

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR
SECRETARY

October 27, 2021

MEMORANDUM TO: Preston Hunter, P.E.

Division 2 Engineer

ATTENTION: Jeff Cabaniss, P.E.

Division Project Development Engineer

FROM:

David Hering, L.G., P.E.

Assistant State Geotechnical Engineer - Eastern Region

David Hering

STATE PROJECT: 45569.1.2 (B-5614) COUNTY: BEAUFORT

DESCRIPTION: Bridge #9 on SR 1112 (Mouth of the Creek Rd) over Blount Creek

SUBJECT: Structure Foundation Recommendations

The Geotechnical Engineering Unit has reviewed and presents the subsurface investigation and foundation recommendations prepared by Schnabel Engineering for the above referenced project.

- □ Design Scour Memo (2) pages
- ☐ Geotechnical Foundation Table (1) page

Please call Thein T. Zan, PE or James R. Batts, P.E. at (919) 662-4710 if there are any questions concerning this memorandum.

Attachment

Website: www.ncdot.gov

FOUNDATION RECOMMENDATIONS

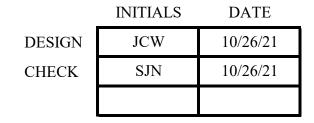
 PROJECT
 45569.1.2

 TIP NO.
 B-5614

 COUNTY
 Beaufort

 STATION
 22+57.00 -L

DESCRIPTION Bridge 9 over Blounts Creek on
SR 1112 (Mouth of the Creek Rd)





	BENT STATION	FOUNDATION TYPE	FACTORED RESISTANCE	ADDITIONAL INFORMATION		
END	17+75.88	Cap on	110 TF / / D'1	Bottom of Cap Elev. = 6.29 ft		
BENT 1	-L-	HP 12 x 53 Steel H-Piles	110 Tons/Pile	Average Estimated Pile Length = 70 ft Number of Piles/Cap = 7		
		Steel II-I lies		Bottom of Cap Elev. = 9.21 ft		
	40.07.00	Column on		Top of Drilled Pier Elev. = 1.0 ft		
BENT	18+97.00	54" Diameter	655 Tons/Pier	Point of Fixity Elev. = -49 ft		
1	-L-	Drilled Pier		Tip No Higher Than Elev. = -98 ft		
				Number of Piers/Cap = 2		
				Bottom of Cap Elev. = 11.25 ft		
BENT	20+17.00	Column on		Top of Drilled Pier Elev. = 1.0 ft		
2	-L-	54" Diameter	655 Tons/Pier	Point of Fixity Elev. = -50 ft		
	L	Drilled Pier		Tip No Higher Than Elev. = -104 ft		
				Number of Piers/Cap = 2		
				Bottom of Cap Elev. = 12.36 ft		
BENT	21+37.00	Column on		Top of Drilled Pier Elev. = 1.0 ft		
3	-L-	54" Diameter	655 Tons/Pier	Point of Fixity Elev. = -46 ft		
	L	Drilled Pier		Tip No Higher Than Elev. = -91 ft		
				Number of Piers/Cap = 2		
				Bottom of Cap Elev. = 12.53 ft		
BENT	22+57.00	Column on		Top of Drilled Pier Elev. = 1.0 ft		
4	-L-	54" Diameter	655 Tons/Pier	Point of Fixity Elev. = -50 ft		
		Drilled Pier		Tip No Higher Than Elev. = -96 ft		
				Number of Piers/Cap = 2		

BENT 5	23+77.00 -L-	Column on 54" Diameter Drilled Pier	655 Tons/Pier	Bottom of Cap Elev. = 11.78 ft Top of Drilled Pier Elev. = 1.0 ft Point of Fixity Elev. = -49 ft Tip No Higher Than Elev. = -97 ft Number of Piers/Cap = 2
BENT 6	24+97.00 -L-	Column on 54" Diameter Drilled Pier	655 Tons/Pier	Bottom of Cap Elev. = 10.09 ft Top of Drilled Pier Elev. = 1.0 ft Point of Fixity Elev. = -49 ft Tip No Higher Than Elev. = -97 ft Number of Piers/Cap = 2
BENT 7	26+17.00 -L-	Column on 54" Diameter Drilled Pier	655 Tons/Pier	Bottom of Cap Elev. = 7.47 ft Top of Drilled Pier Elev. = 1.0 ft Point of Fixity Elev. = -50 ft Tip No Higher Than Elev. = -103 ft Number of Piers/Cap = 2
END BENT 2	27+38.13 -L-	Cap on HP 12 x 53 Steel H-Piles	110 Tons/Pile	Bottom of Cap Elev. = 3.99 ft Average Estimated Pile Length = 65 ft Number of Piles/Cap = 7

(SEE NOTES ON PLANS AND COMMENTS ON FOLLOWING PAGES.)

FOUNDATION RECOMMENDATIONS NOTES ON PLANS

- 1. FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 2. DO NOT DEWATER DRILLED PIER EXCAVATIONS AT BENT NOS. 1 THROUGH 7. CLEAN THE BOTTOM OF EXCAVATIONS WITH A SUBMERSIBLE PUMP OR AN AIRLIFT. WET PLACEMENT OF CONCRETE IS REQUIRED.
- 3. SLURRY CONSTRUCTION IS REQUIRED FOR DRILLED PIERS AT BENT NOS. 1 THROUGH 7.
- 4. OBSERVE A THREE (3) MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT, END BENT AND REINFORCED BRIDGE APPROACH FILL, IF APPLICABLE, BEFORE BEGINNING APPROACH SLAB CONSTRUCTION AT END BENT NO. 1. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS.
- 5. FOR PILES, SEE PILES PROVISION AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOUNDATION RECOMMENDATIONS SPECIAL NOTES ON PLANS

1. INSTALL PERMANENT STEEL CASINGS AT INTERIOR BENTS BY VIBRATING, SCREWING OR DRIVING PERMANENT CASINGS BEFORE EXCAVATING OR DISTURBING ANY MATERIAL BELOW ELEVATIONS -40 FT (BENT NOS. 3, 4, AND 5), -41 FT (BENT NOS. 1, 6, AND 7), AND -43 FT (BENT NO. 2).

FOUNDATION RECOMMENDATIONS COMMENTS

- 1. BRIDGE END BENT SLOPES OF 1½:1 (H:V) ARE OK WITH SLOPE PROTECTION.
- 2. A SINGLE ROW WITH 5 PLUMB PILES AND 2 BRACE PILES ARE PROVIDED FOR BOTH END BENTS.
- 3. THE DESIGN SCOUR ELEVATIONS ARE -15.2 FT AT BENT NO. 1, -23.9 FT AT BENT NO. 2, -21.3 FT AT BENT NO. 3, -21.4 FT AT BENT NO. 4, -19.0 FT AT BENT NO. 5, -17.8 FT AT BENT NO. 6, AND -16.3 FT AT BENT NO. 7.
- 4. A RESISTANCE FACTOR OF 0.75 WAS USED TO REDUCE REQUIRED DRIVING RESISTANCE BASED ON THE REQUIREMENTS FOR PDA TESTING IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATION.

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bent/						Driven Piles		Predrilling for Piles*			Drilled-In Piles		
Bent No, Pile(s) ## (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Lenth per Pile FT	Scour Critical Elevation FT	Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile Exc In Soil per Pile Lin FT
End Bent 1, Piles 1-7	110	7.29	70			165							
End Bent 2, Piles 1-7	110	4.99	65			150	i						
							7						

^{*}Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile TONS	Factored Downdrag Load per Pile TONS	Factored Dead Load* per Pile TONS	Dynamic Resistance Factor	Nominal Downdrag Resistance per Pile TONS	Nominal Scour Resistance per Pile TONS	Scour Resistance Factor (Default = 1.00)
End Bent 1, Piles 1-7	110	8		0.75	6		1.00
End Bent 2, Piles 1-7	110			0.75			1.00

^{*}Factored Dead Load is factored weight of pile above the ground line.

