SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

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ROADWAY SUBSURFACE INVESTIGATION

CROSS SECTIONS

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PROJECT DESCRIPTION BRIDGE NO. 20 ON US 70 BUSINESS OVER CSX RAILROAD

INVENTORY

STATE PROJECT REPERENCE NO. 20 B-4838

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

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W. PESL B. PAINTER

M. ARNOLD

D. TIGNOR

W. SHENBERGER

INVESTIGATED BY F&R, Inc.

DRAWN BY _ M. ARNOLD

CHECKED BY M. ARNOLD

SUBMITTED BY P. ALTON, P.E.

DATE JANUARY 2020

FROEHLING & ROBERTSON, INC.

Prepared in the Office of:

Engineering Stability Since 1881

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Patrick alton

1/14/2020

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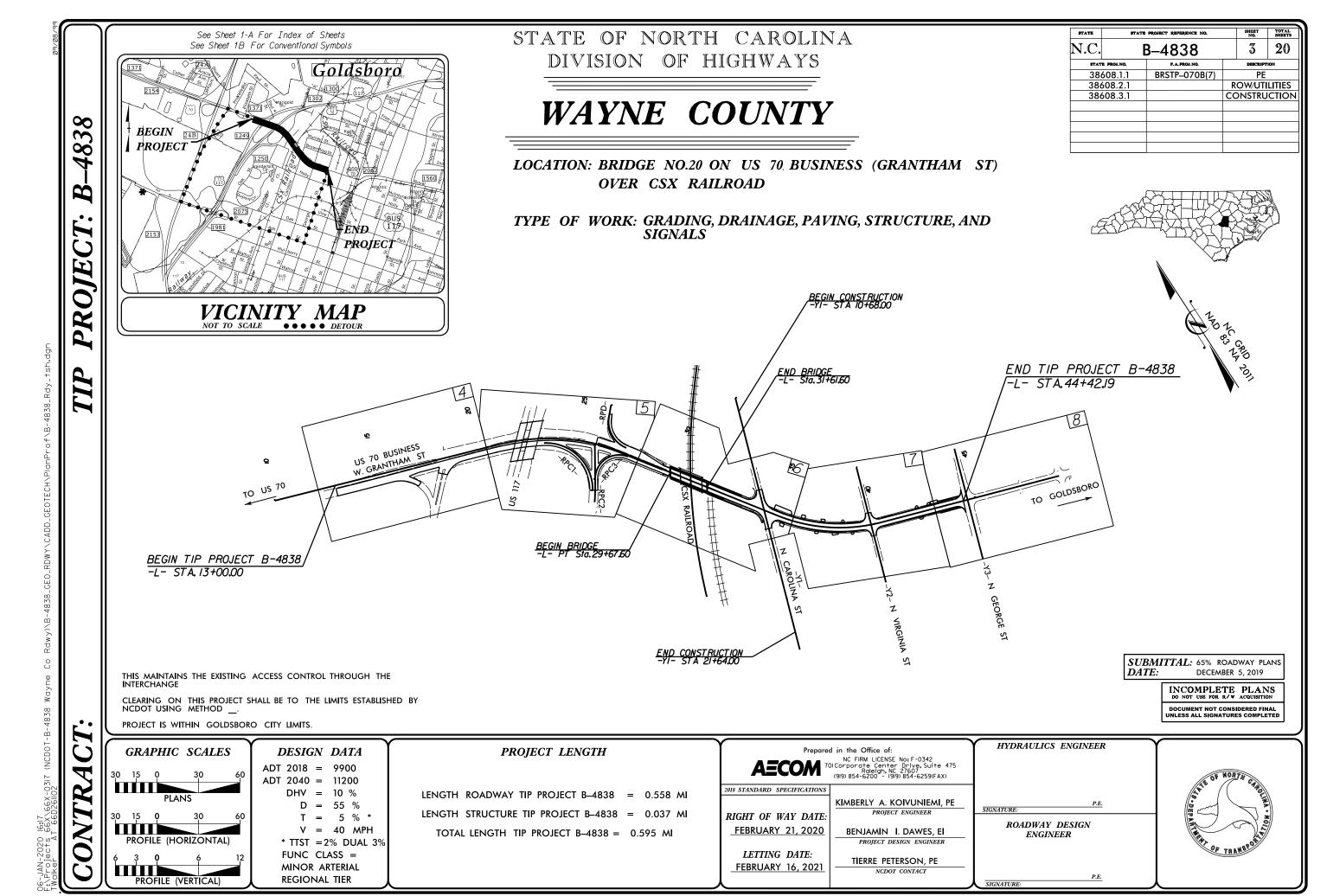
| PROJECT REPERENCE NO. | SHEET NO. |
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

