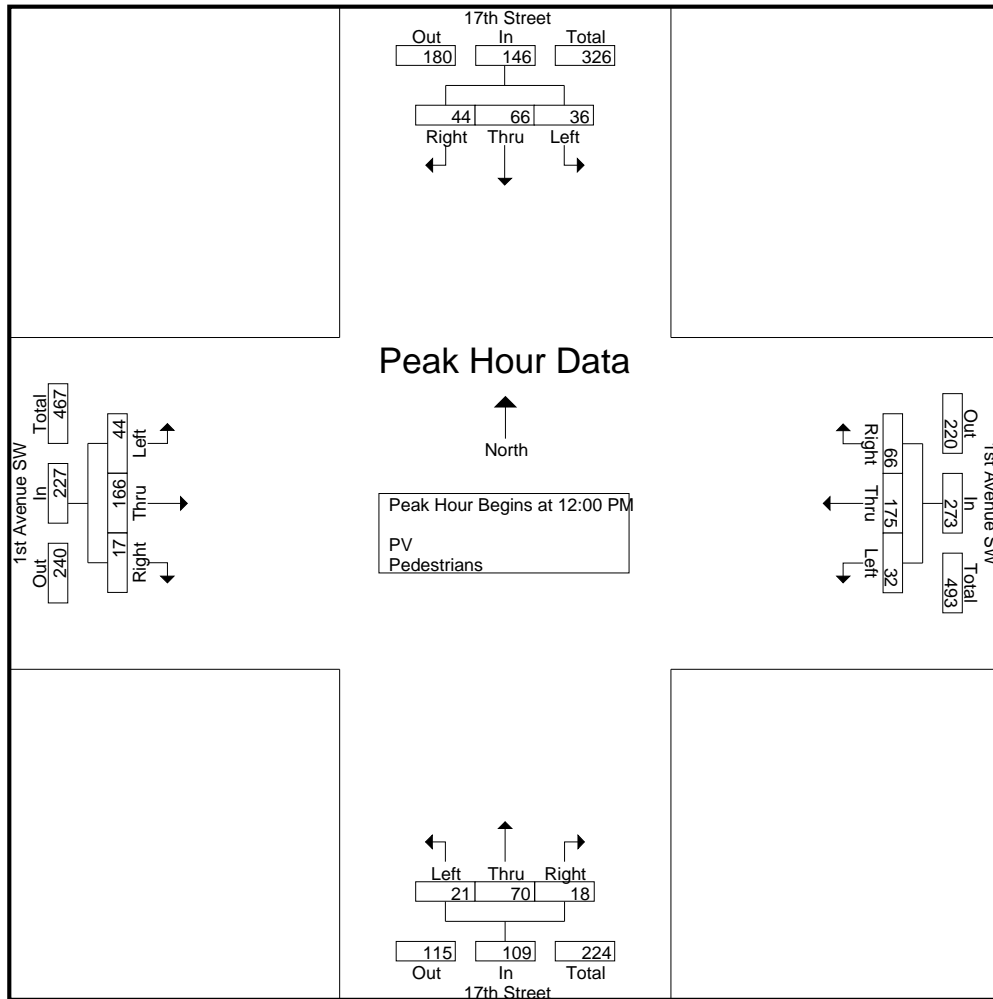


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File Name : 1706132
 Site Code : 17-06132
 Start Date : 6/1/2017
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Start Time	17th Street Southbound				1st Avenue SW Westbound				17th Street Northbound				1st Avenue SW Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 12:00 PM																	
12:00 PM	14	14	14	42	16	48	7	71	5	22	4	31	6	36	6	48	192
12:15 PM	5	17	5	27	18	42	11	71	9	13	5	27	3	38	6	47	172
12:30 PM	13	16	5	34	10	37	12	59	3	17	8	28	2	48	15	65	186
12:45 PM	12	19	12	43	22	48	2	72	1	18	4	23	6	44	17	67	205
Total Volume	44	66	36	146	66	175	32	273	18	70	21	109	17	166	44	227	755
% App. Total	30.1	45.2	24.7		24.2	64.1	11.7		16.5	64.2	19.3		7.5	73.1	19.4		
PHF	.786	.868	.643	.849	.750	.911	.667	.948	.500	.795	.656	.879	.708	.865	.647	.847	.921

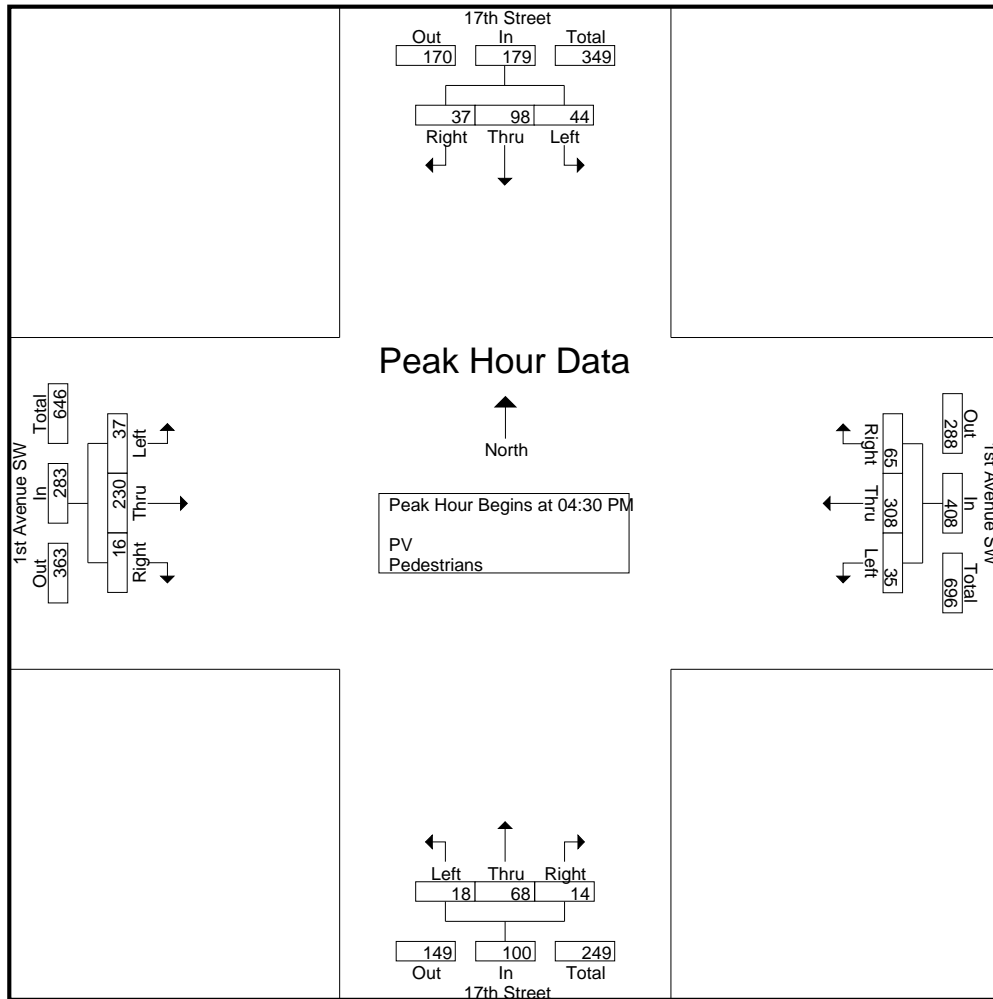


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File Name : 1706132
 Site Code : 17-06132
 Start Date : 6/1/2017
 Page No : 6

Start Time	17th Street Southbound				1st Avenue SW Westbound				17th Street Northbound				1st Avenue SW Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 02:00 PM to 06:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	12	28	19	59	25	75	8	108	8	19	8	35	6	69	10	85	287
04:45 PM	7	26	8	41	14	79	7	100	2	21	2	25	1	37	8	46	212
05:00 PM	8	22	6	36	15	95	12	122	2	14	5	21	7	56	10	73	252
05:15 PM	10	22	11	43	11	59	8	78	2	14	3	19	2	68	9	79	219
Total Volume	37	98	44	179	65	308	35	408	14	68	18	100	16	230	37	283	970
% App. Total	20.7	54.7	24.6		15.9	75.5	8.6		14	68	18		5.7	81.3	13.1		
PHF	.771	.875	.579	.758	.650	.811	.729	.836	.438	.810	.563	.714	.571	.833	.925	.832	.845



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Site Code : 17-06132

Start Date : 6/1/2017

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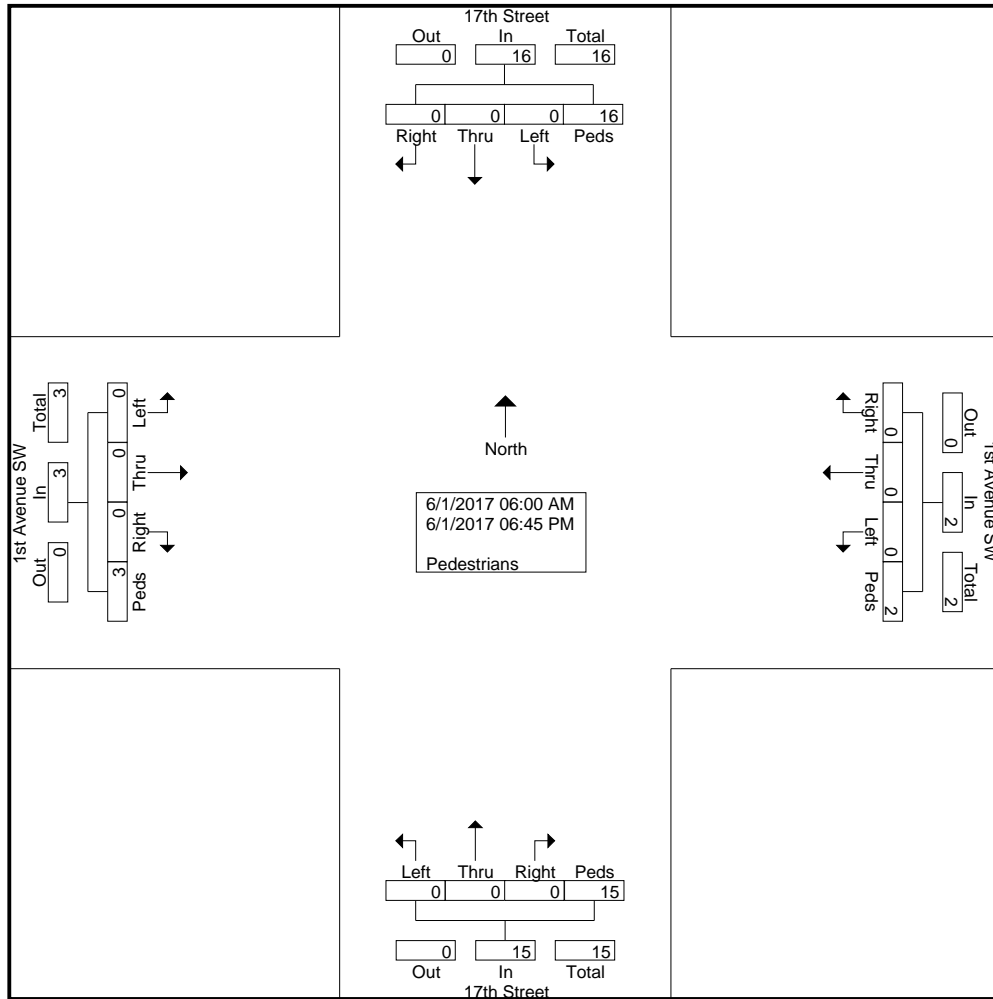
Groups Printed- Pedestrians

Start Time	17th Street Southbound					1st Avenue SW Westbound					17th Street Northbound					1st Avenue SW Eastbound					Int. Total					
	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total						
03:00 PM	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1	1	3	
04:30 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	3
04:45 PM	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total	0	0	0	5	5	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	1	1	1	1	8	
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	0	0	0	1
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	2	2	0	0	0	1	1	0	0	0	1	1	1	0	0	0	0	0	0	0	0	0	4
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	0	0	0	1
Total	0	0	0	2	2	0	0	0	1	1	0	0	0	3	3	3	0	0	0	0	0	0	0	0	0	6
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	2	2	2	0	0	0	0	0	0	0	0	0	3
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	1	0	0	0	0	0	0	0	0	2	2	2	0	0	0	0	0	0	0	0	0	3
Grand Total	0	0	0	16	16	0	0	0	2	2	0	0	0	15	15	15	0	0	0	3	3	3	3	3	36	
Apprch %	0	0	0	100		0	0	0	100		0	0	0	100			0	0	0	100						
Total %	0	0	0	44.4	44.4	0	0	0	5.6	5.6	0	0	0	41.7	41.7	41.7	0	0	0	8.3	8.3	8.3	8.3	8.3		

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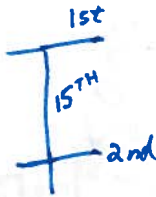


BUILD 1

2016

3700

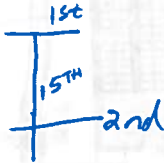
3100



2040

4300

3500



$$R = \left(\frac{V_f}{V_c}\right)^{\frac{1}{r}} - 1 = \left(\frac{4300}{3700}\right)^{\frac{1}{24}} - 1 = .00628$$
$$= .628\%$$

$$= \left(\frac{3500}{3100}\right)^{\frac{1}{24}} - 1 = .00507$$
$$= .507\%$$

$$GF \quad \frac{4300}{3700} = 1.162$$

$$\frac{3500}{3100} = 1.129$$

USE 1.146

USE .6% FOR 17TH

AECOM

701 Corporate Center Drive
Suite 475
Raleigh, NC 27607
Traffic Engineering

File Name : 1706132
Site Code : 17-06132
Start Date : 6/1/2017
Page No : 1

Groups Printed- PV

Start Time	1st Avenue SW Eastbound				1st Avenue SW Westbound				17th Street Northbound				17th Street Southbound				Int. Total
	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	
06:00	6	26	0	0	1	8	7	0	2	3	0	0	5	10	2	0	70
06:15	2	20	1	0	2	13	2	0	2	7	1	0	2	9	3	0	64
06:30	8	49	2	0	9	24	8	0	0	7	1	0	9	10	6	0	133
06:45	5	26	1	0	8	51	11	0	1	8	3	0	15	17	5	0	151
Total	21	121	4	0	20	96	28	0	5	25	5	0	31	46	16	0	418
07:00	2	30	0	0	7	22	11	0	1	11	1	0	5	13	7	0	110
07:15	0	33	5	0	8	31	9	0	0	15	1	0	6	11	5	0	124
07:30	9	54	7	0	8	34	13	0	6	10	1	0	9	20	5	0	176
07:45	6	51	5	0	7	38	6	0	6	21	3	0	10	14	9	0	176
Total	17	168	17	0	30	125	39	0	13	57	6	0	30	58	26	0	586
08:00	5	41	5	0	2	44	18	0	1	25	1	0	8	6	6	0	162
08:15	6	34	2	0	7	26	15	0	3	14	5	0	13	10	10	0	145
08:30	9	31	1	0	5	33	15	0	1	23	2	0	7	14	6	0	147
08:45	7	31	3	0	9	23	10	0	0	14	3	0	7	10	6	0	123
Total	27	137	11	0	23	126	58	0	5	76	11	0	35	40	28	0	577
09:00	7	37	3	0	1	38	31	0	6	16	3	0	9	18	6	0	175
09:15	9	49	2	0	3	22	15	0	7	5	2	0	16	9	11	0	150
09:30	10	41	3	0	8	31	15	0	3	16	2	0	11	13	9	0	162
09:45	11	34	3	0	15	48	17	0	6	15	1	0	10	13	5	0	178
Total	37	161	11	0	27	139	78	0	22	52	8	0	46	53	31	0	665
10:00	11	33	1	0	8	45	18	0	3	15	2	0	8	10	6	0	160
10:15	3	38	2	0	8	31	9	0	3	9	7	0	5	9	9	0	133
10:30	14	32	2	0	5	45	11	0	2	9	3	0	28	13	7	0	171
10:45	3	49	3	0	2	61	14	0	2	21	5	0	8	10	7	0	185
Total	31	152	8	0	23	182	52	0	10	54	17	0	49	42	29	0	649
11:00	11	39	2	0	5	47	14	0	5	9	5	0	8	5	7	0	157
11:15	13	30	3	0	2	39	13	0	1	10	3	0	11	10	13	0	148
11:30	10	47	5	0	9	57	11	0	1	17	3	0	7	16	9	0	192
11:45	7	23	5	0	8	26	14	0	2	16	5	0	10	8	9	0	133
Total	41	139	15	0	24	169	52	0	9	52	16	0	36	39	38	0	630
12:00	7	41	7	0	8	55	18	0	5	25	6	0	16	16	16	0	220
12:15	7	44	3	0	13	48	21	0	6	15	10	0	6	20	6	0	199
12:30	17	55	2	0	14	43	11	0	9	20	3	0	6	18	15	0	213
12:45	20	51	7	0	2	55	25	0	5	21	1	0	14	22	14	0	237
Total	51	191	19	0	37	201	75	0	25	81	20	0	42	76	51	0	869
13:00	20	49	9	0	6	53	13	0	8	14	1	0	8	14	17	0	212
13:15	13	47	7	0	8	37	18	0	1	10	3	0	8	16	16	0	184
13:30	18	44	6	0	11	48	9	0	6	21	6	0	9	17	3	0	198
13:45	21	53	5	0	8	64	17	0	5	11	1	0	10	15	8	0	218
Total	72	193	27	0	33	202	57	0	20	56	11	0	35	62	44	0	812
14:00	8	41	9	0	6	44	16	0	1	22	2	0	7	11	7	0	174
14:15	14	52	3	0	8	51	13	0	5	10	3	0	10	13	13	0	195
14:30	10	68	6	0	8	57	25	0	2	14	5	0	9	9	8	0	221
14:45	5	53	5	0	7	66	29	0	3	15	1	0	11	16	8	0	219
Total	37	214	23	0	29	218	83	0	11	61	11	0	37	49	36	0	809
15:00	7	55	5	0	10	51	18	0	1	16	7	0	17	13	9	0	209
15:15	13	52	3	0	7	79	25	0	9	15	6	0	9	11	7	0	236
15:30	9	79	8	0	14	74	15	0	3	14	6	0	16	13	11	0	262
15:45	17	49	10	0	8	83	18	0	7	26	2	0	9	24	10	0	263
Total	46	235	26	0	39	287	76	0	20	71	21	0	51	61	37	0	970

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701 Corporate Center Drive
Suite 475
Raleigh, NC 27607
Traffic Engineering

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Groups Printed- PV

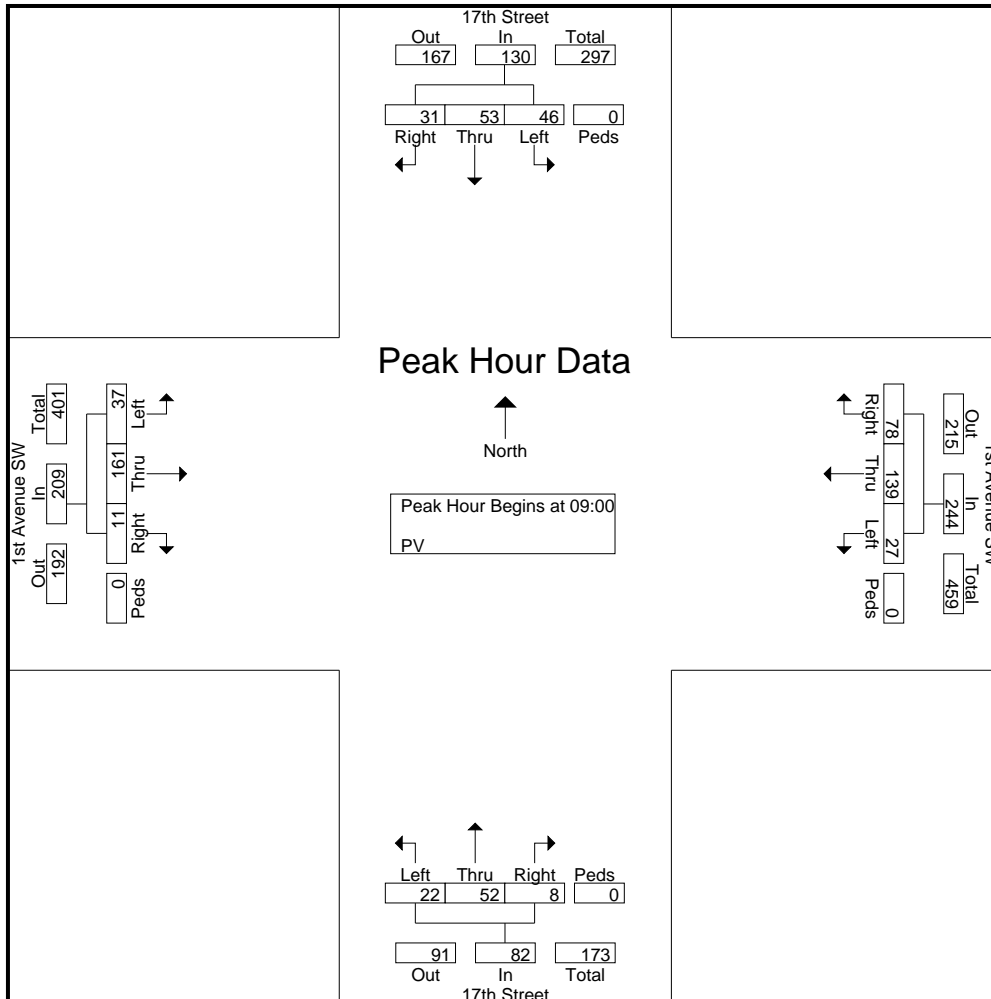
Start Time	1st Avenue SW Eastbound				1st Avenue SW Westbound				17th Street Northbound				17th Street Southbound				Int. Total
	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	
16:00	10	49	5	0	14	72	26	0	1	16	6	0	17	24	11	0	251
16:15	16	59	9	0	8	69	17	0	6	14	3	0	13	11	5	0	230
16:30	11	79	7	0	9	86	29	0	9	22	9	0	22	32	14	0	329
16:45	9	43	1	0	8	91	16	0	2	24	2	0	9	30	8	0	243
Total	46	230	22	0	39	318	88	0	18	76	20	0	61	97	38	0	1053
17:00	11	64	8	0	14	109	17	0	6	16	2	0	7	25	9	0	288
17:15	10	78	2	0	9	68	13	0	3	16	2	0	13	25	11	0	250
17:30	11	34	2	0	6	67	18	0	6	21	5	0	14	26	13	0	223
17:45	16	43	5	0	11	52	20	0	6	20	2	0	11	24	9	0	219
Total	48	219	17	0	40	296	68	0	21	73	11	0	45	100	42	0	980
18:00	6	36	6	0	8	48	14	0	1	15	5	0	13	21	7	0	180
18:15	5	28	5	0	3	56	9	0	3	11	2	0	8	3	6	0	139
18:30	5	31	5	0	7	47	9	0	9	8	1	0	7	15	6	0	150
18:45	13	34	3	0	7	36	13	0	5	14	1	0	1	6	13	0	146
Total	29	129	19	0	25	187	45	0	18	48	9	0	29	45	32	0	615
Grand Total	503	2289	219	0	389	2546	799	0	197	782	166	0	527	768	448	0	9633
Apprch %	16.7	76	7.3	0	10.4	68.2	21.4	0	17.2	68.3	14.5	0	30.2	44.1	25.7	0	
Total %	5.2	23.8	2.3	0	4	26.4	8.3	0	2	8.1	1.7	0	5.5	8	4.7	0	

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Traffic Engineering

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Start Time	1st Avenue SW Eastbound					1st Avenue SW Westbound					17th Street Northbound					17th Street Southbound					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Peak Hour Analysis From 06:00 to 09:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 09:00																					
09:00	7	37	3													9	18	6	0	33	175
09:15	9	49	2	0	60	3	22	15	0	40	7					16		11		36	150
09:30	10	41	3	0	54	8	31	15	0	54	3	16	2	0	21	11	13	9	0	33	162
09:45	11					15	48			80	6	15	1	0	22	10	13	5	0	28	178
Total Volume	37	161	11	0	209	27	139	78	0	244	22	52	8	0	82	46	53	31	0	130	665
% App. Total	17.7	77	5.3	0		11.1	57	32	0		26.8	63.4	9.8	0		35.4	40.8	23.8	0		
PHF	.841	.821	.917	.000	.871	.450	.724	.629	.000	.763	.786	.813	.667	.000	.820	.719	.736	.705	.000	.903	.934

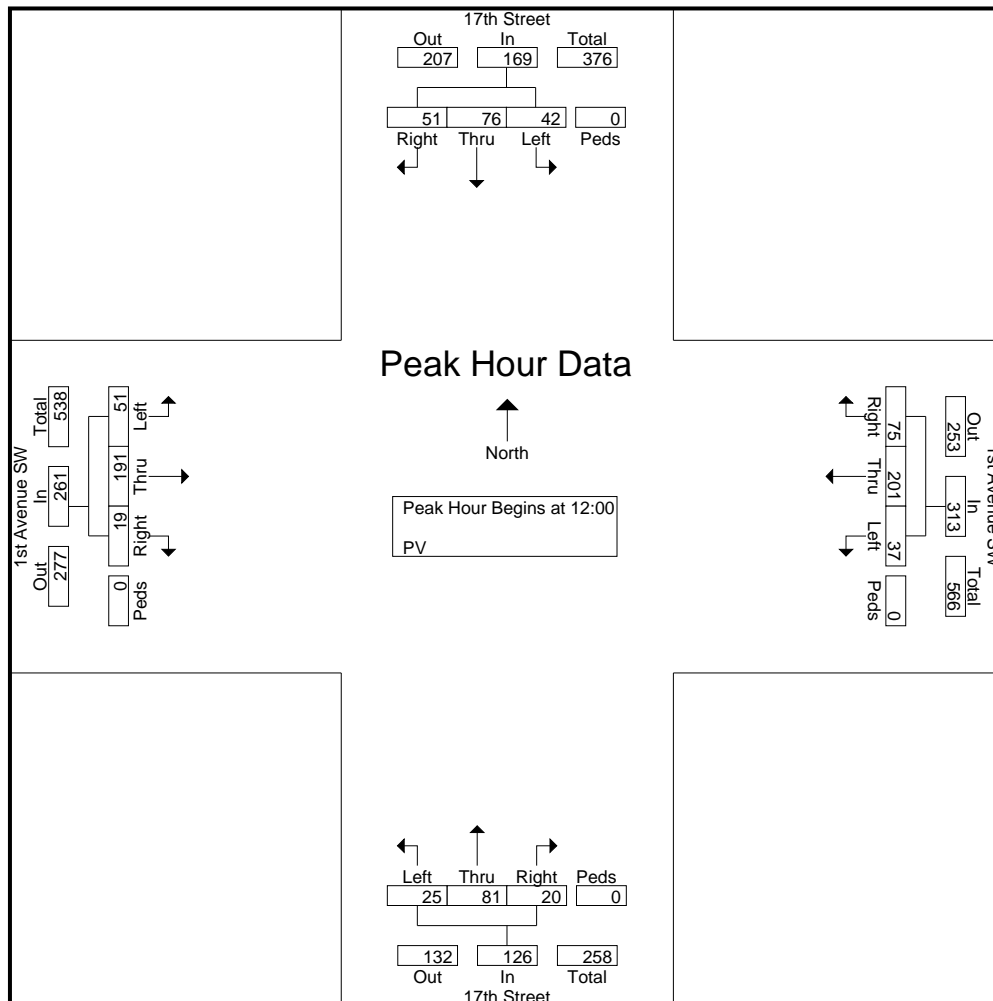


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 Traffic Engineering

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Start Time	1st Avenue SW Eastbound					1st Avenue SW Westbound					17th Street Northbound					17th Street Southbound					Int. Total					
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total						
Peak Hour Analysis From 10:00 to 13:45 - Peak 1 of 1																										
Peak Hour for Entire Intersection Begins at 12:00																										
12:00	7	41	7			55					25					36					16	16				
12:15	7	44	3	0	54	13	48	21	0	82	6	15	10	0	31	6	20	6	0	32	199					
12:30	17	55	2	0	74	14	43	11	0	68	9															
12:45	20	51	7	0	78	2	55	25	0	82	5	21	1	0	27	14	22	14	0	50	237					
Total Volume	51	191	19	0	261	37	201	75	0	313	25	81	20	0	126	42	76	51	0	169	869					
% App. Total	19.5	73.2	7.3	0		11.8	64.2	24	0		19.8	64.3	15.9	0		24.9	45	30.2	0							
PHF	.638	.868	.679	.000	.837	.661	.914	.750	.000	.954	.694	.810	.500	.000	.875	.656	.864	.797	.000	.845	.917					

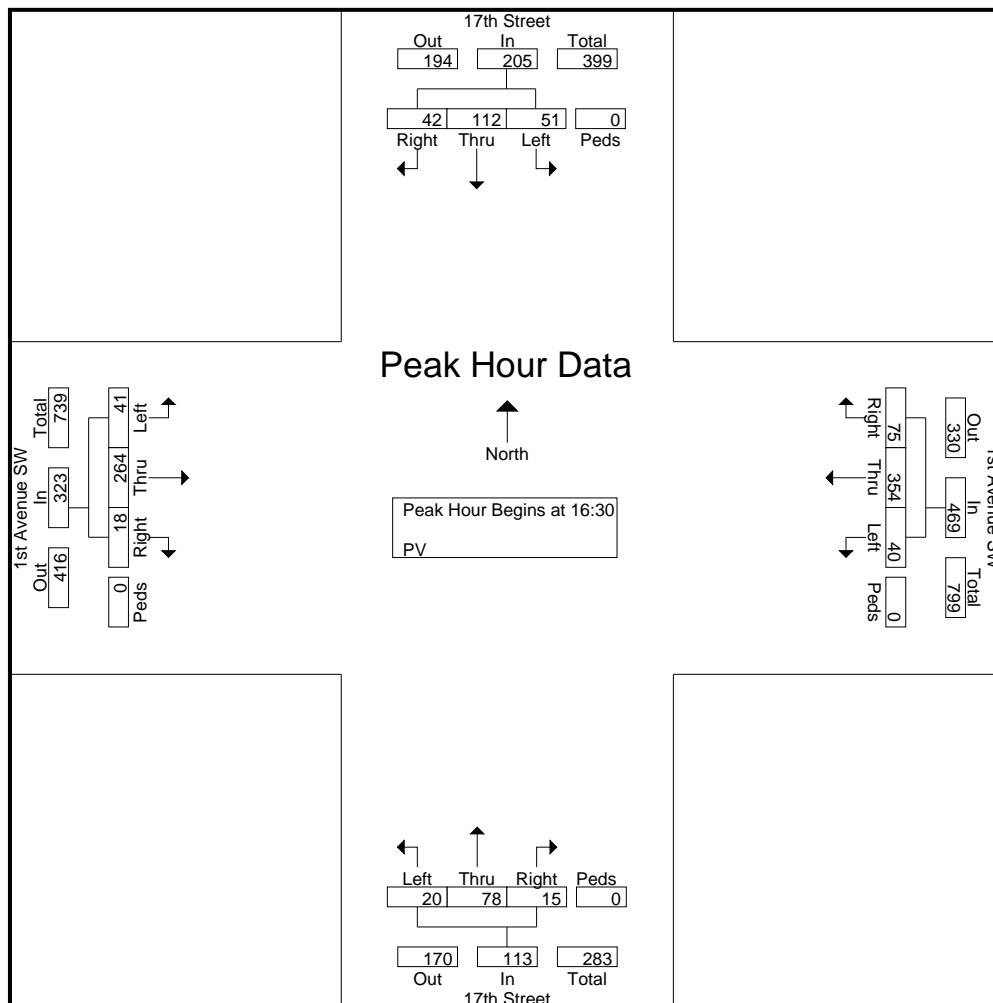


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701 Corporate Center Drive
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Site Code : 17-06132
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





















Start Time	1st Avenue SW Eastbound					1st Avenue SW Westbound					17th Street Northbound					17th Street Southbound					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Peak Hour Analysis From 14:00 to 18:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 16:30																					
16:30	11	79	7	0	97	9	86	29	0	115	9	24	9	0	40	22	32	14	0	68	329
16:45	9	43	1	0	53	8	91	16	0	115	2	24	2	0	24	7	25	9	0	41	288
17:00	11	64	8	0	83	14	109	17	0	140	6	16	2	0	24	13	25	11	0	49	250
17:15	10	78	2	0	90	9	68	13	0	90	3	16	2	0	21	13	25	11	0	49	250
Total Volume	41	264	18	0	323	40	354	75	0	469	20	78	15	0	113	51	112	42	0	205	1110
% App. Total	12.7	81.7	5.6	0		8.5	75.5	16	0		17.7	69	13.3	0		24.9	54.6	20.5	0		
PHF	.932	.835	.563	.000	.832	.714	.812	.647	.000	.838	.556	.813	.417	.000	.706	.580	.875	.750	.000	.754	.843



APPENDIX C
2040 No-Build Alternative Synchro & SimTraffic Reports

Lanes, Volumes, Timings
1: US 321 & 13th Street SW

NCDOT TIP-U4700

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	127	37	16	98	20	68	9	1407	104	90	1759	92
Future Volume (vph)	127	37	16	98	20	68	9	1407	104	90	1759	92
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		200	200		1000	250		400
Storage Lanes	1		0	1		1	1		1	1		2
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.91	0.91	0.95	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.91	1.00
Frt		0.979				0.850			0.850			0.850
Flt Protected	0.950	0.973		0.950			0.950			0.950		
Satd. Flow (prot)	1564	3137	0	3335	1810	1538	1719	3438	1538	1719	4940	1538
Flt Permitted	0.950	0.973		0.950			0.950			0.950		
Satd. Flow (perm)	1564	3137	0	3335	1810	1538	1719	3438	1538	1719	4940	1538
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		468			555			2913			1957	
Travel Time (s)		9.1			10.8			44.1			29.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	141	41	18	109	22	76	10	1563	116	100	1954	102
Shared Lane Traffic (%)	50%											
Lane Group Flow (vph)	70	130	0	109	22	76	10	1563	116	100	1954	102
Turn Type	Split	NA		Split	NA	Free	Prot	NA	Free	Prot	NA	Free
Protected Phases	8	8		4	4		5	2		1	6	
Permitted Phases						Free			Free			Free
Detector Phase	8	8		4	4		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0		7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0		14.0	19.0	
Total Split (s)	20.0	20.0		16.0	16.0		14.0	91.0		23.0	100.0	
Total Split (%)	13.3%	13.3%		10.7%	10.7%		9.3%	60.7%		15.3%	66.7%	
Maximum Green (s)	13.0	13.0		9.0	9.0		7.0	84.0		16.0	93.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)	11.3	11.3		9.0	9.0	150.0	7.5	88.4	150.0	13.3	105.3	150.0
Actuated g/C Ratio	0.08	0.08		0.06	0.06	1.00	0.05	0.59	1.00	0.09	0.70	1.00
v/c Ratio	0.60	0.55		0.54	0.20	0.05	0.12	0.77	0.08	0.66	0.56	0.07
Control Delay	87.6	75.7		78.8	71.4	0.1	70.4	27.4	0.1	83.6	5.3	0.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	87.6	75.7		78.8	71.4	0.1	70.4	27.4	0.1	83.6	5.3	0.0
LOS	F	E		E	E	A	E	C	A	F	A	A
Approach Delay		79.9			49.1			25.8			8.6	

Lanes, Volumes, Timings
 1: US 321 & 13th Street SW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Approach LOS	E			D			C			A			
Queue Length 50th (ft)	73	68		54	21	0	10	611	0	90	49	0	
Queue Length 95th (ft)	132	106		88	52	0	30	732	0	m87	m304	m0	
Internal Link Dist (ft)	388			475			2833			1877			
Turn Bay Length (ft)	300			150		200		200		1000		250	
Base Capacity (vph)	135	271		208	113	1538	86	2028	1538	183	3486	1538	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.52	0.48		0.52	0.19	0.05	0.12	0.77	0.08	0.55	0.56	0.07	

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 116 (77%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 20.8
 Intersection LOS: C
 Intersection Capacity Utilization 72.4%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: US 321 & 13th Street SW



Lanes, Volumes, Timings
 2: US 321 & Main Avenue Drive NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Traffic Volume (vph)	20	4	65	13	4	15	112	1487	21	30	1843	42
Future Volume (vph)	20	4	65	13	4	15	112	1487	21	30	1843	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	200		0	150		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.901			0.934			0.998			0.997	
Flt Protected		0.989			0.980		0.950			0.950		
Satd. Flow (prot)	0	1644	0	0	1656	0	1719	3431	0	1719	3428	0
Flt Permitted		0.989			0.980		0.950			0.950		
Satd. Flow (perm)	0	1644	0	0	1656	0	1719	3431	0	1719	3428	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		892			860			1957			1105	
Travel Time (s)		24.3			23.5			29.7			16.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	22	4	72	14	4	17	124	1652	23	33	2048	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	98	0	0	35	0	124	1675	0	33	2095	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	74.4%
Analysis Period (min)	15
	ICU Level of Service D

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	20	4	65	13	4	15	112	1487	21	30	1843	42
Future Vol, veh/h	20	4	65	13	4	15	112	1487	21	30	1843	42
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	200	-	-	150	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	5	5	5	5	5	5	5	5	5
Mvmt Flow	22	4	72	14	4	17	124	1652	23	33	2048	47

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	3215	4062	1047	3006	4074	838	2094	0	0	1676	0	0
Stage 1	2138	2138	-	1913	1913	-	-	-	-	-	-	-
Stage 2	1077	1924	-	1093	2161	-	-	-	-	-	-	-
Critical Hdwy	7.56	6.56	6.96	7.6	6.6	7	4.2	-	-	4.2	-	-
Critical Hdwy Stg 1	6.56	5.56	-	6.6	5.6	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.56	5.56	-	6.6	5.6	-	-	-	-	-	-	-
Follow-up Hdwy	3.53	4.03	3.33	3.55	4.05	3.35	2.25	-	-	2.25	-	-
Pot Cap-1 Maneuver	~ 4	~ 3	223	~ 6	~ 2	303	249	-	-	365	-	-
Stage 1	50	87	-	68	110	-	-	-	-	-	-	-
Stage 2	232	111	-	223	82	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	-	~ 1	223	-	~ 1	303	249	-	-	365	-	-
Mov Cap-2 Maneuver	-	~ 1	-	-	~ 1	-	-	-	-	-	-	-
Stage 1	25	79	-	34	55	-	-	-	-	-	-	-
Stage 2	101	56	-	129	75	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s			2.3	0.2
HCM LOS	-	-		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	249	-	-	-	365	-	-
HCM Lane V/C Ratio	0.5	-	-	-	0.091	-	-
HCM Control Delay (s)	33.1	-	-	-	15.9	-	-
HCM Lane LOS	D	-	-	-	C	-	-
HCM 95th %tile Q(veh)	2.6	-	-	-	0.3	-	-

Notes
 -: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lanes, Volumes, Timings
3: US 321 & 2nd Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	60	132	96	76	114	52	124	1333	115	98	1659	97
Future Volume (vph)	60	132	96	76	114	52	124	1333	115	98	1659	97
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	250		500	400		0
Storage Lanes	1		0	0		0	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Frt		0.937			0.968				0.850		0.992	
Flt Protected	0.950				0.985		0.950			0.950		
Satd. Flow (prot)	1752	1728	0	0	3342	0	1719	3438	1538	1719	3411	0
Flt Permitted	0.950				0.985		0.950			0.950		
Satd. Flow (perm)	1752	1728	0	0	3342	0	1719	3438	1538	1719	3411	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		627			328			1105			2513	
Travel Time (s)		12.2			6.4			16.7			38.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	67	147	107	84	127	58	138	1481	128	109	1843	108
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	254	0	0	269	0	138	1481	128	109	1951	0
Turn Type	Split	NA		Split	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	8	8		4	4		5	2	4	1	6	
Permitted Phases									2			
Detector Phase	8	8		4	4		5	2	4	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0	7.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0	14.0	14.0	19.0	
Total Split (s)	27.0	27.0		18.0	18.0		18.0	85.0	18.0	20.0	87.0	
Total Split (%)	18.0%	18.0%		12.0%	12.0%		12.0%	56.7%	12.0%	13.3%	58.0%	
Maximum Green (s)	20.0	20.0		11.0	11.0		11.0	78.0	11.0	13.0	80.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0			0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0			7.0		7.0	7.0	7.0	7.0	7.0	
Lead/Lag							Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min	None	None	C-Min	
Act Effect Green (s)	20.0	20.0			11.0		11.0	74.3	85.3	16.7	80.0	
Actuated g/C Ratio	0.13	0.13			0.07		0.07	0.50	0.57	0.11	0.53	
v/c Ratio	0.29	1.10			1.10		1.10	0.87	0.15	0.57	1.07	
Control Delay	62.3	147.7			147.8		161.5	28.1	4.2	65.0	67.8	
Queue Delay	0.0	0.0			0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	62.3	147.7			147.8		161.5	28.1	4.2	65.0	67.8	
LOS	E	F			F		F	C	A	E	E	
Approach Delay		129.9			147.8			36.9			67.7	

Lanes, Volumes, Timings
 3: US 321 & 2nd Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS		F			F			D				E
Queue Length 50th (ft)	60	-282			-156		-151	723	25	99	-1101	
Queue Length 95th (ft)	111	#464			#254		m#245	397	m7	m128	#1235	
Internal Link Dist (ft)		547			248			1025				2433
Turn Bay Length (ft)							250		500	400		
Base Capacity (vph)	233	230			245		126	1787	874	191	1819	
Starvation Cap Reductn	0	0			0		0	0	0	0	0	0
Spillback Cap Reductn	0	0			0		0	0	0	0	0	0
Storage Cap Reductn	0	0			0		0	0	0	0	0	0
Reduced v/c Ratio	0.29	1.10			1.10		1.10	0.83	0.15	0.57	1.07	

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 32 (21%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 170
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.10
 Intersection Signal Delay: 64.9
 Intersection LOS: E
 Intersection Capacity Utilization 99.0%
 ICU Level of Service F
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: US 321 & 2nd Avenue NW



Lanes, Volumes, Timings
4: US 321 & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↖	↗		↖	↕	↗	↖	↕	↗
Traffic Volume (vph)	7	54	36	57	135	7	70	1394	45	7	1724	17
Future Volume (vph)	7	54	36	57	135	7	70	1394	45	7	1724	17
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	150		0	100		200	150		0
Storage Lanes	0		0	1		0	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	0.95
Frt		0.950			0.992				0.850			0.999
Flt Protected		0.996		0.950			0.950			0.950		
Satd. Flow (prot)	0	1745	0	1671	1745	0	1719	3438	1538	1736	3468	0
Flt Permitted		0.905		0.555			0.950			0.950		
Satd. Flow (perm)	0	1586	0	976	1745	0	1719	3438	1538	1736	3468	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		406			154			2513			1038	
Travel Time (s)		7.9			3.0			38.1			15.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	8%	8%	8%	5%	5%	5%	4%	4%	4%
Adj. Flow (vph)	8	60	40	63	150	8	78	1549	50	8	1916	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	108	0	63	158	0	78	1549	50	8	1935	0
Turn Type	Perm	NA		Perm	NA		Prot	NA	Perm	Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8					2			
Detector Phase	4	4		8	8		5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0	12.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0	19.0	14.0	19.0	
Total Split (s)	28.0	28.0		28.0	28.0		18.0	108.0	108.0	14.0	104.0	
Total Split (%)	18.7%	18.7%		18.7%	18.7%		12.0%	72.0%	72.0%	9.3%	69.3%	
Maximum Green (s)	21.0	21.0		21.0	21.0		11.0	101.0	101.0	7.0	97.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		7.0		7.0	7.0		7.0	7.0	7.0	7.0	7.0	
Lead/Lag							Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min	C-Min	None	C-Min	
Act Effect Green (s)		17.8		17.8	17.8		10.4	115.4	115.4	7.0	100.8	
Actuated g/C Ratio		0.12		0.12	0.12		0.07	0.77	0.77	0.05	0.67	
v/c Ratio		0.57		0.55	0.76		0.66	0.59	0.04	0.10	0.83	
Control Delay		74.2		79.3	87.0		57.5	3.8	1.6	75.3	19.0	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.2	
Total Delay		74.2		79.3	87.0		57.5	3.8	1.6	75.3	19.2	
LOS		E		E	F		E	A	A	E	B	
Approach Delay		74.2			84.8			6.2			19.4	

Lanes, Volumes, Timings
4: US 321 & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS		E			F			A				B
Queue Length 50th (ft)		101		59	151		71	12	1	8		418
Queue Length 95th (ft)		166		110	230		m90	m59	m4	m9		452
Internal Link Dist (ft)		326			74			2433				958
Turn Bay Length (ft)				150			100		200	150		
Base Capacity (vph)		222		136	244		129	2644	1183	81		2329
Starvation Cap Reductn		0		0	0		0	0	0	0		49
Spillback Cap Reductn		0		0	0		0	0	0	0		0
Storage Cap Reductn		0		0	0		0	0	0	0		0
Reduced v/c Ratio		0.49		0.46	0.65		0.60	0.59	0.04	0.10		0.85

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 26 (17%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 19.0 Intersection LOS: B
 Intersection Capacity Utilization 81.0% ICU Level of Service D
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: US 321 & 7th Avenue NW



Lanes, Volumes, Timings
5: US 321 & 9th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	53	4	39	56	4	53	131	1299	39	45	1580	216
Future Volume (vph)	53	4	39	56	4	53	131	1299	39	45	1580	216
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		0	150		0	100		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.863			0.860			0.996			0.982	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1592	0	1736	1571	0	1736	3457	0	1736	3409	0
Flt Permitted	0.716			0.726			0.950			0.950		
Satd. Flow (perm)	1321	1592	0	1326	1571	0	1736	3457	0	1736	3409	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		346			350			1038			963	
Travel Time (s)		6.7			6.8			15.7			14.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	59	4	43	62	4	59	146	1443	43	50	1756	240
Shared Lane Traffic (%)												
Lane Group Flow (vph)	59	47	0	62	63	0	146	1486	0	50	1996	0
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0		7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0		14.0	19.0	
Total Split (s)	18.0	18.0		18.0	18.0		25.0	115.0		17.0	107.0	
Total Split (%)	12.0%	12.0%		12.0%	12.0%		16.7%	76.7%		11.3%	71.3%	
Maximum Green (s)	11.0	11.0		11.0	11.0		18.0	108.0		10.0	100.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)	10.5	10.5		10.5	10.5		16.3	112.4		9.0	102.3	
Actuated g/C Ratio	0.07	0.07		0.07	0.07		0.11	0.75		0.06	0.68	
v/c Ratio	0.64	0.43		0.67	0.58		0.78	0.57		0.49	0.86	
Control Delay	97.7	78.6		100.8	88.3		73.0	2.8		96.9	7.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	97.7	78.6		100.8	88.3		73.0	2.8		96.9	7.7	
LOS	F	E		F	F		E	A		F	A	
Approach Delay		89.2			94.5			9.1			9.9	

Lanes, Volumes, Timings
5: US 321 & 9th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS		F			F			A			A	
Queue Length 50th (ft)	56	44		59	60		151	84		50	157	
Queue Length 95th (ft)	#123	90		#131	114		#238	11		m66	m158	
Internal Link Dist (ft)		266			270			958			883	
Turn Bay Length (ft)	300			150			150			100		
Base Capacity (vph)	99	119		100	118		208	2595		115	2331	
Starvation Cap Reductn	0	0		0	0		0	0		0	8	
Spillback Cap Reductn	0	0		0	0		0	0		0	5	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.60	0.39		0.62	0.53		0.70	0.57		0.43	0.86	

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 46 (31%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 14.4 Intersection LOS: B
 Intersection Capacity Utilization 85.1% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: US 321 & 9th Avenue NW



Lanes, Volumes, Timings
6: US 321 & Clement Boulevard

NCDOT TIP-U4700

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	86	82	51	135	137	660	39	1430	61	323	1537	70
Future Volume (vph)	86	82	51	135	137	660	39	1430	61	323	1537	70
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	600		0	300		0	400		250	600		0
Storage Lanes	1		1	1		1	1		1	2		0
Taper Length (ft)	100			100			100			200		
Lane Util. Factor	0.95	0.95	1.00	0.95	0.95	1.00	1.00	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.993	
Flt Protected	0.950	0.995		0.950	0.996		0.950			0.950		
Satd. Flow (prot)	1665	1744	1568	1681	1763	1583	1719	3438	1538	3367	3447	0
Flt Permitted	0.950	0.995		0.950	0.996		0.950			0.950		
Satd. Flow (perm)	1665	1744	1568	1681	1763	1583	1719	3438	1538	3367	3447	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		915			441			963			1050	
Travel Time (s)		17.8			8.6			14.6			15.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	5%	5%	5%	4%	4%	4%
Adj. Flow (vph)	96	91	57	150	152	733	43	1589	68	359	1708	78
Shared Lane Traffic (%)	10%			10%								
Lane Group Flow (vph)	86	101	57	135	167	733	43	1589	68	359	1786	0
Turn Type	Split	NA	pm+ov	Split	NA	pm+ov	Prot	NA	Perm	Prot	NA	
Protected Phases	4	4	5	8	8	1	5	2		1	6	
Permitted Phases			4			8			2			
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	12.0	7.0	12.0	
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	19.0	19.0	14.0	19.0	
Total Split (s)	14.0	14.0	15.0	18.0	18.0	47.0	15.0	71.0	71.0	47.0	103.0	
Total Split (%)	9.3%	9.3%	10.0%	12.0%	12.0%	31.3%	10.0%	47.3%	47.3%	31.3%	68.7%	
Maximum Green (s)	7.0	7.0	8.0	11.0	11.0	40.0	8.0	64.0	64.0	40.0	96.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Lead/Lag			Lag			Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?			Yes			Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	
Act Effect Green (s)	7.0	7.0	20.7	11.0	11.0	58.0	8.1	64.0	64.0	40.0	98.7	
Actuated g/C Ratio	0.05	0.05	0.14	0.07	0.07	0.39	0.05	0.43	0.43	0.27	0.66	
v/c Ratio	1.12	1.25	0.27	1.10	1.29	1.20	0.47	1.08	0.10	0.40	0.79	
Control Delay	199.0	234.7	59.4	170.9	229.7	144.4	68.3	77.8	18.6	43.7	12.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	199.0	234.7	59.4	170.9	229.7	144.4	68.3	77.8	18.6	43.7	12.5	
LOS	F	F	E	F	F	F	E	E	B	D	B	
Approach Delay		181.2			161.6			75.2			17.7	

Lanes, Volumes, Timings
6: US 321 & Clement Boulevard

NCDOT TIP-U4700

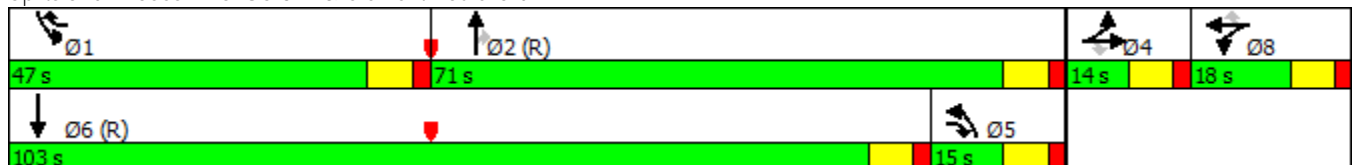


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS	F			F			E			B		
Queue Length 50th (ft)	~101	~128	50	~156	~218	~866	39	~898	23	126	341	
Queue Length 95th (ft)	#226	#263	96	#308	#383	#1116	m73	#1044	m50	m136	m429	
Internal Link Dist (ft)	835			361			883			970		
Turn Bay Length (ft)	600			300			400			250	600	
Base Capacity (vph)	77	81	217	123	129	612	94	1466	656	897	2275	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	4
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.12	1.25	0.26	1.10	1.29	1.20	0.46	1.08	0.10	0.40	0.79	

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 74 (49%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 170
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.29
 Intersection Signal Delay: 73.6
 Intersection LOS: E
 Intersection Capacity Utilization 103.7%
 ICU Level of Service G
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: US 321 & Clement Boulevard



Lanes, Volumes, Timings
7: US 321 & 13th Avenue Drive NW

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	46	4	1683	2486	31
Future Volume (vph)	0	46	4	1683	2486	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	1	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Frt		0.865			0.998	
Flt Protected			0.950			
Satd. Flow (prot)	0	1596	1736	3471	3431	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	1596	1736	3471	3431	0
Link Speed (mph)	35			45	45	
Link Distance (ft)	250			1050	1732	
Travel Time (s)	4.9			15.9	26.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	4%	4%	5%	5%
Adj. Flow (vph)	0	51	4	1870	2762	34
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	51	4	1870	2796	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	79.7% ICU Level of Service D
Analysis Period (min)	15

Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗	↘ ↗	↗ ↗	↗ ↗	
Traffic Vol, veh/h	0	46	4	1683	2486	31
Future Vol, veh/h	0	46	4	1683	2486	31
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	4	4	5	5
Mvmt Flow	0	51	4	1870	2762	34

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	1398	2797 0
Stage 1	-	-	- -
Stage 2	-	-	- -
Critical Hdwy	-	6.96	4.18 -
Critical Hdwy Stg 1	-	-	- -
Critical Hdwy Stg 2	-	-	- -
Follow-up Hdwy	-	3.33	2.24 -
Pot Cap-1 Maneuver	0	129	132 -
Stage 1	0	-	- -
Stage 2	0	-	- -
Platoon blocked, %			- -
Mov Cap-1 Maneuver	-	129	132 -
Mov Cap-2 Maneuver	-	-	- -
Stage 1	-	-	- -
Stage 2	-	-	- -

Approach	EB	NB	SB
HCM Control Delay, s	50.1	0.1	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	132	-	129	-	-
HCM Lane V/C Ratio	0.034	-	0.396	-	-
HCM Control Delay (s)	33.2	-	50.1	-	-
HCM Lane LOS	D	-	F	-	-
HCM 95th %tile Q(veh)	0.1	-	1.7	-	-

Lanes, Volumes, Timings
 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	13	7	9	13	7	166	15	1628	21	393	2387	30
Future Volume (vph)	13	7	9	13	7	166	15	1628	21	393	2387	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		250	0		100	300		150	900		200
Storage Lanes	1		1	0		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950				0.969		0.950			0.950		
Satd. Flow (prot)	1719	1810	1538	0	1805	1583	1736	3471	1553	1736	3471	1553
Flt Permitted	0.950				0.969		0.950			0.950		
Satd. Flow (perm)	1719	1810	1538	0	1805	1583	1736	3471	1553	1736	3471	1553
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		606			480			1732			3358	
Travel Time (s)		11.8			9.4			26.2			50.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	2%	2%	2%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	14	8	10	14	8	184	17	1809	23	437	2652	33
Shared Lane Traffic (%)												
Lane Group Flow (vph)	14	8	10	0	22	184	17	1809	23	437	2652	33
Turn Type	Split	NA	pm+ov	Split	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	5	8	8	1	5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	12.0	7.0	12.0	12.0
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	19.0	19.0	14.0	19.0	19.0
Total Split (s)	14.0	14.0	14.0	14.0	14.0	42.0	14.0	80.0	80.0	42.0	108.0	108.0
Total Split (%)	9.3%	9.3%	9.3%	9.3%	9.3%	28.0%	9.3%	53.3%	53.3%	28.0%	72.0%	72.0%
Maximum Green (s)	7.0	7.0	7.0	7.0	7.0	35.0	7.0	73.0	73.0	35.0	101.0	101.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Lead/Lag			Lead			Lag	Lead	Lead	Lead	Lag	Lag	Lag
Lead-Lag Optimize?			Yes			Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	C-Min
Act Effect Green (s)	7.0	7.0	11.2		7.0	39.2	7.0	84.2	84.2	35.0	120.6	120.6
Actuated g/C Ratio	0.05	0.05	0.07		0.05	0.26	0.05	0.56	0.56	0.23	0.80	0.80
v/c Ratio	0.17	0.10	0.09		0.26	0.45	0.21	0.93	0.03	1.08	0.95	0.03
Control Delay	74.0	71.1	48.4		77.0	39.2	87.7	19.9	10.6	91.6	17.9	4.5
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.0	71.1	48.4		77.0	39.2	87.7	19.9	10.6	91.6	17.9	4.5
LOS	E	E	D		E	D	F	B	B	F	B	A
Approach Delay		65.3			43.2			20.4			28.1	

Lanes, Volumes, Timings
 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

NCDOT TIP-U4700

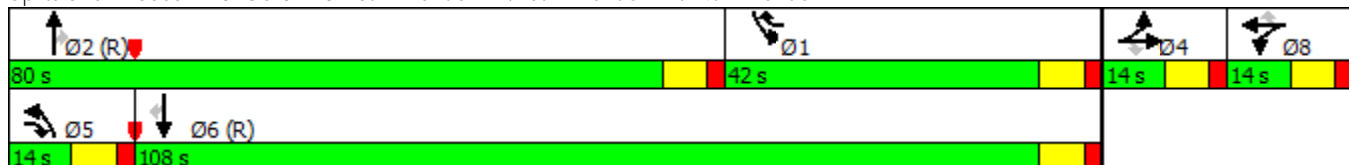


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS	E			D			C			C		
Queue Length 50th (ft)	13	8	8		21	117	17	-1027	7	-476	-1580	9
Queue Length 95th (ft)	39	27	25		53	178	m15	m630	m7	m#463	m#1541	m6
Internal Link Dist (ft)		526			400			1652			3278	
Turn Bay Length (ft)	250		250			100	300		150	900		200
Base Capacity (vph)	80	84	114		84	413	81	1948	871	405	2791	1248
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.10	0.09		0.26	0.45	0.21	0.93	0.03	1.08	0.95	0.03

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 122 (81%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 180
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.08
 Intersection Signal Delay: 26.2 Intersection LOS: C
 Intersection Capacity Utilization 97.1% ICU Level of Service F
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW



Lanes, Volumes, Timings
9: US 321 & Grace Chapel Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	463	33	4	1601	200	21	2345
Future Volume (vph)	463	33	4	1601	200	21	2345
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	250	300		300	400	
Storage Lanes	1	1	1		1	1	
Taper Length (ft)	100		100			100	
Lane Util. Factor	0.97	1.00	1.00	0.95	1.00	1.00	0.95
Frt		0.850			0.850		
Flt Protected	0.950		0.950			0.950	
Satd. Flow (prot)	3433	1583	1736	3471	1553	1736	3471
Flt Permitted	0.950		0.950			0.950	
Satd. Flow (perm)	3433	1583	1736	3471	1553	1736	3471
Right Turn on Red		No			No		
Satd. Flow (RTOR)							
Link Speed (mph)	45			55			55
Link Distance (ft)	983			1234			3568
Travel Time (s)	14.9			15.3			44.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%	4%
Adj. Flow (vph)	514	37	4	1779	222	23	2606
Shared Lane Traffic (%)							
Lane Group Flow (vph)	514	37	4	1779	222	23	2606
Turn Type	Prot	Perm	Prot	NA	Free	Prot	NA
Protected Phases	8		5	2		1	6
Permitted Phases		8			Free		
Detector Phase	8	8	5	2		1	6
Switch Phase							
Minimum Initial (s)	7.0	7.0	7.0	14.0		7.0	14.0
Minimum Split (s)	14.0	14.0	14.0	21.0		14.0	21.0
Total Split (s)	27.0	27.0	14.0	109.0		14.0	109.0
Total Split (%)	18.0%	18.0%	9.3%	72.7%		9.3%	72.7%
Maximum Green (s)	20.0	20.0	7.0	102.0		7.0	102.0
Yellow Time (s)	5.0	5.0	5.0	5.0		5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0		2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0		7.0	7.0
Lead/Lag			Lead	Lead		Lag	Lag
Lead-Lag Optimize?			Yes	Yes		Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	C-Min		None	C-Min
Act Effect Green (s)	20.0	20.0	7.0	107.6	150.0	7.0	113.2
Actuated g/C Ratio	0.13	0.13	0.05	0.72	1.00	0.05	0.75
v/c Ratio	1.12	0.18	0.05	0.71	0.14	0.28	1.00
Control Delay	137.7	60.2	101.8	4.3	0.1	60.0	23.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	137.7	60.2	101.8	4.3	0.1	60.0	23.2
LOS	F	E	F	A	A	E	C
Approach Delay	132.5			4.0			23.5

Lanes, Volumes, Timings
 9: US 321 & Grace Chapel Road

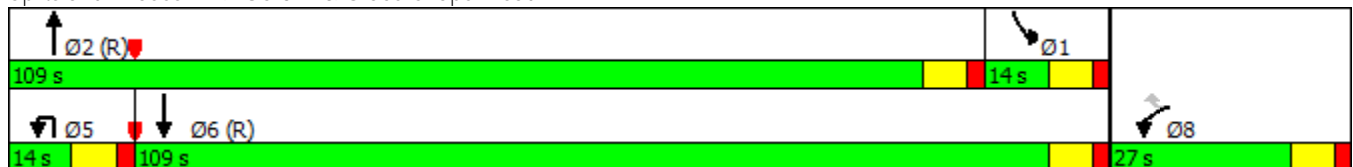


Lane Group	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Approach LOS	F			A		C	
Queue Length 50th (ft)	~297	33	4	15	0	22	132
Queue Length 95th (ft)	#416	70	m5	39	m0	m27	#1631
Internal Link Dist (ft)	903			1154		3488	
Turn Bay Length (ft)	250	250	300	300		400	
Base Capacity (vph)	457	211	81	2490	1553	81	2619
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.12	0.18	0.05	0.71	0.14	0.28	1.00

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 28 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 180
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.12
 Intersection Signal Delay: 27.5
 Intersection LOS: C
 Intersection Capacity Utilization 89.7%
 ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
10: US 321 & Alex Lee Blvd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖	↗		↖	↗	↗	↑↑	↗	↗	↑↔	
Traffic Volume (vph)	4	4	4	90	5	16	13	1352	74	38	2029	7
Future Volume (vph)	4	4	4	90	5	16	13	1352	74	38	2029	7
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	200		350	450		0
Storage Lanes	0		1	0		1	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	0.95
Frt			0.850			0.850			0.850		0.999	
Flt Protected		0.976			0.955		0.950			0.950		
Satd. Flow (prot)	0	1766	1538	0	1227	1442	1736	3471	1442	1612	3501	0
Flt Permitted		0.870			0.733		0.950			0.950		
Satd. Flow (perm)	0	1574	1538	0	942	1442	1736	3471	1442	1612	3501	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		25			35			55			55	
Link Distance (ft)		264			676			3568			1233	
Travel Time (s)		7.2			13.2			44.2			15.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	50%	12%	12%	4%	4%	12%	12%	3%	3%
Adj. Flow (vph)	4	4	4	100	6	18	14	1502	82	42	2254	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	8	4	0	106	18	14	1502	82	42	2262	0
Turn Type	Perm	NA	pm+ov	Perm	NA	pm+ov	Prot	NA	Perm	Prot	NA	
Protected Phases		4	5		8	1	5	2		1	6	
Permitted Phases	4		4	8		8			2			
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	14.0	14.0	7.0	14.0	
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	21.0	21.0	14.0	21.0	
Total Split (s)	26.0	26.0	14.0	26.0	26.0	16.0	14.0	108.0	108.0	16.0	110.0	
Total Split (%)	17.3%	17.3%	9.3%	17.3%	17.3%	10.7%	9.3%	72.0%	72.0%	10.7%	73.3%	
Maximum Green (s)	19.0	19.0	7.0	19.0	19.0	9.0	7.0	101.0	101.0	9.0	103.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Lead/Lag			Lead			Lead	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	
Act Effect Green (s)		19.1	33.1		19.1	34.4	7.0	104.4	104.4	8.3	108.5	
Actuated g/C Ratio		0.13	0.22		0.13	0.23	0.05	0.70	0.70	0.06	0.72	
v/c Ratio		0.04	0.01		0.89	0.05	0.17	0.62	0.08	0.47	0.89	
Control Delay		58.3	46.0		119.8	45.3	59.1	28.7	15.7	80.3	11.6	
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		58.3	46.0		119.8	45.3	59.1	28.7	15.7	80.3	11.6	
LOS		E	D		F	D	E	C	B	F	B	
Approach Delay		54.2			109.0			28.3			12.9	

Lanes, Volumes, Timings
 10: US 321 & Alex Lee Blvd

NCDOT TIP-U4700

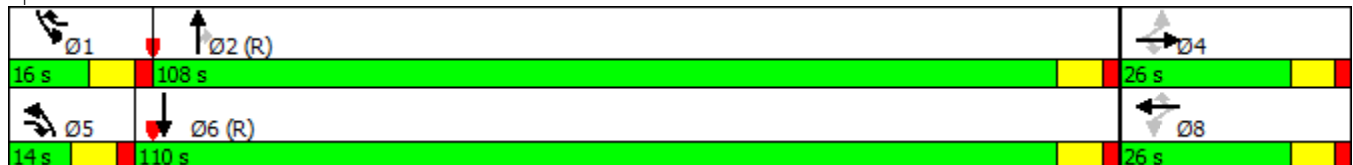


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS		D			F			C			B	
Queue Length 50th (ft)		7	3		104	14	13	593	50	39	335	
Queue Length 95th (ft)		25	14		#224	37	m18	647	m66	m49	373	
Internal Link Dist (ft)		184			596			3488			1153	
Turn Bay Length (ft)							200		350	450		
Base Capacity (vph)		206	339		123	337	81	2425	1007	96	2533	
Starvation Cap Reductn		0	0		0	0	0	0	0	0	0	
Spillback Cap Reductn		0	0		0	0	0	0	0	0	0	
Storage Cap Reductn		0	0		0	0	0	0	0	0	0	
Reduced v/c Ratio		0.04	0.01		0.86	0.05	0.17	0.62	0.08	0.44	0.89	

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 136 (91%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.89
 Intersection Signal Delay: 22.0
 Intersection LOS: C
 Intersection Capacity Utilization 85.5%
 ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: US 321 & Alex Lee Blvd



Lanes, Volumes, Timings
 11: US 321 & Pooveys Grove Church Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↖	↕	↗		↕
Traffic Volume (vph)	0	4	1552	4	0	2345
Future Volume (vph)	0	4	1552	4	0	2345
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.865				
Flt Protected						
Satd. Flow (prot)	0	1611	3505	0	0	3505
Flt Permitted						
Satd. Flow (perm)	0	1611	3505	0	0	3505
Link Speed (mph)	35		55			55
Link Distance (ft)	444		1233			1822
Travel Time (s)	8.6		15.3			22.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	3%	3%
Adj. Flow (vph)	0	4	1724	4	0	2606
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	4	1728	0	0	2606
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	68.2%
Analysis Period (min)	15
	ICU Level of Service C

Intersection

Int Delay, s/veh 0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↕↗			↕↗
Traffic Vol, veh/h	0	4	1552	4	0	2345
Future Vol, veh/h	0	4	1552	4	0	2345
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Free	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	0	4	1724	4	0	2606





























Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	-	862	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.94	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.32	-
Pot Cap-1 Maneuver	0	298	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	298	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	17.3	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBTWBLn1	SBT
Capacity (veh/h)	- 298	-
HCM Lane V/C Ratio	- 0.015	-
HCM Control Delay (s)	- 17.3	-
HCM Lane LOS	- C	-
HCM 95th %tile Q(veh)	- 0	-

Lanes, Volumes, Timings
 12: US 321 & US 321 A/Riverbend Rd

NCDOT TIP-U4700

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				 				 		 	 	
Traffic Volume (vph)	8	164	177	401	171	376	119	937	260	389	1492	8
Future Volume (vph)	8	164	177	401	171	376	119	937	260	389	1492	8
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		100	0		0	250		200	350		200
Storage Lanes	1		1	2		1	1		1	2		1
Taper Length (ft)	100			100			100			200		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1845	1568	3433	1863	1583	1736	3471	1553	3367	3471	1553
Flt Permitted	0.638			0.950			0.950			0.950		
Satd. Flow (perm)	1177	1845	1568	3433	1863	1583	1736	3471	1553	3367	3471	1553
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			55			55	
Link Distance (ft)		491			484			1822			1161	
Travel Time (s)		9.6			9.4			22.6			14.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	9	182	197	446	190	418	132	1041	289	432	1658	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	9	182	197	446	190	418	132	1041	289	432	1658	9
Turn Type	Perm	NA	Free	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	Free
Protected Phases		4		3	8	1	5	2	3	1	6	
Permitted Phases	4		Free			8			2			Free
Detector Phase	4	4		3	8	1	5	2	3	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	14.0	7.0	7.0	14.0	
Minimum Split (s)	14.0	14.0		14.0	14.0	14.0	14.0	21.0	14.0	14.0	21.0	
Total Split (s)	21.0	21.0		26.0	47.0	32.0	20.0	71.0	26.0	32.0	83.0	
Total Split (%)	14.0%	14.0%		17.3%	31.3%	21.3%	13.3%	47.3%	17.3%	21.3%	55.3%	
Maximum Green (s)	14.0	14.0		19.0	40.0	25.0	13.0	64.0	19.0	25.0	76.0	
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	-2.0		-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	0.0	
Total Lost Time (s)	7.0	5.0		5.0	5.0	5.0	7.0	7.0	5.0	5.0	7.0	
Lead/Lag	Lag	Lag		Lead		Lead	Lag	Lag	Lead	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes		Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Min	None	None	C-Min	
Act Effct Green (s)	14.5	16.5	150.0	21.3	42.8	73.1	12.9	64.9	93.2	25.3	75.3	150.0
Actuated g/C Ratio	0.10	0.11	1.00	0.14	0.29	0.49	0.09	0.43	0.62	0.17	0.50	1.00
v/c Ratio	0.08	0.90	0.13	0.91	0.36	0.54	0.89	0.69	0.30	0.76	0.95	0.01
Control Delay	64.1	107.2	0.2	87.2	45.4	29.9	93.7	21.4	10.8	68.7	48.6	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.1	107.2	0.2	87.2	45.4	29.9	93.7	21.4	10.8	68.7	48.6	0.0
LOS	E	F	A	F	D	C	F	C	B	E	D	A
Approach Delay		51.9			56.9			25.8			52.5	

Lanes, Volumes, Timings
 12: US 321 & US 321 A/Riverbend Rd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS	D			E			C			D		
Queue Length 50th (ft)	8	180	0	225	150	276	120	179	77	207	789	0
Queue Length 95th (ft)	28	#332	0	#327	225	382	#257	260	98	269	#968	0
Internal Link Dist (ft)	411			404			1742			1081		
Turn Bay Length (ft)	200	100		250			200	350	200		200	
Base Capacity (vph)	113	202	1568	488	531	789	150	1501	965	606	1758	1553
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.90	0.13	0.91	0.36	0.53	0.88	0.69	0.30	0.71	0.94	0.01

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 8 (5%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 45.6
 Intersection LOS: D
 Intersection Capacity Utilization 87.9%
 ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 12: US 321 & US 321 A/Riverbend Rd



Summary of All Intervals

Run Number	1	10	2	3	4	5	6
Start Time	6:50	6:50	6:50	6:50	6:50	6:50	6:50
End Time	8:00	8:00	8:00	8:00	8:00	8:00	8:00
Total Time (min)	70	70	70	70	70	70	70
Time Recorded (min)	60	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1	1
Vehs Entered	9603	9496	9536	9321	9606	9387	9539
Vehs Exited	9406	9422	9315	9081	9301	9193	9339
Starting Vehs	881	868	814	776	796	797	778
Ending Vehs	1078	942	1035	1016	1101	991	978
Denied Entry Before	22	9	1	42	26	8	10
Denied Entry After	233	289	208	326	342	267	300
Travel Distance (mi)	20256	20100	19884	19295	19929	19858	19700
Travel Time (hr)	1124.4	1126.4	1034.4	1033.0	1163.9	1057.9	1073.8
Total Delay (hr)	678.3	683.5	596.6	607.7	724.1	619.8	638.2
Total Stops	20857	19963	18876	17051	20186	19361	18213
Fuel Used (gal)	825.3	820.7	797.6	779.2	825.3	798.6	800.8

Summary of All Intervals

Run Number	7	8	9	Avg
Start Time	6:50	6:50	6:50	6:50
End Time	8:00	8:00	8:00	8:00
Total Time (min)	70	70	70	70
Time Recorded (min)	60	60	60	60
# of Intervals	2	2	2	2
# of Recorded Intervals	1	1	1	1
Vehs Entered	9349	9408	9600	9480
Vehs Exited	9192	9227	9316	9280
Starting Vehs	755	793	781	791
Ending Vehs	912	974	1065	1004
Denied Entry Before	16	33	48	19
Denied Entry After	270	268	306	277
Travel Distance (mi)	19530	19672	19868	19809
Travel Time (hr)	1051.6	1103.9	1126.1	1089.5
Total Delay (hr)	620.4	669.8	687.4	652.6
Total Stops	18105	19719	19013	19136
Fuel Used (gal)	791.4	807.2	819.4	806.5

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60

Volumes adjusted by Growth Factors.

Run Number	1	10	2	3	4	5	6
Vehs Entered	9603	9496	9536	9321	9606	9387	9539
Vehs Exited	9406	9422	9315	9081	9301	9193	9339
Starting Vehs	881	868	814	776	796	797	778
Ending Vehs	1078	942	1035	1016	1101	991	978
Denied Entry Before	22	9	1	42	26	8	10
Denied Entry After	233	289	208	326	342	267	300
Travel Distance (mi)	20256	20100	19884	19295	19929	19858	19700
Travel Time (hr)	1124.4	1126.4	1034.4	1033.0	1163.9	1057.9	1073.8
Total Delay (hr)	678.3	683.5	596.6	607.7	724.1	619.8	638.2
Total Stops	20857	19963	18876	17051	20186	19361	18213
Fuel Used (gal)	825.3	820.7	797.6	779.2	825.3	798.6	800.8

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60

Volumes adjusted by Growth Factors.

Run Number	7	8	9	Avg
Vehs Entered	9349	9408	9600	9480
Vehs Exited	9192	9227	9316	9280
Starting Vehs	755	793	781	791
Ending Vehs	912	974	1065	1004
Denied Entry Before	16	33	48	19
Denied Entry After	270	268	306	277
Travel Distance (mi)	19530	19672	19868	19809
Travel Time (hr)	1051.6	1103.9	1126.1	1089.5
Total Delay (hr)	620.4	669.8	687.4	652.6
Total Stops	18105	19719	19013	19136
Fuel Used (gal)	791.4	807.2	819.4	806.5

Total Network Performance By Run

Run Number	1	10	2	3	4	5	6
Denied Delay (hr)	126.3	140.4	94.6	158.5	178.1	112.1	150.1
Denied Del/Veh (s)	46.2	51.7	34.9	59.2	64.5	41.8	54.9
Total Delay (hr)	552.0	543.1	502.1	449.2	546.0	507.7	488.1
Total Del/Veh (s)	189.6	188.6	174.6	160.1	189.0	179.5	170.3
Stop Delay (hr)	310.9	306.1	283.9	254.5	308.7	292.4	279.9
Stop Del/Veh (s)	106.8	106.3	98.8	90.7	106.8	103.4	97.7
Total Stops	20857	19963	18876	17051	20186	19361	18213
Stop/Veh	1.99	1.93	1.82	1.69	1.94	1.90	1.77
Travel Dist (mi)	20255.9	20099.8	19883.7	19294.7	19928.6	19858.0	19700.4
Travel Time (hr)	1124.4	1126.4	1034.4	1033.0	1163.9	1057.9	1073.8
Avg Speed (mph)	20	20	21	22	20	21	21
Vehicles Entered	9603	9496	9536	9321	9606	9387	9539
Vehicles Exited	9406	9422	9315	9081	9301	9193	9339
Hourly Exit Rate	9406	9422	9315	9081	9301	9193	9339
Input Volume	66558	66558	66558	66558	66558	66558	66558
% of Volume	14	14	14	14	14	14	14
Denied Entry Before	22	9	1	42	26	8	10
Denied Entry After	233	289	208	326	342	267	300

Total Network Performance By Run

Run Number	7	8	9	Avg
Denied Delay (hr)	140.2	139.6	154.1	139.4
Denied Del/Veh (s)	52.5	51.9	56.0	51.4
Total Delay (hr)	480.1	530.3	533.3	513.2
Total Del/Veh (s)	171.1	187.1	185.0	179.6
Stop Delay (hr)	277.1	311.0	319.0	294.3
Stop Del/Veh (s)	98.7	109.7	110.6	103.0
Total Stops	18105	19719	19013	19136
Stop/Veh	1.79	1.93	1.83	1.86
Travel Dist (mi)	19529.8	19672.2	19867.6	19809.1
Travel Time (hr)	1051.6	1103.9	1126.1	1089.5
Avg Speed (mph)	21	20	20	21
Vehicles Entered	9349	9408	9600	9480
Vehicles Exited	9192	9227	9316	9280
Hourly Exit Rate	9192	9227	9316	9280
Input Volume	66558	66558	66558	66558
% of Volume	14	14	14	14
Denied Entry Before	16	33	48	19
Denied Entry After	270	268	306	277

Intersection: 1: US 321 & 13th Street SW

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	LT	TR	L	L	T	R	L	T	T	R	L
Maximum Queue (ft)	175	213	140	139	172	79	42	98	489	502	11	210
Average Queue (ft)	56	116	33	19	88	21	2	15	294	302	0	88
95th Queue (ft)	145	192	99	87	156	60	24	80	439	454	8	164
Link Distance (ft)		409	409		504	504			2831	2831		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	300			150			200	200			1000	250
Storage Blk Time (%)				0	1				18			
Queuing Penalty (veh)				0	1				2			

Intersection: 1: US 321 & 13th Street SW

Movement	SB	SB	SB
Directions Served	T	T	T
Maximum Queue (ft)	342	356	333
Average Queue (ft)	90	93	80
95th Queue (ft)	232	238	216
Link Distance (ft)	1853	1853	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			400
Storage Blk Time (%)	2	0	0
Queuing Penalty (veh)	2	2	2

Intersection: 2: US 321 & Main Avenue Drive NW

Movement	EB	WB	NB	NB	SB	SB	SB
Directions Served	LTR	LTR	L	TR	L	T	TR
Maximum Queue (ft)	854	362	144	6	67	8	162
Average Queue (ft)	520	159	52	0	13	0	6
95th Queue (ft)	987	385	112	3	44	6	106
Link Distance (ft)	841	788		1853		1020	1020
Upstream Blk Time (%)	25						
Queuing Penalty (veh)	0						
Storage Bay Dist (ft)			200		150		
Storage Blk Time (%)			0				
Queuing Penalty (veh)			0				

Intersection: 3: US 321 & 2nd Avenue NW

Movement	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	TR	LT	TR	L	T	T	R	L	T	TR
Maximum Queue (ft)	394	570	259	216	349	632	611	399	500	1166	1197
Average Queue (ft)	101	364	168	115	159	218	219	37	209	760	782
95th Queue (ft)	374	643	257	211	310	458	453	187	537	1266	1295
Link Distance (ft)	570	570	258	258		1020	1020			2420	2420
Upstream Blk Time (%)	2	11	2	0							
Queuing Penalty (veh)	0	0	0	0							
Storage Bay Dist (ft)					250			500	400		
Storage Blk Time (%)					5	6	1			29	
Queuing Penalty (veh)					35	8	2			28	

Intersection: 4: US 321 & 7th Avenue NW

Movement	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	T	T	R	L	T	TR
Maximum Queue (ft)	183	102	138	185	509	516	276	83	704	708
Average Queue (ft)	81	41	94	73	170	180	24	9	302	321
95th Queue (ft)	158	88	140	153	393	407	132	54	595	611
Link Distance (ft)	356		94		2420	2420			967	967
Upstream Blk Time (%)		3	23						0	0
Queuing Penalty (veh)		0	0						1	1
Storage Bay Dist (ft)		150		100			200	150		
Storage Blk Time (%)		3	23	6	10	6			15	
Queuing Penalty (veh)		4	13	45	7	3			1	

Intersection: 5: US 321 & 9th Avenue NW

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR
Maximum Queue (ft)	129	127	158	170	243	506	534	172	800	822
Average Queue (ft)	56	42	60	66	123	192	189	56	262	277
95th Queue (ft)	109	98	124	137	235	427	439	127	603	621
Link Distance (ft)		297		302		967	967		874	874
Upstream Blk Time (%)				0					0	0
Queuing Penalty (veh)				0					2	3
Storage Bay Dist (ft)	300		150		150			100		
Storage Blk Time (%)			1	1	9	10		5	20	
Queuing Penalty (veh)			0	1	58	13		39	9	

Intersection: 6: US 321 & Clement Boulevard

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB	SB
Directions Served	L	LT	R	L	LT	R	L	T	T	R	L	L
Maximum Queue (ft)	288	326	112	342	410	436	499	840	850	350	174	308
Average Queue (ft)	153	215	37	153	334	404	118	627	637	93	90	114
95th Queue (ft)	292	323	88	334	494	420	412	903	915	348	154	224
Link Distance (ft)		850	850		385	385		874	874			
Upstream Blk Time (%)				0	17	84		2	2			
Queuing Penalty (veh)				0	0	0		12	13			
Storage Bay Dist (ft)	600			300			400			250	600	600
Storage Blk Time (%)				0	9			38	47			
Queuing Penalty (veh)				1	6			15	29			

Intersection: 6: US 321 & Clement Boulevard

Movement	SB	SB
Directions Served	T	TR
Maximum Queue (ft)	625	658
Average Queue (ft)	260	280
95th Queue (ft)	488	510
Link Distance (ft)	964	964
Upstream Blk Time (%)	0	0
Queuing Penalty (veh)	0	0
Storage Bay Dist (ft)		
Storage Blk Time (%)	0	
Queuing Penalty (veh)	1	

Intersection: 7: US 321 & 13th Avenue Drive NW

Movement	EB	NB	SB	SB
Directions Served	R	L	T	TR
Maximum Queue (ft)	134	24	49	47
Average Queue (ft)	44	4	3	3
95th Queue (ft)	100	18	36	38
Link Distance (ft)	196		1656	1656
Upstream Blk Time (%)	0			
Queuing Penalty (veh)	0			
Storage Bay Dist (ft)		200		
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	LT	R	L	T	T	R	L	T	T
Maximum Queue (ft)	54	33	41	150	148	228	755	774	250	590	583	580
Average Queue (ft)	11	7	6	21	42	23	358	373	30	326	217	227
95th Queue (ft)	37	25	23	80	113	123	659	675	144	542	511	511
Link Distance (ft)		546		415			1656	1656			3294	3294
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	250		250		100	300			150	900		
Storage Blk Time (%)				1	3		17	36				8
Queuing Penalty (veh)				1	1		2	8				3

Intersection: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

Movement	SB	B54	B54
Directions Served	R	T	T
Maximum Queue (ft)	194	116	121
Average Queue (ft)	14	4	4
95th Queue (ft)	103	115	119
Link Distance (ft)		1174	1174
Upstream Blk Time (%)			0
Queuing Penalty (veh)			0
Storage Bay Dist (ft)	200		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 9: US 321 & Grace Chapel Road

Movement	WB	WB	WB	NB	NB	NB	NB	B54	B54	SB	SB	SB
Directions Served	L	L	R	U	T	T	R	T	T	L	T	T
Maximum Queue (ft)	345	484	273	32	966	990	400	298	307	247	813	828
Average Queue (ft)	245	289	48	4	238	258	58	10	11	32	452	465
95th Queue (ft)	361	446	180	20	721	759	288	293	296	148	922	950
Link Distance (ft)		928			1174	1174		3294	3294		3473	3473
Upstream Blk Time (%)					0	0						
Queuing Penalty (veh)					1	3						
Storage Bay Dist (ft)	250		250	300			300			400		
Storage Blk Time (%)	12	24			10	11						9
Queuing Penalty (veh)	31	64			0	22						2

Intersection: 10: US 321 & Alex Lee Blvd

Movement	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB	
Directions Served	LT	R	LT	R	L	T	T	R	L	T	TR	
Maximum Queue (ft)	41	42	324	72	129	344	361	69	118	408	444	
Average Queue (ft)	8	7	132	13	16	102	125	11	32	168	180	
95th Queue (ft)	29	29	261	45	77	289	323	46	87	358	377	
Link Distance (ft)	217	217	618	618		3473	3473			1153	1153	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					200				350	450		
Storage Blk Time (%)							3		0		0	
Queuing Penalty (veh)							0		0		0	

Intersection: 11: US 321 & Pooveys Grove Church Road

Movement	WB
Directions Served	R
Maximum Queue (ft)	33
Average Queue (ft)	4
95th Queue (ft)	21
Link Distance (ft)	398
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 12: US 321 & US 321 A/Riverbend Rd

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	T	R	L	L	T	R	L	T	T	R	L
Maximum Queue (ft)	141	414	200	349	291	201	340	270	419	444	300	279
Average Queue (ft)	12	231	136	228	182	103	187	104	195	192	99	164
95th Queue (ft)	77	424	252	316	267	180	312	211	340	346	234	247
Link Distance (ft)		435		418	418	418	418		1744	1744		
Upstream Blk Time (%)		4		0			0					
Queuing Penalty (veh)		0		0			0					
Storage Bay Dist (ft)	200		100					250			200	350
Storage Blk Time (%)		50	15					0	5	9	0	
Queuing Penalty (veh)		93	25					2	6	24	1	

Intersection: 12: US 321 & US 321 A/Riverbend Rd

Movement	SB	SB	SB	SB
Directions Served	L	T	T	R
Maximum Queue (ft)	544	642	624	60
Average Queue (ft)	242	413	395	2
95th Queue (ft)	439	587	574	42
Link Distance (ft)		1113	1113	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	350			200
Storage Blk Time (%)	0	16	28	
Queuing Penalty (veh)	0	62	2	

Intersection: 158: Bend

Movement	SB
Directions Served	T
Maximum Queue (ft)	249
Average Queue (ft)	8
95th Queue (ft)	245
Link Distance (ft)	2831
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 710

Lanes, Volumes, Timings
1: US 321 & 13th Street SW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	91	20	9	104	37	90	16	1759	98	68	1407	127
Future Volume (vph)	91	20	9	104	37	90	16	1759	98	68	1407	127
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		200	200		1000	250		400
Storage Lanes	1		0	1		1	1		1	1		2
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.91	0.91	0.95	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.91	1.00
Frt		0.982				0.850			0.850			0.850
Flt Protected	0.950	0.970		0.950			0.950			0.950		
Satd. Flow (prot)	1564	3137	0	3335	1810	1538	1719	3438	1538	1719	4940	1538
Flt Permitted	0.950	0.970		0.950			0.950			0.950		
Satd. Flow (perm)	1564	3137	0	3335	1810	1538	1719	3438	1538	1719	4940	1538
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		468			555			2913			1957	
Travel Time (s)		9.1			10.8			44.1			29.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	101	22	10	116	41	100	18	1954	109	76	1563	141
Shared Lane Traffic (%)	50%											
Lane Group Flow (vph)	50	83	0	116	41	100	18	1954	109	76	1563	141
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Split	NA		Split	NA	Free	Prot	NA	Free	Prot	NA	Free
Protected Phases	8	8		4	4		5	2		1	6	
Permitted Phases						Free			Free			Free
Detector Phase	8	8		4	4		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0		7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0		14.0	19.0	
Total Split (s)	14.0	14.0		14.0	14.0		14.0	87.0		15.0	88.0	
Total Split (%)	10.8%	10.8%		10.8%	10.8%		10.8%	66.9%		11.5%	67.7%	
Maximum Green (s)	7.0	7.0		7.0	7.0		7.0	80.0		8.0	81.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	

Lanes, Volumes, Timings
1: US 321 & 13th Street SW

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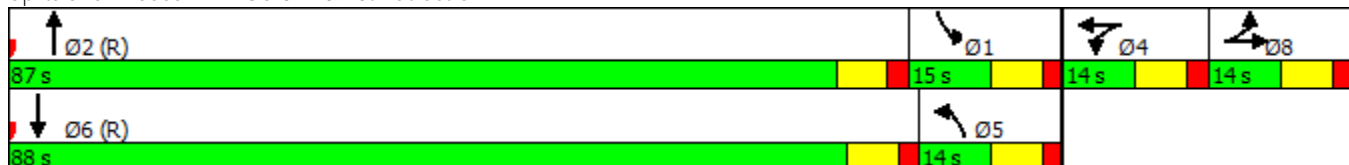


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)	7.0	7.0		7.0	7.0	130.0	13.5	80.1	130.0	7.9	82.9	130.0
Actuated g/C Ratio	0.05	0.05		0.05	0.05	1.00	0.10	0.62	1.00	0.06	0.64	1.00
v/c Ratio	0.60	0.49		0.64	0.42	0.07	0.10	0.92	0.07	0.74	0.50	0.09
Control Delay	88.0	70.2		77.0	72.9	0.1	49.8	31.0	0.1	78.8	6.3	0.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	88.0	70.2		77.0	72.9	0.1	49.8	31.0	0.1	78.8	6.3	0.0
LOS	F	E		E	E	A	D	C	A	E	A	A
Approach Delay		76.9			46.4			29.6			8.9	
Approach LOS		E			D			C			A	
Queue Length 50th (ft)	46	37		50	34	0	15	727	0	65	47	0
Queue Length 95th (ft)	#110	68		#88	73	0	37	871	0	m73	m180	m0
Internal Link Dist (ft)		388			475			2833			1877	
Turn Bay Length (ft)	300			150		200	200		1000	250		400
Base Capacity (vph)	84	168		180	98	1538	178	2118	1538	105	3396	1538
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.60	0.49		0.64	0.42	0.07	0.10	0.92	0.07	0.72	0.46	0.09

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 96 (74%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 23.4 Intersection LOS: C
 Intersection Capacity Utilization 77.8% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: US 321 & 13th Street SW



Lanes, Volumes, Timings
2: US 321 & Main Avenue Drive NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Traffic Volume (vph)	41	4	112	21	4	30	66	1844	13	15	1485	19
Future Volume (vph)	41	4	112	21	4	30	66	1844	13	15	1485	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	200		0	150		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.904			0.926			0.999			0.998	
Flt Protected		0.987			0.981		0.950			0.950		
Satd. Flow (prot)	0	1646	0	0	1644	0	1719	3435	0	1719	3431	0
Flt Permitted		0.987			0.981		0.950			0.950		
Satd. Flow (perm)	0	1646	0	0	1644	0	1719	3435	0	1719	3431	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		892			860			1957			1105	
Travel Time (s)		24.3			23.5			29.7			16.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	46	4	124	23	4	33	73	2049	14	17	1650	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	174	0	0	60	0	73	2063	0	17	1671	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	72.4%
Analysis Period (min)	15
	ICU Level of Service C

Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	41	4	112	21	4	30	66	1844	13	15	1485	19
Future Vol, veh/h	41	4	112	21	4	30	66	1844	13	15	1485	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	200	-	-	150	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	5	5	5	5	5	5	5	5	5
Mvmt Flow	46	4	124	23	4	33	73	2049	14	17	1650	21

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	2867	3904	836	3064	3907	1032	1671	0	0	2063	0	0
Stage 1	1694	1694	-	2203	2203	-	-	-	-	-	-	-
Stage 2	1173	2210	-	861	1704	-	-	-	-	-	-	-
Critical Hdwy	7.56	6.56	6.96	7.6	6.6	7	4.2	-	-	4.2	-	-
Critical Hdwy Stg 1	6.56	5.56	-	6.6	5.6	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.56	5.56	-	6.6	5.6	-	-	-	-	-	-	-
Follow-up Hdwy	3.53	4.03	3.33	3.55	4.05	3.35	2.25	-	-	2.25	-	-
Pot Cap-1 Maneuver	~ 7	~ 3	308	~ 5	~ 3	225	367	-	-	257	-	-
Stage 1	95	146	-	44	78	-	-	-	-	-	-	-
Stage 2	202	79	-	310	141	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	-	~ 2	308	-	~ 2	225	367	-	-	257	-	-
Mov Cap-2 Maneuver	-	~ 2	-	-	~ 2	-	-	-	-	-	-	-
Stage 1	76	136	-	35	62	-	-	-	-	-	-	-
Stage 2	128	63	-	167	132	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s			0.6	0.2
HCM LOS	-	-		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	367	-	-	-	257	-	-
HCM Lane V/C Ratio	0.2	-	-	-	0.065	-	-
HCM Control Delay (s)	17.2	-	-	-	20	-	-
HCM Lane LOS	C	-	-	-	C	-	-
HCM 95th %tile Q(veh)	0.7	-	-	-	0.2	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lanes, Volumes, Timings
3: US 321 & 2nd Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	97	114	124	115	132	98	96	1659	76	52	1333	60
Future Volume (vph)	97	114	124	115	132	98	96	1659	76	52	1333	60
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	250		500	400		0
Storage Lanes	1		0	0		0	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Frt		0.922			0.957				0.850		0.994	
Flt Protected	0.950				0.984		0.950			0.950		
Satd. Flow (prot)	1752	1701	0	0	3300	0	1719	3438	1538	1719	3417	0
Flt Permitted	0.950				0.984		0.950			0.950		
Satd. Flow (perm)	1752	1701	0	0	3300	0	1719	3438	1538	1719	3417	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		627			328			1105			2513	
Travel Time (s)		12.2			6.4			16.7			38.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	108	127	138	128	147	109	107	1843	84	58	1481	67
Shared Lane Traffic (%)												
Lane Group Flow (vph)	108	265	0	0	384	0	107	1843	84	58	1548	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Split	NA		Split	NA		Prot	NA	pm+ov	Prot	NA	
Protected Phases	8	8		4	4		5	2	4	1	6	
Permitted Phases									2			
Detector Phase	8	8		4	4		5	2	4	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0	7.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0	14.0	14.0	19.0	
Total Split (s)	25.0	25.0		20.0	20.0		16.0	71.0	20.0	14.0	69.0	
Total Split (%)	19.2%	19.2%		15.4%	15.4%		12.3%	54.6%	15.4%	10.8%	53.1%	
Maximum Green (s)	18.0	18.0		13.0	13.0		9.0	64.0	13.0	7.0	62.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0		7.0	7.0	7.0	7.0	7.0	
Lead/Lag							Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min	None	None	C-Min	