SUMMARY OF DRILLED PIER INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pier(s) ## (e.g., "Bent 1, Piers 1-3")	Factored Resistance per Pier TONS	Minimum Pier Tip (Tip No Higher Than) Elevation FT	Required Tip Resistance per Pier TSF	Scour Critical Elevation FT	Minimum Drilled Pier Penetration Into Rock per Pier Lin FT	Drilled Pier Length per Pier Lin FT	Drilled Pier Length Not In Soil per Pier Lin FT	Drilled Pier Length In Soil per Pier Lin FT	Permanent Steel Casing Required? YES or MAYBE	Permanent Steel Casing Tip Elevation (Elev Not To Extend Casing Below) FT	Permanent Steel Casing Length* per Pier Lin FT
Bent 1, Piers 1-2	655	-98	5	-18	0.0	99			YES	-41.0	42.0
Bent 2, Piers 1-2	655	-104	10	-26	0.0	105			YES	-43.0	44.0
Bent 3, Piers 1-2	655	-91	5	-24	0.0	92			YES	-40.0	41.0
Bent 4, Piers 1-2	655	-96	10	-24	0.0	97			YES	-40.0	41.0
Bent 5, Piers 1-2	655	-97	10	-21	0.0	98			YES	-40.0	41.0
Bent 6, Piers 1-2	655	-97	5	-20	0.0	98			YES	-41.0	42.0
Bent 7, Piers 1-2	655	-103	5	-19	0.0	104			YES	-41.0	42.0

^{*}Permanent Steel Casing Length equals the difference between the ground line or top of drilled pier elevation, whichever is higher, and the permanent casing tip elevation.

SUMMARY OF PDA/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

Pi	le Driving Analyz	Pile Order Lengths			
End Bent/ Bent No	PDA Testing Required? YES or MAYBE	PDA Test Pile Length FT	Total PDA Testing Quantity EACH	End Bent/ Bent No(s)	Pile Order Length Basis* EST or PDA
End Bent 1, Piles 1-7	YES	75			
End Bent 2, Piles 1-7	YES	70			
			2		

^{*}EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

SUMMARY OF PILE ACCESSORIES

(Blank entries indicate item is not applicable to structure)

Pipe Pile Plates	Pipe Pile			
Required? YES or MAYBE	Cutting Shoes Required? YES	Pipe Pile Conical Points Required? YES	H-Pile Points Required? YES	Steel Pile Tips Required? YES
			YES	
			YES	
			14	
		MAYBE Required?	MAYBE Required? Required?	MAYBE Required? Required? YES Required? YES YES

SUMMARY OF DRILLED PIER TESTING

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pier(s) ## (e.g., "Bent 1, Piers 1-3")	Standard Penetration Test (SPT) Required? YES or MAYBE	Crosshole Sonic Logging (CSL) Required?* YES or MAYBE	Total CSL Tube Length (For All Tubes) per Pier Lin FT	Shaft Inspection Device (SID) Required? YES or MAYBE	Pile Integrity Test (PIT) Required? MAYBE
Bent 1, Piers 1-2	YES	MAYBE	503	YES	
Bent 2, Piers 1-2	YES	MAYBE	533	YES	
Bent 3, Piers 1-2	YES	MAYBE	468	YES	
Bent 4, Piers 1-2	YES	MAYBE	493	YES	
Bent 5, Piers 1-2	YES	MAYBE	498	YES	
Bent 6, Piers 1-2	YES	MAYBE	498	YES	
Bent 7, Piers 1-2	YES	MAYBE	528	YES	
TOTAL QTY:	14	7	3518	14	

^{*}CSL Tubes are required if CSL Testing is or may be required. The number of CSL Tubes per drilled pier is equal to one tube per foot of design pier diameter with at least 4 tubes per pier. The length of each CSL Tube is equal to the drilled pier length plus 1.5 ft.

PROJECT N	NO. <u>B-5614</u>	(45569.1.2)
	BEAUFORT	COUNTY
STATION:	22+5	7 -L-
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NOTES:

- 1. The Pile and Drilled Pier Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Jacob Wessell, P.E., NC PE 030395) on
- 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
- 3. The Engineer will determine the need for CSL Testing when these items may be required.



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

PILE AND DRILLED PIER **FOUNDATION TABLES**

SIGNATURE	DATE			SHEET NO.				
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL			BY:	DATE:	NO.	BY:	DATE:	TOTAL
					3			SHEETS
SIGNATURES COMPLETED					4			

 $^{= \}frac{Factored\ Resistance + \ Factored\ Downdrag\ Load + Factored\ Dead\ Load}{Dynamic\ Resistance\ Factor} + Nominal\ Downdrag\ Resistance + \frac{Nominal\ Scour\ Resistance\ Factor}{Scour\ Resistance\ Factor}$

Ò REFERENCE

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DESCRIPTION

LEGEND (SOIL & ROCK)

SOIL TEST RESULTS ROCK TEST RESULTS SITE PHOTOGRAPHS

BORE LOGS, CORE LOGS & CORE PHOTOGRAPHS

TITLE SHEET

SITE PLAN

PROFILE

SHEET NO.

5-16 17

569. S 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY _	BEAUFOR	T					
PROJECT	DESCRIPTION	N _ <i>BR</i> .	IDGE	NO.9	OVER	?	
BLOUN	TS CREEK	ON	SR1112	(MOU	J TH	OF	THE
CREEK	RD)						
SITE DES	CRIPTION _						

STATE PROJECT REFERENCE NO. 19 B-5614

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABDRATORY SAMPLE DATA AND THE IN SITU (IM-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESION DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESION INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

