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### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

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### **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY ROCK	INGHAM				
PROJECT DESCR		PLACE	BRIDGE	<i>163</i>	ON
US 158 OVER					

STATE PROJECT REFERENCE NO. BR-0155

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THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

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  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

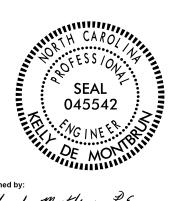
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ESPGNATURE

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REPERENCE NO. SHEET NO. 2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

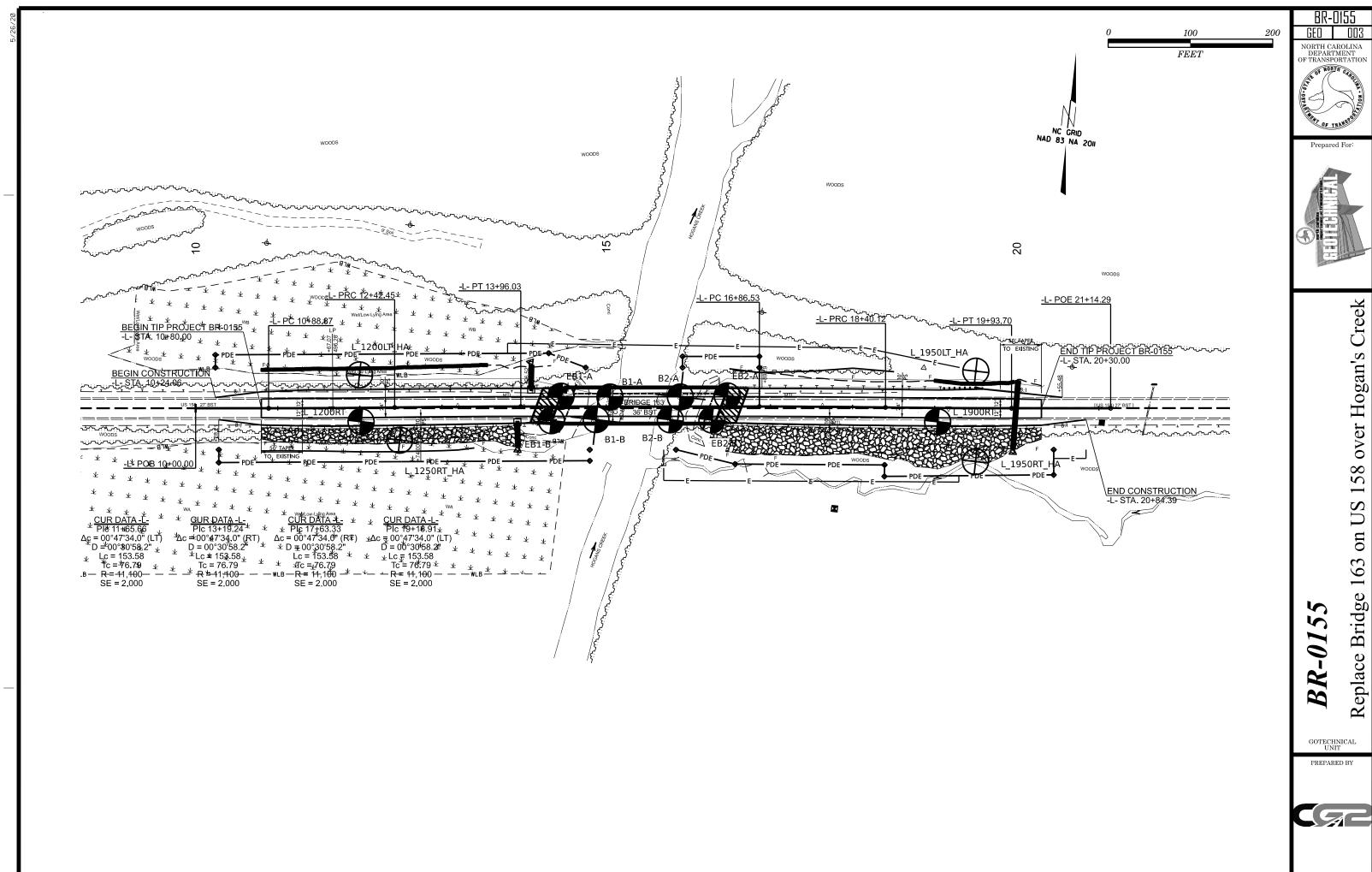
SOIL DESCRIPTION	GRADATION  WELL CRAPED - INDICATES A COOR REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE	ROCK DESCRIPTION  HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	TERMS AND DEFINITIONS				
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	HELOVIOM THELOVA, SOILS THAT HAVE BEEN TRANSFORTED BY WHIER.				
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.				
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.				
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF.GRAY.SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC.A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	SU/EU/A	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.				
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED VILLY NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT				
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE CRYSTA	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND				
LLASS. (\$39% PASSING "200) (>30% PASSING "200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.				
CROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-6 A-2-7 A-4, A-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.  COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM				
000000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.				
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED				
% PASSING SILT-GRANULAR SILT-MUCK,	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.				
#40 30 MX 50 MX 51 MN   SOILS CLAT PEAT	GRANULAR SILT - CLAY	- WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.				
אורים לפל אורים	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE				
MATERIAL PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.				
LL - 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 10 LITTLE OB	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.				
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE OPENALS	GROUND WATER	OF A CRYSTALLINE NATURE.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE				
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.				
USUAL TYPES STUNE FRAUS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.				
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM				
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.				
AS SUBURADE PUUR	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.				
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.				
DANCE OF STANDARD DANCE OF UNCONFINED	MISCELEMILOUS STRIBUES	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.				
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE)  25/025  DIP & DIP DIRECTION  OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	<u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO				
(N-VALUE) (TONS/FT <sup>2</sup> )  VERY LOOSE < 4		SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.				
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL  SOIL SYMBOL  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.				
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.				
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE				
VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING ■ SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.				
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MN MONITORING WELL TEST BORING	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.				
MATERIAL STIFF 8 TO 15 1 TO 2	WITH CORE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE				
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4  HARD > 30 > 4	TTTTT ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY INSTALLATION SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.				
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.				
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNCLASSIFIED EXCAVATION - TAN UNCLASSIFIED EXCAVATION -	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND				
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	WX SUBJECT VALUE WASTE WASTE ACCEPTABLE, BUT NOT TO BE	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO				
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNCLASSIFIED EXCAVATION - UNDERCUT UNCLASSIFIED EXCAVATION - EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.				
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.				
GRAIN MM 305 75 2.0 0.25 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF				
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL				
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY  MOD MODERATELY  7 - UNIT WEIGHT  CPT - CONE PENETRATION TEST  NP - NON PLASTIC  7 - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.				
SOIL MOISTURE SCALE FIELD MOISTURE CHIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY				
(ATTERBERG LIMITS) DESCRIPTION GOIDE FOR FIELD MOISTORE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.				
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY				
LL LIQUID LIMIT	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.				
PLASTIC   SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.				
(PI) PL PLASTIC LIMIT	FRAGS FRAGMENTS	FRACTURE SPACING         BEDDING           1ERM         SPACING         1ERM         THICKNESS	BENCH MARK:				
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: FEET				
OM _ OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE					
SL SHRINKAGE LIMIT	CME-45C CLAY BITS AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:				
- DRY - (D)  REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6° CONTINUOUS FLIGHT AUGER CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	ROADWAY DESIGN FILES PROVIDED BY NCDOT DATED DECEMBER 2, 2024.				
PLASTICITY	CME-55	INDURATION	BRIDGE BORING COLLAR ELEVATION OBTAINED USING CARLSON BR×7 GPS.				
. = : : = : : :		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.					
PLASTICITY INDEX (PI) DRY STRENGTH  NON PLASTIC 0-5 VERY LOW	TUNG-CARRIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS:	FIAD = FILLED IMMEDIATELY AFTER DRILLING				
SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST X CASING X W/ ADVANCER HAND TOOLS:	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.					
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	POST HOLE DIGGER	MODERATELY INDURATED  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  PROMO FACE FACILITY WHEN HIT WITH HAMMER					
COLOR	1 —   AAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.					
	X MOBILE B-29 TRICONE 'TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER.					
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X CORE BIT VANE SHEAR TEST	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;					
HODE IENS SOCIERS EIGHT, DANK, STREAKED, ETC. HAE USED TO DESCRIBE HEFEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-				

