

REFERENCE: U-5797

PROJECT: 44367

SEE SHEET 3 FOR PLAN SHEET LAYOUT  
AT TIME OF INVESTIGATION

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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

ROADWAY  
SUBSURFACE INVESTIGATION

COUNTY ROBESON

PROJECT DESCRIPTION SR 1997 (FAYETTEVILLE ROAD)  
FROM FARRINGDOM STREET TO EAST 22ND STREET

PAVEMENT AND SUBGRADE INVESTIGATION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5797	1	13

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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  - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

ROSS, S. I.

TRIGON EXP.

INVESTIGATED BY FALCON ENG.

DRAWN BY HUNSBERGER, W. S.

CHECKED BY CROCKETT, S. C.

SUBMITTED BY FALCON ENG.

DATE OCTOBER 2024



Signed by: W. Scott Hunsberger 12/20/2024

5A469AC89C9CND4P8RE DATE

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)							ORGANIC MATERIALS															
GROUP CLASS.	A-1		A-3		A-2		A-2-4		A-2-5		A-2-6		A-2-7		A-4		A-5		A-6		A-7		A-1, A-2		A-3		A-4, A-5		A-6, A-7	
SYMBOL																														
% PASSING	50 MX		30 MX		50 MX		51 MN		35 MX		35 MX		35 MX		36 MN		36 MN		36 MN		36 MN		GRANULAR SOILS		SILT-CLAY SOILS		MUCK, PEAT			
MATERIAL PASSING #40	—		—		—		40 MX		41 MN		40 MX		41 MN		40 MX		41 MN		40 MX		41 MN		SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS					
LL	6 MX		NP		NP		10 MX		11 MN		11 MN		11 MN		10 MX		11 MN		11 MN											
GROUP INDEX	0		0		0		4 MX		8 MX		12 MX		16 MX		NO MX															
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS																			
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR							FAIR TO POOR		POOR		UNSATISFACTORY											
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30																														

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.75	2.00	0.42	0.25	0.075	0.053

	BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)
GRAIN SIZE	MM 305	75	2.0	0.25	0.05	0.005	
	IN. 12	3					

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL PLASTIC RANGE (PI) PL	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
	- WET - (W)	SEMI-SOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
OM SL	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:  
ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING  
 STATIC WATER LEVEL AFTER 24 HOURS  
 PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA  
 SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION  
 SOIL SYMBOL  
 ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  
 INFERRED SOIL BOUNDARY  
 INFERRED ROCK LINE  
 ALLUVIAL SOIL BOUNDARY  
 DIP & DIP DIRECTION OF ROCK STRUCTURES  
 SPT  
 TEST BORING  
 AUGER BORING  
 CORE BORING  
 MONITORING WELL  
 PIEZOMETER INSTALLATION  
 SLOPE INDICATOR INSTALLATION  
 CONE PENETROMETER TEST  
 SOUNDING ROD  
 TEST BORING WITH CORE  
 SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT  
 SHALLOW UNDERCUT  
 UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  
 UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  
 UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL  
BT - BORING TERMINATED  
CL - CLAY  
CPT - CONE PENETRATION TEST  
CSE - COARSE  
DMT - DILATOMETER TEST  
DPT - DYNAMIC PENETRATION TEST  
e - VOID RATIO  
F - FINE  
FOSS - FOSSILIFEROUS  
FRAC. - FRACTURED, FRACTURES  
FRAGS. - FRAGMENTS  
HL - HIGHLY  
MED. - MEDIUM  
MICA - MICACEOUS  
MOD. - MODERATELY  
NP - NON PLASTIC  
ORG. - ORGANIC  
PMT - PRESSUREMETER TEST  
SAP. - SAPROLITIC  
SD. - SAND, SANDY  
SL. - SILT, SILTY  
SLI. - SLIGHTLY  
TCR - TRICONE REFUSAL  
w - MOISTURE CONTENT  
V - VERY  
VST - VANE SHEAR TEST  
WEA. - WEATHERED  
γ - UNIT WEIGHT  
γ<sub>d</sub> - DRY UNIT WEIGHT  
SAMPLE ABBREVIATIONS  
S - BULK  
SS - SPLIT SPOON  
ST - SHELBY TUBE  
RS - ROCK  
RT - RECOMPACTED TRIAXIAL  
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:  
☐ CME-45C  
☒ CME-55  
☐ CME-550  
☐ VANE SHEAR TEST  
☐ PORTABLE HOIST  
☐ \_\_\_\_\_  
☐ \_\_\_\_\_

ADVANCING TOOLS:  
☐ CLAY BITS  
☐ 6" CONTINUOUS FLIGHT AUGER  
☐ 8" HOLLOW AUGERS  
☐ HARD FACED FINGER BITS  
☐ TUNG-CARBIDE INSERTS  
☐ CASING ☐ W/ ADVANCER  
☐ TRICONE \_\_\_\_\_ \* STEEL TEETH  
☐ TRICONE \_\_\_\_\_ \* TUNG.-CARB.  
☐ CORE BIT  
☐ \_\_\_\_\_

HAMMER TYPE:  
☒ AUTOMATIC ☐ MANUAL  
CORE SIZE:  
☐ -B \_\_\_\_\_ ☐ -H \_\_\_\_\_  
☐ -N \_\_\_\_\_  
HAND TOOLS:  
☐ POST HOLE DIGGER  
☐ HAND AUGER  
☐ SOUNDING ROD  
☐ VANE SHEAR TEST  
☒ KESSLER DCP