Lanes, Volumes, Timings
3: US 321 & 2nd Avenue NW

NCDOT TIP-U4700

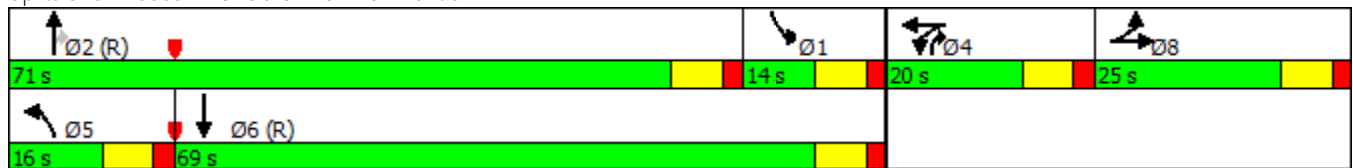


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)	18.0	18.0			13.4		9.0	66.4	81.2	7.0	61.6	
Actuated g/C Ratio	0.14	0.14			0.10		0.07	0.51	0.62	0.05	0.47	
v/c Ratio	0.45	1.13			1.13		0.90	1.05	0.09	0.63	0.96	
Control Delay	58.0	147.3			140.7		103.2	50.5	3.7	64.0	27.5	
Queue Delay	0.0	0.0			0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	58.0	147.3			140.7		103.2	50.5	3.7	64.0	27.5	
LOS	E	F			F		F	D	A	E	C	
Approach Delay		121.4			140.7			51.3			28.9	
Approach LOS		F			F			D			C	
Queue Length 50th (ft)	85	-258			-201		87	-922	15	47	464	
Queue Length 95th (ft)	146	#433			#307		m#113	#1047	m16	m68	#803	
Internal Link Dist (ft)		547			248			1025			2433	
Turn Bay Length (ft)							250		500	400		
Base Capacity (vph)	242	235			340		119	1756	960	92	1629	
Starvation Cap Reductn	0	0			0		0	0	0	0	0	
Spillback Cap Reductn	0	0			0		0	0	0	0	0	
Storage Cap Reductn	0	0			0		0	0	0	0	0	
Reduced v/c Ratio	0.45	1.13			1.13		0.90	1.05	0.09	0.63	0.95	

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 44 (34%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 160
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.13
 Intersection Signal Delay: 56.9 Intersection LOS: E
 Intersection Capacity Utilization 98.7% ICU Level of Service F
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: US 321 & 2nd Avenue NW



Lanes, Volumes, Timings
4: US 321 & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↙	↘		↙	↕	↘	↙	↕	↘
Traffic Volume (vph)	16	135	71	45	54	7	36	1724	58	7	1393	7
Future Volume (vph)	16	135	71	45	54	7	36	1724	58	7	1393	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	150		0	100		200	150		0
Storage Lanes	0		0	1		0	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	0.95
Frt		0.957			0.982				0.850		0.999	
Flt Protected		0.996		0.950			0.950			0.950		
Satd. Flow (prot)	0	1758	0	1671	1728	0	1719	3438	1538	1736	3468	0
Flt Permitted		0.973		0.329			0.950			0.950		
Satd. Flow (perm)	0	1718	0	579	1728	0	1719	3438	1538	1736	3468	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		406			154			2513			1038	
Travel Time (s)		7.9			3.0			38.1			15.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	8%	8%	8%	5%	5%	5%	4%	4%	4%
Adj. Flow (vph)	18	150	79	50	60	8	40	1916	64	8	1548	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	247	0	50	68	0	40	1916	64	8	1556	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		Prot	NA	Perm	Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8					2			
Detector Phase	4	4		8	8		5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0	12.0	7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0	19.0	14.0	19.0	
Total Split (s)	30.0	30.0		30.0	30.0		14.0	86.0	86.0	14.0	86.0	
Total Split (%)	23.1%	23.1%		23.1%	23.1%		10.8%	66.2%	66.2%	10.8%	66.2%	
Maximum Green (s)	23.0	23.0		23.0	23.0		7.0	79.0	79.0	7.0	79.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		7.0		7.0	7.0		7.0	7.0	7.0	7.0	7.0	
Lead/Lag							Lag	Lead	Lead	Lag	Lead	
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min	C-Min	None	C-Min	

Lanes, Volumes, Timings
4: US 321 & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)		22.5		22.5	22.5		7.8	90.7	90.7	7.0	81.5	
Actuated g/C Ratio		0.17		0.17	0.17		0.06	0.70	0.70	0.05	0.63	
v/c Ratio		0.83		0.50	0.23		0.39	0.80	0.06	0.09	0.72	
Control Delay		74.7		66.2	47.5		61.6	25.6	13.9	62.7	6.2	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		74.7		66.2	47.5		61.6	25.6	13.9	62.7	6.2	
LOS		E		E	D		E	C	B	E	A	
Approach Delay		74.7			55.4			26.0			6.5	
Approach LOS		E			E			C			A	
Queue Length 50th (ft)		199		38	49		30	572	28	7	103	
Queue Length 95th (ft)		#338		85	95		m32	m588	m34	m11	153	
Internal Link Dist (ft)		326			74			2433			958	
Turn Bay Length (ft)				150			100		200	150		
Base Capacity (vph)		317		107	319		102	2398	1073	93	2222	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.78		0.47	0.21		0.39	0.80	0.06	0.09	0.70	

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 9 (7%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 22.2 Intersection LOS: C
 Intersection Capacity Utilization 78.3% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: US 321 & 7th Avenue NW



Lanes, Volumes, Timings
5: US 321 & 9th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	216	4	131	39	4	45	39	1580	56	53	1299	53
Future Volume (vph)	216	4	131	39	4	45	39	1580	56	53	1299	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		0	150		0	100		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.84											
Frt		0.854			0.861			0.995			0.994	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1575	0	1736	1573	0	1736	3454	0	1736	3450	0
Flt Permitted	0.722			0.613			0.950			0.950		
Satd. Flow (perm)	1121	1575	0	1120	1573	0	1736	3454	0	1736	3450	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		346			350			1038			963	
Travel Time (s)		6.7			6.8			15.7			14.6	
Confl. Peds. (#/hr)	131											
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	240	4	146	43	4	50	43	1756	62	59	1443	59
Shared Lane Traffic (%)												
Lane Group Flow (vph)	240	150	0	43	54	0	43	1818	0	59	1502	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	12.0		7.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		14.0	19.0		14.0	19.0	
Total Split (s)	37.0	37.0		37.0	37.0		15.0	79.0		14.0	78.0	
Total Split (%)	28.5%	28.5%		28.5%	28.5%		11.5%	60.8%		10.8%	60.0%	
Maximum Green (s)	30.0	30.0		30.0	30.0		8.0	72.0		7.0	71.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	

Lanes, Volumes, Timings
5: US 321 & 9th Avenue NW

NCDOT TIP-U4700

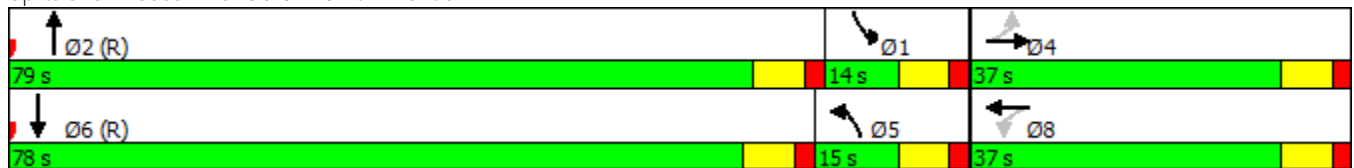


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)	29.3	29.3		29.3	29.3		9.5	75.5		7.0	73.1	
Actuated g/C Ratio	0.23	0.23		0.23	0.23		0.07	0.58		0.05	0.56	
v/c Ratio	0.95	0.42		0.17	0.15		0.34	0.91		0.63	0.78	
Control Delay	95.8	47.2		42.3	41.3		59.5	22.1		66.3	9.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	95.8	47.2		42.3	41.3		59.5	22.1		66.3	9.3	
LOS	F	D		D	D		E	C		E	A	
Approach Delay		77.1			41.8			22.9			11.4	
Approach LOS		E			D			C			B	
Queue Length 50th (ft)	199	108		29	37		39	281		53	98	
Queue Length 95th (ft)	#364	176		64	74		m43	#914		m68	126	
Internal Link Dist (ft)		266			270			958			883	
Turn Bay Length (ft)	300			150			150			100		
Base Capacity (vph)	258	363		258	363		129	2006		93	1983	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.93	0.41		0.17	0.15		0.33	0.91		0.63	0.76	

Intersection Summary

























Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 124 (95%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 24.2 Intersection LOS: C
 Intersection Capacity Utilization 80.8% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: US 321 & 9th Avenue NW



Lanes, Volumes, Timings
6: US 321 & Clement Boulevard

NCDOT TIP-U4700

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	70	137	39	61	82	323	51	1537	135	660	1430	86
Future Volume (vph)	70	137	39	61	82	323	51	1537	135	660	1430	86
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	600		0	300		0	400		250	600		0
Storage Lanes	1		1	1		1	1		1	2		0
Taper Length (ft)	100			100			100			200		
Lane Util. Factor	0.95	0.95	1.00	0.95	0.95	1.00	1.00	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.991	
Flt Protected	0.950	0.998		0.950	0.996		0.950			0.950		
Satd. Flow (prot)	1665	1749	1568	1681	1763	1583	1719	3438	1538	3367	3440	0
Flt Permitted	0.950	0.998		0.950	0.996		0.950			0.950		
Satd. Flow (perm)	1665	1749	1568	1681	1763	1583	1719	3438	1538	3367	3440	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		915			441			963			1050	
Travel Time (s)		17.8			8.6			14.6			15.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	5%	5%	5%	4%	4%	4%
Adj. Flow (vph)	78	152	43	68	91	359	57	1708	150	733	1589	96
Shared Lane Traffic (%)	10%			10%								
Lane Group Flow (vph)	70	160	43	61	98	359	57	1708	150	733	1685	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Split	NA	pm+ov	Split	NA	pm+ov	Prot	NA	Perm	Prot	NA	
Protected Phases	4	4	5	8	8	1	5	2		1	6	
Permitted Phases			4			8			2			
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	12.0	7.0	12.0	
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	19.0	19.0	14.0	19.0	
Total Split (s)	17.0	17.0	14.0	14.0	14.0	32.0	14.0	67.0	67.0	32.0	85.0	
Total Split (%)	13.1%	13.1%	10.8%	10.8%	10.8%	24.6%	10.8%	51.5%	51.5%	24.6%	65.4%	
Maximum Green (s)	10.0	10.0	7.0	7.0	7.0	25.0	7.0	60.0	60.0	25.0	78.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Lead/Lag			Lag			Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?			Yes			Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	

Lanes, Volumes, Timings
6: US 321 & Clement Boulevard

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)	10.0	10.0	23.2	7.0	7.0	39.0	7.6	60.0	60.0	25.0	80.2	
Actuated g/C Ratio	0.08	0.08	0.18	0.05	0.05	0.30	0.06	0.46	0.46	0.19	0.62	
v/c Ratio	0.55	1.19	0.15	0.68	1.04	0.76	0.57	1.08	0.21	1.13	0.79	
Control Delay	74.3	189.2	45.9	94.8	163.2	52.9	52.2	60.7	8.9	125.6	14.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	74.3	189.2	45.9	94.8	163.2	52.9	52.2	60.7	8.9	125.6	14.6	
LOS	E	F	D	F	F	D	D	E	A	F	B	
Approach Delay		137.2			78.7			56.4			48.3	
Approach LOS		F			E			E			D	
Queue Length 50th (ft)	61	-170	31	53	-93	274	45	-832	29	-372	418	
Queue Length 95th (ft)	115	#322	66	#130	#218	396	m52	m#970	m39	#489	563	
Internal Link Dist (ft)		835			361			883			970	
Turn Bay Length (ft)	600			300			400		250	600		
Base Capacity (vph)	128	134	280	90	94	474	100	1586	709	647	2138	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.55	1.19	0.15	0.68	1.04	0.76	0.57	1.08	0.21	1.13	0.79	

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 16 (12%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 180
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.19
 Intersection Signal Delay: 59.1
 Intersection LOS: E
 Intersection Capacity Utilization 97.7%
 ICU Level of Service F
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: US 321 & Clement Boulevard



Lanes, Volumes, Timings
7: US 321 & 13th Avenue Drive NW

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	31	4	2525	1645	46
Future Volume (vph)	0	31	4	2525	1645	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	1	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Frt		0.865			0.996	
Flt Protected			0.950			
Satd. Flow (prot)	0	1596	1736	3471	3424	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	1596	1736	3471	3424	0
Link Speed (mph)	35			45	45	
Link Distance (ft)	250			1050	1732	
Travel Time (s)	4.9			15.9	26.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	4%	4%	5%	5%
Adj. Flow (vph)	0	34	4	2806	1828	51
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	34	4	2806	1879	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	73.1%
Analysis Period (min)	15
	ICU Level of Service D

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	31	4	2525	1645	46
Future Vol, veh/h	0	31	4	2525	1645	46
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	200	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	4	4	5	5
Mvmt Flow	0	34	4	2806	1828	51

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	939	1879 0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.96	4.18 -
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.33	2.24 -
Pot Cap-1 Maneuver	0	263	308 -
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %			-
Mov Cap-1 Maneuver	-	263	308 -
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	20.7	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	308	-	263	-	-
HCM Lane V/C Ratio	0.014	-	0.131	-	-
HCM Control Delay (s)	16.9	-	20.7	-	-
HCM Lane LOS	C	-	C	-	-
HCM 95th %tile Q(veh)	0	-	0.4	-	-

Lanes, Volumes, Timings
 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	8	16	24	8	390	10	2389	15	165	1626	12
Future Volume (vph)	28	8	16	24	8	390	10	2389	15	165	1626	12
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		250	0		100	300		150	900		200
Storage Lanes	1		1	0		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950				0.964		0.950			0.950		
Satd. Flow (prot)	1719	1810	1538	0	1796	1583	1736	3471	1553	1736	3471	1553
Flt Permitted	0.950				0.964		0.950			0.950		
Satd. Flow (perm)	1719	1810	1538	0	1796	1583	1736	3471	1553	1736	3471	1553
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		606			480			1732			3358	
Travel Time (s)		11.8			9.4			26.2			50.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	2%	2%	2%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	31	9	18	27	9	433	11	2654	17	183	1807	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	9	18	0	36	433	11	2654	17	183	1807	13
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Split	NA	pm+ov	Split	NA	pm+ov	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	5	8	8	1	5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	12.0	7.0	12.0	12.0
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	19.0	19.0	14.0	19.0	19.0
Total Split (s)	14.0	14.0	14.0	14.0	14.0	25.0	14.0	77.0	77.0	25.0	88.0	88.0
Total Split (%)	10.8%	10.8%	10.8%	10.8%	10.8%	19.2%	10.8%	59.2%	59.2%	19.2%	67.7%	67.7%
Maximum Green (s)	7.0	7.0	7.0	7.0	7.0	18.0	7.0	70.0	70.0	18.0	81.0	81.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Lead/Lag			Lead			Lag	Lead	Lead	Lead	Lag	Lag	Lag
Lead-Lag Optimize?			Yes			Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	C-Min

Lanes, Volumes, Timings
 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)	7.0	7.0	11.2		7.0	22.2	7.0	81.2	81.2	18.0	100.6	100.6
Actuated g/C Ratio	0.05	0.05	0.09		0.05	0.17	0.05	0.62	0.62	0.14	0.77	0.77
v/c Ratio	0.34	0.09	0.14		0.38	1.60	0.12	1.22	0.02	0.76	0.67	0.01
Control Delay	69.5	60.8	40.6		70.9	319.3	75.8	121.4	6.9	63.2	11.0	7.4
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.5	60.8	40.6		70.9	319.3	75.8	121.4	6.9	63.2	11.0	7.4
LOS	E	E	D		E	F	E	F	A	E	B	A
Approach Delay		59.2			300.2			120.5			15.7	
Approach LOS		E			F			F			B	
Queue Length 50th (ft)	26	7	11		30	~413	10	~1575	4	137	406	4
Queue Length 95th (ft)	60	26	32		67	#620	m15	m#1517	m5	m#254	513	m3
Internal Link Dist (ft)		526			400			1652			3278	
Turn Bay Length (ft)	250		250			100	300		150	900		200
Base Capacity (vph)	92	97	132		96	270	93	2168	970	240	2686	1201
Starvation Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0		0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.09	0.14		0.38	1.60	0.12	1.22	0.02	0.76	0.67	0.01

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 62 (48%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 180
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.60
 Intersection Signal Delay: 95.7 Intersection LOS: F
 Intersection Capacity Utilization 113.5% ICU Level of Service H
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW



Lanes, Volumes, Timings
9: US 321 & Grace Chapel Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	202	18	4	2343	467	28	1604
Future Volume (vph)	202	18	4	2343	467	28	1604
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	250	300		300	400	
Storage Lanes	1	1	1		1	1	
Taper Length (ft)	100		100			100	
Lane Util. Factor	0.97	1.00	1.00	0.95	1.00	1.00	0.95
Frt		0.850			0.850		
Flt Protected	0.950		0.950			0.950	
Satd. Flow (prot)	3433	1583	1736	3471	1553	1736	3471
Flt Permitted	0.950		0.950			0.950	
Satd. Flow (perm)	3433	1583	1736	3471	1553	1736	3471
Right Turn on Red		No			No		
Satd. Flow (RTOR)							
Link Speed (mph)	45			55			55
Link Distance (ft)	983			1234			3568
Travel Time (s)	14.9			15.3			44.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%	4%
Adj. Flow (vph)	224	20	4	2603	519	31	1782
Shared Lane Traffic (%)							
Lane Group Flow (vph)	224	20	4	2603	519	31	1782
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Right	R NA	Left	Right	Left	Left
Median Width(ft)	24			12			12
Link Offset(ft)	0			0			0
Crosswalk Width(ft)	16			16			16
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9		9	15	
Turn Type	Prot	Perm	Prot	NA	Free	Prot	NA
Protected Phases	8		5	2		1	6
Permitted Phases		8			Free		
Detector Phase	8	8	5	2		1	6
Switch Phase							
Minimum Initial (s)	7.0	7.0	7.0	14.0		7.0	14.0
Minimum Split (s)	14.0	14.0	14.0	21.0		14.0	21.0
Total Split (s)	16.0	16.0	14.0	100.0		14.0	100.0
Total Split (%)	12.3%	12.3%	10.8%	76.9%		10.8%	76.9%
Maximum Green (s)	9.0	9.0	7.0	93.0		7.0	93.0
Yellow Time (s)	5.0	5.0	5.0	5.0		5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0		2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0		7.0	7.0
Lead/Lag			Lead	Lead		Lag	Lag
Lead-Lag Optimize?			Yes	Yes		Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	C-Min		None	C-Min

Lanes, Volumes, Timings
 9: US 321 & Grace Chapel Road

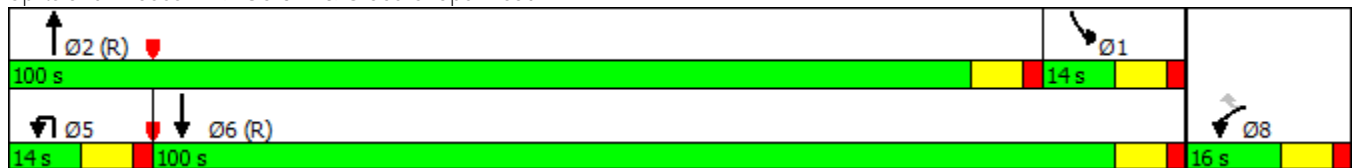


Lane Group	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Act Effect Green (s)	10.6	10.6	7.0	97.0	130.0	7.0	102.6
Actuated g/C Ratio	0.08	0.08	0.05	0.75	1.00	0.05	0.79
v/c Ratio	0.80	0.16	0.04	1.01	0.33	0.33	0.65
Control Delay	79.8	59.8	83.0	17.4	0.1	57.9	7.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.8	59.8	83.0	17.4	0.1	57.9	7.1
LOS	E	E	F	B	A	E	A
Approach Delay	78.2			14.6			8.0
Approach LOS	E			B			A
Queue Length 50th (ft)	98	16	3	-1266	0	25	240
Queue Length 95th (ft)	#180	44	m3	m101	m0	m41	m361
Internal Link Dist (ft)	903			1154			3488
Turn Bay Length (ft)	250	250	300		300	400	
Base Capacity (vph)	280	129	93	2589	1553	93	2738
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.80	0.16	0.04	1.01	0.33	0.33	0.65

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 117 (90%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 140
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 15.3
 Intersection LOS: B
 Intersection Capacity Utilization 82.3%
 ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
10: US 321 & Alex Lee Blvd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕	↗	↗	↕↕	↗	↗	↕↕	
Traffic Volume (vph)	6	5	15	90	4	14	4	2025	75	36	1356	4
Future Volume (vph)	6	5	15	90	4	14	4	2025	75	36	1356	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	200		350	450		0
Storage Lanes	0		1	0		1	1		1	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	0.95
Frt			0.850			0.850			0.850			
Flt Protected		0.974			0.954		0.950			0.950		
Satd. Flow (prot)	0	1762	1538	0	1220	1442	1736	3471	1442	1612	3505	0
Flt Permitted		0.833			0.725		0.950			0.950		
Satd. Flow (perm)	0	1507	1538	0	927	1442	1736	3471	1442	1612	3505	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		25			35			55			55	
Link Distance (ft)		264			676			3568			1233	
Travel Time (s)		7.2			13.2			44.2			15.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	50%	12%	12%	4%	4%	12%	12%	3%	3%
Adj. Flow (vph)	7	6	17	100	4	16	4	2250	83	40	1507	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	17	0	104	16	4	2250	83	40	1511	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			36			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA	pm+ov	Perm	NA	pm+ov	Prot	NA	Perm	Prot	NA	
Protected Phases		4	5		8	1	5	2		1	6	
Permitted Phases	4		4	8		8			2			
Detector Phase	4	4	5	8	8	1	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	14.0	14.0	7.0	14.0	
Minimum Split (s)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	21.0	21.0	14.0	21.0	
Total Split (s)	22.0	22.0	14.0	22.0	22.0	14.0	14.0	94.0	94.0	14.0	94.0	
Total Split (%)	16.9%	16.9%	10.8%	16.9%	16.9%	10.8%	10.8%	72.3%	72.3%	10.8%	72.3%	
Maximum Green (s)	15.0	15.0	7.0	15.0	15.0	7.0	7.0	87.0	87.0	7.0	87.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Lead/Lag			Lead			Lead	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Min	C-Min	None	C-Min	

Lanes, Volumes, Timings
10: US 321 & Alex Lee Blvd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)		15.8	29.8		15.8	29.8	7.0	89.0	89.0	7.0	91.8	
Actuated g/C Ratio		0.12	0.23		0.12	0.23	0.05	0.68	0.68	0.05	0.71	
v/c Ratio		0.07	0.05		0.93	0.05	0.04	0.95	0.08	0.47	0.61	
Control Delay		52.5	40.3		125.4	40.4	47.5	16.6	8.9	93.5	2.8	
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		52.5	40.3		125.4	40.4	47.5	16.6	8.9	93.5	2.8	
LOS		D	D		F	D	D	B	A	F	A	
Approach Delay		45.6			114.0			16.4			5.1	
Approach LOS		D			F			B			A	
Queue Length 50th (ft)		10	11		89	11	4	337	24	35	52	
Queue Length 95th (ft)		31	33		#209	31	m4	m335	m24	m57	58	
Internal Link Dist (ft)		184			596			3488			1153	
Turn Bay Length (ft)							200		350	450		
Base Capacity (vph)		182	352		112	329	93	2377	988	86	2476	
Starvation Cap Reductn		0	0		0	0	0	0	0	0	0	
Spillback Cap Reductn		0	0		0	0	0	0	0	0	0	
Storage Cap Reductn		0	0		0	0	0	0	0	0	0	
Reduced v/c Ratio		0.07	0.05		0.93	0.05	0.04	0.95	0.08	0.47	0.61	

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 124 (95%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 15.2 Intersection LOS: B
 Intersection Capacity Utilization 85.1% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 10: US 321 & Alex Lee Blvd



Lanes, Volumes, Timings
 11: US 321 & Pooveys Grove Church Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	30	2315	4	0	1555
Future Volume (vph)	0	30	2315	4	0	1555
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Fr _t		0.865				
Fl _t Protected						
Satd. Flow (prot)	0	1611	3505	0	0	3505
Fl _t Permitted						
Satd. Flow (perm)	0	1611	3505	0	0	3505
Link Speed (mph)	35		55			55
Link Distance (ft)	444		1233			1822
Travel Time (s)	8.6		15.3			22.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	3%	3%
Adj. Flow (vph)	0	33	2572	4	0	1728
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	33	2576	0	0	1728
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	74.1%
Analysis Period (min)	15
	ICU Level of Service D

Intersection

Int Delay, s/veh 0.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↑	↑↑			↑↑
Traffic Vol, veh/h	0	30	2315	4	0	1555
Future Vol, veh/h	0	30	2315	4	0	1555
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Free	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	0	33	2572	4	0	1728

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	-	1286	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.94	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.32	-
Pot Cap-1 Maneuver	0	155	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	155	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	34.5	0	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBTWBLn1	SBT
Capacity (veh/h)	- 155	-
HCM Lane V/C Ratio	- 0.215	-
HCM Control Delay (s)	- 34.5	-
HCM Lane LOS	- D	-
HCM 95th %tile Q(veh)	- 0.8	-

Lanes, Volumes, Timings
 12: US 321 & US 321 A/Riverbend Rd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	171	119	260	164	389	177	1292	401	376	937	8
Future Volume (vph)	8	171	119	260	164	389	177	1292	401	376	937	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		100	0		0	250		200	350		200
Storage Lanes	1		1	2		1	1		1	2		1
Taper Length (ft)	100			100			100			200		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1845	1568	3433	1863	1583	1736	3471	1553	3367	3471	1553
Flt Permitted	0.643			0.950			0.950			0.950		
Satd. Flow (perm)	1186	1845	1568	3433	1863	1583	1736	3471	1553	3367	3471	1553
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			55				55
Link Distance (ft)		491			484			1822				1161
Travel Time (s)		9.6			9.4			22.6				14.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	9	190	132	289	182	432	197	1436	446	418	1041	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	9	190	132	289	182	432	197	1436	446	418	1041	9
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24				24
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA	Free	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	Free
Protected Phases		4		3	8	1	5	2	3	1	6	
Permitted Phases	4		Free			8			2			Free
Detector Phase	4	4		3	8	1	5	2	3	1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	14.0	7.0	7.0	14.0	
Minimum Split (s)	14.0	14.0		14.0	14.0	14.0	14.0	21.0	14.0	14.0	21.0	
Total Split (s)	21.0	21.0		19.0	40.0	23.0	30.0	67.0	19.0	23.0	60.0	
Total Split (%)	16.2%	16.2%		14.6%	30.8%	17.7%	23.1%	51.5%	14.6%	17.7%	46.2%	
Maximum Green (s)	14.0	14.0		12.0	33.0	16.0	23.0	60.0	12.0	16.0	53.0	
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	-2.0		-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	0.0	
Total Lost Time (s)	7.0	5.0		5.0	5.0	5.0	7.0	7.0	5.0	5.0	7.0	
Lead/Lag	Lag	Lag		Lead		Lag	Lead	Lead	Lead	Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes		Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Min	None	None	C-Min	

Lanes, Volumes, Timings
 12: US 321 & US 321 A/Riverbend Rd

NCDOT TIP-U4700

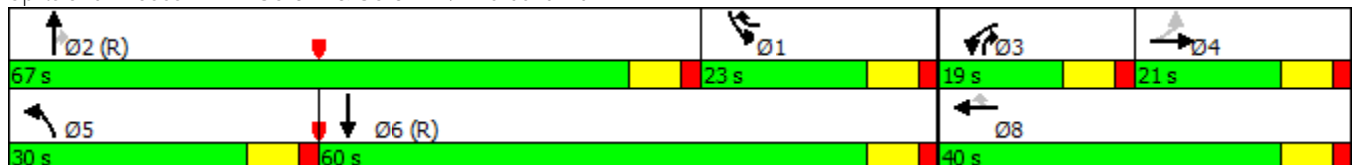


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effect Green (s)	13.9	15.9	130.0	14.3	35.2	59.5	19.1	58.5	74.8	19.3	56.6	130.0
Actuated g/C Ratio	0.11	0.12	1.00	0.11	0.27	0.46	0.15	0.45	0.58	0.15	0.44	1.00
v/c Ratio	0.07	0.84	0.08	0.77	0.36	0.60	0.77	0.92	0.50	0.84	0.69	0.01
Control Delay	53.8	86.2	0.1	70.4	40.9	31.0	61.3	25.5	8.1	69.5	33.1	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.8	86.2	0.1	70.4	40.9	31.0	61.3	25.5	8.1	69.5	33.1	0.0
LOS	D	F	A	E	D	C	E	C	A	E	C	A
Approach Delay		51.0			45.6			25.2				43.3
Approach LOS		D			D			C				D
Queue Length 50th (ft)	7	159	0	124	125	273	151	519	157	180	369	0
Queue Length 95th (ft)	25	#288	0	#187	194	387	m163	m580	m173	#273	468	0
Internal Link Dist (ft)		411			404			1742			1081	
Turn Bay Length (ft)	200		100				250		200	350		200
Base Capacity (vph)	127	227	1568	377	504	725	307	1602	893	500	1512	1553
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.84	0.08	0.77	0.36	0.60	0.64	0.90	0.50	0.84	0.69	0.01

Intersection Summary

Area Type: Other
 Cycle Length: 130
 Actuated Cycle Length: 130
 Offset: 91 (70%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 36.4 Intersection LOS: D
 Intersection Capacity Utilization 81.5% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: US 321 & US 321 A/Riverbend Rd



Summary of All Intervals

Run Number	1	10	2	3	4	5	6
Start Time	4:50	4:50	4:50	4:50	4:50	4:50	4:50
End Time	6:00	6:00	6:00	6:00	6:00	6:00	6:00
Total Time (min)	70	70	70	70	70	70	70
Time Recorded (min)	60	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1	1
Vehs Entered	9473	9628	9203	9410	9533	9341	9498
Vehs Exited	9233	9321	8953	9087	9219	9124	9092
Starting Vehs	817	779	766	728	756	815	716
Ending Vehs	1057	1086	1016	1051	1070	1032	1122
Denied Entry Before	2	2	0	4	1	0	1
Denied Entry After	286	325	441	296	394	352	356
Travel Distance (mi)	19513	19873	19054	19545	19245	19497	19161
Travel Time (hr)	1112.7	1183.6	1127.6	1060.7	1103.6	1082.4	1145.3
Total Delay (hr)	681.5	745.3	705.9	628.0	676.0	651.2	721.0
Total Stops	23978	24542	22695	23902	21495	22088	25182
Fuel Used (gal)	794.9	820.5	783.0	781.9	785.2	786.1	791.3

Summary of All Intervals

Run Number	7	8	9	Avg
Start Time	4:50	4:50	4:50	4:50
End Time	6:00	6:00	6:00	6:00
Total Time (min)	70	70	70	70
Time Recorded (min)	60	60	60	60
# of Intervals	2	2	2	2
# of Recorded Intervals	1	1	1	1
Vehs Entered	9450	9463	9491	9449
Vehs Exited	9137	9250	9138	9152
Starting Vehs	769	803	768	766
Ending Vehs	1082	1016	1121	1055
Denied Entry Before	3	4	0	0
Denied Entry After	382	363	375	355
Travel Distance (mi)	19586	19349	19116	19394
Travel Time (hr)	1134.6	1157.7	1110.7	1121.9
Total Delay (hr)	700.7	727.6	686.5	692.4
Total Stops	23530	23353	24014	23477
Fuel Used (gal)	803.4	801.1	781.1	792.8

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	

Run Number	1	10	2	3	4	5	6
Vehs Entered	9473	9628	9203	9410	9533	9341	9498
Vehs Exited	9233	9321	8953	9087	9219	9124	9092
Starting Vehs	817	779	766	728	756	815	716
Ending Vehs	1057	1086	1016	1051	1070	1032	1122
Denied Entry Before	2	2	0	4	1	0	1
Denied Entry After	286	325	441	296	394	352	356
Travel Distance (mi)	19513	19873	19054	19545	19245	19497	19161
Travel Time (hr)	1112.7	1183.6	1127.6	1060.7	1103.6	1082.4	1145.3
Total Delay (hr)	681.5	745.3	705.9	628.0	676.0	651.2	721.0
Total Stops	23978	24542	22695	23902	21495	22088	25182
Fuel Used (gal)	794.9	820.5	783.0	781.9	785.2	786.1	791.3

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	

Run Number	7	8	9	Avg
Vehs Entered	9450	9463	9491	9449
Vehs Exited	9137	9250	9138	9152
Starting Vehs	769	803	768	766
Ending Vehs	1082	1016	1121	1055
Denied Entry Before	3	4	0	0
Denied Entry After	382	363	375	355
Travel Distance (mi)	19586	19349	19116	19394
Travel Time (hr)	1134.6	1157.7	1110.7	1121.9
Total Delay (hr)	700.7	727.6	686.5	692.4
Total Stops	23530	23353	24014	23477
Fuel Used (gal)	803.4	801.1	781.1	792.8

Total Network Performance By Run

Run Number	1	10	2	3	4	5	6
Denied Delay (hr)	103.1	147.0	163.4	81.5	137.9	120.6	124.4
Denied Del/Veh (s)	38.0	53.2	61.0	30.2	50.0	44.8	45.4
Total Delay (hr)	578.4	598.2	542.5	546.5	538.1	530.6	596.6
Total Del/Veh (s)	202.4	206.9	195.9	194.1	188.3	188.1	210.3
Stop Delay (hr)	361.5	374.3	342.0	323.5	339.9	323.3	377.5
Stop Del/Veh (s)	126.5	129.5	123.5	114.9	118.9	114.6	133.1
Total Stops	23978	24542	22695	23902	21495	22088	25182
Stop/Veh	2.33	2.36	2.28	2.36	2.09	2.17	2.47
Travel Dist (mi)	19513.4	19873.4	19054.1	19545.1	19245.0	19497.1	19160.8
Travel Time (hr)	1112.7	1183.6	1127.6	1060.7	1103.6	1082.4	1145.3
Avg Speed (mph)	19	19	20	20	20	20	19
Vehicles Entered	9473	9628	9203	9410	9533	9341	9498
Vehicles Exited	9233	9321	8953	9087	9219	9124	9092
Hourly Exit Rate	9233	9321	8953	9087	9219	9124	9092
Input Volume	66386	66386	66386	66386	66386	66386	66386
% of Volume	14	14	13	14	14	14	14
Denied Entry Before	2	2	0	4	1	0	1
Denied Entry After	286	325	441	296	394	352	356

Total Network Performance By Run

Run Number	7	8	9	Avg
Denied Delay (hr)	139.0	157.2	126.4	130.1
Denied Del/Veh (s)	50.9	57.6	46.1	47.8
Total Delay (hr)	561.8	570.4	560.1	562.3
Total Del/Veh (s)	197.9	200.0	196.5	198.3
Stop Delay (hr)	344.7	353.3	348.9	348.9
Stop Del/Veh (s)	121.4	123.9	122.4	123.1
Total Stops	23530	23353	24014	23477
Stop/Veh	2.30	2.27	2.34	2.30
Travel Dist (mi)	19585.9	19348.8	19115.9	19393.9
Travel Time (hr)	1134.6	1157.7	1110.7	1121.9
Avg Speed (mph)	20	19	19	20
Vehicles Entered	9450	9463	9491	9449
Vehicles Exited	9137	9250	9138	9152
Hourly Exit Rate	9137	9250	9138	9152
Input Volume	66386	66386	66386	66386
% of Volume	14	14	14	14
Denied Entry Before	3	4	0	0
Denied Entry After	382	363	375	355

Intersection: 1: US 321 & 13th Street SW

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	LT	TR	L	L	T	R	L	T	T	R	L
Maximum Queue (ft)	138	171	120	138	169	112	98	227	646	666	16	159
Average Queue (ft)	23	91	16	24	91	39	9	27	343	351	1	61
95th Queue (ft)	88	154	61	96	154	88	60	123	533	546	9	126
Link Distance (ft)		409	409		504	504			2831	2831		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	300			150			200	200			1000	250
Storage Blk Time (%)				0	1	0			20			
Queuing Penalty (veh)				0	1	0			3			

Intersection: 1: US 321 & 13th Street SW

Movement	SB	SB	SB
Directions Served	T	T	T
Maximum Queue (ft)	327	332	317
Average Queue (ft)	69	79	74
95th Queue (ft)	211	217	206
Link Distance (ft)	1853	1853	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			400
Storage Blk Time (%)	2	0	0
Queuing Penalty (veh)	2	0	0

Intersection: 2: US 321 & Main Avenue Drive NW

Movement	EB	WB	NB	NB	NB	SB	SB
Directions Served	LTR	LTR	L	T	TR	L	TR
Maximum Queue (ft)	872	733	86	196	197	46	16
Average Queue (ft)	748	401	21	7	7	9	1
95th Queue (ft)	1081	824	57	186	189	30	8
Link Distance (ft)	841	788		1853	1853		1020
Upstream Blk Time (%)	72	11		0	0		
Queuing Penalty (veh)	0	0		0	1		
Storage Bay Dist (ft)			200			150	
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 3: US 321 & 2nd Avenue NW

Movement	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	TR	LT	TR	L	T	T	R	L	T	TR
Maximum Queue (ft)	417	586	289	263	350	706	730	381	271	593	598
Average Queue (ft)	180	407	205	164	119	353	373	39	54	290	307
95th Queue (ft)	527	698	302	280	288	673	692	238	159	542	558
Link Distance (ft)	570	570	258	258		1020	1020			2420	2420
Upstream Blk Time (%)	8	26	13	6		0					
Queuing Penalty (veh)	0	0	0	0		0					
Storage Bay Dist (ft)					250			500	400		
Storage Blk Time (%)					0	20	5			6	
Queuing Penalty (veh)					0	19	4			3	

Intersection: 4: US 321 & 7th Avenue NW

Movement	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	T	T	R	L	T	TR
Maximum Queue (ft)	312	104	116	166	720	748	300	42	464	487
Average Queue (ft)	171	38	47	43	405	429	76	7	130	144
95th Queue (ft)	283	85	104	126	711	730	270	28	322	339
Link Distance (ft)	356		94		2420	2420			967	967
Upstream Blk Time (%)	0	3	4							
Queuing Penalty (veh)	0	0	0							
Storage Bay Dist (ft)		150		100			200	150		
Storage Blk Time (%)		3	4	1	25	23			7	
Queuing Penalty (veh)		2	2	6	9	13			0	

Intersection: 5: US 321 & 9th Avenue NW

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR
Maximum Queue (ft)	318	291	88	121	249	905	907	160	468	478
Average Queue (ft)	216	136	31	48	63	511	524	53	135	136
95th Queue (ft)	339	287	71	98	190	947	958	120	342	345
Link Distance (ft)		297		302		967	967		874	874
Upstream Blk Time (%)	11	7				1	1			
Queuing Penalty (veh)	0	0				5	7			
Storage Bay Dist (ft)	300		150		150			100		
Storage Blk Time (%)	11	7		0		39		3	18	
Queuing Penalty (veh)	15	14		0		15		20	10	

Intersection: 6: US 321 & Clement Boulevard

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	SB	SB
Directions Served	L	LT	R	L	LT	R	L	T	T	R	L	L
Maximum Queue (ft)	402	461	85	235	330	400	499	877	889	350	426	431
Average Queue (ft)	195	290	23	76	187	267	97	586	609	147	265	270
95th Queue (ft)	472	542	63	210	334	444	353	983	1018	431	400	405
Link Distance (ft)		850	850		385	385		874	874			
Upstream Blk Time (%)				0	2	14		0	1			
Queuing Penalty (veh)				0	0	0		3	7			
Storage Bay Dist (ft)	600			300			400			250	600	600
Storage Blk Time (%)	0	3			0			24	39			
Queuing Penalty (veh)	0	1			0			12	53			

Intersection: 6: US 321 & Clement Boulevard

Movement	SB	SB
Directions Served	T	TR
Maximum Queue (ft)	522	530
Average Queue (ft)	234	245
95th Queue (ft)	428	442
Link Distance (ft)	964	964
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)	0	
Queuing Penalty (veh)	1	

Intersection: 7: US 321 & 13th Avenue Drive NW

Movement	EB	NB	NB	NB	SB	SB
Directions Served	R	L	T	T	T	TR
Maximum Queue (ft)	65	192	803	814	9	7
Average Queue (ft)	22	13	507	512	0	0
95th Queue (ft)	52	106	977	983	6	6
Link Distance (ft)	196		964	964	1656	1656
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)		200				
Storage Blk Time (%)			36			
Queuing Penalty (veh)			1			

Intersection: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	LT	R	L	T	T	R	L	T	T
Maximum Queue (ft)	74	38	65	427	200	225	1684	1692	227	233	327	314
Average Queue (ft)	24	7	12	223	176	21	1504	1514	21	125	139	137
95th Queue (ft)	59	26	41	464	236	134	2049	2052	125	209	280	274
Link Distance (ft)		546		415			1656	1656			3294	3294
Upstream Blk Time (%)				5			5	6				
Queuing Penalty (veh)				0			60	75				
Storage Bay Dist (ft)	250		250		100	300			150	900		
Storage Blk Time (%)				3	44		41	39				2
Queuing Penalty (veh)				12	14		4	6				0

Intersection: 8: US 321 & 20th Avenue NW/15th Avenue NW/14th Avenue NW

Movement	SB
Directions Served	R
Maximum Queue (ft)	56
Average Queue (ft)	4
95th Queue (ft)	35
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	200
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 9: US 321 & Grace Chapel Road

Movement	WB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	U	T	T	R	L	T	T
Maximum Queue (ft)	175	202	57	30	472	472	266	85	278	288
Average Queue (ft)	77	125	20	4	219	228	25	27	118	108
95th Queue (ft)	168	191	51	20	396	407	165	66	232	223
Link Distance (ft)		928			1174	1174			3473	3473
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	250		250	300			300	400		
Storage Blk Time (%)					2	2				
Queuing Penalty (veh)					0	11				

Intersection: 10: US 321 & Alex Lee Blvd

Movement	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB	
Directions Served	LT	R	LT	R	L	T	T	R	L	T	TR	
Maximum Queue (ft)	50	70	280	58	52	432	431	104	109	179	183	
Average Queue (ft)	13	16	117	9	4	191	210	16	26	44	45	
95th Queue (ft)	40	50	230	34	34	398	416	70	74	128	130	
Link Distance (ft)	217	217	618	618		3473	3473			1153	1153	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)					200				350	450		
Storage Blk Time (%)							6	1				
Queuing Penalty (veh)							0	1				

Intersection: 11: US 321 & Pooveys Grove Church Road

Movement	WB
Directions Served	R
Maximum Queue (ft)	72
Average Queue (ft)	24
95th Queue (ft)	57
Link Distance (ft)	398
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 12: US 321 & US 321 A/Riverbend Rd

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB	SB
Directions Served	L	T	R	L	L	T	R	L	T	T	R	L
Maximum Queue (ft)	93	374	200	225	198	196	327	332	512	546	300	262
Average Queue (ft)	11	182	56	156	111	89	159	131	260	273	159	146
95th Queue (ft)	64	347	194	216	204	168	279	252	434	465	325	235
Link Distance (ft)		435		418	418	418	418		1744	1744		
Upstream Blk Time (%)		1					0					
Queuing Penalty (veh)		0					0					
Storage Bay Dist (ft)	200		100					250			200	350
Storage Blk Time (%)		41	0					0	9	15	1	0
Queuing Penalty (veh)		52	0					3	16	60	6	0

Intersection: 12: US 321 & US 321 A/Riverbend Rd

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	297	329	318
Average Queue (ft)	186	213	193
95th Queue (ft)	266	310	287
Link Distance (ft)		1113	1113
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	350		
Storage Blk Time (%)	0	0	6
Queuing Penalty (veh)	0	1	1

Intersection: 158: Bend

Movement	SB
Directions Served	T
Maximum Queue (ft)	277
Average Queue (ft)	9
95th Queue (ft)	273
Link Distance (ft)	2831
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 554

APPENDIX D

**2040 Build Alternative 13th Street – One-Way Split Partial Cloverleaf
Interchange Synchro, SimTraffic & HCS Reports**

HCM Unsignalized Intersection Capacity Analysis
 3: US 321 & US 321 NB Off Ramp

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑↑	↑↑↑	↗		↘
Traffic Volume (veh/h)	0	1857	1534	116	0	59
Future Volume (Veh/h)	0	1857	1534	116	0	59
Sign Control		Free	Free		Yield	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	2063	1704	129	0	66
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1704				2392	568
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1704				2392	568
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	86
cM capacity (veh/h)	356				27	458

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	WB 4	SB 1
Volume Total	688	688	688	568	568	568	129	66
Volume Left	0	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	129	66
cSH	1700	1700	1700	1700	1700	1700	1700	458
Volume to Capacity	0.40	0.40	0.40	0.33	0.33	0.33	0.08	0.14
Queue Length 95th (ft)	0	0	0	0	0	0	0	12
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	14.2
Lane LOS								B
Approach Delay (s)	0.0			0.0				14.2
Approach LOS								B

Intersection Summary			
Average Delay		0.2	
Intersection Capacity Utilization	40.0%		ICU Level of Service A
Analysis Period (min)	15		

HCM Unsignalized Intersection Capacity Analysis
 10: 15 St SW/15th St Connector & 1st Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↔↔			↕			↕	↕
Traffic Volume (veh/h)	0	0	0	4	413	32	8	66	0	0	144	38
Future Volume (Veh/h)	0	0	0	4	413	32	8	66	0	0	144	38
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	4	459	36	9	73	0	0	160	42
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												4
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	495	0			338			503	0	522	485	248
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	495	0			338			503	0	522	485	248
tC, single (s)	4.2	4.2			7.6			6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2	2.2			3.6			4.1	3.4	3.5	4.0	3.3
p0 queue free %	100	100			98			84	100	100	66	94
cM capacity (veh/h)	1058	1607			404			460	1071	378	473	744

Direction, Lane #	WB 1	WB 2	NB 1	SB 1
Volume Total	234	266	82	202
Volume Left	4	0	9	0
Volume Right	0	36	0	42
cSH	1607	1700	453	597
Volume to Capacity	0.00	0.16	0.18	0.34
Queue Length 95th (ft)	0	0	16	37
Control Delay (s)	0.1	0.0	14.7	15.1
Lane LOS	A		B	C
Approach Delay (s)	0.1		14.7	15.1
Approach LOS			B	C

Intersection Summary			
Average Delay		5.5	
Intersection Capacity Utilization	29.8%	ICU Level of Service	A
Analysis Period (min)	15		

Intersection Sign configuration not allowed in HCM analysis.

Intersection Sign configuration not allowed in HCM analysis.

HCM Unsignalized Intersection Capacity Analysis
 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	66	258	9	0	0	0	0	4	13	149	4	0
Future Volume (Veh/h)	66	258	9	0	0	0	0	4	13	149	4	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	73	287	10	0	0	0	0	4	14	166	4	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			297			440	438	148	306	443	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			297			440	438	148	306	443	0
tC, single (s)	4.2			4.2			7.7	6.7	7.1	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	95			100			100	99	98	71	99	100
cM capacity (veh/h)	1614			1254			467	475	853	579	476	1071
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	216	154	18	170								
Volume Left	73	0	0	166								
Volume Right	0	10	14	0								
cSH	1614	1700	725	576								
Volume to Capacity	0.05	0.09	0.02	0.30								
Queue Length 95th (ft)	4	0	2	31								
Control Delay (s)	2.7	0.0	10.1	13.8								
Lane LOS	A		B	B								
Approach Delay (s)	1.6		10.1	13.8								
Approach LOS			B	B								
Intersection Summary												
Average Delay			5.6									
Intersection Capacity Utilization			31.1%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

33: 14th Street SW & 2nd Avenue SW

NCDOT TIP-U4700



















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	126	197	141	0	0	0	0	16	126	35	40	0
Future Volume (Veh/h)	126	197	141	0	0	0	0	16	126	35	40	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	140	219	157	0	0	0	0	18	140	39	44	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			376			600	578	188	538	656	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			376			600	578	188	538	656	0
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	91			100			100	95	83	88	87	100
cM capacity (veh/h)	1614			1172			323	387	819	315	347	1078
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	250	266	158	83								
Volume Left	140	0	0	39								
Volume Right	0	157	140	0								
cSH	1614	1700	726	331								
Volume to Capacity	0.09	0.16	0.22	0.25								
Queue Length 95th (ft)	7	0	21	24								
Control Delay (s)	4.5	0.0	11.3	19.5								
Lane LOS	A		B	C								
Approach Delay (s)	2.2		11.3	19.5								
Approach LOS			B	C								
Intersection Summary												
Average Delay			6.0									
Intersection Capacity Utilization			36.3%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

34: 2nd Avenue SW & 13th Street SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	25	360	39	0	0	0	0	48	68	4	20	0
Future Volume (Veh/h)	25	360	39	0	0	0	0	48	68	4	20	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	28	400	43	0	0	0	0	53	76	4	22	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									4			
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			443			488	478	222	320	499	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			443			488	478	222	320	499	0
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	98			100			100	89	90	99	95	100
cM capacity (veh/h)	1614			1099			429	466	767	484	453	1068
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	228	243	129	26								
Volume Left	28	0	0	4								
Volume Right	0	43	76	0								
cSH	1614	1700	1135	458								
Volume to Capacity	0.02	0.14	0.11	0.06								
Queue Length 95th (ft)	1	0	10	5								
Control Delay (s)	1.0	0.0	11.6	13.3								
Lane LOS	A		B	B								
Approach Delay (s)	0.5		11.6	13.3								
Approach LOS			B	B								
Intersection Summary												
Average Delay			3.3									
Intersection Capacity Utilization			29.5%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 37: 14th Street SW & 1st Avenue SW
















NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↕↕			↕			↕	
Traffic Volume (veh/h)	0	0	0	44	456	15	74	61	0	0	35	65
Future Volume (Veh/h)	0	0	0	44	456	15	74	61	0	0	35	65
Sign Control		Free			Free			Stop		Stop		
Grade		0%			0%			0%		0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	49	507	17	82	68	0	0	39	72
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	524	0			443		622	0	648	614	262	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	524	0			443		622	0	648	614	262	
tC, single (s)	4.1	4.1			7.6		6.6	7.0	7.6	6.6	7.0	
tC, 2 stage (s)												
tF (s)	2.2	2.2			3.5		4.0	3.3	3.5	4.0	3.3	
p0 queue free %	100	97			80		82	100	100	90	90	
cM capacity (veh/h)	1039	1622			402		385	1078	299	392	734	
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	302	270	150	111								
Volume Left	49	0	82	0								
Volume Right	0	17	0	72								
cSH	1622	1700	394	561								
Volume to Capacity	0.03	0.16	0.38	0.20								
Queue Length 95th (ft)	2	0	44	18								
Control Delay (s)	1.4	0.0	19.6	13.0								
Lane LOS	A		C	B								
Approach Delay (s)	0.7		19.6	13.0								
Approach LOS			C	B								
Intersection Summary												
Average Delay	5.8											
Intersection Capacity Utilization	35.0%			ICU Level of Service					A			
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
 38: 13th Street SW & 1st Avenue SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	30	578	4	62	4	0	0	4	4
Future Volume (Veh/h)	0	0	0	30	578	4	62	4	0	0	4	4
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	33	642	4	69	4	0	0	4	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	646	0			393			712	0	712	710	323
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	646	0			393			712	0	712	710	323
tC, single (s)	4.2	4.2			7.6			6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2	2.2			3.6			4.1	3.4	3.6	4.1	3.4
p0 queue free %	100	98			87			99	100	100	99	99
cM capacity (veh/h)	922	1607			513			339	1068	304	342	661
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	354	325	73	8								
Volume Left	33	0	69	0								
Volume Right	0	4	0	4								
cSH	1607	1700	499	451								
Volume to Capacity	0.02	0.19	0.15	0.02								
Queue Length 95th (ft)	2	0	13	1								
Control Delay (s)	0.8	0.0	13.5	13.1								
Lane LOS	A		B	B								
Approach Delay (s)	0.4		13.5	13.1								
Approach LOS			B	B								
Intersection Summary												
Average Delay	1.8											
Intersection Capacity Utilization	34.0%			ICU Level of Service					A			
Analysis Period (min)	15											

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5064
Vehs Exited	5086
Starting Vehs	136
Ending Vehs	114
Travel Distance (mi)	4804
Travel Time (hr)	126.7
Total Delay (hr)	10.0
Total Stops	1127
Fuel Used (gal)	157.8

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5064
Vehs Exited	5086
Starting Vehs	136
Ending Vehs	114
Travel Distance (mi)	4804
Travel Time (hr)	126.7
Total Delay (hr)	10.0
Total Stops	1127
Fuel Used (gal)	157.8

Intersection: 3: US 321 & US 321 NB Off Ramp

Movement	SB
Directions Served	R
Maximum Queue (ft)	68
Average Queue (ft)	17
95th Queue (ft)	48
Link Distance (ft)	556
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: 15 St SW/15th St Connector & 1st Avenue SW

Movement	NB	SB	SB
Directions Served	LT	T	R
Maximum Queue (ft)	72	93	53
Average Queue (ft)	33	46	25
95th Queue (ft)	52	73	49
Link Distance (ft)	285	433	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			100
Storage Blk Time (%)		0	
Queuing Penalty (veh)		0	

Intersection: 15: US 321 & 13th Street U-turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 16: 15th St Connector & US 321

Movement	NE
Directions Served	R
Maximum Queue (ft)	31
Average Queue (ft)	12
95th Queue (ft)	34
Link Distance (ft)	433
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	68	72
Average Queue (ft)	15	43
95th Queue (ft)	45	68
Link Distance (ft)	298	285
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 33: 14th Street SW & 2nd Avenue SW

Movement	EB	NB	SB
Directions Served	TR	TR	LT
Maximum Queue (ft)	18	63	50
Average Queue (ft)	1	31	27
95th Queue (ft)	6	52	41
Link Distance (ft)	618	349	269
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 34: 2nd Avenue SW & 13th Street SW

Movement	NB	NB	SB
Directions Served	T	R	LT
Maximum Queue (ft)	47	74	27
Average Queue (ft)	22	26	14
95th Queue (ft)	44	48	35
Link Distance (ft)	556		304
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		100	
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	85	82
Average Queue (ft)	41	27
95th Queue (ft)	74	58
Link Distance (ft)	269	446
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 38: 13th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	86	29
Average Queue (ft)	26	8
95th Queue (ft)	54	29
Link Distance (ft)	304	179
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

HCM Unsignalized Intersection Capacity Analysis
 3: US 321 & US 321 NB Off Ramp

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR					
Lane Configurations		↑↑↑	↑↑↑	↗		↘					
Traffic Volume (veh/h)	0	1511	1850	102	0	86					
Future Volume (Veh/h)	0	1511	1850	102	0	86					
Sign Control		Free	Free		Yield						
Grade		0%	0%		0%						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90					
Hourly flow rate (vph)	0	1679	2056	113	0	96					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type		None	None								
Median storage (veh)											
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2056				2616	685					
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
vCu, unblocked vol	2056				2616	685					
tC, single (s)	4.2				6.9	7.0					
tC, 2 stage (s)											
tF (s)	2.2				3.5	3.3					
p0 queue free %	100				100	75					
cM capacity (veh/h)	258				19	383					
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	WB 4	SB 1			
Volume Total	560	560	560	685	685	685	113	96			
Volume Left	0	0	0	0	0	0	0	0			
Volume Right	0	0	0	0	0	0	113	96			
cSH	1700	1700	1700	1700	1700	1700	1700	383			
Volume to Capacity	0.33	0.33	0.33	0.40	0.40	0.40	0.07	0.25			
Queue Length 95th (ft)	0	0	0	0	0	0	0	24			
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	17.5			
Lane LOS									C		
Approach Delay (s)	0.0				0.0			17.5			
Approach LOS									C		
Intersection Summary											
Average Delay			0.4								
Intersection Capacity Utilization			47.7%	ICU Level of Service				A			
Analysis Period (min)			15								

HCM Unsignalized Intersection Capacity Analysis
 10: 15 St SW/15th St Connector & 1st Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↕↕			↕			↕	↕
Traffic Volume (veh/h)	0	0	0	4	436	59	5	45	0	0	178	17
Future Volume (Veh/h)	0	0	0	4	436	59	5	45	0	0	178	17
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	4	484	66	6	50	0	0	198	19
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												4
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	550			0			358	558	0	550	525	275
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	550			0			358	558	0	550	525	275
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.5	4.0	3.3
p0 queue free %	100			100			98	88	100	100	56	97
cM capacity (veh/h)	1009			1607			358	427	1071	374	448	714
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	246	308	56	217								
Volume Left	4	0	6	0								
Volume Right	0	66	0	19								
cSH	1607	1700	418	491								
Volume to Capacity	0.00	0.18	0.13	0.44								
Queue Length 95th (ft)	0	0	11	56								
Control Delay (s)	0.1	0.0	14.9	18.4								
Lane LOS	A		B	C								
Approach Delay (s)	0.1		14.9	18.4								
Approach LOS			B	C								
Intersection Summary												
Average Delay			5.9									
Intersection Capacity Utilization			30.7%	ICU Level of Service	A							
Analysis Period (min)			15									

Intersection Sign configuration not allowed in HCM analysis.

Intersection Sign configuration not allowed in HCM analysis.

HCM Unsignalized Intersection Capacity Analysis
 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	45	277	13	0	0	0	0	4	9	187	4	0
Future Volume (Veh/h)	45	277	13	0	0	0	0	4	9	187	4	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	50	308	14	0	0	0	0	4	10	208	4	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			322			417	415	161	266	422	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			322			417	415	161	266	422	0
tC, single (s)	4.2			4.2			7.7	6.7	7.1	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	97			100			100	99	99	67	99	100
cM capacity (veh/h)	1614			1227			491	497	837	628	497	1071
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	204	168	14	212								
Volume Left	50	0	0	208								
Volume Right	0	14	10	0								
cSH	1614	1700	700	625								
Volume to Capacity	0.03	0.10	0.02	0.34								
Queue Length 95th (ft)	2	0	2	37								
Control Delay (s)	2.0	0.0	10.2	13.7								
Lane LOS	A		B	B								
Approach Delay (s)	1.1		10.2	13.7								
Approach LOS			B	B								
Intersection Summary												
Average Delay			5.8									
Intersection Capacity Utilization			33.3%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

33: 14th Street SW & 2nd Avenue SW

















NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	60	239	176	0	0	0	0	5	92	79	82	0
Future Volume (Veh/h)	60	239	176	0	0	0	0	5	92	79	82	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	67	266	196	0	0	0	0	6	102	88	91	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			462			544	498	231	372	596	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			462			544	498	231	372	596	0
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	96			100			100	99	87	81	77	100
cM capacity (veh/h)	1614			1088			335	451	768	461	394	1078
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	200	329	108	179								
Volume Left	67	0	0	88								
Volume Right	0	196	102	0								
cSH	1614	1700	739	424								
Volume to Capacity	0.04	0.19	0.15	0.42								
Queue Length 95th (ft)	3	0	13	51								
Control Delay (s)	2.7	0.0	10.7	19.5								
Lane LOS	A		B	C								
Approach Delay (s)	1.0		10.7	19.5								
Approach LOS			B	C								
Intersection Summary												
Average Delay			6.4									
Intersection Capacity Utilization			36.0%		ICU Level of Service				A			
Analysis Period (min)			15									
















HCM Unsignalized Intersection Capacity Analysis
 34: 2nd Avenue SW & 13th Street SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	11	367	36	0	0	0	0	24	78	6	50	0
Future Volume (Veh/h)	11	367	36	0	0	0	0	24	78	6	50	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	12	408	40	0	0	0	0	27	87	7	56	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									4			
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			448			480	452	224	285	472	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			448			480	452	224	285	472	0
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	99			100			100	94	89	99	88	100
cM capacity (veh/h)	1614			1095			414	487	764	534	474	1068
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	216	244	114	63								
Volume Left	12	0	0	7								
Volume Right	0	40	87	0								
cSH	1614	1700	1001	480								
Volume to Capacity	0.01	0.14	0.11	0.13								
Queue Length 95th (ft)	1	0	10	11								
Control Delay (s)	0.5	0.0	10.9	13.6								
Lane LOS	A		B	B								
Approach Delay (s)	0.2		10.9	13.6								
Approach LOS			B	B								
Intersection Summary												
Average Delay			3.5									
Intersection Capacity Utilization			29.8%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 37: 14th Street SW & 1st Avenue SW
















NCDOT TIP-U4700

																			
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR							
Lane Configurations																			
Traffic Volume (veh/h)	0	0	0	121	497	66	15	50	0	0	38	25							
Future Volume (Veh/h)	0	0	0	121	497	66	15	50	0	0	38	25							
Sign Control		Free			Free			Stop			Stop								
Grade		0%			0%			0%			0%								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90							
Hourly flow rate (vph)	0	0	0	134	552	73	17	56	0	0	42	28							
Pedestrians																			
Lane Width (ft)																			
Walking Speed (ft/s)																			
Percent Blockage																			
Right turn flare (veh)																			
Median type	None				None														
Median storage (veh)																			
Upstream signal (ft)																			
pX, platoon unblocked																			
vC, conflicting volume	625			0				593		893		0		884		856		312	
vC1, stage 1 conf vol																			
vC2, stage 2 conf vol																			
vCu, unblocked vol	625			0				593		893		0		884		856		312	
tC, single (s)	4.1			4.1				7.6		6.6		7.0		7.6		6.6		7.0	
tC, 2 stage (s)																			
tF (s)	2.2			2.2				3.5		4.0		3.3		3.5		4.0		3.3	
p0 queue free %	100			92				94		78		100		100		84		96	
cM capacity (veh/h)	952			1622				305		253		1078		185		268		680	
Direction, Lane #	WB 1	WB 2	NB 1	SB 1															
Volume Total	410	349	73	70															
Volume Left	134	0	17	0															
Volume Right	0	73	0	28															
cSH	1622	1700	263	353															
Volume to Capacity	0.08	0.21	0.28	0.20															
Queue Length 95th (ft)	7	0	27	18															
Control Delay (s)	2.9	0.0	23.8	17.7															
Lane LOS	A		C	C															
Approach Delay (s)	1.6		23.8	17.7															
Approach LOS			C	C															
Intersection Summary																			
Average Delay				4.6															
Intersection Capacity Utilization				36.2%						ICU Level of Service			A						
Analysis Period (min)				15															

HCM Unsignalized Intersection Capacity Analysis

38: 13th Street SW & 1st Avenue SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	61	603	8	33	4	0	0	4	4
Future Volume (Veh/h)	0	0	0	61	603	8	33	4	0	0	4	4
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	68	670	9	37	4	0	0	4	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	679			0			477	815	0	812	810	340
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	679			0			477	815	0	812	810	340
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	100			96			92	99	100	100	99	99
cM capacity (veh/h)	896			1607			438	288	1068	252	292	645
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	403	344	41	8								
Volume Left	68	0	37	0								
Volume Right	0	9	0	4								
cSH	1607	1700	417	402								
Volume to Capacity	0.04	0.20	0.10	0.02								
Queue Length 95th (ft)	3	0	8	2								
Control Delay (s)	1.6	0.0	14.6	14.1								
Lane LOS	A		B	B								
Approach Delay (s)	0.8		14.6	14.1								
Approach LOS			B	B								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utilization			34.1%		ICU Level of Service				A			
Analysis Period (min)			15									

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5024
Vehs Exited	5031
Starting Vehs	127
Ending Vehs	120
Travel Distance (mi)	4774
Travel Time (hr)	123.6
Total Delay (hr)	9.6
Total Stops	1024
Fuel Used (gal)	155.7

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5024
Vehs Exited	5031
Starting Vehs	127
Ending Vehs	120
Travel Distance (mi)	4774
Travel Time (hr)	123.6
Total Delay (hr)	9.6
Total Stops	1024
Fuel Used (gal)	155.7

Intersection: 3: US 321 & US 321 NB Off Ramp

Movement	SB
Directions Served	R
Maximum Queue (ft)	53
Average Queue (ft)	27
95th Queue (ft)	52
Link Distance (ft)	556
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: 15 St SW/15th St Connector & 1st Avenue SW

Movement	WB	NB	SB	SB
Directions Served	TR	LT	T	R
Maximum Queue (ft)	19	31	72	49
Average Queue (ft)	1	27	42	11
95th Queue (ft)	6	44	67	37
Link Distance (ft)	683	285	433	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				100
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 15: US 321 & 13th Street U-turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 16: 15th St Connector & US 321

Movement	NE
Directions Served	R
Maximum Queue (ft)	31
Average Queue (ft)	8
95th Queue (ft)	28
Link Distance (ft)	433
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	49	93
Average Queue (ft)	11	44
95th Queue (ft)	37	74
Link Distance (ft)	298	285
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 33: 14th Street SW & 2nd Avenue SW

Movement	EB	NB	SB
Directions Served	TR	TR	LT
Maximum Queue (ft)	17	45	142
Average Queue (ft)	1	23	48
95th Queue (ft)	6	42	97
Link Distance (ft)	618	349	269
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 34: 2nd Avenue SW & 13th Street SW

Movement	EB	NB	NB	SB
Directions Served	LT	T	R	LT
Maximum Queue (ft)	26	28	73	49
Average Queue (ft)	1	12	29	24
95th Queue (ft)	9	33	56	39
Link Distance (ft)	527	556		304
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)			100	
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	107	64
Average Queue (ft)	32	19
95th Queue (ft)	72	36
Link Distance (ft)	269	446
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 38: 13th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	49	49
Average Queue (ft)	18	9
95th Queue (ft)	41	33
Link Distance (ft)	304	179
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1650		V _D = 59 veh/h		
		Ramp Volume, V _R			116				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1650	0.90	Rolling	5	0	0.930	1.00	1971	
Ramp	116	0.90	Rolling	5	0	0.930	1.00	139	
UpStream									
DownStream	59	0.90	Rolling	5	0	0.930	1.00	70	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1971 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1971	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1832	Exhibit 13-8	4500	No
					V _R	139	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1971	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 20.3 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.441 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.3 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.3 mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1952		V _D = 86 veh/h		
		Ramp Volume, V _R			102				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1952	0.90	Rolling	5	0	0.930	1.00	2332	
Ramp	102	0.90	Rolling	5	0	0.930	1.00	122	
UpStream									
DownStream	86	0.90	Rolling	5	0	0.930	1.00	103	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2332 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2332	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2210	Exhibit 13-8	4500	No
					V _R	122	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2332	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 23.4 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.439 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.3 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.3 mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp	
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = 200 ft		Deceleration Lane Length L _D						L _{down} = ft	
V _u = 182 veh/h		Freeway Volume, V _F			1759			V _D = veh/h	
		Ramp Volume, V _R			98				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1759	0.90	Rolling	5	0	0.930	1.00	2101	
Ramp	98	0.90	Rolling	5	0	0.930	1.00	117	
UpStream	182	0.90	Rolling	5	0	0.930	1.00	217	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 2101 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2218	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2218	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 19.6 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	0.322 (Exhibit 13-11)				D _S =	(Exhibit 13-12)			
S _R =	50.8 mph (Exhibit 13-11)				S _R =	mph (Exhibit 13-12)			
S ₀ =	N/A mph (Exhibit 13-11)				S ₀ =	mph (Exhibit 13-12)			
S =	50.8 mph (Exhibit 13-13)				S =	mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off		Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 200 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 195 veh/h		Freeway Volume, V _F			1407		V _D = veh/h		
		Ramp Volume, V _R			104				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1407	0.90	Rolling	5	0	0.930	1.00	1681	
Ramp	104	0.90	Rolling	5	0	0.930	1.00	124	
UpStream	195	0.90	Rolling	5	0	0.930	1.00	233	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1681 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1805	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1805	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 16.4 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	0.310 (Exhibit 13-11)				D _S =	(Exhibit 13-12)			
S _R =	51.0 mph (Exhibit 13-11)				S _R =	mph (Exhibit 13-12)			
S ₀ =	N/A mph (Exhibit 13-11)				S ₀ =	mph (Exhibit 13-12)			
S =	51.0 mph (Exhibit 13-13)				S =	mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	203 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp	
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = 200 ft		Deceleration Lane Length L _D						L _{down} = ft	
V _u = 116 veh/h		Freeway Volume, V _F			1650			V _D = veh/h	
		Ramp Volume, V _R			59				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1650	0.90	Rolling	5	0	0.930	1.00	1971	
Ramp	59	0.90	Rolling	5	0	0.930	1.00	70	
UpStream	116	0.90	Rolling	5	0	0.930	1.00	139	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 1971 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2041	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2041	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 18.2 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.316 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.9 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.9 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	203 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp	
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = 200 ft		Deceleration Lane Length L _D						L _{down} = ft	
V _u = 102 veh/h		Freeway Volume, V _F			1850			V _D = veh/h	
		Ramp Volume, V _R			86				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1850	0.90	Rolling	5	0	0.930	1.00	2210	
Ramp	86	0.90	Rolling	5	0	0.930	1.00	103	
UpStream	102	0.90	Rolling	5	0	0.930	1.00	122	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 2210 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2313	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2313	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 20.3 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = C (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.325 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.8 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.8 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1941		V _D = 98 veh/h		
		Ramp Volume, V _R			182				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1941	0.90	Rolling	5	0	0.930	1.00	2318	
Ramp	182	0.90	Rolling	5	0	0.930	1.00	217	
UpStream									
DownStream	98	0.90	Rolling	5	0	0.930	1.00	117	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2318 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2318	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2101	Exhibit 13-8	4500	No
					V _R	217	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2318	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 23.3 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.448 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.2 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.2 mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1602		V _D = 104 veh/h		
		Ramp Volume, V _R			195				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1602	0.90	Rolling	5	0	0.930	1.00	1913	
Ramp	195	0.90	Rolling	5	0	0.930	1.00	233	
UpStream									
DownStream	104	0.90	Rolling	5	0	0.930	1.00	124	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1913 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1913	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1680	Exhibit 13-8	4500	No
					V _R	233	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1913	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 19.8 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.449 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.2 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.2 mph (Exhibit 13-13)			

APPENDIX E

**2040 Build Alternative 13th Street – Partial Cloverleaf Interchange
Synchro, SimTraffic & HCS Reports**

HCM Unsignalized Intersection Capacity Analysis
 1: 13th Street SW & US 321 On/Off Ramp

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	9	104	127	127	68	100
Future Volume (Veh/h)	9	104	127	127	68	100
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	10	116	141	141	76	111
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	419	141			282	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	419	141			282	
tC, single (s)	6.9	7.0			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	87			94	
cM capacity (veh/h)	524	875			1263	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	126	94	188	113	74	
Volume Left	10	0	0	76	0	
Volume Right	116	0	141	0	0	
cSH	831	1700	1700	1263	1700	
Volume to Capacity	0.15	0.06	0.11	0.06	0.04	
Queue Length 95th (ft)	13	0	0	5	0	
Control Delay (s)	10.1	0.0	0.0	5.6	0.0	
Lane LOS	B		A			
Approach Delay (s)	10.1	0.0		3.4		
Approach LOS	B					
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utilization			29.3%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

2: 13th Street SW & US 321 SB Off Ramp

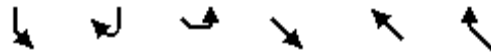
NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	90	92	16	164	11	98
Future Volume (Veh/h)	90	92	16	164	11	98
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	100	102	18	182	12	109
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	194	60	121			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	194	60	121			
tC, single (s)	6.9	7.0	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	87	90	99			
cM capacity (veh/h)	762	986	1450			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	202	79	121	8	113	
Volume Left	100	18	0	0	0	
Volume Right	102	0	0	0	109	
cSH	861	1450	1700	1700	1700	
Volume to Capacity	0.23	0.01	0.07	0.00	0.07	
Queue Length 95th (ft)	23	1	0	0	0	
Control Delay (s)	10.5	1.8	0.0	0.0	0.0	
Lane LOS	B	A				
Approach Delay (s)	10.5	0.7		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization			25.6%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 3: US 321 & US 321 NB Off Ramp

NCDOT TIP-U4700



Movement	SBL	SBR	SEL	SET	NWT	NWR		
Lane Configurations		↗		↖↖↖	↖↖↖	↖		
Traffic Volume (veh/h)	0	195	0	1873	1407	113		
Future Volume (Veh/h)	0	195	0	1873	1407	113		
Sign Control	Yield			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly flow rate (vph)	0	217	0	2081	1563	126		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type				None	None			
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	2257	521	1563					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	2257	521	1563					
tC, single (s)	6.8	6.9	4.1					
tC, 2 stage (s)								
tF (s)	3.5	3.3	2.2					
p0 queue free %	100	57	100					
cM capacity (veh/h)	35	500	419					
Direction, Lane #	SB 1	SE 1	SE 2	SE 3	NW 1	NW 2	NW 3	NW 4
Volume Total	217	694	694	694	521	521	521	126
Volume Left	0	0	0	0	0	0	0	0
Volume Right	217	0	0	0	0	0	0	126
cSH	500	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.43	0.41	0.41	0.41	0.31	0.31	0.31	0.07
Queue Length 95th (ft)	54	0	0	0	0	0	0	0
Control Delay (s)	17.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	C							
Approach Delay (s)	17.6	0.0				0.0		
Approach LOS	C							
Intersection Summary								
Average Delay	1.0							
Intersection Capacity Utilization	45.9%			ICU Level of Service			A	
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis
 4: US 321 & US 321 SB Off Ramp

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations		↗		↑↑↑	↓↓↓	↘		
Traffic Volume (veh/h)	0	114	0	1602	1759	182		
Future Volume (Veh/h)	0	114	0	1602	1759	182		
Sign Control	Yield			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly flow rate (vph)	0	127	0	1780	1954	202		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type				None	None			
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	2547	651	1954					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	2547	651	1954					
tC, single (s)	6.8	6.9	4.1					
tC, 2 stage (s)								
tF (s)	3.5	3.3	2.2					
p0 queue free %	100	69	100					
cM capacity (veh/h)	22	411	295					
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4
Volume Total	127	593	593	593	651	651	651	202
Volume Left	0	0	0	0	0	0	0	0
Volume Right	127	0	0	0	0	0	0	202
cSH	411	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.31	0.35	0.35	0.35	0.38	0.38	0.38	0.12
Queue Length 95th (ft)	32	0	0	0	0	0	0	0
Control Delay (s)	17.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	C							
Approach Delay (s)	17.6	0.0				0.0		
Approach LOS	C							
Intersection Summary								
Average Delay			0.6					
Intersection Capacity Utilization			47.7%	ICU Level of Service			A	
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis
 10: 15 St SW & 1st Avenue SW

NCDOT TIP-U4700



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				↕↕	↕	
Traffic Volume (veh/h)	0	0	4	461	8	0
Future Volume (Veh/h)	0	0	4	461	8	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	4	512	9	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			0		264	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0		264	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	100
cM capacity (veh/h)			1622		701	1084
Direction, Lane #	WB 1	WB 2	NB 1			
Volume Total	175	341	9			
Volume Left	4	0	9			
Volume Right	0	0	0			
cSH	1622	1700	701			
Volume to Capacity	0.00	0.20	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.2	0.0	10.2			
Lane LOS	A		B			
Approach Delay (s)	0.1		10.2			
Approach LOS			B			
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			22.9%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection Sign configuration not allowed in HCM analysis.

HCM Unsignalized Intersection Capacity Analysis
 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	8	324	9	0	0	0	0	4	13	5	4	0
Future Volume (Veh/h)	8	324	9	0	0	0	0	4	13	5	4	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	9	360	10	0	0	0	0	4	14	6	4	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			370			385	383	185	214	388	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			370			385	383	185	214	388	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			100			100	99	98	99	99	100
cM capacity (veh/h)	1622			1185			543	546	826	705	542	1084
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	189	190	18	10								
Volume Left	9	0	0	6								
Volume Right	0	10	14	0								
cSH	1622	1700	741	629								
Volume to Capacity	0.01	0.11	0.02	0.02								
Queue Length 95th (ft)	0	0	2	1								
Control Delay (s)	0.4	0.0	10.0	10.8								
Lane LOS	A		A	B								
Approach Delay (s)	0.2		10.0	10.8								
Approach LOS			A	B								
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utilization			20.8%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

33: 14th Street SW & 2nd Avenue SW

NCDOT TIP-U4700




















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	126	224	36	0	0	0	0	4	105	35	33	0
Future Volume (Veh/h)	126	224	36	0	0	0	0	4	105	35	33	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	140	249	40	0	0	0	0	4	117	39	37	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			289			568	549	144	524	569	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			289			568	549	144	524	569	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	91			100			100	99	87	89	91	100
cM capacity (veh/h)	1622			1270			352	404	877	351	393	1084

Direction, Lane #	EB 1	EB 2	NB 1	SB 1
Volume Total	264	164	121	76
Volume Left	140	0	0	39
Volume Right	0	40	117	0
cSH	1622	1700	844	370
Volume to Capacity	0.09	0.10	0.14	0.21
Queue Length 95th (ft)	7	0	12	19
Control Delay (s)	4.3	0.0	10.0	17.2
Lane LOS	A		A	C
Approach Delay (s)	2.6		10.0	17.2
Approach LOS			A	C

Intersection Summary			
Average Delay		5.8	
Intersection Capacity Utilization	28.0%		ICU Level of Service
Analysis Period (min)	15		A

HCM Unsignalized Intersection Capacity Analysis
 34: 2nd Avenue SW & 13th Street SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	25	287	118	0	0	0	0	102	141	4	59	0
Future Volume (Veh/h)	25	287	118	0	0	0	0	102	141	4	59	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	28	319	131	0	0	0	0	113	157	4	66	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			450			474	440	225	429	506	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			450			474	440	225	429	506	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			100			100	77	80	99	86	100
cM capacity (veh/h)	1622			1107			416	501	778	332	459	1084
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2						
Volume Total	188	290	113	157	4	66						
Volume Left	28	0	0	0	4	0						
Volume Right	0	131	0	157	0	0						
cSH	1622	1700	501	778	332	459						
Volume to Capacity	0.02	0.17	0.23	0.20	0.01	0.14						
Queue Length 95th (ft)	1	0	21	19	1	12						
Control Delay (s)	1.2	0.0	14.3	10.8	16.0	14.1						
Lane LOS	A		B	B	C	B						
Approach Delay (s)	0.5		12.3		14.3							
Approach LOS			B		B							
Intersection Summary												
Average Delay			5.5									
Intersection Capacity Utilization			34.5%	ICU Level of Service	A							
Analysis Period (min)			15									

Lanes, Volumes, Timings
35: 17th Ave NW & 1st Avenue SW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↘			↕	
Traffic Volume (vph)	37	161	11	27	139	78	22	52	8	46	53	31
Future Volume (vph)	37	161	11	27	139	78	22	52	8	46	53	31
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	75		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	50			50			50			50		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.993			0.957			0.980			0.968	
Flt Protected		0.991			0.994		0.950				0.983	
Satd. Flow (prot)	0	1833	0	0	1772	0	1770	1825	0	0	1772	0
Flt Permitted		0.917			0.955		0.696				0.855	
Satd. Flow (perm)	0	1696	0	0	1702	0	1296	1825	0	0	1542	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		634			290			373			463	
Travel Time (s)		12.4			5.6			7.3			9.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	41	179	12	30	154	87	24	58	9	51	59	34
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	232	0	0	271	0	24	67	0	0	144	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		12.0	12.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	19.0	19.0		19.0	19.0		14.0	14.0		14.0	14.0	
Total Split (s)	37.0	37.0		37.0	37.0		23.0	23.0		23.0	23.0	
Total Split (%)	61.7%	61.7%		61.7%	61.7%		38.3%	38.3%		38.3%	38.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		16.0	16.0		16.0	16.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		-2.0			-2.0		-2.0	-2.0			-2.0	
Total Lost Time (s)		5.0			5.0		5.0	5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Min	C-Min		C-Min	C-Min		None	None		None	None	
Act Effct Green (s)		41.0			41.0		12.8	12.8			12.8	
Actuated g/C Ratio		0.68			0.68		0.21	0.21			0.21	
v/c Ratio		0.20			0.23		0.09	0.17			0.44	
Control Delay		6.1			6.3		17.9	19.0			23.9	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		6.1			6.3		17.9	19.0			23.9	
LOS		A			A		B	B			C	
Approach Delay		6.1			6.3			18.7			23.9	
Approach LOS		A			A			B			C	

Lanes, Volumes, Timings
 35: 17th Ave NW & 1st Avenue SW

NCDOT TIP-U4700

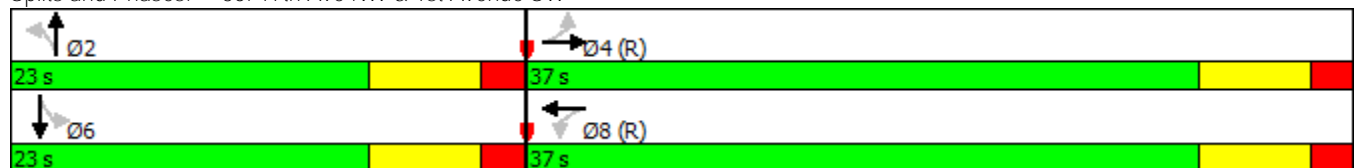


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		31			37		7	20				46
Queue Length 95th (ft)		72			85		21	43				83
Internal Link Dist (ft)		554			210			293				383
Turn Bay Length (ft)							75					
Base Capacity (vph)		1158			1162		388	547				462
Starvation Cap Reductn		0			0		0	0				0
Spillback Cap Reductn		0			0		0	0				0
Storage Cap Reductn		0			0		0	0				0
Reduced v/c Ratio		0.20			0.23		0.06	0.12				0.31

Intersection Summary
















Area Type:	Other
Cycle Length:	60
Actuated Cycle Length:	60
Offset:	29 (48%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
Natural Cycle:	40
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.44
Intersection Signal Delay:	11.2
Intersection LOS:	B
Intersection Capacity Utilization	39.8%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 35: 17th Ave NW & 1st Avenue SW



HCM Unsignalized Intersection Capacity Analysis
 37: 14th Street SW & 1st Avenue SW

NCDOT TIP-U4700

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (veh/h)	0	0	0	37	478	15	58	61	0	0	35	65	
Future Volume (Veh/h)	0	0	0	37	478	15	58	61	0	0	35	65	
Sign Control		Free			Free			Stop			Stop		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	0	0	0	41	531	17	64	68	0	0	39	72	
Pedestrians													
Lane Width (ft)													
Walking Speed (ft/s)													
Percent Blockage													
Right turn flare (veh)													
Median type	None				None								
Median storage (veh)													
Upstream signal (ft)													
pX, platoon unblocked													
vC, conflicting volume	548				0			439	630	0	656	622	274
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	548				0			439	630	0	656	622	274
tC, single (s)	4.1				4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)													
tF (s)	2.2				2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100				97			84	82	100	100	90	90
cM capacity (veh/h)	1018				1622			409	387	1084	298	391	724
Direction, Lane #	WB 1	WB 2	NB 1	SB 1									
Volume Total	306	282	132	111									
Volume Left	41	0	64	0									
Volume Right	0	17	0	72									
cSH	1622	1700	398	557									
Volume to Capacity	0.03	0.17	0.33	0.20									
Queue Length 95th (ft)	2	0	36	18									
Control Delay (s)	1.2	0.0	18.5	13.1									
Lane LOS	A		C	B									
Approach Delay (s)	0.6		18.5	13.1									
Approach LOS			C	B									
Intersection Summary													
Average Delay				5.1									
Intersection Capacity Utilization				34.5%	ICU Level of Service	A							
Analysis Period (min)				15									

HCM Unsignalized Intersection Capacity Analysis
 38: 13th Street SW & 1st Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↔↔			↔			↔	
Traffic Volume (veh/h)	0	0	0	4	523	4	116	4	0	0	4	4
Future Volume (Veh/h)	0	0	0	4	523	4	116	4	0	0	4	4
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	4	581	4	129	4	0	0	4	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	585			0			304	593	0	593	591	292
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	585			0			304	593	0	593	591	292
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			79	99	100	100	99	99
cM capacity (veh/h)	986			1622			616	416	1084	386	417	704
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	294	294	133	8								
Volume Left	4	0	129	0								
Volume Right	0	4	0	4								
cSH	1622	1700	607	524								
Volume to Capacity	0.00	0.17	0.22	0.02								
Queue Length 95th (ft)	0	0	21	1								
Control Delay (s)	0.1	0.0	12.6	12.0								
Lane LOS	A		B	B								
Approach Delay (s)	0.1		12.6	12.0								
Approach LOS			B	B								
Intersection Summary												
Average Delay			2.5									
Intersection Capacity Utilization			34.7%	ICU Level of Service	A							
Analysis Period (min)			15									

Summary of All Intervals

Start Time	6:57
End Time	7:10
Total Time (min)	13
Time Recorded (min)	10
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	956
Vehs Exited	954
Starting Vehs	102
Ending Vehs	104
Travel Distance (mi)	709
Travel Time (hr)	18.0
Total Delay (hr)	1.7
Total Stops	281
Fuel Used (gal)	23.0

Interval #0 Information Seeding

Start Time	6:57
End Time	7:00
Total Time (min)	3
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	7:10
Total Time (min)	10
Volumes adjusted by Growth Factors.	
Vehs Entered	956
Vehs Exited	954
Starting Vehs	102
Ending Vehs	104
Travel Distance (mi)	709
Travel Time (hr)	18.0
Total Delay (hr)	1.7
Total Stops	281
Fuel Used (gal)	23.0

Intersection: 1: 13th Street SW & US 321 On/Off Ramp

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	77	30
Average Queue (ft)	37	12
95th Queue (ft)	71	35
Link Distance (ft)	527	145
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 2: 13th Street SW & US 321 SB Off Ramp

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (ft)	79	53
Average Queue (ft)	60	11
95th Queue (ft)	90	46
Link Distance (ft)	514	866
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 3: NB US 321 & US 321 NB Off Ramp

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement	EB
Directions Served	R
Maximum Queue (ft)	171
Average Queue (ft)	34
95th Queue (ft)	147
Link Distance (ft)	514
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	L
Maximum Queue (ft)	28
Average Queue (ft)	11
95th Queue (ft)	32
Link Distance (ft)	258
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 15: NB US 321 & 13th Street U-turn

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 32: 15 St SW & 2nd Avenue SW & 1st Street SW Connector

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	28	26
Average Queue (ft)	11	10
95th Queue (ft)	33	31
Link Distance (ft)	295	258
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 33: 14th Street SW & 2nd Avenue SW

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	40	26
Average Queue (ft)	25	20
95th Queue (ft)	41	37
Link Distance (ft)	349	268
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: 2nd Avenue SW & 13th Street SW

Movement	NB	NB	SB
Directions Served	T	R	T
Maximum Queue (ft)	67	87	49
Average Queue (ft)	35	31	28
95th Queue (ft)	66	77	56
Link Distance (ft)	145	145	292
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	47	61
Average Queue (ft)	29	26
95th Queue (ft)	44	55
Link Distance (ft)	268	446
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 38: 13th Street SW & 1st Avenue SW

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	49	29
Average Queue (ft)	36	11
95th Queue (ft)	54	35
Link Distance (ft)	292	179
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

HCM Unsignalized Intersection Capacity Analysis
 1: 13th Street SW & US 321 On/Off Ramp

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	16	98	88	91	90	141
Future Volume (Veh/h)	16	98	88	91	90	141
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	18	109	98	101	100	157
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	427	100			199	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	427	100			199	
tC, single (s)	6.9	7.0			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	88			93	
cM capacity (veh/h)	510	930			1356	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	127	65	134	152	105	
Volume Left	18	0	0	100	0	
Volume Right	109	0	101	0	0	
cSH	833	1700	1700	1356	1700	
Volume to Capacity	0.15	0.04	0.08	0.07	0.06	
Queue Length 95th (ft)	13	0	0	6	0	
Control Delay (s)	10.1	0.0	0.0	5.4	0.0	
Lane LOS	B		A			
Approach Delay (s)	10.1	0.0		3.2		
Approach LOS	B					
Intersection Summary						
Average Delay			3.6			
Intersection Capacity Utilization			28.8%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

2: 13th Street SW & US 321 SB Off Ramp

NCDOT TIP-U4700

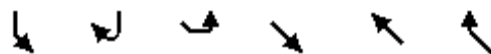


Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	68	127	9	111	53	104
Future Volume (Veh/h)	68	127	9	111	53	104
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	76	141	10	123	59	116
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	198	88	175			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	198	88	175			
tC, single (s)	6.9	7.0	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	90	85	99			
cM capacity (veh/h)	760	947	1384			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	217	51	82	39	136	
Volume Left	76	10	0	0	0	
Volume Right	141	0	0	0	116	
cSH	872	1384	1700	1700	1700	
Volume to Capacity	0.25	0.01	0.05	0.02	0.08	
Queue Length 95th (ft)	25	1	0	0	0	
Control Delay (s)	10.5	1.5	0.0	0.0	0.0	
Lane LOS	B	A				
Approach Delay (s)	10.5	0.6		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			4.5			
Intersection Capacity Utilization			28.2%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

3: US 321 & US 321 NB Off Ramp

NCDOT TIP-U4700



Movement	SBL	SBR	SEL	SET	NWT	NWR		
Lane Configurations		↗		↖↖↖	↖↖↖	↗		
Traffic Volume (veh/h)	0	181	0	1520	1759	114		
Future Volume (Veh/h)	0	181	0	1520	1759	114		
Sign Control	Yield			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly flow rate (vph)	0	201	0	1689	1954	127		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type				None	None			
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	2517	651	1954					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	2517	651	1954					
tC, single (s)	6.8	6.9	4.1					
tC, 2 stage (s)								
tF (s)	3.5	3.3	2.2					
p0 queue free %	100	51	100					
cM capacity (veh/h)	23	411	295					
Direction, Lane #	SB 1	SE 1	SE 2	SE 3	NW 1	NW 2	NW 3	NW 4
Volume Total	201	563	563	563	651	651	651	127
Volume Left	0	0	0	0	0	0	0	0
Volume Right	201	0	0	0	0	0	0	127
cSH	411	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.49	0.33	0.33	0.33	0.38	0.38	0.38	0.07
Queue Length 95th (ft)	65	0	0	0	0	0	0	0
Control Delay (s)	21.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	C							
Approach Delay (s)	21.9	0.0				0.0		
Approach LOS	C							
Intersection Summary								
Average Delay			1.1					
Intersection Capacity Utilization			51.9%	ICU Level of Service	A			
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis

4: US 321 & US 321 SB Off Ramp

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations		↗		↑↑↑	↓↓↓	↘		
Traffic Volume (veh/h)	0	113	0	1940	1407	195		
Future Volume (Veh/h)	0	113	0	1940	1407	195		
Sign Control	Yield			Free	Free			
Grade	0%			0%	0%			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly flow rate (vph)	0	126	0	2156	1563	217		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type				None	None			
Median storage (veh)								
Upstream signal (ft)								
pX, platoon unblocked								
vC, conflicting volume	2282	521	1563					
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	2282	521	1563					
tC, single (s)	6.8	6.9	4.1					
tC, 2 stage (s)								
tF (s)	3.5	3.3	2.2					
p0 queue free %	100	75	100					
cM capacity (veh/h)	33	500	419					
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3	SB 4
Volume Total	126	719	719	719	521	521	521	217
Volume Left	0	0	0	0	0	0	0	0
Volume Right	126	0	0	0	0	0	0	217
cSH	500	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.25	0.42	0.42	0.42	0.31	0.31	0.31	0.13
Queue Length 95th (ft)	25	0	0	0	0	0	0	0
Control Delay (s)	14.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	B							
Approach Delay (s)	14.6	0.0	0.0					
Approach LOS	B							
Intersection Summary								
Average Delay			0.5					
Intersection Capacity Utilization			40.8%	ICU Level of Service				A
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis
 10: 15 St SW & 1st Avenue SW

NCDOT TIP-U4700



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				↕↕	↗	
Traffic Volume (veh/h)	0	0	4	453	5	0
Future Volume (Veh/h)	0	0	4	453	5	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	4	503	6	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			0		260	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0		260	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	100
cM capacity (veh/h)			1622		705	1084
Direction, Lane #	WB 1	WB 2	NB 1			
Volume Total	172	335	6			
Volume Left	4	0	6			
Volume Right	0	0	0			
cSH	1622	1700	705			
Volume to Capacity	0.00	0.20	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.2	0.0	10.1			
Lane LOS	A		B			
Approach Delay (s)	0.1		10.1			
Approach LOS			B			
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			22.6%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection Sign configuration not allowed in HCM analysis.

