

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5302	1	52

CONTENTS

LINE	STATION	PLAN
-L-	17+50 TO 96+40	4-10
-Y3-	10+54 TO 23+50	7, 11
-Y1-	10+36 TO 18+10	

CROSS SECTIONS

LINE	STATION	SHEETS
-L-	36+00 TO 96+50	12-40
-Y3-	10+54 TO 23+50	41-45

APPENDICES

APPENDIX	TITLE	SHEETS
A	SOIL TEST RESULTS	46-47
B	BORING LOGS	48-52

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY WAKE
PROJECT DESCRIPTION US 401 WIDENING FROM
SOUTH OF SR 2782 (LEGEND RD) TO US 70 GARNER

INVENTORY

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

S. BUCHANAN

R. DOYLE

Z. ARDEBILI

G. LANG

A. LOZADA

SUMMIT D&E

GEOTECHNICS

INVESTIGATED BY S. BUCHANAN

DRAWN BY Z. ARDEBILI

CHECKED BY R. DOYLE

SUBMITTED BY AECOM

DATE NOVEMBER 2018

REFERENCE: U-5302

PROJECT: 48000



Signed by:

Ryan P. Doyle

1/8/2026

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SIGNATURE

DATE

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT**

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																																																																																																																									
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</p>										<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>										<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>										<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOROUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (GAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SRC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																																																																																									
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ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p style="text-align: center;">COMPRESSIBILITY</p> <p>SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50</p> <p style="text-align: center;">PERCENTAGE OF MATERIAL</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE 1 - 10%</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE 10 - 20%</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME 20 - 35%</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>> 10%</td> <td>> 20%</td> <td>HIGHLY 35% AND ABOVE</td> </tr> </table> <p style="text-align: center;">GROUND WATER</p> <p> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING</p> <p> STATIC WATER LEVEL AFTER 24 HOURS</p> <p> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA</p> <p> SPRING OR SEEP</p> <p style="text-align: center;">MISCELLANEOUS SYMBOLS</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td></td> <td>25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES</td> </tr> <tr> <td></td> <td>SPT DPT DMT VST PMT TEST BORING</td> </tr> <tr> <td></td> <td>AUGER BORING</td> </tr> <tr> <td></td> <td>CORE BORING</td> </tr> <tr> <td></td> <td>MONITORING WELL</td> </tr> <tr> <td></td> <td>PIEZOMETER INSTALLATION</td> </tr> <tr> <td></td> <td>SLOPE INDICATOR INSTALLATION</td> </tr> <tr> <td></td> <td>CONE PENETROMETER TEST</td> </tr> <tr> <td></td> <td>SOUNDING ROD</td> </tr> <tr> <td></td> <td>TEST BORING WITH CORE</td> </tr> <tr> <td></td> <td>SPT N-VALUE</td> </tr> </table>										ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%	HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE		25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES		SPT DPT DMT VST PMT TEST BORING		AUGER BORING		CORE BORING		MONITORING WELL		PIEZOMETER INSTALLATION		SLOPE INDICATOR INSTALLATION		CONE PENETROMETER TEST		SOUNDING ROD		TEST BORING WITH CORE		SPT N-VALUE	<p style="text-align: center;">ROCK HARDNESS</p> <p>VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.</p> <p>HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.</p> <p>MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.</p> <p>MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.</p> <p>SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.</p> <p>VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PEICES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. 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TIP PROJECT: U-5302

CONTRACT: 48000

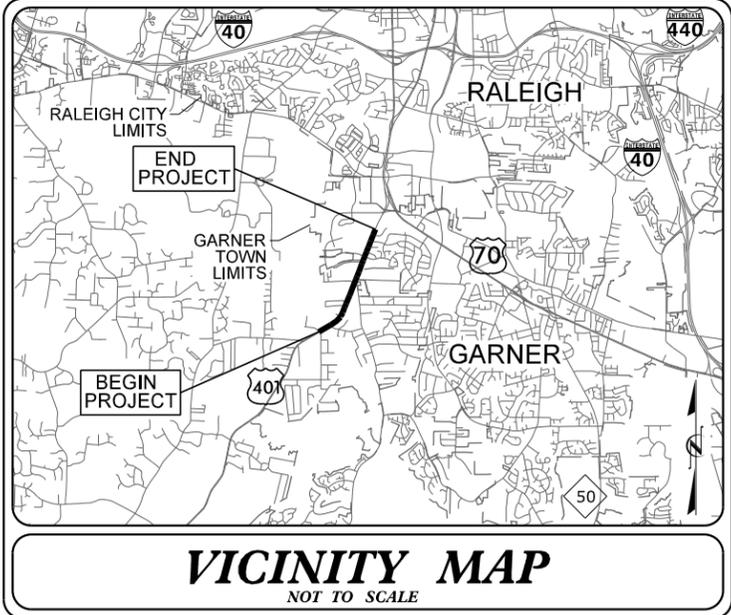
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

WAKE COUNTY

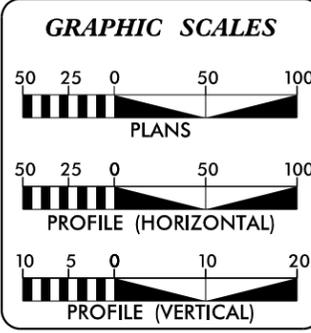
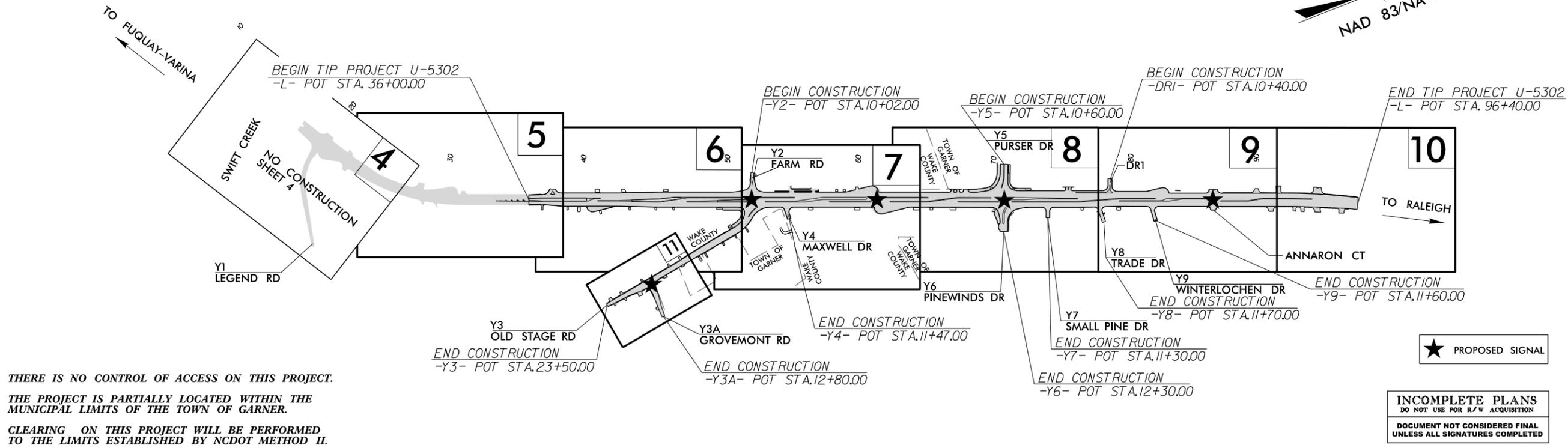
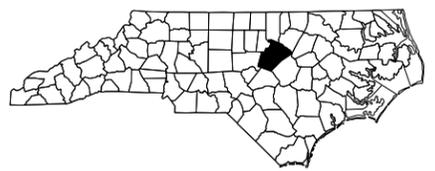
**LOCATION: US 401 SOUTH OF SR 2782 (LEGEND RD) TO US 70
GARNER**

TYPE OF WORK: PAVING, GRADING, DRAINAGE, AND SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5302	3	52
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
48000.1.1	N/A	PE	



25% PRELIMINARY SUBMITTAL



DESIGN DATA

ADT 2015 =	48,300
ADT 2040 =	67,000
K =	8 %
D =	60 %
T =	5 % *
V =	50 MPH
* TTST =	1% DUAL 4%
FUNC CLASS =	MAJOR ARTERIAL

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5302 =	1.144 MILES
LENGTH CULVERT TIP PROJECT U-5302 =	0.000 MILES
TOTAL LENGTH OF TIP PROJECT U-5302 =	1.144 MILES

Prepared in the Office of:

AECOM
NC FIRM LICENSE No: F-0342
701 Corporate Center Drive, Suite 475
Raleigh, NC 27607
(919) 854-6200 - (919) 854-6259(FAX)

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JULY 13, 2018

LETTING DATE:

LEN HILL, P.E.
PROJECT ENGINEER

KEVIN VANMETRE, P.E.
PROJECT DESIGN ENGINEER

BEN UPSHAW, P.E.
NCDOT DIVISION DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

11/21/2018 G:\DCS\Projects\URS\Jobs4\Projects\Geotechnical Support\NCDOT U-5302 US 401\4.0 Deliverables\U5302_GEO_RDWY\CADD_GEO\TECH\PlanProf\U5302_rdy_tsh.dgn Ryan.P.Doyle



AECOM – North Carolina
1600 Perimeter Park Drive, Suite 400
Morrisville, NC 27560
Tel: 919-461-1100
Fax: 919-461-1415

November 21, 2018

WBS NO.: 48000.1.1
NCDOT TIP: U-5302
COUNTY: Wake

DESCRIPTION: US 401 Widening from South of SR 2782 (Legend Rd) to US 70 Garner

SUBJECT: Geotechnical Report – Roadway Inventory

Project Description

This project is for the widening of US 401 (alignment -L-) between Legend Rd (SR 2782) and US 70 Garner. The project is located in Garner, Wake County, NC. The project begins approximately 1,600-ft northwest of the Legend Dr./US 401 intersection and extends east/northeast approximately 6,040-ft. SR 1006 (Old Stage Rd) (alignment -Y3-) will also be widened as part of the project. Minimal cut/fill is required for this site.

A geotechnical investigation was conducted in March 2018. 32 hand-augers with the maximum depth of 6-ft and 23 pavement cores were performed throughout the alignment to classify subgrade soils. Dynamic cone penetration testing was also performed in all pavement core locations and in select hand-auger locations. Representative soil samples were collected for visual classification in the field and select samples were submitted for laboratory analysis. The following alignments were investigated:

<u>Line</u>	<u>Station(±)</u>
-L-	17+50 to 96+40
-Y1-	10+36 to 18+10
-Y3-	10+54 to 23+50

It should be noted that during the time of the field investigation the design included widening along US 401 (alignment -L-) from station 17+50 to 94+65 and widening Legend Dr. from station 10+36 to 18+10; however, NCDOT decided to revise the design and start the project construction at station 36+00 along alignment -L- and remove construction along alignment -Y1- on April 2, 2018. For the purpose of this inventory, data gathered along Legend Dr. (Y1) and US 401 (L) (between stations 17+50 to 36+00) will be presented in boring logs located in Appendix B.

Physiography and Geology

The project is located within the Piedmont Physiographic Province and lies within the Raleigh Belt. Along the project corridor the terrain is relatively flat and land use consists mostly of commercial businesses, residential housing and woods. Residual soil in this area comes from weathering of the underlying Injected Gneiss.

Soil Properties

Soils encountered at the project site include mainly residuum, with little roadway embankment and artificial fills.

Residual soils were encountered throughout the project. These soils generally consisted of red-brown to brown, moist, fine to medium-grained, micaceous, silty, clayey sands to sand with silt (A-2-4, A-1-b); and brown to red-brown, moist, micaceous, sandy silt to silty clay (A-4, A-6).

Roadway embankment material was encountered in borings 401H-04, 401H-09, 401H-10, 401H-16, 401H-17, and 401H-19. These soils generally consisted of brown to red-brown, moist, sandy clay (A-6) to brown, moist, silty sand (A-2-4) and gray, silty, gravelly sand (A-1-a).

Artificial fill was encountered in boring 401H-03, This soil consisted of brown to red-brown, moist, silty sand (A-2-4) with clay.

Groundwater Properties

All borings were dry at zero hour but filled in after drilling due to traffic concerns, and therefore groundwater was not encountered during field investigations. There is a monitoring well at the right side of alignment L, between station 57+50 and 58+00, which is shown on plan view.

Areas of Special Geotechnical Interest

- 1) **Highly Plastic Soils:** Moderately to highly plastic soils were encountered in the majority of the laboratory samples. Atterberg limit tests for six of the eight samples had plasticity indices that ranged from 23 to 42. The following sections were found to contain highly plastic soils:

<u>Line</u>	<u>Stations (±)</u>	<u>Offsets</u>
-L-	36+00 to 40+00	LT
-L-	40+00 to 43+00	RT
-L-	46+00 to 48+00	LT
-L-	50+00 to 52+00	LT
-L-	54+00 to 56+00	LT
-L-	61+50 to 64+00	LT
-L-	68+50 to 91+00	LT
-L-	72+00 to 77+00	RT
-L-	79+00 to 96+40	RT
-L-	93+50 to 94+65	LT
-Y3-	16+00 to 23+50	LT

- 2) Micaceous Soils: Micaceous material was encounter throughout the project, and therefore the following sections were found to contain micaceous soils:

<u>Line</u>	<u>Stations (±)</u>	<u>Offsets</u>
-L-	36+00 to 94+65	LT to RT
-Y3-	10+54 to 23+50	LT to RT

- 3) Artificial Fill: Artificial fill is countered only in the boring 401H-03 at station 40+50. It is started from depth 0.5 ft to 1.0 ft at the very left side of the cross section 40+50. This boring log then terminated by auger refusal at depth 1.0 ft.

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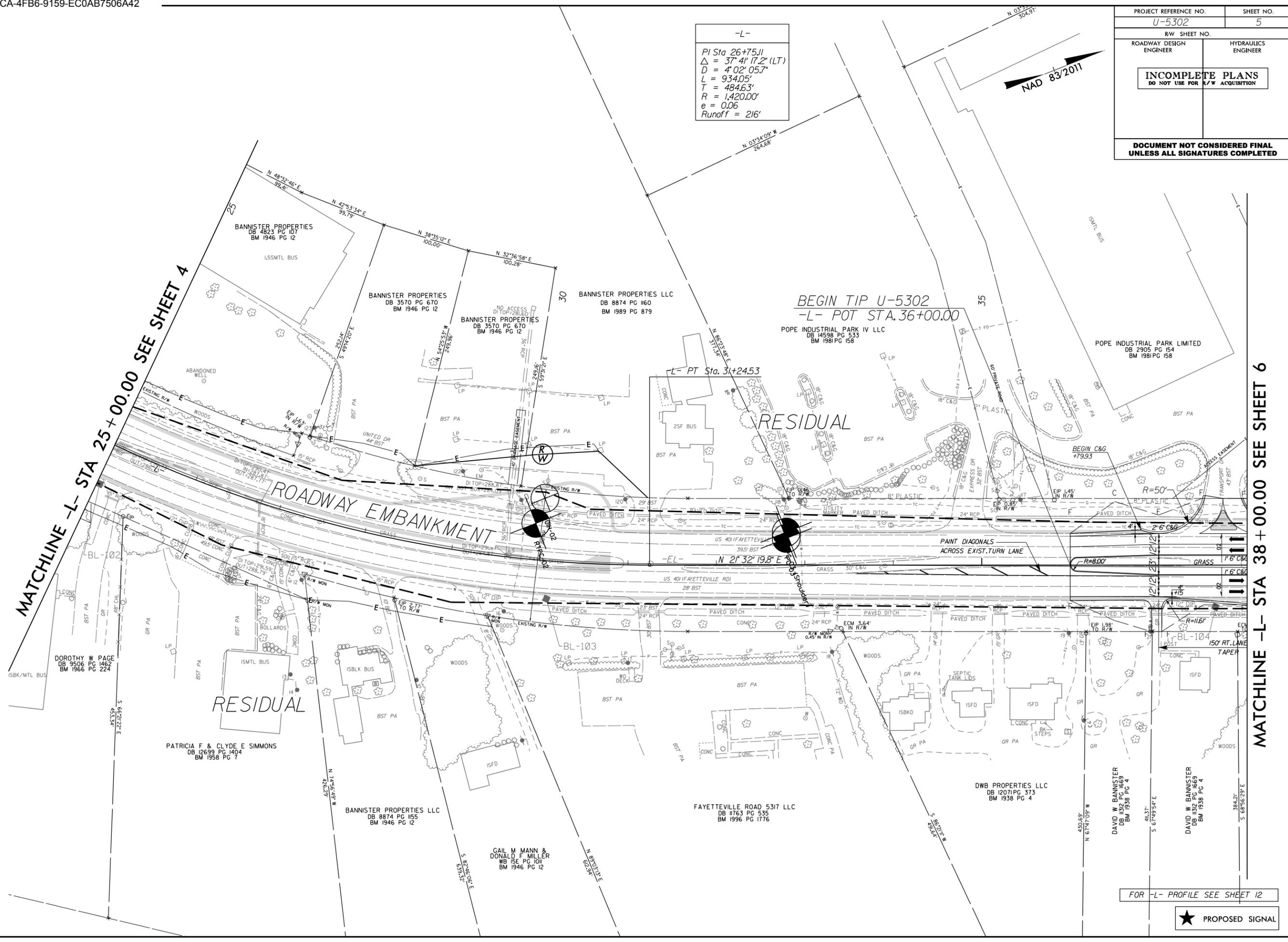
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INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			

-L-

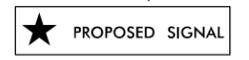
PI Sta 26+75.11
 $\Delta = 37' 41" 17.2" (LT)$
 $D = 4' 02" 05.7"$
 $L = 934.05'$
 $T = 484.63'$
 $R = 1,420.00'$
 $e = 0.06$
 Runoff = 216'



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FOR -L- PROFILE SEE SHEET 12



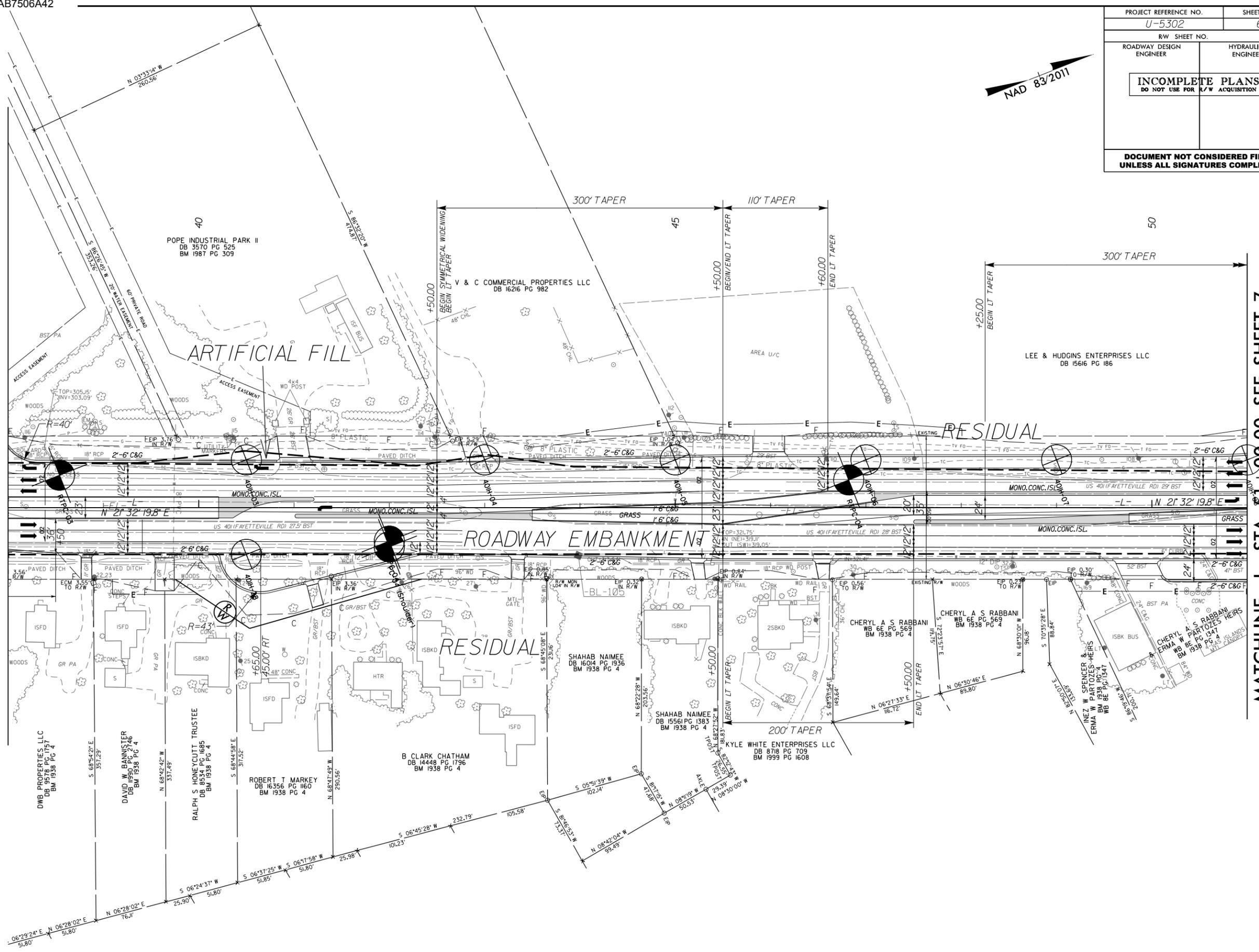
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ROADWAY DESIGN ENGINEER			
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