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)  
 CRYSTALLINE ROCK (CR)  
 NON-CRYSTALLINE ROCK (NCR)  
 COASTAL PLAIN SEDIMENTARY ROCK (CP)

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  
FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  
FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH  
VERY SLIGHT (V SL.)  
SLIGHT (SL.)  
MODERATE (MOD.)  
MODERATELY SEVERE (MOD. SEV.)  
SEVERE (SEV.)  
VERY SEVERE (V SEV.)  
COMPLETE

ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.  
ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.  
ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.  
SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*  
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*  
ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD  
HARD  
MODERATELY HARD  
MEDIUM HARD  
SOFT  
VERY SOFT

CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  
CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  
CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  
CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  
CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  
CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  
FRIABLE  
MODERATELY INDURATED  
INDURATED  
EXTREMELY INDURATED

RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  
GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.  
GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.  
SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  
AQUIFER - A WATER BEARING FORMATION OR STRATA.  
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  
ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.  
CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.  
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.  
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  
DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.  
DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.  
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.  
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOOED FROM PARENT MATERIAL.  
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.  
FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.  
JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.  
LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.  
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.  
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.  
ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.  
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  
SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  
STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  
STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  
STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

ELEVATION: FEET

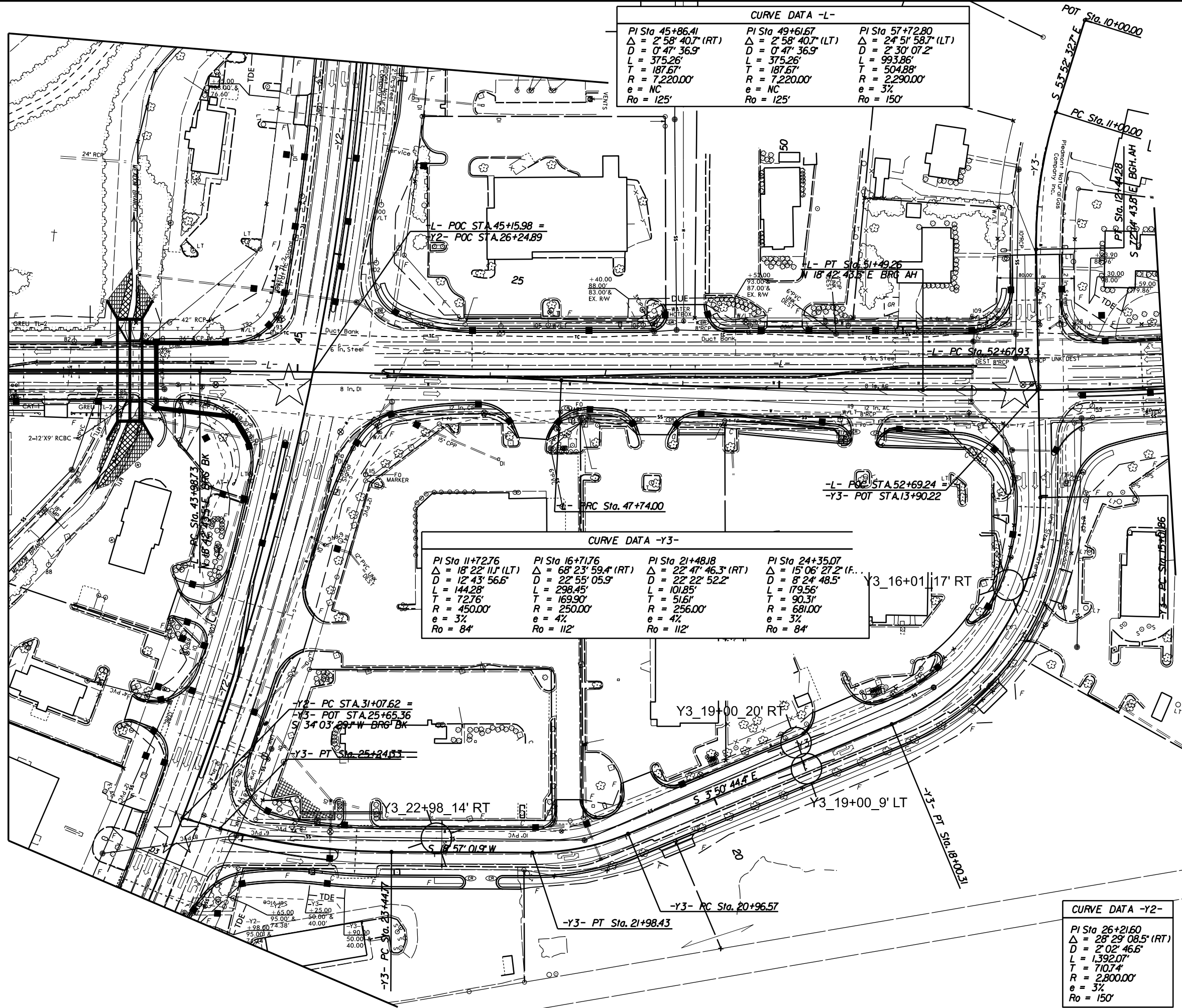
NOTES:

