



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

December 29, 2005

Stormwater Section
Division of Water Quality
127 Cardinal Drive Extension
Wilmington, NC 28405

ATTN: Linda Lewis

Dear Madam:

Subject: Stormwater Permit Application, Second Bridge to Oak Island from SR 1104 (Beach Drive) to NC 211, Brunswick County. TIP No. R-2245, State Project No. 8.2231201. \$420.00 Debit work order 8.2231201, WBS Element 34407.1.1

The North Carolina Department of Transportation (NCDOT) proposes to construct a second bridge to Oak Island on new location from SR 1104 to NC 211 in Brunswick County. The NCDOT will be applying for a Section 404, 401, and CAMA permits in the near future.

Please find enclosed a stormwater permit application form, stormwater management narrative, infiltration supplements, and one copy of the project plans. Please review this project for authorization by your section of NCDWQ. If you have any questions regarding this project or need additional information, please contact Mr. Max Price of NCDOT Hydraulics Unit at (919) 250-4100, or Mr. Brett Feulner of NCDOT Project Development and Environmental Analysis Branch at (919) 715-1488.

Sincerely,

A handwritten signature in black ink that appears to read "E. J. Thorpe".

key Gregory J. Thorpe, Ph.D., Environmental Management Director
Project Development and Environmental Analysis Branch

W/out attachment

Dr. David Chang, Hydraulics
Mr. Dave Timpay, USACE, Washington
Mr. Brian Wrenn, NCDWQ
Mr. Steve Sollod, NCDCM
Mr. Bill Arrington, NCDCM
Mr. Joseph Qubain, PDEA
File

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
PROJECT DEVELOPMENT AND ENVIRONMENTAL ANALYSIS
1598 MAIL SERVICE CENTER
RALEIGH NC 27699-1598

TELEPHONE: 919-715-1500
FAX: 919-715-1501

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION:
2728 CAPITOL BOULEVARD
PARKER LINCOLN BUILDING, SUITE 168
RALEIGH NC 27699

STORMWATER MANAGEMENT PLAN

R-2245, State Project 34407.1

Date: 6 APR 05

Brunswick County

Hydraulics Project Manager: Max S. Price, PE

ROADWAY DESCRIPTION

The project constructs a new, second, route to Oak Island. It begins by widening the existing two lane Middleton Avenue on Oak Island and runs north, crossing the Atlantic Intracoastal Waterway with a new high rise bridge, continuing on a new route on the mainland. It terminates at the intersection of NC-211 and SR 1500, Midway Road. The overall length of the project is 4.52 miles with 0.92 miles on Oak Island and the remaining 3.60 miles on the mainland.

Starting near the ocean, the initial 1,700 feet of the project maintains two lanes, but widens the road from 20 feet to 22 feet and adds 4 foot wide paved shoulder to both sides. The road then begins a taper, widening to provide 5 lanes up to the southern end of the bridge over the Intracoastal Waterway. From this point to the northern terminus, the road provides two lanes in each direction with a raised median separating the traffic.

On the island, the roadway crosses Davis Canal. The project will cross the canal at the same location, but with a new bridge. The roadway will be closed during construction of the new bridge.

The majority of the roadway uses open shoulders. Gutters are used only on approaches to the two bridges.

On the mainland portion of the project, the 17.5 foot wide median is raised, with concrete curb and gutter bordering the mounded, grassy median. The typical section shows the pavement slopes away from the median, draining to the grass shoulders and fill slopes. All fill slopes are 3:1 or flatter except in wetland areas where they are held to 3:1.

A third major stream crossing occurs on NC-211, approximately 540 feet east of the intersection with SR 1500. There, an existing box culvert will be removed and replaced with a larger culvert.

ENVIRONMENTAL DESCRIPTION

Land Use

Beach homes dominate the project area on Oak Island while the corridor on the mainland remains undeveloped. Not far off the mainland corridor, development of the St. James community continues, pushing towards the project route. Along NC-211, at the northern

Topography and Water Resources

The project is located in the Lumber River Basin in the Coastal Plain Physiographic Province. Elevations in the project area range from a low of near seven feet to a high of

approximately 63 feet. The minimum elevation occurs on Oak Island near the ocean and the maximum elevation occurs between the two largest Carolina Bays in the central portion of the project area. North of the apex, the elevation drops to about 45 feet at the intersection of the -L- line with NC-211, creating a slope of about 2 percent. South of the apex, the elevation drops to sea level at the Atlantic Intracoastal Waterway, giving a slope of about 8 percent. Descending the scarp to the Quaternary-aged deposits largely accounts for the steeper slope. Soils in the southern portion of the project area are better drained than in the northern part.

The project -L- line crosses three jurisdictional surface waters: Davis Canal on Oak Island; the Atlantic Intracoastal Waterway which separates Oak Island from the mainland; and, a small, unnamed tributary on the mainland. NC-211 (-Y7-) crosses "River Swamp" with an existing box culvert. Both Davis Canal and the Intracoastal Waterway are SA waters. The unnamed tributary, an intermittent channel through a wetland, is not classified, but it drains to the Intracoastal waterway. This channel will be carried under the -L- line by two 60" diameter concrete pipes, both set one foot below natural ground level. Approximately 247 feet of this stream will be permanently impacted due to the crossing and about 0.009 acres of permanent surface water impacts will also result from the crossing. We estimate temporary surface water impacts at this site will measure 0.002 acres.

River Swamp has been classified as C and Sw water. Currently, water runs through a culvert of two different sizes; it appears to have been built with an initial size of 6' wide by 4' high, then extended on both ends with a size of 6' wide by 5' high. The extensions, each measuring about 7 feet in length, maintain the same top slab elevation, but use bottom slabs that drop one foot below the central, smaller portion. The project will replace this multi-sized culvert with one 10' wide by 8' high box culvert. The larger size will convey stormwater runoff expected from the developing area. Estimated impacts include: 0.010 acres surface water (permanent impact); 0.010 acres surface water (temporary impact); 40 feet of channel impact (permanent); and, 58 feet of channel impact (temporary.)

31 wetland sites will be crossed by the project with fill sections, impacting the wetlands with permanent roadway fill and mechanized clearing. One small wetland area, located in the southwest quadrant of the -L- line intersection with NC-211 will require a roadway ditch. There is an existing ditch, presently within the wetland, running along the south side of NC-211 and the project will require this ditch to be pushed out from 10 to 15 feet further into the wetland.

Total impacts to wetlands caused by permanent fill are estimated at 17.808 acres.

BEST MANAGEMENT PRACTICES AND MAJOR STRUCTURES

Best Management Practices (BMPs) and measures used on the project are an attempt to reduce the potential for surface water runoff and sedimentation and maximize infiltration. The primary BMP is the use of grassed shoulders, as opposed to a curb and gutter roadway drainage system. Rip rap lined ditches are used only near the stream under NC-211. There, the stream banks are lined with rip rap where the two roadside ditches flow down

the banks and into the channel. On the mainland, the outside shoulders and fill slopes are grass, requiring no stormwater collection system. Along the median in the curves, inlets intercept storm water and pipe it to the toe of slope. There, the stormwater will infiltrate into the existing soils. Rip rap pads are used at each outlet to aid in energy attenuation and flow dispersal. Most inlets are single inlets, collecting small areas of roadway runoff, and discharging the small flow to the adjacent land. The largest system on the mainland consists of three inlets and one outlet. This design was used to keep the outlet outside of a wetland area.

The one jurisdictional stream crossing on the mainland by the -L- line will be made by using two 60" concrete pipe culverts. These culverts will be placed one foot below natural ground level.

In the wetland areas, 48" concrete pipes are placed across the -L- line to promote equalization of water levels. Typical spacing is 200 feet and, as for the pipe culvert, these pipes will be placed one foot below natural ground level.

The new bridge crossing Davis Canal will carry stormwater in the shoulder area to inlets off of the bridge on the northern end. From there, the water will be conveyed to a "splitter box", dividing the first flush to "infiltration chambers." The first flush, consisting of the runoff from the first 1.5 inch of rainfall, will enter the buried chambers and infiltrate into the sandy soil. The bypass flow will be diverted away from the infiltration chambers and will flow through a grass swale to the Davis Canal. The swale length is greater than 170'.

The new bridge crossing the Intracoastal Waterway will carry stormwater in the shoulder area, just like the Davis Canal bridge. And as with the Davis Canal bridge, no deck drains will be allowed. Off the southern end of the bridge, storm water will be collected in a series of inlets and piped to a splitter box, similar to the one for the Davis Canal. Here, the runoff from the first 1.5 inch of rainfall will be directed to an infiltration basin. Bypass flows will be directed to an infiltration channel, located along both sides of the fill slopes for the southern approach to the bridge. Based on the topography and sandy soils, bridge deck storm water runoff will not enter SA waters except during large rainfall events, probably exceeded the 50 year event.

Stormwater from the northern end of the bridge collects in inlets off of the bridge deck and is piped to an outlet on the west side of the fill slope. At this location, the roadway is built atop of a sandy spoil pile created by construction and dredging of the Intracoastal Waterway. A soil boring and resultant testing show a high rate of infiltration will occur. Therefore, no additional devices, such as the "storm chambers" used on the Davis Canal or the infiltration basin used for the southern half of the Intracoastal Waterway bridge, are necessary. During large storm events, if the runoff does not infiltrate, it will sheet flow towards a non-jurisdictional pond within the spoil pile area, located approximately 350' west from the pipe outlet.

OFFICE USE ONLY		
Date Received	Fee Paid	Permit Number

State of North Carolina
Department of Environment and Natural Resources
Division of Water Quality

STORMWATER MANAGEMENT PERMIT APPLICATION FORM

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
LINEAR ROADWAY PROJECT

This form may be photocopied for use as an original.

DWQ Stormwater Management Plan Review:

A complete stormwater management plan submittal includes this application form, a supplement form for each BMP proposed (see Section V), design calculations, and plans and specifications showing all road and BMP details.

I. PROJECT INFORMATION

NCDOT Project Number: Q-2245 County: BRUNSWICK
 Project Name: NEW ROUTE FROM SR-1104 (OCEAN BEACH DRIVE) TO NC-211
 Project Location: FROM OAK ISLAND TO MIDWAY SECOND BRIDGE TO OAK ISLAND
 Contact Person: RAY LOVINGGOOD Phone: 1.919.250.4100 Fax: 1.919.250.4108
 Receiving Stream Name: INTRACOASTAL River Basin: LUMBER Class: SA
 Proposed linear feet of project: WATERWAY & DAVIS CANAL 23,865

Proposed Structural BMP and Road Station (*attach a list of station and BMP type if more room is needed*):
-L STA 25+00 RT, STORM CHAMBER ; -L STA 48+00 LT RT INFILTRATION
BASIN

Type of proposed project: (*check all that apply*):

New Widening 2 lane* 4 lane* Curb and Gutter Bridge Replacement

Other (*Describe*)

**2 lane and 4 lane imply that roadside ditches are used unless Curb and Gutter is also checked.*

II. REQUIRED ITEMS CHECKLIST

Initial in the space provided below to indicate the following design requirements have been met and supporting documentation is attached. Supporting documentation shall, at a minimum, consist of a brief narrative description including (1) the scope of the project, (2) how the items below are met, (3) how the proposed best management practices minimize water quality impacts, and (4) any significant constraints and/or justification for not meeting a, b, c and d to the maximum extent practicable.

Designer's Initials

RDL a. The amount of impervious surface has been minimized as much as possible.

RDL b. The runoff from the impervious areas has been diverted away from surface waters as much as possible.

RDL c. Best Management Practices are employed which minimize water quality impacts.

RDL d. Vegetated roadside ditches are 3:1 slope or flatter.

III. OPERATION AND MAINTENANCE AGREEMENT

I acknowledge and agree by my initials below that the North Carolina Department of Transportation is responsible for the implementation of the four maintenance items listed. I agree to notify DWQ of any operational problems with the BMP's that would impact water quality or prior to making any changes to the system or responsible party.

Maintenance Engineer's Initials

DT a. BMP's shall be inspected and maintained in good working order.

DT b. Eroded areas shall be repaired and reseeded as needed.

DT c. Stormwater collection systems, including piping, inlets, and outlets, shall be maintained to insure proper functioning.

Maintenance Engineer's Name: DAVID L. THOMAS, P.E.
Title: DIVISION MAINTENANCE ENGINEER

IV. APPLICATION CERTIFICATION

I, (print or type name) _____ of _____ Branch, certify that the information included on this permit application form is, to the best of my knowledge, correct and that the project will be constructed in conformance with the approved plans and that the proposed project complies with the requirements of 15A NCAC 2H .1000.

Title: _____

Address: _____

Signature: _____ Date: _____

V. SUPPLEMENT FORMS

The applicable state stormwater management permit supplement form(s) listed below must be submitted for each BMP specified for this project. Contact the Stormwater and General Permits Unit at (919) 733-5083 for the status and availability of these forms.

Form SWU-102	Wet Detention Basin Supplement
Form SWU-103	Infiltration Basin Supplement
Form SWU-104	Low Density Supplement
Form SWU-105	Curb Outlet System Supplement
Form SWU-106	Off-Site System Supplement
Form SWU-107	Underground Infiltration Trench Supplement
Form SWU-108	Neuse River Basin Supplement
Form SWU-109	Innovative Best Management Practice Supplement
Form SWU-110	Extended Dry Detention Basin Supplement

State of North Carolina
Department of Environment and Natural Resources
Division of Water Quality

STORMWATER MANAGEMENT PERMIT APPLICATION FORM

INFILTRATION BASIN SUPPLEMENT

This form may be photocopied for use as an original

DWQ Stormwater Management Plan Review:

A complete stormwater management plan submittal includes a stormwater management permit application, an infiltration basin supplement for each system, design calculations, soils report and plans and specifications showing all stormwater conveyances and system details.

I. PROJECT INFORMATION

SECOND BRIDGE TO OAK ISLAND

Project Name: NEW ROUTE FROM SR1104 (OCEAN BEACH DRIVE) TO NC 211

Contact Person: RAY LOVINGGOOD Phone Number: (919) 250.4100

This worksheet applies to: Basin No. INFILTRATION BASIN in Drainage Area BRIDGE
(as identified on plans) INTRACOASTAL WATERWAY
(from Form SWU-101)

II. DESIGN INFORMATION - Attach supporting calculations/documentation. The soils report must be based upon an actual field investigation and soil borings. County soil maps are not an acceptable source of soils information. All elevations shall be in feet mean sea level (fmsl).

Soils Report Summary

Soil Type WANDO VARIANT

Infiltration Rate 64.4 (in/hr) or cf/hr/sf (circle appropriate units)

SHWT Elevation < 20.5 fmsl (Seasonal High Water Table elevation)

Basin Design Parameters

Design Storm 1.5 inch (1.5 inch event for SA waters, 1 inch event for others)

Design Volume 7,600 c.f.

Drawdown Time 1 days

Basin Dimensions

Basin Size 100 ft. x 34 ft. = 3400 sq. ft. (bottom dimensions)

Basin Volume Provided 22,200 c.f.

Basin Elevations

Bottom Elevation 23.0 fmsl

Storage Elevation 23.6 fmsl

Top Elevation 26.0 fmsl

III. REQUIRED ITEMS CHECKLIST

The following checklist outlines design requirements per the Stormwater Best Management Practices Manual (N.C. Department of Environment, Health and Natural Resources, February 1999) and Administrative Code Section: 15 A NCAC 2H .1008.

Initial in the space provided to indicate that the following design requirements have been met and supporting documentation is attached. If the applicant has designated an agent in the Stormwater Management Permit Application Form, the agent may initial below. **Attach justification if a requirement has not been met.**

Applicants Initials

- RDL a. System is located 50 feet from class SA waters and 30 feet from other surface waters.
- RDL b. System is located at least 100 feet from water supply wells.
- RDL c. Bottom of system is at least 2 feet above the seasonal high water table.
- RDL d. Bottom of the system is 3 feet above any bedrock or impervious soil horizon.
- RDL e. System is not sited on or in fill material or DWQ approval has been obtained.
- RDL f. System is located in a recorded drainage easement for the purposes of operation and maintenance and has recorded access easements to the nearest public right-of-way.
- RDL g. Drainage area for the device is less than 5 acres.
- RDL h. Soils have a minimum hydraulic conductivity of 0.52 inches per hour and soils report is attached.
- RDL i. System captures and infiltrates the runoff from the first 1.0 inch of rainfall (1.5 inch event for areas draining to SA waters). Design volume and infiltration calculations attached.
- RDL j. System is sized to take into account the runoff at the ultimate built-out potential from all surfaces draining to the system, including any off-site drainage. Calculations attached.
- RDL k. All side slopes stabilized with vegetated cover are no steeper than 3:1 (H:V).
 - l. A pretreatment device such as a catch basin, grease trap, filter strip, grassed swale or sediment trap is provided.
- RDL m. Bottom of the device is covered with a layer of clean sand to an average depth of 4 inches or dense vegetative cover is provided.
- RDL n. Vegetated filter is provided for overflow and detail is shown on plans (Required minimum length is 50 feet for SA waters, 30 feet for other waters).
- RDL o. Flow distribution mechanism within the basin is provided.
- RDL p. A benchmark is provided to determine the sediment accumulation in the pretreatment device.
- RDL q. Runoff in excess of the design volume bypasses off-line systems (bypass detail provided).
- RDL r. System is designed to draw down the design storage volume to the proposed bottom elevation under seasonal high water conditions within five days. A soils report and all pertinent draw-down calculations are attached.
- RDL s. Plans ensure that the installed system will meet design specifications (constructed or restored) upon initial operation once the project is complete and the entire drainage area is stabilized.