J. WESSELL J. HOLLAND A. STRICKLAND MID-ATLANTIC DRILLING INVESTIGATED BY <u>J.</u> HOLLAND DRAWN BY A. STRICKLAND



CHECKED BY J. WESSELL

DATE APRIL 2021

SUBMITTED BY _SCHNABEL ENG.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

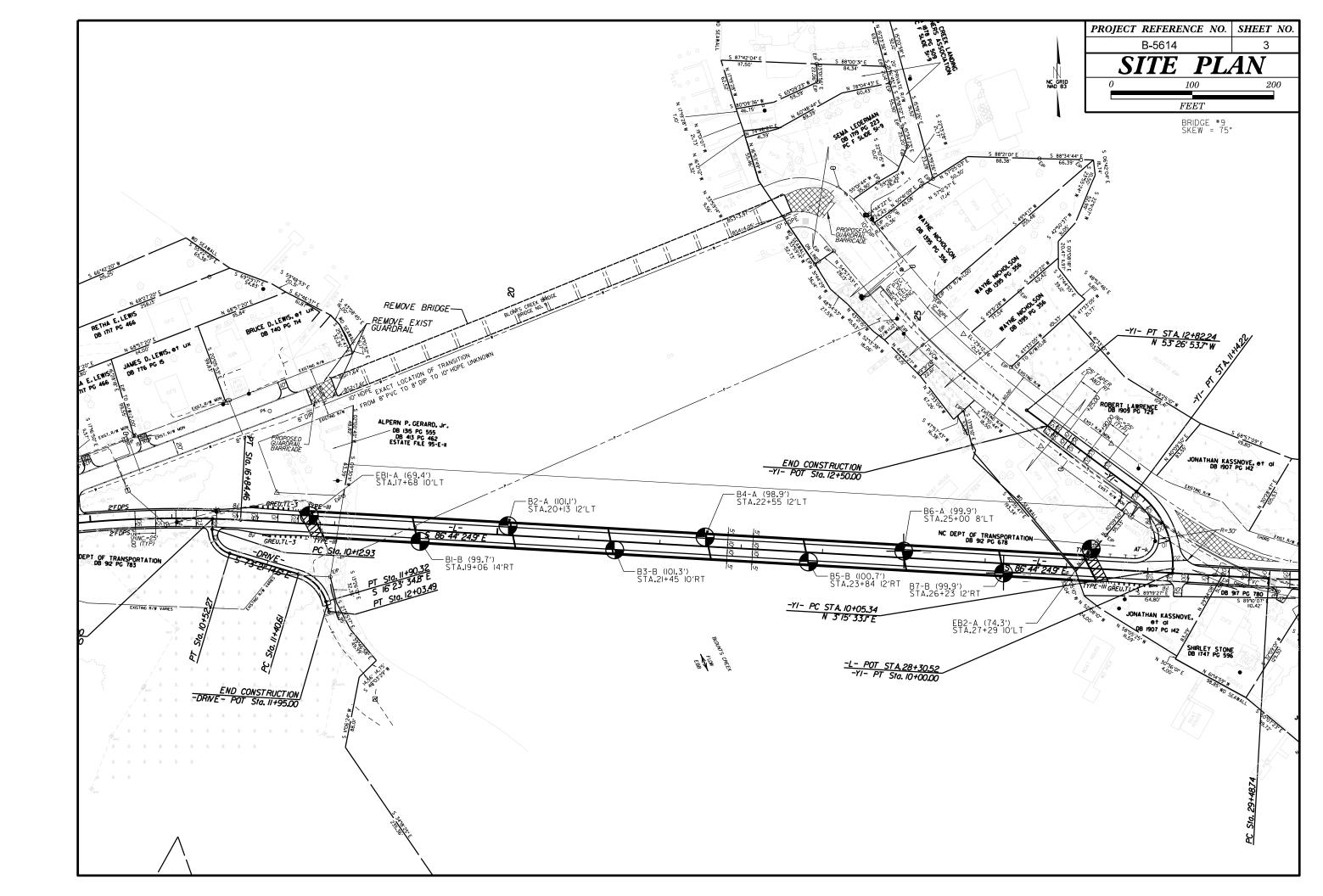
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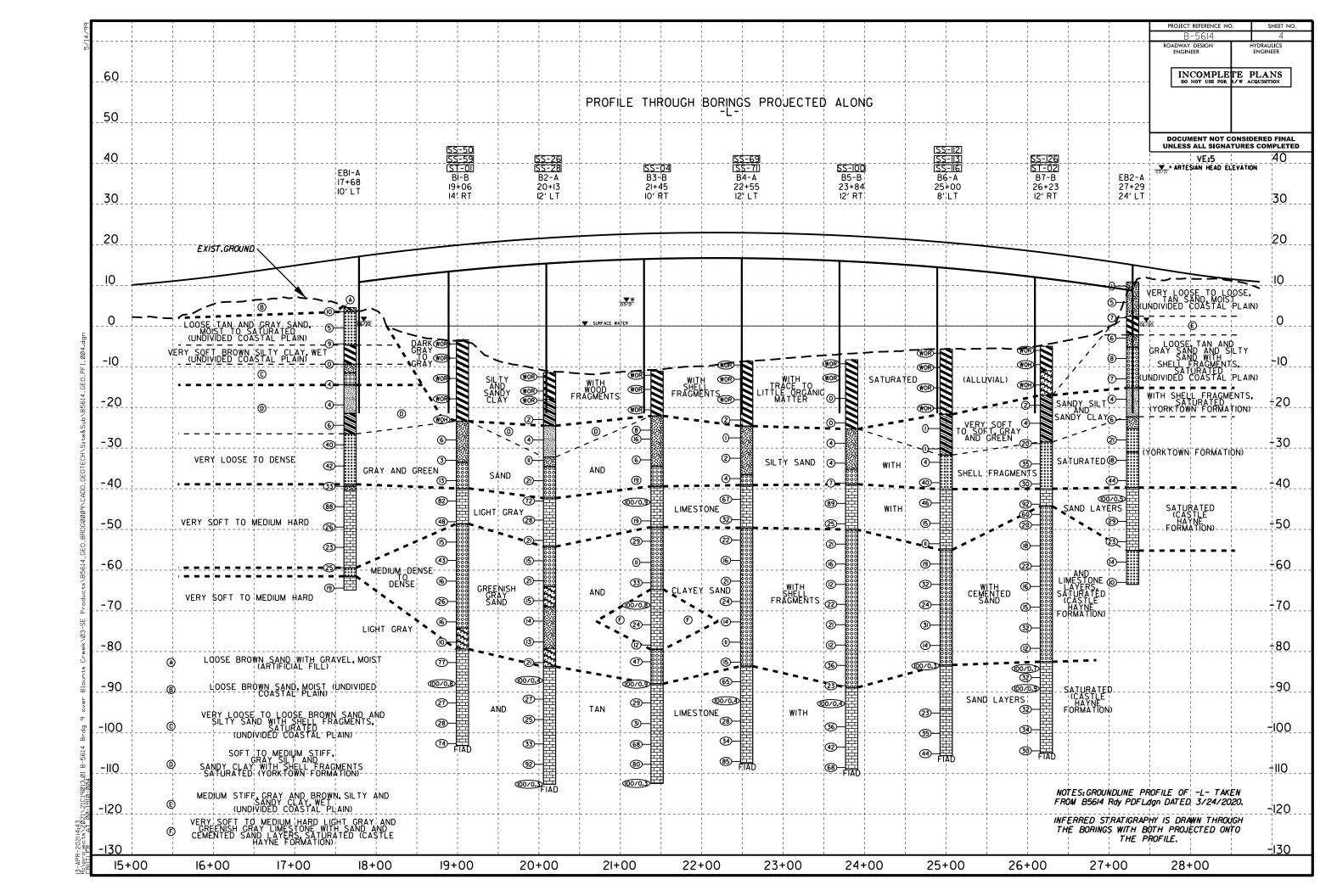
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUYIUM (ALLUY.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CONTROL MATERIALS	MINERALOGICAL COMPOSITION	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNCLOS, GHOBRU, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-0 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-LAYSTALLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.
7. PASSING	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR SILT- MUCK,	PERCENTAGE OF MATERIAL	CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 30 MX 50 MX 51 MN PEAT SOILS S	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL — — 40 MX (41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN LITTLE OR LICELY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CODITION OF A A A A A A A A A A A A A A A A A A	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE EPACS ORGANIC	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND SAND GADE GRAVE AND SAND GODES	▼ STATIC WATER LEVEL AFTER 24 HOURS	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SANU	→ STATIC WATER CEVEL HATEN 21 HOOKS VPW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP		FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED	ET	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
VERY LODGE 4.4	- 1 '	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANII AR LOOSE 4 TO 10	SOIL SYMBOL SOIL SYMBOL SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTILING IN SOILS
MATERIAL DENSE 10 TO 30 N/A MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	J WILL TORONAL ELIGIBATION ELIT	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A DIEZOMETED	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	TT>++4 ALLUVIAL SOIL BOUNDARY A FIEZUMETER SPT N-VALUE	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SHALLOW STEET OF STEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	UNDERCUT STREET DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (GSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CCL - CLAY MOD MODERATELY γ - ONLY WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC γ_d - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION OUDE FOR FIELD MOISTONE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
LL _ LIQUID LIMIT	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PLASTIC LIMITATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	BENCH MARK: N/A
	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: N/A FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: N/A FEET
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS FLIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD = FILLED IN AFTER DRILLING
	CME-55 = Cont 3/20:	INDURATION	TOP OF BORING ELEVATIONS OBTAINED FROM SURVEY-GRADE GPS SUB-CM ACCURACY DATED MARCH 10, 2021 - MARCH 24, 2021
PLASTICITY	1	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	SUBTUM ACCURACT DATED MARCH 10, 2021 - MARCH 24, 2021
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW	L_ CME-550 L_ HARD FACED FINGER BITS X-N Q2	PURRING WITH FINGER ERES NUMBEROUS CRAINS.	
SLIGHTLY PLASTIC 6-15 SLIGHT	I I VANE SHEAR TEST I ──	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE:	
COLOR	PORTABLE HOIST X TRICONE 2 15/16 STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
CULUN	X CME-55C TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST	CHARD HAMMED DI ONC DECITIDED TO RDEAV CAMPLE.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X MUD ROTARY	EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 4-1-21