MATCHLINE -L- STA 38+00.00 SEE SHEET 5

MATCHLINE -L- STA 51+00.00 SEE SHEET 7



FOR -L- PROFILE SEE SHEET 12

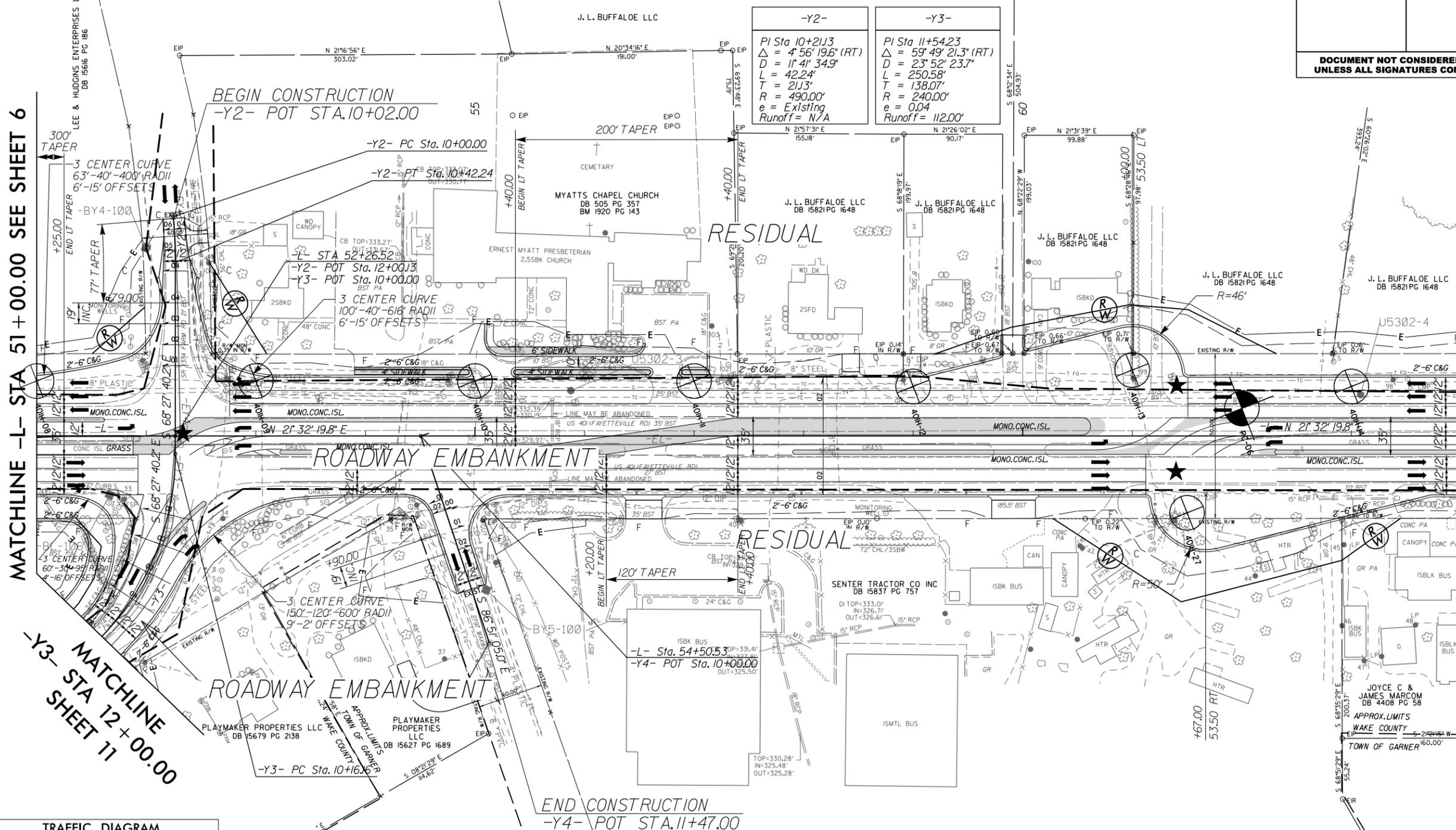
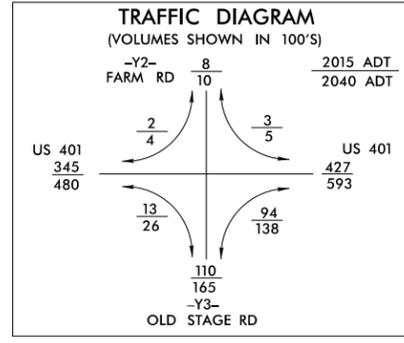
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PROJECT REFERENCE NO. U-5302		SHEET NO. 7	
RW SHEET NO.		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



MATCHLINE -L- STA 51+00.00 SEE SHEET 6

MATCHLINE -L- STA 64+00.00 SEE SHEET 8



-Y2-	-Y3-
PI Sta 10+21.3 $\Delta = 4' 56'' 19.6''$ (RT) $D = 11' 41'' 34.9''$ $L = 42.24'$ $T = 21.13'$ $R = 490.00'$ $e = \text{Existing}$ $\text{Runoff} = \text{N/A}$	PI Sta 11+54.23 $\Delta = 59' 49'' 21.3''$ (RT) $D = 23' 52'' 23.7''$ $L = 250.58'$ $T = 138.07'$ $R = 240.00'$ $e = 0.04$ $\text{Runoff} = 112.00'$

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FOR -L- PROFILE SEE SHEET 12
FOR -Y3- PROFILE SEE SHEET 15



PROJECT REFERENCE NO. U-5302		SHEET NO. 8	
RW SHEET NO.		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

WATER II TOP=342 12" CCP INV=3 IN (NE)=3 IN (SW)=3 24" OUT(NW)=3	2" DIA. 1" WITH TOP=343.23 24" IN=338.28 12" IN=339.28 18" OUT=337.95	2" DIA. 1" WITH TOP=343.86 24" IN=338.86 12" IN=339.86 18" OUT=337.95
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TOP=344.45 IN=341.42 OUT=341.42	TOP=344.66 IN=341.41 OUT=341.39	TOP=343.86 IN=340.86 OUT=340.86	TOP=343.86 IN=340.86 OUT=340.86
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TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86
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TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86	TOP=343.86 IN=339.86 OUT=338.86
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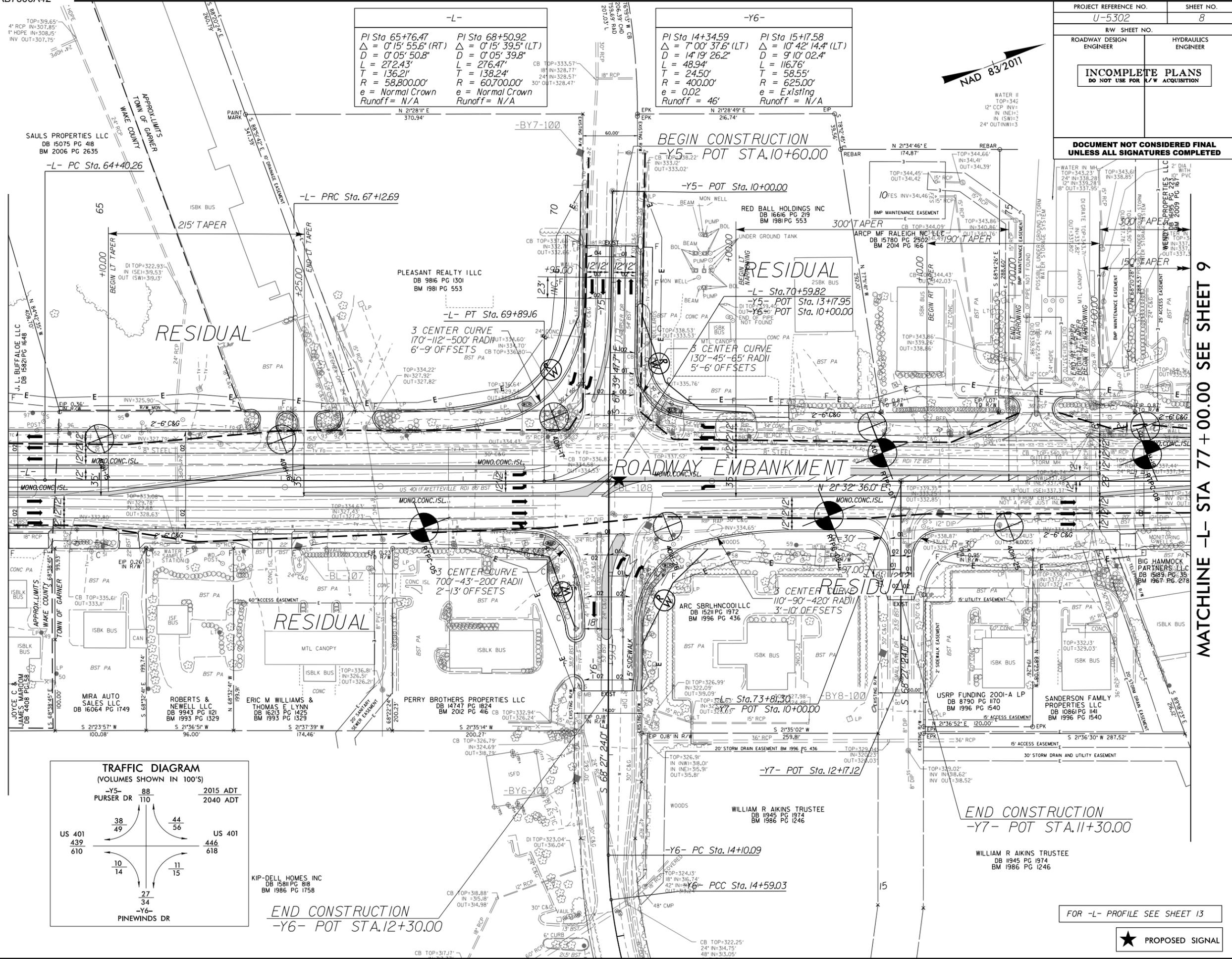
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FOR -L- PROFILE SEE SHEET 13



PROPOSED SIGNAL



-L-

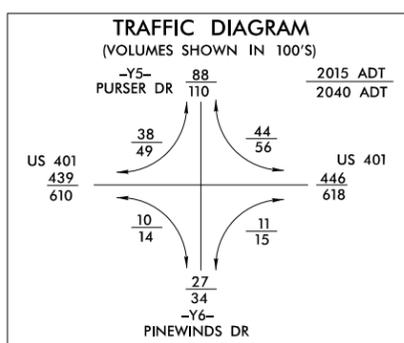
PI Sta 65+76.47 Δ = 0'15" 55.6" (RT) D = 0'05' 50.8" L = 272.43' T = 136.21' R = 58,800.00' e = Normal Crown Runoff = N/A	PI Sta 68+50.92 Δ = 0'15' 39.5" (LT) D = 0'05' 39.8" L = 276.47' T = 138.24' R = 60,700.00' e = Normal Crown Runoff = N/A
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-Y6-

PI Sta 14+34.59 Δ = 7'00" 37.6" (LT) D = 14'19' 26.2" L = 48.94' T = 24.50' R = 400.00' e = 0.02 Runoff = 46'	PI Sta 15+17.58 Δ = 10'42" 14.4" (LT) D = 9'10' 02.4" L = 116.76' T = 58.55' R = 625.00' e = Existing Runoff = N/A
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MATCHLINE -L- STA 64 + 00.00 SEE SHEET 7

MATCHLINE -L- STA 77 + 00.00 SEE SHEET 9



END CONSTRUCTION
-Y6- POT STA.12+30.00

END CONSTRUCTION
-Y7- POT STA.11+30.00

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PROJECT REFERENCE NO. U-5302		SHEET NO. 9	
RW SHEET NO. ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
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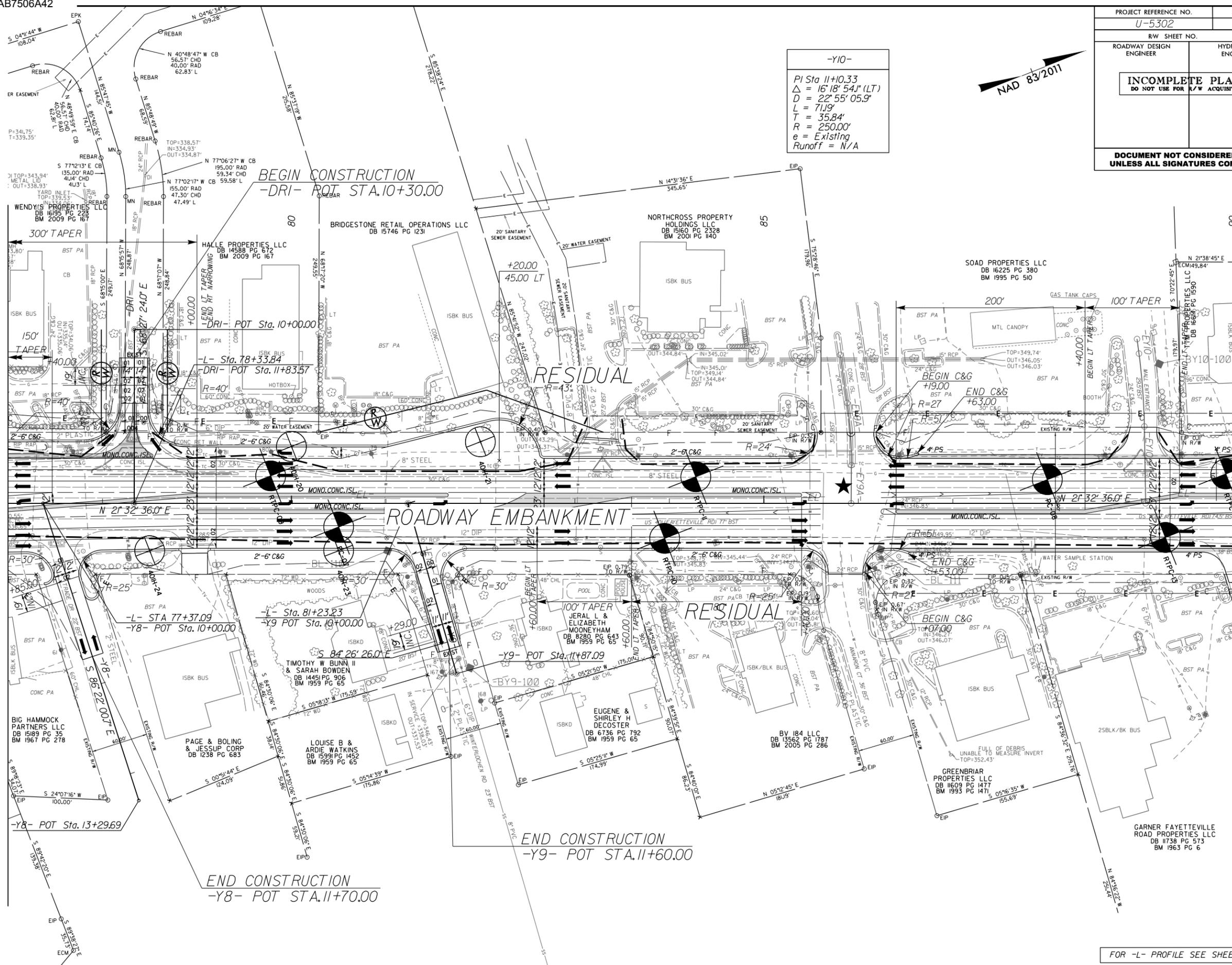
-Y10-

Pi Sta 11+10.33
 $\Delta = 16' 18" 54" (LT)$
 $D = 22' 55" 05.9"$
 $L = 71.9'$
 $T = 35.84'$
 $R = 250.00'$
 $e = \text{Existing}$
 $\text{Runoff} = N/A$



MATCHLINE -L- STA 77 + 00.00 SEE SHEET 8

MATCHLINE -L- STA 90 + 00.00 SEE SHEET 10



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FOR -L- PROFILE SEE SHEETS 13



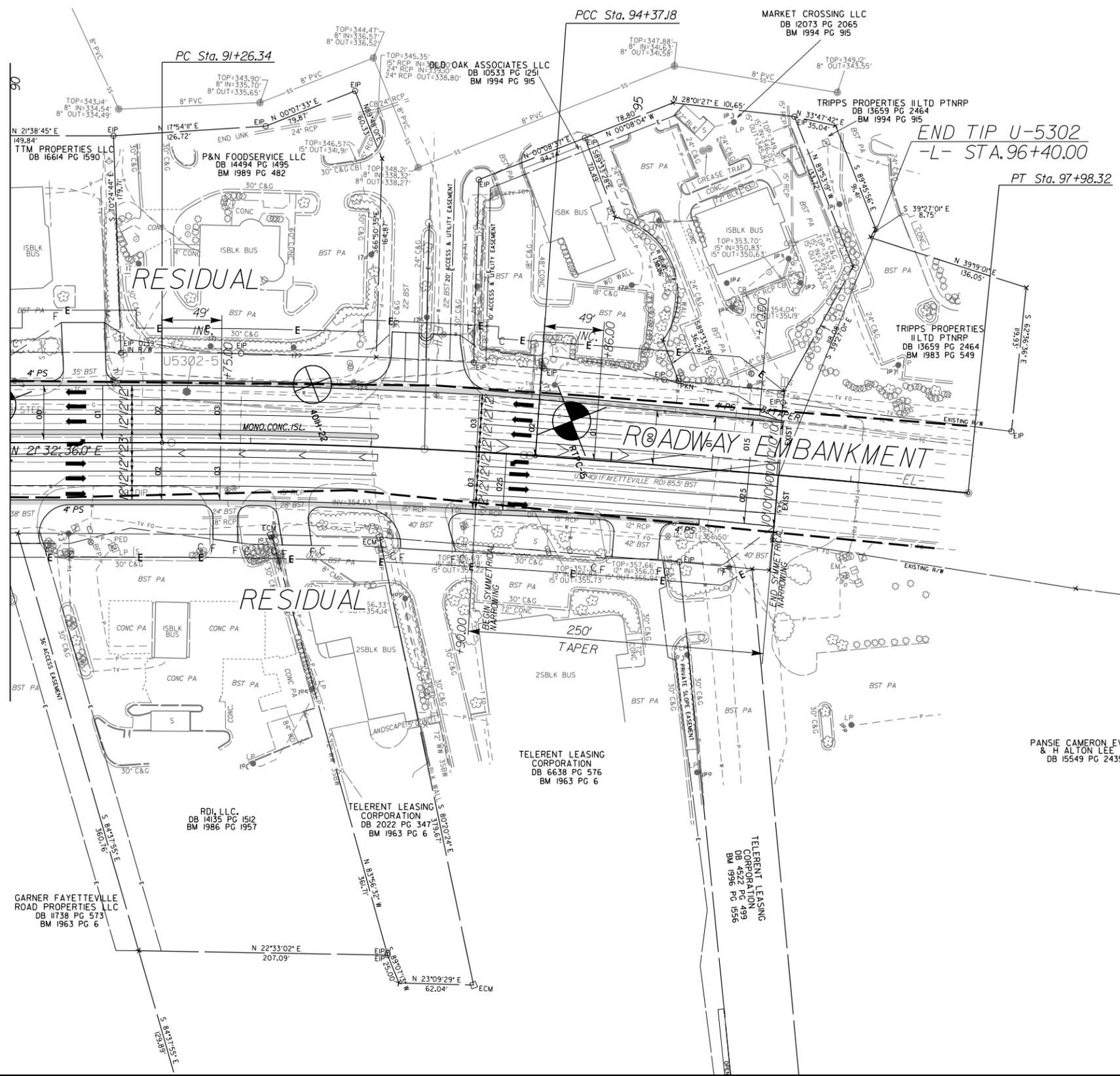
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PROJECT REFERENCE NO.		SHEET NO.	
U-5302		10	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER		
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