IV. INFILTRATION BASIN OPERATION AND MAINTENANCE AGREEMENT

1. After every runoff producing rainfall event and at least monthly inspect the infiltration system for erosion, trash accumulation, vegetative cover, and general condition.
2. Repair eroded areas immediately, re-seed as necessary to maintain adequate vegetative cover, mow vegetated cover to maintain a maximum height of six inches, and remove trash as needed.
3. After every runoff producing rainfall event and at least monthly inspect the bypass, inflow and overflow structures for blockage and deterioration. Remove any blockage and repair the structure to approved design specifications.
4. Remove accumulated sediment from the pretreatment system and infiltration basin annually or when depth in the pretreatment unit is reduced to 75% of the original design depth. The system shall be restored to the original design depth without over-excavating. Over-excavating may cause the required water table separation to be reduced and may compromise the ability of the system to perform as designed. Removed sediment shall be disposed of in an appropriate manner and shall not be handled in a manner that will adversely impact water quality (i.e. stockpiling near a stormwater treatment device or stream, etc.).

A benchmark shall be established in the pretreatment unit. The benchmark will document the original design depth so that accurate sediment accumulation readings can be taken. The measuring device used to determine the depth at the benchmark shall be such that it will give an accurate depth reading and not readily penetrate into accumulated sediments.

When the design depth reads 0.3 feet in the pretreatment unit, the sediment shall be removed from both the pretreatment unit and the infiltration basin.

5. If the Division determines that the system is failing, the system will immediately be repaired to original design specifications. If the system cannot be repaired to perform its design function, other stormwater control devices as allowed by NCAC 2H .1000 must be designed, approved and constructed.

DO THE NC DOT

I acknowledge and agree by my signature below that I am responsible for the performance of the five maintenance procedures listed above. I agree to notify DWQ of any problems with the system or prior to any changes to the system or responsible party.

Print Name and Title: DAVID L. THOMAS, P.E. DIVISION MAINTENANCE ENGR.

Address: 124 DIVISION DRIVE WILMINGTON NC 28401

Phone: 910 251-5724 Date: 12/1/05

Signature: 

Note: The legally responsible party should not be a homeowners association unless more than 50% of the lots have been sold and a resident of the subdivision has been named the president.

I, _____, a Notary Public for the State of _____, County of _____, do hereby certify that _____ personally appeared before me this _____ day of _____, _____, and acknowledge the due execution of the forgoing infiltration basin maintenance requirements. Witness my hand and official seal,

SEAL

My commission expires _____

S&ME
"IN-SITU" CONSTANT HEAD PERMEAMETER

Date: 9/16/2004
 Location: Ksat 6
 Horizon: C
 Client: NCDOT
 Project Name: Oak Island NCDOT Project
 Project #: 1051-04-188

	Hole Depth:	1.83	Feet
	Hole Radius (r):	0.08	Feet
	Bubble Tube to Surface:	0.25	Feet
	Reference Tube to Hole Bottom (D):	2.08	Feet
	Water Depth in Hole (H):	0.16	Feet
	CHT Tube(s) Setting (h ₁):	1.92	Feet

	Chamber Used:	0.11	▼	ft ²
	Initial Water in Hole:	0.16	Feet	
	Final Water in Hole:	0.16	Feet	

$$K_{sat} = CQ/(2\pi H^2)$$

$$C = \sinh^{-1} (H/r) - [(r/H)^2 + 1]^{1/2} + r/H$$

\sinh^{-1} = inverse hyperbolic sin of a number

H = Height of water in hole (cm)

r = radius of hole (cm)

Q = Constant Flow Rate (Gal/day)

= Cross Sectional Area of Reservoir x Length of Drop in Water Column over Time

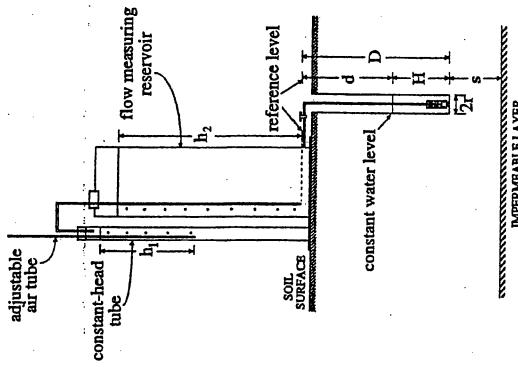
$r = \frac{0.08}{H = \frac{0.16}{ft}}$	Time	Drop in Water Column (ft)	Time (min) =
	0-5 min.	0.492	5
	5 - 10 min.	0.705	
	12 - 17 min.	0.837	
	17 - 22 min.	0.754	
	23 - 28 min.	0.902	
	28 - 33 min.	0.722	
	34 - 39 min.	0.840	
	39 - 43 min.	0.748	
	Avg.	0.770	

$$\frac{\text{Cross Sectional Area} = 0.11 \text{ ft}^2}{\text{Length of Drop in Water Column} = 221.76 \text{ ft/day}}$$

$$K_{sat} = 962.85 \text{ Gallons/Day/ft}^2$$

$$\text{Inches/Hour} = 64.36$$

Note: Ksat calculations are based on average drop in Water Column (ft) after equilibrium is reached.



S&ME, INC.
SOIL PROFILE DESCRIPTIONS

ICWW SITE

Client:	North Carolina DOT
Project Name:	Oak Island
County:	Brunswick
Location:	Oak Island, NC
Soil Series:	Wando Variant
Apparent Water Table:	>72 Inches
Vegetation:	Pines & Oaks
Boring Terminated at	72 Inches

Date: 9-16-04
Project No. 1051-04-188
State NC
Site/Field No. Ksat-6
Water Table: >72 Inches
Slope: 0-1 %

COMMENTS:

LEGEND: Attached at the end of logs

DESCRIBED BY: Rob Willcox DATE 9-16-04

State of North Carolina
Department of Environment and Natural Resources
Division of Water Quality

STORMWATER MANAGEMENT PERMIT APPLICATION FORM

UNDERGROUND INFILTRATION TRENCH SUPPLEMENT

This form may be photocopied for use as an original

DWQ Stormwater Management Plan Review:

A complete stormwater management plan submittal includes a stormwater management permit application, an underground infiltration trench supplement for each system, design calculations, and plans and specifications showing all stormwater conveyances and system details.

I. PROJECT INFORMATION

SECOND BRIDGE TO OAK ISLAND

Project Name: NEW ROUTE FROM SR-1104 (OCEAN BEACH DRIVE) TO NC-211

Contact Person: RAY LOVINGGOOD Phone Number: (919) 250. 4100

For projects with multiple infiltration systems, a supplement form must be completed for each system. This worksheet applies to: Trench STORM CHAMBER in Drainage Area DAVIS CANAL BRIDGE
(from plans) (from Form SWU-101)

II. DESIGN INFORMATION (attach supporting calculations/documentation):

Soils Report Summary (based upon an actual field investigation and soil borings)

Soil Type FILL

Infiltration Rate 23.3 (in/hr) or cf/hr/sf (circle appropriate units)

SHWT Elevation < 11 fmsl (Seasonal High Water Table elevation)

Trench Design Parameters

Design Volume 1,765 c.f.

Design Storm 1.5 inch event (1.5 inch event for SA waters, 1 inch event for others)

Drawdown Time _____ days

Perforated Pipe Size 51" w x 30" h inch diameter storm chamber

Perforated Pipe Length 170 ft.

No. Observation Wells _____ (may be required on a case-by-case basis)

Stone Type (if used) WASHED # 57

Stone Void Ratio 40%

Trench Elevations (in feet mean sea level)

Bottom Elevation 11.75' BOTTOM fmsl OF STORM CHAMBER

Storage/Overflow Elevation 14.25' fmsl

Trench Dimensions

Height	<u>2.5</u> ft.	Length	<u>170</u> ft.
Width	<u>4.25</u> ft.	Volume Provided	<u>1,800</u> cu.ft.

III. REQUIRED ITEMS CHECKLIST

The following checklist outlines design requirements per the Stormwater Best Management Practices manual (N.C. Department of Environment, Health and Natural Resources, November 1995) and Administrative Code Section: 15 A NCAC 2H .1008.

Initial in the space provided to indicate that the following design requirements have been met and supporting documentation is attached. If the applicant has designated an agent in the Stormwater Management Permit Application Form, the agent may initial below. Attach justification if a requirement has not been met.

Applicants Initials

KDL a. System is located 50 feet from class SA waters and 30 feet from other surface waters.

KDL b. System is located at least 100 feet from water supply wells.

KDL c. Bottom of system is at least 2 feet above the seasonal high water table.

KDL d. Bottom of the system is 3 feet above any bedrock or impervious soil horizon.

KDL e. Off-line system, runoff in excess of the design volume bypasses the system (bypass detail provided).

KDL f. System is designed to draw down the design storage volume to the proposed bottom elevation under seasonal high water conditions within five days based upon infiltration through the bottom only (a hydrogeologic evaluation may be required).

KDL g. Soils have a minimum hydraulic conductivity of 0.52 inches per hour.

h. System is not sited on or in fill material or DWQ approval has been obtained.

KDL i. Plans ensure that the installed system will meet design specifications (constructed or restored) upon initial operation once the project is complete and the entire drainage area is stabilized.

KDL j. System is sized to take into account the runoff at the ultimate built-out potential from all surfaces draining to the system, including any off-site drainage.

KDL k. System is located in a recorded drainage easement for the purposes of operation and maintenance and has recorded access easements to the nearest public right-of-way.

KDL l. System captures and infiltrates the runoff from the first 1.0 inch of rainfall (1.5 inch event for areas draining to SA waters).

KDL m. Drainage area for the device is less than 5 acres.

_____ n. A pretreatment device (filter strip, grassed swale, sediment trap, etc.) is provided.

_____ o. Trench bottom is covered with a layer of clean sand to an average depth of 4 inches.

KDL p. Sides of the infiltration trench are lined with geotextile fabric.

_____ q. Rock used is free of fines (washed stone) and has a large void ratio.

_____ r. Side to bottom area ratio is less than 4:1.

- s. Observation well(s) are provided (case-by-case basis).
- k'OL* t. Vegetated filter is provided for overflow (50 feet for SA waters, 30 feet for other waters) and detail is shown on plans.
- k'DL* u. A benchmark for checking sediment accumulation is provided.

IV. UNDERGROUND INFILTRATION TRENCH OPERATION AND MAINTENANCE AGREEMENT

1. After every runoff producing rainfall event and at least monthly inspect the bypass/overflow structure for blockage and deterioration and the infiltration system for erosion, trash accumulation, grass cover, and general condition.
2. Repair eroded areas immediately, re-seed as necessary to maintain adequate vegetative cover, mow vegetative cover to maintain a maximum height of six inches, and remove trash and blockages as needed to maintain system performance.
3. Remove accumulated sediment annually or when depth is reduced to 75% of the original design depth. Restore depth to original design depth without over-excavating. Over-excavating may cause the required water table separation to be reduced and may compromise the ability of the system to perform as designed.
4. The water level in any monitoring wells will be recorded after a 1 inch rainfall event and at least once a month. Chronic high water table elevations (within 1 foot of the bottom of the system for a period of three months) shall be reported to DWQ immediately.
5. If DWQ determines that the system is failing, the system will immediately be repaired or replaced to original design specifications. If the system cannot be repaired to perform its design function, other stormwater control devices as allowed by NCAC 2H .1000 must be designed, approved and constructed.
6. Remove accumulated sediment from the infiltration system annually or when depth in the unit is reduced to 75% of the original design depth. The system shall be restored to the original design depth without over-excavating. Over-excavating may cause the required water table separation to be reduced and may compromise the ability of the system to perform as designed. Removed sediment shall be disposed of in an appropriate manner and shall not be handled in a manner that will adversely impact water quality (i.e. stockpiling near a stormwater treatment device or stream, etc.).

A benchmark shall be established in the infiltration system. The benchmark will document the original design depth so that accurate sediment accumulation readings can be taken. The measuring device used to determine the depth at the benchmark shall be such that it will give an accurate depth reading and not readily penetrate into accumulated sediments.

When the depth at the benchmark reads 0.5', the accumulated sediment shall be removed from the system.

IN THE NAME

I acknowledge and agree by my signature below that I am responsible for maintaining the stormwater collection system in accordance with the six maintenance procedures listed above. I agree to notify DWQ of any problems with the system or prior to any changes to the system or responsible party.

Print Name and Title: DAVID L. THOMAS, P.E. DIVISION MAINTENANCE COORD

Address: 124 DIVISION MAINT. DRIVE

Phone: (910) 251-5724

Date: 12/1/05

Signature: David Thomas

Note: The legally responsible party should not be a homeowners association unless more than 50% of the lots have been sold and a resident of the subdivision has been named the president.

I, _____, a Notary Public for the State of _____, County of _____, do hereby certify that _____ personally appeared before me this _____ day of _____, _____, and acknowledge the due execution of the forgoing infiltration basin maintenance requirements. Witness my hand and official seal,

SEAL

My commission expires _____

S&ME
"IN-SITU" CONSTANT HEAD PERMEAMETER

Date: 9/17/2004
Location: Ksat 3
Horizon: C
Client: NCDOT
Project Name: Oak Island NCDOT Project
Project #: 1051-04-188

	Hole Depth:	2.08	Feet
	Hole Radius (r):	0.08	Feet
	Bubble Tube to Surface:	0.25	Feet
	Reference Tube to Hole Bottom (D):	2.33	Feet
	Water Depth in Hole (H):	0.16	Feet
	CHT Tube(s) Setting (h ₁):	2.17	Feet

	Chamber Used:	0.11	►	R ²
	Initial Water in Hole:	0.16	Feet	
	Final Water in Hole:	0.16	Feet	

$$K_{sat} = CQ/(2\pi H^2)$$

$$C = \sinh^{-1}(H/r) - [(r/H)^2 + 1]^{1/2} + r/H$$

\sinh^{-1} = inverse hyperbolic sin of a number

H = Height of water in hole (cm)

r = radius of hole (cm)

Q = Constant Flow Rate (Gal/day)

= Cross Sectional Area of Reservoir x Length of Drop in Water Column over Time

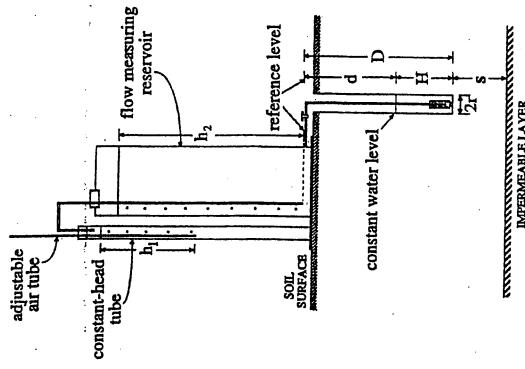
$$\begin{aligned} r &= \frac{0.08}{H} \text{ ft} & \text{Time} &= \text{Drop in Water Column (ft)} \\ H &= \frac{0.16}{\text{ft}} & 0.5 \text{ min.} &= 0.269 \\ & & 5 - 10 \text{ min.} &= 0.259 \\ C &= 0.83 & 10 - 15 \text{ min.} &= 0.311 \\ Q &= 67.77 \text{ Gallons/Day} & 15 - 20 \text{ min.} &= 0.262 \\ & & 20 - 25 \text{ min.} &= 0.262 \\ & & \text{Avg.} &= 0.278 \end{aligned}$$

$$\begin{aligned} \text{Cross Sectional Area} &= 0.11 \text{ ft}^2 \\ \text{Length of Drop in Water Column} &= \frac{80.16}{80.16} \text{ ft/day} \end{aligned}$$

$$K_{sat} = 348.04 \text{ Gallons/Day/ft}^2$$

$$\text{Inches/Hour} = 23.26$$

Note: K_{sat} calculations are based on average drop in Water Column (ft) after equilibrium is reached.



DAVIS CANAL SITE

S&ME, INC.
SOIL PROFILE DESCRIPTIONS

DAVIS CANAL SITE

Client: North Carolina DOT
 Project Name: Oak Island
 County: Brunswick
 Location: Oak Island, NC
 Soil Series: Fill
 Apparent Water Table: >72 Inches
 Vegetation: R/W
 Boring Terminated at 72 Inches

Date: 9-17-04
 Project No. 1051-04-188
 State NC
 Site/Field No. Ksat-3

Seasonal High Water Table: >72 Inches
 Slope: 1-2 %

Horizon	Depth	Matrix	Mottles	Texture	Structure	Consistence	Boundary
Fill	0-10	10YR 6/3	-	s	sg	ml	cw
Fill	10-21	10YR 5/6	-	s	sg	ml	gw
Fill	21-27	10YR 3/3	-	s	sg	ml	gw
C1	27-35	2.5Y 6/6	-	s	sg	ml	gw
C2	35-43	2.5Y 6/4	-	s	sg	ml	gw
C3	43-72+	2.5Y 6/6	-	s	sg	ml	-

COMMENTS:

LEGEND: Attached at the end of logs

DESCRIBED BY: Rob Willcox DATE 9-17-04

JUSTIFICATION FOR CERTAIN REQUIREMENTS ON REQUIRED ITEM CHECKLIST NOT BEING MET

R-2245, State Project 34407.1

Date: 14 NOV 05

Brunswick County

Hydraulics Project Manager: Max S. Price, PE

DAVIS CANAL BRIDGE

Item h. System is not sited on or in fill material or DWQ approval has been obtained.

The location for the infiltration chambers was discussed with regional DWQ staff.

Item n. A pretreatment device (filter strip, grassed swale, sediment trap, etc.) is provided.

The proposed drainage area is comprised entirely of bridge deck and paved shoulder. Sedimentation should not be a problem, and large debris will be prevented from entering the system by the bridge deck drain size and grated inlets.

Item o. Trench bottom is covered with a layer of clean sand to an average depth of 4 inches.

The storm chambers will be bedded on a minimum of six inches of washed no. 57 stone.

Item r. Side to bottom ratio is less than 4:1.

The storm chambers are buried in a trench that will have vertical walls and backfilled with native, sandy material. There will not be an open trench or swale providing infiltration with this installation.

INTRACOASTAL WATERWAY BRIDGE

Item l. A pretreatment device such as a catch basin, grease trap, filter strip, grassed swale or sediment trap is provided.

The proposed drainage area is comprised entirely of bridge deck and paved shoulder. Sedimentation should not be a problem, and large debris will be prevented from entering the system by the bridge deck drain size and grated inlets.