PROJECT REFERENCE NO.	SHEET NO.
BR-0155	2A

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	ted Rock Mass (Mar				OGE DESIGN SPECIFICATIONS  AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Defo	ormed Heterogeneous Rock	Masses (Marinos and Hoe	ek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)	у Ф О	D.	8 & C	aces	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)			
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS  VERY GOOD  Very rough, fresh unweathered surface	COOD Rough, slightly weathered, iron stained surfaces Surfaces FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfac with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfactions soft clay coatings or fillings	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.	VERY GOOD - Very Rough, fresh unweathered surfaces GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	R - Very sr highly wear
STRUCTURE	DE	CREASING SURFACE (		<b>⇒</b>	COMPOSITION AND STRUCTURE			
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	BIECES 80		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70 A		
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	OF ROCK P	70 60			8. Sand- stone with stone and thin inter- sultstone sultstone with sand- with sand- with sand- with sand-	50 B	C D E	
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	OCKING	50			layers of sultstone siltstone with state with state with sandstone layers	40	C / D / E	
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL	40	30		C.D.E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.		30 F 20	
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECREK		20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers  While III H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed		<b>F</b>	H 10
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	<b>∛</b> N/A	N/A		10	into small rock pieces.  Means deformation after tectonic disturbance			DATE: 8-19-



