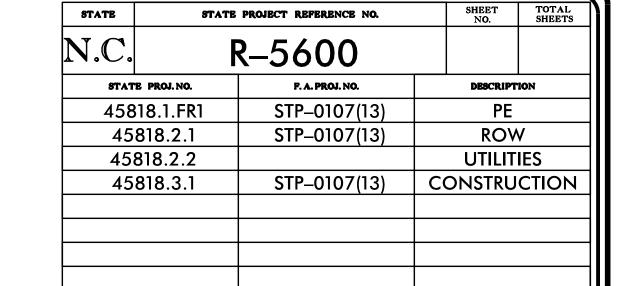
BEGIN TIP R-5600

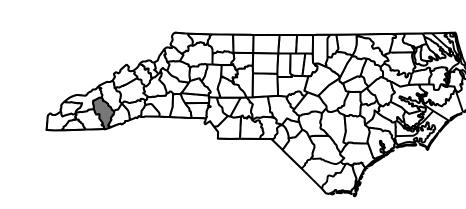
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

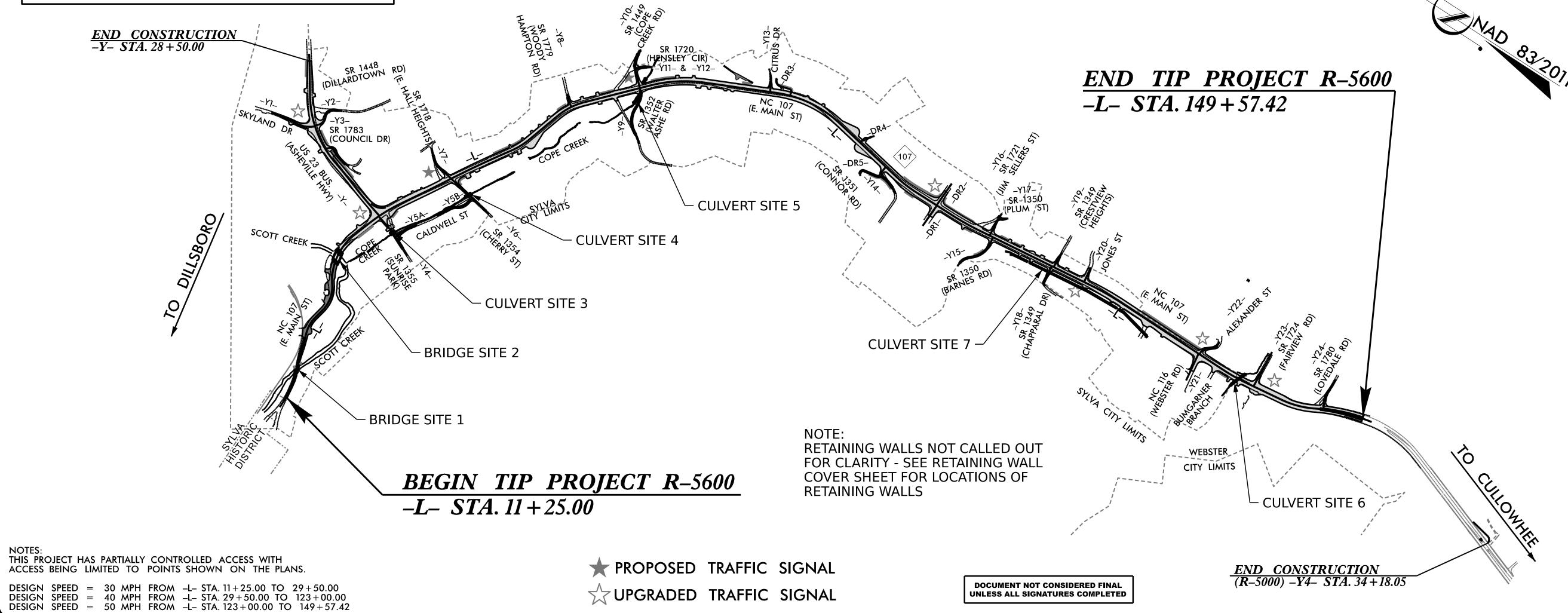
JACKSON COUNTY

LOCATION: SYLVA - N.C. 107 FROM WEST OF U.S. 23 BUSINESS (ASHEVILLE HIGHWAY) TO SOUTH OF N.C. 116 (WEBSTER ROAD) AND U.S. 23 BUS. FROM SKYLAND DRIVE TO WEST OF MUNICIPAL DRIVE

TYPE OF WORK: GRADING, PAVING, DRAINAGE, RETAINING WALLS, STRUCTURES AND SIGNALS







DESIGN DATA

END TIP

R–5600

VICINITY MAP

ADT 2025 = 37,340ADT 2045 = 42,420= 9 %

= 55 % V = 30 MPHTO 50 MPH

TTST = 4% DUAL = 3%FUNC CLASS = ARTERIALREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-5600 = 2.603 MILES LENGTH STRUCTURES TIP PROJECT R-5600 = 0.017 MILES

TOTAL LENGTH TIP PROJECT R-5600 = 2.620 MILES

Prepared in the Office of:

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

2024 STANDARD SPECIFICATIONS

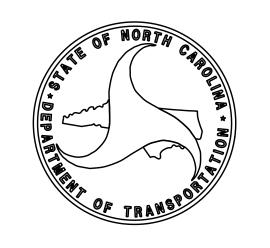
RIGHT OF WAY DATE: JULY 16, 2021

LETTING DATE: **NOVEMBER 18, 2025** PHILLIP E. ROGERS, PE PROJECT ENGINEER

ALEXANDER D. SNIDER, PE PROJECT DESIGN ENGINEER

JEANETTE L. WHITE, PE NCDOT CONTACT





Docusign Envelope ID: 191C9EC7-2F50-4117-BC6D-C63D46077E79

SITE NO.	STATION	STRUCTURE DESCRIPTION	SHEET NOS.
1	-	BRIDGE 490050 PRESERVATION ON NC107 (-L-) OVER SCOTT CREEK	S01-1 THRU S01-18
2	27+24.00 -L-	REPLACE BRIDGE 490077 ON NC107 (-L-) OVER SCOTT CREEK	S02-1 THRU S02-41
3	12+33.00 -Y4-	TRIPLE BARREL 8 FT. X 5 FT. RCBC SR1355 (-Y4-) OVER COPE CREEK	S03-1 THRU S03-8
4	12+94.00 -Y6-	TRIPLE BARREL 7 FT. X 8 FT. RCBC SR1354 (-Y6-) OVER COPE CREEK	S04-1 THRU S04-8
5	64+35.31 -L-	TRIPLE BARREL 6 FT. X 7 FT. RCBC EXTENSIONS NC107 (-L-) OVER COPE CREEK	S05-1 THRU S05-14
6	135+82.00 -L-	DOUBLE BARREL 7 FT. X 8 FT. RCBC NC107 (-L-) OVER BUMGARNER BRANCH	S06-1 THRU S06-12
7	11+63.55 -Y18-	SINGLE BARREL 10 FT. X 5 FT. RCBC SR1349 (-Y18-) OVER MILL CREEK	S07-1 THRU S07-10

PROJECT NO. R-5600 **JACKSON** _ COUNTY

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH INDEX OF SHEETS BRIDGES AND

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

SEAL 043031	CULVERTS	
A January T. Name	REVISIONS	SHEET

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NO.	BY:	DATE:	NO.	BY:	DATE:	
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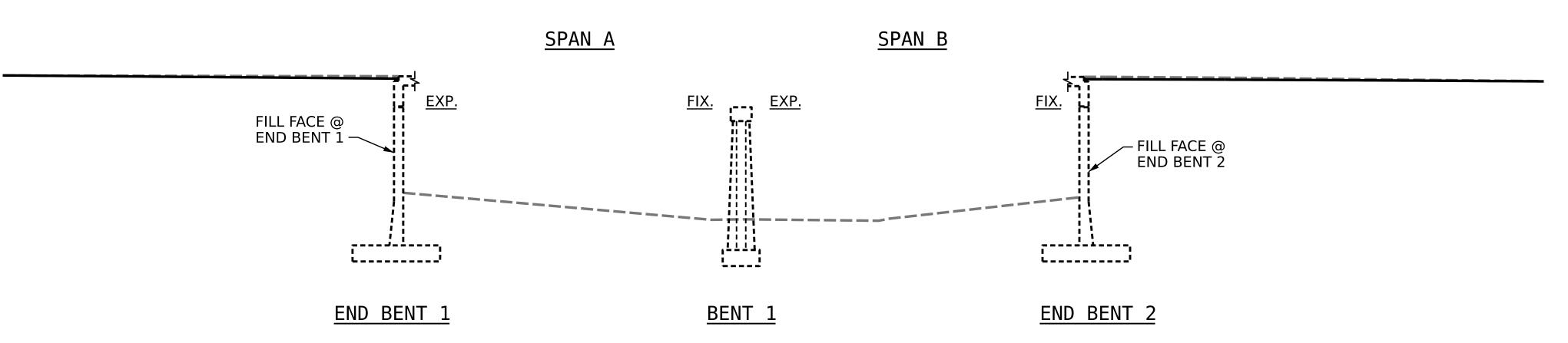
DWG BY: B. PETERSON
CHK BY: K. DICKENS DATE: 05/25
DATE: 06/25 DES BY: _--

STATION:____



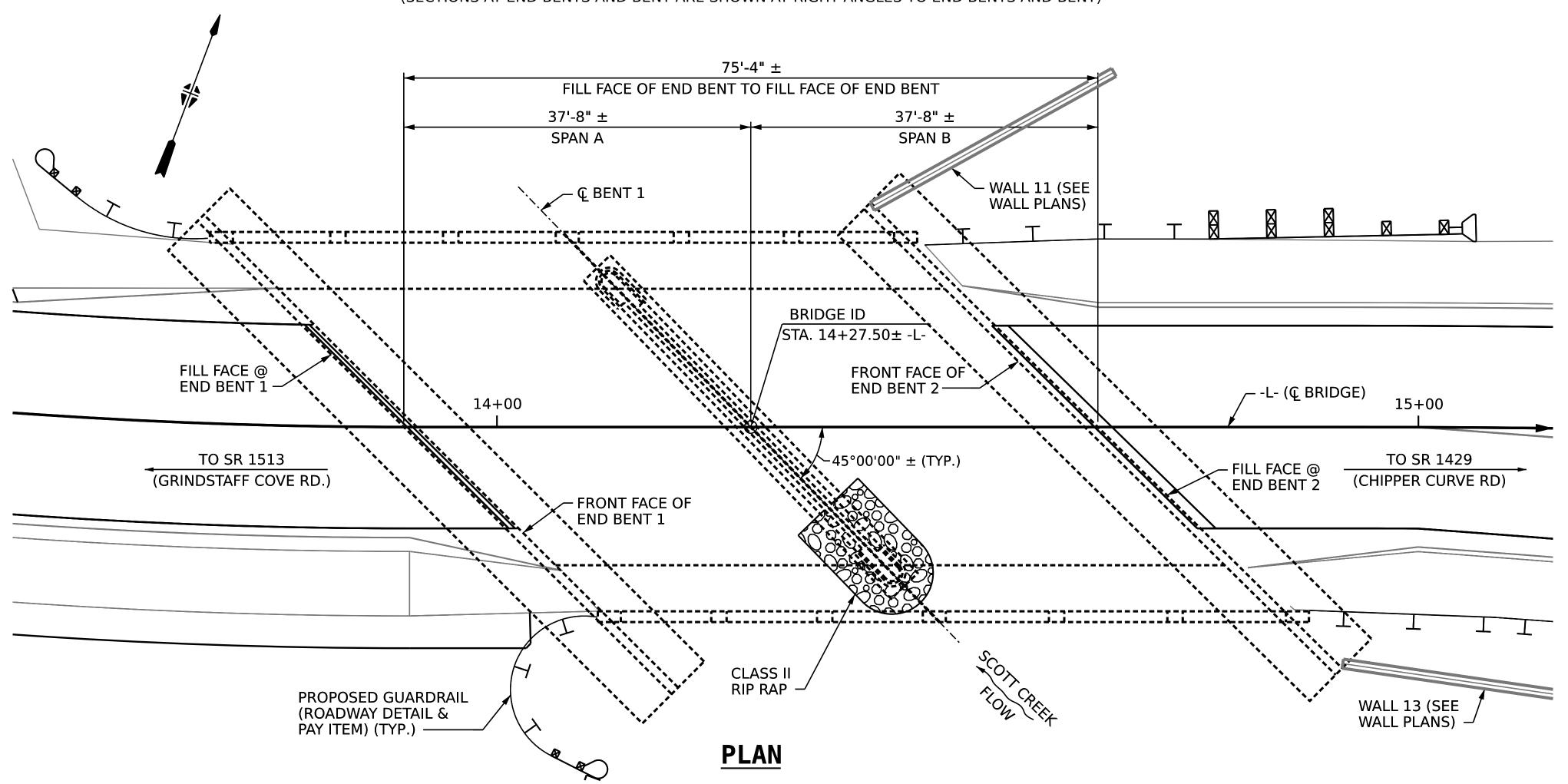
GENERAL DRAWING INFORMATION IS TAKEN FROM THE ORIGINAL PLANS AND THE ROUTINE INSPECTION REPORT DATED 05/28/2024.

BRIDGE ORIENTATION IS REVERSED FROM THE ORIGINAL PLANS.



SECTION ALONG -L-

(SECTIONS AT END BENTS AND BENT ARE SHOWN AT RIGHT ANGLES TO END BENTS AND BENT)



SCOPE OF WORK

- REMOVE ASPHALT OVERLAY AND CONCRETE WEARING SURFACE ON BRIDGE DECK.
- PARTIALLY REMOVE BRIDGE DECK CONCRETE BY SCARIFICATION AND SHOT BLASTING METHODS.
- PLACE REINFORCED CONCRETE WEARING SURFACE.
- GROOVE REINFORCED CONCRETE WEARING SURFACE.
- REMOVE UNSOUND CONCRETE AND PROPERLY PREPARE SHOTCRETE AND CONCRETE REPAIR AREAS.
- HAND CHIP SURFACE OF SIDEWALKS AND PLACE CONCRETE OVERLAY.
- INSTALL POURABLE SILICONE JOINT SEAL AT INTERIOR BENT JOINT.
- PERFORM SHOTCRETE AND CONCRETE REPAIRS IN PREPARED AREAS.
- EPOXY INJECTION OF CONCRETE CRACKS.
- APPLY EPOXY COATING TO THE TOP OF CAPS.
- PLACE RIP RAP AT INTERIOR BENT.

 DES BY:
 J. PATT
 DATE:
 03/25
 DWG BY:
 B. PETERSON
 DATE:
 03/25

 DES CHK:
 M. NEIHEISEL
 DATE:
 05/25
 CHK BY:
 J. PATT
 DATE:
 06/25

SEAL 043031 VCINE P. 1. NELLING 1

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I HEREBY CERTIFY THAT THIS STRUCTURE WAS REHABILITATED ACCORDING TO THESE PLANS OR AS NOTED HEREIN.

RESIDENT ENGINEER

DATE

PROJECT NO. R-5600

JACKSON

BRIDGE NO. 490050

SHEET 1 OF 2

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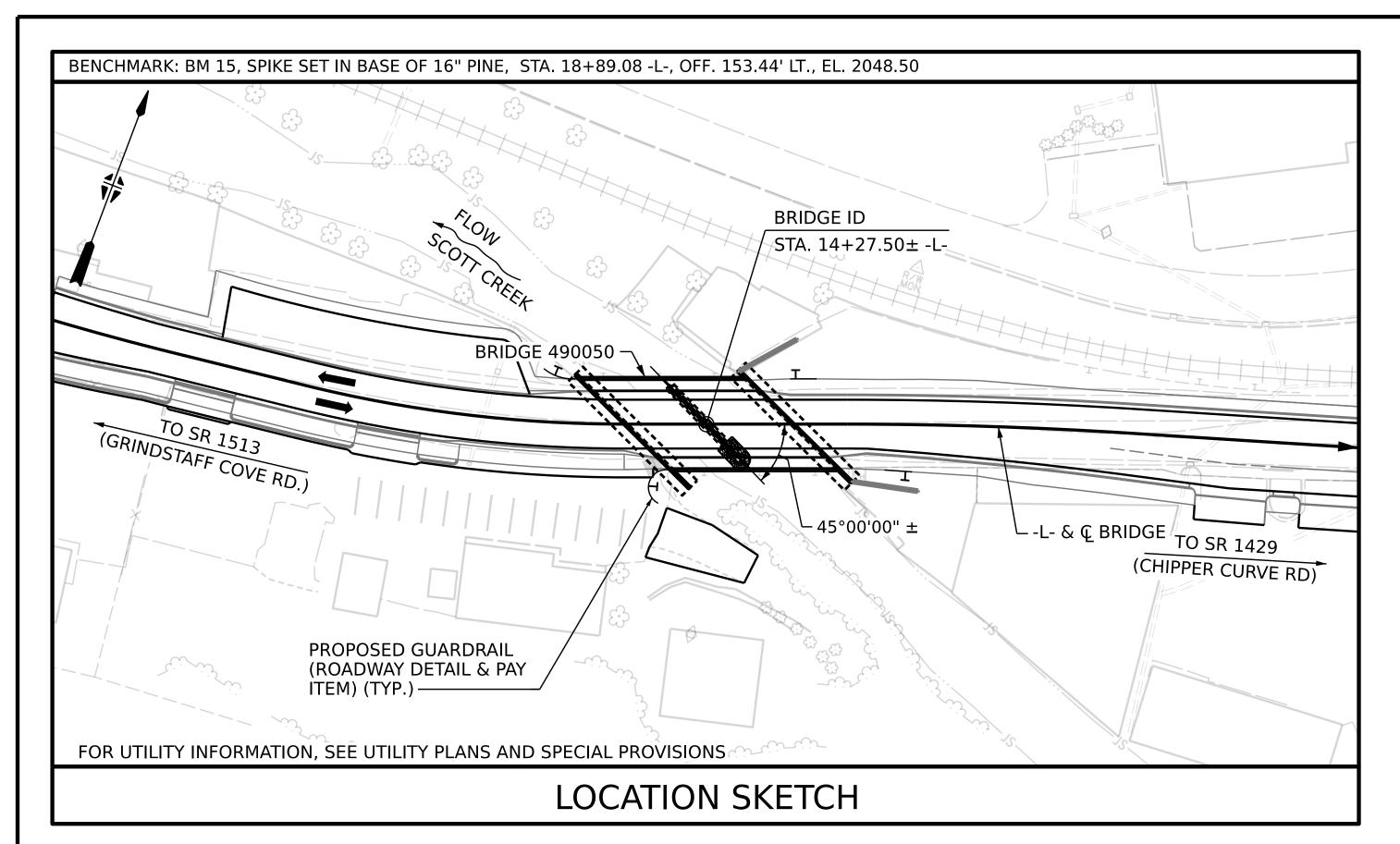
DEPARTMENT OF TRANSPORTATION
RALEIGH

COUNTY

GENERAL DRAWING

FOR BRIDGE ON NC 107 (-L-) (US BUS. 23) OVER SCOTT CREEK BETWEEN SR 1513 (GRINDSTAFF COVE ROAD) & SR 1429 (CHIPPER CURVE ROAD)

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LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE COORDINATES

LATITUDE: 35°22'26.4" LONGITUDE: 83°13'10.2"

DWG BY: B. PETERSON

CHK BY: J. PATT

DATE: 05/25

_ DATE : 02/25

DES BY: J. PATT

DES CHK: M. NEIHEISEL

NOTES

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT DUE TO THE NATURE OF PRESERVATION PROJECTS, THE EXTENT OF WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. REPAIR LOCATIONS AND ESTIMATES OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS.

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THAT SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, EXCEPT WHERE THE CONTRACTOR'S PLAN USE PLATFORMS, NETS, SCREENS OR OTHER PROTECTIVE DEVICES TO CATCH THE MATERIAL. THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PERFORM ALL WORK WITH CARE SO THAT THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE WILL NOT BE DAMAGED. IF THE CONTRACTOR DAMAGES ANY PART OF THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE, THE DAMAGED AREA SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE ENGINEER AT NO ADDITIONAL COST TO THE DEPARTMENT.

ANY DAMAGE TO EXISTING REINFORCING STEEL, DURING CONTRACTOR'S OPERATIONS, SHALL BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST TO THE DEPARTMENT.

FOR TRAFFIC CONTROL AND LIMITS OF PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLANS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ALL PAVEMENT MARKING WILL BE IN ACCORDANCE WITH THE PAVEMENT MARKING PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

EXISTING JOINTS SHALL BE SEALED AND DECK DRAINS SHALL BE CUT AND PLUGGED PRIOR TO BEGINNING SURFACE PREPARATIONS OF THE BRIDGE DECK.

LONGITUDINAL CONSTRUCTION JOINTS OF CONCRETE WEARING SURFACE SHALL BE LOCATED ALONG THE CENTERLINE OR EDGE OF TRAVEL LANES.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THERE ARE STREAM MONITORING DEVICES ATTACHED TO STRUCTURE 490050 THAT SHOULD NOT BE DISTURBED WITHOUT APPROVAL FROM THE ENGINEER.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS.

FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

WEARING SURFACE CONSTRUCTION JOINTS WILL BE SEALED WITH HMWM. SEE SPECIAL PROVISION FOR CONCRETE BRIDGE DECK CRACK SEALING.

FOR SIDEWALK CONCRETE OVERLAY, SEE SPECIAL PROVISIONS

FOR EPOXY COATING. SEE SPECIAL PROVISIONS.

	TOTAL BILL OF MATERIAL										
BRIDGE NO. 490050	CONCRETE WEARING SURFACE	GROOVING BRIDGE FLOORS	RIP RAP CLASS II	CONCRETE REPAIRS	SHOTCRETE REPAIRS	EPOXY RESIN INJECTION	POURABLE SILICONE JOINT SEALANT	SIDEWALK CONCRETE OVERLAY	EPOXY COATING	SCARIFYING BRIDGE DECK	SHOTBLASTING BRIDGE DECK
TOTALS	SQ. FT.	SQ. FT.	TON	CU. FT.	CU. FT.	LIN. FT.	LIN. FT.	SQ. FT.	SQ. FT.	SQ. YDS.	SQ. YDS.
IOIALS	2217.6	1984	20	14.9	23.6	159.5	57.6	753.5	10.9	246.4	246.4

PROJECT NO. R-5600 **JACKSON**

COUNTY

BRIDGE NO._

490050

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

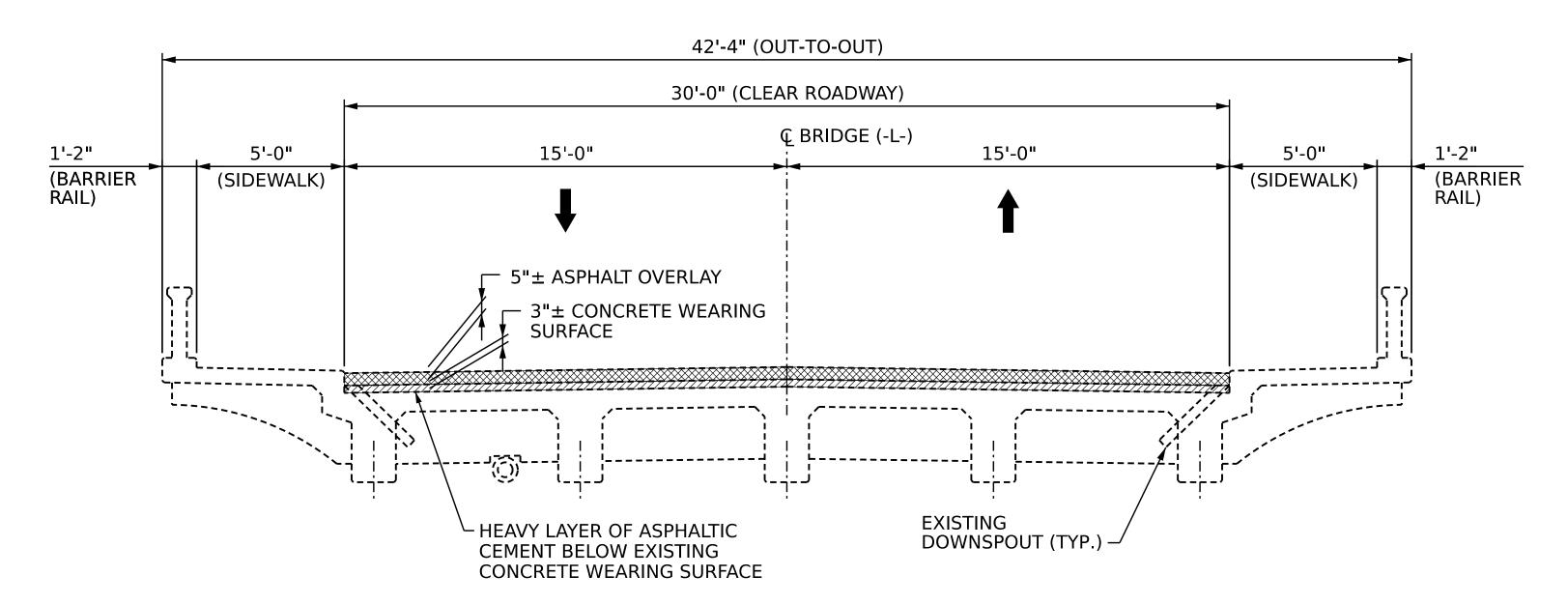
FOR BRIDGE ON NC 107 (-L-) (US BUS. 23) OVER SCOTT CREEK BETWEEN SR 1513 (GRINDSTAFF COVE ROAD) & SR 1429 (CHIPPER CURVE ROAD)

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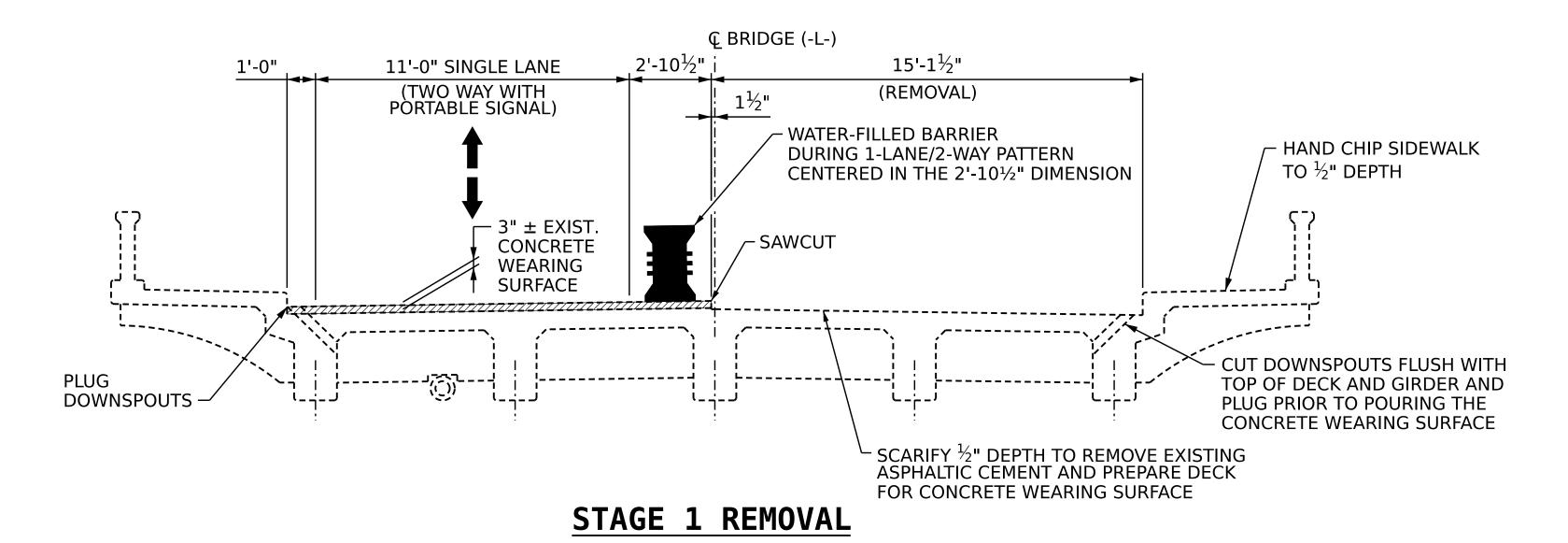


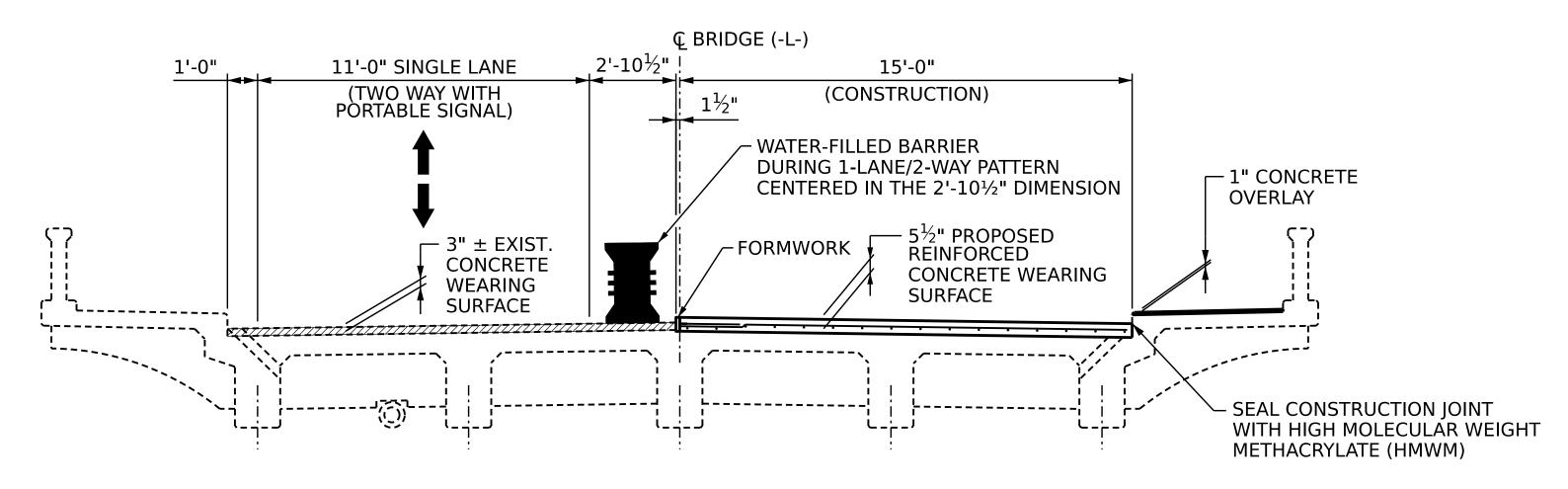
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DOCUMENT NOT CONSIDERED FINAL



EXISTING BRIDGE





STAGE 1 CONSTRUCTION

DES BY: M. NEIHEISEL	DATE :05/25	DWG BY: D. CARTER	DATE : 05/25
DES CHK: J. PATT	DATE: 05/25	CHK BY: J. PATT	DATE: 06/25

NOTES

STAGE 1 REMOVAL

- 1. REMOVE ENTIRE 5" ASPHALT OVERLAY OVERNIGHT USING FLAGGING. MILL THE ASPHALT APPROACHES AND WEDGE BACK IN EACH DIRECTION PER THE ROADWAY DETAILS. RETURN TRAFFIC BACK TO EXISTING PATTERN THE NEXT MORNING. PAYMENT FOR APPROACH PAVEMENT WORK TO BE INCLUDED WITH THE ROADWAY WORK.
- 2. PLACE TRAFFIC BARRIER FOR ONE LANE/TWO WAY PATTERN.
- 3. REMOVE THE EXISTING 3" CONCRETE WEARING SURFACE ON THE STAGE 1 SIDE.
- 4. CUT THE EXISTING DECK DRAINS FLUSH WITH THE TOP OF DECK AND PLUG ON BOTH SIDES OF THE BRIDGE.
- 5. SCARIFY DECK $\frac{1}{2}$ " WITH MILLING AND PREPARE THE SURFACE WITH SHOT BLASTING.
- 6. HAND CHIP SIDEWALK TO A DEPTH OF $\frac{1}{2}$ " AND REMOVE AREAS OF DELAMINATION.

STAGE 1 CONSTRUCTION

- 1. PLACE 1" CONCRETE OVERLAY ON SIDEWALK AND PLACE THE $5\frac{1}{2}$ " REINFORCED CONCRETE WEARING SURFACE ON HALF OF THE BRIDGE.
- 2. INSTALL POURABLE SILICONE JOINT AT THE INTERIOR BENT.
- 3. SEAL THE CONSTRUCTION JOINT AT THE CURB WITH HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM). PLACE HMWM ON THE JOINT AND AT LEAST 6" FROM THE CURB. SEE SPECIAL PROVISION FOR CONCRETE BRIDGE DECK CRACK SEALING. PAYMENT FOR HMWM WILL BE INCIDENTAL TO THE CONCRETE WEARING SURFACE.
- 4. PLACE THE APPROACH PAVEMENT BEFORE AND AFTER THE BRIDGE TO MEET THE NEW CONCRETE WEARING SURFACE ON THE BRIDGE. PAYMENT FOR PAVEMENT WORK TO BE INCLUDED AS PART OF THE ROADWAY WORK.
- 5. THE REINFORCED CONCRETE WEARING SURFACE MUST REACH A STRENGTH OF 2,500 PSI AND THE APPROACH PAVEMENT MUST BE PLACED PRIOR TO SWITCHING TRAFFIC TO THE OTHER SIDE.

PROJECT NO. R-5600

JACKSON COUNTY

BRIDGE NO.____

490050

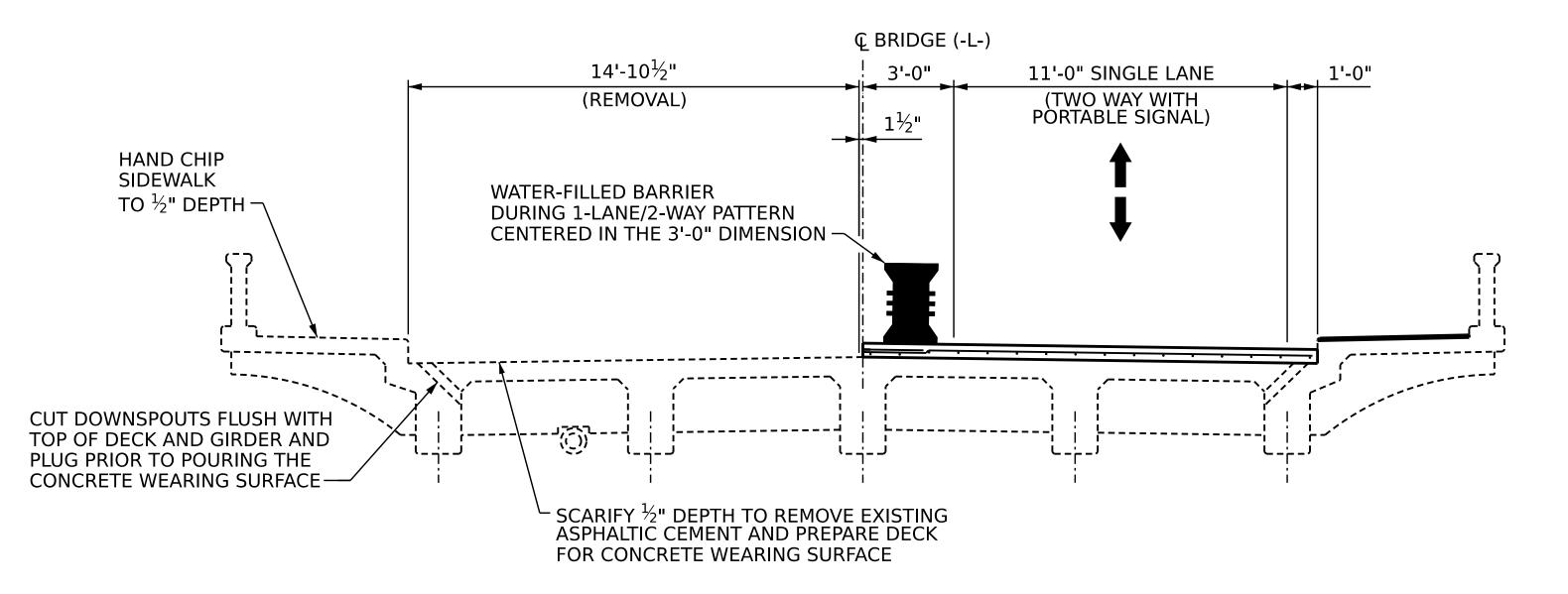
SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH

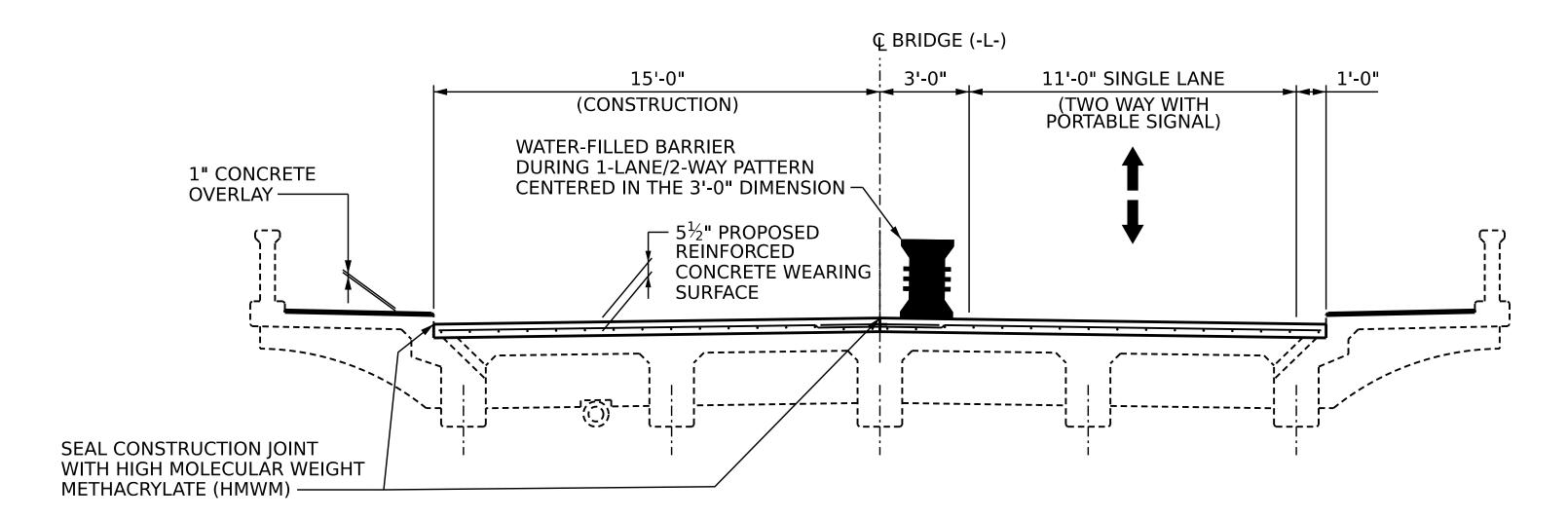
STAGED CONSTRUCTION DETAILS



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STAGE 2 REMOVAL



STAGE 2 CONSTRUCTION

 DES BY:
 M. NEIHEISEL
 DATE:
 05/25
 DWG BY:
 D. CARTER
 DATE:
 05/25

 DES CHK:
 J. PATT
 DATE:
 05/25
 CHK BY:
 J. PATT
 DATE:
 06/25

NOTES

STAGE 2 REMOVAL

- 1. PLACE TRAFFIC BARRIER FOR THE STAGE 2, ONE LANE/TWO WAY PATTERN.
- 2. REMOVE THE REMAINING EXISTING 3" CONCRETE WEARING
- 3. CUT THE EXISTING DECK DRAINS FLUSH WITH THE TOP OF DECK AND GIRDER AND PLUG.
- 4. SCARIFY DECK $\frac{1}{2}$ " WITH MILLING AND PREPARE THE SURFACE WITH BEAD BLASTING.
- 5. HAND CHIP SIDEWALK TO A DEPTH OF $\frac{1}{2}$ " AND REMOVE AREAS OF DELAMINATION.

STAGE 2 CONSTRUCTION

- 1. PLACE 1" CONCRETE OVERLAY ON SIDEWALK AND PLACE THE $5\frac{1}{2}$ " REINFORCED CONCRETE WEARING SURFACE ON THE REMAINING HALF OF THE BRIDGE.
- 2. INSTALL POURABLE SILICONE JOINT AT THE INTERIOR BENT.
- 3. SEAL THE CONSTRUCTION JOINTS AT THE CURB AND CROWN WITH HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM). PLACE HMWM ON THE JOINT AND AT LEAST 6" FROM THE CURB AND AT LEAST 6" ON EACH SIDE OF THE CROWN. SEE SPECIAL PROVISION FOR CONCRETE BRIDGE DECK CRACK SEALING. PAYMENT FOR HMWM WILL BE INCIDENTAL TO THE CONCRETE WEARING SURFACE.
- 4. GROOVE THE CONCRETE WEARING SURFACE PER THE STANDARD SPECIFICATIONS.
- 5. PLACE THE APPROACH PAVEMENT BEFORE AND AFTER THE BRIDGE TO MEET THE NEW CONCRETE WEARING SURFACE ON THE BRIDGE. PAYMENT FOR PAVEMENT WORK TO BE INCLUDED AS PART OF THE ROADWAY WORK.
- 6. THE REINFORCED CONCRETE WEARING SURFACE MUST REACH A STRENGTH OF 2,500 PSI AND THE APPROACH PAVEMENT MUST BE PLACED PRIOR TO REOPENING TO TRAFFIC.

PROJECT NO. R-5600

JACKSON

COUNTY

BRIDGE NO. 490050

SHEET 2 OF 2

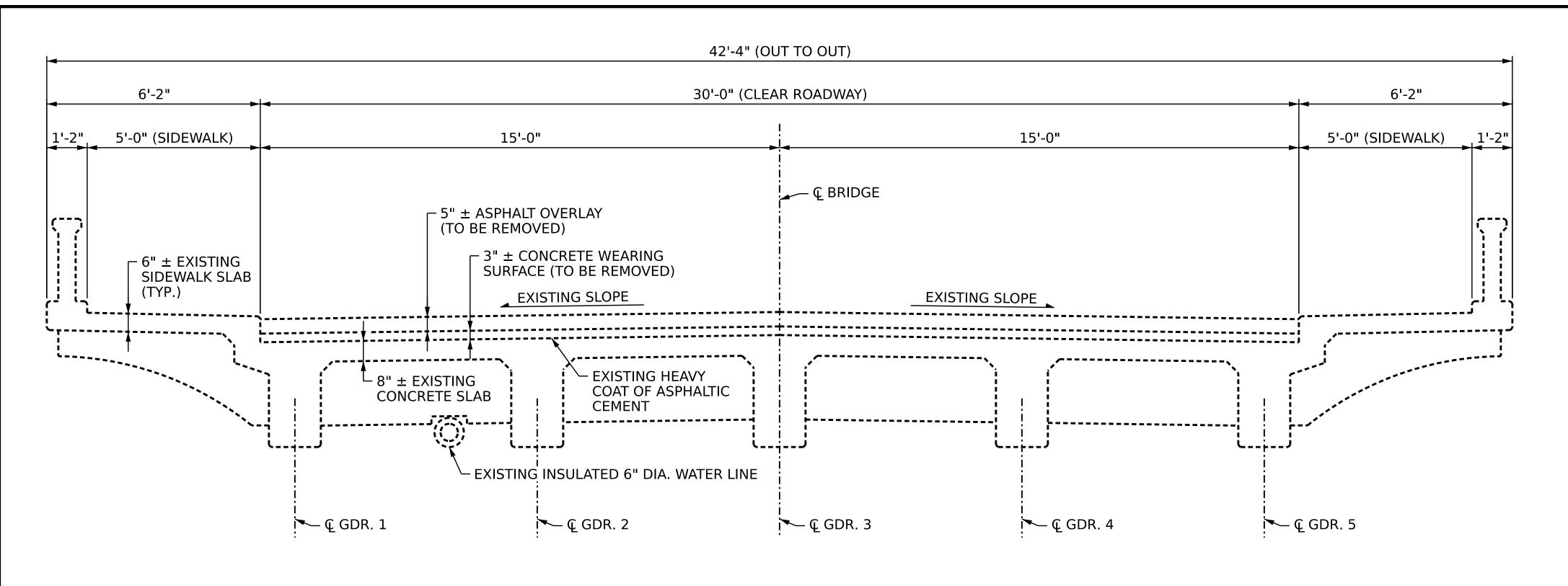
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALETCH

STAGED CONSTRUCTION DETAILS

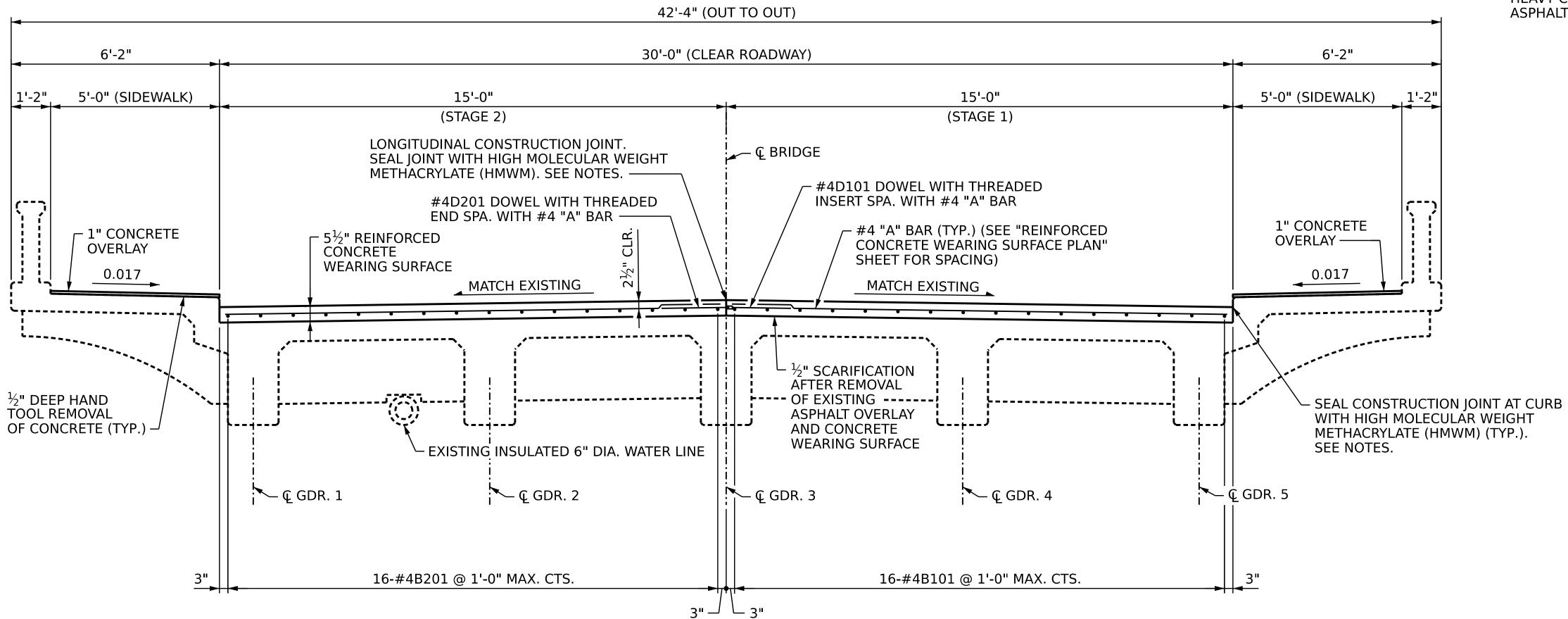


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TYPICAL SECTION - EXISTING



TYPICAL SECTION - PROPOSED

DWG BY: D. CARTER DES BY: J. PATT _ DATE : 05/25 DATE: 05/25
DATE: 06/25 DES CHK: M. NEIHEISEL _ DATE : 06/25 CHK BY: M. NEIHEISEL

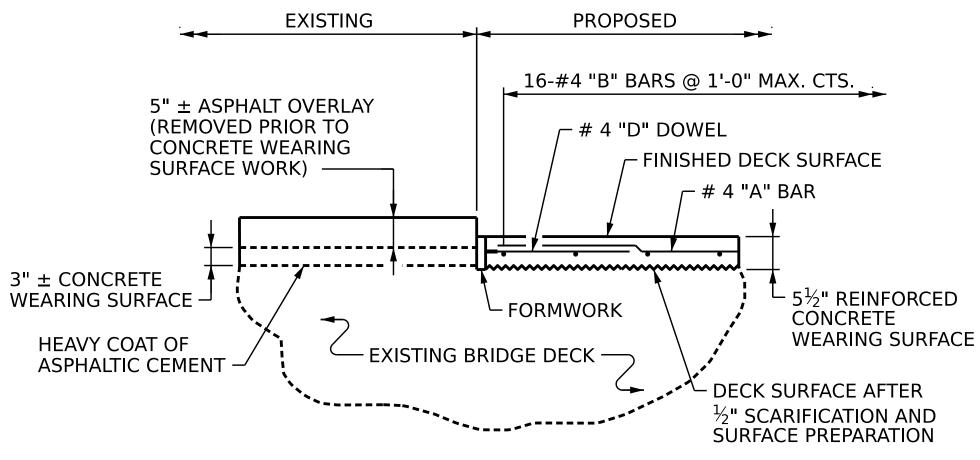
NOTES

SEE TRANSPORTATION MANAGEMENT PLANS FOR LANE WIDTH, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF CONCRETE WEARING SURFACE PREPARATION AND CONCRETE PLACEMENT.