**Item n. Vegetated filter is provided for overflow and detail is shown on plans
(Required minimum length is 50 feet for SA waters, 30 feet for other waters).**

The proposed design provides for infiltration of the overflow and does not convey the overflow to any receiving channel or body of water. The overflow “swales” are similar to the main infiltration basin, but uses side slopes of 4:1 rather than the 6:1 in the main basin. The steeper sides were required to fit the swales in the available space.

STATE OF NORTH CAROLINA

SUBJECT STORM CHAMBER

R-2245

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

HIGHWAY BUILDING

P. O. BOX 25201

RALEIGH, NORTH CAROLINA 27611

PROJECT

COUNTY

PREPARED BY RDL DATE 22 NOV 05 STATION _____CHECKED BY MSP DATE _____ STR NO _____ SHEET _____ OF _____

DETERMINE DRAW DOWN

$$Q = (0.00023) \times K \times A \times \Delta H / \Delta L$$

$$Q = (0.00023) \times (25\% \times 23.26 \text{ in/hr}) \times (4' \times 170') \times 1$$

$$Q = 0.9095 \text{ cfs}$$

$$\text{DECK RUNOFF} = 1,765 \text{ ft}^3$$

$$\text{TIME FOR DRAWDOWN} = 1,765 \text{ ft}^3 / 0.9095 \text{ ft}^3/\text{sec} = 1,940 \text{ SEC}$$

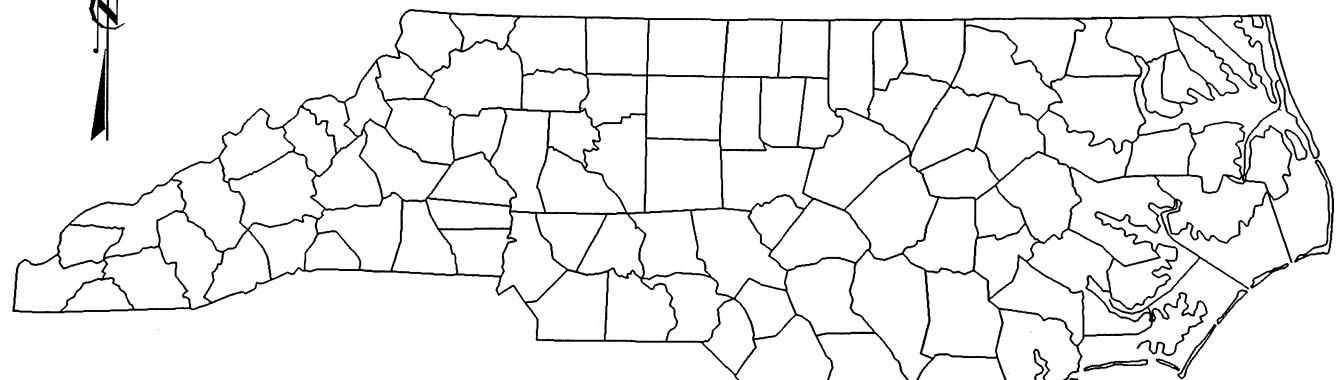
$$= 32 \text{ min}$$

Hydrograph for Pond 2P: Infiltration Basin, South

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Primary (cfs)	Secondary (cfs)
0.00	0.00	0	23.00	0.00	0.00	0.00
1.00	0.00	0	23.00	0.00	0.00	0.00
2.00	0.00	0	23.00	0.00	0.00	0.00
3.00	0.00	2	23.00	0.00	0.00	0.00
4.00	0.01	17	23.00	0.00	0.00	0.00
5.00	0.01	40	23.01	0.01	0.01	0.00
6.00	0.02	70	23.01	0.01	0.01	0.00
7.00	0.03	110	23.02	0.02	0.02	0.00
8.00	0.04	168	23.04	0.02	0.02	0.00
9.00	0.07	256	23.05	0.04	0.04	0.00
10.00	0.10	388	23.08	0.06	0.06	0.00
11.00	0.16	588	23.12	0.09	0.09	0.00
12.00	1.64	1,689	23.35	0.25	0.25	0.00
13.00	0.20	2,898	23.59	0.42	0.42	0.00
14.00	0.13	2,136	23.45	0.32	0.32	0.00
15.00	0.10	1,556	23.33	0.23	0.23	0.00
16.00	0.07	1,136	23.24	0.17	0.17	0.00
17.00	0.05	831	23.17	0.12	0.12	0.00
18.00	0.04	618	23.13	0.09	0.09	0.00
19.00	0.04	469	23.10	0.07	0.07	0.00
20.00	0.03	371	23.08	0.05	0.05	0.00
21.00	0.03	304	23.06	0.05	0.05	0.00
22.00	0.03	258	23.05	0.04	0.04	0.00
23.00	0.02	222	23.05	0.03	0.03	0.00
24.00	0.02	194	23.04	0.03	0.03	0.00
25.00	0.00	117	23.02	0.02	0.02	0.00
26.00	0.00	69	23.01	0.01	0.01	0.00
27.00	0.00	40	23.01	0.01	0.01	0.00
28.00	0.00	24	23.00	0.00	0.00	0.00
29.00	0.00	14	23.00	0.00	0.00	0.00
30.00	0.00	8	23.00	0.00	0.00	0.00
31.00	0.00	5	23.00	0.00	0.00	0.00
32.00	0.00	3	23.00	0.00	0.00	0.00
33.00	0.00	2	23.00	0.00	0.00	0.00
34.00	0.00	1	23.00	0.00	0.00	0.00
35.00	0.00	1	23.00	0.00	0.00	0.00
36.00	0.00	0	23.00	0.00	0.00	0.00
37.00	0.00	0	23.00	0.00	0.00	0.00
38.00	0.00	0	23.00	0.00	0.00	0.00
39.00	0.00	0	23.00	0.00	0.00	0.00
40.00	0.00	0	23.00	0.00	0.00	0.00
41.00	0.00	0	23.00	0.00	0.00	0.00
42.00	0.00	0	23.00	0.00	0.00	0.00
43.00	0.00	0	23.00	0.00	0.00	0.00
44.00	0.00	0	23.00	0.00	0.00	0.00
45.00	0.00	0	23.00	0.00	0.00	0.00
46.00	0.00	0	23.00	0.00	0.00	0.00
47.00	0.00	0	23.00	0.00	0.00	0.00
48.00	0.00	0	23.00	0.00	0.00	0.00

DRAW DOWN TIME

NORTH CAROLINA

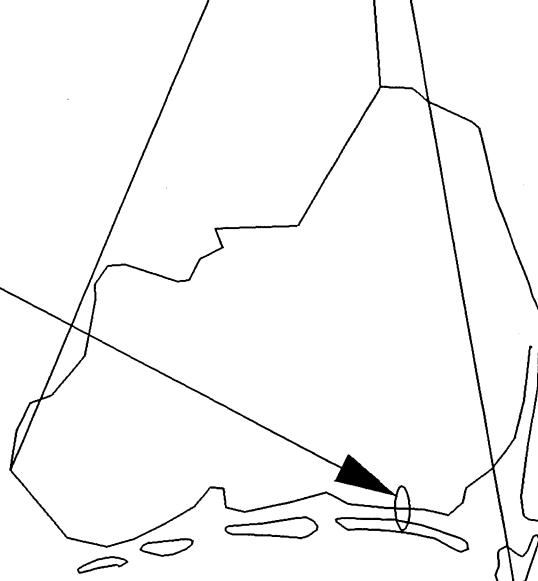


**BRUNSWICK
COUNTY**

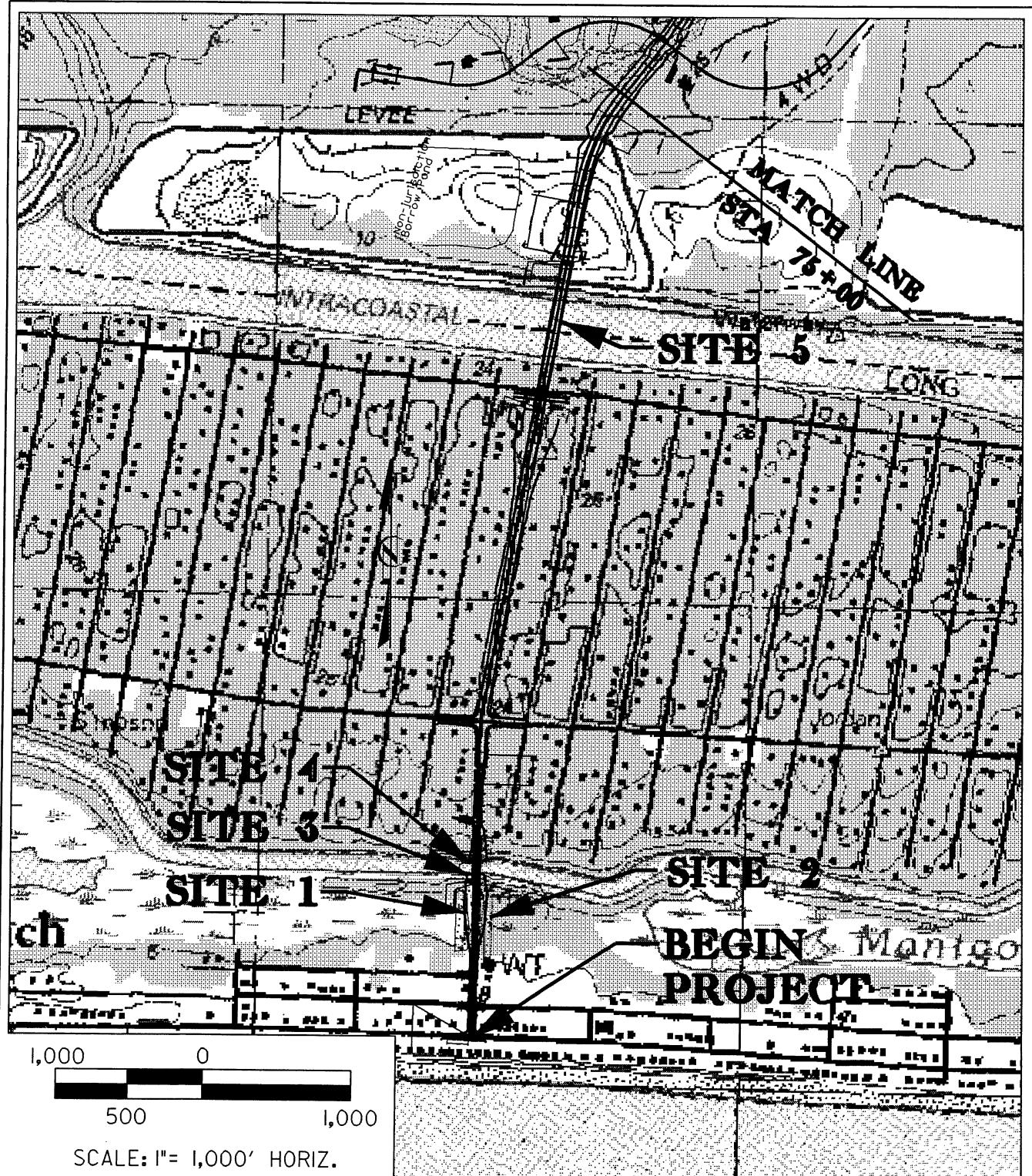
**PROJECT
LOCATION**



**LOCATION
MAP**



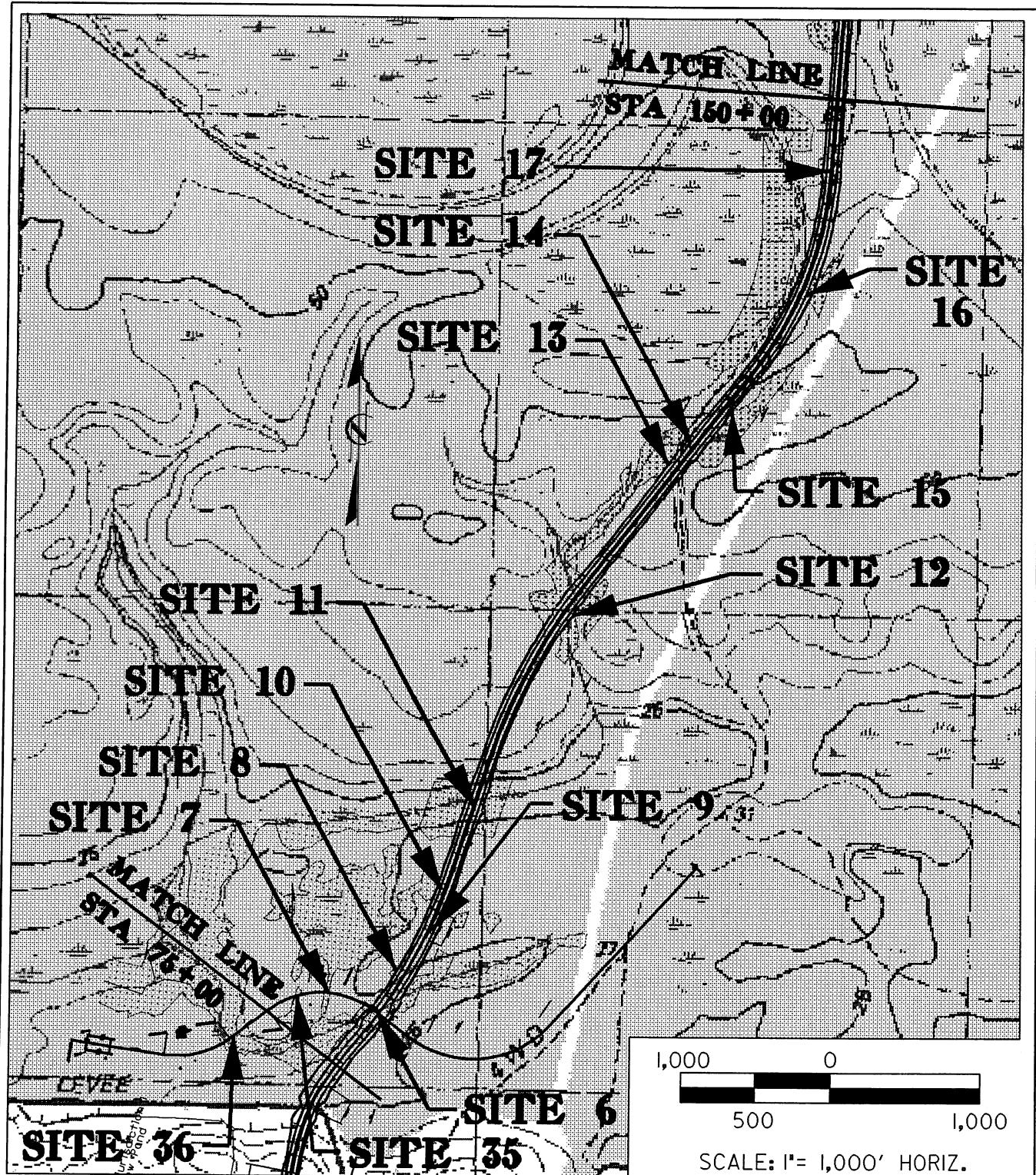
**NCDOT
DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
SECOND BRIDGE TO
OAK ISLAND**



SITE
MAP
1 of 4

NCDOT

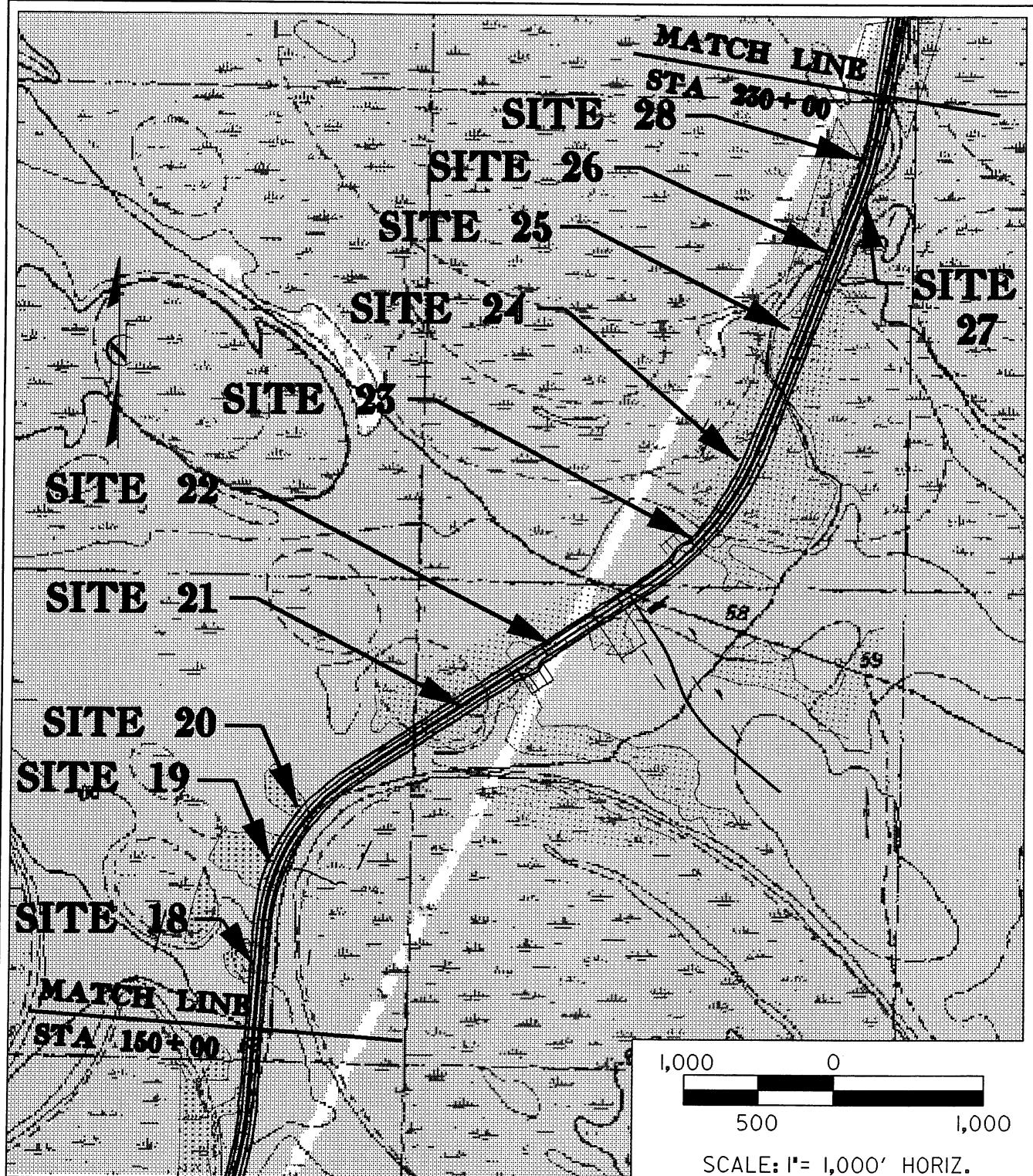
DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
NEW ROUTE FROM SR 1104
(BEACH DR) TO NC 211
(SECOND BRIDGE TO OAK ISLAND)