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION



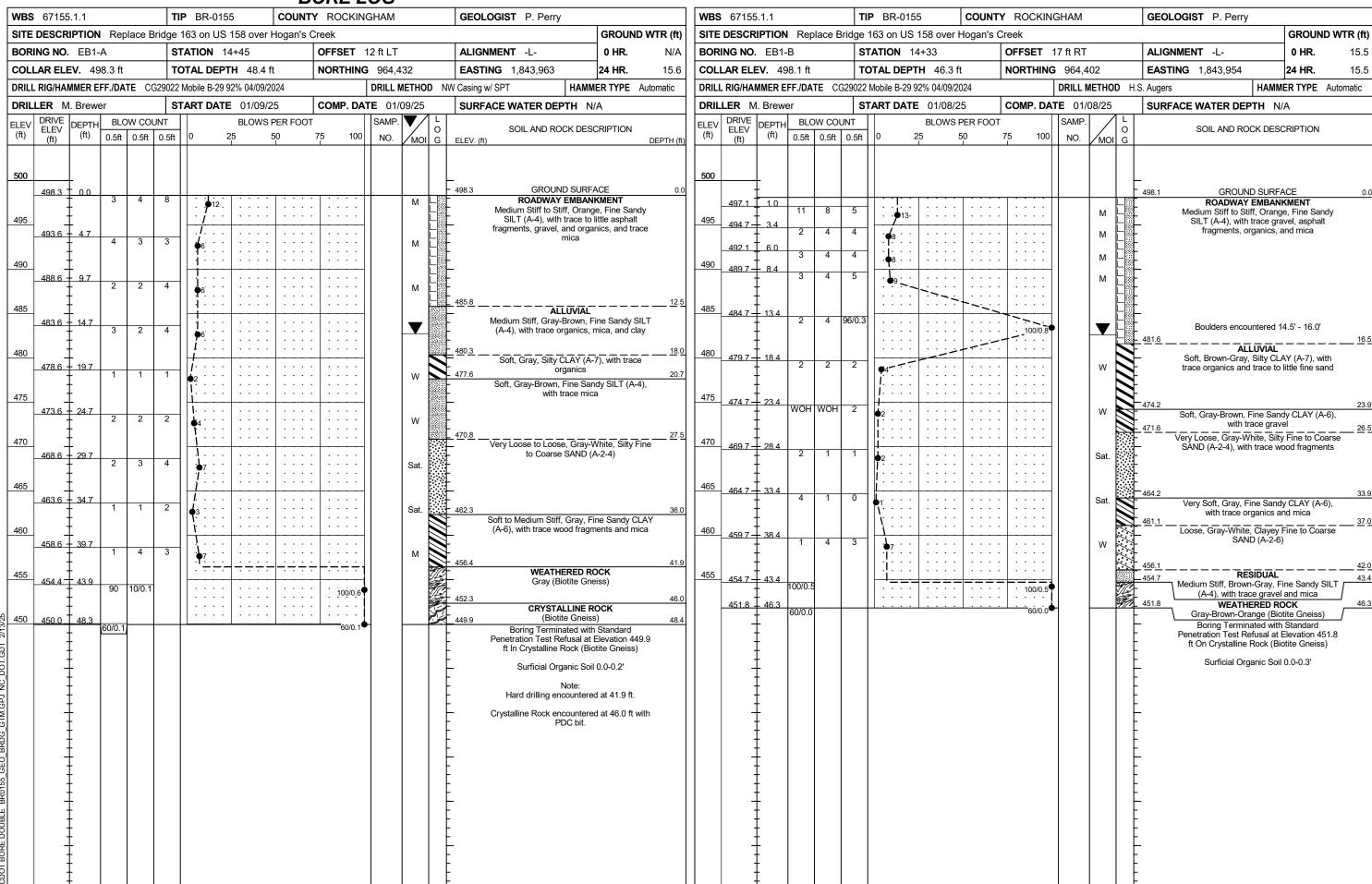




over Hogan's Creek

Wide   Color   True   Design   Color
DRILLER NA
COLLAR ELEY   494.2   1
DRILL RIGHAMMER EFF.DATE   NA
DRILLER NA   START DATE   0.103/25   COMP. DATE   0.103/25   SURFACE WATER DEPTH NA
ELEV   CREV   CR
W   (r)   W   O   O   Sh   O

									ORE L								
WBS	67155	.1.1			TI	<b>P</b> BR-015	5	COUNT	Y ROCKING	SHAM			GEOLOGI	ST M. Malis	sher		
SITE	DESCR	IPTION	Repl	ace Br	idge 16	63 on US 1	58 over Ho	gan's Cre	ek							GROUND V	VTR (ft)
BORI	NG NO.	L 125	50RT	HA	S	TATION 1	2+50		OFFSET 4	10 ft RT			ALIGNME	NT -L-		0 HR.	0.5
	AR ELE				_	OTAL DEP			NORTHING		 58			1,843,773		24 HR.	FIAD
	RIG/HAN			Ε N/Λ					11011111111	1		<b>.</b> ⊔ <sub>2</sub>	nd Auger	.,0.0,0	ППУММЕ	R TYPE N/A	
	LER N		וואטויו	L IN/A		TART DAT	E 04/02/3	) <i>E</i>	COMP. DA			) IIa		WATER DE			
			DI C	NA/ 00		HARI DAT			L	_	J3/25	1 L T	SURFACE	WATER DE	PIN N/A	4	
(ft)	DRIVE ELEV (ft)	DEPTH (ft)		0.5ft		0		PER FOO	75 100	SAMP. NO.	MOI	0	ELEV. (ft)	SOIL AND RO	OCK DESC		DEPTH (ft
485		-										-	_				
	-												484.0		ND SURFA	ACE	0.0
	-	-									W		_ Ve	ry Soft, Brown- (A-6), with trac	LLUVIAL Gray, Fine ce organics	Sandy CLAY and mica	
											''						
	•					1	1						480.5	ing Terminate	nd at Elevat	ion 480 5 ft In	3.5
	-	-											-	Alluvial S	andy CLAY Note:	′ (A-6)	
	-	-											. На -	nd auger term Multiple o	inated due ffsets atter	hole cave-in. npted.	
	-	-											-				
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	<i>D</i>	ORE LOG	
<b>WBS</b> 67155.1.1	TIP BR-0155 COUNT	Y ROCKINGHAM	GEOLOGIST P. Perry
SITE DESCRIPTION Replace Br	idge 163 on US 158 over Hogan's (	Creek	GROUND WTR (ft)
BORING NO. B1-A	STATION 15+05	OFFSET 12 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 483.6 ft	TOTAL DEPTH 31.4 ft	<b>NORTHING</b> 964,438	<b>EASTING</b> 1,844,022 <b>24 HR.</b> 0.9
DRILL RIG/HAMMER EFF./DATE CG	29022 Mobile B-29 92% 04/09/2024	DRILL METHOD NW	Casing w/ SPT HAMMER TYPE Automatic
DRILLER M. Brewer	<b>START DATE</b> 01/09/25	<b>COMP. DATE</b> 01/09/25	SURFACE WATER DEPTH N/A
DRIVE   DEPTH   BLOW COU	NT BLOWS PER FOOT 0.5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION  ELEV. (ft) DEPTH (ft)
483.6 + 0.0			483.6 GROUND SURFACE 0.0
480	1 1		ALLUVIAL  Very Soft, Gray-Brown, Slity CLAY (A-7),  with trace fine to coarse sand, gravel, and  organics
475	0 1	W   -	Very Soft, Orange-Brown, Fine to Coarse Sandy SILT (A-4)  475.6 Medium Dense, White-Gray, Silty Fine to
473.7 + 9.9 2 8	4	W	Coarse SAND (A-2-4) - 13.4
468.7 + 14.9 4 1	3	Sat.	Loose, Gray-White-Orange, Clayey Fine to Coarse SAND (A-2-6), with trace gravel  466.1  RESIDUAL
463.7 19.9 7 11	29	M	Hard, Orange, Fine Sandy SILT (A-4), with trace to little mica
456 0 + 27 6	0/0.2	. 100/0.7	458.2 25.  WEATHERED ROCK  Brown-Gray-Orange, (Biotite Gneiss)
455 454.7 — 28.9 60 40/0.3		100/0.8	452.2 31.  Boring Terminated with Standard
			Penetration Test Refusal at Elevation 452.2 ft On Crystalline Rock (Biotite Gneiss)