DATE: 8-15-14



8/17/99


REVISIONS

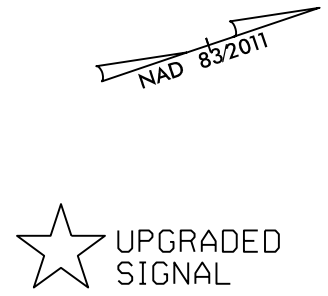


CURVE DATA -L-		
PI Sta 45+86.41	PI Sta 49+61.67	PI Sta 57+72.80
$\Delta = 2^\circ 58' 40.7''$ (RT)	$\Delta = 2^\circ 58' 40.7''$ (LT)	$\Delta = 2^\circ 51' 58.7''$ (LT)
$D = 0^\circ 47' 36.9''$	$D = 0^\circ 47' 36.9''$	$D = 2^\circ 30' 07.2''$
$L = 375.26'$	$L = 375.26'$	$L = 993.86'$
$T = 187.67'$	$T = 187.67'$	$T = 504.88'$
$R = 7,220.00'$	$R = 7,220.00'$	$R = 2,290.00'$
$e = NC$	$e = NC$	$e = 3\%$
$Ro = 125'$	$Ro = 125'$	$Ro = 150'$

CURVE DATA -Y3-			
PI Sta 11+72.76	PI Sta 16+71.76	PI Sta 21+48.18	PI Sta 24+35.07
$\Delta = 18^\circ 22' 11.1''$ (LT)	$\Delta = 68^\circ 23' 59.4''$ (RT)	$\Delta = 22^\circ 47' 46.3''$ (RT)	$\Delta = 15^\circ 06' 27.2''$ (F...)
$D = 12^\circ 43' 56.6''$	$D = 22^\circ 55' 05.9''$	$D = 22^\circ 22' 52.2''$	$D = 8^\circ 24' 48.5''$
$L = 144.28'$	$L = 298.45'$	$L = 101.85'$	$L = 179.56'$
$T = 72.76'$	$T = 169.90'$	$T = 51.61'$	$T = 90.31'$
$R = 450.00'$	$R = 250.00'$	$R = 256.00'$	$R = 681.00'$
$e = 3\%$	$e = 4\%$	$e = 4\%$	$e = 3\%$
$Ro = 84'$	$Ro = 112'$	$Ro = 112'$	$Ro = 84'$

CURVE DATA -Y2-	
PI Sta 26+21.60	
$\Delta = 28^\circ 29' 08.5''$ (RT)	
$D = 2^\circ 02' 46.6''$	
$L = 1,392.07'$	
$T = 710.74'$	
$R = 2,800.00'$	
$e = 3\%$	
$Ro = 150'$	

PROJECT REFERENCE NO. <b>U-5797</b>		SHEET NO. <b>4</b>	
RW SHEET NO.		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER			
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>			
 STV Engineers, Inc. 1600 Perimeter Park Dr. Suite 225 Morrisville, NC 27560 NC License Number F-0991			



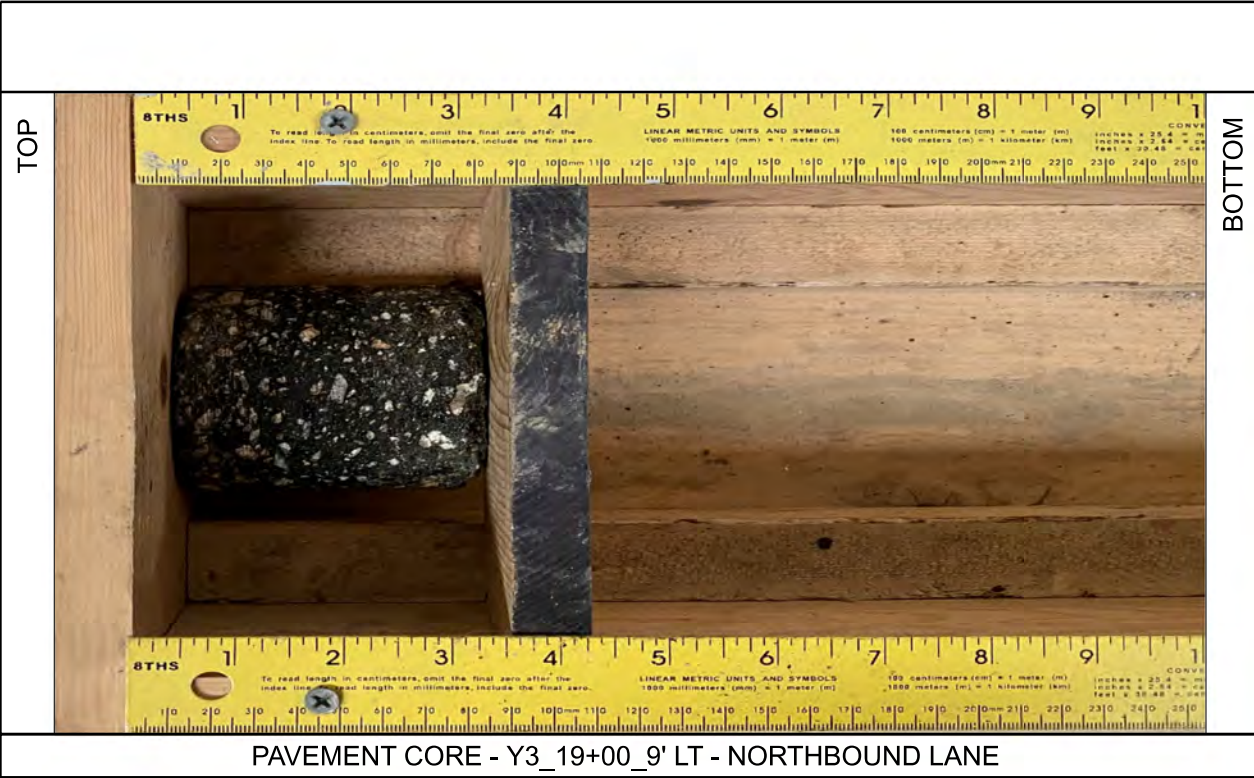




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CARY, NC 27513  
PHONE: 919.871.0800