SITE
MAP
2 of 4

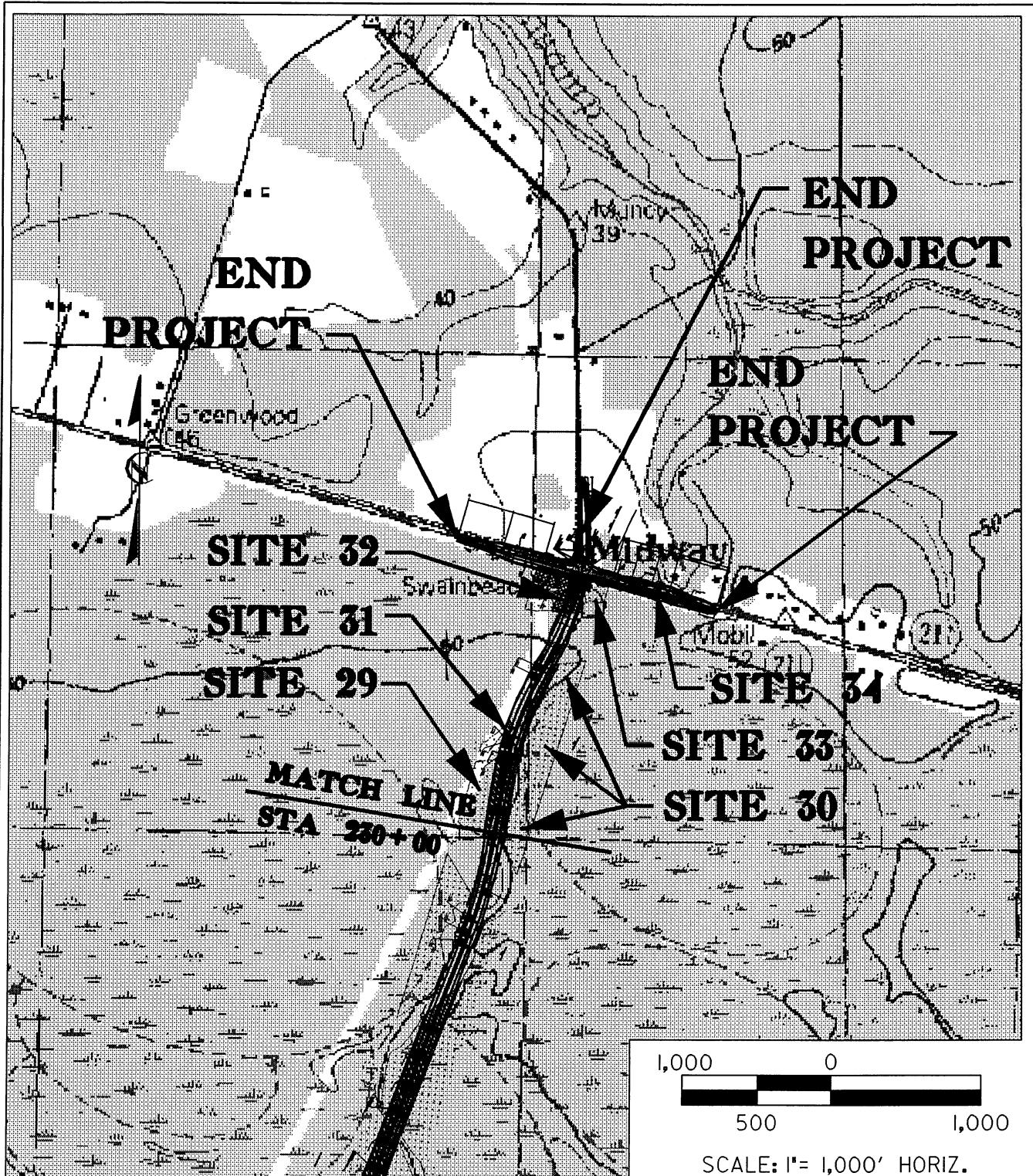
NCDOT

DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
NEW ROUTE FROM SR 1104
(BEACH DR) TO NC 211
(SECOND BRIDGE TO OAK ISLAND)



SITE
 MAP
 3 of 4

NCDOT
 DIVISION OF HIGHWAYS
 BRUNSWICK COUNTY
 PROJECT 34407.1.1 (R-2245)
 NEW ROUTE FROM SR 1104
 (BEACH DR) TO NC 211
 (SECOND BRIDGE TO OAK ISLAND)



SITE
MAP
4 of 4

NCDOT

DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
NEW ROUTE FROM SR 1104
(BEACH DR) TO NC 211
(SECOND BRIDGE TO OAK ISLAND)

PROPERTY OWNERS
NAMES AND ADDRESSES

PARCEL NO.	NAMES	ADDRESSES
5	BOBBY A. LONDON	129 HELEN AVE. BESSEMER CITY, NC 28013
7	OAK ISLAND ELKS LODGE #2769	EAST DOLPHIN DRIVE OAK ISLAND, NC 28465
8	JAMES A. FERRELL	(no address)
9	HOBOKE, INC.	P.O. BOX 433 OAK ISLAND, NC 28465
11	ROBERT ROBBINS	119 SE 1st STREET OAK ISLAND, NC 28465
104	MAS PROPERTIES LLC	131 OCEAN BLVD. WEST HOLDEN BEACH, NC 28462
106	LADANE WILLIAMSON	130 N. FRONT STREET, STE 201 WILMINGTON, NC 28401

NCDOT
DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
SECOND BRIDGE TO
OAK ISLAND

PROPERTY OWNERS

NAMES AND ADDRESSES

PARCEL NO.	NAMES	ADDRESSES
(not numbered)	BRUNSWICK ELEC MEMBERSHIP	P.O. BOX 826 SHALLOTTE, NC 28459
107	CBS LLC	36 RICHMOND PLAZA HWY 74 ROCKINGHAM, NC 28379
109	YOUNGS GAS & GROCERY OF WINNABOW INC.	P.O. BOX 14 WINNABOW, NC 28479

NCDOT
DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT 34407.1.1 (R-2245)
SECOND BRIDGE TO
OAK ISLAND

WETLAND PERMIT IMPACT SUMMARY											
Site No.	Station (From/To)	Structure Size / Type	WETLAND IMPACTS				SURFACE WATER IMPACTS				
			Permanent Fill In Wetlands (ac)	Temp. Fill In Wetlands (ac)	Excavation in Wetlands (ac)	Mechanized Clearing in Wetlands (ac)	Hand Clearing in Wetlands (ac)	Permanent SW impacts (ac)	Temp. SW impacts (ac)	Existing Channel Impacts Permanent (ft)	Existing Channel Impacts Temp. (ft)
7	ACCESS ROAD 2	CLEARING FOR 13+65 to 14+05 +/- FENCE					0.009				
8	82+10 -L- +/- to 83+00 -L- +/- LT	ROADWAY FILL	0.046				0.020	0.014			
9	85+95 -L- +/- to 86+65 -L- +/- RT	ROADWAY FILL	0.099				0.007				
10	88+05 -L- +/- to 89+75 -L- +/- LT	ROADWAY FILL	0.108				0.029	0.035			
11	92+65 -L- +/- to 97+50 -L- +/-	ROADWAY FILL	1.203				0.204	0.151	0.010	0.002	207
12	108+20 -L- +/- to 112+65 -L- +/-	ROADWAY FILL	1.013				0.159	0.055			
13	120+85 -L- +/- to 123+05 -L- +/- LT	ROADWAY FILL	0.029				0.041	0.015			
14	122+85 -L- +/- to 126+55 -L- +/-	ROADWAY FILL	0.669				0.081	0.023			
TOTALS, This Sheet:			3.167				0.541	0.302	0.010	0.002	207
											52

WETLAND PERMIT IMPACT SUMMARY									
Site No.	Station (From/To)	Structure Size / Type	WETLAND IMPACTS			SURFACE WATER IMPACTS			
			Permanent Fill in Wetlands (ac)	Temp. Fill in Wetlands (ac)	Excavation in Wetlands (ac)	Mechanized Clearing in Wetlands (ac)	Hand Clearing in Wetlands (ac)	Permanent SW impacts (ac)	Temp SW impacts (ac)
15	127+23 -L- +/- to 127+85 -L- +/- RT	ROADWAY FILL	0.003				0.007	0.014	
16	136+35 -L- +/- to 138+30 -L- +/- RT	ROADWAY FILL	0.231				0.045		
17	144+25 -L- +/- to 146+10 -L- +/- LT	ROADWAY FILL	0.325				0.040	0.030	
18	153+40 -L- +/- to 155+25 -L- +/- LT	ROADWAY FILL	0.105				0.042	0.035	
19	159+25 -L- +/- to 163+65 -L- +/-	ROADWAY FILL	1.070				0.091		
20	164+55 -L- +/- to 165+70 -L- +/- LT	ROADWAY FILL	0.142				0.026		
21	173+70 -L- +/- to 182+40 -L- +/-	ROADWAY FILL	2.388				0.385	0.125	
TOTALS, This Sheet:			4.264				0.636	0.204	

WETLAND PERMIT IMPACT SUMMARY							SURFACE WATER IMPACTS				
Site No.	Station (From/To)	Structure Size / Type	WETLAND IMPACTS			Hand Clearing in Wetlands (ac)	Permanent SW impacts (ac)	Temp. SW impacts (ac)	Existing Channel Impacts (ft)	Existing Channel Impacts (ft)	Natural Stream Design (ft)
			Permanent Fill In Wetlands (ac)	Temp. Fill In Wetlands (ac)	Excavation in Wetlands (ac)						
22	184+50 -L- +/- to 191+85 -L- +/-	ROADWAY FILL	1.313			0.243	0.222				
23	197+05 -L- +/- to 197+70 -L- +/-	ROADWAY FILL	0.161			0.021	0.008				
24	198+55 -L- +/- to 209+30 -L- +/-	ROADWAY FILL	2.889			0.456	0.089				
25	208+60 -L- +/- to 219+90 -L- +/-	ROADWAY FILL	2.497			0.411	0.167				
26	218+65 -L- +/- to 220+45 -L- +/- LT	ROADWAY FILL					0.013				
27	222+25 -L- +/- to 223+65 -L- +/- RT	ROADWAY FILL	0.004			0.021	0.007				
28	225+20 -L- +/- to 225+65 -L- +/- LT	ROADWAY FILL					0.002				
29	229+50 -L- +/- to 235+30 -L- +/-	ROADWAY FILL	0.682				0.123	0.019			
TOTALS, This Sheet:			7.546				1.275	0.527			

NC DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

BRUNSWICK COUNTY
PROJECT: 34407.1.1 (R-2245)

SHEET OF

WETLAND PERMIT IMPACT SUMMARY									
Site No.	Station (From/To)	Structure Size / Type	WETLAND IMPACTS			SURFACE WATER IMPACTS			
			Permanent Fill in Wetlands (ac)	Temp. Fill in Wetlands (ac)	Excavation in Wetlands (ac)	Mechanized Clearing in Wetlands (ac)	Hand Clearing in Wetlands (ac)	Permanent SW impacts (ac)	Temp. SW impacts (ac)
30	229+15 -L- +/- to 242+45 -L- +/-	ROADWAY FILL	0.687			0.297	0.091		
31	236+55 -L- +/- to 237+60 -L- +/- LT	ROADWAY FILL	0.073			0.015			
*32	SOUTH WEST CORNER OF L- and NC-211	ROADWAY FILL	0.514	0.166					
		*Drawdown Effect	0.671						
33	SOUTH EAST CORNER OF L- and NC-211	ROADWAY FILL	0.047			0.050			
34	24+00 -Y7- (NC-211)	10' w x 8' h RCBC					0.009	0.011	34
35	ACCESS ROAD 2 15+65 +/- to 16+45 +/-	HAND CLEARING FOR RIGHT OF WAY FENCE				0.006			61
TOTALS, This Sheet:			1.992	0.166	0.312	0.147	0.009	0.011	34
									61

* NOTE: Area of drawdown as a result of moving existing roadway ditch. 0.671 Acres

NC DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

BRUNSWICK COUNTY
PROJECT: 34407.1.1 (R-2245)

SHEET 0F

WETLAND PERMIT IMPACT SUMMARY											
Site No.	Station (From/To)	Structure Size / Type	WETLAND IMPACTS				SURFACE WATER IMPACTS				
			Permanent Fill in Wetlands (ac)	Temp. Fill in Wetlands (ac)	Excavation in Wetlands (ac)	Mechanized Clearing in Wetlands (ac)	Hand Clearing in Wetlands (ac)	Permanent SW Impacts (ac)	Temp. SW Impacts (ac)	Existing Channel Impacts Permanent (ft)	Existing Channel Impacts Temp. (ft)
*1	16+00-L- +/- to	ROADWAY FILL	0.099				0.039				
	19+80 -L- +/- LT										
*2	15+40 -L- +/- to	ROADWAY FILL	0.162				0.047				
	19+80 -L- +/- RT										
3	20+70 -L- +/- to	CLASS II RIP RAP FILL						0.044	0.001		
	21+20 -L- +/-	SLOPE PROTECTION & TEMP. WORK BRIDGE									
*4	21+75 -L- +/- to	CLASS II RIP RAP FILL	0.018					0.005	0.001		
	22+35 -L- +/-	SLOPE PROTECTION & MECH. CLEARING									
5	56+40 -L- +/- to	BRIDGE OVER INTRACOASTAL WATERWAY						0.013			
	60+70 -L- +/-										
6	75+40 -L- +/- to	ROADWAY FILL	1.395				0.142	0.075			
	83+00 -L- +/-										
TOTALS, This Sheet:			1.674				0.142	0.161	0.049	0.015	

* NOTE: Wetlands at Sites 1, 2, and 4 are Coastal Wetlands

Total impacts to Coastal Wetlands:

Permanent Fill in Coastal Wetlands: 0.279 acres
Hand Clearing in Coastal Wetlands: 0.086 acres

SPECIAL EROSION CONTROL FENCE (TEMPORARY FILL) WILL BE PLACED IN SOME AREAS OF MECH. CLEARING.

NC DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
BRUNSWICK COUNTY
PROJECT: 34407.1.1 (R-2245)
SHEET OF

WETLAND PERMIT IMPACT SUMMARY

NC DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

BRUNSWICK COUNTY
PROJECT: 34407.1.1 (R-2245)

SHEET OF

MATCH LINE SEE SHEET 6 -L- STA. 50+20.00

GEORGE L. FISHER, ETUX

BENNY E. PRINCE

HENRY G. HARRIS

RANDY W. GOODNIGHT

CHARLOTTE M. HAWTHORNE

BENNY E. PRINCE

REVISIONS

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REVIEWS

SITE 12

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

NAD 83

MATCH LINE SEE SHEET 10 -L- STA. 104+25.00

REVISIONS

WOODS

BURY CROSS PIPE

1.0'

HDWL

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PROJECT REFERENCE NO.	SHEET NO.
R-2245	14
MY SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

ENGLIS

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

REVISIONS

MATCH LINE SEE SHEET L STA. 146+15.00

150

NAD 83

155

SITE 18

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

SITE 19

FILL IN WETLANDS
&
MECHANIZED CLEARING

MATCH LINE SEE SHEET 15-L STA. 159+50.00

5 @ 60" x 46" CORR. ALUM. PIPE-ARCH
FOR ANIMAL PASSAGE
WITH CONTINUOUS ALUMINUM HEADWALLS
ON BOTH ENDS

BURY PIPES 1.0' AND BACKFILL
ENTIRE LENGTH OF EACH PIPE
WITH 1.0' OF SOIL

106 LaDANE WILLIAMSON

SCALE

LEGEND

FILL IN WETLANDS

MECHANIZED CLEARING

HAND CLEARING

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

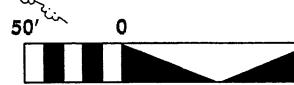
ENGLISH

8/17/93

REVISIONS

MATCH LINE SEE SHEET 15 -I- STA. 173. +90.00

SCALE



0

50'

100'

LODANE WILLIAMSON

LEGEND



SITE 21

FILL IN WETLANDS,
 MECHANIZED & HAND CLEARING

175

180

NAD 83

185

SITE 22

FILL IN WETLANDS,
 MECHANIZED & HAND CLEARING

106

LODANE WILLIAMSON

MATCH LINE SEE SHEET 17 -I- STA. 187 +50.00

107
 RESERVE DEVELOPMENT CO., LLC

NO REVISIONS CAN BE MADE TO THE CONTROL
 OF ACCESS WITHOUT APPROVAL OF NCDOT,
 DENR AND USACE.