## GEOTECHNICAL BORING REPORT

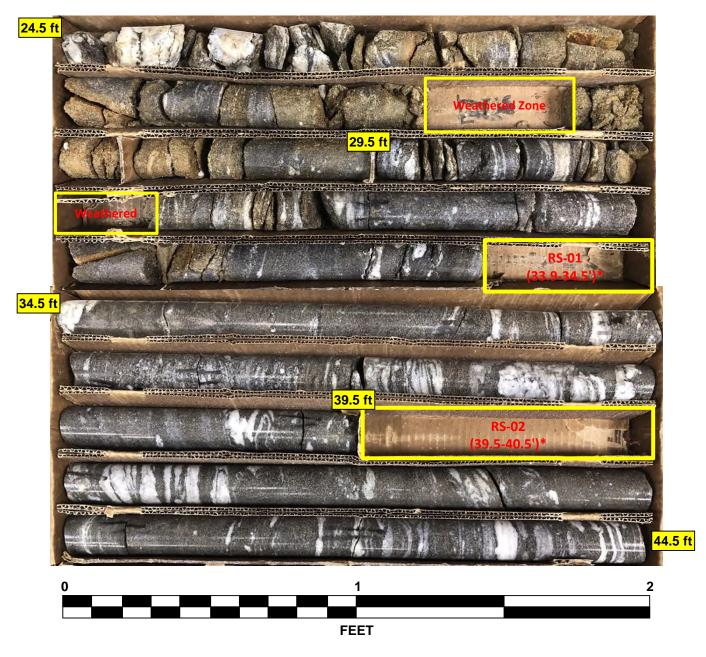
#### **BORE LOG** COUNTY ROCKINGHAM **WBS** 67155.1.1 **TIP** BR-0155 **GEOLOGIST** T. Wenner SITE DESCRIPTION Replace Bridge 163 on US 158 over Hogan's Creek **GROUND WTR (ft)** ALIGNMENT -L-**STATION** 14+88 OFFSET 16 ft RT **BORING NO.** B1-B 0 HR. N/A COLLAR ELEV. 484.5 ft TOTAL DEPTH 44.5 ft **NORTHING** 964,409 **EASTING** 1,844,008 24 HR. 1.5 DRILL RIG/HAMMER EFF./DATE CG29022 Mobile B-29 92% 04/09/2024 **DRILL METHOD** NW Casing W/SPT & Core HAMMER TYPE Automatic DRILLER M. Brewer **START DATE** 01/13/25 COMP. DATE 01/13/25 SURFACE WATER DEPTH N/A ELEV DRIVE DEPTH BLOW COUNT SAMP. **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G (ft) ELEV. (ft) DEPTH (ft **GROUND SURFACE** ALLUVIAL Soft, Brown-Gray. Fine Sandy SILT (A-4), with trace mica and organics 480 479.5 W Very Loose to Loose, Gray, Silty Fine to 475 474.5 Coarse SAND (A-2-4), with trace gravel W 470 469.5 + 15.0 W RESIDUAL W Very Stiff, Tan, Fine to Coarse Sandy SILT 464.5 + 20.0 48 52/0.4 (A-4) with trace mica WEATHERED ROCK 100/0.8 460 460.0 24.5 60/0.0 CRYSTALLINE ROCK Brown-Gray, (Biotite Gneiss) 27.7 28.7 REC = 100% RQD = 0% 455 GSI = 15-20 WEATHERED ROCK Brown-Gray, (Biotite Gneiss) 450 CRYSTALLINE ROCK RS-01 Gray-White, (Biotite Gneiss) REC = 98% RQD =75% 445 GSI = 55-60 RS-02 440 Boring Terminated at Elevation 440.0 ft In Crystalline Rock (Biotite Gneiss)

									<u></u>	U	RE LOG
WBS	67155	5.1.1			TIP	BR-0	155	C	OUNT	Υ	ROCKINGHAM GEOLOGIST T. Wenner
SITE	DESCR	IPTION	Rep	lace Brid	ge 16	3 on U	S 158 ov	er Hog	gan's (	Cree	eek GROUND WTR (
BOR	ING NO.	B1-B			STA	TION	14+88			OF	DFFSET 16 ft RT ALIGNMENT -L- 0 HR. N/
	LAR ELI						<b>PTH</b> 44			NC	NORTHING 964,409 EASTING 1,844,008 24 HR. 1
DRILI	RIG/HAI	MMER E	FF./DA	TE CG29	022 Mo	bile B-2	9 92% 04/0	9/2024			DRILL METHOD NW Casing W/SPT & Core HAMMER TYPE Automatic
DRIL	LER M	l. Brewe	er		STAI	RT DA	<b>TE</b> 01/1	3/25		C	COMP. DATE 01/13/25 SURFACE WATER DEPTH N/A
COR	E SIZE	NQ			TOT	AL RU	<b>N</b> 20.0 f				
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft)	JN RQD (ft) %	SAMP. NO.	REC. (ft)	RQD (ft) %	L O G	D DESCRIPTION AND REMARKS
460										_	Begin Coring @ 24.5 ft
455	460.0 - - 455.0	24.5	5.0	N=60/0.0 4:50/1.0 1:37/1.0 1:17/1.0 1:50/1.0 3:02/1.0	(4.5) 90%	(0.4) 8%		(3.2) 100% (0.5) \ 50% /	(0.0)		460.0 CRYSTALLINE ROCK  Moderately Severe Weathering to Moderate Weathering, Soft to Moderately  Hard, Brown-Gray, (Biotite Gneiss), with Very Close to Close Fracture  Spacing  2 2 2 2
450	450.0	34.5	5.0	2:43/1.0 3:27/1.0 4:18/1.0 4:55/1.0 4:55/1.0	94%	(1.7) 34%	DC 04	(15.5) 98%	(11.9) 75%		GSI = 15-20  WEATHERED ROCK  Brown-Gray, (Biotite Gneiss)  CRYSTALLINE ROCK
	-		5.0	4:33/1.0 3:13/1.0 4:43/1.0		(4.9) 98%	RS-01		COUNTY r Hogan's Cro ft // 2024 // 25  STRATA REC. RQD (ft) (ft) (ft) (3.2) (0.0) (0.0) (0.5) (0.0) (0.5) (0.0) (0.5) (0.0) (15.5) (11.9)		Moderate Weathering to Fresh, Moderately Hard to Hard, Gray-White (Biotite Gneiss), with Close to Wide Fracture Spacing  GSI = 55-60
445	445.0	39.5	5.0	5:15/1.0 3:03/1.0 2:50/1.0	(5.0)	(4.9)	RS-02				RS-01: 33.9'-34.5'
440	440.0	44.5		3:33/1.0 1:53/1.0 3:06/1.0 4:30/1.0	100%	98%					Unit Weight: 173.0 pcf Unconfined Compressive Strength: 9,180 psi (1,320 ksf)  RS-02: 39.5'-40.5'
											Unit Weight: 171.8 pcf Unconfined Compressive Strength: 10,560 psi (1,520 ksf) Boring Terminated at Elevation 440.0 ft in Crystalline Rock (Biotite Gneiss)