HCM Unsignalized Intersection Capacity Analysis
 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	5	322	13	0	0	0	0	4	9	9	4	0
Future Volume (Veh/h)	5	322	13	0	0	0	0	4	9	9	4	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	358	14	0	0	0	0	4	10	10	4	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			372			379	377	186	203	384	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			372			379	377	186	203	384	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	99	99	99	99	100
cM capacity (veh/h)	1622			1183			549	551	824	722	546	1084
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	185	193	14	14								
Volume Left	6	0	0	10								
Volume Right	0	14	10	0								
cSH	1622	1700	722	661								
Volume to Capacity	0.00	0.11	0.02	0.02								
Queue Length 95th (ft)	0	0	1	2								
Control Delay (s)	0.3	0.0	10.1	10.6								
Lane LOS	A		B	B								
Approach Delay (s)	0.1		10.1	10.6								
Approach LOS			B	B								
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization			23.5%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

33: 14th Street SW & 2nd Avenue SW

NCDOT TIP-U4700




















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔						↔			↔	
Traffic Volume (veh/h)	60	249	33	0	0	0	0	4	77	79	61	0
Future Volume (Veh/h)	60	249	33	0	0	0	0	4	77	79	61	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	67	277	37	0	0	0	0	4	86	88	68	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			314			464	430	157	360	448	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			314			464	430	157	360	448	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	96			100			100	99	90	82	86	100
cM capacity (veh/h)	1622			1243			417	495	861	494	484	1084

Direction, Lane #	EB 1	EB 2	NB 1	SB 1
Volume Total	206	176	90	156
Volume Left	67	0	0	88
Volume Right	0	37	86	0
cSH	1622	1700	833	489
Volume to Capacity	0.04	0.10	0.11	0.32
Queue Length 95th (ft)	3	0	9	34
Control Delay (s)	2.6	0.0	9.8	15.8
Lane LOS	A		A	C
Approach Delay (s)	1.4		9.8	15.8
Approach LOS			A	C

Intersection Summary			
Average Delay		6.2	
Intersection Capacity Utilization		30.6%	ICU Level of Service
Analysis Period (min)		15	A

HCM Unsignalized Intersection Capacity Analysis
 34: 2nd Avenue SW & 13th Street SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	11	301	97	0	0	0	0	46	144	6	130	0
Future Volume (Veh/h)	11	301	97	0	0	0	0	46	144	6	130	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	12	334	108	0	0	0	0	51	160	7	144	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	0			442			484	412	221	376	466	0
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0			442			484	412	221	376	466	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			100			100	90	80	98	71	100
cM capacity (veh/h)	1622			1114			358	525	783	407	489	1084
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2						
Volume Total	179	275	51	160	7	144						
Volume Left	12	0	0	0	7	0						
Volume Right	0	108	0	160	0	0						
cSH	1622	1700	525	783	407	489						
Volume to Capacity	0.01	0.16	0.10	0.20	0.02	0.29						
Queue Length 95th (ft)	1	0	8	19	1	30						
Control Delay (s)	0.5	0.0	12.6	10.8	14.0	15.4						
Lane LOS	A		B	B	B	C						
Approach Delay (s)	0.2		11.2		15.3							
Approach LOS			B		C							
Intersection Summary												
Average Delay			5.9									
Intersection Capacity Utilization			34.0%	ICU Level of Service	A							
Analysis Period (min)			15									

Lanes, Volumes, Timings
 35: 17th Ave NW & 1st Avenue SW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↘			↕	
Traffic Volume (vph)	41	264	18	40	354	75	20	78	15	51	112	42
Future Volume (vph)	41	264	18	40	354	75	20	78	15	51	112	42
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	75		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	50			50			50			50		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.992			0.978			0.975			0.972	
Flt Protected		0.994			0.996		0.950				0.988	
Satd. Flow (prot)	0	1837	0	0	1814	0	1770	1816	0	0	1789	0
Flt Permitted		0.899			0.947		0.568				0.886	
Satd. Flow (perm)	0	1661	0	0	1725	0	1058	1816	0	0	1604	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		634			290			373			463	
Travel Time (s)		12.4			5.6			7.3			9.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	46	293	20	44	393	83	22	87	17	57	124	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	359	0	0	520	0	22	104	0	0	228	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	12.0	12.0		12.0	12.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	21.0	21.0		21.0	21.0		21.0	21.0		21.0	21.0	
Total Split (s)	38.0	38.0		38.0	38.0		22.0	22.0		22.0	22.0	
Total Split (%)	63.3%	63.3%		63.3%	63.3%		36.7%	36.7%		36.7%	36.7%	
Maximum Green (s)	31.0	31.0		31.0	31.0		15.0	15.0		15.0	15.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		-2.0			-2.0		-2.0	-2.0			-2.0	
Total Lost Time (s)		5.0			5.0		5.0	5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Min	C-Min		C-Min	C-Min		None	None		None	None	
Act Effct Green (s)		35.3			35.3		14.7	14.7			14.7	
Actuated g/C Ratio		0.59			0.59		0.24	0.24			0.24	
v/c Ratio		0.37			0.51		0.08	0.23			0.58	
Control Delay		8.5			10.3		16.9	18.5			25.7	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		8.5			10.3		16.9	18.5			25.7	
LOS		A			B		B	B			C	
Approach Delay		8.5			10.3			18.2			25.7	
Approach LOS		A			B			B			C	

Lanes, Volumes, Timings
 35: 17th Ave NW & 1st Avenue SW

NCDOT TIP-U4700

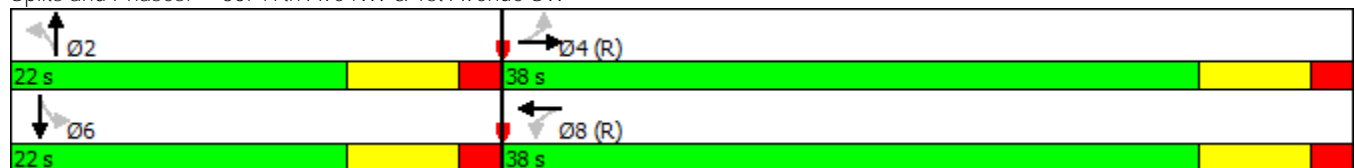


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		62			101		6	29				71
Queue Length 95th (ft)		117			185		21	61				127
Internal Link Dist (ft)		554			210			293				383
Turn Bay Length (ft)							75					
Base Capacity (vph)		979			1017		301	518				457
Starvation Cap Reductn		0			0		0	0				0
Spillback Cap Reductn		0			0		0	0				0
Storage Cap Reductn		0			0		0	0				0
Reduced v/c Ratio		0.37			0.51		0.07	0.20				0.50

Intersection Summary

Area Type:	Other
Cycle Length:	60
Actuated Cycle Length:	60
Offset:	0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
Natural Cycle:	45
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	13.4
Intersection LOS:	B
Intersection Capacity Utilization	57.0%
ICU Level of Service	B
Analysis Period (min)	15

Splits and Phases: 35: 17th Ave NW & 1st Avenue SW



HCM Unsignalized Intersection Capacity Analysis

37: 14th Street SW & 1st Avenue SW
















NCDOT TIP-U4700



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↔			↔			↔	
Traffic Volume (veh/h)	0	0	0	100	460	66	10	50	0	0	38	25
Future Volume (Veh/h)	0	0	0	100	460	66	10	50	0	0	38	25
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	111	511	73	11	56	0	0	42	28
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None				None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	584			0			526	806	0	798	770	292
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	584			0			526	806	0	798	770	292
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			93			97	81	100	100	86	96
cM capacity (veh/h)	987			1622			354	293	1084	224	307	704
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	366	328	67	70								
Volume Left	111	0	11	0								
Volume Right	0	73	0	28								
cSH	1622	1700	301	397								
Volume to Capacity	0.07	0.19	0.22	0.18								
Queue Length 95th (ft)	6	0	21	16								
Control Delay (s)	2.7	0.0	20.3	16.0								
Lane LOS	A		C	C								
Approach Delay (s)	1.4		20.3	16.0								
Approach LOS			C	C								
Intersection Summary												
Average Delay				4.2								
Intersection Capacity Utilization				34.2%	ICU Level of Service	A						
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis
 38: 13th Street SW & 1st Avenue SW

NCDOT TIP-U4700

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	141	523	8	55	4	0	0	4	4
Future Volume (Veh/h)	0	0	0	141	523	8	55	4	0	0	4	4
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	157	581	9	61	4	0	0	4	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	590	0			610			904	0	902	900	295
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	590	0			610			904	0	902	900	295
tC, single (s)	4.1	4.1			7.5			6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)												
tF (s)	2.2	2.2			3.5			4.0	3.3	3.5	4.0	3.3
p0 queue free %	100	90			82			98	100	100	98	99
cM capacity (veh/h)	982	1622			344			249	1084	213	250	701
Direction, Lane #	WB 1	WB 2	NB 1	SB 1								
Volume Total	448	300	65	8								
Volume Left	157	0	61	0								
Volume Right	0	9	0	4								
cSH	1622	1700	336	369								
Volume to Capacity	0.10	0.18	0.19	0.02								
Queue Length 95th (ft)	8	0	18	2								
Control Delay (s)	3.2	0.0	18.3	15.0								
Lane LOS	A		C	B								
Approach Delay (s)	1.9		18.3	15.0								
Approach LOS			C	B								
Intersection Summary												
Average Delay	3.3											
Intersection Capacity Utilization	35.4%			ICU Level of Service	A							
Analysis Period (min)	15											

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5284
Vehs Exited	5273
Starting Vehs	106
Ending Vehs	117
Travel Distance (mi)	4312
Travel Time (hr)	109.1
Total Delay (hr)	9.3
Total Stops	1467
Fuel Used (gal)	135.6

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5284
Vehs Exited	5273
Starting Vehs	106
Ending Vehs	117
Travel Distance (mi)	4312
Travel Time (hr)	109.1
Total Delay (hr)	9.3
Total Stops	1467
Fuel Used (gal)	135.6

Intersection: 1: 13th Street SW & US 321 On/Off Ramp

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	77	49
Average Queue (ft)	34	5
95th Queue (ft)	59	26
Link Distance (ft)	527	145
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 2: 13th Street SW & US 321 SB Off Ramp

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	78	31	34
Average Queue (ft)	37	7	1
95th Queue (ft)	58	27	11
Link Distance (ft)	514	866	534
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: NB US 321 & US 321 NB Off Ramp

Movement	SB
Directions Served	R
Maximum Queue (ft)	56
Average Queue (ft)	4
95th Queue (ft)	26
Link Distance (ft)	527
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement	EB
Directions Served	R
Maximum Queue (ft)	106
Average Queue (ft)	6
95th Queue (ft)	42
Link Distance (ft)	514
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	L
Maximum Queue (ft)	28
Average Queue (ft)	5
95th Queue (ft)	22
Link Distance (ft)	258
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 15: NB US 321 & 13th Street U-turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 32: 15 St SW & 2nd Avenue SW & 1st Street SW Connector

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	28	26
Average Queue (ft)	14	8
95th Queue (ft)	36	27
Link Distance (ft)	295	258
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 33: 14th Street SW & 2nd Avenue SW

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	39	50
Average Queue (ft)	18	33
95th Queue (ft)	30	53
Link Distance (ft)	349	268
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: 2nd Avenue SW & 13th Street SW

Movement	NB	NB	SB	SB
Directions Served	T	R	L	T
Maximum Queue (ft)	43	64	27	68
Average Queue (ft)	17	32	3	36
95th Queue (ft)	35	58	16	59
Link Distance (ft)	145	145	292	292
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	WB	NB	SB
Directions Served	TR	LT	TR
Maximum Queue (ft)	17	66	37
Average Queue (ft)	1	25	16
95th Queue (ft)	6	50	27
Link Distance (ft)	474	268	446
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 38: 13th Street SW & 1st Avenue SW

Movement	WB	NB	SB
Directions Served	LT	LT	TR
Maximum Queue (ft)	29	48	29
Average Queue (ft)	1	22	9
95th Queue (ft)	10	43	31
Link Distance (ft)	309	292	179
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1520		V _D = 74 veh/h		
		Ramp Volume, V _R			113				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1520	0.90	Rolling	5	0	0.930	1.00	1816	
Ramp	113	0.90	Rolling	5	0	0.930	1.00	135	
UpStream									
DownStream	74	0.90	Rolling	5	0	0.930	1.00	88	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1816 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1816	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1681	Exhibit 13-8	4500	No
					V _R	135	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1816	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 19.0 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.440 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.3 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.3 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB						
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street						
Date Performed	2/10/17		Jurisdiction							
Analysis Time Period	PM Peak		Analysis Year	2040						
Project Description US 321 at 13th Street										
Inputs										
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1			<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A						<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100			L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1873			V _D = 74 veh/h		
		Ramp Volume, V _R			114					
		Freeway Free-Flow Speed, S _{FF}			55.0					
		Ramp Free-Flow Speed, S _{FR}			35.0					
Conversion to pc/h Under Base Conditions										
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p		
Freeway	1873	0.90	Rolling	5	0	0.930	1.00	2237		
Ramp	114	0.90	Rolling	5	0	0.930	1.00	136		
UpStream										
DownStream	74	0.90	Rolling	5	0	0.930	1.00	88		
Merge Areas					Diverge Areas					
Estimation of v ₁₂					Estimation of v ₁₂					
V ₁₂ = V _F (P _{FM})		V ₁₂ = V _R + (V _F - V _R)P _{FD}								
L _{EQ} = (Equation 13-6 or 13-7)		L _{EQ} = (Equation 13-12 or 13-13)								
P _{FM} = using Equation (Exhibit 13-6)		P _{FD} = 1.000 using Equation (Exhibit 13-7)								
V ₁₂ = pc/h		V ₁₂ = 2237 pc/h								
V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17)		V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17)								
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No		Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No								
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No		Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No								
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)		If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)								
Capacity Checks					Capacity Checks					
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?	
V _{FO}		Exhibit 13-8			V _F	2237	Exhibit 13-8	4500	No	
					V _{FO} = V _F - V _R	2101	Exhibit 13-8	4500	No	
					V _R	136	Exhibit 13-10	2000	No	
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area					
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?	
V _{R12}		Exhibit 13-8			V ₁₂	2237	Exhibit 13-8	4400:All	No	
Level of Service Determination (if not F)					Level of Service Determination (if not F)					
D _R = 5.475 + 0.00734 v _R + 0.0078 V ₁₂ - 0.00627 L _A		D _R = 4.252 + 0.0086 V ₁₂ - 0.009 L _D								
D _R = (pc/mi/ln)		D _R = 22.6 (pc/mi/ln)								
LOS = (Exhibit 13-2)		LOS = C (Exhibit 13-2)								
Speed Determination					Speed Determination					
M _S = (Exhibit 13-11)		D _S = 0.440 (Exhibit 13-12)								
S _R = mph (Exhibit 13-11)		S _R = 49.3 mph (Exhibit 13-12)								
S ₀ = mph (Exhibit 13-11)		S ₀ = N/A mph (Exhibit 13-12)								
S = mph (Exhibit 13-13)		S = 49.3 mph (Exhibit 13-13)								

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB
Agency or Company	AECOM	Junction	202 - US 321 at 13th Street
Date Performed	2/10/17	Jurisdiction	
Analysis Time Period	AM Peak	Analysis Year	2040

Project Description US 321 at 13th Street

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off L _{up} = 200 ft V _u = 182 veh/h	Terrain: Rolling S _{FF} = 55.0 mph S _{FR} = 35.0 mph Sketch (show lanes, L _A , L _D , V _R , V _f)	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
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Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1759	0.90	Rolling	5	0	0.930	1.00	2101
Ramp	114	0.90	Rolling	5	0	0.930	1.00	136
UpStream	182	0.90	Rolling	5	0	0.930	1.00	217
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$ (Equation 25-2 or 25-3) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 25-5) V ₁₂ = 2101 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 25-4 or 25-5) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-8)	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 25-8 or 25-9) L _{EQ} = P _{FD} = using Equation (Exhibit 25-12) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 25-15 or 25-16) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-18)
--	--

Capacity Checks

Capacity Checks

	Actual	Capacity	LOS F?		Actual	Capacity	LOS F?
V _{FO}	2237	Exhibit 25-7	No	V _F		Exhibit 25-14	
				V _{FO} = V _F - V _R		Exhibit 25-14	
				V _R		Exhibit 25-3	

Flow Entering Merge Influence Area

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?		Actual	Max Desirable	Violation?
V _{R12}	2237	Exhibit 25-7	4600:All	No	V ₁₂	Exhibit 25-14	

Level of Service Determination (if not F)

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 19.7 (pc/mi/ln) LOS = B (Exhibit 25-4)	$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 25-4)
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Speed Determination

Speed Determination

M _S = 0.323 (Exhibit 25-19)	D _s = (Exhibit 25-19)
S _R = 50.8 mph (Exhibit 25-19)	S _R = mph (Exhibit 25-19)
S ₀ = N/A mph (Exhibit 25-19)	S ₀ = mph (Exhibit 25-19)
S = 50.8 mph (Exhibit 25-14)	S = mph (Exhibit 25-15)

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB
Agency or Company	AECOM	Junction	202 - US 321 at 13th Street
Date Performed	2/10/17	Jurisdiction	
Analysis Time Period	PM Peak	Analysis Year	2040

Project Description US 321 at 13th Street

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off L _{up} = 200 ft V _u = 195 veh/h	Terrain: Rolling S _{FF} = 55.0 mph S _{FR} = 35.0 mph Sketch (show lanes, L _A , L _D , V _R , V _f)	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
---	--	--

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1407	0.90	Rolling	5	0	0.930	1.00	1681
Ramp	195	0.90	Rolling	5	0	0.930	1.00	233
UpStream	195	0.90	Rolling	5	0	0.930	1.00	233
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$
(Equation 25-2 or 25-3)

L_{EQ} =
P_{FM} = 1.000 using Equation (Exhibit 25-5)
V₁₂ = 1681 pc/h
V₃ or V_{av34} = 0 pc/h (Equation 25-4 or 25-5)
Is V₃ or V_{av34} > 2,700 pc/h? Yes No
Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
If Yes, V_{12a} = pc/h (Equation 25-8)

$V_{12} = V_R + (V_F - V_R)P_{FD}$
(Equation 25-8 or 25-9)

L_{EQ} =
P_{FD} = using Equation (Exhibit 25-12)
V₁₂ = pc/h
V₃ or V_{av34} = pc/h (Equation 25-15 or 25-16)
Is V₃ or V_{av34} > 2,700 pc/h? Yes No
Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
If Yes, V_{12a} = pc/h (Equation 25-18)

Capacity Checks

Capacity Checks

	Actual	Capacity	LOS F?		Actual	Capacity	LOS F?
V _{FO}	1914	Exhibit 25-7	No	V _F		Exhibit 25-14	
				V _{FO} = V _F - V _R		Exhibit 25-14	
				V _R		Exhibit 25-3	

Flow Entering Merge Influence Area

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?		Actual	Max Desirable	Violation?
V _{R12}	1914	Exhibit 25-7	4600:All	No	V ₁₂	Exhibit 25-14	

Level of Service Determination (if not F)

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$

D_R = 17.2 (pc/mi/ln)
LOS = B (Exhibit 25-4)

$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$

D_R = (pc/mi/ln)
LOS = (Exhibit 25-4)

Speed Determination

Speed Determination

M _S = 0.312 (Exhibit 25-19)	S _R = 50.9 mph (Exhibit 25-19)	S ₀ = N/A mph (Exhibit 25-19)	S = 50.9 mph (Exhibit 25-14)	D _s = (Exhibit 25-19)	S _R = mph (Exhibit 25-19)	S ₀ = mph (Exhibit 25-19)	S = mph (Exhibit 25-15)
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RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB
Agency or Company	AECOM	Junction	203 - US 321 at 13th Street
Date Performed	2/10/17	Jurisdiction	
Analysis Time Period	AM Peak	Analysis Year	2040

Project Description US 321 at 13th Street

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off L _{up} = 200 ft V _u = 113 veh/h	Terrain: Rolling S _{FF} = 55.0 mph S _{FR} = 35.0 mph Sketch (show lanes, L _A , L _D , V _R , V _f)	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
---	--	--

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1407	0.90	Rolling	5	0	0.930	1.00	1681
Ramp	195	0.90	Rolling	5	0	0.930	1.00	233
UpStream	113	0.90	Rolling	5	0	0.930	1.00	135
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$ (Equation 25-2 or 25-3) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 25-5) V ₁₂ = 1681 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 25-4 or 25-5) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-8)	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 25-8 or 25-9) L _{EQ} = P _{FD} = using Equation (Exhibit 25-12) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 25-15 or 25-16) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-18)
--	--

Capacity Checks

Capacity Checks

	Actual	Capacity	LOS F?		Actual	Capacity	LOS F?
V _{FO}	1914	Exhibit 25-7	No	V _F		Exhibit 25-14	
				V _{FO} = V _F - V _R		Exhibit 25-14	
				V _R		Exhibit 25-3	

Flow Entering Merge Influence Area

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?		Actual	Max Desirable	Violation?
V _{R12}	1914	Exhibit 25-7	4600:All	No	V ₁₂	Exhibit 25-14	

Level of Service Determination (if not F)

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 17.2 (pc/mi/ln) LOS = B (Exhibit 25-4)	$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 25-4)
---	--

Speed Determination

Speed Determination

M _S = 0.312 (Exhibit 25-19)	D _s = (Exhibit 25-19)
S _R = 50.9 mph (Exhibit 25-19)	S _R = mph (Exhibit 25-19)
S ₀ = N/A mph (Exhibit 25-19)	S ₀ = mph (Exhibit 25-19)
S = 50.9 mph (Exhibit 25-14)	S = mph (Exhibit 25-15)

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB
Agency or Company	AECOM	Junction	203 - US 321 at 13th Street
Date Performed	2/10/17	Jurisdiction	
Analysis Time Period	PM Peak	Analysis Year	2040

Project Description US 321 at 13th Street

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off L _{up} = 200 ft V _u = 114 veh/h	Terrain: Rolling S _{FF} = 55.0 mph S _{FR} = 35.0 mph Sketch (show lanes, L _A , L _D , V _R , V _f)	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
---	--	--

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1759	0.90	Rolling	5	0	0.930	1.00	2101
Ramp	181	0.90	Rolling	5	0	0.930	1.00	216
UpStream	114	0.90	Rolling	5	0	0.930	1.00	136
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$ (Equation 25-2 or 25-3) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 25-5) V ₁₂ = 2101 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 25-4 or 25-5) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-8)	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 25-8 or 25-9) L _{EQ} = P _{FD} = using Equation (Exhibit 25-12) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 25-15 or 25-16) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 25-18)
--	--

Capacity Checks

Capacity Checks

	Actual	Capacity	LOS F?		Actual	Capacity	LOS F?
V _{FO}	2317	Exhibit 25-7	No	V _F		Exhibit 25-14	
				V _{FO} = V _F - V _R		Exhibit 25-14	
				V _R		Exhibit 25-3	

Flow Entering Merge Influence Area

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?		Actual	Max Desirable	Violation?
V _{R12}	2317	Exhibit 25-7	4600:All	No	V ₁₂	Exhibit 25-14	

Level of Service Determination (if not F)

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 20.3 (pc/mi/ln) LOS = C (Exhibit 25-4)	$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 25-4)
---	--

Speed Determination

Speed Determination

M _S = 0.326 (Exhibit 25-19)	D _s = (Exhibit 25-19)
S _R = 50.8 mph (Exhibit 25-19)	S _R = mph (Exhibit 25-19)
S ₀ = N/A mph (Exhibit 25-19)	S ₀ = mph (Exhibit 25-19)
S = 50.8 mph (Exhibit 25-14)	S = mph (Exhibit 25-15)

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1941		V _D = 332 veh/h		
		Ramp Volume, V _R			182				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1941	0.90	Rolling	5	0	0.930	1.00	2318	
Ramp	182	0.90	Rolling	5	0	0.930	1.00	217	
UpStream									
DownStream	332	0.90	Rolling	5	0	0.930	1.00	397	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2318 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2318	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2101	Exhibit 13-8	4500	No
					V _R	217	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2318	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 23.3 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.448 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.2 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1602		V _D = 332 veh/h		
		Ramp Volume, V _R			195				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1602	0.90	Rolling	5	0	0.930	1.00	1913	
Ramp	195	0.90	Rolling	5	0	0.930	1.00	233	
UpStream									
DownStream	332	0.90	Rolling	5	0	0.930	1.00	397	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1913 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1913	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1680	Exhibit 13-8	4500	No
					V _R	233	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1913	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 19.8 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.449 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.2 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.2 mph (Exhibit 13-13)			

APPENDIX F

**2040 Build Alternative 13th Street – Two-Way Partial Cloverleaf
Interchange Synchro, SimTraffic & HCS Reports**

Intersection

Int Delay, s/veh 2.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	24	102	76	215	28	114
Future Vol, veh/h	24	102	76	215	28	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	27	113	84	239	31	127

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	393	204	0	0	323	0
Stage 1	204	-	-	-	-	-
Stage 2	189	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	606	829	-	-	1220	-
Stage 1	823	-	-	-	-	-
Stage 2	836	-	-	-	-	-
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	591	829	-	-	1220	-
Mov Cap-2 Maneuver	591	-	-	-	-	-
Stage 1	823	-	-	-	-	-
Stage 2	815	-	-	-	-	-

Approach	WB		NB		SB
HCM Control Delay, s	10.7		0		1.6
HCM LOS	B				

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	770	1220
HCM Lane V/C Ratio	-	-	0.182	0.026
HCM Control Delay (s)	-	-	10.7	8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.7	0.1

Intersection

Int Delay, s/veh 3.9

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	38	156	42	253	44	94
Future Vol, veh/h	38	156	42	253	44	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	0	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	42	173	47	281	49	104

Major/Minor	Minor2	Major1		Major2
Conflicting Flow All	475	101	153	0
Stage 1	101	-	-	-
Stage 2	374	-	-	-
Critical Hdwy	6.45	6.25	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-
Follow-up Hdwy	3.545	3.345	2.245	-
Pot Cap-1 Maneuver	543	946	1409	-
Stage 1	916	-	-	-
Stage 2	689	-	-	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	525	946	1409	-
Mov Cap-2 Maneuver	525	-	-	-
Stage 1	916	-	-	-
Stage 2	666	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	11	1.1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1409	-	818	-	-
HCM Lane V/C Ratio	0.033	-	0.264	-	-
HCM Control Delay (s)	7.6	-	11	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0.1	-	1.1	-	-

32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Intersection

Int Delay, s/veh 5.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔		↔		↔			↔	
Traffic Vol, veh/h	4	4	4	4	0	4	0	4	4	4	9	0
Future Vol, veh/h	4	4	4	4	0	4	0	4	4	4	9	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	0	-	1	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	8	8	8	8	8	8	6	6	6
Mvmt Flow	4	4	4	4	0	4	0	4	4	4	10	0

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	0	0	9	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.13	-	4.18	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.227	-	2.272	-
Pot Cap-1 Maneuver	-	-	1572	0
Stage 1	-	-	-	0
Stage 2	-	-	-	0
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	-	1572	-
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s		3.6	8.8	9.1
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBR	SBLn1
Capacity (veh/h)	946	-	-	-	1572	-	889
HCM Lane V/C Ratio	0.009	-	-	-	0.003	-	0.016
HCM Control Delay (s)	8.8	-	-	-	7.3	-	9.1
HCM Lane LOS	A	-	-	-	A	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0	-	0.1

Intersection

Int Delay, s/veh 5.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	277	88	0	0	0	0	0	199	5	41	0
Future Vol, veh/h	0	277	88	0	0	0	0	0	199	5	41	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	-	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	2	2	2	7	7	7	7	7	7
Mvmt Flow	0	308	98	0	0	0	0	0	221	6	46	0

Major/Minor	Major1			Minor1			Minor2		
Conflicting Flow All	-	0	0	-	-	357	357	406	-
Stage 1	-	-	-	-	-	-	0	0	-
Stage 2	-	-	-	-	-	-	357	406	-
Critical Hdwy	-	-	-	-	-	6.27	7.17	6.57	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.17	5.57	-
Follow-up Hdwy	-	-	-	-	-	3.363	3.563	4.063	-
Pot Cap-1 Maneuver	0	-	-	0	0	676	589	526	0
Stage 1	0	-	-	0	0	-	-	-	0
Stage 2	0	-	-	0	0	-	650	589	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	676	396	526	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	396	526	-
Stage 1	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	437	589	-

Approach	EB	NB	SB
HCM Control Delay, s	0	12.9	12.9
HCM LOS		B	B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	SBLn1
Capacity (veh/h)	676	-	-	508
HCM Lane V/C Ratio	0.327	-	-	0.101
HCM Control Delay (s)	12.9	-	-	12.9
HCM Lane LOS	B	-	-	B
HCM 95th %tile Q(veh)	1.4	-	-	0.3

Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4TB						TB			TB		
Traffic Vol, veh/h	85	390	4	0	0	0	0	8	8	4	4	0
Future Vol, veh/h	85	390	4	0	0	0	0	8	8	4	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	-	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	2	2	2	2	2	2	2	2	2
Mvmt Flow	94	433	4	0	0	0	0	9	9	4	4	0

Major/Minor	Major1			Minor1			Minor2		
Conflicting Flow All	0	0	0	-	624	219	410	627	-
Stage 1	-	-	-	-	624	-	0	0	-
Stage 2	-	-	-	-	0	-	410	627	-
Critical Hdwy	4.18	-	-	-	6.54	6.94	7.54	6.54	-
Critical Hdwy Stg 1	-	-	-	-	5.54	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-
Follow-up Hdwy	2.24	-	-	-	4.02	3.32	3.52	4.02	-
Pot Cap-1 Maneuver	-	-	-	0	400	785	526	399	0
Stage 1	-	-	-	0	476	-	-	-	0
Stage 2	-	-	-	0	-	-	589	474	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	400	785	511	399	-
Mov Cap-2 Maneuver	-	-	-	-	400	-	511	399	-
Stage 1	-	-	-	-	476	-	-	-	-
Stage 2	-	-	-	-	-	-	571	474	-

Approach	EB	NB	SB
HCM Control Delay, s		12	13.2
HCM LOS		B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	SBLn1
Capacity (veh/h)	530	-	-	-	448
HCM Lane V/C Ratio	0.034	-	-	-	0.02
HCM Control Delay (s)	12	-	-	-	13.2
HCM Lane LOS	B	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1

Intersection

Int Delay, s/veh 0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶		↷		↶	
Traffic Vol, veh/h	306	9	4	421	8	18
Future Vol, veh/h	306	9	4	421	8	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	4	4	6	6
Mvmt Flow	340	10	4	468	9	20

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	350	822
Stage 1	-	-	345
Stage 2	-	-	477
Critical Hdwy	-	4.14	6.46
Critical Hdwy Stg 1	-	-	5.46
Critical Hdwy Stg 2	-	-	5.46
Follow-up Hdwy	-	2.236	3.554
Pot Cap-1 Maneuver	-	1198	338
Stage 1	-	-	708
Stage 2	-	-	616
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	1198	336
Mov Cap-2 Maneuver	-	-	336
Stage 1	-	-	708
Stage 2	-	-	613

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	12.3
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	521	-	-	1198	-
HCM Lane V/C Ratio	0.055	-	-	0.004	-
HCM Control Delay (s)	12.3	-	-	8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0.2	-	-	0	-

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	0	484	14	0	4
Future Vol, veh/h	4	0	484	14	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	3	3
Mvmt Flow	4	0	538	16	0	4

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	553	-	546
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.12	-	6.23
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.218	-	3.327
Pot Cap-1 Maneuver	1017	0	536
Stage 1	-	0	-
Stage 2	-	0	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	1017	-	536
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	8.6	0	11.8
HCM LOS			B

Minor Lane/Major Mvmt	EBL	WBT	WBR	SBLn1
Capacity (veh/h)	1017	-	-	536
HCM Lane V/C Ratio	0.004	-	-	0.008
HCM Control Delay (s)	8.6	-	-	11.8
HCM Lane LOS	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	0

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			↶			↷
Traffic Vol, veh/h	0	0	592	4	0	4
Future Vol, veh/h	0	0	592	4	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	-	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	6	6
Mvmt Flow	0	0	658	4	0	4

Major/Minor

	Major2	Minor2
Conflicting Flow All	-	0
Stage 1	-	-
Stage 2	-	-
Critical Hdwy	-	-
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	-
Follow-up Hdwy	-	-
Pot Cap-1 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	-
Mov Cap-2 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-

Approach

	WB	SB
HCM Control Delay, s	0	13
HCM LOS		B

Minor Lane/Major Mvmt

	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	456
HCM Lane V/C Ratio	-	-	0.01
HCM Control Delay (s)	-	-	13
HCM Lane LOS	-	-	B
HCM 95th %tile Q(veh)	-	-	0

Intersection

Int Delay, s/veh 1.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↕↕			↕			↕	
Traffic Vol, veh/h	0	0	0	4	560	4	90	4	0	0	4	4
Future Vol, veh/h	0	0	0	4	560	4	90	4	0	0	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	-	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	4	4	4	2	2	2	2	2	2
Mvmt Flow	0	0	0	4	622	4	100	4	0	0	4	4

Major/Minor	Major2	Minor1	Minor2
Conflicting Flow All	0	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.18	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.24	-	-
Pot Cap-1 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s		12.5	12.3
HCM LOS		B	B

Minor Lane/Major Mvmt	NBLn1	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	585	-	-	-	501
HCM Lane V/C Ratio	0.179	-	-	-	0.018
HCM Control Delay (s)	12.5	-	-	-	12.3
HCM Lane LOS	B	-	-	-	B
HCM 95th %tile Q(veh)	0.6	-	-	-	0.1

Summary of All Intervals

Start Time	6:57
End Time	7:10
Total Time (min)	13
Time Recorded (min)	10
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	904
Vehs Exited	896
Starting Vehs	118
Ending Vehs	126
Travel Distance (mi)	830
Travel Time (hr)	21.8
Total Delay (hr)	2.8
Total Stops	190
Fuel Used (gal)	26.4

Interval #0 Information Seeding

Start Time	6:57
End Time	7:00
Total Time (min)	3
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	7:10
Total Time (min)	10
Volumes adjusted by Growth Factors.	
Vehs Entered	904
Vehs Exited	896
Starting Vehs	118
Ending Vehs	126
Travel Distance (mi)	830
Travel Time (hr)	21.8
Total Delay (hr)	2.8
Total Stops	190
Fuel Used (gal)	26.4

Intersection: 1: 13th Street SW & US 321 On/Off Ramp

Movement	WB	SB
Directions Served	LR	L
Maximum Queue (ft)	49	31
Average Queue (ft)	33	18
95th Queue (ft)	46	43
Link Distance (ft)	545	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 2: 13th Street SW & US 321 SB Off Ramp

Movement	EB
Directions Served	LR
Maximum Queue (ft)	51
Average Queue (ft)	32
95th Queue (ft)	63
Link Distance (ft)	523
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: NB US 321 & US 321 NB Off Ramp

Movement	SB
Directions Served	R
Maximum Queue (ft)	207
Average Queue (ft)	121
95th Queue (ft)	236
Link Distance (ft)	545
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 13: 1st street SW Connector & 1st Avenue SW

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 15: NB US 321 & 13th Street U-turn

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 18: 1st Avenue SW

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	WB	NB	SB
Directions Served	L	TR	LT
Maximum Queue (ft)	27	31	50
Average Queue (ft)	5	12	15
95th Queue (ft)	23	37	48
Link Distance (ft)	244	304	256
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)	0		
Queuing Penalty (veh)	0		

Intersection: 34: 2nd Ave SW/2nd Avenue SW

Movement	NB	SB
Directions Served	R	LT
Maximum Queue (ft)	100	30
Average Queue (ft)	37	17
95th Queue (ft)	98	41
Link Distance (ft)	171	368
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 35: 12th Street SW & 2nd Avenue SW/2nd Avenue

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	23	27
Average Queue (ft)	14	11
95th Queue (ft)	32	32
Link Distance (ft)	140	272
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 36: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	LR
Maximum Queue (ft)	51
Average Queue (ft)	21
95th Queue (ft)	53
Link Distance (ft)	256
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 37: 1st Avenue SW & 14th Street SW

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 38: 1st Avenue SW & Driveway

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 39: 12th Street SW & 1st Avenue SW/1st Avenue

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	47	22
Average Queue (ft)	38	4
95th Queue (ft)	56	19
Link Distance (ft)	272	266
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

Intersection

Int Delay, s/veh 3.4

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	43	94	48	156	38	140
Future Vol, veh/h	43	94	48	156	38	140
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	48	104	53	173	42	156

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	380	140	0	0	227	0
Stage 1	140	-	-	-	-	-
Stage 2	240	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	616	900	-	-	1324	-
Stage 1	879	-	-	-	-	-
Stage 2	793	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	596	900	-	-	1324	-
Mov Cap-2 Maneuver	596	-	-	-	-	-
Stage 1	879	-	-	-	-	-
Stage 2	768	-	-	-	-	-

Approach	WB		NB		SB
HCM Control Delay, s	10.8		0		1.7
HCM LOS	B				

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	- 776	1324	-
HCM Lane V/C Ratio	-	- 0.196	0.032	-
HCM Control Delay (s)	-	- 10.8	7.8	-
HCM Lane LOS	-	- B	A	-
HCM 95th %tile Q(veh)	-	- 0.7	0.1	-

Intersection

Int Delay, s/veh 4.7

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	28	215	25	176	81	102
Future Vol, veh/h	28	215	25	176	81	102
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	0	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	31	239	28	196	90	113

Major/Minor	Minor2	Major1		Major2
Conflicting Flow All	398	147	203	0
Stage 1	147	-	-	-
Stage 2	251	-	-	-
Critical Hdwy	6.45	6.25	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-
Follow-up Hdwy	3.545	3.345	2.245	-
Pot Cap-1 Maneuver	602	892	1351	-
Stage 1	873	-	-	-
Stage 2	784	-	-	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	590	892	1351	-
Mov Cap-2 Maneuver	590	-	-	-
Stage 1	873	-	-	-
Stage 2	768	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	11.3	1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1351	-	842	-	-
HCM Lane V/C Ratio	0.021	-	0.321	-	-
HCM Control Delay (s)	7.7	-	11.3	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0.1	-	1.4	-	-

32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Intersection

Int Delay, s/veh 5.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔		↔		↔			↔	
Traffic Vol, veh/h	4	4	4	4	0	4	0	4	4	4	13	0
Future Vol, veh/h	4	4	4	4	0	4	0	4	4	4	13	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	0	-	1	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	8	8	8	8	8	8	6	6	6
Mvmt Flow	4	4	4	4	0	4	0	4	4	4	14	0

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	0	0	9	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.13	-	4.18	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.227	-	2.272	-
Pot Cap-1 Maneuver	-	-	1572	0
Stage 1	-	-	0	870
Stage 2	-	-	0	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	-	1572	-
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s		3.6	8.8	9.2
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBR	SBLn1
Capacity (veh/h)	946	-	-	-	1572	-	881
HCM Lane V/C Ratio	0.009	-	-	-	0.003	-	0.021
HCM Control Delay (s)	8.8	-	-	-	7.3	-	9.2
HCM Lane LOS	A	-	-	-	A	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0	-	0.1

Intersection

Int Delay, s/veh 5.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↻							↻		↻	
Traffic Vol, veh/h	0	278	73	0	0	0	0	0	156	13	90	0
Future Vol, veh/h	0	278	73	0	0	0	0	0	156	13	90	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	-	-	-	-	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	-	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	2	2	2	7	7	7	7	7	7
Mvmt Flow	0	309	81	0	0	0	0	0	173	14	100	0

Major/Minor	Major1			Minor1			Minor2		
Conflicting Flow All	-	0	0	-	-	349	349	390	-
Stage 1	-	-	-	-	-	-	0	0	-
Stage 2	-	-	-	-	-	-	349	390	-
Critical Hdwy	-	-	-	-	-	6.27	7.17	6.57	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.17	5.57	-
Follow-up Hdwy	-	-	-	-	-	3.363	3.563	4.063	-
Pot Cap-1 Maneuver	0	-	-	0	0	683	596	538	0
Stage 1	0	-	-	0	0	-	-	-	0
Stage 2	0	-	-	0	0	-	657	599	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	683	445	538	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	445	538	-
Stage 1	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	490	599	-

Approach	EB	NB	SB
HCM Control Delay, s	0	12.1	13.8
HCM LOS		B	B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	SBLn1
Capacity (veh/h)	683	-	-	524
HCM Lane V/C Ratio	0.254	-	-	0.218
HCM Control Delay (s)	12.1	-	-	13.8
HCM Lane LOS	B	-	-	B
HCM 95th %tile Q(veh)	1	-	-	0.8

Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB						TB			TB	
Traffic Vol, veh/h	38	400	8	0	0	0	0	4	4	4	8	0
Future Vol, veh/h	38	400	8	0	0	0	0	4	4	4	8	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	-	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	2	2	2	2	2	2	2	2	2
Mvmt Flow	42	444	9	0	0	0	0	4	4	4	9	0

Major/Minor	Major1			Minor1			Minor2		
Conflicting Flow All	0	0	0	-	533	227	309	538	-
Stage 1	-	-	-	-	533	-	0	0	-
Stage 2	-	-	-	-	0	-	309	538	-
Critical Hdwy	4.18	-	-	-	6.54	6.94	7.54	6.54	-
Critical Hdwy Stg 1	-	-	-	-	5.54	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-
Follow-up Hdwy	2.24	-	-	-	4.02	3.32	3.52	4.02	-
Pot Cap-1 Maneuver	-	-	-	0	451	776	620	448	0
Stage 1	-	-	-	0	523	-	-	-	0
Stage 2	-	-	-	0	-	-	676	521	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	451	776	612	448	-
Mov Cap-2 Maneuver	-	-	-	-	451	-	612	448	-
Stage 1	-	-	-	-	523	-	-	-	-
Stage 2	-	-	-	-	-	-	666	521	-

Approach	EB	NB	SB
HCM Control Delay, s		11.4	12.5
HCM LOS		B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	SBLn1
Capacity (veh/h)	570	-	-	-	492
HCM Lane V/C Ratio	0.016	-	-	-	0.027
HCM Control Delay (s)	11.4	-	-	-	12.5
HCM Lane LOS	B	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.1

Intersection

Int Delay, s/veh 0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	304	13	4	423	5	18
Future Vol, veh/h	304	13	4	423	5	18
Conflicting Peds, #/hr	0	0	0	0	18	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	4	4	6	6
Mvmt Flow	338	14	4	470	6	20

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	352	842
Stage 1	-	-	345
Stage 2	-	-	497
Critical Hdwy	-	4.14	6.46
Critical Hdwy Stg 1	-	-	5.46
Critical Hdwy Stg 2	-	-	5.46
Follow-up Hdwy	-	2.236	3.554
Pot Cap-1 Maneuver	-	1196	329
Stage 1	-	-	708
Stage 2	-	-	603
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	1196	322
Mov Cap-2 Maneuver	-	-	322
Stage 1	-	-	708
Stage 2	-	-	591

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	11.8
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	552	-	-	1196	-
HCM Lane V/C Ratio	0.046	-	-	0.004	-
HCM Control Delay (s)	11.8	-	-	8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0.1	-	-	0	-

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	0	478	26	0	4
Future Vol, veh/h	4	0	478	26	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	3	3
Mvmt Flow	4	0	531	29	0	4

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	560	-	546
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.12	-	6.23
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.218	-	3.327
Pot Cap-1 Maneuver	1011	0	536
Stage 1	-	0	-
Stage 2	-	0	-
Platoon blocked, %			
Mov Cap-1 Maneuver	1011	-	536
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	8.6	0	11.8
HCM LOS			B

Minor Lane/Major Mvmt	EBL	WBT	WBR	SBLn1
Capacity (veh/h)	1011	-	-	536
HCM Lane V/C Ratio	0.004	-	-	0.008
HCM Control Delay (s)	8.6	-	-	11.8
HCM Lane LOS	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	0

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			↶			↷
Traffic Vol, veh/h	0	0	527	9	0	4
Future Vol, veh/h	0	0	527	9	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	-	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	6	6
Mvmt Flow	0	0	586	10	0	4

Major/Minor

	Major2	Minor2
Conflicting Flow All	-	0
Stage 1	-	-
Stage 2	-	-
Critical Hdwy	-	-
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	-
Follow-up Hdwy	-	-
Pot Cap-1 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	-
Mov Cap-2 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-

Approach

	WB	SB
HCM Control Delay, s	0	12.3
HCM LOS		B

Minor Lane/Major Mvmt

	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	499
HCM Lane V/C Ratio	-	-	0.009
HCM Control Delay (s)	-	-	12.3
HCM Lane LOS	-	-	B
HCM 95th %tile Q(veh)	-	-	0

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↕↕			↕			↕		
Traffic Vol, veh/h	0	0	0	8	610	4	38	4	0	0	4	4
Future Vol, veh/h	0	0	0	8	610	4	38	4	0	0	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	-	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	4	4	4	2	2	2	2	2	2
Mvmt Flow	0	0	0	9	678	4	42	4	0	0	4	4
Major/Minor	Major2			Minor1			Minor2					
Conflicting Flow All	0			0			359			700		
Stage 1	-			-			0			0		
Stage 2	-			-			359			700		
Critical Hdwy	4.18			-			7.54			6.54		
Critical Hdwy Stg 1	-			-			-			5.54		
Critical Hdwy Stg 2	-			-			6.54			5.54		
Follow-up Hdwy	2.24			-			3.52			4.02		
Pot Cap-1 Maneuver	-			-			572			362		
Stage 1	-			-			-			0		
Stage 2	-			-			632			440		
Platoon blocked, %	-			-			-			-		
Mov Cap-1 Maneuver	-			-			563			362		
Mov Cap-2 Maneuver	-			-			563			362		
Stage 1	-			-			-			440		
Stage 2	-			-			621			440		
Approach	WB			NB			SB					
HCM Control Delay, s							12.4			12.9		
HCM LOS							B			B		
Minor Lane/Major Mvmt	NBLn1	WBL	WBT	WBR	SBLn1							
Capacity (veh/h)	535	-	-	-	467							
HCM Lane V/C Ratio	0.087	-	-	-	0.019							
HCM Control Delay (s)	12.4	-	-	-	12.9							
HCM Lane LOS	B	-	-	-	B							
HCM 95th %tile Q(veh)	0.3	-	-	-	0.1							

Summary of All Intervals

Start Time	6:57
End Time	7:10
Total Time (min)	13
Time Recorded (min)	10
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	859
Vehs Exited	826
Starting Vehs	109
Ending Vehs	142
Travel Distance (mi)	788
Travel Time (hr)	20.3
Total Delay (hr)	2.2
Total Stops	184
Fuel Used (gal)	24.6

Interval #0 Information Seeding

Start Time	6:57
End Time	7:00
Total Time (min)	3
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	7:10
Total Time (min)	10
Volumes adjusted by Growth Factors.	
Vehs Entered	859
Vehs Exited	826
Starting Vehs	109
Ending Vehs	142
Travel Distance (mi)	788
Travel Time (hr)	20.3
Total Delay (hr)	2.2
Total Stops	184
Fuel Used (gal)	24.6

Intersection: 1: 13th Street SW & US 321 On/Off Ramp

Movement	WB	SB
Directions Served	LR	L
Maximum Queue (ft)	53	31
Average Queue (ft)	34	6
95th Queue (ft)	50	27
Link Distance (ft)	545	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 2: 13th Street SW & US 321 SB Off Ramp

Movement	EB
Directions Served	LR
Maximum Queue (ft)	76
Average Queue (ft)	52
95th Queue (ft)	84
Link Distance (ft)	523
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: NB US 321 & US 321 NB Off Ramp

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 13: 1st street SW Connector & 1st Avenue SW

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 15: NB US 321 & 13th Street U-turn

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 18: 1st Avenue SW

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	53	74
Average Queue (ft)	17	31
95th Queue (ft)	53	70
Link Distance (ft)	304	256
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: 2nd Ave SW/2nd Avenue SW

Movement	NB	SB
Directions Served	R	LT
Maximum Queue (ft)	63	73
Average Queue (ft)	20	50
95th Queue (ft)	62	73
Link Distance (ft)	171	368
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 35: 12th Street SW & 2nd Avenue SW/2nd Avenue

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	23	27
Average Queue (ft)	13	11
95th Queue (ft)	32	32
Link Distance (ft)	140	272
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 36: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	LR
Maximum Queue (ft)	28
Average Queue (ft)	21
95th Queue (ft)	38
Link Distance (ft)	256
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 37: 1st Avenue SW & 14th Street SW

Movement	SB
Directions Served	R
Maximum Queue (ft)	15
Average Queue (ft)	3
95th Queue (ft)	13
Link Distance (ft)	453
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 38: 1st Avenue SW & Driveway

Movement	SB
Directions Served	R
Maximum Queue (ft)	28
Average Queue (ft)	6
95th Queue (ft)	24
Link Distance (ft)	190
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 39: 12th Street SW & 1st Avenue SW/1st Avenue

Movement	NB	SB
Directions Served	LT	TR
Maximum Queue (ft)	24	22
Average Queue (ft)	19	4
95th Queue (ft)	35	19
Link Distance (ft)	272	266
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB						
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street						
Date Performed	2/10/17		Jurisdiction							
Analysis Time Period	AM Peak		Analysis Year	2040						
Project Description US 321 at 13th Street										
Inputs										
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1			<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A						<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100			L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1647			V _D = 243 veh/h		
		Ramp Volume, V _R			126					
		Freeway Free-Flow Speed, S _{FF}			55.0					
		Ramp Free-Flow Speed, S _{FR}			35.0					
Conversion to pc/h Under Base Conditions										
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p		
Freeway	1647	0.90	Rolling	5	0	0.930	1.00	1967		
Ramp	126	0.90	Rolling	5	0	0.930	1.00	150		
UpStream										
DownStream	243	0.90	Rolling	5	0	0.930	1.00	290		
Merge Areas					Diverge Areas					
Estimation of v ₁₂					Estimation of v ₁₂					
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1967 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					
Capacity Checks					Capacity Checks					
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?	
V _{FO}		Exhibit 13-8			V _F	1967	Exhibit 13-8	4500	No	
					V _{FO} = V _F - V _R	1817	Exhibit 13-8	4500	No	
					V _R	150	Exhibit 13-10	2000	No	
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area					
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?	
V _{R12}		Exhibit 13-8			V ₁₂	1967	Exhibit 13-8	4400:All	No	
Level of Service Determination (if not F)					Level of Service Determination (if not F)					
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 20.3 (pc/mi/ln) LOS = C (Exhibit 13-2)					
Speed Determination					Speed Determination					
M _S = (Exhibit 13-11)					D _S = 0.442 (Exhibit 13-12)					
S _R = mph (Exhibit 13-11)					S _R = 49.3 mph (Exhibit 13-12)					
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)					
S = mph (Exhibit 13-13)					S = 49.3 mph (Exhibit 13-13)					

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			2055		V _D = 194 veh/h		
		Ramp Volume, V _R			137				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	2055	0.90	Rolling	5	0	0.930	1.00	2455	
Ramp	137	0.90	Rolling	5	0	0.930	1.00	164	
UpStream									
DownStream	194	0.90	Rolling	5	0	0.930	1.00	232	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
L _{EQ} =		V ₁₂ = V _F (P _{FM})			(Equation 13-6 or 13-7)		L _{EQ} =		
P _{FM} =		using Equation (Exhibit 13-6)					V ₁₂ = V _R + (V _F - V _R)P _{FD}		
V ₁₂ =		pc/h					(Equation 13-12 or 13-13)		
V ₃ or V _{av34}		pc/h (Equation 13-14 or 13-17)					P _{FD} = 1.000 using Equation (Exhibit 13-7)		
Is V ₃ or V _{av34} > 2,700 pc/h?		<input type="checkbox"/> Yes <input type="checkbox"/> No					V ₁₂ = 2455 pc/h		
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2		<input type="checkbox"/> Yes <input type="checkbox"/> No					V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17)		
If Yes, V _{12a} =		pc/h (Equation 13-16, 13-18, or 13-19)					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
							Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
							If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)		
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2455	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2291	Exhibit 13-8	4500	No
					V _R	164	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2455	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
D _R = 5.475 + 0.00734 v _R + 0.0078 V ₁₂ - 0.00627 L _A					D _R = 4.252 + 0.0086 V ₁₂ - 0.009 L _D				
D _R = (pc/mi/ln)					D _R = 24.5 (pc/mi/ln)				
LOS = (Exhibit 13-2)					LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.443 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.2 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off		Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 700 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 136 veh/h		Freeway Volume, V _F			1917		V _D = veh/h		
		Ramp Volume, V _R			194				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1917	0.90	Rolling	5	0	0.930	1.00	2290	
Ramp	194	0.90	Rolling	5	0	0.930	1.00	232	
UpStream	136	0.90	Rolling	5	0	0.930	1.00	162	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 2290 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2522	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2522	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 21.9 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = C (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.335 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.7 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.7 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off		Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 700 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 243 veh/h		Freeway Volume, V _F			1520		V _D = veh/h		
		Ramp Volume, V _R			127				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1520	0.90	Rolling	5	0	0.930	1.00	1816	
Ramp	127	0.90	Rolling	5	0	0.930	1.00	152	
UpStream	243	0.90	Rolling	5	0	0.930	1.00	290	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 1816 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1968	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1968	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 17.6 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.314 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.9 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.9 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	203 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off		Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 200 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 116 veh/h		Freeway Volume, V _F			1534		V _D = veh/h		
		Ramp Volume, V _R			59				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1534	0.90	Rolling	5	0	0.930	1.00	1832	
Ramp	59	0.90	Rolling	5	0	0.930	1.00	70	
UpStream	116	0.90	Rolling	5	0	0.930	1.00	139	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 1832 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1902	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1902	Exhibit 13-8	4600:All	No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 17.1 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.312 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.9 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.9 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB		Junction		203 - US 321 at 13th Street	
Agency or Company	AECOM		Jurisdiction	2040		Analysis Year		2040	
Date Performed	2/10/17		Project Description US 321 at 13th Street						
Analysis Time Period	PM Peak								
Inputs									
Upstream Adj Ramp	Freeway Number of Lanes, N		2		Downstream Adj Ramp				
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N		1		<input type="checkbox"/> Yes <input type="checkbox"/> On				
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A		500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off				
L _{up} = 200 ft	Deceleration Lane Length L _D				L _{down} = ft				
V _u = 102 veh/h	Freeway Volume, V _F		1850		V _D = veh/h				
	Ramp Volume, V _R		86						
	Freeway Free-Flow Speed, S _{FF}		55.0						
	Ramp Free-Flow Speed, S _{FR}		35.0						
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1850	0.90	Rolling	5	0	0.930	1.00	2210	
Ramp	86	0.90	Rolling	5	0	0.930	1.00	103	
UpStream	102	0.90	Rolling	5	0	0.930	1.00	122	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 2210 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2313	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2313	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 20.3 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = C (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.325 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.8 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.8 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 700 ft		
V _u = veh/h		Freeway Volume, V _F			2111		V _D = 136 veh/h		
		Ramp Volume, V _R			194				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	2111	0.90	Rolling	5	0	0.930	1.00	2521	
Ramp	194	0.90	Rolling	5	0	0.930	1.00	232	
UpStream									
DownStream	136	0.90	Rolling	5	0	0.930	1.00	162	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2521 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2521	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2289	Exhibit 13-8	4500	No
					V _R	232	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2521	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 25.0 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	(Exhibit 13-11)				D _S =	0.449 (Exhibit 13-12)			
S _R =	mph (Exhibit 13-11)				S _R =	49.2 mph (Exhibit 13-12)			
S ₀ =	mph (Exhibit 13-11)				S ₀ =	N/A mph (Exhibit 13-12)			
S =	mph (Exhibit 13-13)				S =	49.2 mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp	Freeway Number of Lanes, N		2		Downstream Adj Ramp				
<input type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N		1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On				
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Acceleration Lane Length, L _A				<input type="checkbox"/> No <input type="checkbox"/> Off				
L _{up} = ft	Deceleration Lane Length L _D		100		L _{down} = 700 ft				
V _u = veh/h	Freeway Volume, V _F		1763		V _D = 127 veh/h				
	Ramp Volume, V _R		243						
	Freeway Free-Flow Speed, S _{FF}		55.0						
	Ramp Free-Flow Speed, S _{FR}		35.0						
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1763	0.90	Rolling	5	0	0.930	1.00	2106	
Ramp	243	0.90	Rolling	5	0	0.930	1.00	290	
UpStream									
DownStream	127	0.90	Rolling	5	0	0.930	1.00	152	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2106 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2106	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1816	Exhibit 13-8	4500	No
					V _R	290	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2106	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 21.5 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.454 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.1 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.1 mph (Exhibit 13-13)				

APPENDIX G




**2040 Build Alternative 13th Street – Two-Way Split Partial Cloverleaf
Interchange Synchro, SimTraffic & HCS Reports**

32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Intersection												
Int Delay, s/veh	3.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	152	5	92	270	8	12	4	130	5	4	4
Future Vol, veh/h	4	152	5	92	270	8	12	4	130	5	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	8	8	8	8	8	8	6	6	6
Mvmt Flow	4	169	6	102	300	9	13	4	144	6	4	4
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	309	0	0	174	0	0	694	694	172	764	692	304
Stage 1	-	-	-	-	-	-	181	181	-	509	509	-
Stage 2	-	-	-	-	-	-	513	513	-	255	183	-
Critical Hdwy	4.13	-	-	4.18	-	-	7.18	6.58	6.28	7.16	6.56	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	6.18	5.58	-	6.16	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.18	5.58	-	6.16	5.56	-
Follow-up Hdwy	2.227	-	-	2.272	-	-	3.572	4.072	3.372	3.554	4.054	3.354
Pot Cap-1 Maneuver	1246	-	-	1367	-	-	349	359	856	316	362	726
Stage 1	-	-	-	-	-	-	807	739	-	539	532	-
Stage 2	-	-	-	-	-	-	533	526	-	741	741	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1246	-	-	1367	-	-	319	325	856	241	328	726
Mov Cap-2 Maneuver	-	-	-	-	-	-	319	325	-	241	328	-
Stage 1	-	-	-	-	-	-	804	736	-	537	484	-
Stage 2	-	-	-	-	-	-	478	479	-	610	738	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			2			11.4			16.1		
HCM LOS							B			C		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	724	1246	-	-	1367	-	-	338				
HCM Lane V/C Ratio	0.224	0.004	-	-	0.075	-	-	0.043				
HCM Control Delay (s)	11.4	7.9	0	-	7.8	0	-	16.1				
HCM Lane LOS	B	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	0.9	0	-	-	0.2	-	-	0.1				

Intersection

Int Delay, s/veh 5.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	186	103	95	287	78	125
Future Vol, veh/h	186	103	95	287	78	125
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	3	5	5
Mvmt Flow	207	114	106	319	87	139

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	321
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.13
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.227
Pot Cap-1 Maneuver	-	-	1233
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1233
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	2	18.2
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	495	-	-	1233	-
HCM Lane V/C Ratio	0.456	-	-	0.086	-
HCM Control Delay (s)	18.2	-	-	8.2	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2.3	-	-	0.3	-

Intersection												
Int Delay, s/veh	14.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Traffic Vol, veh/h	9	251	98	152	234	4	125	4	62	4	4	4
Future Vol, veh/h	9	251	98	152	234	4	125	4	62	4	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	4	4	4	5	5	5	7	7	7
Mvmt Flow	10	279	109	169	260	4	139	4	69	4	4	4
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	264	0	0	388	0	0	957	955	333	956	1008	262
Stage 1	-	-	-	-	-	-	353	353	-	600	600	-
Stage 2	-	-	-	-	-	-	604	602	-	356	408	-
Critical Hdwy	4.13	-	-	4.14	-	-	7.15	6.55	6.25	7.17	6.57	6.27
Critical Hdwy Stg 1	-	-	-	-	-	-	6.15	5.55	-	6.17	5.57	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.15	5.55	-	6.17	5.57	-
Follow-up Hdwy	2.227	-	-	2.236	-	-	3.545	4.045	3.345	3.563	4.063	3.363
Pot Cap-1 Maneuver	1294	-	-	1160	-	-	234	255	702	233	236	765
Stage 1	-	-	-	-	-	-	658	626	-	479	482	-
Stage 2	-	-	-	-	-	-	480	484	-	651	588	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1294	-	-	1160	-	-	197	209	702	178	194	765
Mov Cap-2 Maneuver	-	-	-	-	-	-	197	209	-	178	194	-
Stage 1	-	-	-	-	-	-	651	620	-	474	400	-
Stage 2	-	-	-	-	-	-	391	401	-	577	582	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			3.4			61.9			20.3		
HCM LOS							F			C		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	257	1294	-	-	1160	-	-	248				
HCM Lane V/C Ratio	0.826	0.008	-	-	0.146	-	-	0.054				
HCM Control Delay (s)	61.9	7.8	0	-	8.6	0	-	20.3				
HCM Lane LOS	F	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	6.6	0	-	-	0.5	-	-	0.2				

Lanes, Volumes, Timings
 34: 13th St SW & 2nd Avenue SW

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	9	251	98	152	234	4	125	4	62	4	4	4
Future Volume (vph)	9	251	98	152	234	4	125	4	62	4	4	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.963			0.999			0.956			0.955	
Flt Protected		0.999			0.981			0.968			0.984	
Satd. Flow (prot)	0	1775	0	0	1790	0	0	1675	0	0	1669	0
Flt Permitted		0.987			0.725			0.795			0.883	
Satd. Flow (perm)	0	1753	0	0	1323	0	0	1375	0	0	1497	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			25	
Link Distance (ft)		984			317			579			323	
Travel Time (s)		19.2			6.2			11.3			8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	4%	4%	4%	5%	5%	5%	7%	7%	7%
Adj. Flow (vph)	10	279	109	169	260	4	139	4	69	4	4	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	398	0	0	433	0	0	212	0	0	12	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	17.0	17.0		17.0	17.0		14.0	14.0		14.0	14.0	
Total Split (s)	37.0	37.0		37.0	37.0		23.0	23.0		23.0	23.0	
Total Split (%)	61.7%	61.7%		61.7%	61.7%		38.3%	38.3%		38.3%	38.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		16.0	16.0		16.0	16.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		-2.0			-2.0			-2.0			-2.0	
Total Lost Time (s)		5.0			5.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	None		None	None	
Act Effct Green (s)		25.7			25.7			14.3			14.3	
Actuated g/C Ratio		0.51			0.51			0.28			0.28	
v/c Ratio		0.44			0.64			0.54			0.03	
Control Delay		10.1			14.8			21.6			14.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		10.1			14.8			21.6			14.4	
LOS		B			B			C			B	
Approach Delay		10.1			14.8			21.6			14.4	
Approach LOS		B			B			C			B	
Queue Length 50th (ft)		66			82			46			2	
Queue Length 95th (ft)		135			183			121			13	

Lanes, Volumes, Timings
 34: 13th St SW & 2nd Avenue SW

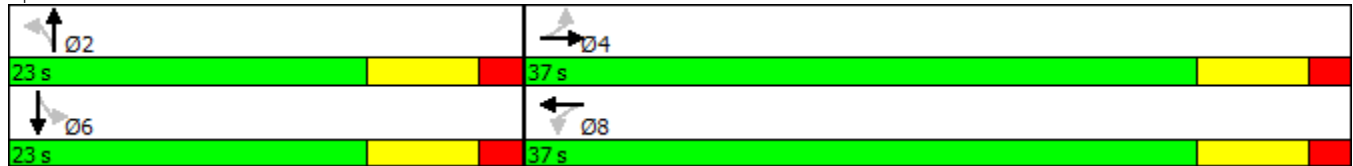
NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		904			237			499			243	
Turn Bay Length (ft)												
Base Capacity (vph)		1145			864			505			550	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.35			0.50			0.42			0.02	

Intersection Summary	
Area Type:	Other
Cycle Length:	60
Actuated Cycle Length:	50.2
Natural Cycle:	45
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.64
Intersection Signal Delay:	14.4
Intersection LOS:	B
Intersection Capacity Utilization	70.7%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 34: 13th St SW & 2nd Avenue SW



Intersection

Int Delay, s/veh 0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	200	4	4	191	8	4
Future Vol, veh/h	200	4	4	191	8	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	100	4	4	6	6
Mvmt Flow	222	4	4	212	9	4

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	227
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.14
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.236
Pot Cap-1 Maneuver	-	-	1330
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1330
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	10.9
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	624	-	-	1330	-
HCM Lane V/C Ratio	0.021	-	-	0.003	-
HCM Control Delay (s)	10.9	-	-	7.7	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0.1	-	-	0	-

Intersection

Int Delay, s/veh 0.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	200	4	12	203	15	6	4	4	4	4	4
Future Vol, veh/h	4	200	4	12	203	15	6	4	4	4	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	3	2	3
Mvmt Flow	4	222	4	13	226	17	7	4	4	4	4	4

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	242	0	0	227	0	0	498	502	224	499	497	234
Stage 1	-	-	-	-	-	-	233	233	-	261	261	-
Stage 2	-	-	-	-	-	-	265	269	-	238	236	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.13	6.52	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.13	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.13	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.527	4.018	3.327
Pot Cap-1 Maneuver	1324	-	-	1341	-	-	483	471	815	480	475	803
Stage 1	-	-	-	-	-	-	770	712	-	742	692	-
Stage 2	-	-	-	-	-	-	740	687	-	763	710	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1324	-	-	1341	-	-	472	464	815	469	468	803
Mov Cap-2 Maneuver	-	-	-	-	-	-	472	464	-	469	468	-
Stage 1	-	-	-	-	-	-	768	710	-	740	684	-
Stage 2	-	-	-	-	-	-	723	679	-	752	708	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.4	11.9	11.8
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	534	1324	-	-	1341	-	-	544
HCM Lane V/C Ratio	0.029	0.003	-	-	0.01	-	-	0.025
HCM Control Delay (s)	11.9	7.7	0	-	7.7	0	-	11.8
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

Intersection

Int Delay, s/veh 0.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	200	4	4	218	4	10	4	4	4	4	4
Future Vol, veh/h	4	200	4	4	218	4	10	4	4	4	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	4	4	4	7	7	7	6	6	6
Mvmt Flow	4	222	4	4	242	4	11	4	4	4	4	4

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	247	0	0	227	0	0	491	489	224	491	489	244
Stage 1	-	-	-	-	-	-	233	233	-	253	253	-
Stage 2	-	-	-	-	-	-	258	256	-	238	236	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.17	6.57	6.27	7.16	6.56	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	6.17	5.57	-	6.16	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.17	5.57	-	6.16	5.56	-
Follow-up Hdwy	2.236	-	-	2.236	-	-	3.563	4.063	3.363	3.554	4.054	3.354
Pot Cap-1 Maneuver	1307	-	-	1330	-	-	480	472	803	481	474	785
Stage 1	-	-	-	-	-	-	759	703	-	742	690	-
Stage 2	-	-	-	-	-	-	736	687	-	756	702	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1307	-	-	1330	-	-	471	469	803	472	471	785
Mov Cap-2 Maneuver	-	-	-	-	-	-	471	469	-	472	471	-
Stage 1	-	-	-	-	-	-	756	700	-	739	688	-
Stage 2	-	-	-	-	-	-	725	685	-	744	699	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1	12.2	11.8
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	518	1307	-	-	1330	-	-	544
HCM Lane V/C Ratio	0.039	0.003	-	-	0.003	-	-	0.025
HCM Control Delay (s)	12.2	7.8	0	-	7.7	0	-	11.8
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5297
Vehs Exited	5295
Starting Vehs	130
Ending Vehs	132
Travel Distance (mi)	4900
Travel Time (hr)	128.1
Total Delay (hr)	15.2
Total Stops	792
Fuel Used (gal)	156.6

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5297
Vehs Exited	5295
Starting Vehs	130
Ending Vehs	132
Travel Distance (mi)	4900
Travel Time (hr)	128.1
Total Delay (hr)	15.2
Total Stops	792
Fuel Used (gal)	156.6

Intersection: 3: NB US 321 & 13th St SW

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 15: NB US 321 & 13th Street U-turn

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	56	117	124	28
Average Queue (ft)	2	19	52	11
95th Queue (ft)	20	67	92	33
Link Distance (ft)	376	239	310	248
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 33: US 321 SB Off Ramp & 2nd Avenue SW

Movement	EB	WB	NB
Directions Served	TR	LT	LR
Maximum Queue (ft)	30	90	111
Average Queue (ft)	1	25	52
95th Queue (ft)	10	71	87
Link Distance (ft)	239	926	688
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 34: 13th St SW & 2nd Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	74	90	111	24
Average Queue (ft)	6	42	52	8
95th Queue (ft)	34	74	93	26
Link Distance (ft)	926	281	518	252
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 36: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	LR
Maximum Queue (ft)	28
Average Queue (ft)	11
95th Queue (ft)	32
Link Distance (ft)	248
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	28	50	22	16
Average Queue (ft)	1	3	9	4
95th Queue (ft)	9	21	26	15
Link Distance (ft)	687	485	106	460
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 38: 13th ST SW/Driveway & 1st Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	29	30	46	29
Average Queue (ft)	1	1	10	6
95th Queue (ft)	9	10	33	26
Link Distance (ft)	485	310	252	191
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Intersection												
Int Delay, s/veh	3.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	270	12	129	152	5	5	4	91	8	4	4
Future Vol, veh/h	4	270	12	129	152	5	5	4	91	8	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	8	8	8	8	8	8	6	6	6
Mvmt Flow	4	300	13	143	169	6	6	4	101	9	4	4
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	174	0	0	313	0	0	779	777	307	826	780	172
Stage 1	-	-	-	-	-	-	316	316	-	458	458	-
Stage 2	-	-	-	-	-	-	463	461	-	368	322	-
Critical Hdwy	4.13	-	-	4.18	-	-	7.18	6.58	6.28	7.16	6.56	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	6.18	5.58	-	6.16	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.18	5.58	-	6.16	5.56	-
Follow-up Hdwy	2.227	-	-	2.272	-	-	3.572	4.072	3.372	3.554	4.054	3.354
Pot Cap-1 Maneuver	1397	-	-	1214	-	-	306	321	719	287	322	861
Stage 1	-	-	-	-	-	-	683	644	-	575	560	-
Stage 2	-	-	-	-	-	-	568	555	-	644	644	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1397	-	-	1214	-	-	270	278	719	219	279	861
Mov Cap-2 Maneuver	-	-	-	-	-	-	270	278	-	219	279	-
Stage 1	-	-	-	-	-	-	681	642	-	573	487	-
Stage 2	-	-	-	-	-	-	487	482	-	548	642	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			3.8			12			18.3		
HCM LOS							B			C		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	627	1397	-	-	1214	-	-	288				
HCM Lane V/C Ratio	0.177	0.003	-	-	0.118	-	-	0.062				
HCM Control Delay (s)	12	7.6	0	-	8.4	0	-	18.3				
HCM Lane LOS	B	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	0.6	0	-	-	0.4	-	-	0.2				

Intersection

Int Delay, s/veh 6.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	T		T		T	
Traffic Vol, veh/h	239	125	62	191	97	152
Future Vol, veh/h	239	125	62	191	97	152
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	3	5	5
Mvmt Flow	266	139	69	212	108	169

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	404	685
Stage 1	-	-	335
Stage 2	-	-	350
Critical Hdwy	-	4.13	6.45
Critical Hdwy Stg 1	-	-	5.45
Critical Hdwy Stg 2	-	-	5.45
Follow-up Hdwy	-	2.227	3.545
Pot Cap-1 Maneuver	-	1149	409
Stage 1	-	-	718
Stage 2	-	-	707
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	1149	381
Mov Cap-2 Maneuver	-	-	381
Stage 1	-	-	718
Stage 2	-	-	659

Approach	EB	WB	NB
HCM Control Delay, s	0	2	19.1
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	528	-	-	1149	-
HCM Lane V/C Ratio	0.524	-	-	0.06	-
HCM Control Delay (s)	19.1	-	-	8.3	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	3	-	-	0.2	-

Intersection												
Int Delay, s/veh	8.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	287	77	125	130	4	103	4	95	4	4	9
Future Vol, veh/h	4	287	77	125	130	4	103	4	95	4	4	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	4	4	4	5	5	5	7	7	7
Mvmt Flow	4	319	86	139	144	4	114	4	106	4	4	10
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	149	0	0	404	0	0	803	798	362	797	837	147
Stage 1	-	-	-	-	-	-	371	371	-	424	424	-
Stage 2	-	-	-	-	-	-	432	427	-	373	413	-
Critical Hdwy	4.13	-	-	4.14	-	-	7.15	6.55	6.25	7.17	6.57	6.27
Critical Hdwy Stg 1	-	-	-	-	-	-	6.15	5.55	-	6.17	5.57	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.15	5.55	-	6.17	5.57	-
Follow-up Hdwy	2.227	-	-	2.236	-	-	3.545	4.045	3.345	3.563	4.063	3.363
Pot Cap-1 Maneuver	1426	-	-	1144	-	-	298	316	676	299	297	887
Stage 1	-	-	-	-	-	-	643	614	-	598	579	-
Stage 2	-	-	-	-	-	-	596	580	-	638	585	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1426	-	-	1144	-	-	260	273	676	223	256	887
Mov Cap-2 Maneuver	-	-	-	-	-	-	260	273	-	223	256	-
Stage 1	-	-	-	-	-	-	640	612	-	596	502	-
Stage 2	-	-	-	-	-	-	506	503	-	532	583	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			4.1			29.2			14.7		
HCM LOS							D			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	366	1426	-	-	1144	-	-	389				
HCM Lane V/C Ratio	0.613	0.003	-	-	0.121	-	-	0.049				
HCM Control Delay (s)	29.2	7.5	0	-	8.6	0	-	14.7				
HCM Lane LOS	D	A	A	-	A	A	-	B				
HCM 95th %tile Q(veh)	3.9	0	-	-	0.4	-	-	0.2				

Intersection

Int Delay, s/veh 0.3

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶		↷		↶	
Traffic Vol, veh/h	200	4	4	193	5	4
Future Vol, veh/h	200	4	4	193	5	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	100	4	4	6	6
Mvmt Flow	222	4	4	214	6	4

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	227	0	447	224
Stage 1	-	-	-	-	224	-
Stage 2	-	-	-	-	223	-
Critical Hdwy	-	-	4.14	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	-	-	2.236	-	3.554	3.354
Pot Cap-1 Maneuver	-	-	1330	-	562	806
Stage 1	-	-	-	-	804	-
Stage 2	-	-	-	-	805	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1330	-	560	806
Mov Cap-2 Maneuver	-	-	-	-	560	-
Stage 1	-	-	-	-	804	-
Stage 2	-	-	-	-	803	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	10.6
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	648	-	-	1330	-
HCM Lane V/C Ratio	0.015	-	-	0.003	-
HCM Control Delay (s)	10.6	-	-	7.7	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Traffic Vol, veh/h	4	200	4	7	197	27	11	4	4	4	4	4
Future Vol, veh/h	4	200	4	7	197	27	11	4	4	4	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	3	2	3
Mvmt Flow	4	222	4	8	219	30	12	4	4	4	4	4
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	249	0	0	227	0	0	487	497	224	487	485	234
Stage 1	-	-	-	-	-	-	233	233	-	249	249	-
Stage 2	-	-	-	-	-	-	254	264	-	238	236	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.13	6.52	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.13	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.13	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.527	4.018	3.327
Pot Cap-1 Maneuver	1317	-	-	1341	-	-	491	475	815	489	482	803
Stage 1	-	-	-	-	-	-	770	712	-	753	701	-
Stage 2	-	-	-	-	-	-	750	690	-	763	710	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1317	-	-	1341	-	-	481	470	815	479	477	803
Mov Cap-2 Maneuver	-	-	-	-	-	-	481	470	-	479	477	-
Stage 1	-	-	-	-	-	-	768	710	-	751	696	-
Stage 2	-	-	-	-	-	-	736	685	-	752	708	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.2			12.2			11.7		
HCM LOS	B			B			B			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	524	1317	-	-	1341	-	-	553				
HCM Lane V/C Ratio	0.04	0.003	-	-	0.006	-	-	0.024				
HCM Control Delay (s)	12.2	7.7	0	-	7.7	0	-	11.7				
HCM Lane LOS	B	A	A	-	A	A	-	B				
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1				

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	4	200	4	12	213	8	5	4	4	4	4	4
Future Vol, veh/h	4	200	4	12	213	8	5	4	4	4	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	4	4	4	7	7	7	6	6	6
Mvmt Flow	4	222	4	13	237	9	6	4	4	4	4	4
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	246	0	0	227	0	0	505	505	224	506	504	241
Stage 1	-	-	-	-	-	-	233	233	-	268	268	-
Stage 2	-	-	-	-	-	-	272	272	-	238	236	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.17	6.57	6.27	7.16	6.56	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	6.17	5.57	-	6.16	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.17	5.57	-	6.16	5.56	-
Follow-up Hdwy	2.236	-	-	2.236	-	-	3.563	4.063	3.363	3.554	4.054	3.354
Pot Cap-1 Maneuver	1308	-	-	1330	-	-	470	462	803	470	465	788
Stage 1	-	-	-	-	-	-	759	703	-	729	680	-
Stage 2	-	-	-	-	-	-	723	676	-	756	702	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1308	-	-	1330	-	-	459	455	803	459	458	788
Mov Cap-2 Maneuver	-	-	-	-	-	-	459	455	-	459	458	-
Stage 1	-	-	-	-	-	-	756	700	-	726	673	-
Stage 2	-	-	-	-	-	-	706	669	-	744	699	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.4			12			11.9		
HCM LOS							B			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	527	1308	-	-	1330	-	-	533				
HCM Lane V/C Ratio	0.027	0.003	-	-	0.01	-	-	0.025				
HCM Control Delay (s)	12	7.8	0	-	7.7	0	-	11.9				
HCM Lane LOS	B	A	A	-	A	A	-	B				
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1				

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5286
Vehs Exited	5314
Starting Vehs	143
Ending Vehs	115
Travel Distance (mi)	4921
Travel Time (hr)	129.0
Total Delay (hr)	15.1
Total Stops	835
Fuel Used (gal)	158.0

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5286
Vehs Exited	5314
Starting Vehs	143
Ending Vehs	115
Travel Distance (mi)	4921
Travel Time (hr)	129.0
Total Delay (hr)	15.1
Total Stops	835
Fuel Used (gal)	158.0

Intersection: 3: NB US 321 & 13th St SW

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 4: NB US 321 & US 321 SB Off Ramp

Movement	NB
Directions Served	T
Maximum Queue (ft)	52
Average Queue (ft)	2
95th Queue (ft)	19
Link Distance (ft)	647
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 15: NB US 321 & 13th Street U-turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 32: 15 St SW & 1st Street SW Connector/2nd Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	13	75	77	49
Average Queue (ft)	0	25	39	14
95th Queue (ft)	4	65	66	38
Link Distance (ft)	376	239	310	248
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 33: US 321 SB Off Ramp & 2nd Avenue SW

Movement	WB	NB
Directions Served	LT	LR
Maximum Queue (ft)	71	134
Average Queue (ft)	19	61
95th Queue (ft)	49	105
Link Distance (ft)	926	688
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: 13th St SW & 2nd Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	52	55	130	43
Average Queue (ft)	4	29	50	11
95th Queue (ft)	23	64	91	33
Link Distance (ft)	926	281	518	252
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 36: 15 St SW & 1st Avenue SW

Movement	NB
Directions Served	LR
Maximum Queue (ft)	28
Average Queue (ft)	7
95th Queue (ft)	25
Link Distance (ft)	248
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 37: 14th Street SW & 1st Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	28	26	46	18
Average Queue (ft)	2	1	14	6
95th Queue (ft)	13	9	34	19
Link Distance (ft)	687	485	106	460
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 38: 13th ST SW/Driveway & 1st Avenue SW

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	48	30	46	29
Average Queue (ft)	2	2	9	7
95th Queue (ft)	16	14	31	26
Link Distance (ft)	485	310	252	191
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1780		V _D = 254 veh/h		
		Ramp Volume, V _R			191				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1780	0.90	Rolling	5	0	0.930	1.00	2126	
Ramp	191	0.90	Rolling	5	0	0.930	1.00	228	
UpStream									
DownStream	254	0.90	Rolling	5	0	0.930	1.00	303	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2126 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2126	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1898	Exhibit 13-8	4500	No
					V _R	228	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2126	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 21.6 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.449 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.2 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB					
Agency or Company	AECOM		Junction	201 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp	Freeway Number of Lanes, N		2		Downstream Adj Ramp				
<input type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N		1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On				
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Acceleration Lane Length, L _A				<input type="checkbox"/> No <input type="checkbox"/> Off				
L _{up} = ft	Deceleration Lane Length L _D		100		L _{down} = 200 ft				
V _u = veh/h	Freeway Volume, V _F		2189		V _D = 206 veh/h				
	Ramp Volume, V _R		202						
	Freeway Free-Flow Speed, S _{FF}		55.0						
	Ramp Free-Flow Speed, S _{FR}		35.0						
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	2189	0.90	Rolling	5	0	0.930	1.00	2615	
Ramp	202	0.90	Rolling	5	0	0.930	1.00	241	
UpStream									
DownStream	206	0.90	Rolling	5	0	0.930	1.00	246	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2615 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2615	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2374	Exhibit 13-8	4500	No
					V _R	241	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2615	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 25.8 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.450 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.2 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off		Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 700 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 203 veh/h		Freeway Volume, V _F			1987		V _D = veh/h		
		Ramp Volume, V _R			198				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1987	0.90	Rolling	5	0	0.930	1.00	2373	
Ramp	198	0.90	Rolling	5	0	0.930	1.00	236	
UpStream	203	0.90	Rolling	5	0	0.930	1.00	242	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 2373 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2609	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2609	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 22.6 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = C (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _s = 0.339 (Exhibit 13-11)					D _s = (Exhibit 13-12)				
S _R = 50.6 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.6 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	202 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2			Downstream Adj Ramp	
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = 200 ft		Deceleration Lane Length L _D						L _{down} = ft	
V _u = 249 veh/h		Freeway Volume, V _F			1589			V _D = veh/h	
		Ramp Volume, V _R			187				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1589	0.90	Rolling	5	0	0.930	1.00	1898	
Ramp	187	0.90	Rolling	5	0	0.930	1.00	223	
UpStream	249	0.90	Rolling	5	0	0.930	1.00	297	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 1898 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2121	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2121	Exhibit 13-8	4600:All	No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 18.8 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.319 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.9 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.9 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB		Junction		203 - US 321 at 13th Street	
Agency or Company	AECOM		Jurisdiction	AM Peak		Analysis Year		2040	
Date Performed	2/10/17		Project Description US 321 at 13th Street						
Analysis Time Period	AM Peak		Analysis Year 2040						
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 200 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 191 veh/h		Freeway Volume, V _F			1589		V _D = veh/h		
		Ramp Volume, V _R			254				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1589	0.90	Rolling	5	0	0.930	1.00	1898	
Ramp	254	0.90	Rolling	5	0	0.930	1.00	303	
UpStream	191	0.90	Rolling	5	0	0.930	1.00	228	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v₁₂					Estimation of v₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
P _{FM} = 1.000 using Equation (Exhibit 13-6)					P _{FD} = using Equation (Exhibit 13-7)				
V ₁₂ = 1898 pc/h					V ₁₂ = pc/h				
V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17)					V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2201	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2201	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
D _R = 19.4 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.321 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 50.8 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 50.8 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / NB		Junction		203 - US 321 at 13th Street	
Agency or Company	AECOM		Jurisdiction	2040		Analysis Year		2040	
Date Performed	2/10/17		Project Description		US 321 at 13th Street				
Analysis Time Period	PM Peak		Inputs						
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> On	Ramp Number of Lanes, N			1		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input type="checkbox"/> No	<input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A			500		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = 200 ft		Deceleration Lane Length L _D					L _{down} = ft		
V _u = 202 veh/h		Freeway Volume, V _F			1987		V _D = veh/h		
		Ramp Volume, V _R			206				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1987	0.90	Rolling	5	0	0.930	1.00	2373	
Ramp	206	0.90	Rolling	5	0	0.930	1.00	246	
UpStream	202	0.90	Rolling	5	0	0.930	1.00	241	
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 2373 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	2619	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	2619	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 22.7 (pc/mi/ln) LOS = C (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S =	0.340 (Exhibit 13-11)				D _S =	(Exhibit 13-12)			
S _R =	50.6 mph (Exhibit 13-11)				S _R =	mph (Exhibit 13-12)			
S ₀ =	N/A mph (Exhibit 13-11)				S ₀ =	mph (Exhibit 13-12)			
S =	50.6 mph (Exhibit 13-13)				S =	mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	AM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 500 ft		
V _u = veh/h		Freeway Volume, V _F			2190		V _D = 198 veh/h		
		Ramp Volume, V _R			203				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	2190	0.90	Rolling	5	0	0.930	1.00	2616	
Ramp	203	0.90	Rolling	5	0	0.930	1.00	242	
UpStream									
DownStream	198	0.90	Rolling	5	0	0.930	1.00	236	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2616 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2616	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2374	Exhibit 13-8	4500	No
					V _R	242	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2616	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 25.8 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.450 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.2 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM		Freeway/Dir of Travel	US 321 / SB					
Agency or Company	AECOM		Junction	204 - US 321 at 13th Street					
Date Performed	2/10/17		Jurisdiction						
Analysis Time Period	PM Peak		Analysis Year	2040					
Project Description US 321 at 13th Street									
Inputs									
Upstream Adj Ramp		Freeway Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Ramp Number of Lanes, N			1		<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Deceleration Lane Length L _D			100		L _{down} = 200 ft		
V _u = veh/h		Freeway Volume, V _F			1838		V _D = 187 veh/h		
		Ramp Volume, V _R			249				
		Freeway Free-Flow Speed, S _{FF}			55.0				
		Ramp Free-Flow Speed, S _{FR}			35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1838	0.90	Rolling	5	0	0.930	1.00	2195	
Ramp	249	0.90	Rolling	5	0	0.930	1.00	297	
UpStream									
DownStream	187	0.90	Rolling	5	0	0.930	1.00	223	
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2195 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2195	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	1898	Exhibit 13-8	4500	No
					V _R	297	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2195	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 22.2 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.455 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 49.1 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 49.1 mph (Exhibit 13-13)				

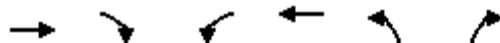
APPENDIX H

**2040 Build Alternative Clement Boulevard – Partial Cloverleaf
Interchange Synchro, SimTraffic & HCS Reports**

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

1: US 321 SB Ramps & Clement Blvd

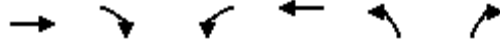


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Volume (vph)	142	47	230	183	96	978
Future Volume (vph)	142	47	230	183	96	978
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	400		0	250
Storage Lanes		1	1		1	1
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1719	1538
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1863	1583	1770	1863	1719	1538
Right Turn on Red		No				No
Satd. Flow (RTOR)						
Link Speed (mph)	35			35	35	
Link Distance (ft)	912			836	1022	
Travel Time (s)	17.8			16.3	19.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	5%
Adj. Flow (vph)	158	52	256	203	107	1087
Shared Lane Traffic (%)						
Lane Group Flow (vph)	158	52	256	203	107	1087
Turn Type	NA	pm+ov	Prot	NA	Prot	Free
Protected Phases	4	2	3	8	2	
Permitted Phases		4				Free
Detector Phase	4	2	3	8	2	
Switch Phase						
Minimum Initial (s)	10.0	7.0	7.0	10.0	7.0	
Minimum Split (s)	17.0	14.0	14.0	17.0	14.0	
Total Split (s)	29.0	24.0	37.0	66.0	24.0	
Total Split (%)	32.2%	26.7%	41.1%	73.3%	26.7%	
Maximum Green (s)	22.0	17.0	30.0	59.0	17.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Min	None	None	C-Min	None	
Act Effct Green (s)	44.4	59.6	20.4	70.8	13.0	90.0
Actuated g/C Ratio	0.49	0.66	0.23	0.79	0.14	1.00
v/c Ratio	0.17	0.05	0.64	0.14	0.43	0.71
Control Delay	16.9	6.8	17.7	1.5	39.9	2.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	16.9	6.8	17.7	1.5	39.9	2.8
LOS	B	A	B	A	D	A
Approach Delay	14.4			10.5	6.1	

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

1: US 321 SB Ramps & Clement Blvd

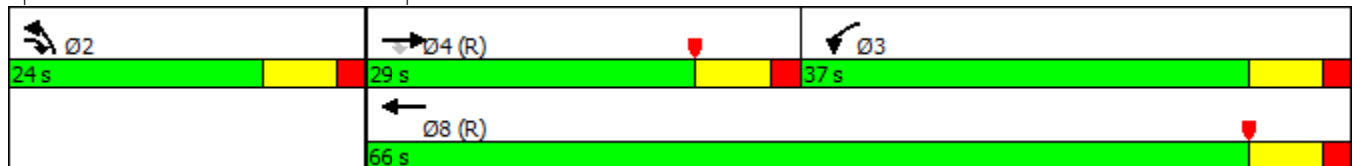


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Approach LOS	B			B		A
Queue Length 50th (ft)	51	9	22	9	56	0
Queue Length 95th (ft)	111	26	76	20	101	0
Internal Link Dist (ft)	832			756		942
Turn Bay Length (ft)		250	400			250
Base Capacity (vph)	920	1035	629	1465	362	1538
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.05	0.41	0.14	0.30	0.71

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	68 (76%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow
Natural Cycle:	45
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.71
Intersection Signal Delay:	8.1
Intersection LOS:	A
Intersection Capacity Utilization	39.4%
ICU Level of Service	A
Analysis Period (min)	15

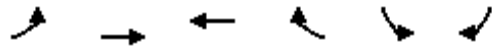
Splits and Phases: 1: US 321 SB Ramps & Clement Blvd



Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

2: Clement Blvd & US 321 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	50	1070	356	700	204	57
Future Volume (vph)	50	1070	356	700	204	57
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			0	250	0
Storage Lanes	1			1	1	1
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1770	3539	1863	1583	1719	1538
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	3539	1863	1583	1719	1538
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		35	
Link Distance (ft)		836	935		1109	
Travel Time (s)		16.3	18.2		21.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	5%
Adj. Flow (vph)	56	1189	396	778	227	63
Shared Lane Traffic (%)						
Lane Group Flow (vph)	56	1189	396	778	227	63
Turn Type	Prot	NA	NA	pm+ov	Prot	pm+ov
Protected Phases	7	4	8	6	6	7
Permitted Phases				8		6
Detector Phase	7	4	8	6	6	7
Switch Phase						
Minimum Initial (s)	7.0	10.0	10.0	7.0	7.0	7.0
Minimum Split (s)	14.0	17.0	17.0	14.0	14.0	14.0
Total Split (s)	14.0	52.0	38.0	38.0	38.0	14.0
Total Split (%)	15.6%	57.8%	42.2%	42.2%	42.2%	15.6%
Maximum Green (s)	7.0	45.0	31.0	31.0	31.0	7.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag			Lead
Lead-Lag Optimize?	Yes		Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Min	C-Min	None	None	None
Act Effct Green (s)	9.2	42.1	30.7	74.6	37.9	52.1
Actuated g/C Ratio	0.10	0.47	0.34	0.83	0.42	0.58
v/c Ratio	0.31	0.72	0.62	0.59	0.31	0.07
Control Delay	43.2	19.9	30.1	5.2	19.0	8.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.2	19.9	30.1	5.2	19.0	8.8
LOS	D	B	C	A	B	A
Approach Delay		20.9	13.6		16.8	