-L-	
PI Sta 92+81.83	PI Sta 96+17.77
$\Delta = 4' 02' 51.7''$ (RT)	$\Delta = 1' 41' 55.9''$ (RT)
$D = 1' 18' 07.8''$	$D = 0' 28' 13.5''$
$L = 310.84'$	$L = 361.14'$
$T = 155.49'$	$T = 180.58'$
$R = 4,400.00'$	$R = 12,180.00'$
$e = 3\%$	$e = NC$
Runoff = 147'	Runoff = N/A

MATCHLINE -L- STA 90 + 00.00 SEE SHEET 9



BM 3
ELEV=354.00'
-BL- STA. 106+09.26 286' LEFT
"BENCH TIE" NAIL SET IN 12" OAK TREE

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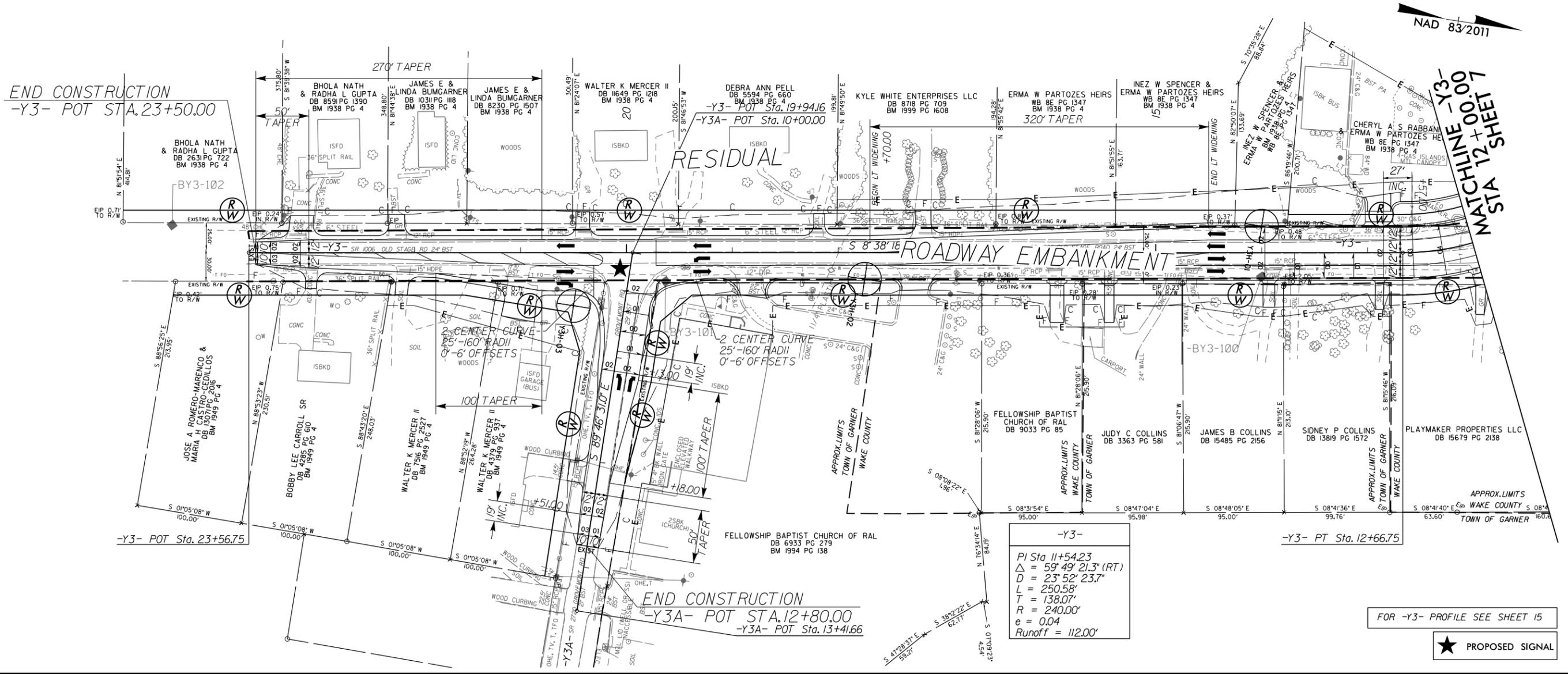
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PROJECT REFERENCE NO.		SHEET NO.	
U-5302		11	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
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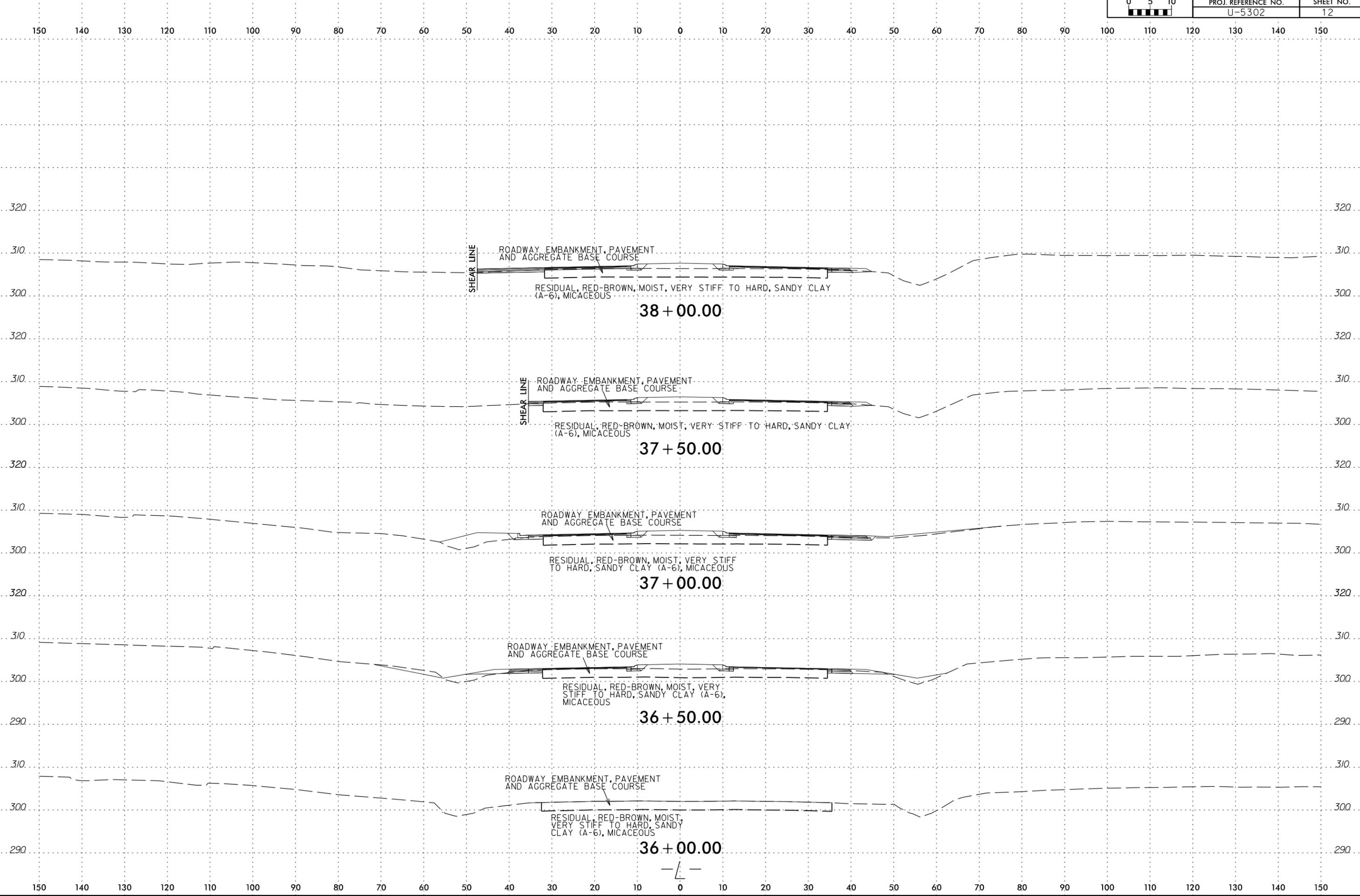


-Y3-
 PI Sta 11+54.23
 $\Delta = 59^{\circ} 49' 21.3''$ (RT)
 $D = 23^{\circ} 52' 23.7''$
 $L = 250.58'$
 $T = 138.07'$
 $R = 240.00'$
 $e = 0.04$
 Runoff = 112.00'

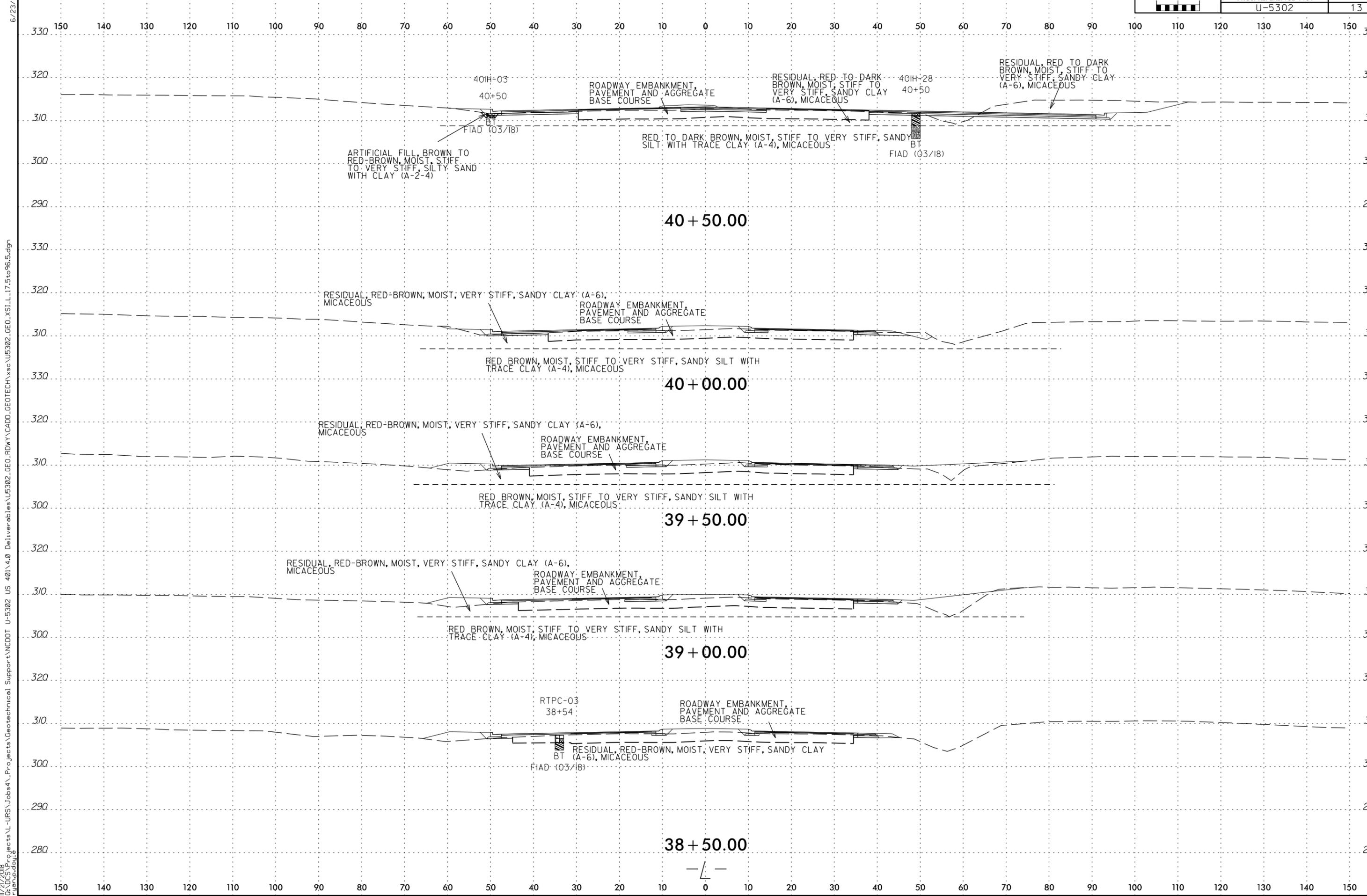
FOR -Y3- PROFILE SEE SHEET 15

★ PROPOSED SIGNAL

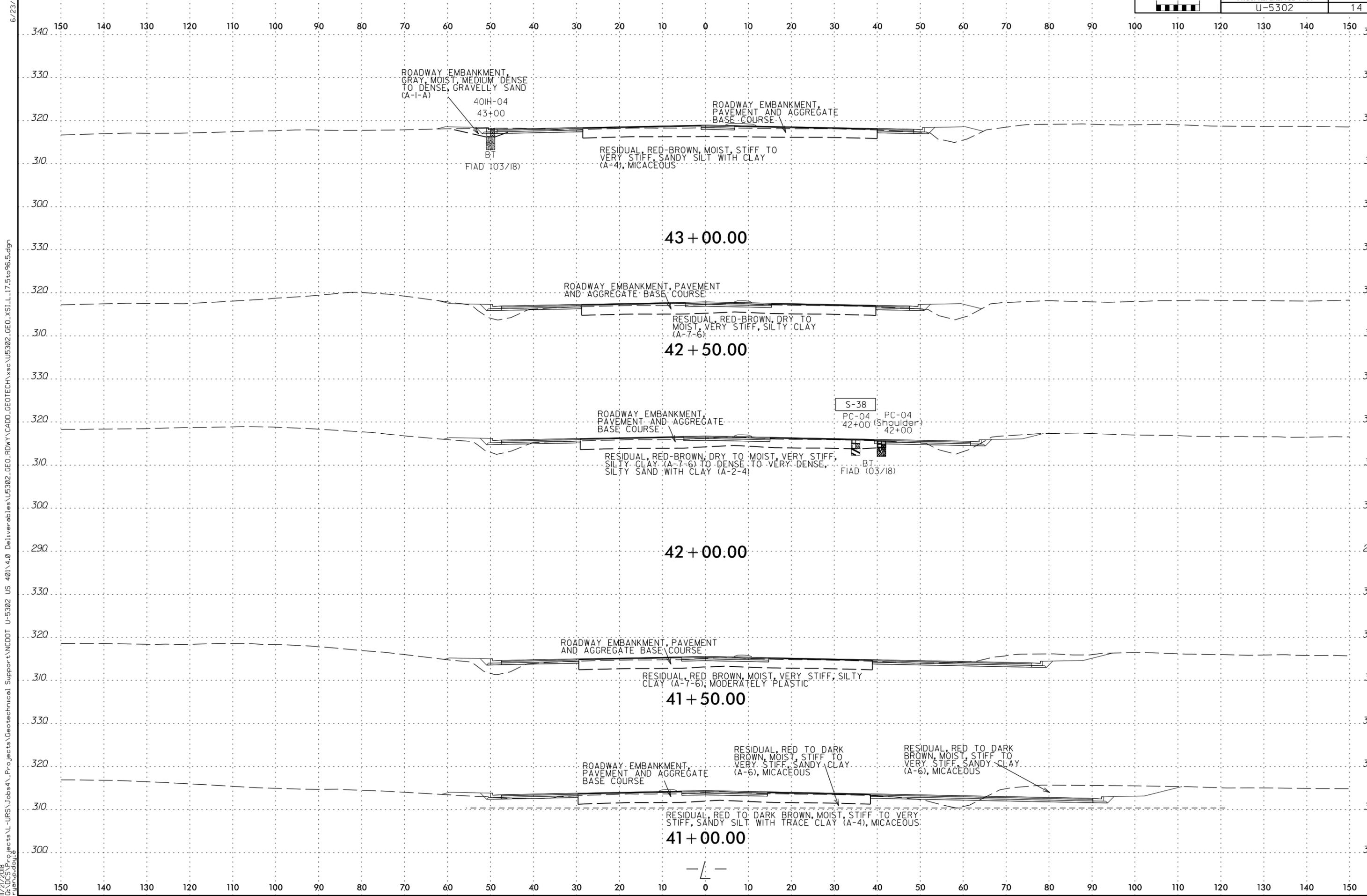
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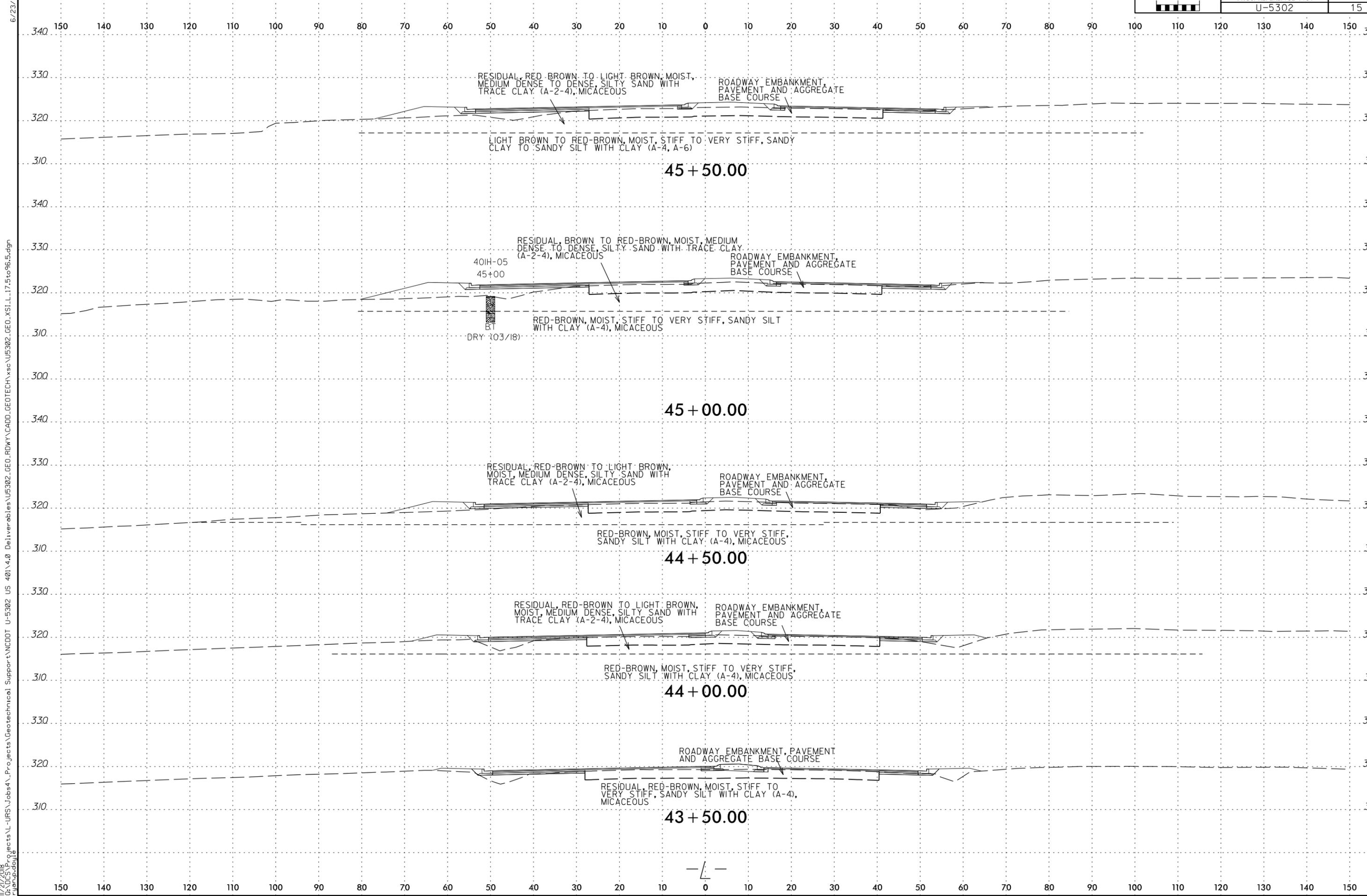