PAVEMENT CORE PHOTOGRAPHS

SR 1997 (FAYETTEVILLE ROAD) FROM  
FARRINGDOM STREET TO EAST 22ND STREET  
WBS NO.:44367.1.1 | TIP NO.: U-5797  
FALCON PROJECT NO.: G24042.00



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PAVEMENT CORE PHOTOGRAPHS

SR 1997 (FAYETTEVILLE ROAD) FROM  
FARRINGDOM STREET TO EAST 22ND STREET  
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PAVEMENT CORE EVALUATION  
44367.1.1, U-5797, Robeson County

LINE	STATION	ABC (in)	LAYER THICKNESS (in)	Lifts	LAYERS	REMARKS
-Y3-	Y3_1601_17'RT	-	3.50	2	Surface	Low oxidation
	3.5" Asphalt					
-Y3-	Y3_1900_9' LT	5.0	2.50	1	Surface	Core broken at 1.0" in Asphalt
	2.5" Asphalt					Low oxidation
-Y3-	Y3_1900_20' RT	-	3.50	2	Surface	Low oxidation
	3.5" Asphalt					
-Y3-	Y3_22+98_14' RT	4.0	3.00	1	Surface	Low oxidation
	3.0" Asphalt					





CONE PENETROMETER  
DATA CODE SHEET

CONE PENETROMETER DATA CODE SHEET										PROJECT NUMBER				PROJECT I.D.				ROUTE							
										U-5797				44367.1.1				(-Y3-) BOOMERANG DRIVE							
										COUNTY				GEOLOGIST				TECHNICIANS							
										Robeson				Ross, S. I.											
Station (location) information										Date run				Station (location) information								Date run			
Y3_16+01_17' RT										6/20/2024				Y3_19+00_20' RT								6/20/2024			
Datum		cut or fill		Remarks						Datum		cut or fill		Remarks											
Soil Subgrade		Fill								Soil Subgrae		Fill													
Cumulative Penetration in Centimeters										Cumulative Penetration in Centimeters															
0.3		24.4		89.2						0.4		21.3													
2.0		25.0		90.9						1.5		22.4													
2.5		25.5		92.5						2.6		23.4													
3.0		26.0		93.9						3.8		24.4													
3.5		26.5								4.1		25.4													
4.0		27.0								4.4		26.4													
4.7		27.9								4.7		27.5													
5.0		28.9								5.0		28.6													
5.3		30.0								5.4		29.7													
5.6		31.5								5.6		30.2													
5.9		33.1								5.8		30.7													
6.3		35.3								6.0		31.2													
6.5		37.2								6.2		31.7													
6.7		39.0								6.4		32.3													
6.9		40.4								6.6		32.9													
7.1		41.6								6.8		33.5													
7.6		42.9								7.4		34.1													
7.9		44.2								7.6		34.7													
8.2		45.7								7.8		35.3													
8.5		47.2								8.0		36.0													
8.8		49.0								8.2		36.7													
9.3		51.0								8.4		37.4													
9.6		52.0								8.6		38.2													
9.9		53.1								8.8		39.0													
10.2		53.7								9.0		39.8													
10.5		54.3								9.2		40.6													
11.1		54.9								9.9		41.5													
11.5		55.5								10.1		42.7													
11.9		56.2								10.3		44.0													
12.3		56.8								10.5		44.8													
12.7		57.4								10.7		46.7													
13.1		58.0								10.9		48.0													
13.5		58.6								11.1		49.4													
14.3		59.5								11.3		51.0													
14.8		60.4								11.5		52.6													
15.3		61.3								11.7		55.1													
15.8		62.2								12.7		57.6													
16.6		63.1								13.0		60.2													
17.0		64.3								13.3		63.1													
17.4		65.7								13.6		67.0													
17.8		67.2								13.9		71.2													
18.2		68.5								14.2		75.7													
19.0		69.9								14.5		84.9													
19.4		71.5								15.4															
19.8		73.2								15.8															
20.2		75.1								16.2															
20.6		77.1								16.6															
21.0		78.9								17.0															
21.4		80.8								17.7															
22.0		82.6								18.3															
22.6		84.4								18.9															
23.2		86.0								19.5															
23.8		87.6								20.4															