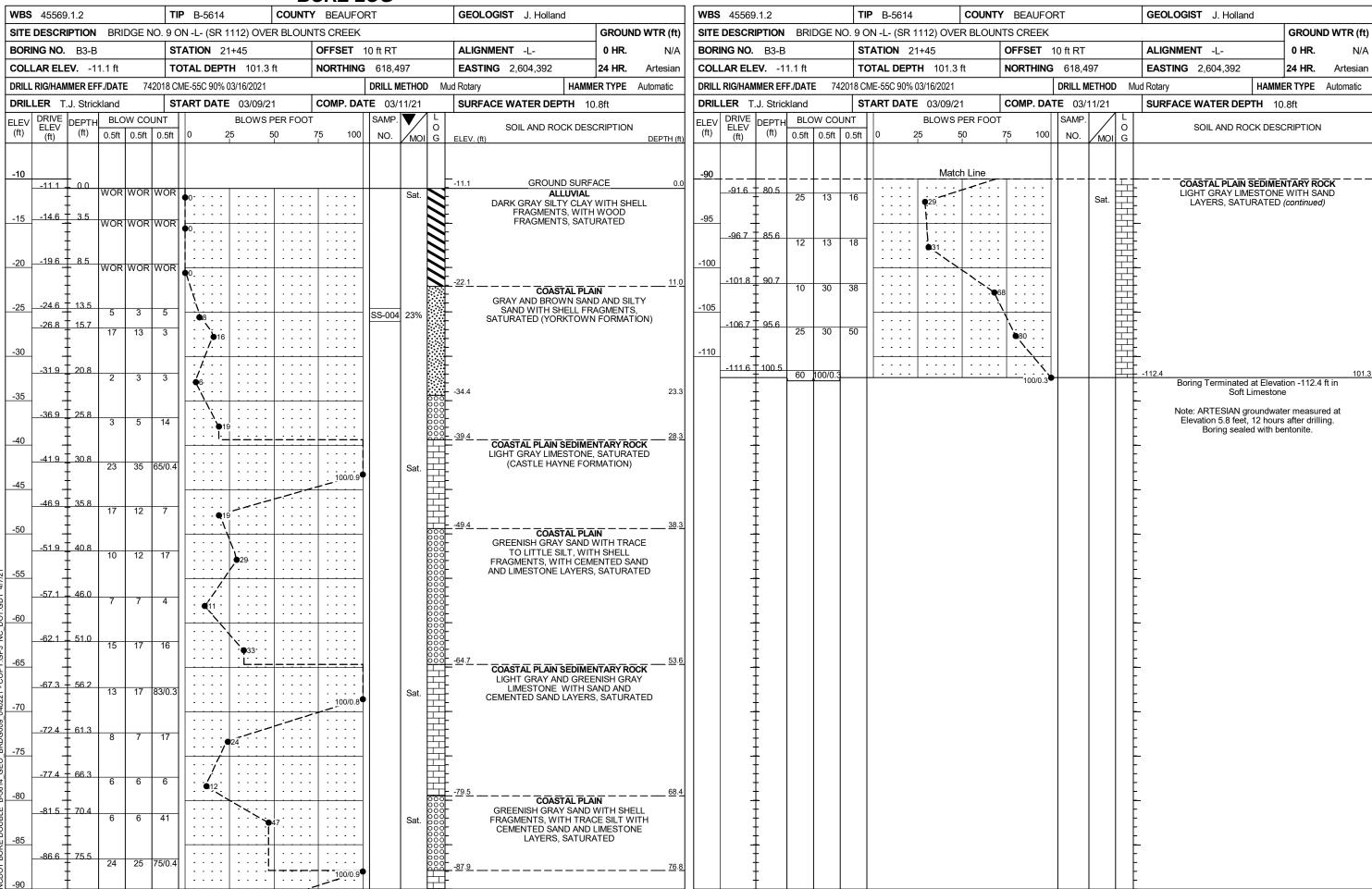


							D	<u>URE L</u>	UG				
WBS	45569	9.1.2			TI	IP B-5614	COUNTY	BEAUFO	RT			GEOLOGIST Argenbright, D. N	١.
SITE	DESCR	IPTION	BRI	DGE N	10. 9 (ON -L- (SR 1112) OVE	R BLOUN	ITS CREEK					GROUND WTR (ft
BORI	NG NO.	EB1-	·A		S	TATION 17+68		OFFSET 1	10 ft LT			ALIGNMENT -L-	0 HR. N//
OLI	AR ELI	EV. 4.	5 ft		Т	OTAL DEPTH 69.4 ff		NORTHING	618.5	30		EASTING 2,604,026	24 HR. 3.
	RIG/HAN			F GF		5 CME-45C 89% 08/19/2019			DRILL M) Mi		ER TYPE Automatic
	LER S					TART DATE 06/10/2		COMP. DAT			- 1410	SURFACE WATER DEPTH N	
	DRIVE	1		ow col			PER FOOT		SAMP.	10/20	1 L T	SORFACE WATER BEFTH IN	A
LEV (ft)	ELEV (ft)	DEPTH (ft)	0.5ft		0.5ft	-		75 100	NO.	MOI	O G	SOIL AND ROCK DES	CRIPTION DEPTH
5													
	4.5	0.0	6	6	4	10 - 10				М		_4.5 GROUND SURF - 3.5 ARTIFICIAL FI	
		‡				- -				•	0000	BROWN SAND WITH GR	
0	0.5	4.0	3	2	3	$ j' \cdots \cdots $				0.1		TAN AND GRAY SAND,	MOIST TO
		+		_		∮ 5				Sat.	0000	SATURATED (UNDIVIDE PLAIN)	D COASTAL
	-3.4	7.9	_	7] :/: : : : : : :						•	
-5	-	‡	5	'	2	9						4.7 BROWN SILTY CLA	Y. WET
		‡										•	,
	-8.4	12.9	1	0	0					w		- 9.4	1
10	-	+				0		 		VV		BROWN SAND AND SILT'11.5 SHELL FRAGMENTS, S	Y SAND WITH
		F										- 11.5 SHELL FRAGINENTS, S	ATURATED
15	-13.4	17.9	2	2	2		: : : :			Sat.	000	- 14.5	1
10	-	‡				 						COASTAL PLA GRAY SILT WITH SHELL	
	-18.4	22.9										WET (YORKTOWN FO	
20	-10.4		2	2	2	1 4: : : : : : :				W	De la Contraction de la Contra	• -	
	-	Ŧ				1						- 21.5	
	-23.4	27.9] 🕯 : : : : : : : :						GRAY SANDY CLAY W FRAGMENTS, V	
25	_	‡	2	3	3	♦ 6				W		-	
		<u> </u>										26.5 - GRAY SAND, SATU	
	-28.2	32.7	15	20	20	`><				Cat	0000	GIVAT SAND, SATO	IVATED
30	-	Ŧ	"	20		40		1		Sat.	0000	-	
		‡				:::: :::					0000	•	
	-33.4	37.9	19	22	20	1					0000	• •	
35	-	t				● 42		 				-	
		40.0				· · · · · ·/· ·					0000	•	
40	-38.4	42.9	7	21	12	33	: : : :			Sat.		-38.9 -39.4 COASTAL PLAIN SEDIME	NTARY ROCK ~_4
	-	‡					++	 - 				LIGHT GRAY LIMESTONE (CASTLE HAYNE FOR	, SATURATED 🖟 4
	-43.4	47.9									臣	COASTAL PLA	IN
! 5			34	50	38	1		88		Sat.		- GRAY SAND WITH SHELL - SATURATED	
		<u> </u>						1::::			井	COASTAL PLAIN SEDIME	NTARY ROCK
	-48.4	52.9	20	15	11	: : : : : : : :					臣	LIGHT GRAY LIMESTONE	, SATURATED
50	-	‡	28	15	11	26					岸	-	
	:	‡					: : : :				井	•	
	-53.4	57.9	10	11	12	1	: : : :	: : : :			岸	• •	
55	-	‡				■ 2 3		+			旪	-	
	F0.4	1									臣	•	
30	-58.4	62.9	36	17	8	25	: : : :			Sat.		59.4	6
	-	‡				 						– COASTAL PLA 61.5 GRAY SAND, SATU	
	-63.4	67.9] ::::/ ::::	: : : :	: : : :			口	COASTAL PLAIN SEDIME	NTARY ROCK
	JU.4	- "."	12	8	11	19]	Sat.	田	LIGHT GRAY LIMESTONE	6
	-	+									F	Boring Terminated at Eleva Very Soft Limest	
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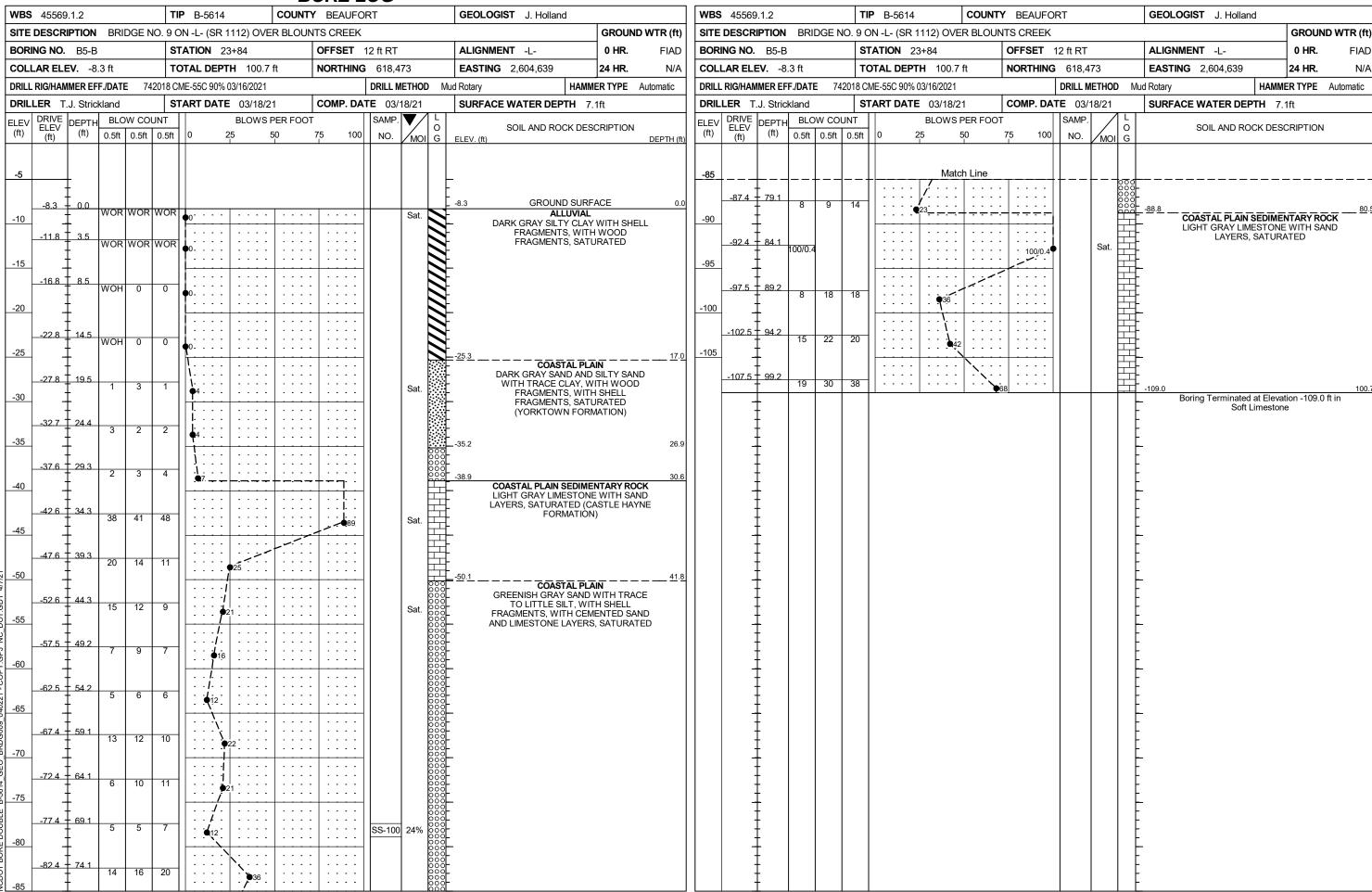