| Sign SECRETION | MINERALS, OR HAVING E, ETC. DVE THE LEVEL AT DVE THE GROUND CARBONATE. LOPE OR AT BOTTOM CORE BARREL DIVIDED TO F ADJACENT |
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| PORTABLE HOIST COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. MICHIELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO SEPARATE WITH HAMMER. SHAPPHER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS. | |





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NC Engineering License # F-0266

January 9, 2020

State Project No.: 38608.1.1

TIP No.: B-4838

F.A. Number: N/A

County: Wayne

Description: Bridge No. 20 on US 70 Business over CSX Railroad

SUBJECT: Geotechnical Report – Inventory

Project Description

This project consists of the replacement of the existing Bridge 20 on US 70 Business/West Grantham Street (-L-) over CSX Railroad in Goldsboro, Wayne County, North Carolina. The bridge replacement will necessitate: realignment of existing on and off ramps, construction of two retaining walls, vertical grade adjustments, and road widening. The projects extends from approximately 200 feet southeast of the intersection of US 70 and North Elizabeth Street (-L- station 13+00.00) to the intersection of US 70 and North George Street (-L- station 44+42.19)- a distance of approximately 0.6 miles. The new alignment will typically consist of one eastbound and one westbound lane separated by a center left turn lane or concrete island. More specifically, the typical section will incorporate 12-foot lanes and 4 to 8-foot paved shoulders.

The realignment of existing on and off ramps to US 117/13/70 is proposed along -RPC1-, -RPC3-, and -RPD-. The grade along US 70 in the vicinity of the bridge replacement is proposed to be raised by up to about 4 feet. The grade increase will necessitate the construction of two retaining walls. The walls are proposed on the left and right sides of -L- from proposed bridge End Bent 2 to approximately station 33+30, left and 32+50, right. The walls be located at the top of the existing embankments. The remainder of the proposed construction east of the bridge will generally consist of minor widening with fill depths on the order of 1 to 3 feet.

The geotechnical field investigation was performed from October 2019 to November 2019. During this time period, a total of eight Standard Penetration Tests (SPT) borings were advanced with an ATV-mounted CME-55 drill rig with an automatic hammer. In addition, eleven hand auger borings were completed due to restrictive underground and overhead utilities. Representative soil samples were collected from the split spoon and hand auger cuttings for visual classification in the field and for analysis by F&R's testing laboratory.

The following alignments were investigated:

| <u>Alignment</u> | <u>Station (±)</u> |
|------------------|--------------------|
| -L- | 26+50 to 44+42.19 |
| -RPC3- | 10+00 to 12+25.81 |

Areas of Special Geotechnical Interest

1) <u>Soft, Loose and/or Wet Soils</u>: The following areas contain relatively soft or loose (SPT N<6 bpf) and/or wet, near-surface soils that have the potential to cause subgrade problems during construction:

| <u>Alignment</u> | <u>Station (±)</u> |
|------------------|-----------------------|
| -L- | 27+25 to 27+75 left |
| -L- | 28+25 to 29+67.60 |
| -L- | 31+61.60 to 33+50 |
| -L- | 35+25 to 39+75 |
| -L- | 41+50 to 44+00, right |

2) <u>Cohesive Soils</u>: The following areas contain cohesive soils (AASHTO A-5, A-6 & A-7 soils) at existing subgrade in fill areas or at/near proposed subgrade in cut areas that have the potential to cause subgrade problems during construction:

| <u>Alignment</u> | <u>Station (±)</u> |
|------------------|-----------------------|
| -L- | 26+25 to 26+75, right |
| -L- | 39+25 to 39+75 |
| -L- | 43+50 to 44+00, left |