### BR-0155 - Replace Bridge 163 on US 158 over Hogans Creek Rockingham County, North Carolina Rock Core Photographs Boring: B1-B

24.5 to 44.5 Feet



\*Note: Sample was removed before the rock core box could be photographed.

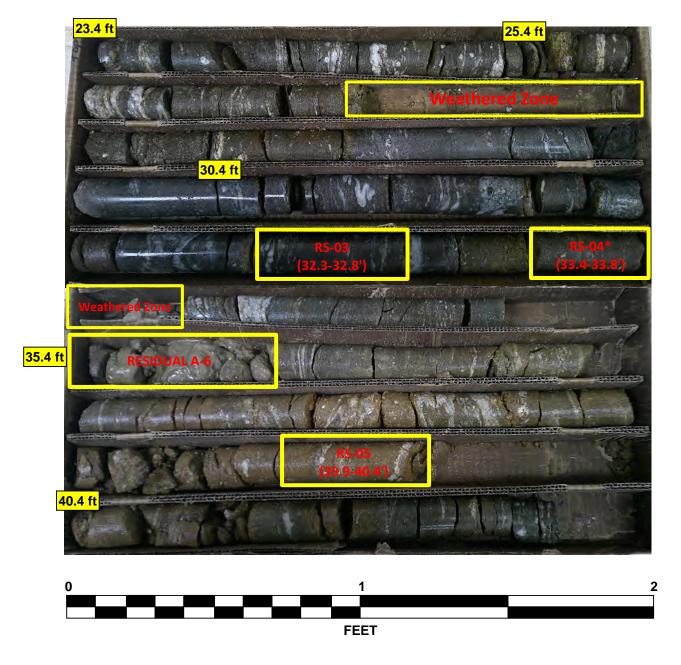
#### COUNTY ROCKINGHAM **WBS** 67155.1.1 **GEOLOGIST** P. Perry **TIP** BR-0155 SITE DESCRIPTION Replace Bridge 163 on US 158 over Hogan's Creek **GROUND WTR (ft) STATION** 15+90 OFFSET 12 ft LT ALIGNMENT -L-BORING NO. B2-A 0 HR. N/A COLLAR ELEV. 486.3 ft TOTAL DEPTH 50.4 ft **NORTHING** 964,447 **EASTING** 1,844,107 24 HR. 3.0 DRILL RIG/HAMMER EFF./DATE CG29022 Mobile B-29 92% 04/09/2024 **DRILL METHOD** NW Casing W/SPT & Core HAMMER TYPE Automatic DRILLER M. Brewer **START DATE** 01/10/25 **COMP. DATE** 01/10/25 SURFACE WATER DEPTH N/A SAMP. **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G ELEV. (ft) DEPTH (ft 490 GROUND SURFACE ALLUVIAL 485 Soft, Brown-Gray, Fine Sandy CLAY (A-6), $\blacksquare$ with trace organics and gravel Very Soft, Gray-Green, Fine Sandy SILT (A-4), with trace organics and mica WOH WOH WOH Sat. Very Soft, Gray, Fine Sandy CLAY (A-6), with trace mica and organics WOH WOH . . . . 475 W RESIDUAL Very Stiff, Orange-Brown-White, Fine Sandy 6 6 М SILT (A-4), with trace to little mica 470 . . . . 466.3 7 20.0 13 87/0.4 WEATHERED ROCK -100/0.9 Brown-Orange-White (Biotite Gneiss) 462.9 1 23.4 60/0.0 CRYSTALLINE ROCK Gray-Brown-White (Biotite Gneiss) 460 26.7 27.7 REC = 75% RQD = 0% GSI = 15-20 WEATHERED ROCK 455 Orange-Brown (Biotite Gneiss) CRYSTALLINE ROCK RS-03 Gray-Brown-Orange (Biotite Gneiss) 34.3 35.4 35.9 . . . . 450 REC = 98% RQD = 48% GSI = 35-40 WEATHERED ROCK Brown-Orange (Biotite Gneiss) RS-05 445 CRYSTALLINE ROCK 42.1 RS-06 Brown-Orange-White (Biotite Gneiss) REC = 100% 440 RQD = 0% GSI = 15-20 . . . . RESIDUAL Soil Seam from 35.4' - 35.9' 50.4 CRYSTALLINE ROCK Orange-Brown-White (Biotite Gneiss) REC = 100% RQD = 13% GSI = 20-25 Gray (Biotite Gneiss) REC = 99% **RQD = 94%** GSI = 75-80 Boring Terminated at Elevation 435.9 ft In Crystalline Rock (Biotite Gneiss)