		ORE LUG					
WBS 45569.1.2	TIP B-5614 COUNT	Y BEAUFORT	GEOLOGIST J. Holland	WBS 45569.1.2	TIP B-5614 COUN	TY BEAUFORT	GEOLOGIST J. Holland
SITE DESCRIPTION BRIDGE NO	9 ON -L- (SR 1112) OVER BLOU	NTS CREEK	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO.	9 ON -L- (SR 1112) OVER BLOU	JNTS CREEK	GROUND WTR (ft)
BORING NO. B1-B	STATION 19+06	OFFSET 14 ft RT	ALIGNMENT -L- 0 HR. FIAD	BORING NO. B1-B	STATION 19+06	OFFSET 14 ft RT	ALIGNMENT -L- 0 HR. FIAD
COLLAR ELEV3.4 ft	TOTAL DEPTH 99.7 ft	NORTHING 618,498	EASTING 2,604,162 24 HR. N/A	COLLAR ELEV3.4 ft	TOTAL DEPTH 99.7 ft	NORTHING 618,498	EASTING 2,604,162 24 HR. N/A
DRILL RIG/HAMMER EFF./DATE 7420		DRILL METHOD MU	<u> </u>	DRILL RIG/HAMMER EFF./DATE 7420		DRILL METHOD M	
DRILLER T.J. Strickland	START DATE 03/12/21	COMP. DATE 03/15/21	SURFACE WATER DEPTH 3.0ft	DRILLER T.J. Strickland	START DATE 03/12/21	COMP. DATE 03/15/21	SURFACE WATER DEPTH 3.0ft
		 	SON AGE WATER BEFTT 5.01		L		SORT AGE WATER DEFITT 5.010
ELEV (ft) DRIVE (ELEV (ft) DEPTH BLOW COUN (ft) 0.5ft 0.5ft 0	5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUN (ft) 0.5ft 0	.5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION
					Match Line		
			-	<u>-80</u>			COASTAL PLAIN SEDIMENTARY ROCK LIGHT GRAY AND GREENISH GRAY
-3.4 + 0.0 WOR WOR W	OR 1	· · · · ·	-3.4 GROUND SURFACE 0.0 ALLUVIAL	+	44	Sat.	LIMESTONE WITH SAND LAYERS, SATURATED (continued)
-6.9 3.5 WOR WOR W		: :::::	- DARK GRAY SILTY CLAY WITH WOOD FRAGMENTS, WITH TRACE ORGANICS,	-85 T 83.2			- -
-0.9 T S.S WOR WOR W	OR		SATURATED	53 80 20	70.3	100/0.8	
-10			-	-90			
-11.9 8.5 WOR WOR W	OR 0	: :::: 📑		-91.6 † 88.2 11 11 1	16 27		-
-15		· ···· 	-	-95			<u>-</u>
-16.9 T 13.5 WOR WOR W	OR 0			<u>-96.6 † 93.2 </u>	15		
-20		<u> </u>	-	-100			<u>-</u>
-22.0 18.6 WOH WOH W	OH		-23.4 20.0	<u>-101.6 + 98.2 </u>	43	- · · · · ·	- -103.1 99.
-25	10	28%	COASTAL PLAIN - GRAY AND DARK GRAY SAND AND				Boring Terminated at Elevation -103.1 ft in Soft Limestone
-27.0 23.6 2 4			SILTY SAND WITH SHELL FRAGMENTS, SATURATED (YORKTOWN FORMATION)				- - <u>Other Samples:</u> - ST-01 (20.1 - 22.1)
-30	6		,				- ST-01 (20.1 - 22.1)
-32.0 28.6	<u> </u>		-				
-35	1 •3 · · · · · · · · · · ·	SS-050 58% SS-050	-33.5 30.1				-
37.0 33.6			-				-
4 7	6		20.0				
-40 38.6			COASTAL PLAIN SEDIMENTARY ROCK				-
44 42 6	0	82 · · Sat.	LIGHT GRAY LIMESTONE, SATURATED (CASTLE HAYNE FORMATION)				
72 -45			-				<u> </u>
-47.1 <u>43.7</u> 19 26 :	22		_48.6 45.2				
-50 -52.1 -48.7 3 7			GREENISH GRAY SAND WITH TRACE				
2 -52.1 <u>48.7</u> 3 7	8		SILT, WITH SHELL FRAGMENTS, WITH CEMENTED SAND AND LIMESTONE LAYERS, SATURATED				<u>.</u> -
3 / 9 -55 -56.6 53.2 -11 -11 -11		-	-				<u>-</u>
'	32						
000000000000000000000000000000000000000			_				
8 -61.7 + 58.3	8						-
a	\\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	- · · · ·					<u>.</u> -
0 -66.7 -63.3		-	-				-
7 13	3 26						
			-				<u></u>
-/1./ + 68.3 6 8	8						
-71.7			-74.2				<u>-</u>
9 -76.7 + 73.3 5 5 5	5	1 188-0591 33% 🐼 🔊	SAND AND LIMESTONE LAYERS, SATURATED				
ŌZ -80 T			-79.4 76.0				