SG = Subgrade  
SS = Stabilized Soil  
ABC = Aggregate Base Course

CONE PENETROMETER  
DATA CODE SHEET

CONE PENETROMETER DATA CODE SHEET										PROJECT NUMBER				PROJECT I.D.				ROUTE							
										U-5797				44367.1.1				(-Y3-) BOOMERANG DRIVE							
										COUNTY				GEOLOGIST				TECHNICIANS							
Robeson				Ross, S. I.																					
Station (location) information										Date run				Station (location) information								Date run			
Y3_19+00_9' LT										6/20/2024				Y3_22+98_14' RT								6/20/2024			
Datum		cut or fill		Remarks						Datum		cut or fill		Remarks											
ABC		Fill								ABC		Fill													
Cumulative Penetration in Centimeters										Cumulative Penetration in Centimeters															
0.4		45.8								0.2		81.1													
2.7		46.6								2.0		83.7													
3.8		47.5								2.8		85.6													
4.9		48.2								3.6		87.4													
6.1		48.9								4.4		89.1													
6.9		49.8								5.5		90.7													
7.7		50.7								6.5		92.2													
8.5		51.6								7.5		93.7													
9.4		52.6								8.7															
10.3		53.7								10.3															
11.4		54.8								11.9															
12.2		56.2								13.0															
13.0		57.7								14.2															
14.0		59.5								15.2															
15.1		63.8								16.2															
16.2		66.7								17.3															
17.4		69.4								18.5															
18.6		70.4								20.1															
19.9		71.4								22.0															
21.2		72.4								23.9															
22.6		73.5								25.0															
23.7		74.6								26.2															
24.9		75.7								27.1															
25.5		76.8								28.0															
26.1		77.9								28.9															
26.8		79.0								29.6															
27.4		79.7								30.3															
28.0		80.4								31.0															
28.6		81.1								32.0															
29.1		81.8								33.0															
29.6		82.5								34.2															
30.1		83.1								36.0															
30.6		83.7								37.8															
31.3		84.5								38.8															
31.7		85.3								39.8															
32.1		86.1								41.0															
32.5		87.0								42.2															
32.9		87.8								43.4															
33.5		88.6								44.9															
34.0		89.5								46.5															
34.5		90.2								48.7															
35.0		91.0								51.9															
35.5		91.9								55.9															
36.4		92.6								58.7															
37.2		93.4								61.3															
38.0										63.6															
39.0										65.9															
40.0										68.2															
41.0										70.7															
42.1										72.8															
43.0										74.6															
43.9										76.5															
45.0										78.7															

SG = Subgrade  
SS = Stabilized Soil  
ABC = Aggregate Base Course

## DCP TEST DATA

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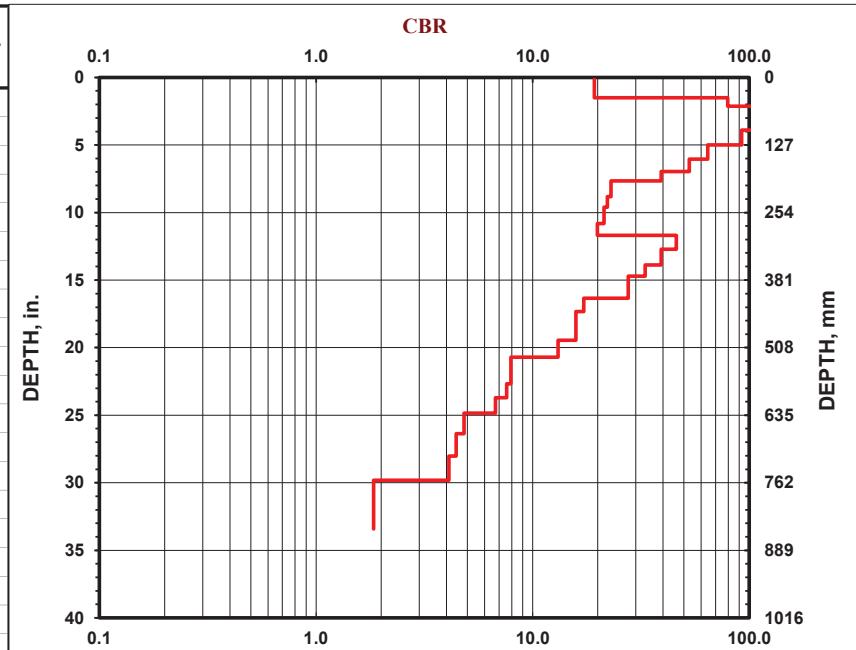
Date: 27-Jun-24  
Soil Type(s): A-4