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

2: Clement Blvd & US 321 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Approach LOS		C	B		B	
Queue Length 50th (ft)	28	305	215	87	75	12
Queue Length 95th (ft)	m47	251	m273	296	153	37
Internal Link Dist (ft)		756	855		1029	
Turn Bay Length (ft)	250				250	
Base Capacity (vph)	182	1848	739	1311	723	890
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.64	0.54	0.59	0.31	0.07

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 78 (87%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 17.3

Intersection LOS: B

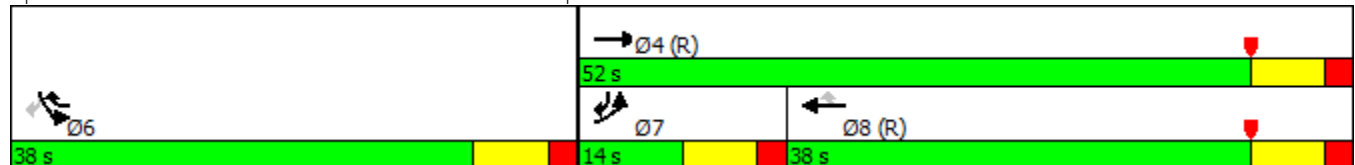
Intersection Capacity Utilization 57.5%

ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Clement Blvd & US 321 NB Ramps



Lanes, Volumes, Timings

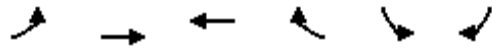
NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

3: Clement Blvd/Old Lenoir & 12th Avenue



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	572	691	300	42	129	765
Future Volume (vph)	572	691	300	42	129	765
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			250	175	0
Storage Lanes	1			0	1	1
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	1.00	0.95	0.95	1.00	1.00
Frt			0.981			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	1863	3472	0	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	1863	3472	0	1770	1583
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		35	
Link Distance (ft)		935	1298		476	
Travel Time (s)		18.2	25.3		9.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	636	768	333	47	143	850
Shared Lane Traffic (%)						
Lane Group Flow (vph)	636	768	380	0	143	850
Turn Type	Prot	NA	NA		Prot	pm+ov
Protected Phases	7	4	8		6	7
Permitted Phases						6
Detector Phase	7	4	8		6	7
Switch Phase						
Minimum Initial (s)	7.0	10.0	10.0		7.0	7.0
Minimum Split (s)	14.0	17.0	17.0		14.0	14.0
Total Split (s)	51.0	73.0	22.0		17.0	51.0
Total Split (%)	56.7%	81.1%	24.4%		18.9%	56.7%
Maximum Green (s)	44.0	66.0	15.0		10.0	44.0
Yellow Time (s)	5.0	5.0	5.0		5.0	5.0
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0		-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0		5.0	5.0
Lead/Lag	Lag		Lead			Lag
Lead-Lag Optimize?	Yes		Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Min	C-Min		None	None
Act Effct Green (s)	44.3	67.8	18.5		12.2	61.5
Actuated g/C Ratio	0.49	0.75	0.21		0.14	0.68
v/c Ratio	0.38	0.55	0.53		0.60	0.79
Control Delay	6.2	2.7	35.6		33.5	6.4
Queue Delay	0.0	0.0	0.0		0.0	0.4
Total Delay	6.2	2.7	35.6		33.5	6.8
LOS	A	A	D		C	A
Approach Delay		4.3	35.6		10.7	
Approach LOS		A	D		B	

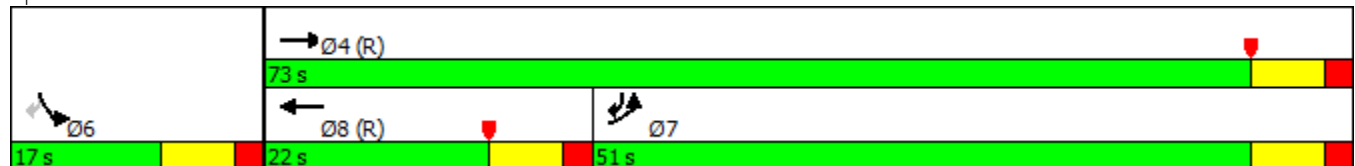
Lanes, Volumes, Timings NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM
3: Clement Blvd/Old Lenoir & 12th Avenue



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Queue Length 50th (ft)	31	17	104		78	62
Queue Length 95th (ft)	55	82	151		m96	58
Internal Link Dist (ft)		855	1218		396	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	1754	1416	714		248	1058
Starvation Cap Reductn	0	0	0		0	30
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.36	0.54	0.53		0.58	0.83

Intersection Summary
 Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 48 (53%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 10.9 Intersection LOS: B
 Intersection Capacity Utilization 65.3% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Clement Blvd/Old Lenoir & 12th Avenue



Lanes, Volumes, Timings
4: 12th Avenue & 12St Dr

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø2
Lane Configurations	↻		↻	↻	↻	↻	
Traffic Volume (vph)	57	169	727	29	65	547	
Future Volume (vph)	57	169	727	29	65	547	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)		0	600		100	0	
Storage Lanes		0	1		1	2	
Taper Length (ft)			100		100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.88	
Frt	0.899					0.850	
Flt Protected			0.950		0.950		
Satd. Flow (prot)	1675	0	1770	1863	1770	2787	
Flt Permitted			0.950		0.950		
Satd. Flow (perm)	1675	0	1770	1863	1770	2787	
Right Turn on Red		No				No	
Satd. Flow (RTOR)							
Link Speed (mph)	35			35	35		
Link Distance (ft)	1434			992	476		
Travel Time (s)	27.9			19.3	9.3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	63	188	808	32	72	608	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	251	0	808	32	72	608	
Turn Type	NA		Prot	NA	Prot	pm+ov	
Protected Phases	4		3	8	5	3	2
Permitted Phases						2	
Detector Phase	4		3	8	5	3	
Switch Phase							
Minimum Initial (s)	10.0		7.0	10.0	7.0	7.0	7.0
Minimum Split (s)	17.0		14.0	17.0	14.0	14.0	14.0
Total Split (s)	22.0		54.0	76.0	14.0	54.0	14.0
Total Split (%)	24.4%		60.0%	84.4%	15.6%	60.0%	16%
Maximum Green (s)	15.0		47.0	69.0	7.0	47.0	7.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0	
Lead/Lag	Lead		Lag			Lag	
Lead-Lag Optimize?	Yes		Yes			Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0	3.0
Recall Mode	None		C-Min	None	None	C-Min	None
Act Effct Green (s)	16.9		51.8	74.7	9.1	63.1	
Actuated g/C Ratio	0.19		0.58	0.83	0.10	0.70	
v/c Ratio	0.80		0.79	0.02	0.40	0.31	
Control Delay	55.2		23.8	2.1	45.5	1.6	
Queue Delay	0.0		0.0	0.0	0.0	0.0	
Total Delay	55.2		23.8	2.1	45.5	1.6	
LOS	E		C	A	D	A	
Approach Delay	55.2			23.0	6.3		
Approach LOS	E			C	A		

Lanes, Volumes, Timings
4: 12th Avenue & 12St Dr

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

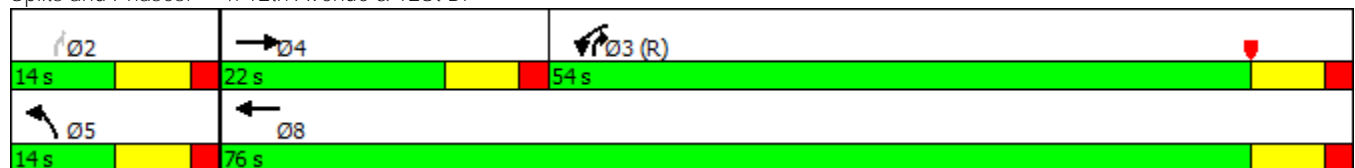


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø2
Queue Length 50th (ft)	137		362	3	45	14	
Queue Length 95th (ft)	#258		#612	8	m89	17	
Internal Link Dist (ft)	1354			912	396		
Turn Bay Length (ft)			600		100		
Base Capacity (vph)	322		1027	1548	179	1937	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.78		0.79	0.02	0.40	0.31	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 16 (18%), Referenced to phase 3:WBL, Start of Yellow
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 21.2 Intersection LOS: C
 Intersection Capacity Utilization 72.0% ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: 12th Avenue & 12St Dr



SimTraffic Simulation Summary NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	2869
Vehs Exited	2851
Starting Vehs	95
Ending Vehs	113
Travel Distance (mi)	1967
Travel Time (hr)	116.8
Total Delay (hr)	52.2
Total Stops	3640
Fuel Used (gal)	90.0

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	2869
Vehs Exited	2851
Starting Vehs	95
Ending Vehs	113
Travel Distance (mi)	1967
Travel Time (hr)	116.8
Total Delay (hr)	52.2
Total Stops	3640
Fuel Used (gal)	90.0

Queuing and Blocking Report NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Intersection: 1: US 321 SB Ramps & Clement Blvd

Movement	EB	EB	WB	WB	NB
Directions Served	T	R	L	T	L
Maximum Queue (ft)	97	53	246	116	150
Average Queue (ft)	43	18	125	28	50
95th Queue (ft)	81	48	223	87	107
Link Distance (ft)	876			781	972
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	400		
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: Clement Blvd & US 321 NB Ramps

Movement	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	T	T	T	R	L	R
Maximum Queue (ft)	93	238	311	248	221	197	133
Average Queue (ft)	41	139	160	138	109	96	28
95th Queue (ft)	78	209	251	233	183	180	78
Link Distance (ft)		781	781	874	874		1066
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)	250					250	
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Intersection: 3: Clement Blvd/Old Lenoir & 12th Avenue

Movement	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	L	T	T	TR	L	R
Maximum Queue (ft)	349	371	182	187	204	274	400
Average Queue (ft)	74	94	57	80	123	113	235
95th Queue (ft)	188	199	131	157	192	230	392
Link Distance (ft)		874	874	1253	1253		377
Upstream Blk Time (%)							7
Queuing Penalty (veh)							60
Storage Bay Dist (ft)	250					175	
Storage Blk Time (%)	1	1				1	17
Queuing Penalty (veh)	3	4				6	22

Queuing and Blocking Report NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Intersection: 4: 12th Avenue & 12St Dr

Movement	EB	WB	WB	B6	NB	NB	NB
Directions Served	TR	L	T	T	L	R	R
Maximum Queue (ft)	421	700	994	136	112	95	155
Average Queue (ft)	223	362	182	12	49	20	38
95th Queue (ft)	409	676	789	70	96	65	86
Link Distance (ft)	1396		922	121		377	377
Upstream Blk Time (%)			6	2			
Queuing Penalty (veh)			0	0			
Storage Bay Dist (ft)		600			100		
Storage Blk Time (%)		12	1		1	0	
Queuing Penalty (veh)		4	5		3	0	

Network Summary

Network wide Queuing Penalty: 107

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

1: US 321 SB Ramps & Clement Blvd



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	222	57	204	139	50	700
Future Volume (vph)	222	57	204	139	50	700
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	400		0	250
Storage Lanes		1	1		1	1
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1719	1538
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1863	1583	1770	1863	1719	1538
Right Turn on Red		No				No
Satd. Flow (RTOR)						
Link Speed (mph)	35			35	35	
Link Distance (ft)	912			836	1022	
Travel Time (s)	17.8			16.3	19.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	5%
Adj. Flow (vph)	247	63	227	154	56	778
Shared Lane Traffic (%)						
Lane Group Flow (vph)	247	63	227	154	56	778
Turn Type	NA	pm+ov	Prot	NA	Prot	Free
Protected Phases	4	2	3	8	2	
Permitted Phases		4				Free
Detector Phase	4	2	3	8	2	
Switch Phase						
Minimum Initial (s)	10.0	7.0	7.0	10.0	7.0	
Minimum Split (s)	17.0	14.0	14.0	17.0	14.0	
Total Split (s)	37.0	18.0	35.0	72.0	18.0	
Total Split (%)	41.1%	20.0%	38.9%	80.0%	20.0%	
Maximum Green (s)	30.0	11.0	28.0	65.0	11.0	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Min	None	None	C-Min	None	
Act Effct Green (s)	48.3	61.2	18.8	73.1	10.7	90.0
Actuated g/C Ratio	0.54	0.68	0.21	0.81	0.12	1.00
v/c Ratio	0.25	0.06	0.61	0.10	0.28	0.51
Control Delay	14.6	6.1	15.4	0.9	39.1	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.6	6.1	15.4	0.9	39.1	1.2
LOS	B	A	B	A	D	A
Approach Delay	12.9			9.5	3.7	

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

1: US 321 SB Ramps & Clement Blvd



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Approach LOS	B			A		
Queue Length 50th (ft)	76	10	20	6	30	0
Queue Length 95th (ft)	152	29	m126	m4	64	0
Internal Link Dist (ft)	832			756	942	
Turn Bay Length (ft)	250		400		250	
Base Capacity (vph)	999	1051	590	1515	249	1538
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.06	0.38	0.10	0.22	0.51

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 77 (86%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow
 Natural Cycle: 45
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 7.0
 Intersection LOS: A
 Intersection Capacity Utilization 41.3%
 ICU Level of Service A
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

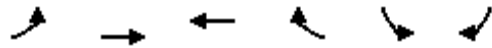
Splits and Phases: 1: US 321 SB Ramps & Clement Blvd



Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

2: Clement Blvd & US 321 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	96	826	296	978	230	47
Future Volume (vph)	96	826	296	978	230	47
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			0	250	0
Storage Lanes	1			1	1	1
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1770	3539	1863	1583	1719	1538
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	3539	1863	1583	1719	1538
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		35	
Link Distance (ft)		836	935		1109	
Travel Time (s)		16.3	18.2		21.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	5%
Adj. Flow (vph)	107	918	329	1087	256	52
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	918	329	1087	256	52
Turn Type	Prot	NA	NA	pm+ov	Prot	pm+ov
Protected Phases	7	4	8	6	6	7
Permitted Phases				8		6
Detector Phase	7	4	8	6	6	7
Switch Phase						
Minimum Initial (s)	7.0	10.0	10.0	7.0	7.0	7.0
Minimum Split (s)	14.0	17.0	17.0	14.0	14.0	14.0
Total Split (s)	14.0	41.0	27.0	49.0	49.0	14.0
Total Split (%)	15.6%	45.6%	30.0%	54.4%	54.4%	15.6%
Maximum Green (s)	7.0	34.0	20.0	42.0	42.0	7.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead		Lag			Lead
Lead-Lag Optimize?	Yes		Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Min	C-Min	None	None	None
Act Effect Green (s)	9.0	33.5	22.3	74.8	46.5	60.5
Actuated g/C Ratio	0.10	0.37	0.25	0.83	0.52	0.67
v/c Ratio	0.60	0.70	0.72	0.83	0.29	0.05
Control Delay	55.2	23.5	33.2	12.6	14.2	6.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.2	23.5	33.2	12.6	14.2	6.0
LOS	E	C	C	B	B	A
Approach Delay		26.8	17.4		12.8	

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

2: Clement Blvd & US 321 NB Ramps

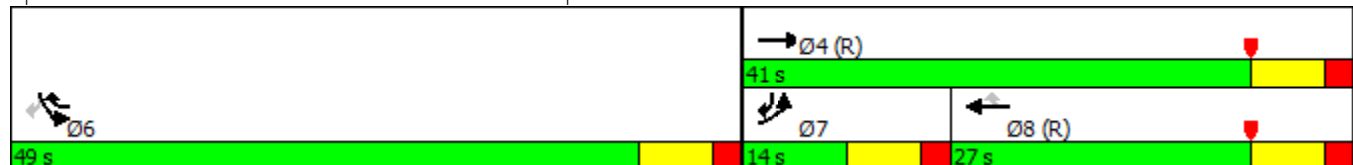


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Approach LOS		C	B		B	
Queue Length 50th (ft)	51	217	122	395	84	10
Queue Length 95th (ft)	#128	213	m220	#799	136	22
Internal Link Dist (ft)		756	855		1029	
Turn Bay Length (ft)	250				250	
Base Capacity (vph)	177	1415	472	1315	888	1034
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.60	0.65	0.70	0.83	0.29	0.05

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 68 (76%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 20.4
 Intersection LOS: C
 Intersection Capacity Utilization 74.7%
 ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

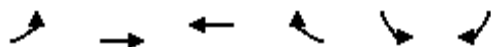
Splits and Phases: 2: Clement Blvd & US 321 NB Ramps



Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

3: Clement Blvd/Old Lenoir & 12th Avenue

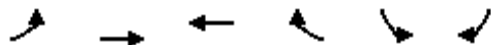


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	765	300	691	129	42	572
Future Volume (vph)	765	300	691	129	42	572
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			250	175	0
Storage Lanes	1			0	1	1
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	1.00	0.95	0.95	1.00	1.00
Frt			0.976			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	1863	3454	0	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	1863	3454	0	1770	1583
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		35	
Link Distance (ft)		935	1298		476	
Travel Time (s)		18.2	25.3		9.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	850	333	768	143	47	636
Shared Lane Traffic (%)						
Lane Group Flow (vph)	850	333	911	0	47	636
Turn Type	Prot	NA	NA		Prot	pm+ov
Protected Phases	7	4	8		6	7
Permitted Phases						6
Detector Phase	7	4	8		6	7
Switch Phase						
Minimum Initial (s)	7.0	10.0	10.0		7.0	7.0
Minimum Split (s)	14.0	17.0	17.0		14.0	14.0
Total Split (s)	39.0	76.0	37.0		14.0	39.0
Total Split (%)	43.3%	84.4%	41.1%		15.6%	43.3%
Maximum Green (s)	32.0	69.0	30.0		7.0	32.0
Yellow Time (s)	5.0	5.0	5.0		5.0	5.0
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0		-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0		5.0	5.0
Lead/Lag	Lag		Lead			Lag
Lead-Lag Optimize?	Yes		Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Min	C-Min		None	None
Act Effct Green (s)	36.2	78.6	35.4		9.0	44.6
Actuated g/C Ratio	0.40	0.87	0.39		0.10	0.50
v/c Ratio	0.62	0.20	0.67		0.27	0.81
Control Delay	14.1	0.9	26.4		27.7	14.8
Queue Delay	0.0	0.0	0.0		0.0	1.2
Total Delay	14.1	0.9	26.4		27.7	16.0
LOS	B	A	C		C	B
Approach Delay		10.4	26.4		16.8	
Approach LOS		B	C		B	

Lanes, Volumes, Timings

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

3: Clement Blvd/Old Lenoir & 12th Avenue

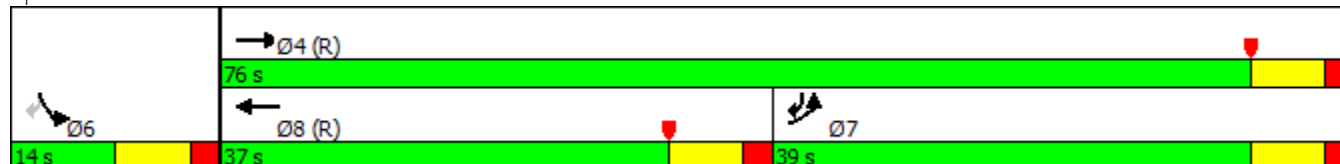


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Queue Length 50th (ft)	216	8	235		27	311
Queue Length 95th (ft)	297	24	308		m39	85
Internal Link Dist (ft)		855	1218		396	
Turn Bay Length (ft)	250				175	
Base Capacity (vph)	1380	1627	1358		177	784
Starvation Cap Reductn	0	0	0		0	42
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.62	0.20	0.67		0.27	0.86

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 60 (67%), Referenced to phase 4:EBT and 8:WBT, Start of Yellow
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.81
 Intersection Signal Delay: 17.2
 Intersection LOS: B
 Intersection Capacity Utilization 67.0%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Clement Blvd/Old Lenoir & 12th Avenue



Lanes, Volumes, Timings
4: 12th Avenue & 12St Dr

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø2
Lane Configurations	↔		↶	↷	↶	↷	
Traffic Volume (vph)	29	65	547	57	169	727	
Future Volume (vph)	29	65	547	57	169	727	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)		0	600		100	0	
Storage Lanes		0	1		1	2	
Taper Length (ft)			50		50		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.88	
Frt	0.907					0.850	
Flt Protected			0.950		0.950		
Satd. Flow (prot)	1690	0	1770	1863	1770	2787	
Flt Permitted			0.950		0.950		
Satd. Flow (perm)	1690	0	1770	1863	1770	2787	
Right Turn on Red		No				No	
Satd. Flow (RTOR)							
Link Speed (mph)	35			35	35		
Link Distance (ft)	1434			992	476		
Travel Time (s)	27.9			19.3	9.3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	32	72	608	63	188	808	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	104	0	608	63	188	808	
Turn Type	NA		Prot	NA	Prot	pm+ov	
Protected Phases	4		3	8	5	3	2
Permitted Phases						2	
Detector Phase	4		3	8	5	3	
Switch Phase							
Minimum Initial (s)	10.0		7.0	10.0	7.0	7.0	7.0
Minimum Split (s)	17.0		14.0	17.0	14.0	14.0	14.0
Total Split (s)	17.0		51.0	68.0	22.0	51.0	22.0
Total Split (%)	18.9%		56.7%	75.6%	24.4%	56.7%	24%
Maximum Green (s)	10.0		44.0	61.0	15.0	44.0	15.0
Yellow Time (s)	5.0		5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0		-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0	
Lead/Lag	Lead		Lag			Lag	
Lead-Lag Optimize?	Yes		Yes			Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0	3.0
Recall Mode	None		C-Min	None	None	C-Min	None
Act Effct Green (s)	12.2		50.7	64.5	15.5	72.2	
Actuated g/C Ratio	0.14		0.56	0.72	0.17	0.80	
v/c Ratio	0.46		0.61	0.05	0.62	0.36	
Control Delay	43.1		18.4	4.3	39.9	1.6	
Queue Delay	0.0		0.0	0.0	0.0	0.0	
Total Delay	43.1		18.4	4.3	39.9	1.6	
LOS	D		B	A	D	A	
Approach Delay	43.1			17.1	8.8		
Approach LOS	D			B	A		

Lanes, Volumes, Timings
4: 12th Avenue & 12St Dr

NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

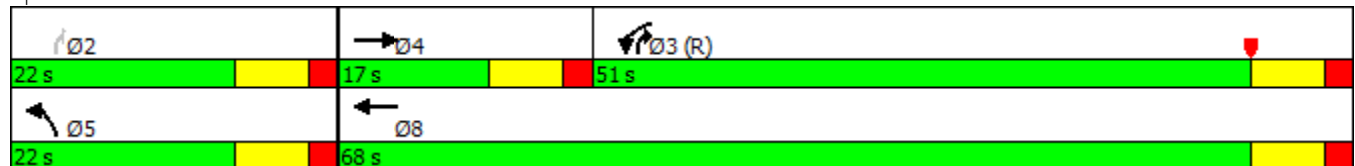


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø2
Queue Length 50th (ft)	55		248	9	118	23	
Queue Length 95th (ft)	107		367	21	m180	32	
Internal Link Dist (ft)	1354			912	396		
Turn Bay Length (ft)			600		100		
Base Capacity (vph)	228		1004	1339	339	2186	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.46		0.61	0.05	0.55	0.37	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	20 (22%), Referenced to phase 3:WBL, Start of Yellow
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.62
Intersection Signal Delay:	14.0
Intersection LOS:	B
Intersection Capacity Utilization	54.7%
ICU Level of Service	A
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 4: 12th Avenue & 12St Dr



SimTraffic Simulation Summary NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	2856
Vehs Exited	2829
Starting Vehs	93
Ending Vehs	120
Travel Distance (mi)	1921
Travel Time (hr)	108.7
Total Delay (hr)	45.6
Total Stops	3766
Fuel Used (gal)	89.4

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	2856
Vehs Exited	2829
Starting Vehs	93
Ending Vehs	120
Travel Distance (mi)	1921
Travel Time (hr)	108.7
Total Delay (hr)	45.6
Total Stops	3766
Fuel Used (gal)	89.4

Queuing and Blocking Report NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Intersection: 1: US 321 SB Ramps & Clement Blvd

Movement	EB	EB	WB	WB	NB
Directions Served	T	R	L	T	L
Maximum Queue (ft)	177	52	232	74	129
Average Queue (ft)	60	14	107	15	37
95th Queue (ft)	136	41	193	52	95
Link Distance (ft)	876			781	972
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	400		
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: Clement Blvd & US 321 NB Ramps

Movement	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	T	T	T	R	L	R
Maximum Queue (ft)	119	309	249	243	333	244	48
Average Queue (ft)	71	181	122	118	170	97	14
95th Queue (ft)	117	277	212	211	294	174	39
Link Distance (ft)		781	781	874	874		1066
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)	250					250	
Storage Blk Time (%)		2				0	
Queuing Penalty (veh)		2				0	

Intersection: 3: Clement Blvd/Old Lenoir & 12th Avenue

Movement	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	L	T	T	TR	L	R
Maximum Queue (ft)	247	256	139	308	342	225	397
Average Queue (ft)	116	133	17	165	214	48	229
95th Queue (ft)	207	217	67	264	306	133	393
Link Distance (ft)		874	874	1253	1253		377
Upstream Blk Time (%)							2
Queuing Penalty (veh)							13
Storage Bay Dist (ft)	250					175	
Storage Blk Time (%)	0	0					21
Queuing Penalty (veh)	0	1					9

Queuing and Blocking Report NCDOT TIP-U4700 2040 Clement Blvd Partial Cloverleaf AM

Intersection: 4: 12th Avenue & 12St Dr

Movement	EB	WB	WB	NB	NB	NB
Directions Served	TR	L	T	L	R	R
Maximum Queue (ft)	111	354	53	149	284	290
Average Queue (ft)	54	184	15	101	79	78
95th Queue (ft)	99	325	44	160	221	200
Link Distance (ft)	1396		922		377	377
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)		600		100		
Storage Blk Time (%)				13	3	
Queuing Penalty (veh)				48	5	

Network Summary

Network wide Queuing Penalty: 78

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB						
Agency or Company	AECOM	Junction	201 - US 321 at Clement Blvd						
Date Performed	2/9/17	Jurisdiction							
Analysis Time Period	AM Peak	Analysis Year	2040						
Project Description US 321 at Clement Boulevard (Partial Cloverleaf)									
Inputs									
Upstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Freeway Number of Lanes, N	2	Downstream Adj Ramp <input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off						
$L_{up} =$ ft	Ramp Number of Lanes, N	1	$L_{down} =$ 500 ft						
$V_u =$ veh/h	Acceleration Lane Length, L_A		$V_D =$ 750 veh/h						
	Deceleration Lane Length L_D	100							
	Freeway Volume, V_F	1522							
	Ramp Volume, V_R	261							
	Freeway Free-Flow Speed, S_{FF}	55.0							
	Ramp Free-Flow Speed, S_{FR}	25.0							
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	$v = V/PHF \times f_{HV} \times f_p$	
Freeway	1522	0.90	Rolling	5	0	0.930	1.00	1818	
Ramp	261	0.90	Rolling	2	0	0.971	1.00	299	
UpStream									
DownStream	750	0.90	Rolling	2	0	0.971	1.00	858	
Merge Areas					Diverge Areas				
Estimation of v_{12}					Estimation of v_{12}				
$L_{EQ} =$	$V_{12} = V_F (P_{FM})$	(Equation 13-6 or 13-7)			$L_{EQ} =$	$V_{12} = V_R + (V_F - V_R)P_{FD}$	(Equation 13-12 or 13-13)		
$P_{FM} =$	using Equation (Exhibit 13-6)				$P_{FD} =$	1.000 using Equation (Exhibit 13-7)			
$V_{12} =$	pc/h				$V_{12} =$	1818 pc/h			
V_3 or V_{av34}	pc/h (Equation 13-14 or 13-17)				V_3 or V_{av34}	0 pc/h (Equation 13-14 or 13-17)			
Is V_3 or $V_{av34} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input type="checkbox"/> No				Is V_3 or $V_{av34} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No			
Is V_3 or $V_{av34} > 1.5 * V_{12}/2$	<input type="checkbox"/> Yes <input type="checkbox"/> No				Is V_3 or $V_{av34} > 1.5 * V_{12}/2$	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No			
If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)				If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)			
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V_{FO}		Exhibit 13-8			V_F	1818	Exhibit 13-8	4500	No
					$V_{FO} = V_F - V_R$	1519	Exhibit 13-8	4500	No
					V_R	299	Exhibit 13-10	1900	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable	Violation?			Actual	Max Desirable	Violation?	
V_{R12}		Exhibit 13-8			V_{12}	1818	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 V_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
$D_R =$ (pc/mi/ln)					$D_R =$ 19.0 (pc/mi/ln)				
LOS = (Exhibit 13-2)					LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
$M_S =$ (Exhibit 13-11)					$D_S =$ 0.585 (Exhibit 13-12)				
$S_R =$ mph (Exhibit 13-11)					$S_R =$ 47.4 mph (Exhibit 13-12)				
$S_0 =$ mph (Exhibit 13-11)					$S_0 =$ N/A mph (Exhibit 13-12)				
$S =$ mph (Exhibit 13-13)					$S =$ 47.4 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB		Agency or Company	AECOM	Junction	201 - US 321 at Clement Blvd	
Date Performed	2/7/17	Jurisdiction			Analysis Time Period	PM Peak	Analysis Year	2040	
Project Description US 321 at Clement Boulevard (Partial Cloverleaf)									
Inputs									
Upstream Adj Ramp	Freeway Number of Lanes, N = 2					Downstream Adj Ramp			
<input type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N = 1					<input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On			
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Acceleration Lane Length, L _A					<input type="checkbox"/> No <input type="checkbox"/> Off			
L _{up} = ft	Deceleration Lane Length L _D = 100					L _{down} = 500 ft			
V _u = veh/h	Freeway Volume, V _F = 2257					V _D = 1074 veh/h			
	Ramp Volume, V _R = 277								
	Freeway Free-Flow Speed, S _{FF} = 55.0								
	Ramp Free-Flow Speed, S _{FR} = 25.0								
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	2257	0.90	Rolling	5	0	0.930	1.00	2696	
Ramp	277	0.90	Rolling	2	0	0.971	1.00	317	
UpStream									
DownStream	1074	0.90	Rolling	2	0	0.971	1.00	1229	
Merge Areas					Diverge Areas				
Estimation of v₁₂					Estimation of v₁₂				
$V_{12} = V_F (P_{FM})$ L _{EQ} = (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ L _{EQ} = (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 2696 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	2696	Exhibit 13-8	4500	No
					V _{FO} = V _F - V _R	2379	Exhibit 13-8	4500	No
					V _R	317	Exhibit 13-10	1900	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	2696	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = 26.5 (pc/mi/ln) LOS = C (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11)					D _S = 0.587 (Exhibit 13-12)				
S _R = mph (Exhibit 13-11)					S _R = 47.4 mph (Exhibit 13-12)				
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)				
S = mph (Exhibit 13-13)					S = 47.4 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB
Agency or Company	AECOM	Junction	202 - US 321 at Clement Blvd
Date Performed	10/14/11	Jurisdiction	
Analysis Time Period	AM Peak	Analysis Year	2040

Project Description US 321 at Clement Boulevard - (Partial Cloverleaf)

Inputs

Upstream Adj Ramp	Freeway Number of Lanes, N	2	Downstream Adj Ramp
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N	1	<input type="checkbox"/> Yes <input type="checkbox"/> On
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A	500	<input checked="" type="checkbox"/> No <input type="checkbox"/> Off
L _{up} = 500 ft	Deceleration Lane Length L _D		L _{down} = ft
V _u = 1074 veh/h	Freeway Volume, V _F	1980	V _D = veh/h
	Ramp Volume, V _R	277	
	Freeway Free-Flow Speed, S _{FF}	55.0	
	Ramp Free-Flow Speed, S _{FR}	35.0	

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1980	0.90	Rolling	5	0	0.930	1.00	2365
Ramp	277	0.90	Rolling	2	0	0.971	1.00	317
UpStream	1074	0.90	Rolling	2	0	0.971	1.00	1229
DownStream								

Merge Areas

Estimation of v₁₂

$$V_{12} = V_F (P_{FM})$$

(Equation 13-6 or 13-7)

L_{EQ} =

P_{FM} = 1.000 using Equation (Exhibit 13-6)

V₁₂ = 2365 pc/h

V₃ or V_{av34} = 0 pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Diverge Areas

Estimation of v₁₂

$$V_{12} = V_R + (V_F - V_R)P_{FD}$$

(Equation 13-12 or 13-13)

L_{EQ} =

P_{FD} = using Equation (Exhibit 13-7)

V₁₂ = pc/h

V₃ or V_{av34} = pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Capacity Checks

	Actual	Capacity	LOS F?
V _{FO}	2682	Exhibit 13-8	No

Capacity Checks

	Actual	Capacity	LOS F?
V _F		Exhibit 13-8	
V _{FO} = V _F - V _R		Exhibit 13-8	
V _R		Exhibit 13-10	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
V _{R12}	2682	Exhibit 13-8	4600:All
			No

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
V ₁₂		Exhibit 13-8	

Level of Service Determination (if not F)

$$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$$

D_R = 23.1 (pc/mi/ln)

LOS = C (Exhibit 13-2)

Level of Service Determination (if not F)

$$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$$

D_R = (pc/mi/ln)

LOS = (Exhibit 13-2)

Speed Determination

M_S = 0.343 (Exhibit 13-11)

S_R = 50.5 mph (Exhibit 13-11)

S₀ = N/A mph (Exhibit 13-11)

S = 50.5 mph (Exhibit 13-13)

Speed Determination

D_S = (Exhibit 13-12)

S_R = mph (Exhibit 13-12)

S₀ = mph (Exhibit 13-12)

S = mph (Exhibit 13-13)

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB
Agency or Company	AECOM	Junction	202 - US 321 at Clement Blvd
Date Performed	2/9/17	Jurisdiction	
Analysis Time Period	PM Peak	Analysis Year	2040

Project Description US 321 at Clement Boulevard - (Partial Cloverleaf)

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off	Freeway Number of Lanes, N 2 Ramp Number of Lanes, N 1 Acceleration Lane Length, L _A 500 Deceleration Lane Length L _D Freeway Volume, V _F 1261 Ramp Volume, V _R 261 Freeway Free-Flow Speed, S _{FF} 55.0 Ramp Free-Flow Speed, S _{FR} 35.0	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
L _{up} = 500 ft		
V _u = 750 veh/h		

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1261	0.90	Rolling	5	0	0.930	1.00	1506
Ramp	261	0.90	Rolling	2	0	0.971	1.00	299
UpStream	750	0.90	Rolling	2	0	0.971	1.00	858
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$
 (Equation 13-6 or 13-7)
 P_{FM} = 1.000 using Equation (Exhibit 13-6)
 V₁₂ = 1506 pc/h
 V₃ or V_{av34} = 0 pc/h (Equation 13-14 or 13-17)
 Is V₃ or V_{av34} > 2,700 pc/h? Yes No
 Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
 If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Estimation of v₁₂

$V_{12} = V_R + (V_F - V_R)P_{FD}$
 (Equation 13-12 or 13-13)
 P_{FD} = using Equation (Exhibit 13-7)
 V₁₂ = pc/h
 V₃ or V_{av34} = pc/h (Equation 13-14 or 13-17)
 Is V₃ or V_{av34} > 2,700 pc/h? Yes No
 Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
 If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Capacity Checks

	Actual	Capacity	LOS F?
V _{FO}	1805	Exhibit 13-8	No

Capacity Checks

	Actual	Capacity	LOS F?
V _F		Exhibit 13-8	
V _{FO} = V _F - V _R		Exhibit 13-8	
V _R		Exhibit 13-10	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
V _{R12}	1805	Exhibit 13-8	4600:All No

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
V ₁₂		Exhibit 13-8	

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$
 D_R = 16.3 (pc/mi/ln)
 LOS = B (Exhibit 13-2)

Level of Service Determination (if not F)

$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$
 D_R = (pc/mi/ln)
 LOS = (Exhibit 13-2)

Speed Determination

M_S = 0.310 (Exhibit 13-11)
 S_R = 51.0 mph (Exhibit 13-11)
 S₀ = N/A mph (Exhibit 13-11)
 S = 51.0 mph (Exhibit 13-13)

Speed Determination

D_s = (Exhibit 13-12)
 S_R = mph (Exhibit 13-12)
 S₀ = mph (Exhibit 13-12)
 S = mph (Exhibit 13-13)

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB
Agency or Company	AECOM	Junction	203 - US 321 at Clement Blvd
Date Performed	2/9/17	Jurisdiction	
Analysis Time Period	AM Peak	Analysis Year	2040

Project Description US 321 at Clement Boulevard - (Partial Cloverleaf)

Inputs

Upstream Adj Ramp	Freeway Number of Lanes, N	2	Downstream Adj Ramp
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N	1	<input type="checkbox"/> Yes <input type="checkbox"/> On
<input type="checkbox"/> No <input checked="" type="checkbox"/> Off	Acceleration Lane Length, L _A	500	<input checked="" type="checkbox"/> No <input type="checkbox"/> Off
L _{up} = 500 ft	Deceleration Lane Length L _D		L _{down} = ft
V _u = 261 veh/h	Freeway Volume, V _F	1261	V _D = veh/h
	Ramp Volume, V _R	750	
	Freeway Free-Flow Speed, S _{FF}	55.0	
	Ramp Free-Flow Speed, S _{FR}	35.0	

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1261	0.90	Rolling	5	0	0.930	1.00	1506
Ramp	750	0.90	Rolling	2	0	0.971	1.00	858
UpStream	261	0.90	Rolling	2	0	0.971	1.00	299
DownStream								

Merge Areas

Estimation of v₁₂

$$V_{12} = V_F (P_{FM})$$

(Equation 13-6 or 13-7)

L_{EQ} =

P_{FM} = 1.000 using Equation (Exhibit 13-6)

V₁₂ = 1506 pc/h

V₃ or V_{av34} = 0 pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Diverge Areas

Estimation of v₁₂

$$V_{12} = V_R + (V_F - V_R)P_{FD}$$

(Equation 13-12 or 13-13)

L_{EQ} =

P_{FD} = using Equation (Exhibit 13-7)

V₁₂ = pc/h

V₃ or V_{av34} = pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Capacity Checks

	Actual	Capacity	LOS F?
V _{FO}	2364	Exhibit 13-8	No

Capacity Checks

	Actual	Capacity	LOS F?
V _F		Exhibit 13-8	
V _{FO} = V _F - V _R		Exhibit 13-8	
V _R		Exhibit 13-10	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
V _{R12}	2364	Exhibit 13-8	4600:All
			No

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
V ₁₂		Exhibit 13-8	

Level of Service Determination (if not F)

$$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$$

D_R = 20.4 (pc/mi/ln)

LOS = C (Exhibit 13-2)

Level of Service Determination (if not F)

$$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$$

D_R = (pc/mi/ln)

LOS = (Exhibit 13-2)

Speed Determination

M_S = 0.327 (Exhibit 13-11)

S_R = 50.7 mph (Exhibit 13-11)

S₀ = N/A mph (Exhibit 13-11)

S = 50.7 mph (Exhibit 13-13)

Speed Determination

D_S = (Exhibit 13-12)

S_R = mph (Exhibit 13-12)

S₀ = mph (Exhibit 13-12)

S = mph (Exhibit 13-13)

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 / NB
Agency or Company	AECOM	Junction	203 - US 321 at Clement Blvd
Date Performed	2/9/17	Jurisdiction	
Analysis Time Period	PM Peak	Analysis Year	2040

Project Description US 321 at Clement Boulevard - (Partial Cloverleaf)

Inputs

Upstream Adj Ramp <input checked="" type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off	Freeway Number of Lanes, N 2 Ramp Number of Lanes, N 1 Acceleration Lane Length, L _A 500 Deceleration Lane Length L _D Freeway Volume, V _F 1980 Ramp Volume, V _R 1074 Freeway Free-Flow Speed, S _{FF} 55.0 Ramp Free-Flow Speed, S _{FR} 35.0	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
L _{up} = 500 ft		
V _u = 277 veh/h		

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1980	0.90	Rolling	5	0	0.930	1.00	2365
Ramp	1074	0.90	Rolling	2	0	0.971	1.00	1229
UpStream	277	0.90	Rolling	2	0	0.971	1.00	317
DownStream								

Merge Areas

Estimation of v₁₂

$$V_{12} = V_F (P_{FM})$$

(Equation 13-6 or 13-7)

L_{EQ} =

P_{FM} = 1.000 using Equation (Exhibit 13-6)

V₁₂ = 2365 pc/h

V₃ or V_{av34} = 0 pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Diverge Areas

Estimation of v₁₂

$$V_{12} = V_R + (V_F - V_R)P_{FD}$$

(Equation 13-12 or 13-13)

L_{EQ} =

P_{FD} = using Equation (Exhibit 13-7)

V₁₂ = pc/h

V₃ or V_{av34} = pc/h (Equation 13-14 or 13-17)

Is V₃ or V_{av34} > 2,700 pc/h? Yes No

Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No

If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Capacity Checks

	Actual	Capacity	LOS F?
V _{FO}	3594	Exhibit 13-8	No

Capacity Checks

	Actual	Capacity	LOS F?
V _F		Exhibit 13-8	
V _{FO} = V _F - V _R		Exhibit 13-8	
V _R		Exhibit 13-10	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
V _{R12}	3594	Exhibit 13-8	4600:All No

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
V ₁₂		Exhibit 13-8	

Level of Service Determination (if not F)

$$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$$

D_R = 29.8 (pc/mi/ln)

LOS = D (Exhibit 13-2)

Level of Service Determination (if not F)

$$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$$

D_R = (pc/mi/ln)

LOS = (Exhibit 13-2)

Speed Determination

M_S = 0.428 (Exhibit 13-11)

S_R = 49.4 mph (Exhibit 13-11)

S₀ = N/A mph (Exhibit 13-11)

S = 49.4 mph (Exhibit 13-13)

Speed Determination

D_s = (Exhibit 13-12)

S_R = mph (Exhibit 13-12)

S₀ = mph (Exhibit 13-12)

S = mph (Exhibit 13-13)

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB						
Agency or Company	AECOM	Junction	204 - US 321 at Clement Blvd						
Date Performed	2/7/17	Jurisdiction							
Analysis Time Period	AM Peak	Analysis Year	2040						
Project Description US 321 at Clement Boulevard (Partial Cloverleaf)									
Inputs									
Upstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Freeway Number of Lanes, N	2	Downstream Adj Ramp <input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Ramp Number of Lanes, N	1				
$L_{up} =$ ft	Acceleration Lane Length, L_A		$L_{down} =$ 500 ft	Deceleration Lane Length L_D	100				
$V_u =$ veh/h	Freeway Volume, V_F	3054	$V_D =$ 277 veh/h	Freeway Free-Flow Speed, S_{FF}	55.0				
	Ramp Volume, V_R	1074		Ramp Free-Flow Speed, S_{FR}	35.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	$v = V/PHF \times f_{HV} \times f_p$	
Freeway	3054	0.90	Rolling	5	0	0.930	1.00	3648	
Ramp	1074	0.90	Rolling	2	0	0.971	1.00	1229	
UpStream									
DownStream	277	0.90	Rolling	2	0	0.971	1.00	317	
Merge Areas					Diverge Areas				
Estimation of v_{12}					Estimation of v_{12}				
$L_{EQ} =$	$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)				$L_{EQ} =$	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)			
$P_{FM} =$	using Equation (Exhibit 13-6)				$P_{FD} =$	1.000 using Equation (Exhibit 13-7)			
$V_{12} =$	pc/h				$V_{12} =$	3648 pc/h			
V_3 or V_{av34}	pc/h (Equation 13-14 or 13-17)				V_3 or V_{av34}	0 pc/h (Equation 13-14 or 13-17)			
Is V_3 or $V_{av34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No					Is V_3 or $V_{av34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No				
Is V_3 or $V_{av34} > 1.5 * V_{12}/2$ <input type="checkbox"/> Yes <input type="checkbox"/> No					Is V_3 or $V_{av34} > 1.5 * V_{12}/2$ <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No				
If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)				If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)			
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V_{FO}		Exhibit 13-8			V_F	3648	Exhibit 13-8	4500	No
					$V_{FO} = V_F - V_R$	2419	Exhibit 13-8	4500	No
					V_R	1229	Exhibit 13-10	2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable	Violation?			Actual	Max Desirable	Violation?	
V_{R12}		Exhibit 13-8			V_{12}	3648	Exhibit 13-8	4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$				
$D_R =$ (pc/mi/ln)					$D_R =$ 34.7 (pc/mi/ln)				
LOS = (Exhibit 13-2)					LOS = D (Exhibit 13-2)				
Speed Determination					Speed Determination				
$M_S =$ (Exhibit 13-11)					$D_S =$ 0.539 (Exhibit 13-12)				
$S_R =$ mph (Exhibit 13-11)					$S_R =$ 48.0 mph (Exhibit 13-12)				
$S_0 =$ mph (Exhibit 13-11)					$S_0 =$ N/A mph (Exhibit 13-12)				
$S =$ mph (Exhibit 13-13)					$S =$ 48.0 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst	AECOM	Freeway/Dir of Travel	US 321 / SB		Agency or Company	AECOM	Junction	204 - US 321 at Clement Blvd		
Date Performed	2/9/17	Jurisdiction			Analysis Time Period	PM Peak	Analysis Year	2040		
Project Description US 321 at Clement Boulevard (Partial Cloverleaf)										
Inputs										
Upstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Freeway Number of Lanes, N	2			Downstream Adj Ramp <input checked="" type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Ramp Number of Lanes, N	1			
$L_{up} =$ ft	Acceleration Lane Length, L_A				$L_{down} =$ 500 ft	Deceleration Lane Length L_D	100			
$V_u =$ veh/h	Freeway Volume, V_F	2011			$V_D =$ 261 veh/h	Freeway Free-Flow Speed, S_{FF}	55.0			
	Ramp Volume, V_R	750				Ramp Free-Flow Speed, S_{FR}	35.0			
Conversion to pc/h Under Base Conditions										
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	$v = V/PHF \times f_{HV} \times f_p$		
Freeway	2011	0.90	Rolling	5	0	0.930	1.00	2402		
Ramp	750	0.90	Rolling	2	0	0.971	1.00	858		
UpStream										
DownStream	261	0.90	Rolling	2	0	0.971	1.00	299		
Merge Areas					Diverge Areas					
Estimation of v_{12}					Estimation of v_{12}					
$L_{EQ} =$	$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)				$L_{EQ} =$	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
$P_{FM} =$	using Equation (Exhibit 13-6)				$P_{FD} =$	1.000 using Equation (Exhibit 13-7)				
$V_{12} =$	pc/h				$V_{12} =$	2402 pc/h				
V_3 or V_{av34}	pc/h (Equation 13-14 or 13-17)				V_3 or V_{av34}	0 pc/h (Equation 13-14 or 13-17)				
Is V_3 or $V_{av34} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input type="checkbox"/> No				Is V_3 or $V_{av34} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No				
Is V_3 or $V_{av34} > 1.5 * V_{12}/2$	<input type="checkbox"/> Yes <input type="checkbox"/> No				Is V_3 or $V_{av34} > 1.5 * V_{12}/2$	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No				
If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)				If Yes, $V_{12a} =$	pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks					
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?	
V_{FO}		Exhibit 13-8			V_F	2402	Exhibit 13-8		4500	No
					$V_{FO} = V_F - V_R$	1544	Exhibit 13-8		4500	No
					V_R	858	Exhibit 13-10		2000	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area					
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?	
V_{R12}		Exhibit 13-8			V_{12}	2402	Exhibit 13-8		4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)					
$D_R = 5.475 + 0.00734 V_R + 0.0078 V_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$					
$D_R =$ (pc/mi/ln)					$D_R =$ 24.0 (pc/mi/ln)					
LOS = (Exhibit 13-2)					LOS = C (Exhibit 13-2)					
Speed Determination					Speed Determination					
$M_S =$ (Exhibit 13-11)					$D_S =$ 0.505 (Exhibit 13-12)					
$S_R =$ mph (Exhibit 13-11)					$S_R =$ 48.4 mph (Exhibit 13-12)					
$S_0 =$ mph (Exhibit 13-11)					$S_0 =$ N/A mph (Exhibit 13-12)					
$S =$ mph (Exhibit 13-13)					$S =$ 48.4 mph (Exhibit 13-13)					

APPENDIX I

**2040 Build Alternative Clement Boulevard – Superstreet Synchro &
SimTraffic Reports**

Lanes, Volumes, Timings
 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	729	34	178	453	169	7	18	60	377	119	66
Future Volume (vph)	47	729	34	178	453	169	7	18	60	377	119	66
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	175		0	150		125	125		125	0		450
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.993				0.850			0.850		0.947	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3514	0	1770	3539	1583	1770	1863	1583	1770	1764	0
Flt Permitted	0.434			0.950			0.579			0.744		
Satd. Flow (perm)	808	3514	0	1770	3539	1583	1079	1863	1583	1386	1764	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35				45
Link Distance (ft)		444			444			649				551
Travel Time (s)		8.6			8.6			12.6				8.3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	52	810	38	198	503	188	8	20	67	419	132	73
Shared Lane Traffic (%)												
Lane Group Flow (vph)	52	848	0	198	503	188	8	20	67	419	205	0
Turn Type	Perm	NA		Prot	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2				6
Permitted Phases	4					8	2		2	6		
Detector Phase	4	4		3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		7.0	10.0	10.0	7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	17.0	17.0		14.0	17.0	17.0	14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	33.0	33.0		18.0	51.0	51.0	39.0	39.0	39.0	39.0	39.0	39.0
Total Split (%)	36.7%	36.7%		20.0%	56.7%	56.7%	43.3%	43.3%	43.3%	43.3%	43.3%	43.3%
Maximum Green (s)	26.0	26.0		11.0	44.0	44.0	32.0	32.0	32.0	32.0	32.0	32.0
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lead		Lag								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Min	C-Min		None	C-Min	C-Min	None	None	None	None	None	None
Act Effct Green (s)	30.3	30.3		13.2	48.5	48.5	31.5	31.5	31.5	31.5	31.5	31.5
Actuated g/C Ratio	0.34	0.34		0.15	0.54	0.54	0.35	0.35	0.35	0.35	0.35	0.35
v/c Ratio	0.19	0.72		0.76	0.26	0.22	0.02	0.03	0.12	0.86	0.33	
Control Delay	16.1	20.6		61.4	14.2	14.3	17.9	18.0	19.3	46.0	22.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	16.1	20.6		61.4	14.2	14.3	17.9	18.0	19.3	46.0	22.5	
LOS	B	C		E	B	B	B	B	B	D	C	
Approach Delay		20.4			24.8			18.9			38.3	
Approach LOS		C			C			B			D	

Lanes, Volumes, Timings
 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

NCDOT TIP-U4700

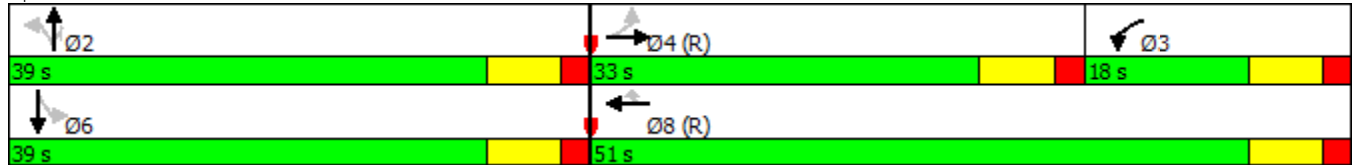


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	15	137		115	106	73	3	7	24	208	81	
Queue Length 95th (ft)	32	172		#222	143	123	12	22	52	#367	135	
Internal Link Dist (ft)		364			364			569			471	
Turn Bay Length (ft)	175			150		125	125		125			
Base Capacity (vph)	271	1181		261	1905	852	407	703	598	523	666	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.19	0.72		0.76	0.26	0.22	0.02	0.03	0.11	0.80	0.31	

Intersection Summary

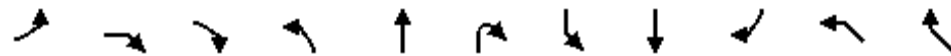
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 55 (61%), Referenced to phase 4:EBTL and 8:WBT, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 26.3 Intersection LOS: C
 Intersection Capacity Utilization 71.1% ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd



Lanes, Volumes, Timings
 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

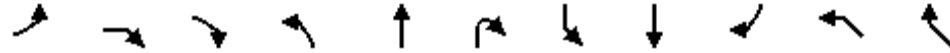
NCDOT TIP-U4700



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	189	0	0	0	0	1742	368	201	0
Future Volume (vph)	0	0	189	0	0	0	0	1742	368	201	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	450		0		0	375		250	0	0
Storage Lanes	0	0		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	0	0	0	3471	1583	1770	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	0	0	0	3471	1583	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	518				815			1204		221	
Travel Time (s)	10.1				10.1			14.9		6.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	210	0	0	0	0	1936	409	223	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	210	0	0	0	0	1936	409	223	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			24.0					66.0	66.0	24.0	
Total Split (%)			26.7%					73.3%	73.3%	26.7%	
Maximum Green (s)			17.0					59.0	59.0	17.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			17.1					62.9	62.9	17.1	
Actuated g/C Ratio			0.19					0.70	0.70	0.19	
v/c Ratio			0.69					0.80	0.37	0.66	
Control Delay			46.2					6.3	3.8	38.4	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			46.2					6.3	3.8	38.4	
LOS			D					A	A	D	
Approach Delay	46.2							5.9		38.4	

Lanes, Volumes, Timings
 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Approach LOS	D						A			D	
Queue Length 50th (ft)			109				151		50	117	
Queue Length 95th (ft)			184				m168		m58	171	
Internal Link Dist (ft)	438					735	1124		141		
Turn Bay Length (ft)			450						250		
Base Capacity (vph)			336						2424	1105	373
Starvation Cap Reductn			0						0	0	0
Spillback Cap Reductn			0						0	0	0
Storage Cap Reductn			0						0	0	0
Reduced v/c Ratio			0.63						0.80	0.37	0.60

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 20 (22%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 11.5 Intersection LOS: B
 Intersection Capacity Utilization 68.2% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW



Lanes, Volumes, Timings
44: NB US 321 & 7th Avenue NW U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↘	
Traffic Volume (vph)	0	1509	0	0	114	0
Future Volume (vph)	0	1509	0	0	114	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		55	55		25	
Link Distance (ft)		897	685		100	
Travel Time (s)		11.1	8.5		2.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	5%	2%
Adj. Flow (vph)	0	1677	0	0	127	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1677	0	0	127	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		14.0			7.0	
Minimum Split (s)		21.0			14.0	
Total Split (s)		70.0			20.0	
Total Split (%)		77.8%			22.2%	
Maximum Green (s)		63.0			13.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		66.8			13.2	
Actuated g/C Ratio		0.74			0.15	
v/c Ratio		0.66			0.50	
Control Delay		7.7			41.9	
Queue Delay		0.0			0.0	
Total Delay		7.7			41.9	
LOS		A			D	
Approach Delay		7.7			41.9	
Approach LOS		A			D	
Queue Length 50th (ft)		214			66	
Queue Length 95th (ft)		293			m80	

Lanes, Volumes, Timings
 44: NB US 321 & 7th Avenue NW U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		817	605		20	
Turn Bay Length (ft)						
Base Capacity (vph)		2558			289	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.66			0.44	

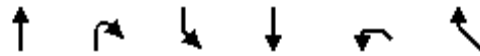
Intersection Summary
 Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 71 (79%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.66
 Intersection Signal Delay: 10.1
 Intersection LOS: B
 Intersection Capacity Utilization 56.4%
 ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 44: NB US 321 & 7th Avenue NW U-Turn



Lanes, Volumes, Timings
 47: SB US 321 & 7th Avenue NW U-Turn

NCDOT TIP-U4700












Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	114	1817	0	0
Future Volume (vph)	0	0	114	1817	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	375		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	771			815	100	
Travel Time (s)	9.6			10.1	1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	127	2019	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	127	2019	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	56.4%
Analysis Period (min)	15
	ICU Level of Service B

Lanes, Volumes, Timings
 48: NB US 321 & 7th Avenue NW Left-Over

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		 				
Traffic Volume (vph)	201	1422	0	0	0	0
Future Volume (vph)	201	1422	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1770	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1770	3438	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		685	1246		221	
Travel Time (s)		8.5	15.4		2.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	223	1580	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	223	1580	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	99.0% ICU Level of Service F
Analysis Period (min)	15

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	932	0	1516	143	0	0	0	323	0
Future Volume (vph)	0	0	932	0	1516	143	0	0	0	323	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	0		0		0	0		0	0	0
Storage Lanes	1	3		0		1	0		0	2	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.76	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	3438	1583	0	0	0	3433	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	3438	1583	0	0	0	3433	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	444				116			987		235	
Travel Time (s)	8.6				1.4			12.2		6.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	2%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	1036	0	1684	159	0	0	0	359	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	1036	0	1684	159	0	0	0	359	0
Turn Type			Prot		NA	Free				Prot	
Protected Phases			8		2					8	
Permitted Phases			8			Free				8	
Detector Phase			8		2					8	
Switch Phase											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			34.0		56.0					34.0	
Total Split (%)			37.8%		62.2%					37.8%	
Maximum Green (s)			27.0		49.0					27.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effect Green (s)			28.8		51.2	90.0				28.8	
Actuated g/C Ratio			0.32		0.57	1.00				0.32	
v/c Ratio			0.90		0.86	0.10				0.33	
Control Delay			33.3		10.3	0.1				22.8	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			33.3		10.3	0.1				22.8	
LOS			C		B	A				C	
Approach Delay	33.3				9.4					22.8	

Lanes, Volumes, Timings
 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Approach LOS	C				A				C			
Queue Length 50th (ft)			254			161	0				93	
Queue Length 95th (ft)			#346			177	m0				m106	
Internal Link Dist (ft)	364				36				907		155	
Turn Bay Length (ft)												
Base Capacity (vph)			1163			1956	1583				1106	
Starvation Cap Reductn			0			0	0				0	
Spillback Cap Reductn			0			0	0				0	
Storage Cap Reductn			0			0	0				0	
Reduced v/c Ratio			0.89			0.86	0.10				0.32	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 18.5
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lanes, Volumes, Timings

NCDOT TIP-U4700

62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations												
Traffic Volume (vph)	0	0	46	0	0	0	0	2486	31	272	0	0
Future Volume (vph)	0	0	46	0	0	0	0	2486	31	272	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00	1.00
Fr _t			0.865					0.998				
Fl _t Protected										0.950		
Satd. Flow (prot)	0	0	1565	0	0	0	0	4930	0	1719	0	0
Fl _t Permitted										0.950		
Satd. Flow (perm)	0	0	1565	0	0	0	0	4930	0	1719	0	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55			25	
Link Distance (ft)	391				835			456			104	
Travel Time (s)	7.6				10.4			5.7			2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	2%	2%	2%	2%	5%	5%	5%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	100	0	0	0
Adj. Flow (vph)	0	0	51	0	0	0	0	2762	34	302	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	51	0	0	0	0	2796	0	302	0	0
Turn Type			Free					NA		Prot		
Protected Phases								6		8		
Permitted Phases			Free									
Detector Phase								6		8		
Switch Phase												
Minimum Initial (s)								14.0		7.0		
Minimum Split (s)								21.0		14.0		
Total Split (s)								63.0		27.0		
Total Split (%)								70.0%		30.0%		
Maximum Green (s)								56.0		20.0		
Yellow Time (s)								5.0		5.0		
All-Red Time (s)								2.0		2.0		
Lost Time Adjust (s)								-2.0		-2.0		
Total Lost Time (s)								5.0		5.0		
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0		3.0		
Recall Mode								C-Min		None		
Act Effct Green (s)			90.0					59.6		20.4		
Actuated g/C Ratio			1.00					0.66		0.23		
v/c Ratio			0.03					0.86		0.77		
Control Delay			0.0					15.9		41.0		
Queue Delay			0.0					0.0		0.0		
Total Delay			0.0					15.9		41.0		
LOS			A					B		D		
Approach Delay								15.9			41.0	
Approach LOS								B			D	
Queue Length 50th (ft)			0					428		161		

62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Queue Length 95th (ft)			0					512		m188		
Internal Link Dist (ft)	311				755			376			24	
Turn Bay Length (ft)												
Base Capacity (vph)			1565					3262		420		
Starvation Cap Reductn			0					0		0		
Spillback Cap Reductn			0					0		0		
Storage Cap Reductn			0					0		0		
Reduced v/c Ratio			0.03					0.86		0.72		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 86 (96%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 18.1 Intersection LOS: B
 Intersection Capacity Utilization 71.3% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lanes, Volumes, Timings
 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

NCDOT TIP-U4700



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	↓	↓
Traffic Volume (vph)	0	0	219	0	0	0	0	1672	207	248	39	0
Future Volume (vph)	0	0	219	0	0	0	0	1672	207	248	39	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2787	0	0	0	0	3438	1583	1681	1681	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2787	0	0	0	0	3438	1583	1681	1681	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55				25
Link Distance (ft)	749				781			222				128
Travel Time (s)	14.6				9.7			2.8				3.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	0	0	243	0	0	0	0	1858	230	276	43	0
Shared Lane Traffic (%)										42%		
Lane Group Flow (vph)	0	0	243	0	0	0	0	1858	230	160	159	0
Turn Type			Prot					NA	Perm	Perm	Prot	
Protected Phases			7					6				8
Permitted Phases									6	8		
Detector Phase			7					6	6	8	8	
Switch Phase												
Minimum Initial (s)			7.0					14.0	14.0	7.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	14.0	
Total Split (s)			15.0					58.0	58.0	17.0	17.0	
Total Split (%)			16.7%					64.4%	64.4%	18.9%	18.9%	
Maximum Green (s)			8.0					51.0	51.0	10.0	10.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	5.0	
Lead/Lag			Lag							Lead	Lead	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0					3.0	3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	None	
Act Effct Green (s)			10.3					52.9	52.9	11.8	11.8	
Actuated g/C Ratio			0.11					0.59	0.59	0.13	0.13	
v/c Ratio			0.76					0.92	0.25	0.73	0.72	
Control Delay			55.7					14.5	5.9	53.0	52.7	
Queue Delay			0.0					0.0	0.0	0.0	0.0	
Total Delay			55.7					14.5	5.9	53.0	52.7	
LOS			E					B	A	D	D	
Approach Delay	55.7							13.5			52.8	
Approach LOS	E							B			D	
Queue Length 50th (ft)			77					125	29	97	97	
Queue Length 95th (ft)			#142					#481	m43	m#143	m142	

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Internal Link Dist (ft)	669				701			142			48	
Turn Bay Length (ft)												
Base Capacity (vph)			319					2024	932	224	224	
Starvation Cap Reductn			0					0	0	0	0	
Spillback Cap Reductn			0					0	0	0	0	
Storage Cap Reductn			0					0	0	0	0	
Reduced v/c Ratio			0.76					0.92	0.25	0.71	0.71	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	4 (4%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.92
Intersection Signal Delay:	22.1
Intersection LOS:	C
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lanes, Volumes, Timings
64: NB US 321 & 9th Ave NW & Clement Boulevard

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↔↔		↑↑↑						↔	
Traffic Volume (vph)	0	0	308	0	1345	138	0	0	0	168	52	0
Future Volume (vph)	0	0	308	0	1345	138	0	0	0	168	52	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		375	0		0		0	0
Storage Lanes	0	1		0		1	0		0		1	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.986							
Flt Protected											0.950	
Satd. Flow (prot)	0	0	2733	0	4884	0	0	0	0	0	1731	0
Flt Permitted											0.950	
Satd. Flow (perm)	0	0	2733	0	4884	0	0	0	0	0	1731	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	25				55			55			25	
Link Distance (ft)	322				1246			837			179	
Travel Time (s)	8.8				15.4			10.4			4.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	2%	5%	2%	2%	2%	2%	5%	2%	2%
Adj. Flow (vph)	0	0	342	0	1494	153	0	0	0	187	58	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	342	0	1647	0	0	0	0	0	245	0
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			3		2					4	4	
Permitted Phases												
Detector Phase			3		2					4	4	
Switch Phase												
Minimum Initial (s)			7.0		14.0					7.0	7.0	
Minimum Split (s)			14.0		21.0					14.0	14.0	
Total Split (s)			22.0		43.0					25.0	25.0	
Total Split (%)			24.4%		47.8%					27.8%	27.8%	
Maximum Green (s)			15.0		36.0					18.0	18.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Lost Time Adjust (s)			-2.0		-2.0						-2.0	
Total Lost Time (s)			5.0		5.0						5.0	
Lead/Lag			Lag							Lead	Lead	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effect Green (s)			16.2		40.7						18.1	
Actuated g/C Ratio			0.18		0.45						0.20	
v/c Ratio			0.69		0.75						0.71	
Control Delay			42.6		19.4						32.6	
Queue Delay			0.0		0.0						0.0	
Total Delay			42.6		19.4						32.6	
LOS			D		B						C	
Approach Delay	42.6				19.4						32.6	

Lanes, Volumes, Timings
 64: NB US 321 & 9th Ave NW & Clement Boulevard

NCDOT TIP-U4700

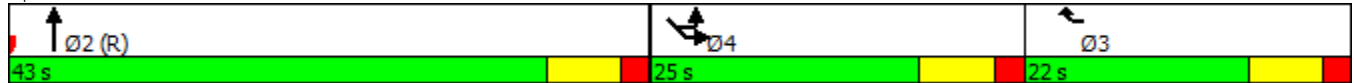


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Approach LOS	D				B				C			
Queue Length 50th (ft)				103		299					120	
Queue Length 95th (ft)				153		263					m142	
Internal Link Dist (ft)	242					1166		757				
Turn Bay Length (ft)				250								
Base Capacity (vph)				517		2207					384	
Starvation Cap Reductn				0		0					0	
Spillback Cap Reductn				0		0					0	
Storage Cap Reductn				0		0					0	
Reduced v/c Ratio				0.66		0.75					0.64	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 4 (4%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.75
 Intersection Signal Delay: 24.4
 Intersection LOS: C
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 64: NB US 321 & 9th Ave NW & Clement Boulevard



Lanes, Volumes, Timings
 65: NB US 321 & Clement Boulevard U-Turn

NCDOT TIP-U4700



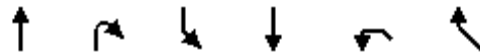
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	272	2176	0	0
Future Volume (vph)	0	0	272	2176	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	104			987	471	
Travel Time (s)	2.8			12.2	5.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	4%	4%
Adj. Flow (vph)	0	0	302	2418	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	302	2418	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.5%
Analysis Period (min)	15
	ICU Level of Service B

Lanes, Volumes, Timings
 66: SB US 321 & Clement Boulevard Left-Over

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↔↔	↑↑↑		
Traffic Volume (vph)	0	0	323	1879	0	0
Future Volume (vph)	0	0	323	1879	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	225		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3433	4940	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3433	4940	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	222			835	235	
Travel Time (s)	2.8			10.4	2.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	5%	2%	2%
Adj. Flow (vph)	0	0	359	2088	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	359	2088	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	39.6%
Analysis Period (min)	15
	ICU Level of Service A

Lanes, Volumes, Timings
 67: SB US 321 & Clement Boulevard












NCDOT TIP-U4700

	↑	↖	↘	↓	↙	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↘	↑↑		
Traffic Volume (vph)	0	0	220	2037	0	0
Future Volume (vph)	0	0	220	2037	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	175		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1204			781	179	
Travel Time (s)	14.9			9.7	2.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	244	2263	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	244	2263	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	59.6% ICU Level of Service B
Analysis Period (min)	15

Lanes, Volumes, Timings
 68: NB US 321 & Clement Boulevard Left-Over

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations	 	  				
Traffic Volume (vph)	287	1659	0	0	0	0
Future Volume (vph)	287	1659	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	375			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3433	4940	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3433	4940	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		837	116		128	
Travel Time (s)		10.4	1.4		1.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	319	1843	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	319	1843	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	35.4% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
441: Old Lenoir Rd & 12 Ave NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↶↷	↶↷	↶↷		↶	↷
Traffic Volume (vph)	509	650	269	45	115	535
Future Volume (vph)	509	650	269	45	115	535
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	0	300
Storage Lanes	2			0	1	1
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.95	0.95	0.95	1.00	1.00
Frt			0.979			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	3539	3465	0	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	3539	3465	0	1770	1583
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		35	
Link Distance (ft)		444	1015		470	
Travel Time (s)		6.7	15.4		9.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	566	722	299	50	128	594
Shared Lane Traffic (%)						
Lane Group Flow (vph)	566	722	349	0	128	594
Turn Type	Prot	NA	NA		Prot	Free
Protected Phases	7	4	8		6	
Permitted Phases						Free
Detector Phase	7	4	8		6	
Switch Phase						
Minimum Initial (s)	7.0	12.0	12.0		7.0	
Minimum Split (s)	14.0	19.0	19.0		14.0	
Total Split (s)	37.0	66.0	29.0		24.0	
Total Split (%)	41.1%	73.3%	32.2%		26.7%	
Maximum Green (s)	30.0	59.0	22.0		17.0	
Yellow Time (s)	5.0	5.0	5.0		5.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0		-2.0	
Total Lost Time (s)	5.0	5.0	5.0		5.0	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Recall Mode	None	C-Min	C-Min		None	
Act Effct Green (s)	22.3	66.2	39.0		13.8	90.0
Actuated g/C Ratio	0.25	0.74	0.43		0.15	1.00
v/c Ratio	0.67	0.28	0.23		0.47	0.38
Control Delay	18.8	0.7	18.3		39.9	0.7
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	18.8	0.7	18.3		39.9	0.7
LOS	B	A	B		D	A
Approach Delay		8.7	18.3		7.6	
Approach LOS		A	B		A	

Lanes, Volumes, Timings
 441: Old Lenoir Rd & 12 Ave NW



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Queue Length 50th (ft)	83	4	63		67	0
Queue Length 95th (ft)	m149	30	114		115	0
Internal Link Dist (ft)		364	935		390	
Turn Bay Length (ft)	200					300
Base Capacity (vph)	1220	2604	1499		373	1583
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.46	0.28	0.23		0.34	0.38

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 44 (49%), Referenced to phase 4:EBT and 8:WBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 9.8
 Intersection LOS: A
 Intersection Capacity Utilization 43.4%
 ICU Level of Service A
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 441: Old Lenoir Rd & 12 Ave NW



Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	7590
Vehs Exited	7642
Starting Vehs	244
Ending Vehs	192
Travel Distance (mi)	4767
Travel Time (hr)	250.1
Total Delay (hr)	137.7
Total Stops	9376
Fuel Used (gal)	235.7

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	7590
Vehs Exited	7642
Starting Vehs	244
Ending Vehs	192
Travel Distance (mi)	4767
Travel Time (hr)	250.1
Total Delay (hr)	137.7
Total Stops	9376
Fuel Used (gal)	235.7

Intersection: 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	L	TR
Maximum Queue (ft)	113	264	203	250	276	94	131	26	52	348	132
Average Queue (ft)	46	175	112	113	71	48	39	6	9	196	77
95th Queue (ft)	84	242	184	208	192	89	91	23	35	299	131
Link Distance (ft)		332	332		379	379			586	490	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	175			150			125	125			450
Storage Blk Time (%)		13		11	0		0				
Queuing Penalty (veh)		6		24	0		0				

Intersection: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

Movement	EB	SB	SB	SB	NW
Directions Served	>	T	T	R	L
Maximum Queue (ft)	252	271	257	157	234
Average Queue (ft)	116	145	136	83	125
95th Queue (ft)	209	229	221	144	208
Link Distance (ft)	454	1132	1132		164
Upstream Blk Time (%)					7
Queuing Penalty (veh)					14
Storage Bay Dist (ft)				250	
Storage Blk Time (%)				0	
Queuing Penalty (veh)				1	

Intersection: 44: NB US 321 & 7th Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	282	227	108
Average Queue (ft)	112	74	75
95th Queue (ft)	206	162	115
Link Distance (ft)	864	864	48
Upstream Blk Time (%)			37
Queuing Penalty (veh)			42
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 47: SB US 321 & 7th Avenue NW U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	89
Average Queue (ft)	11
95th Queue (ft)	50
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	375
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 48: NB US 321 & 7th Avenue NW Left-Over

Movement	NB
Directions Served	L
Maximum Queue (ft)	69
Average Queue (ft)	6
95th Queue (ft)	37
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	100
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

Movement	WB	WB	WB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	R	L	L
Maximum Queue (ft)	234	246	297	134	142	54	174	175
Average Queue (ft)	158	159	152	110	113	2	88	82
95th Queue (ft)	228	234	247	123	131	18	153	149
Link Distance (ft)		332	332	12	12	12	62	62
Upstream Blk Time (%)				34	36		16	15
Queuing Penalty (veh)				191	199		26	25
Storage Bay Dist (ft)	250							
Storage Blk Time (%)	0	0						
Queuing Penalty (veh)	0	0						

Intersection: 62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW

Movement	SB	SB	SB	NW
Directions Served	T	T	TR	<
Maximum Queue (ft)	462	462	449	141
Average Queue (ft)	384	310	229	108
95th Queue (ft)	495	484	449	122
Link Distance (ft)	410	410	410	45
Upstream Blk Time (%)	12	10	5	62
Queuing Penalty (veh)	0	0	0	167
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

Movement	EB	EB	SB	SB	SB	NW	NW
Directions Served	>	>	T	T	R	<	<L
Maximum Queue (ft)	191	184	248	222	156	131	173
Average Queue (ft)	108	50	182	188	49	81	112
95th Queue (ft)	178	137	240	231	108	117	151
Link Distance (ft)	671	671	86	86	86	43	43
Upstream Blk Time (%)			28	32	2	30	49
Queuing Penalty (veh)			177	199	15	43	71
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 64: NB US 321 & 9th Ave NW & Clement Boulevard

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	273	252	328	352	324	230
Average Queue (ft)	184	99	210	221	175	132
95th Queue (ft)	274	234	309	314	280	203
Link Distance (ft)	258		1191	1191		110
Upstream Blk Time (%)	1	0				16
Queuing Penalty (veh)	0	0				35
Storage Bay Dist (ft)		250			375	
Storage Blk Time (%)	1	0				
Queuing Penalty (veh)	2	0				

Intersection: 65: NB US 321 & Clement Boulevard U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	320
Average Queue (ft)	113
95th Queue (ft)	233
Link Distance (ft)	852
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 66: SB US 321 & Clement Boulevard Left-Over

Movement	SB	SB	SB	SB
Directions Served	L	L	T	T
Maximum Queue (ft)	23	19	201	179
Average Queue (ft)	1	1	46	60
95th Queue (ft)	8	9	147	151
Link Distance (ft)			789	789
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	225	225		
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 67: SB US 321 & Clement Boulevard

Movement	SB
Directions Served	L
Maximum Queue (ft)	132
Average Queue (ft)	13
95th Queue (ft)	66
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	175
Storage Blk Time (%)	0
Queuing Penalty (veh)	0

Intersection: 68: NB US 321 & Clement Boulevard Left-Over

Movement	NB	NB	NB	NB	NB
Directions Served	L	L	T	T	T
Maximum Queue (ft)	24	98	262	275	74
Average Queue (ft)	1	8	96	100	7
95th Queue (ft)	8	49	217	218	35
Link Distance (ft)			768	768	768
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	375	375			
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 441: Old Lenoir Rd & 12 Ave NW

Movement	EB	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	L	T	T	T	TR	L	R
Maximum Queue (ft)	199	190	198	96	136	90	144	104
Average Queue (ft)	95	118	28	30	68	48	60	15
95th Queue (ft)	166	178	96	75	125	81	109	67
Link Distance (ft)			379	379	948	948	396	
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	200	200						300
Storage Blk Time (%)	0	0	0					
Queuing Penalty (veh)	0	0	0					

Network Summary

Network wide Queuing Penalty: 1237

Lanes, Volumes, Timings
 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

NCDOT TIP-U4700



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	66	453	7	60	729	377	34	119	178	169	18	47
Future Volume (vph)	66	453	7	60	729	377	34	119	178	169	18	47
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	175		0	150		125	125		125	0		450
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850			0.850		0.892	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3532	0	1770	3539	1583	1770	1863	1583	1770	1662	0
Flt Permitted	0.346			0.950			0.710			0.657		
Satd. Flow (perm)	645	3532	0	1770	3539	1583	1323	1863	1583	1224	1662	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35				45
Link Distance (ft)		444			444			649				551
Travel Time (s)		8.6			8.6			12.6				8.3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	73	503	8	67	810	419	38	132	198	188	20	52
Shared Lane Traffic (%)												
Lane Group Flow (vph)	73	511	0	67	810	419	38	132	198	188	72	0
Turn Type	Perm	NA		Prot	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2				6
Permitted Phases	4					8	2		2	6		
Detector Phase	4	4		3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		7.0	10.0	10.0	7.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	17.0	17.0		14.0	17.0	17.0	14.0	14.0	14.0	14.0	14.0	
Total Split (s)	38.0	38.0		16.0	54.0	54.0	36.0	36.0	36.0	36.0	36.0	
Total Split (%)	42.2%	42.2%		17.8%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	
Maximum Green (s)	31.0	31.0		9.0	47.0	47.0	29.0	29.0	29.0	29.0	29.0	
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Min	C-Min		None	C-Min	C-Min	None	None	None	None	None	
Act Effct Green (s)	46.0	46.0		11.0	59.2	59.2	20.8	20.8	20.8	20.8	20.8	
Actuated g/C Ratio	0.51	0.51		0.12	0.66	0.66	0.23	0.23	0.23	0.23	0.23	
v/c Ratio	0.22	0.28		0.31	0.35	0.40	0.12	0.31	0.54	0.67	0.19	
Control Delay	7.1	5.3		46.6	4.3	5.2	25.7	28.9	35.0	42.3	26.7	
Queue Delay	0.0	0.0		0.0	0.1	0.2	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.1	5.3		46.6	4.4	5.4	25.7	28.9	35.0	42.3	26.7	
LOS	A	A		D	A	A	C	C	C	D	C	
Approach Delay		5.5			6.9			31.8			38.0	
Approach LOS		A			A			C			D	

Lanes, Volumes, Timings
 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

NCDOT TIP-U4700

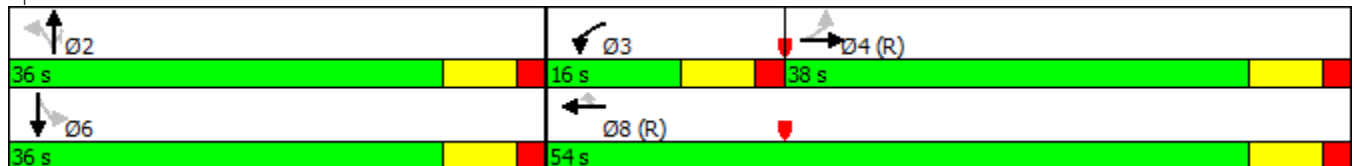


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	9	32		35	36	36	17	62	99	97	33	
Queue Length 95th (ft)	m16	m51		63	60	68	38	100	149	152	61	
Internal Link Dist (ft)		364			364			569			471	
Turn Bay Length (ft)	175			150		125	125		125			
Base Capacity (vph)	330	1809		232	2329	1042	455	641	545	421	572	
Starvation Cap Reductn	0	0		0	514	172	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.28		0.29	0.45	0.48	0.08	0.21	0.36	0.45	0.13	

Intersection Summary

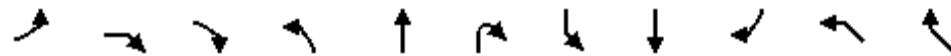
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 4:EBTL and 8:WBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 13.4
 Intersection LOS: B
 Intersection Capacity Utilization 57.0%
 ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd



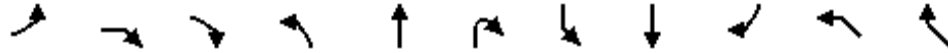
Lanes, Volumes, Timings
 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

NCDOT TIP-U4700



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	569	0	0	0	0	1307	114	75	0
Future Volume (vph)	0	0	569	0	0	0	0	1307	114	75	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	450		0		0	375		250	0	0
Storage Lanes	0	0		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	0	0	0	3471	1583	1770	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	0	0	0	3471	1583	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	518				815			1204		221	
Travel Time (s)	10.1				10.1			14.9		6.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	632	0	0	0	0	1452	127	83	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	632	0	0	0	0	1452	127	83	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			44.0					46.0	46.0	44.0	
Total Split (%)			48.9%					51.1%	51.1%	48.9%	
Maximum Green (s)			37.0					39.0	39.0	37.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			38.1					41.9	41.9	38.1	
Actuated g/C Ratio			0.42					0.47	0.47	0.42	
v/c Ratio			0.93					0.90	0.17	0.11	
Control Delay			48.0					21.8	8.3	15.0	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			48.0					21.8	8.3	15.0	
LOS			D					C	A	B	
Approach Delay	48.0							20.7		15.0	

43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

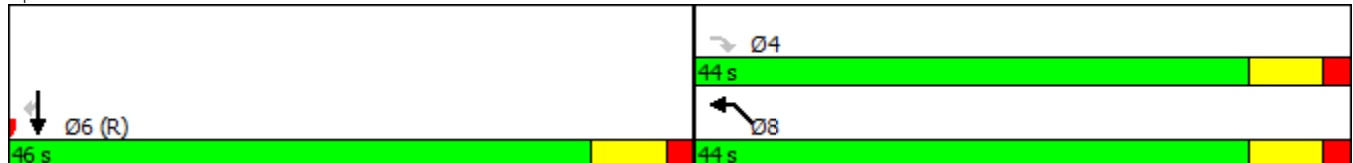


Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Approach LOS	D						C			B	
Queue Length 50th (ft)			327				424		27	31	
Queue Length 95th (ft)			#549				#541		m33	m32	
Internal Link Dist (ft)	438					735		1124		141	
Turn Bay Length (ft)			450							250	
Base Capacity (vph)			691					1614		736	767
Starvation Cap Reductn			0					0		0	0
Spillback Cap Reductn			0					0		0	0
Storage Cap Reductn			0					0		0	0
Reduced v/c Ratio			0.91					0.90		0.17	0.11

Intersection Summary







Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 51 (57%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.93
 Intersection Signal Delay: 28.0 Intersection LOS: C
 Intersection Capacity Utilization 79.7% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW



Lanes, Volumes, Timings
44: NB US 321 & 7th Avenue NW U-Turn

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↘	
Traffic Volume (vph)	0	1818	0	0	367	0
Future Volume (vph)	0	1818	0	0	367	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		55	55		25	
Link Distance (ft)		897	685		100	
Travel Time (s)		11.1	8.5		2.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	5%	2%
Adj. Flow (vph)	0	2020	0	0	408	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2020	0	0	408	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		14.0			7.0	
Minimum Split (s)		21.0			14.0	
Total Split (s)		61.0			29.0	
Total Split (%)		67.8%			32.2%	
Maximum Green (s)		54.0			22.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		56.4			23.6	
Actuated g/C Ratio		0.63			0.26	
v/c Ratio		0.94			0.90	
Control Delay		25.8			45.6	
Queue Delay		0.0			0.0	
Total Delay		25.8			45.6	
LOS		C			D	
Approach Delay		25.8			45.6	
Approach LOS		C			D	
Queue Length 50th (ft)		502			232	
Queue Length 95th (ft)		#735			m265	

Lanes, Volumes, Timings
 44: NB US 321 & 7th Avenue NW U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		817	605		20	
Turn Bay Length (ft)						
Base Capacity (vph)		2153			458	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.94			0.89	

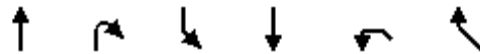
Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 44 (49%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 29.1
 Intersection LOS: C
 Intersection Capacity Utilization 78.9%
 ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 44: NB US 321 & 7th Avenue NW U-Turn



Lanes, Volumes, Timings
 47: SB US 321 & 7th Avenue NW U-Turn












Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	367	1509	0	0
Future Volume (vph)	0	0	367	1509	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	375		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	771			815	100	
Travel Time (s)	9.6			10.1	1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	408	1677	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	408	1677	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	71.3%
Analysis Period (min)	15
	ICU Level of Service C

Lanes, Volumes, Timings
 48: NB US 321 & 7th Avenue NW Left-Over

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		 				
Traffic Volume (vph)	75	2110	0	0	0	0
Future Volume (vph)	75	2110	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1770	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1770	3438	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		685	1246		221	
Travel Time (s)		8.5	15.4		2.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	83	2344	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	83	2344	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	140.6% ICU Level of Service H
Analysis Period (min)	15

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	466	0	1607	272	0	0	0	660	0
Future Volume (vph)	0	0	466	0	1607	272	0	0	0	660	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	0		0		0	0		0	0	0
Storage Lanes	1	3		0		1	0		0	2	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.76	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	3438	1583	0	0	0	3433	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	3438	1583	0	0	0	3433	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	444				116			962		235	
Travel Time (s)	8.6				1.4			11.9		6.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	2%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	518	0	1786	302	0	0	0	733	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	518	0	1786	302	0	0	0	733	0
Turn Type			Prot		NA	Free				Prot	
Protected Phases			8		2					8	
Permitted Phases			8			Free				8	
Detector Phase			8		2					8	
Switch Phase											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			29.0		61.0					29.0	
Total Split (%)			32.2%		67.8%					32.2%	
Maximum Green (s)			22.0		54.0					22.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effect Green (s)			23.8		56.2	90.0				23.8	
Actuated g/C Ratio			0.26		0.62	1.00				0.26	
v/c Ratio			0.54		0.83	0.19				0.81	
Control Delay			21.7		6.1	0.1				35.0	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			21.7		6.1	0.1				35.0	
LOS			C		A	A				C	
Approach Delay	21.7				5.3					35.0	

Lanes, Volumes, Timings
 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Approach LOS	C				A				C		
Queue Length 50th (ft)			114		109	0				203	
Queue Length 95th (ft)			103		126	m0				246	
Internal Link Dist (ft)	364				36			882		155	
Turn Bay Length (ft)											
Base Capacity (vph)			974		2157	1583				926	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.53		0.83	0.19				0.79	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 78 (87%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 14.3
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lanes, Volumes, Timings

NCDOT TIP-U4700

62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations												
Traffic Volume (vph)	0	0	31	0	0	0	0	1645	46	143	0	0
Future Volume (vph)	0	0	31	0	0	0	0	1645	46	143	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00	1.00
Fr _t			0.865					0.996				
Flt Protected										0.950		
Satd. Flow (prot)	0	0	1565	0	0	0	0	4920	0	1719	0	0
Flt Permitted										0.950		
Satd. Flow (perm)	0	0	1565	0	0	0	0	4920	0	1719	0	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55			25	
Link Distance (ft)	391				835			456			117	
Travel Time (s)	7.6				10.4			5.7			3.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	2%	2%	2%	2%	5%	5%	5%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	100	0	0	0
Adj. Flow (vph)	0	0	34	0	0	0	0	1828	51	159	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	34	0	0	0	0	1879	0	159	0	0
Turn Type			Free					NA		Prot		
Protected Phases								6		8		
Permitted Phases			Free									
Detector Phase								6		8		
Switch Phase												
Minimum Initial (s)								14.0		7.0		
Minimum Split (s)								21.0		14.0		
Total Split (s)								63.0		27.0		
Total Split (%)								70.0%		30.0%		
Maximum Green (s)								56.0		20.0		
Yellow Time (s)								5.0		5.0		
All-Red Time (s)								2.0		2.0		
Lost Time Adjust (s)								-2.0		-2.0		
Total Lost Time (s)								5.0		5.0		
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)								3.0		3.0		
Recall Mode								C-Min		None		
Act Effct Green (s)			90.0					64.4		15.6		
Actuated g/C Ratio			1.00					0.72		0.17		
v/c Ratio			0.02					0.53		0.54		
Control Delay			0.0					7.0		42.2		
Queue Delay			0.0					0.0		0.0		
Total Delay			0.0					7.0		42.2		
LOS			A					A		D		
Approach Delay								7.0			42.2	
Approach LOS								A			D	
Queue Length 50th (ft)			0					151		94		

62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Queue Length 95th (ft)			0					230		m105		
Internal Link Dist (ft)	311				755			376			37	
Turn Bay Length (ft)												
Base Capacity (vph)			1565					3522		420		
Starvation Cap Reductn			0					0		0		
Spillback Cap Reductn			0					0		0		
Storage Cap Reductn			0					0		0		
Reduced v/c Ratio			0.02					0.53		0.38		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32 (36%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 40
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.54
 Intersection Signal Delay: 9.6 Intersection LOS: A
 Intersection Capacity Utilization 48.2% ICU Level of Service A
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW



Lanes, Volumes, Timings

NCDOT TIP-U4700

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	↑	↑
Traffic Volume (vph)	0	0	246	0	0	0	0	1491	168	138	61	0
Future Volume (vph)	0	0	246	0	0	0	0	1491	168	138	61	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2787	0	0	0	0	3438	1583	1681	1681	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2787	0	0	0	0	3438	1583	1681	1681	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55				25
Link Distance (ft)	749				781			222				128
Travel Time (s)	14.6				9.7			2.8				3.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	0	0	273	0	0	0	0	1657	187	153	68	0
Shared Lane Traffic (%)										28%		
Lane Group Flow (vph)	0	0	273	0	0	0	0	1657	187	110	111	0
Turn Type			Prot					NA	Perm	Perm	Prot	
Protected Phases			7					6			8	
Permitted Phases									6	8		
Detector Phase			7					6	6	8	8	
Switch Phase												
Minimum Initial (s)			7.0					14.0	14.0	7.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	14.0	
Total Split (s)			18.0					57.0	57.0	15.0	15.0	
Total Split (%)			20.0%					63.3%	63.3%	16.7%	16.7%	
Maximum Green (s)			11.0					50.0	50.0	8.0	8.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	5.0	
Lead/Lag			Lag							Lead	Lead	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0					3.0	3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	None	
Act Effct Green (s)			12.8					52.3	52.3	9.9	9.9	
Actuated g/C Ratio			0.14					0.58	0.58	0.11	0.11	
v/c Ratio			0.69					0.83	0.20	0.59	0.60	
Control Delay			46.7					16.1	7.9	54.0	54.2	
Queue Delay			0.0					0.0	0.0	0.0	0.0	
Total Delay			46.7					16.1	7.9	54.0	54.2	
LOS			D					B	A	D	D	
Approach Delay	46.7							15.3			54.1	
Approach LOS	D							B			D	
Queue Length 50th (ft)			84					394	30	68	69	
Queue Length 95th (ft)			131					274	65	m80	m80	

Lanes, Volumes, Timings
 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

NCDOT TIP-U4700

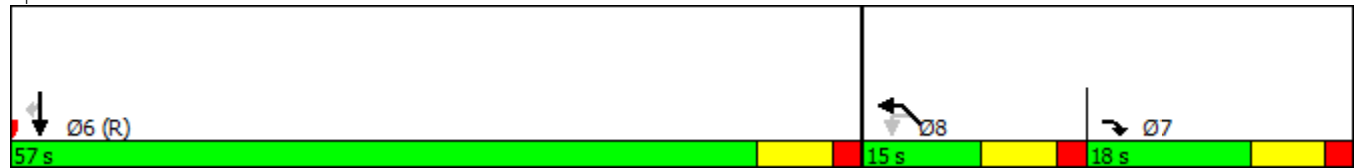


Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Internal Link Dist (ft)	669				701			142			48	
Turn Bay Length (ft)												
Base Capacity (vph)			402					2003	922	188	188	
Starvation Cap Reductn			0					0	0	0	0	
Spillback Cap Reductn			0					0	0	0	0	
Storage Cap Reductn			0					0	0	0	0	
Reduced v/c Ratio			0.68					0.83	0.20	0.59	0.59	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 16 (18%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 22.6
 Intersection LOS: C
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lanes, Volumes, Timings
64: NB US 321 & 9th Ave NW & Clement Boulevard