BRUNSWICK ELECTRONIC STRIP CORP.
 08 160 PG 1

PROJECT REFERENCE NO.	SHEET NO.
R-2245	17
NW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

ENGLISH

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

LODANE WILLIAMSON

(106)

BRUNSWICK ELECTRIC MEMBERSHIP CORP.
08 100 PG 1S 85° 48' 0" W
126° 30'

EP

NAD 83

200

SITE 22

FILL IN WETLAND,
MECHANIZED &
HAND CLEARING

190

195

SITE 23

FILL IN WETLAND,
MECHANIZED & HAND CLEARING

(107) RESERVE DEVELOPMENT CO., LLC

MATCH LINE SEE SHEET 16 -L- STA. 187+50.00

18-NOV-2005 15:32 Permits\2245\Permit psb 17.dgn
f:\Hydro\autodesk\Permits\2245\Permit psb 17.dgnYOUNG'S GAS & GROCERY
OF WINNABOW, INC

(109)

(109)

YOUNG'S GAS & GROCERY
OF WINNABOW, INC

(109)

200

STA. 201+00

STA. 201+10

STA. 201+20

STA. 201+30

STA. 201+40

STA. 201+50

STA. 201+60

STA. 201+70

STA. 201+80

STA. 201+90

STA. 201+100

STA. 201+110

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STA. 201+130

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STA. 201+1610



PROJECT REFERENCE NO.	18
R-2245	
REV SHEET NO.	18
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

REVISIONS

SITE 24

FILL IN WETLAND,
MECHANIZED AND HAND CLEARING

205

MATCH LINE SEE SHEET 17-L STA 201+20.00

(109) YOUNG'S GAS & GROCERY
OF WINNABOW, INC.

SITE 25

FILL IN WETLAND,
MECHANIZED AND HAND CLEARING

210

NAD 83

(109) YOUNG'S GAS & GROCERY
OF WINNABOW, INC.

BURY SMALL ANIMAL PIPE 1.0' AND
BACKFILL ENTIRE LENGTH OF PIPE
WITH 1.0' OF SOIL

BURY EQUALIZER PIPES
1.0'

BURY EQUALIZER PIPE
1.0'

48° WW FENCE

48° WW FENCE

48° WW FENCE

RESERVE DEVELOPMENT CO., LLC

BURY EQUALIZER PIPE
1.0'

MATCH LINE SEE SHEET 19-L STA 215+00.00

(107) RESERVE DEVELOPMENT CO., LLC

SCALE
50' 0 100'

LEGEND

FILL IN WETLANDS

MECHANIZED CLEARING

HAND CLEARING

REVISIONS

— 18 I STA 215±00.00

— SITE 25

**FILL IN WETLAND,
MECHANIZED & HAND CLEARI**

109 YOUNG'S GAS & CO.

**HAND CLEARING WILL BE REQUIRED
IN AREAS DESIGNATED BY THE PERMIT
AND AS DIRECTED BY THE ENGINEER.**

NAD 83

* SITE 2

HAND CLE

SITE 28

HAND CLEAR

SITE 27

**FILL IN WETLAND,
MECHANIZED & HAND CLEARING**

SCALE

LEGEND

FILL IN WETLANDS → 

MECHANIZED CLEARING

HAND CLEARING

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.



PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NAD 83

SITE 32

FILL and EXCAVATION IN WETLANDS
&
MECHANICAL CLEARING
&
WETLAND IMPACTS FROM
DITCH DRAWDOWN

MATCH LINE SEE SHEET 20 -I- STA. 242+75.00

MATCH LINE SEE SHEET 22 -Y7- STA. 13+15.00

REVISIONS

YOUNG'S GAS & GROCERY
OF WINNABOW, INC

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SITE 34

TEMPORARY AND PERMANENT
SURFACE WATER IMPACTS
&
TEMPORARY AND PERMANENT
CHANNEL IMPACTS

REVISIONS

MATCH LINE SEE SHEET 21-Y2- STA 22 ± 55 00

SITE 34

RAR Y AND PERMANENT FACE WATER IMPACTS & RAR Y AND PERMANENT CHANNEL IMPACTS

113 WILLIAM J. BATUYIC

114 JAMES E. McCRACKEN

115 GENE ALAN BARRETT

**RIP RAP AT
EMBANKMENT
SEE DETAIL
TDE**

A. 22 + 55.00

TDE **SPECIAL CUT DITCH**

An aerial map showing a coastal area with various landmarks and a red circle. Inside the red circle, the letter 'A' is written, and the letters 'DDE' are written above it. There are also other markings like 'S' and '25' on the map.

18° CONC 151

28
NC 211 SOUTH
3' BS

— — —

SCALE

LEGEND

PERMANENT FILL IN SURFACE WATERS

TEMPORARY FILL IN SURFACE WATERS

TERS —

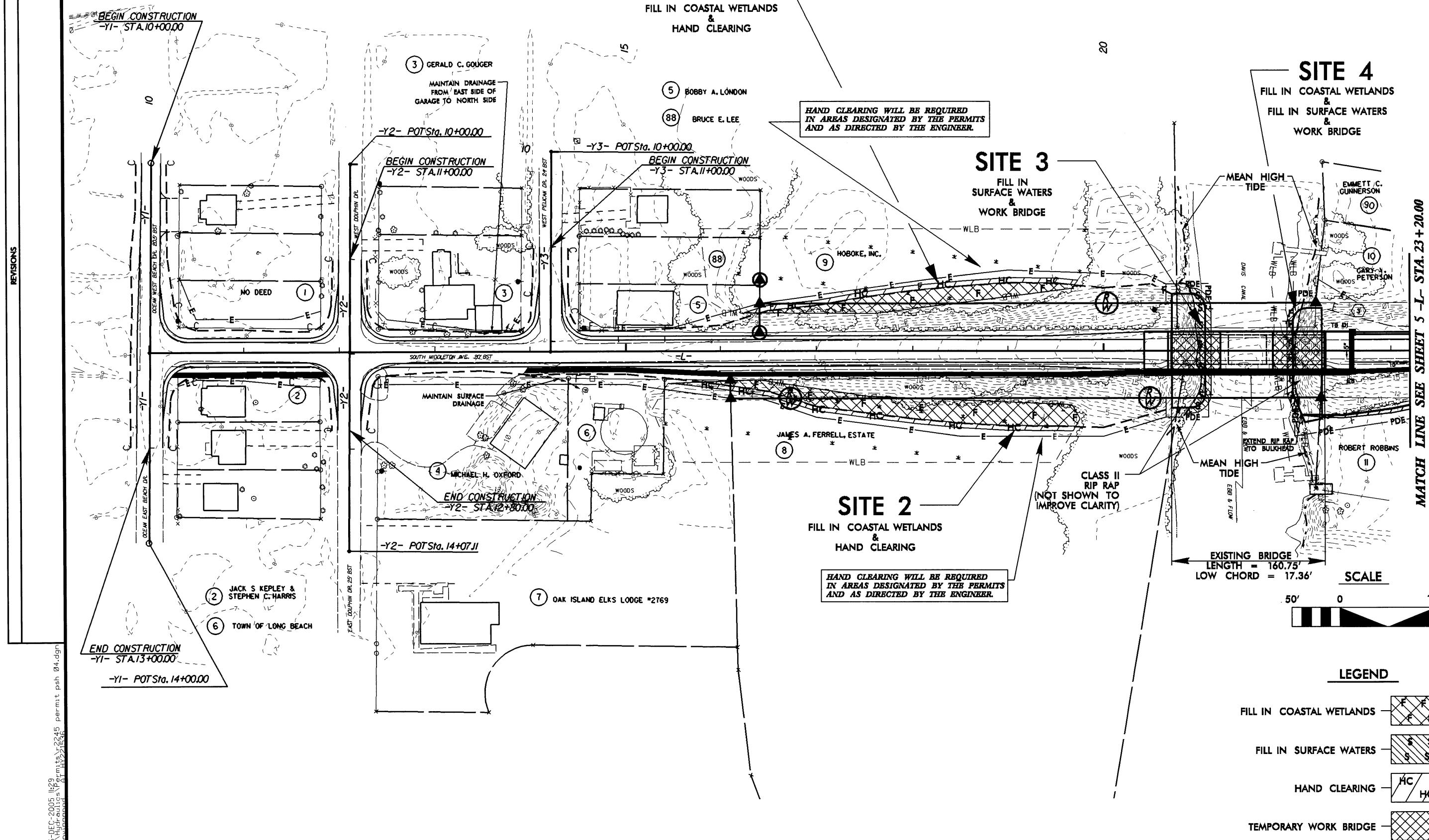
TERS →

PROJECT REFERENCE NO.	SHEET NO.
R-2245	4
REV SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NAD 83



PROJECT REFERENCE NO.	8
SHET NO.	8
MR SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

ENGLISH

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

REVISIONS

MATCH LINE SEE SHEET 1 STA 64+05.00

MATCH LINE SEE SHEET 7 STA 77+20.00

MATCH LINE SEE SHEET 9 STA 77+20.00

NAD 83

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SITE 12

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

NAD 83

MATCH LINE SEE SHEET 10 - STA. 104+25.00

REVISIONS

D.A. = 33.2 Ac
 * Q2 = 13.3 cfs
 V2 = 1.3 ft/sec
 Q10 = 21 cfs
 V10 = 1.5 ft/sec

NO REVISIONS CAN BE MADE TO THE CONTROL
 OF ACCESS WITHOUT APPROVAL OF NCDOT,
 DENR AND USACE.

05-DEC-2005 14:19 Permit#12245 psh 11.dgn
 File#12245

BURY CROSS PIPE

1.0'

HDWL

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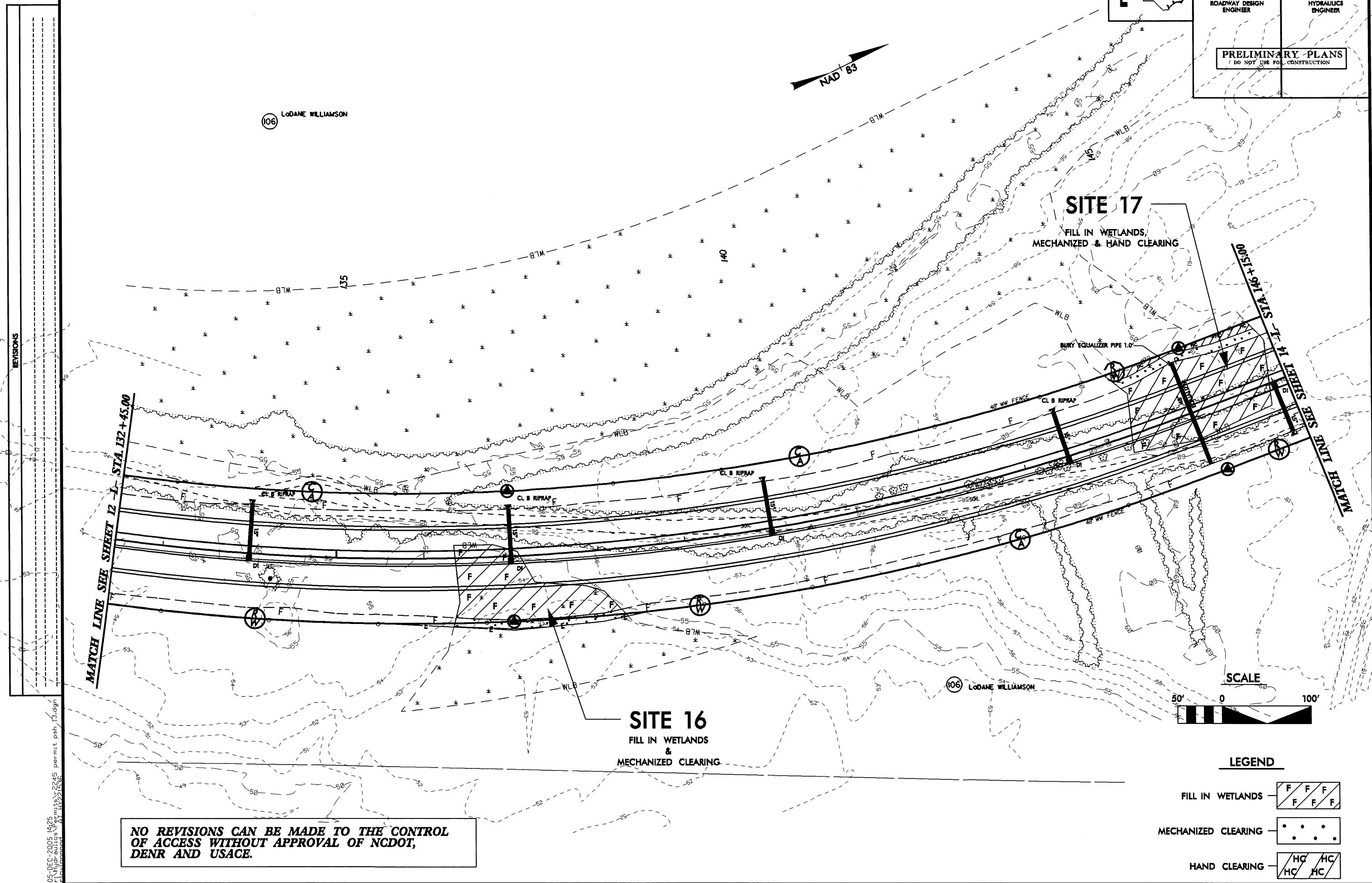
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**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

卷之三

ENGLISH



ENGLISH

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

ROADWAY DESIGN ENGINEER

HYDRAULICS
ENGINEER

STA 116 ± 15.00

— SITE 18 —

**FILL IN WETLANDS,
MACHINIZED & HAND CLEARING**

SITE 19

~~FILL IN WETLANDS~~
&
~~MECHANIZED CLEARING~~

106 LODANE WILLIAMSON

NAD' 83

REVIEWS

5 @ 60" x 46" CORR. ALUM. PIPE-ARCH
FOR ANIMAL PASSAGE
WITH CONTINUOUS ALUMINUM HEADWALLS
ON BOTH ENDS

BURY PIPES 1' AND BACKFILL
ENTIRE LENGTH OF EACH PIPE
WITH 1' OF SOIL

SCALE

LEGEND

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

MECHANIZED CLEARING — * * * * .

HAND CLEARING — HC / HC

SCALP

50' 0 10'

LEGEND

FILL IN WETLANDS

MECHANIZED CLEARING

HAND CLEARING



SITE 21

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

175

180

185

106

LODANE WILLIAMSON

NAD 83

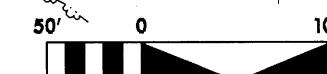
SITE 22

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

MATCH LINE SEE SHEET 15 - STA. 183+90.00

MATCH LINE SEE SHEET 17 - STA. 187+90.00

REVISIONS



LEGEND



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 File Number: R-2245

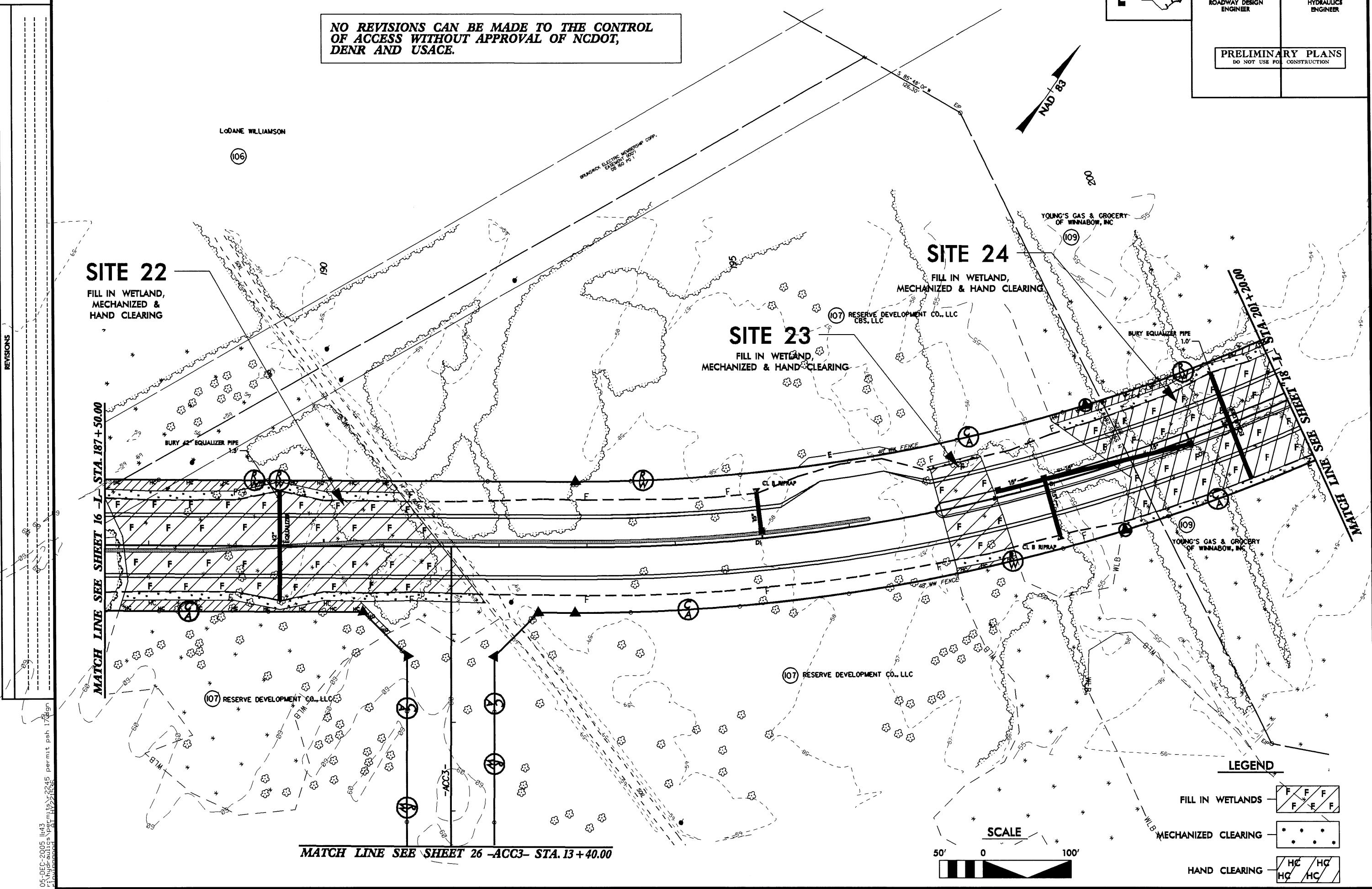
RESERVE DEVELOPMENT CO., LLC

NO REVISIONS CAN BE MADE TO THE CONTROL
 OF ACCESS WITHOUT APPROVAL OF NCDOT,
 DENR AND USACE.