WBS 45569.1.2 TIP B-5	614 COUNTY BEAUFORT	GEOLOGIST J. Holland		WRS	45569	1.2		т	IP B-5614 COUN	ITY BEAUFORT		GEOLOGIST J. Holland	
SITE DESCRIPTION BRIDGE NO. 9 ON -L- (GEOEOGIST 3. Holland	GROUND WTR (ft)	—			BRIDGI		ON -L- (SR 1112) OVER BLO			GEOLOGIST 5. Floriand	GROUND WTR (ft)
BORING NO. B2-A STATION		ALIGNMENT -L-	⊣ `′l		ING NO.		DINIDGI		TATION 20+13	OFFSET 12 ft L	т	ALIGNMENT -L-	0 HR. FIAD
	EPTH 101.1 ft NORTHING 618		⊣ 1		LAR ELE		5 ft		OTAL DEPTH 101.1 ft	NORTHING 618		EASTING 2,604,270	24 HR. N/A
DRILL RIG/HAMMER EFF./DATE 742018 CME-55C			MER TYPE Automatic	_					ME-55C 90% 03/16/2021		L METHOD		IER TYPE Automatic
	ATE 03/11/21 COMP. DATE 0				LER T.		-		TART DATE 03/11/21	COMP. DATE		SURFACE WATER DEPTH 12	
ELEV DRIVE DEPTH BLOW COUNT					DRIVE		BLOW (BLOWS PER FO			-	-
(ft) ELEV (ft) 0.5ft 0.5ft 0.5ft 0		MOI G ELEV. (ft)	SCRIPTION DEPTH (ft)	(ft)	ELEV (ft)	(ft)		5ft 0.5ft	0 25 50	75 100 NO	D. MOI C	00.27.1.2 1.00.1.220	CRIPTION
-10				-90					Match Line				
-11.5 T 0.0 WOR WOR WOR		11.5 GROUND SURF			-90.7	79.2	25 1	2 15	27			COASTAL PLAIN SEDIME LIGHT GRAY AND GREE	
WOR WOR WOR O		Sat. DARK GRAY SILTY CLAY	Y WITH TRACE]							LIMESTONE WITH LAYERS, SATURATED	SAND
-15 -15.0 3.5 WOR WOR WOR		ORGANICS, SATU	JRATED	-95	-95.7	84.2	15						(John Hada)
-17.3 - 5.8 WOR WOR WOR		-18.0	6.5]	[15 1:	2 13	25			<u> </u>	
-20		GRAY SANDY CLAY W FRAGMENTS, SAT		-100	_				\			_	
-22.0 10.5					-101.7	90.2	04	0 10	1				
WOR WOH 2		Sat24.5	42.0		-	-	21 2	0 13	33			<u> </u>	
-25		COASTAL PL		-105	│	-						COASTAL PLAIN SEDIME LIGHT GRAY AND GREIF LIMESTONE WITH LAYERS, SATURATED LAYERS, SATURATED LAYERS, SATURATED LAYERS, SATURATED LAYERS, SATURATED LAYERS, SATURATED	
-27.0 15.5 1 1 3		GRAY SILT WITH LITTL SATURATED (YORKTOW			-106.7	95.2	24 4	5 47		92			
		7 - 00 E		-110	_							<u> </u>	
-32.0 20.5		-32.2	20.7		-111.8	100.3	05 100	10.0				-112.6	101.1
3 6 5	11 -	GRAY AND BROWN SAN 34.5 SAND WITH SHELL FF	ND AND SILTY		-		25 100	/0.3		100/0.3		Boring Terminated at Eleva	
-35		SATURATE			-	-						Soft Limeston	ne
37.0 25.5 4 9 12	.\.\.\.\.\.\.\.\.\.\.\.\.\.\.\.\.\.\.\	000 			_							_	
40 -40	. []	000			-							-	
-42.0 30.5		000 000 000 000 -42.3	30.8		-	-						_	
14 32 40	72	COASTAL PLAIN SEDIME LIGHT GRAY LIMESTONE	ENTARY ROCK		-							_	
<u>-45</u> <u> </u>		(CASTLE HAYNE FO			-							_	
-46.7 † 35.2 16 17 11 · ·		Sat.			-							_	
50]/				-	-						-	
-51.7 + 40.2	: :/ : : : : : : : : : : : :				-	-						_	
	$\begin{bmatrix} lacksquare 21 \dots & & & & & & & & & & & & & & & & & & $	-54.2	42.7		-	-						-	
-55	./. 	COASTAL PL	AIN		-	-						_	
-56.7	15	Sat. Sat. Sat. Sat. Sat. Sat. Sat. Sat.			-							‡	
6 -60	<u>'</u>	000 000 000			-							-	
61.6 + 50.1 9 10 11					-							‡	
	. 9 21	LIGHT GREENISH GRAY	52.5		-							ţ	
-65 - 55.0	7.	WITH CEMENTED S	SAND AND		-							F	
8 6 9	15	Sat. LIMESTONE LAYERS,	OATUKATED		-							F	
3 -70 +		GREENISH GRAY SILTY			-	<u> </u>						_	
-71.5 + 60.0 8 7 7		Sat. CEMENTED SAND AND LAYERS, SATUR			-							‡	
8 7 7					-							-	
-76.6 T 65.1					-							F	
8 6 7	13.	70.0			-							-	
-80 +	<u> </u>	LIGHT GREENISH GRAY	CLAYEY SAND 67.7		-							<u> </u>	
81.7 + 70.2		WITH SHELL FRAGMENT	S, SATUKATED		-							-	
-85		COASTAL PLAIN SEDIME	ENTARY ROCK 72.2		-							-	
-85.7 74.2 34 36 64/0.4		LIGHT GRAY AND GRE	ENISH GRAY		-							F	
	100/0.4	LAYERS,SATUR			-							‡	
<u> </u>	·· ····											t	



					_		1	ORE			1			¬ [T	_				
	45569.1.2				P B-5614		l	Y BEAUF			GEOL	OGIST J. Holland			3S 455					B-5614		BEAUFO	RT		GEOLOGIST J. Holland	<u> </u>
SITE	DESCRIPTION	DN BF	RIDGE 1	VO. 9 O	N -L- (SR	1112) OVE	ER BLOU	NTS CREEK	<				GROUND WTR (ft) SIT	TE DESC	CRIPTION	BRID	GE NO	0. 9 01	N -L- (SR 1112) OV	/ER BLOUN	TS CREEK				GROUND WTR (ft)
BOR	ING NO. B4	l-A		ST	ATION 2	2+55		OFFSET	12 ft LT		ALIGN	MENT -L-	0 HR. FIAD	ВС	RING N	O . B4-A			STA	ATION 22+55		OFFSET	12 ft LT		ALIGNMENT -L-	0 HR. FIAD
COL	LAR ELEV.	-8.6 ft		TC	TAL DEP	TH 98.91	ft	NORTHIN	IG 618,	504	EASTI	NG 2,604,512	24 HR . N/A	CC	LLAR E	LEV. -8.	6 ft		TO	TAL DEPTH 98.9	ft	NORTHING	618,504		EASTING 2,604,512	24 HR . N/A
DRILI	. RIG/HAMMER	EFF./DA	TE 74	42018 CN	лЕ-55С 90%	03/16/2021			DRILL I	METHOD	Mud Rotary	HAN	MMER TYPE Automatic	DR	ILL RIG/H	AMMER EFF	./DATE	742	2018 CM	E-55C 90% 03/16/202	<u>'</u> 1		DRILL METHOD) Mu	ud Rotary HAMI	MER TYPE Automatic
	LER T.J. St					E 03/17/2	P1	COMP. D				ACE WATER DEPTH		⊣		T.J. Strick				ART DATE 03/17		COMP. DA	TE 03/17/21		SURFACE WATER DEPTH 8	
ELEV	DRIVE DEP		ow co		1		PER FOO			. 🔻 🖊	L			ELE				v cou			S PER FOOT		SAMP.	1 L T		
(ft)	ELEV (ft)) 0.5f	t 0.5ft		0		50	75 100		1 1/ 1	O G ELEV. (ft)	SOIL AND ROCK DI	ESCRIPTION DEPTH (I I (ft	ELE'	DEPTH (ft)	0.5ft (0.5ft	0.5ft	0 25		75 100	1 /	O G	SOIL AND ROCK DES	SCRIPTION
	(14)									IVIOI	J ELEV. (II)		DEFIN	4 —	(1.5)								7 IVIOI	Ĭ		
-5											_			-8		+	+	-4-	+	Ma Ma	tch Line		++	┨╾╂	COASTAL PLAIN SEDIMI	
	‡										_				-86.4	4 + 77.8 +	23	47	18				Sat.	F	LIGHT GRAY LIMESTON	IE WITH SAND
	-8.6 + 0.0		RIWOR	WOR	<u> </u>	1	1	.		Sat.	8.6	GROUND SUI		∸l I		‡						\:::::		臣	LAYERS, SATURATE) (continued)
-10	+	""	T WOI		P 0		+		+	Sat.) -	DARK GRAY SILTY CL	AY WITH SHELL	-90		1 _ 82.5						 \		H	_	
	-12.1 I 3.5	5 WO	R WOR	WOR	: : : :			.			7	FRAGMENTS, SA	TURATED			Ŧ	100/0.4					100/0.4	,		- -	
15	‡	""	T WOI		₱ 0· · · ·			.			3				_	‡								H	- -	
-15						 		: : : : :	$\exists 1$		\$			-98		1 + 87.5								井井	-	
	-17.1 - 8.5	5 WO	R WOR	WOR			1:::	: : : : :			3					± 1	12	15	13	•28	.	: : : :			<u>.</u>	
-20					T o: : : :			. : : : :			7			-10	00	Ŧ					. : : : :				-	
	‡	_			1	: : : :		: : : : :	7		5					0 92.4	14	18	16	.\	: : : : :			岸	<u>-</u> -	
	-22.1 <u>13.</u>	5 1	1	1	2	: : : :					\$					‡	' -	"	10	34				田	<u>.</u> -	
-25	±				T		1	-	∐		-24.6	COASTAL P	<u> </u>	<u>-10</u>	15	<u>†</u>									- -	
	-26.5 + 17.	9			j			.			<u></u>	GRAY AND DARK GR	RAY SAND AND		-106	0 97.4	17	37	48		$\cdot \mid \cdot \cdot$	+< <u></u> . ⋅		井	-	
	‡	1	0	1	# 1			: : : : :	SS-069	24%	·	SILTY SAND, WITH SHE SATURATED (YORKTO)				+	+	+				₹85			107.5 - Boring Terminated at Elev	98. ation -107.5 ft in
-30	‡				i::::	1 : : : :	1 : : :		_		<u></u>	SATORVILLE (FORUMO)	VIVI OTAVI (TION)			‡									Soft Limesto	ne
	-31.5 22.	9	1	1	1			.			_					± 1								<u> </u>	-	
	l Ŧ	'	'	'	P 2		: : :	.								ŦI								F	- -	
-35	‡				1	1	1		_		:: <u>-</u>					‡									- -	
	-36.5 = 27.	9 2	1	3	1				SS-07	1 26%	-36.5 -		27	9		‡									- -	
	+	-			P ⁴ · · ·			.	33-07	20%	00- 00- 0039.1		30	5		+									-	
-40	1 7										_	COASTAL PLAIN SEDIN LIGHT GRAY LIMESTON		7		Ŧ								F	- -	
	-41.6 ‡ 33.	0 23	37	30			: : : '			Sat.	王	(CASTLE HAYNE F				‡									- -	
45	‡															‡									- -	
-45	+					 	1		\dashv		_					†								-	-	
	-46.6 † 38. †	10	14	18		32		.			-					+								-	-	
50 -50					: : : :	/		.			<u>-49.6</u>		41	<u>o</u>		Ŧ								F	-	
///4	-51.6 T 43.					/::::	1		71	000		COASTAL P GREENISH GRAY SAN	'LAIN ID WITH SHELL			‡									-	
GDT	<u> </u>	8	8	14	: : : :	22	: : :	: : : : :		Sat. o	00- 00-	FRAGMENTS, WITH CE AND LIMESTONE LAYE	EMENTED SAND			‡									<u>.</u> -	
<u>-55</u>	±				<u> </u>		ļ	-	∐ I	000	5 ŏ -	AND LINESTONE LAYE	NO, SATURATED			<u>†</u>								<u> </u>	- -	
-COPY.GPJ NC_DOT.GDT 4/721	-56.6 48.	م ا م		\perp	$ \cdot\cdot\cdotar{\iota} $					000	00 00 00 00 00 00 00 00					<u>†</u>								F	-	
I	‡	5	8	8	1. 16		: : :	.		2000	00 -					Ŧ l								F	-	
60 -60	‡				ļ <u>i</u> .	1::::	1:::	1	_	1000	56 -					‡									- -	
Θ̄	-61.7 ‡ 53.	1 20	13	۵	1:::;			: : : : :		000	ŏŏ-					‡									- -	
221 .		20	13		: : • 🛉) 21 		.		000	00- 00- 00-					<u>†</u>								-	<u>-</u> -	
BORE DOUBLE B-5614 GEO BRDG009 040221	‡					1	+		+1	000	5 6 -					Ŧ								F	- -	
6000	-66.7 ‡ 58.	1 9	12	12	: : : :	<u>j</u>	: : :	: : : : :		000	50-					‡									- -	
IRDG	‡				: : : : ;	24	: : :	: : : : :		0000	0-					‡									- -	
<u>-70</u>	+				<i>/</i>	+	+		+1	000	, , , , , , , , , , , , , , , , , , ,					+								-	_	
B	<u>-71.7 † 63.</u>	1 6	6	8	/ .			.		000	200 200 200 200 200 200 200 200 200 200					Į Į								F	- -	
2614	‡				T. 14.		1 : : :	: : : : :		000	00 -					‡									<u>.</u> -	
m -/5	+				· . · .	1	1 : : :	: : : : :	$\exists 1$	000	ŏŏ <u>–</u>					†								-	<u>-</u> -	
UBL	-76.7 T 68.	1 6	5	6	1.1			.		000	000					+								F	-	
-80	‡				: ','' :					000	00 - 00 -					Ŧ l								F	- -	
S S S S S S S S S S S S S S S S S S S	-81.6 T 73.	١			1		1		71	000) - 					‡									<u>-</u> ·	
DOT B	<u> </u>	7	8	7	15			: : : : :		1 0	20 - 2083.6		75			‡									<u>.</u>	
ON -85					!=	+	 1	-		ľ				 		+								F	-	
				-																						