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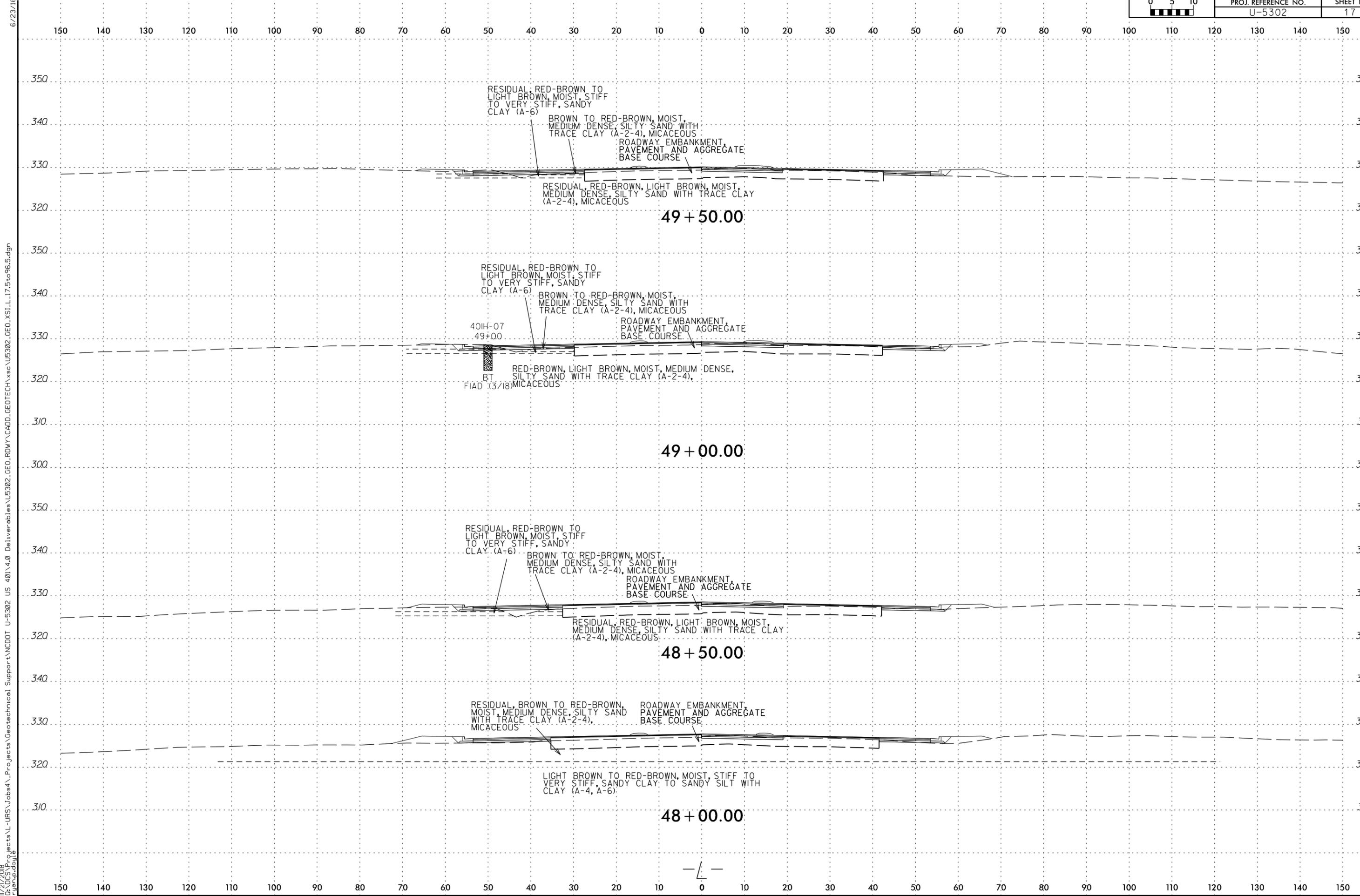
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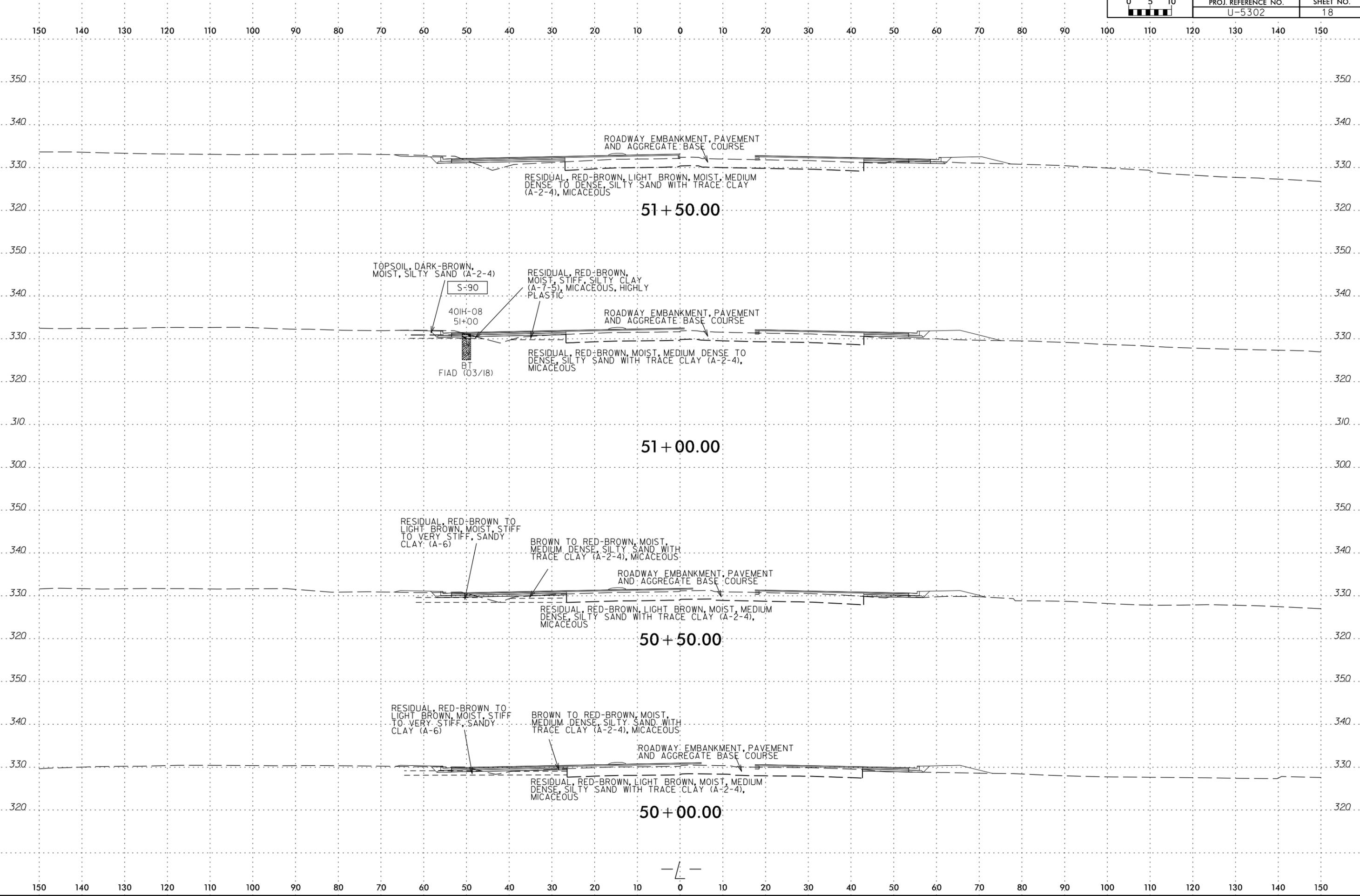
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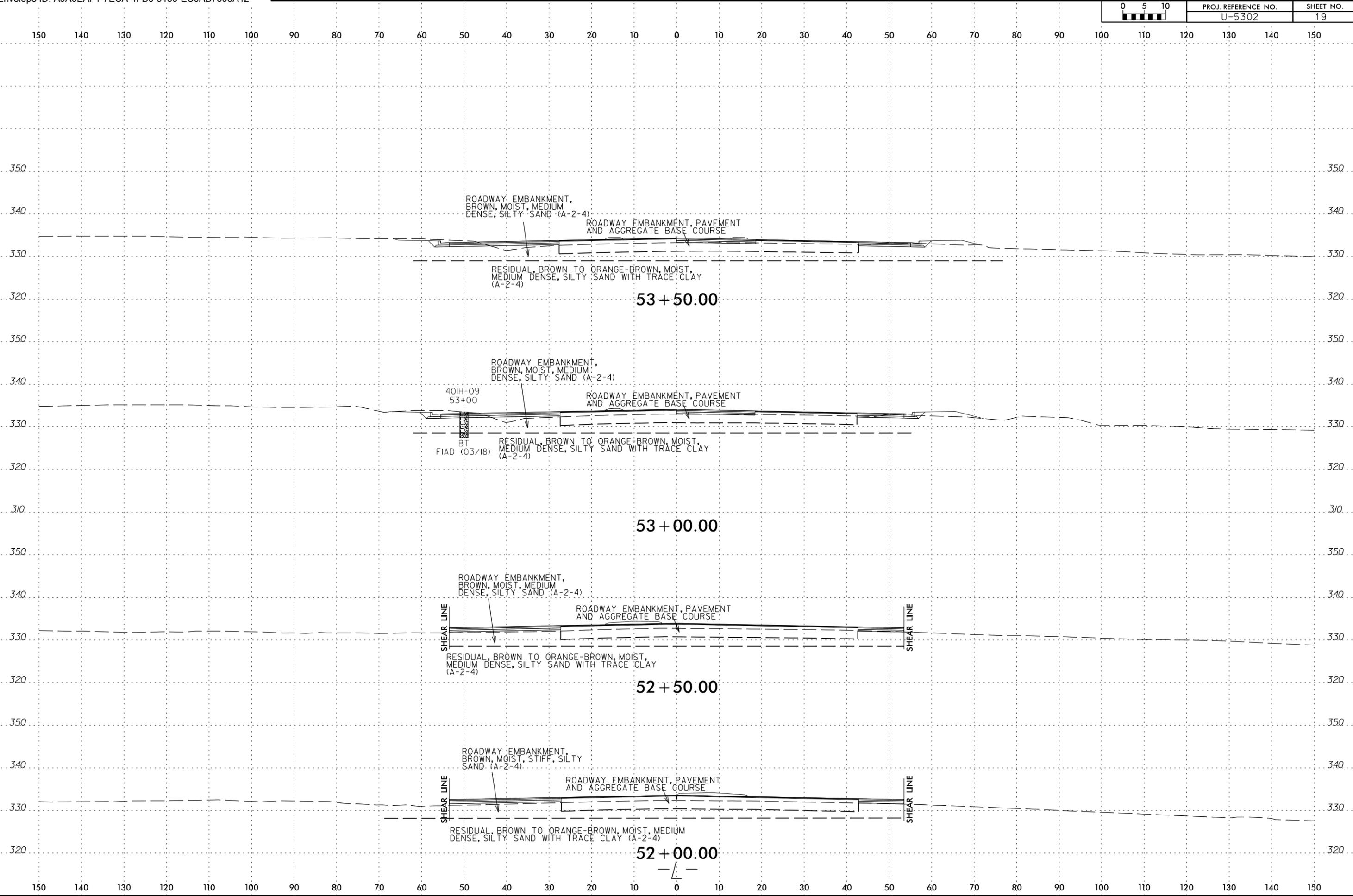
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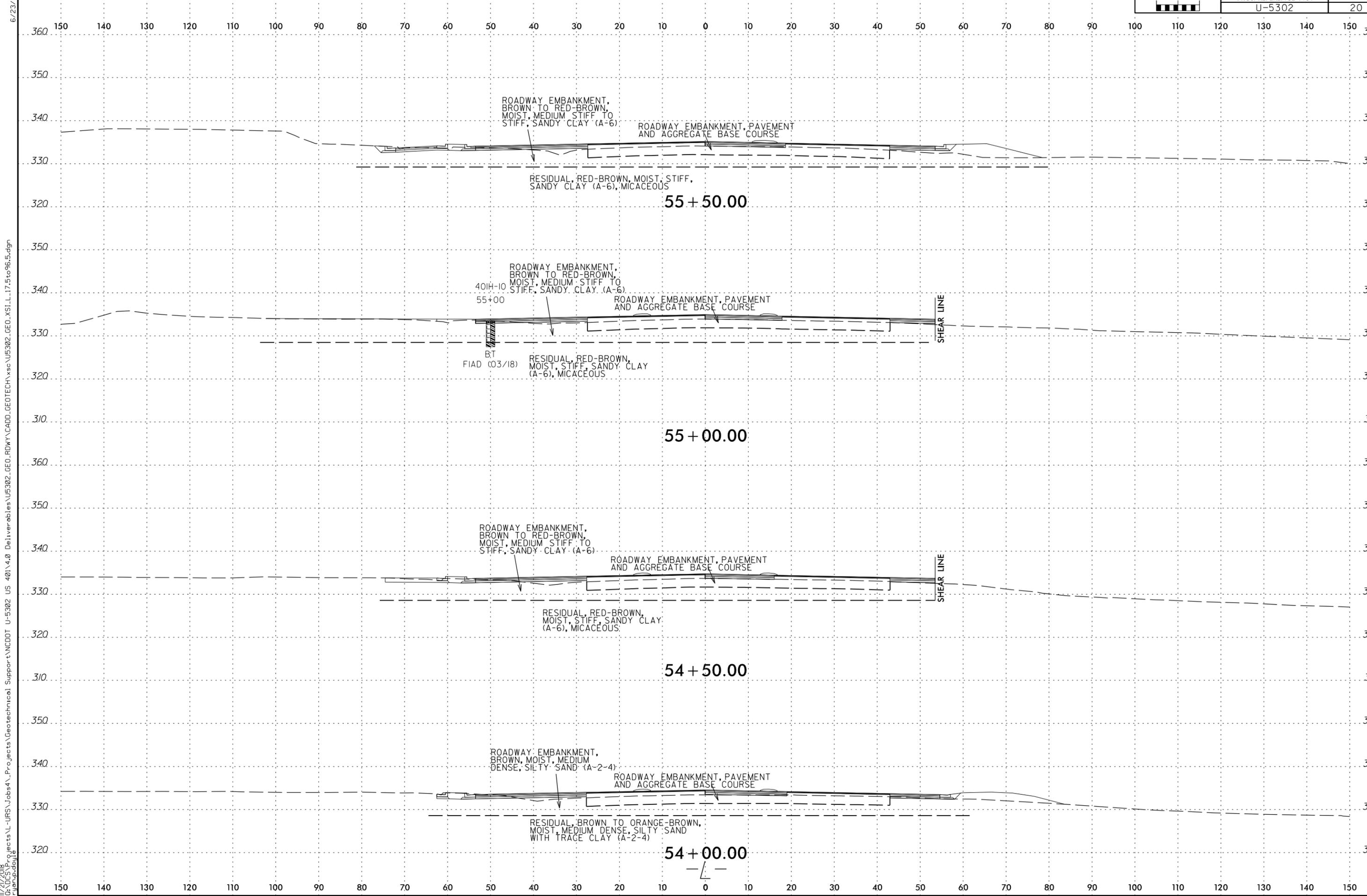
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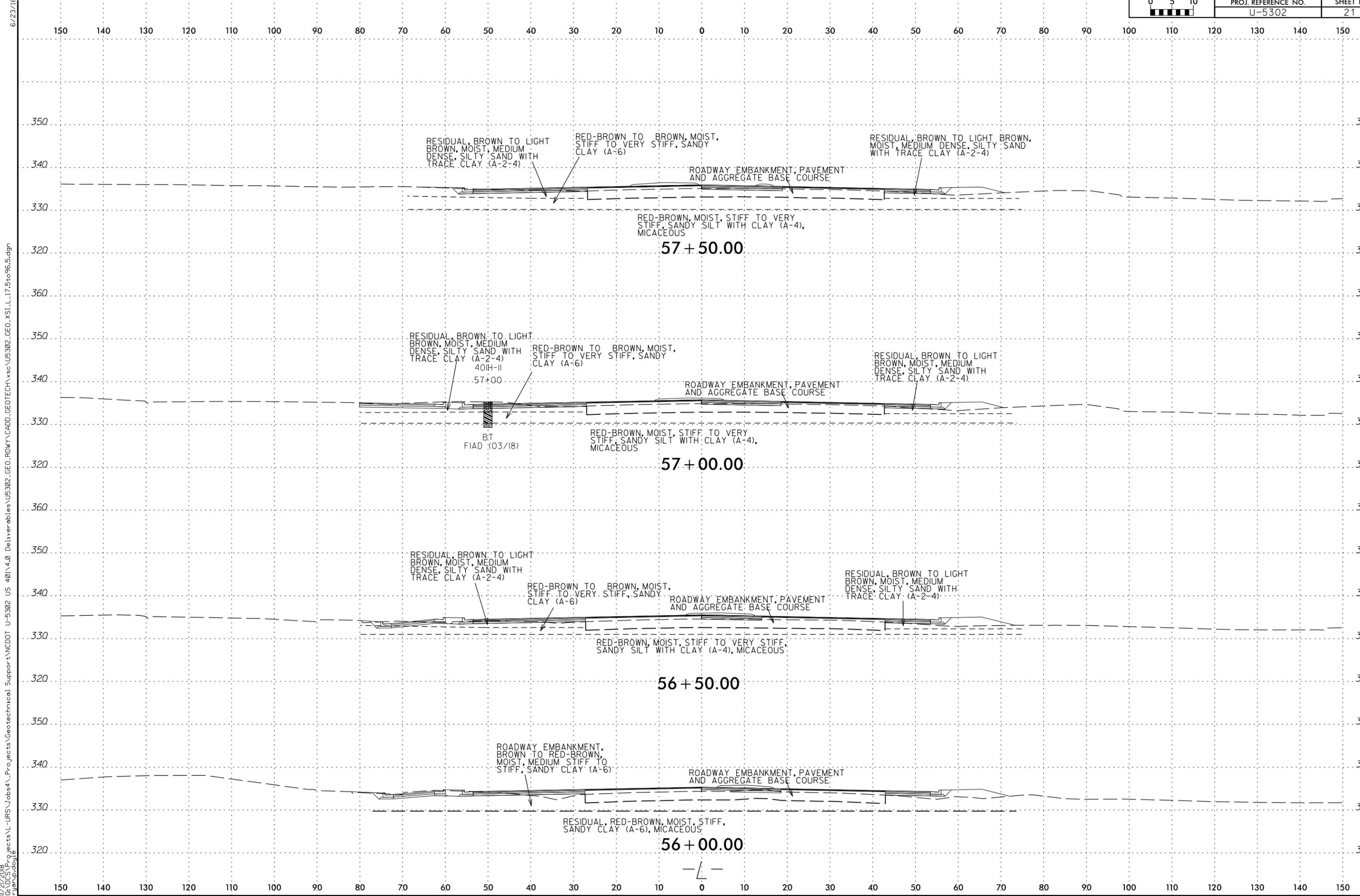
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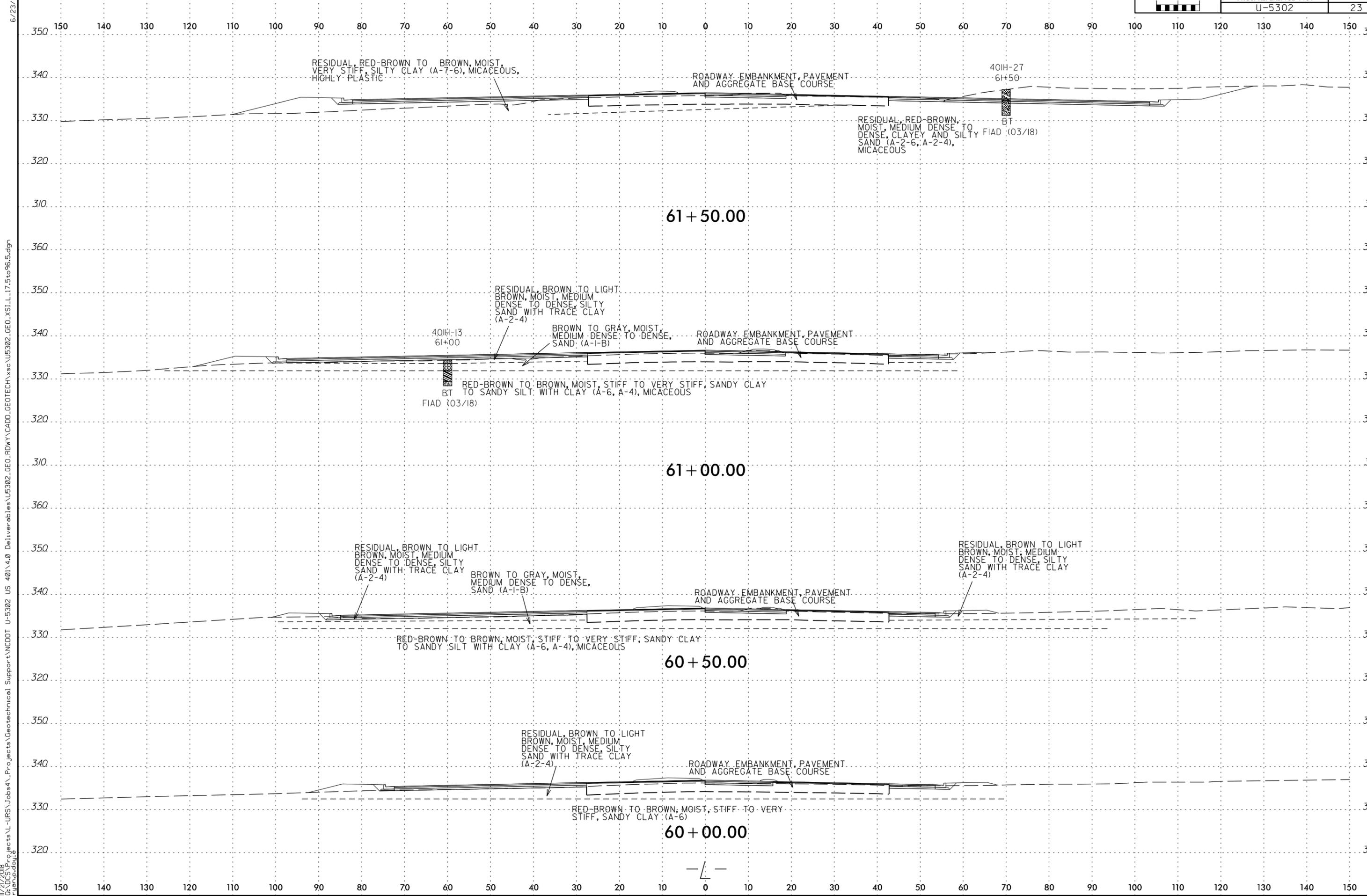