DRILLER M. Brewer	I
BORING NO.   B2-A   STATION   15+90   OFFSET   12 ft LT   ALIGNMENT -L-	
COLLAR ELEV. 486.3 ft   TOTAL DEPTH   50.4 ft   NORTHING   964,447   EASTING   1,844,107	GROUND WTR (ft)
DRILL RIGHHAMMER EFF.JDATE   CG29022 Mobile B-29 92% 04/09/2024   DRILL METHOD NW Casing WISPT & Core   FIA	<b>0 HR.</b> N/A
DRILLER M. Brewer   START DATE   01/10/25   COMP. DATE   01/10/25   SURFACE WATER DEPTH	<b>24 HR.</b> 3.0
CORE SIZE   NQ   TOTAL RUN   27.0 ft	MMER TYPE Automatic
ELEV   RUN   City   C	N/A
ELEV (ft)   Chi (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	
462 9 23.4 2.0 N=60/0.0   1.50   0.00   88	DEPTH (ft)
460   25.4   3.29/10   75%   0%   88%   0%   45.9   30.4   1.08/10	23.4
455.9 30.4 2:101.0 2:101.0 3:18/1.0 4.55    455.9 30.4 3:18/1.0 4.55    5.0 3:18/1.0 4.55    450.9 35.4 5.0 3:18/1.0 4.55    5.0 2:22/1.0 10.0 16%   2:29/1.0 10.0 16%   2:29/1.0 10.0 16%   2:29/1.0 10.0 16%   2:29/1.0 10.0 16%   3:100% 10% 16%   3:100% 10% 16%   3:100% 10% 16%   3:100% 10% 16%   3:100% 10% 16% 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.	d, Very Soft to Soft,
1.50	
445.9	
445.9	35.9
440.9 45.4 1.54/1.0 2:33/1.0 1.54/1.0 (5.0) 435.9 50.4 1.29/1.0 1.29/1.0 2.55/1.0 1.29/1.0 1.	1.010 ksf) 42.1
2:32/1.0 2:50/1.0 2:50/1.0 1:29/1.0 96%  435.9 50.4 1:29/1.0 1:29/1.0 GSI = 15-20  RS-04: 33.4-33.8' (RS-04 did not produce a qualified sample for RESIDUAL Soil Seam from 35.4' - 35.9' CRYSTALLINE ROCK Moderately Severe to Moderately Weathered, Moderately Fractive (Biotite Gneiss), with Very Close to Close Fractue (RS-04 did not produce a qualified sample for CRYSTALLINE ROCK Moderately Severe to Moderately Weathered, Very Soft to Orange-Brown-White (Biotite Gneiss), with Very Close Spacing GSI = 20-25	
RS-04: 33.4-33.8'  (RS-04 did not produce a qualified sample for RESIDUAL  Soil Seam from 35.4' - 35.9'  CRYSTALLINE ROCK  Moderately Severe to Moderately Weathered, Very Soft to Orange-Brown-White (Biotite Gneiss), with Very Close Spacing  GSI = 20-25	
Soil Seam from 35.4' - 35.9'  CRYSTALLINE ROCK  Moderately Severe to Moderately Weathered, Very Soft t  Orange-Brown-White (Biotite Gneiss), with Very Close Spacing  GSI = 20-25	
Orange-Brown-White (Biotite Gneiss), with Very Close Spacing  GSI = 20-25	
	o Moderately Hard, to Close Fracture
Unit Weight: 154.5 pcf Unconfined Compressive Strength: 970 psi (1	ray (Biotite Gneiss),
with Close to Moderately Close Fracture Sp  GSI = 75-80	pacing
RS-06: 42.2'-43.0' Unit Weight: 167.0 pcf	
Unconfined Compressive Strength: 11,540 psi ( Boring Terminated at Elevation 435.9 ft In Crystalline Ro	
Somiginal and a control of the contr	on (Blothe Greiss)



### BR-0155 - Replace Bridge 163 on US 158 over Hogans Creek Rockingham County, North Carolina Rock Core Photographs Boring: B2-A