FOR SIDEWALK OVERLAY DETAILS, SEE "PLAN OF SPANS SPAN B"

FOR REINFORCED CONCRETE WEARING SURFACE DETAILS. SEE "REINFORCED CONCRETE WEARING SURFACE PLAN" SHEET.

SEAL LONGITUDINAL CONSTRUCTION JOINTS AT CROWN AND CURBS WITH HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM). THE SEALANT SHALL COAT THE JOINT AND AT LEAST 6" FROM THE CURB OR CROWN. PAYMENT FOR SEALING WILL BE INCIDENTAL TO THE CONCRETE WEARING SURFACE. SEE SPECIAL PROVISION FOR CONCRETE BRIDGE DECK CRACK SEALING.



DETAIL OF REINFORCED **CONCRETE WEARING SURFACE**

PROJECT NO. R-5600 COUNTY

JACKSON

490050 BRIDGE NO._

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > TYPICAL SECTION

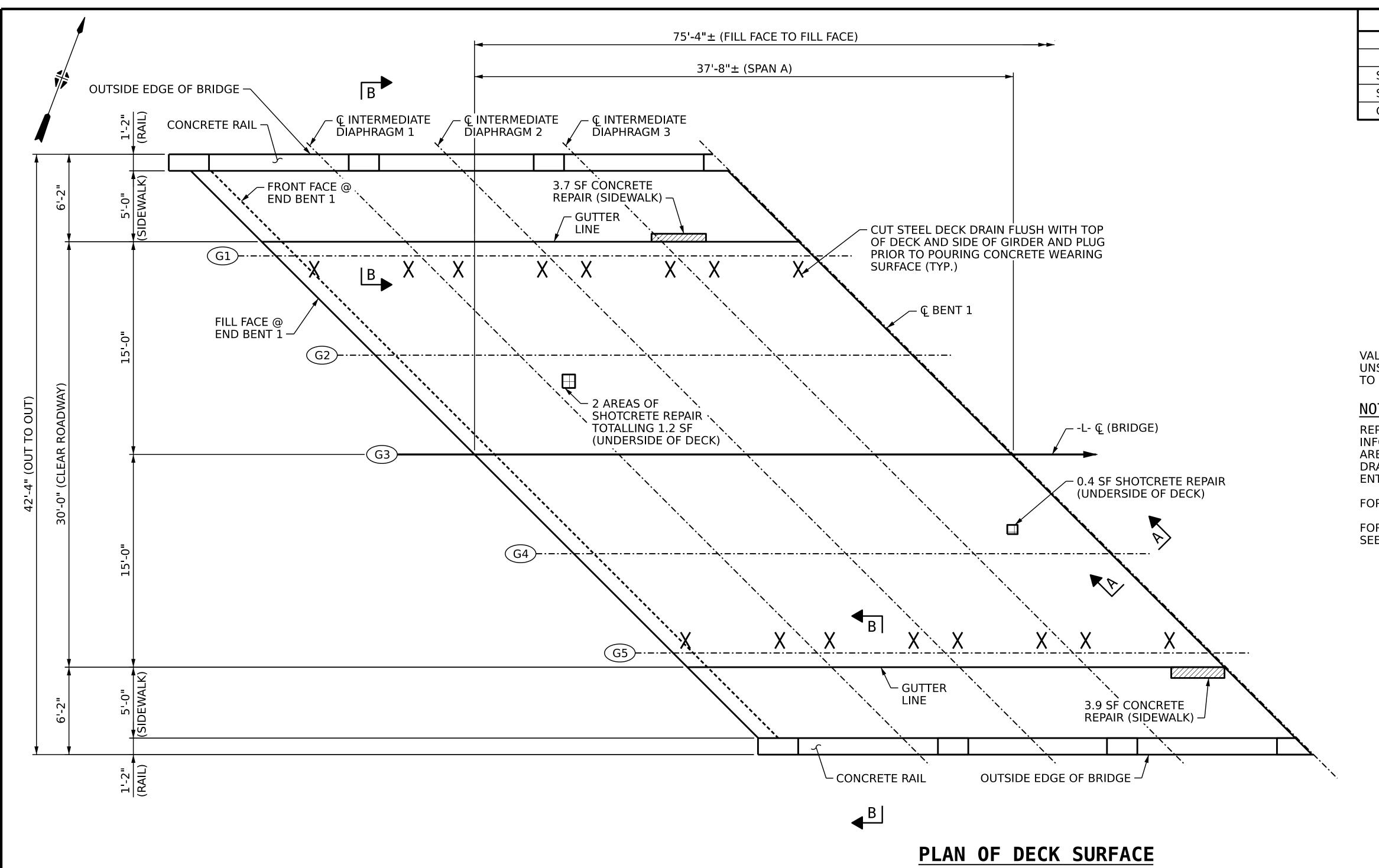


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SHEET NO. REVISIONS S01-05 NO. BY: DATE: DATE:



AS-BUILT REPAIR Q	UANTITY TA	ABLE
TOP OF DECK	REPAIRS	
	ESTIMATE	AS-BUILT
SCARIFYING BRIDGE DECK	123.2 SQ. YDS.	
SHOTBLASTING BRIDGE DECK	123.2 SQ. YDS.	
GROOVING BRIDGE FLOOR	991.6 SQ. FT.	

UNDERSIDE OF DECK REPAIRS								
	ESTI	MATE	AS-BUILT					
SHOTCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.				
UNDERSIDE OF DECK	1.6	0.6						

SIDEWALK REPAIRS									
	ESTI	MATE	AS-B	UILT					
CONCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.					
SIDEWALK	7.6	2.6							

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM 2" CLEARANCE TO SAWCUT. SEE "SUPERSTRUCTURE REPAIR DETAILS" SHEET.

NOTES

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE AS-BUILT REPAIR QUANTITY TABLE.

FOR SECTION A-A, SEE "JOINT DETAILS" SHEET.

FOR SECTION B-B AND SIDEWALK CONCRETE OVERLAY DETAILS AND NOTES, SEE SHEET 2 OF 2.

LEGEND

DECK DRAIN TO BE CUT AND PLUGGED

CONCRETE REPAIR AREA



SHOTCRETE REPAIR AREA

PROJECT NO. R-5600

JACKSON

490050

BRIDGE NO. ___

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

PLAN OF SPANS

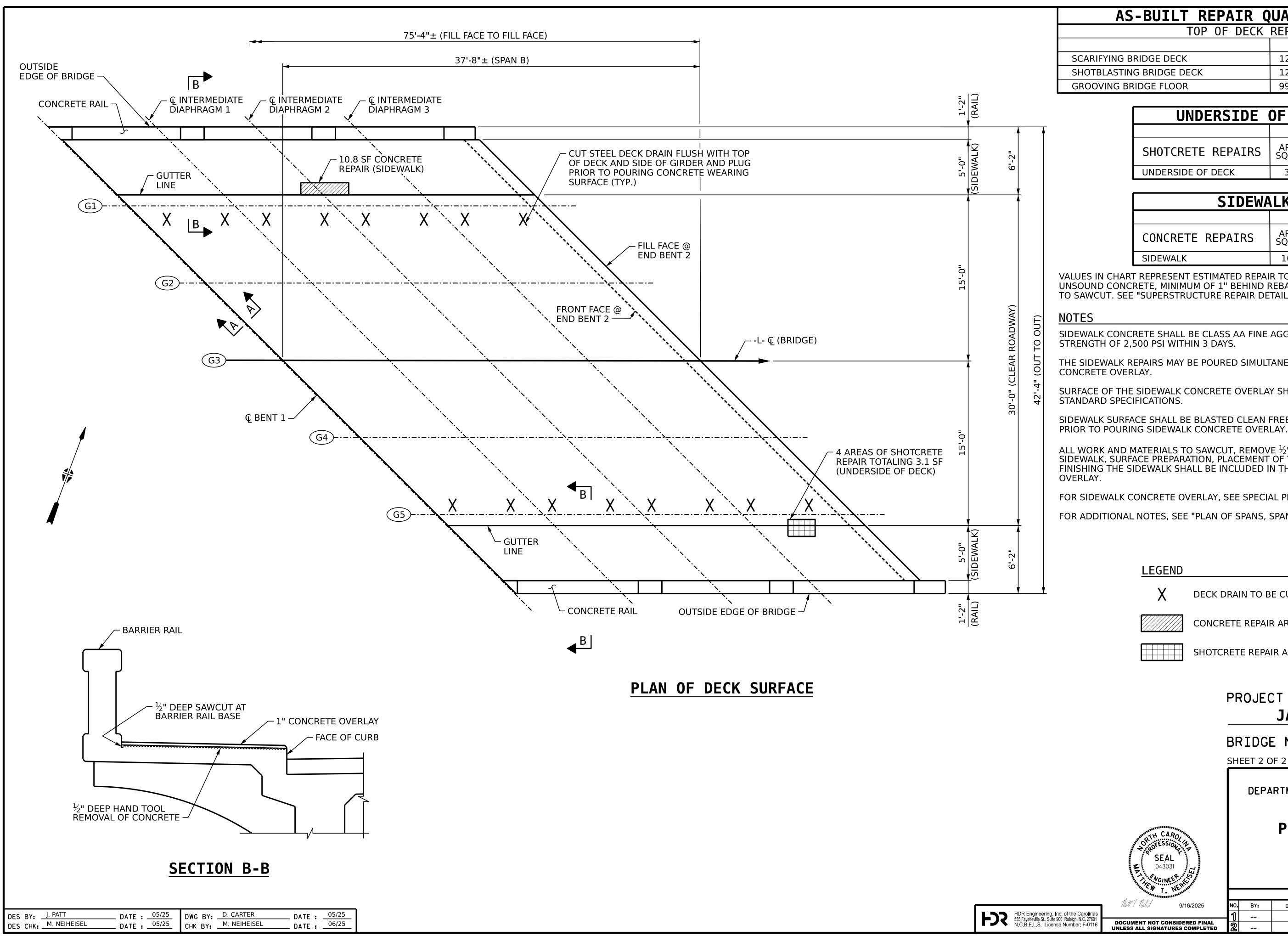


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REVISIONS SHEET NO. S01-06 NO. BY: DATE: 9/16/2025 DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DES BY: J. PATT __ DATE : 05/25 __ DATE : 05/25 DWG BY: D. CARTER
CHK BY: M. NEIHEISEL __ DATE : 05/25 __ DATE : 06/25 DES CHK: M. NEIHEISEL



AS-BUILT REPAIR QUANTITY TABLE TOP OF DECK REPAIRS **AS-BUILT ESTIMATE** 123.2 SQ. YDS. SCARIFYING BRIDGE DECK 123.2 SQ. YDS. SHOTBLASTING BRIDGE DECK 991.6 SQ. FT. GROOVING BRIDGE FLOOR

UNDERSIDE OF DECK REPAIRS									
	ESTI	MATE	AS-BUILT						
SHOTCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.					
UNDERSIDE OF DECK	3.1	1.1							

SIDEWALK REPAIRS								
	ESTI	MATE	AS-BUILT					
CONCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.				
SIDEWALK	10.8	4.6						

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM 2" CLEARANCE TO SAWCUT. SEE "SUPERSTRUCTURE REPAIR DETAILS" SHEET.

SIDEWALK CONCRETE SHALL BE CLASS AA FINE AGGREGATE MIX THAT WILL REACH A STRENGTH OF 2,500 PSI WITHIN 3 DAYS.

THE SIDEWALK REPAIRS MAY BE POURED SIMULTANEOUSLY WITH THE SIDEWALK CONCRETE OVERLAY.

SURFACE OF THE SIDEWALK CONCRETE OVERLAY SHALL BE FINISHED PER THE STANDARD SPECIFICATIONS.

SIDEWALK SURFACE SHALL BE BLASTED CLEAN FREE OF OIL, GREASE, AND LAITANCE

ALL WORK AND MATERIALS TO SAWCUT, REMOVE $\frac{1}{2}$ " OF THE EXISTING CONCRETE SIDEWALK, SURFACE PREPARATION, PLACEMENT OF THE CONCRETE OVERLAY, AND FINISHING THE SIDEWALK SHALL BE INCLUDED IN THE PAY ITEM SIDEWALK CONCRETE

FOR SIDEWALK CONCRETE OVERLAY, SEE SPECIAL PROVISIONS.

FOR ADDITIONAL NOTES, SEE "PLAN OF SPANS, SPAN A" SHEET.

LEGEND DECK DRAIN TO BE CUT AND PLUGGED CONCRETE REPAIR AREA

SHOTCRETE REPAIR AREA

PROJECT NO. R-5600 **JACKSON** COUNTY

490050 BRIDGE NO.___

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> PLAN OF SPANS SPAN B



MANUEL -	NEE: HELLER							
T. Newson			REVISIONS					
Matt Milul	9/16/2025	NO.	BY:	DATE:	NO.	BY:	DATE:	S01-
		-11			3			TOTAL SHEET
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DWG BY: D. CARTER

CHK BY: M. NEIHEISEL

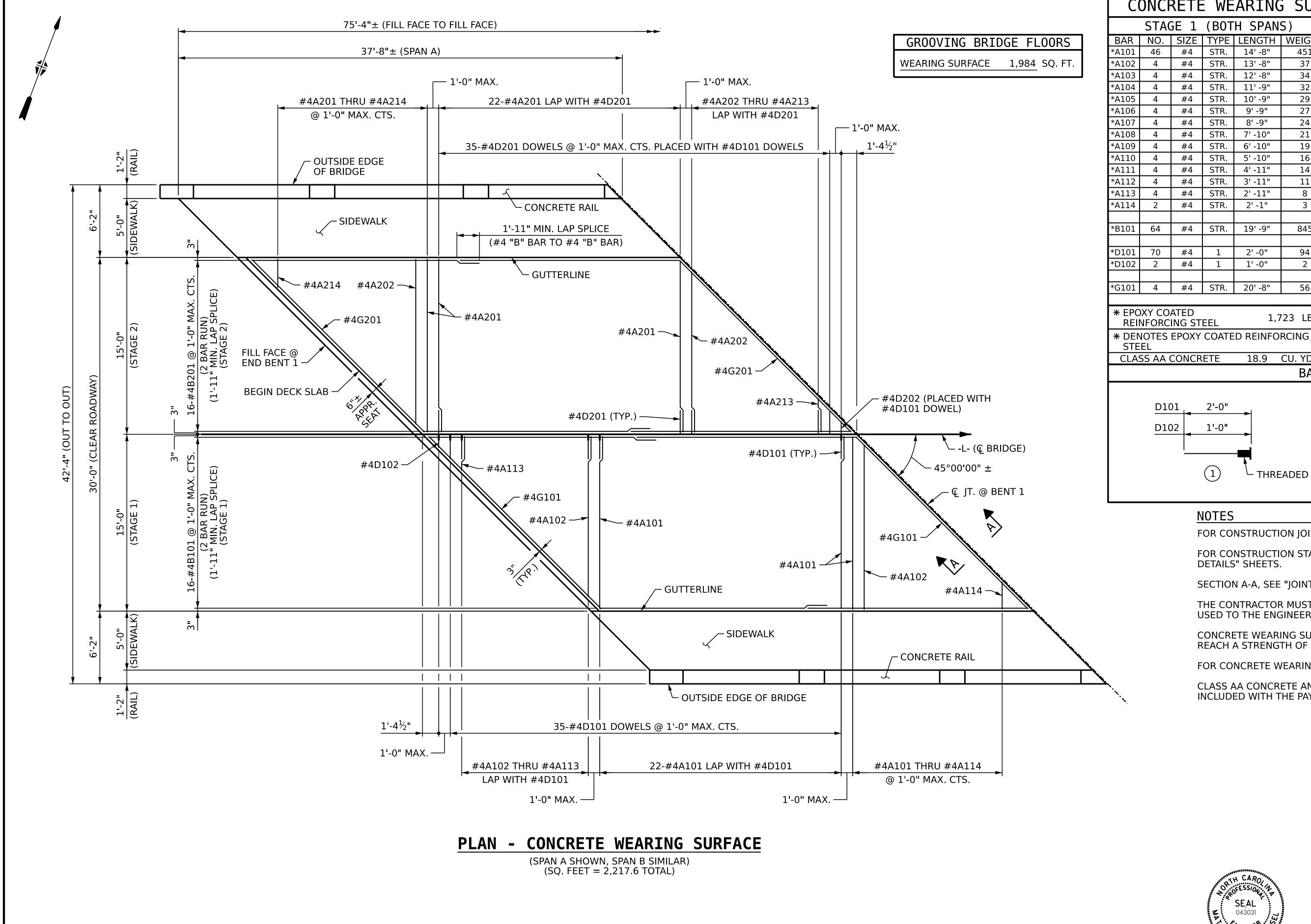
_ DATE : 06/25

_ DATE : __05/25

_ DATE : 05/25

DES BY: J. PATT

DES CHK: M. NEIHEISEL



CONCRETE WEARING SURFACE BILL OF MATERIAL STAGE 1 (BOTH SPANS) STAGE 2 (BOTH SPANS) NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. SIZE TYPE LENGTH WEIGHT STR. 14' -8" 451 #4 STR. *A201 46 14' -8" STR. 13' -8" *A202 #4 STR. 37 13' -8" STR. 12'-8" #4 STR. 34 12' -8" *A204 STR. 11' -9" 32 #4 STR. 32 11' -9" *A205 STR. 10' -9" 29 #4 STR. 10' -9" 29 *A206 STR. 9' -9" 27 #4 | STR. 27 9' -9" STR. 8'-9" 24 #4 STR. 8' -9" 24 *A208 STR. 7'-10" 21 #4 STR. 7' -10'' 21 *A209 STR. | 6'-10" 19 #4 STR. 6' -10'' 19 *A210 5' -10" 16 #4 STR. 16 5' -10'' *A211 STR. 4' -11" 14 #4 STR. 4' -11" *A212 STR. 3' -11" 11 #4 STR. 3' -11" *A213 4 | #4 | STR. | 2'-11" #4 | STR. | 2'-11" *A214 2 #4 STR. 2' -1" #4 STR. 2' -1" 64 | #4 | STR. | 19'-9" 845 *B201 64 #4 STR. 19' -9'' 845 *D201 70 #4 2 2'-0" 1 2' -0" 94 94 *D202 #4 2 1' -0" 2 1' -0" 4 | #4 | STR. | 20'-8" 56 *G201 #4 STR. 20' -8" 4 * EPOXY COATED 1,723 LBS. 1,723 LBS REINFORCING STEEL

D202 1'-0" - THREADED END (1) lacksquare Threaded insert

BAR TYPE

NOTES

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

FOR CONSTRUCTION JOINT DETAILS, SEE "TYPICAL SECTION".

FOR CONSTRUCTION STAGING DETAILS, SEE "STAGED CONSTRUCTION DETAILS" SHEETS.

STEEL

CLASS AA CONCRETE

SECTION A-A, SEE "JOINT DETAILS" SHEET.

18.9 CU. YDS.

THE CONTRACTOR MUST SUBMIT THE TYPE OF #4 "D" DOWELS TO BE USED TO THE ENGINEER FOR APPROVAL.

CONCRETE WEARING SURFACE WILL BE CLASS AA CONCRETE THAT WILL REACH A STRENGTH OF 2500 PSI WITHIN 3 DAYS.

FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

CLASS AA CONCRETE AND EPOXY COATED REINFORCING STEEL ARE INCLUDED WITH THE PAY ITEM FOR CONCRETE WEARING SURFACE.

> PROJECT NO. R-5600 **JACKSON** COUNTY 490050 BRIDGE NO. __

> > STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

* DENOTES EPOXY COATED REINFORCING

18.9 CU. YDS.

REINFORCED CONCRETE CONCRETE WEARING **SURFACE PLAN**

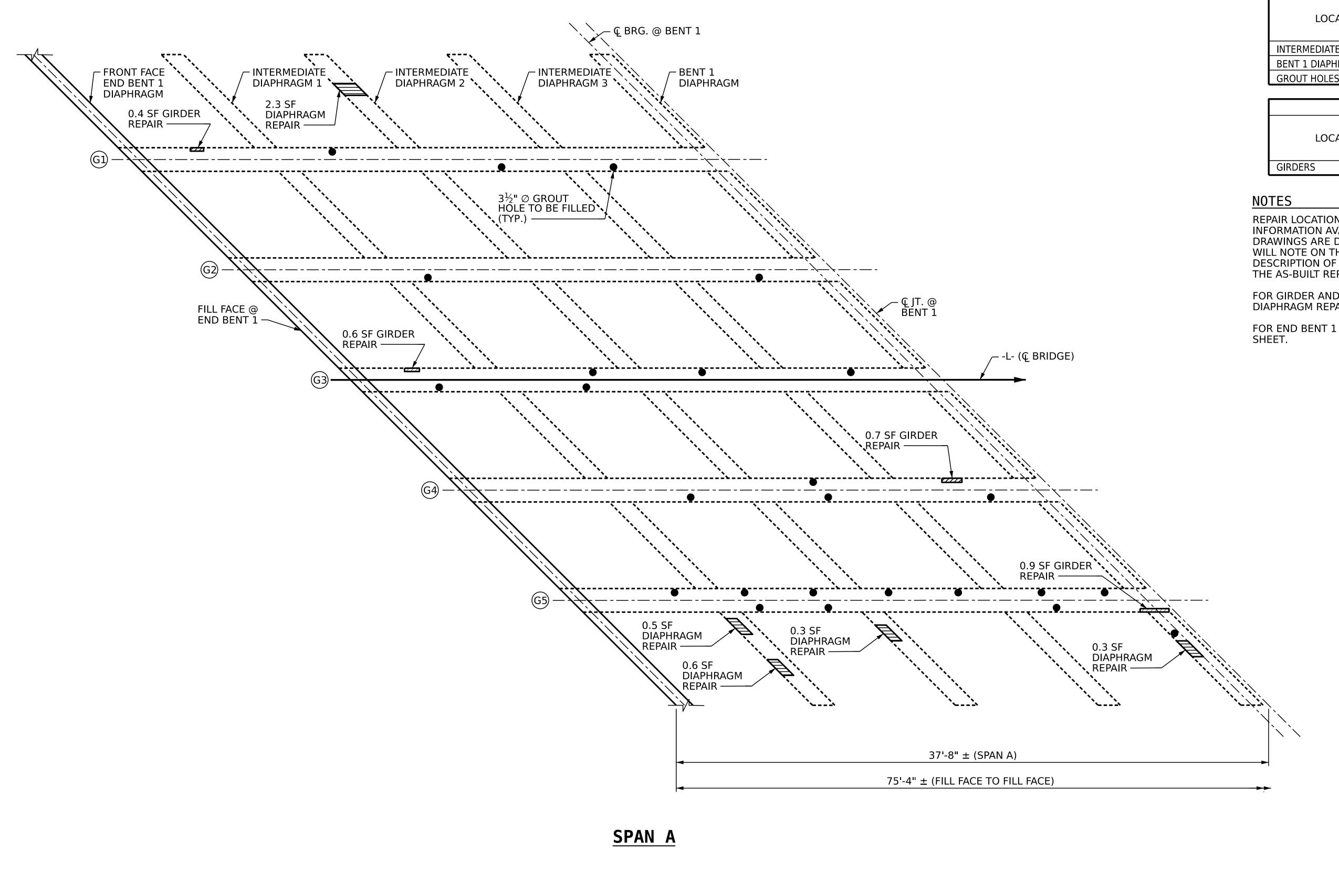
SHEET NO. REVISIONS S01-08 NO. BY: DATE: 9/16/2025 DATE: DOCUMENT NOT CONSIDERED FINAL

DES BY: J. PATT
DES CHK: M. NEIHEISEL

DWG BY: D. CARTER
CHK BY: M. NEIHEISEL

__ DATE : 03/25 __ DATE : 05/25

__ DATE : 05/25 __ DATE : 05/25



	SHOTCRETE REPAIRS								
	ESTI	MATE	AS-BUILT						
	LOCATION	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.				
	INTERMEDIATE DIAPHGRAMS	3.7	1.3						
	BENT 1 DIAPHRAGMS	0.3	0.1						
	GROUT HOLES	1.8	0.3						

CONCRETE REPAIRS								
	ESTII	MATE	AS-BUILT					
LOCATION	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.				
GIRDERS	2.6	1.0						

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE AS-BUILT REPAIR QUANTITY TABLE.

FOR GIRDER AND DIAPHRAGM REPAIRS, SEE DECK, GIRDER, AND DIAPHRAGM REPAIR SHEET DETAILS " SHEET.

FOR END BENT 1 DIAPHRAGM REPAIRS, SEE "SUBSTRUCTURE END BENT 1"

LEGEND

DIAPHRAGM REPAIR AREA

GIRDER REPAIR AREA

GIRDER NUMBER

 $3\frac{1}{2}$ " \varnothing GROUT HOLE TO BE FILLED

PROJECT NO. R-5600

JACKSON

COUNTY

490050 BRIDGE NO.___

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> FRAMING PLAN SPAN A

> > SHEET NO. S01-09

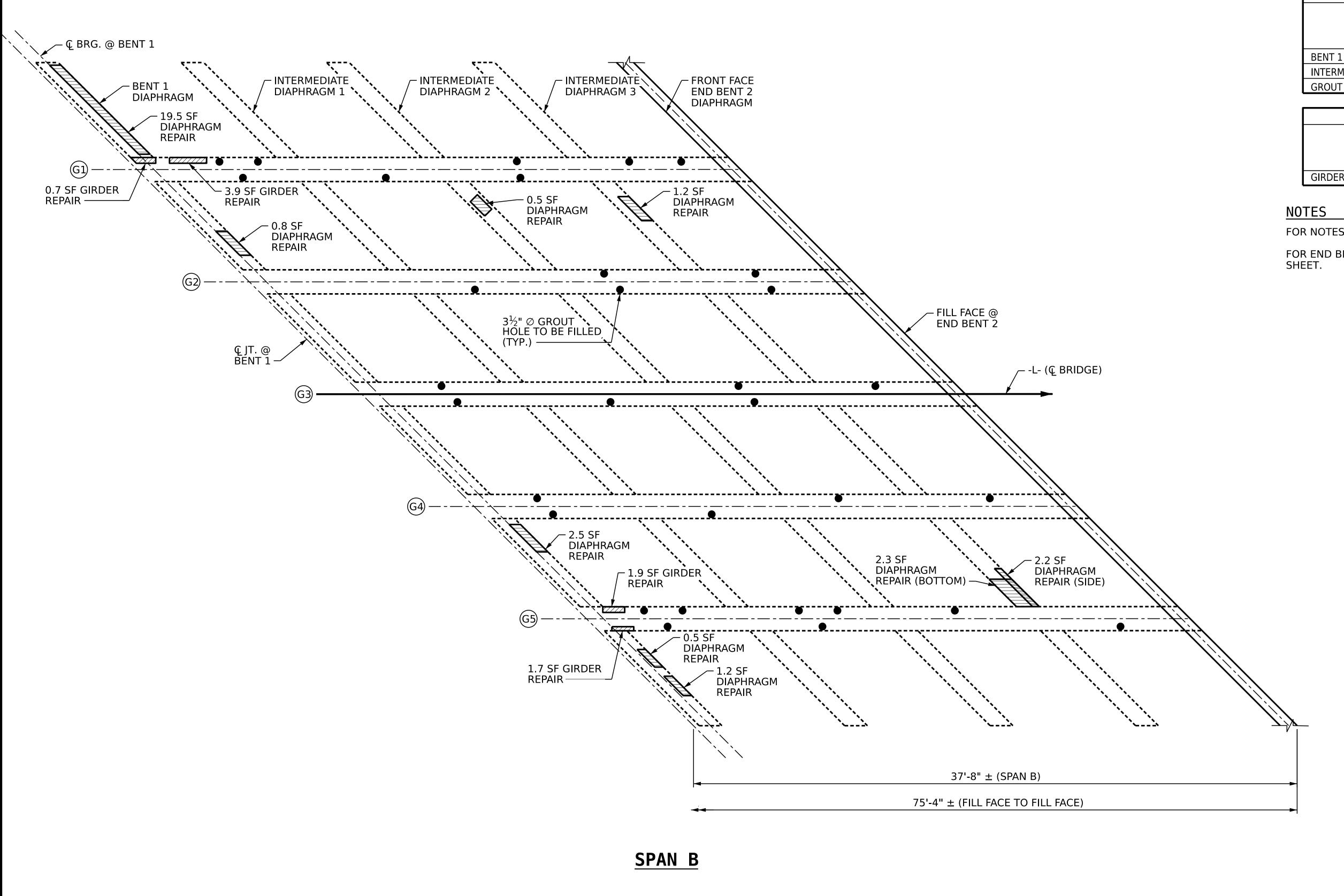
REVISIONS NO. BY: DATE: 9/16/2025 DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DES BY: J. PATT
DES CHK: M. NEIHEISEL

DWG BY: D. CARTER
CHK BY: M. NEIHEISEL

DATE : 05/25
DATE : 05/25

__ DATE : 03/25 __ DATE : 05/25



SHOTCRETE REPAIRS									
	ESTI	MATE	AS-BUILT						
LOCATION	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.					
BENT 1 DIAPHRAGM	24.5	8.3							
INTERMEDIATE DIAPHGRAMS	6.2	2.3							
GROUT HOLES	2.2	0.2							

CONCRETE REPAIRS									
	ESTII	MATE	AS-BUILT						
LOCATION	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUME CU. FT.					
GIRDERS	8.2	2.9							

FOR NOTES, SEE "FRAMING PLAN SPAN A" SHEET.

FOR END BENT 2 DIAPHRAGM REPAIRS, SEE "SUBSTRUCTURE END BENT 2" SHEET.

<u>LEGEND</u>

DIAPHR

DIAPHRAGM REPAIR AREA



GIRDER REPAIR AREA



GIRDER NUMBER

 $3\frac{1}{2}$ " \oslash GROUT HOLE TO BE FILLED

PROJECT NO. R-5600

JACKSON

N COUNTY

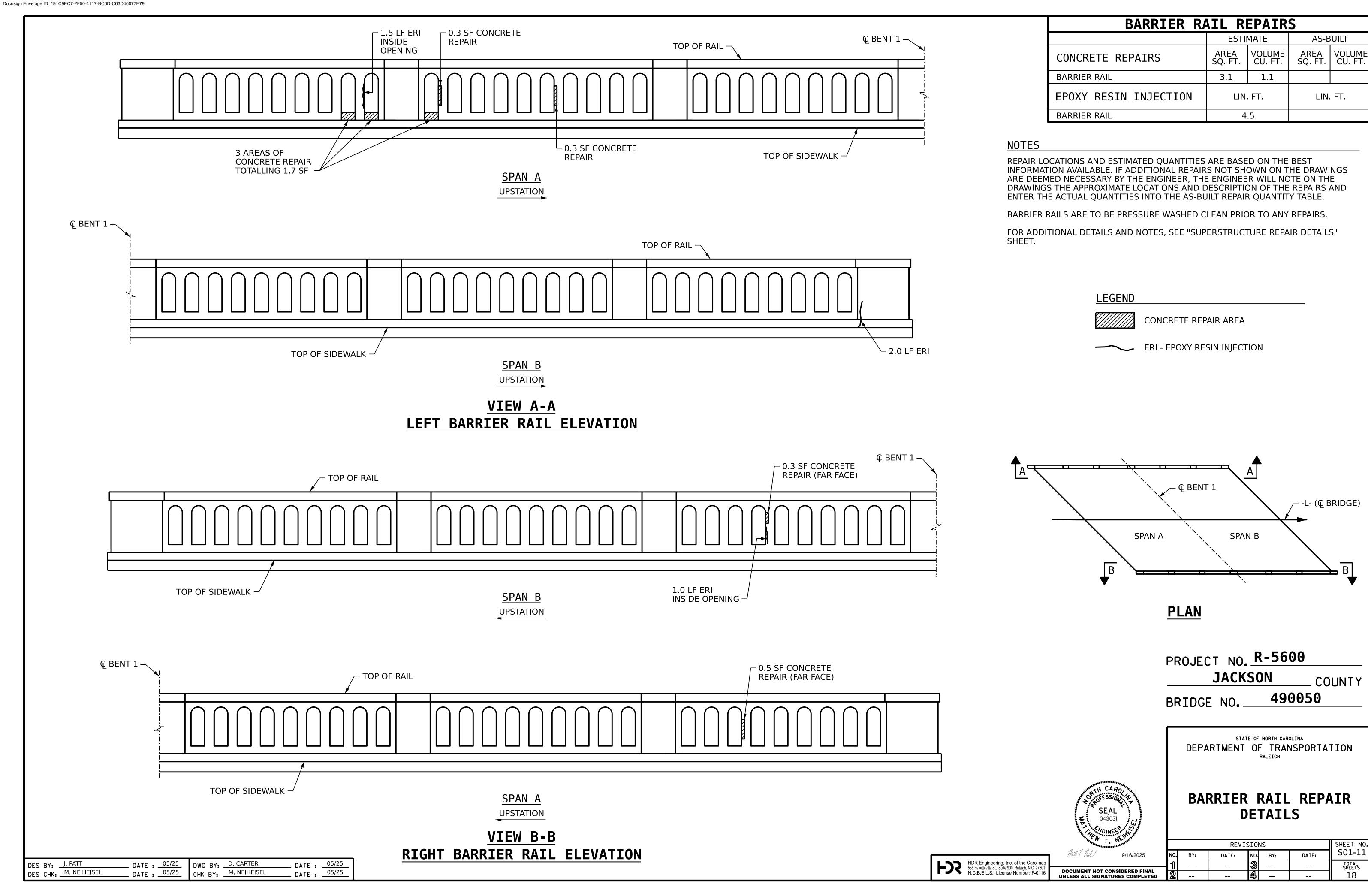
BRIDGE NO. 490050

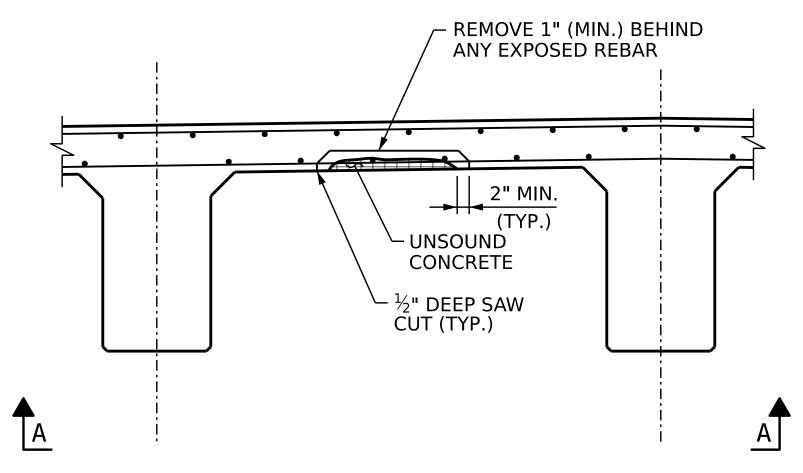
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2 OF 2

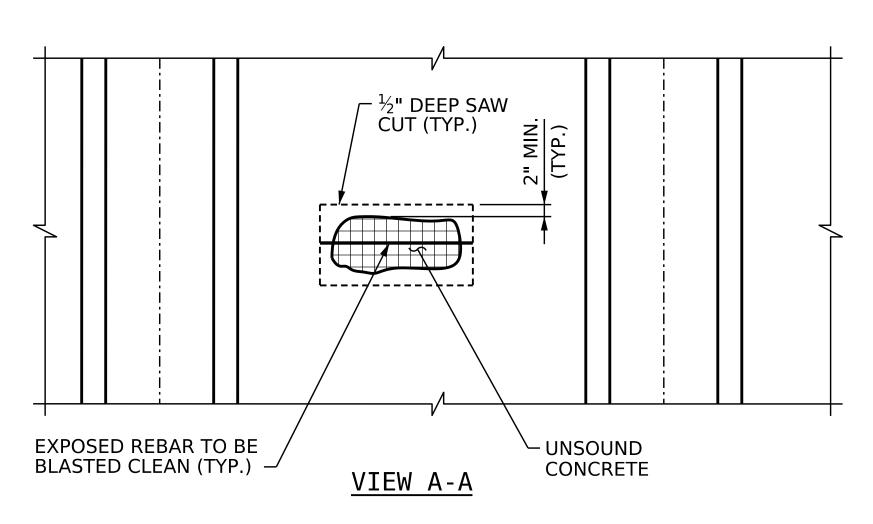
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN SPAN B

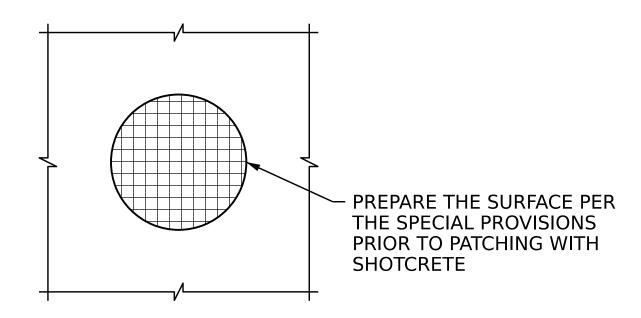




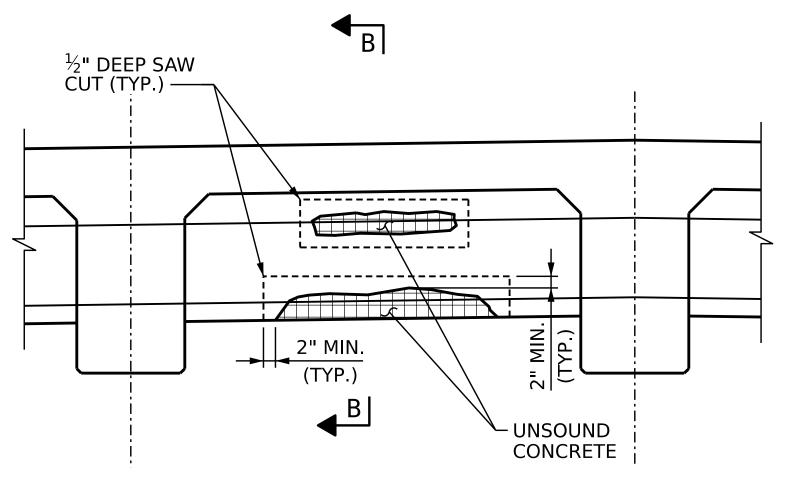
TYPICAL SECTION



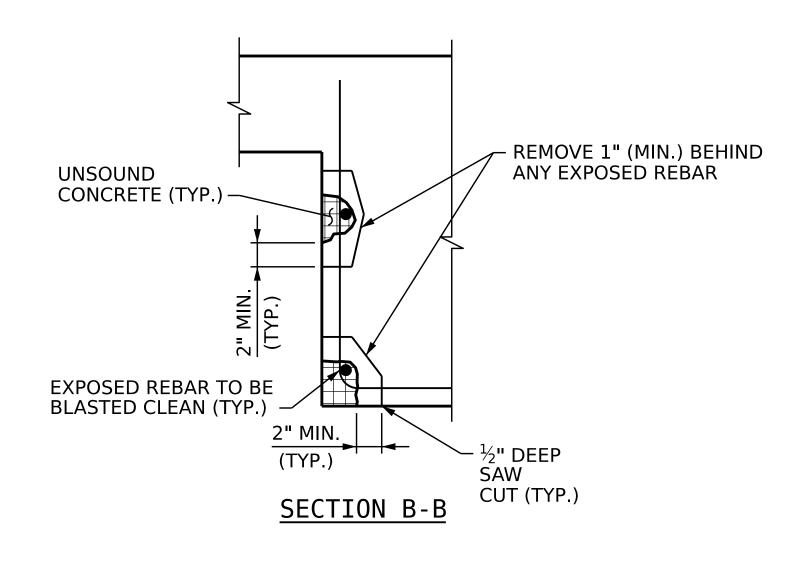
UNDERSIDE DECK REPAIR DETAILS



3½" Ø GROUT HOLE REPAIR

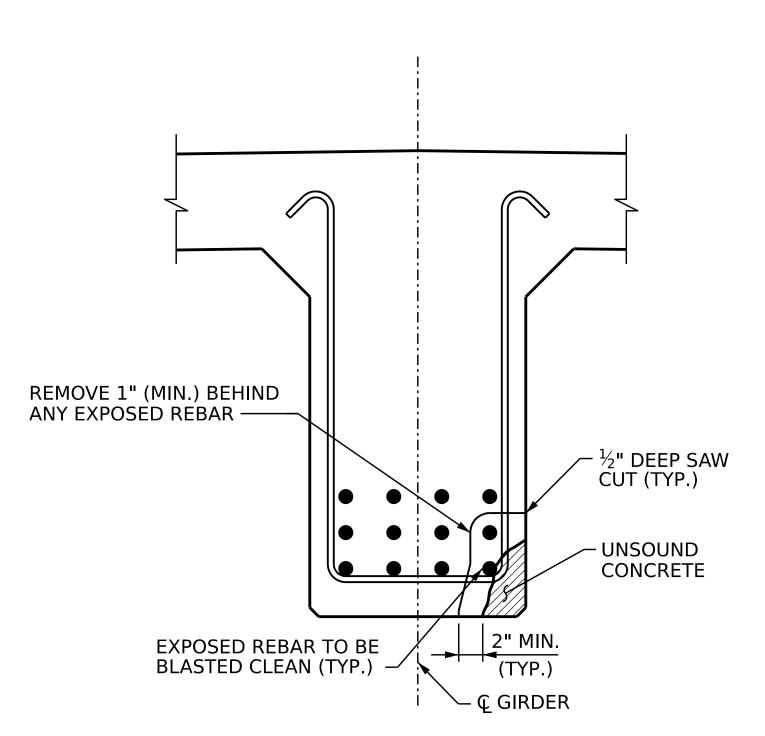


TYPICAL SECTION - DIAPHRAGM



DIAPHRAGM REPAIR DETAILS

INTERIOR DIAPHRAGM SHOWN, END DIAPHRAGMS AND OVERHANGS SIMILAR



SIDEWALK AND BARRIER RAIL REPAIRS SIMILAR

REPAIR SEQUENCE:

- 1. SOUND CONCRETE TO DETERMINE EXTENT OF REPAIR LOCATION.
- 2. REMOVE SURFACE CONCRETE TO VERIFY SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL. SAW CUT AROUND REPAIR AREA TO A NOMINAL DEPTH OF $\frac{1}{2}$ ".
- 3. REMOVE CONCRETE WITHIN SAW CUT AREA TO A MINIMUM $\frac{1}{2}$ " DEPTH.
- 4. USE A WIRE BRUSH TO CLEAN ALL EXPOSED REINFORCING STEEL. FOR BARS WITH MORE THAN 10% SECTION LOSS, SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED.
- 5. REMOVE ALL LOOSE OR WEAKENED MATERIAL THEN CLEAN THE REPAIR AREA OF DIRT, GREASE, OIL, AND FOREIGN MATTER.
- 6. PREPARE SURFACE AND PLACE APPROVED MATERIAL ACCORDING TO MANUFACTURERS SPECIFICATIONS.

NOTES

CONTRACTOR SHALL CUT THE REPAIR AREAS SO THAT THE CORNERS ARE SQUARE AS INDICATED IN THE DETAILS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

SHOTCRETE REPAIRS MAY BE REPLACED WITH CONCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

REPAIRS SHALL PROVIDE A MINIMUM OF $1\frac{1}{2}$ " OF CONCRETE COVER TO ANY EXPOSED REINFORCING STEEL.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.





SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA

PROJECT NO. R-5600

JACKSON

490050 BRIDGE NO.__

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

SUPERSTRUCTURE REPAIR DETAILS



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CONCRETE GIRDER REPAIR DETAIL

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SPLICE LENGTH TABLE

BAR SIZE MIN. SPLICE LENGTH

#4

#5

#6

#7

#8

#9

#10

#11

1'-5"

1'-11"

2'-8"

3'-8"

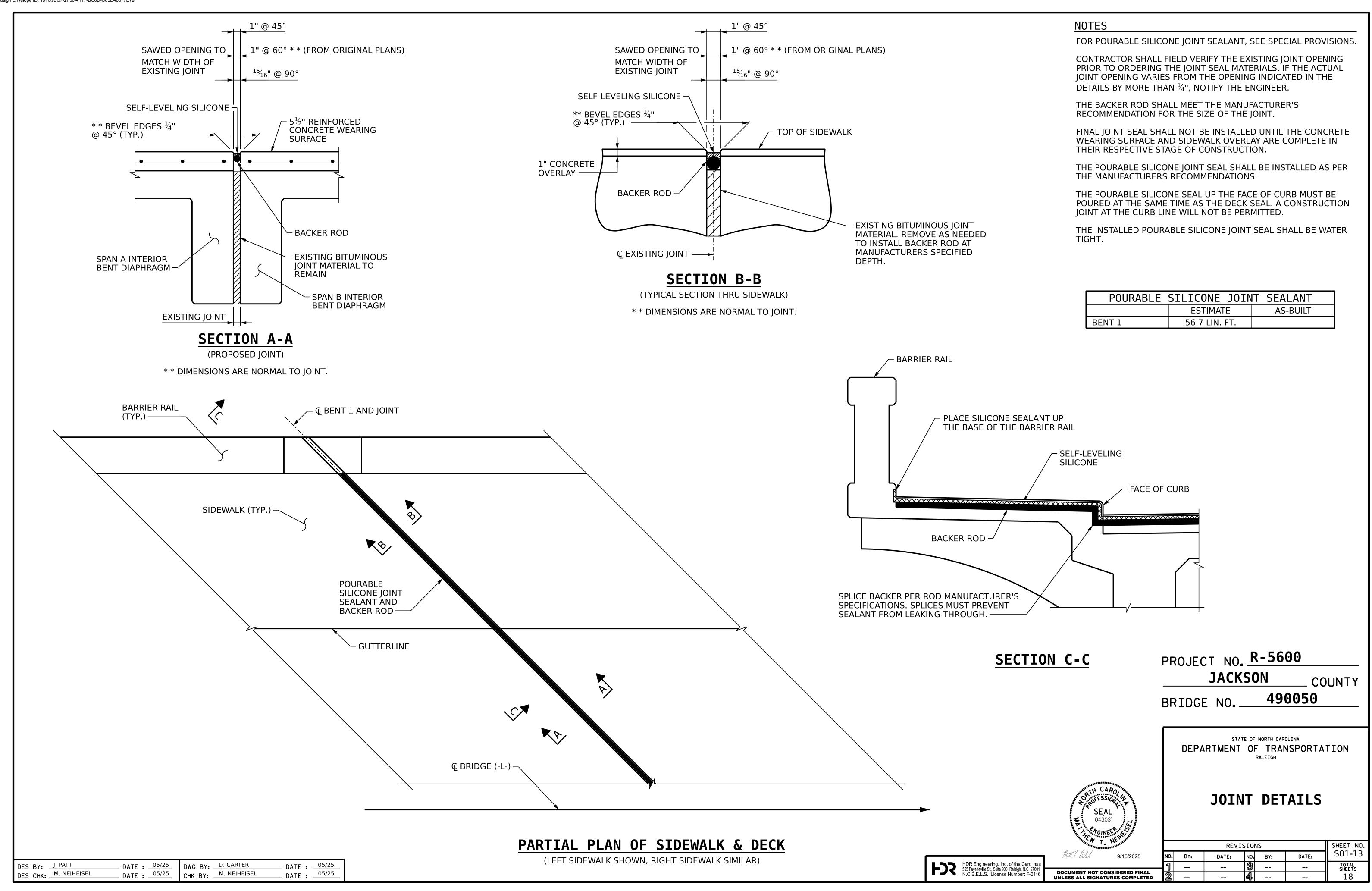
4'-9"

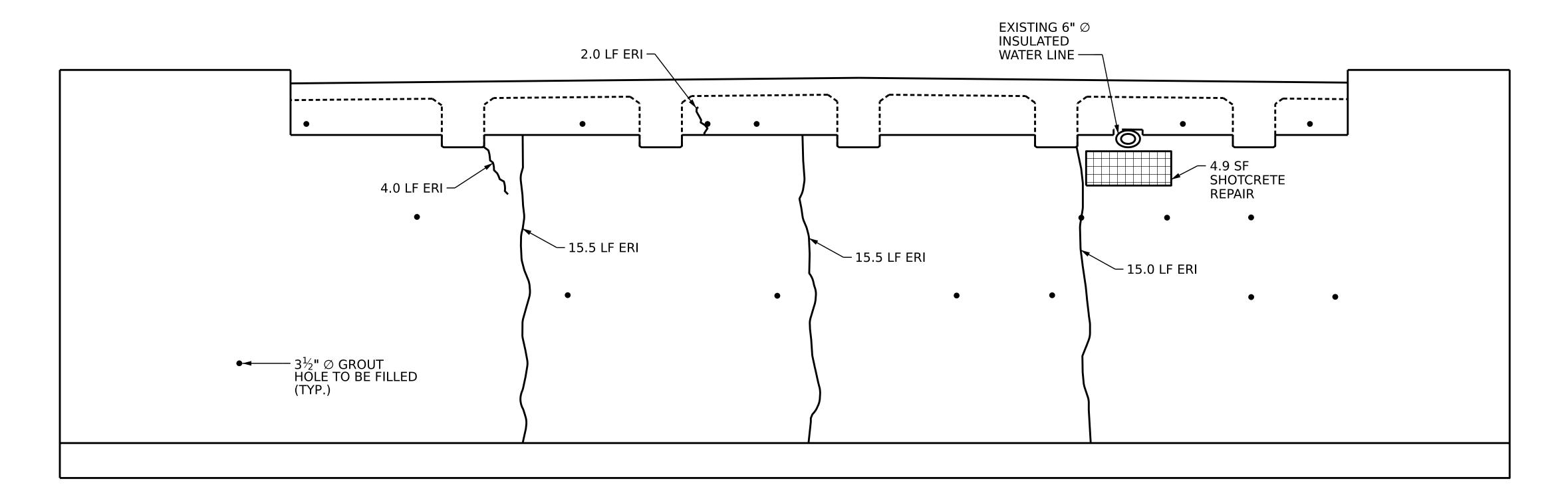
6'-0"

7'-8"

9'-5"

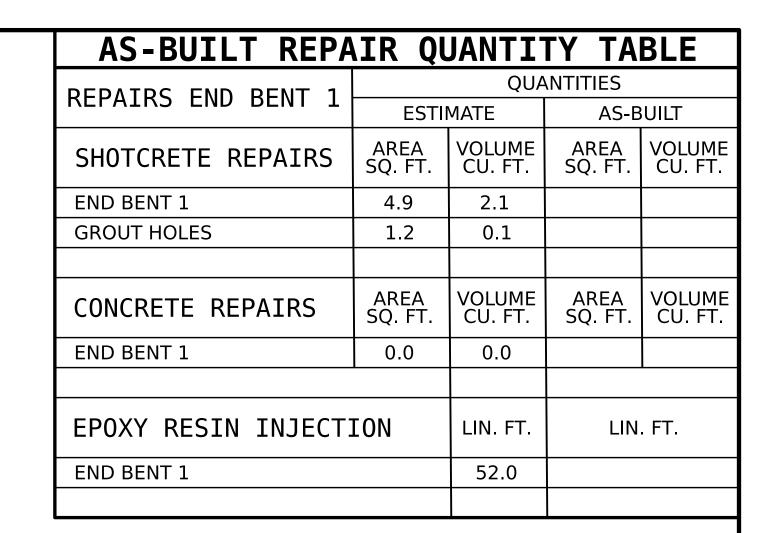
DWG BY: D. CARTER DES BY: M. NEIHEISEL __ DATE : 05/25 __ DATE : 05/25 DATE: 05/25
DATE: 05/25 DES CHK: J PATT CHK BY: J. PATT





ELEVATION

(LOOKING DOWNSTATION)



VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT. SEE "OVERHANG BEAM, AND DIAPHRAGM REPAIR DETAILS" SHEET.