ENGLISH

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION



SITE 24

**FILL IN WETLAND,
MECHANIZED AND HAND CLEARING**

SINCAVIS

MATCH LINE SEE SHEET 17-1

SITE 25

**FILL IN WETLAND,
MECHANIZED AND HAND CLEARING**

A logo consisting of a diagonal arrow pointing upwards and to the right, with the text "NAD 83" written along the arrow.

BURY SMALL ANIMAL PIPE 1.5" BACKFILL FIBREGLASS LINED

(109) YOUNG'S GAS & GROCER
OF WINNABOW, INC

BURY SMALL ANIMAL PIPE
BACKFILL ENTIRE LENGTH
WITH 10%

BURY EQUALIZER PIPE

BURY EQUALIZER PIPE

VE DEVELOPMENT CO., LLC

EQUALIZER PIPE

~~MATCH LINE SEE SHEET 19-L STA. 215+00~~

A scale bar diagram with markings at 60', 0, and 100'. The 60' and 100' markings are on the left and right respectively, with a central '0' marking. A shaded area is present between the 0 and 100' markings, with a larger triangular shaded region extending from the 0 marking towards the right.

LEGEND

ANIZED CLEARING * *
HAND CLEARING HC / HC / HC

**HAND CLEARING WILL BE REQUIRED
IN AREAS DESIGNATED BY THE PERMITS
AND AS DIRECTED BY THE ENGINEER.**

NAD⁺ 83

SITE 25

**FILL IN WETLAND,
MECHANIZED & HAND CLEARING**

(109) YOUNG'S GAS & CO.
OF WINNABOW, N.H.

— * SITE 26

HAND CLEARING

SITE 28

HAND CLEARING

— 7 STM 215±00.00

REVISIONS

SITE 27

**FILL IN WETLAND,
MECHANIZED & HAND CLEARING**

LEGEND

FILL IN WETLANDS → 

MECHANIZED CLEARING

HAND CLEARING — **HC** **HC**
 HC

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**



ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NAD 83

SITE 29

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

SITE 31

FILL IN WETLANDS
&
MECHANIZED CLEARING

235

REVISIONS

STA 228+75.00

STA 19-1

MATCH LINE SEE SHEET 19-1

MATCH LINE SEE SHEET 21-L STA 242+75.00

SITE 30

FILL IN WETLANDS,
MECHANIZED & HAND CLEARING

LEGEND
FILL IN WETLANDS
MECHANIZED CLEARING
HAND CLEARING

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.

PROJECT REFERENCE NO.	SHEET NO.
R-2245	21
IVW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

ENGLIS

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

SITE 32

FILL and EXCAVATION IN WETLANDS
&
MECHANICAL CLEARING
&
WETLAND IMPACTS FROM
DITCH DRAWDOWN

MATCH LINE SEE SHEET 22 -Y7- STA. 13 +15.00

MATCH LINE SEE SHEET 20 -L- STA. 242 +75.00

SITE 33

FILL IN WETLANDS
&
HAND CLEARING

MATCH LINE SEE SHEET 23 -Y7- STA. 22 +55.00

END TIP PROJECT R-2245
-L- POT STA. 248 +66.02

NAD 83

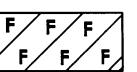
JOSEPH M. EDWARDS, SR.

END CONSTRUCTION
-Y8- POT STA. 16 +50.00

SCALE
50' 0 100'

LEGEND

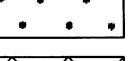
FILL IN WETLANDS



EXCAVATION IN WETLANDS



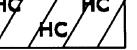
MECHANIZED CLEARING



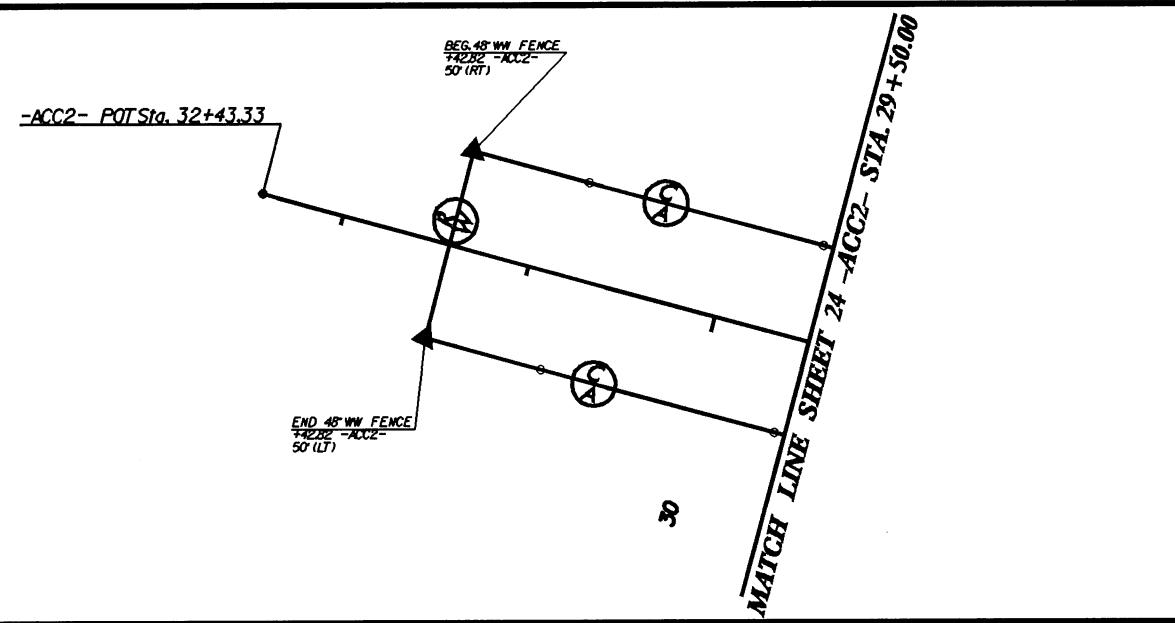
DITCH DRAWDOWN



HAND CLEARING



05-DEC-2005 [3:58 PM] hydraulics\permits\r2245 permit psh 24.dgn



MATCH LINE SHEET 24 - ACC2- STA. 29 + 50

SITE 3

HAN
CLEAR

2-

104 MAS

**NO REVISIONS CAN BE MADE TO THE CONTROL
OF ACCESS WITHOUT APPROVAL OF NCDOT,
DENR AND USACE.**

PROJECT REFERENCE NO.		SHEET NO.
R-2245		24
RW SHEET NO.		
Roadway Design Engineer	Hydraulics Engineer	
<div style="border: 1px solid black; padding: 10px; text-align: center;"> PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION </div>		

SITE 35

**HAND
CLEARING**

MATCHLINE SEE SHEET 9
ACC- STA. 15+20.00

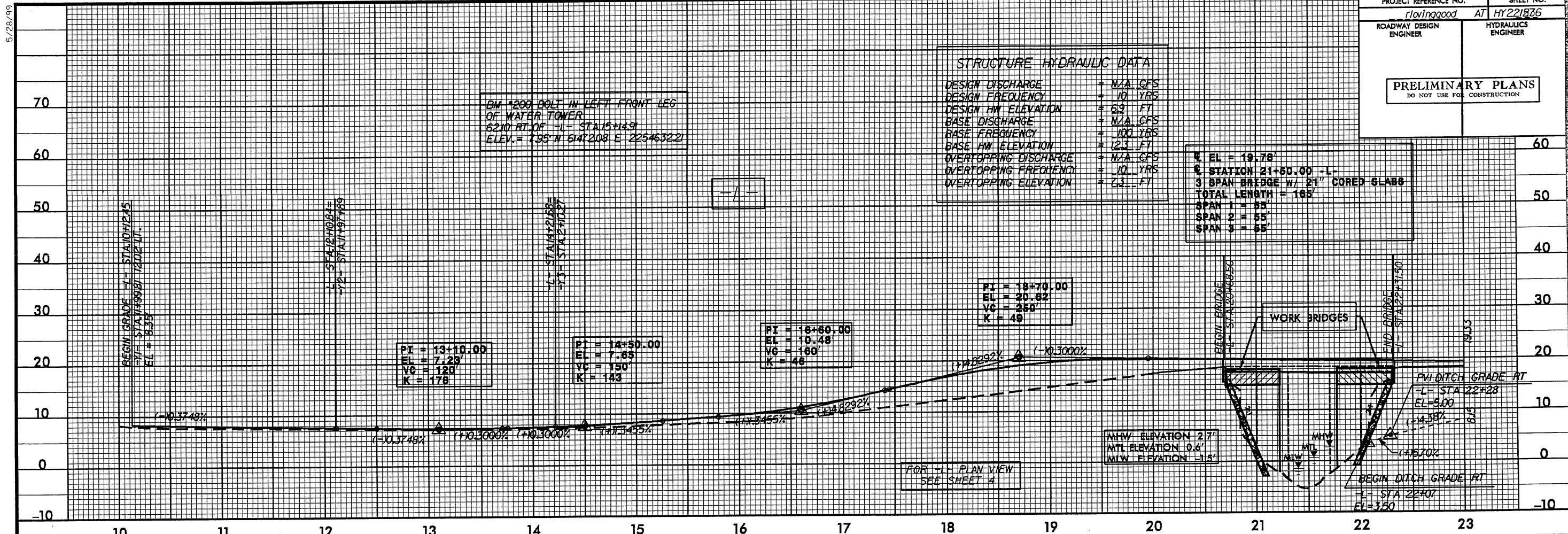
1 : WL SC

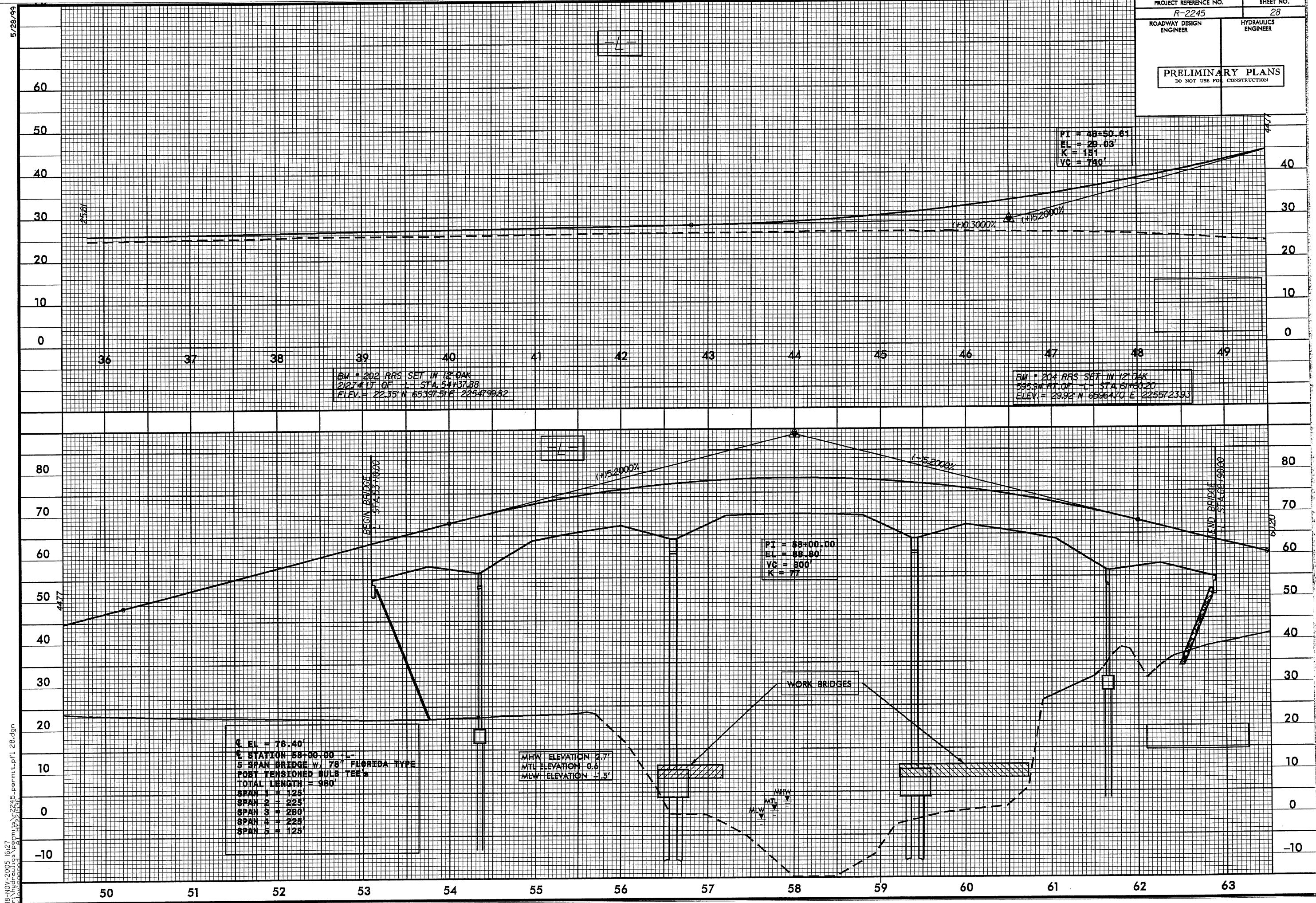
LEGEND

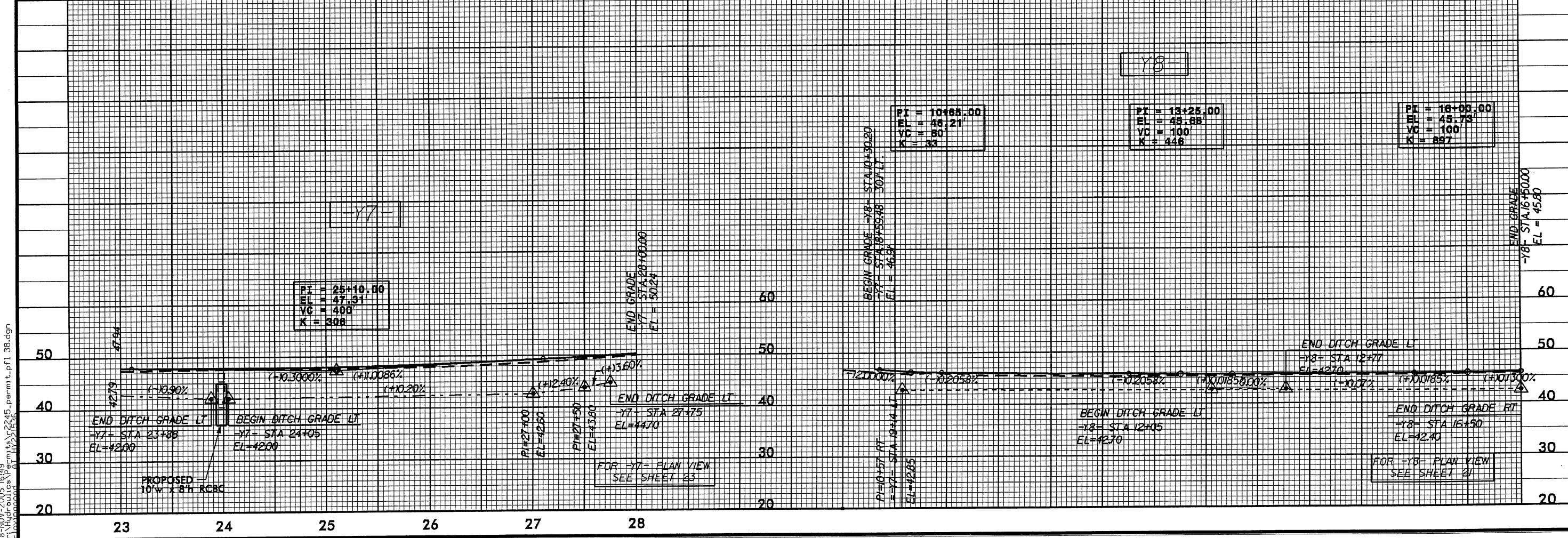
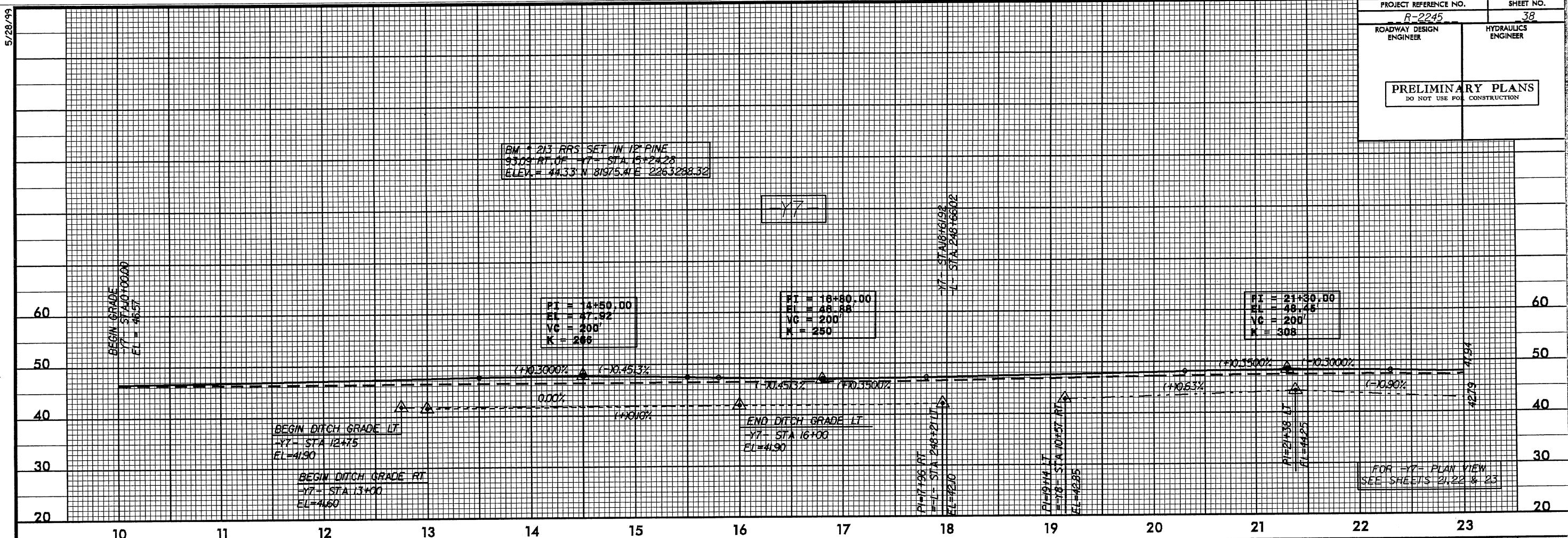
HAND CLEAR