		BURE LUG	T		T		T
WBS 45569.1.2		TY BEAUFORT	GEOLOGIST J. Holland	WBS 45569.1.2		TY BEAUFORT	GEOLOGIST J. Holland
SITE DESCRIPTION BRIDGE NO	. 9 ON -L- (SR 1112) OVER BLOU	INTS CREEK	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO.	· '	JNTS CREEK	GROUND WTR (ft)
BORING NO. B6-A	STATION 25+00	OFFSET 8 ft LT	ALIGNMENT -L- 0 HR. FIAD	BORING NO. B6-A	STATION 25+00	OFFSET 8 ft LT	ALIGNMENT -L- 0 HR. FIAD
COLLAR ELEV5.7 ft	TOTAL DEPTH 99.9 ft	NORTHING 618,486	EASTING 2,604,756 24 HR. N/A	COLLAR ELEV5.7 ft	TOTAL DEPTH 99.9 ft	NORTHING 618,486	EASTING 2,604,756 24 HR. N/A
DRILL RIG/HAMMER EFF./DATE 7420	18 CME-55C 90% 03/16/2021	DRILL METHOD SF	PT Core Boring HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE 7420	18 CME-55C 90% 03/16/2021	DRILL METHOD S	SPT Core Boring HAMMER TYPE Automatic
DRILLER T.J. Strickland	START DATE 03/18/21	COMP. DATE 03/23/21	SURFACE WATER DEPTH 5.9ft	DRILLER T.J. Strickland	START DATE 03/18/21	COMP. DATE 03/23/21	SURFACE WATER DEPTH 5.9ft
ELEV DRIVE DEPTH BLOW COUN	1	OT SAMP.		ELEV DRIVE DEPTH BLOW COUN	<u> </u>	OT SAMP.	
(ft) ELEV (ft) 0.5ft 0.5ft (75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	(ft) ELEV (ft) 0.5ft 0.5ft 0		75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION
					Match Line		
-5 -5.7 0.0 WOR WOR V	(OD		-5.7 GROUND SURFACE 0.0	-85	- Watch Line		COASTAL PLAIN SEDIMENTARY ROCK
	OR 0	Sat.	ALLUVIAL DARK GRAY SILTY CLAY WITH SHELL				LIGHT GRAY AND TAN, FRIABLE TO INDURATED LIMESTONE WITH SAND
-10 -9.2 3.5 WOR WOR W	(OR)	: :::: 💆	FRAGMENTS, SATURATED	₋₉₀	:::: :::: ::::/		LAYERS (CASTLE HAYNE FORMATION) RS-01: 86.4'-87.1'
	T		-				1
			-			: : : : RS-01	
-15 -14.2 + 8.5 WOR WOR V	/OR	· ···· 	-	<u>-95</u> <u>-94.1 + 88.4</u> 12 10	13	SS-124 17%	94.1 (88.4
			-	±		. []	<u> </u>
-19.2 + 13.5			-	-99.1 + 93.4			-
-20 + 13.5 WOH WOH V	/OH •0	· · · · · ·	-	-100 14 14 14 2	21 35		-
							 - -
25 -24.2 + 18.5			GRAY SANDY CLAY WITH WOOD FRAGMENTS, WITH SHELL	-105 -104.1 98.4 17 22 3			 -
-25 -24.2 + 10.5 WOH 1	0 1	Sat.	FRAGMENTS, SATURATED	-105 17 22 2	22 44 4		-
±		: ::::	(YORKTOWN FORMATION)	±			Very Soft Limestone
-30 -29.2 -23.5 1 0	1	. 📖 💮	-				_
T I I I	1 1	SS-112 47%	 31.7 26.0	‡			-
-32.5 + 26.8 2 2	${2}$	SS-113 25%	BROWN AND GRAY SAND WITH SHELL FRAGMENTS, SATURATED				- -
_35	4		- TRAGINENTS, SATURATED				<u>L</u>
			_				-
-37.5 + 31.8 8 17	23		-				F
-40	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0000		‡			F
-42.5 + 36.8			LIGHT GRAY LIMESTONE, SATURATED				- -
	23	Sat.	- (CASTLE HAYNE FORMATION)				- -
<u>-45</u> <u>T</u>			-	±			
-47.5 + 41.8			-	±			_
[-50 + " "	° • • 15		-				-
<u> </u>			-				F
-52.5 + 46.8 6 6	5		-				F
55 +	7	- · · · · ·					<u> </u>
-52.5 + 46.8		.	GREENISH GRAY SAND WITH SHELL				<u> </u>
-37.3 + 31.8 6 8	11	Sat. 0000 0000 0000 0000 0000 0000 0000 00	FRAGMENTS, WITH CEMENTED SAND AND LIMESTONE LAYERS, SATURATED				Ė
<u>-60</u>			_	+			-
-62.5 - 56.8	14		-				-
12 18	14 32 32 32 32 32 32 32 3		-				F
		-	- -	‡			F
67.5 + 61.8 9 9	15	· · · · ·	-				- -
-67.5	24	-					
			-				E
-72.5 + 66.8 + 12 16			-				-
¹ / ₂ −75			- -				F
<u>-77.5 + 71.8</u>			-				-
7 6	8 •14		-				<u> </u>
뿐 <mark>-80</mark> +			-	±			<u> </u>
-82.5 T 76.8 14 100/0.3			- -83 3 77 6				-
O -85		100/0.3					F
					-		