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↔↔		↑↑↑							↔
Traffic Volume (vph)	0	0	190	0	1789	249	0	0	0	207	60	0
Future Volume (vph)	0	0	190	0	1789	249	0	0	0	207	60	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		375	0		0		0	0
Storage Lanes	0	1		0		1	0		0		1	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.982							
Flt Protected												0.950
Satd. Flow (prot)	0	0	2733	0	4868	0	0	0	0	0	1730	0
Flt Permitted												0.950
Satd. Flow (perm)	0	0	2733	0	4868	0	0	0	0	0	1730	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	25				55			55				25
Link Distance (ft)	322				1246			837				179
Travel Time (s)	8.8				15.4			10.4				4.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	2%	5%	2%	2%	2%	2%	5%	2%	2%
Adj. Flow (vph)	0	0	211	0	1988	277	0	0	0	230	67	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	211	0	2265	0	0	0	0	0	297	0
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			3		2					4	4	
Permitted Phases												
Detector Phase			3		2					4	4	
Switch Phase												
Minimum Initial (s)			7.0		14.0					7.0	7.0	
Minimum Split (s)			14.0		21.0					14.0	14.0	
Total Split (s)			14.0		52.0					24.0	24.0	
Total Split (%)			15.6%		57.8%					26.7%	26.7%	
Maximum Green (s)			7.0		45.0					17.0	17.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Lost Time Adjust (s)			-2.0		-2.0							-2.0
Total Lost Time (s)			5.0		5.0							5.0
Lead/Lag			Lead							Lag	Lag	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effct Green (s)			9.0		47.5							18.5
Actuated g/C Ratio			0.10		0.53							0.21
v/c Ratio			0.77		0.88							0.84
Control Delay			59.7		13.8							41.0
Queue Delay			0.0		0.0							0.0
Total Delay			59.7		13.8							41.0
LOS			E		B							D
Approach Delay	59.7				13.8							41.0

Lanes, Volumes, Timings
 64: NB US 321 & 9th Ave NW & Clement Boulevard

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Approach LOS	E				B				D			
Queue Length 50th (ft)			67			319					151	
Queue Length 95th (ft)			#128			m355					m#214	
Internal Link Dist (ft)	242					1166			757			99
Turn Bay Length (ft)			250									
Base Capacity (vph)			273			2570					365	
Starvation Cap Reductn			0			0					0	
Spillback Cap Reductn			0			0					0	
Storage Cap Reductn			0			0					0	
Reduced v/c Ratio			0.77			0.88					0.81	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 76 (84%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.88
 Intersection Signal Delay: 20.2
 Intersection LOS: C
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 64: NB US 321 & 9th Ave NW & Clement Boulevard



Lanes, Volumes, Timings
 65: NB US 321 & Clement Boulevard U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	143	2525	0	0
Future Volume (vph)	0	0	143	2525	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	117			962	471	
Travel Time (s)	3.2			11.9	5.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	4%	4%
Adj. Flow (vph)	0	0	159	2806	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	159	2806	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	73.1%
Analysis Period (min)	15
	ICU Level of Service D

Lanes, Volumes, Timings
 66: SB US 321 & Clement Boulevard Left-Over

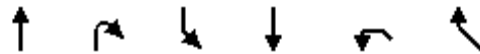
NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↖↙	↑↑↑		
Traffic Volume (vph)	0	0	660	1659	0	0
Future Volume (vph)	0	0	660	1659	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	225		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3433	4940	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3433	4940	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	222			835	235	
Travel Time (s)	2.8			10.4	2.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	5%	2%	2%
Adj. Flow (vph)	0	0	733	1843	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	733	1843	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	35.4% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
 67: SB US 321 & Clement Boulevard

NCDOT TIP-U4700














Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	267	1483	0	0
Future Volume (vph)	0	0	267	1483	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	175		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1204			781	179	
Travel Time (s)	14.9			9.7	2.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	297	1648	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	297	1648	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	44.3%
Analysis Period (min)	15
	ICU Level of Service A

Lanes, Volumes, Timings
 68: NB US 321 & Clement Boulevard Left-Over

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations	 	  				
Traffic Volume (vph)	199	1879	0	0	0	0
Future Volume (vph)	199	1879	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	375			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3433	4940	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3433	4940	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		837	116		128	
Travel Time (s)		10.4	1.4		1.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Adj. Flow (vph)	221	2088	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	221	2088	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	39.6% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
441: Old Lenoir Rd & 12 Ave NW

NCDOT TIP-U4700



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↶↷	↶↷	↶↷		↶	↷
Traffic Volume (vph)	535	269	650	115	45	509
Future Volume (vph)	535	269	650	115	45	509
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	0	300
Storage Lanes	2			0	1	1
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.95	0.95	0.95	1.00	1.00
Frt			0.977			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	3433	3539	3458	0	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3433	3539	3458	0	1770	1583
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		35	
Link Distance (ft)		444	1015		470	
Travel Time (s)		6.7	15.4		9.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	594	299	722	128	50	566
Shared Lane Traffic (%)						
Lane Group Flow (vph)	594	299	850	0	50	566
Turn Type	Prot	NA	NA		Prot	Free
Protected Phases	7	4	8		6	
Permitted Phases						Free
Detector Phase	7	4	8		6	
Switch Phase						
Minimum Initial (s)	7.0	12.0	12.0		7.0	
Minimum Split (s)	14.0	19.0	19.0		14.0	
Total Split (s)	32.0	75.0	43.0		15.0	
Total Split (%)	35.6%	83.3%	47.8%		16.7%	
Maximum Green (s)	25.0	68.0	36.0		8.0	
Yellow Time (s)	5.0	5.0	5.0		5.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0		-2.0	
Total Lost Time (s)	5.0	5.0	5.0		5.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Vehicle Extension (s)	3.0	3.0	3.0		3.0	
Recall Mode	None	C-Min	C-Min		None	
Act Effct Green (s)	22.5	77.7	48.2		9.9	90.0
Actuated g/C Ratio	0.25	0.86	0.54		0.11	1.00
v/c Ratio	0.69	0.10	0.46		0.26	0.36
Control Delay	24.7	2.8	16.1		40.1	0.6
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	24.7	2.8	16.1		40.1	0.6
LOS	C	A	B		D	A
Approach Delay		17.4	16.1		3.8	
Approach LOS		B	B		A	

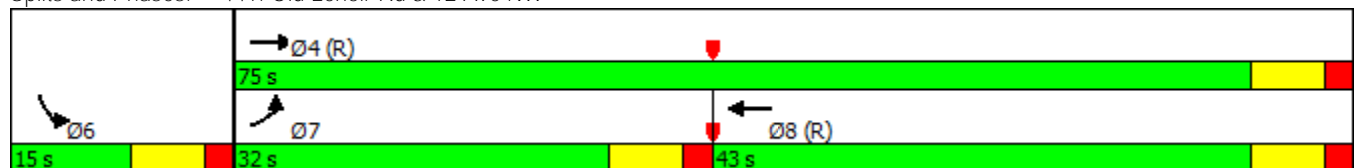
Lanes, Volumes, Timings
 441: Old Lenoir Rd & 12 Ave NW



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Queue Length 50th (ft)	105	17	171		27	0
Queue Length 95th (ft)	157	44	251		61	0
Internal Link Dist (ft)		364	935		390	
Turn Bay Length (ft)	200					300
Base Capacity (vph)	1029	3068	1855		202	1583
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.58	0.10	0.46		0.25	0.36

Intersection Summary	
Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	31 (34%), Referenced to phase 4:EBT and 8:WBT, Start of Green
Natural Cycle:	55
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.69
Intersection Signal Delay:	13.4
Intersection LOS:	B
Intersection Capacity Utilization	55.2%
ICU Level of Service	B
Analysis Period (min)	15

Splits and Phases: 441: Old Lenoir Rd & 12 Ave NW



Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	7491
Vehs Exited	7461
Starting Vehs	226
Ending Vehs	256
Travel Distance (mi)	4664
Travel Time (hr)	248.2
Total Delay (hr)	137.0
Total Stops	9126
Fuel Used (gal)	227.4

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	7491
Vehs Exited	7461
Starting Vehs	226
Ending Vehs	256
Travel Distance (mi)	4664
Travel Time (hr)	248.2
Total Delay (hr)	137.0
Total Stops	9126
Fuel Used (gal)	227.4

Intersection: 25: 12th St Dr NW & Clement Boulevard & Old Lenoir Rd

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	L	TR
Maximum Queue (ft)	74	235	118	102	160	180	151	88	115	156	65
Average Queue (ft)	39	90	29	27	68	68	58	30	57	98	27
95th Queue (ft)	74	197	84	66	135	136	126	73	101	154	54
Link Distance (ft)		332	332		377	377			584	490	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	175			150			125	125			450
Storage Blk Time (%)		1			0	1	1		0		
Queuing Penalty (veh)		1			0	5	3		0		

Intersection: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

Movement	EB	SB	SB	SB	NW
Directions Served	>	T	T	R	L
Maximum Queue (ft)	469	280	250	111	91
Average Queue (ft)	290	157	140	47	31
95th Queue (ft)	443	249	221	97	71
Link Distance (ft)	454	1132	1132		164
Upstream Blk Time (%)	1				
Queuing Penalty (veh)	0				
Storage Bay Dist (ft)				250	
Storage Blk Time (%)			0		
Queuing Penalty (veh)			0		

Intersection: 44: NB US 321 & 7th Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	436	422	142
Average Queue (ft)	244	241	109
95th Queue (ft)	364	381	124
Link Distance (ft)	864	864	48
Upstream Blk Time (%)			63
Queuing Penalty (veh)			230
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 47: SB US 321 & 7th Avenue NW U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	370
Average Queue (ft)	159
95th Queue (ft)	282
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	375
Storage Blk Time (%)	0
Queuing Penalty (veh)	2

Intersection: 48: NB US 321 & 7th Avenue NW Left-Over

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

Movement	WB	WB	WB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	R	L	L
Maximum Queue (ft)	140	181	200	147	136	114	192	194
Average Queue (ft)	59	93	90	106	109	25	165	170
95th Queue (ft)	132	154	167	137	135	91	194	190
Link Distance (ft)		332	332	12	12	12	62	62
Upstream Blk Time (%)				31	35		60	58
Queuing Penalty (veh)				194	218		198	191
Storage Bay Dist (ft)	250							
Storage Blk Time (%)								
Queuing Penalty (veh)								

Intersection: 62: SB US 321 & Clement Boulevard U-Turn & 13th Avenue Drive NW

Movement	SB	SB	SB	NW
Directions Served	T	T	TR	<
Maximum Queue (ft)	422	287	55	134
Average Queue (ft)	213	120	26	94
95th Queue (ft)	368	258	56	150
Link Distance (ft)	446	446	446	59
Upstream Blk Time (%)				34
Queuing Penalty (veh)				49
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

Movement	EB	EB	SB	SB	SB	NW	NW
Directions Served	>	>	T	T	R	<	<L
Maximum Queue (ft)	169	134	224	199	116	114	137
Average Queue (ft)	109	50	188	180	42	60	85
95th Queue (ft)	171	125	237	233	89	100	127
Link Distance (ft)	671	671	86	86	86	43	43
Upstream Blk Time (%)			28	25	1	24	51
Queuing Penalty (veh)			153	136	4	24	51
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 64: NB US 321 & 9th Ave NW & Clement Boulevard

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	287	258	284	298	268	184
Average Queue (ft)	144	64	190	214	169	143
95th Queue (ft)	248	201	275	295	249	209
Link Distance (ft)	258		1191	1191		110
Upstream Blk Time (%)	3	0				29
Queuing Penalty (veh)	0	0				76
Storage Bay Dist (ft)		250			375	
Storage Blk Time (%)	3	0				
Queuing Penalty (veh)	3	0				

Intersection: 65: NB US 321 & Clement Boulevard U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	84
Average Queue (ft)	12
95th Queue (ft)	49
Link Distance (ft)	815
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 66: SB US 321 & Clement Boulevard Left-Over

Movement	SB	SB	SB	SB
Directions Served	L	L	T	T
Maximum Queue (ft)	214	220	215	210
Average Queue (ft)	63	58	48	50
95th Queue (ft)	166	157	139	146
Link Distance (ft)			790	790
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	225	225		
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 67: SB US 321 & Clement Boulevard

Movement	SB
Directions Served	L
Maximum Queue (ft)	157
Average Queue (ft)	25
95th Queue (ft)	95
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	175
Storage Blk Time (%)	1
Queuing Penalty (veh)	5

Intersection: 68: NB US 321 & Clement Boulevard Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	40	351	368	160
Average Queue (ft)	3	115	127	51
95th Queue (ft)	19	283	297	126
Link Distance (ft)		768	768	768
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	375			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 441: Old Lenoir Rd & 12 Ave NW

Movement	EB	EB	EB	EB	WB	WB	SB	SB
Directions Served	L	L	T	T	T	TR	L	R
Maximum Queue (ft)	249	299	384	92	206	247	163	258
Average Queue (ft)	130	162	26	19	111	125	30	66
95th Queue (ft)	233	254	177	57	179	221	88	186
Link Distance (ft)			377	377	948	948	396	
Upstream Blk Time (%)			0					
Queuing Penalty (veh)			2					
Storage Bay Dist (ft)	200	200						300
Storage Blk Time (%)	1	5						
Queuing Penalty (veh)	1	6						

Network Summary

Network wide Queuing Penalty: 1554

APPENDIX J

**2040 Build Alternative Grace Chapel Road – Superstreet Synchro &
SimTraffic Reports**

Lanes, Volumes, Timings
5: NB US 321

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↙	↑↑↑		
Traffic Volume (vph)	0	0	21	3112	0	0
Future Volume (vph)	0	0	21	3112	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	75		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1770	5085	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1770	5085	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1257			483	113	
Travel Time (s)	15.6			6.0	1.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	23	3458	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	23	3458	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	72.7%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road & NB US 321

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑		↑↑↑	↑				↑	
Traffic Volume (vph)	0	0	454	0	1829	179	0	0	0	21	0
Future Volume (vph)	0	0	454	0	1829	179	0	0	0	21	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	300		300		400	0		0	0	0
Storage Lanes	0	1		0		1	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	4988	1568	0	0	0	1770	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	4988	1568	0	0	0	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	982				1247			484		113	
Travel Time (s)	19.1				15.5			6.0		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	3%	2%	4%	3%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	504	0	2032	199	0	0	0	23	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	504	0	2032	199	0	0	0	23	0
Turn Type			Prot		NA	Free				Prot	
Protected Phases			8		2					4	
Permitted Phases						Free					
Detector Phase			8		2					4	
Switch Phase											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			32.0		58.0					32.0	
Total Split (%)			35.6%		64.4%					35.6%	
Maximum Green (s)			25.0		51.0					25.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effect Green (s)			23.1		56.9	90.0				23.1	
Actuated g/C Ratio			0.26		0.63	1.00				0.26	
v/c Ratio			0.71		0.64	0.13				0.05	
Control Delay			36.2		12.0	0.2				26.0	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			36.2		12.0	0.2				26.0	
LOS			D		B	A				C	
Approach Delay	36.2				10.9					26.0	

91: US 321 & Grace Chapel Road & NB US 321



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Approach LOS	D				B				C			
Queue Length 50th (ft)			147			239	0					10
Queue Length 95th (ft)			196			324	0					m10
Internal Link Dist (ft)	902				1167				404		33	
Turn Bay Length (ft)				300			400					
Base Capacity (vph)				828			3155			1568		531
Starvation Cap Reductn				0			0			0		0
Spillback Cap Reductn				0			0			0		0
Storage Cap Reductn				0			0			0		0
Reduced v/c Ratio				0.61			0.64		0.13		0.04	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT, Start of Green, Master Intersection
 Natural Cycle: 45
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.71
 Intersection Signal Delay: 15.7 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 91: US 321 & Grace Chapel Road & NB US 321



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑	↖↗	
Traffic Volume (vph)	0	0	0	2713	420	0
Future Volume (vph)	0	0	0	2713	420	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.97	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	4988	3367	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	4988	3367	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	55			55	25	
Link Distance (ft)	483			1103	111	
Travel Time (s)	6.0			13.7	3.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	3014	467	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3014	467	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				68.0	22.0	
Total Split (%)				75.6%	24.4%	
Maximum Green (s)				61.0	15.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Lost Time Adjust (s)				-2.0	-2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				63.4	16.6	
Actuated g/C Ratio				0.70	0.18	
v/c Ratio				0.86	0.75	
Control Delay				13.4	36.1	
Queue Delay				0.0	0.0	
Total Delay				13.4	36.1	
LOS				B	D	
Approach Delay				13.4	36.1	
Approach LOS				B	D	
Queue Length 50th (ft)				407	134	
Queue Length 95th (ft)				489	166	

Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Internal Link Dist (ft)	403			1023	31	
Turn Bay Length (ft)						
Base Capacity (vph)				3512	635	
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.86	0.74	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	47 (52%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.86
Intersection Signal Delay:	16.5
Intersection LOS:	B
Intersection Capacity Utilization	95.9%
ICU Level of Service	F
Analysis Period (min)	15

Splits and Phases: 92: Grace Chapel Road U-Turn & SB US 321



Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑			↙	
Traffic Volume (vph)	0	2008	0	0	4	0
Future Volume (vph)	0	2008	0	0	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	4988	0	0	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	4988	0	0	1736	0
Link Speed (mph)		55	55		25	
Link Distance (ft)		578	1247		112	
Travel Time (s)		7.2	15.5		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	4%	2%
Adj. Flow (vph)	0	2231	0	0	4	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2231	0	0	4	0
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.5%
ICU Level of Service	B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
 93: NB US 321 & Grace Chapel Road U-Turn












NCDOT TIP-U4700



Movement	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑			↵	
Traffic Volume (veh/h)	0	2008	0	0	4	0
Future Volume (Veh/h)	0	2008	0	0	4	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	2231	0	0	4	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)			1247			
pX, platoon unblocked						
vC, conflicting volume	0				744	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0				744	0
tC, single (s)	4.1				6.9	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1622				346	1084
Direction, Lane #	NB 1	NB 2	NB 3	SE 1		
Volume Total	744	744	744	4		
Volume Left	0	0	0	4		
Volume Right	0	0	0	0		
cSH	1700	1700	1700	346		
Volume to Capacity	0.44	0.44	0.44	0.01		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	0.0	0.0	15.5		
Lane LOS				C		
Approach Delay (s)	0.0			15.5		
Approach LOS				C		
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			63.5%		ICU Level of Service	B
Analysis Period (min)			15			

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations	 	  				
Traffic Volume (vph)	420	1863	0	0	0	0
Future Volume (vph)	420	1863	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	125			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3367	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3367	4988	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		484	1105		111	
Travel Time (s)		6.0	13.7		2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	467	2070	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	467	2070	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	95.9% ICU Level of Service F
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↙	↑↑↑		
Traffic Volume (vph)	0	0	4	3112	0	0
Future Volume (vph)	0	0	4	3112	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	125		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	683			1257	112	
Travel Time (s)	15.5			15.6	2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	4	3458	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	4	3458	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.5% ICU Level of Service B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	2008	0	0	0	3112
Future Volume (vph)	0	2008	0	0	0	3112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		1796	578		683	
Travel Time (s)		22.3	7.2		8.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	2231	0	0	0	3458
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2231	0	0	0	3458
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.9%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5361
Vehs Exited	5359
Starting Vehs	144
Ending Vehs	146
Travel Distance (mi)	5203
Travel Time (hr)	141.8
Total Delay (hr)	41.9
Total Stops	2227
Fuel Used (gal)	186.2

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5361
Vehs Exited	5359
Starting Vehs	144
Ending Vehs	146
Travel Distance (mi)	5203
Travel Time (hr)	141.8
Total Delay (hr)	41.9
Total Stops	2227
Fuel Used (gal)	186.2

Intersection: 5: NB US 321

Movement	SB
Directions Served	T
Maximum Queue (ft)	84
Average Queue (ft)	3
95th Queue (ft)	28
Link Distance (ft)	413
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 91: US 321 & Grace Chapel Road & NB US 321

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	T	L
Maximum Queue (ft)	478	312	226	238	243	50
Average Queue (ft)	197	139	137	158	142	15
95th Queue (ft)	314	254	210	229	220	44
Link Distance (ft)	919		1187	1187	1187	38
Upstream Blk Time (%)						3
Queuing Penalty (veh)						1
Storage Bay Dist (ft)		300				
Storage Blk Time (%)	1	0				
Queuing Penalty (veh)	2	0				

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	SB	SB	SB	NW	NW
Directions Served	T	T	T	L	L
Maximum Queue (ft)	281	288	264	115	138
Average Queue (ft)	201	178	124	85	112
95th Queue (ft)	276	265	224	121	134
Link Distance (ft)	1072	1072	1072	28	28
Upstream Blk Time (%)				62	74
Queuing Penalty (veh)				130	155
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	SE
Directions Served	L
Maximum Queue (ft)	28
Average Queue (ft)	1
95th Queue (ft)	9
Link Distance (ft)	46
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement	NB	NB
Directions Served	L	L
Maximum Queue (ft)	83	116
Average Queue (ft)	12	28
95th Queue (ft)	50	83
Link Distance (ft)		
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	125	125
Storage Blk Time (%)		0
Queuing Penalty (veh)		0

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 996: US 321/NB US 321 & SB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 288

Lanes, Volumes, Timings
5: NB US 321

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↙	↑↑↑		
Traffic Volume (vph)	0	0	29	2014	0	0
Future Volume (vph)	0	0	29	2014	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	75		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1770	5085	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1770	5085	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1257			483	113	
Travel Time (s)	15.6			6.0	1.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	32	2238	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	32	2238	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	50.1%
Analysis Period (min)	15
	ICU Level of Service A

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road & NB US 321

NCDOT TIP-U4700



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↑↑↑	↖				↘	
Traffic Volume (vph)	0	0	200	0	2690	424	0	0	0	29	0
Future Volume (vph)	0	0	200	0	2690	424	0	0	0	29	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	300		300		400	0		0	0	0
Storage Lanes	0	1		0		1	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	4988	1568	0	0	0	1770	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	4988	1568	0	0	0	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	982				1247			484		113	
Travel Time (s)	19.1				15.5			6.0		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	3%	2%	4%	3%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	222	0	2989	471	0	0	0	32	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	222	0	2989	471	0	0	0	32	0
Turn Type			Prot		NA	Free				Prot	
Protected Phases			8		2					4	
Permitted Phases						Free					
Detector Phase			8		2					4	
Switch Phase											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			18.0		72.0					18.0	
Total Split (%)			20.0%		80.0%					20.0%	
Maximum Green (s)			11.0		65.0					11.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effct Green (s)			12.5		67.5	90.0				12.5	
Actuated g/C Ratio			0.14		0.75	1.00				0.14	
v/c Ratio			0.58		0.80	0.30				0.13	
Control Delay			42.8		9.3	0.5				33.5	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			42.8		9.3	0.5				33.5	
LOS			D		A	A				C	
Approach Delay	42.8				8.1					33.5	

91: US 321 & Grace Chapel Road & NB US 321



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Approach LOS	D				A				C			
Queue Length 50th (ft)			67		322	0				16		
Queue Length 95th (ft)			108		388	0				m26		
Internal Link Dist (ft)	902				1167			404		33		
Turn Bay Length (ft)			300			400						
Base Capacity (vph)			398		3741	1568				255		
Starvation Cap Reductn			0		0	0				0		
Spillback Cap Reductn			0		0	0				0		
Storage Cap Reductn			0		0	0				0		
Reduced v/c Ratio			0.56		0.80	0.30				0.13		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT, Start of Green, Master Intersection
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 10.4
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 91: US 321 & Grace Chapel Road & NB US 321



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑	↖↗	
Traffic Volume (vph)	0	0	0	1861	182	0
Future Volume (vph)	0	0	0	1861	182	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.97	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	4988	3367	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	4988	3367	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	55			55	25	
Link Distance (ft)	483			1103	111	
Travel Time (s)	6.0			13.7	3.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	2068	202	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2068	202	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				71.0	19.0	
Total Split (%)				78.9%	21.1%	
Maximum Green (s)				64.0	12.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Lost Time Adjust (s)				-2.0	-2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				67.3	12.7	
Actuated g/C Ratio				0.75	0.14	
v/c Ratio				0.55	0.43	
Control Delay				5.8	33.2	
Queue Delay				0.0	0.0	
Total Delay				5.8	33.2	
LOS				A	C	
Approach Delay				5.8	33.2	
Approach LOS				A	C	
Queue Length 50th (ft)				150	48	
Queue Length 95th (ft)				210	m62	

Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Internal Link Dist (ft)	403			1023	31	
Turn Bay Length (ft)						
Base Capacity (vph)				3748	535	
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.55	0.38	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	66 (73%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	40
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.55
Intersection Signal Delay:	8.2
Intersection LOS:	A
Intersection Capacity Utilization	95.8%
ICU Level of Service	F
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 92: Grace Chapel Road U-Turn & SB US 321



Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑			↙	
Traffic Volume (vph)	0	3114	0	0	4	0
Future Volume (vph)	0	3114	0	0	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	4988	0	0	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	4988	0	0	1736	0
Link Speed (mph)		55	55		25	
Link Distance (ft)		578	1247		112	
Travel Time (s)		7.2	15.5		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	4%	2%
Adj. Flow (vph)	0	3460	0	0	4	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3460	0	0	4	0
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	70.2%
Analysis Period (min)	15
	ICU Level of Service C

Intersection

Int Delay, s/veh 0

Movement	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑			↑	
Traffic Vol, veh/h	0	3114	0	0	4	0
Future Vol, veh/h	0	3114	0	0	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	-	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	4	2	2	4	2
Mvmt Flow	0	3460	0	0	4	0

Major/Minor

	Major1	Minor2
Conflicting Flow All	- 0	1384 -
Stage 1	- -	0 -
Stage 2	- -	1384 -
Critical Hdwy	- -	5.78 -
Critical Hdwy Stg 1	- -	- -
Critical Hdwy Stg 2	- -	6.08 -
Follow-up Hdwy	- -	3.84 -
Pot Cap-1 Maneuver	0 -	194 0
Stage 1	0 -	- 0
Stage 2	0 -	173 0
Platoon blocked, %	- -	- -
Mov Cap-1 Maneuver	- -	194 -
Mov Cap-2 Maneuver	- -	194 -
Stage 1	- -	- -
Stage 2	- -	173 -

Approach












	NB	SE
HCM Control Delay, s	0	24
HCM LOS		C

Minor Lane/Major Mvmt

	NBT SELn1
Capacity (veh/h)	- 194
HCM Lane V/C Ratio	- 0.023
HCM Control Delay (s)	- 24
HCM Lane LOS	- C
HCM 95th %tile Q(veh)	- 0.1

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700

						
Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations	 	  				
Traffic Volume (vph)	182	2708	0	0	0	0
Future Volume (vph)	182	2708	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	125			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	100				100	
Lane Util. Factor	0.97	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3367	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3367	4988	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		484	1105		111	
Travel Time (s)		6.0	13.7		2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	202	3009	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	202	3009	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	95.8% ICU Level of Service F
Analysis Period (min)	15

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↙	↑↑↑		
Traffic Volume (vph)	0	0	4	2014	0	0
Future Volume (vph)	0	0	4	2014	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	125		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	683			1257	112	
Travel Time (s)	15.5			15.6	2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	4	2238	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	4	2238	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	70.2% ICU Level of Service C
Analysis Period (min)	15

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	3114	0	0	0	2014
Future Volume (vph)	0	3114	0	0	0	2014
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		1796	578		683	
Travel Time (s)		22.3	7.2		8.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	3460	0	0	0	2238
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3460	0	0	0	2238
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.5%
ICU Level of Service	B
Analysis Period (min)	15

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5182
Vehs Exited	5179
Starting Vehs	108
Ending Vehs	111
Travel Distance (mi)	5024
Travel Time (hr)	122.6
Total Delay (hr)	27.3
Total Stops	1202
Fuel Used (gal)	170.4

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5182
Vehs Exited	5179
Starting Vehs	108
Ending Vehs	111
Travel Distance (mi)	5024
Travel Time (hr)	122.6
Total Delay (hr)	27.3
Total Stops	1202
Fuel Used (gal)	170.4

Intersection: 5: NB US 321

Movement	SB
Directions Served	L
Maximum Queue (ft)	31
Average Queue (ft)	3
95th Queue (ft)	15
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	75
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 91: US 321 & Grace Chapel Road & NB US 321

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	T	L
Maximum Queue (ft)	226	174	213	189	196	53
Average Queue (ft)	107	51	105	134	127	21
95th Queue (ft)	176	131	178	190	184	51
Link Distance (ft)	919		1187	1187	1187	38
Upstream Blk Time (%)						4
Queuing Penalty (veh)						1
Storage Bay Dist (ft)		300				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	SB	SB	SB	NW	NW
Directions Served	T	T	T	L	L
Maximum Queue (ft)	140	160	116	92	133
Average Queue (ft)	93	64	35	46	75
95th Queue (ft)	144	130	73	79	119
Link Distance (ft)	1072	1072	1072	28	28
Upstream Blk Time (%)				40	68
Queuing Penalty (veh)				36	62
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	SE
Directions Served	L
Maximum Queue (ft)	29
Average Queue (ft)	3
95th Queue (ft)	16
Link Distance (ft)	46
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	34
Average Queue (ft)	1
95th Queue (ft)	11
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	125
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 996: US 321/NB US 321 & SB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

















Network wide Queuing Penalty: 99

APPENDIX K

**2040 Build Alternative Grace Chapel Road – Reverse Superstreet
Synchro & SimTraffic Reports**

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road

NCDOT TIP-U4700

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	 		  			  
Traffic Volume (vph)	420	34	1829	200	0	2713
Future Volume (vph)	420	34	1829	200	0	2713
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300	0		400	0	
Storage Lanes	2	1		1	0	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	1.00	0.91	1.00	1.00	0.91
Frt		0.850		0.850		
Flt Protected	0.950					
Satd. Flow (prot)	3400	1568	4988	1568	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1568	4988	1568	0	4988
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		55			55
Link Distance (ft)	982		651			221
Travel Time (s)	19.1		8.1			2.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	4%	3%	4%	4%
Adj. Flow (vph)	467	38	2032	222	0	3014
Shared Lane Traffic (%)						
Lane Group Flow (vph)	467	38	2032	222	0	3014
Turn Type	Prot	Prot	NA	Free		NA
Protected Phases	8	8	2			6
Permitted Phases				Free		
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	7.0	7.0	14.0			14.0
Minimum Split (s)	14.0	14.0	21.0			21.0
Total Split (s)	29.0	29.0	91.0			91.0
Total Split (%)	24.2%	24.2%	75.8%			75.8%
Maximum Green (s)	22.0	22.0	84.0			84.0
Yellow Time (s)	5.0	5.0	5.0			5.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0			-2.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	22.3	22.3	87.7	120.0		87.7
Actuated g/C Ratio	0.19	0.19	0.73	1.00		0.73
v/c Ratio	0.74	0.13	0.56	0.14		0.83
Control Delay	53.7	41.1	8.2	0.2		13.9
Queue Delay	0.0	0.0	0.0	0.0		0.0
Total Delay	53.7	41.1	8.2	0.2		13.9
LOS	D	D	A	A		B
Approach Delay	52.8		7.4			13.9

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	D		A		B	
Queue Length 50th (ft)	175	25	239	0	533	
Queue Length 95th (ft)	232	56	279	0	613	
Internal Link Dist (ft)	902		571		141	
Turn Bay Length (ft)	300			400		
Base Capacity (vph)	680	313	3646	1568	3646	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.69	0.12	0.56	0.14	0.83	

Intersection Summary

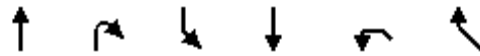
Area Type:	Other
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green, Master Intersection
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.83
Intersection Signal Delay:	14.8
Intersection LOS:	B
Intersection Capacity Utilization	72.7%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 91: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑↑	↘	
Traffic Volume (vph)	0	0	0	2709	4	0
Future Volume (vph)	0	0	0	2709	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	4988	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	4988	1736	0
Link Speed (mph)	55			55	25	
Link Distance (ft)	370			1103	63	
Travel Time (s)	4.6			13.7	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	3010	4	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3010	4	0
Sign Control	Free			Free	Yield	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.3%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700

	↑	↗	↘	↓	↖	↙
Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑	↖	
Traffic Volume (veh/h)	0	0	0	2709	4	0
Future Volume (Veh/h)	0	0	0	2709	4	0
Sign Control	Free			Free	Yield	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	3010	4	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	0			1003	0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0			1003	0	
tC, single (s)	4.1			6.9	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			98	100	
cM capacity (veh/h)	1622			235	1084	
Direction, Lane #	SB 1	SB 2	SB 3	NW 1		
Volume Total	1003	1003	1003	4		
Volume Left	0	0	0	4		
Volume Right	0	0	0	0		
cSH	1700	1700	1700	235		
Volume to Capacity	0.59	0.59	0.59	0.02		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	0.0	0.0	20.6		
Lane LOS					C	
Approach Delay (s)	0.0			20.6		
Approach LOS					C	
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	62.3%			ICU Level of Service	B	
Analysis Period (min)	15					

Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	21	0	0	2008	0	0
Future Volume (vph)	21	0	0	2008	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	88			578	601	
Travel Time (s)	2.4			7.2	7.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	23	0	0	2231	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	23	0	0	2231	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.6%
Analysis Period (min)	15
	ICU Level of Service G

HCM Unsignalized Intersection Capacity Analysis
 93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↶			↷↷↷		
Traffic Volume (veh/h)	21	0	0	2008	0	0
Future Volume (Veh/h)	21	0	0	2008	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	23	0	0	2231	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	744	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	744	0	0			
tC, single (s)	6.9	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	100	100			
cM capacity (veh/h)	346	1084	1622			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	23	744	744	744		
Volume Left	23	0	0	0		
Volume Right	0	0	0	0		
cSH	346	1700	1700	1700		
Volume to Capacity	0.07	0.44	0.44	0.44		
Queue Length 95th (ft)	5	0	0	0		
Control Delay (s)	16.1	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	16.1	0.0				
Approach LOS	C					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			105.6%		ICU Level of Service	G
Analysis Period (min)			15			

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	4	1859	0	0	0	0
Future Volume (vph)	4	1859	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	40				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		320	1105		63	
Travel Time (s)		4.0	13.7		1.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	4	2066	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	2066	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.3%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	21	3112
Future Volume (vph)	0	0	0	0	21	3112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	30		30			55
Link Distance (ft)	88		647			536
Travel Time (s)	2.0		14.7			6.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	23	3458
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	23	3458
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	125.5%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 96: SB US 321/US 321 & NB US 321

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑		↑↑↑
Traffic Volume (vph)	0	0	0	3133	0	2029
Future Volume (vph)	0	0	0	3133	0	2029
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	4988	0	3541
Flt Permitted						
Satd. Flow (perm)	0	0	0	4988	0	3541
Link Speed (mph)	55			55	55	
Link Distance (ft)	536			651	601	
Travel Time (s)	6.6			8.1	7.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	4%
Adj. Flow (vph)	0	0	0	3481	0	2254
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3481	0	2254
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	114.5%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
97: US 321 & NB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	1863	0	0	0	2713
Future Volume (vph)	0	1863	0	0	0	2713
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		221	320		370	
Travel Time (s)		2.7	4.0		4.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	2070	0	0	0	3014
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2070	0	0	0	3014
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	66.6%
ICU Level of Service	C
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	2008	0	0	0	3112
Future Volume (vph)	0	2008	0	0	0	3112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		1796	578		647	
Travel Time (s)		22.3	7.2		8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	2231	0	0	0	3458
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2231	0	0	0	3458
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.9%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5143
Vehs Exited	5112
Starting Vehs	111
Ending Vehs	142
Travel Distance (mi)	5038
Travel Time (hr)	129.2
Total Delay (hr)	33.8
Total Stops	1525
Fuel Used (gal)	170.8

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5143
Vehs Exited	5112
Starting Vehs	111
Ending Vehs	142
Travel Distance (mi)	5038
Travel Time (hr)	129.2
Total Delay (hr)	33.8
Total Stops	1525
Fuel Used (gal)	170.8

Intersection: 91: US 321 & Grace Chapel Road

Movement	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	T	T	T	T	T	T
Maximum Queue (ft)	227	272	106	152	160	175	201	201	214
Average Queue (ft)	146	179	35	112	118	119	181	179	142
95th Queue (ft)	227	249	83	141	161	165	213	218	204
Link Distance (ft)			906	607	607	607	138	138	138
Upstream Blk Time (%)							11	9	4
Queuing Penalty (veh)							101	84	33
Storage Bay Dist (ft)	300	300							
Storage Blk Time (%)									
Queuing Penalty (veh)									

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	NW
Directions Served	L
Maximum Queue (ft)	22
Average Queue (ft)	2
95th Queue (ft)	13
Link Distance (ft)	-5
Upstream Blk Time (%)	8
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	45
Average Queue (ft)	10
95th Queue (ft)	31
Link Distance (ft)	22
Upstream Blk Time (%)	9
Queuing Penalty (veh)	2
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 96: SB US 321/US 321 & NB US 321

Movement	SB
Directions Served	T
Maximum Queue (ft)	37
Average Queue (ft)	1
95th Queue (ft)	12
Link Distance (ft)	607
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 97: US 321 & NB US 321

Movement	SE	SE	SE
Directions Served	R	R	R
Maximum Queue (ft)	117	113	61
Average Queue (ft)	31	25	9
95th Queue (ft)	83	76	39
Link Distance (ft)	320	320	320
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 996: US 321/NB US 321 & SB US 321

















Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 221

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road

NCDOT TIP-U4700

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	 		  			  
Traffic Volume (vph)	182	18	2690	453	0	1865
Future Volume (vph)	182	18	2690	453	0	1865
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300	0		400	0	
Storage Lanes	2	1		1	0	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	1.00	0.91	1.00	1.00	0.91
Frt		0.850		0.850		
Flt Protected	0.950					
Satd. Flow (prot)	3400	1568	4988	1568	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1568	4988	1568	0	4988
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		55			55
Link Distance (ft)	982		651			221
Travel Time (s)	19.1		8.1			2.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	4%	3%	4%	4%
Adj. Flow (vph)	202	20	2989	503	0	2072
Shared Lane Traffic (%)						
Lane Group Flow (vph)	202	20	2989	503	0	2072
Turn Type	Prot	Prot	NA	Free		NA
Protected Phases	8	8	2			6
Permitted Phases				Free		
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	7.0	7.0	14.0			14.0
Minimum Split (s)	14.0	14.0	21.0			21.0
Total Split (s)	19.0	19.0	101.0			101.0
Total Split (%)	15.8%	15.8%	84.2%			84.2%
Maximum Green (s)	12.0	12.0	94.0			94.0
Yellow Time (s)	5.0	5.0	5.0			5.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0			-2.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	13.2	13.2	96.8	120.0		96.8
Actuated g/C Ratio	0.11	0.11	0.81	1.00		0.81
v/c Ratio	0.54	0.12	0.74	0.32		0.52
Control Delay	56.1	49.3	7.1	0.5		4.4
Queue Delay	0.0	0.0	0.0	0.0		0.0
Total Delay	56.1	49.3	7.1	0.5		4.4
LOS	E	D	A	A		A
Approach Delay	55.5		6.2			4.4

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	E		A			A
Queue Length 50th (ft)	76	14	341	0		162
Queue Length 95th (ft)	116	39	387	0		185
Internal Link Dist (ft)	902		571			141
Turn Bay Length (ft)	300			400		
Base Capacity (vph)	396	182	4023	1568		4023
Starvation Cap Reductn	0	0	0	0		0
Spillback Cap Reductn	0	0	0	0		0
Storage Cap Reductn	0	0	0	0		0
Reduced v/c Ratio	0.51	0.11	0.74	0.32		0.52

Intersection Summary

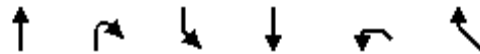
Area Type:	Other
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green, Master Intersection
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.74
Intersection Signal Delay:	7.5
Intersection LOS:	A
Intersection Capacity Utilization	66.1%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 91: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑↑	↘	
Traffic Volume (vph)	0	0	0	1861	4	0
Future Volume (vph)	0	0	0	1861	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	4988	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	4988	1736	0
Link Speed (mph)	55			55	25	
Link Distance (ft)	370			1103	63	
Travel Time (s)	4.6			13.7	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	2068	4	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2068	4	0
Sign Control	Free			Free	Yield	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.6% ICU Level of Service B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
 92: Grace Chapel Road U-Turn & SB US 321

NCDOT TIP-U4700

	↑	↗	↘	↓	↙	↖
Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑	↗	
Traffic Volume (veh/h)	0	0	0	1861	4	0
Future Volume (Veh/h)	0	0	0	1861	4	0
Sign Control	Free			Free	Yield	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	2068	4	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	0			689	0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0			689	0	
tC, single (s)	4.1			6.9	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			99	100	
cM capacity (veh/h)	1622			375	1084	
Direction, Lane #	SB 1	SB 2	SB 3	NW 1		
Volume Total	689	689	689	4		
Volume Left	0	0	0	4		
Volume Right	0	0	0	0		
cSH	1700	1700	1700	375		
Volume to Capacity	0.41	0.41	0.41	0.01		
Queue Length 95th (ft)	0	0	0	1		
Control Delay (s)	0.0	0.0	0.0	14.7		
Lane LOS				B		
Approach Delay (s)	0.0			14.7		
Approach LOS				B		
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	55.6%			ICU Level of Service	B	
Analysis Period (min)	15					

Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	29	0	0	3114	0	0
Future Volume (vph)	29	0	0	3114	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	88			578	601	
Travel Time (s)	2.4			7.2	7.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	32	0	0	3460	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	32	0	0	3460	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.8%
Analysis Period (min)	15
	ICU Level of Service G

HCM Unsignalized Intersection Capacity Analysis

93: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↵			↑↑↑		
Traffic Volume (veh/h)	29	0	0	3114	0	0
Future Volume (Veh/h)	29	0	0	3114	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	32	0	0	3460	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1153	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1153	0	0			
tC, single (s)	6.9	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	83	100	100			
cM capacity (veh/h)	188	1084	1622			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	32	1153	1153	1153		
Volume Left	32	0	0	0		
Volume Right	0	0	0	0		
cSH	188	1700	1700	1700		
Volume to Capacity	0.17	0.68	0.68	0.68		
Queue Length 95th (ft)	15	0	0	0		
Control Delay (s)	28.1	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	28.1	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			105.8%		ICU Level of Service	G
Analysis Period (min)			15			

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	4	2704	0	0	0	0
Future Volume (vph)	4	2704	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	40				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		320	1105		63	
Travel Time (s)		4.0	13.7		1.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	4	3004	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	3004	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.6%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	29	2018
Future Volume (vph)	0	0	0	0	29	2018
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	30		30			55
Link Distance (ft)	88		647			536
Travel Time (s)	2.0		14.7			6.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	32	2242
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	32	2242
Sign Control	Stop		Stop			Free

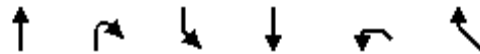
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	130.5%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 96: SB US 321/US 321 & NB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑↑		↑↑↑↑
Traffic Volume (vph)	0	0	0	2047	0	3143
Future Volume (vph)	0	0	0	2047	0	3143
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	4988	0	3541
Flt Permitted						
Satd. Flow (perm)	0	0	0	4988	0	3541
Link Speed (mph)	55			55	55	
Link Distance (ft)	536			651	601	
Travel Time (s)	6.6			8.1	7.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	4%
Adj. Flow (vph)	0	0	0	2274	0	3492
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2274	0	3492
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	119.5%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
97: US 321 & NB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	2708	0	0	0	1865
Future Volume (vph)	0	2708	0	0	0	1865
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		221	320		370	
Travel Time (s)		2.7	4.0		4.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	3009	0	0	0	2072
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3009	0	0	0	2072
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.7%
ICU Level of Service	B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	3114	0	0	0	2018
Future Volume (vph)	0	3114	0	0	0	2018
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		1796	578		647	
Travel Time (s)		22.3	7.2		8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	3460	0	0	0	2242
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3460	0	0	0	2242
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.5%
ICU Level of Service	B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	5182
Vehs Exited	5163
Starting Vehs	112
Ending Vehs	131
Travel Distance (mi)	5085
Travel Time (hr)	120.1
Total Delay (hr)	24.4
Total Stops	907
Fuel Used (gal)	168.0

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	5182
Vehs Exited	5163
Starting Vehs	112
Ending Vehs	131
Travel Distance (mi)	5085
Travel Time (hr)	120.1
Total Delay (hr)	24.4
Total Stops	907
Fuel Used (gal)	168.0

Intersection: 91: US 321 & Grace Chapel Road

Movement	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	T	T	T	T	T	T
Maximum Queue (ft)	240	273	76	212	202	193	166	135	153
Average Queue (ft)	46	109	19	115	120	117	103	82	57
95th Queue (ft)	144	185	50	177	178	177	151	134	128
Link Distance (ft)			906	607	607	607	138	138	138
Upstream Blk Time (%)							1	0	0
Queuing Penalty (veh)							6	2	2
Storage Bay Dist (ft)	300	300							
Storage Blk Time (%)									
Queuing Penalty (veh)									

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	NW
Directions Served	L
Maximum Queue (ft)	23
Average Queue (ft)	4
95th Queue (ft)	18
Link Distance (ft)	-5
Upstream Blk Time (%)	3
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	67
Average Queue (ft)	25
95th Queue (ft)	55
Link Distance (ft)	22
Upstream Blk Time (%)	45
Queuing Penalty (veh)	13
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 96: SB US 321/US 321 & NB US 321

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 97: US 321 & NB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Intersection: 996: US 321/NB US 321 & SB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 23

APPENDIX L

**2040 Build Alternative Dudley Shoals Road – At-Grade Intersection
Synchro & SimTraffic Reports**

Lanes, Volumes, Timings
1: Dudley Shoals



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔			
Traffic Volume (vph)	10	111	134	112	0	0
Future Volume (vph)	10	111	134	112	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.939			
Flt Protected		0.996				
Satd. Flow (prot)	0	1855	1749	0	0	0
Flt Permitted		0.996				
Satd. Flow (perm)	0	1855	1749	0	0	0
Link Speed (mph)		35	35		55	
Link Distance (ft)		1050	1626		1046	
Travel Time (s)		20.5	31.7		13.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	11	123	149	124	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	134	273	0	0	0
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	17.5%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis

1: Dudley Shoals

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	10	111	134	112	0	0
Future Volume (Veh/h)	10	111	134	112	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	123	149	124	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	273				356	211
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	273				356	211
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				100	100
cM capacity (veh/h)	1290				637	829
Direction, Lane #						
	EB 1	WB 1				
Volume Total	134	273				
Volume Left	11	0				
Volume Right	0	124				
cSH	1290	1700				
Volume to Capacity	0.01	0.16				
Queue Length 95th (ft)	1	0				
Control Delay (s)	0.7	0.0				
Lane LOS	A					
Approach Delay (s)	0.7	0.0				
Approach LOS						
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			17.5%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings
2: Dudley Shoals

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	216	90	58	53	156	188
Future Volume (vph)	216	90	58	53	156	188
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.960				0.926	
Flt Protected	0.966			0.975		
Satd. Flow (prot)	1727	0	0	1816	1725	0
Flt Permitted	0.966			0.975		
Satd. Flow (perm)	1727	0	0	1816	1725	0
Link Speed (mph)	45			45	45	
Link Distance (ft)	639			746	415	
Travel Time (s)	9.7			11.3	6.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	240	100	64	59	173	209
Shared Lane Traffic (%)						
Lane Group Flow (vph)	340	0	0	123	382	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	53.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

2: Dudley Shoals

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	216	90	58	53	156	188
Future Volume (Veh/h)	216	90	58	53	156	188
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	240	100	64	59	173	209
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	464	278	382			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	464	278	382			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	54	87	95			
cM capacity (veh/h)	526	761	1176			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	340	123	382			
Volume Left	240	64	0			
Volume Right	100	0	209			
cSH	578	1176	1700			
Volume to Capacity	0.59	0.05	0.22			
Queue Length 95th (ft)	95	4	0			
Control Delay (s)	19.7	4.5	0.0			
Lane LOS	C	A				
Approach Delay (s)	19.7	4.5	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			8.6			
Intersection Capacity Utilization		53.2%		ICU Level of Service		A
Analysis Period (min)			15			

Lanes, Volumes, Timings
7: US 321

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	122	0	1459	2058	0
Future Volume (vph)	0	122	0	1459	2058	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Frt	0.865					
Flt Protected						
Satd. Flow (prot)	0	1611	0	3539	3539	0
Flt Permitted						
Satd. Flow (perm)	0	1611	0	3539	3539	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1046			2666	352	
Travel Time (s)	13.0			33.0	4.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	136	0	1621	2287	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	136	0	1621	2287	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	71.1% ICU Level of Service C
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

7: US 321

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↕	↕	
Traffic Volume (veh/h)	0	122	0	1459	2058	0
Future Volume (Veh/h)	0	122	0	1459	2058	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	136	0	1621	2287	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	3098	1144	2287			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	3098	1144	2287			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	30	100			
cM capacity (veh/h)	9	194	218			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	136	810	810	1144	1144	
Volume Left	0	0	0	0	0	
Volume Right	136	0	0	0	0	
cSH	194	1700	1700	1700	1700	
Volume to Capacity	0.70	0.48	0.48	0.67	0.67	
Queue Length 95th (ft)	110	0	0	0	0	
Control Delay (s)	58.4	0.0	0.0	0.0	0.0	
Lane LOS	F					
Approach Delay (s)	58.4	0.0		0.0		
Approach LOS	F					
Intersection Summary						
Average Delay			2.0			
Intersection Capacity Utilization			71.1%	ICU Level of Service		C
Analysis Period (min)			15			

Lanes, Volumes, Timings

NCDOT TIP-U4700

131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations												
Traffic Volume (vph)	0	0	256	0	1252	154	0	0	0	182	144	0
Future Volume (vph)	0	0	256	0	1252	154	0	0	0	182	144	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	200		0		0	0		0		0	0
Storage Lanes	0	1		0		1	0		0		2	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850						
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2787	0	3471	1583	0	0	0	1770	1770	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2787	0	3471	1583	0	0	0	1770	1770	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55				25
Link Distance (ft)	1199				166			766				230
Travel Time (s)	23.4				2.1			9.5				6.3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	0	284	0	1391	171	0	0	0	202	160	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	284	0	1391	171	0	0	0	202	160	0
Turn Type			Prot		NA	Prot				Prot	Prot	
Protected Phases			8		2	2				7	4	
Permitted Phases												
Minimum Initial (s)			7.0		14.0	14.0				7.0	7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	14.0	
Total Split (s)			25.0		46.0	46.0				19.0	44.0	
Total Split (%)			27.8%		51.1%	51.1%				21.1%	48.9%	
Maximum Green (s)			18.0		39.0	39.0				12.0	37.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	5.0	
Lead/Lag			Lag							Lead		
Lead-Lag Optimize?			Yes							Yes		
Vehicle Extension (s)			3.0		3.0	3.0				3.0	3.0	
Recall Mode			None		C-Min	C-Min				None	None	
Act Effct Green (s)			16.2		45.1	45.1				13.7	34.9	
Actuated g/C Ratio			0.18		0.50	0.50				0.15	0.39	
v/c Ratio			0.57		0.80	0.22				0.75	0.23	
Control Delay			37.8		19.1	10.4				43.1	12.2	
Queue Delay			0.0		0.0	0.0				0.0	0.0	
Total Delay			37.8		19.1	10.4				43.1	12.2	
LOS			D		B	B				D	B	
Approach Delay	37.8				18.1							29.4
Approach LOS	D				B							C
Queue Length 50th (ft)			84		351	43				107	48	
Queue Length 95th (ft)			122		#347	m62				m118	m49	

131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

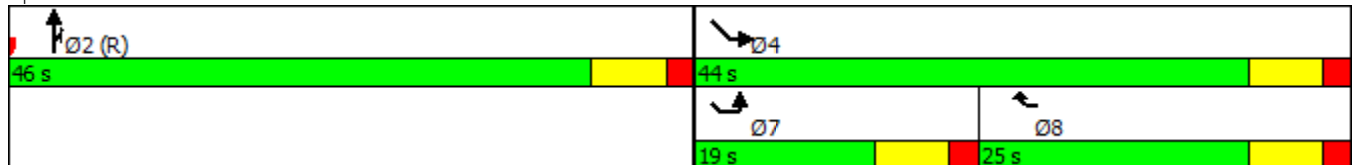


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Internal Link Dist (ft)	1119				86			686			150	
Turn Bay Length (ft)			200									
Base Capacity (vph)			619		1739	793				275	767	
Starvation Cap Reductn			0		0	0				0	0	
Spillback Cap Reductn			0		0	0				0	0	
Storage Cap Reductn			0		0	0				0	0	
Reduced v/c Ratio			0.46		0.80	0.22				0.73	0.21	

Intersection Summary

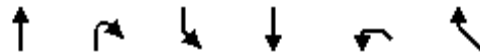
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 89 (99%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 22.5 Intersection LOS: C
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 131: NB US 321 & Pinewood Road & Pinewood Road Left-Over



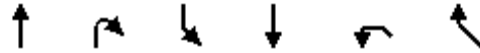
Lanes, Volumes, Timings
 132: SB US 321 & Pinewood Road U-Turn

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	2283	118	0
Future Volume (vph)	0	0	0	2283	118	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3471	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3471	1736	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	30			55	25	
Link Distance (ft)	587			861	64	
Travel Time (s)	13.3			10.7	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	2537	131	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2537	131	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				76.0	14.0	
Total Split (%)				84.4%	15.6%	
Maximum Green (s)				69.0	7.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				70.8	9.2	
Actuated g/C Ratio				0.79	0.10	
v/c Ratio				0.93	0.74	
Control Delay				4.5	61.2	
Queue Delay				0.1	0.0	
Total Delay				4.6	61.2	
LOS				A	E	
Approach Delay				4.6	61.2	
Approach LOS				A	E	
Queue Length 50th (ft)				86	73	
Queue Length 95th (ft)				m83	m#101	
Internal Link Dist (ft)	507			781	1	
Turn Bay Length (ft)						
Base Capacity (vph)				2738	178	

Lanes, Volumes, Timings
 132: SB US 321 & Pinewood Road U-Turn



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Starvation Cap Reductn				10	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.93	0.74	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 1 (1%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.93
 Intersection Signal Delay: 7.4 Intersection LOS: A
 Intersection Capacity Utilization 114.1% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 132: SB US 321 & Pinewood Road U-Turn

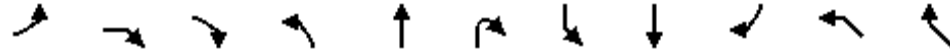


133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	333	0	0	0	0	1864	211	192	0
Future Volume (vph)	0	0	333	0	0	0	0	1864	211	192	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	300		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2733	0	0	0	0	3471	1553	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2733	0	0	0	0	3471	1553	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	770				688			243		154	
Travel Time (s)	15.0				15.6			3.0		4.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	2%	2%	2%	2%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	370	0	0	0	0	2071	234	213	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	370	0	0	0	0	2071	234	213	0
Turn Type			Prot					NA	Perm	Prot	
Protected Phases			4					6		8	
Permitted Phases									6		
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			22.0					68.0	68.0	22.0	
Total Split (%)			24.4%					75.6%	75.6%	24.4%	
Maximum Green (s)			15.0					61.0	61.0	15.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			16.5					63.5	63.5	16.5	
Actuated g/C Ratio			0.18					0.71	0.71	0.18	
v/c Ratio			0.74					0.85	0.21	0.67	
Control Delay			44.4					5.8	1.9	43.6	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			44.4					5.8	1.9	43.6	
LOS			D					A	A	D	
Approach Delay	44.4							5.4		43.6	
Approach LOS	D							A		D	
Queue Length 50th (ft)			113					185	14	104	
Queue Length 95th (ft)			167					m253	m15	#183	

133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	690				608			163		74	
Turn Bay Length (ft)			300								
Base Capacity (vph)			518					2449	1096	328	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.71					0.85	0.21	0.65	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 19 (21%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.85
 Intersection Signal Delay: 13.2 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lanes, Volumes, Timings
 134: NB US 321 & Pinewood Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	139	0	0	1459	0	0
Future Volume (vph)	139	0	0	1459	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	71			478	600	
Travel Time (s)	1.9			5.9	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	154	0	0	1621	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	154	0	0	1621	0	0
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	22.0			68.0		
Total Split (%)	24.4%			75.6%		
Maximum Green (s)	15.0			61.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	14.5			65.5		
Actuated g/C Ratio	0.16			0.73		
v/c Ratio	0.55			0.64		
Control Delay	36.3			8.1		
Queue Delay	0.0			0.0		
Total Delay	36.3			8.1		
LOS	D			A		
Approach Delay	36.3			8.1		
Approach LOS	D			A		
Queue Length 50th (ft)	73			213		
Queue Length 95th (ft)	m93			299		
Internal Link Dist (ft)	1			398	520	
Turn Bay Length (ft)						
Base Capacity (vph)	328			2526		

Lanes, Volumes, Timings
 134: NB US 321 & Pinewood Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.47			0.64		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 86 (96%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 10.6 Intersection LOS: B
 Intersection Capacity Utilization 56.4% ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 134: NB US 321 & Pinewood Road U-Turn



Lanes, Volumes, Timings
 135: NB US 321 & Pinewood Road U-Turn

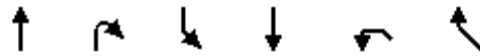


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	118	1572	0	0
Future Volume (vph)	0	0	118	1572	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	64			766	838	
Travel Time (s)	1.5			9.5	10.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	131	1747	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	131	1747	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	114.1% ICU Level of Service H
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 136: SB US 321 & Pinewood Road Left-Over



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↙↘	↑↑↑		
Traffic Volume (vph)	0	0	326	2075	0	0
Future Volume (vph)	0	0	326	2075	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	100		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3433	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3433	4988	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	243			587	230	
Travel Time (s)	5.5			7.3	2.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%
Adj. Flow (vph)	0	0	362	2306	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	362	2306	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	78.0%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 137: SB US 321 & Pinewood Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	139	2058
Future Volume (vph)	0	0	0	0	139	2058
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	30		30			55
Link Distance (ft)	71		536			688
Travel Time (s)	1.6		12.2			8.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	154	2287
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	154	2287
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	58.7%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 138: NB US 321 & Pinewood Road Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	192	1406	0	0
Future Volume (vph)	0	0	192	1406	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	154			600	166	
Travel Time (s)	1.9			7.4	2.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	213	1562	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	213	1562	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	86.9%
Analysis Period (min)	15
	ICU Level of Service E

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

NCDOT TIP-U4700

141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations											
Traffic Volume (vph)	0	0	145	0	1605	30	0	0	0	6	0
Future Volume (vph)	0	0	145	0	1605	30	0	0	0	6	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200		0	200		300	0	0
Storage Lanes	0	1		0		0	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt			0.865		0.997						
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	3461	0	0	0	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	3461	0	0	0	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	499				224			926		66	
Travel Time (s)	9.7				2.8			11.5		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	2%	4%	3%	2%	4%	2%	3%	2%
Adj. Flow (vph)	0	0	161	0	1783	33	0	0	0	7	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	161	0	1816	0	0	0	0	7	0
Turn Type			Prot		NA					Prot	
Protected Phases			8		2					4	
Permitted Phases											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			22.0		68.0					22.0	
Total Split (%)			24.4%		75.6%					24.4%	
Maximum Green (s)			15.0		61.0					15.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			Min		C-Min					None	
Act Effct Green (s)			15.0		65.0					15.0	
Actuated g/C Ratio			0.17		0.72					0.17	
v/c Ratio			0.61		0.73					0.02	
Control Delay			44.6		4.7					36.8	
Queue Delay			0.0		0.0					0.0	
Total Delay			44.6		4.7					36.8	
LOS			D		A					D	
Approach Delay	44.6				4.7					36.8	
Approach LOS	D				A					D	
Queue Length 50th (ft)			84		84					4	
Queue Length 95th (ft)			147		157					m5	

141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over

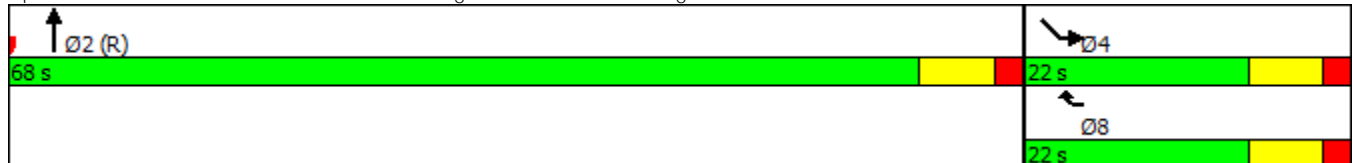


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	419				144			846		1	
Turn Bay Length (ft)											
Base Capacity (vph)			301		2501					330	
Starvation Cap Reductn			0		0					0	
Spillback Cap Reductn			0		0					0	
Storage Cap Reductn			0		0					0	
Reduced v/c Ratio			0.53		0.73					0.02	

Intersection Summary

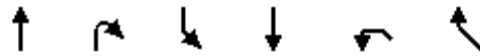
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 8 (9%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 8.1 Intersection LOS: A
 Intersection Capacity Utilization 62.6% ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over



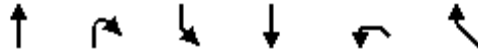
Lanes, Volumes, Timings
 142: SB US 321 & N. Highland Avenue U-Turn

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	2022	122	0
Future Volume (vph)	0	0	0	2022	122	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3471	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3471	1736	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	30			55	25	
Link Distance (ft)	894			2338	46	
Travel Time (s)	20.3			29.0	1.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	2247	136	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2247	136	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				74.0	16.0	
Total Split (%)				82.2%	17.8%	
Maximum Green (s)				67.0	9.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				68.9	11.1	
Actuated g/C Ratio				0.77	0.12	
v/c Ratio				0.85	0.64	
Control Delay				11.1	39.0	
Queue Delay				0.0	0.0	
Total Delay				11.1	39.0	
LOS				B	D	
Approach Delay				11.1	39.0	
Approach LOS				B	D	
Queue Length 50th (ft)				350	74	
Queue Length 95th (ft)				468	m103	
Internal Link Dist (ft)	814			2258	1	
Turn Bay Length (ft)						
Base Capacity (vph)				2670	218	

Lanes, Volumes, Timings
 142: SB US 321 & N. Highland Avenue U-Turn



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.84	0.62	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 73 (81%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.85
 Intersection Signal Delay: 12.7 Intersection LOS: B
 Intersection Capacity Utilization 108.4% ICU Level of Service G
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 142: SB US 321 & N. Highland Avenue U-Turn



143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	231	0	0	0	0	2103	35	6	0
Future Volume (vph)	0	0	231	0	0	0	0	2103	35	6	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00
Fr _t			0.865					0.998			
Fl _t Protected										0.950	
Satd. Flow (prot)	0	0	1611	0	0	0	0	3465	0	1770	0
Fl _t Permitted										0.950	
Satd. Flow (perm)	0	0	1611	0	0	0	0	3465	0	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	804				861			210		86	
Travel Time (s)	15.7				19.6			2.6		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	4%	2%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	257	0	0	0	0	2337	39	7	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	257	0	0	0	0	2376	0	7	0
Turn Type			Prot					NA		Prot	
Protected Phases			4					6		8	
Permitted Phases			4								
Minimum Initial (s)			7.0					14.0		7.0	
Minimum Split (s)			14.0					21.0		14.0	
Total Split (s)			21.0					69.0		21.0	
Total Split (%)			23.3%					76.7%		23.3%	
Maximum Green (s)			14.0					62.0		14.0	
Yellow Time (s)			5.0					5.0		5.0	
All-Red Time (s)			2.0					2.0		2.0	
Total Lost Time (s)			5.0					5.0		5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0		3.0	
Recall Mode			None					C-Max		Min	
Act Effct Green (s)			16.0					64.0		16.0	
Actuated g/C Ratio			0.18					0.71		0.18	
v/c Ratio			0.90					0.96		0.02	
Control Delay			70.9					17.6		30.7	
Queue Delay			0.0					3.0		0.0	
Total Delay			70.9					20.6		30.7	
LOS			E					C		C	
Approach Delay	70.9							20.6		30.7	
Approach LOS	E							C		C	
Queue Length 50th (ft)			144					591		4	
Queue Length 95th (ft)			#285					#850		m5	
Internal Link Dist (ft)	724				781			130		6	
Turn Bay Length (ft)											
Base Capacity (vph)			286					2464		314	

143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Starvation Cap Reductn			0					0		0	
Spillback Cap Reductn			0					59		0	
Storage Cap Reductn			0					0		0	
Reduced v/c Ratio			0.90					0.99		0.02	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 88 (98%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.96
 Intersection Signal Delay: 25.6 Intersection LOS: C
 Intersection Capacity Utilization 81.9% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lanes, Volumes, Timings
 145: NB US 321 & N. Highland Avenue U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	122	1628	0	0
Future Volume (vph)	0	0	122	1628	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	46			926	2360	
Travel Time (s)	1.0			11.5	29.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	136	1809	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	136	1809	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	108.4%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 146: SB US 321 & N. Highland Avenue Left-Over



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	6	2138
Future Volume (vph)	0	0	0	0	6	2138
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	50	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3471
Link Speed (mph)	30		30			55
Link Distance (ft)	66		210			894
Travel Time (s)	1.5		4.8			11.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	4%
Adj. Flow (vph)	0	0	0	0	7	2376
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	7	2376
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	71.0% ICU Level of Service C
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 148: NB US 321 & N. Highland Avenue Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	6	1635	0	0
Future Volume (vph)	0	0	6	1635	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	50			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1770	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1770	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	86			838	224	
Travel Time (s)	2.0			10.4	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%
Adj. Flow (vph)	0	0	7	1817	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	7	1817	0	0
Sign Control	Stop			Free	Stop	

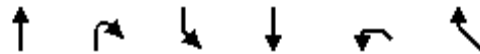
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	48.5%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 991: SB US 321/US 321 & NB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑		↑↑
Traffic Volume (vph)	0	0	0	2022	0	1628
Future Volume (vph)	0	0	0	2022	0	1628
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3471	0	2733
Flt Permitted						
Satd. Flow (perm)	0	0	0	3471	0	2733
Link Speed (mph)	55			55	55	
Link Distance (ft)	2338			158	2360	
Travel Time (s)	29.0			2.0	29.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	4%
Adj. Flow (vph)	0	0	0	2247	0	1809
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2247	0	1809
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	60.3%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 992: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑				↑↑
Traffic Volume (vph)	0	1459	0	0	0	2058
Future Volume (vph)	0	1459	0	0	0	2058
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	3471	0	0	0	2733
Flt Permitted						
Satd. Flow (perm)	0	3471	0	0	0	2733
Link Speed (mph)		55	55		55	
Link Distance (ft)		352	478		536	
Travel Time (s)		4.4	5.9		6.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	1621	0	0	0	2287
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1621	0	0	0	2287
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.3%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	4821
Vehs Exited	4848
Starting Vehs	239
Ending Vehs	212
Travel Distance (mi)	7320
Travel Time (hr)	239.7
Total Delay (hr)	90.2
Total Stops	4845
Fuel Used (gal)	277.2

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	4821
Vehs Exited	4848
Starting Vehs	239
Ending Vehs	212
Travel Distance (mi)	7320
Travel Time (hr)	239.7
Total Delay (hr)	90.2
Total Stops	4845
Fuel Used (gal)	277.2

Intersection: 1: Dudley Shoals

Movement	EB
Directions Served	LT
Maximum Queue (ft)	31
Average Queue (ft)	4
95th Queue (ft)	21
Link Distance (ft)	1029
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 2: Dudley Shoals

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	175	55	51
Average Queue (ft)	72	15	2
95th Queue (ft)	119	45	19
Link Distance (ft)	582	694	380
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 7: US 321

Movement	EB
Directions Served	R
Maximum Queue (ft)	200
Average Queue (ft)	76
95th Queue (ft)	150
Link Distance (ft)	970
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

Movement	WB	WB	NB	NB	SE	SE
Directions Served	>	>	T	T	<	L
Maximum Queue (ft)	159	156	184	180	176	96
Average Queue (ft)	76	70	135	142	102	57
95th Queue (ft)	128	120	192	196	159	94
Link Distance (ft)	1112		82	82	117	117
Upstream Blk Time (%)			16	19	10	
Queuing Penalty (veh)			74	91	17	
Storage Bay Dist (ft)		200				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 132: SB US 321 & Pinewood Road U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	309	397	79
Average Queue (ft)	120	135	75
95th Queue (ft)	227	242	88
Link Distance (ft)	742	742	22
Upstream Blk Time (%)			78
Queuing Penalty (veh)			93
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 133: SB US 321 & Pinewood Road Left-Over & Pinewood Road

Movement	EB	EB	SB	SB	NW
Directions Served	>	>	T	T	L
Maximum Queue (ft)	299	281	217	221	160
Average Queue (ft)	173	127	90	111	116
95th Queue (ft)	268	250	171	196	162
Link Distance (ft)	696		108	108	65
Upstream Blk Time (%)			3	7	33
Queuing Penalty (veh)			21	46	64
Storage Bay Dist (ft)		300			
Storage Blk Time (%)	0	0			
Queuing Penalty (veh)	0	0			

Intersection: 134: NB US 321 & Pinewood Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	100	222	225
Average Queue (ft)	67	86	89
95th Queue (ft)	98	156	163
Link Distance (ft)	19	432	432
Upstream Blk Time (%)	64		
Queuing Penalty (veh)	88		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 135: NB US 321 & Pinewood Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	122
Average Queue (ft)	45
95th Queue (ft)	111
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	150
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 136: SB US 321 & Pinewood Road Left-Over

Movement	SB	SB
Directions Served	T	T
Maximum Queue (ft)	133	40
Average Queue (ft)	5	3
95th Queue (ft)	45	20
Link Distance (ft)	536	536
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 137: SB US 321 & Pinewood Road U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	137
Average Queue (ft)	44
95th Queue (ft)	114
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	250
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 138: NB US 321 & Pinewood Road Left-Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	94	146	151
Average Queue (ft)	9	29	34
95th Queue (ft)	47	90	104
Link Distance (ft)		545	545
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	200		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over

Movement	WB	NB	NB	SE
Directions Served	>	T	TR	L
Maximum Queue (ft)	188	222	222	45
Average Queue (ft)	90	86	87	8
95th Queue (ft)	155	180	180	29
Link Distance (ft)	452	163	163	6
Upstream Blk Time (%)		1	1	8
Queuing Penalty (veh)		6	7	0
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 142: SB US 321 & N. Highland Avenue U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	220	222	71
Average Queue (ft)	116	116	49
95th Queue (ft)	203	208	71
Link Distance (ft)	2297	2297	2
Upstream Blk Time (%)			51
Queuing Penalty (veh)			63
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue

Movement	EB	SB	SB	NW
Directions Served	>	T	TR	L
Maximum Queue (ft)	265	239	234	25
Average Queue (ft)	146	166	172	3
95th Queue (ft)	232	252	256	16
Link Distance (ft)	716	157	157	22
Upstream Blk Time (%)		11	11	4
Queuing Penalty (veh)		122	120	0
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 145: NB US 321 & N. Highland Avenue U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	112
Average Queue (ft)	34
95th Queue (ft)	89
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	150
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 146: SB US 321 & N. Highland Avenue Left-Over

Movement	SB	SB
Directions Served	T	T
Maximum Queue (ft)	204	162
Average Queue (ft)	30	35
95th Queue (ft)	118	122
Link Distance (ft)	851	851
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)	4	
Queuing Penalty (veh)	0	

Intersection: 148: NB US 321 & N. Highland Avenue Left-Over

Movement	NB	NB
Directions Served	T	T
Maximum Queue (ft)	50	104
Average Queue (ft)	3	5
95th Queue (ft)	23	37
Link Distance (ft)	795	795
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 991: SB US 321/US 321 & NB US 321

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 992: US 321/NB US 321 & SB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 813

Lanes, Volumes, Timings
1: Dudley Shoals



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	10	139	116	75	0	0
Future Volume (vph)	10	139	116	75	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.947			
Flt Protected		0.997				
Satd. Flow (prot)	0	1857	1764	0	0	0
Flt Permitted		0.997				
Satd. Flow (perm)	0	1857	1764	0	0	0
Link Speed (mph)		35	35		55	
Link Distance (ft)		1050	1626		1046	
Travel Time (s)		20.5	31.7		13.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	11	154	129	83	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	165	212	0	0	0
Sign Control		Free	Free		Stop	

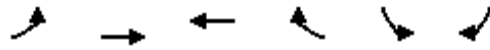
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	18.9%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis

1: Dudley Shoals

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	10	139	116	75	0	0
Future Volume (Veh/h)	10	139	116	75	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	154	129	83	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	212				346	170
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	212				346	170
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				100	100
cM capacity (veh/h)	1358				645	873
Direction, Lane #						
	EB 1	WB 1				
Volume Total	165	212				
Volume Left	11	0				
Volume Right	0	83				
cSH	1358	1700				
Volume to Capacity	0.01	0.12				
Queue Length 95th (ft)	1	0				
Control Delay (s)	0.6	0.0				
Lane LOS	A					
Approach Delay (s)	0.6	0.0				
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			18.9%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings
2: Dudley Shoals

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	273	85	68	71	106	163
Future Volume (vph)	273	85	68	71	106	163
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.968			0.918		
Flt Protected	0.963			0.976		
Satd. Flow (prot)	1736	0	0	1818	1710	0
Flt Permitted	0.963			0.976		
Satd. Flow (perm)	1736	0	0	1818	1710	0
Link Speed (mph)	45			45	45	
Link Distance (ft)	639			746	415	
Travel Time (s)	9.7			11.3	6.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	303	94	76	79	118	181
Shared Lane Traffic (%)						
Lane Group Flow (vph)	397	0	0	155	299	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	53.4% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

2: Dudley Shoals

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	273	85	68	71	106	163
Future Volume (Veh/h)	273	85	68	71	106	163
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	303	94	76	79	118	181
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	440	208	299			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	440	208	299			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	44	89	94			
cM capacity (veh/h)	540	832	1262			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	397	155	299			
Volume Left	303	76	0			
Volume Right	94	0	181			
cSH	589	1262	1700			
Volume to Capacity	0.67	0.06	0.18			
Queue Length 95th (ft)	128	5	0			
Control Delay (s)	22.8	4.2	0.0			
Lane LOS	C	A				
Approach Delay (s)	22.8	4.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			11.4			
Intersection Capacity Utilization		53.4%		ICU Level of Service		A
Analysis Period (min)			15			

Lanes, Volumes, Timings
7: US 321

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	85	0	2170	1384	0
Future Volume (vph)	0	85	0	2170	1384	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Frt	0.865					
Flt Protected						
Satd. Flow (prot)	0	1611	0	3539	3539	0
Flt Permitted						
Satd. Flow (perm)	0	1611	0	3539	3539	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	1046			2666	352	
Travel Time (s)	13.0			33.0	4.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	94	0	2411	1538	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	94	0	2411	1538	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.3%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis
7: US 321

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↕	↕	
Traffic Volume (veh/h)	0	85	0	2170	1384	0
Future Volume (Veh/h)	0	85	0	2170	1384	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	94	0	2411	1538	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2744	769	1538			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2744	769	1538			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	73	100			
cM capacity (veh/h)	16	344	428			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	94	1206	1206	769	769	
Volume Left	0	0	0	0	0	
Volume Right	94	0	0	0	0	
cSH	344	1700	1700	1700	1700	
Volume to Capacity	0.27	0.71	0.71	0.45	0.45	
Queue Length 95th (ft)	27	0	0	0	0	
Control Delay (s)	19.4	0.0	0.0	0.0	0.0	
Lane LOS	C					
Approach Delay (s)	19.4	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			63.3%		ICU Level of Service	B
Analysis Period (min)			15			

Lanes, Volumes, Timings

NCDOT TIP-U4700

131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↗↗		↕↕	↗				↘	↘	
Traffic Volume (vph)	0	0	223	0	1957	230	0	0	0	35	138	0
Future Volume (vph)	0	0	223	0	1957	230	0	0	0	35	138	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	200		0		0	0		0		0	0
Storage Lanes	0	1		0		1	0		0		2	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850						
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2787	0	3471	1583	0	0	0	1770	1770	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2787	0	3471	1583	0	0	0	1770	1770	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55				25
Link Distance (ft)	1199				166			766				230
Travel Time (s)	23.4				2.1			9.5				6.3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	0	0	248	0	2174	256	0	0	0	39	153	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	248	0	2174	256	0	0	0	39	153	0
Turn Type			Prot		NA	Prot				Prot	Prot	
Protected Phases			8		2	2				7	4	
Permitted Phases												
Minimum Initial (s)			7.0		14.0	14.0				7.0	7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	14.0	
Total Split (s)			14.0		62.0	62.0				14.0	28.0	
Total Split (%)			15.6%		68.9%	68.9%				15.6%	31.1%	
Maximum Green (s)			7.0		55.0	55.0				7.0	21.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	5.0	
Lead/Lag			Lag							Lead		
Lead-Lag Optimize?			Yes							Yes		
Vehicle Extension (s)			3.0		3.0	3.0				3.0	3.0	
Recall Mode			None		C-Min	C-Min				None	None	
Act Effect Green (s)			10.9		60.7	60.7				9.0	19.3	
Actuated g/C Ratio			0.12		0.67	0.67				0.10	0.21	
v/c Ratio			0.73		0.93	0.24				0.22	0.40	
Control Delay			53.9		12.1	2.9				35.7	25.4	
Queue Delay			0.0		0.0	0.0				0.0	0.0	
Total Delay			53.9		12.1	2.9				35.7	25.4	
LOS			D		B	A				D	C	
Approach Delay	53.9				11.1							27.5
Approach LOS	D				B							C
Queue Length 50th (ft)			80		527	27				21	74	
Queue Length 95th (ft)			#157		m#751	m28				m34	101	

131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

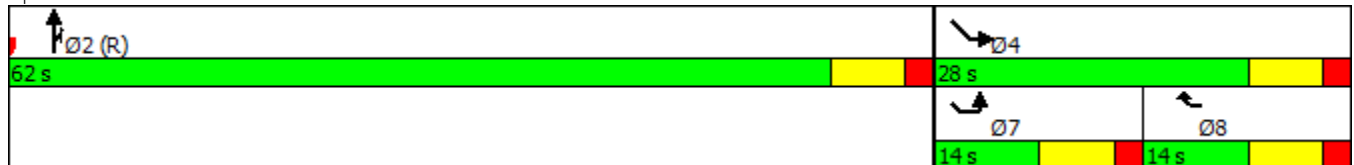


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Internal Link Dist (ft)	1119				86			686			150	
Turn Bay Length (ft)			200									
Base Capacity (vph)			338		2340	1067				177	452	
Starvation Cap Reductn			0		0	0				0	0	
Spillback Cap Reductn			0		0	0				0	0	
Storage Cap Reductn			0		0	0				0	0	
Reduced v/c Ratio			0.73		0.93	0.24				0.22	0.34	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 86 (96%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.93
 Intersection Signal Delay: 15.9 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

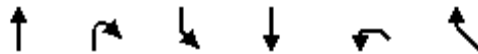


Lanes, Volumes, Timings
 132: SB US 321 & Pinewood Road U-Turn

NCDOT TIP-U4700

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	1425	79	0
Future Volume (vph)	0	0	0	1425	79	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3471	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3471	1736	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	30			55	25	
Link Distance (ft)	587			861	64	
Travel Time (s)	13.3			10.7	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	1583	88	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	1583	88	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				72.0	18.0	
Total Split (%)				80.0%	20.0%	
Maximum Green (s)				65.0	11.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				71.8	12.0	
Actuated g/C Ratio				0.80	0.13	
v/c Ratio				0.57	0.38	
Control Delay				3.2	42.7	
Queue Delay				0.0	0.0	
Total Delay				3.2	42.7	
LOS				A	D	
Approach Delay				3.2	42.7	
Approach LOS				A	D	
Queue Length 50th (ft)				74	53	
Queue Length 95th (ft)				157	m50	
Internal Link Dist (ft)	507			781	1	
Turn Bay Length (ft)						
Base Capacity (vph)				2790	262	

Lanes, Volumes, Timings
 132: SB US 321 & Pinewood Road U-Turn

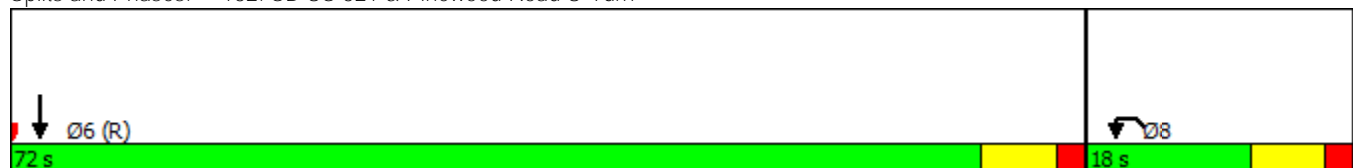


Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.57	0.34	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	32 (36%), Referenced to phase 6:SBT, Start of Green
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.57
Intersection Signal Delay:	5.3
Intersection LOS:	A
Intersection Capacity Utilization	105.9%
ICU Level of Service	G
Analysis Period (min)	15
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 132: SB US 321 & Pinewood Road U-Turn



133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	403	0	0	0	0	1192	139	194	0
Future Volume (vph)	0	0	403	0	0	0	0	1192	139	194	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	300		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2733	0	0	0	0	3471	1553	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2733	0	0	0	0	3471	1553	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	770				688			243		154	
Travel Time (s)	15.0				15.6			3.0		4.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	2%	2%	2%	2%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	448	0	0	0	0	1324	154	216	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	448	0	0	0	0	1324	154	216	0
Turn Type			Prot					NA	Perm	Prot	
Protected Phases			4					6		8	
Permitted Phases									6		
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			31.0					59.0	59.0	31.0	
Total Split (%)			34.4%					65.6%	65.6%	34.4%	
Maximum Green (s)			24.0					52.0	52.0	24.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			21.6					58.4	58.4	21.6	
Actuated g/C Ratio			0.24					0.65	0.65	0.24	
v/c Ratio			0.68					0.59	0.15	0.52	
Control Delay			36.3					5.5	3.2	33.4	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			36.3					5.5	3.2	33.4	
LOS			D					A	A	C	
Approach Delay	36.3							5.2		33.4	
Approach LOS	D							A		C	
Queue Length 50th (ft)			131					102	12	98	
Queue Length 95th (ft)			176					36	12	m96	

133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	690				608			163		74	
Turn Bay Length (ft)			300								
Base Capacity (vph)			790					2251	1007	501	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.57					0.59	0.15	0.43	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 36 (40%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 14.6 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 133: SB US 321 & Pinewood Road Left-Over & Pinewood Road



Lanes, Volumes, Timings
 134: NB US 321 & Pinewood Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	211	0	0	2170	0	0
Future Volume (vph)	211	0	0	2170	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	71			478	600	
Travel Time (s)	1.9			5.9	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	234	0	0	2411	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	234	0	0	2411	0	0
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	19.0			71.0		
Total Split (%)	21.1%			78.9%		
Maximum Green (s)	12.0			64.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	14.0			66.0		
Actuated g/C Ratio	0.16			0.73		
v/c Ratio	0.87			0.95		
Control Delay	61.3			20.8		
Queue Delay	0.0			0.0		
Total Delay	61.3			20.8		
LOS	E			C		
Approach Delay	61.3			20.8		
Approach LOS	E			C		
Queue Length 50th (ft)	137			520		
Queue Length 95th (ft)	#259			#853		
Internal Link Dist (ft)	1			398	520	
Turn Bay Length (ft)						
Base Capacity (vph)	270			2545		

Lanes, Volumes, Timings
 134: NB US 321 & Pinewood Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.87			0.95		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 68 (76%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 24.3 Intersection LOS: C
 Intersection Capacity Utilization 80.0% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 134: NB US 321 & Pinewood Road U-Turn



Lanes, Volumes, Timings
 135: NB US 321 & Pinewood Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	79	2136	0	0
Future Volume (vph)	0	0	79	2136	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	64			766	838	
Travel Time (s)	1.5			9.5	10.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	88	2373	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	88	2373	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.9% ICU Level of Service G
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 136: SB US 321 & Pinewood Road Left-Over

	↑	↖	↙	↓	↘	↗
Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↖↙	↑↑↑		
Traffic Volume (vph)	0	0	173	1331	0	0
Future Volume (vph)	0	0	173	1331	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	100		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			100		100	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3433	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3433	4988	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	243			587	230	
Travel Time (s)	5.5			7.3	2.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%
Adj. Flow (vph)	0	0	192	1479	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	192	1479	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	53.6% ICU Level of Service A
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 137: SB US 321 & Pinewood Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↖	↗↗
Traffic Volume (vph)	0	0	0	0	211	1384
Future Volume (vph)	0	0	0	0	211	1384
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	30		30			55
Link Distance (ft)	71		536			688
Travel Time (s)	1.6		12.2			8.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	234	1538
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	234	1538
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	78.3%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 138: NB US 321 & Pinewood Road Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	194	2187	0	0
Future Volume (vph)	0	0	194	2187	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	154			600	166	
Travel Time (s)	1.9			7.4	2.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	216	2430	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	216	2430	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	125.6%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

NCDOT TIP-U4700

141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations											
Traffic Volume (vph)	0	0	35	0	2017	123	0	0	0	22	0
Future Volume (vph)	0	0	35	0	2017	123	0	0	0	22	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200		0	200		300	0	0
Storage Lanes	0	1		0		0	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt			0.865		0.991						
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	3442	0	0	0	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	3442	0	0	0	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	499				224			926		66	
Travel Time (s)	9.7				2.8			11.5		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	2%	4%	3%	2%	4%	2%	3%	2%
Adj. Flow (vph)	0	0	39	0	2241	137	0	0	0	24	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	39	0	2378	0	0	0	0	24	0
Turn Type			Prot		NA					Prot	
Protected Phases			8		2					4	
Permitted Phases											
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			14.0		76.0					14.0	
Total Split (%)			15.6%		84.4%					15.6%	
Maximum Green (s)			7.0		69.0					7.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			Min		C-Min					None	
Act Effct Green (s)			9.0		71.0					9.0	
Actuated g/C Ratio			0.10		0.79					0.10	
v/c Ratio			0.25		0.88					0.14	
Control Delay			41.6		4.8					43.5	
Queue Delay			0.0		0.0					0.0	
Total Delay			41.6		4.8					43.5	
LOS			D		A					D	
Approach Delay	41.6				4.8					43.5	
Approach LOS	D				A					D	
Queue Length 50th (ft)			21		32					13	
Queue Length 95th (ft)			52		m36					m23	

141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over



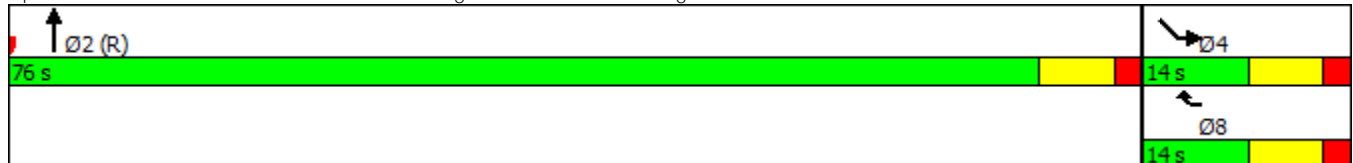
Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	419				144			846		1	
Turn Bay Length (ft)											
Base Capacity (vph)			159		2715					175	
Starvation Cap Reductn			0		0					0	
Spillback Cap Reductn			0		0					0	
Storage Cap Reductn			0		0					0	
Reduced v/c Ratio			0.25		0.88					0.14	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	10 (11%), Referenced to phase 2:NBT, Start of Green
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.88
Intersection Signal Delay:	5.8
Intersection LOS:	A
Intersection Capacity Utilization	73.8%
ICU Level of Service	D
Analysis Period (min)	15

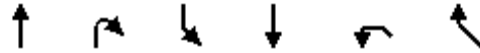
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over



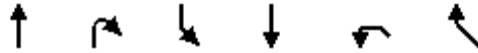
Lanes, Volumes, Timings
 142: SB US 321 & N. Highland Avenue U-Turn

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↗	
Traffic Volume (vph)	0	0	0	1626	30	0
Future Volume (vph)	0	0	0	1626	30	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3471	1736	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3471	1736	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	30			55	25	
Link Distance (ft)	894			2338	46	
Travel Time (s)	20.3			29.0	1.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	2%
Adj. Flow (vph)	0	0	0	1807	33	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	1807	33	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.0	14.0	
Total Split (s)				76.0	14.0	
Total Split (%)				84.4%	15.6%	
Maximum Green (s)				69.0	7.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				77.9	9.7	
Actuated g/C Ratio				0.87	0.11	
v/c Ratio				0.60	0.18	
Control Delay				4.5	31.2	
Queue Delay				0.0	0.0	
Total Delay				4.5	31.2	
LOS				A	C	
Approach Delay				4.5	31.2	
Approach LOS				A	C	
Queue Length 50th (ft)				181	16	
Queue Length 95th (ft)				275	m19	
Internal Link Dist (ft)	814			2258	1	
Turn Bay Length (ft)						
Base Capacity (vph)				3031	187	

Lanes, Volumes, Timings
 142: SB US 321 & N. Highland Avenue U-Turn



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.60	0.18	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 12 (13%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.60
 Intersection Signal Delay: 4.9 Intersection LOS: A
 Intersection Capacity Utilization 108.3% ICU Level of Service G
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 142: SB US 321 & N. Highland Avenue U-Turn



143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	41	0	0	0	0	1453	181	51	0
Future Volume (vph)	0	0	41	0	0	0	0	1453	181	51	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00
Fr _t			0.865					0.983			
Fl _t Protected										0.950	
Satd. Flow (prot)	0	0	1611	0	0	0	0	3419	0	1770	0
Fl _t Permitted										0.950	
Satd. Flow (perm)	0	0	1611	0	0	0	0	3419	0	1770	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	804				861			210		86	
Travel Time (s)	15.7				19.6			2.6		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	4%	4%	2%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	46	0	0	0	0	1614	201	57	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	46	0	0	0	0	1815	0	57	0
Turn Type			Prot					NA		Prot	
Protected Phases			4					6		8	
Permitted Phases			4								
Minimum Initial (s)			7.0					14.0		7.0	
Minimum Split (s)			14.0					21.0		14.0	
Total Split (s)			14.0					76.0		14.0	
Total Split (%)			15.6%					84.4%		15.6%	
Maximum Green (s)			7.0					69.0		7.0	
Yellow Time (s)			5.0					5.0		5.0	
All-Red Time (s)			2.0					2.0		2.0	
Total Lost Time (s)			5.0					5.0		5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0		3.0	
Recall Mode			None					C-Max		Min	
Act Effect Green (s)			9.0					71.0		9.0	
Actuated g/C Ratio			0.10					0.79		0.10	
v/c Ratio			0.29					0.67		0.32	
Control Delay			42.6					3.7		36.2	
Queue Delay			0.0					0.0		0.0	
Total Delay			42.6					3.7		36.2	
LOS			D					A		D	
Approach Delay	42.6							3.7		36.2	
Approach LOS	D							A		D	
Queue Length 50th (ft)			25					11		33	
Queue Length 95th (ft)			59					165		m39	
Internal Link Dist (ft)	724				781			130		6	
Turn Bay Length (ft)											
Base Capacity (vph)			161					2697		177	

143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Starvation Cap Reductn			0					0		0	
Spillback Cap Reductn			0					0		0	
Storage Cap Reductn			0					0		0	
Reduced v/c Ratio			0.29					0.67		0.32	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	88 (98%), Referenced to phase 6:SBT, Start of Green
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.67
Intersection Signal Delay:	5.6
Intersection LOS:	A
Intersection Capacity Utilization	60.1%
ICU Level of Service	B
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue



Lanes, Volumes, Timings
 145: NB US 321 & N. Highland Avenue U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	30	2022	0	0
Future Volume (vph)	0	0	30	2022	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	46			926	2360	
Travel Time (s)	1.0			11.5	29.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	33	2247	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	33	2247	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	108.3%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 146: SB US 321 & N. Highland Avenue Left-Over



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	22	1634
Future Volume (vph)	0	0	0	0	22	1634
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	50	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3471
Link Speed (mph)	30		30			55
Link Distance (ft)	66		210			894
Travel Time (s)	1.5		4.8			11.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	4%
Adj. Flow (vph)	0	0	0	0	24	1816
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	24	1816
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	59.1% ICU Level of Service B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 148: NB US 321 & N. Highland Avenue Left-Over

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	51	2140	0	0
Future Volume (vph)	0	0	51	2140	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	50			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1770	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1770	3471	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	86			838	224	
Travel Time (s)	2.0			10.4	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%
Adj. Flow (vph)	0	0	57	2378	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	57	2378	0	0
Sign Control	Stop			Free	Stop	

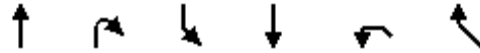
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.5%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 991: SB US 321/US 321 & NB US 321