23.4 to 50.4 Feet

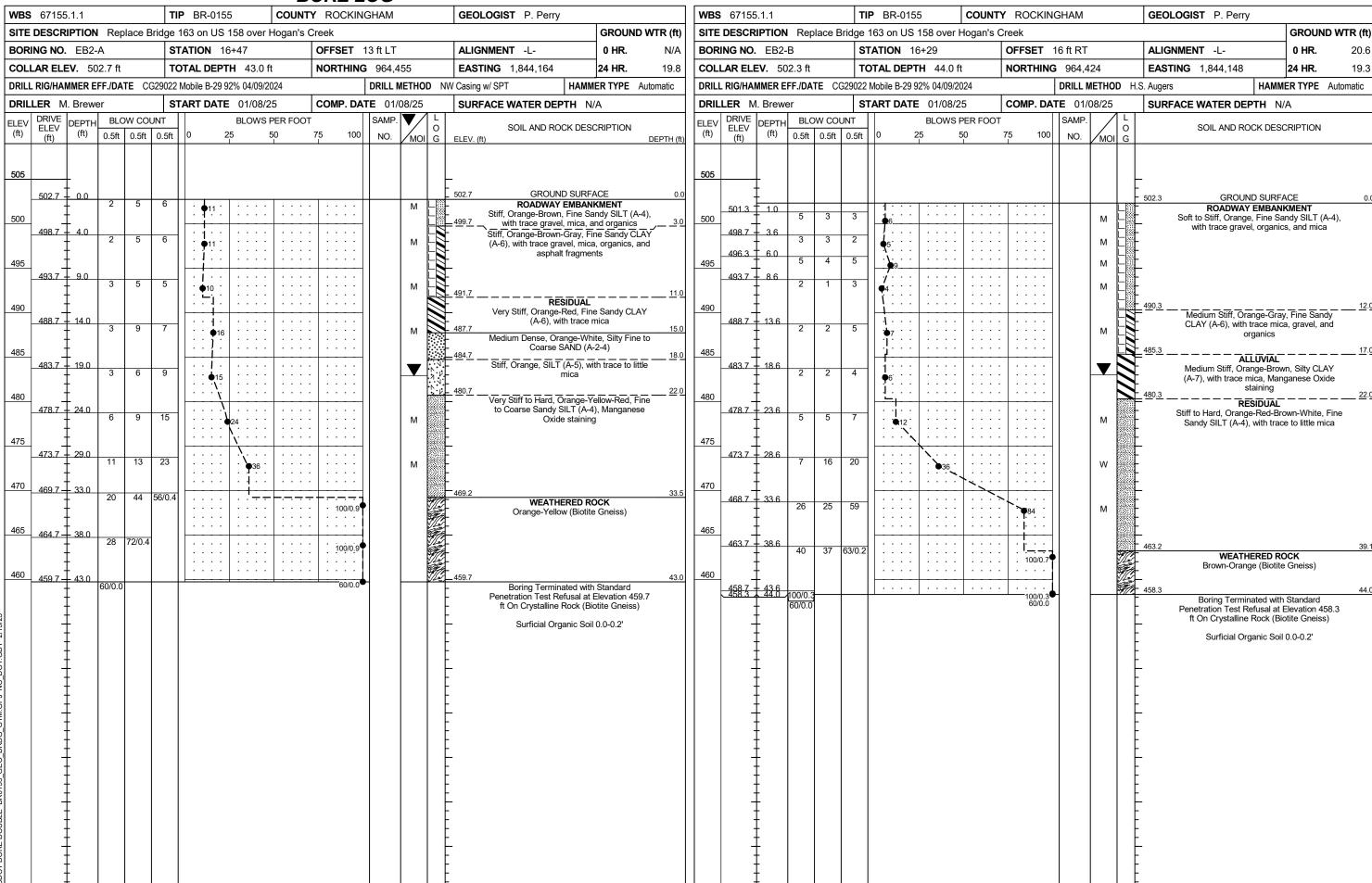






\*NOTE: Sample RS-04 was unable produce a qualified sample for testing during trimming.

	<b>D</b>	ORE LOG	
<b>NBS</b> 67155.1.1	TIP BR-0155 COUNT	ROCKINGHAM	GEOLOGIST P. Perry
SITE DESCRIPTION Replace Br	idge 163 on US 158 over Hogan's (	Creek	GROUND WTR (ft)
BORING NO. B2-B	STATION 15+78	OFFSET 16 ft RT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 486.1 ft	TOTAL DEPTH 29.4 ft	<b>NORTHING</b> 964,419	<b>EASTING</b> 1,844,098 <b>24 HR</b> . 3.5
DRILL RIG/HAMMER EFF./DATE CG2	29022 Mobile B-29 92% 04/09/2024	DRILL METHOD NW	Casing w/ SPT HAMMER TYPE Automatic
DRILLER M. Brewer	<b>START DATE</b> 01/10/25	COMP. DATE 01/10/25	SURFACE WATER DEPTH N/A
ELEV CHI	NT BLOWS PER FOOT 0.5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION  ELEV. (ft)  DEPTH (f
490			
486.1 0.0 1 1 1	1 2		486.1 GROUND SURFACE 0.  ALLUVIAL Soft, Orange-Gray-Brown, Fine to Coarse
481.1 5.0 1 1	1 02		Sandy SILT (A-4), with trace organics and gravel and trace to little mica
476.1 10.0 WOH WOH V	<del> </del>	Sat. Sat.	477.6 Very Loose, Gray, Silty Fine SAND (A-2-4)
471.1 15.0 12 26	34		472.6
467.4 18.7 70 30/0.1		100/0.6♥	467.4 WEATHERED ROCK Orange-Brown-White (Biotite Gneiss)
461.7 + 24.4   60/0.0			461.7 24.  CRYSTALLINE ROCK  (Pictite Chairs)
456.7 + 29.4   60/0.0		60/0.0	(Biotite Gneiss)  456.7 29.  Boring Terminated with Standard
			Penetration Test Refusal at Elevation 456.7 ft In Crystalline Rock (Biotite Gneiss)



									D	UK	$\vdash$	UG							
WBS	67155	.1.1			TIE	<b>P</b> BR-0	155		COUNT	Y RO	CKING	HAM			GEOLOGIST P. Perr	у			
SITE	DESCRI	PTION	Repla	ace Br	idge 16	3 on US	158 ov	/er Hog	jan's Cre	ek							GROUN	ID WTR	₹ (ft)
BORI	NG NO.	L_190	00RT		ST	ATION	19+04	1		OFFS	SET 1	7 ft RT			ALIGNMENT -L-		0 HR.		Dry
COLI	AR ELE	<b>V</b> . 51	1.3 ft		TC	TAL DE	PTH	13.0 ft		NOR	THING	964,4	52		<b>EASTING</b> 1,844,422		24 HR.		Dry
DRILL	RIG/HAM	MER EF	F./DATE	CG2	9022 M	obile B-29	92% 04	/09/2024	1			DRILL M	ETHO	) H.S	. Augers	HAMN	IER TYPE	Automat	tic
DRIL	LER M	. Brewe	er		ST	ART DA	ATE 0	1/08/2	5	СОМ	P. DA1	<b>E</b> 01/0	)8/25		SURFACE WATER D	EPTH N	/A		
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLO 0.5ft	W CO		0	BL 25		PER FOO	T 75	100	SAMP. NO.	MOI	L O G	SOIL AND F	OCK DES	CRIPTION	DEP	TH (f
515	- - -	- - -												-	- 511.3 GROL	IND SURF	FACE		0.
510	510.3	1.0	3	3	3	1							М		ROADWA Loose to	Y EMBAN Medium	IKMENT Dense,		
	507.8 -	- - 3.5	4	5	8	٠,٠.				-		SS-34	15%		Orange-Gray-Bi SAND (A-2-4), y	own, Silty vith trace r	Fine to Coa	arse nics,	
505	505.3	6.0	4	3			3 .	: : : 				33-34			clay, and -	asphalt fra	agments		
	- 502.8 -	- - 8.5			3	<b>6</b> 6	<u> -   :   :   :                         </u>			.			M		503.3	RESIDUAL			8
500	-	-	3	6	16		22			-			М		Very Stiff, Orang (A-4), with ti	e-Brown, F	ine Sandy	SILT	
300	498.3	- - 13 N					<del>-   -</del>								- (A-4), with the fragm	ents and r	mica	•	13.
	- - - - - - - -	- - - - - - - - -	60/0.0								60/0.0♥			-	Boring Tern Penetration Test ft On Crystallir Surficial C	Refusal at e Rock (B	Elevation 4 liotite Gneis	498.3	
															-				
		- - - - - - - - - -													-				