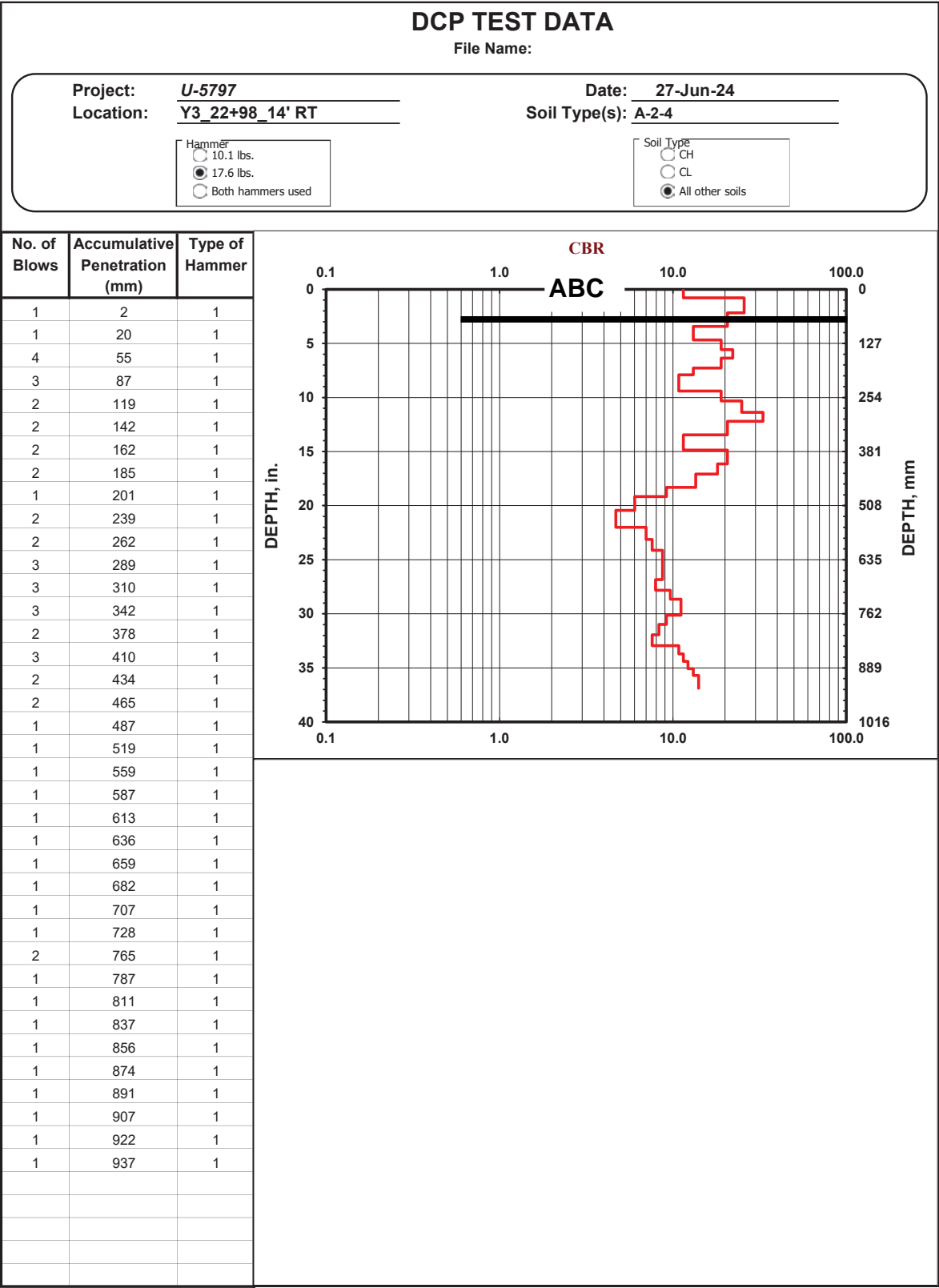
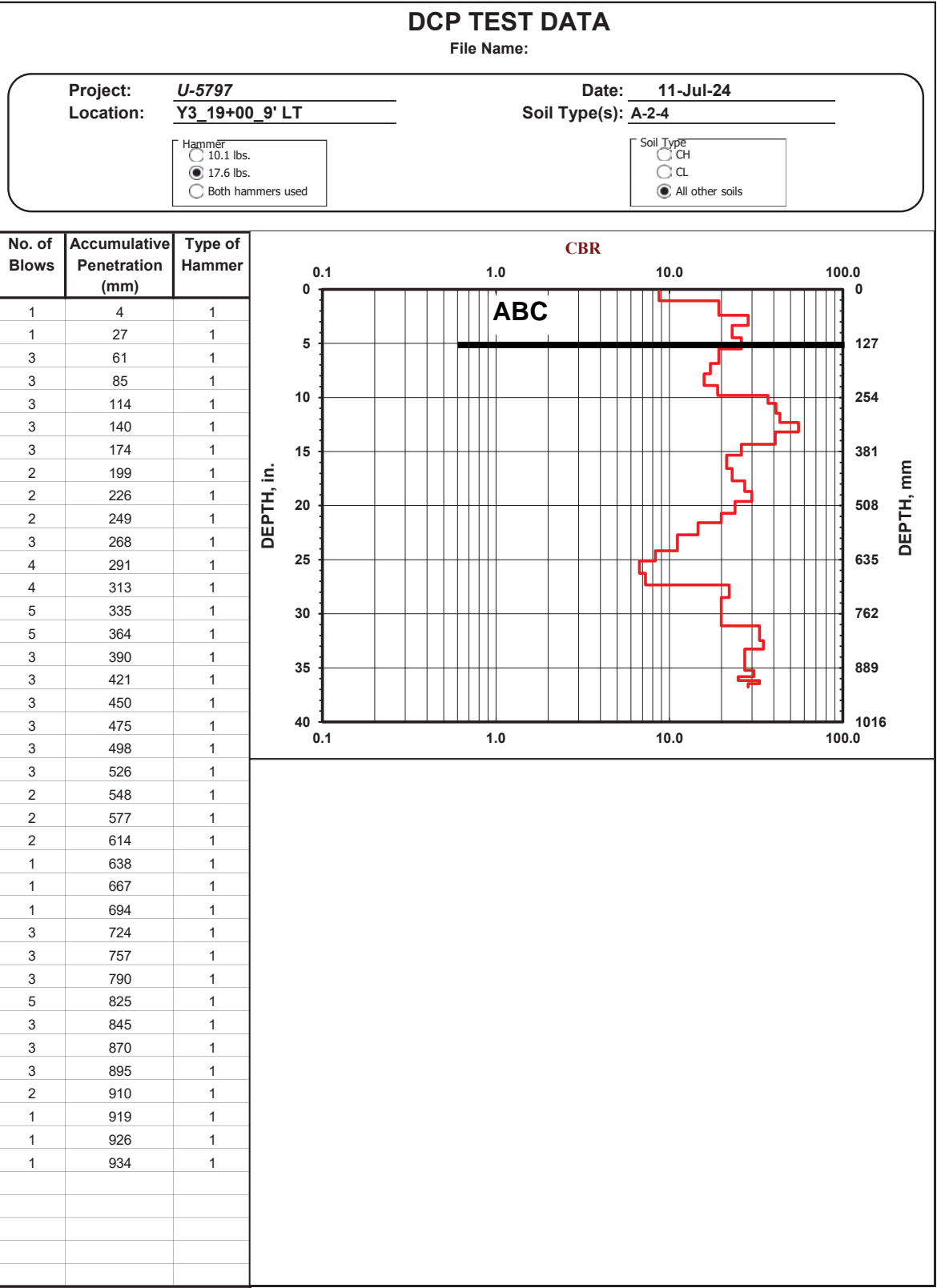
Soil Type