NOTES

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE AS-BUILT REPAIR QUANTITY TABLE.

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT. FOR REPAIR DETAILS, SEE "TYPICAL BENT CAP AND BREASTWALL REPAIR DETAILS" SHEET

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

SHOTCRETE REPAIRS MAY BE REPLACED WITH CONCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



ERI - EPOXY RESIN INJECTION

 $3\frac{1}{2}$ "Ø GROUT HOLE TO BE FILLED

PROJECT NO. R-5600

JACKSON

COUNTY

SHEET NO. S01-14

TOTAL SHEETS

490050 BRIDGE NO.___

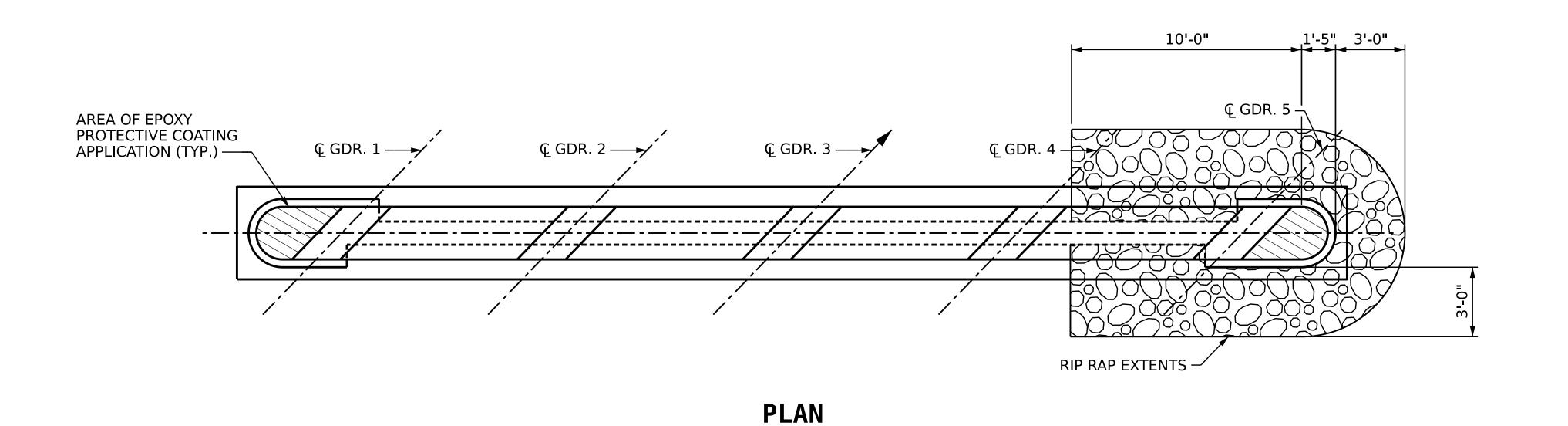
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE END BENT 1



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T. NEwalist			REVISIONS					
Matt Milit	9/16/2025	NO.	BY:	DATE:	NO.	BY:	DATE:	
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DES BY: J. PATT __ DATE : 03/25 __ DATE : 03/25 DWG BY: D. CARTER
CHK BY: M. NEIHEISEL __ DATE : 05/25 __ DATE : 05/25 DES CHK: M. NEIHEISEL



LIMITS OF EPOXY PROTECTIVE COATING -LIMITS OF EPOXY EXISTING 6" Ø PROTECTIVE COATING INSULATED WATER LINE -2.1 SF 4.5 SF SHOTCRETE CONCRETE ─ 1.8 SF REPAIR REPAIR -CONCRETE REPAIR 1.3 SF SHOTCRETE SHOTCRETE REPAIR → 14.0 LF ERI REPAIR − 14.0 LF ERI $3\frac{1}{2}$ " Ø GROUT HOLE TO BE FILLED 14.0 LF ERI-20 TONS CLASS II RIP RAP, TO EXTEND 3' ABOVE TOP OF FOOTING

VIEW A-A (LEFT SIDE VIEW)

SPAN A ELEVATION (LOOKING UPSTATION)

LEGEND



CONCRETE REPAIR AREA

CLASS II RIP RAP

EPOXY PROTECTIVE COATING

ERI - EPOXY RESIN INJECTION

3½"Ø GROUT HOLE TO BE FILLED



AS-BUILT REPA	IR QU	JANTI	ΓΥ ΤΑ	BLE
BENT 1		QUA	NTITIES	
SPAN A FACE	ESTI	MATE	AS-E	BUILT
SHOTCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUMI CU. FT.
CAP	0.0	0.0		
WALL	4.0	1.8		
GROUT HOLES	2.3	0.2		
CONCRETE REPAIRS	AREA SQ. FT.	VOLUME CU. FT.	AREA SQ. FT.	VOLUMI CU. FT
CAP	6.3	2.7		
WALL	0.0	0.0		
EPOXY RESIN INJECT	ION	LIN. FT.	LIN	. FT.
CAP		0.0		
WALL		42.0		
EPOXY COATING		SQ. FT.	SQ	. FT.
TOP OF CAP		10.9		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT. SEE "OVERHANG BEAM, AND DIAPHRAGM REPAIR DETAILS" SHEET.

NOTES

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE AS-BUILT REPAIR QUANTITY TABLE.

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT, FOR REPAIR DETAILS, SEE "TYPICAL BENT CAP AND BREASTWALL REPAIR DETAILS" SHEET

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

SHOTCRETE REPAIRS MAY BE REPLACED WITH CONCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

> PROJECT NO. R-5600 **JACKSON** COUNTY 490050 BRIDGE NO.__

SHEET 1 OF 2

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE BENT 1 SPAN A FACE

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		1			3			TOTAL SHEETS
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		2			4			18



DWG BY: D. CARTER DES BY: J. PATT _ DATE : 03/25 __ DATE : 05/25 __ DATE : 05/25 DES CHK: M. NEIHEISEL _ DATE : 03/25 CHK BY: M. NEIHEISEL

VIEW B-B

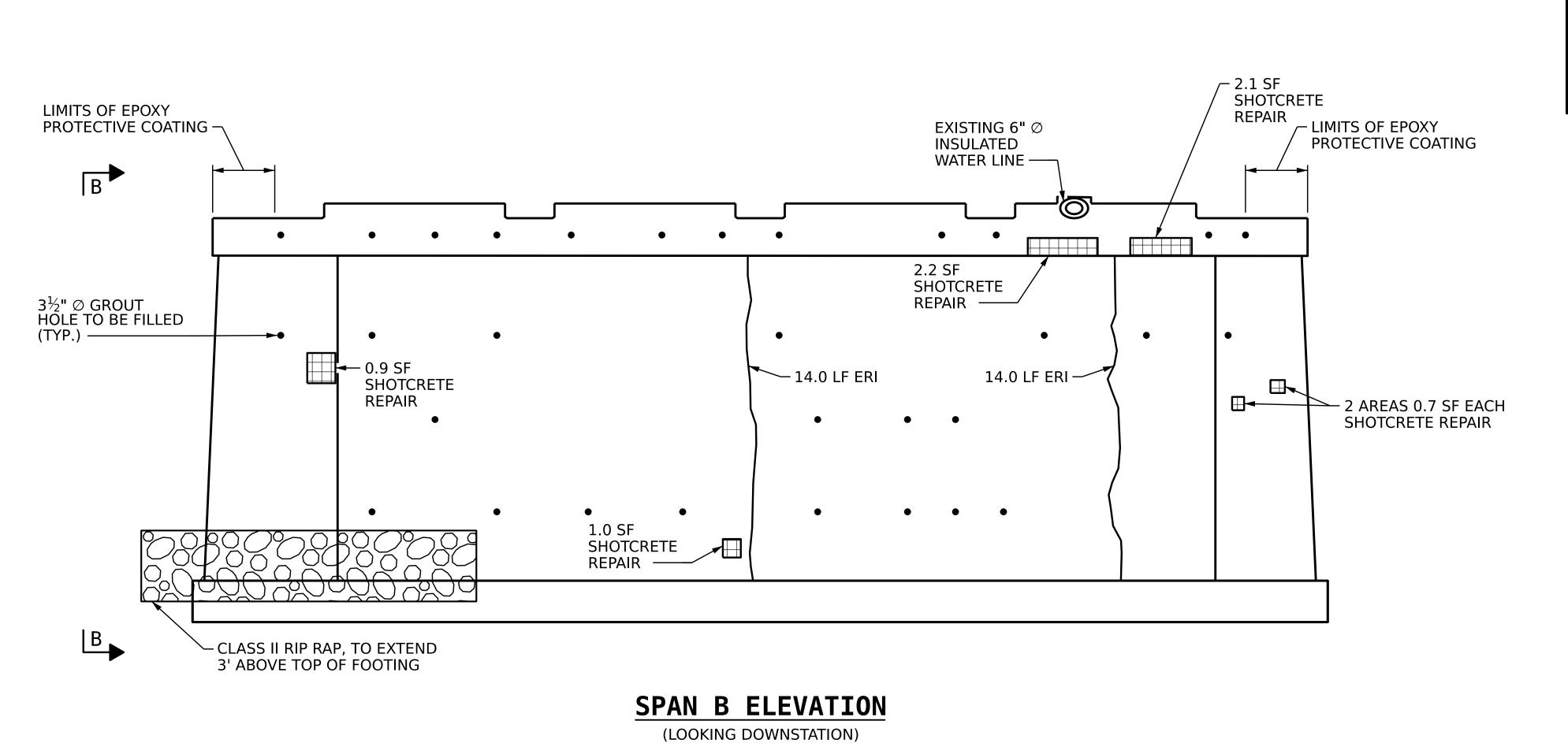
(RIGHT SIDE VIEW)

DES BY: J. PATT
DES CHK: M. NEIHEISEL

DWG BY: D. CARTER
CHK BY: M. NEIHEISEL

__ DATE : 05/25 __ DATE : 05/25

__ DATE : 03/25 __ DATE : 03/25



AS-BUILT REPAIR QUANTITY TABLE BENT 1 QUANTITIES SPAN B FACE **ESTIMATE AS-BUILT** AREA VOLUME SQ. FT. CU. FT. AREA SQ. FT. VOLUME CU. FT. SHOTCRETE REPAIRS CAP 1.9 4.3 WALL 1.5 3.3 **GROUT HOLES** 2.1 0.2 AREA VOLUME SQ. FT. CU. FT. AREA SQ. FT. VOLUME CU. FT. CONCRETE REPAIRS CAP 0.0 0.0 WALL 0.0 0.0 EPOXY RESIN INJECTION LIN. FT. LIN. FT. CAP 0.0 WALL 28.0

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT. SEE "OVERHANG BEAM, AND DIAPHRAGM REPAIR DETAILS" SHEET.

NOTES

FOR NOTES, SEE "SUBSTRUCTURE BENT 1 SPAN A FACE" SHEET.

LEGEND

SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

CLASS II RIP RAP

ERI - EPOXY RESIN INJECTION

 $3\frac{1}{2}$ "Ø GROUT HOLE TO BE FILLED

PROJECT NO. R-5600

JACKSON

COUNTY

490050 BRIDGE NO.___

SHEET 2 OF 2

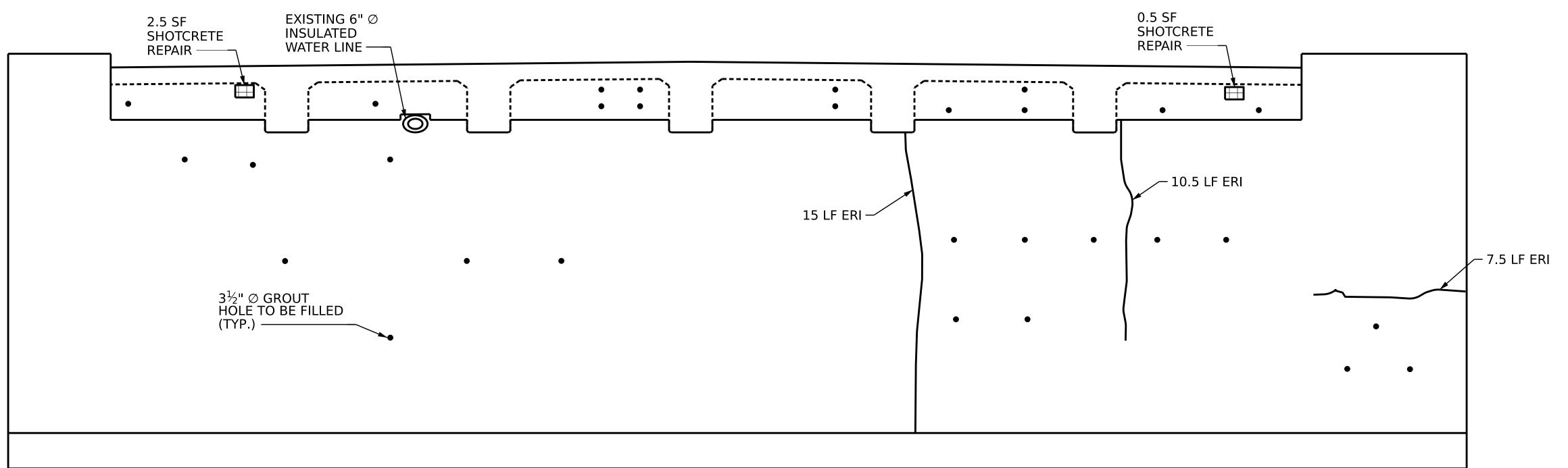
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> **SUBSTRUCTURE** BENT 1 SPAN B FACE

REVISIONS SHEET NO. S01-16 NO. BY: DATE: DATE: TOTAL SHEETS

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ELEVATION

(LOOKING UPSTATION)

AS-BUILT REPAIR QUANTITY TABLE QUANTITIES REPAIRS END BENT 2 **ESTIMATE AS-BUILT** AREA VOLUME SQ. FT. CU. FT. VOLUME CU. FT. AREA SQ. FT. SHOTCRETE REPAIRS END BENT 2 3.0 1.3 **GROUT HOLES** 2.0 0.2 AREA VOLUME AREA SQ. FT. VOLUME CU. FT. CONCRETE REPAIRS SQ. FT. CU. FT. END BENT 2 0.0 0.0 EPOXY RESIN INJECTION LIN. FT. LIN. FT. END BENT 2 33.0

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT. SEE "OVERHANG BEAM, AND DIAPHRAGM REPAIR DETAILS" SHEET.

NOTES

FOR NOTES SEE, "SUBSTRUCTURE END BENT 1" SHEET.

LEGEND

SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

ERI - EPOXY RESIN INJECTION

 $3\frac{1}{2}$ "Ø GROUT HOLE TO BE FILLED

PROJECT NO. R-5600

JACKSON

COUNTY

DATE:

SHEET NO. S01-17

TOTAL SHEETS

490050 BRIDGE NO.___

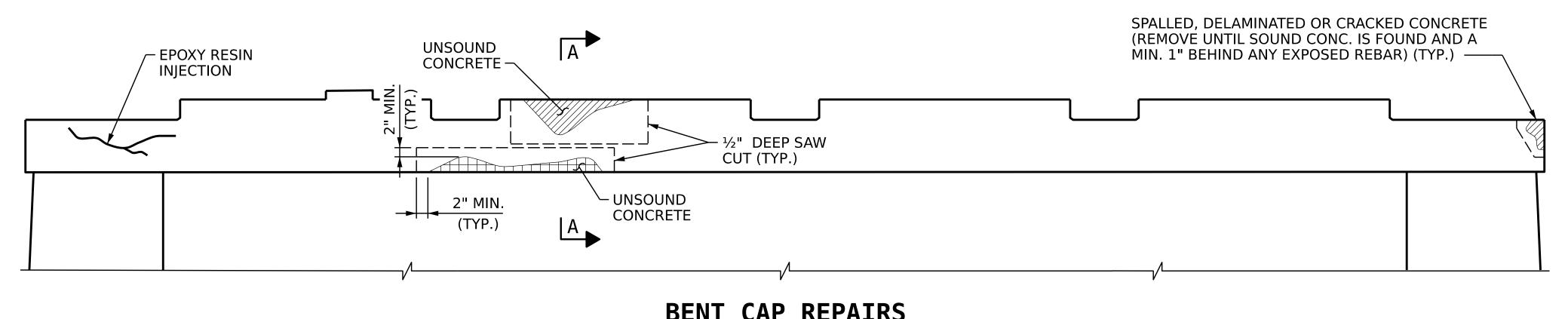
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> > **SUBSTRUCTURE** END BENT 2

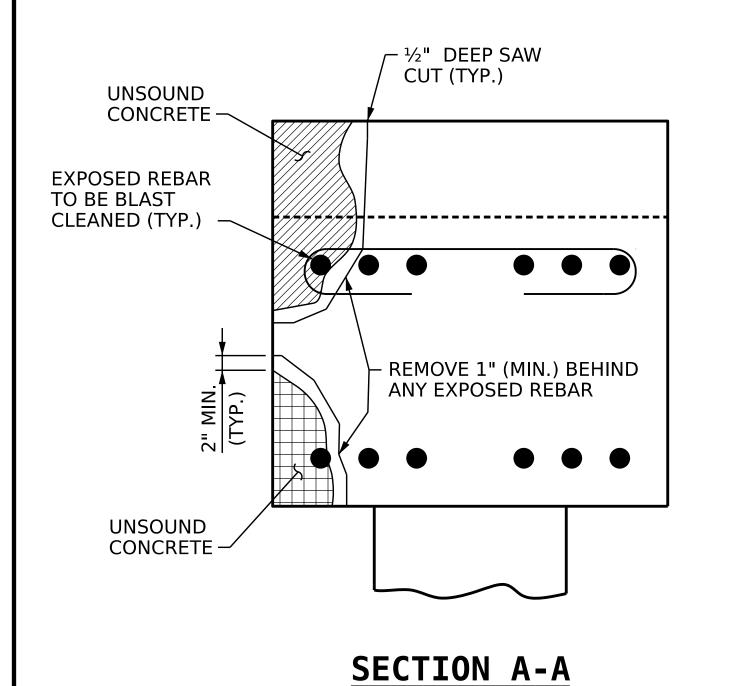
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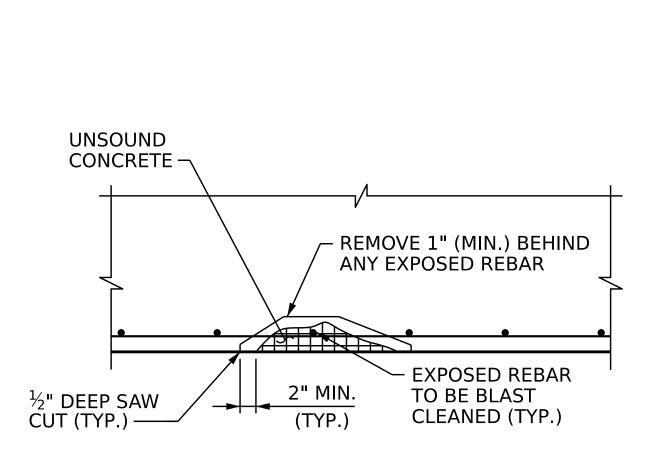
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DWG BY: D. CARTER
CHK BY: M. NEIHEISEL DES BY: J. PATT
DES CHK: M. NEIHEISEL __ DATE : 03/25 __ DATE : 03/25 __ DATE : 05/25 __ DATE : 05/25



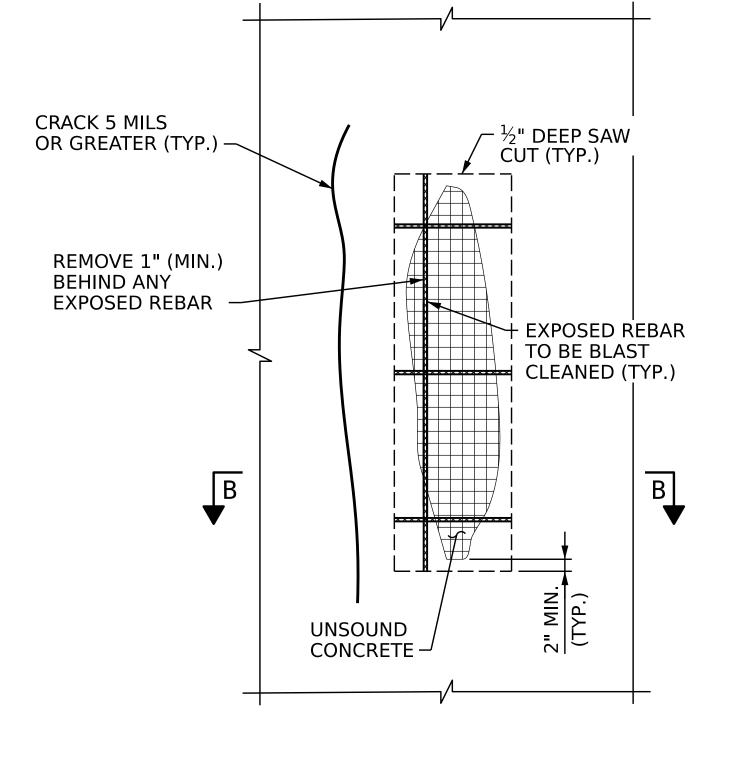
BENT CAP REPAIRS





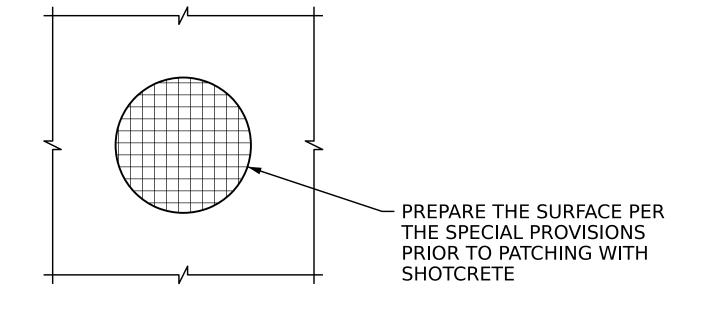
SECTION B-B

(TYPICAL FOR BENT AND END BENT REPAIR)



ELEVATION OF BREASTWALL

(TYPICAL FOR BENT AND END BENT REPAIR)



3½" Ø GROUT HOLE REPAIR

LEGEND

CONCRETE REPAIR AREA

SHOTCRETE REPAIR AREA

ERI - EPOXY RESIN INJECTION

SPLICE	LENGTH TABLE
BAR SIZE	MIN. SPLICE LENGTH
#4	1'-5"
#5	1'-11"
#6	2'-8"
#7	3'-8"
#8	4'-9"
#9	6'-0"
#10	7'-8"
#11	9'-5"

REPAIR SEQUENCE:

- 1. SOUND CONCRETE TO DETERMINE EXTENT OF REPAIR LOCATION.
- 2. REMOVE SURFACE CONCRETE TO VERIFY SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL. SAW CUT AROUND REPAIR AREA TO A NOMINAL DEPTH OF $\frac{1}{2}$ ".
- 3. REMOVE CONCRETE WITHIN SAW CUT AREA TO A MINIMUM $\frac{1}{2}$ " DEPTH. (PICTURE REQUIRED)
- 4. USE A WIRE BRUSH TO CLEAN ALL EXPOSED REINFORCING STEEL. FOR BARS WITH MORE THAN 10% SECTION LOSS, SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED.
- 5. REMOVE ALL LOOSE OR WEAKENED MATERIAL THEN CLEAN THE REPAIR AREA OF DIRT, GREASE, OIL, AND FOREIGN MATTER. (PICTURE REQUIRED)
- 6. PREPARE SURFACE AND PLACE APPROVED MATERIAL ACCORDING TO MANUFACTURERS SPECIFICATIONS. (PICTURE REQUIRED)

NOTES

TYPICAL BENT REPAIRS ARE SHOWN. REPAIR DETAILS SIMILAR FOR END BENTS.

CONTRACTOR SHALL SAW CUT THE REPAIR AREAS SO THAT THE CORNERS ARE SOUARE AS INDICATED ON THE DETAILS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP CROSS SECTIONAL AREA SHALL BE REMOVED AT ONE TIME. SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN ONE-THIRD OF THE CAP CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR BREASTWALL, IF THE AREAS OF REMOVAL ARE NOT ADJACENT TO OR DIRECTLY OPPOSITE ONE ANOTHER. IF REMOVAL EXTENDS MORE THAN $1\frac{1}{2}$ " BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED. SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

SHOTCRETE REPAIRS MAY BE REPLACED WITH CONCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER

REPAIRS SHALL PROVIDE A MINIMUM OF $1\frac{1}{2}$ " OF CONCRETE COVER TO ANY EXPOSED REINFORCING STEEL.

PROJECT NO. R-5600 **JACKSON** COUNTY 490050 BRIDGE NO. ___

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

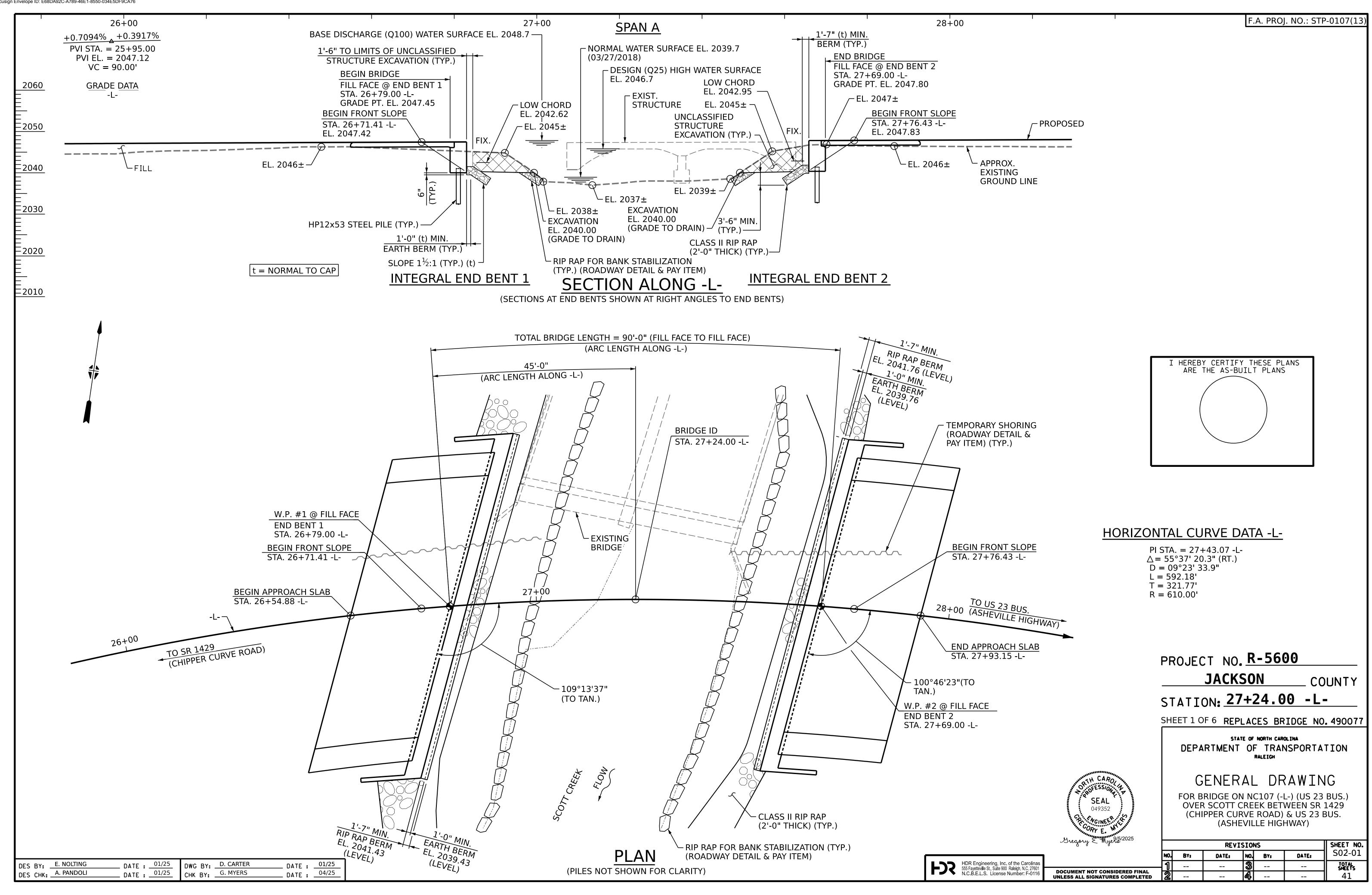
> > TYPICAL CAP AND BREASTWALL

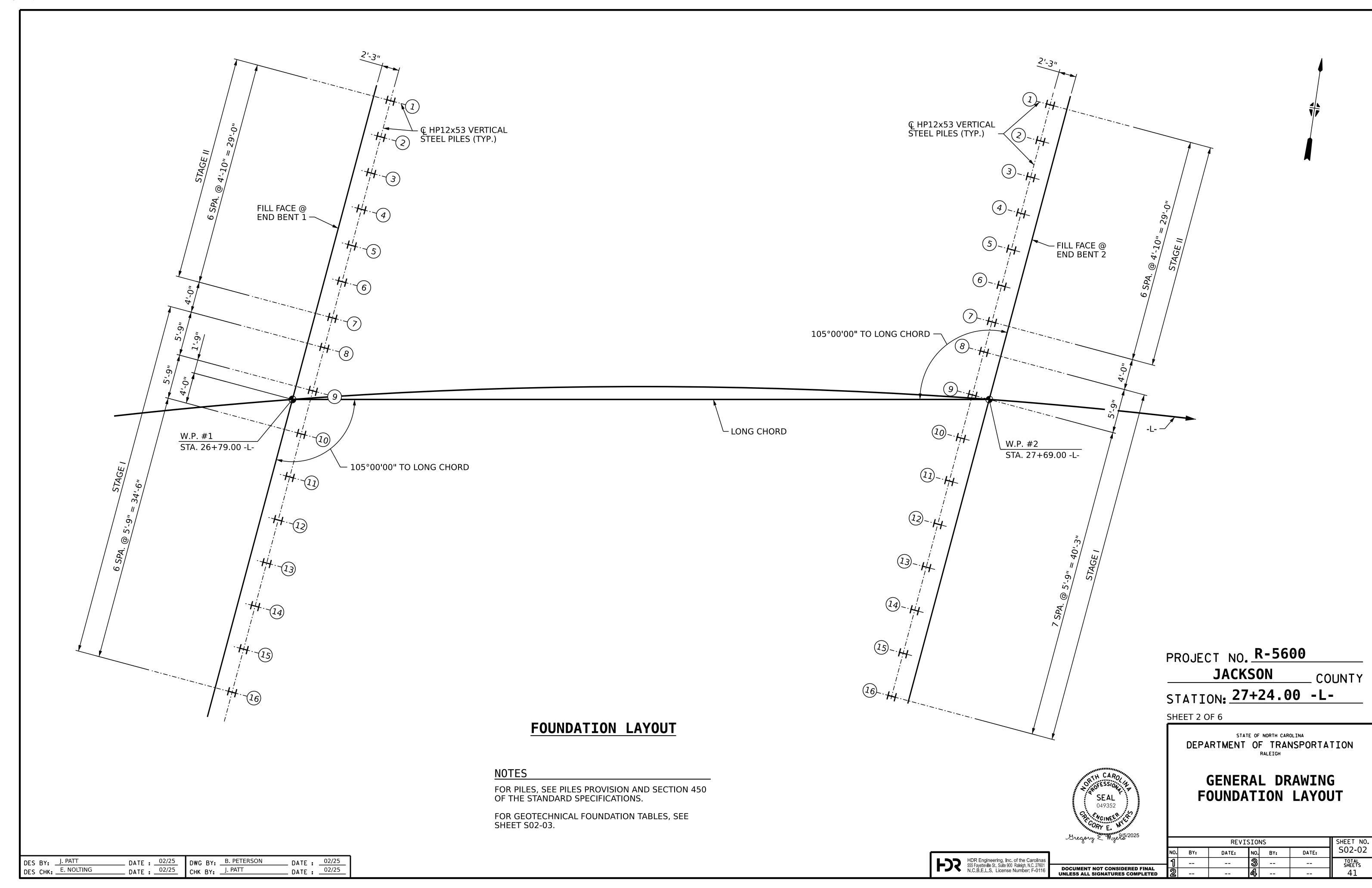
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DES BY: M. NEIHEISEL DWG BY: D. CARTER __ DATE : ____05/25 __ DATE : ____05/25 DATE: 05/25
DATE: 05/25 DES CHK: J. PATT CHK BY: J. PATT







SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

							Driven Piles		Predrilling for Piles **		Drilled-In Piles			
End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Number of Piles per Line	Factored Resistance per Pile KIPS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Length per Pile FT	Scour Critical Elevation FT	Minimum Pile Tip (Tip No Higher Than) Elevation FT	Required Driving Resistance (RDR)* per pile KIPS	Pile Redrives Quantity EACH	Predrilling Length per Pile LIN FT	Predrilling Elevation (Elevation Not To Predrill Below) FT	Maximum Predrilling Diameter INCHES	Pile Excavation (Bottom of Hole) Elevation FT	Pile Excavation Not In Soil per Pile LIN FT	Pile Excavation In Soil per Pile LIN FT
End Bent 1, Pile 1-16	16	215	2040.93	65			360							
End Bent 2, Pile 1-16	16	215	2041.26	50			360							
TOTAL QUANTITY:														

 $^{^*}RDR = \frac{Factored\ Resistance + Factored\ Drag\ Load + Factored\ Dead\ Load}{Powerform} + Nominal\ Drag\ Load\ Resistance + Nominal\ Resistance\ from\ Scourable\ Material$

Dynamic Resistance Factor

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile KIPS	Factored Drag Load per Pile KIPS	Factored Dead Load * per Pile KIPS	Dynamic Resistance Factor	Nominal Drag Resistance per Pile KIPS	Nominal Scour Resistance per Pile KIPS
End Bent 1, Pile 1-16	215			0.60		
End Bent 2, Pile 1-16	215			0.60		
				_		

^{*} Factored Dead Load is factored weight of pile above the ground line.

SUMMARY OF PILE ACCESSORIES

(Blank entries indicate item is not applicable to structure)

	Dina	,	Steel Pile Points	3
End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates EACH	Pipe Pile Cutting Shoes EACH	Pipe Pile Conical Points EACH	H-Pile Points EACH
End Bent 1, Pile 1-16				16
End Bent 2, Pile 1-16	,			16
TOTAL QUANTITY:	,			32

SUMMARY OF DPT/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No (e.g., "Bent 1 - Bent 3")	DPT Test Pile Length FT	DPT Testing Quantity EACH
End Bent 1, Pile 1-16	65	
End Bent 2, Pile 1-16	50	1

Pile Order Lengths for C	oncrete Piles
End Bent / Bent No (e.g., "Bent 1 - Bent 3")	Pile Order Length Basis* EST or DPT

* EST = Pile order lengths from estimated pile lengths; DPT = Pile order lengths based on Dynamic Pile Testing. For groups of end bents/bents with pile order lengths based on DPT testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the DPT.

NOTES:

- 1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Saket Kabra, #053059) on 02-25-2025.
- 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
- 3. The Engineer may adjust the quantity for DPT Testing and Pipe Pile Plates when necessary.



PROJECT NO. R-5600 **JACKSON**

_ COUNTY STATION: 27+24.00 -L-

SHEET 3 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> GENERAL DRAWING **GEOTECHNICAL**

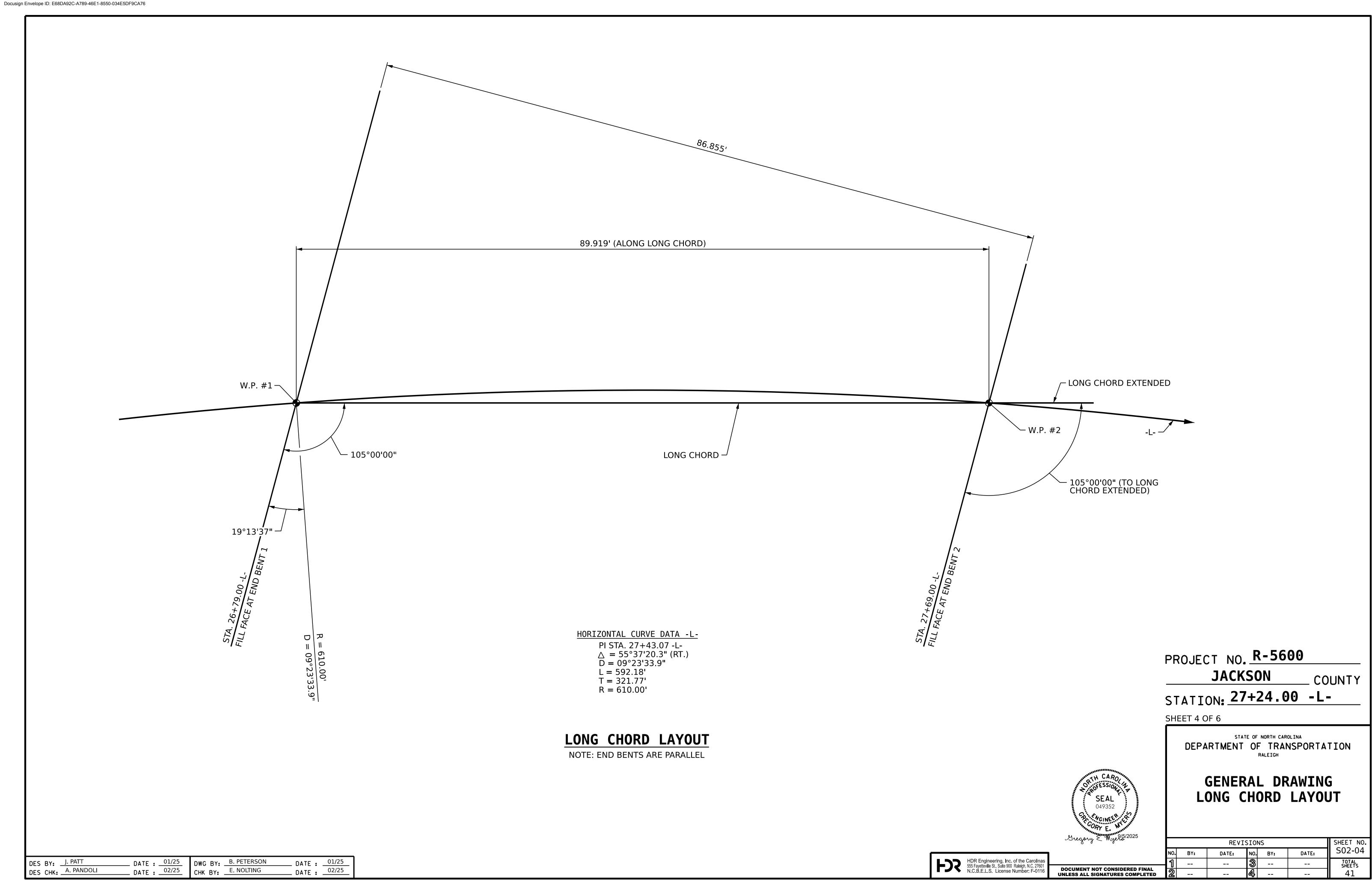
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DATE:	NO.	BY:	DATE:	S02-03

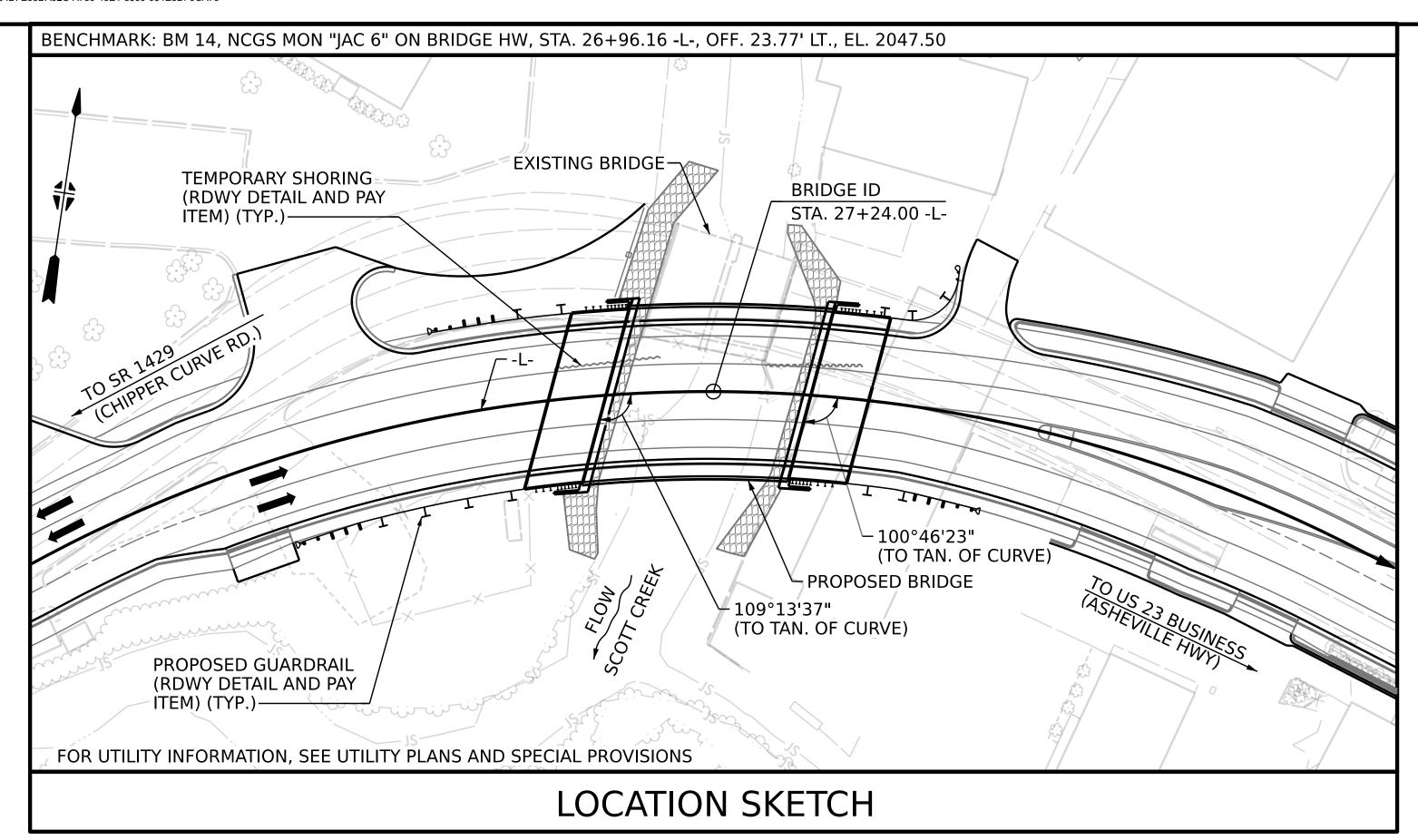
HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

S BY: .	S. KABRA	DATE : 02/25	DWG BY: B. PETERSON	DATE :	02/25
S CHK:	P. ZHANG	00/05	CHK BY: G. MYERS	DATE :	04/25

^{**} Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.





HYDRAULIC DATA

DESIGN DISCHARGE ____ = 3,480 CFS FREQUENCY OF DESIGN FLOOD ____ = 25 YR. DESIGN HIGH WATER ELEVATION ____ = 2046.7 FT. DRAINAGE AREA _____ = 51.6 SQ. MI. BASE DISCHARGE (Q100) _____ = 4,580 CFS BASE HIGH WATER ELEVATION ____ = 2048.7 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE ____ = 3,600 CFS FREOUENCY OF OVERTOPPING FLOOD.__= > 25 YR. OVERTOPPING FLOOD ELEVATION ____= 2046.9 FT.

	TOTAL BILL OF MATERIAL											
	REMOVAL OF EXISTING STRUCTURE AT STATION 27+24.00 -L-	ASBESTOS ASSESSMENT	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 27+24.00 -L-	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS, STATION 27+24.00 -L-	REINFORCING STEEL (BRIDGE)		FIB 36" ESTRESSED CONCRETE GIRDERS		
	LUMP SUM	LUMP SUM	LUMP SUM	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	NO.	LIN. FT.		
SUPERSTRUCTURE				6,672	8,097		LUMP SUM		10	866.8		
END BENT 1						38.1		7,029				
END BENT 2						36.9		6,792				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	6,672	8,097	75.0	LUMP SUM	13,821	10	866.8		

	TOTAL BILL OF MATERIAL												
	PILE DRIVING EQUIPMENT SETUP FOR HP12x53 STEEL PILES		12 X 53 EEL PILES	STEEL PILE POINTS	DYNAMIC PILE TESTING (DPT)	THREE BAR METAL RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS				
	EA.	NO.	LIN. FT.	EA.	EA.	LIN. FT.	TONS	SQ. YD.	LUMP SUM				
SUPERSTRUCTURE						161.0			LUMP SUM				
END BENT 1	16	16	1,040	16			245	270					
END BENT 2	16	16	800	16	1		190	210					
TOTAL	32	32	1,840	32	1	161.0	435	480	LUMP SUM				

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS
REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH
SAMPLES OF EACH SIZE BAR USED. THE SAMPLE BARS SHOULD COME FROM STEEL ACTUALLY USED IN THE PROJECT AND THE SAMPLE BARS SHOULD BE REPLACED BY SPLICED BARS AS SPECIFIED IN THE SAMPLE BAR REPLACEMENT CHART. PAYMENT FOR THE SAMPLE BARS AND REPLACEMENT REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

ALL PAVEMENT MARKING WILL BE IN ACCORDANCE WITH THE PAVEMENT MARKING PLANS AND SHALL PROVIDE FOR BICYCLES.