3) <u>Cohesive Soils</u>: The following areas contain deeper deposits of relatively soft cohesive soils (AASHTO A-5, A-6 & A-7 soils) that have the potential to cause embankment instability or long-term settlement problems:

| <u>Alignment</u> | <u>Station (±)</u> |
|------------------|--------------------|
| -L- | 29+25 to 29.67+60 |
| -L- | 31+61.60 to 33+50 |

Physiography and Geology

The proposed grade changes and re-alignment in the area of this project generally follows the existing roads and run in a northwest-to-southeast direction, primarily through residential and commercial properties. The existing ground surface along the proposed road generally slopes upward from an elevation (EL) of ± 95 feet near -L- station 24+00 to EL ± 123 feet around End Bent 1 of the existing bridge (-L- station 30+00). From there, the ground surface slopes downward to EL ± 101 feet near -L- station 31+00, which is near the center

of the existing railroad tracks, and then back up to EL ± 125 feet, near End Bent 2 of the existing bridge (-L- station 31+50). From there, the ground surface generally slopes downward to EL ± 116 feet at the end of the project near station 44+00.

The project site is geologically located in the Coastal Plain physiographic province of North Carolina. The Coastal Plain Province is a broad, flat plain, with widely spaced low rolling hills, where the near surface soils have their origin from the deposition of sediments several million years ago during the period that the ocean receded from this area to its present location along the Atlantic Coast. According to the Geologic Map of North Carolina (1985), the site is within an area mapped as Cretaceous period deposits and is comprised of sediments that are identified as being located within the Yorktown Formation. The Yorktown Formation is described as blueish gray fossiliferous clay with varying amounts of fine-grained sand.

Soils Properties

The subsurface conditions discussed below and those shown on the attached drawings, represent an estimate of the subsurface conditions based on interpretation of the boring data using normally-accepted geotechnical engineering judgments. The transitions between different soil strata are usually less distinct than those shown. Sometimes the relatively small sample obtained in the field is insufficient to definitively describe the origin of the subsurface material. Although individual soil test borings are representative of the subsurface conditions at the boring locations on the dates shown, they are not necessarily indicative of subsurface conditions at other locations or at other times.

Soils within the area of this project have been divided into two categories: roadway embankment and coastal plain soils.

Roadway Embankment: Roadway embankment (RE) soils were encountered at the surface of all SPT and hand auger borings. The RE was typically associated with existing US 70/West Grantham Street. The RE extended to depths up to 8 feet. The roadway embankment soil was variable and described as moist to wet, very loose to dense silty and clayey SAND (A-2-4 & A-2-6) and medium stiff to stiff sandy CLAY (A-6 & A-7). Most samples contained trace organic matter and gravel.

Coastal Plain Soils: A majority of the soils encountered on this project were coastal plain soils belonging to the Yorktown formation. The coastal plain soils were typically described as moist to saturated, very loose to very dense silty and clayey SAND (A-2-4 & A-2-6) and moist to wet, very soft to hard sandy and silty CLAY (A-6 & A-7). Some of the samples contained trace amounts of organics, mica, and gravel.

Groundwater Properties

Generally, groundwater measurements were attempted in a majority of the borings along the project immediately upon their completion. Eight of the hand auger borings and two of the SPT borings were backfilled immediately upon their completion. Groundwater was not encountered in any borings immediately after completion; however groundwater was not measured in 3 SPT borings due to using mud rotary drilling techniques. Stabilized groundwater was only encountered in one hand auger boring at a depth

of 6.6 feet and an elevation of 109.1 feet. It should be noted that the groundwater levels fluctuate depending upon seasonal factors such as precipitation and temperature. As such, soil moisture and groundwater conditions at other times may vary or be different from those described in this report.

We appreciate the opportunity to work with you on this project. Please contact us if you have any questions regarding this report or if we may be of further service.

Sincerely,

FROEHLING & ROBERTSON, INC.

Meredith Arnold, G.I.T. Staff Geologist W. Patrick Alton, P.E.
Transportation Services Manager

Appendix A

Bulk Samples

The following bulk sample was obtained and transported to our laboratory for testing to determine the engineering properties of the soil:

| Sample No. | Boring No. | Line | Station | Offset | Depth (ft) | Test(s) Performed |
|---------------|------------|------|---------|---------|---------------|-----------------------|
| CBR-1 | L4150R | -L- | 41+75 | 38' Rt. | 0.5-2.0 | Standard Proctor, CBR |