☐ CH

☐ CL

☒ All other soils

[illegible]





**LABORATORY TEST RESULTS**  
**Fayetteville Road Proposed Widening from Farringdom Street to 22nd Street**  
**Lumberton, NC**  
**NCDOT Project: U-5797**  
**Falcon Engineering Project No: G24042.00**

NO.	SAMPLE LOCATION	DEPTH INTERVAL	AASHTO CLASS.	ATTERBERG LIMITS		PERCENT BY WEIGHT				PERCENT PASSING SIEVE			MOISTURE (%)	SPECIFIC GRAVITY	ORGANICS (%)
				LL	PI	C.SAND	F.SAND	SILT	CLAY	#10	#40	#200			
BS-01	Y3_19+00_20' RT & 9' LT	1.0-3.0	A-4(0)	19	5	24	45	12	19	100	86	37	10	2.604	N/A
S-01	Y3_16+01_17' RT	1.0-2.5	A-2-4(0)	17	3	37	38	7	18	100	77	29	10	N/A	N/A
S-02	Y3_19+00_9' RT	1.0-2.0	A-4(0)	18	3	23	45	12	20	100	86	37	11	N/A	N/A
S-03	Y3_19+00_20' RT	1.0-3.0	A-2-4(0)	19	5	31	40	11	18	100	81	33	10	N/A	N/A
S-04	Y3_22+98_14' RT	4.0-5.0	A-7-6(15)	48	30	4	44	14	38	100	98	61	18	N/A	N/A

**Reviewed By**





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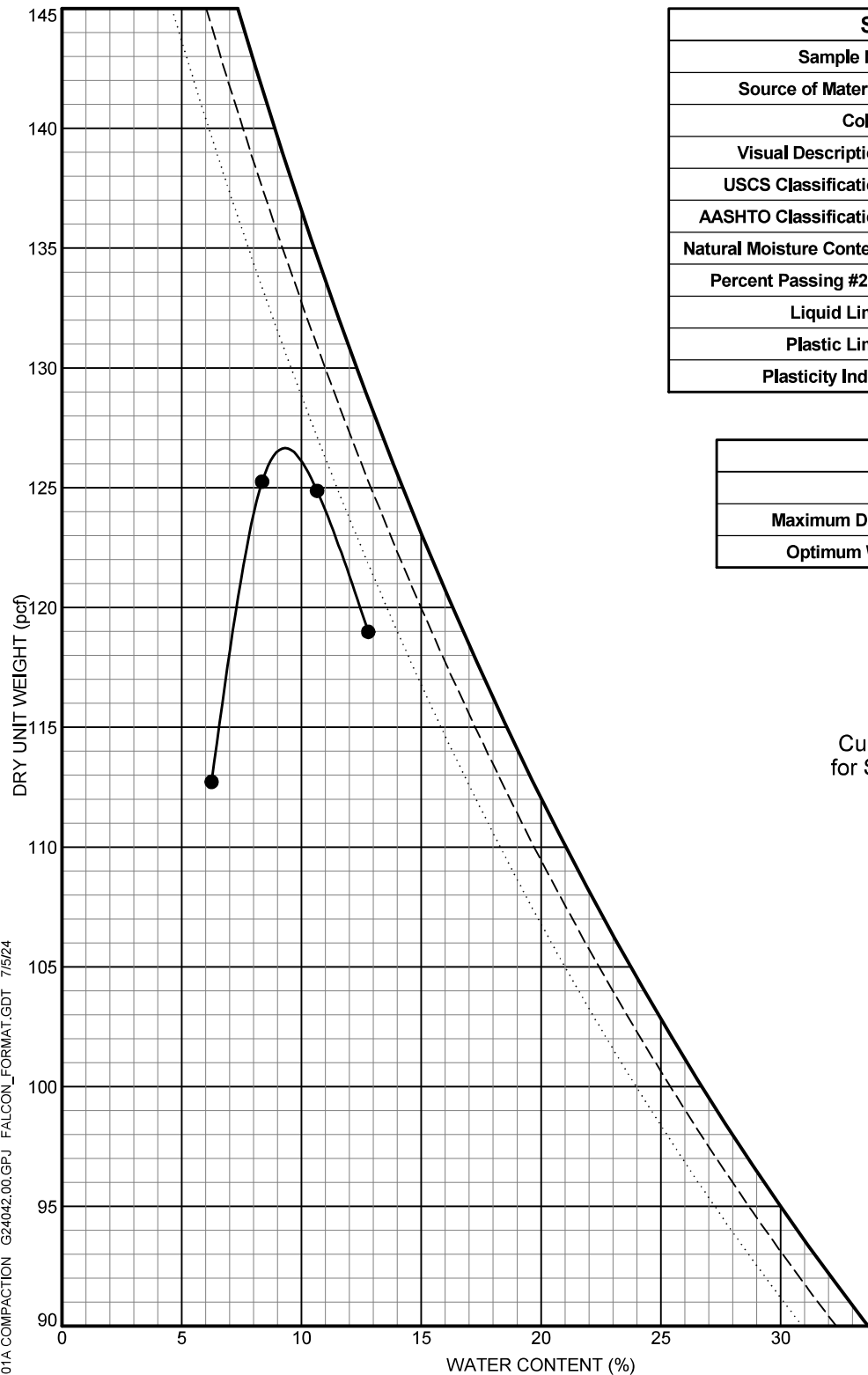
## LABORATORY COMPACTION TEST RESULTS