NCDOT TIP-U4700



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑		↑↑
Traffic Volume (vph)	0	0	0	1626	0	2022
Future Volume (vph)	0	0	0	1626	0	2022
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3471	0	2733
Flt Permitted						
Satd. Flow (perm)	0	0	0	3471	0	2733
Link Speed (mph)	55			55	55	
Link Distance (ft)	2338			158	2360	
Travel Time (s)	29.0			2.0	29.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	4%	2%	4%
Adj. Flow (vph)	0	0	0	1807	0	2247
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	1807	0	2247
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	74.1% ICU Level of Service D
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 992: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑				↑↑
Traffic Volume (vph)	0	2170	0	0	0	1384
Future Volume (vph)	0	2170	0	0	0	1384
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	3471	0	0	0	2733
Flt Permitted						
Satd. Flow (perm)	0	3471	0	0	0	2733
Link Speed (mph)		55	55		55	
Link Distance (ft)		352	478		536	
Travel Time (s)		4.4	5.9		6.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	2%	4%
Adj. Flow (vph)	0	2411	0	0	0	1538
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2411	0	0	0	1538
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.3%
ICU Level of Service	B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	4814
Vehs Exited	4820
Starting Vehs	205
Ending Vehs	199
Travel Distance (mi)	7189
Travel Time (hr)	216.0
Total Delay (hr)	70.7
Total Stops	3862
Fuel Used (gal)	266.5

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	4814
Vehs Exited	4820
Starting Vehs	205
Ending Vehs	199
Travel Distance (mi)	7189
Travel Time (hr)	216.0
Total Delay (hr)	70.7
Total Stops	3862
Fuel Used (gal)	266.5

Intersection: 1: Dudley Shoals

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 2: Dudley Shoals

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	146	54	22
Average Queue (ft)	76	16	1
95th Queue (ft)	132	46	7
Link Distance (ft)	582	694	380
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 7: US 321

Movement	EB
Directions Served	R
Maximum Queue (ft)	94
Average Queue (ft)	29
95th Queue (ft)	57
Link Distance (ft)	970
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 131: NB US 321 & Pinewood Road & Pinewood Road Left-Over

Movement	WB	WB	NB	NB	SE	SE
Directions Served	>	>	T	T	<	L
Maximum Queue (ft)	139	126	177	161	94	156
Average Queue (ft)	64	68	104	112	33	80
95th Queue (ft)	105	108	165	171	77	147
Link Distance (ft)	1112		82	82	117	117
Upstream Blk Time (%)			9	13		3
Queuing Penalty (veh)			65	92		3
Storage Bay Dist (ft)		200				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 132: SB US 321 & Pinewood Road U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	150	157	103
Average Queue (ft)	40	47	55
95th Queue (ft)	103	109	98
Link Distance (ft)	742	742	22
Upstream Blk Time (%)			37
Queuing Penalty (veh)			29
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 133: SB US 321 & Pinewood Road Left-Over & Pinewood Road

Movement	EB	EB	SB	SB	NW
Directions Served	>	>	T	T	L
Maximum Queue (ft)	256	224	222	223	188
Average Queue (ft)	171	115	103	110	110
95th Queue (ft)	259	241	189	187	158
Link Distance (ft)	696		108	108	65
Upstream Blk Time (%)			6	6	36
Queuing Penalty (veh)			26	28	70
Storage Bay Dist (ft)		300			
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 134: NB US 321 & Pinewood Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	100	312	342
Average Queue (ft)	77	170	192
95th Queue (ft)	86	277	311
Link Distance (ft)	19	432	432
Upstream Blk Time (%)	71		
Queuing Penalty (veh)	151		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 135: NB US 321 & Pinewood Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	54
Average Queue (ft)	8
95th Queue (ft)	35
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	150
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 136: SB US 321 & Pinewood Road Left-Over

Movement	SB	SB	SB
Directions Served	T	T	T
Maximum Queue (ft)	46	40	68
Average Queue (ft)	2	1	9
95th Queue (ft)	18	13	40
Link Distance (ft)	536	536	536
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 137: SB US 321 & Pinewood Road U-Turn

Movement	SB	SB
Directions Served	L	T
Maximum Queue (ft)	246	235
Average Queue (ft)	78	8
95th Queue (ft)	180	78
Link Distance (ft)		653
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	250	
Storage Blk Time (%)	0	0
Queuing Penalty (veh)	1	0

Intersection: 138: NB US 321 & Pinewood Road Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	45	35	56	21
Average Queue (ft)	6	2	6	1
95th Queue (ft)	28	13	32	7
Link Distance (ft)		545	545	545
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	200			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 141: NB US 321 & N. Highland Avenue & N. Highland Avenue Left-Over

Movement	WB	NB	NB	SE
Directions Served	>	T	TR	L
Maximum Queue (ft)	72	204	222	44
Average Queue (ft)	25	121	118	18
95th Queue (ft)	62	191	202	38
Link Distance (ft)	452	163	163	6
Upstream Blk Time (%)		1	2	23
Queuing Penalty (veh)		11	17	5
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 142: SB US 321 & N. Highland Avenue U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	151	115	51
Average Queue (ft)	40	41	20
95th Queue (ft)	109	107	45
Link Distance (ft)	2297	2297	2
Upstream Blk Time (%)			18
Queuing Penalty (veh)			5
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 143: SB US 321 & N. Highland Avenue Left-Over & N. Highland Avenue

Movement	EB	SB	SB	NW
Directions Served	>	T	TR	L
Maximum Queue (ft)	84	151	156	94
Average Queue (ft)	24	78	81	41
95th Queue (ft)	61	145	144	86
Link Distance (ft)	716	157	157	22
Upstream Blk Time (%)		0	0	34
Queuing Penalty (veh)		1	1	18
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 145: NB US 321 & N. Highland Avenue U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 146: SB US 321 & N. Highland Avenue Left-Over

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 148: NB US 321 & N. Highland Avenue Left-Over

Movement	NB	NB
Directions Served	L	T
Maximum Queue (ft)	58	116
Average Queue (ft)	4	4
95th Queue (ft)	25	38
Link Distance (ft)		795
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	50	
Storage Blk Time (%)	1	
Queuing Penalty (veh)	9	

Intersection: 991: SB US 321/US 321 & NB US 321

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 992: US 321/NB US 321 & SB US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 531

APPENDIX M

2040 Build Alternative Dudley Shoals Road – Freeway Ramp Reports

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 SB
Agency or Company	AECOM	Junction	201 - Dudley Shoals Ramp
Date Performed	3/27/2017	Jurisdiction	NCDOT
Analysis Time Period	AM Peak Hour	Analysis Year	2040

Project Description U-4700 US 321

Inputs			
Upstream Adj Ramp	Freeway Number of Lanes, N	2	Downstream Adj Ramp
<input type="checkbox"/> Yes <input type="checkbox"/> On	Ramp Number of Lanes, N	1	<input type="checkbox"/> Yes <input type="checkbox"/> On
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	Acceleration Lane Length, L _A	500	<input checked="" type="checkbox"/> No <input type="checkbox"/> Off
L _{up} = ft	Deceleration Lane Length L _D		L _{down} = ft
V _u = veh/h	Freeway Volume, V _F	2058	V _D = veh/h
	Ramp Volume, V _R	122	
	Freeway Free-Flow Speed, S _{FF}	55.0	
	Ramp Free-Flow Speed, S _{FR}	25.0	

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	2058	0.90	Rolling	4	0	0.943	1.00	2424
Ramp	122	0.90	Rolling	4	0	0.943	1.00	144
UpStream								
DownStream								

Merge Areas	Diverge Areas
-------------	---------------

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 2424 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)	$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)
--	---

Capacity Checks

	Actual		Capacity		LOS F?
V _{FO}	2568	Exhibit 13-8	4600:All	No	V _F
					V _{FO} = V _F - V _R
					V _R

Flow Entering Merge Influence Area	Flow Entering Diverge Influence Area
------------------------------------	--------------------------------------

	Actual	Max Desirable	Violation?		Actual	Max Desirable	Violation?
V _{R12}	2568	Exhibit 13-8	4600:All	No	V ₁₂	Exhibit 13-8	

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 22.3 (pc/mi/ln) LOS = C (Exhibit 13-2)	$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)
--	---

Speed Determination

M _s = 0.347 (Exhibit 13-11) S _R = 50.5 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 50.5 mph (Exhibit 13-13)	D _s = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)
---	---

RAMPS AND RAMP JUNCTIONS WORKSHEET

General Information		Site Information	
Analyst	AECOM	Freeway/Dir of Travel	US 321 SB
Agency or Company	AECOM	Junction	201 - Dudley Shoals Ramp
Date Performed	3/27/2017	Jurisdiction	NCDOT
Analysis Time Period	PM Peak Hour	Analysis Year	2040

Project Description U-4700 US 321

Inputs

Upstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{up} = ft V _u = veh/h	Freeway Number of Lanes, N 2 Ramp Number of Lanes, N 1 Acceleration Lane Length, L _A 500 Deceleration Lane Length L _D Freeway Volume, V _F 1384 Ramp Volume, V _R 85 Freeway Free-Flow Speed, S _{FF} 55.0 Ramp Free-Flow Speed, S _{FR} 25.0	Downstream Adj Ramp <input type="checkbox"/> Yes <input type="checkbox"/> On <input checked="" type="checkbox"/> No <input type="checkbox"/> Off L _{down} = ft V _D = veh/h
--	--	--

Conversion to pc/h Under Base Conditions

(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p
Freeway	1384	0.90	Rolling	4	0	0.943	1.00	1630
Ramp	85	0.90	Rolling	4	0	0.943	1.00	100
UpStream								
DownStream								

Merge Areas

Diverge Areas

Estimation of v₁₂

$V_{12} = V_F (P_{FM})$
 (Equation 13-6 or 13-7)
 P_{FM} = 1.000 using Equation (Exhibit 13-6)
 V₁₂ = 1630 pc/h
 V₃ or V_{av34} = 0 pc/h (Equation 13-14 or 13-17)
 Is V₃ or V_{av34} > 2,700 pc/h? Yes No
 Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
 If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Estimation of v₁₂

$V_{12} = V_R + (V_F - V_R)P_{FD}$
 (Equation 13-12 or 13-13)
 P_{FD} = using Equation (Exhibit 13-7)
 V₁₂ = pc/h
 V₃ or V_{av34} = pc/h (Equation 13-14 or 13-17)
 Is V₃ or V_{av34} > 2,700 pc/h? Yes No
 Is V₃ or V_{av34} > 1.5 * V₁₂/2 Yes No
 If Yes, V_{12a} = pc/h (Equation 13-16, 13-18, or 13-19)

Capacity Checks

	Actual	Capacity	LOS F?
V _{FO}	1730	Exhibit 13-8	No

Capacity Checks

	Actual	Capacity	LOS F?
V _F		Exhibit 13-8	
V _{FO} = V _F - V _R		Exhibit 13-8	
V _R		Exhibit 13-10	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
V _{R12}	1730	Exhibit 13-8	4600:All No

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
V ₁₂		Exhibit 13-8	

Level of Service Determination (if not F)

$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$
 D_R = 15.8 (pc/mi/ln)
 LOS = B (Exhibit 13-2)

Level of Service Determination (if not F)

$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$
 D_R = (pc/mi/ln)
 LOS = (Exhibit 13-2)

Speed Determination

M_S = 0.318 (Exhibit 13-11)
 S_R = 50.9 mph (Exhibit 13-11)
 S₀ = N/A mph (Exhibit 13-11)
 S = 50.9 mph (Exhibit 13-13)

Speed Determination

D_S = (Exhibit 13-12)
 S_R = mph (Exhibit 13-12)
 S₀ = mph (Exhibit 13-12)
 S = mph (Exhibit 13-13)

APPENDIX N

**2040 Build Alternative Lower Cedar Valley Road & Pine Mountain
Road Synchro & SimTraffic Reports**

Lanes, Volumes, Timings

NCDOT TIP-U4700

161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↔↔		↑↑↑						↔	
Traffic Volume (vph)	0	0	254	0	1556	52	0	0	0	17	82	0
Future Volume (vph)	0	0	254	0	1556	52	0	0	0	17	82	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		0		0	0		0		0	0
Storage Lanes	0	1		0		0	0		0		1	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.995							
Flt Protected											0.950	
Satd. Flow (prot)	0	0	2787	0	4963	0	0	0	0	0	1752	0
Flt Permitted											0.950	
Satd. Flow (perm)	0	0	2787	0	4963	0	0	0	0	0	1752	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55			25	
Link Distance (ft)	760				118			761			116	
Travel Time (s)	14.8				1.5			9.4			3.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	3%	4%	4%	2%	2%	2%	3%	3%	3%
Adj. Flow (vph)	0	0	282	0	1729	58	0	0	0	19	91	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	282	0	1787	0	0	0	0	0	110	0
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			8		2					7	7	
Permitted Phases												
Minimum Initial (s)			7.0		14.0					7.0	7.0	
Minimum Split (s)			14.0		21.0					14.0	14.0	
Total Split (s)			21.0		51.0					18.0	18.0	
Total Split (%)			23.3%		56.7%					20.0%	20.0%	
Maximum Green (s)			14.0		44.0					11.0	11.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Total Lost Time (s)			5.0		5.0						5.0	
Lead/Lag			Lag							Lead	Lead	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effct Green (s)			15.0		51.0						11.8	
Actuated g/C Ratio			0.17		0.57						0.13	
v/c Ratio			0.61		0.64						0.48	
Control Delay			40.7		11.7						43.4	
Queue Delay			0.0		0.0						0.0	
Total Delay			40.7		11.7						43.4	
LOS			D		B						D	
Approach Delay	40.7				11.7						43.4	
Approach LOS	D				B						D	
Queue Length 50th (ft)			83		196						54	
Queue Length 95th (ft)			129		182						m62	

161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Internal Link Dist (ft)	680				38			681				36
Turn Bay Length (ft)			150									
Base Capacity (vph)			497		2815							253
Starvation Cap Reductn			0		0							0
Spillback Cap Reductn			0		0							0
Storage Cap Reductn			0		0							0
Reduced v/c Ratio			0.57		0.63							0.43

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 30 (33%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 17.1 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lanes, Volumes, Timings
 162: SB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	204	0	0	0	0	2087
Future Volume (vph)	204	0	0	0	0	2087
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	0	0	3505
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	0	0	3505
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		30			55
Link Distance (ft)	69		391			709
Travel Time (s)	1.9		8.9			8.8
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	2%	2%	3%
Adj. Flow (vph)	227	0	0	0	0	2319
Shared Lane Traffic (%)						
Lane Group Flow (vph)	227	0	0	0	0	2319
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Minimum Initial (s)	7.0					14.0
Minimum Split (s)	14.0					21.0
Total Split (s)	20.0					70.0
Total Split (%)	22.2%					77.8%
Maximum Green (s)	13.0					63.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	14.7					65.3
Actuated g/C Ratio	0.16					0.73
v/c Ratio	0.79					0.91
Control Delay	58.2					10.0
Queue Delay	0.0					0.0
Total Delay	58.2					10.0
LOS	E					A
Approach Delay	58.2					10.0
Approach LOS	E					A
Queue Length 50th (ft)	108					180
Queue Length 95th (ft)	#232					#373
Internal Link Dist (ft)	1		311			629
Turn Bay Length (ft)						
Base Capacity (vph)	292					2543

Lanes, Volumes, Timings
 162: SB US 321 & Lower Cedar Valley Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Starvation Cap Reductn	0					0
Spillback Cap Reductn	0					0
Storage Cap Reductn	0					0
Reduced v/c Ratio	0.78					0.91

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 58 (64%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.91
 Intersection Signal Delay: 14.3 Intersection LOS: B
 Intersection Capacity Utilization 111.0% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

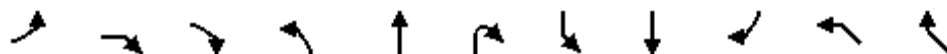
Splits and Phases: 162: SB US 321 & Lower Cedar Valley Road U-Turn



Lanes, Volumes, Timings

NCDOT TIP-U4700

163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	265	0	0	0	0	1826	320	164	0
Future Volume (vph)	0	0	265	0	0	0	0	1826	320	164	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3505	1568	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3505	1568	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	661				1107			120		112	
Travel Time (s)	12.9				13.7			1.5		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%	2%	3%	3%	4%	3%
Adj. Flow (vph)	0	0	294	0	0	0	0	2029	356	182	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	294	0	0	0	0	2029	356	182	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			21.0					69.0	69.0	21.0	
Total Split (%)			23.3%					76.7%	76.7%	23.3%	
Maximum Green (s)			14.0					62.0	62.0	14.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			15.1					64.9	64.9	15.1	
Actuated g/C Ratio			0.17					0.72	0.72	0.17	
v/c Ratio			0.64					0.80	0.31	0.63	
Control Delay			41.6					4.1	2.0	40.2	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			41.6					4.1	2.0	40.2	
LOS			D					A	A	D	
Approach Delay	41.6							3.8		40.2	
Approach LOS	D							A		D	
Queue Length 50th (ft)			88					65	22	89	
Queue Length 95th (ft)			134					m77	m25	m132	

163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	581				1027			40		32	
Turn Bay Length (ft)			250								
Base Capacity (vph)			490					2529	1131	308	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.60					0.80	0.31	0.59	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 72 (80%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 10.0 Intersection LOS: A
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lanes, Volumes, Timings
 164: NB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	157	0	0	1615	0	0
Future Volume (vph)	157	0	0	1615	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	56			1125	1035	
Travel Time (s)	1.5			13.9	12.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	174	0	0	1794	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	174	0	0	1794	0	0
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	22.0			68.0		
Total Split (%)	24.4%			75.6%		
Maximum Green (s)	15.0			61.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	15.3			64.7		
Actuated g/C Ratio	0.17			0.72		
v/c Ratio	0.59			0.72		
Control Delay	43.0			5.4		
Queue Delay	0.0			0.0		
Total Delay	43.0			5.4		
LOS	D			A		
Approach Delay	43.0			5.4		
Approach LOS	D			A		
Queue Length 50th (ft)	92			149		
Queue Length 95th (ft)	m113			143		
Internal Link Dist (ft)	1			1045	955	
Turn Bay Length (ft)						
Base Capacity (vph)	332			2506		

Lanes, Volumes, Timings
 164: NB US 321 & Lower Cedar Valley Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.52			0.72		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 25 (28%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.72
 Intersection Signal Delay: 8.8
 Intersection LOS: A
 Intersection Capacity Utilization 61.7%
 ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 164: NB US 321 & Lower Cedar Valley Road U-Turn



Lanes, Volumes, Timings
 165: NB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	204	1659	0	0
Future Volume (vph)	0	0	204	1659	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	3505	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	3505	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	69			282	760	
Travel Time (s)	0.9			3.5	9.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	227	1843	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	227	1843	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	111.0%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 166: Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↑↑↑
Traffic Volume (vph)	0	0	0	0	99	2146
Future Volume (vph)	0	0	0	0	99	2146
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	5036
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	5036
Link Speed (mph)	25		55			55
Link Distance (ft)	116		120			594
Travel Time (s)	3.2		1.5			7.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	110	2384
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	110	2384
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	44.8%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 166: Lower Cedar Valley Road Left-Over

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↘	↑↑↑
Traffic Volume (veh/h)	0	0	0	0	99	2146
Future Volume (Veh/h)	0	0	0	0	99	2146
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	0	110	2384
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage veh)						
Upstream signal (ft)			120		985	
pX, platoon unblocked						
vC, conflicting volume	0.55				1015	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0		0		0	
tC, single (s)	6.8		6.9		4.2	
tC, 2 stage (s)						
tF (s)	3.5		3.3		2.2	
p0 queue free %	100		100		93	
cM capacity (veh/h)	529		1084		1614	
Direction, Lane #	SB 1	SB 2	SB 3	SB 4		
Volume Total	110	795	795	795		
Volume Left	110	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.07	0.47	0.47	0.47		
Queue Length 95th (ft)	5	0	0	0		
Control Delay (s)	7.4	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.3					
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			44.8%		ICU Level of Service	
Analysis Period (min)			15			
			A			

Lanes, Volumes, Timings
 167: SB US 321 & Lower Cedar Valley Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	157	1934
Future Volume (vph)	0	0	0	0	157	1934
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	300	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	55		55			55
Link Distance (ft)	56		1159			1107
Travel Time (s)	0.7		14.4			13.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	174	2149
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	174	2149
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	61.7%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 168: NB US 321 & Lower Cedar Valley Road Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	164	1608	0	0
Future Volume (vph)	0	0	164	1608	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	112			1035	118	
Travel Time (s)	1.4			12.8	1.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	182	1787	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	182	1787	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	96.1% ICU Level of Service F
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 171: NB US 321 & Quarry Estates Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	63	1800	27	0	0
Future Volume (vph)	0	63	1800	27	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Frt		0.865	0.998			
Flt Protected						
Satd. Flow (prot)	0	1611	5026	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1611	5026	0	0	0
Link Speed (mph)	35		55			55
Link Distance (ft)	468		761			282
Travel Time (s)	9.1		9.4			3.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	4%	4%
Adj. Flow (vph)	0	70	2000	30	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	70	2030	0	0	0
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	45.9%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 171: NB US 321 & Quarry Estates Road

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	63	1800	27	0	0
Future Volume (Veh/h)	0	63	1800	27	0	0
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	70	2000	30	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			761			
pX, platoon unblocked	0.75	0.75			0.75	
vC, conflicting volume	2015	682			2030	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1189	0			1209	
tC, single (s)	6.8	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	91			100	
cM capacity (veh/h)	136	814			422	

Direction, Lane #	WB 1	NB 1	NB 2	NB 3
Volume Total	70	800	800	430
Volume Left	0	0	0	0
Volume Right	70	0	0	30
cSH	814	1700	1700	1700
Volume to Capacity	0.09	0.47	0.47	0.25
Queue Length 95th (ft)	7	0	0	0
Control Delay (s)	9.8	0.0	0.0	0.0
Lane LOS	A			
Approach Delay (s)	9.8	0.0		
Approach LOS	A			

Intersection Summary			
Average Delay		0.3	
Intersection Capacity Utilization		45.9%	ICU Level of Service
Analysis Period (min)		15	A

Lanes, Volumes, Timings
 172: Quarry Estates Road & SB US 321



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	15	0	0	2230	61
Future Volume (vph)	0	15	0	0	2230	61
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	0			250
Storage Lanes	0	1	0			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	0.91	0.91
Frt		0.865			0.996	
Flt Protected						
Satd. Flow (prot)	0	1580	0	0	5016	0
Flt Permitted						
Satd. Flow (perm)	0	1580	0	0	5016	0
Link Speed (mph)	35			55	55	
Link Distance (ft)	724			594	391	
Travel Time (s)	14.1			7.4	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	3%	3%
Adj. Flow (vph)	0	17	0	0	2478	68
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	17	0	0	2546	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	54.4%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 172: Quarry Estates Road & SB US 321

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑	↑↑↑↘	
Traffic Volume (veh/h)	0	15	0	0	2230	61
Future Volume (Veh/h)	0	15	0	0	2230	61
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	17	0	0	2478	68
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
				None	None	
Median storage veh						
Upstream signal (ft)						
				714	391	
pX, platoon unblocked	0.49	0.49	0.49			
vC, conflicting volume	2512	860	2546			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	438	0	508			
tC, single (s)	6.8	7.0	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	97	100			
cM capacity (veh/h)	268	528	516			
Direction, Lane #	EB 1	SB 1	SB 2	SB 3		
Volume Total	17	991	991	564		
Volume Left	0	0	0	0		
Volume Right	17	0	0	68		
cSH	528	1700	1700	1700		
Volume to Capacity	0.03	0.58	0.58	0.33		
Queue Length 95th (ft)	2	0	0	0		
Control Delay (s)	12.1	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	12.1	0.0				
Approach LOS	B					
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			54.4%		ICU Level of Service	A
Analysis Period (min)			15			

181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over

Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↕↕	↗				↘	
Traffic Volume (vph)	0	0	515	0	1487	250	0	0	0	194	0
Future Volume (vph)	0	0	515	0	1487	250	0	0	0	194	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	400		0		0	400		0	0	0
Storage Lanes	0	1		0		1	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	3505	1568	0	0	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	3505	1568	0	0	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	1087				138			816		103	
Travel Time (s)	21.2				1.7			10.1		2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	3%	3%	3%	4%	4%	2%	3%	2%
Adj. Flow (vph)	0	0	572	0	1652	278	0	0	0	216	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	572	0	1652	278	0	0	0	216	0
Turn Type			Prot		NA	Free				Prot	
Protected Phases			4		2					4	
Permitted Phases						Free					
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			32.0		58.0					32.0	
Total Split (%)			35.6%		64.4%					35.6%	
Maximum Green (s)			25.0		51.0					25.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effect Green (s)			24.5		55.5	90.0				24.5	
Actuated g/C Ratio			0.27		0.62	1.00				0.27	
v/c Ratio			0.75		0.76	0.18				0.45	
Control Delay			36.6		7.0	0.2				29.1	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			36.6		7.0	0.2				29.1	
LOS			D		A	A				C	
Approach Delay	36.6				6.1					29.1	
Approach LOS	D				A					C	
Queue Length 50th (ft)			164		142	0				116	
Queue Length 95th (ft)			226		148	m0				m130	

181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	1007				58			736		23	
Turn Bay Length (ft)			400								
Base Capacity (vph)			836		2160	1568				525	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.68		0.76	0.18				0.41	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 8 (9%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.76
 Intersection Signal Delay: 14.3 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over



Lanes, Volumes, Timings
 182: SB US 321 & Pine Mountain Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	287	0	0	0	0	1990
Future Volume (vph)	287	0	0	0	0	1990
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	0	0	3505
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	0	0	3505
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		55			55
Link Distance (ft)	63		255			1015
Travel Time (s)	1.7		3.2			12.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	2%	2%	3%
Adj. Flow (vph)	319	0	0	0	0	2211
Shared Lane Traffic (%)						
Lane Group Flow (vph)	319	0	0	0	0	2211
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Minimum Initial (s)	7.0					14.0
Minimum Split (s)	14.0					21.0
Total Split (s)	25.0					65.0
Total Split (%)	27.8%					72.2%
Maximum Green (s)	18.0					58.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	19.4					60.6
Actuated g/C Ratio	0.22					0.67
v/c Ratio	0.84					0.94
Control Delay	49.9					23.0
Queue Delay	0.0					0.0
Total Delay	49.9					23.0
LOS	D					C
Approach Delay	49.9					23.0
Approach LOS	D					C
Queue Length 50th (ft)	178					525
Queue Length 95th (ft)	m#279					#793
Internal Link Dist (ft)	1		175			935
Turn Bay Length (ft)						
Base Capacity (vph)	389					2358

Lanes, Volumes, Timings
 182: SB US 321 & Pine Mountain Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Starvation Cap Reductn	0					0
Spillback Cap Reductn	0					0
Storage Cap Reductn	0					0
Reduced v/c Ratio	0.82					0.94

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 89 (99%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 26.4 Intersection LOS: C
 Intersection Capacity Utilization 109.9% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 182: SB US 321 & Pine Mountain Road U-Turn



183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑		↑	
Traffic Volume (vph)	0	0	400	0	0	0	0	1904	0	102	0
Future Volume (vph)	0	0	400	0	0	0	0	1904	0	102	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	200		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850								
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	0	0	0	3505	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	0	0	0	3505	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	538				984			106		137	
Travel Time (s)	10.5				12.2			1.3		3.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	3%	2%
Adj. Flow (vph)	0	0	444	0	0	0	0	2116	0	113	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	444	0	0	0	0	2116	0	113	0
Turn Type			Perm					NA		Prot	
Protected Phases								6		8	
Permitted Phases			4								
Minimum Initial (s)			7.0					14.0		7.0	
Minimum Split (s)			14.0					21.0		14.0	
Total Split (s)			23.0					67.0		23.0	
Total Split (%)			25.6%					74.4%		25.6%	
Maximum Green (s)			16.0					60.0		16.0	
Yellow Time (s)			5.0					5.0		5.0	
All-Red Time (s)			2.0					2.0		2.0	
Total Lost Time (s)			5.0					5.0		5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0		3.0	
Recall Mode			None					C-Min		None	
Act Effct Green (s)			17.7					62.3		17.7	
Actuated g/C Ratio			0.20					0.69		0.20	
v/c Ratio			0.81					0.87		0.33	
Control Delay			47.6					6.2		39.6	
Queue Delay			0.0					0.0		0.0	
Total Delay			47.6					6.2		39.6	
LOS			D					A		D	
Approach Delay	47.6							6.2		39.6	
Approach LOS	D							A		D	
Queue Length 50th (ft)			137					92		66	
Queue Length 95th (ft)			#215					m101		m87	

183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	458				904			26		57	
Turn Bay Length (ft)			200								
Base Capacity (vph)			557					2426		350	
Starvation Cap Reductn			0					0		0	
Spillback Cap Reductn			0					0		0	
Storage Cap Reductn			0					0		0	
Reduced v/c Ratio			0.80					0.87		0.32	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 7 (8%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.87
 Intersection Signal Delay: 14.5 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lanes, Volumes, Timings
 184: NB US 321 & Pine Mountain Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	225	0	0	1614	0	0
Future Volume (vph)	225	0	0	1614	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	3505	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	3505	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	70			834	917	
Travel Time (s)	1.9			10.3	11.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	3%	2%	2%
Adj. Flow (vph)	250	0	0	1793	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	250	0	0	1793	0	0
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	26.0			64.0		
Total Split (%)	28.9%			71.1%		
Maximum Green (s)	19.0			57.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	18.6			61.4		
Actuated g/C Ratio	0.21			0.68		
v/c Ratio	0.69			0.75		
Control Delay	32.9			5.0		
Queue Delay	0.0			0.0		
Total Delay	32.9			5.0		
LOS	C			A		
Approach Delay	32.9			5.0		
Approach LOS	C			A		
Queue Length 50th (ft)	125			57		
Queue Length 95th (ft)	m145			97		
Internal Link Dist (ft)	1			754	837	
Turn Bay Length (ft)						
Base Capacity (vph)	408			2391		

Lanes, Volumes, Timings
 184: NB US 321 & Pine Mountain Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.61			0.75		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 84 (93%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.75
 Intersection Signal Delay: 8.4 Intersection LOS: A
 Intersection Capacity Utilization 65.4% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 184: NB US 321 & Pine Mountain Road U-Turn



Lanes, Volumes, Timings
 185: Pine Mountain Road & Ramp to Pine Mountain Road



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑			↗
Traffic Volume (vph)	0	400	102	0	0	179
Future Volume (vph)	0	400	102	0	0	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	1863	1863	0	0	1596
Flt Permitted						
Satd. Flow (perm)	0	1863	1863	0	0	1596
Link Speed (mph)		35	35		35	
Link Distance (ft)		566	538		146	
Travel Time (s)		11.0	10.5		2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	3%
Adj. Flow (vph)	0	444	113	0	0	199
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	444	113	0	0	199
Sign Control		Free	Free		Yield	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	24.4%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 185: Pine Mountain Road & Ramp to Pine Mountain Road

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑			↗
Traffic Volume (veh/h)	0	400	102	0	0	179
Future Volume (Veh/h)	0	400	102	0	0	179
Sign Control		Free	Free		Yield	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	444	113	0	0	199
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)			538			
pX, platoon unblocked						
vC, conflicting volume	113				557	113
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	113				557	113
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	79
cM capacity (veh/h)	1476				491	937
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	444	113	199			
Volume Left	0	0	0			
Volume Right	0	0	199			
cSH	1700	1700	937			
Volume to Capacity	0.26	0.07	0.21			
Queue Length 95th (ft)	0	0	20			
Control Delay (s)	0.0	0.0	9.9			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.9			
Approach LOS			A			
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utilization			24.4%	ICU Level of Service		A
Analysis Period (min)			15			

Lanes, Volumes, Timings
 186: NB US 321 & Pine Mountain Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	287	1715	0	0
Future Volume (vph)	0	0	287	1715	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	300			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	3505	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	3505	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	63			816	983	
Travel Time (s)	0.8			10.1	12.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	319	1906	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	319	1906	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	109.9%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 187: SB US 321 & Pine Mountain Road Left-Over



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	194	1904
Future Volume (vph)	0	0	0	0	194	1904
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3505
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3505
Link Speed (mph)	55		55			55
Link Distance (ft)	103		106			502
Travel Time (s)	1.3		1.3			6.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	216	2116
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	216	2116
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	61.3%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 188: SB US 321 & Pine Mountain Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	225	2079
Future Volume (vph)	0	0	0	0	225	2079
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3505
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3505
Link Speed (mph)	55		55			55
Link Distance (ft)	70		894			984
Travel Time (s)	0.9		11.1			12.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	250	2310
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	250	2310
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	65.4%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 189: NB US 321 & Pine Mountain Road Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	102	1737	0	0
Future Volume (vph)	0	0	102	1737	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	5036	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	5036	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	137			917	138	
Travel Time (s)	3.7			11.4	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	113	1930	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	113	1930	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	102.3%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 950: Ramp to Pine Mountain Road & SB US 321



Lane Group	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations			↑↑	↗		
Traffic Volume (vph)	0	0	2098	179	0	0
Future Volume (vph)	0	0	2098	179	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00
Fr _t				0.850		
Fl _t Protected						
Satd. Flow (prot)	0	0	3505	1568	0	0
Fl _t Permitted						
Satd. Flow (perm)	0	0	3505	1568	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		502	255		146	
Travel Time (s)		6.2	3.2		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	2331	199	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	2331	199	0	0
Sign Control		Stop	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	61.3%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 988: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑				↑↑
Traffic Volume (vph)	0	1614	0	0	0	2079
Future Volume (vph)	0	1614	0	0	0	2079
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	3505	0	0	0	2760
Flt Permitted						
Satd. Flow (perm)	0	3505	0	0	0	2760
Link Speed (mph)		55	55		55	
Link Distance (ft)		2395	834		894	
Travel Time (s)		29.7	10.3		11.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	2%	2%	3%
Adj. Flow (vph)	0	1793	0	0	0	2310
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1793	0	0	0	2310
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	76.1%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 989: SB US 321 & NB US 321/US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations		↑↑		↑↑		
Traffic Volume (vph)	0	1659	0	2087	0	0
Future Volume (vph)	0	1659	0	2087	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.88	1.00	1.00
Fr _t				0.850		
Flt Protected						
Satd. Flow (prot)	0	3505	0	2760	0	0
Flt Permitted						
Satd. Flow (perm)	0	3505	0	2760	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		760	2395		709	
Travel Time (s)		9.4	29.7		8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	3%	2%	2%
Adj. Flow (vph)	0	1843	0	2319	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1843	0	2319	0	0
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	76.3%
ICU Level of Service	D
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
 989: SB US 321 & NB US 321/US 321

NCDOT TIP-U4700



Movement	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations		↑↑		↑↑		
Traffic Volume (veh/h)	0	1659	0	2087	0	0
Future Volume (Veh/h)	0	1659	0	2087	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1843	0	2319	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2319				922	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2319				922	0
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	212				269	1084
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volume Total	922	922	1160	1160		
Volume Left	0	0	0	0		
Volume Right	0	0	1160	1160		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.54	0.54	0.68	0.68		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			76.3%		ICU Level of Service	D
Analysis Period (min)			15			

Summary of All Intervals

Start Time	6:50
End Time	8:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	3679
Vehs Exited	3659
Starting Vehs	269
Ending Vehs	289
Travel Distance (mi)	8163
Travel Time (hr)	293.6
Total Delay (hr)	126.8
Total Stops	7411
Fuel Used (gal)	316.0

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	3679
Vehs Exited	3659
Starting Vehs	269
Ending Vehs	289
Travel Distance (mi)	8163
Travel Time (hr)	293.6
Total Delay (hr)	126.8
Total Stops	7411
Fuel Used (gal)	316.0

Intersection: 161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	203	177	134	155	165	118
Average Queue (ft)	121	71	84	111	114	69
95th Queue (ft)	182	160	139	146	148	125
Link Distance (ft)	697		42	42	42	46
Upstream Blk Time (%)			18	32	34	25
Queuing Penalty (veh)			97	169	183	25
Storage Bay Dist (ft)		150				
Storage Blk Time (%)	3	0				
Queuing Penalty (veh)	4	0				

Intersection: 162: SB US 321 & Lower Cedar Valley Road U-Turn

Movement	WB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	97	269	372
Average Queue (ft)	63	153	173
95th Queue (ft)	79	241	278
Link Distance (ft)	4	669	669
Upstream Blk Time (%)	70		
Queuing Penalty (veh)	143		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	249	226	133	132	116	140
Average Queue (ft)	166	108	76	90	56	107
95th Queue (ft)	233	222	137	142	110	140
Link Distance (ft)	601		58	58	58	43
Upstream Blk Time (%)			11	14	10	56
Queuing Penalty (veh)			79	99	69	91
Storage Bay Dist (ft)		250				
Storage Blk Time (%)	0					
Queuing Penalty (veh)	0					

Intersection: 164: NB US 321 & Lower Cedar Valley Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	86	175	146
Average Queue (ft)	61	84	88
95th Queue (ft)	76	163	151
Link Distance (ft)	6	1083	1083
Upstream Blk Time (%)	62		
Queuing Penalty (veh)	97		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 165: NB US 321 & Lower Cedar Valley Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	235
Average Queue (ft)	100
95th Queue (ft)	180
Link Distance (ft)	248
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 166: Lower Cedar Valley Road Left-Over

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	30	97	115
Average Queue (ft)	3	7	13
95th Queue (ft)	16	39	56
Link Distance (ft)		550	550
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 167: SB US 321 & Lower Cedar Valley Road U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	136
Average Queue (ft)	45
95th Queue (ft)	103
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 168: NB US 321 & Lower Cedar Valley Road Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	98	110	143	206
Average Queue (ft)	24	25	60	77
95th Queue (ft)	74	86	142	167
Link Distance (ft)		978	978	978
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	200			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 171: NB US 321 & Quarry Estates Road

Movement	WB
Directions Served	R
Maximum Queue (ft)	67
Average Queue (ft)	24
95th Queue (ft)	53
Link Distance (ft)	403
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 172: Quarry Estates Road & SB US 321

Movement	EB
Directions Served	R
Maximum Queue (ft)	36
Average Queue (ft)	8
95th Queue (ft)	24
Link Distance (ft)	651
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	282	249	186	191	56	119
Average Queue (ft)	184	127	139	146	20	80
95th Queue (ft)	277	241	195	183	64	114
Link Distance (ft)	1036		79	79	79	34
Upstream Blk Time (%)			19	24		50
Queuing Penalty (veh)			112	139		98
Storage Bay Dist (ft)		400				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 182: SB US 321 & Pine Mountain Road U-Turn

Movement	WB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	96	610	571
Average Queue (ft)	64	300	253
95th Queue (ft)	83	513	483
Link Distance (ft)	4	993	993
Upstream Blk Time (%)	64		
Queuing Penalty (veh)	183		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road

Movement	EB	EB	SB	SB	NW
Directions Served	>	>	T	T	L
Maximum Queue (ft)	255	183	84	93	116
Average Queue (ft)	146	117	82	84	57
95th Queue (ft)	215	177	92	88	105
Link Distance (ft)	451		23	23	48
Upstream Blk Time (%)			28	28	24
Queuing Penalty (veh)			270	270	24
Storage Bay Dist (ft)		200			
Storage Blk Time (%)	1	0			
Queuing Penalty (veh)	3	0			

Intersection: 184: NB US 321 & Pine Mountain Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	112	288	298
Average Queue (ft)	72	111	127
95th Queue (ft)	91	231	230
Link Distance (ft)	15	784	784
Upstream Blk Time (%)	59		
Queuing Penalty (veh)	133		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 185: Pine Mountain Road & Ramp to Pine Mountain Road

Movement	SB
Directions Served	R
Maximum Queue (ft)	78
Average Queue (ft)	13
95th Queue (ft)	47
Link Distance (ft)	78
Upstream Blk Time (%)	0
Queuing Penalty (veh)	1
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 186: NB US 321 & Pine Mountain Road U-Turn

Movement	NB	NB
Directions Served	L	T
Maximum Queue (ft)	243	216
Average Queue (ft)	115	7
95th Queue (ft)	196	71
Link Distance (ft)		754
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	300	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 187: SB US 321 & Pine Mountain Road Left-Over

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	147	361	324
Average Queue (ft)	29	99	130
95th Queue (ft)	88	248	263
Link Distance (ft)		447	447
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	200		
Storage Blk Time (%)		2	
Queuing Penalty (veh)		4	

Intersection: 188: SB US 321 & Pine Mountain Road U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	167
Average Queue (ft)	67
95th Queue (ft)	144
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	200
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 189: NB US 321 & Pine Mountain Road Left-Over

Movement	NB	NB
Directions Served	T	T
Maximum Queue (ft)	228	214
Average Queue (ft)	59	72
95th Queue (ft)	163	176
Link Distance (ft)	862	862
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)	1	
Queuing Penalty (veh)	1	

Intersection: 950: Ramp to Pine Mountain Road & SB US 321

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 988: US 321/NB US 321 & SB US 321

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 989: SB US 321 & NB US 321/US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 2294

Lanes, Volumes, Timings

NCDOT TIP-U4700

161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↔↔		↑↑↑						↔	
Traffic Volume (vph)	0	0	134	0	2021	125	0	0	0	43	129	0
Future Volume (vph)	0	0	134	0	2021	125	0	0	0	43	129	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		0		0	0		0		0	0
Storage Lanes	0	1		0		0	0		0		1	0
Taper Length (ft)	100			100			100				100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.991							
Flt Protected											0.950	
Satd. Flow (prot)	0	0	2787	0	4943	0	0	0	0	0	1752	0
Flt Permitted											0.950	
Satd. Flow (perm)	0	0	2787	0	4943	0	0	0	0	0	1752	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				55			55			25	
Link Distance (ft)	760				118			761			116	
Travel Time (s)	14.8				1.5			9.4			3.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	3%	4%	4%	2%	2%	2%	3%	3%	3%
Adj. Flow (vph)	0	0	149	0	2246	139	0	0	0	48	143	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	149	0	2385	0	0	0	0	0	191	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	0				0			0			12	
Link Offset(ft)	0				0			0			0	
Crosswalk Width(ft)	16				16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15		9	15		9	15	15	9
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			8		2					7	7	
Permitted Phases												
Minimum Initial (s)			7.0		14.0					7.0	7.0	
Minimum Split (s)			14.0		21.0					14.0	14.0	
Total Split (s)			14.0		56.0					20.0	20.0	
Total Split (%)			15.6%		62.2%					22.2%	22.2%	
Maximum Green (s)			7.0		49.0					13.0	13.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Total Lost Time (s)			5.0		5.0						5.0	
Lead/Lag			Lag							Lead	Lead	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effct Green (s)			9.0		51.7						14.3	
Actuated g/C Ratio			0.10		0.57						0.16	
v/c Ratio			0.53		0.84						0.69	

161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Control Delay			46.1		9.5							44.9
Queue Delay			0.0		0.0							0.0
Total Delay			46.1		9.5							44.9
LOS			D		A							D
Approach Delay	46.1				9.5							44.9
Approach LOS	D				A							D
Queue Length 50th (ft)			46		203							97
Queue Length 95th (ft)			80		m258							m#159
Internal Link Dist (ft)	680				38			681				36
Turn Bay Length (ft)			150									
Base Capacity (vph)			279		2839							292
Starvation Cap Reductn			0		0							0
Spillback Cap Reductn			0		0							0
Storage Cap Reductn			0		0							0
Reduced v/c Ratio			0.53		0.84							0.65

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 33 (37%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.84
 Intersection Signal Delay: 14.0 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over



Lanes, Volumes, Timings
 162: SB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	79	0	0	0	0	1685
Future Volume (vph)	79	0	0	0	0	1685
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	0	0	3505
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	0	0	3505
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		30			55
Link Distance (ft)	69		391			709
Travel Time (s)	1.9		8.9			8.8
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	2%	2%	3%
Adj. Flow (vph)	88	0	0	0	0	1872
Shared Lane Traffic (%)						
Lane Group Flow (vph)	88	0	0	0	0	1872
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Minimum Initial (s)	7.0					14.0
Minimum Split (s)	14.0					21.0
Total Split (s)	16.0					74.0
Total Split (%)	17.8%					82.2%
Maximum Green (s)	9.0					67.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	11.1					72.7
Actuated g/C Ratio	0.12					0.81
v/c Ratio	0.41					0.66
Control Delay	49.4					3.2
Queue Delay	0.0					0.0
Total Delay	49.4					3.2

Lanes, Volumes, Timings
162: SB US 321 & Lower Cedar Valley Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
LOS	D					A
Approach Delay	49.4					3.2
Approach LOS	D					A
Queue Length 50th (ft)	52					115
Queue Length 95th (ft)	m54					110
Internal Link Dist (ft)	1		311			629
Turn Bay Length (ft)						
Base Capacity (vph)	226					2856
Starvation Cap Reductn	0					0
Spillback Cap Reductn	0					0
Storage Cap Reductn	0					0
Reduced v/c Ratio	0.39					0.66

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	66 (73%), Referenced to phase 6:SBT, Start of Green
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.66
Intersection Signal Delay:	5.3
Intersection LOS:	A
Intersection Capacity Utilization:	112.2%
ICU Level of Service:	H
Analysis Period (min):	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 162: SB US 321 & Lower Cedar Valley Road U-Turn



Lanes, Volumes, Timings

163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	484	0	0	0	0	1451	157	108	0
Future Volume (vph)	0	0	484	0	0	0	0	1451	157	108	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3505	1568	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3505	1568	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	661				1107			120		112	
Travel Time (s)	12.9				13.7			1.5		3.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%	2%	3%	3%	4%	3%
Adj. Flow (vph)	0	0	538	0	0	0	0	1612	174	120	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	538	0	0	0	0	1612	174	120	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Right
Median Width(ft)	0				12			12		12	
Link Offset(ft)	0				0			0		0	
Crosswalk Width(ft)	16				16			16		16	
Two way Left Turn Lane											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15		9	15		9	15	9
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			31.0					59.0	59.0	31.0	
Total Split (%)			34.4%					65.6%	65.6%	34.4%	
Maximum Green (s)			24.0					52.0	52.0	24.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			23.4					56.6	56.6	23.4	
Actuated g/C Ratio			0.26					0.63	0.63	0.26	
v/c Ratio			0.75					0.73	0.18	0.27	

163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Control Delay			37.5					8.7	4.2	23.5	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			37.5					8.7	4.2	23.5	
LOS			D					A	A	C	
Approach Delay	37.5							8.3		23.5	
Approach LOS	D							A		C	
Queue Length 50th (ft)			156					233	27	44	
Queue Length 95th (ft)			215					184	m30	m46	
Internal Link Dist (ft)	581				1027			40		32	
Turn Bay Length (ft)			250								
Base Capacity (vph)			797					2202	985	501	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.68					0.73	0.18	0.24	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 79 (88%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.75
 Intersection Signal Delay: 15.4 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road



Lanes, Volumes, Timings
 164: NB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	320	0	0	1934	0	0
Future Volume (vph)	320	0	0	1934	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	56			1125	1035	
Travel Time (s)	1.5			13.9	12.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	4%	2%	2%
Adj. Flow (vph)	356	0	0	2149	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	356	0	0	2149	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	28.0			62.0		
Total Split (%)	31.1%			68.9%		
Maximum Green (s)	21.0			55.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	22.1			57.9		
Actuated g/C Ratio	0.25			0.64		
v/c Ratio	0.84			0.96		
Control Delay	48.0			24.5		
Queue Delay	0.0			0.0		
Total Delay	48.0			24.5		

Lanes, Volumes, Timings
 164: NB US 321 & Lower Cedar Valley Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
LOS	D			C		
Approach Delay	48.0			24.5		
Approach LOS	D			C		
Queue Length 50th (ft)	178			572		
Queue Length 95th (ft)	m#316			#790		
Internal Link Dist (ft)	1			1045	955	
Turn Bay Length (ft)						
Base Capacity (vph)	443			2232		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.80			0.96		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 15 (17%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.96
 Intersection Signal Delay: 27.8 Intersection LOS: C
 Intersection Capacity Utilization 79.5% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 164: NB US 321 & Lower Cedar Valley Road U-Turn



Lanes, Volumes, Timings
 165: NB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	79	2103	0	0
Future Volume (vph)	0	0	79	2103	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	3505	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	3505	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	69			282	760	
Travel Time (s)	0.9			3.5	9.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	88	2337	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	88	2337	0	0
Enter Blocked Intersection	No	No	Yes	Yes	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	25			9
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	112.2%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 166: Lower Cedar Valley Road Left-Over



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↑↑↑
Traffic Volume (vph)	0	0	0	0	172	1608
Future Volume (vph)	0	0	0	0	172	1608
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	5036
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	5036
Link Speed (mph)	25		55			55
Link Distance (ft)	116		120			594
Travel Time (s)	3.2		1.5			7.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	191	1787
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	191	1787
Enter Blocked Intersection	No	No	No	No	Yes	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	25	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	34.4%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
 166: Lower Cedar Valley Road Left-Over

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↘	↑↑↑
Traffic Volume (veh/h)	0	0	0	0	172	1608
Future Volume (Veh/h)	0	0	0	0	172	1608
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	0	191	1787
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)			120		985	
pX, platoon unblocked	0.89					
vC, conflicting volume	978	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	547	0	0			
tC, single (s)	6.8	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	88			
cM capacity (veh/h)	367	1084	1614			

Direction, Lane #	SB 1	SB 2	SB 3	SB 4
Volume Total	191	596	596	596
Volume Left	191	0	0	0
Volume Right	0	0	0	0
cSH	1614	1700	1700	1700
Volume to Capacity	0.12	0.35	0.35	0.35
Queue Length 95th (ft)	10	0	0	0
Control Delay (s)	7.5	0.0	0.0	0.0
Lane LOS	A			
Approach Delay (s)	0.7			
Approach LOS				

Intersection Summary				
Average Delay	0.7			
Intersection Capacity Utilization	34.4%	ICU Level of Service		A
Analysis Period (min)	15			

Lanes, Volumes, Timings
 167: SB US 321 & Lower Cedar Valley Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	320	1615
Future Volume (vph)	0	0	0	0	320	1615
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	300	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	55		55			55
Link Distance (ft)	56		1159			1107
Travel Time (s)	0.7		14.4			13.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Adj. Flow (vph)	0	0	0	0	356	1794
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	356	1794
Enter Blocked Intersection	No	No	No	No	Yes	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	25	
Sign Control	Stop		Stop			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	79.5%			ICU Level of Service D		
Analysis Period (min)	15					

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 168: NB US 321 & Lower Cedar Valley Road Left-Over

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	108	2146	0	0
Future Volume (vph)	0	0	108	2146	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	112			1035	118	
Travel Time (s)	1.4			12.8	1.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	120	2384	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	120	2384	0	0
Enter Blocked Intersection	No	No	Yes	Yes	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	25			9
Sign Control	Stop			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	124.3%			ICU Level of Service H		
Analysis Period (min)	15					

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 171: NB US 321 & Quarry Estates Road

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	27	2155	63	0	0
Future Volume (vph)	0	27	2155	63	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Frt		0.865	0.996			
Flt Protected						
Satd. Flow (prot)	0	1611	5016	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1611	5016	0	0	0
Link Speed (mph)	35		55			55
Link Distance (ft)	468		761			282
Travel Time (s)	9.1		9.4			3.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	4%	4%
Adj. Flow (vph)	0	30	2394	70	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	30	2464	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	53.0%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 171: NB US 321 & Quarry Estates Road

NCDOT TIP-U4700



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↑↑↑			
Traffic Volume (veh/h)	0	27	2155	63	0	0
Future Volume (Veh/h)	0	27	2155	63	0	0
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	30	2394	70	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (ft)			761			
pX, platoon unblocked	0.60	0.60			0.60	
vC, conflicting volume	2429	833			2464	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1043	0			1102	
tC, single (s)	6.8	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	95			100	
cM capacity (veh/h)	135	650			370	

Direction, Lane #	WB 1	NB 1	NB 2	NB 3
Volume Total	30	958	958	549
Volume Left	0	0	0	0
Volume Right	30	0	0	70
cSH	650	1700	1700	1700
Volume to Capacity	0.05	0.56	0.56	0.32
Queue Length 95th (ft)	4	0	0	0
Control Delay (s)	10.8	0.0	0.0	0.0
Lane LOS	B			
Approach Delay (s)	10.8	0.0		
Approach LOS	B			

Intersection Summary			
Average Delay		0.1	
Intersection Capacity Utilization		53.0%	ICU Level of Service A
Analysis Period (min)		15	

Lanes, Volumes, Timings
 172: Quarry Estates Road & SB US 321

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	61	0	0	1737	16
Future Volume (vph)	0	61	0	0	1737	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	0			250
Storage Lanes	0	1	0			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	0.91	0.91
Frt		0.865			0.999	
Flt Protected						
Satd. Flow (prot)	0	1580	0	0	5031	0
Flt Permitted						
Satd. Flow (perm)	0	1580	0	0	5031	0
Link Speed (mph)	35			55	55	
Link Distance (ft)	724			594	391	
Travel Time (s)	14.1			7.4	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	4%	2%	2%	3%	3%
Adj. Flow (vph)	0	68	0	0	1930	18
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	68	0	0	1948	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	44.4%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 172: Quarry Estates Road & SB US 321

NCDOT TIP-U4700



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	61	0	0	1737	16
Future Volume (Veh/h)	0	61	0	0	1737	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	68	0	0	1930	18
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked	0.78	0.78	0.78			
vC, conflicting volume	1939	652	1948			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1215	0	1227			
tC, single (s)	6.8	7.0	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	92	100			
cM capacity (veh/h)	135	840	440			
Direction, Lane #	EB 1	SB 1	SB 2	SB 3		
Volume Total	68	772	772	404		
Volume Left	0	0	0	0		
Volume Right	68	0	0	18		
cSH	840	1700	1700	1700		
Volume to Capacity	0.08	0.45	0.45	0.24		
Queue Length 95th (ft)	7	0	0	0		
Control Delay (s)	9.7	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	9.7	0.0				
Approach LOS	A					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			44.4%	ICU Level of Service	A	
Analysis Period (min)			15			

Lanes, Volumes, Timings

NCDOT TIP-U4700

181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↑↑	↗				↘	
Traffic Volume (vph)	0	0	444	0	1796	287	0	0	0	228	0
Future Volume (vph)	0	0	444	0	1796	287	0	0	0	228	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	400		0		0	400		0	0	0
Storage Lanes	0	1		0		1	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	3505	1568	0	0	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	3505	1568	0	0	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	1087				138			816		103	
Travel Time (s)	21.2				1.7			10.1		2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	3%	3%	3%	4%	4%	2%	3%	2%
Adj. Flow (vph)	0	0	493	0	1996	319	0	0	0	253	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	493	0	1996	319	0	0	0	253	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Right
Median Width(ft)	0				12			12		12	
Link Offset(ft)	0				0			0		0	
Crosswalk Width(ft)	16				16			16		16	
Two way Left Turn Lane											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15		9	15		9	15	9
Turn Type			Prot		NA	Free				Prot	
Protected Phases			4		2					4	
Permitted Phases						Free					
Minimum Initial (s)			7.0		14.0					7.0	
Minimum Split (s)			14.0		21.0					14.0	
Total Split (s)			26.0		64.0					26.0	
Total Split (%)			28.9%		71.1%					28.9%	
Maximum Green (s)			19.0		57.0					19.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effect Green (s)			20.2		59.8	90.0				20.2	
Actuated g/C Ratio			0.22		0.66	1.00				0.22	
v/c Ratio			0.79		0.86	0.20				0.64	

181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Control Delay			42.9		7.0	0.1				45.7	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			42.9		7.0	0.1				45.7	
LOS			D		A	A				D	
Approach Delay	42.9				6.0					45.7	
Approach LOS	D				A					D	
Queue Length 50th (ft)			148		60	0				151	
Queue Length 95th (ft)			#212		360	m0				m190	
Internal Link Dist (ft)	1007				58			736		23	
Turn Bay Length (ft)			400								
Base Capacity (vph)			650		2327	1568				408	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.76		0.86	0.20				0.62	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 14 (16%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 15.2 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over



Lanes, Volumes, Timings
 182: SB US 321 & Pine Mountain Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	250	0	0	0	0	1715
Future Volume (vph)	250	0	0	0	0	1715
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	0	0	3505
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	0	0	3505
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		55			55
Link Distance (ft)	63		255			1015
Travel Time (s)	1.7		3.2			12.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	2%	2%	3%
Adj. Flow (vph)	278	0	0	0	0	1906
Shared Lane Traffic (%)						
Lane Group Flow (vph)	278	0	0	0	0	1906
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Minimum Initial (s)	7.0					14.0
Minimum Split (s)	14.0					21.0
Total Split (s)	26.0					64.0
Total Split (%)	28.9%					71.1%
Maximum Green (s)	19.0					57.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	19.3					60.7
Actuated g/C Ratio	0.21					0.67
v/c Ratio	0.74					0.81
Control Delay	34.4					14.4
Queue Delay	0.0					0.0
Total Delay	34.4					14.4

Lanes, Volumes, Timings
 182: SB US 321 & Pine Mountain Road U-Turn



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
LOS	C					B
Approach Delay	34.4					14.4
Approach LOS	C					B
Queue Length 50th (ft)	140					382
Queue Length 95th (ft)	m172					493
Internal Link Dist (ft)	1		175		935	
Turn Bay Length (ft)						
Base Capacity (vph)	408					2364
Starvation Cap Reductn	0					0
Spillback Cap Reductn	0					0
Storage Cap Reductn	0					0
Reduced v/c Ratio	0.68					0.81

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 89 (99%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.81
 Intersection Signal Delay: 17.0 Intersection LOS: B
 Intersection Capacity Utilization 109.9% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 182: SB US 321 & Pine Mountain Road U-Turn



183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑		↑	
Traffic Volume (vph)	0	0	281	0	0	0	0	1512	0	175	0
Future Volume (vph)	0	0	281	0	0	0	0	1512	0	175	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	200		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		0	1	0
Taper Length (ft)	100			100			100			100	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850								
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	0	0	0	3505	0	1752	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	0	0	0	3505	0	1752	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	538				984			106		137	
Travel Time (s)	10.5				12.2			1.3		3.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	2%	3%	2%
Adj. Flow (vph)	0	0	312	0	0	0	0	1680	0	194	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	312	0	0	0	0	1680	0	194	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Right
Median Width(ft)	0				12			12		12	
Link Offset(ft)	0				0			0		0	
Crosswalk Width(ft)	16				16			16		16	
Two way Left Turn Lane											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15		9	15		9	15	9
Turn Type			Perm					NA		Prot	
Protected Phases								6		8	
Permitted Phases			4								
Minimum Initial (s)			7.0					14.0		7.0	
Minimum Split (s)			14.0					21.0		14.0	
Total Split (s)			25.0					65.0		25.0	
Total Split (%)			27.8%					72.2%		27.8%	
Maximum Green (s)			18.0					58.0		18.0	
Yellow Time (s)			5.0					5.0		5.0	
All-Red Time (s)			2.0					2.0		2.0	
Total Lost Time (s)			5.0					5.0		5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0		3.0	
Recall Mode			None					C-Min		None	
Act Effct Green (s)			17.0					63.0		17.0	
Actuated g/C Ratio			0.19					0.70		0.19	
v/c Ratio			0.59					0.68		0.59	

183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Control Delay			37.9					4.8		32.3	
Queue Delay			0.0					0.0		0.0	
Total Delay			37.9					4.8		32.3	
LOS			D					A		C	
Approach Delay	37.9							4.8		32.3	
Approach LOS	D							A		C	
Queue Length 50th (ft)			92					131		100	
Queue Length 95th (ft)			134					167		m130	
Internal Link Dist (ft)	458				904			26		57	
Turn Bay Length (ft)			200								
Base Capacity (vph)			619					2455		389	
Starvation Cap Reductn			0					0		0	
Spillback Cap Reductn			0					0		0	
Storage Cap Reductn			0					0		0	
Reduced v/c Ratio			0.50					0.68		0.50	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 12 (13%), Referenced to phase 6:SBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 12.0 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road



Lanes, Volumes, Timings
 184: NB US 321 & Pine Mountain Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	179	0	0	2079	0	0
Future Volume (vph)	179	0	0	2079	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1752	0	0	3505	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1752	0	0	3505	0	0
Right Turn on Red	No	No				No
Satd. Flow (RTOR)						
Link Speed (mph)	25			55	55	
Link Distance (ft)	70			834	917	
Travel Time (s)	1.9			10.3	11.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	2%	2%	3%	2%	2%
Adj. Flow (vph)	199	0	0	2310	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	199	0	0	2310	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot			NA		
Protected Phases	4			2		
Permitted Phases						
Minimum Initial (s)	7.0			14.0		
Minimum Split (s)	14.0			21.0		
Total Split (s)	18.0			72.0		
Total Split (%)	20.0%			80.0%		
Maximum Green (s)	11.0			65.0		
Yellow Time (s)	5.0			5.0		
All-Red Time (s)	2.0			2.0		
Total Lost Time (s)	5.0			5.0		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0		
Recall Mode	None			C-Min		
Act Effect Green (s)	12.9			67.1		
Actuated g/C Ratio	0.14			0.75		
v/c Ratio	0.79			0.88		
Control Delay	55.5			8.0		
Queue Delay	0.0			0.0		
Total Delay	55.5			8.0		

Lanes, Volumes, Timings
 184: NB US 321 & Pine Mountain Road U-Turn



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
LOS	E			A		
Approach Delay	55.5			8.0		
Approach LOS	E			A		
Queue Length 50th (ft)	107			150		
Queue Length 95th (ft)	m#207			125		
Internal Link Dist (ft)	1			754	837	
Turn Bay Length (ft)						
Base Capacity (vph)	253			2612		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.79			0.88		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT, Start of Green
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.88
 Intersection Signal Delay: 11.8 Intersection LOS: B
 Intersection Capacity Utilization 75.7% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 184: NB US 321 & Pine Mountain Road U-Turn



Lanes, Volumes, Timings
 185: Pine Mountain Road & Ramp to Pine Mountain Road

NCDOT TIP-U4700



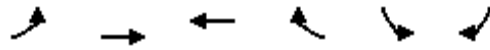
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑			↗
Traffic Volume (vph)	0	281	175	0	0	225
Future Volume (vph)	0	281	175	0	0	225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	1863	1863	0	0	1596
Flt Permitted						
Satd. Flow (perm)	0	1863	1863	0	0	1596
Link Speed (mph)		35	35		35	
Link Distance (ft)		566	538		146	
Travel Time (s)		11.0	10.5		2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	2%	3%
Adj. Flow (vph)	0	312	194	0	0	250
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	312	194	0	0	250
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	29.8%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis
 185: Pine Mountain Road & Ramp to Pine Mountain Road

NCDOT TIP-U4700



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑			↗
Traffic Volume (veh/h)	0	281	175	0	0	225
Future Volume (Veh/h)	0	281	175	0	0	225
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	312	194	0	0	250
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)			538			
pX, platoon unblocked						
vC, conflicting volume	194				506	194
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	194				506	194
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	70
cM capacity (veh/h)	1379				526	845
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	312	194	250			
Volume Left	0	0	0			
Volume Right	0	0	250			
cSH	1700	1700	845			
Volume to Capacity	0.18	0.11	0.30			
Queue Length 95th (ft)	0	0	31			
Control Delay (s)	0.0	0.0	11.0			
Lane LOS			B			
Approach Delay (s)	0.0	0.0	11.0			
Approach LOS			B			
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			29.8%	ICU Level of Service		A
Analysis Period (min)			15			

Lanes, Volumes, Timings
 186: NB US 321 & Pine Mountain Road U-Turn

NCDOT TIP-U4700



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	250	1990	0	0
Future Volume (vph)	0	0	250	1990	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	300			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	3505	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	3505	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	63			816	983	
Travel Time (s)	0.8			10.1	12.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	278	2211	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	278	2211	0	0
Enter Blocked Intersection	No	No	Yes	Yes	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	25			9
Sign Control	Stop			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	109.9%			ICU Level of Service H		
Analysis Period (min)	15					

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 187: SB US 321 & Pine Mountain Road Left-Over

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↵	↕↕
Traffic Volume (vph)	0	0	0	0	228	1512
Future Volume (vph)	0	0	0	0	228	1512
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3505
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3505
Link Speed (mph)	55		55			55
Link Distance (ft)	103		106			502
Travel Time (s)	1.3		1.3			6.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	253	1680
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	253	1680
Enter Blocked Intersection	No	No	No	No	Yes	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	25	
Sign Control	Stop		Stop			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	51.4%			ICU Level of Service A		
Analysis Period (min)	15					

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 188: SB US 321 & Pine Mountain Road U-Turn

NCDOT TIP-U4700



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	179	1614
Future Volume (vph)	0	0	0	0	179	1614
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1752	3505
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1752	3505
Link Speed (mph)	55		55			55
Link Distance (ft)	70		894			984
Travel Time (s)	0.9		11.1			12.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	0	0	0	0	199	1793
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	199	1793
Enter Blocked Intersection	No	No	No	No	Yes	Yes
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Stop			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	75.7%			ICU Level of Service D		
Analysis Period (min)	15					

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 189: NB US 321 & Pine Mountain Road Left-Over



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	175	2083	0	0
Future Volume (vph)	0	0	175	2083	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	150			0
Storage Lanes	0	0	1			0
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1752	5036	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1752	5036	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	137			917	138	
Travel Time (s)	3.7			11.4	1.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	194	2314	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	194	2314	0	0
Enter Blocked Intersection	No	No	Yes	Yes	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	25			9
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	119.3%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 950: Ramp to Pine Mountain Road & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations			↑↑	↑		
Traffic Volume (vph)	0	0	1740	225	0	0
Future Volume (vph)	0	0	1740	225	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00
Fr _t				0.850		
Fl _t Protected						
Satd. Flow (prot)	0	0	3505	1568	0	0
Fl _t Permitted						
Satd. Flow (perm)	0	0	3505	1568	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		502	255		146	
Travel Time (s)		6.2	3.2		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Adj. Flow (vph)	0	0	1933	250	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1933	250	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			25	15	9
Sign Control		Stop	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	51.4%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 988: US 321/NB US 321 & SB US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑				↑↑
Traffic Volume (vph)	0	2079	0	0	0	1614
Future Volume (vph)	0	2079	0	0	0	1614
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	3505	0	0	0	2760
Flt Permitted						
Satd. Flow (perm)	0	3505	0	0	0	2760
Link Speed (mph)		55	55		55	
Link Distance (ft)		2395	834		894	
Travel Time (s)		29.7	10.3		11.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	2%	2%	3%
Adj. Flow (vph)	0	2310	0	0	0	1793
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2310	0	0	0	1793
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	60
Sign Control		Free	Stop		Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	60.8%
	ICU Level of Service B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 989: SB US 321 & NB US 321/US 321

NCDOT TIP-U4700



Lane Group	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations		↑↑		↑↑		
Traffic Volume (vph)	0	2103	0	1685	0	0
Future Volume (vph)	0	2103	0	1685	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.88	1.00	1.00
Fr _t				0.850		
Fl _t Protected						
Satd. Flow (prot)	0	3505	0	2760	0	0
Fl _t Permitted						
Satd. Flow (perm)	0	3505	0	2760	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		760	2395		709	
Travel Time (s)		9.4	29.7		8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	2%	3%	2%	2%
Adj. Flow (vph)	0	2337	0	1872	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2337	0	1872	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			55	15	9
Sign Control		Free	Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.3%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis
 989: SB US 321 & NB US 321/US 321

NCDOT TIP-U4700



Movement	NBL	NBT	SBT	SBR	NEL	NER
Lane Configurations		↑↑		↑↑		
Traffic Volume (veh/h)	0	2103	0	1685	0	0
Future Volume (Veh/h)	0	2103	0	1685	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	2337	0	1872	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1872			1168	0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1872			1168	0	
tC, single (s)	4.1			6.8	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			100	100	
cM capacity (veh/h)	318			186	1084	
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volume Total	1168	1168	936	936		
Volume Left	0	0	0	0		
Volume Right	0	0	936	936		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.69	0.69	0.55	0.55		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			62.3%		ICU Level of Service	B
Analysis Period (min)			15			

Summary of All Intervals

Start Time	4:50
End Time	6:00
Total Time (min)	70
Time Recorded (min)	60
# of Intervals	2
# of Recorded Intervals	1
Vehs Entered	3601
Vehs Exited	3631
Starting Vehs	271
Ending Vehs	241
Travel Distance (mi)	7843
Travel Time (hr)	280.8
Total Delay (hr)	121.0
Total Stops	7115
Fuel Used (gal)	304.6

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00
End Time	6:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	
Vehs Entered	3601
Vehs Exited	3631
Starting Vehs	271
Ending Vehs	241
Travel Distance (mi)	7843
Travel Time (hr)	280.8
Total Delay (hr)	121.0
Total Stops	7115
Fuel Used (gal)	304.6

Intersection: 161: NB US 321 & Lower Cedar Valley Road & Lower Cedar Valley Road Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	131	41	166	165	141	154
Average Queue (ft)	59	18	97	124	122	104
95th Queue (ft)	112	40	153	144	135	137
Link Distance (ft)	697		42	42	42	46
Upstream Blk Time (%)			18	33	36	46
Queuing Penalty (veh)			126	236	256	80
Storage Bay Dist (ft)		150				
Storage Blk Time (%)	0					
Queuing Penalty (veh)	0					

Intersection: 162: SB US 321 & Lower Cedar Valley Road U-Turn

Movement	WB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	64	137	149
Average Queue (ft)	33	56	59
95th Queue (ft)	65	119	124
Link Distance (ft)	4	669	669
Upstream Blk Time (%)	36		
Queuing Penalty (veh)	28		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 163: SB US 321 & Lower Cedar Valley Road Left-Over & Lower Cedar Valley Road

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	285	266	157	150	131	136
Average Queue (ft)	188	136	112	120	44	58
95th Queue (ft)	254	238	166	153	92	110
Link Distance (ft)	601		58	58	58	43
Upstream Blk Time (%)			21	26	7	24
Queuing Penalty (veh)			115	139	38	26
Storage Bay Dist (ft)		250				
Storage Blk Time (%)	1	0				
Queuing Penalty (veh)	1	0				

Intersection: 164: NB US 321 & Lower Cedar Valley Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	86	356	377
Average Queue (ft)	65	200	197
95th Queue (ft)	78	327	328
Link Distance (ft)	6	1083	1083
Upstream Blk Time (%)	64		
Queuing Penalty (veh)	204		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 165: NB US 321 & Lower Cedar Valley Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	64
Average Queue (ft)	6
95th Queue (ft)	31
Link Distance (ft)	248
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 166: Lower Cedar Valley Road Left-Over

Movement	SB	SB	SB	SB
Directions Served	L	T	T	T
Maximum Queue (ft)	80	206	218	24
Average Queue (ft)	17	65	72	1
95th Queue (ft)	57	182	182	8
Link Distance (ft)		550	550	550
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	150			
Storage Blk Time (%)		2		
Queuing Penalty (veh)		4		

Intersection: 167: SB US 321 & Lower Cedar Valley Road U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	241
Average Queue (ft)	114
95th Queue (ft)	197
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 168: NB US 321 & Lower Cedar Valley Road Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	56	274	324	309
Average Queue (ft)	2	110	161	191
95th Queue (ft)	18	224	279	283
Link Distance (ft)		978	978	978
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	200			
Storage Blk Time (%)		0		
Queuing Penalty (veh)		0		

Intersection: 171: NB US 321 & Quarry Estates Road

Movement	WB	NB	NB
Directions Served	R	T	TR
Maximum Queue (ft)	63	556	20
Average Queue (ft)	17	19	1
95th Queue (ft)	46	183	6
Link Distance (ft)	403	675	675
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 172: Quarry Estates Road & SB US 321

Movement	EB
Directions Served	R
Maximum Queue (ft)	106
Average Queue (ft)	24
95th Queue (ft)	59
Link Distance (ft)	651
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 181: NB US 321 & Pine Mountain Road & Pine Mountain Road Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	304	254	155	154	56	132
Average Queue (ft)	174	139	87	110	13	92
95th Queue (ft)	259	239	143	157	51	117
Link Distance (ft)	1036		79	79	79	34
Upstream Blk Time (%)			9	12		64
Queuing Penalty (veh)			60	80		146
Storage Bay Dist (ft)		400				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 182: SB US 321 & Pine Mountain Road U-Turn

Movement	WB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	78	333	346
Average Queue (ft)	60	191	145
95th Queue (ft)	70	297	255
Link Distance (ft)	4	993	993
Upstream Blk Time (%)	60		
Queuing Penalty (veh)	151		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 183: SB US 321 & Pine Mountain Road Left-Over & Pine Mountain Road

Movement	EB	EB	SB	SB	NW
Directions Served	>	>	T	T	L
Maximum Queue (ft)	194	108	108	103	138
Average Queue (ft)	92	53	78	79	75
95th Queue (ft)	145	108	112	104	135
Link Distance (ft)	451		23	23	48
Upstream Blk Time (%)			25	26	26
Queuing Penalty (veh)			187	195	46
Storage Bay Dist (ft)		200			
Storage Blk Time (%)	0				
Queuing Penalty (veh)	0				

Intersection: 184: NB US 321 & Pine Mountain Road U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	99	184	186
Average Queue (ft)	75	112	123
95th Queue (ft)	93	165	182
Link Distance (ft)	15	784	784
Upstream Blk Time (%)	71		
Queuing Penalty (veh)	127		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 185: Pine Mountain Road & Ramp to Pine Mountain Road

Movement	SB
Directions Served	R
Maximum Queue (ft)	136
Average Queue (ft)	40
95th Queue (ft)	102
Link Distance (ft)	78
Upstream Blk Time (%)	3
Queuing Penalty (veh)	6
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 186: NB US 321 & Pine Mountain Road U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	195
Average Queue (ft)	93
95th Queue (ft)	166
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 187: SB US 321 & Pine Mountain Road Left-Over

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	125	174	168
Average Queue (ft)	46	64	70
95th Queue (ft)	118	150	162
Link Distance (ft)		447	447
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	200		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 188: SB US 321 & Pine Mountain Road U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	187
Average Queue (ft)	81
95th Queue (ft)	182
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	200
Storage Blk Time (%)	0
Queuing Penalty (veh)	0

Intersection: 189: NB US 321 & Pine Mountain Road Left-Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	56	46	27
Average Queue (ft)	10	2	3
95th Queue (ft)	42	15	17
Link Distance (ft)		862	862
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 950: Ramp to Pine Mountain Road & SB US 321

Movement	SB
Directions Served	R
Maximum Queue (ft)	50
Average Queue (ft)	2
95th Queue (ft)	16
Link Distance (ft)	224
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 988: US 321/NB US 321 & SB US 321

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 989: SB US 321 & NB US 321/US 321

Movement

- Directions Served
- Maximum Queue (ft)
- Average Queue (ft)
- 95th Queue (ft)
- Link Distance (ft)
- Upstream Blk Time (%)
- Queuing Penalty (veh)
- Storage Bay Dist (ft)
- Storage Blk Time (%)
- Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 2251

APPENDIX O

**2040 Build Alternative Mount Herman Road & Caldwell Community
College and Technical Institute Synchro & SimTraffic Reports**

APPENDIX P

**2040 Build Alternative MDI Simulation Analysis Synchro & SimTraffic
Reports**

Lanes, Volumes, Timings
 11: NB US 321 & 13th Street SW

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	136	0	0	243	0	1521	126	0	1917	194
Future Volume (vph)	0	0	136	0	0	243	0	1521	126	0	1917	194
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		200	0		200
Storage Lanes	0		1	0		1	0		1	0		1
Taper Length (ft)	100			100			50			50		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.865			0.865			0.850			0.850
Flt Protected												
Satd. Flow (prot)	0	0	1565	0	0	1565	0	3438	1538	0	3438	1538
Flt Permitted												
Satd. Flow (perm)	0	0	1565	0	0	1565	0	3438	1538	0	3438	1538
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		468			369			2914			1118	
Travel Time (s)		9.1			7.2			44.2			16.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	151	0	0	270	0	1690	140	0	2130	216
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	151	0	0	270	0	1690	140	0	2130	216
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	68.1%
Analysis Period (min)	15
	ICU Level of Service C

HCM Unsignalized Intersection Capacity Analysis

11: NB US 321 & 13th Street SW

04/07/2017



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗			↗		↕	↗		↕	↗
Traffic Volume (veh/h)	0	0	136	0	0	243	0	1521	126	0	1917	194
Future Volume (Veh/h)	0	0	136	0	0	243	0	1521	126	0	1917	194
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	151	0	0	270	0	1690	140	0	2130	216
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2975	3820	1065	2755	3820	845	2130			1690		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2975	3820	1065	2755	3820	845	2130			1690		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	29	100	100	10	100			100		
cM capacity (veh/h)	1	4	214	3	4	300	241			361		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	151	270	845	845	140	1065	1065	216
Volume Left	0	0	0	0	0	0	0	0
Volume Right	151	270	0	0	140	0	0	216
cSH	214	300	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.71	0.90	0.50	0.50	0.08	0.63	0.63	0.13
Queue Length 95th (ft)	114	209	0	0	0	0	0	0
Control Delay (s)	54.5	67.7	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	F	F						
Approach Delay (s)	54.5	67.7	0.0			0.0		
Approach LOS	F	F						

Intersection Summary		
Average Delay		5.8
Intersection Capacity Utilization	68.1%	ICU Level of Service
Analysis Period (min)		15
		C

HCM 2010 TWSC
 11: NB US 321 & 13th Street SW

04/07/2017

Intersection

Int Delay, s/veh 0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗			↗		↕	↗		↕	↗
Traffic Vol, veh/h	0	0	136	0	0	243	0	1521	126	0	1917	194
Future Vol, veh/h	0	0	136	0	0	243	0	1521	126	0	1917	194
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	Free	-	-	Free	-	-	Free	-	-	Free
Storage Length	-	-	0	-	-	0	-	-	200	-	-	200
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	0	0	151	0	0	270	0	1690	140	0	2130	216

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	-	-	-	-	-	-	-	0	-	-	-	0
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-	-	-	-	-	-	-
Pot Cap-1 Maneuver	0	0	0	0	0	0	0	-	0	0	-	0
Stage 1	0	0	0	0	0	0	0	-	0	0	-	0
Stage 2	0	0	0	0	0	0	0	-	0	0	-	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	0
HCM LOS	A	A		

Minor Lane/Major Mvmt	NBT	EBLn1	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	0	0	-
HCM Lane LOS	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	-

Lanes, Volumes, Timings
 15: NB US 321 & SB US 321

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑
Traffic Volume (vph)	0	1764	0	0	0	2111
Future Volume (vph)	0	1764	0	0	0	2111
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4940	0	0	0	2707
Flt Permitted						
Satd. Flow (perm)	0	4940	0	0	0	2707
Link Speed (mph)		45	45		45	
Link Distance (ft)		1118	1025		856	
Travel Time (s)		16.9	15.5		13.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	1960	0	0	0	2346
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1960	0	0	0	2346
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	77.2%
ICU Level of Service	D
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 21: SB US 321 & Main Avenue Drive NW

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	68	0	0	2043	51
Future Volume (vph)	0	68	0	0	2043	51
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	1.00
Frt		0.865				0.850
Flt Protected						
Satd. Flow (prot)	0	1596	0	0	3438	1538
Flt Permitted						
Satd. Flow (perm)	0	1596	0	0	3438	1538
Link Speed (mph)	25			45	45	
Link Distance (ft)	869			856	251	
Travel Time (s)	23.7			13.0	3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	3%	2%	2%	5%	5%
Adj. Flow (vph)	0	76	0	0	2270	57
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	76	0	0	2270	57
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	67.4%
Analysis Period (min)	15
	ICU Level of Service C

HCM Unsignalized Intersection Capacity Analysis

21: SB US 321 & Main Avenue Drive NW

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗			↕↗	↗
Traffic Volume (veh/h)	0	68	0	0	2043	51
Future Volume (Veh/h)	0	68	0	0	2043	51
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	76	0	0	2270	57
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2270	1135	2327			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2270	1135	2327			
tC, single (s)	6.8	7.0	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	61	100			
cM capacity (veh/h)	35	195	210			
Direction, Lane #	EB 1	SB 1	SB 2	SB 3		
Volume Total	76	1135	1135	57		
Volume Left	0	0	0	0		
Volume Right	76	0	0	57		
cSH	195	1700	1700	1700		
Volume to Capacity	0.39	0.67	0.67	0.03		
Queue Length 95th (ft)	43	0	0	0		
Control Delay (s)	34.9	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	34.9	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			67.4%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↑			↑↑	↑
Traffic Vol, veh/h	0	68	0	0	2043	51
Future Vol, veh/h	0	68	0	0	2043	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage, #	0	-	-	-	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	2	2	5	5
Mvmt Flow	0	76	0	0	2270	57

Major/Minor

	Minor2	Major2
Conflicting Flow All	-	1135
Stage 1	-	-
Stage 2	-	-
Critical Hdwy	-	6.96
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	-
Follow-up Hdwy	-	3.33
Pot Cap-1 Maneuver	0	195
Stage 1	0	-
Stage 2	0	-
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	195
Mov Cap-2 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-

Approach

	EB	SB
HCM Control Delay, s	34.7	0
HCM LOS	D	

Minor Lane/Major Mvmt

	EBLn1	SBT	SBR
Capacity (veh/h)	195	-	-
HCM Lane V/C Ratio	0.387	-	-
HCM Control Delay (s)	34.7	-	-
HCM Lane LOS	D	-	-
HCM 95th %tile Q(veh)	1.7	-	-

Lanes, Volumes, Timings

31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↕↕	↗				↖	
Traffic Volume (vph)	0	0	232	0	1575	229	0	0	0	104	0
Future Volume (vph)	0	0	232	0	1575	229	0	0	0	104	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0		0	400		0	0	0
Storage Lanes	0	2		0		1	0		0	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	3438	1538	0	0	0	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	3438	1538	0	0	0	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	275				152			917		184	
Travel Time (s)	5.4				2.3			13.9		5.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	258	0	1750	254	0	0	0	116	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	258	0	1750	254	0	0	0	116	0
Turn Type			Prot		NA	Perm				Prot	
Protected Phases			4		2					4	
Permitted Phases						2					
Detector Phase			4		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		12.0	12.0				7.0	
Minimum Split (s)			14.0		19.0	19.0				14.0	
Total Split (s)			20.0		70.0	70.0				20.0	
Total Split (%)			22.2%		77.8%	77.8%				22.2%	
Maximum Green (s)			13.0		63.0	63.0				13.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effect Green (s)			14.2		65.8	65.8				14.2	
Actuated g/C Ratio			0.16		0.73	0.73				0.16	
v/c Ratio			0.59		0.70	0.23				0.43	
Control Delay			41.1		4.8	2.4				41.6	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			41.1		4.8	2.4				41.6	
LOS			D		A	A				D	
Approach Delay	41.1				4.5					41.6	

Lanes, Volumes, Timings

31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Approach LOS	D				A				D			
Queue Length 50th (ft)			76			67	18			60		
Queue Length 95th (ft)			121			233	m23			m63		
Internal Link Dist (ft)	195					72			837	104		
Turn Bay Length (ft)												
Base Capacity (vph)			464			2518	1126			288		
Starvation Cap Reductn			0			0	0			0		
Spillback Cap Reductn			0			0	0			0		
Storage Cap Reductn			0			0	0			0		
Reduced v/c Ratio			0.56			0.69	0.23			0.40		

Intersection Summary

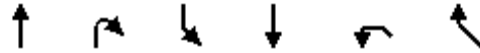
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 20 (22%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.70
 Intersection Signal Delay: 10.3
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over



Lanes, Volumes, Timings
 32: SB US 321 & 2nd Avenue NW U-Turn

04/07/2017

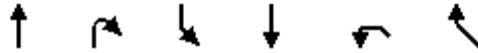


Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	2087	173	0
Future Volume (vph)	0	0	0	2087	173	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3438	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3438	1719	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	45			45	25	
Link Distance (ft)	833			712	94	
Travel Time (s)	12.6			10.8	2.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	0	2319	192	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2319	192	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				12.0	7.0	
Minimum Split (s)				19.0	14.0	
Total Split (s)				72.0	18.0	
Total Split (%)				80.0%	20.0%	
Maximum Green (s)				65.0	11.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Lost Time Adjust (s)				-2.0	-2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				67.1	12.9	
Actuated g/C Ratio				0.75	0.14	
v/c Ratio				0.90	0.78	
Control Delay				6.8	53.0	
Queue Delay				0.0	0.0	
Total Delay				6.8	53.0	
LOS				A	D	
Approach Delay				6.8	53.0	
Approach LOS				A	D	
Queue Length 50th (ft)				66	102	
Queue Length 95th (ft)				89	m#184	

Lanes, Volumes, Timings

32: SB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Internal Link Dist (ft)	753			632	14	
Turn Bay Length (ft)						
Base Capacity (vph)				2564	248	
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.90	0.77	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	66 (73%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.90
Intersection Signal Delay:	10.3
Intersection LOS:	B
Intersection Capacity Utilization	110.4%
ICU Level of Service	H
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

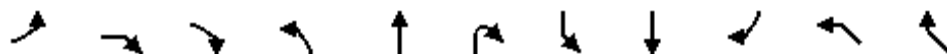
Splits and Phases: 32: SB US 321 & 2nd Avenue NW U-Turn



Lanes, Volumes, Timings

33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

04/07/2017

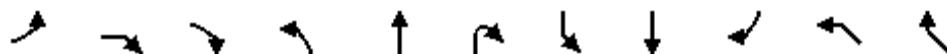


Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	366	0	0	0	0	1897	259	256	0
Future Volume (vph)	0	0	366	0	0	0	0	1897	259	256	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3438	1538	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3438	1538	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	616				840			154		188	
Travel Time (s)	12.0				12.7			2.3		5.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	407	0	0	0	0	2108	288	284	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	407	0	0	0	0	2108	288	284	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					12.0	12.0	7.0	
Minimum Split (s)			14.0					19.0	19.0	14.0	
Total Split (s)			24.0					66.0	66.0	24.0	
Total Split (%)			26.7%					73.3%	73.3%	26.7%	
Maximum Green (s)			17.0					59.0	59.0	17.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			18.3					61.7	61.7	18.3	
Actuated g/C Ratio			0.20					0.69	0.69	0.20	
v/c Ratio			0.72					0.90	0.27	0.81	
Control Delay			41.6					9.0	2.9	39.6	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			41.6					9.0	2.9	39.6	
LOS			D					A	A	D	
Approach Delay	41.6							8.3		39.6	
Approach LOS	D							A		D	
Queue Length 50th (ft)			122					179	21	157	
Queue Length 95th (ft)			178					370	m29	m181	

Lanes, Volumes, Timings

33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	536				760			74		108	
Turn Bay Length (ft)											
Base Capacity (vph)			582					2355	1053	362	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.70					0.90	0.27	0.78	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 82 (91%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 15.5 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW



Lanes, Volumes, Timings

34: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↵	
Traffic Volume (vph)	0	1817	0	0	243	0
Future Volume (vph)	0	1817	0	0	243	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		25	
Link Distance (ft)		1025	741		87	
Travel Time (s)		15.5	11.2		2.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	2%
Adj. Flow (vph)	0	2019	0	0	270	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2019	0	0	270	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		12.0			7.0	
Minimum Split (s)		19.0			14.0	
Total Split (s)		66.0			24.0	
Total Split (%)		73.3%			26.7%	
Maximum Green (s)		59.0			17.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		61.8			18.2	
Actuated g/C Ratio		0.69			0.20	
v/c Ratio		0.86			0.78	
Control Delay		15.8			43.3	
Queue Delay		0.0			0.0	
Total Delay		15.8			43.3	
LOS		B			D	
Approach Delay		15.8			43.3	
Approach LOS		B			D	
Queue Length 50th (ft)		412			144	
Queue Length 95th (ft)		539			m181	

Lanes, Volumes, Timings
 34: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		945	661		7	
Turn Bay Length (ft)						
Base Capacity (vph)		2361			362	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.86			0.75	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 19.1
 Intersection LOS: B
 Intersection Capacity Utilization 72.0%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 34: NB US 321 & 2nd Avenue NW U-Turn



Lanes, Volumes, Timings
 35: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	173	1634	0	0	0	0
Future Volume (vph)	173	1634	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	400			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	3438	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		917	715		94	
Travel Time (s)		13.9	10.8		1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	192	1816	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	192	1816	0	0	0	0
Sign Control		Free	Stop		Stop	

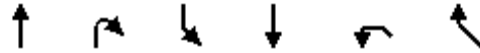
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	110.4%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 36: SB US 321 & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	104	2156	0	0
Future Volume (vph)	0	0	104	2156	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	4940	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	4940	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	154			833	184	
Travel Time (s)	2.3			12.6	2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	116	2396	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	116	2396	0	0
Sign Control	Stop			Free	Stop	

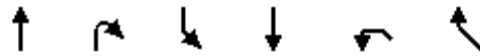
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.6%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 37: SB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	243	2020	0	0
Future Volume (vph)	0	0	243	2020	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	251			840	87	
Travel Time (s)	3.8			12.7	1.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	270	2244	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	270	2244	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	72.0%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 38: NB US 321 & 2nd Avenue NW Left -Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	256	1804	0	0	0	0
Future Volume (vph)	256	1804	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	550			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	4940	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	4940	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		741	152		188	
Travel Time (s)		11.2	2.3		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	2%	2%	2%	2%
Adj. Flow (vph)	284	2004	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	284	2004	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

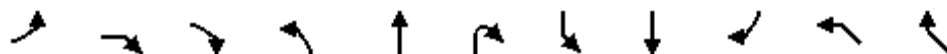
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	110.2%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↗					↕	↗	↖	
Traffic Volume (vph)	0	0	145	0	0	0	0	1934	158	163	0
Future Volume (vph)	0	0	145	0	0	0	0	1934	158	163	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	0	0	0	3471	1553	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	0	0	0	3471	1553	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	410				820			1170		97	
Travel Time (s)	8.0				12.4			17.7		2.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	4%	4%	4%	5%	5%
Adj. Flow (vph)	0	0	161	0	0	0	0	2149	176	181	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	161	0	0	0	0	2149	176	181	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					12.0	12.0	7.0	
Minimum Split (s)			14.0					19.0	19.0	14.0	
Total Split (s)			20.0					70.0	70.0	20.0	
Total Split (%)			22.2%					77.8%	77.8%	22.2%	
Maximum Green (s)			13.0					63.0	63.0	13.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			14.1					65.9	65.9	14.1	
Actuated g/C Ratio			0.16					0.73	0.73	0.16	
v/c Ratio			0.64					0.85	0.15	0.67	
Control Delay			48.0					4.4	1.7	51.2	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			48.0					4.4	1.7	51.2	
LOS			D					A	A	D	
Approach Delay	48.0							4.2		51.2	
Approach LOS	D							A		D	
Queue Length 50th (ft)			86					75	10	108	
Queue Length 95th (ft)			152					84	m12	m167	

Lanes, Volumes, Timings

43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	330				740			1090		17	
Turn Bay Length (ft)											
Base Capacity (vph)			266					2540	1136	286	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.61					0.85	0.15	0.63	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 47 (52%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.85
 Intersection Signal Delay: 10.0 Intersection LOS: B
 Intersection Capacity Utilization 70.8% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

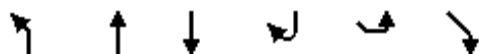
Splits and Phases: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW



Lanes, Volumes, Timings

44: NB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↵	
Traffic Volume (vph)	0	1730	0	0	64	0
Future Volume (vph)	0	1730	0	0	64	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		25	
Link Distance (ft)		715	820		90	
Travel Time (s)		10.8	12.4		2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	1922	0	0	71	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1922	0	0	71	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		12.0			7.0	
Minimum Split (s)		19.0			14.0	
Total Split (s)		76.0			14.0	
Total Split (%)		84.4%			15.6%	
Maximum Green (s)		69.0			7.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		73.6			10.2	
Actuated g/C Ratio		0.82			0.11	
v/c Ratio		0.68			0.37	
Control Delay		3.1			32.1	
Queue Delay		0.0			0.0	
Total Delay		3.1			32.1	
LOS		A			C	
Approach Delay		3.1			32.1	
Approach LOS		A			C	
Queue Length 50th (ft)		111			34	
Queue Length 95th (ft)		94			m45	

Lanes, Volumes, Timings
 44: NB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		635	740		10	
Turn Bay Length (ft)						
Base Capacity (vph)		2857			194	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.67			0.37	

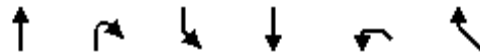
Intersection Summary	
Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	39 (43%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.68
Intersection Signal Delay:	4.2
Intersection LOS:	A
Intersection Capacity Utilization	62.0%
ICU Level of Service	B
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 44: NB US 321 & 7th Avenue NW U-Turn



Lanes, Volumes, Timings
 47: SB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	64	2015	0	0
Future Volume (vph)	0	0	64	2015	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	712			820	90	
Travel Time (s)	10.8			12.4	1.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	71	2239	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	71	2239	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.0%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 48: NB US 321 & 7th Avenue NW Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	163	1631	0	0	0	0
Future Volume (vph)	163	1631	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	3438	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		820	1115		97	
Travel Time (s)		12.4	16.9		1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	4%	4%	5%	5%
Adj. Flow (vph)	181	1812	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	181	1812	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	110.4%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	1056	0	1311	296	0	0	0	978	0
Future Volume (vph)	0	0	1056	0	1311	296	0	0	0	978	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.76	1.00	0.91	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	4940	1538	0	0	0	3367	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	4940	1538	0	0	0	3367	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	400				125			983		205	
Travel Time (s)	7.8				1.9			14.9		5.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	5%	5%	5%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	1173	0	1457	329	0	0	0	1087	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	1173	0	1457	329	0	0	0	1087	0
Turn Type			Prot		NA	Perm				Prot	
Protected Phases			8		2					8	
Permitted Phases			8			2				8	
Detector Phase			8		2	2				8	
Switch Phase											
Minimum Initial (s)			7.0		12.0	12.0				7.0	
Minimum Split (s)			14.0		19.0	19.0				14.0	
Total Split (s)			47.0		43.0	43.0				47.0	
Total Split (%)			52.2%		47.8%	47.8%				52.2%	
Maximum Green (s)			40.0		36.0	36.0				40.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	0.0				-2.0	
Total Lost Time (s)			5.0		5.0	7.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effct Green (s)			39.8		40.2	38.2				39.8	
Actuated g/C Ratio			0.44		0.45	0.42				0.44	
v/c Ratio			0.73		0.66	0.50				0.73	
Control Delay			23.7		9.4	10.0				16.5	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			23.7		9.4	10.0				16.5	
LOS			C		A	B				B	
Approach Delay	23.7				9.5					16.5	
Approach LOS	C				A					B	
Queue Length 50th (ft)			222		132	58				226	
Queue Length 95th (ft)			284		163	m120				m238	

Lanes, Volumes, Timings

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

04/07/2017

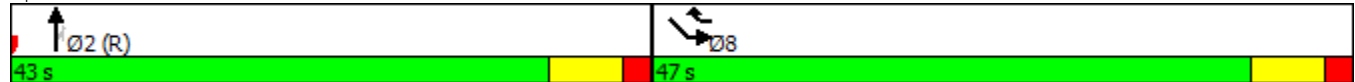


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	320				45			903		125	
Turn Bay Length (ft)											
Base Capacity (vph)			1684		2206	653				1571	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.70		0.66	0.50				0.69	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 4 (4%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 15.5
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lanes, Volumes, Timings

62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗	↘							↖	↗
Traffic Volume (vph)	0	0	46	356	0	0	0	0	0	0	3037	31
Future Volume (vph)	0	0	46	356	0	0	0	0	0	0	3037	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91
Frt			0.865								0.999	
Flt Protected				0.950								
Satd. Flow (prot)	0	0	1596	3367	0	0	0	0	0	0	4935	0
Flt Permitted				0.950								
Satd. Flow (perm)	0	0	1596	3367	0	0	0	0	0	0	4935	0
Right Turn on Red			No	No		No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			25			45				45
Link Distance (ft)		250			97			866				889
Travel Time (s)		4.9			2.6			13.1				13.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	4%	2%	2%	4%	4%	4%	5%	5%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	100
Adj. Flow (vph)	0	0	51	396	0	0	0	0	0	0	3374	34
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	51	396	0	0	0	0	0	0	3408	0
Turn Type			Free	Prot								NA
Protected Phases				8								6
Permitted Phases			Free									
Detector Phase				8								6
Switch Phase												
Minimum Initial (s)				7.0								12.0
Minimum Split (s)				14.0								19.0
Total Split (s)				18.0								72.0
Total Split (%)				20.0%								80.0%
Maximum Green (s)				11.0								65.0
Yellow Time (s)				5.0								5.0
All-Red Time (s)				2.0								2.0
Lost Time Adjust (s)				-2.0								-2.0
Total Lost Time (s)				5.0								5.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)				3.0								3.0
Recall Mode				None								C-Min
Act Effct Green (s)			90.0	13.0								67.0
Actuated g/C Ratio			1.00	0.14								0.74
v/c Ratio			0.03	0.81								0.93
Control Delay			0.0	47.8								15.6
Queue Delay			0.0	0.0								0.0
Total Delay			0.0	47.8								15.6
LOS			A	D								B
Approach Delay					47.8							15.6
Approach LOS					D							B
Queue Length 50th (ft)			0	110								478

Lanes, Volumes, Timings

62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)			0	#184							586	
Internal Link Dist (ft)		170			17			786			809	
Turn Bay Length (ft)												
Base Capacity (vph)			1596	486							3673	
Starvation Cap Reductn			0	0							0	
Spillback Cap Reductn			0	0							0	
Storage Cap Reductn			0	0							0	
Reduced v/c Ratio			0.03	0.81							0.93	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 4 (4%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.93
 Intersection Signal Delay: 18.7
 Intersection LOS: B
 Intersection Capacity Utilization 106.5%
 ICU Level of Service G
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn



Lanes, Volumes, Timings

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations			↑↑					↑↑↑		↑	↑	
Traffic Volume (vph)	0	0	189	0	0	0	0	2210	222	70	57	0
Future Volume (vph)	0	0	189	0	0	0	0	2210	222	70	57	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00	1.00
Fr't			0.850					0.986				
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	4918	0	1770	1770	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	4918	0	1770	1770	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				45			45				25
Link Distance (ft)	910				822			181				143
Travel Time (s)	17.7				12.5			2.7				3.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	5%	5%	5%	4%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	210	0	0	0	0	2456	247	78	63	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	210	0	0	0	0	2703	0	78	63	0
Turn Type			Prot					NA		Prot	Prot	
Protected Phases			7					6		8	8	
Permitted Phases												
Detector Phase			7					6		8	8	
Switch Phase												
Minimum Initial (s)			7.0					12.0		7.0	7.0	
Minimum Split (s)			14.0					19.0		14.0	14.0	
Total Split (s)			14.0					62.0		14.0	14.0	
Total Split (%)			15.6%					68.9%		15.6%	15.6%	
Maximum Green (s)			7.0					55.0		7.0	7.0	
Yellow Time (s)			5.0					5.0		5.0	5.0	
All-Red Time (s)			2.0					2.0		2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0		-2.0	-2.0	
Total Lost Time (s)			5.0					5.0		5.0	5.0	
Lead/Lag			Lead							Lag	Lag	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0					3.0		3.0	3.0	
Recall Mode			None					C-Min		None	None	
Act Effct Green (s)			9.4					59.4		9.0	9.0	
Actuated g/C Ratio			0.10					0.66		0.10	0.10	
v/c Ratio			0.73					0.83		0.44	0.36	
Control Delay			55.5					9.0		43.0	41.0	
Queue Delay			0.0					0.0		0.0	0.0	
Total Delay			55.5					9.0		43.0	41.0	
LOS			E					A		D	D	
Approach Delay	55.5							9.0				42.1
Approach LOS	E							A				D
Queue Length 50th (ft)			67					298		34	27	
Queue Length 95th (ft)			#127					m372		m55	m44	

Lanes, Volumes, Timings

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Internal Link Dist (ft)	830				742			101			63	
Turn Bay Length (ft)												
Base Capacity (vph)			288					3246		177	177	
Starvation Cap Reductn			0					0		0	0	
Spillback Cap Reductn			0					0		0	0	
Storage Cap Reductn			0					0		0	0	
Reduced v/c Ratio			0.73					0.83		0.44	0.36	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	24 (27%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.83
Intersection Signal Delay:	13.7
Intersection LOS:	B
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lanes, Volumes, Timings

64: NB US 321 & 9th Avenue NW & Clement Boulevard

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↔↔		↑↑↑						↔	
Traffic Volume (vph)	0	0	138	0	1612	52	0	0	0	142	62	0
Future Volume (vph)	0	0	138	0	1612	52	0	0	0	142	62	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		375	0		0		0	0
Storage Lanes	0	1		0		1	0		0		1	0
Taper Length (ft)	50			50			50				50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.995							
Flt Protected											0.950	
Satd. Flow (prot)	0	0	2733	0	4963	0	0	0	0	0	1736	0
Flt Permitted											0.950	
Satd. Flow (perm)	0	0	2733	0	4963	0	0	0	0	0	1736	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				45			45			25	
Link Distance (ft)	319				1115			832			149	
Travel Time (s)	6.2				16.9			12.6			4.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	153	0	1791	58	0	0	0	158	69	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	153	0	1849	0	0	0	0	0	227	0
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			8		2					4	4	
Permitted Phases												
Detector Phase			8		2					4	4	
Switch Phase												
Minimum Initial (s)			7.0		12.0					7.0	7.0	
Minimum Split (s)			14.0		19.0					14.0	14.0	
Total Split (s)			15.0		50.0					25.0	25.0	
Total Split (%)			16.7%		55.6%					27.8%	27.8%	
Maximum Green (s)			8.0		43.0					18.0	18.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Lost Time Adjust (s)			-2.0		-2.0						-2.0	
Total Lost Time (s)			5.0		5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effect Green (s)			10.2		47.2						17.6	
Actuated g/C Ratio			0.11		0.52						0.20	
v/c Ratio			0.49		0.71						0.67	
Control Delay			43.3		12.1						45.9	
Queue Delay			0.0		0.0						0.0	
Total Delay			43.3		12.1						45.9	
LOS			D		B						D	
Approach Delay	43.3				12.1						45.9	

Lanes, Volumes, Timings

64: NB US 321 & 9th Avenue NW & Clement Boulevard

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Approach LOS	D				B				D			
Queue Length 50th (ft)				46		280					108	
Queue Length 95th (ft)				81		227					m135	
Internal Link Dist (ft)	239					1035		752				
Turn Bay Length (ft)				250								
Base Capacity (vph)				316		2600					385	
Starvation Cap Reductn				0		0					0	
Spillback Cap Reductn				0		0					0	
Storage Cap Reductn				0		0					0	
Reduced v/c Ratio				0.48		0.71					0.59	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	75 (83%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.71
Intersection Signal Delay:	17.7
Intersection LOS:	B
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 64: NB US 321 & 9th Avenue NW & Clement Boulevard



Lanes, Volumes, Timings
 65: NB US 321 & Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↗↗	↖↖↖
Traffic Volume (vph)	0	0	0	0	356	2050
Future Volume (vph)	0	0	0	0	356	2050
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	300	
Storage Lanes	0	0		0	2	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.97	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	3367	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	3367	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	97		939			983
Travel Time (s)	2.6		14.2			14.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	4%	4%
Adj. Flow (vph)	0	0	0	0	396	2278
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	396	2278
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	106.5%
Analysis Period (min)	15
	ICU Level of Service G

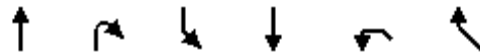
Intersection has too many lanes per leg.