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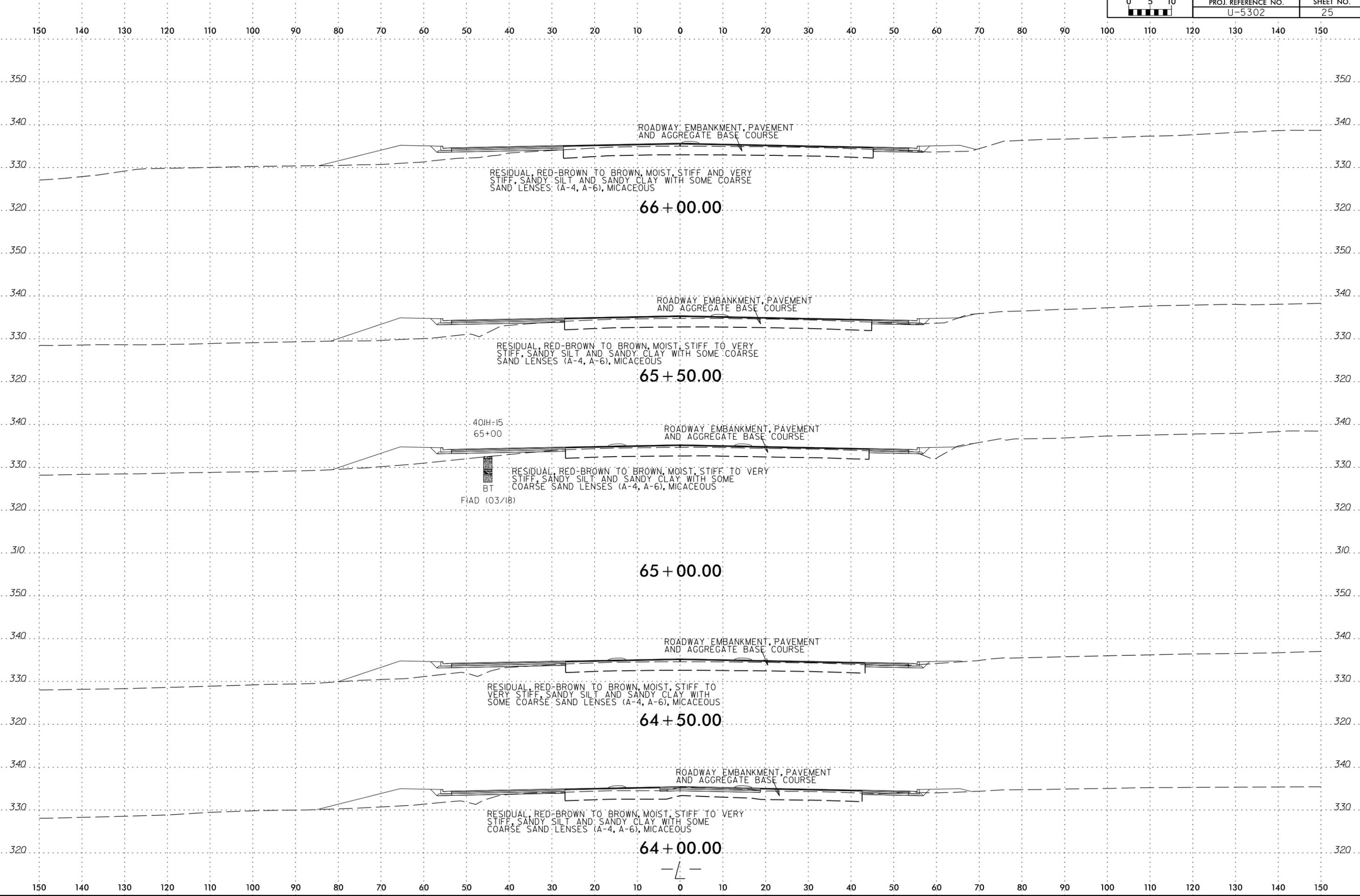


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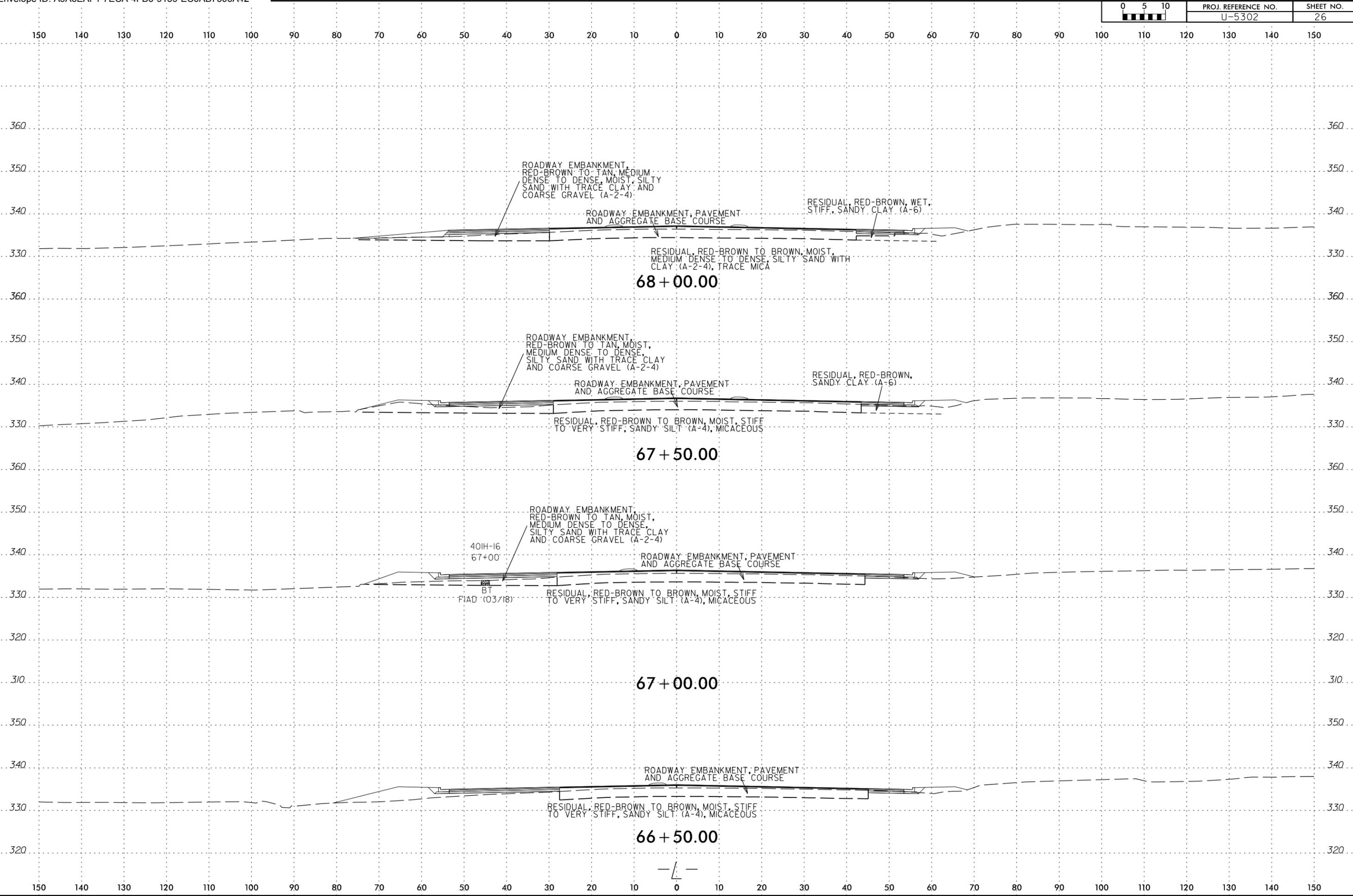
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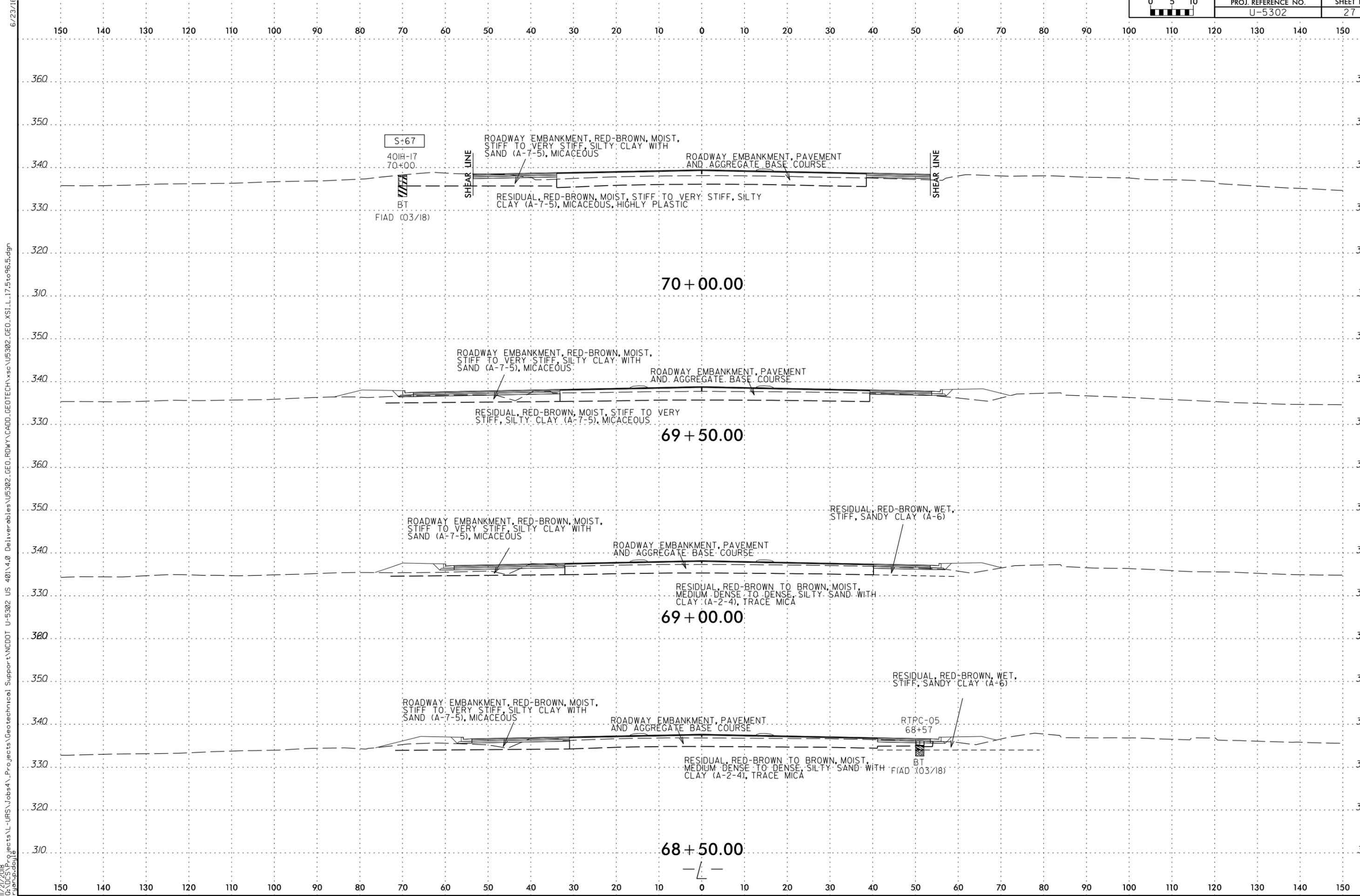


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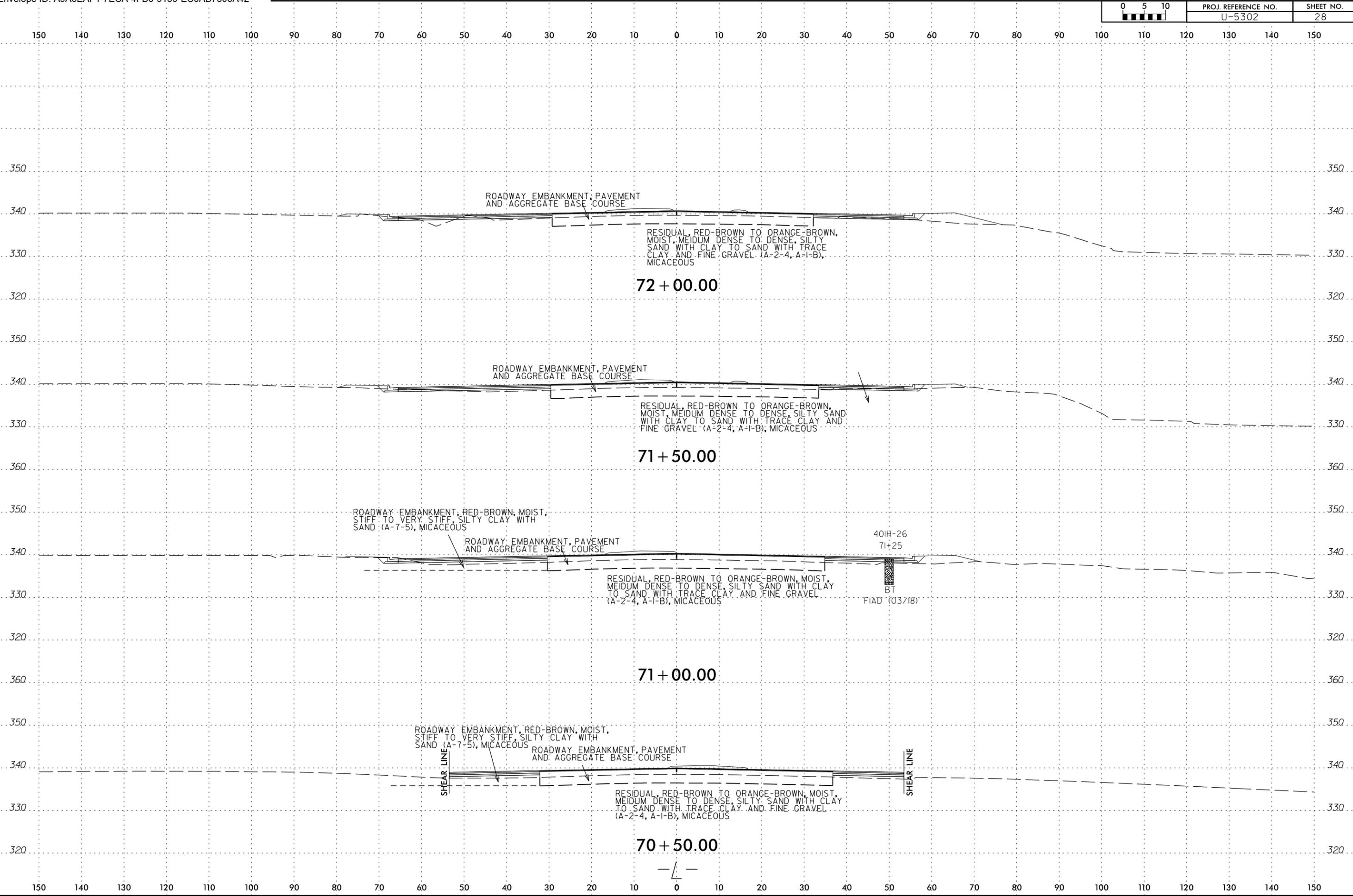