SAMPLE BAR REPLACEMENT SIZE LENGTH 6'-2" 7'-4" #4 8'-6" #5 9'-8" #7 10'-10"

12'-0"

13'-2"

14'-6"

15'-10"

#8

SAMPLE BAR REPLACEMENT LENGTHS BASED ON 30" (SAMPLE LENGTH) PLUS TWO SPLICE LENGTHS AND $f_V = 60 \text{ksi}$.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 27+24.00 -L-".

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S02-01 SHALL BE EXCAVATED FOR A DISTANCE OF 95 FT LEFT AND 68 FT RIGHT OF CENTERLINE ROADWAY AT END BENT 1 AND 68 FT EACH SIDE OF CENTERLINE ROADWAY AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

STEEL SHEET PILING REQUIRED FOR SHORING SHALL BE HOT ROLLED.

TEMPORARY SHORING WILL BE REQUIRED IN THE AREA INDICATED IN THE PLAN VIEW.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF (2) 27'-6" CONCRETE ARCH CULVERTS AND STEEL ROLLED BEAM SPANS, 39'-3" TO 45'-0" CLEAR ROADWAY WIDTH AND CONCRETE SLAB ON C.I.P. CONCRETE BENT CAPS SUPPORTED ON CONCRETE FOOTINGS AND LOCATED 44 FT UPSTREAM FROM PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 -**EVALUATING SCOUR AT BRIDGES."**

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 5 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY



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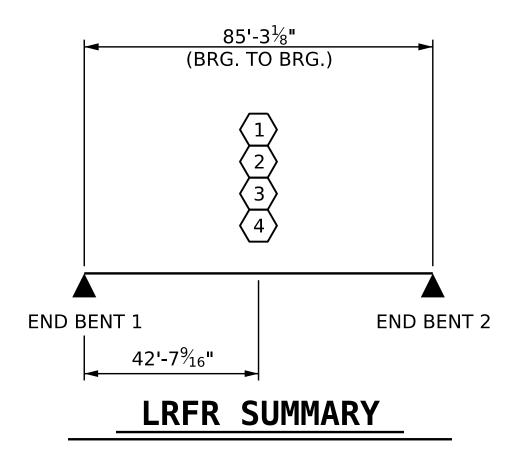
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DWG BY: B. PETERSON DES BY: J. PATT DATE : 02/25 CHK BY: E. NOLTING DES CHK: E. NOLTING

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

DOCUMENT NOT

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																								
										STF	RENGT	H I LIMIT	STAT	E					SER	VICE II	I LIMI	Γ STATE		
				(#)						MC	OMENT	Γ			SHE	AR					MOM	ENT		<u>~</u>
LOAD TYPE		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A		1.18		1.75	0.77	1.43	Α	EL	42.63	0.85		Α	ı	7.96	0.80	0.77	1.18	Α	EL	42.63	
DESIG		HL-93 (OPERATING)	N/A		1.86			0.77	1.86	Α	EL	42.63	 		Α		7.96	N/A						
LOA	ا ر	HS-20 (INVENTORY)	36.000	(2)	1.59	57.24	1.75	0.77	1.93	Α	EL	42.63	-		Α		7.96	0.80	0.77	1.59	Α	EL	42.63	
		HS-20 (OPERATING)	36.000		2.45	88.20	1.35	0.77	2.50	Α	EL	42.63	0.85	2.45	Α		7.96	N/A						
		SNSH	13.500		3.69	49.82	1.40	0.77	5.60	Α	EL	42.63	0.85	5.86	Α	I	7.96	0.80	0.77	3.69	Α	EL	42.63	
	L L	SNGARBS2	20.000		2.71	54.20	1.40	0.77	4.11	Α	EL	42.63	0.85	4.10	Α		7.96	0.80	0.77	2.71	Α	EL	42.63	
	SINGLE VEHICLE (SV)	SNAGRIS2	22.000		2.55	56.10	1.40	0.77	3.86	Α	EL	42.63	0.85	3.79	Α	I	7.96	0.80	0.77	2.55	Α	EL	42.63	
	VE SV	SNCOTTS3	27.250		1.84	50.14	1.40	0.77	2.79	Α	EL	42.63	0.85	2.87	Α	1	7.96	0.80	0.77	1.84	Α	EL	42.63	
) (9)	SNAGGRS4	34.925		1.52	53.09	1.40	0.77	2.30	Α	EL	42.63	0.85	2.34	Α	ı	7.96	0.80	0.77	1.52	Α	EL	42.63	
	<u>N</u>	SNS5A	35.550		1.49	52.97	1.40	0.77	2.25	Α	EL	42.63	0.85	2.36	Α	ı	7.96	0.80	0.77	1.49	Α	EL	42.63	
	S	SNS6A	39.950		1.36	54.33	1.40	0.77	2.06	Α	EL	42.63	0.85	2.13	Α		7.96	0.80	0.77	1.36	Α	EL	42.63	
LEGAL		SNS7B	42.000		1.29	54.18	1.40	0.77	1.96	Α	EL	42.63	0.85	2.08	Α		7.96	0.80	0.77	1.29	Α	EL	42.63	
LOAD		TNAGRIT3	33.000		1.65	54.45	1.40	0.77	2.51	Α	EL	42.63	0.85	2.57	Α		7.96	0.80	0.77	1.65	Α	EL	42.63	
	O.R R	TNT4A	33.075		1.66	54.90	1.40	0.77	2.51	Α	EL	42.63	0.85	2.51	Α		7.96	0.80	0.77	1.66	Α	EL	42.63	
	CT(LEF	TNT6A	41.600		1.35	56.16	1.40	0.77	2.04	Α	EL	42.63	0.85	2.20	Α		7.96	0.80	0.77	1.35	Α	EL	42.63	
	RA RAI ST)	TNT7A	42.000		1.35	56.70	1.40	0.77	2.05	Α	EL	42.63	0.85	2.16	Α		7.96	0.80	0.77	1.35	Α	EL	42.63	
	T = E	TNT7B	42.000		1.39	58.38	1.40	0.77	2.11	Α	EL	42.63	0.85	2.04	Α		7.96	0.80	0.77	1.39	Α	EL	42.63	
	TRUCK TRACTC SEMI-TRAILER (TTST)	TNAGRIT4	43.000		1.33	57.19	1.40	0.77	2.01	Α	EL	42.63	0.85	1.98	Α		7.96	0.80	0.77	1.33	Α	EL	42.63	
	± ''	TNAGT5A	45.000		1.26	56.70	1.40	0.77	1.90	Α	EL	42.63	0.85	1.95	Α	ı	7.96	0.80	0.77	1.26	Α	EL	42.63	
		TNAGT5B	45.000	3	1.24	55.80	1.40	0.77	1.89	Α	EL	42.63	0.85	1.88	Α		7.96	0.80	0.77	1.24	Α	EL	42.63	
EMERG	ENCY	EV2	28.750		1.91	54.91	1.30	0.77	3.12	Α	EL	42.63	0.85	3.07	Α		7.96	0.80	0.77	1.91	Α	EL	42.63	
VEHICLI	E (EV)	EV3	43.000	4	1.25	53.75	1.30	0.77	2.05	Α	EL	42.63	0.85	2.03	Α		7.96	0.80	0.77	1.25	Α	EL	42.63	



LOAD FACTORS:

DESIGN	LIMIT STATE	γDC	γDW
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

CONTROLLING LOAD RATING

DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING * *

4 EMERGENCY VEHICLE LOAD RATING **

* * SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER- EXTERIOR RIGHT GIRDER

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 6 OF 6

STATE OF NORTH CAROLINA

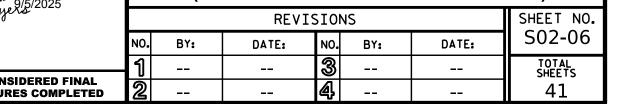
DEPARTMENT OF TRANSPORTATION

RALEIGH

LRFR SUMMARY FOR **PRESTRESSED CONCRETE GIRDERS**

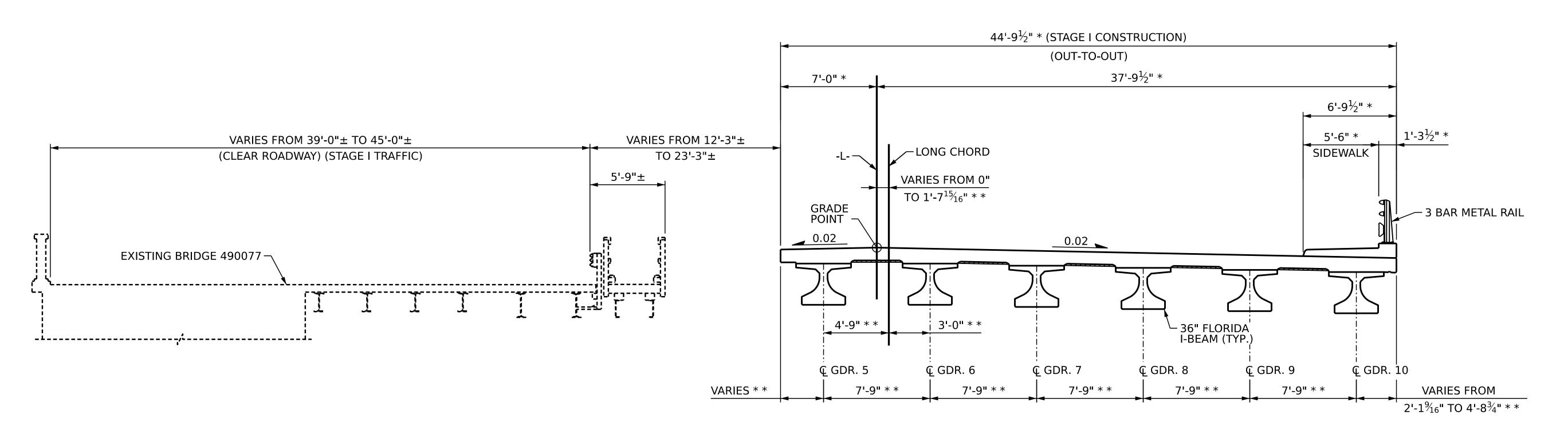
_ COUNTY

(NON-INTERSTATE TRAFFIC)



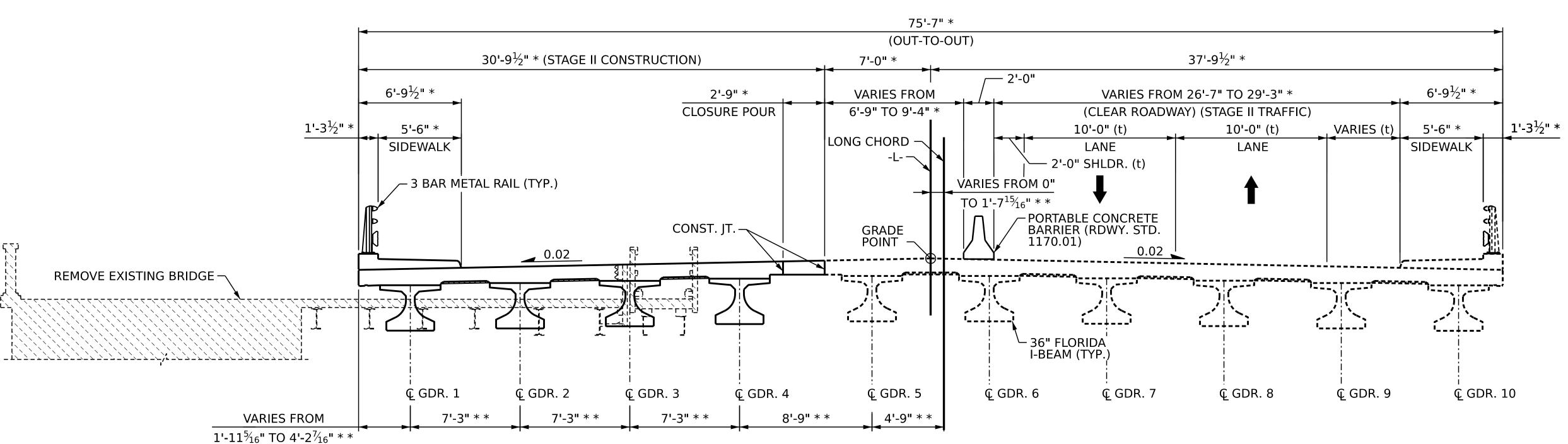
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STAGE I

(CONSTRUCT STAGE I OF PROPOSED STRUCTURE WITH EXISTING STRUCTURE IN PLACE)



PROJECT NO. R-5600

JACKSON COUNTY

STATION: 27+24.00 -L-

STAGE II (SHIFT TRAFFIC, REMOVE EXISTING STRUCTURE AND CONSTRUCT REMAINDER OF PROPOSED STRUCTURE)

- * DIMENSION RADIAL TO -L-
- ** DIMENSION NORMAL TO LONG CHORD
- (t) DIMENSION RADIAL TO PORTABLE CONCRETE BARRIER

SEAL
049352

Megory & Myer/5/2025

SUPERSTRUCTURE

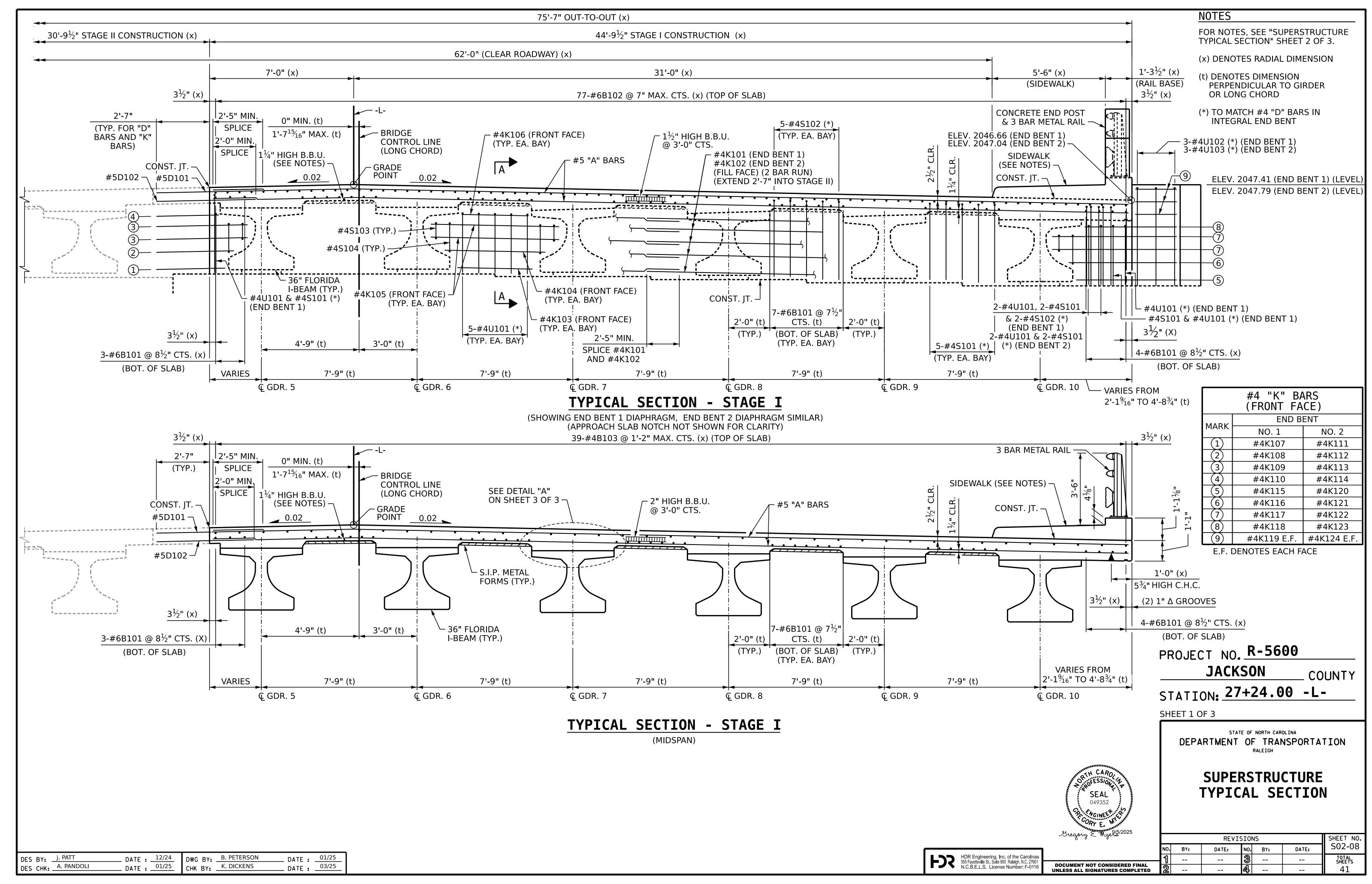
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE CONSTRUCTION SEQUENCE

0 0 0			KE V I :		SHEET NO.		
	NO.	BY:	DATE:	NO.	BY:	DATE:	S02-07
	1			3			TOTAL SHEETS
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	2			4			41

DES BY: E. NOLTING DATE: 01/25 DWG BY: B. PETERSON DATE: 01/25
DES CHK: A. PANDOLI DATE: 01/25 CHK BY: E. NOLTING DATE: 02/25



DWG BY: B. PETERSON

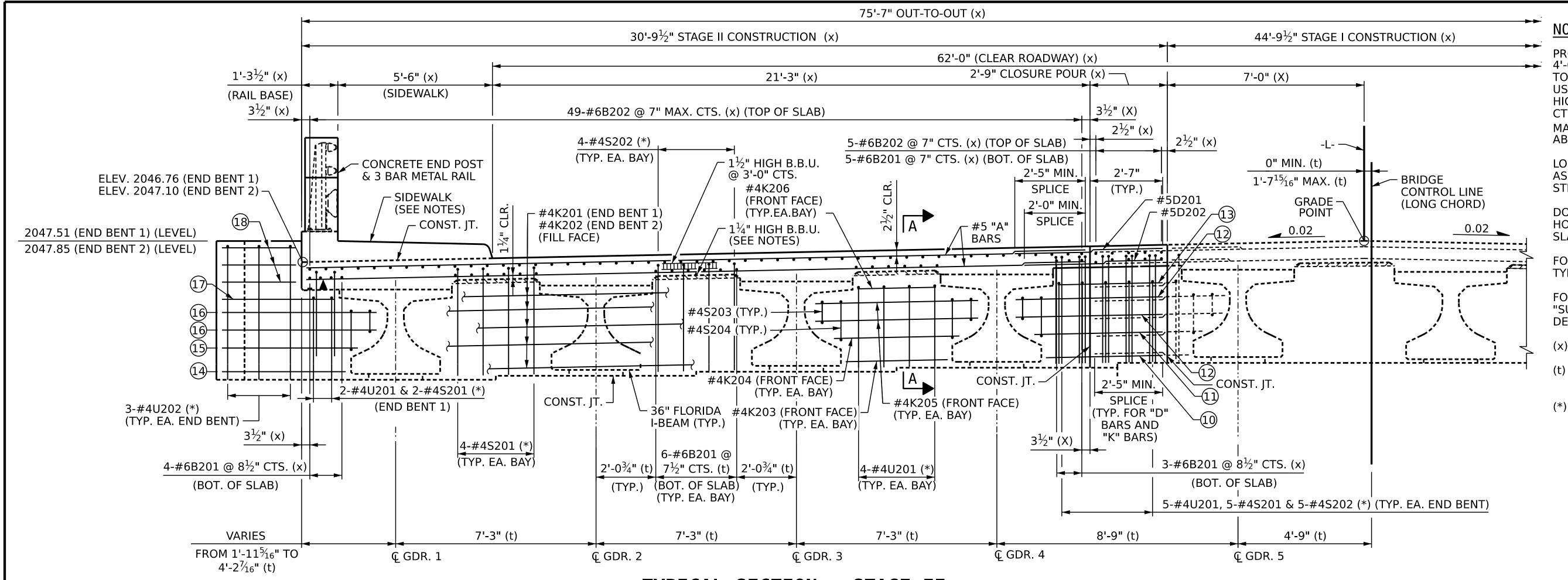
CHK BY: K. DICKENS

DATE: 01/25
DATE: 03/25

DATE : 12/24 DATE : 01/25

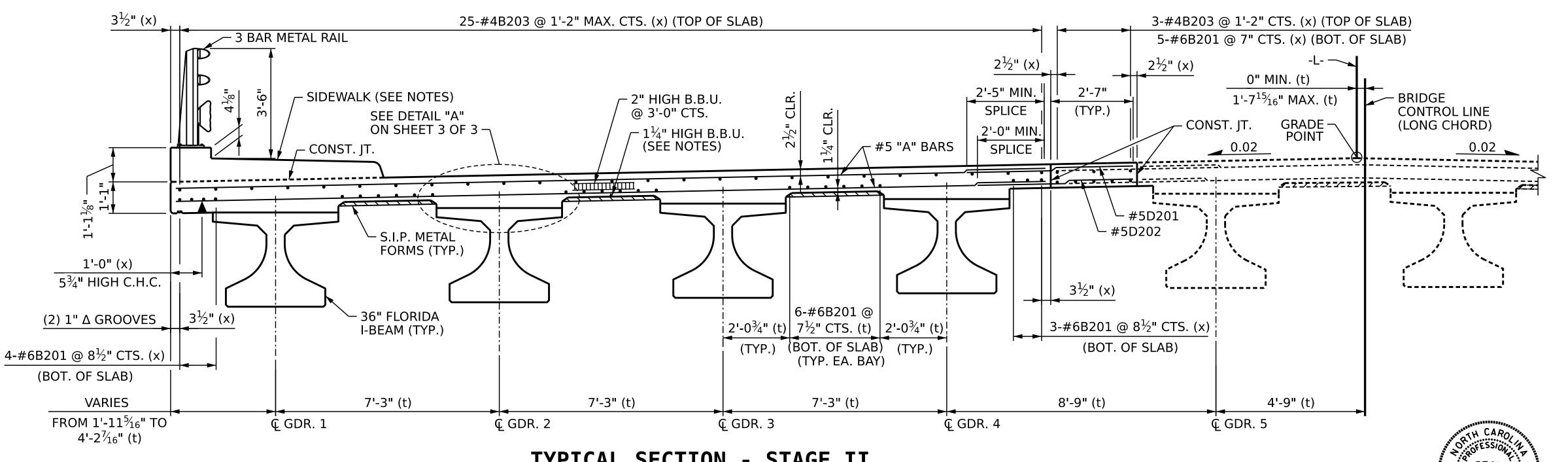
DES BY: J. PATT

DES CHK: A. PANDOLI



TYPICAL SECTION - STAGE II

(SHOWING END BENT 1 DIAPHRAGM, END BENT 2 DIAPHRAGM SIMILAR) (APPROACH SLAB NOTCH NOT SHOWN FOR CLARITY)



TYPICAL SECTION - STAGE II

(MIDSPAN)

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DOCUMENT NOT CONSIDERED FINAL

NOTES

PROVIDE $1\frac{1}{4}$ " HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF $2\frac{1}{2}$ " ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP AND BOTTOM SLAB REINFORCING STEEL

FOR SECTION A-A, SEE "SUPERSTRUCTURE TYPICAL SECTION" SHEET 3 OF 3.

FOR SIDEWALK DETAILS AND NOTES, SEE "SUPERSTRUCTURE CONCRETE SIDEWALK DETAILS" SHEET.

- (x) DENOTES RADIAL DIMENSION
- (t) DENOTES DIMENSION PERPENDICULAR TO GIRDER OR LONG CHORD.
- (*) TO MATCH #4 "D" BARS IN INTEGRAL END BENT

	#4 "K" E	_								
	(FRONT F	ACE)								
MARK	END BENT									
MARK	NO. 1	NO. 2								
(10)	#K207	#K211								
(11)	#K208	#K212								
(12)	#K209	#K213								
13	#K210	#K214								
(14)	#K215	#K220								
(15)	#K216	#K221								
(16)	#K217	#K222								
(17)	#K218	#K223								
(18)	#4K219 E.F.	#4K224 E.F.								

E.F. DENOTES EACH FACE

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

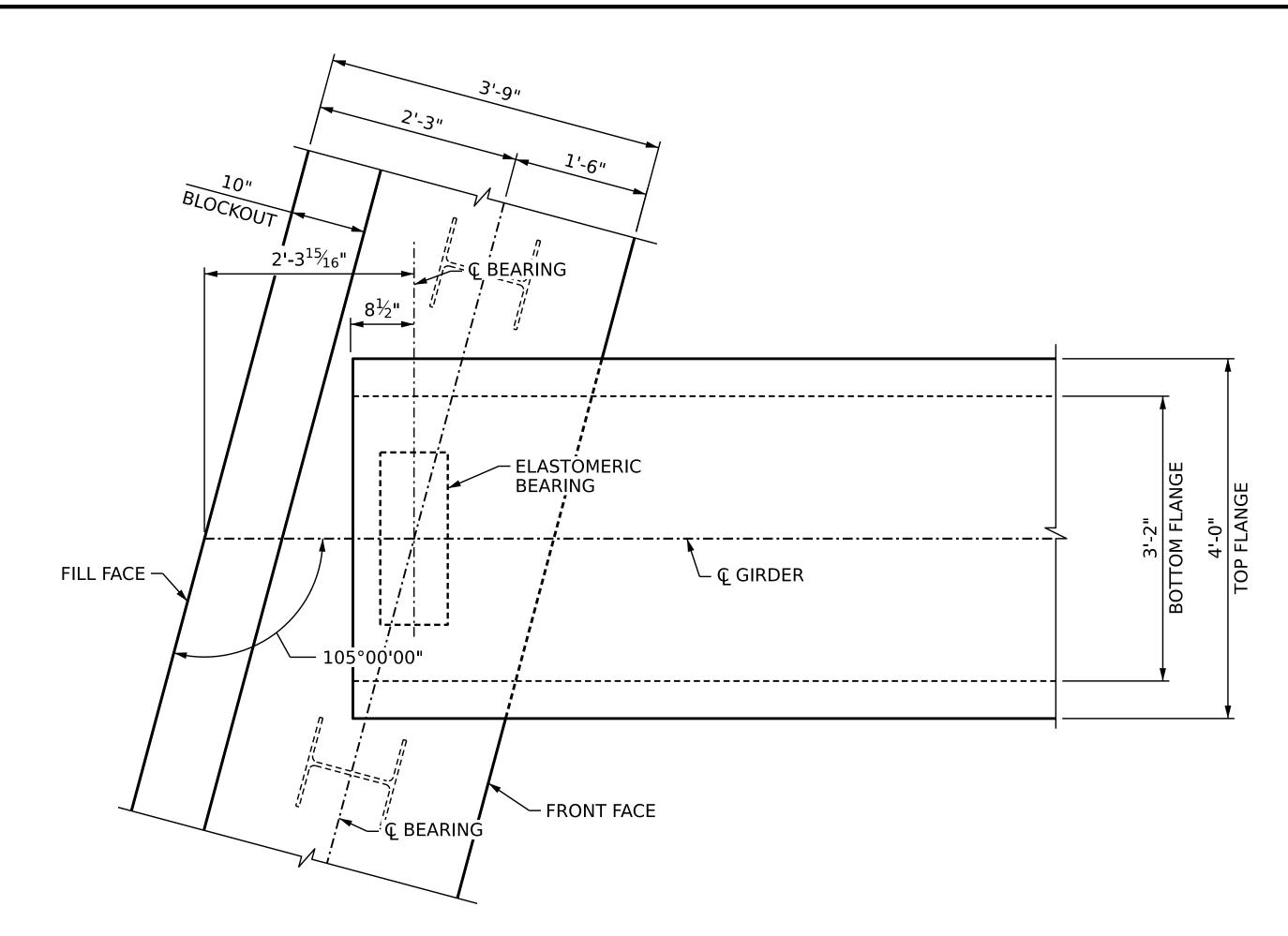
SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

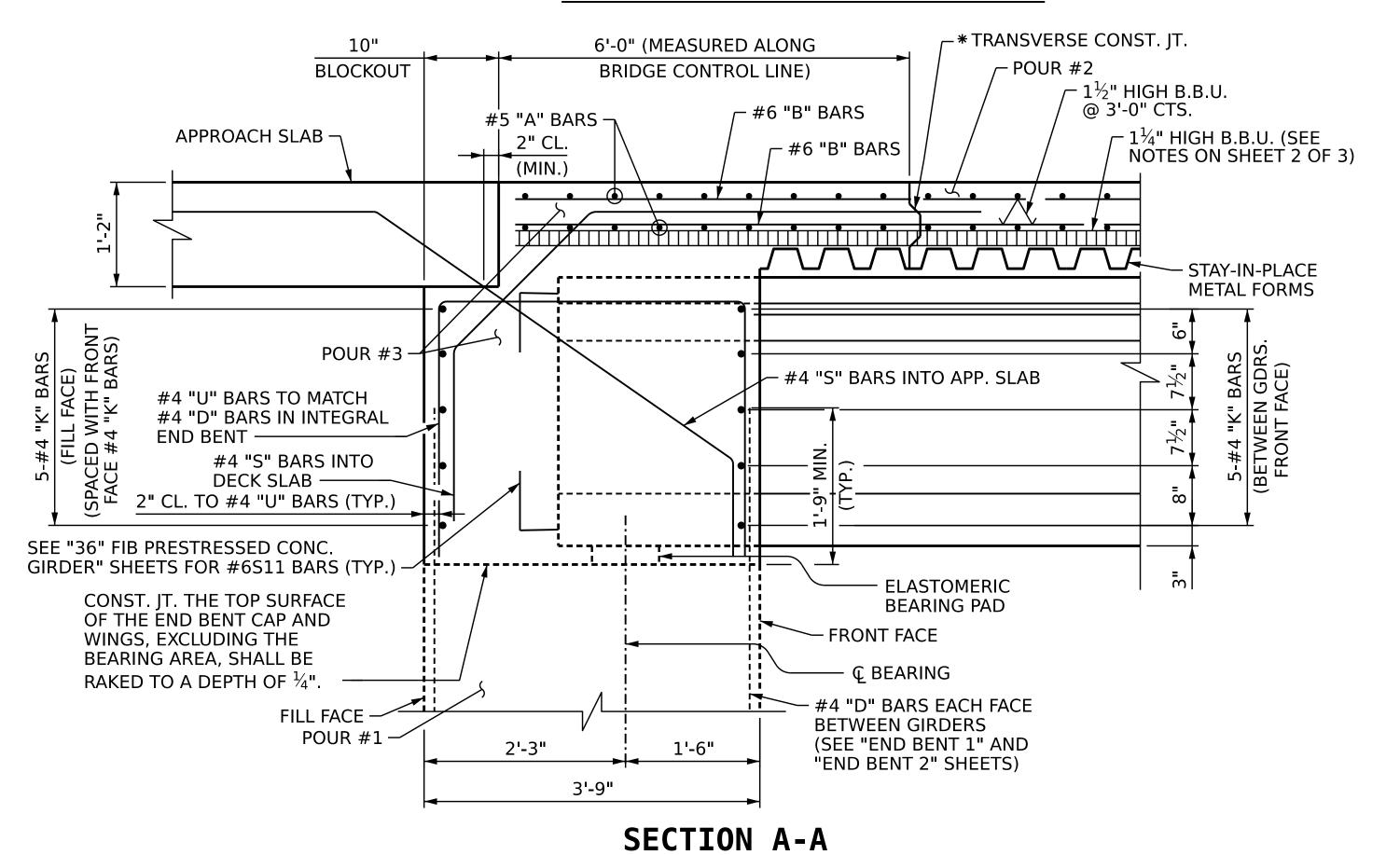
COUNTY

SUPERSTRUCTURE TYPICAL SECTION

	REVISIONS											
BY:	DATE:	NO.	BY:	DATE:	S02-09							
-		®	1		TOTAL SHEETS							
-		4			41							



PLAN OF GIRDER AT END BENT



* FOR CONSTRUCTION JOINT DETAIL, SEE

"SUPERSTRUCTURE BILL OF MATERIALS"

SHEET 1 OF 2.

DWG BY: B. PETERSON

CHK BY: K. DICKENS

DATE: 01/25
DATE: 03/25

__ DATE : 12/24 __ DATE : 01/25

DES BY: J. PATT

DES CHK: A. PANDOLI

8½" UNIFORM SLAB 1'-1½" @ GIRDER BUILDUP € BEARING (SEE TABLE) Ų́ GDR.

DETAIL "A"

GIRDER BUILDUP									
	SPAN A								
	ℚ BRG.	MIDSPAN ▲							
GIRDERS 1 & 9	5"	41/4"							
GIRDERS 2, 3, 6, 7, 8 & 10	5"	41/8"							
GIRDER 4	5"	4"							
GIRDER 5	5"	4"							

▲ = MAXIMUM BUILDUP BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS.

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> **SUPERSTRUCTURE** TYPICAL SECTION **DETAILS**

COUNTY

SHEET NO. S02-10

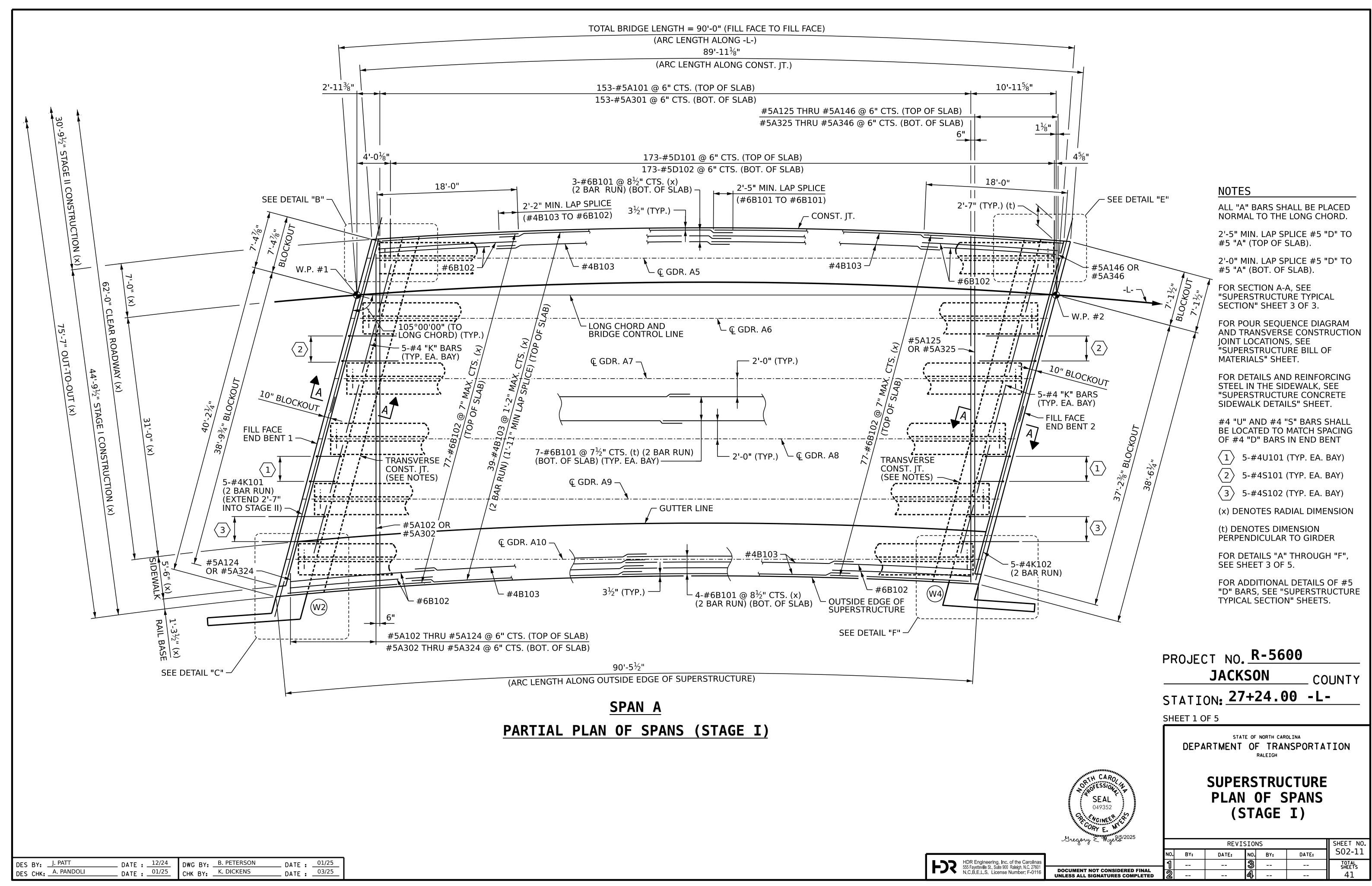
TOTAL SHEETS

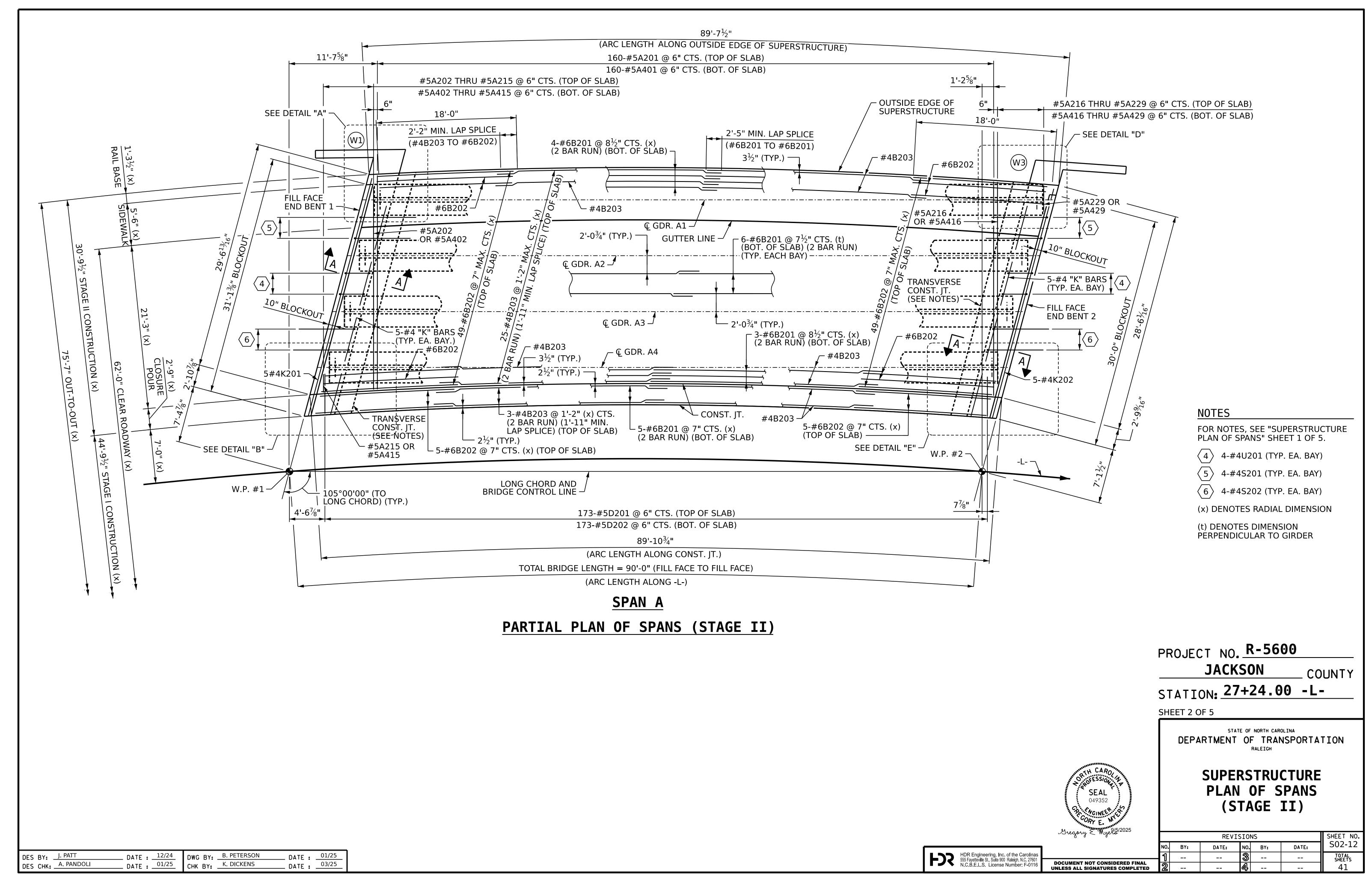
REVISIONS NO. BY: DATE: DATE:

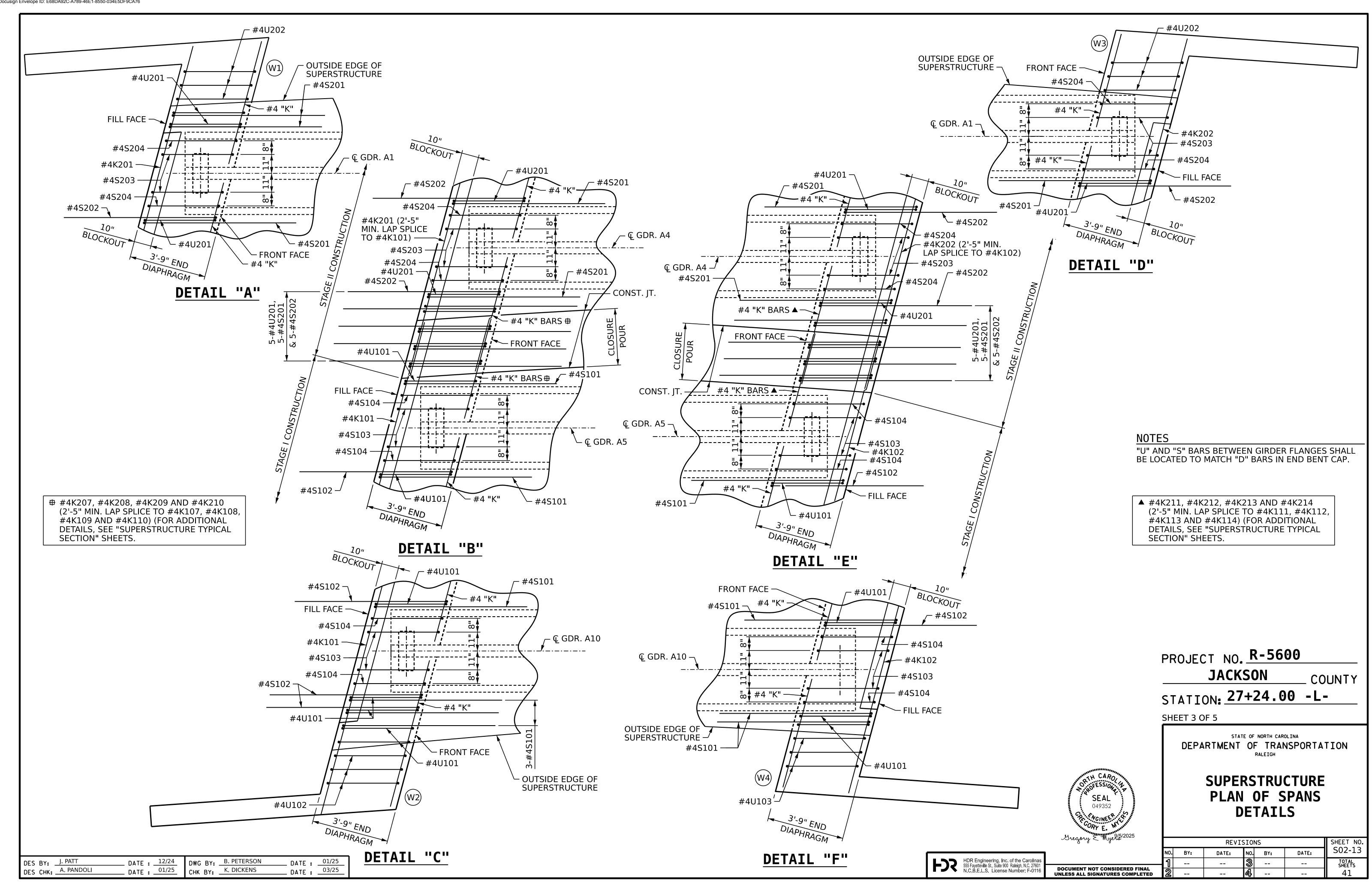


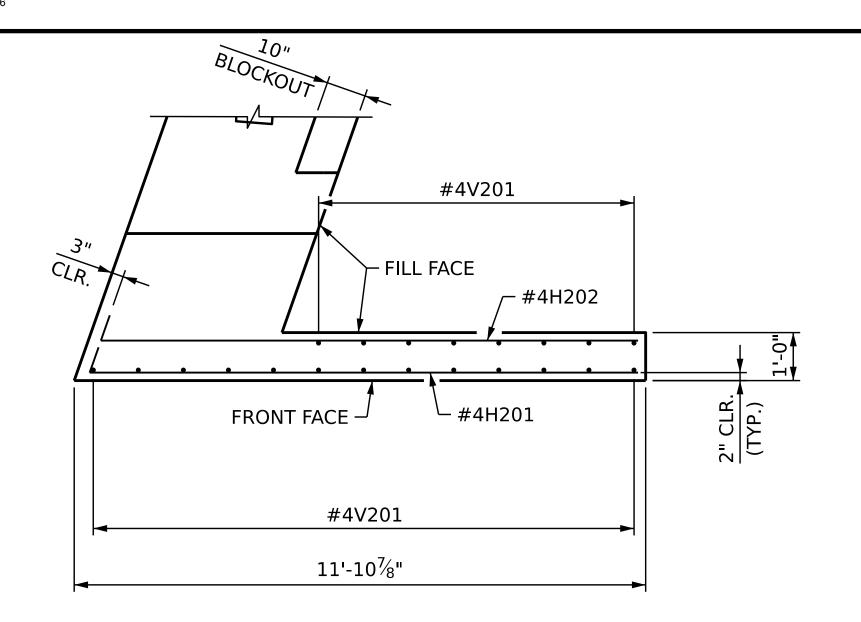
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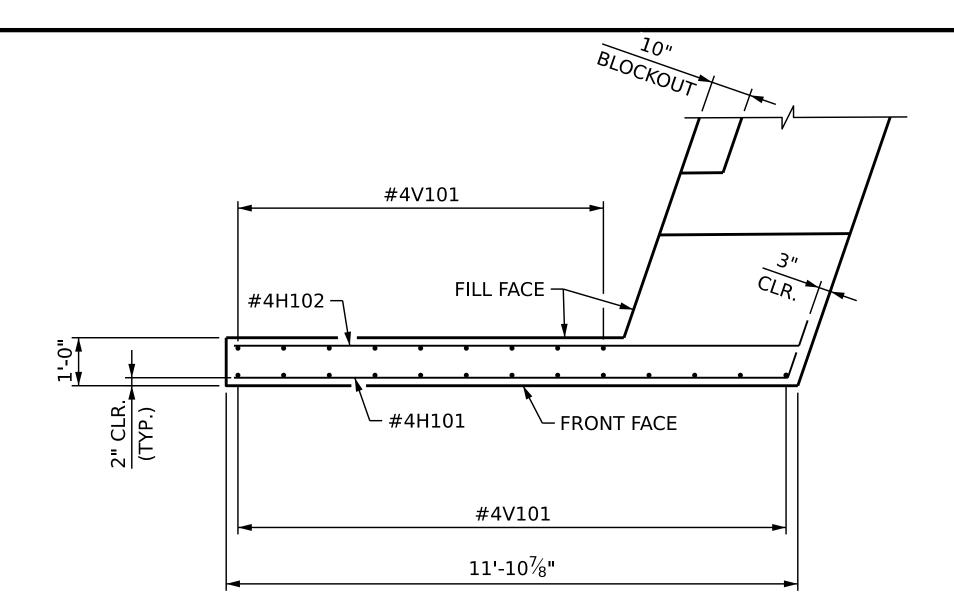




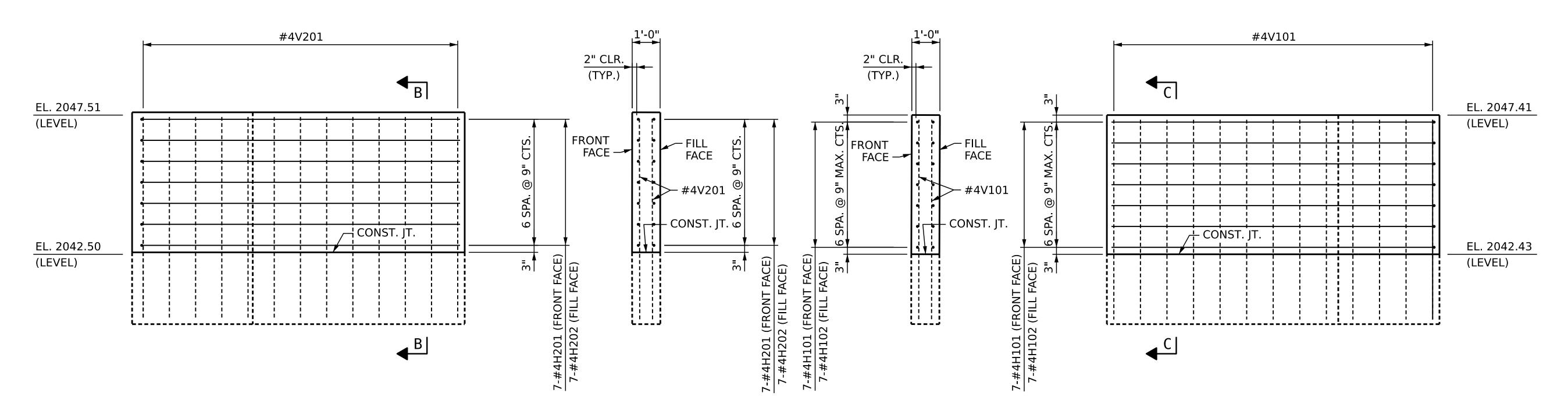




PLAN - WINGWALL "W1" "D" BARS IN END BENT NOT SHOWN FOR CLARITY



PLAN - WINGWALL "W2" "D" BARS IN END BENT NOT SHOWN FOR CLARITY



ELEVATION - WINGWALL "W1"

SECTION B-B

SECTION C-C

ELEVATION - WINGWALL "W2"

UPPER WINGWALLS AT END BENT 1

(FOR LOWER PART OF WINGWALL DETAILS AND REINFORCING STEEL, INCLUDING #4 "V" BARS, SEE "SUBSTRUCTURE END BENT 1 WINGWALLS" SHEET.)