HCM All-Way analysis is limited to two lanes per leg.

Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
 66: SB US 321 & Clement Boulevard Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↔↔	↑↑↑		
Traffic Volume (vph)	0	0	978	2432	0	0
Future Volume (vph)	0	0	978	2432	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	400		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3367	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3367	4988	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	181			866	205	
Travel Time (s)	2.7			13.1	2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	1087	2702	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1087	2702	0	0
Sign Control	Stop			Free	Stop	

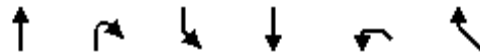
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	50.3%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 67: SB US 321 & Clement Boulevard

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	204	2048	0	0
Future Volume (vph)	0	0	204	2048	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	1170			822	149	
Travel Time (s)	17.7			12.5	1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	227	2276	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	227	2276	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	42.9%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
68: NB US 321 & Clement Boulevard Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	127	1607	0	0	0	0
Future Volume (vph)	127	1607	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	400			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	0.86	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3335	6225	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3335	6225	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		832	125		143	
Travel Time (s)		12.6	1.9		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	4%	42%	5%	5%
Adj. Flow (vph)	141	1786	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	141	1786	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	26.6%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑		↑↑↑					↑	
Traffic Volume (vph)	0	0	63	0	1984	28	0	0	0	115	0
Future Volume (vph)	0	0	63	0	1984	28	0	0	0	115	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998						
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	4932	0	0	0	0	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	4932	0	0	0	0	1736	0
Right Turn on Red			No			No				No	No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	478				179			784		210	
Travel Time (s)	9.3				2.7			11.9		5.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	2%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	70	0	2204	31	0	0	0	128	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	70	0	2235	0	0	0	0	128	0
Turn Type			Perm		NA					Prot	
Protected Phases					2					4	
Permitted Phases			8								
Detector Phase			8		2					4	
Switch Phase											
Minimum Initial (s)			7.0		12.0					7.0	
Minimum Split (s)			14.0		19.0					14.0	
Total Split (s)			22.0		68.0					22.0	
Total Split (%)			24.4%		75.6%					24.4%	
Maximum Green (s)			15.0		61.0					15.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effct Green (s)			13.6		66.4					13.6	
Actuated g/C Ratio			0.15		0.74					0.15	
v/c Ratio			0.17		0.61					0.49	
Control Delay			32.9		3.9					40.8	
Queue Delay			0.0		0.0					0.0	
Total Delay			32.9		3.9					40.8	
LOS			C		A					D	
Approach Delay	32.9				3.9					40.8	
Approach LOS	C				A					D	
Queue Length 50th (ft)			19		62					67	
Queue Length 95th (ft)			39		197					118	

Lanes, Volumes, Timings

81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

04/07/2017

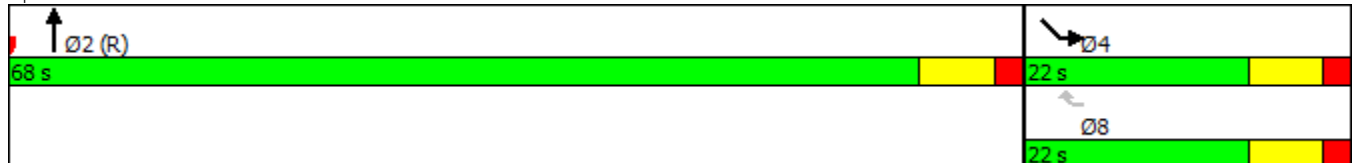


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	398				99			704		130	
Turn Bay Length (ft)											
Base Capacity (vph)			526		3638					327	
Starvation Cap Reductn			0		0					0	
Spillback Cap Reductn			0		0					0	
Storage Cap Reductn			0		0					0	
Reduced v/c Ratio			0.13		0.61					0.39	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	16 (18%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	40
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.61
Intersection Signal Delay:	6.7
Intersection LOS:	A
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15

Splits and Phases: 81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over



Lanes, Volumes, Timings
 82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	18	0	0	0	0	3094
Future Volume (vph)	18	0	0	0	0	3094
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	1719	0	0	0	0	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	85		652			641
Travel Time (s)	2.3		9.9			9.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	2%	4%	4%	4%	4%
Adj. Flow (vph)	20	0	0	0	0	3438
Shared Lane Traffic (%)						
Lane Group Flow (vph)	20	0	0	0	0	3438
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.7%
Analysis Period (min)	15
	ICU Level of Service G

HCM Unsignalized Intersection Capacity Analysis

82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	18	0	0	0	0	3094
Future Volume (Veh/h)	18	0	0	0	0	3094
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	20	0	0	0	0	3438
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1146	0			0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1146	0			0	
tC, single (s)	6.9	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	89	100			100	
cM capacity (veh/h)	188	1084			1607	
Direction, Lane #	WB 1	SB 1	SB 2	SB 3		
Volume Total	20	1146	1146	1146		
Volume Left	20	0	0	0		
Volume Right	0	0	0	0		
cSH	188	1700	1700	1700		
Volume to Capacity	0.11	0.67	0.67	0.67		
Queue Length 95th (ft)	9	0	0	0		
Control Delay (s)	26.4	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	26.4	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			105.7%		ICU Level of Service	G
Analysis Period (min)			15			

HCM 2010 TWSC
 82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔					↑↑↑
Traffic Vol, veh/h	18	0	0	0	0	3094
Future Vol, veh/h	18	0	0	0	0	3094
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	2	4	4	4	4
Mvmt Flow	20	0	0	0	0	3438

Major/Minor

	Minor1		Major2
Conflicting Flow All	1375	-	-
Stage 1	0	-	-
Stage 2	1375	-	-
Critical Hdwy	5.8	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	6.1	-	-
Follow-up Hdwy	3.85	-	-
Pot Cap-1 Maneuver	195	0	0
Stage 1	-	0	0
Stage 2	174	0	0
Platoon blocked, %			-
Mov Cap-1 Maneuver	195	-	-
Mov Cap-2 Maneuver	195	-	-
Stage 1	-	-	-
Stage 2	174	-	-

Approach

	WB	SB
HCM Control Delay, s	25.6	0
HCM LOS	D	

Minor Lane/Major Mvmt

	WBLn1	SBT
Capacity (veh/h)	195	-
HCM Lane V/C Ratio	0.103	-
HCM Control Delay (s)	25.6	-
HCM Lane LOS	D	-
HCM 95th %tile Q(veh)	0.3	-

Lanes, Volumes, Timings

83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	29	0	0	0	0	2965	32	20	0
Future Volume (vph)	0	0	29	0	0	0	0	2965	32	20	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00
Frt			0.865					0.998			
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1565	0	0	0	0	4978	0	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1565	0	0	0	0	4978	0	1736	0
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	560				842			224		173	
Travel Time (s)	10.9				12.8			3.4		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	4%	4%	4%	4%	4%	4%	4%	0%
Adj. Flow (vph)	0	0	32	0	0	0	0	3294	36	22	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	32	0	0	0	0	3330	0	22	0
Sign Control	Stop				Free			Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	68.0%
Analysis Period (min)	15
	ICU Level of Service C

HCM Unsignalized Intersection Capacity Analysis

83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

04/07/2017



Movement	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (veh/h)	0	0	29	0	0	0	0	2965	32	20	0
Future Volume (Veh/h)	0	0	29	0	0	0	0	2965	32	20	0
Sign Control	Stop			Free			Free			Stop	
Grade	0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	32	0	0	0	0	3294	36	22	0
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type											
Median storage veh											
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	3323	3312	1116	3330			0			3330	0
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
vCu, unblocked vol	3323	3312	1116	3330			0			3330	0
tC, single (s)	7.6	6.6	7.0	4.2			4.2			6.6	6.9
tC, 2 stage (s)											
tF (s)	3.5	4.0	3.3	2.2			2.2			4.0	3.3
p0 queue free %	0	100	84	100			100			0	100
cM capacity (veh/h)	0	8	197	80			1607			8	1091

Direction, Lane #	EB 1	SB 1	SB 2	SB 3	NW 1
Volume Total	32	1318	1318	695	22
Volume Left	0	0	0	0	0
Volume Right	32	0	0	36	0
cSH	197	1700	1700	1700	8
Volume to Capacity	0.16	0.78	0.78	0.41	2.84
Queue Length 95th (ft)	14	0	0	0	97
Control Delay (s)	26.8	0.0	0.0	0.0	1756.3
Lane LOS	D				F
Approach Delay (s)	26.8	0.0			1756.3
Approach LOS	D				F

Intersection Summary	
Average Delay	11.7
Intersection Capacity Utilization	68.0%
ICU Level of Service	C
Analysis Period (min)	15

Intersection

Int Delay, s/veh 1.3

Movement	EBL	EBR	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations							↑↑↑		↑	
Traffic Vol, veh/h	0	0	0	0	0	0	2965	32	20	0
Future Vol, veh/h	0	0	0	0	0	0	2965	32	20	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	None	-	-	None	-	None
Storage Length	-	0	-	-	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	-	-	-	0	-	0	-
Grade, %	0	-	-	0	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	4	4	4	4	4	4	4	0
Mvmt Flow	0	0	0	0	0	0	3294	36	22	0

Major/Minor	Minor2	Major2	Minor1				
Conflicting Flow All	-	1665	-	-	0	1318	-
Stage 1	-	-	-	-	-	0	-
Stage 2	-	-	-	-	-	1318	-
Critical Hdwy	-	7.2	-	-	-	6.48	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	6.78	-
Follow-up Hdwy	-	3.95	-	-	-	3.84	-
Pot Cap-1 Maneuver	0	71	0	-	-	163	0
Stage 1	0	-	0	-	-	-	0
Stage 2	0	-	0	-	-	146	0
Platoon blocked, %							
Mov Cap-1 Maneuver	-	71	-	-	-	89	-
Mov Cap-2 Maneuver	-	-	-	-	-	89	-
Stage 1	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	80	-

Approach	EB	SB	NW
HCM Control Delay, s	92.4	0	58.4
HCM LOS	F		F

Minor Lane/Major Mvmt	NWLn1	EBLn1	SBT	SBR
Capacity (veh/h)	89	71	-	-
HCM Lane V/C Ratio	0.25	0.454	-	-
HCM Control Delay (s)	58.4	92.4	-	-
HCM Lane LOS	F	F	-	-
HCM 95th %tile Q(veh)	0.9	1.8	-	-

Lanes, Volumes, Timings
 84: NB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	16	0	0	2016	0	0
Future Volume (vph)	16	0	0	2016	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	0	0	4940	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	0	0	4940	0	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	53			939	719	
Travel Time (s)	1.4			14.2	10.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	2%	5%	5%	4%	4%
Adj. Flow (vph)	18	0	0	2240	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	18	0	0	2240	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	49.0%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis

84: NB US 321 & 15th Avenue NW U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	16	0	0	2016	0	0
Future Volume (Veh/h)	16	0	0	2016	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	18	0	0	2240	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	747	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	747	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	100	100			
cM capacity (veh/h)	343	1084	1600			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	18	747	747	747		
Volume Left	18	0	0	0		
Volume Right	0	0	0	0		
cSH	343	1700	1700	1700		
Volume to Capacity	0.05	0.44	0.44	0.44		
Queue Length 95th (ft)	4	0	0	0		
Control Delay (s)	16.1	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	16.1	0.0				
Approach LOS	C					
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			49.0%		ICU Level of Service	A
Analysis Period (min)			15			

HCM 2010 TWSC
 84: NB US 321 & 15th Avenue NW U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	16	0	0	2016	0	0
Future Vol, veh/h	16	0	0	2016	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	2	5	5	4	4
Mvmt Flow	18	0	0	2240	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	896	-	-	0
Stage 1	0	-	-	-
Stage 2	896	-	-	-
Critical Hdwy	5.8	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	-
Follow-up Hdwy	3.85	-	-	-
Pot Cap-1 Maneuver	343	0	0	-
Stage 1	-	0	0	-
Stage 2	318	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	343	-	-	-
Mov Cap-2 Maneuver	343	-	-	-
Stage 1	-	-	-	-
Stage 2	318	-	-	-

Approach

	EB	NB
HCM Control Delay, s	16.1	0
HCM LOS	C	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	343
HCM Lane V/C Ratio	-	0.052
HCM Control Delay (s)	-	16.1
HCM Lane LOS	-	C
HCM 95th %tile Q(veh)	-	0.2

Lanes, Volumes, Timings
 85: NB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	18	2029	0	0
Future Volume (vph)	0	0	18	2029	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	4988	0	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	85			784	717	
Travel Time (s)	2.3			11.9	10.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	5%	4%	4%	4%
Adj. Flow (vph)	0	0	20	2254	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	20	2254	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.7%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 86: SB US 321 & 15th Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↑↑↑
Traffic Volume (vph)	0	0	0	0	115	2997
Future Volume (vph)	0	0	0	0	115	2997
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	55		45			45
Link Distance (ft)	210		224			652
Travel Time (s)	2.6		3.4			9.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	128	3330
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	128	3330
Sign Control	Stop		Stop			Free

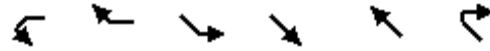
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	69.8%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 87: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	16	2978	0	0
Future Volume (vph)	0	0	16	2978	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	250			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			45	45	
Link Distance (ft)	53			842	889	
Travel Time (s)	0.7			12.8	13.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	5%	5%
Adj. Flow (vph)	0	0	18	3309	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	18	3309	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	60.9%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 88: NB US 321 & 15th Avenue NW Left-Over

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↗	↑↑↑
Traffic Volume (vph)	0	0	0	0	20	2012
Future Volume (vph)	0	0	0	0	20	2012
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	173		179			719
Travel Time (s)	4.7		2.7			10.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	22	2236
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	22	2236
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	92.3%
Analysis Period (min)	15
	ICU Level of Service F

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

91: US 321 & Grace Chapel Road

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	420	34	1829	200	0	2713
Future Volume (vph)	420	34	1829	200	0	2713
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	500	500		550	0	
Storage Lanes	1	1		1	0	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	0.88	0.91	1.00	1.00	0.91
Frt		0.850		0.850		
Flt Protected	0.950					
Satd. Flow (prot)	3433	2787	4988	1553	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3433	2787	4988	1553	0	4988
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		55			55
Link Distance (ft)	986		624			167
Travel Time (s)	19.2		7.7			2.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	467	38	2032	222	0	3014
Shared Lane Traffic (%)						
Lane Group Flow (vph)	467	38	2032	222	0	3014
Turn Type	Prot	Prot	NA	Free		NA
Protected Phases	8	8	2			6
Permitted Phases				Free		
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	7.0	7.0	14.0			14.0
Minimum Split (s)	14.0	14.0	21.0			21.0
Total Split (s)	22.0	22.0	68.0			68.0
Total Split (%)	24.4%	24.4%	75.6%			75.6%
Maximum Green (s)	15.0	15.0	61.0			61.0
Yellow Time (s)	5.0	5.0	5.0			5.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0			-2.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	16.6	16.6	63.4	90.0		63.4
Actuated g/C Ratio	0.18	0.18	0.70	1.00		0.70
v/c Ratio	0.74	0.07	0.58	0.14		0.86
Control Delay	42.5	30.6	6.5	0.2		13.4
Queue Delay	0.0	0.0	0.0	0.0		0.0
Total Delay	42.5	30.6	6.5	0.2		13.4
LOS	D	C	A	A		B
Approach Delay	41.6		5.9			13.4

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	D		A		B	
Queue Length 50th (ft)	129	10	223	0	407	
Queue Length 95th (ft)	182	25	148	0	489	
Internal Link Dist (ft)	906		544		87	
Turn Bay Length (ft)	500	500	550			
Base Capacity (vph)	648	526	3514	1553	3514	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.72	0.07	0.58	0.14	0.86	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green, Master Intersection
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.86
Intersection Signal Delay:	12.9
Intersection LOS:	B
Intersection Capacity Utilization	72.7%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 91: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↶					↑↑↑
Traffic Volume (vph)	4	0	0	0	0	2713
Future Volume (vph)	4	0	0	0	0	2713
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	1	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	0	0	4988
Link Speed (mph)	25		55			55
Link Distance (ft)	91		428			2189
Travel Time (s)	2.5		5.3			27.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	4	0	0	0	0	3014
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	0	0	0	0	3014
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.4%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis

92: Grace Chapel Road U-Turn & SB US 321

04/07/2017



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	4	0	0	0	0	2713
Future Volume (Veh/h)	4	0	0	0	0	2713
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	4	0	0	0	0	3014
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1005	0			0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1005	0			0	
tC, single (s)	6.9	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	100			100	
cM capacity (veh/h)	235	1084			1607	
Direction, Lane #	WB 1	SB 1	SB 2	SB 3		
Volume Total	4	1005	1005	1005		
Volume Left	4	0	0	0		
Volume Right	0	0	0	0		
cSH	235	1700	1700	1700		
Volume to Capacity	0.02	0.59	0.59	0.59		
Queue Length 95th (ft)	1	0	0	0		
Control Delay (s)	20.6	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	20.6	0.0				
Approach LOS	C					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			62.4%		ICU Level of Service	B
Analysis Period (min)			15			

HCM 2010 TWSC
 92: Grace Chapel Road U-Turn & SB US 321

04/07/2017

Intersection

Int Delay, s/veh 0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘					↑↑↑
Traffic Vol, veh/h	4	0	0	0	0	2713
Future Vol, veh/h	4	0	0	0	0	2713
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	500	-
Veh in Median Storage, #	0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	4	0	0	0	0	3014

Major/Minor

	Minor1		Major2	
Conflicting Flow All	1206	-	-	-
Stage 1	0	-	-	-
Stage 2	1206	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	240	0	0	-
Stage 1	-	0	0	-
Stage 2	217	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	240	-	-	-
Mov Cap-2 Maneuver	240	-	-	-
Stage 1	-	-	-	-
Stage 2	217	-	-	-

Approach

	WB	SB
HCM Control Delay, s	20.3	0
HCM LOS	C	

Minor Lane/Major Mvmt

	WBLn1	SBT
Capacity (veh/h)	240	-
HCM Lane V/C Ratio	0.019	-
HCM Control Delay (s)	20.3	-
HCM Lane LOS	C	-
HCM 95th %tile Q(veh)	0.1	-

Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	21	0	0	2008	0	0
Future Volume (vph)	21	0	0	2008	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			45	55	
Link Distance (ft)	169			796	599	
Travel Time (s)	4.6			12.1	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	23	0	0	2231	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	23	0	0	2231	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.6%
ICU Level of Service	G
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↶			↷↷↷		
Traffic Volume (veh/h)	21	0	0	2008	0	0
Future Volume (Veh/h)	21	0	0	2008	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	23	0	0	2231	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	744	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	744	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	100	100			
cM capacity (veh/h)	346	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	23	744	744	744		
Volume Left	23	0	0	0		
Volume Right	0	0	0	0		
cSH	346	1700	1700	1700		
Volume to Capacity	0.07	0.44	0.44	0.44		
Queue Length 95th (ft)	5	0	0	0		
Control Delay (s)	16.1	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	16.1	0.0				
Approach LOS	C					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			105.6%		ICU Level of Service	G
Analysis Period (min)			15			

HCM 2010 TWSC
 93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	21	0	0	2008	0	0
Future Vol, veh/h	21	0	0	2008	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	23	0	0	2231	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	892	-	-	0
Stage 1	0	-	-	-
Stage 2	892	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	347	0	0	-
Stage 1	-	0	0	-
Stage 2	322	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	347	-	-	-
Mov Cap-2 Maneuver	347	-	-	-
Stage 1	-	-	-	-
Stage 2	322	-	-	-

Approach

	EB	NB
HCM Control Delay, s	16.1	0
HCM LOS	C	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	347
HCM Lane V/C Ratio	-	0.067
HCM Control Delay (s)	-	16.1
HCM Lane LOS	-	C
HCM 95th %tile Q(veh)	-	0.2

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	4	1863	0	0
Future Volume (vph)	0	0	4	1863	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		40			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	91			347	2226	
Travel Time (s)	2.1			4.3	27.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	4	2070	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	4	2070	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.4%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	21	3112
Future Volume (vph)	0	0	0	0	21	3112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	350	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	30		45			55
Link Distance (ft)	169		960			435
Travel Time (s)	3.8		14.5			5.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	23	3458
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	23	3458
Sign Control	Stop		Stop			Free

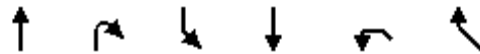
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	123.5%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 96: SB US 321/US 321 & NB US 321

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑↑		↑↑↑↑
Traffic Volume (vph)	0	0	0	3133	0	2029
Future Volume (vph)	0	0	0	3133	0	2029
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	4988	0	3541
Flt Permitted						
Satd. Flow (perm)	0	0	0	4988	0	3541
Link Speed (mph)	55			55	55	
Link Distance (ft)	435			624	599	
Travel Time (s)	5.4			7.7	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	3481	0	2254
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3481	0	2254
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	114.5%
ICU Level of Service	H
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 97: US 321 & NB US 321

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	1863	0	0	0	2713
Future Volume (vph)	0	1863	0	0	0	2713
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		167	347		428	
Travel Time (s)		2.1	4.3		5.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	4%	4%
Adj. Flow (vph)	0	2070	0	0	0	3014
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2070	0	0	0	3014
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	66.6%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↑↑	↖				↘	
Traffic Volume (vph)	0	0	110	0	1560	74	0	0	0	8	0
Future Volume (vph)	0	0	110	0	1560	74	0	0	0	8	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0		0	250		0	0	0
Storage Lanes	0	2		0		1	0		0	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1895	0	3471	1442	0	0	0	1612	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1895	0	3471	1442	0	0	0	1612	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	682				200			676		124	
Travel Time (s)	13.3				2.5			8.4		3.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	12%	12%	50%	2%	4%	12%	12%	4%	4%	12%	4%
Adj. Flow (vph)	0	0	122	0	1733	82	0	0	0	9	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	122	0	1733	82	0	0	0	9	0
Turn Type			Perm		NA	Perm				Prot	
Protected Phases					2					4	
Permitted Phases			8			2					
Detector Phase			8		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		14.0	14.0				7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	
Total Split (s)			19.0		71.0	71.0				19.0	
Total Split (%)			21.1%		78.9%	78.9%				21.1%	
Maximum Green (s)			12.0		64.0	64.0				12.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effect Green (s)			12.4		67.6	67.6				12.4	
Actuated g/C Ratio			0.14		0.75	0.75				0.14	
v/c Ratio			0.47		0.67	0.08				0.04	
Control Delay			41.3		2.9	0.7				31.5	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			41.3		2.9	0.7				31.5	
LOS			D		A	A				C	
Approach Delay	41.3				2.8					31.5	

Lanes, Volumes, Timings

101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Approach LOS	D			A			C				
Queue Length 50th (ft)			36	41		3					4
Queue Length 95th (ft)			66	12		m2					m4
Internal Link Dist (ft)	602			120			596			44	
Turn Bay Length (ft)											
Base Capacity (vph)			297	2611		1084					253
Starvation Cap Reductn			0	0		0					0
Spillback Cap Reductn			0	0		0					0
Storage Cap Reductn			0	0		0					0
Reduced v/c Ratio			0.41	0.66		0.08					0.04

Intersection Summary

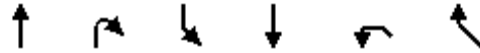
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 44 (49%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 5.4
 Intersection LOS: A
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over



Lanes, Volumes, Timings
 102: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	2357	96	0
Future Volume (vph)	0	0	0	2357	96	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3505	1203	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3505	1203	0
Link Speed (mph)	30			55	25	
Link Distance (ft)	653			1220	102	
Travel Time (s)	14.8			15.1	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	3%	50%	2%
Adj. Flow (vph)	0	0	0	2619	107	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2619	107	0
Sign Control	Free			Free	Stop	

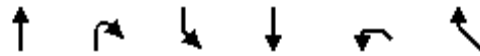
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	115.3%
Analysis Period (min)	15
	ICU Level of Service H

HCM Unsignalized Intersection Capacity Analysis

102: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↗	
Traffic Volume (veh/h)	0	0	0	2357	96	0
Future Volume (Veh/h)	0	0	0	2357	96	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	2619	107	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			0	1310	0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0	1310	0	
tC, single (s)			4.2	7.8	6.9	
tC, 2 stage (s)						
tF (s)			2.2	4.0	3.3	
p0 queue free %			100	0	100	
cM capacity (veh/h)			1607	100	1084	
Direction, Lane #	SB 1	SB 2	NW 1			
Volume Total	1310	1310	107			
Volume Left	0	0	107			
Volume Right	0	0	0			
cSH	1700	1700	100			
Volume to Capacity	0.77	0.77	1.07			
Queue Length 95th (ft)	0	0	170			
Control Delay (s)	0.0	0.0	189.0			
Lane LOS			F			
Approach Delay (s)	0.0		189.0			
Approach LOS			F			
Intersection Summary						
Average Delay			7.4			
Intersection Capacity Utilization			115.3%	ICU Level of Service	H	
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 7.4

Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↑	
Traffic Vol, veh/h	0	0	0	2357	96	0
Future Vol, veh/h	0	0	0	2357	96	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	3	50	2
Mvmt Flow	0	0	0	2619	107	0

Major/Minor

	Major2	Minor1
Conflicting Flow All	-	1309
Stage 1	-	0
Stage 2	-	1309
Critical Hdwy	-	7.8
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	6.8
Follow-up Hdwy	-	4
Pot Cap-1 Maneuver	0	~ 100
Stage 1	0	-
Stage 2	0	144
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	~ 100
Mov Cap-2 Maneuver	-	~ 100
Stage 1	-	-
Stage 2	-	144

Approach

	SB	NW
HCM Control Delay, s	0	188.3
HCM LOS		F

Minor Lane/Major Mvmt

	NWLn1	SBT
Capacity (veh/h)	100	-
HCM Lane V/C Ratio	1.067	-
HCM Control Delay (s)	188.3	-
HCM Lane LOS	F	-
HCM 95th %tile Q(veh)	6.8	-

Notes

-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lanes, Volumes, Timings

103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↗					↕	↘	↙	↘
Traffic Volume (vph)	0	0	4	0	0	0	0	2432	13	13	0
Future Volume (vph)	0	0	4	0	0	0	0	2432	13	13	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1565	0	0	0	0	3471	1553	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1565	0	0	0	0	3471	1553	1719	0
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	227				766			154		171	
Travel Time (s)	4.4				17.4			1.9		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	4%	4%	4%	4%	4%	4%	5%	2%
Adj. Flow (vph)	0	0	4	0	0	0	0	2702	14	14	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	4	0	0	0	0	2702	14	14	0
Sign Control	Stop				Free			Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	77.2%
Analysis Period (min)	15
	ICU Level of Service D

HCM Unsignalized Intersection Capacity Analysis

103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

04/07/2017



Movement	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↗					↕	↗	↖	
Traffic Volume (veh/h)	0	0	4	0	0	0	0	2432	13	13	0
Future Volume (Veh/h)	0	0	4	0	0	0	0	2432	13	13	0
Sign Control	Stop			Free			Free			Stop	
Grade	0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	4	0	0	0	0	2702	14	14	0
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type	None					None					
Median storage (veh)											
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2709	2702	1351	2716			0			2716	0
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
vCu, unblocked vol	2709	2702	1351	2716			0			2716	0
tC, single (s)	7.6	6.6	7.0	4.2			4.2			6.6	6.9
tC, 2 stage (s)											
tF (s)	3.5	4.0	3.3	2.2			2.2			4.0	3.3
p0 queue free %	100	100	97	100			100			29	100
cM capacity (veh/h)	4	20	137	142			1607			20	1084
Direction, Lane #	EB 1	SB 1	SB 2	SB 3	NW 1						
Volume Total	4	1351	1351	14	14						
Volume Left	0	0	0	0	0						
Volume Right	4	0	0	14	0						
cSH	137	1700	1700	1700	20						
Volume to Capacity	0.03	0.79	0.79	0.01	0.71						
Queue Length 95th (ft)	2	0	0	0	49						
Control Delay (s)	32.2	0.0	0.0	0.0	375.6						
Lane LOS	D				F						
Approach Delay (s)	32.2	0.0			375.6						
Approach LOS	D				F						
Intersection Summary											
Average Delay			2.0								
Intersection Capacity Utilization			77.2%		ICU Level of Service				D		
Analysis Period (min)			15								

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations							↑↑	↑	↑	
Traffic Vol, veh/h	0	0	0	0	0	0	2432	13	13	0
Future Vol, veh/h	0	0	0	0	0	0	2432	13	13	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	None	-	-	None	-	None
Storage Length	-	0	-	-	-	-	-	0	0	-
Veh in Median Storage, #	0	-	-	-	-	-	0	-	0	-
Grade, %	0	-	-	0	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	4	4	4	4	4	4	5	2
Mvmt Flow	0	0	0	0	0	0	2702	14	14	0

Major/Minor	Minor2	Major2	Minor1				
Conflicting Flow All	-	1351	-	-	0	1351	-
Stage 1	-	-	-	0	-	-	-
Stage 2	-	-	-	1351	-	-	-
Critical Hdwy	-	7	-	-	-	7.6	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	6.6	-
Follow-up Hdwy	-	3.35	-	-	-	3.55	-
Pot Cap-1 Maneuver	0	137	0	-	-	106	0
Stage 1	0	-	0	-	-	-	0
Stage 2	0	-	0	-	-	154	0
Platoon blocked, %							
Mov Cap-1 Maneuver	-	137	-	-	-	103	-
Mov Cap-2 Maneuver	-	-	-	-	-	103	-
Stage 1	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	149	-

Approach	EB	SB	NW
HCM Control Delay, s	32.2	0	45.6
HCM LOS	D		E

Minor Lane/Major Mvmt	NWLn1	EBLn1	SBT	SBR
Capacity (veh/h)	103	137	-	-
HCM Lane V/C Ratio	0.14	0.032	-	-
HCM Control Delay (s)	45.6	32.2	-	-
HCM Lane LOS	E	D	-	-
HCM 95th %tile Q(veh)	0.5	0.1	-	-

Lanes, Volumes, Timings
 104: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	4	0	0	1646	0	0
Future Volume (vph)	4	0	0	1646	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	57			2226	645	
Travel Time (s)	1.6			27.6	8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	4	0	0	1829	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	0	0	1829	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	41.8%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis

104: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↶			↷↷↷		
Traffic Volume (veh/h)	4	0	0	1646	0	0
Future Volume (Veh/h)	4	0	0	1646	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	4	0	0	1829	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	610	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	610	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	422	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	4	610	610	610		
Volume Left	4	0	0	0		
Volume Right	0	0	0	0		
cSH	422	1700	1700	1700		
Volume to Capacity	0.01	0.36	0.36	0.36		
Queue Length 95th (ft)	1	0	0	0		
Control Delay (s)	13.6	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	13.6	0.0				
Approach LOS	B					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			41.8%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	4	0	0	1646	0	0
Future Vol, veh/h	4	0	0	1646	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	4	0	0	1829	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	732	-	-	0
Stage 1	0	-	-	-
Stage 2	732	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	417	0	0	-
Stage 1	-	0	0	-
Stage 2	392	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	417	-	-	-
Mov Cap-2 Maneuver	417	-	-	-
Stage 1	-	-	-	-
Stage 2	392	-	-	-

Approach

	EB	NB
HCM Control Delay, s	13.7	0
HCM LOS	B	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	417
HCM Lane V/C Ratio	-	0.011
HCM Control Delay (s)	-	13.7
HCM Lane LOS	-	B
HCM 95th %tile Q(veh)	-	0

Lanes, Volumes, Timings
 105: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	96	1574	0	0	0	0
Future Volume (vph)	96	1574	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1203	3471	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1203	3471	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		676	1243		102	
Travel Time (s)		8.4	15.4		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	50%	4%	4%	4%	2%	2%
Adj. Flow (vph)	107	1749	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	1749	0	0	0	0
Sign Control		Free	Stop		Stop	

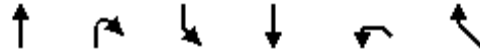
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	115.3%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 106: SB US 321 & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	8	2445	0	0
Future Volume (vph)	0	0	8	2445	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	400		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1612	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1612	3471	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	154			653	124	
Travel Time (s)	3.5			8.1	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	12%	4%	2%	2%
Adj. Flow (vph)	0	0	9	2717	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	9	2717	0	0
Sign Control	Stop			Free	Stop	

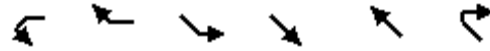
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	77.1% ICU Level of Service D
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 107: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations			↙	↕↕		
Traffic Volume (vph)	0	0	4	2434	0	0
Future Volume (vph)	0	0	4	2434	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	250			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	57			766	2189	
Travel Time (s)	1.3			9.5	49.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	4	2704	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	4	2704	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	51.0% ICU Level of Service A
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 108: NB US 321 & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	13	1634	0	0	0	0
Future Volume (vph)	13	1634	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		25	
Link Distance (ft)		645	200		171	
Travel Time (s)		8.0	2.5		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	2%	2%
Adj. Flow (vph)	14	1816	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	14	1816	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	77.9%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	948	0	1240	404	0	0	0	409	0
Future Volume (vph)	0	0	948	0	1240	404	0	0	0	409	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.76	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	3471	1553	0	0	0	3367	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	3471	1553	0	0	0	3367	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	489				152			678		131	
Travel Time (s)	9.5				1.9			8.4		3.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%	4%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	1053	0	1378	449	0	0	0	454	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	1053	0	1378	449	0	0	0	454	0
Turn Type			Perm		NA	Perm				Prot	
Protected Phases					2					4	
Permitted Phases			8			2					
Detector Phase			8		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		14.0	14.0				7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	
Total Split (s)			39.0		51.0	51.0				39.0	
Total Split (%)			43.3%		56.7%	56.7%				43.3%	
Maximum Green (s)			32.0		44.0	44.0				32.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effct Green (s)			32.4		47.6	47.6				32.4	
Actuated g/C Ratio			0.36		0.53	0.53				0.36	
v/c Ratio			0.81		0.75	0.55				0.38	
Control Delay			31.7		10.7	8.6				18.7	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			31.7		10.7	8.6				18.7	
LOS			C		B	A				B	
Approach Delay	31.7				10.2					18.7	
Approach LOS	C				B					B	
Queue Length 50th (ft)			226		216	80				70	
Queue Length 95th (ft)			292		271	145				m96	

Lanes, Volumes, Timings

121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

04/07/2017

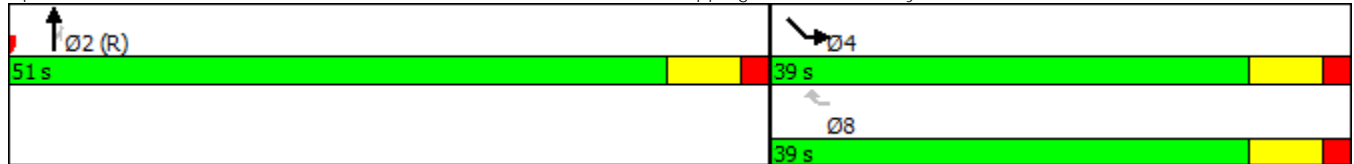


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	409				72			598		51	
Turn Bay Length (ft)											
Base Capacity (vph)			1363		1837	821				1271	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.77		0.75	0.55				0.36	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	55
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.81
Intersection Signal Delay:	18.1
Intersection LOS:	B
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over



Lanes, Volumes, Timings

122: SB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙↘					↑↑↑
Traffic Volume (vph)	543	0	0	0	0	2354
Future Volume (vph)	543	0	0	0	0	2354
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	2	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3367	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3367	0	0	0	0	4988
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		55			55
Link Distance (ft)	118		659			370
Travel Time (s)	3.2		8.2			4.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	2%	2%	4%
Adj. Flow (vph)	603	0	0	0	0	2616
Shared Lane Traffic (%)						
Lane Group Flow (vph)	603	0	0	0	0	2616
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Detector Phase	8					6
Switch Phase						
Minimum Initial (s)	14.0					14.0
Minimum Split (s)	21.0					21.0
Total Split (s)	28.0					62.0
Total Split (%)	31.1%					68.9%
Maximum Green (s)	21.0					55.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Lost Time Adjust (s)	-2.0					-2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	21.6					58.4
Actuated g/C Ratio	0.24					0.65
v/c Ratio	0.75					0.81
Control Delay	33.3					13.8
Queue Delay	0.0					0.0
Total Delay	33.3					13.8
LOS	C					B
Approach Delay	33.3					13.8

Lanes, Volumes, Timings

122: SB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	C			B		
Queue Length 50th (ft)	141			357		
Queue Length 95th (ft)	211			451		
Internal Link Dist (ft)	38		579		290	
Turn Bay Length (ft)						
Base Capacity (vph)	860			3236		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.70			0.81		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	36 (40%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.81
Intersection Signal Delay:	17.5
Intersection LOS:	B
Intersection Capacity Utilization	98.5%
ICU Level of Service	F
Analysis Period (min)	15

Splits and Phases: 122: SB US 321 & Walmart Shopping Center Driveway U-Turn



Lanes, Volumes, Timings
 123: SB US 321 & US 321 A Left-Over & US 321 A

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	184	0	0	0	0	2353	135	22	0
Future Volume (vph)	0	0	184	0	0	0	0	2353	135	22	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	350		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3471	1553	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3471	1553	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35			55			55		25		
Link Distance (ft)	456			1044			127		156		
Travel Time (s)	8.9			12.9			1.6		4.3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	204	0	0	0	0	2614	150	24	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	204	0	0	0	0	2614	150	24	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			14.0					76.0	76.0	14.0	
Total Split (%)			15.6%					84.4%	84.4%	15.6%	
Maximum Green (s)			7.0					69.0	69.0	7.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			9.0					71.0	71.0	9.0	
Actuated g/C Ratio			0.10					0.79	0.79	0.10	
v/c Ratio			0.74					0.95	0.12	0.14	
Control Delay			56.7					12.1	1.4	34.2	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			56.7					12.1	1.4	34.2	
LOS			E					B	A	C	
Approach Delay	56.7							11.5		34.2	

Lanes, Volumes, Timings
 123: SB US 321 & US 321 A Left-Over & US 321 A

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Approach LOS	E						B			C	
Queue Length 50th (ft)			65				154		10	11	
Queue Length 95th (ft)			#122				#926		m10	m20	
Internal Link Dist (ft)	376				964				47	76	
Turn Bay Length (ft)			350								
Base Capacity (vph)			276				2738		1225	173	
Starvation Cap Reductn			0				0		0	0	
Spillback Cap Reductn			0				0		0	0	
Storage Cap Reductn			0				0		0	0	
Reduced v/c Ratio			0.74				0.95		0.12	0.14	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32 (36%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 14.8 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 123: SB US 321 & US 321 A Left-Over & US 321 A



Lanes, Volumes, Timings
 124: NB US 321 & US 321 A U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	147	0	0	1519	0	0
Future Volume (vph)	147	0	0	1519	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	85			1243	972	
Travel Time (s)	2.3			15.4	12.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	163	0	0	1688	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	163	0	0	1688	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	56.8%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis

124: NB US 321 & US 321 A U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	147	0	0	1519	0	0
Future Volume (Veh/h)	147	0	0	1519	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	163	0	0	1688	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	844	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	844	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	45	100	100			
cM capacity (veh/h)	298	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2			
Volume Total	163	844	844			
Volume Left	163	0	0			
Volume Right	0	0	0			
cSH	298	1700	1700			
Volume to Capacity	0.55	0.50	0.50			
Queue Length 95th (ft)	77	0	0			
Control Delay (s)	30.7	0.0	0.0			
Lane LOS	D					
Approach Delay (s)	30.7	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utilization		56.8%		ICU Level of Service		B
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 2.7

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑		
Traffic Vol, veh/h	147	0	0	1519	0	0
Future Vol, veh/h	147	0	0	1519	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	163	0	0	1688	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	844	-	-	0
Stage 1	0	-	-	-
Stage 2	844	-	-	-
Critical Hdwy	6.88	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	5.88	-	-	-
Follow-up Hdwy	3.54	-	-	-
Pot Cap-1 Maneuver	298	0	0	-
Stage 1	-	0	0	-
Stage 2	377	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	298	-	-	-
Mov Cap-2 Maneuver	298	-	-	-
Stage 1	-	-	-	-
Stage 2	377	-	-	-

Approach

	EB	NB
HCM Control Delay, s	30.8	0
HCM LOS	D	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	298
HCM Lane V/C Ratio	-	0.548
HCM Control Delay (s)	-	30.8
HCM Lane LOS	-	D
HCM 95th %tile Q(veh)	-	3.1

Lanes, Volumes, Timings
 125: NB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↕↕	↕↕
Traffic Volume (vph)	0	0	0	0	543	1645
Future Volume (vph)	0	0	0	0	543	1645
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.97	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	3367	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	3367	3471
Link Speed (mph)	55		55			55
Link Distance (ft)	118		483			678
Travel Time (s)	1.5		6.0			8.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	603	1828
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	603	1828
Sign Control	Stop		Stop			Free

Intersection Summary

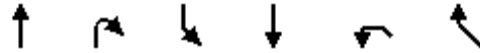
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	98.5%
Analysis Period (min)	15
	ICU Level of Service F

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

126: SB US 321 & Walmart Shopping Center Driveway Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↔↔	↑↑↑		
Traffic Volume (vph)	0	0	409	2488	0	0
Future Volume (vph)	0	0	409	2488	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	500		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3367	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3367	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	127			659	131	
Travel Time (s)	1.6			8.2	1.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	454	2764	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	454	2764	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	69.3%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 127: SB US 321 & US 321 A U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	0	0	0	147	2390
Future Volume (vph)	0	0	0	0	147	2390
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	55		30			55
Link Distance (ft)	85		1220			1044
Travel Time (s)	1.1		27.7			12.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	163	2656
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	163	2656
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	66.8%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 128: NB US 321 & US 321 A Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	22	1644	0	0	0	0
Future Volume (vph)	22	1644	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	500			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		972	152		156	
Travel Time (s)		12.0	1.9		1.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	24	1827	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	24	1827	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	91.9%
Analysis Period (min)	15
	ICU Level of Service F

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	3094	0	0	0	2029
Future Volume (vph)	0	3094	0	0	0	2029
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.76	1.00	1.00	1.00	0.91
Frt		0.850				
Flt Protected						
Satd. Flow (prot)	0	3541	0	0	0	4988
Flt Permitted						
Satd. Flow (perm)	0	3541	0	0	0	4988
Link Speed (mph)	45		45			45
Link Distance (ft)	960		796			1065
Travel Time (s)	14.5		12.1			16.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	4%	4%
Adj. Flow (vph)	0	3438	0	0	0	2254
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3438	0	0	0	2254
Sign Control	Free		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.5%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 997: SB US 321 & NB US 321/US 321

04/07/2017



Lane Group	NBL	NBR	SET	SER	NWL	NWT
Lane Configurations				↑↑↑↑		↑↑↑↑
Traffic Volume (vph)	0	0	0	3094	0	2029
Future Volume (vph)	0	0	0	3094	0	2029
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.76	1.00	0.91
Fr _t				0.850		
Flt Protected						
Satd. Flow (prot)	0	0	0	3541	0	4988
Flt Permitted						
Satd. Flow (perm)	0	0	0	3541	0	4988
Link Speed (mph)	45		45			45
Link Distance (ft)	641		1065			717
Travel Time (s)	9.7		16.1			10.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	3438	0	2254
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3438	0	2254
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.5%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Run Number	1	10	2	3	4	5	6
Start Time	6:50	6:50	6:50	6:50	6:50	6:50	6:50
End Time	8:00	8:00	8:00	8:00	8:00	8:00	8:00
Total Time (min)	70	70	70	70	70	70	70
Time Recorded (min)	60	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1	1
Vehs Entered	7751	7868	7621	7724	7862	7719	7690
Vehs Exited	7633	7722	7496	7572	7738	7528	7548
Starting Vehs	701	682	682	677	685	658	656
Ending Vehs	819	828	807	829	809	849	798
Denied Entry Before	0	0	2	0	2	0	0
Denied Entry After	5	4	1	0	0	0	0
Travel Distance (mi)	23481	23624	23078	23531	23603	23248	22867
Travel Time (hr)	812.8	838.4	805.6	814.7	827.2	834.2	799.8
Total Delay (hr)	289.4	311.7	290.6	290.0	299.9	314.4	288.6
Total Stops	16624	17009	16127	16089	16923	17261	16123
Fuel Used (gal)	847.3	856.2	834.8	845.3	854.4	840.6	826.5

Summary of All Intervals

Run Number	7	8	9	Avg
Start Time	6:50	6:50	6:50	6:50
End Time	8:00	8:00	8:00	8:00
Total Time (min)	70	70	70	70
Time Recorded (min)	60	60	60	60
# of Intervals	2	2	2	2
# of Recorded Intervals	1	1	1	1
Vehs Entered	7732	7706	7795	7743
Vehs Exited	7539	7479	7627	7590
Starting Vehs	668	680	669	669
Ending Vehs	861	907	837	828
Denied Entry Before	0	0	0	0
Denied Entry After	2	21	3	3
Travel Distance (mi)	23280	23109	23317	23314
Travel Time (hr)	840.5	825.1	797.1	819.5
Total Delay (hr)	321.2	308.8	276.3	299.1
Total Stops	17419	17363	15733	16664
Fuel Used (gal)	850.5	837.2	836.6	843.0

Interval #0 Information Seeding

Start Time	6:50
End Time	7:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	

Run Number	1	10	2	3	4	5	6
Vehs Entered	7751	7868	7621	7724	7862	7719	7690
Vehs Exited	7633	7722	7496	7572	7738	7528	7548
Starting Vehs	701	682	682	677	685	658	656
Ending Vehs	819	828	807	829	809	849	798
Denied Entry Before	0	0	2	0	2	0	0
Denied Entry After	5	4	1	0	0	0	0
Travel Distance (mi)	23481	23624	23078	23531	23603	23248	22867
Travel Time (hr)	812.8	838.4	805.6	814.7	827.2	834.2	799.8
Total Delay (hr)	289.4	311.7	290.6	290.0	299.9	314.4	288.6
Total Stops	16624	17009	16127	16089	16923	17261	16123
Fuel Used (gal)	847.3	856.2	834.8	845.3	854.4	840.6	826.5

Interval #1 Information Recording

Start Time	7:00
End Time	8:00
Total Time (min)	60
Volumes adjusted by Growth Factors.	

Run Number	7	8	9	Avg
Vehs Entered	7732	7706	7795	7743
Vehs Exited	7539	7479	7627	7590
Starting Vehs	668	680	669	669
Ending Vehs	861	907	837	828
Denied Entry Before	0	0	0	0
Denied Entry After	2	21	3	3
Travel Distance (mi)	23280	23109	23317	23314
Travel Time (hr)	840.5	825.1	797.1	819.5
Total Delay (hr)	321.2	308.8	276.3	299.1
Total Stops	17419	17363	15733	16664
Fuel Used (gal)	850.5	837.2	836.6	843.0

Total Network Performance By Run

Run Number	1	10	2	3	4	5	6
Denied Delay (hr)	1.8	3.7	1.7	2.2	3.3	1.7	1.7
Denied Del/Veh (s)	0.8	1.7	0.8	1.0	1.5	0.8	0.8
Total Delay (hr)	287.6	307.9	288.9	287.9	296.7	312.8	286.9
Total Del/Veh (s)	122.5	129.7	125.2	123.4	125.0	134.4	123.8
Stop Delay (hr)	122.8	136.9	124.2	126.2	128.9	142.8	129.0
Stop Del/Veh (s)	52.3	57.7	53.8	54.1	54.3	61.4	55.7
Total Stops	16624	17009	16127	16089	16923	17261	16123
Stop/Veh	1.97	1.99	1.94	1.92	1.98	2.06	1.93
Travel Dist (mi)	23481.3	23623.6	23077.9	23531.1	23602.8	23248.5	22867.5
Travel Time (hr)	812.8	838.4	805.6	814.7	827.2	834.2	799.8
Avg Speed (mph)	29	28	29	29	29	28	29
Vehicles Entered	7751	7868	7621	7724	7862	7719	7690
Vehicles Exited	7633	7722	7496	7572	7738	7528	7548
Hourly Exit Rate	7633	7722	7496	7572	7738	7528	7548
Input Volume	162207	162207	162207	162207	162207	162207	162207
% of Volume	5	5	5	5	5	5	5
Denied Entry Before	0	0	2	0	2	0	0
Denied Entry After	5	4	1	0	0	0	0

Total Network Performance By Run

Run Number	7	8	9	Avg
Denied Delay (hr)	1.9	3.6	2.0	2.4
Denied Del/Veh (s)	0.9	1.7	0.9	1.1
Total Delay (hr)	319.3	305.3	274.2	296.8
Total Del/Veh (s)	136.8	131.0	116.6	126.9
Stop Delay (hr)	157.4	137.7	115.7	132.2
Stop Del/Veh (s)	67.5	59.1	49.2	56.5
Total Stops	17419	17363	15733	16664
Stop/Veh	2.07	2.07	1.86	1.98
Travel Dist (mi)	23280.0	23109.0	23316.5	23313.8
Travel Time (hr)	840.5	825.1	797.1	819.5
Avg Speed (mph)	28	28	29	29
Vehicles Entered	7732	7706	7795	7743
Vehicles Exited	7539	7479	7627	7590
Hourly Exit Rate	7539	7479	7627	7590
Input Volume	162207	162207	162207	162207
% of Volume	5	5	5	5
Denied Entry Before	0	0	0	0
Denied Entry After	2	21	3	3

Intersection: 11: NB US 321 & 13th Street SW

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 15: NB US 321 & SB US 321

Movement

NB NB

Directions Served	T	T
Maximum Queue (ft)	14	15
Average Queue (ft)	1	1
95th Queue (ft)	12	14
Link Distance (ft)	1064	1064
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 21: SB US 321 & Main Avenue Drive NW

Movement

EB SB SB

Directions Served	R	T	T
Maximum Queue (ft)	130	3	3
Average Queue (ft)	51	0	0
95th Queue (ft)	109	3	3
Link Distance (ft)	813	187	187
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	202	149	150	147	96	143
Average Queue (ft)	113	40	63	81	32	58
95th Queue (ft)	184	114	121	134	74	118
Link Distance (ft)	208	208	79	79	79	89
Upstream Blk Time (%)	0		2	5	1	5
Queuing Penalty (veh)	0		14	30	5	5
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 32: SB US 321 & 2nd Avenue NW U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	260	293	134
Average Queue (ft)	105	121	99
95th Queue (ft)	208	225	140
Link Distance (ft)	652	652	42
Upstream Blk Time (%)			56
Queuing Penalty (veh)			96
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	232	188	151	154	117	233
Average Queue (ft)	146	91	97	119	47	153
95th Queue (ft)	212	177	160	163	100	235
Link Distance (ft)	549	549	74	74	74	146
Upstream Blk Time (%)			12	19	3	15
Queuing Penalty (veh)			86	138	25	37
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 34: NB US 321 & 2nd Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	307	341	131
Average Queue (ft)	166	181	104
95th Queue (ft)	273	294	132
Link Distance (ft)	981	981	38
Upstream Blk Time (%)			52
Queuing Penalty (veh)			127
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 35: NB US 321 & 2nd Avenue NW U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	159
Average Queue (ft)	39
95th Queue (ft)	115
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	400
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 36: SB US 321 & 2nd Avenue NW Left-Over

Movement	SB	SB	SB	SB
Directions Served	L	T	T	T
Maximum Queue (ft)	23	173	190	12
Average Queue (ft)	1	25	43	1
95th Queue (ft)	11	106	131	13
Link Distance (ft)		790	790	790
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	300			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 37: SB US 321 & 2nd Avenue NW U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	183
Average Queue (ft)	58
95th Queue (ft)	142
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 38: NB US 321 & 2nd Avenue NW Left -Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	130	41	46
Average Queue (ft)	15	1	2
95th Queue (ft)	78	19	23
Link Distance (ft)		687	687
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	550		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

Movement	EB	SB	SB	SB	NW
Directions Served	>	T	T	R	L
Maximum Queue (ft)	208	335	352	102	117
Average Queue (ft)	101	145	146	27	85
95th Queue (ft)	182	270	276	72	121
Link Distance (ft)	347	1100	1100	1100	37
Upstream Blk Time (%)					45
Queuing Penalty (veh)					74
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 44: NB US 321 & 7th Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	165	176	105
Average Queue (ft)	44	53	49
95th Queue (ft)	115	130	94
Link Distance (ft)	655	655	37
Upstream Blk Time (%)			39
Queuing Penalty (veh)			25
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 47: SB US 321 & 7th Avenue NW U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	26
Average Queue (ft)	1
95th Queue (ft)	16
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 48: NB US 321 & 7th Avenue NW Left-Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	146	58	34
Average Queue (ft)	36	2	1
95th Queue (ft)	109	46	33
Link Distance (ft)		778	778
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)	0	0	
Queuing Penalty (veh)	4	0	

Intersection: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

Movement	WB	WB	WB	NB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	T	R	L	L
Maximum Queue (ft)	351	293	169	136	140	142	133	197	210
Average Queue (ft)	265	175	45	83	89	94	55	131	151
95th Queue (ft)	365	276	126	140	141	147	127	200	214
Link Distance (ft)	335	335	335	34	34	34	34	88	88
Upstream Blk Time (%)	2	0		38	41	40	9	19	26
Queuing Penalty (veh)	0	0		153	165	160	38	95	125
Storage Bay Dist (ft)									
Storage Blk Time (%)									
Queuing Penalty (veh)									

Intersection: 62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

Movement	EB	WB	WB	SB	SB	SB
Directions Served	R	L	L	T	T	TR
Maximum Queue (ft)	8	133	138	492	373	295
Average Queue (ft)	0	104	108	265	193	165
95th Queue (ft)	8	138	142	426	310	256
Link Distance (ft)	202	36	36	845	845	845
Upstream Blk Time (%)		70	69			
Queuing Penalty (veh)		124	123			
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

Movement	EB	EB	SB	SB	SB	NW	NW
Directions Served	>	>	T	T	TR	<	L
Maximum Queue (ft)	191	159	178	177	177	118	109
Average Queue (ft)	98	44	125	145	119	50	43
95th Queue (ft)	168	127	182	189	178	97	87
Link Distance (ft)	834	834	49	49	49	58	58
Upstream Blk Time (%)			23	30	26	20	13
Queuing Penalty (veh)			188	241	209	13	8
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 64: NB US 321 & 9th Avenue NW & Clement Boulevard

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	167	107	249	221	244	167
Average Queue (ft)	80	20	128	114	135	123
95th Queue (ft)	141	67	208	190	217	174
Link Distance (ft)	252		1052	1052		79
Upstream Blk Time (%)						37
Queuing Penalty (veh)						76
Storage Bay Dist (ft)		250			375	
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 65: NB US 321 & Clement Boulevard U-Turn

Movement	NW	NW
Directions Served	L	L
Maximum Queue (ft)	102	106
Average Queue (ft)	24	22
95th Queue (ft)	73	77
Link Distance (ft)		
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	300	300
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 66: SB US 321 & Clement Boulevard Left-Over

Movement	SB	SB	SB	SB	SB
Directions Served	L	L	T	T	T
Maximum Queue (ft)	46	63	176	257	228
Average Queue (ft)	3	7	16	34	14
95th Queue (ft)	20	34	103	153	127
Link Distance (ft)			809	809	809
Upstream Blk Time (%)			0		0
Queuing Penalty (veh)			0		0
Storage Bay Dist (ft)	400	400			
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 67: SB US 321 & Clement Boulevard

Movement	SB	SB
Directions Served	L	T
Maximum Queue (ft)	86	64
Average Queue (ft)	13	2
95th Queue (ft)	52	63
Link Distance (ft)		748
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 68: NB US 321 & Clement Boulevard Left-Over

Movement	NB	NB	NB	NB	NB
Directions Served	L	T	T	T	T
Maximum Queue (ft)	4	112	126	129	72
Average Queue (ft)	0	13	13	18	4
95th Queue (ft)	4	62	66	75	33
Link Distance (ft)		767	767	767	
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	400				400
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	L
Maximum Queue (ft)	93	29	154	172	180	161
Average Queue (ft)	37	4	73	79	97	72
95th Queue (ft)	76	20	131	145	171	130
Link Distance (ft)	410	410	100	100	100	133
Upstream Blk Time (%)			2	2	5	1
Queuing Penalty (veh)			13	15	34	1
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 82: SB US 321 & 15th Avenue NW U-Turn

Movement	WB
Directions Served	L
Maximum Queue (ft)	67
Average Queue (ft)	18
95th Queue (ft)	53
Link Distance (ft)	31
Upstream Blk Time (%)	20
Queuing Penalty (veh)	4
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

Movement	EB	SB	SB	NW
Directions Served	>	T	TR	L
Maximum Queue (ft)	106	6	4	140
Average Queue (ft)	37	0	0	62
95th Queue (ft)	109	5	4	154
Link Distance (ft)	502	158	158	114
Upstream Blk Time (%)				21
Queuing Penalty (veh)				4
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 84: NB US 321 & 15th Avenue NW U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	41
Average Queue (ft)	11
95th Queue (ft)	34
Link Distance (ft)	-1
Upstream Blk Time (%)	3
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 85: NB US 321 & 15th Avenue NW U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	10
Average Queue (ft)	0
95th Queue (ft)	8
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	200
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 86: SB US 321 & 15th Avenue NW Left-Over

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 87: SB US 321 & 15th Avenue NW U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 88: NB US 321 & 15th Avenue NW Left-Over

Movement	NW	NW	NW	NW
Directions Served	L	T	T	T
Maximum Queue (ft)	16	6	25	43
Average Queue (ft)	6	0	1	3
95th Queue (ft)	44	5	14	24
Link Distance (ft)		664	664	664
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	200			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 91: US 321 & Grace Chapel Road

Movement	WB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	R	T	T	T	T	T	T
Maximum Queue (ft)	211	246	58	29	227	245	189	140	150	159
Average Queue (ft)	118	163	21	3	109	131	98	109	126	131
95th Queue (ft)	203	228	50	18	192	211	166	159	154	149
Link Distance (ft)		910	910		580	580	580	73	73	73
Upstream Blk Time (%)								11	18	22
Queuing Penalty (veh)								98	160	203
Storage Bay Dist (ft)	500			500						
Storage Blk Time (%)										
Queuing Penalty (veh)										

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	WB
Directions Served	L
Maximum Queue (ft)	21
Average Queue (ft)	3
95th Queue (ft)	14
Link Distance (ft)	13
Upstream Blk Time (%)	3
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	47
Average Queue (ft)	16
95th Queue (ft)	43
Link Distance (ft)	82
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement	NB
Directions Served	T
Maximum Queue (ft)	2
Average Queue (ft)	0
95th Queue (ft)	2
Link Distance (ft)	314
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 96: SB US 321/US 321 & NB US 321

Movement	SB	SB	SB
Directions Served	T	T	T
Maximum Queue (ft)	119	286	175
Average Queue (ft)	4	13	6
95th Queue (ft)	81	155	102
Link Distance (ft)	580	580	580
Upstream Blk Time (%)	0	0	0
Queuing Penalty (veh)	0	0	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 97: US 321 & NB US 321

Movement	SE	SE	SE
Directions Served	R	R	R
Maximum Queue (ft)	95	148	125
Average Queue (ft)	22	43	55
95th Queue (ft)	71	106	107
Link Distance (ft)	375	375	375
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	186	145	175	185	60	46
Average Queue (ft)	71	37	70	91	10	6
95th Queue (ft)	152	102	161	183	39	27
Link Distance (ft)	621	621	95	95	95	30
Upstream Blk Time (%)			7	9	0	6
Queuing Penalty (veh)			36	52	0	0
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 102: SB US 321 & Alex Lee Blvd U-Turn

Movement	SB	NW
Directions Served	T	L
Maximum Queue (ft)	6	168
Average Queue (ft)	0	128
95th Queue (ft)	5	185
Link Distance (ft)	1163	47
Upstream Blk Time (%)		83
Queuing Penalty (veh)		80
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

Movement	EB	SB	NW
Directions Served	>	R	L
Maximum Queue (ft)	22	2	96
Average Queue (ft)	2	0	35
95th Queue (ft)	13	2	111
Link Distance (ft)	164	84	102
Upstream Blk Time (%)			10
Queuing Penalty (veh)			1
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 104: NB US 321 & Alex Lee Blvd U-Turn

Movement	EB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	26	52	52	52
Average Queue (ft)	4	9	10	4
95th Queue (ft)	18	100	105	70
Link Distance (ft)	4	2164	2164	2164
Upstream Blk Time (%)	2			
Queuing Penalty (veh)	0			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 105: NB US 321 & Alex Lee Blvd U-Turn

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	324	382	329
Average Queue (ft)	160	95	77
95th Queue (ft)	371	423	395
Link Distance (ft)		571	571
Upstream Blk Time (%)		2	1
Queuing Penalty (veh)		16	12
Storage Bay Dist (ft)	300		
Storage Blk Time (%)	17	0	
Queuing Penalty (veh)	131	0	

Intersection: 106: SB US 321 & Alex Lee Blvd Left-Over

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 107: SB US 321 & Alex Lee Blvd U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 108: NB US 321 & Alex Lee Blvd Left-Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	30	105	130
Average Queue (ft)	3	29	31
95th Queue (ft)	44	225	229
Link Distance (ft)		590	590
Upstream Blk Time (%)		1	1
Queuing Penalty (veh)		4	5
Storage Bay Dist (ft)	250		
Storage Blk Time (%)		3	
Queuing Penalty (veh)		0	

Intersection: 121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

Movement	WB	WB	WB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	R	L	L
Maximum Queue (ft)	283	334	216	161	180	173	113	127
Average Queue (ft)	159	216	125	104	153	128	63	74
95th Queue (ft)	262	307	211	166	176	181	110	120
Link Distance (ft)	428	428	428	66	66	66	25	25
Upstream Blk Time (%)				20	41	29	39	50
Queuing Penalty (veh)				107	227	160	81	102
Storage Bay Dist (ft)								
Storage Blk Time (%)								
Queuing Penalty (veh)								

Intersection: 122: SB US 321 & Walmart Shopping Center Driveway U-Turn

Movement	WB	WB	SB	SB	SB	B993	B993
Directions Served	L	L	T	T	T	T	T
Maximum Queue (ft)	123	159	298	354	368	40	42
Average Queue (ft)	66	132	208	248	254	2	3
95th Queue (ft)	117	147	323	333	351	26	29
Link Distance (ft)	30	30		321	321	410	410
Upstream Blk Time (%)	49	75		1	2		
Queuing Penalty (veh)	133	204		11	17		
Storage Bay Dist (ft)			250				
Storage Blk Time (%)			2	7			
Queuing Penalty (veh)			17	51			

Intersection: 123: SB US 321 & US 321 A Left-Over & US 321 A

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	233	189	164	158	80	62
Average Queue (ft)	136	67	117	124	29	16
95th Queue (ft)	215	185	169	165	66	47
Link Distance (ft)	394		32	32	32	77
Upstream Blk Time (%)			19	19	8	0
Queuing Penalty (veh)			155	160	70	0
Storage Bay Dist (ft)		350				
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 124: NB US 321 & US 321 A U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	116
Average Queue (ft)	77
95th Queue (ft)	115
Link Distance (ft)	30
Upstream Blk Time (%)	74
Queuing Penalty (veh)	108
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 125: NB US 321 & Walmart Shopping Center Driveway U-Turn

Movement	NW	NW	NW
Directions Served	L	L	T
Maximum Queue (ft)	443	452	35
Average Queue (ft)	145	240	1
95th Queue (ft)	394	449	34
Link Distance (ft)		601	601
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		1	
Storage Bay Dist (ft)	500		
Storage Blk Time (%)	0	1	
Queuing Penalty (veh)	1	3	

Intersection: 126: SB US 321 & Walmart Shopping Center Driveway Left-Over

Movement	SB	SB	SB	SB	SB
Directions Served	L	L	T	T	T
Maximum Queue (ft)	14	36	346	352	60
Average Queue (ft)	1	2	54	69	2
95th Queue (ft)	12	17	208	206	42
Link Distance (ft)		591	591	591	
Upstream Blk Time (%)			0	0	
Queuing Penalty (veh)			0	0	
Storage Bay Dist (ft)	500				250
Storage Blk Time (%)				0	
Queuing Penalty (veh)				3	

Intersection: 127: SB US 321 & US 321 A U-Turn

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	191	190	167
Average Queue (ft)	55	18	14
95th Queue (ft)	178	190	178
Link Distance (ft)		984	984
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	0
Storage Bay Dist (ft)	250		
Storage Blk Time (%)	3	0	
Queuing Penalty (veh)	38	0	

Intersection: 128: NB US 321 & US 321 A Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	55	472	492	183
Average Queue (ft)	4	124	206	24
95th Queue (ft)	77	432	487	113
Link Distance (ft)		917	917	917
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	500			
Storage Blk Time (%)		2		
Queuing Penalty (veh)		0		

Intersection: 996: US 321/NB US 321 & SB US 321

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 997: SB US 321 & NB US 321/US 321

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 4901

Lanes, Volumes, Timings
 11: NB US 321 & 13th Street SW

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗			↗		↕	↗		↕	↗
Traffic Volume (vph)	0	0	127	0	0	194	0	1918	137	0	1520	243
Future Volume (vph)	0	0	127	0	0	194	0	1918	137	0	1520	243
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		200	0		200
Storage Lanes	0		1	0		1	0		1	0		1
Taper Length (ft)	100			100			50			50		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.865			0.865			0.850			0.850
Flt Protected												
Satd. Flow (prot)	0	0	1565	0	0	1565	0	3438	1538	0	3438	1538
Flt Permitted												
Satd. Flow (perm)	0	0	1565	0	0	1565	0	3438	1538	0	3438	1538
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		468			369			2914			1118	
Travel Time (s)		9.1			7.2			44.2			16.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	141	0	0	216	0	2131	152	0	1689	270
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	141	0	0	216	0	2131	152	0	1689	270
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	71.7%
Analysis Period (min)	15
	ICU Level of Service C

HCM Unsignalized Intersection Capacity Analysis

11: NB US 321 & 13th Street SW

04/07/2017



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗			↗		↕	↗		↕	↗
Traffic Volume (veh/h)	0	0	127	0	0	194	0	1918	137	0	1520	243
Future Volume (Veh/h)	0	0	127	0	0	194	0	1918	137	0	1520	243
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	141	0	0	216	0	2131	152	0	1689	270
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2754	3820	844	2976	3820	1066	1689			2131		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2754	3820	844	2976	3820	1066	1689			2131		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	100	53	100	100	0	100			100		
cM capacity (veh/h)	0	4	300	3	4	213	361			241		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	141	216	1066	1066	152	844	844	270				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	141	216	0	0	152	0	0	270				
cSH	300	213	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.47	1.01	0.63	0.63	0.09	0.50	0.50	0.16				
Queue Length 95th (ft)	59	229	0	0	0	0	0	0				
Control Delay (s)	27.2	112.4	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	D	F										
Approach Delay (s)	27.2	112.4	0.0			0.0						
Approach LOS	D	F										
Intersection Summary												
Average Delay			6.1									
Intersection Capacity Utilization			71.7%	ICU Level of Service	C							
Analysis Period (min)			15									

HCM 2010 TWSC
 11: NB US 321 & 13th Street SW

04/07/2017

Intersection

Int Delay, s/veh 0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↗			↗		↕	↗		↕	↗
Traffic Vol, veh/h	0	0	127	0	0	194	0	1918	137	0	1520	243
Future Vol, veh/h	0	0	127	0	0	194	0	1918	137	0	1520	243
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	Free	-	-	Free	-	-	Free	-	-	Free
Storage Length	-	-	0	-	-	0	-	-	200	-	-	200
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	0	0	141	0	0	216	0	2131	152	0	1689	270

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	-	-	-	-	-	-	-	0	-	-	-	0
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-	-	-	-	-	-	-
Pot Cap-1 Maneuver	0	0	0	0	0	0	0	-	0	0	-	0
Stage 1	0	0	0	0	0	0	0	-	0	0	-	0
Stage 2	0	0	0	0	0	0	0	-	0	0	-	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	0
HCM LOS	A	A		

Minor Lane/Major Mvmt	NBT	EBLn1	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	0	0	-
HCM Lane LOS	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	-

Lanes, Volumes, Timings
 15: NB US 321 & SB US 321

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑
Traffic Volume (vph)	0	2112	0	0	0	1763
Future Volume (vph)	0	2112	0	0	0	1763
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.88
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4940	0	0	0	2707
Flt Permitted						
Satd. Flow (perm)	0	4940	0	0	0	2707
Link Speed (mph)		45	45		45	
Link Distance (ft)		1118	1025		856	
Travel Time (s)		16.9	15.5		13.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	2347	0	0	0	1959
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2347	0	0	0	1959
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	65.0%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 21: SB US 321 & Main Avenue Drive NW

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	118	0	0	1650	23
Future Volume (vph)	0	118	0	0	1650	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	1.00
Frt		0.865				0.850
Flt Protected						
Satd. Flow (prot)	0	1596	0	0	3438	1538
Flt Permitted						
Satd. Flow (perm)	0	1596	0	0	3438	1538
Link Speed (mph)	25			45	45	
Link Distance (ft)	869			856	251	
Travel Time (s)	23.7			13.0	3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	3%	2%	2%	5%	5%
Adj. Flow (vph)	0	131	0	0	1833	26
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	131	0	0	1833	26
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	59.6%
ICU Level of Service	B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

21: SB US 321 & Main Avenue Drive NW

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗			↕↕	↗
Traffic Volume (veh/h)	0	118	0	0	1650	23
Future Volume (Veh/h)	0	118	0	0	1650	23
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	131	0	0	1833	26
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1833	916	1859			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1833	916	1859			
tC, single (s)	6.8	7.0	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	52	100			
cM capacity (veh/h)	69	273	321			
Direction, Lane #	EB 1	SB 1	SB 2	SB 3		
Volume Total	131	916	916	26		
Volume Left	0	0	0	0		
Volume Right	131	0	0	26		
cSH	273	1700	1700	1700		
Volume to Capacity	0.48	0.54	0.54	0.02		
Queue Length 95th (ft)	61	0	0	0		
Control Delay (s)	29.8	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	29.8	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			2.0			
Intersection Capacity Utilization			59.6%		ICU Level of Service	B
Analysis Period (min)			15			

HCM 2010 TWSC
 21: SB US 321 & Main Avenue Drive NW

04/07/2017

Intersection

Int Delay, s/veh 2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↑			↑↑	↑
Traffic Vol, veh/h	0	118	0	0	1650	23
Future Vol, veh/h	0	118	0	0	1650	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage, #	0	-	-	-	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	2	2	5	5
Mvmt Flow	0	131	0	0	1833	26

Major/Minor

	Minor2	Major2
Conflicting Flow All	-	917
Stage 1	-	-
Stage 2	-	-
Critical Hdwy	-	6.96
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	-
Follow-up Hdwy	-	3.33
Pot Cap-1 Maneuver	0	272
Stage 1	0	-
Stage 2	0	-
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	272
Mov Cap-2 Maneuver	-	-
Stage 1	-	-
Stage 2	-	-

Approach

	EB	SB
HCM Control Delay, s	30	0
HCM LOS	D	

Minor Lane/Major Mvmt

	EBLn1	SBT	SBR
Capacity (veh/h)	272	-	-
HCM Lane V/C Ratio	0.482	-	-
HCM Control Delay (s)	30	-	-
HCM Lane LOS	D	-	-
HCM 95th %tile Q(veh)	2.5	-	-

Lanes, Volumes, Timings

31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↑↑	↖				↘	
Traffic Volume (vph)	0	0	333	0	1983	173	0	0	0	59	0
Future Volume (vph)	0	0	333	0	1983	173	0	0	0	59	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0		0	400		0	0	0
Storage Lanes	0	2		0		1	0		0	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	3438	1538	0	0	0	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	3438	1538	0	0	0	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	275				152			917		184	
Travel Time (s)	5.4				2.3			13.9		5.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	370	0	2203	192	0	0	0	66	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	370	0	2203	192	0	0	0	66	0
Turn Type			Prot		NA	Perm				Prot	
Protected Phases			4		2					4	
Permitted Phases						2					
Detector Phase			4		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		12.0	12.0				7.0	
Minimum Split (s)			14.0		19.0	19.0				14.0	
Total Split (s)			20.0		70.0	70.0				20.0	
Total Split (%)			22.2%		77.8%	77.8%				22.2%	
Maximum Green (s)			13.0		63.0	63.0				13.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effect Green (s)			14.9		65.1	65.1				14.9	
Actuated g/C Ratio			0.17		0.72	0.72				0.17	
v/c Ratio			0.81		0.89	0.17				0.23	
Control Delay			51.3		5.2	1.8				30.2	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			51.3		5.2	1.8				30.2	
LOS			D		A	A				C	
Approach Delay	51.3				4.9					30.2	

Lanes, Volumes, Timings

31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Approach LOS	D				A				C			
Queue Length 50th (ft)			116		95	13					30	
Queue Length 95th (ft)			#192		m97	m14					m40	
Internal Link Dist (ft)	195				72			837			104	
Turn Bay Length (ft)												
Base Capacity (vph)			460		2485	1111					286	
Starvation Cap Reductn			0		0	0					0	
Spillback Cap Reductn			0		0	0					0	
Storage Cap Reductn			0		0	0					0	
Reduced v/c Ratio			0.80		0.89	0.17					0.23	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 24 (27%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.89
 Intersection Signal Delay: 11.6
 Intersection LOS: B
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

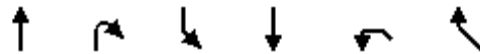
Splits and Phases: 31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over



Lanes, Volumes, Timings

32: SB US 321 & 2nd Avenue NW U-Turn

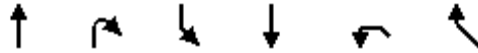
04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↘	
Traffic Volume (vph)	0	0	0	1634	229	0
Future Volume (vph)	0	0	0	1634	229	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3438	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3438	1719	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	45			45	25	
Link Distance (ft)	833			712	94	
Travel Time (s)	12.6			10.8	2.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	0	1816	254	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	1816	254	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				12.0	7.0	
Minimum Split (s)				19.0	14.0	
Total Split (s)				64.0	26.0	
Total Split (%)				71.1%	28.9%	
Maximum Green (s)				57.0	19.0	
Yellow Time (s)				5.0	5.0	
All-Red Time (s)				2.0	2.0	
Lost Time Adjust (s)				-2.0	-2.0	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				3.0	3.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				61.2	18.8	
Actuated g/C Ratio				0.68	0.21	
v/c Ratio				0.78	0.71	
Control Delay				5.2	42.3	
Queue Delay				0.0	0.0	
Total Delay				5.2	42.3	
LOS				A	D	
Approach Delay				5.2	42.3	
Approach LOS				A	D	
Queue Length 50th (ft)				109	138	
Queue Length 95th (ft)				136	m159	

Lanes, Volumes, Timings
 32: SB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Internal Link Dist (ft)	753			632	14	
Turn Bay Length (ft)						
Base Capacity (vph)				2339	401	
Starvation Cap Reductn				0	0	
Spillback Cap Reductn				0	0	
Storage Cap Reductn				0	0	
Reduced v/c Ratio				0.78	0.63	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	55 (61%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.78
Intersection Signal Delay:	9.7
Intersection LOS:	A
Intersection Capacity Utilization	110.4%
ICU Level of Service	H
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 32: SB US 321 & 2nd Avenue NW U-Turn



Lanes, Volumes, Timings

33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	448	0	0	0	0	1585	219	192	0
Future Volume (vph)	0	0	448	0	0	0	0	1585	219	192	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3438	1538	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3438	1538	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	616				840			154		188	
Travel Time (s)	12.0				12.7			2.3		5.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	0	498	0	0	0	0	1761	243	213	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	498	0	0	0	0	1761	243	213	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					12.0	12.0	7.0	
Minimum Split (s)			14.0					19.0	19.0	14.0	
Total Split (s)			28.0					62.0	62.0	28.0	
Total Split (%)			31.1%					68.9%	68.9%	31.1%	
Maximum Green (s)			21.0					55.0	55.0	21.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			21.5					58.5	58.5	21.5	
Actuated g/C Ratio			0.24					0.65	0.65	0.24	
v/c Ratio			0.76					0.79	0.24	0.52	
Control Delay			39.8					7.4	4.4	33.2	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			39.8					7.4	4.4	33.2	
LOS			D					A	A	C	
Approach Delay	39.8							7.1		33.2	
Approach LOS	D							A		C	
Queue Length 50th (ft)			146					103	26	93	
Queue Length 95th (ft)			207					159	m46	m94	

Lanes, Volumes, Timings

33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	536				760			74		108	
Turn Bay Length (ft)											
Base Capacity (vph)			705					2235	1000	439	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.71					0.79	0.24	0.49	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 63 (70%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 15.1 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW



Lanes, Volumes, Timings
 34: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017

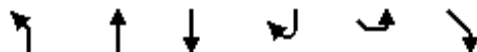


Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↘	
Traffic Volume (vph)	0	2114	0	0	308	0
Future Volume (vph)	0	2114	0	0	308	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		25	
Link Distance (ft)		1025	741		87	
Travel Time (s)		15.5	11.2		2.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	2%
Adj. Flow (vph)	0	2349	0	0	342	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2349	0	0	342	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		12.0			7.0	
Minimum Split (s)		19.0			14.0	
Total Split (s)		67.0			23.0	
Total Split (%)		74.4%			25.6%	
Maximum Green (s)		60.0			16.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		62.0			18.0	
Actuated g/C Ratio		0.69			0.20	
v/c Ratio		0.99			1.00	
Control Delay		31.6			74.9	
Queue Delay		0.0			0.0	
Total Delay		31.6			74.9	
LOS		C			E	
Approach Delay		31.6			74.9	
Approach LOS		C			E	
Queue Length 50th (ft)		597			186	
Queue Length 95th (ft)		#870			m#325	

Lanes, Volumes, Timings

34: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		945	661		7	
Turn Bay Length (ft)						
Base Capacity (vph)		2368			343	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.99			1.00	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	14 (16%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.00
Intersection Signal Delay:	37.1
Intersection LOS:	D
Intersection Capacity Utilization	83.8%
ICU Level of Service	E
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 34: NB US 321 & 2nd Avenue NW U-Turn



Lanes, Volumes, Timings
 35: NB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	229	2087	0	0	0	0
Future Volume (vph)	229	2087	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	400			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	3438	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		917	715		94	
Travel Time (s)		13.9	10.8		1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	254	2319	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	254	2319	0	0	0	0
Sign Control		Free	Stop		Stop	

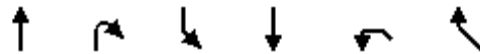
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	110.4%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 36: SB US 321 & 2nd Avenue NW Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	59	1804	0	0
Future Volume (vph)	0	0	59	1804	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	4940	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	4940	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	154			833	184	
Travel Time (s)	2.3			12.6	2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	66	2004	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	66	2004	0	0
Sign Control	Stop			Free	Stop	

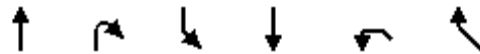
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	66.2%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 37: SB US 321 & 2nd Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	308	1725	0	0
Future Volume (vph)	0	0	308	1725	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	251			840	87	
Travel Time (s)	3.8			12.7	1.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	342	1917	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	342	1917	0	0
Sign Control	Stop			Free	Stop	

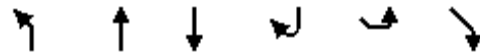
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	83.8%
Analysis Period (min)	15
	ICU Level of Service E

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 38: NB US 321 & 2nd Avenue NW Left -Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	192	2156	0	0	0	0
Future Volume (vph)	192	2156	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	550			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	4940	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	4940	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		741	152		188	
Travel Time (s)		11.2	2.3		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	2%	2%	2%	2%
Adj. Flow (vph)	213	2396	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	213	2396	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

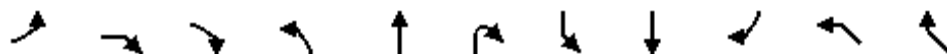
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	128.8%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

04/07/2017

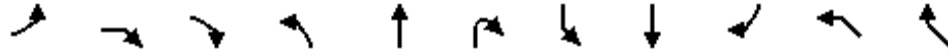


Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	321	0	0	0	0	1567	64	81	0
Future Volume (vph)	0	0	321	0	0	0	0	1567	64	81	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1596	0	0	0	0	3471	1553	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1596	0	0	0	0	3471	1553	1719	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	410				820			1170		97	
Travel Time (s)	8.0				12.4			17.7		2.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	4%	4%	4%	5%	5%
Adj. Flow (vph)	0	0	357	0	0	0	0	1741	71	90	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	357	0	0	0	0	1741	71	90	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					12.0	12.0	7.0	
Minimum Split (s)			14.0					19.0	19.0	14.0	
Total Split (s)			32.0					58.0	58.0	32.0	
Total Split (%)			35.6%					64.4%	64.4%	35.6%	
Maximum Green (s)			25.0					51.0	51.0	25.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			24.9					55.1	55.1	24.9	
Actuated g/C Ratio			0.28					0.61	0.61	0.28	
v/c Ratio			0.81					0.82	0.07	0.19	
Control Delay			45.7					11.0	5.6	18.1	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			45.7					11.0	5.6	18.1	
LOS			D					B	A	B	
Approach Delay	45.7							10.8		18.1	
Approach LOS	D							B		B	
Queue Length 50th (ft)			182					150	10	37	
Queue Length 95th (ft)			#310					190	m19	m39	

Lanes, Volumes, Timings

43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Internal Link Dist (ft)	330				740			1090		17	
Turn Bay Length (ft)											
Base Capacity (vph)			478					2126	951	515	
Starvation Cap Reductn			0					0	0	0	
Spillback Cap Reductn			0					0	0	0	
Storage Cap Reductn			0					0	0	0	
Reduced v/c Ratio			0.75					0.82	0.07	0.17	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	40 (44%), Referenced to phase 6:SBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.82
Intersection Signal Delay:	16.6
Intersection LOS:	B
Intersection Capacity Utilization	71.5%
ICU Level of Service	C
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