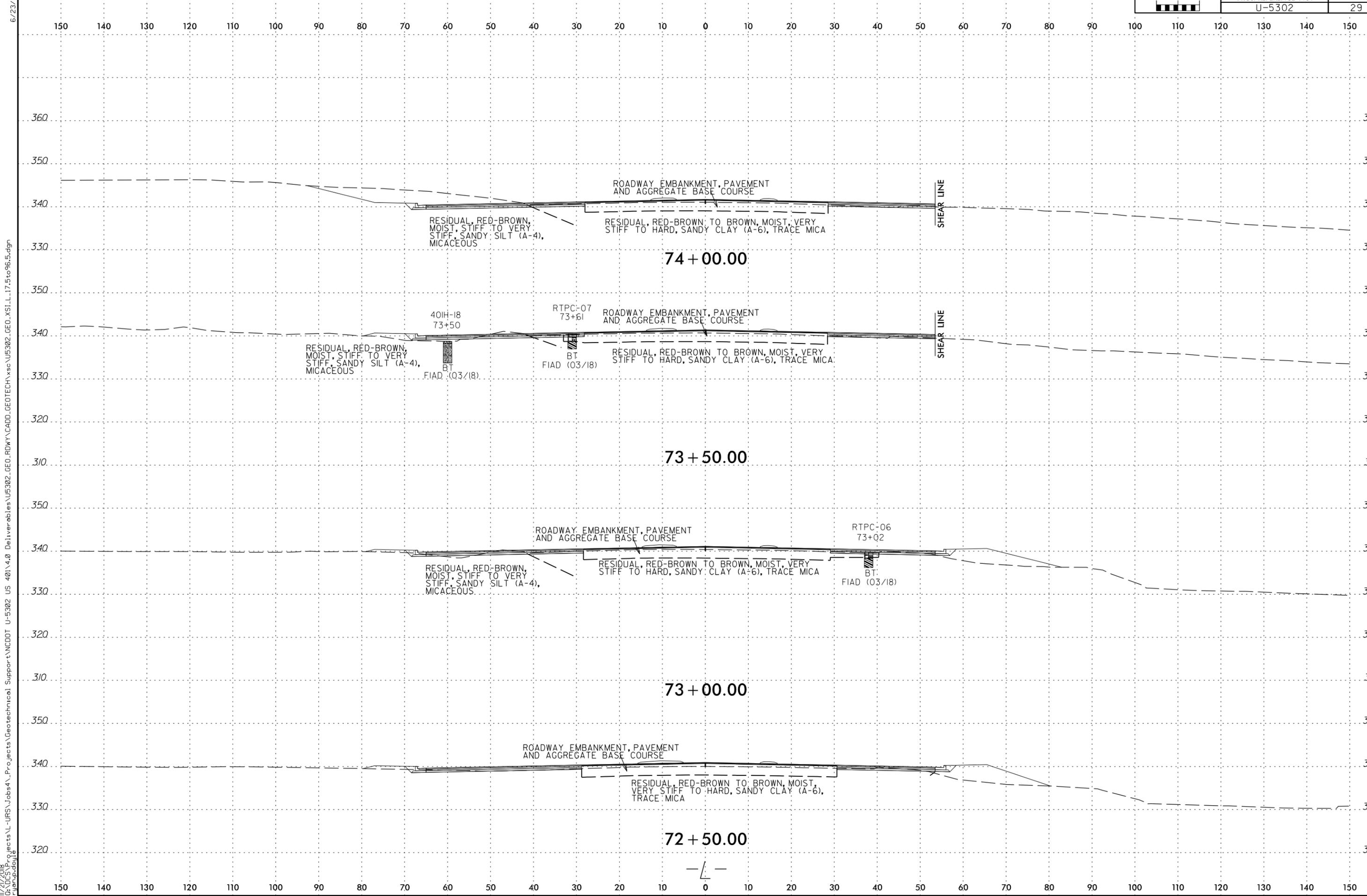
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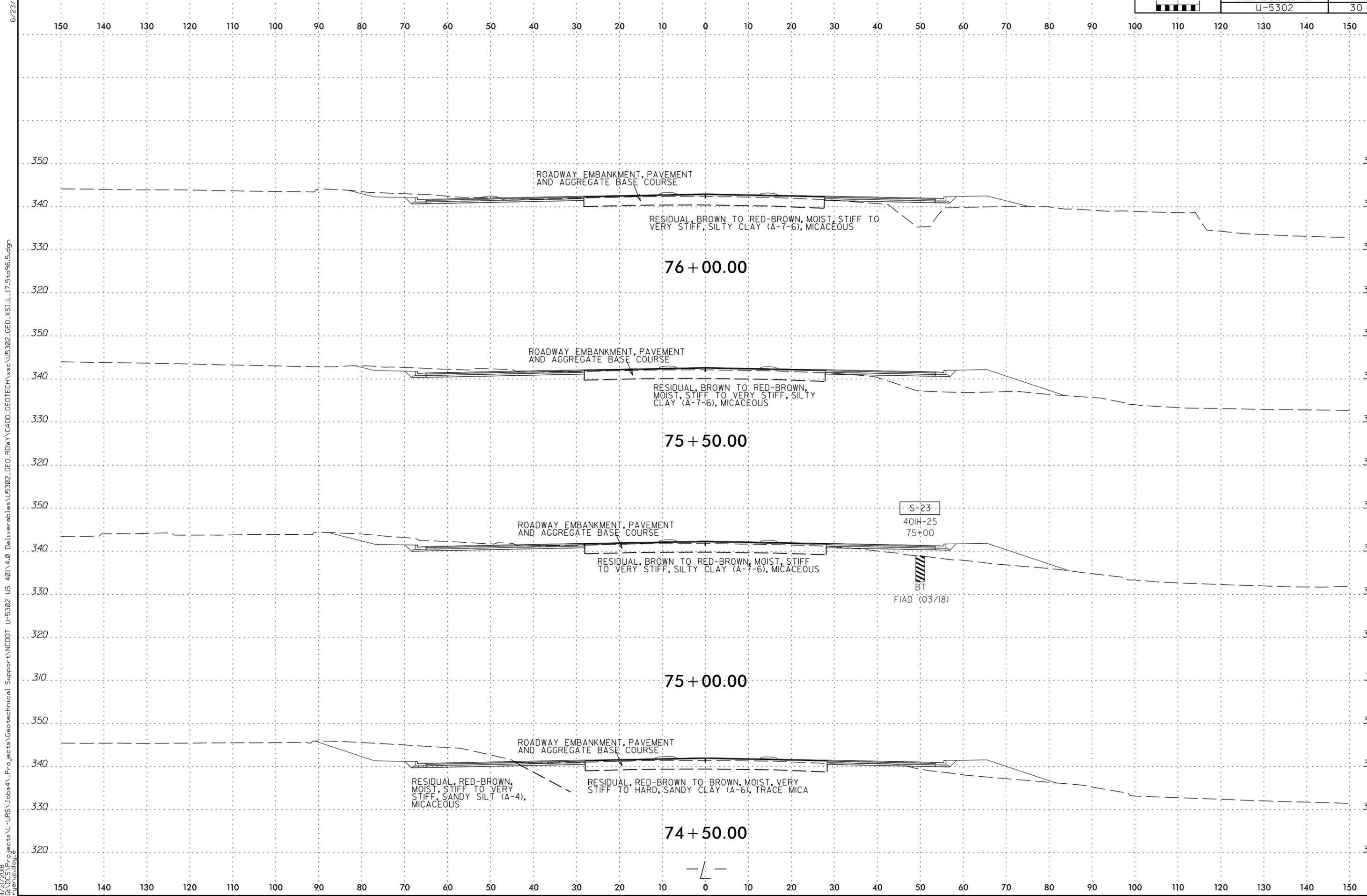
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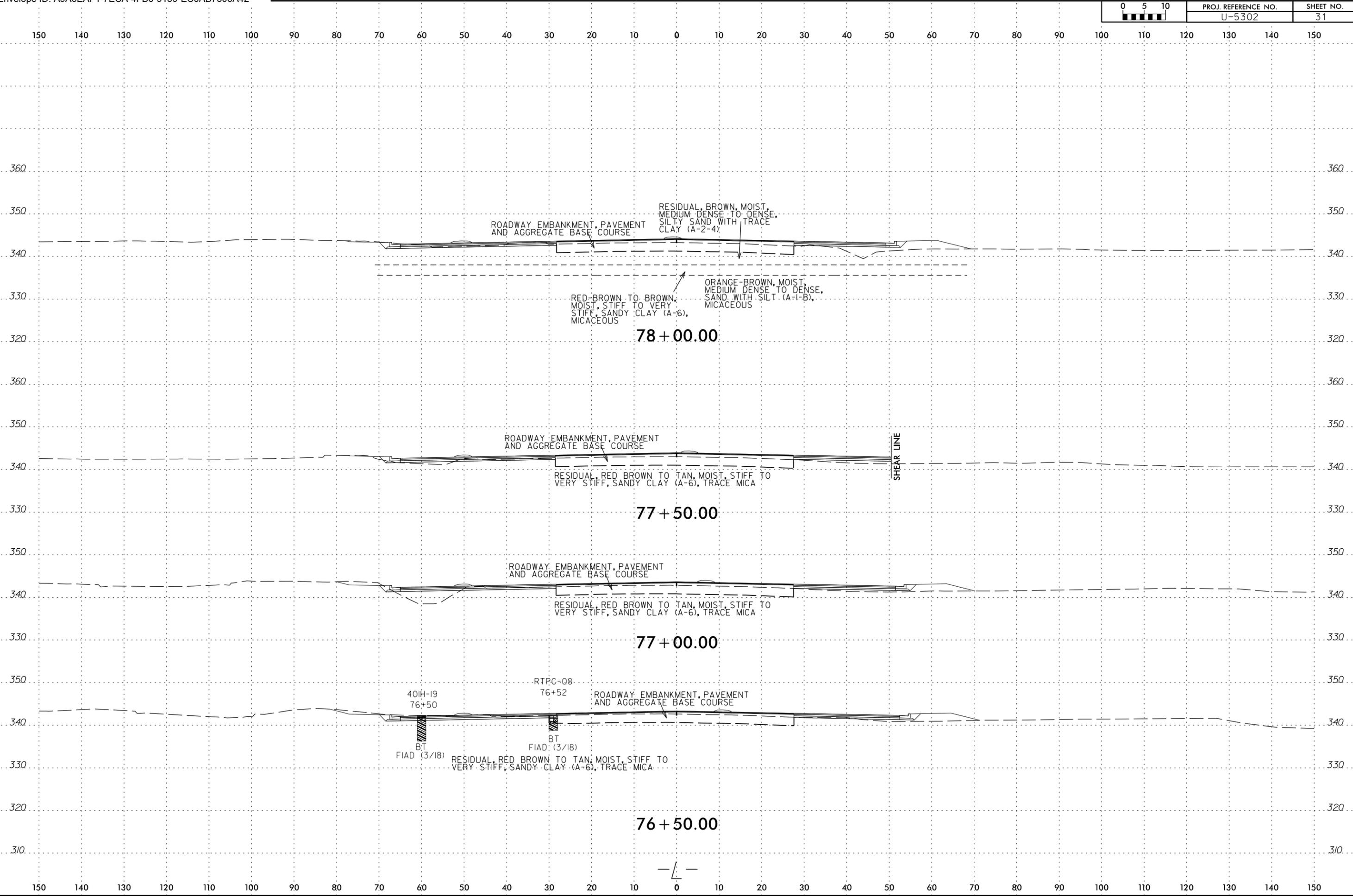


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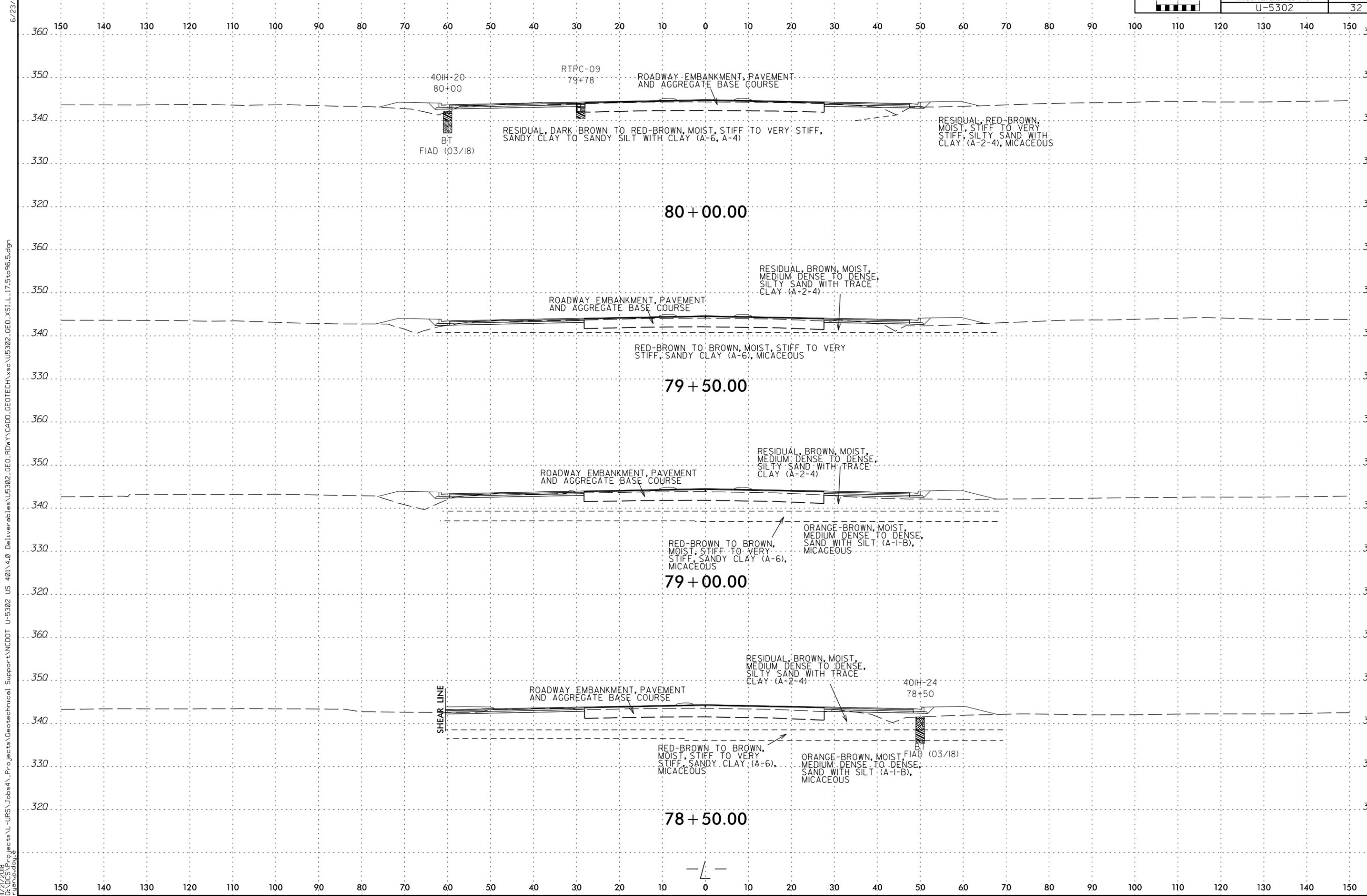


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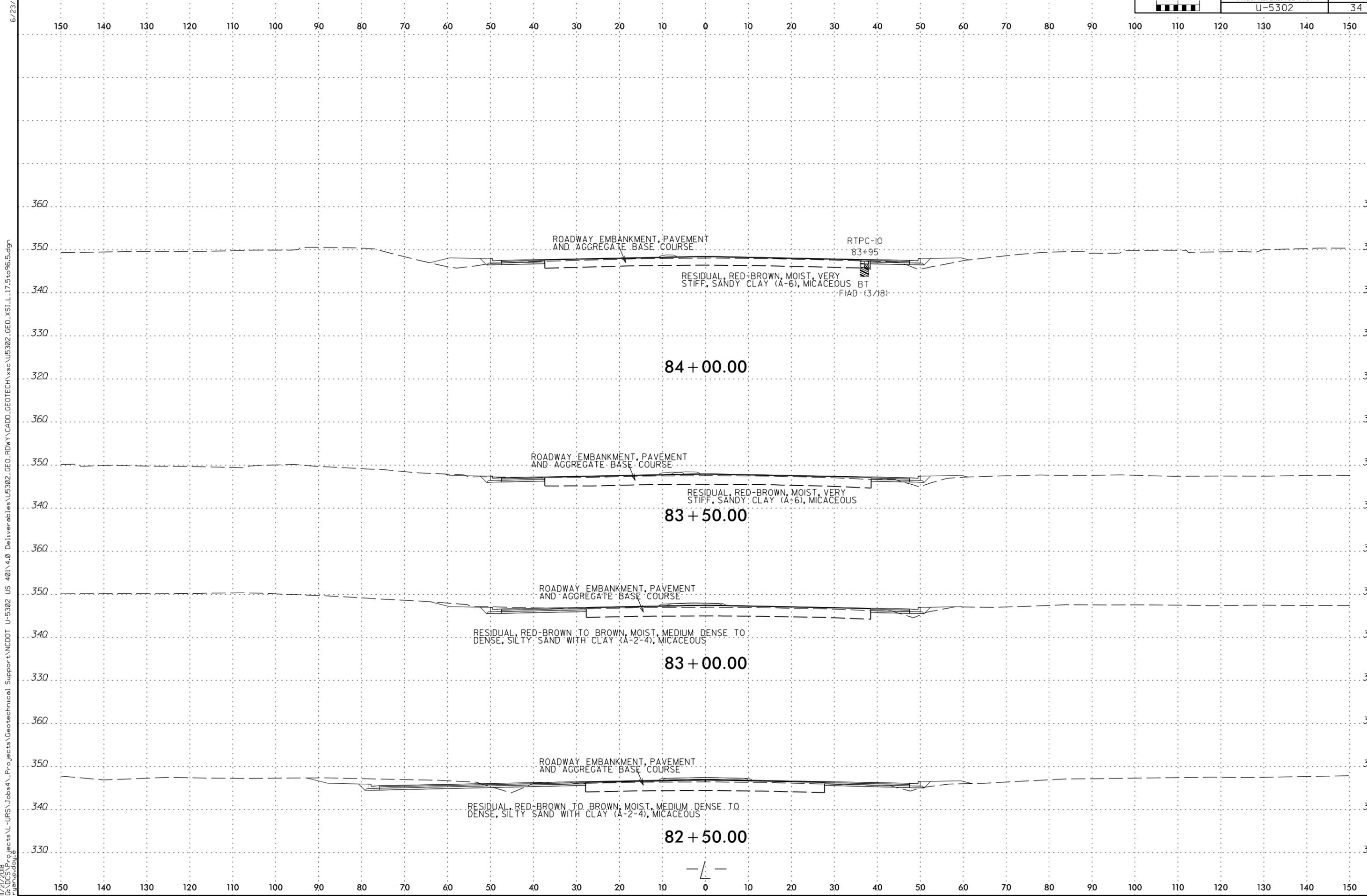
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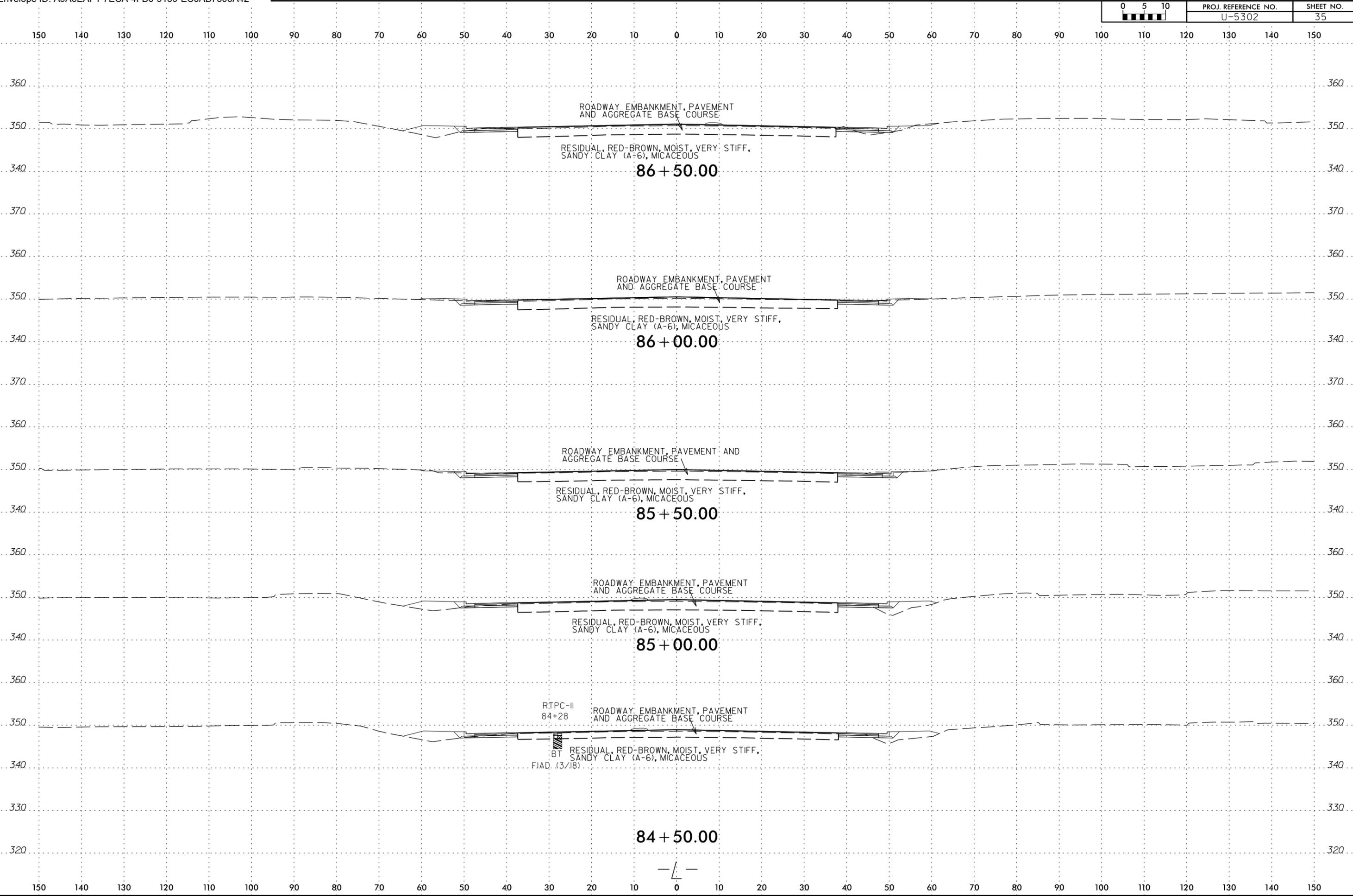