PAGE 1 OF 1

Project No.: G24042.00  
Project Name: U-5797  
Project Location: Lumberton, NC

SPECIMEN DATA	
Sample No:	BS-01
Source of Material:	Y3_19+00_20' RT AND Y3_19+00_9' LT
Color:	Gray
Visual Description:	
USCS Classification:	SILTY, CLAYEY SAND(SC-SM)
AASHTO Classification:	A-4
Natural Moisture Content:	10.4 %
Percent Passing #200:	36.9 %
Liquid Limit:	19
Plastic Limit:	14
Plasticity Index:	5

TEST RESULTS	
Test Method:	AASHTO T-99
Maximum Dry Unit Weight:	126.7 pcf
Optimum Water Content:	9.3 %



01A COMPACTION G24042.00.GPJ FALCON\_FORMAT.GDT 7/5/24



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## CALIFORNIA BEARING RATIO TEST RESULTS

ASTM D1883 / AASHTO T193

Project No.: G24042.00		Tested By: J. Grabner		Test Date: 2024-15-07	
Project Name: U-5797					
Boring ID: Y3_19+00_20' RT AND Y3_19+00_9' LT		Sample ID: BS-01		Sample Depth: 1.0-3.0 ft	
MOLDED SPECIMEN TEST DATA					
Wt. of Mold + Wet Soil:	21111 g	Moisture Content <u>Before</u> Molding	<u>After</u> Molding	Max. Dry Unit Weight:	126.7 pcf
Wt. of Mold:	16478 g	Tare Wt.:	201.30 g	184.80 g	Optimum Moisture Content: 9.3%
Wt. of Wet Soil:	4633 g	Wt. Tare + Wet Soil:	585.90 g	916.20 g	Percent Compaction: 98.1%
Mold Volume:	0.0752 cf	Wt. Tare + Dry Soil:	553.40 g	853.10 g	Compaction Method: 698C
Wet Unit Weight:	135.8 pcf	Moisture Content:	9.2%	9.4%	<u>Conversion Factors</u> 1 lb = 453.6 gram    1 cu. foot = 1728 cu. inch
Dry Unit Weight:	124.2 pcf	Average Moisture Content:	9.3%		
LOAD TEST DATA					
Penetration (in)	Load (lb)	Stress (psi)	<u>Piston Calibration</u>		
0.000	0	0.0	Strain Rate: 0.05 inch/minute		
0.025	127	42.4	Piston Diameter: 1.954 inch		
0.050	248	82.7	Piston Area: 2.999 sq. inch		
0.075	406	135.4			
0.100	548	182.7			
0.125	683	227.8			
0.150	817	272.4	<u>Swell Readings</u>		
0.175	947	315.8	Soak Time: 96 hours		
0.200	1072	357.5	Surcharge Weight: 10 lb		
0.225	1178	392.8	Surcharge Stress: 51 psf		
0.250	1268	422.8	Molded Sample Height: 4.594 inch		
0.275	1365	455.2	Initial Dial Reading: 0.050 inch		
0.300	1455	485.2	Final Dial Reading: 0.050 inch		
0.400	1790	596.9	Percent Swell: 0.00%		
0.500	2082	694.3			
<u>Readings After Soak</u>		<u>Additional Specimen Data</u>			
Wt. Mold + Soaked Soil:	21150 g	Liquid Limit:	19	Percent Passing #4:	%
Wt. Tare:	6 g	Plastic Limit:	14	Percent Passing #10:	100%
Wt. Wet Soil + Tare:	176 g	Plasticity Index:	5	Percent Passing #40:	86%
Wt. Tare + Dry Soil:	161 g			Percent Passing #200:	36.9%
Moisture Content:	9.8%				
Wet Unit Weight:	137.0 pcf				
Dry Unit Weight:	124.8 pcf				
BEARING RATIO		Color:	Gray		
		Visual Description:			
		USCS Classification:	SILTY, CLAYEY SAND (SC-SM)		
		AASHTO Classification:	A-4 (0)		
CBR at 0.1 inch:		18.3			
CBR at 0.2 inch:		23.8			

207 REPORT SHEET - CBR G24042.00.GPJ FALCON\_FORMAT.GDT 7/16/24