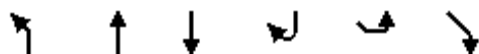
Splits and Phases: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW



Lanes, Volumes, Timings

44: NB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑			↵	
Traffic Volume (vph)	0	2015	0	0	158	0
Future Volume (vph)	0	2015	0	0	158	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	3438	0	0	1719	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	3438	0	0	1719	0
Right Turn on Red				No	No	No
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		25	
Link Distance (ft)		715	820		90	
Travel Time (s)		10.8	12.4		2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%
Adj. Flow (vph)	0	2239	0	0	176	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2239	0	0	176	0
Turn Type		NA			Prot	
Protected Phases		2			4	
Permitted Phases						
Detector Phase		2			4	
Switch Phase						
Minimum Initial (s)		12.0			7.0	
Minimum Split (s)		19.0			14.0	
Total Split (s)		72.0			18.0	
Total Split (%)		80.0%			20.0%	
Maximum Green (s)		65.0			11.0	
Yellow Time (s)		5.0			5.0	
All-Red Time (s)		2.0			2.0	
Lost Time Adjust (s)		-2.0			-2.0	
Total Lost Time (s)		5.0			5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)		3.0			3.0	
Recall Mode		C-Min			None	
Act Effct Green (s)		67.3			12.7	
Actuated g/C Ratio		0.75			0.14	
v/c Ratio		0.87			0.73	
Control Delay		7.0			50.3	
Queue Delay		0.0			0.0	
Total Delay		7.0			50.3	
LOS		A			D	
Approach Delay		7.0			50.3	
Approach LOS		A			D	
Queue Length 50th (ft)		135			100	
Queue Length 95th (ft)		207			m129	

Lanes, Volumes, Timings

44: NB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Internal Link Dist (ft)		635	740		10	
Turn Bay Length (ft)						
Base Capacity (vph)		2571			248	
Starvation Cap Reductn		0			0	
Spillback Cap Reductn		0			0	
Storage Cap Reductn		0			0	
Reduced v/c Ratio		0.87			0.71	

Intersection Summary

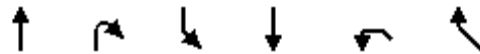
Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	43 (48%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.87
Intersection Signal Delay:	10.1
Intersection LOS:	B
Intersection Capacity Utilization	72.8%
ICU Level of Service	C
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 44: NB US 321 & 7th Avenue NW U-Turn



Lanes, Volumes, Timings
 47: SB US 321 & 7th Avenue NW U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	158	1730	0	0
Future Volume (vph)	0	0	158	1730	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	300		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	3438	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	3438	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	712			820	90	
Travel Time (s)	10.8			12.4	1.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	5%	5%	2%	2%
Adj. Flow (vph)	0	0	176	1922	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	176	1922	0	0
Sign Control	Stop			Free	Stop	

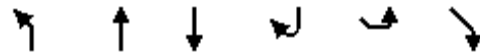
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	72.8%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 48: NB US 321 & 7th Avenue NW Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	81	2092	0	0	0	0
Future Volume (vph)	81	2092	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	3438	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	3438	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		820	1115		97	
Travel Time (s)		12.4	16.9		1.2	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	4%	4%	5%	5%
Adj. Flow (vph)	90	2324	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	2324	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	133.9%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	1274	0	2076	356	0	0	0	700	0
Future Volume (vph)	0	0	1274	0	2076	356	0	0	0	700	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.76	1.00	0.91	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	4940	1538	0	0	0	3367	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	4940	1538	0	0	0	3367	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	400				125			983		205	
Travel Time (s)	7.8				1.9			14.9		5.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	5%	5%	5%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	1416	0	2307	396	0	0	0	778	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	1416	0	2307	396	0	0	0	778	0
Turn Type			Prot		NA	Perm				Prot	
Protected Phases			8		2					8	
Permitted Phases			8			2				8	
Detector Phase			8		2	2				8	
Switch Phase											
Minimum Initial (s)			7.0		12.0	12.0				7.0	
Minimum Split (s)			14.0		19.0	19.0				14.0	
Total Split (s)			41.0		49.0	49.0				41.0	
Total Split (%)			45.6%		54.4%	54.4%				45.6%	
Maximum Green (s)			34.0		42.0	42.0				34.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	0.0				-2.0	
Total Lost Time (s)			5.0		5.0	7.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effct Green (s)			36.0		44.0	42.0				36.0	
Actuated g/C Ratio			0.40		0.49	0.47				0.40	
v/c Ratio			0.98		0.96	0.55				0.58	
Control Delay			47.3		17.8	9.4				22.2	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			47.3		17.8	9.4				22.2	
LOS			D		B	A				C	
Approach Delay	47.3				16.6					22.2	
Approach LOS	D				B					C	
Queue Length 50th (ft)			340		202	52				188	
Queue Length 95th (ft)			#473		#561	m68				244	

Lanes, Volumes, Timings

61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	320				45			903		125	
Turn Bay Length (ft)											
Base Capacity (vph)			1444		2415	717				1346	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.98		0.96	0.55				0.58	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	10 (11%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.98
Intersection Signal Delay:	26.3
Intersection LOS:	C
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over



Lanes, Volumes, Timings

62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	31	296	0	0	0	0	0	0	2012	46
Future Volume (vph)	0	0	31	296	0	0	0	0	0	0	2012	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91
Frt			0.865								0.997	
Flt Protected				0.950								
Satd. Flow (prot)	0	0	1596	3367	0	0	0	0	0	0	4925	0
Flt Permitted				0.950								
Satd. Flow (perm)	0	0	1596	3367	0	0	0	0	0	0	4925	0
Right Turn on Red			No	No		No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			25			45				45
Link Distance (ft)		250			97			866				889
Travel Time (s)		4.9			2.6			13.1				13.5
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	4%	2%	2%	4%	4%	4%	5%	5%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	100
Adj. Flow (vph)	0	0	34	329	0	0	0	0	0	0	2236	51
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	34	329	0	0	0	0	0	0	2287	0
Turn Type			Free	Prot								NA
Protected Phases				8								6
Permitted Phases			Free									
Detector Phase				8								6
Switch Phase												
Minimum Initial (s)				7.0								12.0
Minimum Split (s)				14.0								19.0
Total Split (s)				22.0								68.0
Total Split (%)				24.4%								75.6%
Maximum Green (s)				15.0								61.0
Yellow Time (s)				5.0								5.0
All-Red Time (s)				2.0								2.0
Lost Time Adjust (s)				-2.0								-2.0
Total Lost Time (s)				5.0								5.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)				3.0								3.0
Recall Mode				None								C-Min
Act Effct Green (s)			90.0	15.3								64.7
Actuated g/C Ratio			1.00	0.17								0.72
v/c Ratio			0.02	0.58								0.65
Control Delay			0.0	35.9								7.9
Queue Delay			0.0	0.0								0.0
Total Delay			0.0	35.9								7.9
LOS			A	D								A
Approach Delay					35.9							7.9
Approach LOS					D							A
Queue Length 50th (ft)			0	89								217

Lanes, Volumes, Timings

62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)			0	m92							272	
Internal Link Dist (ft)		170			17			786			809	
Turn Bay Length (ft)												
Base Capacity (vph)			1596	635							3539	
Starvation Cap Reductn			0	0							0	
Spillback Cap Reductn			0	0							0	
Storage Cap Reductn			0	0							0	
Reduced v/c Ratio			0.02	0.52							0.65	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 18 (20%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 45
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.65
 Intersection Signal Delay: 11.3 Intersection LOS: B
 Intersection Capacity Utilization 106.8% ICU Level of Service G
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn



Lanes, Volumes, Timings

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Lane Configurations			↑↑					↑↑↑		↑	↑	
Traffic Volume (vph)	0	0	279	0	0	0	0	1471	142	53	47	0
Future Volume (vph)	0	0	279	0	0	0	0	1471	142	53	47	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00	1.00
Fr't			0.850					0.987				
Flt Protected										0.950	0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	4923	0	1770	1770	0
Flt Permitted										0.950	0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	4923	0	1770	1770	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				45			45				25
Link Distance (ft)	910				822			181				143
Travel Time (s)	17.7				12.5			2.7				3.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	3%	5%	5%	5%	4%	4%	4%	2%	2%	2%
Adj. Flow (vph)	0	0	310	0	0	0	0	1634	158	59	52	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	310	0	0	0	0	1792	0	59	52	0
Turn Type			Prot					NA		Prot	Prot	
Protected Phases			7					6		8	8	
Permitted Phases												
Detector Phase			7					6		8	8	
Switch Phase												
Minimum Initial (s)			7.0					12.0		7.0	7.0	
Minimum Split (s)			14.0					19.0		14.0	14.0	
Total Split (s)			23.0					53.0		14.0	14.0	
Total Split (%)			25.6%					58.9%		15.6%	15.6%	
Maximum Green (s)			16.0					46.0		7.0	7.0	
Yellow Time (s)			5.0					5.0		5.0	5.0	
All-Red Time (s)			2.0					2.0		2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0		-2.0	-2.0	
Total Lost Time (s)			5.0					5.0		5.0	5.0	
Lead/Lag			Lead							Lag	Lag	
Lead-Lag Optimize?			Yes							Yes	Yes	
Vehicle Extension (s)			3.0					3.0		3.0	3.0	
Recall Mode			None					C-Min		None	None	
Act Effct Green (s)			16.3					52.2		9.3	9.3	
Actuated g/C Ratio			0.18					0.58		0.10	0.10	
v/c Ratio			0.62					0.63		0.32	0.28	
Control Delay			39.7					10.3		52.2	51.8	
Queue Delay			0.0					0.0		0.0	0.0	
Total Delay			39.7					10.3		52.2	51.8	
LOS			D					B		D	D	
Approach Delay	39.7							10.3			52.0	
Approach LOS	D							B			D	
Queue Length 50th (ft)			92					152		33	29	
Queue Length 95th (ft)			138					170		m41	m36	

Lanes, Volumes, Timings

63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

04/07/2017

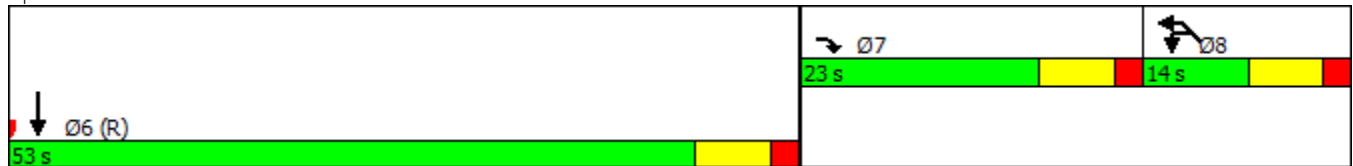


Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL2	NWL	NWR
Internal Link Dist (ft)	830				742			101			63	
Turn Bay Length (ft)												
Base Capacity (vph)			552					2864		183	183	
Starvation Cap Reductn			0					0		0	0	
Spillback Cap Reductn			0					0		0	0	
Storage Cap Reductn			0					0		0	0	
Reduced v/c Ratio			0.56					0.63		0.32	0.28	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 24 (27%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.63
 Intersection Signal Delay: 16.5 Intersection LOS: B
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard



Lanes, Volumes, Timings

64: NB US 321 & 9th Avenue NW & Clement Boulevard

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Lane Configurations			↑↑		↑↑↑						↓	
Traffic Volume (vph)	0	0	114	0	1978	71	0	0	0	222	67	0
Future Volume (vph)	0	0	114	0	1978	71	0	0	0	222	67	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	250		0		375	0		0		0	0
Storage Lanes	0	1		0		1	0		0		1	0
Taper Length (ft)	50			50			50				50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.995							
Flt Protected											0.950	
Satd. Flow (prot)	0	0	2733	0	4963	0	0	0	0	0	1736	0
Flt Permitted											0.950	
Satd. Flow (perm)	0	0	2733	0	4963	0	0	0	0	0	1736	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)	35				45			45			25	
Link Distance (ft)	319				1115			832			149	
Travel Time (s)	6.2				16.9			12.6			4.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	127	0	2198	79	0	0	0	247	74	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	127	0	2277	0	0	0	0	0	321	0
Turn Type			Prot		NA					Prot	Prot	
Protected Phases			8		2					4	4	
Permitted Phases												
Detector Phase			8		2					4	4	
Switch Phase												
Minimum Initial (s)			7.0		12.0					7.0	7.0	
Minimum Split (s)			14.0		19.0					14.0	14.0	
Total Split (s)			14.0		50.0					26.0	26.0	
Total Split (%)			15.6%		55.6%					28.9%	28.9%	
Maximum Green (s)			7.0		43.0					19.0	19.0	
Yellow Time (s)			5.0		5.0					5.0	5.0	
All-Red Time (s)			2.0		2.0					2.0	2.0	
Lost Time Adjust (s)			-2.0		-2.0						-2.0	
Total Lost Time (s)			5.0		5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)			3.0		3.0					3.0	3.0	
Recall Mode			None		C-Min					None	None	
Act Effct Green (s)			9.0		45.8						20.2	
Actuated g/C Ratio			0.10		0.51						0.22	
v/c Ratio			0.47		0.90						0.82	
Control Delay			44.3		18.4						43.0	
Queue Delay			0.0		0.0						0.0	
Total Delay			44.3		18.4						43.0	
LOS			D		B						D	
Approach Delay	44.3				18.4						43.0	

Lanes, Volumes, Timings

64: NB US 321 & 9th Avenue NW & Clement Boulevard

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL2	SEL	SER
Approach LOS	D				B				D			
Queue Length 50th (ft)			39	414						127		
Queue Length 95th (ft)			71	#460						#295		
Internal Link Dist (ft)	239			1035				752			69	
Turn Bay Length (ft)			250									
Base Capacity (vph)			273		2524						405	
Starvation Cap Reductn			0		0						0	
Spillback Cap Reductn			0		0						0	
Storage Cap Reductn			0		0						0	
Reduced v/c Ratio			0.47		0.90						0.79	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	86 (96%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.90
Intersection Signal Delay:	22.5
Intersection LOS:	C
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	

Splits and Phases: 64: NB US 321 & 9th Avenue NW & Clement Boulevard



Lanes, Volumes, Timings
 65: NB US 321 & Clement Boulevard U-Turn

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↗↗	↖↖↖
Traffic Volume (vph)	0	0	0	0	296	3075
Future Volume (vph)	0	0	0	0	296	3075
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	300	
Storage Lanes	0	0		0	2	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.97	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	3367	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	3367	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	97		939			983
Travel Time (s)	2.6		14.2			14.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	5%	4%	4%
Adj. Flow (vph)	0	0	0	0	329	3417
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	329	3417
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	106.8%
Analysis Period (min)	15
	ICU Level of Service G

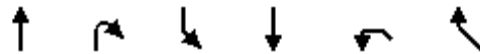
Intersection has too many lanes per leg.

HCM All-Way analysis is limited to two lanes per leg.

Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
 66: SB US 321 & Clement Boulevard Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↔↔	↑↑↑		
Traffic Volume (vph)	0	0	700	1613	0	0
Future Volume (vph)	0	0	700	1613	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	400		0	0
Storage Lanes		0	2		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3367	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3367	4988	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	181			866	205	
Travel Time (s)	2.7			13.1	2.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	778	1792	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	778	1792	0	0
Sign Control	Stop			Free	Stop	

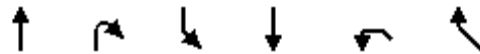
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	34.5%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
67: SB US 321 & Clement Boulevard

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	289	1665	0	0
Future Volume (vph)	0	0	289	1665	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	45			45	55	
Link Distance (ft)	1170			822	149	
Travel Time (s)	17.7			12.5	1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	321	1850	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	321	1850	0	0
Sign Control	Stop			Free	Stop	

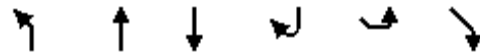
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	35.5%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 68: NB US 321 & Clement Boulevard Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	100	2432	0	0	0	0
Future Volume (vph)	100	2432	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	400			0	0	0
Storage Lanes	2			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	0.86	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3335	6225	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3335	6225	0	0	0	0
Link Speed (mph)		45	45		55	
Link Distance (ft)		832	125		143	
Travel Time (s)		12.6	1.9		1.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	4%	42%	5%	5%
Adj. Flow (vph)	111	2702	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	111	2702	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	38.6%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑		↑↑↑					↓	
Traffic Volume (vph)	0	0	144	0	2978	20	0	0	0	43	0
Future Volume (vph)	0	0	144	0	2978	20	0	0	0	43	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.88	1.00	0.91	0.91	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999						
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2787	0	4936	0	0	0	0	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2787	0	4936	0	0	0	0	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	478				179			784		210	
Travel Time (s)	9.3				2.7			11.9		5.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	5%	2%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	160	0	3309	22	0	0	0	48	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	160	0	3331	0	0	0	0	48	0
Turn Type			Perm		NA					Prot	
Protected Phases					2					4	
Permitted Phases			8								
Detector Phase			8		2					4	
Switch Phase											
Minimum Initial (s)			7.0		12.0					7.0	
Minimum Split (s)			14.0		19.0					14.0	
Total Split (s)			14.0		76.0					14.0	
Total Split (%)			15.6%		84.4%					15.6%	
Maximum Green (s)			7.0		69.0					7.0	
Yellow Time (s)			5.0		5.0					5.0	
All-Red Time (s)			2.0		2.0					2.0	
Lost Time Adjust (s)			-2.0		-2.0					-2.0	
Total Lost Time (s)			5.0		5.0					5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0					3.0	
Recall Mode			None		C-Min					None	
Act Effct Green (s)			9.0		71.0					9.0	
Actuated g/C Ratio			0.10		0.79					0.10	
v/c Ratio			0.58		0.86					0.28	
Control Delay			47.5		4.2					42.0	
Queue Delay			0.0		0.0					0.0	
Total Delay			47.5		4.2					42.0	
LOS			D		A					D	
Approach Delay	47.5				4.2					42.0	
Approach LOS	D				A					D	
Queue Length 50th (ft)			50		183					26	
Queue Length 95th (ft)			85		m208					60	

Lanes, Volumes, Timings

81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

04/07/2017

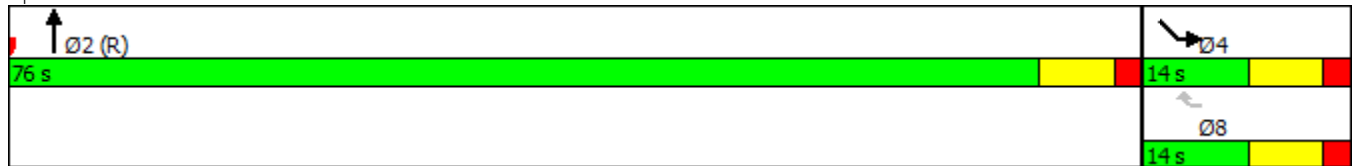


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	398				99			704		130	
Turn Bay Length (ft)											
Base Capacity (vph)			278		3893					173	
Starvation Cap Reductn			0		0					0	
Spillback Cap Reductn			0		0					0	
Storage Cap Reductn			0		0					0	
Reduced v/c Ratio			0.58		0.86					0.28	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	25 (28%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.86
Intersection Signal Delay:	6.7
Intersection LOS:	A
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over



Lanes, Volumes, Timings
 82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	32	0	0	0	0	2024
Future Volume (vph)	32	0	0	0	0	2024
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	1719	0	0	0	0	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	85		652			641
Travel Time (s)	2.3		9.9			9.7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	2%	4%	4%	4%	4%
Adj. Flow (vph)	36	0	0	0	0	2249
Shared Lane Traffic (%)						
Lane Group Flow (vph)	36	0	0	0	0	2249
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.5%
ICU Level of Service	G
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	32	0	0	0	0	2024
Future Volume (Veh/h)	32	0	0	0	0	2024
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	36	0	0	0	0	2249
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	750	0			0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	750	0			0	
tC, single (s)	6.9	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	89	100			100	
cM capacity (veh/h)	341	1084			1607	

Direction, Lane #	WB 1	SB 1	SB 2	SB 3
Volume Total	36	750	750	750
Volume Left	36	0	0	0
Volume Right	0	0	0	0
cSH	341	1700	1700	1700
Volume to Capacity	0.11	0.44	0.44	0.44
Queue Length 95th (ft)	9	0	0	0
Control Delay (s)	16.8	0.0	0.0	0.0
Lane LOS	C			
Approach Delay (s)	16.8	0.0		
Approach LOS	C			

Intersection Summary			
Average Delay		0.3	
Intersection Capacity Utilization		105.5%	ICU Level of Service
Analysis Period (min)		15	G

HCM 2010 TWSC
 82: SB US 321 & 15th Avenue NW U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔					↑↑↑
Traffic Vol, veh/h	32	0	0	0	0	2024
Future Vol, veh/h	32	0	0	0	0	2024
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	2	4	4	4	4
Mvmt Flow	36	0	0	0	0	2249

Major/Minor

	Minor1		Major2
Conflicting Flow All	900	-	-
Stage 1	0	-	-
Stage 2	900	-	-
Critical Hdwy	5.8	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	6.1	-	-
Follow-up Hdwy	3.85	-	-
Pot Cap-1 Maneuver	342	0	0
Stage 1	-	0	0
Stage 2	317	0	0
Platoon blocked, %			-
Mov Cap-1 Maneuver	342	-	-
Mov Cap-2 Maneuver	342	-	-
Stage 1	-	-	-
Stage 2	317	-	-

Approach

	WB	SB
HCM Control Delay, s	16.7	0
HCM LOS	C	

Minor Lane/Major Mvmt

	WBLn1	SBT
Capacity (veh/h)	342	-
HCM Lane V/C Ratio	0.104	-
HCM Control Delay (s)	16.7	-
HCM Lane LOS	C	-
HCM 95th %tile Q(veh)	0.3	-

Lanes, Volumes, Timings

83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	52	0	0	0	0	1998	15	14	0
Future Volume (vph)	0	0	52	0	0	0	0	1998	15	14	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	1.00
Fr _t			0.865					0.999			
Fl _t Protected										0.950	
Satd. Flow (prot)	0	0	1565	0	0	0	0	4983	0	1736	0
Fl _t Permitted										0.950	
Satd. Flow (perm)	0	0	1565	0	0	0	0	4983	0	1736	0
Link Speed (mph)	35				45			45		25	
Link Distance (ft)	560				842			224		173	
Travel Time (s)	10.9				12.8			3.4		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	4%	4%	4%	4%	4%	4%	4%	0%
Adj. Flow (vph)	0	0	58	0	0	0	0	2220	17	16	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	58	0	0	0	0	2237	0	16	0
Sign Control	Stop				Free			Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	48.9%
Analysis Period (min)	15
	ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis

83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

04/07/2017



Movement	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (veh/h)	0	0	52	0	0	0	0	1998	15	14	0
Future Volume (Veh/h)	0	0	52	0	0	0	0	1998	15	14	0
Sign Control	Stop			Free			Free			Stop	
Grade	0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	58	0	0	0	0	2220	17	16	0
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type											
Median storage veh											
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	2236	2228	748	2237			0			2237	0
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
vCu, unblocked vol	2236	2228	748	2237			0			2237	0
tC, single (s)	7.6	6.6	7.0	4.2			4.2			6.6	6.9
tC, 2 stage (s)											
tF (s)	3.5	4.0	3.3	2.2			2.2			4.0	3.3
p0 queue free %	100	100	83	100			100			61	100
cM capacity (veh/h)	16	41	348	222			1607			41	1091

Direction, Lane #	EB 1	SB 1	SB 2	SB 3	NW 1
Volume Total	58	888	888	461	16
Volume Left	0	0	0	0	0
Volume Right	58	0	0	17	0
cSH	348	1700	1700	1700	41
Volume to Capacity	0.17	0.52	0.52	0.27	0.39
Queue Length 95th (ft)	15	0	0	0	34
Control Delay (s)	17.4	0.0	0.0	0.0	141.6
Lane LOS	C				F
Approach Delay (s)	17.4	0.0			141.6
Approach LOS	C				F

Intersection Summary				
Average Delay			1.4	
Intersection Capacity Utilization		48.9%		ICU Level of Service
Analysis Period (min)		15		A

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBR	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations							↑↑↑		↑	
Traffic Vol, veh/h	0	0	0	0	0	0	1998	15	14	0
Future Vol, veh/h	0	0	0	0	0	0	1998	15	14	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	None	-	-	None	-	None
Storage Length	-	0	-	-	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	-	-	-	0	-	0	-
Grade, %	0	-	-	0	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	4	4	4	4	4	4	4	0
Mvmt Flow	0	0	0	0	0	0	2220	17	16	0

Major/Minor	Minor2	Major2	Minor1				
Conflicting Flow All	-	1118	-	-	0	888	-
Stage 1	-	-	-	-	-	0	-
Stage 2	-	-	-	-	-	888	-
Critical Hdwy	-	7.2	-	-	-	6.48	-
Critical Hdwy Stg 1	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	6.78	-
Follow-up Hdwy	-	3.95	-	-	-	3.84	-
Pot Cap-1 Maneuver	0	169	0	-	-	293	0
Stage 1	0	-	0	-	-	-	0
Stage 2	0	-	0	-	-	272	0
Platoon blocked, %					-	-	
Mov Cap-1 Maneuver	-	169	-	-	-	193	-
Mov Cap-2 Maneuver	-	-	-	-	-	193	-
Stage 1	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	179	-

Approach	EB	SB	NW
HCM Control Delay, s	37	0	25.3
HCM LOS	E		D

Minor Lane/Major Mvmt	NWLn1	EBLn1	SBT	SBR
Capacity (veh/h)	193	169	-	-
HCM Lane V/C Ratio	0.081	0.342	-	-
HCM Control Delay (s)	25.3	37	-	-
HCM Lane LOS	D	E	-	-
HCM 95th %tile Q(veh)	0.3	1.4	-	-

Lanes, Volumes, Timings
 84: NB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	30	0	0	2982	0	0
Future Volume (vph)	30	0	0	2982	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1719	0	0	4940	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1719	0	0	4940	0	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	53			939	719	
Travel Time (s)	1.4			14.2	10.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	2%	5%	5%	4%	4%
Adj. Flow (vph)	33	0	0	3313	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	33	0	0	3313	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	67.6%
Analysis Period (min)	15
	ICU Level of Service C

HCM Unsignalized Intersection Capacity Analysis

84: NB US 321 & 15th Avenue NW U-Turn

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↶			↷↷↷		
Traffic Volume (veh/h)	30	0	0	2982	0	0
Future Volume (Veh/h)	30	0	0	2982	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	33	0	0	3313	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1104	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1104	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	84	100	100			
cM capacity (veh/h)	200	1084	1600			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	33	1104	1104	1104		
Volume Left	33	0	0	0		
Volume Right	0	0	0	0		
cSH	200	1700	1700	1700		
Volume to Capacity	0.16	0.65	0.65	0.65		
Queue Length 95th (ft)	14	0	0	0		
Control Delay (s)	26.5	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	26.5	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			67.6%		ICU Level of Service	C
Analysis Period (min)			15			

HCM 2010 TWSC
 84: NB US 321 & 15th Avenue NW U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	30	0	0	2982	0	0
Future Vol, veh/h	30	0	0	2982	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	2	5	5	4	4
Mvmt Flow	33	0	0	3313	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	1325	-	-	0
Stage 1	0	-	-	-
Stage 2	1325	-	-	-
Critical Hdwy	5.8	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	-
Follow-up Hdwy	3.85	-	-	-
Pot Cap-1 Maneuver	207	0	0	-
Stage 1	-	0	0	-
Stage 2	185	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	207	-	-	-
Mov Cap-2 Maneuver	207	-	-	-
Stage 1	-	-	-	-
Stage 2	185	-	-	-

Approach

	EB	NB
HCM Control Delay, s	25.7	0
HCM LOS	D	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	207
HCM Lane V/C Ratio	-	0.161
HCM Control Delay (s)	-	25.7
HCM Lane LOS	-	D
HCM 95th %tile Q(veh)	-	0.6

Lanes, Volumes, Timings
 85: NB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	32	3090	0	0
Future Volume (vph)	0	0	32	3090	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1719	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1719	4988	0	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	85			784	717	
Travel Time (s)	2.3			11.9	10.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	5%	4%	4%	4%
Adj. Flow (vph)	0	0	36	3433	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	36	3433	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.5%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 86: SB US 321 & 15th Avenue NW Left-Over

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↑↑↑
Traffic Volume (vph)	0	0	0	0	43	2013
Future Volume (vph)	0	0	0	0	43	2013
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	55		45			45
Link Distance (ft)	210		224			652
Travel Time (s)	2.6		3.4			9.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	48	2237
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	48	2237
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	49.1%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 87: SB US 321 & 15th Avenue NW U-Turn

04/07/2017



Lane Group	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	30	2020	0	0
Future Volume (vph)	0	0	30	2020	0	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	250			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	55			45	45	
Link Distance (ft)	53			842	889	
Travel Time (s)	0.7			12.8	13.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	5%	5%
Adj. Flow (vph)	0	0	33	2244	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	33	2244	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	67.6%
Analysis Period (min)	15
	ICU Level of Service C

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 88: NB US 321 & 15th Avenue NW Left-Over

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↗	↑↑↑
Traffic Volume (vph)	0	0	0	0	14	2998
Future Volume (vph)	0	0	0	0	14	2998
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	200	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	25		45			45
Link Distance (ft)	173		179			719
Travel Time (s)	4.7		2.7			10.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	16	3331
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	16	3331
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	130.0%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	182	18	2690	453	0	1861
Future Volume (vph)	182	18	2690	453	0	1861
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	500	500		550	0	
Storage Lanes	1	1		1	0	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	0.88	0.91	1.00	1.00	0.91
Frt		0.850		0.850		
Flt Protected	0.950					
Satd. Flow (prot)	3433	2787	4988	1553	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3433	2787	4988	1553	0	4988
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	35		55			55
Link Distance (ft)	986		624			167
Travel Time (s)	19.2		7.7			2.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	202	20	2989	503	0	2068
Shared Lane Traffic (%)						
Lane Group Flow (vph)	202	20	2989	503	0	2068
Turn Type	Prot	Prot	NA	Free		NA
Protected Phases	8	8	2			6
Permitted Phases				Free		
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	7.0	7.0	14.0			14.0
Minimum Split (s)	14.0	14.0	21.0			21.0
Total Split (s)	15.0	15.0	75.0			75.0
Total Split (%)	16.7%	16.7%	83.3%			83.3%
Maximum Green (s)	8.0	8.0	68.0			68.0
Yellow Time (s)	5.0	5.0	5.0			5.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0			-2.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	9.9	9.9	70.1	90.0		70.1
Actuated g/C Ratio	0.11	0.11	0.78	1.00		0.78
v/c Ratio	0.53	0.07	0.77	0.32		0.53
Control Delay	43.5	36.5	2.2	0.3		4.4
Queue Delay	0.0	0.0	0.0	0.0		0.0
Total Delay	43.5	36.5	2.2	0.3		4.4
LOS	D	D	A	A		A
Approach Delay	42.8		2.0			4.4

Lanes, Volumes, Timings
 91: US 321 & Grace Chapel Road

04/07/2017

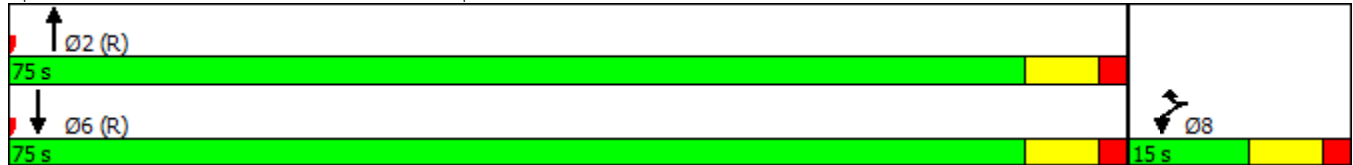


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	D		A		A	
Queue Length 50th (ft)	56	5	41	0	126	
Queue Length 95th (ft)	91	18	56	m0	150	
Internal Link Dist (ft)	906		544		87	
Turn Bay Length (ft)	500	500	550			
Base Capacity (vph)	381	309	3882	1553	3882	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.53	0.06	0.77	0.32	0.53	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green, Master Intersection
 Natural Cycle: 60
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 4.4
 Intersection LOS: A
 Intersection Capacity Utilization 66.1%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 91: US 321 & Grace Chapel Road



Lanes, Volumes, Timings
 92: Grace Chapel Road U-Turn & SB US 321

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	4	0	0	0	0	1861
Future Volume (vph)	4	0	0	0	0	1861
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	1	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	0	0	4988
Link Speed (mph)	25		55			55
Link Distance (ft)	91		428			2189
Travel Time (s)	2.5		5.3			27.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	4	0	0	0	0	2068
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	0	0	0	0	2068
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.7%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis
 92: Grace Chapel Road U-Turn & SB US 321

04/07/2017



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↶					↷↷↷
Traffic Volume (veh/h)	4	0	0	0	0	1861
Future Volume (Veh/h)	4	0	0	0	0	1861
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	4	0	0	0	0	2068
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	689	0			0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	689	0			0	
tC, single (s)	6.9	6.9			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	100			100	
cM capacity (veh/h)	375	1084			1607	

Direction, Lane #	WB 1	SB 1	SB 2	SB 3
Volume Total	4	689	689	689
Volume Left	4	0	0	0
Volume Right	0	0	0	0
cSH	375	1700	1700	1700
Volume to Capacity	0.01	0.41	0.41	0.41
Queue Length 95th (ft)	1	0	0	0
Control Delay (s)	14.7	0.0	0.0	0.0
Lane LOS	B			
Approach Delay (s)	14.7	0.0		
Approach LOS	B			

Intersection Summary			
Average Delay		0.0	
Intersection Capacity Utilization		55.7%	ICU Level of Service B
Analysis Period (min)		15	

Intersection

Int Delay, s/veh 0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔					↑↑↑
Traffic Vol, veh/h	4	0	0	0	0	1861
Future Vol, veh/h	4	0	0	0	0	1861
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	500	-
Veh in Median Storage, #	0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	4	0	0	0	0	2068

Major/Minor

	Minor1		Major2	
Conflicting Flow All	827	-	-	-
Stage 1	0	-	-	-
Stage 2	827	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	374	0	0	-
Stage 1	-	0	0	-
Stage 2	349	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	374	-	-	-
Mov Cap-2 Maneuver	374	-	-	-
Stage 1	-	-	-	-
Stage 2	349	-	-	-

Approach

	WB	SB
HCM Control Delay, s	14.7	0
HCM LOS	B	

Minor Lane/Major Mvmt

	WBLn1	SBT
Capacity (veh/h)	374	-
HCM Lane V/C Ratio	0.012	-
HCM Control Delay (s)	14.7	-
HCM Lane LOS	B	-
HCM 95th %tile Q(veh)	0	-

Lanes, Volumes, Timings
 93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	29	0	0	3114	0	0
Future Volume (vph)	29	0	0	3114	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			45	55	
Link Distance (ft)	169			796	599	
Travel Time (s)	4.6			12.1	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	32	0	0	3460	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	32	0	0	3460	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	105.7%
ICU Level of Service	G
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↵			↑↑↑		
Traffic Volume (veh/h)	29	0	0	3114	0	0
Future Volume (Veh/h)	29	0	0	3114	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	32	0	0	3460	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1153	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1153	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	83	100	100			
cM capacity (veh/h)	188	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	32	1153	1153	1153		
Volume Left	32	0	0	0		
Volume Right	0	0	0	0		
cSH	188	1700	1700	1700		
Volume to Capacity	0.17	0.68	0.68	0.68		
Queue Length 95th (ft)	15	0	0	0		
Control Delay (s)	28.1	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	28.1	0.0				
Approach LOS	D					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			105.7%		ICU Level of Service	G
Analysis Period (min)			15			

HCM 2010 TWSC
 93: NB US 321 & Grace Chapel Road U-Turn

04/07/2017

Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	29	0	0	3114	0	0
Future Vol, veh/h	29	0	0	3114	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	32	0	0	3460	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	1384	-	-	0
Stage 1	0	-	-	-
Stage 2	1384	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	194	0	0	-
Stage 1	-	0	0	-
Stage 2	173	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	194	-	-	-
Mov Cap-2 Maneuver	194	-	-	-
Stage 1	-	-	-	-
Stage 2	173	-	-	-

Approach

	EB	NB
HCM Control Delay, s	27.2	0
HCM LOS	D	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	194
HCM Lane V/C Ratio	-	0.166
HCM Control Delay (s)	-	27.2
HCM Lane LOS	-	D
HCM 95th %tile Q(veh)	-	0.6

Lanes, Volumes, Timings
 94: NB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	4	2708	0	0
Future Volume (vph)	0	0	4	2708	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	200			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		40			
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	4988	0	0
Link Speed (mph)	30			55	55	
Link Distance (ft)	91			347	2226	
Travel Time (s)	2.1			4.3	27.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	4	3009	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	4	3009	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.7%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 95: SB US 321 & Grace Chapel Road U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↑↑↑
Traffic Volume (vph)	0	0	0	0	29	2014
Future Volume (vph)	0	0	0	0	29	2014
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	350	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	4988
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	4988
Link Speed (mph)	30		45			55
Link Distance (ft)	169		960			435
Travel Time (s)	3.8		14.5			5.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	32	2238
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	32	2238
Sign Control	Stop		Stop			Free

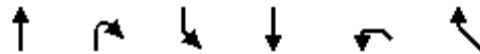
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	128.4%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 96: SB US 321/US 321 & NB US 321

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑↑↑		↑↑↑↑
Traffic Volume (vph)	0	0	0	2043	0	3143
Future Volume (vph)	0	0	0	2043	0	3143
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	4988	0	3541
Flt Permitted						
Satd. Flow (perm)	0	0	0	4988	0	3541
Link Speed (mph)	55			55	55	
Link Distance (ft)	435			624	599	
Travel Time (s)	5.4			7.7	7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	2270	0	3492
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2270	0	3492
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	119.4%
ICU Level of Service	H
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 97: US 321 & NB US 321

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations		↑↑↑				↑↑↑
Traffic Volume (vph)	0	2708	0	0	0	1861
Future Volume (vph)	0	2708	0	0	0	1861
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	0.76
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	4988	0	0	0	3541
Flt Permitted						
Satd. Flow (perm)	0	4988	0	0	0	3541
Link Speed (mph)		55	55		55	
Link Distance (ft)		167	347		428	
Travel Time (s)		2.1	4.3		5.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	4%	4%
Adj. Flow (vph)	0	3009	0	0	0	2068
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	3009	0	0	0	2068
Sign Control		Free	Stop		Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.7%
ICU Level of Service	B
Analysis Period (min)	15

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↗↗		↕↕	↗				↖	
Traffic Volume (vph)	0	0	104	0	2343	80	0	0	0	36	0
Future Volume (vph)	0	0	104	0	2343	80	0	0	0	36	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0		0	250		0	0	0
Storage Lanes	0	2		0		1	0		0	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2538	0	3471	1442	0	0	0	1612	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2538	0	3471	1442	0	0	0	1612	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	682				200			676		124	
Travel Time (s)	13.3				2.5			8.4		3.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	12%	12%	12%	2%	4%	12%	12%	4%	4%	12%	4%
Adj. Flow (vph)	0	0	116	0	2603	89	0	0	0	40	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	116	0	2603	89	0	0	0	40	0
Turn Type			Perm		NA	Perm				Prot	
Protected Phases					2					4	
Permitted Phases			8			2					
Detector Phase			8		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		14.0	14.0				7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	
Total Split (s)			14.0		76.0	76.0				14.0	
Total Split (%)			15.6%		84.4%	84.4%				15.6%	
Maximum Green (s)			7.0		69.0	69.0				7.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effect Green (s)			9.0		74.8	74.8				9.0	
Actuated g/C Ratio			0.10		0.83	0.83				0.10	
v/c Ratio			0.46		0.90	0.07				0.25	
Control Delay			44.6		6.3	0.2				41.2	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			44.6		6.3	0.2				41.2	
LOS			D		A	A				D	
Approach Delay	44.6				6.1					41.2	

Lanes, Volumes, Timings

101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

04/07/2017

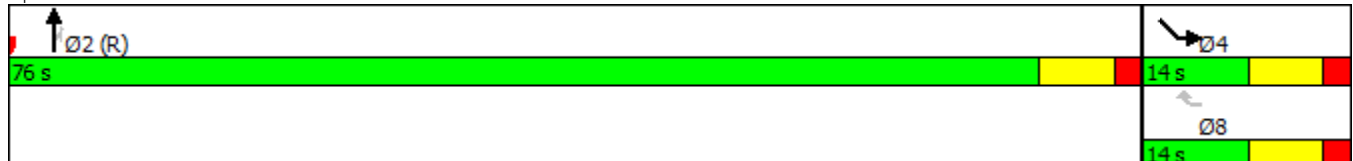


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Approach LOS	D			A						D	
Queue Length 50th (ft)			35			37	0		23		
Queue Length 95th (ft)			65	#906		m1				m40	
Internal Link Dist (ft)	602			120			596			44	
Turn Bay Length (ft)											
Base Capacity (vph)			253	2885		1198				161	
Starvation Cap Reductn			0	0		0				0	
Spillback Cap Reductn			0	0		0				0	
Storage Cap Reductn			0	0		0				0	
Reduced v/c Ratio			0.46	0.90		0.07				0.25	

Intersection Summary

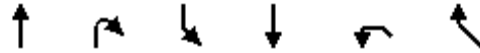
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 43 (48%), Referenced to phase 2:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 8.2 Intersection LOS: A
 Intersection Capacity Utilization Err% ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over



Lanes, Volumes, Timings
 102: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↗	
Traffic Volume (vph)	0	0	0	1601	90	0
Future Volume (vph)	0	0	0	1601	90	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3505	1203	0
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	3505	1203	0
Link Speed (mph)	30			55	25	
Link Distance (ft)	653			1220	102	
Travel Time (s)	14.8			15.1	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	3%	50%	2%
Adj. Flow (vph)	0	0	0	1779	100	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	1779	100	0
Sign Control	Free			Free	Stop	

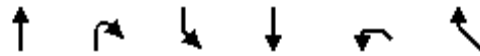
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	116.1%
Analysis Period (min)	15
	ICU Level of Service H

HCM Unsignalized Intersection Capacity Analysis

102: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↗	
Traffic Volume (veh/h)	0	0	0	1601	90	0
Future Volume (Veh/h)	0	0	0	1601	90	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	1779	100	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			0	890	0	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0	890	0	
tC, single (s)			4.2	7.8	6.9	
tC, 2 stage (s)						
tF (s)			2.2	4.0	3.3	
p0 queue free %			100	52	100	
cM capacity (veh/h)			1607	206	1084	
Direction, Lane #	SB 1	SB 2	NW 1			
Volume Total	890	890	100			
Volume Left	0	0	100			
Volume Right	0	0	0			
cSH	1700	1700	206			
Volume to Capacity	0.52	0.52	0.48			
Queue Length 95th (ft)	0	0	60			
Control Delay (s)	0.0	0.0	37.9			
Lane LOS			E			
Approach Delay (s)	0.0		37.9			
Approach LOS			E			
Intersection Summary						
Average Delay			2.0			
Intersection Capacity Utilization			116.1%	ICU Level of Service	H	
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 2

Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations				↑↑	↙	
Traffic Vol, veh/h	0	0	0	1601	90	0
Future Vol, veh/h	0	0	0	1601	90	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	4	4	3	50	2
Mvmt Flow	0	0	0	1779	100	0

Major/Minor

	Major2	Minor1
Conflicting Flow All	-	889
Stage 1	-	0
Stage 2	-	889
Critical Hdwy	-	7.8
Critical Hdwy Stg 1	-	-
Critical Hdwy Stg 2	-	6.8
Follow-up Hdwy	-	4
Pot Cap-1 Maneuver	0	206
Stage 1	0	-
Stage 2	0	264
Platoon blocked, %	-	-
Mov Cap-1 Maneuver	-	206
Mov Cap-2 Maneuver	-	206
Stage 1	-	-
Stage 2	-	264

Approach

	SB	NW
HCM Control Delay, s	0	37.9
HCM LOS		E

Minor Lane/Major Mvmt

	NWLn1	SBT
Capacity (veh/h)	206	-
HCM Lane V/C Ratio	0.485	-
HCM Control Delay (s)	37.9	-
HCM Lane LOS	E	-
HCM 95th %tile Q(veh)	2.4	-

Lanes, Volumes, Timings

103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (vph)	0	0	25	0	0	0	0	1654	4	4	0
Future Volume (vph)	0	0	25	0	0	0	0	1654	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.865						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	1565	0	0	0	0	3471	1553	1719	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	1565	0	0	0	0	3471	1553	1719	0
Link Speed (mph)	35				30			55		25	
Link Distance (ft)	227				766			154		171	
Travel Time (s)	4.4				17.4			1.9		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	5%	5%	5%	4%	4%	4%	4%	4%	4%	5%	2%
Adj. Flow (vph)	0	0	28	0	0	0	0	1838	4	4	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	28	0	0	0	0	1838	4	4	0
Sign Control	Stop				Free			Free		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.7%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis

103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

04/07/2017



Movement	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations											
Traffic Volume (veh/h)	0	0	25	0	0	0	0	1654	4	4	0
Future Volume (Veh/h)	0	0	25	0	0	0	0	1654	4	4	0
Sign Control	Stop			Free			Free			Stop	
Grade	0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	28	0	0	0	0	1838	4	4	0
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type											
Median storage veh											
Upstream signal (ft)											
pX, platoon unblocked											
vC, conflicting volume	1840	1838	919	1842			0			1842	0
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
vCu, unblocked vol	1840	1838	919	1842			0			1842	0
tC, single (s)	7.6	6.6	7.0	4.2			4.2			6.6	6.9
tC, 2 stage (s)											
tF (s)	3.5	4.0	3.3	2.2			2.2			4.0	3.3
p0 queue free %	100	100	90	100			100			94	100
cM capacity (veh/h)	43	72	268	318			1607			72	1084
Direction, Lane #											
	EB 1	SB 1	SB 2	SB 3	NW 1						
Volume Total	28	919	919	4	4						
Volume Left	0	0	0	0	0						
Volume Right	28	0	0	4	0						
cSH	268	1700	1700	1700	72						
Volume to Capacity	0.10	0.54	0.54	0.00	0.06						
Queue Length 95th (ft)	9	0	0	0	4						
Control Delay (s)	20.0	0.0	0.0	0.0	57.9						
Lane LOS	C				F						
Approach Delay (s)	20.0	0.0			57.9						
Approach LOS	C				F						
Intersection Summary											
Average Delay			0.4								
Intersection Capacity Utilization			55.7%		ICU Level of Service				B		
Analysis Period (min)			15								

Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBR	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations							↑↑	↑	↑	
Traffic Vol, veh/h	0	0	0	0	0	0	1654	4	4	0
Future Vol, veh/h	0	0	0	0	0	0	1654	4	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	None	-	-	None	-	None
Storage Length	-	0	-	-	-	-	-	0	0	-
Veh in Median Storage, #	0	-	-	-	-	-	0	-	0	-
Grade, %	0	-	-	0	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	4	4	4	4	4	4	5	2
Mvmt Flow	0	0	0	0	0	0	1838	4	4	0

Major/Minor	Minor2	Major2	Minor1
Conflicting Flow All	-	919	-
Stage 1	-	-	0
Stage 2	-	-	919
Critical Hdwy	-	7	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	6.6
Follow-up Hdwy	-	3.35	-
Pot Cap-1 Maneuver	0	268	0
Stage 1	0	-	-
Stage 2	0	-	286
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	268	-
Mov Cap-2 Maneuver	-	-	199
Stage 1	-	-	-
Stage 2	-	-	256

Approach	EB	SB	NW
HCM Control Delay, s	20	0	23.5
HCM LOS	C		C

Minor Lane/Major Mvmt	NWLn1	EBLn1	SBT	SBR
Capacity (veh/h)	199	268	-	-
HCM Lane V/C Ratio	0.022	0.104	-	-
HCM Control Delay (s)	23.5	20	-	-
HCM Lane LOS	C	C	-	-
HCM 95th %tile Q(veh)	0.1	0.3	-	-

Lanes, Volumes, Timings
 104: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	11	0	0	2414	0	0
Future Volume (vph)	11	0	0	2414	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	4988	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	4988	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	57			2226	645	
Travel Time (s)	1.6			27.6	8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	12	0	0	2682	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	12	0	0	2682	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	56.6%
Analysis Period (min)	15
	ICU Level of Service B

HCM Unsignalized Intersection Capacity Analysis

104: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	11	0	0	2414	0	0
Future Volume (Veh/h)	11	0	0	2414	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	12	0	0	2682	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	894	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	894	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	100	100			
cM capacity (veh/h)	277	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3		
Volume Total	12	894	894	894		
Volume Left	12	0	0	0		
Volume Right	0	0	0	0		
cSH	277	1700	1700	1700		
Volume to Capacity	0.04	0.53	0.53	0.53		
Queue Length 95th (ft)	3	0	0	0		
Control Delay (s)	18.6	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	18.6	0.0				
Approach LOS	C					
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			56.6%		ICU Level of Service	B
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑↑		
Traffic Vol, veh/h	11	0	0	2414	0	0
Future Vol, veh/h	11	0	0	2414	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	12	0	0	2682	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	1073	-	-	0
Stage 1	0	-	-	-
Stage 2	1073	-	-	-
Critical Hdwy	5.78	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	6.08	-	-	-
Follow-up Hdwy	3.84	-	-	-
Pot Cap-1 Maneuver	281	0	0	-
Stage 1	-	0	0	-
Stage 2	257	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	281	-	-	-
Mov Cap-2 Maneuver	281	-	-	-
Stage 1	-	-	-	-
Stage 2	257	-	-	-

Approach

	EB	NB
HCM Control Delay, s	18.4	0
HCM LOS	C	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	281
HCM Lane V/C Ratio	-	0.043
HCM Control Delay (s)	-	18.4
HCM Lane LOS	-	C
HCM 95th %tile Q(veh)	-	0.1

Lanes, Volumes, Timings
 105: NB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	90	2357	0	0	0	0
Future Volume (vph)	90	2357	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1203	3471	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1203	3471	0	0	0	0
Link Speed (mph)		55	55		30	
Link Distance (ft)		676	1243		102	
Travel Time (s)		8.4	15.4		2.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	50%	4%	4%	4%	2%	2%
Adj. Flow (vph)	100	2619	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	100	2619	0	0	0	0
Sign Control		Free	Stop		Stop	

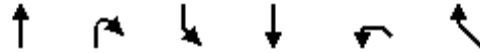
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	116.1%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 106: SB US 321 & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Volume (vph)	0	0	36	1655	0	0
Future Volume (vph)	0	0	36	1655	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	400		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	154			653	124	
Travel Time (s)	3.5			8.1	2.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	40	1839	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	40	1839	0	0
Sign Control	Stop			Free	Stop	

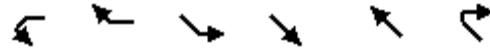
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.9%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 107: SB US 321 & Alex Lee Blvd U-Turn

04/07/2017



Lane Group	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations			↙	↗↗		
Traffic Volume (vph)	0	0	11	1668	0	0
Future Volume (vph)	0	0	11	1668	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	250			0
Storage Lanes	0	0	1			0
Taper Length (ft)	50		50			
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	1736	3471	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1736	3471	0	0
Link Speed (mph)	30			55	30	
Link Distance (ft)	57			766	2189	
Travel Time (s)	1.3			9.5	49.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	12	1853	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	12	1853	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	56.6%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 108: NB US 321 & Alex Lee Blvd Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	4	2423	0	0	0	0
Future Volume (vph)	4	2423	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		25	
Link Distance (ft)		645	200		171	
Travel Time (s)		8.0	2.5		4.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	2%	2%
Adj. Flow (vph)	4	2692	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	2692	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	108.0%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings

121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

04/07/2017



Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Lane Configurations			↑↑↑		↑↑	↑				↑↑	
Traffic Volume (vph)	0	0	813	0	1945	543	0	0	0	405	0
Future Volume (vph)	0	0	813	0	1945	543	0	0	0	405	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.76	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00
Frt			0.850			0.850					
Flt Protected										0.950	
Satd. Flow (prot)	0	0	3610	0	3471	1553	0	0	0	3367	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	3610	0	3471	1553	0	0	0	3367	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	489				152			678		131	
Travel Time (s)	9.5				1.9			8.4		3.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%	4%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	903	0	2161	603	0	0	0	450	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	903	0	2161	603	0	0	0	450	0
Turn Type			Perm		NA	Perm				Prot	
Protected Phases					2					4	
Permitted Phases			8			2					
Detector Phase			8		2	2				4	
Switch Phase											
Minimum Initial (s)			7.0		14.0	14.0				7.0	
Minimum Split (s)			14.0		21.0	21.0				14.0	
Total Split (s)			28.0		62.0	62.0				28.0	
Total Split (%)			31.1%		68.9%	68.9%				31.1%	
Maximum Green (s)			21.0		55.0	55.0				21.0	
Yellow Time (s)			5.0		5.0	5.0				5.0	
All-Red Time (s)			2.0		2.0	2.0				2.0	
Lost Time Adjust (s)			-2.0		-2.0	-2.0				-2.0	
Total Lost Time (s)			5.0		5.0	5.0				5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0		3.0	3.0				3.0	
Recall Mode			None		C-Min	C-Min				None	
Act Effct Green (s)			23.0		57.0	57.0				23.0	
Actuated g/C Ratio			0.26		0.63	0.63				0.26	
v/c Ratio			0.98		0.98	0.61				0.52	
Control Delay			59.6		21.7	7.0				27.9	
Queue Delay			0.0		0.0	0.0				0.0	
Total Delay			59.6		21.7	7.0				27.9	
LOS			E		C	A				C	
Approach Delay	59.6				18.5					27.9	
Approach LOS	E				B					C	
Queue Length 50th (ft)			223		462	107				117	
Queue Length 95th (ft)			#331		#802	m143				151	

Lanes, Volumes, Timings

121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

04/07/2017

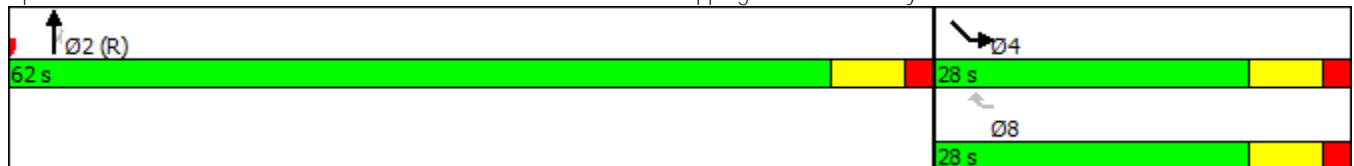


Lane Group	WBL	WBR	WBR2	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
Internal Link Dist (ft)	409				72			598		51	
Turn Bay Length (ft)											
Base Capacity (vph)			922		2198	983				860	
Starvation Cap Reductn			0		0	0				0	
Spillback Cap Reductn			0		0	0				0	
Storage Cap Reductn			0		0	0				0	
Reduced v/c Ratio			0.98		0.98	0.61				0.52	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.98
Intersection Signal Delay:	28.6
Intersection LOS:	C
Intersection Capacity Utilization Err%	ICU Level of Service H
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over



Lanes, Volumes, Timings

122: SB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙↘					↑↑↑
Traffic Volume (vph)	404	0	0	0	0	1645
Future Volume (vph)	404	0	0	0	0	1645
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	2	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	0.91
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	3367	0	0	0	0	4988
Flt Permitted	0.950					
Satd. Flow (perm)	3367	0	0	0	0	4988
Right Turn on Red	No	No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	25		55			55
Link Distance (ft)	118		659			370
Travel Time (s)	3.2		8.2			4.6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	2%	2%	4%
Adj. Flow (vph)	449	0	0	0	0	1828
Shared Lane Traffic (%)						
Lane Group Flow (vph)	449	0	0	0	0	1828
Turn Type	Prot					NA
Protected Phases	8					6
Permitted Phases						
Detector Phase	8					6
Switch Phase						
Minimum Initial (s)	14.0					14.0
Minimum Split (s)	21.0					21.0
Total Split (s)	29.0					61.0
Total Split (%)	32.2%					67.8%
Maximum Green (s)	22.0					54.0
Yellow Time (s)	5.0					5.0
All-Red Time (s)	2.0					2.0
Lost Time Adjust (s)	-2.0					-2.0
Total Lost Time (s)	5.0					5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0					3.0
Recall Mode	None					C-Min
Act Effct Green (s)	19.4					60.6
Actuated g/C Ratio	0.22					0.67
v/c Ratio	0.62					0.54
Control Delay	33.2					7.3
Queue Delay	0.0					0.0
Total Delay	33.2					7.3
LOS	C					A
Approach Delay	33.2					7.3

Lanes, Volumes, Timings

122: SB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach LOS	C			A		
Queue Length 50th (ft)	120			128		
Queue Length 95th (ft)	m113			m207		
Internal Link Dist (ft)	38		579		290	
Turn Bay Length (ft)						
Base Capacity (vph)	897			3357		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.50			0.54		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 38 (42%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 45
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.62
 Intersection Signal Delay: 12.4
 Intersection LOS: B
 Intersection Capacity Utilization 104.4%
 ICU Level of Service G
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 122: SB US 321 & Walmart Shopping Center Driveway U-Turn



Lanes, Volumes, Timings
 123: SB US 321 & US 321 A Left-Over & US 321 A

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Lane Configurations			↑↑					↑↑	↑	↑	
Traffic Volume (vph)	0	0	157	0	0	0	0	1497	147	37	0
Future Volume (vph)	0	0	157	0	0	0	0	1497	147	37	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	350		0		0	0		0	0	0
Storage Lanes	0	1		0		0	0		1	1	0
Taper Length (ft)	50			50			50			50	
Lane Util. Factor	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Frt			0.850						0.850		
Flt Protected										0.950	
Satd. Flow (prot)	0	0	2760	0	0	0	0	3471	1553	1736	0
Flt Permitted										0.950	
Satd. Flow (perm)	0	0	2760	0	0	0	0	3471	1553	1736	0
Right Turn on Red			No			No			No		No
Satd. Flow (RTOR)											
Link Speed (mph)	35				55			55		25	
Link Distance (ft)	456				1044			127		156	
Travel Time (s)	8.9				12.9			1.6		4.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	4%	4%	4%	2%
Adj. Flow (vph)	0	0	174	0	0	0	0	1663	163	41	0
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	174	0	0	0	0	1663	163	41	0
Turn Type			Perm					NA	Perm	Prot	
Protected Phases								6		8	
Permitted Phases			4						6		
Detector Phase			4					6	6	8	
Switch Phase											
Minimum Initial (s)			7.0					14.0	14.0	7.0	
Minimum Split (s)			14.0					21.0	21.0	14.0	
Total Split (s)			16.0					74.0	74.0	16.0	
Total Split (%)			17.8%					82.2%	82.2%	17.8%	
Maximum Green (s)			9.0					67.0	67.0	9.0	
Yellow Time (s)			5.0					5.0	5.0	5.0	
All-Red Time (s)			2.0					2.0	2.0	2.0	
Lost Time Adjust (s)			-2.0					-2.0	-2.0	-2.0	
Total Lost Time (s)			5.0					5.0	5.0	5.0	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)			3.0					3.0	3.0	3.0	
Recall Mode			None					C-Min	C-Min	None	
Act Effct Green (s)			11.9					68.1	68.1	11.9	
Actuated g/C Ratio			0.13					0.76	0.76	0.13	
v/c Ratio			0.48					0.63	0.14	0.18	
Control Delay			40.7					4.4	2.3	39.5	
Queue Delay			0.0					0.0	0.0	0.0	
Total Delay			40.7					4.4	2.3	39.5	
LOS			D					A	A	D	
Approach Delay	40.7							4.2		39.5	

Lanes, Volumes, Timings

123: SB US 321 & US 321 A Left-Over & US 321 A

04/07/2017



Lane Group	EBL	EBR	EBR2	NBL	NBT	NBR	SBL	SBT	SBR	NWL	NWR
Approach LOS	D						A			D	
Queue Length 50th (ft)			52				164		15	23	
Queue Length 95th (ft)			89				131		24	m25	
Internal Link Dist (ft)	376					964		47		76	
Turn Bay Length (ft)			350								
Base Capacity (vph)			373					2672		1195	234
Starvation Cap Reductn			0					0		0	0
Spillback Cap Reductn			0					0		0	0
Storage Cap Reductn			0					0		0	0
Reduced v/c Ratio			0.47					0.62		0.14	0.18

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 30 (33%), Referenced to phase 6:SBT, Start of Green
 Natural Cycle: 50
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.63
 Intersection Signal Delay: 8.0
 Intersection LOS: A
 Intersection Capacity Utilization Err%
 ICU Level of Service H
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 123: SB US 321 & US 321 A Left-Over & US 321 A



Lanes, Volumes, Timings
 124: NB US 321 & US 321 A U-Turn

04/07/2017



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	135	0	0	2390	0	0
Future Volume (vph)	135	0	0	2390	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	0	0	3471	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	0	0	3471	0	0
Link Speed (mph)	25			55	55	
Link Distance (ft)	85			1243	972	
Travel Time (s)	2.3			15.4	12.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	4%	4%	4%	4%
Adj. Flow (vph)	150	0	0	2656	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	150	0	0	2656	0	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	80.2%
Analysis Period (min)	15
	ICU Level of Service D

HCM Unsignalized Intersection Capacity Analysis

124: NB US 321 & US 321 A U-Turn

04/07/2017



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	135	0	0	2390	0	0
Future Volume (Veh/h)	135	0	0	2390	0	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	150	0	0	2656	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1328	0	0			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1328	0	0			
tC, single (s)	6.9	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	0	100	100			
cM capacity (veh/h)	144	1084	1607			
Direction, Lane #	EB 1	NB 1	NB 2			
Volume Total	150	1328	1328			
Volume Left	150	0	0			
Volume Right	0	0	0			
cSH	144	1700	1700			
Volume to Capacity	1.04	0.78	0.78			
Queue Length 95th (ft)	197	0	0			
Control Delay (s)	148.2	0.0	0.0			
Lane LOS	F					
Approach Delay (s)	148.2	0.0				
Approach LOS	F					
Intersection Summary						
Average Delay			7.9			
Intersection Capacity Utilization		80.2%		ICU Level of Service		D
Analysis Period (min)			15			

Intersection

Int Delay, s/veh 7.9

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↑↑		
Traffic Vol, veh/h	135	0	0	2390	0	0
Future Vol, veh/h	135	0	0	2390	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	2	4	4	4	4
Mvmt Flow	150	0	0	2656	0	0

Major/Minor

	Minor2	Major1		
Conflicting Flow All	1328	-	-	0
Stage 1	0	-	-	-
Stage 2	1328	-	-	-
Critical Hdwy	6.88	-	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	5.88	-	-	-
Follow-up Hdwy	3.54	-	-	-
Pot Cap-1 Maneuver	- 144	0	0	-
Stage 1	-	0	0	-
Stage 2	208	0	0	-
Platoon blocked, %				-
Mov Cap-1 Maneuver	- 144	-	-	-
Mov Cap-2 Maneuver	- 144	-	-	-
Stage 1	-	-	-	-
Stage 2	208	-	-	-

Approach

	EB	NB
HCM Control Delay, s	148	0
HCM LOS	F	

Minor Lane/Major Mvmt

	NBT	EBLn1
Capacity (veh/h)	-	144
HCM Lane V/C Ratio	-	1.042
HCM Control Delay (s)	-	148
HCM Lane LOS	-	F
HCM 95th %tile Q(veh)	-	7.9

Notes

-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lanes, Volumes, Timings
 125: NB US 321 & Walmart Shopping Center Driveway U-Turn

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations					↕↕	↕↕
Traffic Volume (vph)	0	0	0	0	404	2354
Future Volume (vph)	0	0	0	0	404	2354
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	500	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	0.97	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	3367	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	3367	3471
Link Speed (mph)	55		55			55
Link Distance (ft)	118		483			678
Travel Time (s)	1.5		6.0			8.4
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	449	2616
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	449	2616
Sign Control	Stop		Stop			Free

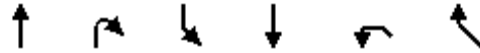
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	104.4%
Analysis Period (min)	15
	ICU Level of Service G

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 126: SB US 321 & Walmart Shopping Center Driveway Left-Over

04/07/2017



Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations			↔↔	↑↑↑		
Traffic Volume (vph)	0	0	405	1644	0	0
Future Volume (vph)	0	0	405	1644	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	500		0	0
Storage Lanes		0	1		0	0
Taper Length (ft)			50		50	
Lane Util. Factor	1.00	1.00	0.97	0.91	1.00	1.00
Frt						
Flt Protected			0.950			
Satd. Flow (prot)	0	0	3367	4988	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	3367	4988	0	0
Link Speed (mph)	55			55	55	
Link Distance (ft)	127			659	131	
Travel Time (s)	1.6			8.2	1.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	0	0	450	1827	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	450	1827	0	0
Sign Control	Stop			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	51.8%
Analysis Period (min)	15
	ICU Level of Service A

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 127: SB US 321 & US 321 A U-Turn

04/07/2017



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations					↙	↕↕
Traffic Volume (vph)	0	0	0	0	135	1519
Future Volume (vph)	0	0	0	0	135	1519
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	250	
Storage Lanes	0	0		0	1	
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	0	1736	3471
Flt Permitted					0.950	
Satd. Flow (perm)	0	0	0	0	1736	3471
Link Speed (mph)	55		30			55
Link Distance (ft)	85		1220			1044
Travel Time (s)	1.1		27.7			12.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	0	150	1688
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	150	1688
Sign Control	Stop		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	80.2%
Analysis Period (min)	15
	ICU Level of Service D

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 128: NB US 321 & US 321 A Left-Over

04/07/2017



Lane Group	NBL	NBT	SBT	SBR	SEL	SER
Lane Configurations						
Traffic Volume (vph)	37	2488	0	0	0	0
Future Volume (vph)	37	2488	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	500			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	50				50	
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00
Frt						
Flt Protected	0.950					
Satd. Flow (prot)	1736	4988	0	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	1736	4988	0	0	0	0
Link Speed (mph)		55	55		55	
Link Distance (ft)		972	152		156	
Travel Time (s)		12.0	1.9		1.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	41	2764	0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	41	2764	0	0	0	0
Sign Control		Free	Stop		Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	131.6%
Analysis Period (min)	15
	ICU Level of Service H

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 996: US 321/NB US 321 & SB US 321

04/07/2017



Lane Group	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations		↑↑↑↑				↑↑↑↑
Traffic Volume (vph)	0	2024	0	0	0	3090
Future Volume (vph)	0	2024	0	0	0	3090
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.76	1.00	1.00	1.00	0.91
Fr _t		0.850				
Flt Protected						
Satd. Flow (prot)	0	3541	0	0	0	4988
Flt Permitted						
Satd. Flow (perm)	0	3541	0	0	0	4988
Link Speed (mph)	45		45			45
Link Distance (ft)	960		796			1065
Travel Time (s)	14.5		12.1			16.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	2%	2%	4%	4%
Adj. Flow (vph)	0	2249	0	0	0	3433
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	2249	0	0	0	3433
Sign Control	Free		Stop			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.0%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Lanes, Volumes, Timings
 997: SB US 321 & NB US 321/US 321

04/07/2017



Lane Group	NBL	NBR	SET	SER	NWL	NWT
Lane Configurations				↑↑↑↑		↑↑↑↑
Traffic Volume (vph)	0	0	0	2024	0	3090
Future Volume (vph)	0	0	0	2024	0	3090
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.76	1.00	0.91
Fr _t				0.850		
Flt Protected						
Satd. Flow (prot)	0	0	0	3541	0	4988
Flt Permitted						
Satd. Flow (perm)	0	0	0	3541	0	4988
Link Speed (mph)	45		45			45
Link Distance (ft)	641		1065			717
Travel Time (s)	9.7		16.1			10.9
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	0	0	2249	0	3433
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2249	0	3433
Sign Control	Stop		Free			Free

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.0%
Analysis Period (min)	15
	ICU Level of Service B

Intersection Sign configuration not allowed in HCM analysis.

Summary of All Intervals

Run Number	1	10	2	3	4	5	6
Start Time	4:50	4:50	4:50	4:50	4:50	4:50	4:50
End Time	6:00	6:00	6:00	6:00	6:00	6:00	6:00
Total Time (min)	70	70	70	70	70	70	70
Time Recorded (min)	60	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1	1
Vehs Entered	7814	7849	7813	7770	7871	7813	7797
Vehs Exited	7699	7631	7519	7201	7598	7587	7701
Starting Vehs	686	659	673	686	656	672	698
Ending Vehs	801	877	967	1255	929	898	794
Denied Entry Before	2	1	0	0	0	0	0
Denied Entry After	1	2	1	65	2	2	1
Travel Distance (mi)	23264	23183	23231	22526	23031	22858	23238
Travel Time (hr)	793.9	792.2	822.8	925.6	819.9	816.6	807.7
Total Delay (hr)	274.4	273.4	304.0	422.7	305.1	305.7	287.7
Total Stops	15454	15623	16144	18535	16806	16361	15746
Fuel Used (gal)	833.7	826.6	836.8	834.8	832.9	828.9	835.4

Summary of All Intervals

Run Number	7	8	9	Avg
Start Time	4:50	4:50	4:50	4:50
End Time	6:00	6:00	6:00	6:00
Total Time (min)	70	70	70	70
Time Recorded (min)	60	60	60	60
# of Intervals	2	2	2	2
# of Recorded Intervals	1	1	1	1
Vehs Entered	7733	7894	7873	7823
Vehs Exited	7564	7534	7625	7567
Starting Vehs	646	636	695	669
Ending Vehs	815	996	943	921
Denied Entry Before	0	0	1	0
Denied Entry After	2	2	2	8
Travel Distance (mi)	23001	23066	23385	23078
Travel Time (hr)	788.9	842.6	826.6	823.7
Total Delay (hr)	274.7	327.5	303.8	307.9
Total Stops	15522	16654	16634	16346
Fuel Used (gal)	823.4	840.5	845.9	833.9

Interval #0 Information Seeding

Start Time	4:50
End Time	5:00
Total Time (min)	10
Volumes adjusted by Growth Factors.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	5:00						
End Time	6:00						
Total Time (min)	60						
Volumes adjusted by Growth Factors.							

Run Number	1	10	2	3	4	5	6
Vehs Entered	7814	7849	7813	7770	7871	7813	7797
Vehs Exited	7699	7631	7519	7201	7598	7587	7701
Starting Vehs	686	659	673	686	656	672	698
Ending Vehs	801	877	967	1255	929	898	794
Denied Entry Before	2	1	0	0	0	0	0
Denied Entry After	1	2	1	65	2	2	1
Travel Distance (mi)	23264	23183	23231	22526	23031	22858	23238
Travel Time (hr)	793.9	792.2	822.8	925.6	819.9	816.6	807.7
Total Delay (hr)	274.4	273.4	304.0	422.7	305.1	305.7	287.7
Total Stops	15454	15623	16144	18535	16806	16361	15746
Fuel Used (gal)	833.7	826.6	836.8	834.8	832.9	828.9	835.4

Interval #1 Information Recording

Start Time	5:00			
End Time	6:00			
Total Time (min)	60			
Volumes adjusted by Growth Factors.				

Run Number	7	8	9	Avg
Vehs Entered	7733	7894	7873	7823
Vehs Exited	7564	7534	7625	7567
Starting Vehs	646	636	695	669
Ending Vehs	815	996	943	921
Denied Entry Before	0	0	1	0
Denied Entry After	2	2	2	8
Travel Distance (mi)	23001	23066	23385	23078
Travel Time (hr)	788.9	842.6	826.6	823.7
Total Delay (hr)	274.7	327.5	303.8	307.9
Total Stops	15522	16654	16634	16346
Fuel Used (gal)	823.4	840.5	845.9	833.9

Total Network Performance By Run

Run Number	1	10	2	3	4	5	6
Denied Delay (hr)	1.4	1.5	1.1	4.6	0.8	1.8	0.9
Denied Del/Veh (s)	0.6	0.7	0.5	2.1	0.4	0.8	0.4
Total Delay (hr)	273.1	271.9	302.9	418.1	304.2	303.9	286.7
Total Del/Veh (s)	115.7	115.0	128.5	178.0	128.4	128.9	121.5
Stop Delay (hr)	119.0	118.3	141.0	253.4	147.0	150.5	129.4
Stop Del/Veh (s)	50.4	50.1	59.8	107.9	62.0	63.8	54.8
Total Stops	15454	15623	16144	18535	16806	16361	15746
Stop/Veh	1.82	1.84	1.90	2.19	1.97	1.93	1.85
Travel Dist (mi)	23263.5	23182.7	23231.5	22525.9	23031.0	22857.9	23238.0
Travel Time (hr)	793.9	792.2	822.8	925.6	819.9	816.6	807.7
Avg Speed (mph)	29	29	28	24	28	28	29
Vehicles Entered	7814	7849	7813	7770	7871	7813	7797
Vehicles Exited	7699	7631	7519	7201	7598	7587	7701
Hourly Exit Rate	7699	7631	7519	7201	7598	7587	7701
Input Volume	162433	162433	162433	162433	162433	162433	162433
% of Volume	5	5	5	4	5	5	5
Denied Entry Before	2	1	0	0	0	0	0
Denied Entry After	1	2	1	65	2	2	1

Total Network Performance By Run

Run Number	7	8	9	Avg
Denied Delay (hr)	2.0	2.1	2.3	1.9
Denied Del/Veh (s)	0.9	0.9	1.0	0.9
Total Delay (hr)	272.7	325.4	301.6	306.0
Total Del/Veh (s)	117.2	137.3	126.7	129.8
Stop Delay (hr)	119.6	159.3	136.3	147.4
Stop Del/Veh (s)	51.4	67.2	57.3	62.5
Total Stops	15522	16654	16634	16346
Stop/Veh	1.85	1.95	1.94	1.93
Travel Dist (mi)	23001.0	23066.2	23385.3	23078.3
Travel Time (hr)	788.9	842.6	826.6	823.7
Avg Speed (mph)	29	27	28	28
Vehicles Entered	7733	7894	7873	7823
Vehicles Exited	7564	7534	7625	7567
Hourly Exit Rate	7564	7534	7625	7567
Input Volume	162433	162433	162433	162433
% of Volume	5	5	5	5
Denied Entry Before	0	0	1	0
Denied Entry After	2	2	2	8

Intersection: 11: NB US 321 & 13th Street SW

Movement

Directions Served
 Maximum Queue (ft)
 Average Queue (ft)
 95th Queue (ft)
 Link Distance (ft)
 Upstream Blk Time (%)
 Queuing Penalty (veh)
 Storage Bay Dist (ft)
 Storage Blk Time (%)
 Queuing Penalty (veh)

Intersection: 15: NB US 321 & SB US 321

Movement	NB	NB	NB
Directions Served	T	T	T
Maximum Queue (ft)	12	81	84
Average Queue (ft)	0	7	8
95th Queue (ft)	9	41	48
Link Distance (ft)	1064	1064	1064
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 21: SB US 321 & Main Avenue Drive NW

Movement	EB	SB
Directions Served	R	T
Maximum Queue (ft)	145	5
Average Queue (ft)	64	0
95th Queue (ft)	122	5
Link Distance (ft)	813	187
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 31: NB US 321 & 2nd Avenue NW & 2nd Avenue NW Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	228	197	151	165	69	90
Average Queue (ft)	160	97	75	101	22	30
95th Queue (ft)	231	191	138	162	57	69
Link Distance (ft)	208	208	79	79	79	89
Upstream Blk Time (%)	3	0	5	9	0	0
Queuing Penalty (veh)	0	0	35	61	1	0
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 32: SB US 321 & 2nd Avenue NW U-Turn

Movement	SB	SB	NW
Directions Served	T	T	L
Maximum Queue (ft)	268	284	133
Average Queue (ft)	103	121	106
95th Queue (ft)	209	227	136
Link Distance (ft)	652	652	42
Upstream Blk Time (%)			51
Queuing Penalty (veh)			117
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 33: SB US 321 & 2nd Avenue NW Left -Over & 2nd Avenue NW

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	251	218	159	152	124	202
Average Queue (ft)	155	103	99	118	50	106
95th Queue (ft)	229	198	158	157	102	176
Link Distance (ft)	549	549	74	74	74	146
Upstream Blk Time (%)			11	19	3	4
Queuing Penalty (veh)			67	112	21	8
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 34: NB US 321 & 2nd Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	451	464	134
Average Queue (ft)	237	255	108
95th Queue (ft)	391	407	129
Link Distance (ft)	981	981	38
Upstream Blk Time (%)			70
Queuing Penalty (veh)			216
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 35: NB US 321 & 2nd Avenue NW U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	182
Average Queue (ft)	50
95th Queue (ft)	133
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	400
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 36: SB US 321 & 2nd Avenue NW Left-Over

Movement	SB	SB	SB
Directions Served	T	T	T
Maximum Queue (ft)	159	166	15
Average Queue (ft)	24	35	1
95th Queue (ft)	98	116	11
Link Distance (ft)	790	790	790
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 37: SB US 321 & 2nd Avenue NW U-Turn

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	275	150	51
Average Queue (ft)	116	19	7
95th Queue (ft)	262	162	96
Link Distance (ft)		779	779
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	300		
Storage Blk Time (%)	2		
Queuing Penalty (veh)	21		

Intersection: 38: NB US 321 & 2nd Avenue NW Left -Over

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	22	64	100
Average Queue (ft)	1	3	8
95th Queue (ft)	10	34	53
Link Distance (ft)		687	687
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	550		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 43: SB US 321 & 7th Avenue NW Left-Over & 7th Avenue NW

Movement	EB	SB	SB	SB	NW
Directions Served	>	T	T	R	L
Maximum Queue (ft)	340	414	431	72	94
Average Queue (ft)	185	197	188	20	41
95th Queue (ft)	304	345	348	56	86
Link Distance (ft)	347	1100	1100	1100	37
Upstream Blk Time (%)	1				15
Queuing Penalty (veh)	0				12
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 44: NB US 321 & 7th Avenue NW U-Turn

Movement	NB	NB	SE
Directions Served	T	T	L
Maximum Queue (ft)	221	241	132
Average Queue (ft)	92	110	92
95th Queue (ft)	176	194	133
Link Distance (ft)	655	655	37
Upstream Blk Time (%)			61
Queuing Penalty (veh)			96
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 47: SB US 321 & 7th Avenue NW U-Turn

Movement	SB
Directions Served	L
Maximum Queue (ft)	132
Average Queue (ft)	25
95th Queue (ft)	87
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	300
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 48: NB US 321 & 7th Avenue NW Left-Over

Movement	NB
Directions Served	L
Maximum Queue (ft)	29
Average Queue (ft)	1
95th Queue (ft)	16
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	150
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 61: NB US 321 & Clement Boulevard & Clement Boulevard Left-Over

Movement	WB	WB	WB	NB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	T	R	L	L
Maximum Queue (ft)	366	350	267	150	147	150	137	196	202
Average Queue (ft)	332	260	142	113	107	110	56	123	141
95th Queue (ft)	386	343	241	157	152	154	130	181	195
Link Distance (ft)	335	335	335	34	34	34	34	88	88
Upstream Blk Time (%)	13	1	0	44	45	45	9	15	22
Queuing Penalty (veh)	0	0	0	265	273	273	57	54	76
Storage Bay Dist (ft)									
Storage Blk Time (%)									
Queuing Penalty (veh)									

Intersection: 62: SB US 321 & 13th Avenue Drive NW/Clement Boulevard U-Turn

Movement	WB	WB	SB	SB	SB
Directions Served	L	L	T	T	TR
Maximum Queue (ft)	123	132	236	164	148
Average Queue (ft)	89	90	109	73	62
95th Queue (ft)	131	133	201	142	123
Link Distance (ft)	36	36	845	845	845
Upstream Blk Time (%)	57	57			
Queuing Penalty (veh)	84	85			
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 63: SB US 321 & Clement Boulevard Left-Over & Clement Boulevard

Movement	EB	EB	SB	SB	SB	NW	NW
Directions Served	>	>	T	T	TR	<	L
Maximum Queue (ft)	210	177	179	175	175	109	96
Average Queue (ft)	122	63	130	136	100	42	39
95th Queue (ft)	194	158	187	190	170	88	79
Link Distance (ft)	834	834	49	49	49	58	58
Upstream Blk Time (%)			28	31	22	12	10
Queuing Penalty (veh)			151	165	119	6	5
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 64: NB US 321 & 9th Avenue NW & Clement Boulevard

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	<L
Maximum Queue (ft)	149	93	258	262	282	176
Average Queue (ft)	75	18	156	154	180	130
95th Queue (ft)	136	60	239	238	260	182
Link Distance (ft)	252		1052	1052		79
Upstream Blk Time (%)						41
Queuing Penalty (veh)						119
Storage Bay Dist (ft)		250			375	
Storage Blk Time (%)					0	
Queuing Penalty (veh)					0	

Intersection: 65: NB US 321 & Clement Boulevard U-Turn

Movement	NW	NW	NW
Directions Served	L	L	T
Maximum Queue (ft)	79	67	98
Average Queue (ft)	12	7	3
95th Queue (ft)	49	37	96
Link Distance (ft)			911
Upstream Blk Time (%)			0
Queuing Penalty (veh)			0
Storage Bay Dist (ft)	300	300	
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 66: SB US 321 & Clement Boulevard Left-Over

Movement	SB	SB	SB	SB	SB
Directions Served	L	L	T	T	T
Maximum Queue (ft)	34	43	192	182	90
Average Queue (ft)	2	2	22	24	6
95th Queue (ft)	17	22	116	117	38
Link Distance (ft)			809	809	809
Upstream Blk Time (%)			0	0	
Queuing Penalty (veh)			0	0	
Storage Bay Dist (ft)	400	400			
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 67: SB US 321 & Clement Boulevard

Movement	SB	SB
Directions Served	L	T
Maximum Queue (ft)	150	41
Average Queue (ft)	30	2
95th Queue (ft)	104	48
Link Distance (ft)		748
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)	0	
Queuing Penalty (veh)	1	

Intersection: 68: NB US 321 & Clement Boulevard Left-Over

Movement	NB	NB	NB	NB	NB
Directions Served	L	T	T	T	T
Maximum Queue (ft)	6	173	168	176	64
Average Queue (ft)	0	35	30	35	6
95th Queue (ft)	7	112	104	115	34
Link Distance (ft)		767	767	767	
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	400			400	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 81: NB US 321 & 15th Avenue NW & 15th Avenue NW Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	TR	L
Maximum Queue (ft)	162	121	176	172	182	85
Average Queue (ft)	84	26	93	98	141	33
95th Queue (ft)	143	85	164	161	195	73
Link Distance (ft)	410	410	100	100	100	133
Upstream Blk Time (%)			4	3	11	
Queuing Penalty (veh)			36	34	114	
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 82: SB US 321 & 15th Avenue NW U-Turn

Movement	WB
Directions Served	L
Maximum Queue (ft)	60
Average Queue (ft)	21
95th Queue (ft)	49
Link Distance (ft)	31
Upstream Blk Time (%)	11
Queuing Penalty (veh)	4
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 83: SB US 321 & 15th Avenue NW Left-Over & 20th Avenue NW/15th Avenue NW

Movement	EB	NW
Directions Served	>	L
Maximum Queue (ft)	79	52
Average Queue (ft)	31	13
95th Queue (ft)	65	41
Link Distance (ft)	502	114
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 84: NB US 321 & 15th Avenue NW U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	63
Average Queue (ft)	21
95th Queue (ft)	52
Link Distance (ft)	-1
Upstream Blk Time (%)	20
Queuing Penalty (veh)	6
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 85: NB US 321 & 15th Avenue NW U-Turn

Movement	NB
Directions Served	L
Maximum Queue (ft)	3
Average Queue (ft)	0
95th Queue (ft)	3
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	200
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 86: SB US 321 & 15th Avenue NW Left-Over

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 87: SB US 321 & 15th Avenue NW U-Turn

Movement	SE
Directions Served	L
Maximum Queue (ft)	51
Average Queue (ft)	6
95th Queue (ft)	44
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	250
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 88: NB US 321 & 15th Avenue NW Left-Over

Movement	NW	NW	NW
Directions Served	T	T	T
Maximum Queue (ft)	23	16	62
Average Queue (ft)	1	1	8
95th Queue (ft)	11	9	35
Link Distance (ft)	664	664	664
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 91: US 321 & Grace Chapel Road

Movement	WB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	R	T	T	T	T	T	T
Maximum Queue (ft)	139	171	45	22	204	210	188	96	126	121
Average Queue (ft)	36	92	11	1	112	128	89	32	58	73
95th Queue (ft)	104	154	35	11	185	194	158	78	109	112
Link Distance (ft)		910	910		580	580	580	73	73	73
Upstream Blk Time (%)								1	2	5
Queuing Penalty (veh)								3	13	30
Storage Bay Dist (ft)	500			500						
Storage Blk Time (%)										
Queuing Penalty (veh)										

Intersection: 92: Grace Chapel Road U-Turn & SB US 321

Movement	WB
Directions Served	L
Maximum Queue (ft)	21
Average Queue (ft)	3
95th Queue (ft)	14
Link Distance (ft)	13
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 93: NB US 321 & Grace Chapel Road U-Turn

Movement	EB
Directions Served	L
Maximum Queue (ft)	80
Average Queue (ft)	23
95th Queue (ft)	59
Link Distance (ft)	82
Upstream Blk Time (%)	1
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 94: NB US 321 & Grace Chapel Road U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 95: SB US 321 & Grace Chapel Road U-Turn

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 96: SB US 321/US 321 & NB US 321

Movement	NW
Directions Served	R
Maximum Queue (ft)	9
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	534
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 97: US 321 & NB US 321

Movement	SE	SE	SE
Directions Served	R	R	R
Maximum Queue (ft)	2	17	12
Average Queue (ft)	0	1	1
95th Queue (ft)	2	7	9
Link Distance (ft)	375	375	375
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 101: NB US 321 & Alex Lee Blvd & Alex Lee Blvd Left-Over

Movement	WB	WB	NB	NB	NB	SE
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	146	103	208	195	71	79
Average Queue (ft)	57	22	127	147	13	28
95th Queue (ft)	116	70	217	226	46	70
Link Distance (ft)	621	621	95	95	95	30
Upstream Blk Time (%)			9	12	0	25
Queuing Penalty (veh)			74	100	0	9
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 102: SB US 321 & Alex Lee Blvd U-Turn

Movement	NW
Directions Served	L
Maximum Queue (ft)	129
Average Queue (ft)	58
95th Queue (ft)	106
Link Distance (ft)	47
Upstream Blk Time (%)	14
Queuing Penalty (veh)	12
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 103: SB US 321 & Alex Lee Blvd Left-Over & Alex Lee Blvd

Movement	EB	NW
Directions Served	>	L
Maximum Queue (ft)	60	22
Average Queue (ft)	16	2
95th Queue (ft)	45	15
Link Distance (ft)	164	102
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 104: NB US 321 & Alex Lee Blvd U-Turn

Movement	EB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	37	40	38	40
Average Queue (ft)	8	1	1	1
95th Queue (ft)	28	39	37	40
Link Distance (ft)	4	2164	2164	2164
Upstream Blk Time (%)	14			
Queuing Penalty (veh)	2			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 105: NB US 321 & Alex Lee Blvd U-Turn

Movement	NB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	26	60	58
Average Queue (ft)	1	4	4
95th Queue (ft)	18	80	82
Link Distance (ft)		571	571
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		4	4
Storage Bay Dist (ft)	300		
Storage Blk Time (%)		0	
Queuing Penalty (veh)		0	

Intersection: 106: SB US 321 & Alex Lee Blvd Left-Over

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 107: SB US 321 & Alex Lee Blvd U-Turn

Movement	SE
Directions Served	L
Maximum Queue (ft)	2
Average Queue (ft)	0
95th Queue (ft)	2
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	250
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 108: NB US 321 & Alex Lee Blvd Left-Over

Movement	NB	NB
Directions Served	T	T
Maximum Queue (ft)	196	204
Average Queue (ft)	20	33
95th Queue (ft)	114	134
Link Distance (ft)	590	590
Upstream Blk Time (%)	0	0
Queuing Penalty (veh)	1	1
Storage Bay Dist (ft)		
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 121: NB US 321 & River Bend Dr & Walmart Shopping Center Driveway Left-Over

Movement	WB	WB	WB	NB	NB	NB	SE	SE
Directions Served	>	>	>	T	T	R	L	L
Maximum Queue (ft)	372	414	323	179	175	178	126	132
Average Queue (ft)	221	279	192	142	145	133	86	88
95th Queue (ft)	361	405	309	181	184	187	131	131
Link Distance (ft)	428	428	428	66	66	66	25	25
Upstream Blk Time (%)	1	3	1	26	27	24	49	57
Queuing Penalty (veh)	0	0	0	217	225	200	99	116
Storage Bay Dist (ft)								
Storage Blk Time (%)								
Queuing Penalty (veh)								

Intersection: 122: SB US 321 & Walmart Shopping Center Driveway U-Turn

Movement	WB	WB	SB	SB	SB	B993	B993
Directions Served	L	L	T	T	T	T	T
Maximum Queue (ft)	119	156	227	259	297	206	204
Average Queue (ft)	57	124	82	121	136	36	36
95th Queue (ft)	105	151	199	256	282	252	248
Link Distance (ft)	30	30		321	321	410	410
Upstream Blk Time (%)	42	67		4	5	4	5
Queuing Penalty (veh)	85	136		50	61	50	53
Storage Bay Dist (ft)			250				
Storage Blk Time (%)			1	5			
Queuing Penalty (veh)			4	26			

Intersection: 123: SB US 321 & US 321 A Left-Over & US 321 A

Movement	EB	EB	SB	SB	SB	NW
Directions Served	>	>	T	T	R	L
Maximum Queue (ft)	219	186	158	152	104	83
Average Queue (ft)	111	42	88	105	31	26
95th Queue (ft)	202	155	170	167	80	65
Link Distance (ft)	394		32	32	32	77
Upstream Blk Time (%)	0		24	26	9	1
Queuing Penalty (veh)	0		133	141	52	0
Storage Bay Dist (ft)		350				
Storage Blk Time (%)	0	0				
Queuing Penalty (veh)	0	0				

Intersection: 124: NB US 321 & US 321 A U-Turn

Movement	EB	NB	NB
Directions Served	L	T	T
Maximum Queue (ft)	118	125	125
Average Queue (ft)	85	19	19
95th Queue (ft)	112	240	241
Link Distance (ft)	30	1173	1173
Upstream Blk Time (%)	94	1	1
Queuing Penalty (veh)	126	6	6
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 125: NB US 321 & Walmart Shopping Center Driveway U-Turn

Movement	NW	NW	NW	NW
Directions Served	L	L	T	T
Maximum Queue (ft)	161	267	168	164
Average Queue (ft)	13	69	33	32
95th Queue (ft)	95	214	255	250
Link Distance (ft)		601	601	601
Upstream Blk Time (%)		0	2	1
Queuing Penalty (veh)		1	14	12
Storage Bay Dist (ft)	500			
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 126: SB US 321 & Walmart Shopping Center Driveway Left-Over

Movement	SB	SB	SB	SB	SB
Directions Served	L	L	T	T	T
Maximum Queue (ft)	93	325	395	398	152
Average Queue (ft)	7	49	89	94	24
95th Queue (ft)	82	307	386	392	158
Link Distance (ft)		591	591	591	
Upstream Blk Time (%)		2	3	3	
Queuing Penalty (veh)		13	19	22	
Storage Bay Dist (ft)	500				250
Storage Blk Time (%)		0		9	
Queuing Penalty (veh)		0		50	

Intersection: 127: SB US 321 & US 321 A U-Turn

Movement	SB	SB	SB
Directions Served	L	T	T
Maximum Queue (ft)	300	912	893
Average Queue (ft)	186	340	320
95th Queue (ft)	382	1005	1000
Link Distance (ft)		984	984
Upstream Blk Time (%)		8	6
Queuing Penalty (veh)		64	50
Storage Bay Dist (ft)	250		
Storage Blk Time (%)	45	0	
Queuing Penalty (veh)	340	0	

Intersection: 128: NB US 321 & US 321 A Left-Over

Movement	NB	NB	NB	NB
Directions Served	L	T	T	T
Maximum Queue (ft)	55	285	293	225
Average Queue (ft)	5	82	82	40
95th Queue (ft)	96	342	335	216
Link Distance (ft)		917	917	917
Upstream Blk Time (%)		1	1	
Queuing Penalty (veh)		8	8	
Storage Bay Dist (ft)	500			
Storage Blk Time (%)		2		
Queuing Penalty (veh)		1		

Intersection: 996: US 321/NB US 321 & SB US 321

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 997: SB US 321 & NB US 321/US 321

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 5341