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84 + 50.00

85 + 00.00

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86 + 50.00

ROADWAY EMBANKMENT, PAVEMENT AND AGGREGATE BASE COURSE

RESIDUAL, RED-BROWN, MOIST, VERY STIFF, SANDY CLAY (A-6), MICACEOUS

ROADWAY EMBANKMENT, PAVEMENT AND AGGREGATE BASE COURSE

RESIDUAL, RED-BROWN, MOIST, VERY STIFF, SANDY CLAY (A-6), MICACEOUS

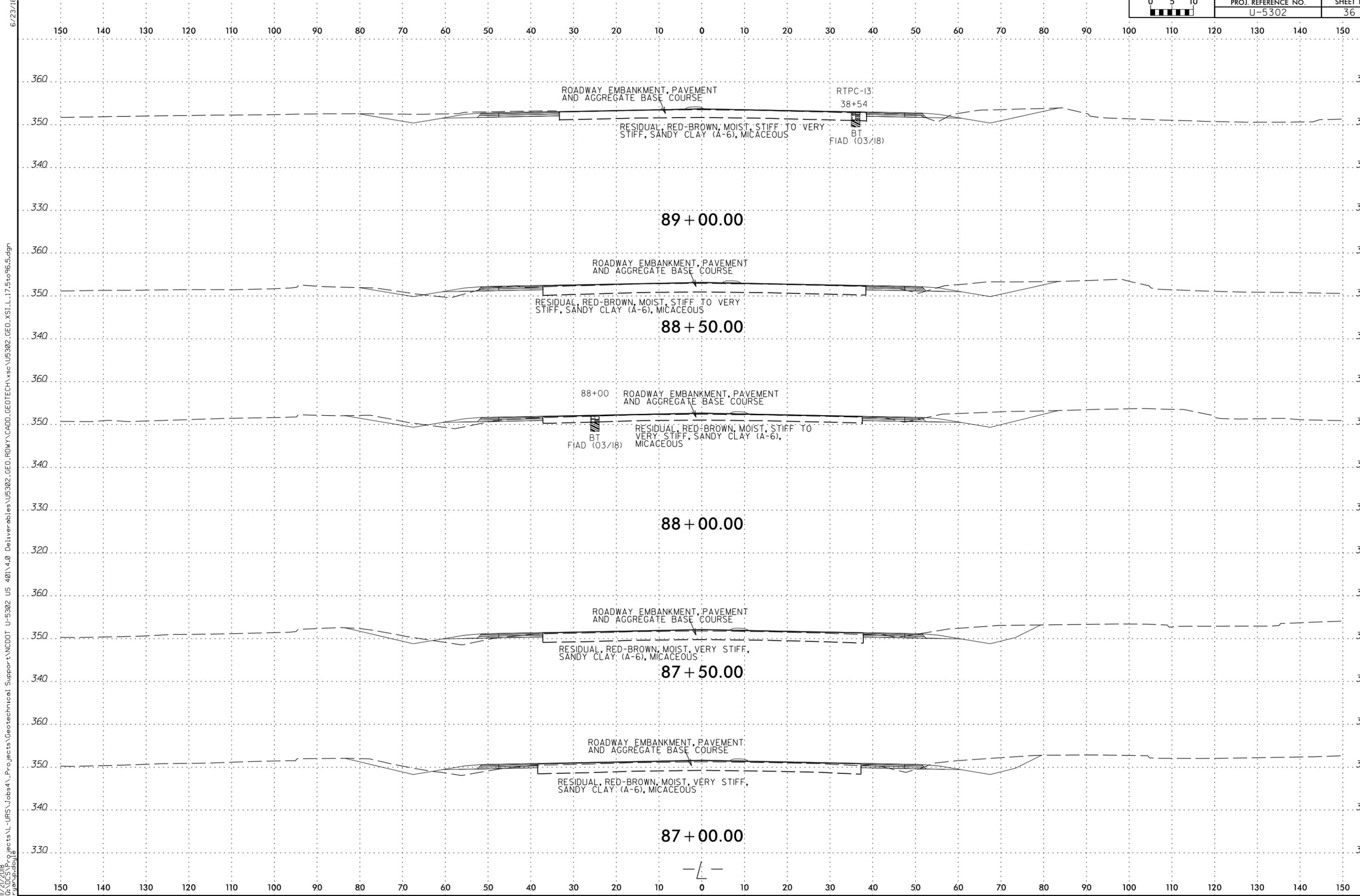
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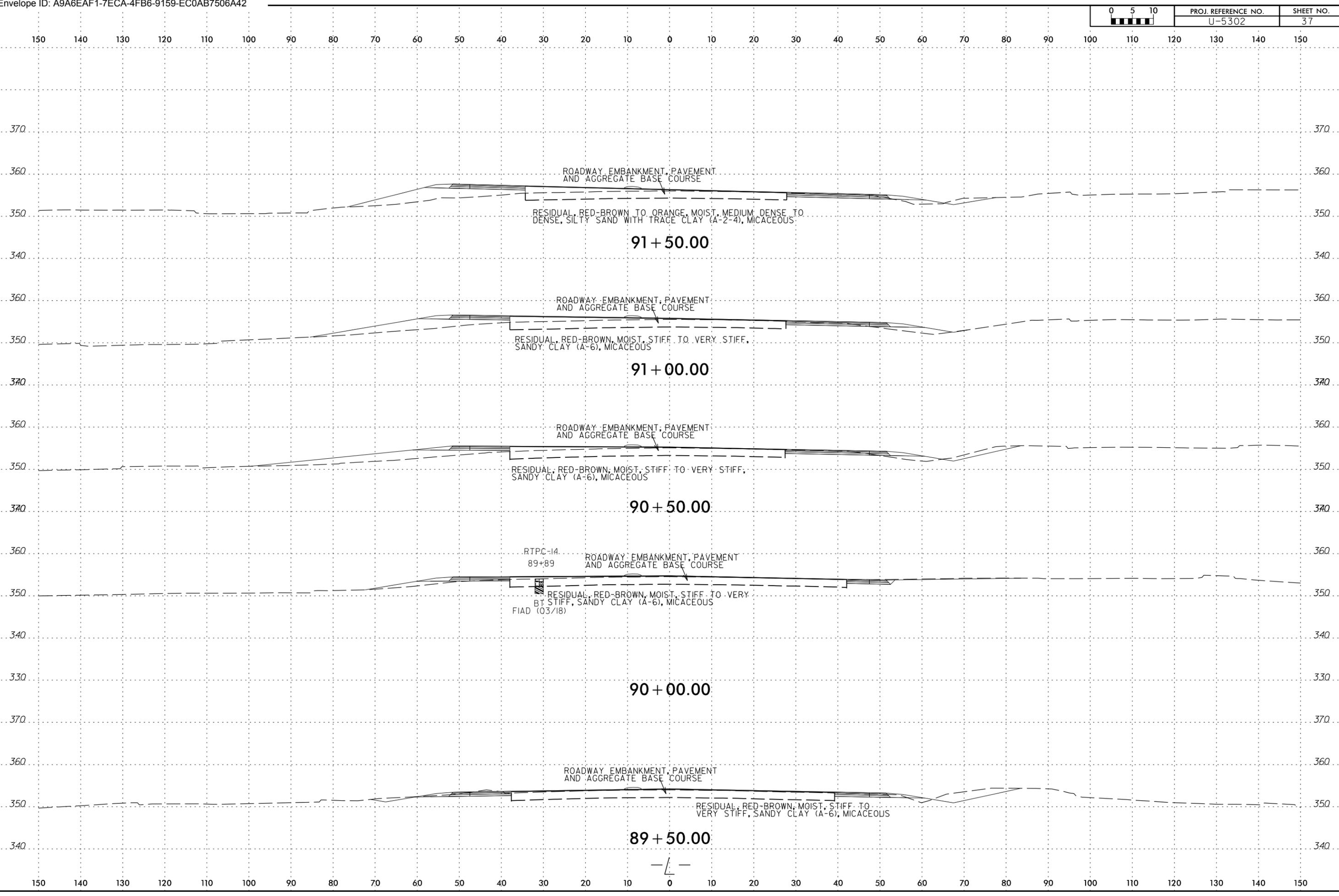
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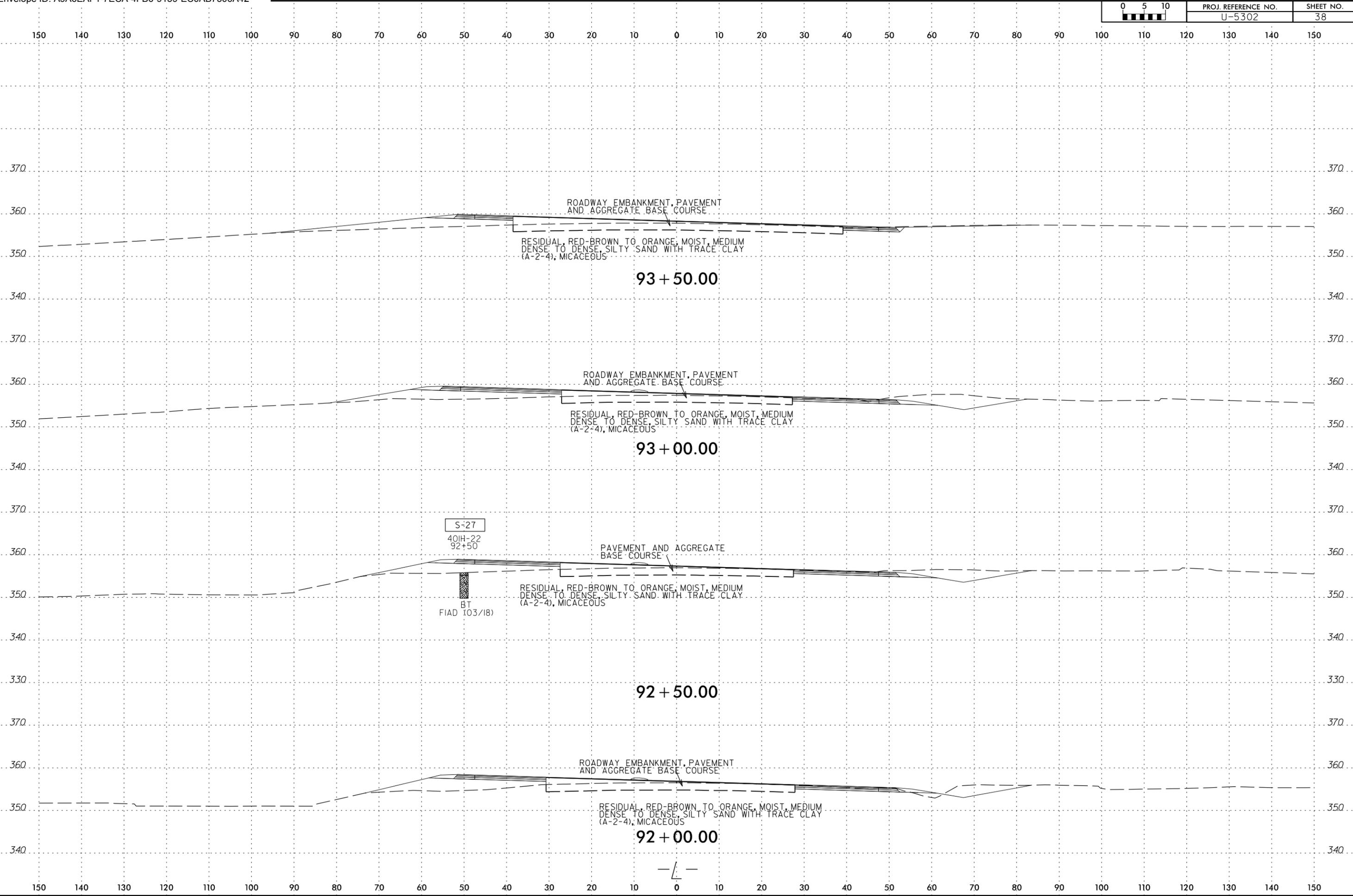


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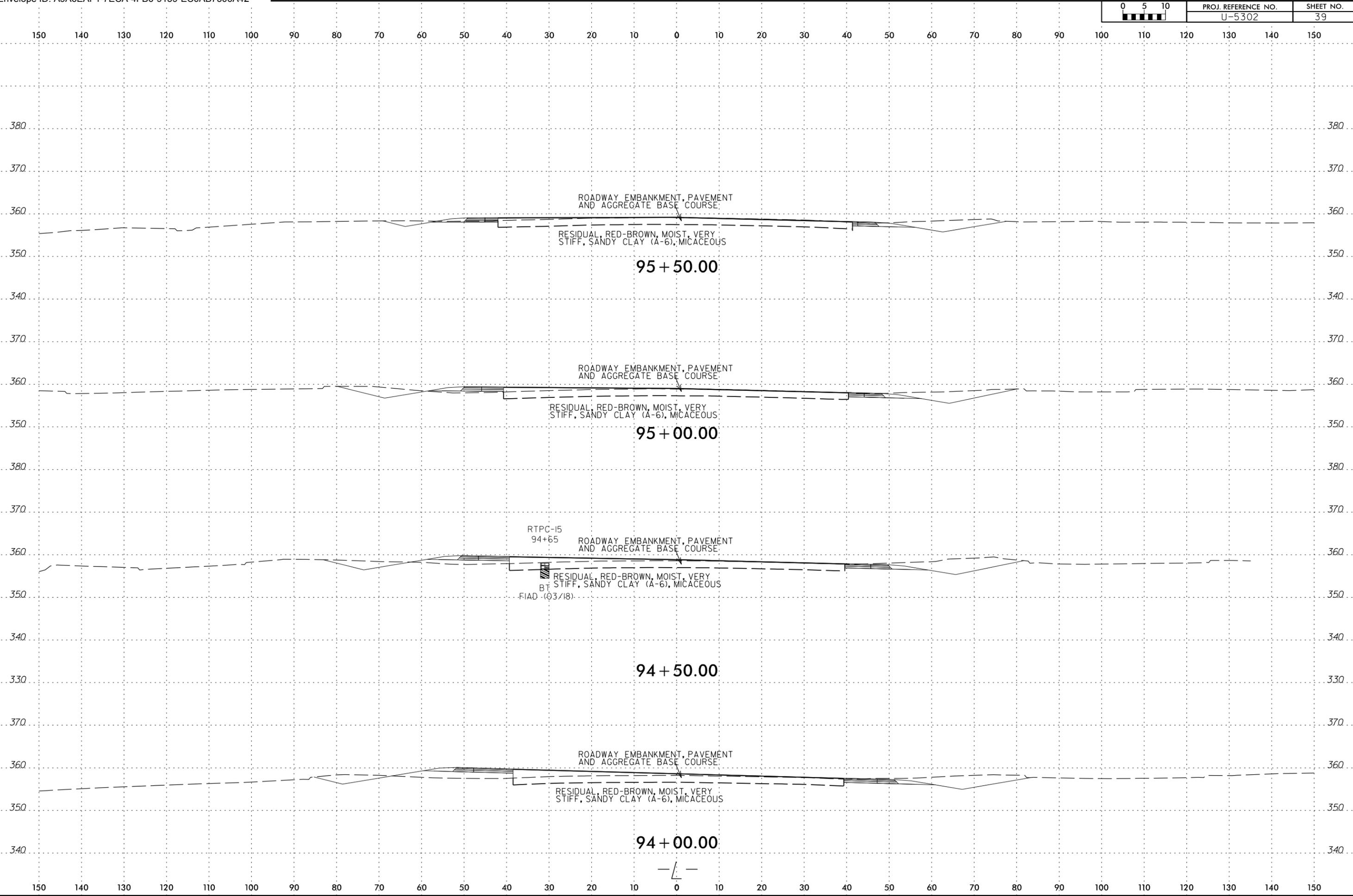
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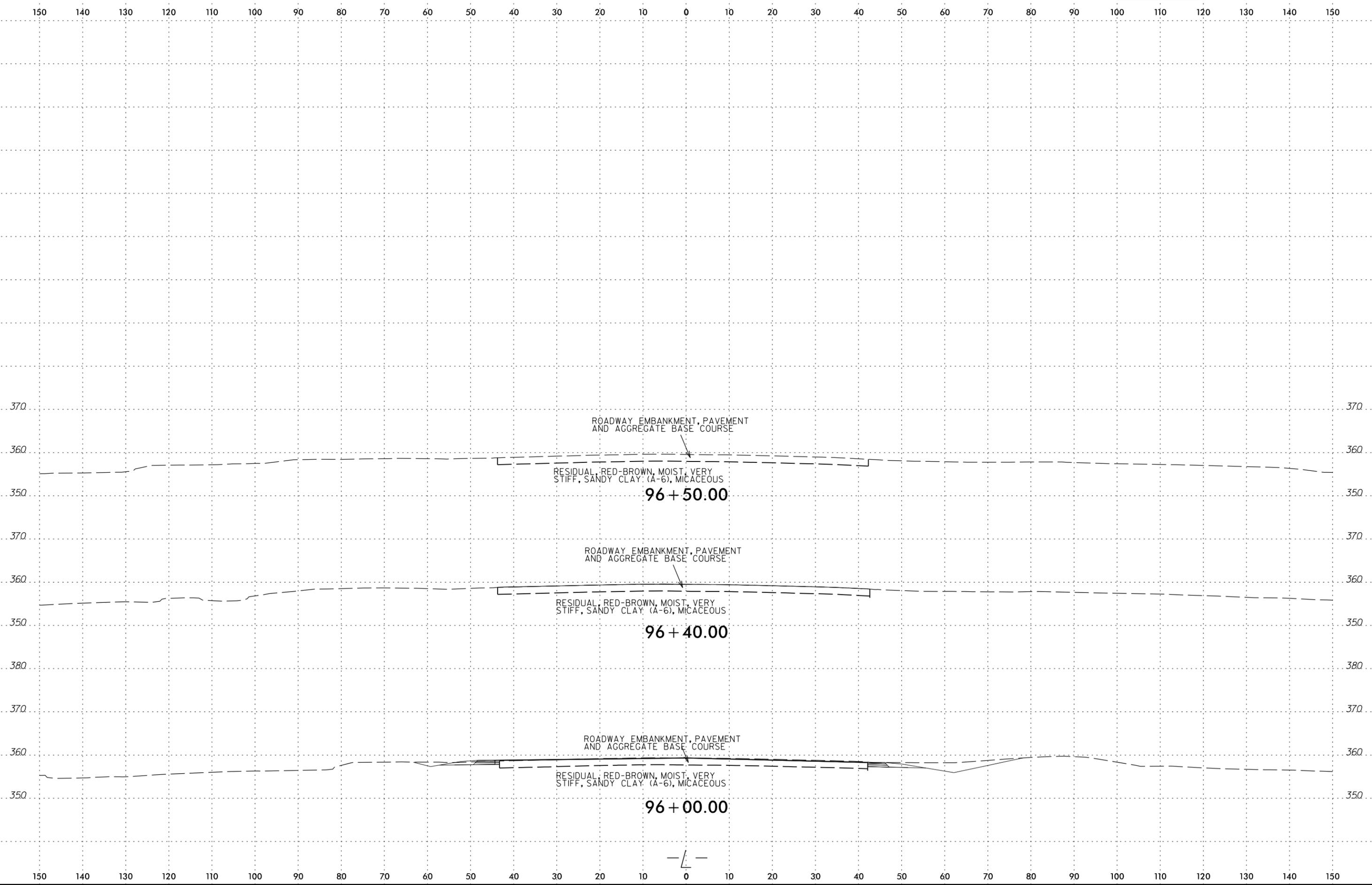


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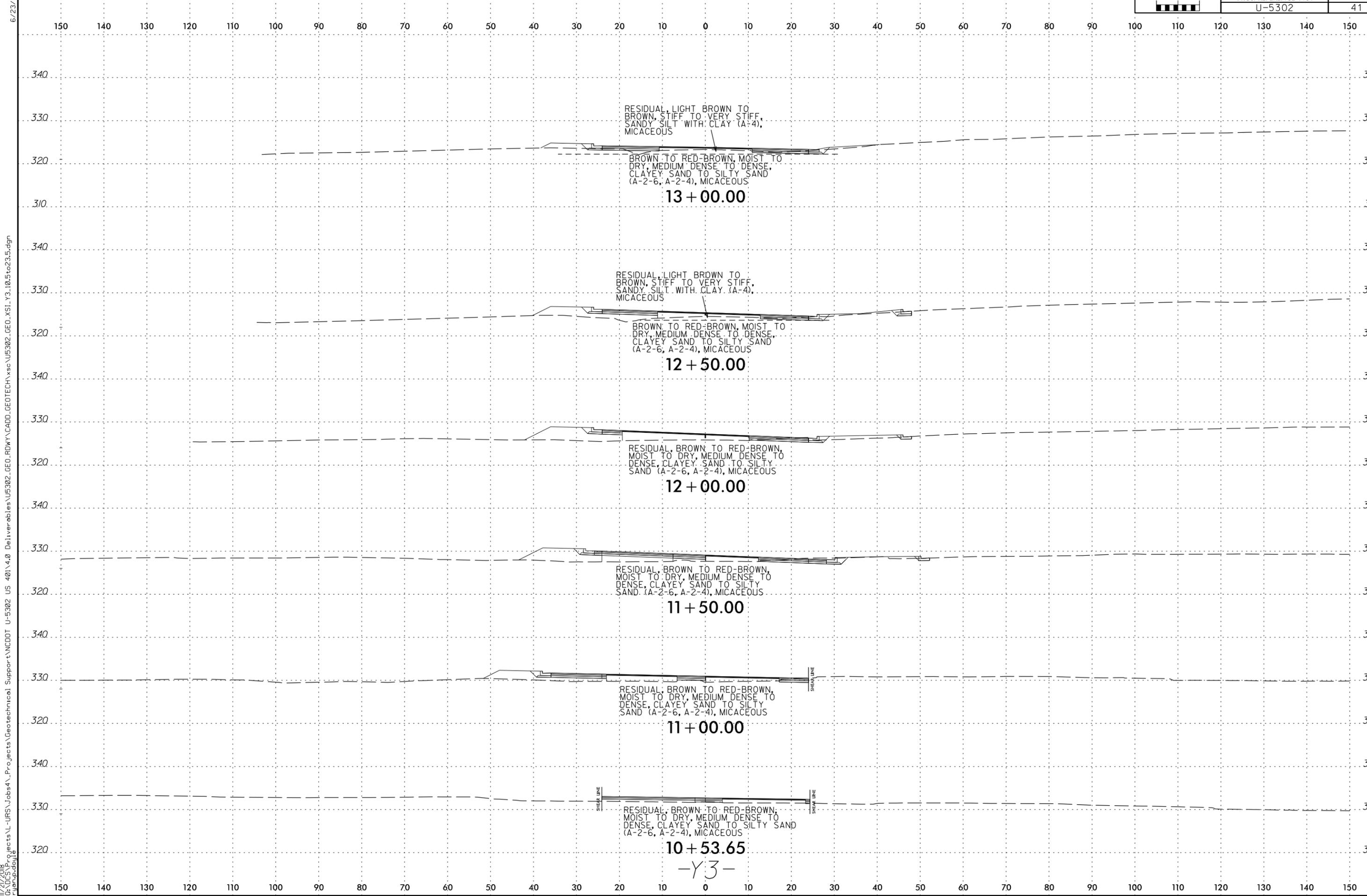