	BORE	i				
<b>WBS</b> 67155.1.1	TIP BR-0155 COUNTY ROCKI	KINGHAM GEOLOGIST M. Malisher		<b>WBS</b> 67155.1.1	TIP BR-0155 COUNTY ROCKINGHAM	GEOLOGIST M. Malisher
SITE DESCRIPTION Replace Brid	dge 163 on US 158 over Hogan's Creek		GROUND WTR (ft)	SITE DESCRIPTION Replace Bridg	e 163 on US 158 over Hogan's Creek	GROUND WTR (ft)
BORING NO. L_1950LT_HA	STATION 19+50 OFFSET	ET 45 ft LT ALIGNMENT -L-	<b>0 HR</b> . Dry	BORING NO. L_1950RT_HA	STATION 19+50 OFFSET 65 ft RT	ALIGNMENT -L- 0 HR. 1.6
COLLAR ELEV. 520.4 ft	TOTAL DEPTH 6.4 ft NORTHII	HING 964,518 EASTING 1,844,461	24 HR. FIAD	COLLAR ELEV. 489.8 ft	TOTAL DEPTH 5.5 ft NORTHING 964,409	<b>EASTING</b> 1,844,473 <b>24 HR.</b> FIAD
DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD Hand Auger HAMME		DRILL RIG/HAMMER EFF./DATE N/A	DRILL METHOD	Hand Auger HAMMER TYPE N/A
DRILLER N/A		SURFACE WATER DEPTH N/A	4	DRILLER N/A	START DATE 01/03/25 COMP. DATE 01/03/25	SURFACE WATER DEPTH N/A
ELEV CRIVE CHARACTER SECULOR COUNTY COUNTY CRIP CRIP COUNTY CRIP C	I	100 SAMP. L O SOIL AND ROCK DESC	CRIPTION DEPTH (ft)	DRILLER         N/A           ELEV (ft)         DRIVE ELEV (ft)         DEPTH (ft)         BLOW COUN           0.5ft         0.5ft         0		O SOIL AND ROCK DESCRIPTION G
525				490		_489.8 GROUND SURFACE 0.0
520		520.4 GROUND SURFA RESIDUAL Soft to Medium Stiff, Red-Bi	Brown. Fine to	485		ALLUVIAL  Very Loose to Loose, Brown-Gray, Silty Fine to Coarse SAND (A-2-4), with trace mica, roots, and gravel  484.3
515		Coarse Sandy CLAY (A-6), wand roots  Medium Dense, Red-Brow SAND (A-2-4), with tra	with trace mica			Boring Terminated at Elevation 484.3 ft In Alluvial Silty SAND (A-2-4) Surficial Organic Soil 0.0-0.1'
		514.0  Boring Terminated at Elevat Residual Silty SAND  Surficial Organic Soil	(A-2-4)			- - - - - -
NCDOT BORE DOUBLE BR0155_GEO_BRDG_GTM.GPJ NC_DOT.GDT 2/11/25						

PROJECT REFERENCE NO.	SHEET NO.
BR-0155	16

	SOIL TEST RESULTS																	
BORING	SAMPLE	OFFSET	CTATION	NODWIING	EASTING	DEPTH	AASHTO	7.7	рт	% BY WEIGHT			% PASSING (SIEVES)		%	%		
ID	NO.	OFFSEI	STATION	NORTHING		INTERVAL	CLASS.	L.L.	P.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
L_1200RT	SS-1	17' RT	12+01 -L-	964375	1843724	1.0 - 2.5'	A-2-4	32	5	24.3	42.3	23.3	10.1	59.2	51.1	23.6	11.8	-
$L_{1900RT}$	SS-34	17' RT	19+04 -L-	964452	1844422	3.5 - 5.0'	A-2-4	32	8	29.2	33.0	23.5	14.3	79.8	65.1	33.6	15.1	-

	$ROCK\ TEST\ RESULTS$								
SAMPLE NO.*	BORING	STATION	OFFSET	NORTHING	EASTING	DEPTH INTERVAL	ROCK TYPE	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH
RS-01	B1-B	14+88 -L-	16' RT	964409	1844008	33.9 – 34.5'	BIOTITE GNEISS	173.0	9,180 psi (1,320 ksf)
RS-02	B1-B	14+88 -L-	16' RT	964409	1844008	39.5 - 40.5'	BIOTITE GNEISS	171.8	10,560 psi (1,520 ksf)
RS-03	B2–A	15+90 -L-	12' LT	964447	1844107	32.3 - 32.8'	BIOTITE GNEISS	170.7	7,020 psi (1,010 ksf)
RS-05	B2–A	15+90 -L-	12' LT	964447	1844107	39.9 - 40.4	BIOTITE GNEISS	154.5	970 psi (140 ksf)
RS-06	B2–A	15+90 -L-	12' RT	964447	1844107	42.2 - 43.0'	BIOTITE GNEISS	167.0	11,540 psi (1,660 ksf)

<sup>\*</sup>NOTE: RS-04 SPECIMEN WAS UNABLE TO PRODUCE A TESTABLE SAMPLE DURING TRIMMING.

AUTHORIZED SIGNATURE NCDOT CERT NO. 130–0212

Alx M Atmilly

Prepared in the Office of:

F&ME CONSULTANTS, INC.

COLUMBIA, SOUTH CAROLINA

NCDOT LAB CERT. NO. 130–0212

PROJECT REFERENCE NO.	SHEET NO.
BR-0155	17
SITE PHO	TOS



PHOTO #1: VIEW OF BRIDGE 163 PROFILE FACING NORTH.



PHOTO #2: VIEW ALONG BRIDGE 163, FACING WEST, DOWNSTATION.