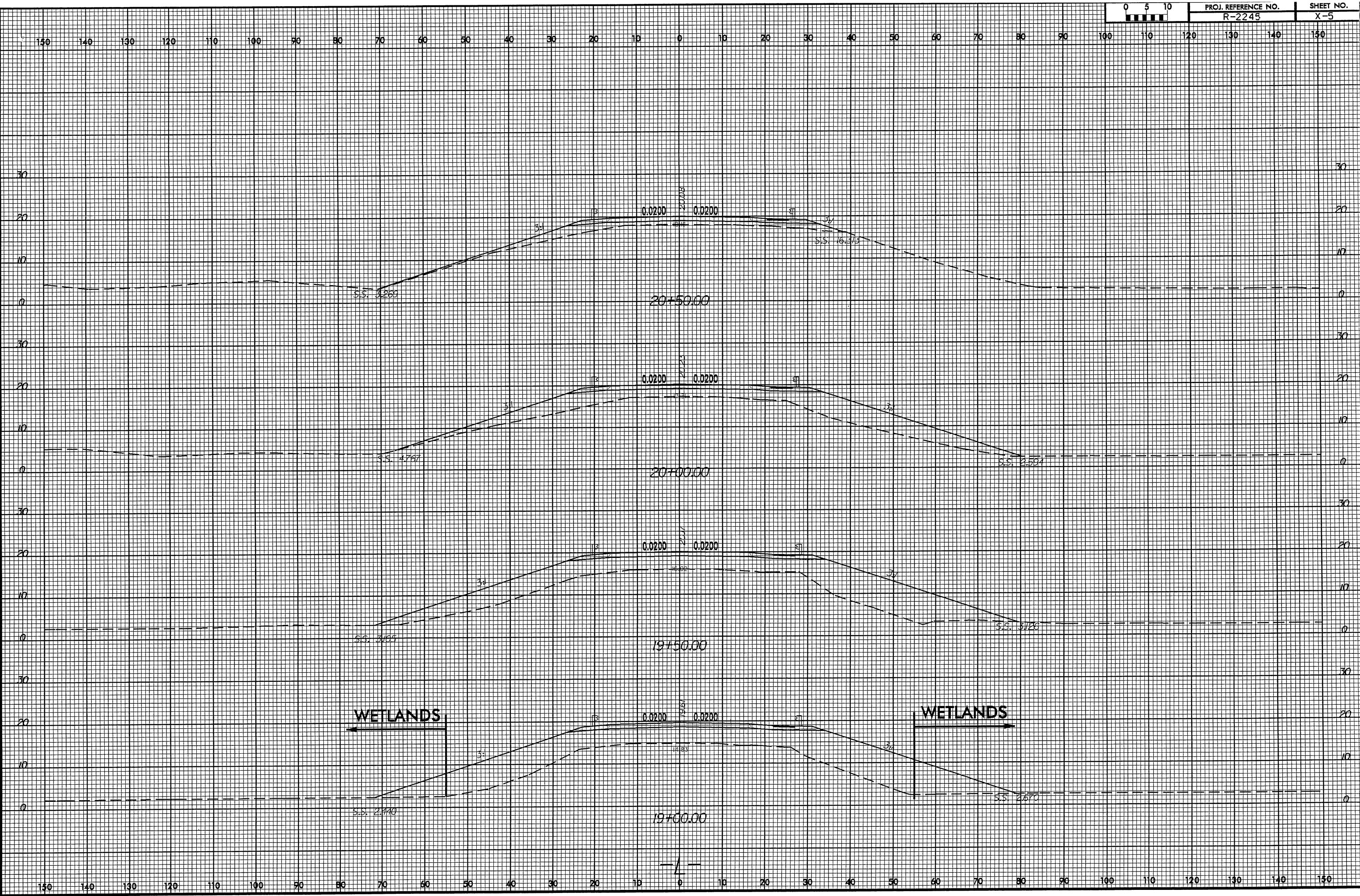
NOTE:

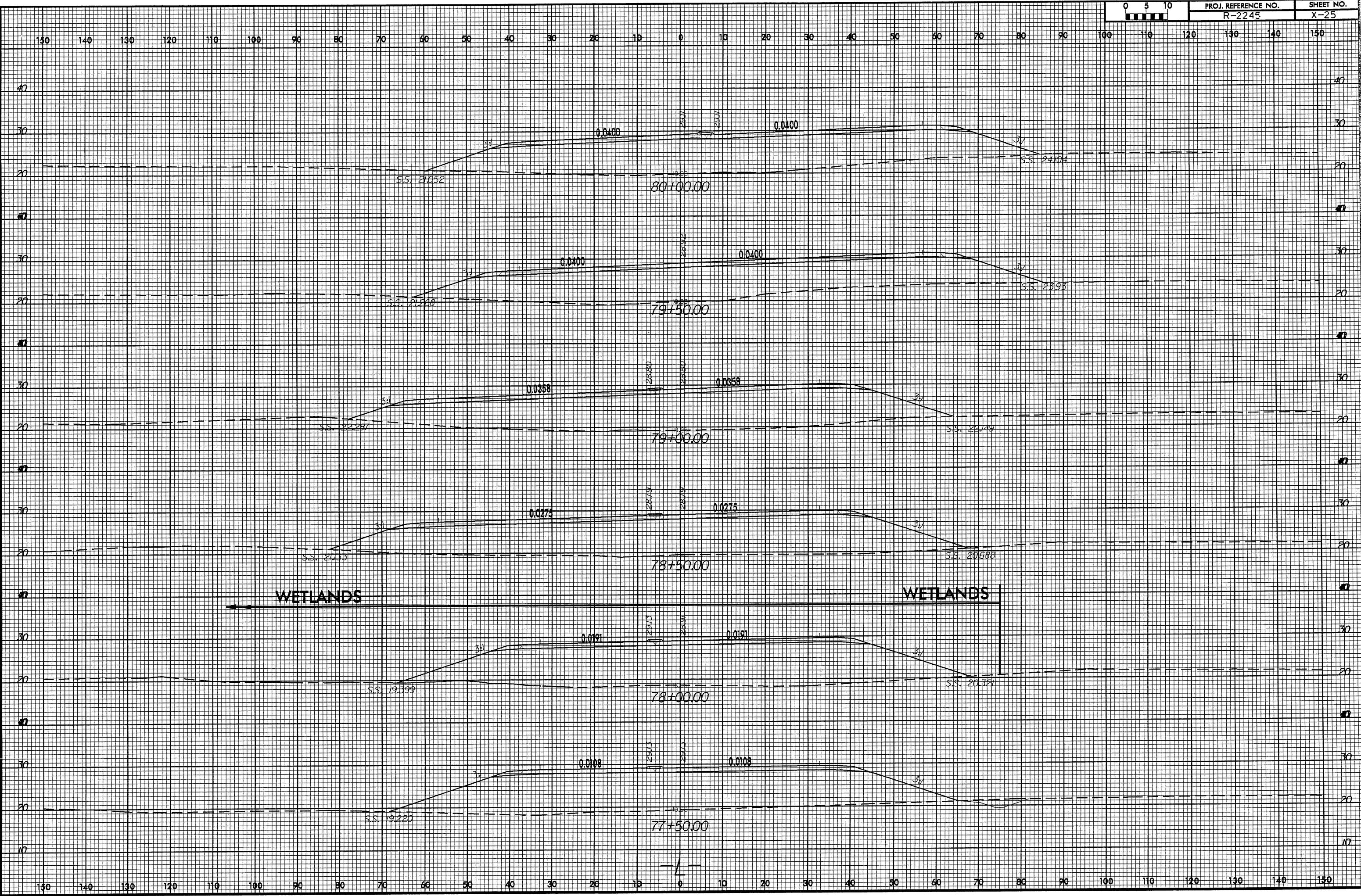
1) ACCESS ROAD TO BE CONSTRUCTED BY OTHERS.