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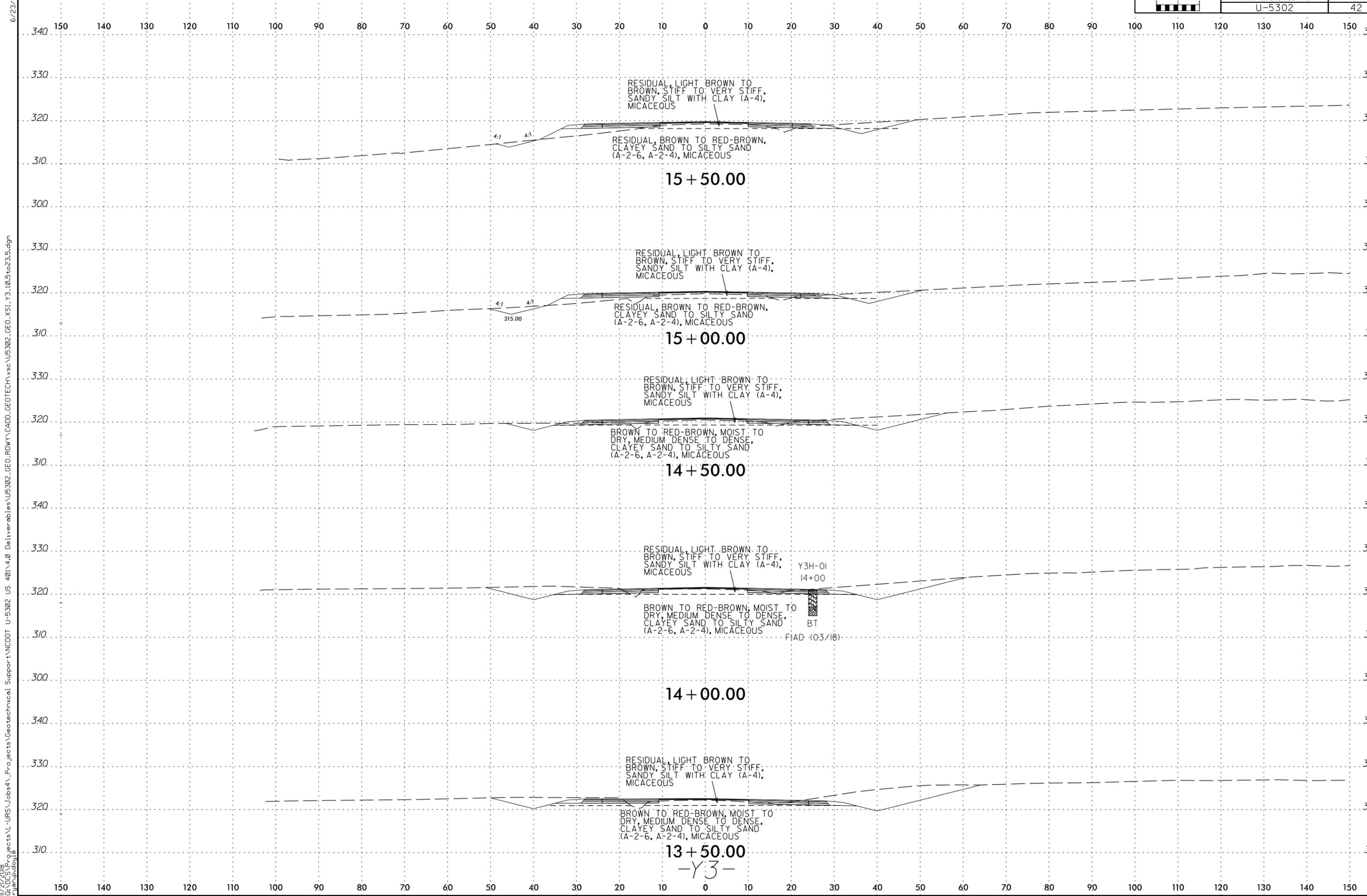


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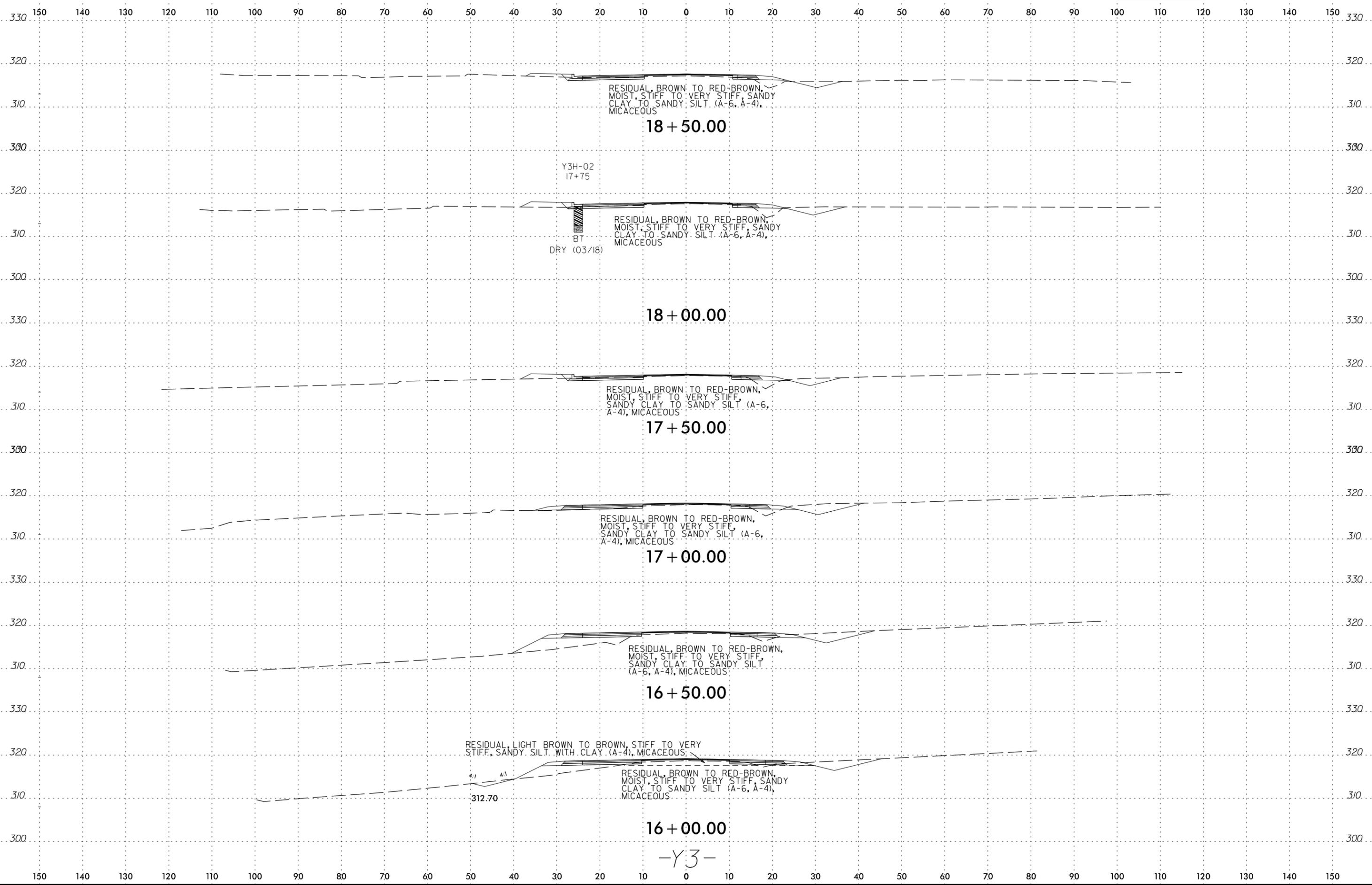
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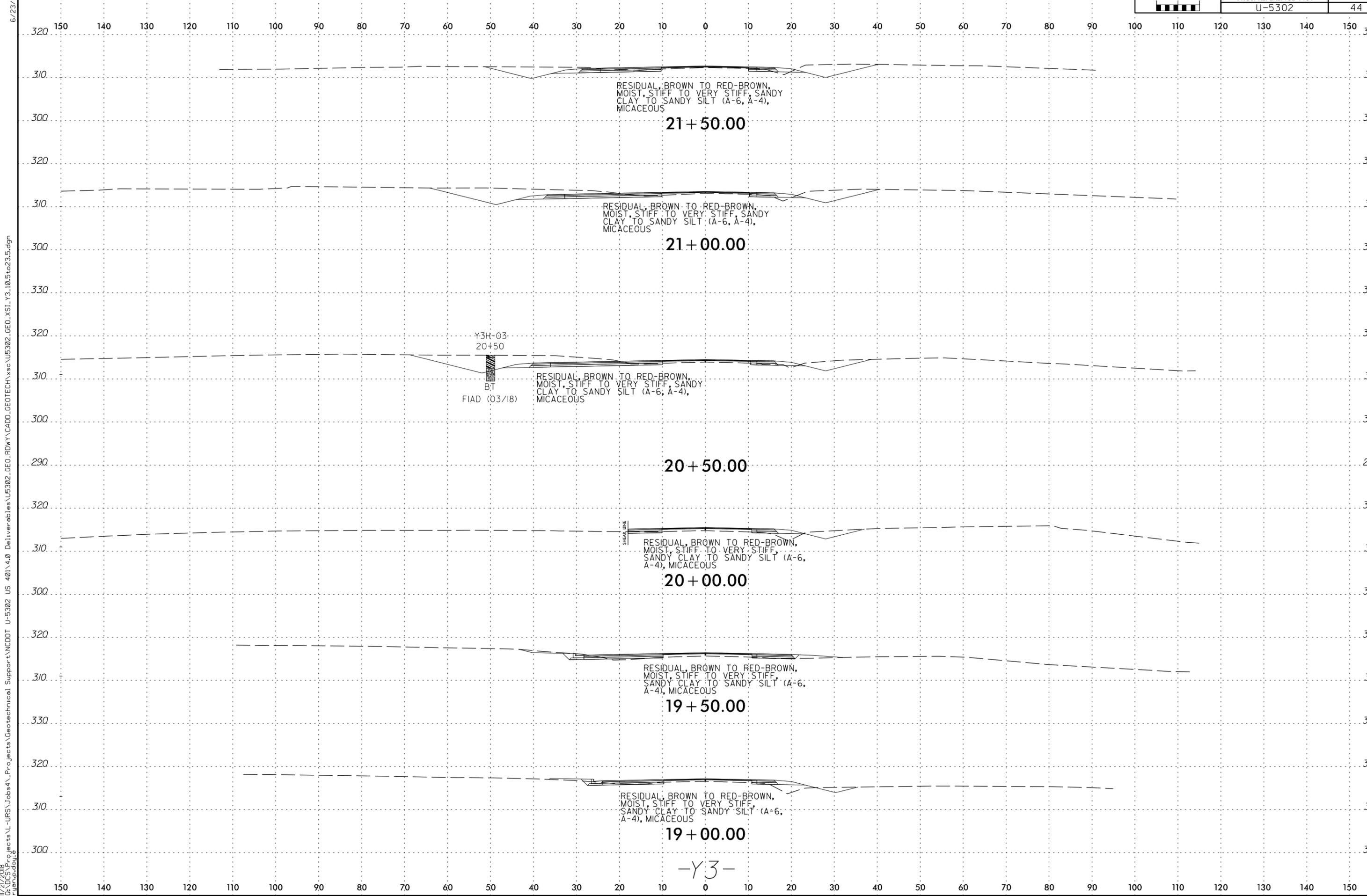
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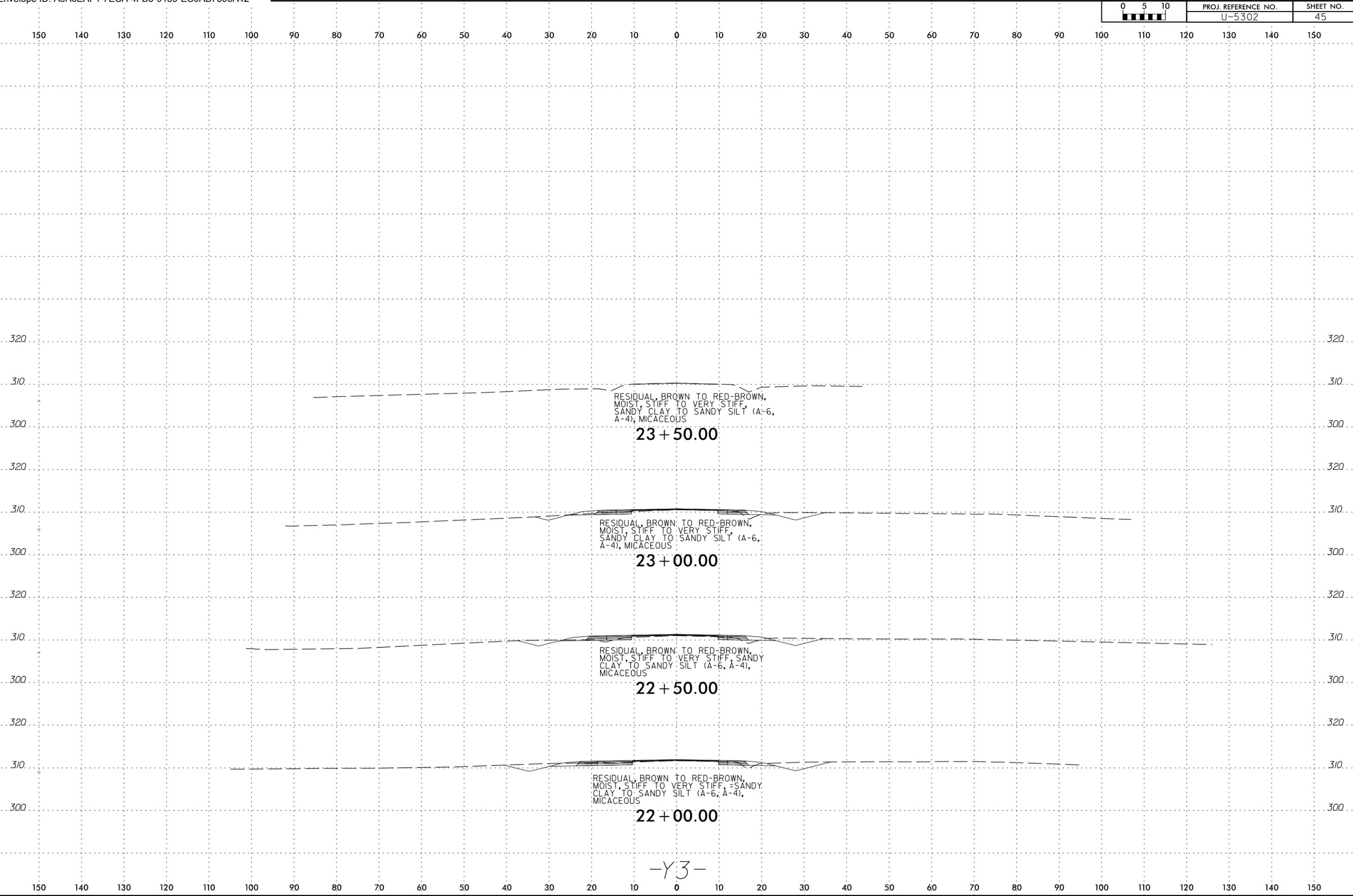
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U-5302

46

REFERENCE: U-5302

PROJECT: 48000

***NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT***

SUBSURFACE INVESTIGATION

***APPENDIX A
SOIL TEST RESULTS***

SOIL TEST RESULTS

BORING NO.	SAMPLE NO.	OFFSET	STATION	ALIGNMENT	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE
									GRAVEL	C. SAND	F. SAND	FINES	10	40	200	
401H-01	S-3	70' RT	20+50	-L-	1.0-2.0	A-4(0)	28	2	9.6	15.9	36.3	38.2	90.4	74.5	38.2	12.8
401H-25	S-23	50' RT	75+00	-L-	1.0-2.0	A-7-6(11)	53	24	3.5	10.7	31.0	54.8	96.5	85.7	54.8	14.3
401H-22	S-27	50' LT	92+50	-L-	1.5-3.5	A-2-4(0)	19	5	5.4	24.7	44.9	25.1	94.6	70.0	25.1	7.4
PC-04	S-38	35' RT	42+00	-L-	2.0-3.5	A-7-6(11)	43	23	1.3	11.4	29.3	58.1	98.8	87.4	58.1	13.5
PC-07	S-43	25' RT	80+50	-L-	2.0-3.5	A-7-5(34)	85	42	1.1	13.2	13.0	72.7	98.9	85.7	72.7	24.8
PC-06	S-54	25' LT	62+00	-L-	1.8-3.0	A-7-6(12)	46	26	3.2	13.8	26.3	56.7	96.8	83.0	56.7	13.4
401H-17	S-67	70' LT	70+00	-L-	3.0-4.0	A-7-5(24)	69	33	0.3	9.4	21.7	68.6	99.7	90.3	68.6	21.9
401H-08	S-90	50' LT	51+00	-L-	0.2-1.0	A-7-5(18)	64	27	0.7	12.4	22.7	64.2	99.3	86.9	64.2	21.3

TESTED BY: Michael P. Simon

NCDOT NO.: 129-03-0411

PROJECT REFERENCE NO.	SHEET NO.
<i>U-5302</i>	48

***NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT***

SUBSURFACE INVESTIGATION

***APPENDIX B
BORING LOGS***

REFERENCE: U-5302

PROJECT: 48000

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 48000.1.1		TIP U-5302		COUNTY WAKE		GEOLOGIST Sean Buchanan										
SITE DESCRIPTION Legend Rd							GROUND WTR (ft)									
BORING NO. Y1H-01		STATION 13+00		OFFSET 15 ft LT		ALIGNMENT -Y1-	0 HR. Dry									
COLLAR ELEV. 283.4 ft		TOTAL DEPTH 6.0 ft		NORTHING 711,374		EASTING 2,100,263	24 HR. FIAD									
DRILL RIG/HAMMER EFF./DATE N/A - Hand Auger				DRILL METHOD Hand Auger		HAMMER TYPE N/A										
DRILLER Mike Moseley		START DATE 03/05/18		COMP. DATE 03/05/18		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft)	DEPTH (ft)		
285														283.4	0.0	GROUND SURFACE
												M		262.9	0.9	TOPSOIL
												M		279.4	4.0	BROWN, SANDY CLAY (A-6)
												M		277.4	6.0	RESIDUAL RED TO DARK BROWN, STIFF TO VERY STIFF, SANDY CLAY (A-6), MICACEOUS DARK BROWN TO DARK RED, STIFF TO VERY STIFF, SANDY SILT WITH TRACE CLAY (A-4), MICACEOUS
																Boring Terminated at Elevation 277.4 ft in sandy silt

NCDOT BORE DOUBLE U5302_GEO_RDWY.GPJ NC_DOT_GDT 9/25/18