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

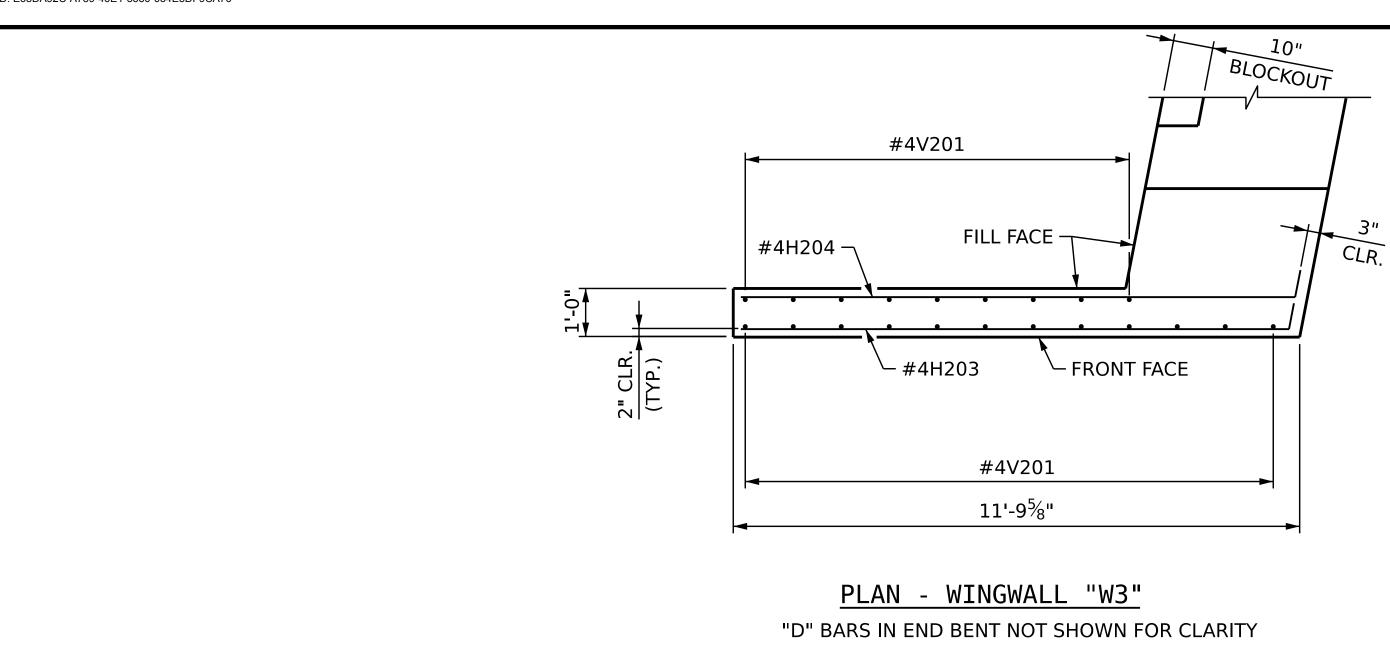
COUNTY

SUPERSTRUCTURE PLAN OF SPANS DETAILS

SHEET NO. SO2-14 REVISIONS NO. BY: DATE: DATE: HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 TOTAL SHEETS DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



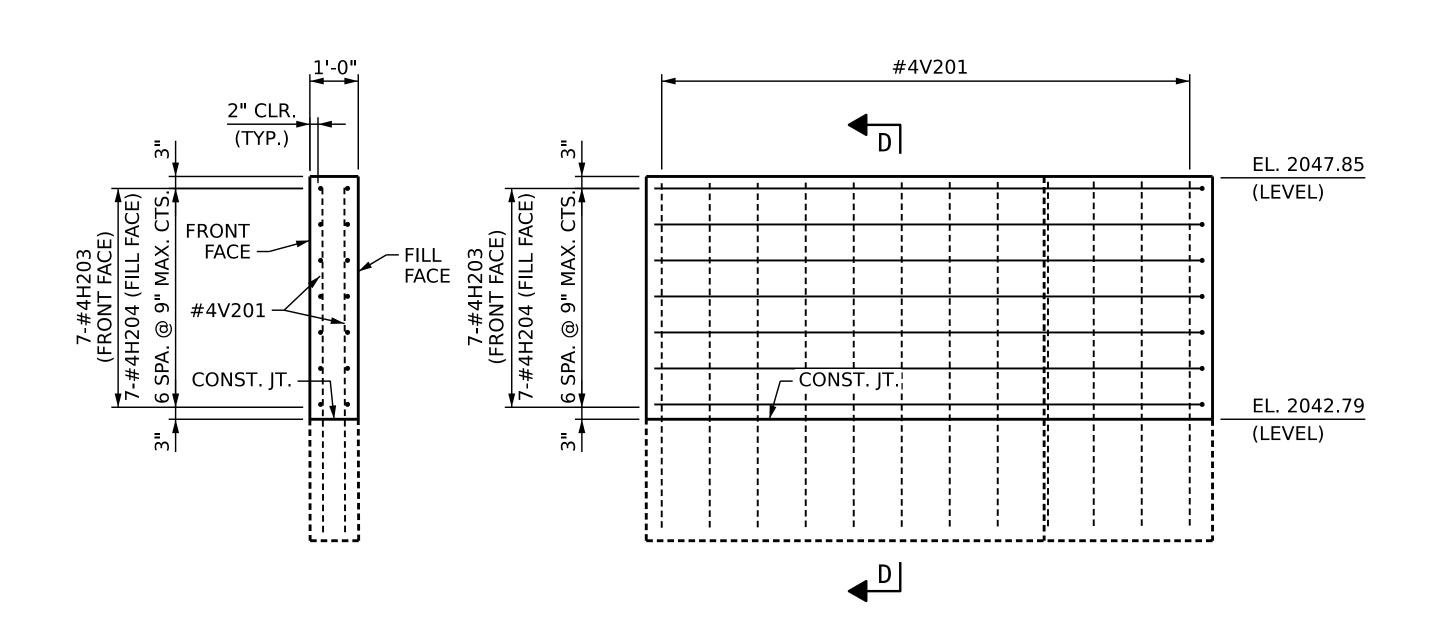
DES BY: J. PATT
DES CHK: K. DICKENS __ DATE : 04/25 __ DATE : 04/25 DWG BY: B. PETERSON
CHK BY: G. MYERS __ DATE : 03/25 __ DATE : 04/25

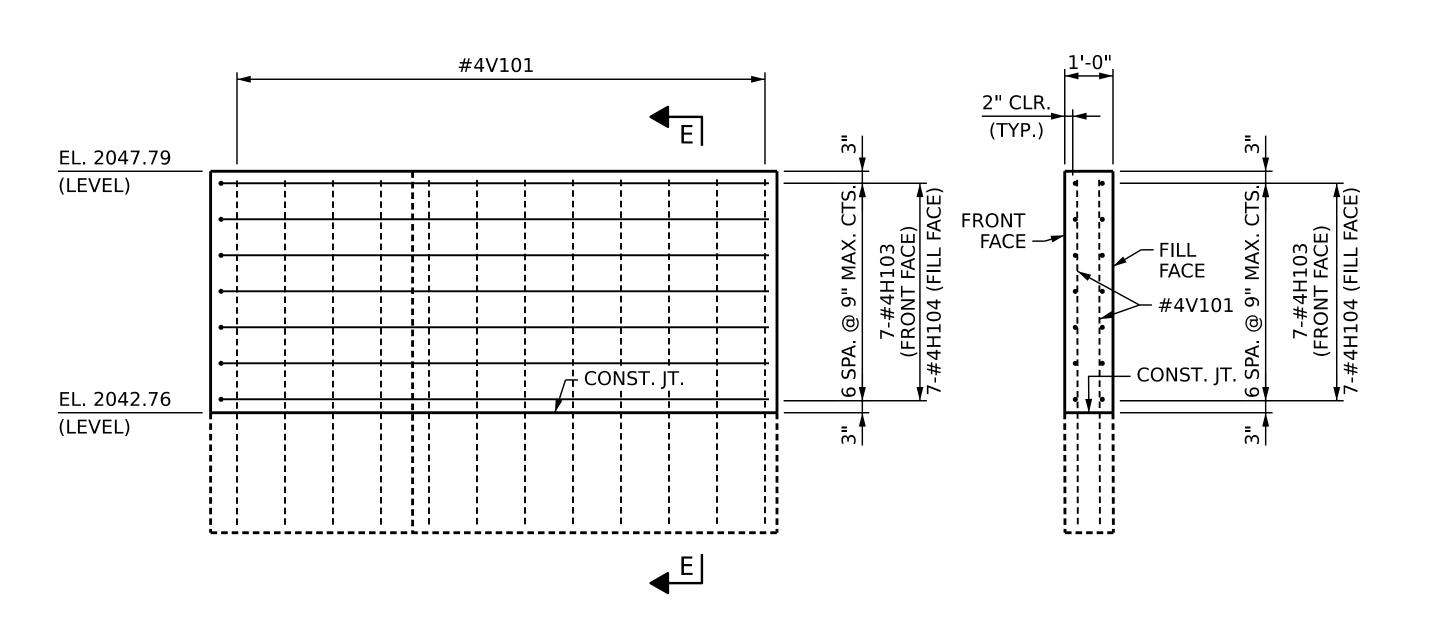


BLOCKOUT #4V101 - FILL FACE _ #4H104 \widehat{CLR} 2" CLR. (TYP.) FRONT FACE -₩4H103 #4V101 11'-9⁵⁄8"

PLAN - WINGWALL "W4"

"D" BARS IN END BENT NOT SHOWN FOR CLARITY





SECTION D-D

ELEVATION - WINGWALL "W3"

ELEVATION - WINGWALL "W4"

SECTION E-E

UPPER WINGWALLS AT END BENT 2

(FOR LOWER PART OF WINGWALL DETAILS AND REINFORCING STEEL, INCLUDING #4 "V" BARS, SEE "SUBSTRUCTURE END BENT 2 WINGWALLS" SHEET.) PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

COUNTY

SHEET 5 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE PLAN OF SPANS DETAILS

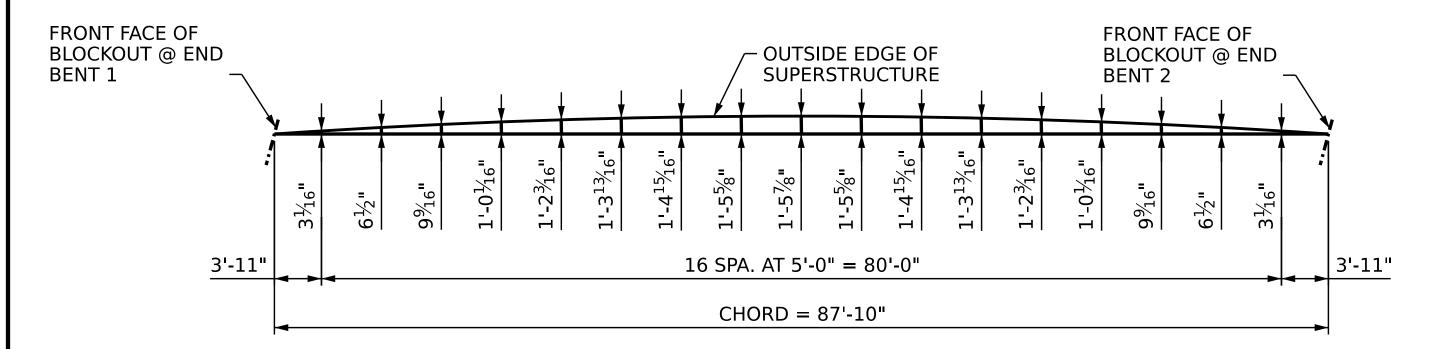
REVISIONS

SHEET NO. S02**-**15 NO. BY: DATE: DATE: TOTAL SHEETS

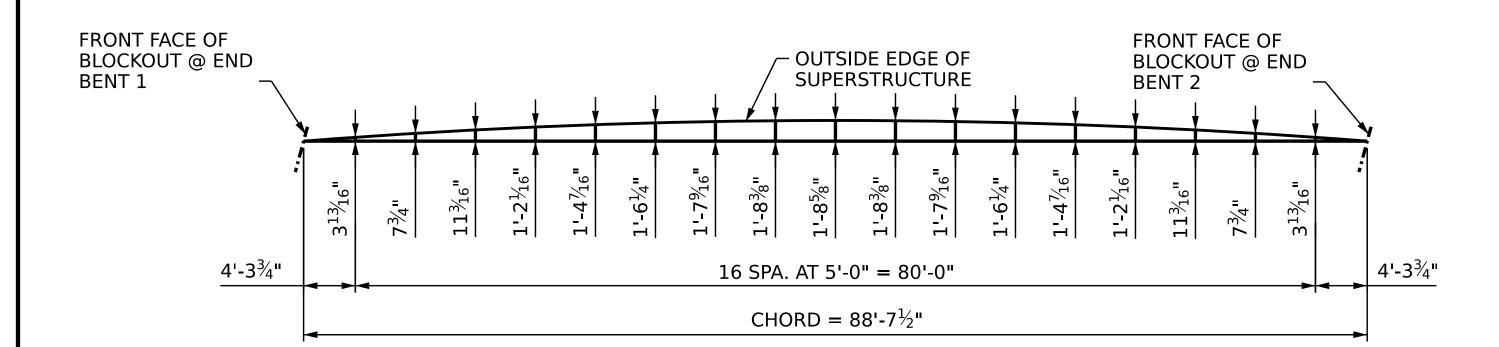
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DWG BY: B. PETERSON
CHK BY: G. MYERS DES BY: J. PATT
DES CHK: K. DICKENS __ DATE : 04/25 __ DATE : 04/25 __ DATE : 03/25 __ DATE : 04/25

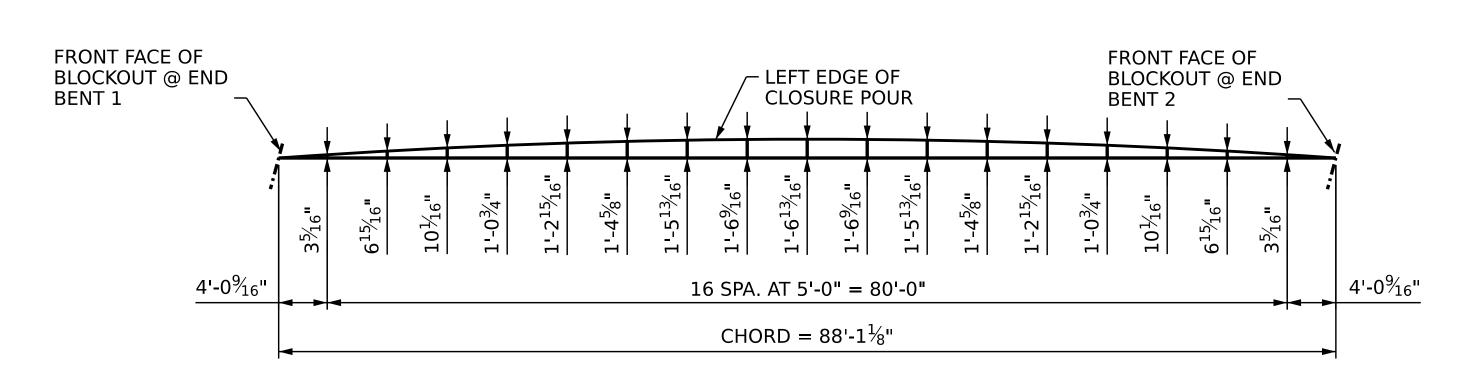


LEFT SIDE

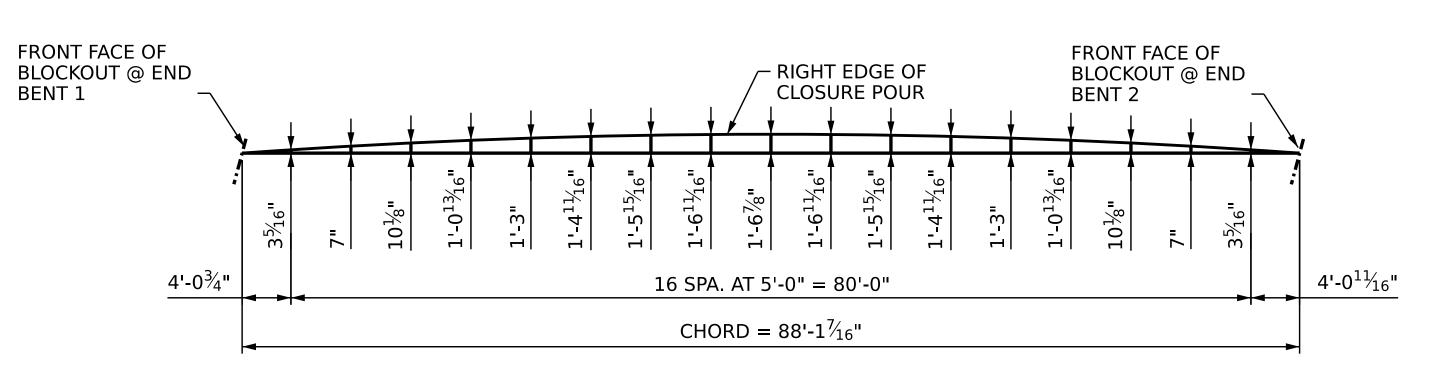


RIGHT SIDE

OUTSIDE OF DECK ARC OFFSETS



LEFT SIDE



RIGHT SIDE

OUTSIDE OF CLOSURE POUR ARC OFFSETS

PROJECT NO. R-5600 **JACKSON** COUNTY STATION: 27+24.00 -L-

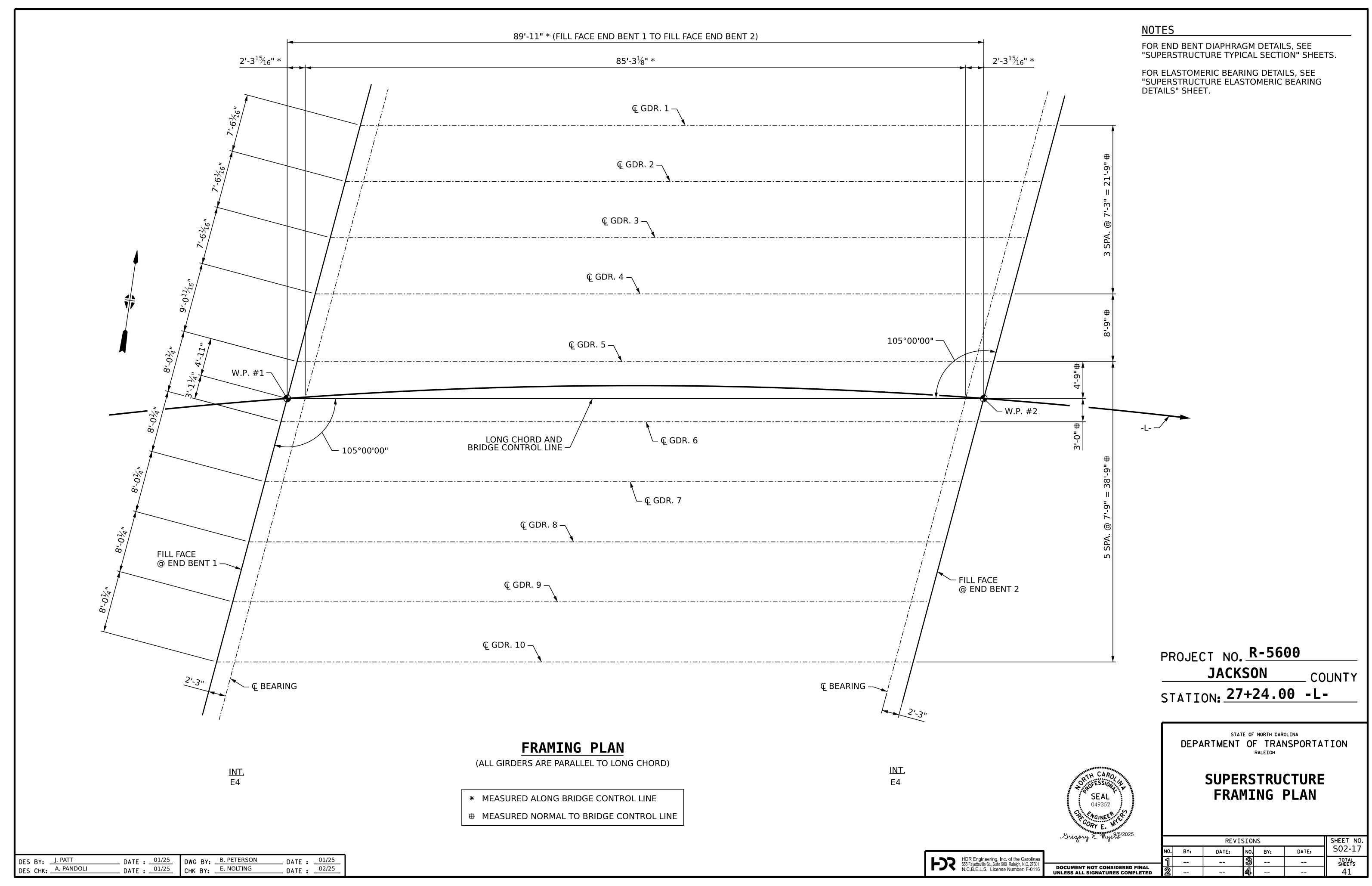
DEPARTMENT OF TRANSPORTATION

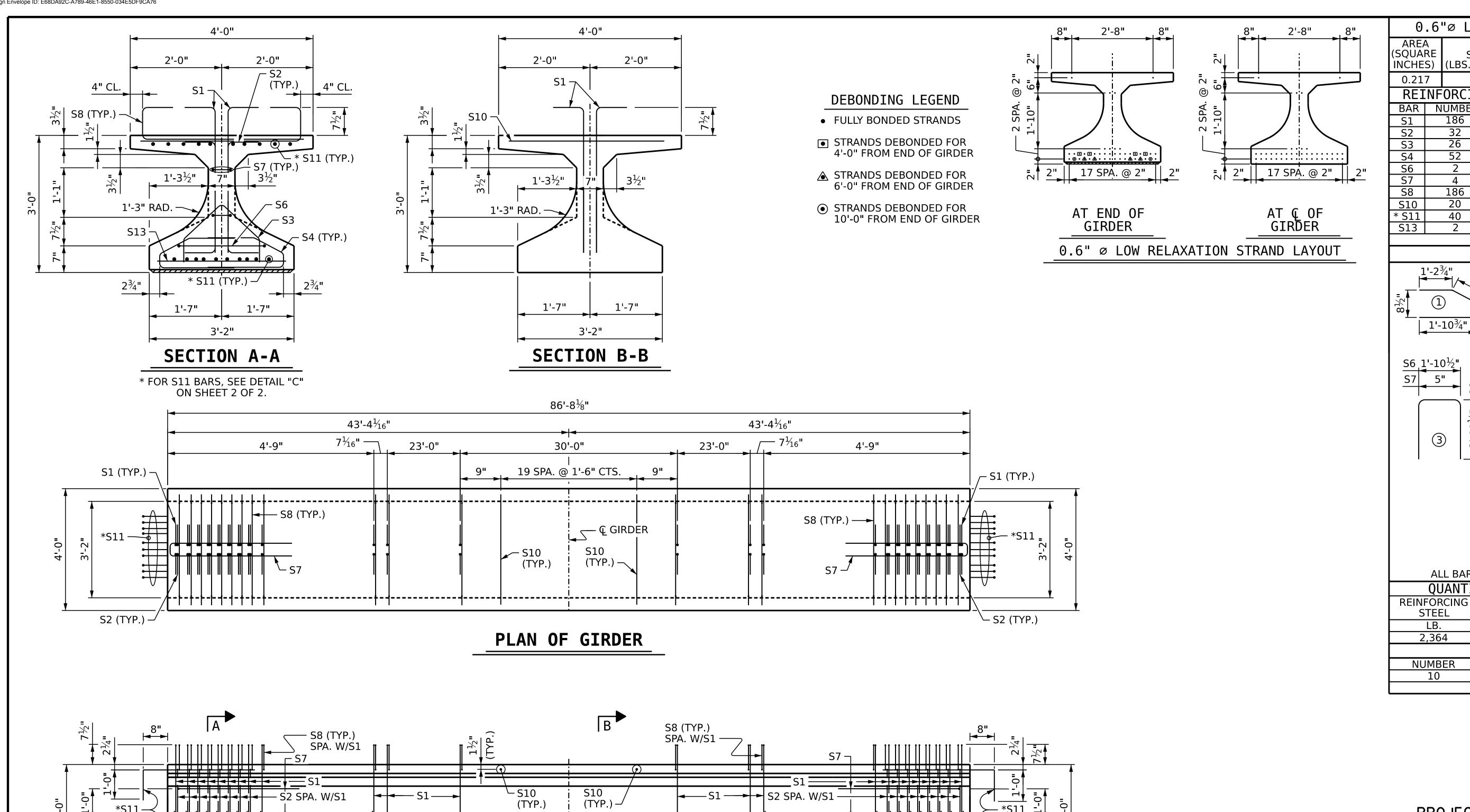
SUPERSTRUCTURE ARC OFFSETS

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S02-16
		3			TOTAL SHEETS
					<i>l</i> 11

DES BY: G. MYERS
DES CHK: J. PATT DWG BY: D. CARTER
CHK BY: G. MYERS DATE: 02/25
DATE: 04/25 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED





S3 (TYP.)

23 SPA.

@ 1-'0" CTS.

`— S4 (TYP.) /

1√7½6"

S6 –

4 SPA. @ 6" CTS.

© BEARING —

FIX.

; ← Q GIRDER

20 SPA. @ 1'-6" CTS.

ELEVATION OF GIRDER

S11 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL

* NOTE:

NOT BE ALLOWED.

└__{S13} \ S4 (TYP.) __

4 SPA. @ 6" CTS.

E Q BEARING

DWG BY: B. PETERSON
CHK BY: G. MYERS

FIX.

DATE : 01/25
DATE : 01/25

DES BY: J. PATT

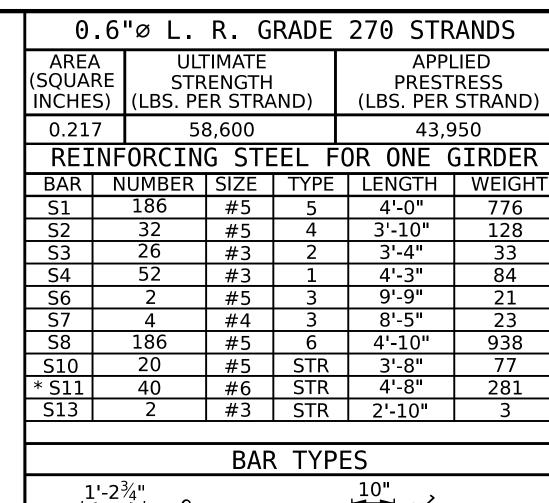
DES CHK: A. PANDOLI

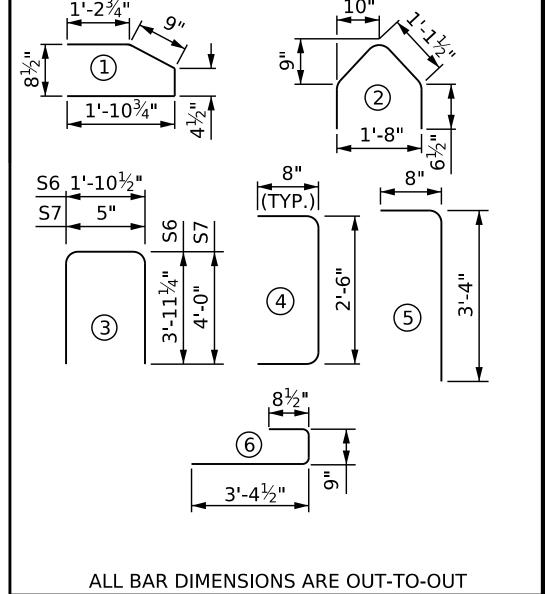
— S3 (TYP.)

23 SPA.

@ 1-'0" CTS.

DATE : 01/25
DATE : 03/25





9,000 PSI

CONCRETE

18.0

LENGTH

86'-8¹⁄8"

GIRDERS REQUIRED

L.R. STRANDS

TOTAL LENGTH

866'-9¹⁄₄"

COUNTY

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 1 OF 2

STEEL

LB.

2,364

10

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE 36" FIB PRESTRESSED CONC. GIRDER

SHEET NO. REVISIONS S02-18 NO. BY: DATE: DATE: TOTAL SHEETS



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DEAD LOAD DEFLECTION TABLE FOR CIRCLES																						
DEAD LOAD DEFLECTION TABLE FOR GIRDERS																						
GIRDERS 1 & 9 20TH POINT 0.000 0.050 0.100 0.150 0.200 0.250 0.300 0.350 0.400 0.450 0.500 0.550 0.600 0.650 0.700 0.750 0.800 0.850 0.900 0.950 1.000																						
20TH POINT		0.000	0.050	0.100	0.150	0.200	0.250	0.300	0.350	0.400	0.450	0.500	0.550	0.600	0.650	0.700	0.750	0.800	0.850	0.900	0.950	1.00
CAMBER (GIRDER ALONE IN PLACE)	<u> </u>	0.000	0.044	0.087	0.128	0.164	0.198	0.226	0.249	0.265	0.275	0.278	0.275	0.265	0.249	0.226	0.198	0.164	0.128	0.087	0.044	0.00
DEFLECTION DUE TO SDL *	<u></u>	0.000	0.030	0.059	0.087	0.112	0.135	0.154	0.169	0.180	0.187	0.189	0.187	0.180	0.169	0.154	0.135	0.112	0.087	0.059	0.030	0.00
FINAL CAMBER	↑	0''	3/16"	⁵ ⁄16''	1/211	5/811	3/411	7/811	15⁄ ₁₆ "	1''	11/16"	11/16 "	11/16 "	1''	15/ ₁₆ "	7/811	3/411	5/811	1/2"	⁵ ⁄16''	3/16"	0'
	DEAD LOAD DEFLECTION TABLE FOR GIRDERS																					
GIRDERS 2, 3, 6, 7, 8 & 10 20TH POINT 0.000 0.050 0.100 0.150 0.200 0.250 0.300 0.350 0.400 0.450 0.500 0.550 0.600 0.650 0.700 0.750 0.800 0.850 0.900 0.950 1.000																						
20TH POINT		0.000	0.050	0.100	0.150	0.200	0.250	0.300	0.350	0.400	0.450	0.500	0.550	0.600	0.650	0.700	0.750	0.800	0.850	0.900	0.950	1.00
CAMBER (GIRDER ALONE IN PLACE)	†	0.000	0.044	0.087	0.128	0.164	0.198	0.226	0.249	0.265	0.275	0.278	0.275	0.265	0.249	0.226	0.198	0.164	0.128	0.087	0.044	0.00
DEFLECTION DUE TO SDL *	\	0.000	0.028	0.055	0.080	0.103	0.125	0.142	0.156	0.166	0.173	0.175	0.173	0.166	0.156	0.142	0.125	0.103	0.080	0.055	0.028	0.00
FINAL CAMBER	<u> </u>	0''	3/16"	3/811	9/16"	3/411	7/811	1''	1½''	13/16"	11/4"	11/4"	11/4"	13/16"	11/8"	1''	7/811	3/411	9/16"	3/811	3/16"	0''
				DEA	AD LC	AD D	EFLE(CTION	I TABI	E FO	R GIR	DERS	5						•	•		•
							GI	RDER 4	ļ.													
20TH POINT		0.000	0.050	0.100	0.150	0.200	0.250	0.300	0.350	0.400	0.450	0.500	0.550	0.600	0.650	0.700	0.750	0.800	0.850	0.900	0.950	1.00
CAMBER (GIRDER ALONE IN PLACE)	†	0.000	0.044	0.087	0.128	0.164	0.198	0.226	0.249	0.265	0.275	0.278	0.275	0.265	0.249	0.226	0.198	0.164	0.128	0.087	0.044	0.00
DEFLECTION DUE TO SDL *	↓	0.000	0.022	0.043	0.064	0.082	0.099	0.112	0.124	0.132	0.137	0.138	0.137	0.132	0.124	0.112	0.099	0.082	0.064	0.043	0.022	0.00
FINAL CAMBER	<u></u>	0''	1/4"	1/211	3/411	1''	13/16"	13/8"	1½''	1 ⁵ ⁄8''	1 ¹¹ ⁄ ₁₆ ''	111/16"	111/16"	1 ⁵ ⁄8''	1½''	13/8"	13/16"	1''	3/4"	1/211	1/4"	0''
				DEA	AD LC	•			TABI								•	•	•	•		•
							GI	RDER 5	-													
20TH POINT		0.000	0.050	0.100	0.150	0.200	0.250	0.300	0.350	0.400	0.450	0.500	0.550	0.600	0.650	0.700	0.750	0.800	0.850	0.900	0.950	1.00
CAMBER (GIRDER ALONE IN PLACE)	<u></u>	0.000	0.044	0.087	0.128	0.164	0.198	0.226	0.249	0.265	0.275	0.278	0.275	0.265	0.249	0.226	0.198	0.164	0.128	0.087	0.044	0.00
DEFLECTION DUE TO SDL *	↓	0.000	0.025	0.049	0.071	0.092	0.111	0.126	0.139	0.148	0.154	0.155	0.154	0.148	0.139	0.126	0.111	0.092	0.071	0.049	0.025	0.0
FINAL CAMBER	<u></u>	0''	1/4"	7/16"	11/16"	7/811	1½16''	1 ³ / ₁₆ "	1 ⁵ ⁄16''	13/8"	17/16"	1½''	17/16"	13/8"	1 ⁵ ⁄ ₁₆ ''	1 ³ / ₁₆ "	11/16"	7/811	11/16"	7/16"	1/4"	0'

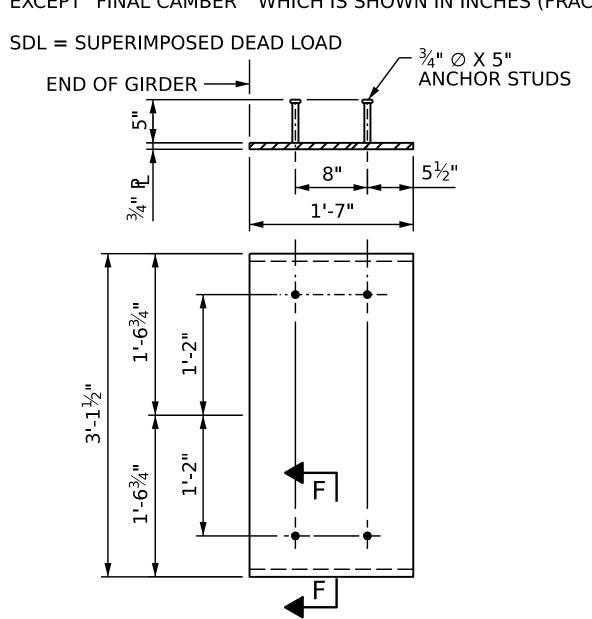
= UPWARD CAMBER

DES BY: J. PATT

= DOWNWARD DEFLECTION

* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.

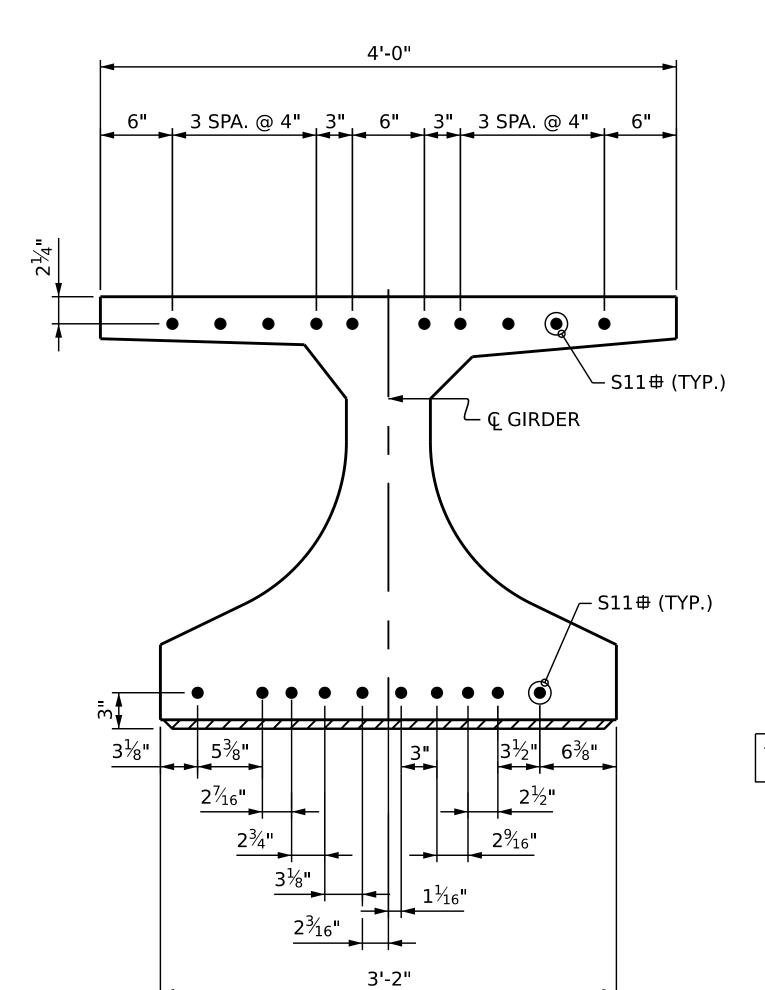
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER" WHICH IS SHOWN IN INCHES (FRACTION FORM).



EMBEDDED PLATE "B-1" DETAILS FOR FLORIDA I BEAM

> (2 REQ'D PER GIRDER) ³/₄" BEVEL EDGE SECTION "F"

(SEE NOTES) DWG BY: B. PETERSON
CHK BY: G. MYERS DATE : 01/25
DATE : 03/25 DES CHK: A. PANDOLI



DETAIL "C"

(36" FLORIDA I BEAM)

S11 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUB SECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7,200 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4500 lbs.

> PROJECT NO. R-5600 **JACKSON** COUNTY

STATION: 27+24.00 -L-

SHEET 2 OF 2

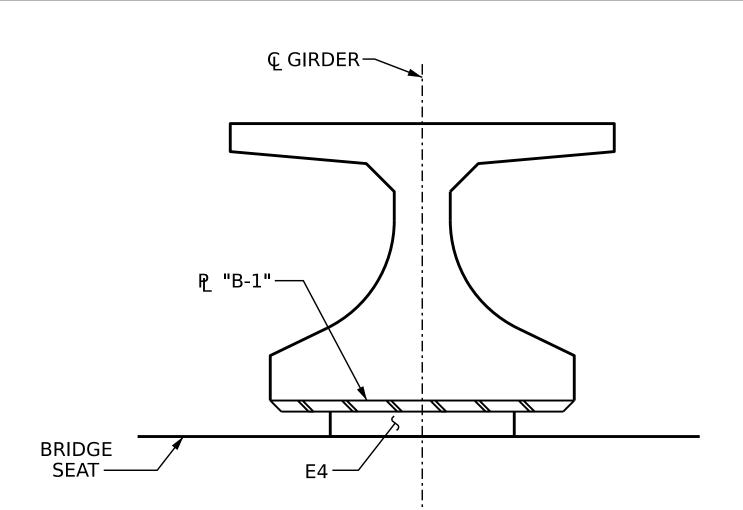
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE FIB PRESTRESSED CONC. GIRDER DETAILS

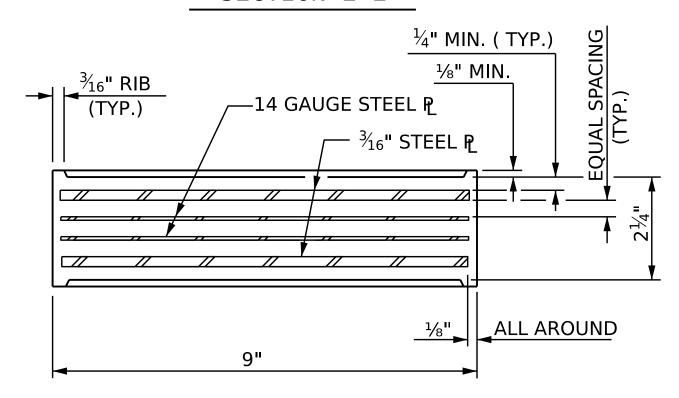


· OIV			PLIX	DLIF	AILS
	REVI	SION	S		SHEET NO
Y:	DATE:	NO.	BY:	DATE:	S02-19
_		2			TOTAL

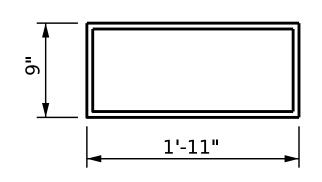
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SECTION E-E



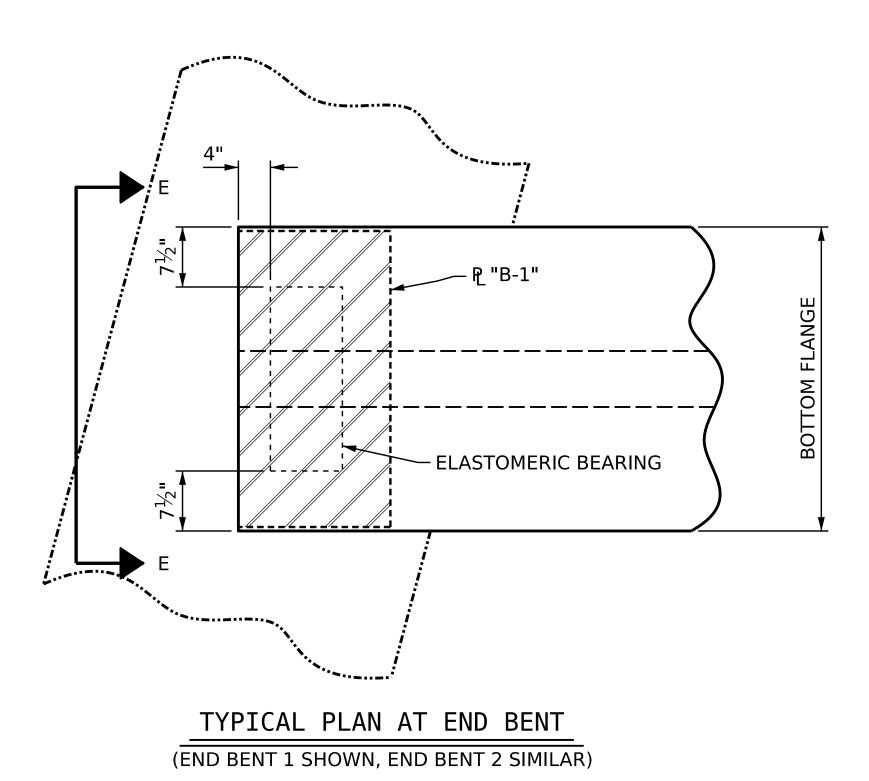
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E4 (20 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE V



NOTES

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE STANDARD SPECIFICATIONS.