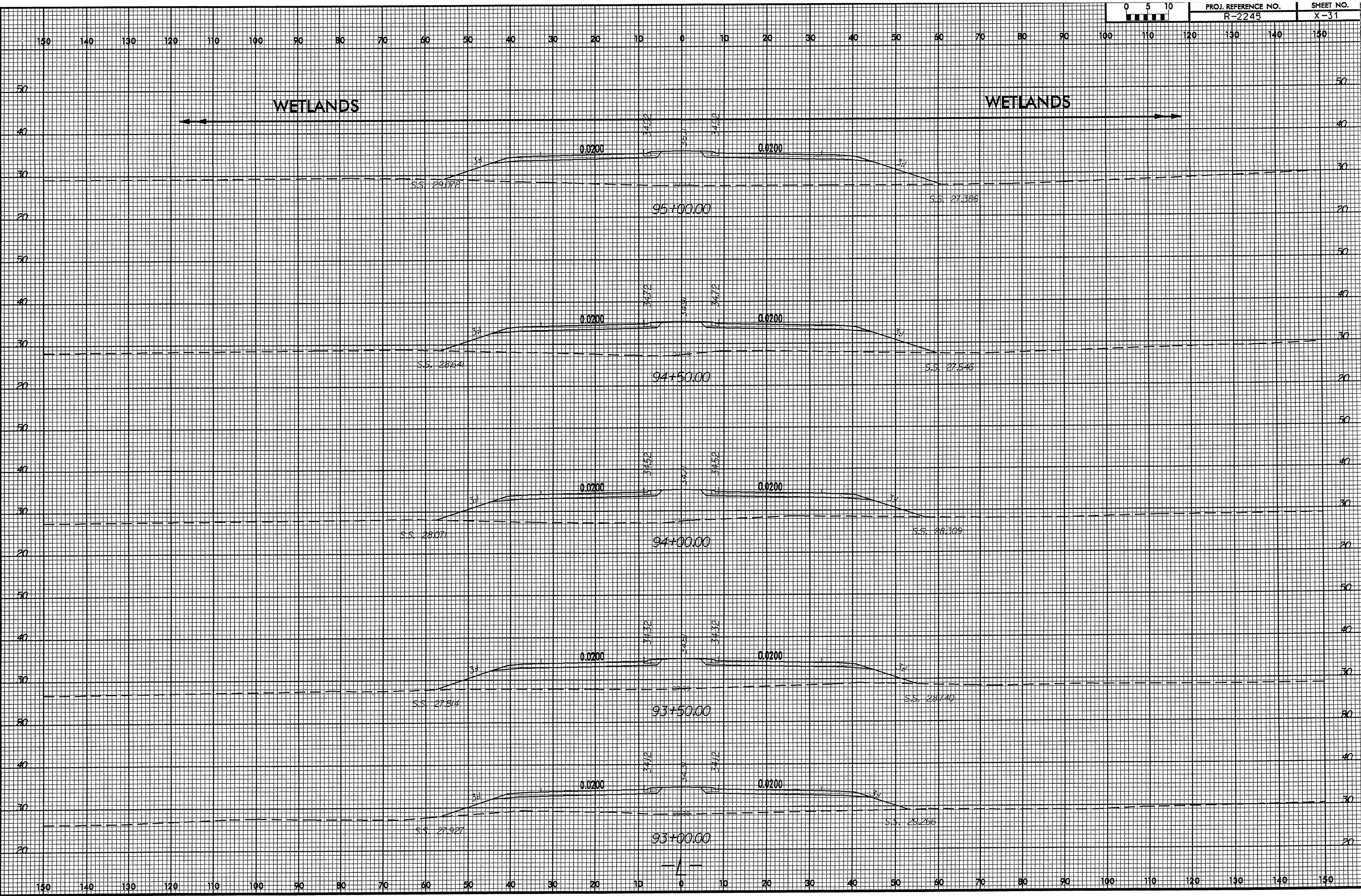


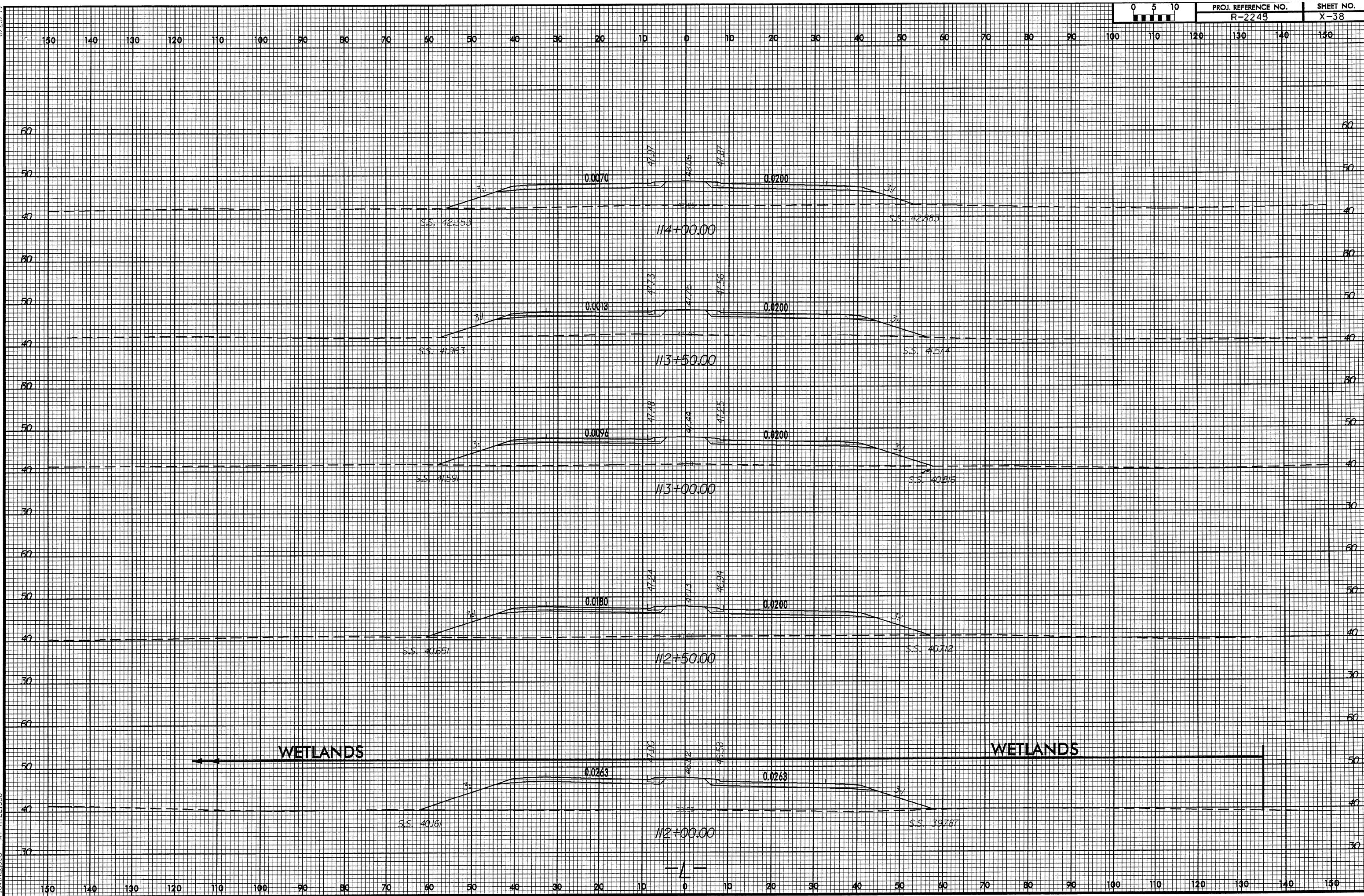


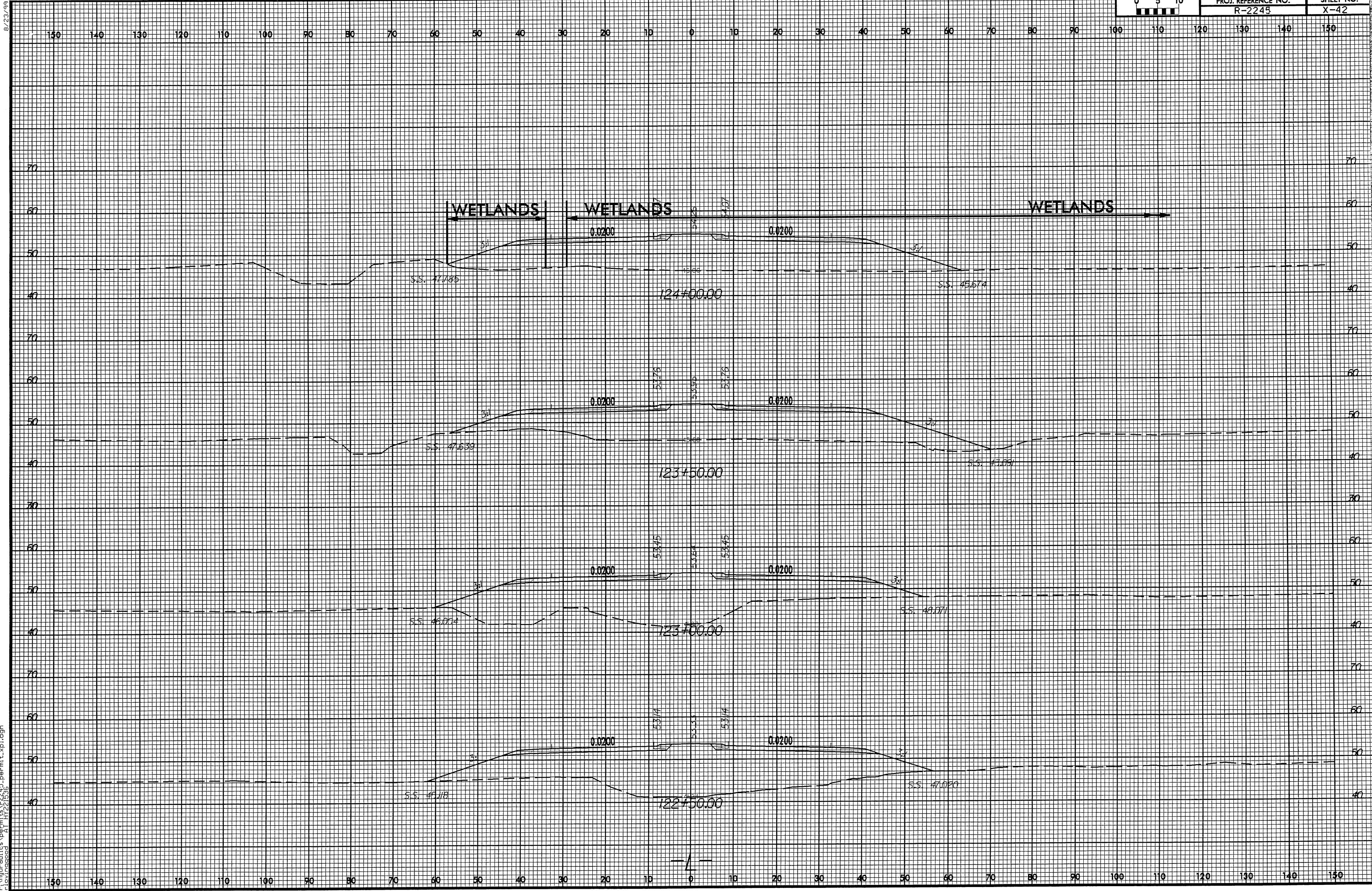


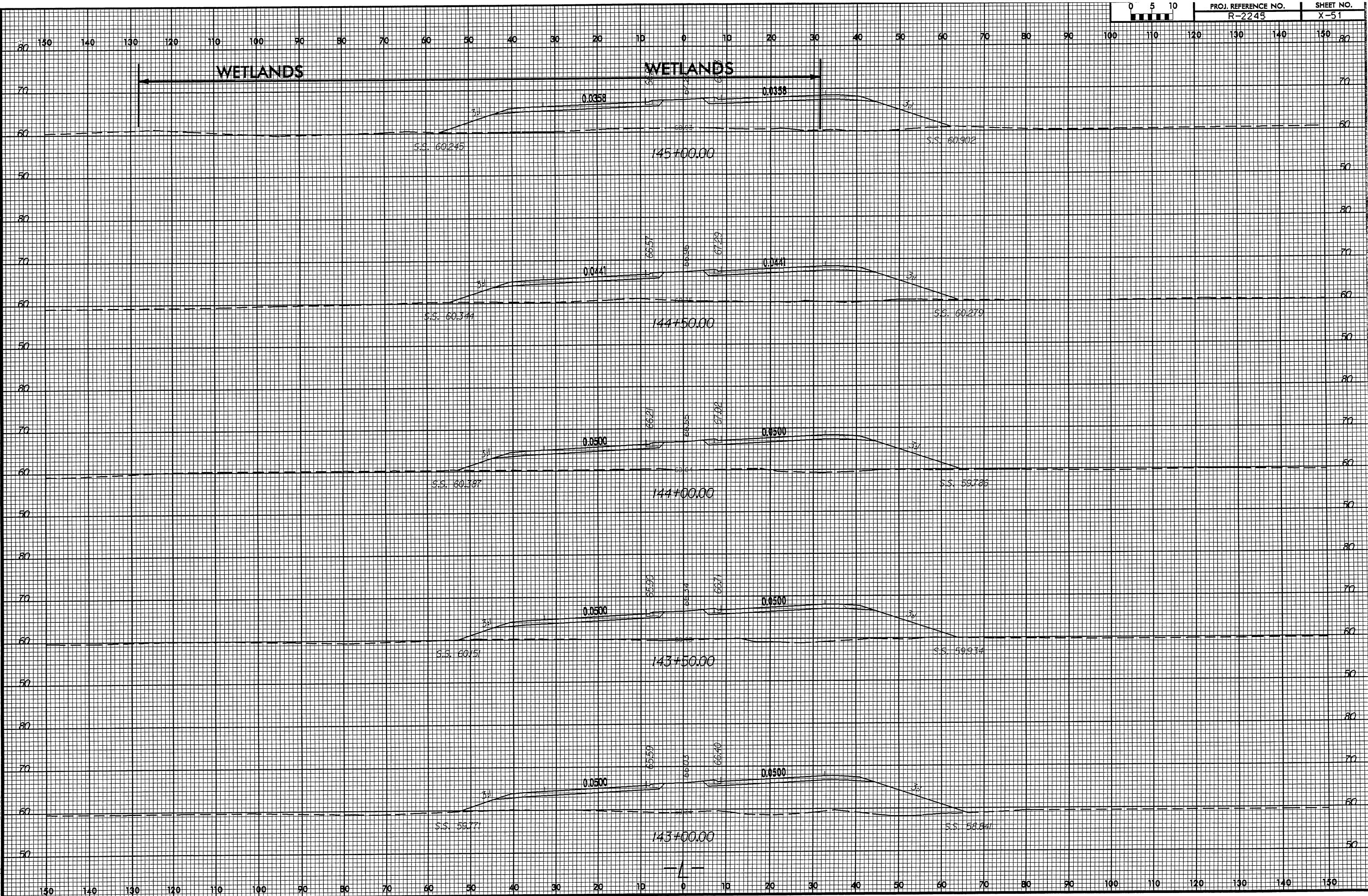


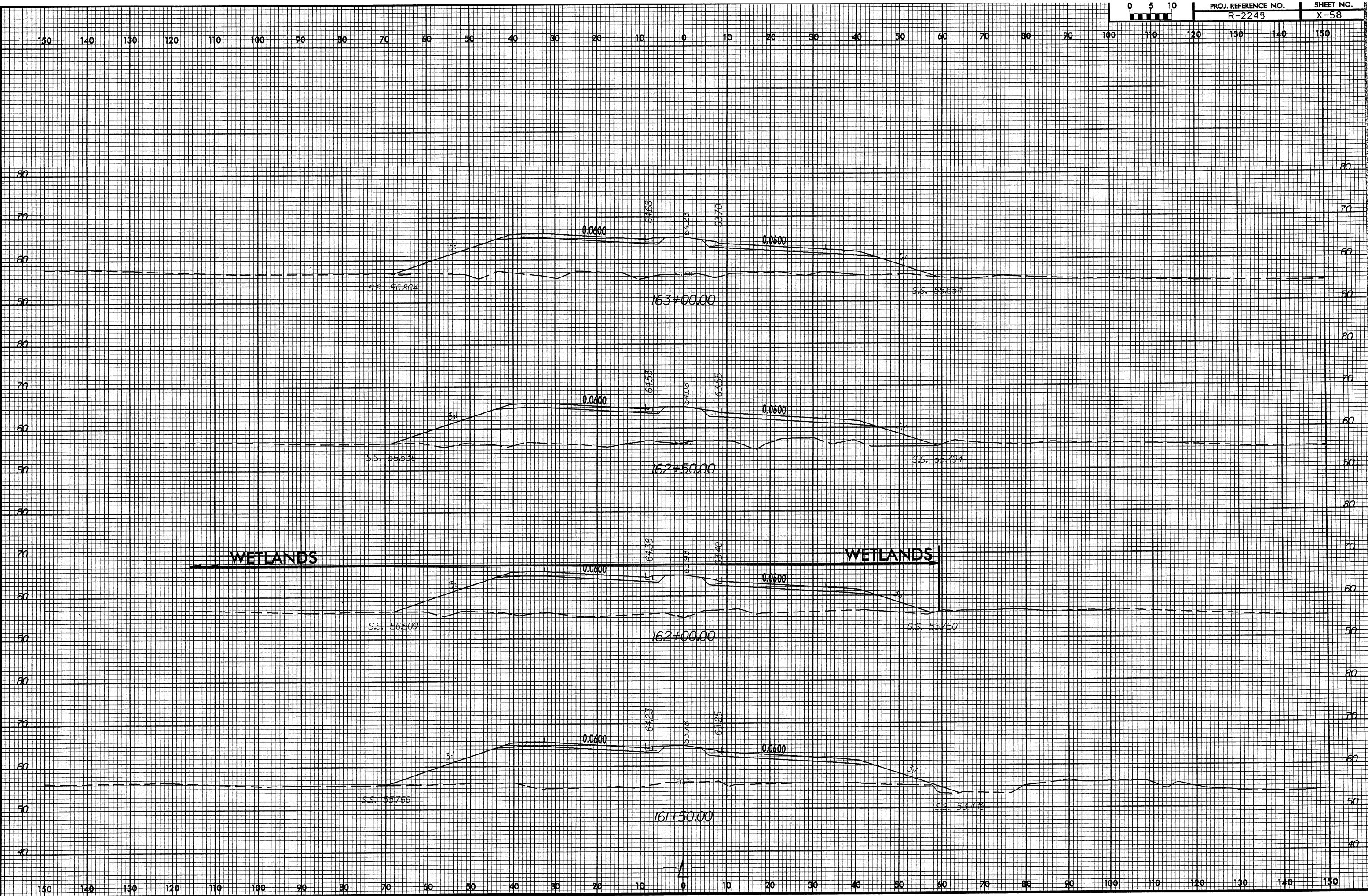


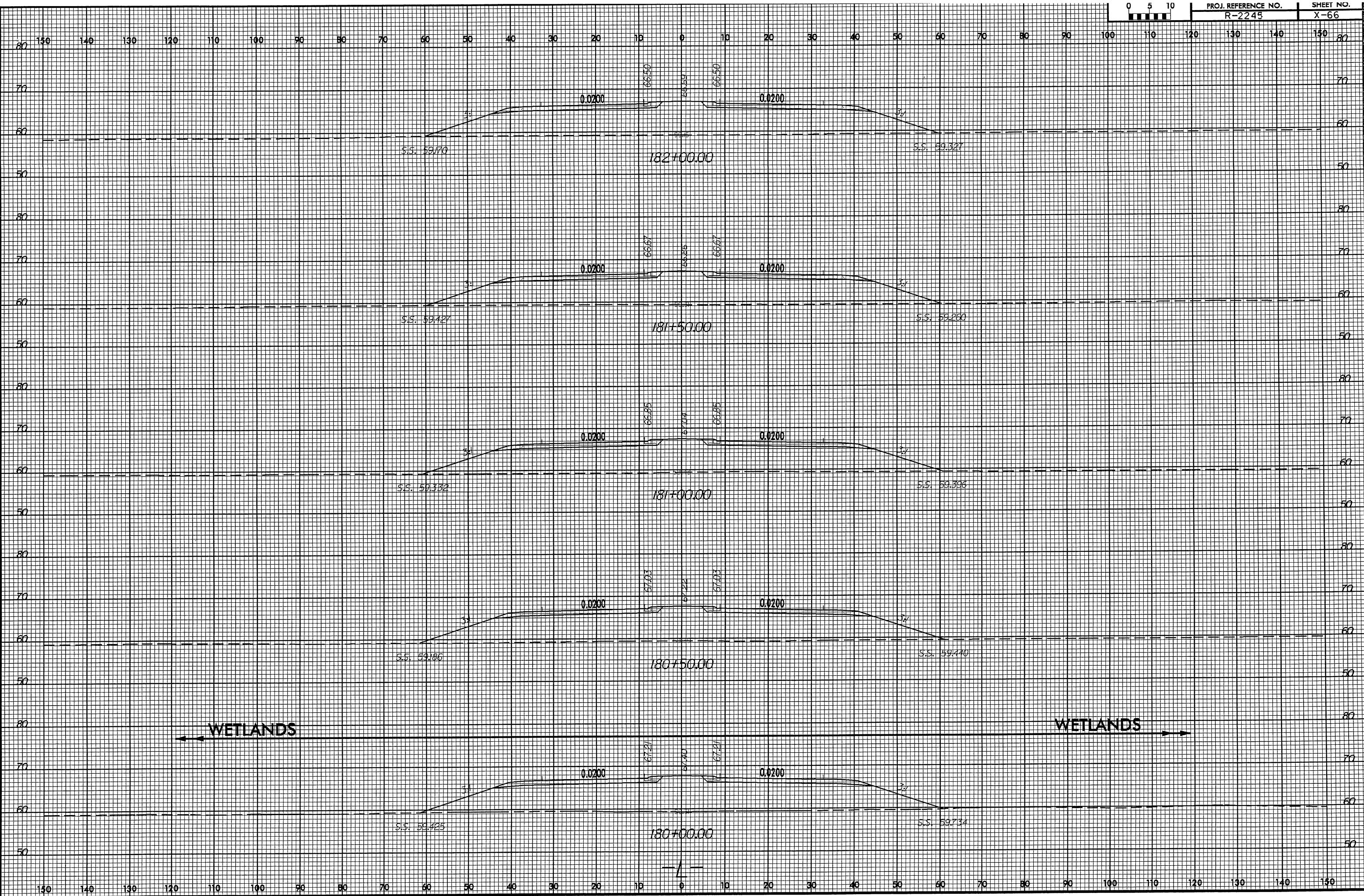


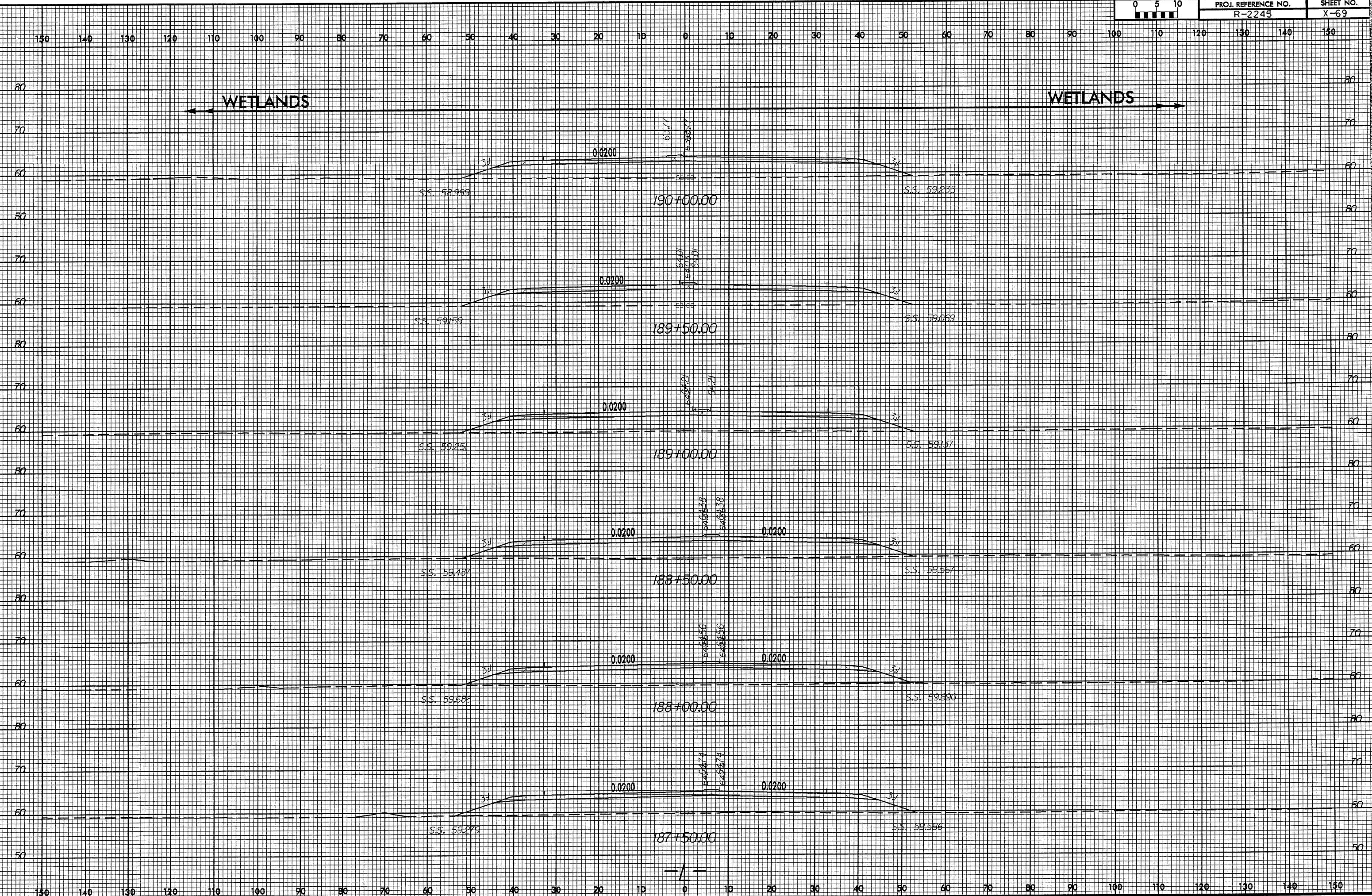












WETLANDS

WETLANDS