MAXIMUM ALLOWABLE SERVICE LOADS D.L.+L.L. (NO IMPACT) TYPE V 365 k

PROJECT NO. R-5600 **JACKSON**

_ COUNTY STATION: 27+24.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
> RALEIGH



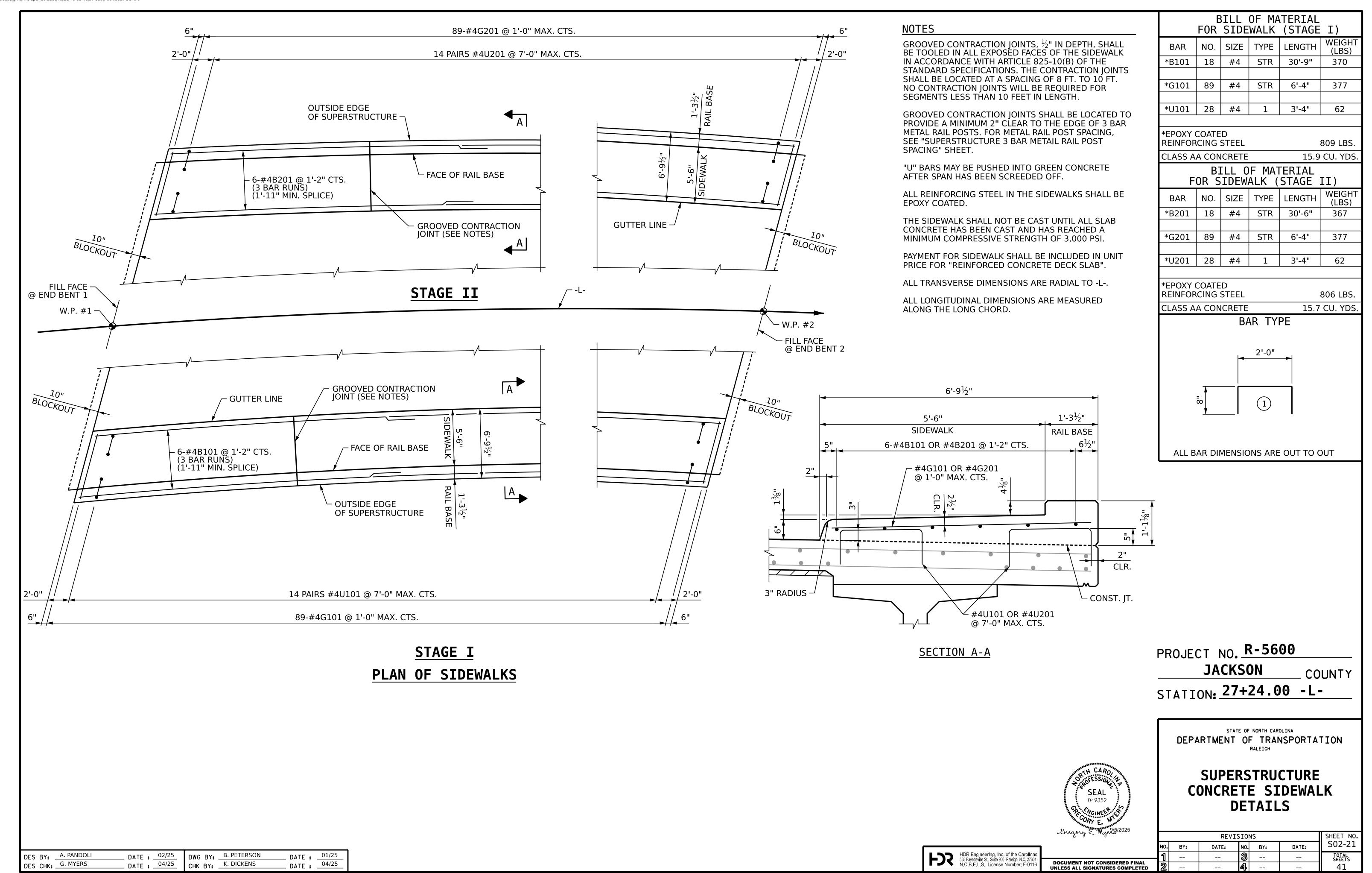
SUPERSTRUCTURE ELASTOMERIC BEARING DETAILS

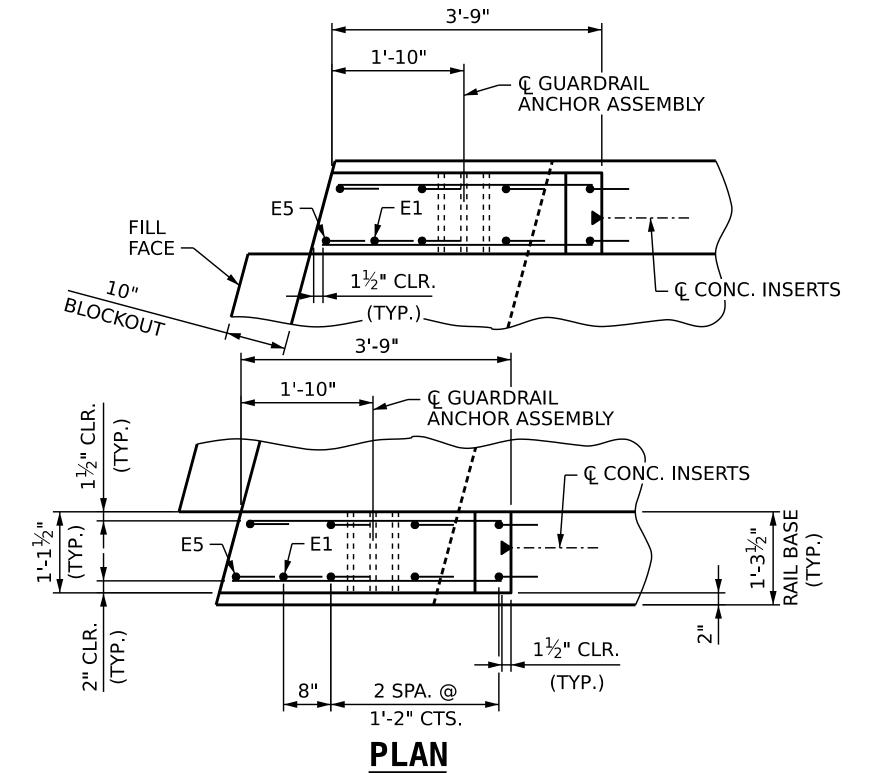
Myet/21/2029			REV:	ISION	S		SHEET NO
	NO.	BY:	DATE:	NO.	BY:	DATE:	S02-20
	-11			3			TOTAL SHEETS
CONSIDERED FINAL ATURES COMPLETED	2	-		4			41

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DOCUMENT NOT CO UNLESS ALL SIGNAT

DWG BY: B. PETERSON
CHK BY: E. NOLTING DES BY: J. PATT
DES CHK: A. PANDOLI __ DATE : 01/25 __ DATE : 01/25 __ DATE : 01/25 __ DATE : 02/25





(SHOWN AT END BENT 1, END BENT 2 SIMILAR BY ROTATION)

1'-3½"

RAIL BASE

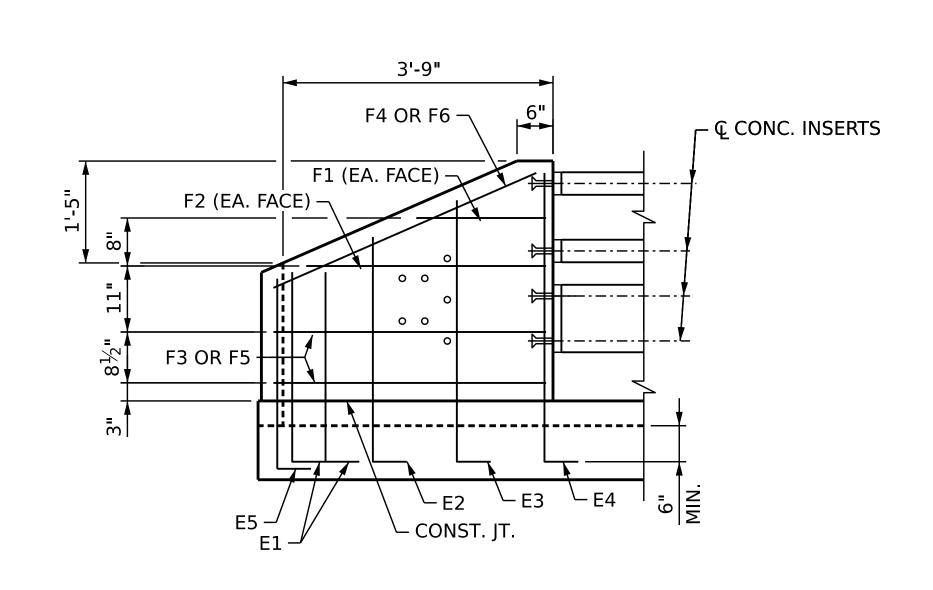
END VIEW

€ GUARDRAIL

ANCHOR ASSEMBLY

1'-1½"

– Ç CONC. INSERTS



ELEVATION

		FOR	ONE C	ONCRET	TE END PO	ST
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT (LBS)
	* E1	2	#7	1	3'-5"	14
	* E2	2	#7	1	4'-0"	17
	* E3	2	#7	1	4'-7"	19
	* E4	2	#7	1	5'-0"	21
	* E5	1	#7	1	3'-6"	8
	*F1	2	#6	STR	1'-8"	6
	* F2	2	#6	STR	3'-2"	10
	* F3	2	#6	STR	3'-5''	11
	* F4	1	#6	STR	3'-9"	6
	* F5	2	#6	STR	3'-7"	11
	* F6	1	#6	STR	3'-11"	6
			DATED NG STE	EL		129 LBS.
	CLAS	S AA (CONCRI	ETE	0.4	CU. YDS.
	T	.	BA	AR TY	PE	I
	2'-5" E1	3'-0" E2	3'-7" E3	4'-0" E4 2'-6" E5		1
	٨١	I DAD		CIONC	ADE OUT T	1'-0"
	AL	L RAK	DIMEN	SIUNS A	ARE OUT T	0 001
) T F	- C					

BILL OF MATERIAL

NOTES

PAYMENT FOR END POSTS SHALL BE INCLUDED IN UNIT PRICE FOR "REINFORCED CONCRETE DECK SLAB".

PROJECT NO. R-5600 **JACKSON** _ COUNTY

STATION: 27+24.00 -L-

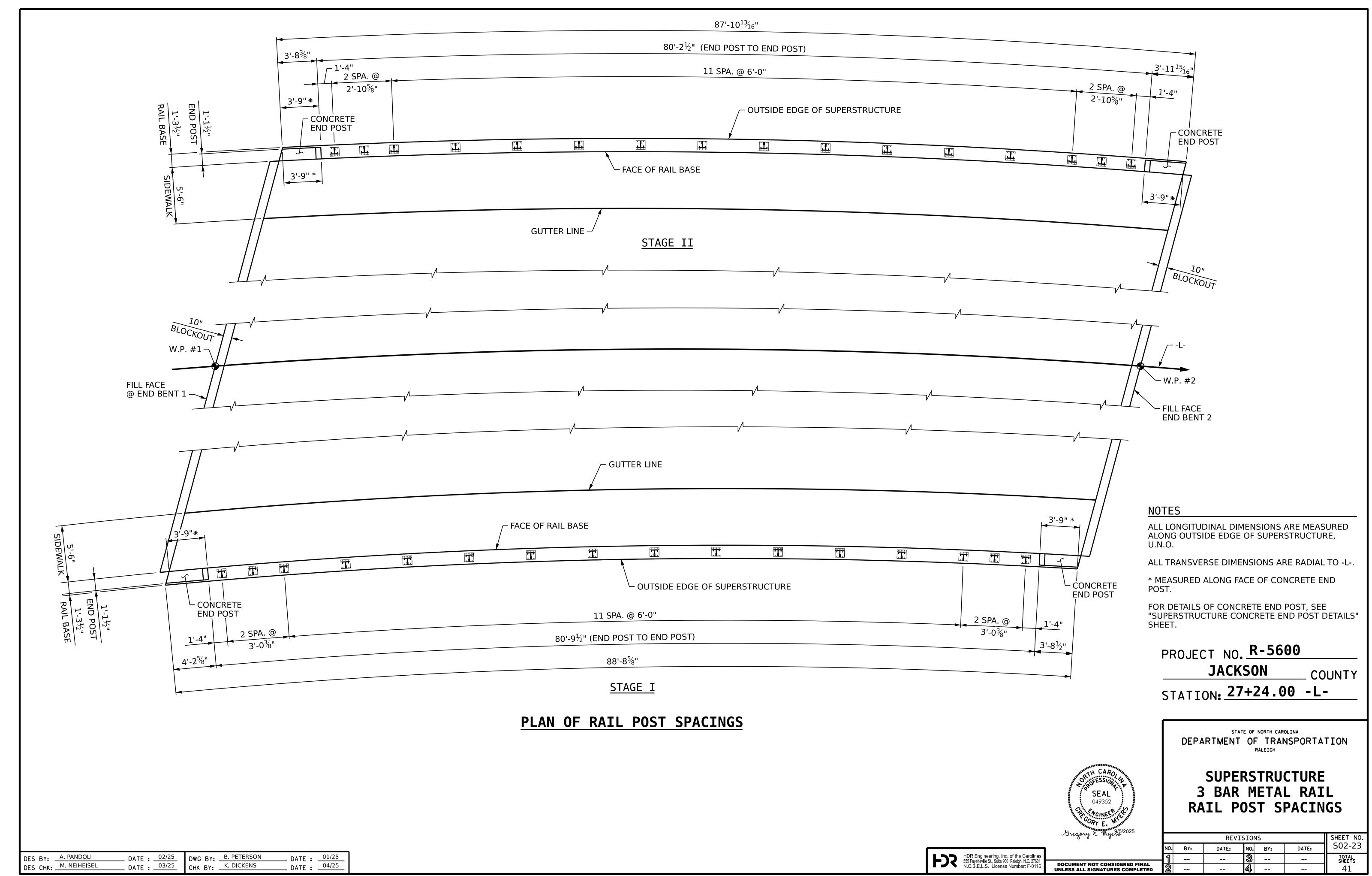
DEPARTMENT OF TRANSPORTATION
RALEIGH

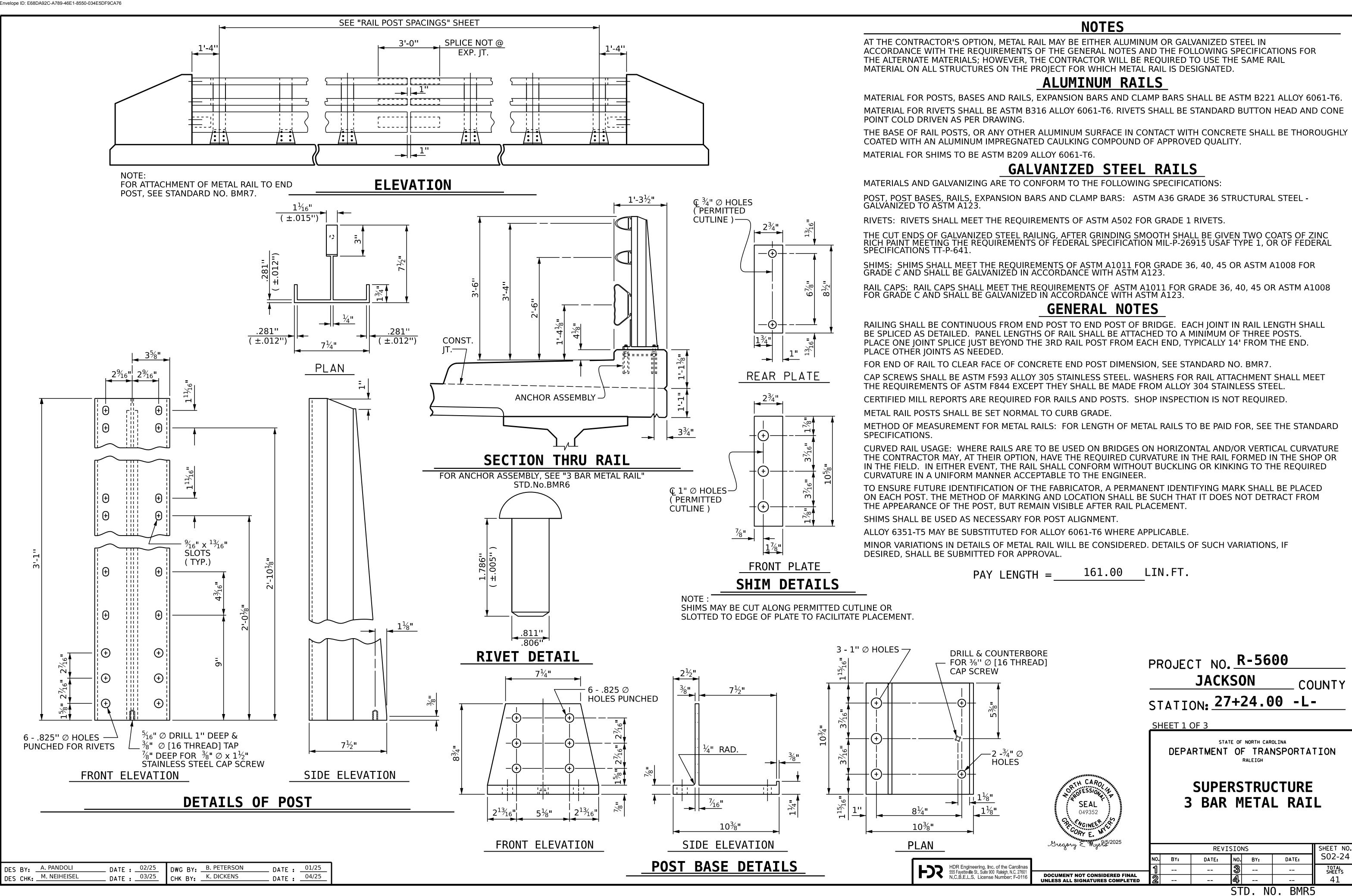
SUPERSTRUCTURE CONCRETE END POST DETAILS

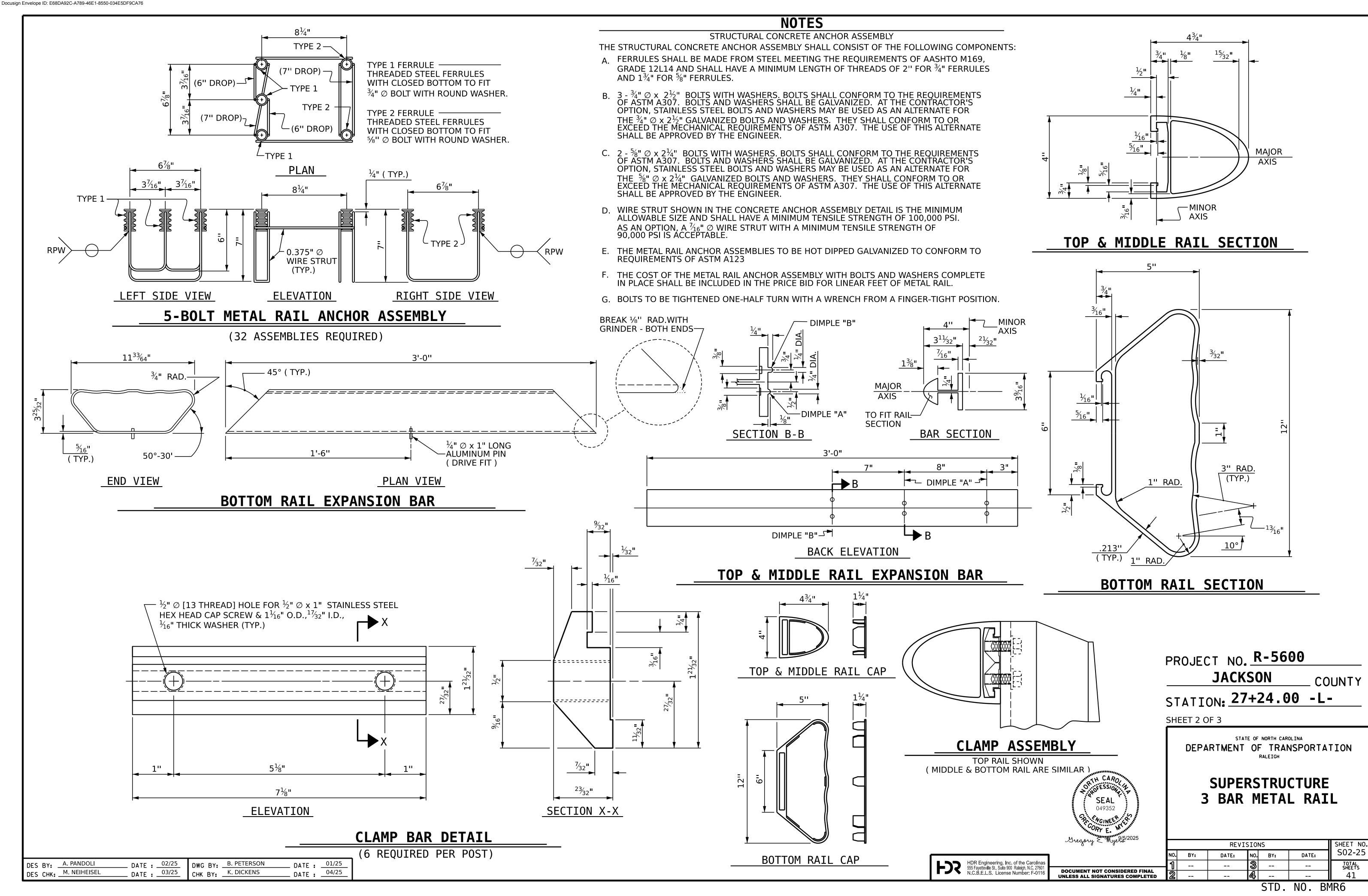
SHEET NO. S02-22 REVISIONS NO. BY: DATE: DATE:

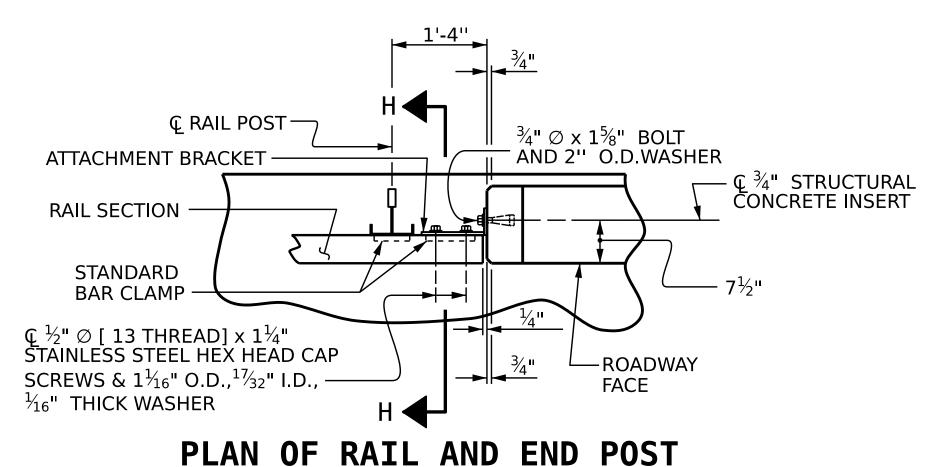
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DES BY:	A. PANDOLI	DATE : 02/25	DWG BY: B. PETERSON	DATE :	01/25
DES CHK:	G. MYERS	0.4/0.5	CHK BY: K. DICKENS	DATE :	04/25

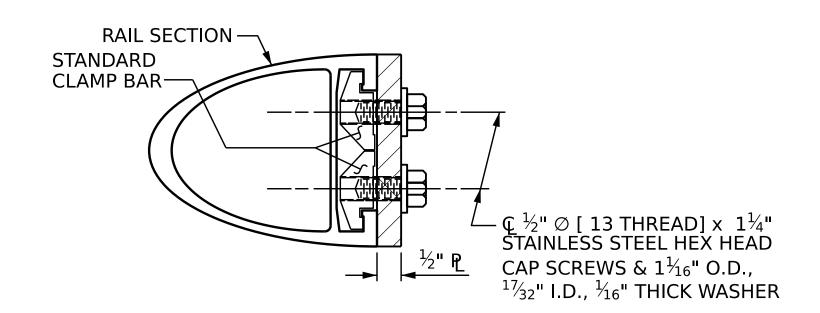




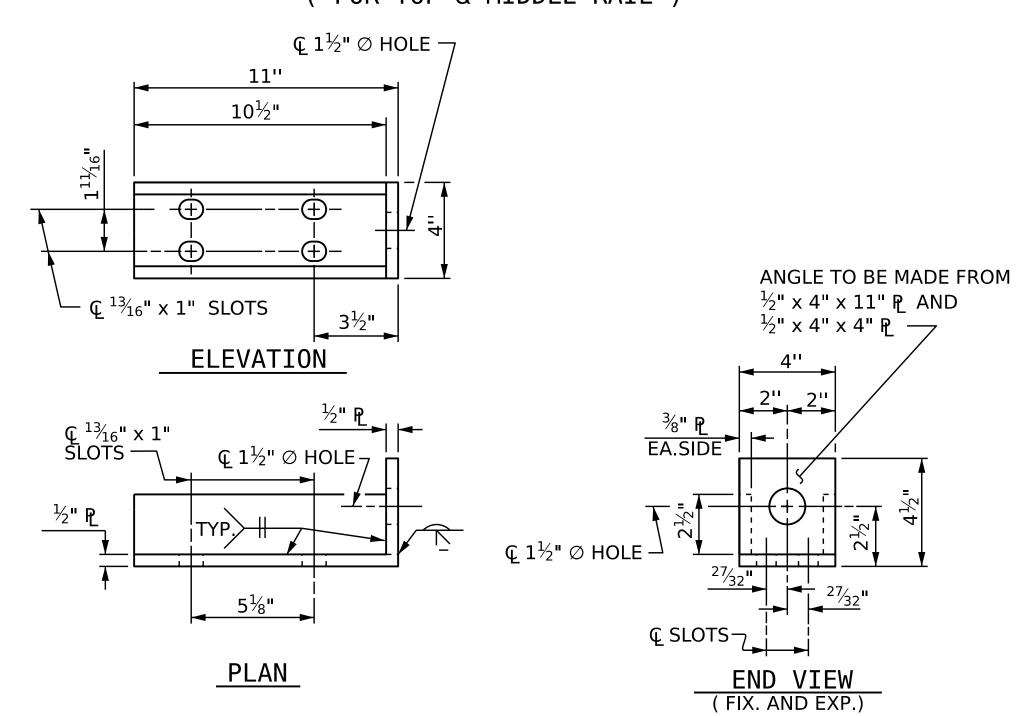




(STIFFENER ON $\frac{1}{2}$ " P NOT SHOWN FOR CLARITY)



SECTION H-H FOR TOP & MIDDLE RAIL



DETAILS FOR ATTACHMENT BRACKET TOP & MIDDLE RAIL ONLY)

DWG BY: B. PETERSON DES BY: A. PANDOLI __ DATE : 02/25 __ DATE : 03/25 DATE : 01/25
DATE : 04/25 DES CHK: M. NEIHEISEL CHK BY: K. DICKENS

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO ASTM A36 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A $\frac{3}{4}$ " \emptyset x $1\frac{5}{8}$ " BOLT WITH 2" O.D. WASHER IN PLACE. THE $\frac{3}{4}$ " \emptyset x $1\frac{5}{8}$ " BOLT SHALL HAVE N.C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F. WASHERS FOR RAIL ATTACHMENT SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.
- D. STANDARD CLAMP BARS (STD. No. BMR6)

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 3 BAR METAL RAIL

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

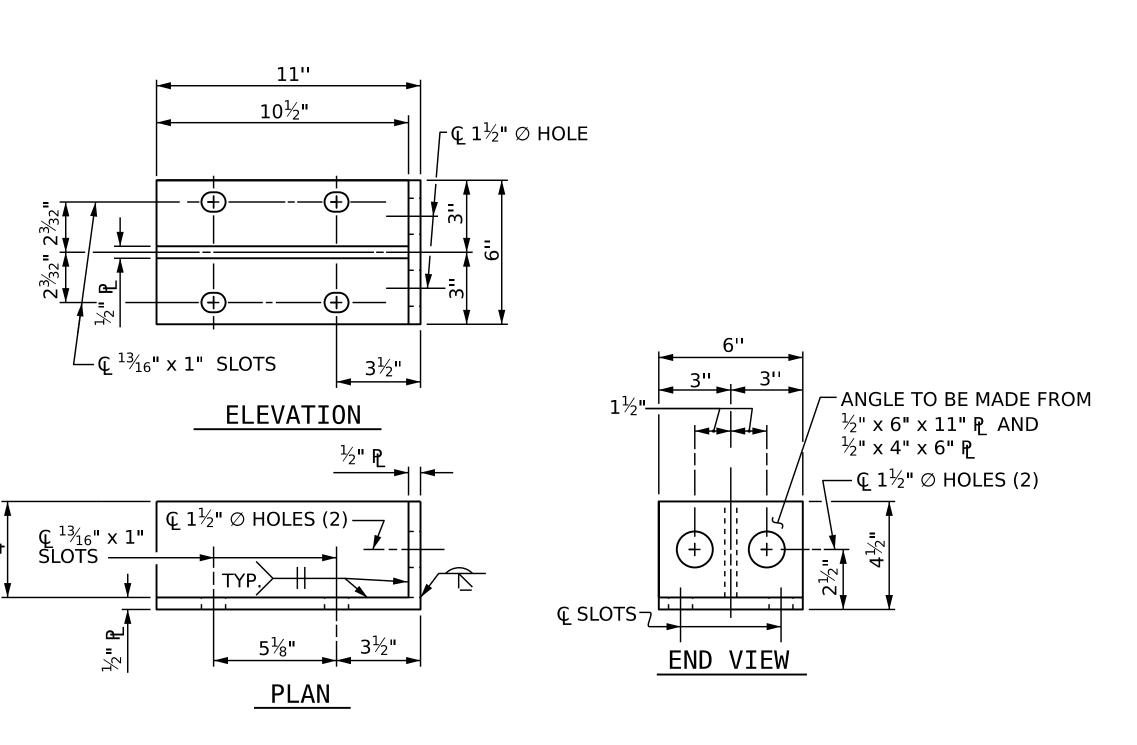
THE CONTRACTOR, AT THEIR OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED. THE 34 " \varnothing x 1^58 " BOLT WITH WASHER SHALL BE REPLACED WITH A $^3\!4$ " \varnothing x $6^1\!\!2$ " BOLT AND 2" O.D.WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $^3\!4$ " \varnothing x 1 $^5\!8$ " BOLT SHALL APPLY TO THE $^3\!4$ " \varnothing x 6 $^1\!2$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{1}{2}$ "
- 1 $^3\!\!4$ " Ø x $1^5\!\!8$ " BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. AT THE CONTRACTORS OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $^3\!4$ " \oslash x $1^5\!8$ " GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER
- WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_{16} " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.



DETAILS FOR ATTACHMENT BRACKET

 $\mathbb{Q}^{\frac{1}{2}}$ " \emptyset [13 THREAD] x $1\frac{1}{4}$ " STAINLESS STEEL HEX HEAD

 17 ₃₂" I.D., 1 ₁₆" THICK WASHER -

SECTION H-H

FOR BOTTOM RAIL

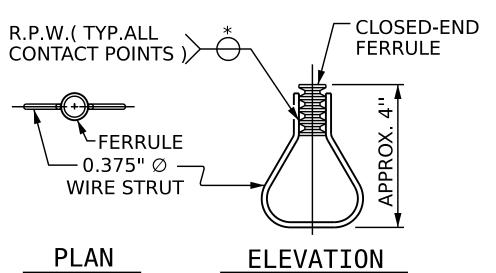
CAP SCREWS & $1\frac{1}{16}$ " O.D.,

RAIL SECTION

STANDARD

CLAMP BAR -

BOTTOM RAIL ONLY



STRUCTURAL CONCRETE

INSERT * EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE

STRENGTH OF THE WIRE. PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE 3 BAR METAL RAIL

REVISIONS

DATE:



	NO.	BY:
	1	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	2	

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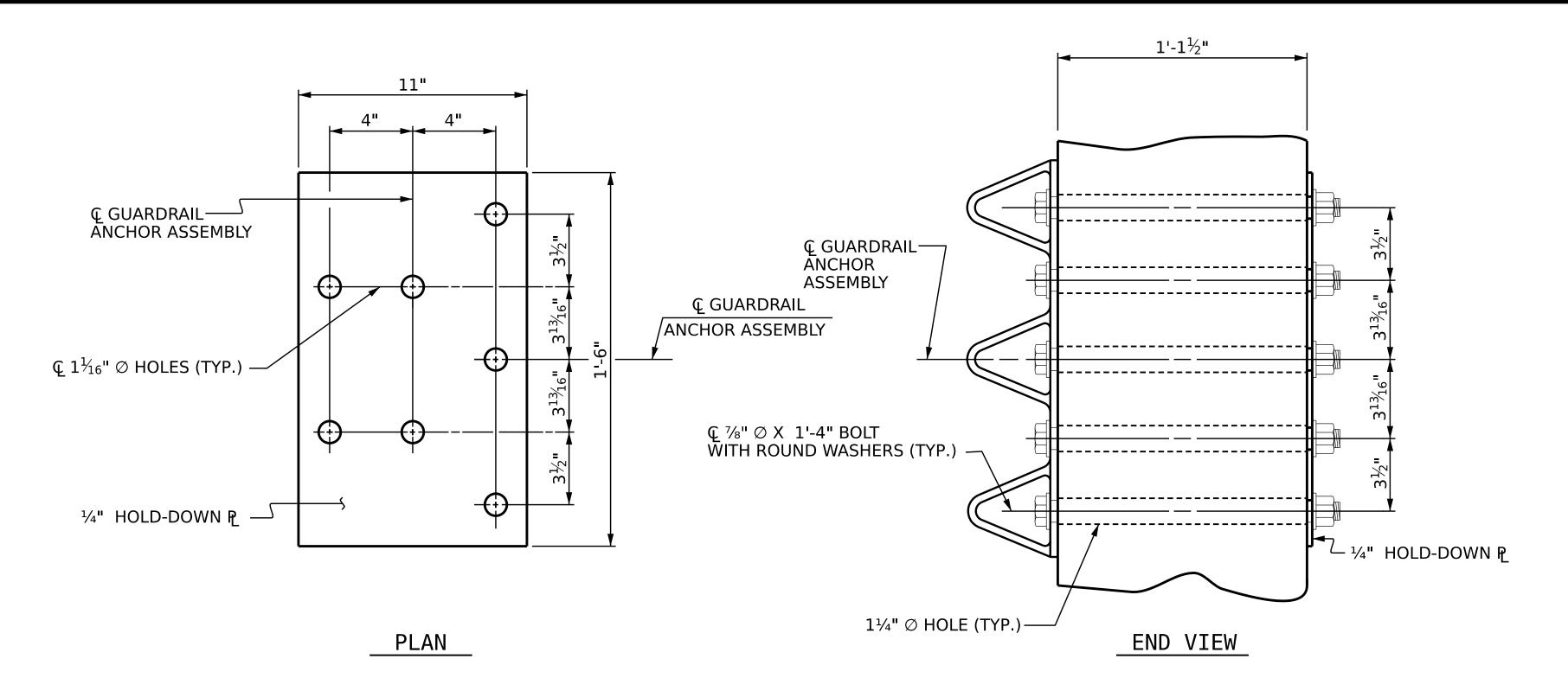
STD. NO. BMR7

DATE:

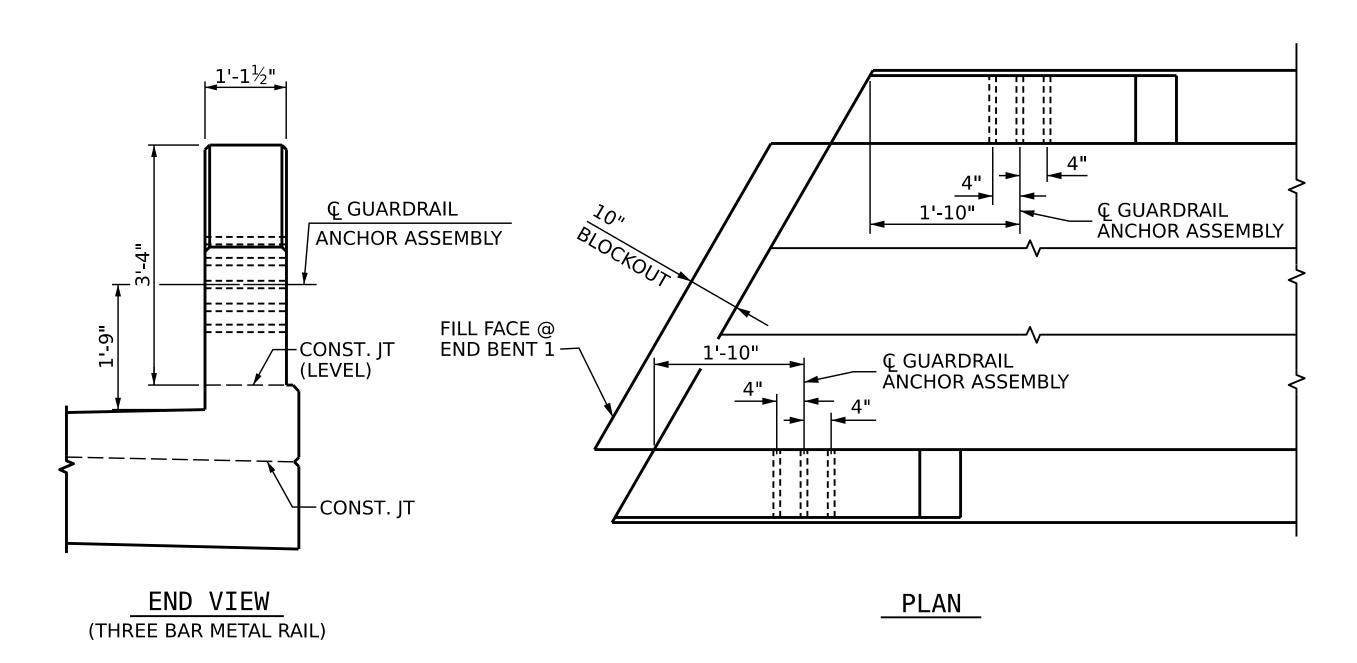
COUNTY

SHEET NO.

S02-26



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

END BENT 1 SHOWN, END BENT 2 SIMILAR

DES BY: A. PANDOLI DATE : 01/25 DWG BY: B. PETERSON DATE : 01/25 DES CHK: E. NOLTING DATE : 02/25 CHK BY: E. NOLTING DATE : 02/25

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " \oslash BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE ½ GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

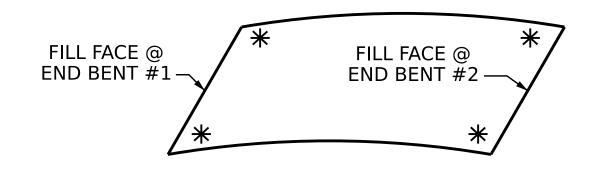
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " \oslash HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. R-5600

JACKSON COUNTY

STATION: 27+24.00 -L-

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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

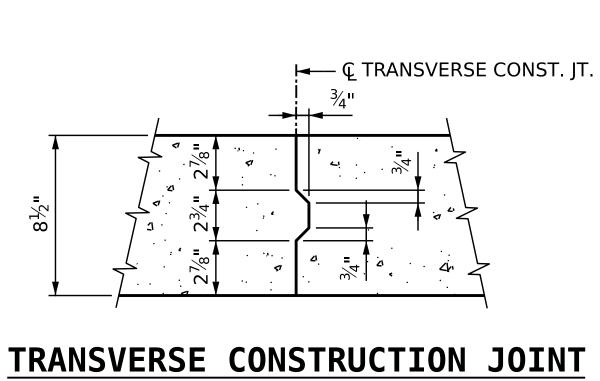
SUPERSTRUCTURE GUARDRAIL ANCHORAGE DETAILS

resory E Myers/2025							
regory < Myers 2020				SHEET NO.			
	NO.	BY:	DATE:	NO.	BY:	DATE:	S02-27
	-1			3			TOTAL SHEETS
UMENT NOT CONSIDERED FINAL	മ						41

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NOTES

LONGITUDINAL DIMENSIONS ARE MEASURED ALONG THE LONG CHORD AND TRANSVERSE DIMENSIONS RADIAL TO -L-.



(REINFORCING STEEL IN SLAB NOT SHOWN)
(LONGITITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT)

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PROJECT NO. R-5600

JACKSON COUNTY

STATION: 27+24.00 -L-

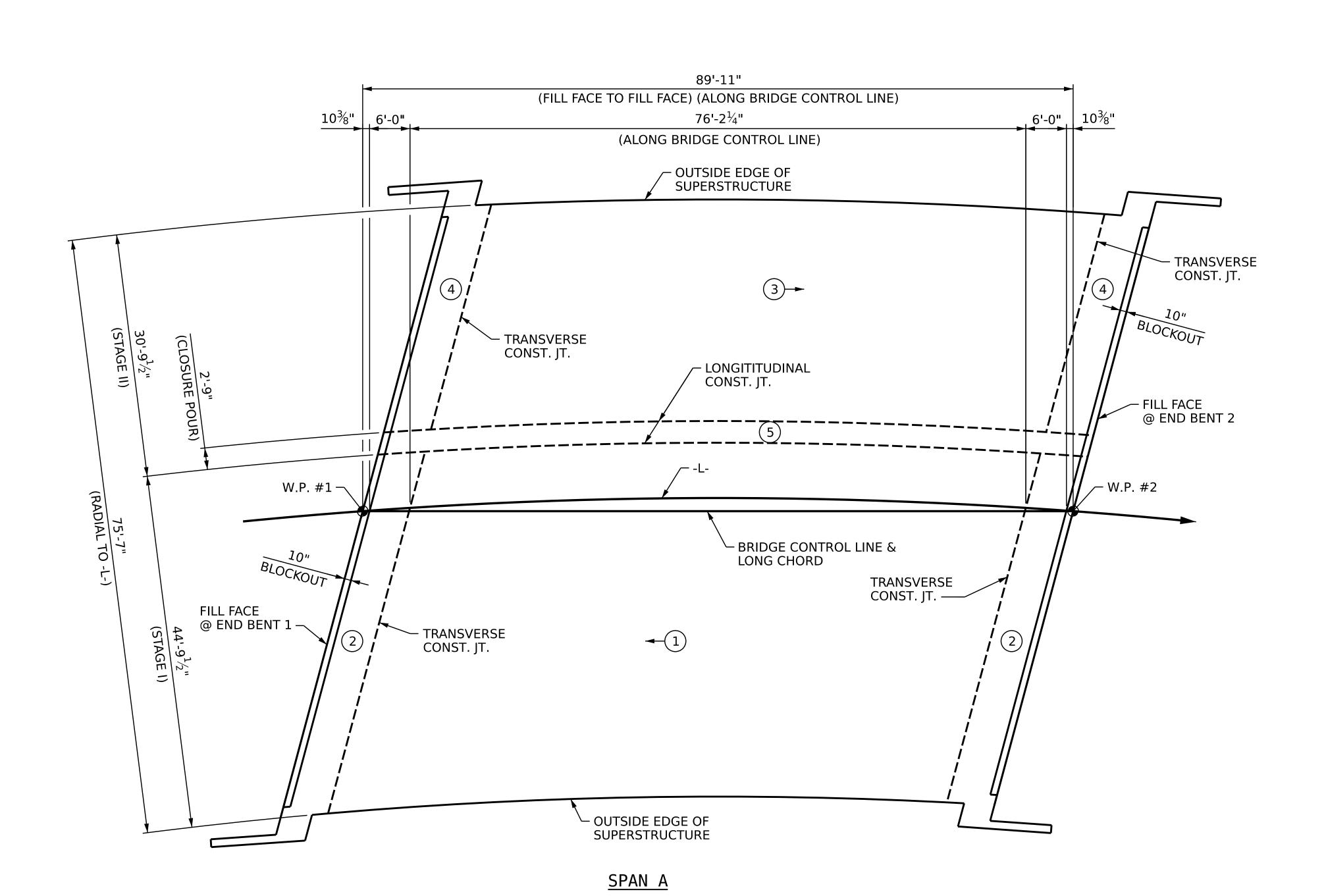
SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

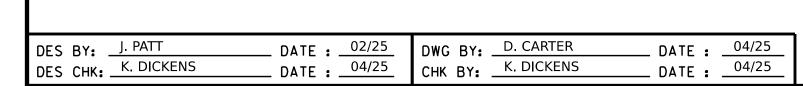
SUPERSTRUCTURE BILL OF MATERIALS



CONCRETE POURING SEQUENCE AND LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB

(REINFORCED CONCRETE DECK SLAB AREA = 6672 SQ. FT.)

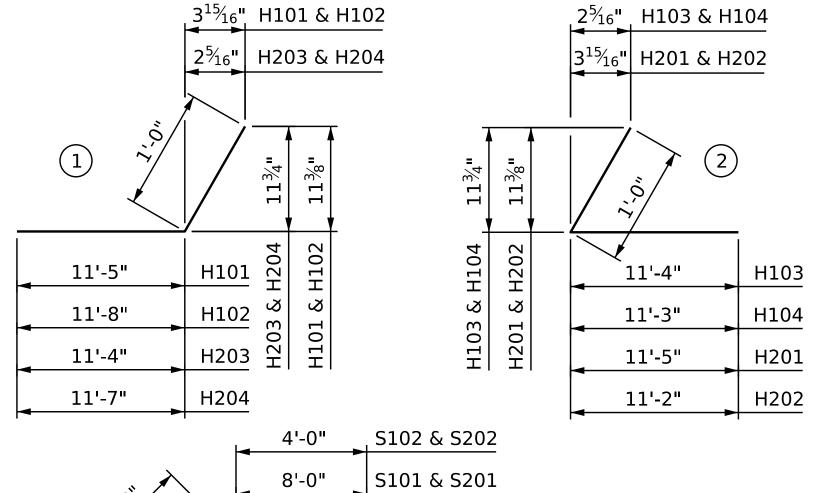
(#) → = INDICATES POUR NUMBER AND DIRECTION OF POUR



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SUPERSTRUCTURE BILL OF MATERIAL																												
					CΤΛ	GE I	DEC	K VVIL	D DIAPH	1D V CMC										<u> </u>	TACE T	T DECK	AND DIAF	OHDAG	MC			215/,
DAD LNG	Lore	T TVDE	LIENOTU	LWEIGHT	317	L L	LOUZE	ANL		INAUIIO		NIO	Loize	I TVDE	LIENGTH	LVEIGUE	DAD	NIO	CIZE	<u> </u>	TIAGE 1.	T			T = 1 / 5 =	LENGTH	LWEIGHT	3 ¹⁵ ⁄ ₁₆ " ◀ ▶
BAR NO.	SIZE	IYPE	LENGIH	WEIGHT	BAK	NO.	SIZE	ITPE	LENGTH	WEIGHT	K103	NO.	SIZE #4	ITPE	LENGTH 4' -4"	WEIGHT	BAR	NO.	SIZE #F	IYPE	27' -8"	WEIGHT	BAR NO.	SIZE			WEIGHT	25/16"
*A101 153 *A102 1	#5 #5	STR STR	44' -5" 42' -9"	7088 45	A308 A309	1	#5 #5	STR STR	31' -10" 30' -0"	33 31	K103	10 10	#4	STR STR	5' -4"	29 36	*A201 *A202	160	#5 #5	STR STR	27' -8"	4617 28	B201 60 *B202 108	#6 #6	STR STR	45' -2" 18' -0"	4070 2920	.
*A102 1	#5	STR	40' -11"	43	A310	1 1	#5	STR	28' -2"	29	K104	20	#4	STR	6' -10"	91	*A202	1	#5 #5	STR	25' -2"	26	*B203 56	#4	STR	29' -6"	1104	!
*A104 1	#5	STR	39' -1"	41	A310	1	#5	STR	26' -4"	27	K105	10	#4	STR	3' -6"	23	*A204	1	#5	STR	23 -2	24	10203 30	#4	3111	29 -0	1104	$\dot{\delta}/\dot{\delta}$
*A105 1	#5	STR	37' -3"	39	A311	1	#5	STR	24' -6"	26	K107	1	#4	STR	3' -8"	23	*A205	1	#5 #5	STR	21' -6"	22	*D201 173	#5	STR	5' -3"	947	
*A106 1	#5	STR	35' -6"	37	A312	1	#5	STR	22' -8"	24	K108	1	#4	STR	4' -1"	3	*A206	1	#5	STR	19' -8"	21	D202 173		STR	4' -10"	872	
*A107 1	#5	STR	33' -8"	35	A314	1	#5	STR	20' -10"	22	K109	2	#4	STR	4' -10"	6	*A207	1	#5	STR	17' -10"	19	3232 273	" 3	1 3	1 20	1 3/2	!
*A108 1	#5	STR	31' -10"	33	A315	1	#5	STR	19' -0"	20	K110	1	#4	STR	3' -2"	2	*A208	1	#5	STR	16' -0"	17	H201 7	#4	2	12' -5"	58	·
*A109 1	#5	STR	30' -0"	31	A316	1	#5	STR	17' -3"	18	K111	1	#4	STR	3' -3"	2	*A209	1	#5	STR	14' -2"	15	H202 7	#4	2	12' -2"	57	1
*A110 1	#5	STR	28' -2"	29	A317	1	#5	STR	15' -5"	16	K112	1	#4	STR	3' -9"	3	*A210	1	#5	STR	12' -4"	13	H203 7	#4	1	12' -4"	58	11'-5" H101
*A111 1	#5	STR	26' -4"	27	A318	1	#5	STR	13' -7"	14	K113	2	#4	STR	4' -6"	6	*A211	1	#5	STR	10' -6"	11	H204 7	#4	1	12' -7"	59	11'-8" H102
*A112 1	#5	STR	24' -6"	26	A319	1	#5	STR	11' -9"	12	K114	1	#4	STR	2' -10"	2	*A212	1	#5	STR	8' -8"	9						- 11-0
*A113 1	#5	STR	22' -8"	24	A320	1	#5	STR	9' -11"	10	K115	1	#4	STR	6' -0"	4	*A213	1	#5	STR	6' -10"	7	K201 5	#4	STR	35' -3"	118	11'-4" H203
*A114 1	#5	STR	20' -10"	22	A321	1	#5	STR	8' -1"	8	K116	1	#4	STR	6' -5"	4	*A214	1	#5	STR	5' -0"	5	K202 5	#4	STR	34' -1"	114	
*A115 1	#5	STR	19' -0"	20	A322	1	#5	STR	6' -3"	7	K117	2	#4	STR	7' -2"	10	*A215	1	#5	STR	3' -2"	3	K203 6	#4	STR	3' -10"	15	11'-7" H204
*A116 1	#5	STR	17' -3"	18	A323	1	#5	STR	4' -6"	5	K118	1	#4	STR	5' -6"	4	*A216	1	#5	STR	27' -3"	28	K204 6	#4	STR	4' -10"	19] '
*A117 1	#5	STR	15' -5"	16	A324	1	#5	STR	2' -8"	3	K119	6	#4	STR	2' -11"	12	*A217	1	#5	STR	25' -5"	27	K205 12	#4	STR	6' -4"	51	」 ⊢
*A118 1	#5	STR	13' -7"	14	A325	1	#5	STR	42' -8"	45	K120	1	#4	STR	4' -1"	3	*A218	1	#5	STR	23' -6"	25	K206 6	#4	STR	3' -0"	12	
*A119 1	#5	STR	11' -9"	12	A326	1	#5	STR	40' -10"	43	K121	1	#4	STR	4' -7"	3	*A219	1	#5	STR	21' -7"	23	K207 1	#4	STR	4' -3"	3	
*A120 1	#5	STR	9' -11"	10	A327	1	#5	STR	38' -11"	41	K122	2	#4	STR	5' -4"	7	*A220	1	#5	STR	19' -8"	21	K208 1	#4	STR	4' -9"	3	
*A121 1	#5	STR	8' -1"	8	A328	1	#5	STR	37' -0"	39	K123	1	#4	STR	3' -8"	2	*A221	1	#5	STR	17' -9"	19	K209 2	#4	STR	5' -6"	7	
*A122 1	#5	STR	6' -3"	7	A329	1	#5	STR	35' -1"	37	K124	6	#4	STR	2' -8"	11	*A222	1	#5	STR	15' -10"	17	K210 1	#4	STR	3' -10"	3	32/20/
*A123 1	#5	STR	4' -6"	5	A330	1	#5	STR	33' -3"	35	WC7.01	F.6			111111	116	*A223	1	#5	STR	14' -0"	15	K211 1	#4	STR	4' -7"	3	24/52/
*A124 1	#5	STR	2' -8"	3	A331	1	#5	STR	31' -4"	33	*S101	56	#4	3	11' -11"	446	*A224	1	#5	STR	12' -1"	13	K212 1	#4	STR	5' -1"	3	500 00 -
*A125 1	#5	STR	42' -8"	45	A332	1	#5	STR	29' -5"	31	*S102	52	#4	3	10' -4"	359	*A225	1	#5	STR	10' -2"	11	K213 2	#4	STR	5' -10"	8	5) 50 0
*A126 1	#5	STR	40' -10"	43	A333	1 1	#5	STR	27' -6"	29	S103	24	#4	5	5' -8"	91	*A226	<u>1</u>	#5 #5	STR	8' -3"	9	K214 1	#4	STR	4' -1"	3	.
*A127 1	#5	STR	38' -11"		A334	1	#5	STR	25' -8"	27	S104	24	#4	3	6' -11"	111	*A227	1	#5 #5	STR	6' -4"	/	K215 1	#4	STR STR	4' -10"	3	
*A128 1	#5	STR STR	37' -0"	39 37	A335 A336	1	#5 #5	STR STR	23' -9" 21' -10"	25 23	U101	57	#4	1 1	9' -2"	349	*A228 *A229	1	#5 #5	STR STR	4' -5" 2' -6"	2	K216 1 K217 2	#4	STR	5' -3" 6' -0"	9	S101 & S201 1'-8½"
*A129 1 *A130 1	#5 #5	STR	35' -1" 33' -3"	35	A336	1	#5	STR	19' -11"	23	U102	37	#4	4	13' -1"	26	*A229		#5	SIR	2 -0	3	K217 2 K218 1	#4	STR	4' -4"	3	S102 & S202 4'-1 ¹³ / ₁₆ "
*A131 1	#5	STR	31' -4"	33	A337	1 1	#5	STR	18' -0"	19	U103	ر م	#4	4	13'-3"	27	A401	160	#5	STR	27' -8"	4617	K218 1 6	#4	STR	2' -9"	11	3102 Q 3202 4-1 716
*A132 1	#5	STR	29' -5"	31	A339	1	#5	STR	16' -2"	17	0103		# -	+ -	15 -5	21	A401 A402	100	#5	STR	27'-0"	28	K219 0	#4	STR	3' -4"	2	1
*A133 1	#5	STR	27' -6"	29	A340	1	#5	STR	14' -3"	15			<u> </u>	1	l	<u>l</u>	A403	1	#5 #5	STR	25' -2"	26	K221 1	#4	STR	3' -10"	3	1
*A134 1	#5	STR	25' -8"	27	A341	1	#5	STR	12' -4"	13	¥ EDC	XY CO	ATED				A404	1	#5	STR	23' -4"	24	K222 2	#4	STR	4' -7"	6	1
*A135 1	#5	STR	23' -9"	25	A342	1	#5	STR	10' -5"	11				ΓEEL (∄	_{∄)} 15,6	514 LBS.	A405	1	#5	STR	21' -6"	22	K223 1	#4	STR	2' -11"	2	1 =
*A136 1	#5	STR	21' -10"	23	A343	1	#5	STR	8' -6"	9	-			FEEL (230 LBS.	A406	1	#5	STR	19' -8"	21	K224 6	#4	STR	2' -9"	11	<u>-</u> 2
*A137 1	#5	STR	19' -11"		A344	1	#5	STR	6' -8"	7	1				ED REINFO		A407	1	#5	STR	17' -10"	19						1
*A138 1	#5	STR	18' -0"	19	A345	1	#5	STR	4' -9"	5	STE		21 0/(1	00/111	LD INLINII O	r.cii te	A408	1	#5	STR	16' -0"	17	*S201 36	#4	3	11' -11"	287	1
*A139 1	#5	STR	16' -2"	17	A346	1	#5	STR	2' -10"	3		$C \perp \Lambda$	$CC \Lambda$	<u>۸ (۸</u>	NCDETE		A409	1	#5	STR	14' -2"	15	*S202 34	#4	3	10' -4"	235	i c
*A140 1	#5	STR	14' -3"	15							1	CLA	33 A	A CUI	NCRETE		A410	1	#5	STR	12' -4"	13	S203 16	#4	5	5' -8"	61	<u>ر</u> د
*A141 1	#5	STR	12' -4"	13	B101	84	#6	STR	45' -5"	5730		POU	R NO.		CU	. YDS.	A411	1	#5	STR	10' -6"	11	S204 16	#4	5	6' -11"	74	
*A142 1	#5	STR	10' -5"	11	*B102	154	#6	STR	18' -0"	4164			1		1	18.6	A412	1	#5	STR	8' -8"	9]
*A143 1	#5	STR	8' -6"	9	*B103	78	#4	STR	29' -6"	1537			2			64.0	A413	1	#5	STR	6' -10"	7	U201 36	#4	4	9' -2"	220	J
*A144 1	#5	STR	6' -8"	7								TOTA	L (毋)		1	82.6	A414	1	#5	STR	5' -0"	5	U202 6	#4	4	13' -2"	53	1
*A145 1	#5	STR	4' -9"	5	*D101	173	#5	STR	5' -3"	947	(曲)				ARAPETS A		A415	1	#5	STR	3' -2"	3						(ALL
*A146 1	#5	STR	2' -10"	3	D102	173	#5	STR	4' -10"	872		SIDE	WALK	ARE NO	OT INCLUD	ED	A416	1	#5		27' -3"	28	* EPOXY CO			 10	553 LBS.	
				1							1						A417	1	#5	+	25' -5"	27	REINFOR			サ)		G
A301 153	#5	STR	44' -5"	7088	H101	7	#4	1 1	12' -5"	58	1						A418	1	#5	STR	23' -6"	25	REINFOR		-		057 LBS.	↓
A302 1	#5	STR	42' -9"	45	H102	7	#4	1 1	12' -8"	59	4						A419	1	#5 =	+	21' -7"	23	* DENOTES	S EPOXY	COATI	ED REINFO	RCING	BRID
A303 1	#5	STR	40' -11"		H103	7 -	#4	2	12' -4"	58	4						A420	1	#5		19' -8"	21	STEEL					
A304 1	#5	STR	39' -1"	41	H104	7	#4	2	12' -3"	57	-						A421	1	#5	STR	17' -9"	19	- CLA	ASS A	A CO	NCRETE		APPE
A305 1	#5	STR	37' -3"	39	1/1 01	1.0	11.0	CTS	201 0"	107	4						A422	1	#5 #5	+	15' -10"	17						TOTA
A306 1	#5	STR	35' -6"	37	K101	10	#4	STR	28' -0"	187	-						A423	1	#5 #5		14' -0"	15	POL	JR NO.			. YDS.	· _
A307 1	#5	STR	33' -8"	35	K102	10	#4	STR	26' -10"	179	J						A424	1	#5 #5	STR	12' -1"	13	 	<u>خ</u>			76.4	1
																	A425	1	#5 #5	STR	10' -2"	11	E (CLOC	H HDE DO	IID)		42.7 0.2	1
																	A426 A427	1	#5 #5	+	8' -3" 6' -4"	9 7	5 (CLOS	ORE PO AL(申)			9.2 28.3	1
																	A427	1	#5 #5	+	4' -5"	5						1
																	A428 A429	1	#5		2' -6"	3	(曲)QUA SID			OT INCLUD		
																	1123	-	, , <u>,</u>	1 3111		<u> </u>	3,0	_ , , , (LIX	14\	J. 1.1020D		1
																	1 0	HDED	CTDII	CTUD	F RETNI	EUDUTNIC	CTEEL					

SUPERSTRUCTURE LENGTHS ARE FOLLOWING MINIM	BASED ON	THE	
 CURERCERUCTURE	_		

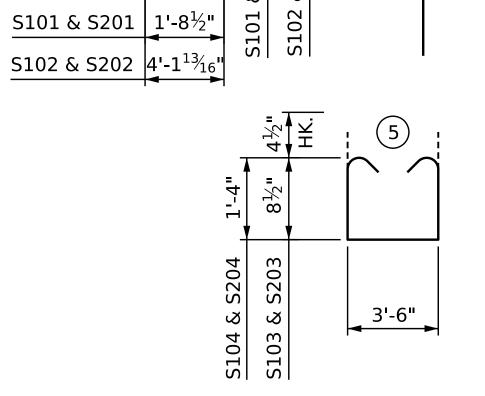
BAR SIZE	EXCEPT A SLABS,	RUCTURE APPROACH PARAPET, RIER RAIL	APPROAG	PARAPET AND BARRIER			
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL		
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"		
#5	2'-5"	2'-0"	2'-5"	2'-0"	3'-1"		
#6	2'-10"	2'-5"	3'-7"	2'-5"	3'-8"		
#7	4'-2"	2'-9"					
#8	4'-9"	3'-2"					



3'-6"

4

BAR TYPES



& 5201

(ALL BAR DIMENSIONS ARE OUT TO OUT)

GROOVING B	RIDGE	FL00RS
BRIDGE DECK	5,208	SQ.FT.
APPROACH SLABS	2,889	SQ.FT.
TOTAL	8,097	SQ.FT.

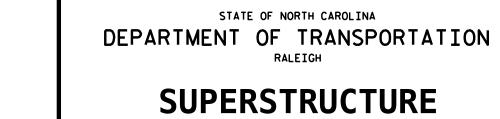
PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

_ COUNTY

SHEET 2 OF 2

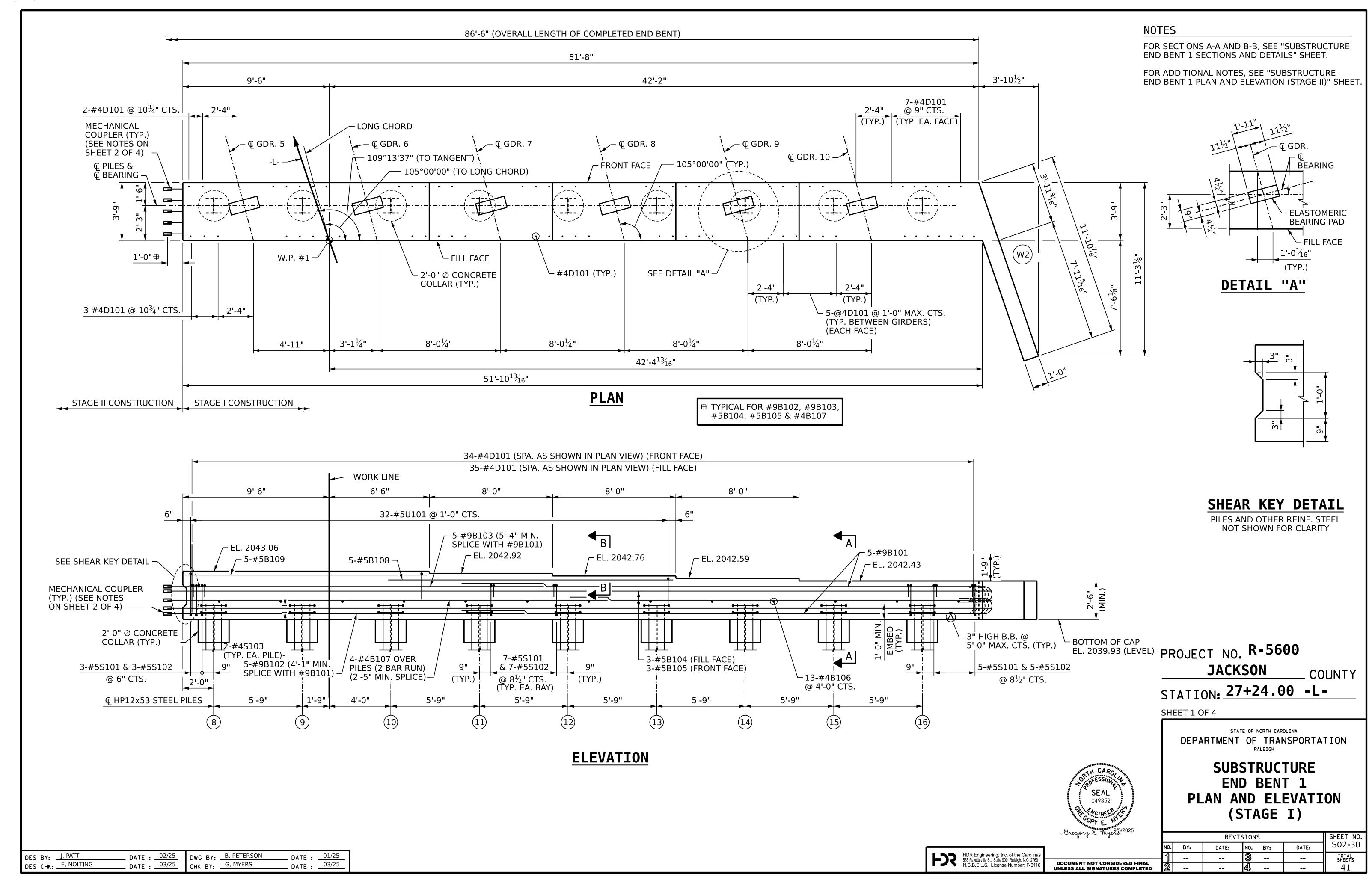




SHEET NO. S02-29 REVISIONS DATE: NO. BY: DATE:

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DES BY: J. PATT
DES CHK: K. DICKENS DWG BY: D. CARTER
CHK BY: K. DICKENS DATE: 04/25
DATE: 04/25



5'-0" MAX. CTS. (TYP.) —

DWG BY: B. PETERSON

CHK BY: G. MYERS

DATE : 02/25
DATE : 03/25

DES BY: J. PATT

DES CHK: E. NOLTING

€ HP12x53 STEEL PILES

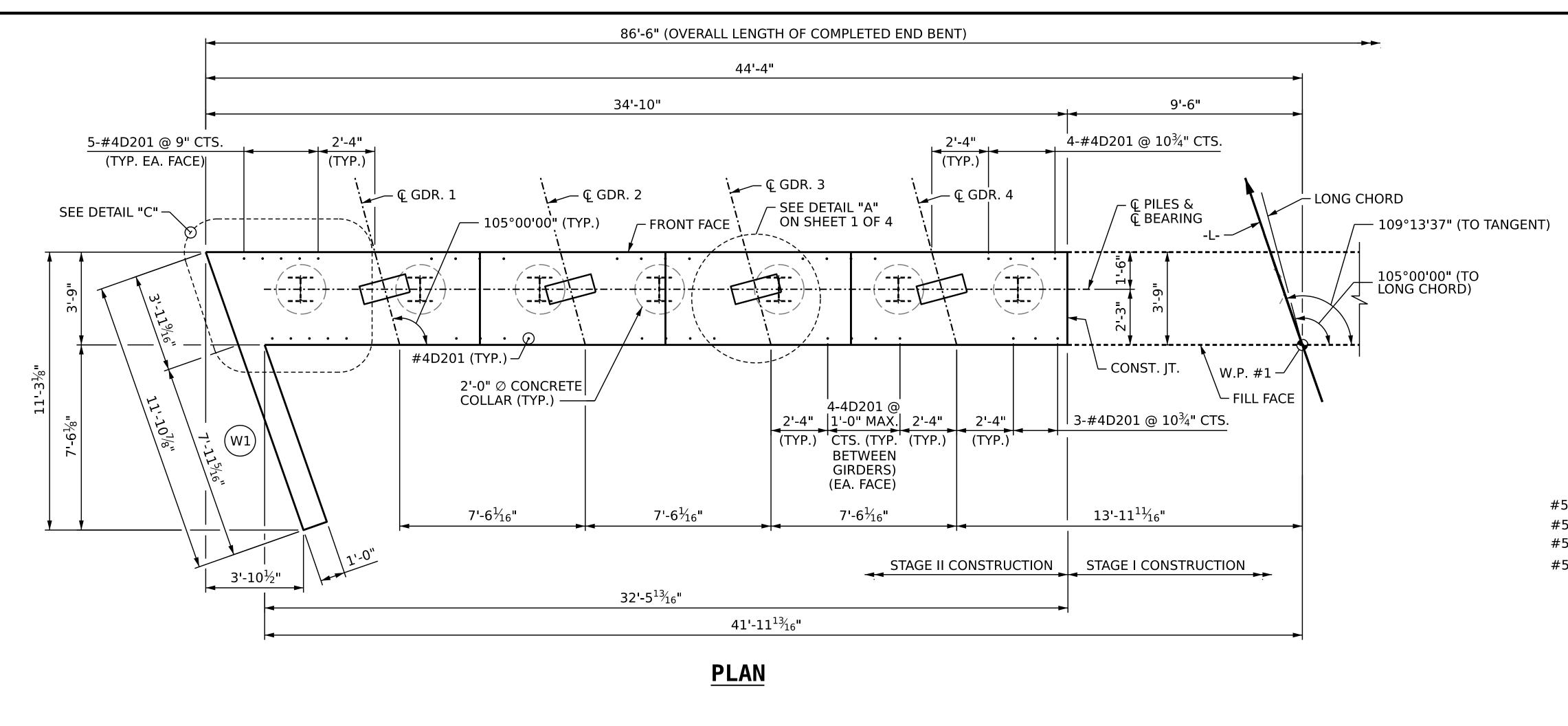
(TYP. EA. BAY)

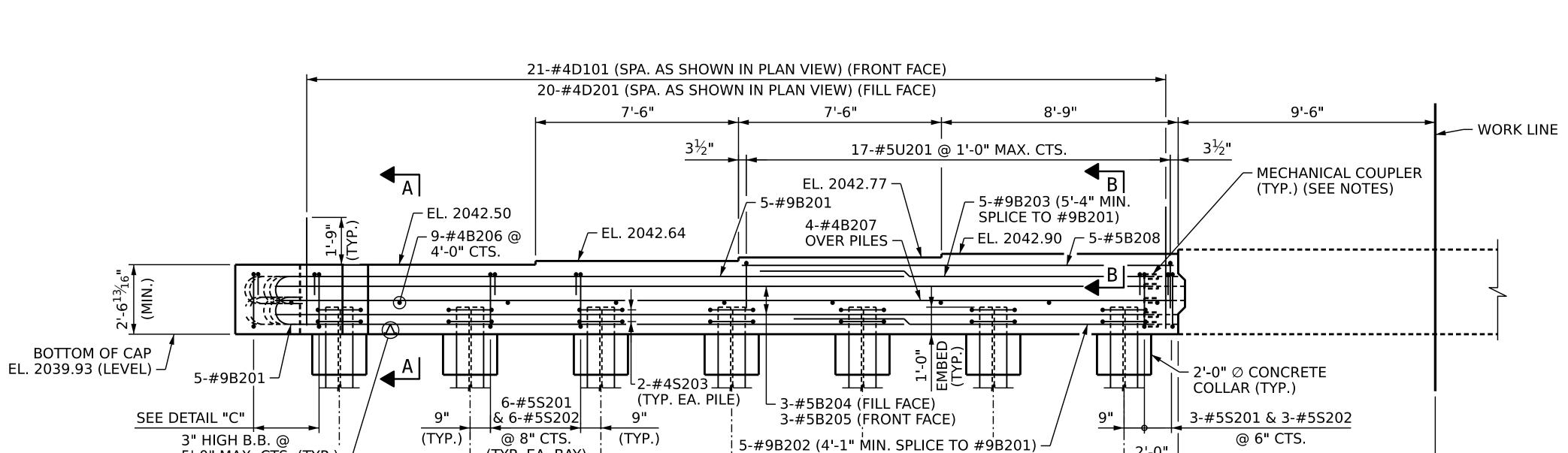
4'-10"

4'-10"

4'-10"

__ DATE : 01/25 __ DATE : 03/25





4'-10"

ELEVATION

i 2'-0"

11'-6"

4'-10"

4'-10"

NOTES

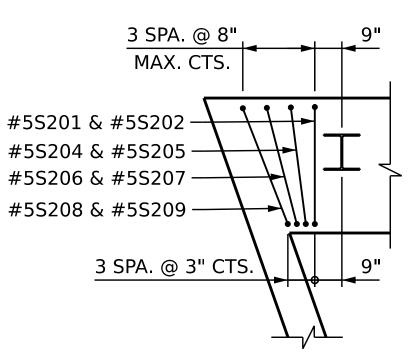
FOR SECTIONS A-A, B-B, PILE SPLICE DETAILS AND TEMPORARY DRAINAGE AT END BENT DETAIL, SEE "SUBSTRUCTURE END BENT 1 SECTIONS AND DETAILS" SHEET.

THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".

THE #4D101 AND #4D201 BARS MAY BE SHIFTED SLIGHTLY TO AVOID STIRRUPS IN CAP.

FOR MECHANICAL COUPLERS, SEE SECTION 1070-9 OF THE STANDARD SPECIFICATIONS.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT 1, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.



MAIN REINFORCING NOT SHOWN FOR CLARITY

DETAIL "C"

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

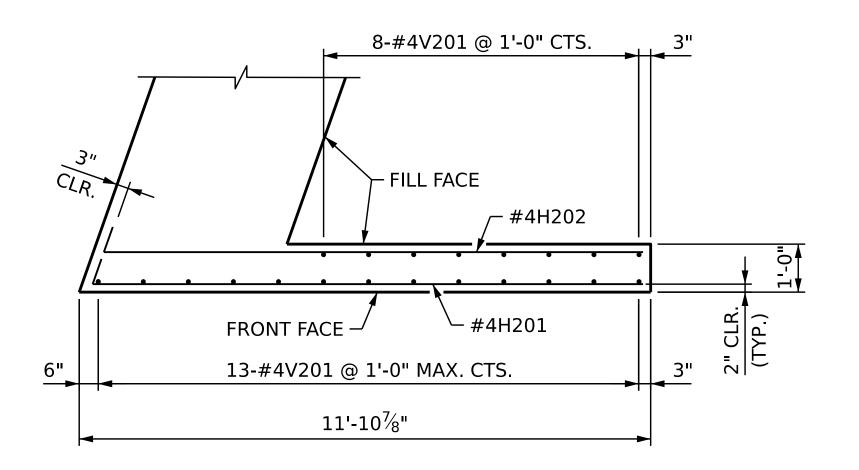
COUNTY

SUBSTRUCTURE END BENT 1 PLAN AND ELEVATION (STAGE II)

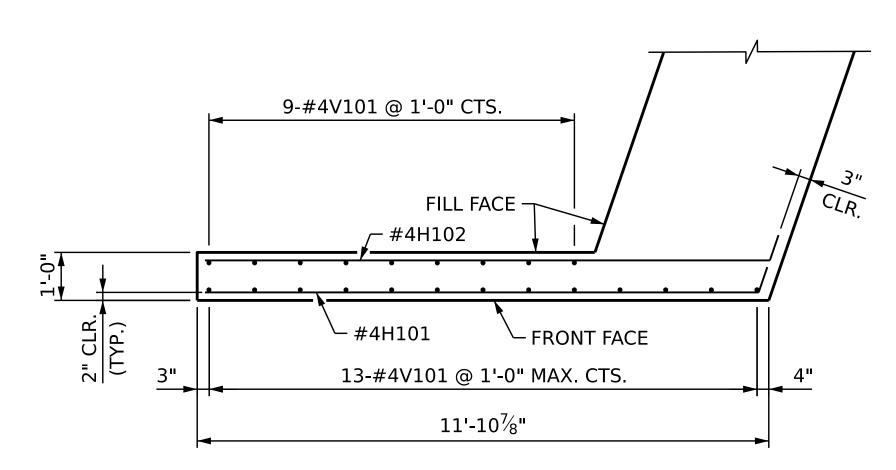
SHEET NO. REVISIONS S02-31 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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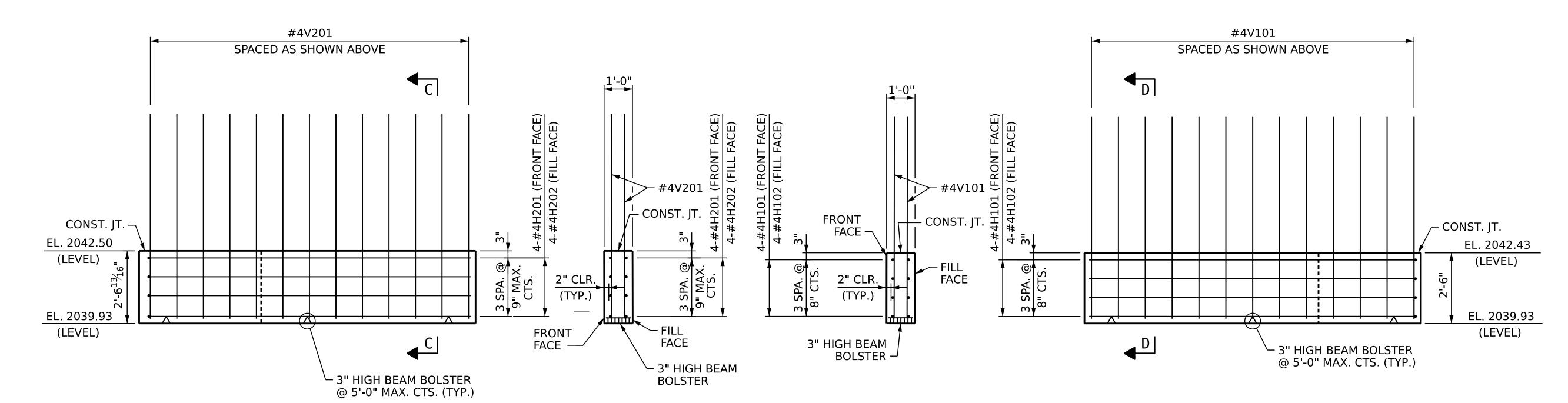
DES BY: J. PATT
DES CHK: E. NOLTING



PLAN - WINGWALL "W1" (STAGE II)



PLAN - WINGWALL "W2" (STAGE I)



ELEVATION - WINGWALL "W1" (STAGE II)

__ DATE : 01/25 __ DATE : 03/25

DWG BY: B. PETERSON
CHK BY: G. MYERS

DATE: 02/25
DATE: 03/25

SECTION C-C

SECTION D-D

ELEVATION - WINGWALL "W2" (STAGE I)

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PROJECT NO. R-5600

JACKSON COUNTY

STATION: 27+24.00 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE END BENT 1 WINGWALLS

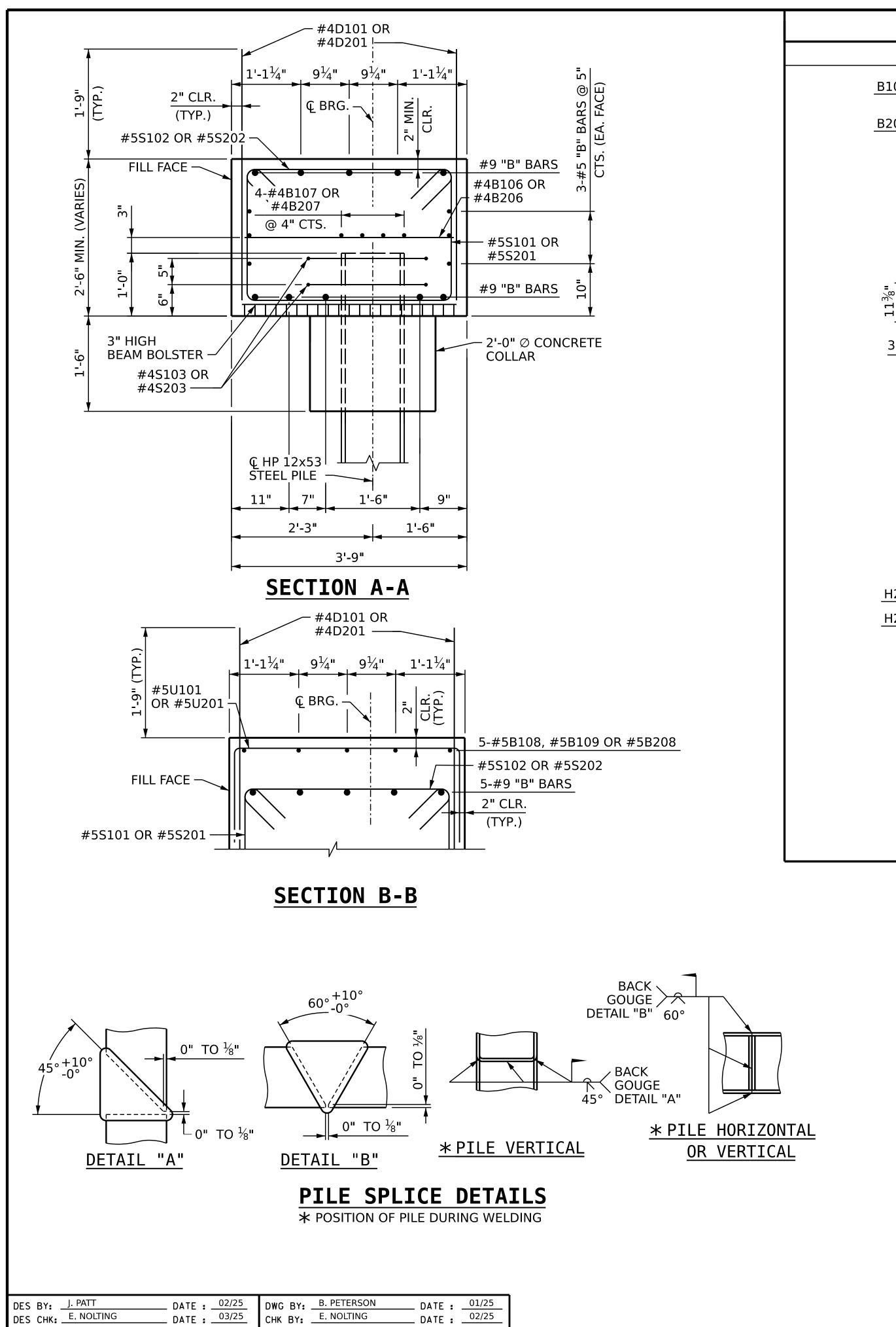
REVISIONS

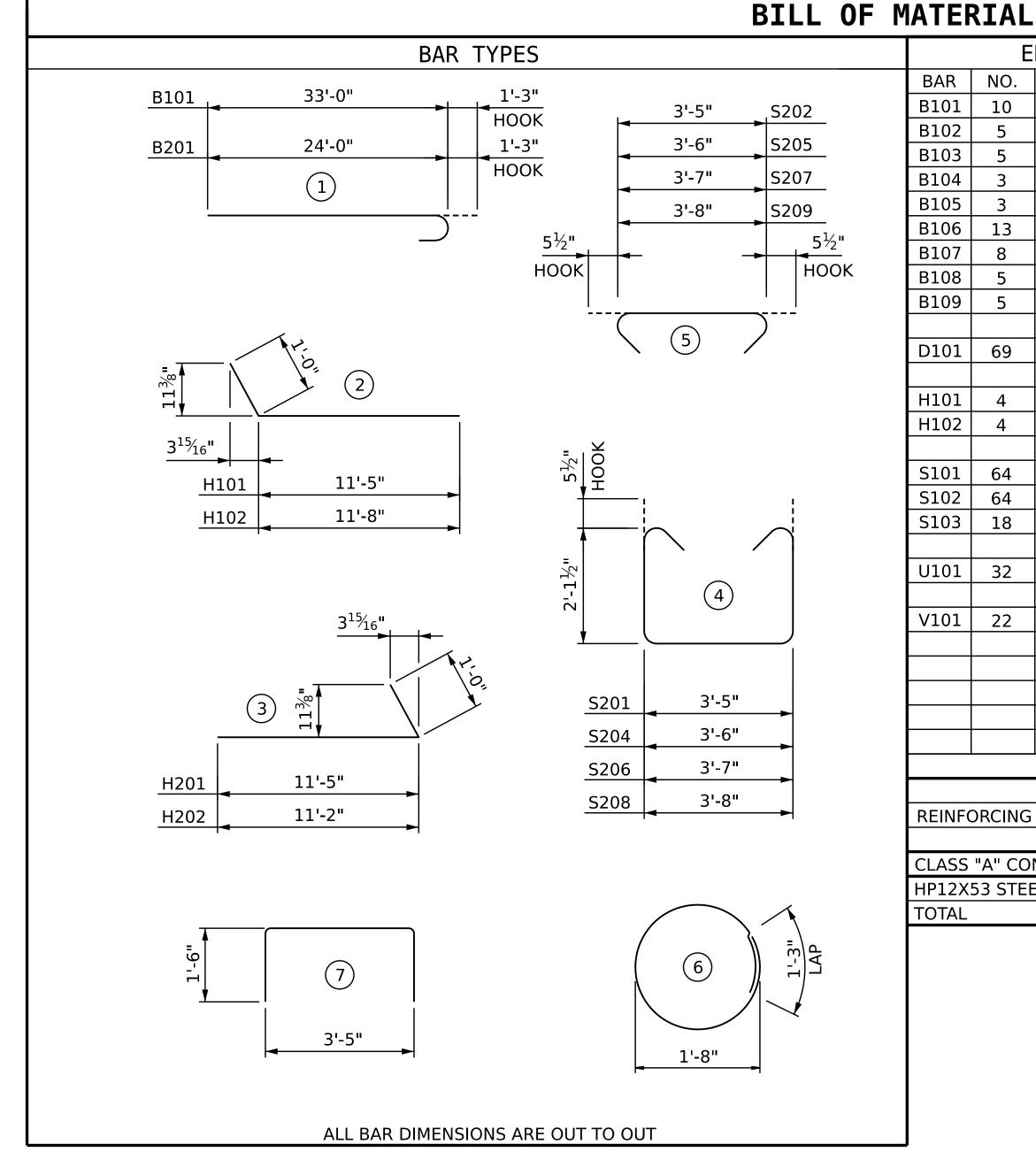
SY: DATE: NO. BY: DATE: S02-32

-- -- 3 -- -- TOTAL SHEETS
-- -- 41

SEAL
049352

MELONY E. MARTINETTE STATE OF THE SEAL
049352





	END BENT 1 STAGE I						END	BENT	1 STA	AGE II		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
B101	10	#9	1	34'-3"	1165	B201	10	#9	1	25'-3"	859	
B102	5	#9	STR	25'-0"	425	B202	5	#9	STR	14'-2"	241	
B103	5	#9	STR	26'-3"	446	B203	5	#9	STR	15'-5"	262	
B104	3	#5	STR	53'-8"	168	B204	3	#5	STR	32'-4"	101	
B105	3	#5	STR	52'-6"	164	B205	3	#5	STR	33'-5"	105	
B106	13	#4	STR	3'-5"	30	B206	9	#4	STR	3'-5"	21	
B107	8	#4	STR	28'-2"	151	B207	4	#4	STR	32'-11"	88	
B108	5	#5	STR	19'-0"	99	B208	5	#5	STR	15'-11"	83	
B109	5	#5	STR	15'-8"	82							
						D201	41	#4	STR	4'-0"	110	
D101	69	#4	STR	4'-0"	184							
						H201	4	#4	3	12'-5"	33	
H101	4	#4	2	12'-5"	33	H202	4	#4	3	12'-2"	33	
H102	4	#4	2	12'-8"	34							
						S201	40	#5	4	8'-7"	358	
S101	64	#5	4	8'-7"	573	S202	40	#5	5	4'-4"	181	
S102	64	#5	5	4'-4"	289	S203	14	#4	6	6'-6"	61	
S103	18	#4	6	6'-6"	78	S204	1	#5	4	8'-8"	9	
						S205	1	#5	5	4'-5"	5	
U101	32	#5	7	6'-5"	214	S206	1	#5	4	8'-9"	9	
						S207	1	#5	5	4'-6"	5	
V101	22	#4	STR	7'-0"	103	S208	1	#5	4	8'-10"	9	
						S209	1	#5	5	4'-7"	5	
						U201	17	#5	7	6'-5"	114	
						V201	21	#4	STR	7'-1"	99	
DEIX:	ODCING	CTEE:			1220 DC	DEINIE		CTEE			701 100	
KEINF	ORCING	SIEEL			1238 LBS	KEINF	ORCING	SIEEL		2	791 LBS	
CLASS	CLASS "A" CONCRETE 23.0 CU. YDS						OS. CLASS "A" CONCRETE 15.1 CU. YDS.					
				23.0						13.1		
	K53 STEE	L PILES	•		NO. 9			EL PILES)		NO. 7	
TOTAL	_				9	TOTAL	-				/	
1												

MINIMUM OF 3-ONE CUBIC
FOOT BAGS OF #78M STONE.
BAGS SHALL BE OF POROUS
FABRIC, SECURELY TIED.

GRADE TO DRAIN

GRADE TO DRAIN

6" (MIN.) PIPE
FOR DRAINAGE
TOE OF SLOPE

TOE OF SLOPE

TOE OF SLOPE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

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PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 4 OF 4

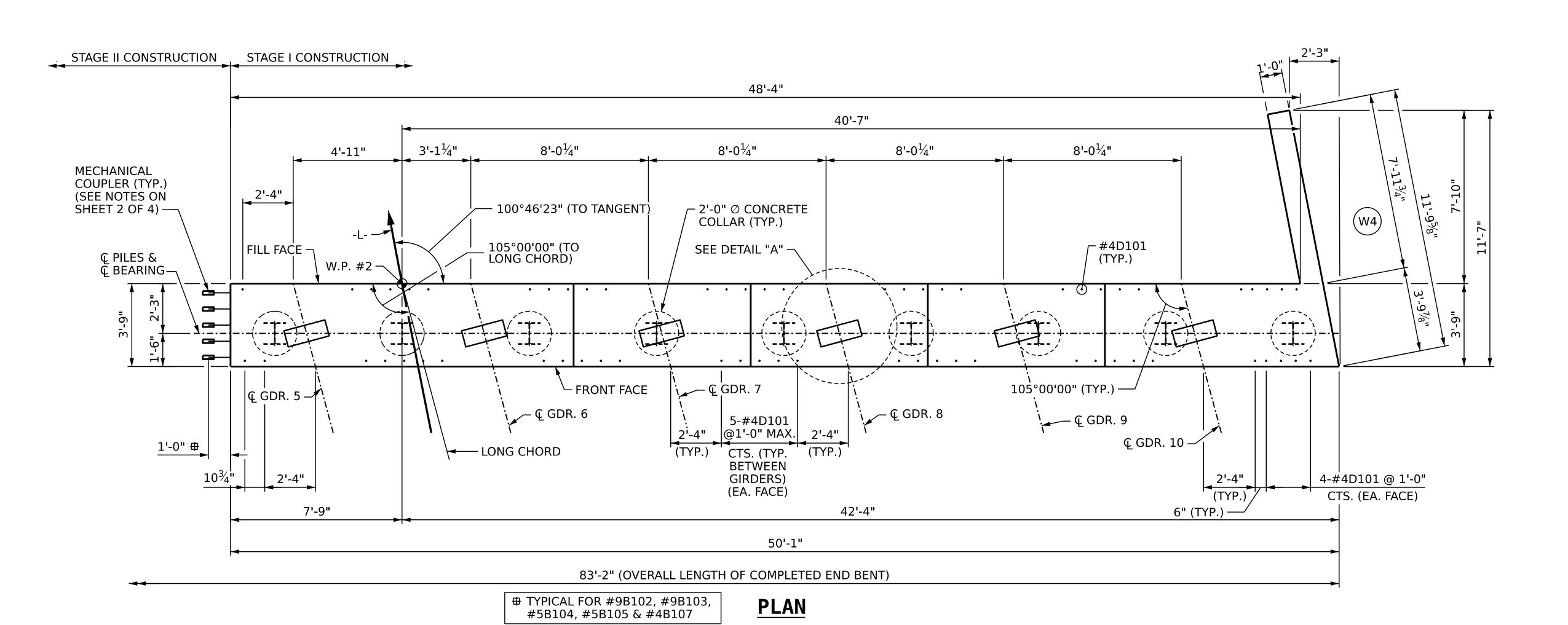
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

COUNTY

SUBSTRUCTURE END BENT 1 SECTIONS AND DETAILS



32-#4D101 (SPA. AS SHOWN IN PLAN VIEW) (FRONT FACE) 31-#4D101 (SPA. AS SHOWN IN PLAN VIEW) (FILL FACE) ₩ORK LINE 7'-9" 7'-9" 8'-0" 8'-0" 8'-0" 32-#5U101 @ 1'-0" CTS. __ 5-#9B103 (5'-4" MIN. SPLICE _ 5-5B109 TO #9B101) _ 5-#9B101 SEE SHEAR KEY ← EL. 2043.25 - EL. 2043.39 _ 5-#5Bİ08 1'-9" (TYP.) _ EL. 2042.92 DETAIL -- EL. 2042.76 _ EL. 2043.09 MECHANICAL COUPLER (TYP.) (SEE NOTES ON SHEET 2 OF 4)— • - - - -**13-#4B106** 2'-0" Ø CONCRETE @ 4'-0" BOTTOM OF CAP COLLAR (TYP.) -EL. 2040.26 (LEVEL) 3" HIGH B.B. @ 5'-0" MAX. CTS. (TYP.)-7-#5S101 └ 2-#4S103 3-#5B104 (FILL FACE) 3-#5B105(FRONT FACE) 4-#4B107 OVER PILES (2 BAR RUN) (2'-5" MIN. SPLICE) (TYP. @ EA. PILE) 2-#5S101 & 2-#5S102 & 7-#5S102 3-#5S101 & 3-#5S102 5 - #9B102 (4'-1" MIN. SPLICE TO #9B101) —— @ 6" CTS. @ $8\frac{1}{2}$ " CTS. (TYP. EA. BAY) @ 6" CTS.

ELEVATION

5'-9"

5'-9"

(10)

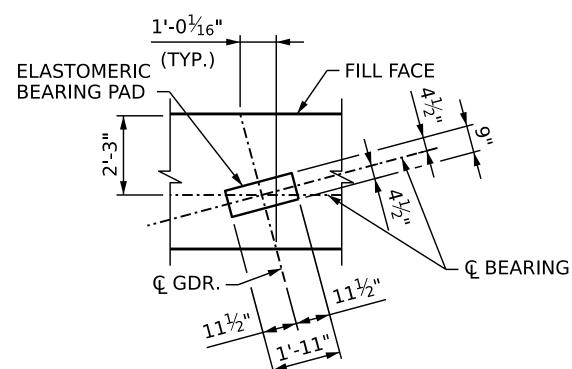
5'-9"

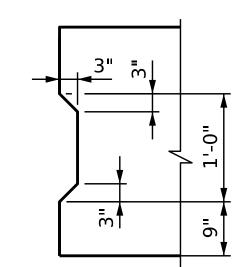
5'-9"

NOTES NOTES

FOR SECTIONS A-A AND B-B, SEE "SUBSTRUCTURE END BENT 2 SECTIONS AND DETAILS" SHEET.

FOR ADDITIONAL NOTES, SEE "SUBSTRUCTURE END BENT 2 PLAN AND ELEVATION (STAGE II)" SHEET.





DETAIL "A"

SHEAR KEY DETAIL

PILES AND OTHER REINF. STEEL NOT SHOWN FOR CLARITY

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

COUNTY

SUBSTRUCTURE
END BENT 2
PLAN AND ELEVATION
(STAGE I)

 REVISIONS
 SHEET NO.

 BY:
 DATE:
 No.
 BY:
 DATE:
 SO2-34

 - - 3
 - - TOTAL SHEET'S HEET'S

 - - 41
 - 41

5'-9"

5'-9"

 DES BY:
 J. PATT
 DATE:
 02/25
 DWG BY:
 B. PETERSON
 DATE:
 01/25

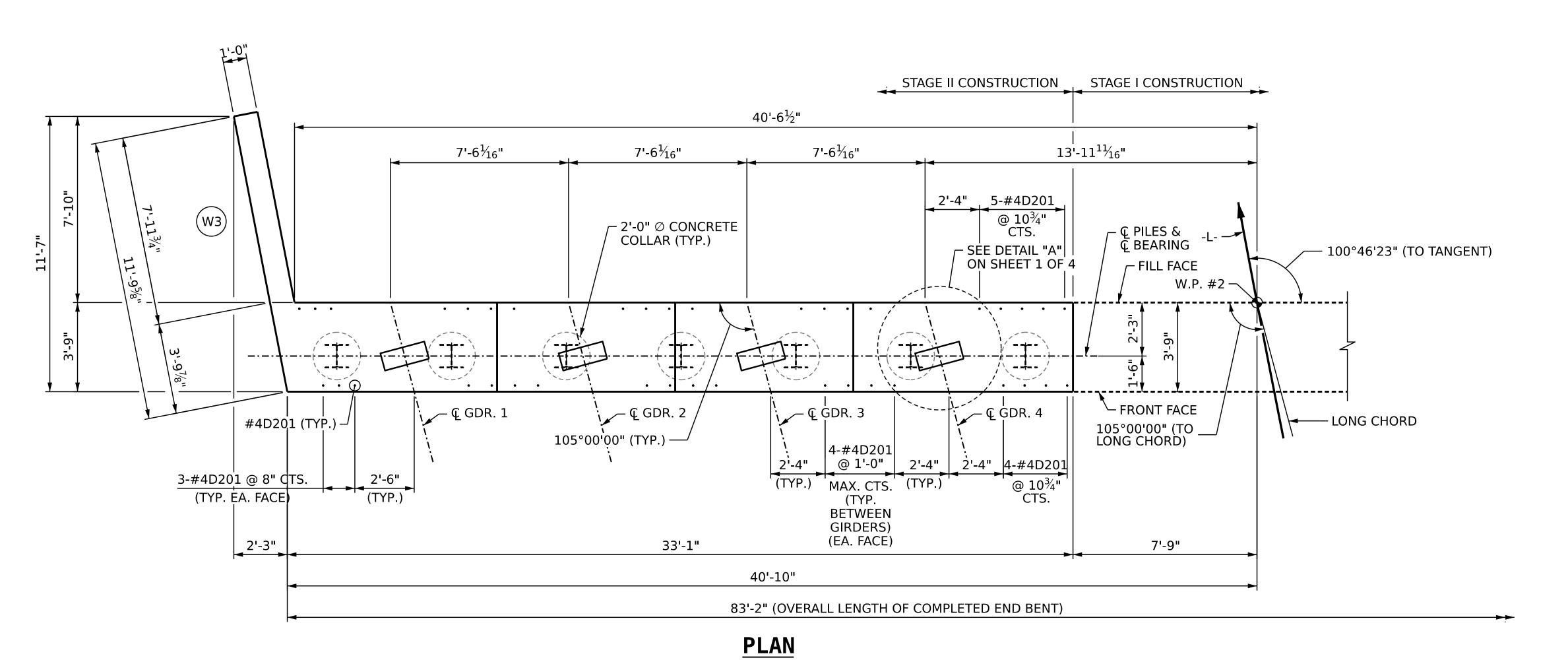
 DES CHK:
 E. NOLTING
 DATE:
 03/25
 CHK BY:
 G. MYERS
 DATE:
 03/25

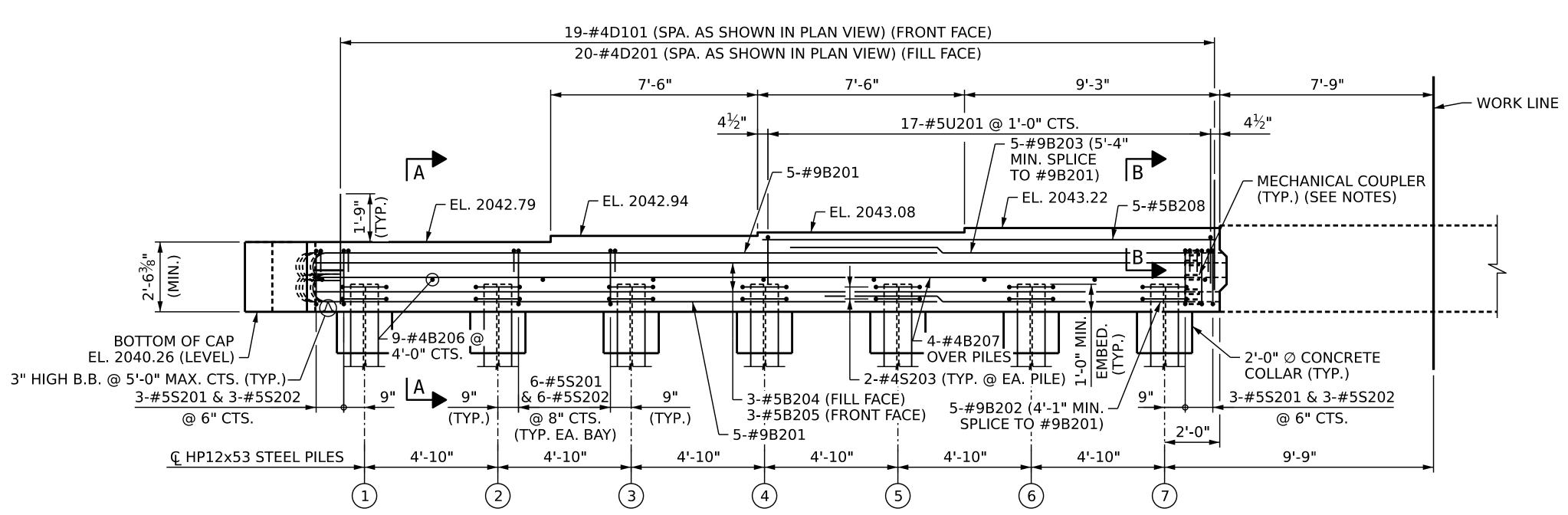
5'-9"

5'-9"

€ HP12x53 STEEL PILES

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555 Fayetteville St., Suite 900 Raleigh, N.C. 27601
N.C.B.E.L.S. License Number: F-0116
UNLESS ALL SIGNATURES COMPLETED





ELEVATION

DWG BY: B. PETERSON DES BY: J. PATT DATE : 02/25
DATE : 03/25 __ DATE : 01/25 __ DATE : 03/25 DES CHK: E. NOLTING CHK BY: G. MYERS

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NOTES

FOR SECTIONS A-A & B-B, SEE "SUBSTRUCTURE END BENT 2 SECTIONS AND DETAILS" SHEET.

THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".

THE #4D101 AND #4D201 BARS MAY BE SHIFTED SLIGHTLY TO AVOID STIRRUPS IN CAP.

FOR MECHANICAL COUPLERS, SEE SECTION 1070-9 OF THE STANDARD SPECIFICATIONS.

FOR PILE SPLICE DETAILS AND TEMPORARY DRAINAGE AT END BENT DETAIL, SEE "SUBSTRUCTURE END BENT 1 SECTIONS AND DETAILS" SHEET.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT 2, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. R-5600

JACKSON

COUNTY

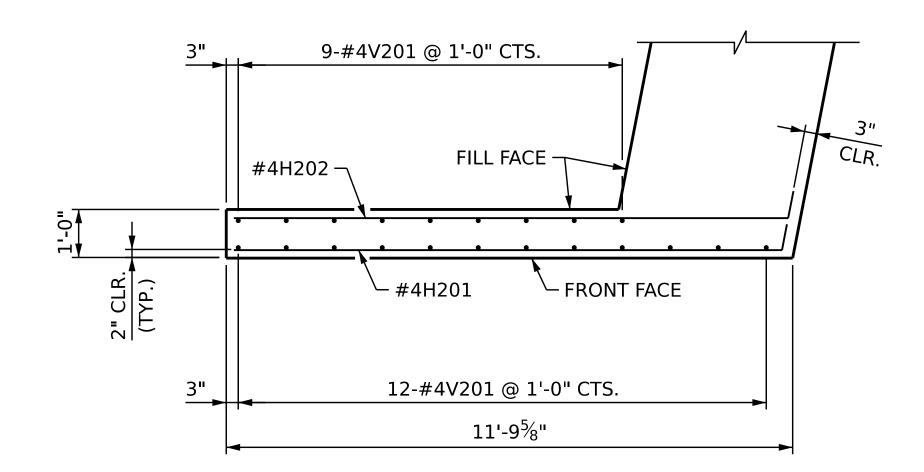
STATION: 27+24.00 -L-

SHEET 2 OF 4

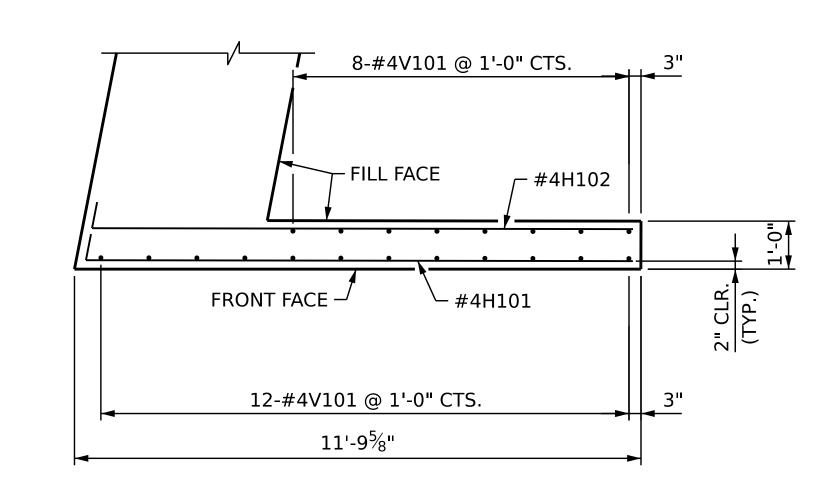
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE END BENT 2 PLAN AND ELEVATION (STAGE II)

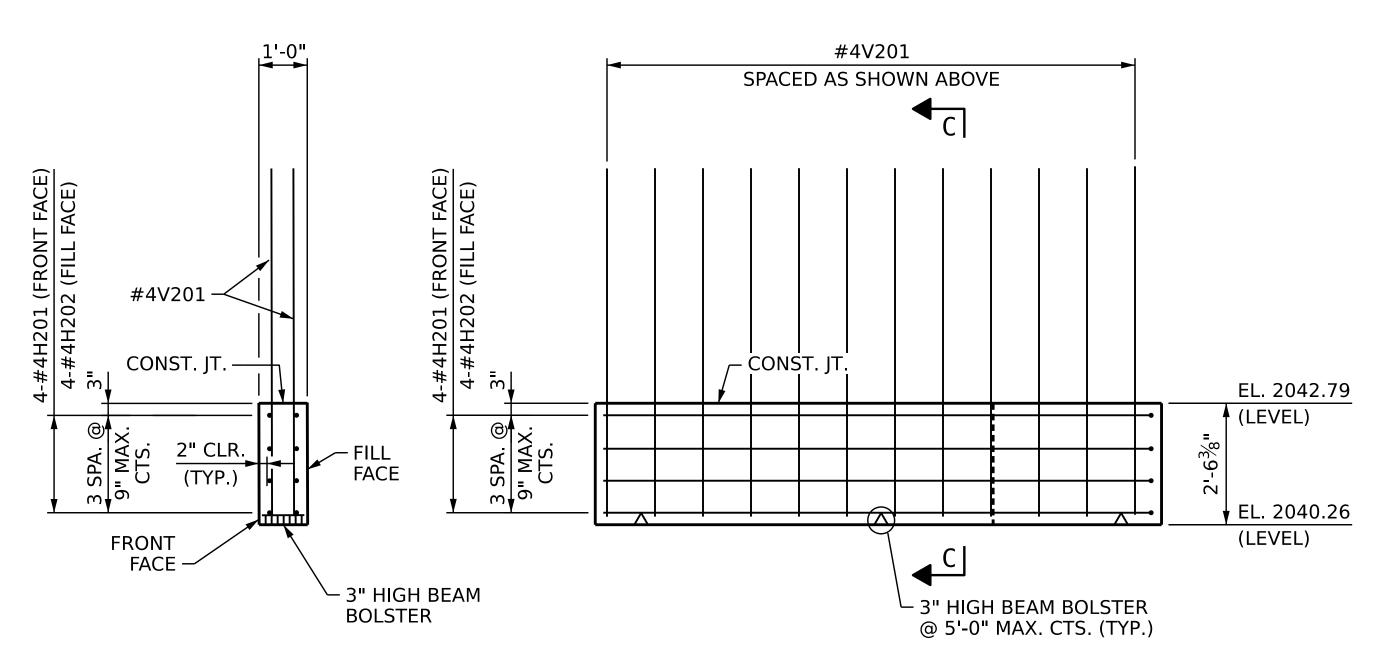
		SHEET NO.						
NO.	BY:	DATE:	NO. BY: DATE: S02-35					
1			3			TOTAL SHEETS		
2			4			41		



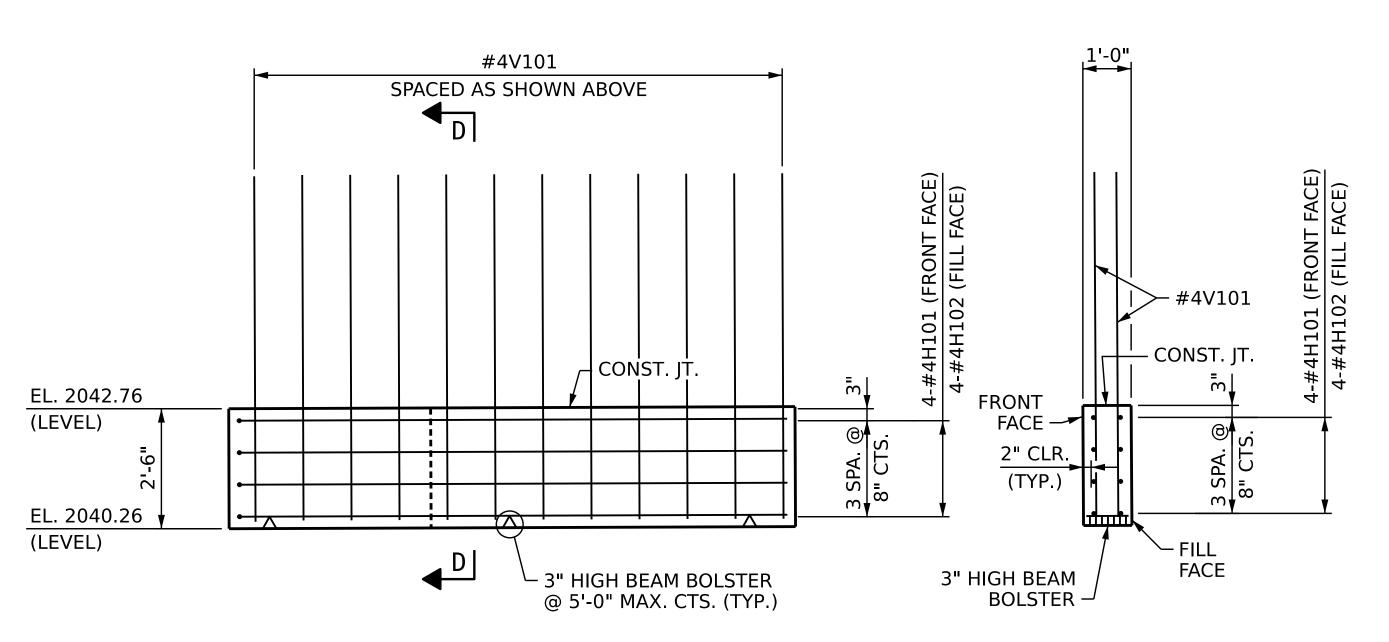
PLAN - WINGWALL "W3" (STAGE II)



PLAN - WINGWALL "W4" (STAGE I)



ELEVATION - WINGWALL "W3" (STAGE II)



ELEVATION - WINGWALL "W4" (STAGE I)

SECTION D-D

PROJECT NO. R-5600 **JACKSON** COUNTY STATION: 27+24.00 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> **SUBSTRUCTURE** END BENT 2

WINGWALLS

SHEET NO. S02-36 REVISIONS NO. BY: DATE: DATE: TOTAL SHEETS

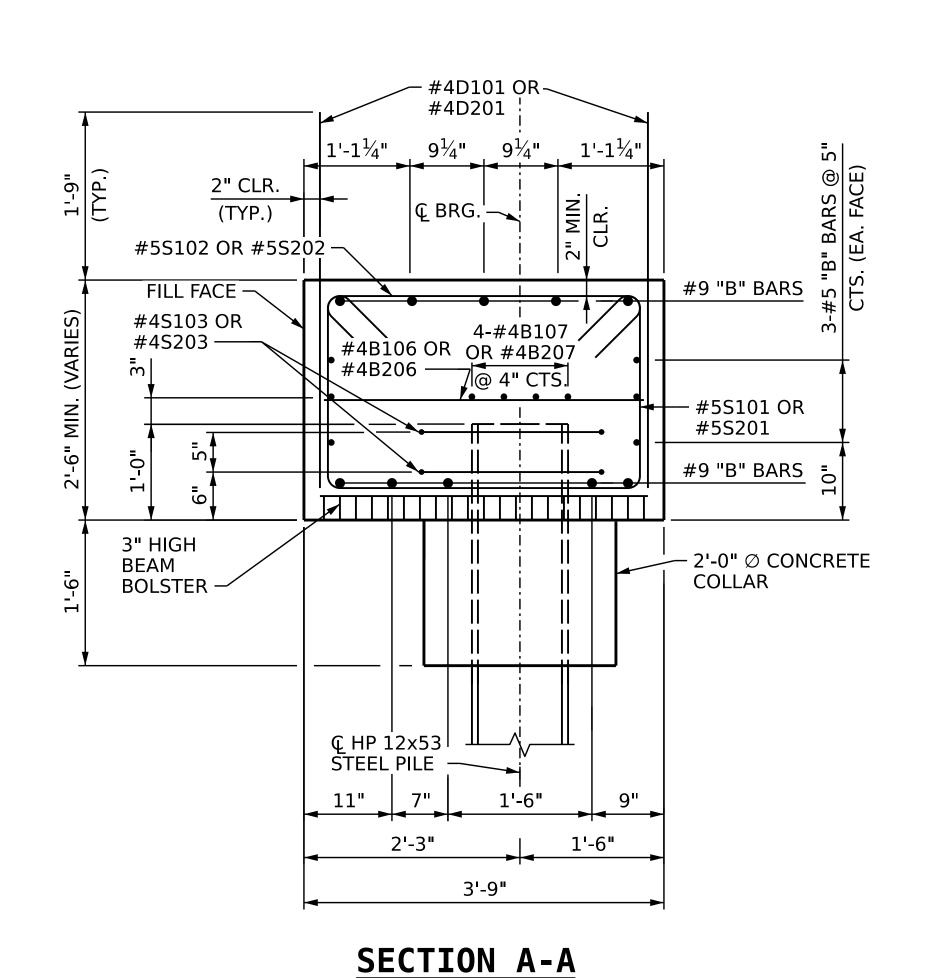
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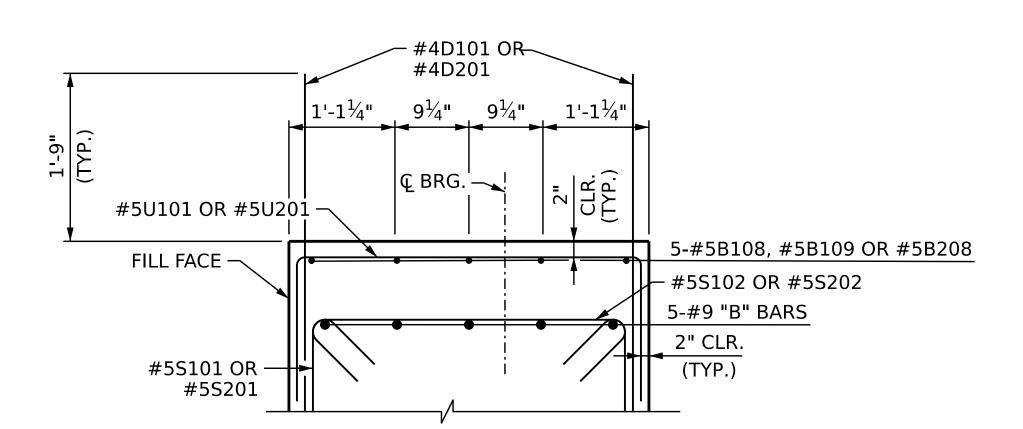
ES BY: J. PATT	DATE : 02/25	DWG BY: B. PETERSON	DATE : .	01/25
ES CHK: E. NOLTING	02/25	CHK BY: G. MYERS	DATE :	03/25

SECTION C-C

DES BY: J. PATT
DES CHK: E. NOLTING

__ DATE : 02/25 __ DATE : 03/25

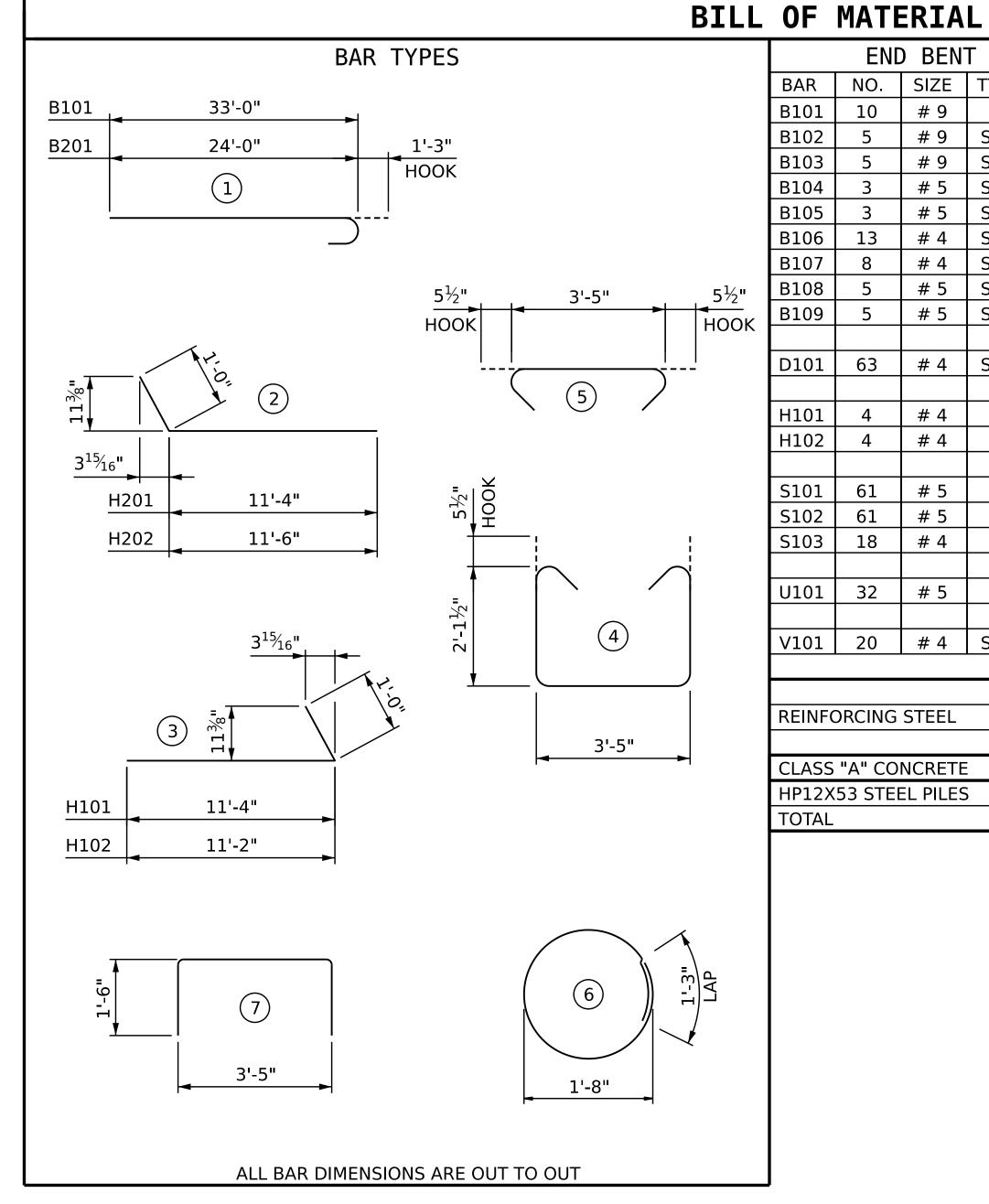




SECTION B-B

DWG BY: B. PETERSON
CHK BY: G. MYERS

__ DATE : 01/25 __ DATE : 03/25



	END BENT 2 STAGE I					END BENT 2 STAGE II						
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	B101	10	# 9	1	34'-3"	1165	B201	10	# 9	1	25'-3"	859
	B102	5	# 9	STR	22'-2"	377	B202	5	# 9	STR	13'-5"	228
	B103	5	# 9	STR	23'-5"	398	B203	5	# 9	STR	14'-8"	249
	B104	3	# 5	STR	50'-3"	157	B204	3	# 5	STR	32'-6"	102
	B105	3	# 5	STR	50'-10"	159	B205	3	# 5	STR	31'-10"	100
	B106	13	# 4	STR	3'-5"	30	B206	9	# 4	STR	3'-5"	21
	B107	8	# 4	STR	26'-6"	142	B207	4	# 4	STR	32'-0"	86
	B108	5	# 5	STR	19'-0"	99	B208	5	# 5	STR	16'-5"	86
	B109	5	# 5	STR	15'-2"	79						
`							D201	39	# 4	STR	4'-0"	104
	D101	63	# 4	STR	4'-0"	168						
							H201	4	# 4	2	12'-4"	33
	H101	4	# 4	3	12'-4"	33	H202	4	# 4	2	12'-6"	33
	H102	4	# 4	3	12'-2"	33						
							S201	42	# 5	4	8'-7"	376
	S101	61	# 5	4	8'-7"	546	S202	42	# 5	5	4'-4"	190
	S102	61	# 5	5	4'-4"	276	S203	14	# 4	6	6'-6"	61
	S103	18	# 4	6	6'-6"	78						
							U201	17	# 5	7	6'-5"	114
	U101	32	# 5	7	6'-5"	214						
							V201	21	# 4	STR	7'-2"	101
	V101	20	# 4	STR	7'-1"	95						
	REINFORCING STEEL 4049 LBS				REINFORCING STEEL 2743 LBS							
	CLASS "A" CONCRETE 22.1 CU. YDS.				CLASS "A" CONCRETE 14.8 CU. YDS.							
	HP12X53 STEEL PILES NO.					HP12X53 STEEL PILES NO.						
	TOTAL 9						TOTAL 7					

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JACKSON

COUNTY

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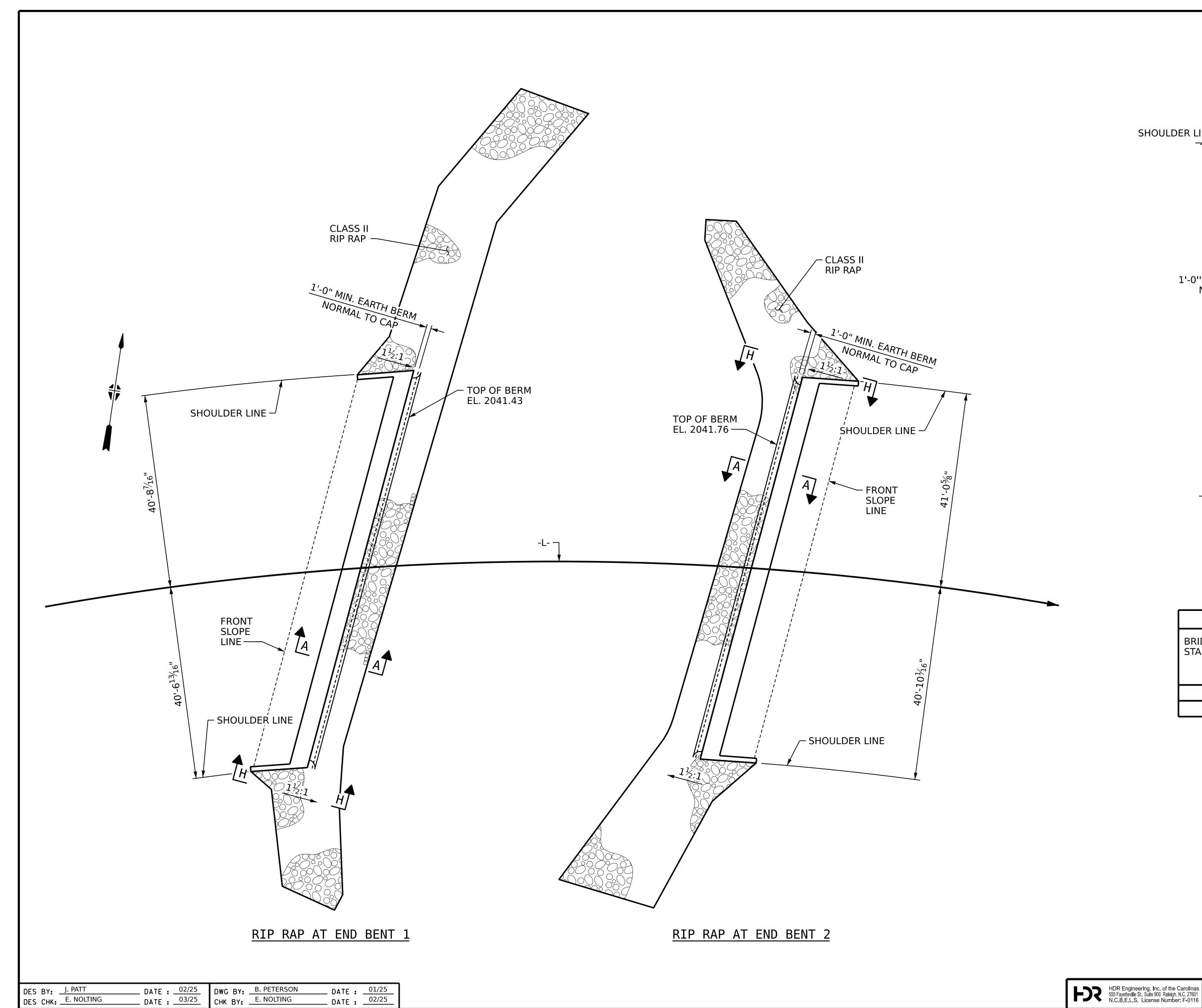
SHEET 4 OF 4

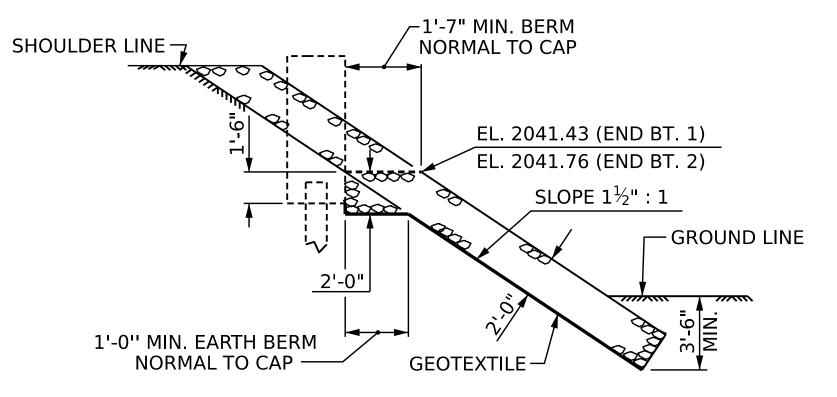
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBST	TRUC 1	ΓURE
END	BEN 7	Γ2
SECTIONS	AND	DETAILS

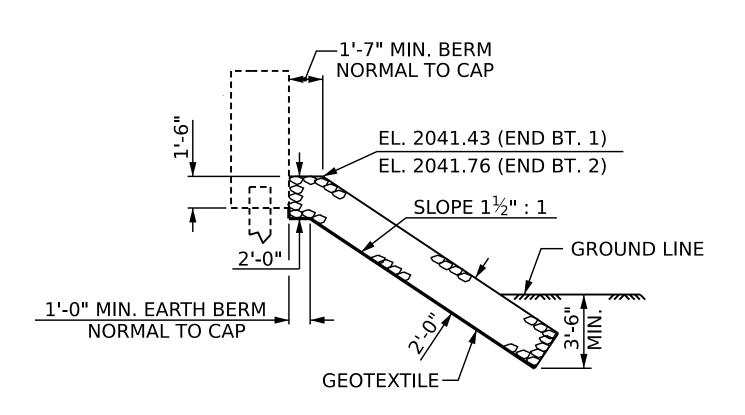
SHEET NO. S02-37 REVISIONS NO. BY: DATE: DATE: TOTAL SHEETS DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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SECTION H-H



SECTION A-A

ESTIMATED QUANTITIES						
BRIDGE @ STA. 27+24.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
END BENT 1	245	270				
END BENT 2	190	210				

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JACKSON COUNTY

STATION: 27+24.00 -L-

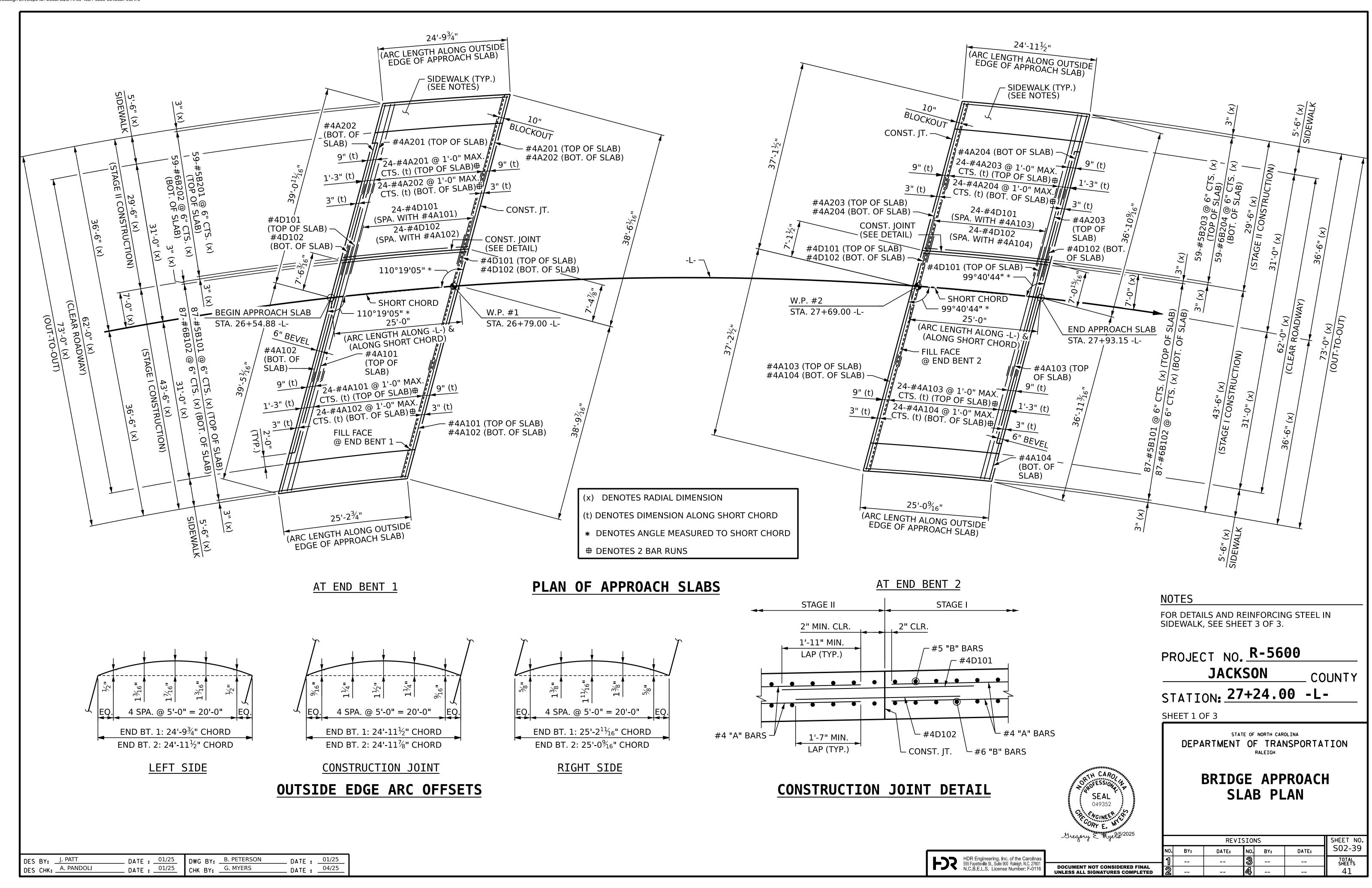
STATE OF NORTH CAROLINA

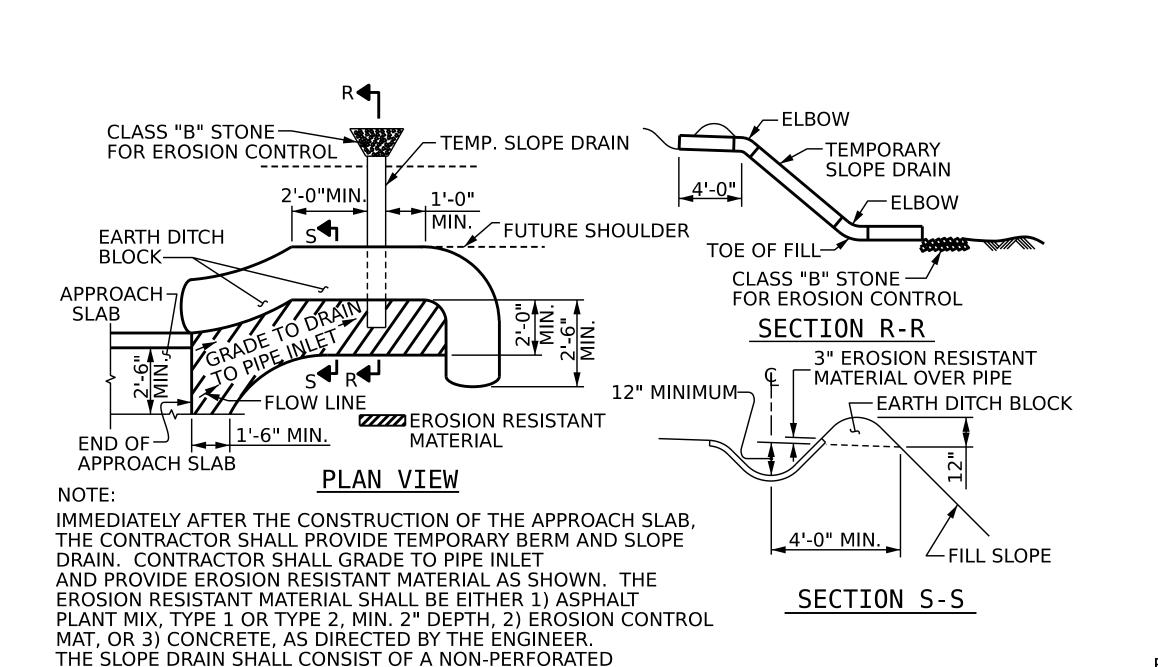
DEPARTMENT OF TRANSPORTATION

RALEIGH



RIP RAP DETAILS





TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

NOTES

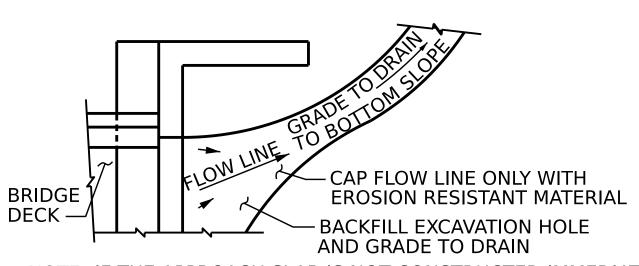
FOR BRIDGE APPROACH FILL, SEE ROADWAY PLANS.

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

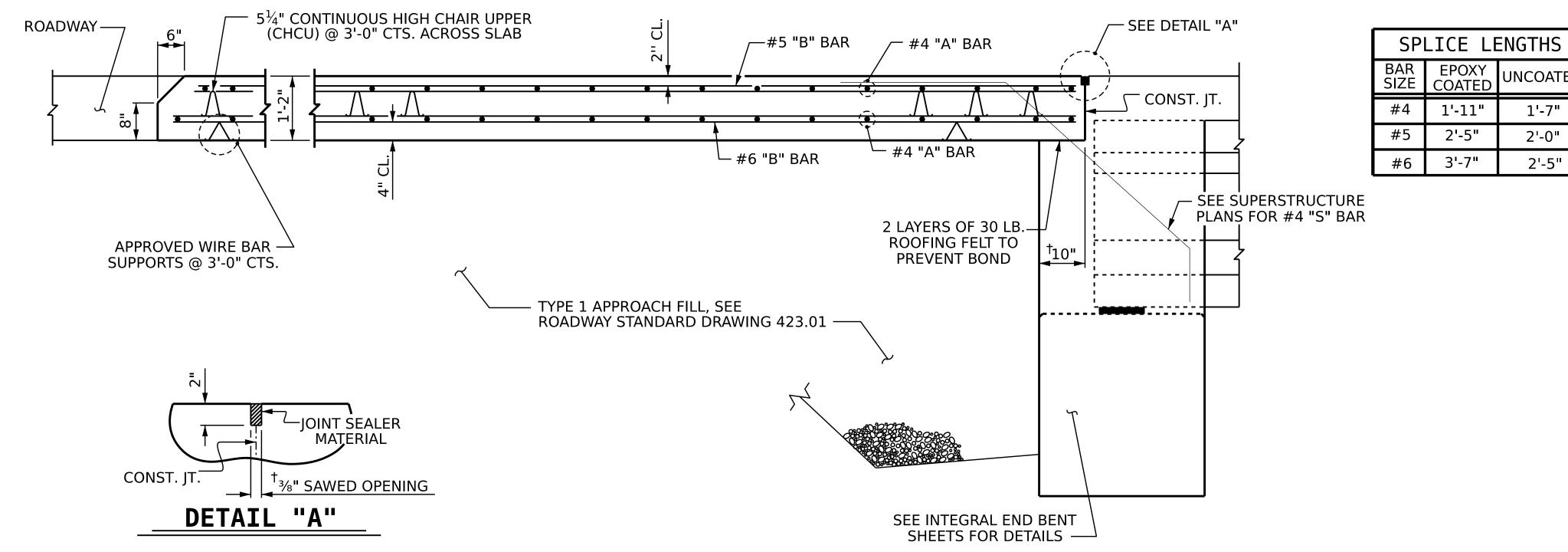
THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION "TYPE 1A - ALTERNATE APPROACH FILL (ROADWAY STD. 423.02) MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT IN LIEU OF "TYPE 1 - APPROACH FILL".



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



1'-11"	1'-7"	REINFORCING STEEL	LBS. 3244	REINFORCING STEEL	LBS. 2216
2'-5"	2'-0"				
3'-7"	2'-5"	CLASS "AA" CONCRETE APPROACH SLAB & SIDEWALK	C. Y. 50.0	CLASS "AA" CONCRETE APPROACH SLAB & SIDEWALK	C. Y. 34.9
		BAR TYPE			
		2'-0"			

ALL BAR DIMENSIONS ARE OUT TO OUT

PROJECT NO. R-5600

JACKSON

STATION: 27+24.00 -L-

SHEET 2 OF 3

BILL OF MATERIAL

STAGE II

16'-7"

24'-3"

24'-5"

5'-3"

6'-7"

3'-4"

576

570

1472

2149

82

84

4

18

LBS. 2719

LBS. 2236

550

544

1482

2171

82

80

4

18

LBS. 2715

COUNTY

4 | STR |

4 | STR |

4 | STR |

STAGE II

A204 52 # 4 STR 15'-8"

6 | STR |

4 | STR |

4 | STR | 15'-10"

24'-1"

24'-6"

24'-7"

5'-0"

6'-4"

3'-4"

#4|

A202 52 # 4 STR 16'-5"

*B201 59 # 5 STR 23'-11"

| B202 | 59 | # 6 | STR |

*G201 24 | # 4 | STR |

APPROACH SLAB AT END BENT

BAR NO. | SIZE TYPE | LENGTH | WEIGHT | BAR NO. | SIZE TYPE | LENGTH | WEIGHT

842

837

2185

3202

83

71

86

18

LBS. 3300

793

787

2185

3202

82

82

71

18

*A201 52

*B205 5

*G202 1

*U201| 8 |

LBS. 4110 REINFORCING STEEL

APPROACH SLAB & SIDEWALK C. Y. 50.2 APPROACH SLAB & SIDEWALK C. Y. 34.8

APPROACH SLAB AT END BENT 2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

*A203 52

B204 59

*G204 1

LBS. 4060 REINFORCING STEEL

|*U201| 8 | # 4

EPOXY COATED

REINFORCING STEEL

EPOXY COATED

REINFORCING STEEL

CLASS "AA" CONCRETE

*B203 59 | # 5 | STR |

*B206 5 # 4 STR

*G203 24 | # 4 | STR |

STAGE I

6 | STR

4 | STR

#4 | STR

STAGE

*A103 52 | # 4 | STR | 22'-10"

| # 6 | STR

4 | STR

*A101 52 # 4 STR

A102 52 # 4 STR

*B101 | 87 | # 5 | STR

*D101 26 | # 4 | STR

D102 26 # 4 STR

|*G101| 24 | # 4 | STR

*U101| 8 | # 4

EPOXY COATED

REINFORCING STEEL

REINFORCING STEEL

CLASS "AA" CONCRETE

A104 52 # 4 STR

*B101 87 # 5 STR

*B104 | 5 | # 4 | STR

*D101 | 26 | # 4 | STR

D102 | 26 | # 4 | STR

*G103 | 24 | # 4 | STR

*U101 8 # 4 | 1

REINFORCING STEEL

EPOXY COATED

B102 | 87 |

*G104

UNCOATED

B102 87

*B103 5

*G102 1

<u>24'-3"</u>

24'-1"

24'-1"

24'-6"

24'-10"

4'-9"

4'-1"

5'-4"

6'-8"

3'-4"

22'-8"

24'-1"

24'-6"

24'-8"

4'-9"

4'-1"

5'-0"

6'-5"

3'-4"

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> **BRIDGE APPROACH SLAB DETAILS**

SHEET NO REVISIONS S02-40 NO. BY: DATE: DATE:

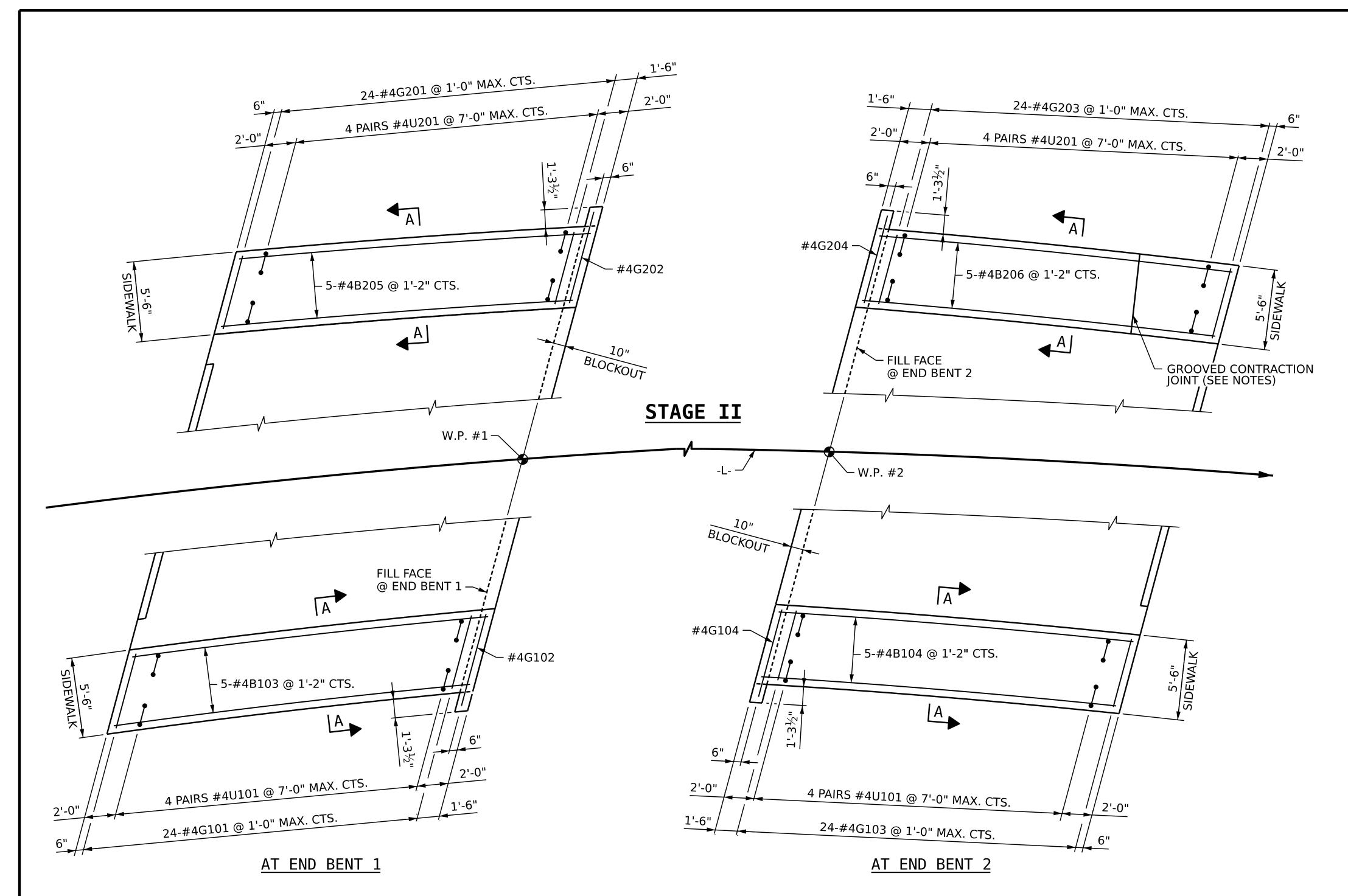
SECTION THRU SLAB

[†]NORMAL TO END BENT

DWG BY: B. PETERSON DES BY: J. PATT DATE : 01/25 DATE : 04/25 _ DATE : 01/25 CHK BY: G. MYERS DES CHK: A. PANDOLI

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STD. NO. BAS5



STAGE I
PLAN OF SIDEWALKS

NOTES

GROOVED CONTRACTION JOINTS, ½" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE SIDEWALK IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT A SPACING OF 8 FT. TO 10 FT. NO CONTRACTION JOINTS WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FEET IN LENGTH.

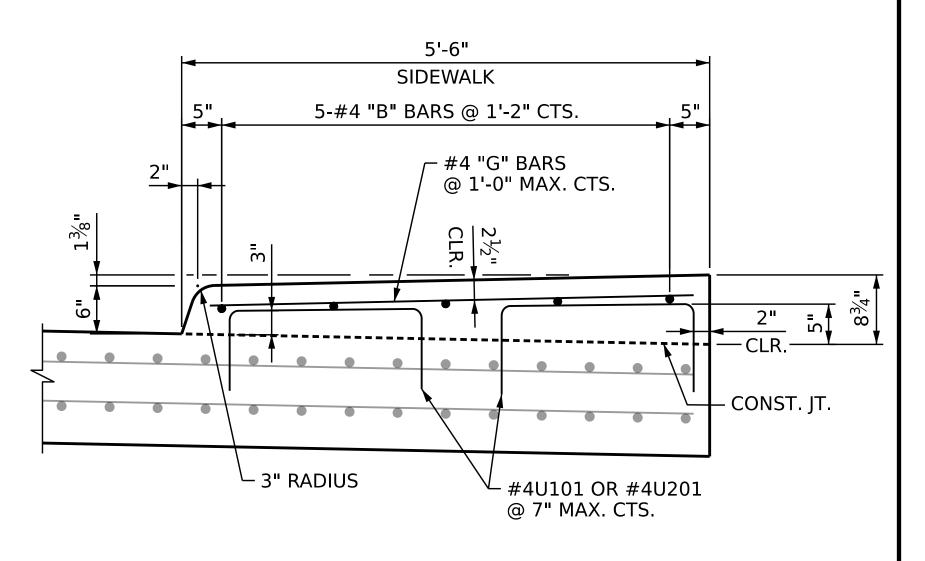
"U" BARS MAY BE PUSHED INTO GREEN CONCRETE AFTER APPROACH SLAB HAS BEEN SCREEDED OFF.

ALL REINFORCING STEEL IN THE SIDEWALKS SHALL BE EPOXY COATED.

PAYMENT FOR SIDEWALK SHALL BE INCLUDED IN UNIT PRICE FOR "BRIDGE APPROACH SLABS".

ALL TRANSVERSE DIMENSIONS ARE RADIAL TO -L-.

ALL LONGITUDINAL DIMENSIONS ARE MEASURED ALONG SHORT CHORDS.



SECTION A-A

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JACKSON

KSON COUNTY

STATION: 27+24.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALETCH

BRIDGE APPROACH SLAB CONCRETE SIDEWALK DETAILS

REVISIONS

BY: DATE: NO. BY: DATE: S02-41

-- -- 3 -- -- SHEET NO. S02-41

-- -- 4 -- -- 41

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 DES BY:
 A. PANDOLI
 DATE:
 02/25
 DWG BY:
 B. PETERSON
 DATE:
 01/25

 DES CHK:
 G. MYERS
 DATE:
 04/25
 CHK BY:
 K. DICKENS
 DATE:
 04/25

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