	C	ORE LOG		
WBS 45569.1.2	TIP B-5614 COUNT	Y BEAUFORT	GEOLOGIST J. Holland	
SITE DESCRIPTION BRIDGE NO.	. 9 ON -L- (SR 1112) OVER BLOUN	ITS CREEK		GROUND WTR (ft)
BORING NO. B6-A	STATION 25+00	OFFSET 8 ft LT	ALIGNMENT -L-	0 HR. FIAD
COLLAR ELEV5.7 ft	TOTAL DEPTH 99.9 ft	NORTHING 618,486	EASTING 2,604,756	24 HR . N/A
DRILL RIG/HAMMER EFF./DATE 74201	18 CME-55C 90% 03/16/2021	DRILL METHOD SP	Γ Core Boring HAMM	ER TYPE Automatic
DRILLER T.J. Strickland	START DATE 03/18/21	COMP. DATE 03/23/21	SURFACE WATER DEPTH 5.9	9ft
CORE SIZE NQ2	TOTAL RUN 10.8 ft			
ELEV (ft) DEPTH RUN RATE (Min/ft)	RUN SAMP. REC. RQD RQD REC. RQD RQD	L O D D G ELEV. (ft)	ESCRIPTION AND REMARKS	DEPTH (fi
-83.3			Begin Coring @ 77.6 ft	
-85	(1.1) (0.6) 22% 12% 20% 19% (1.8) (1.0) 36% 20% RS-01	LIGHT GRAY AND T	STAL PLAIN SEDIMENTARY ROCK AN, FRIABLE TO INDURATED LIME LYERS (CASTLE HAYNE FORMATIC RS-01: 86.4'-87.1' RMR= 12	
-105 N=44		-105.6 Boring Terminat	ed at Elevation -105.6 ft in Very Soft I	99.

SHEET 12

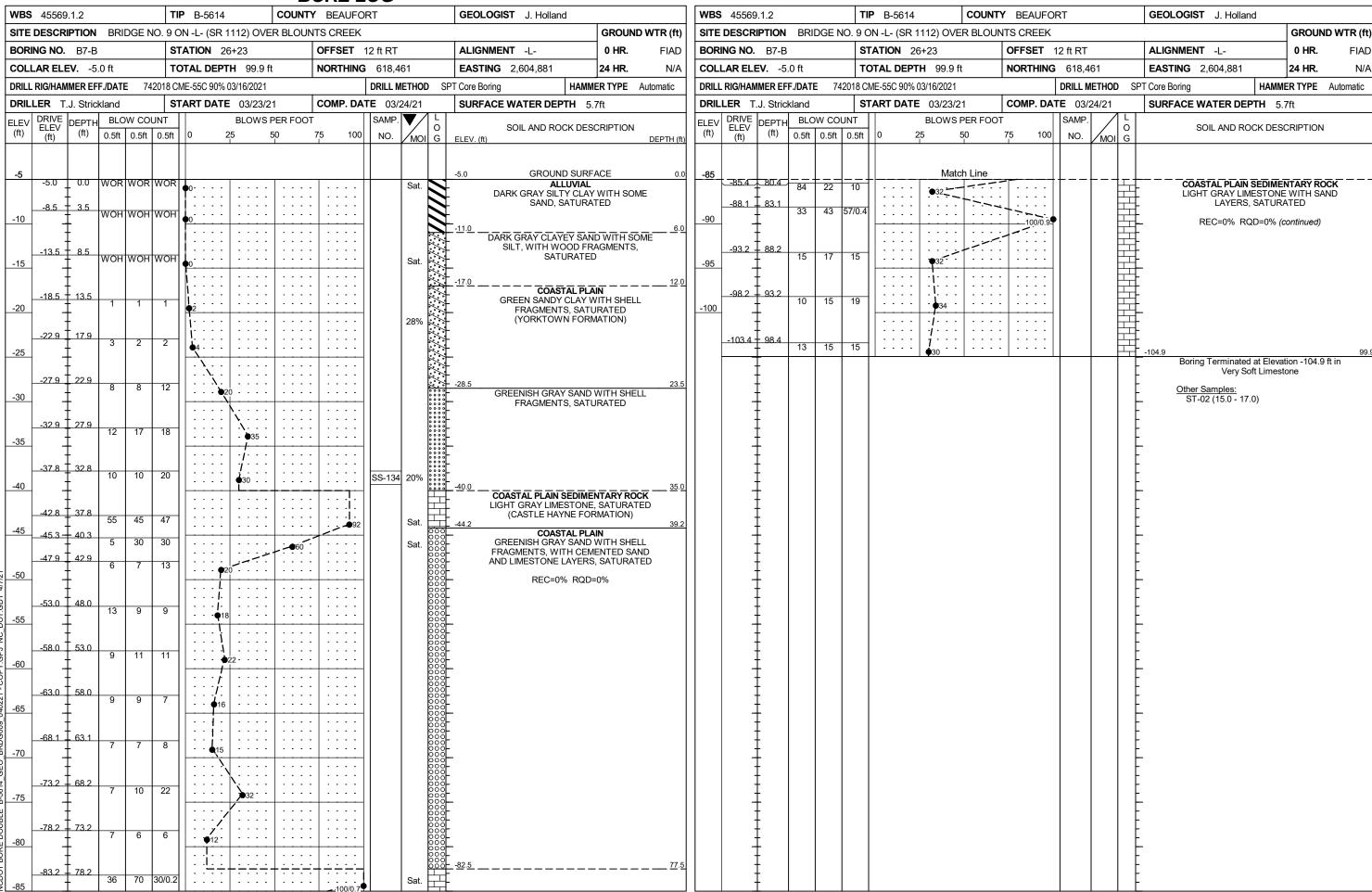
CORE PHOTOGRAPH BRIDGE NO. 9 OVER BLOUNTS CREEK ON SR 1112 (MOUTH OF CREEK RD)

B6-A BOX 1 OF 1: 77.6 - 88.4 FEET





APPROXIMATE SCALE IN FEET



											KE LUG				
WBS	45569	.1.2			TIP	B-561	4	С	OUNT	Y B	EAUFORT	GEOLOGIST J. H	lolland		
SITE	DESCR	IPTION	BRI	DGE NO.	9 ON	-L- (SF	R 1112) C	OVER I	BLOU	NTS (CREEK			GROUN	ND WTR (ft)
BORI	NG NO.	B7-B			STA	TION	26+23			OF	FSET 12 ft RT	ALIGNMENT -L-		0 HR.	FIAD
COLL	AR ELI	EV 5	.0 ft		TOT	AL DE	PTH 99	.9 ft		NO	RTHING 618,461	EASTING 2,604,8	881	24 HR.	N/A
DRILL	RIG/HAN	IMER EF	F./DAT	E 74201	18 CME	-55C 90	% 03/16/20)21			DRILL METHOD SPT	Core Boring	HAMME	R TYPE	Automatic
DRIL	LER T	.J. Stric	kland		STA	RT DA	TE 03/2	23/21		СО	MP. DATE 03/24/21	SURFACE WATER	DEPTH 5.7	7ft	
CORI	E SIZE	NQ2			1		N 2.0 ft			L.					
ELEV	RUN ELEV	DEPTH		DRILL RATE	REC.	JN RQD	SAMP.	REC.	RATA	L	Di	ESCRIPTION AND RE	MARKS		
(ft)	(ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	G	ELEV. (ft)				DEPTH (ft
-44.3 -45	-44.3 -45.3	39.3	1.0	:54/1.0	(0.0)	(0.0)		<u> </u>	-	000		Begin Coring @ 39 COASTAL PLAN	.3 ft N		
	-45.3 -	40.3		N=60	0%	(0%)				0000	- GREENISH GR - CEMENTED SA	AY SAND WITH SHEL ND AND LIMESTONE	L FRAGMENT	S, WITH	
	-	_		N=20				1		0000	- -	(continued)	L (1 L) (0, 0) (1	OTTT	
-50	-	-						1		0000	- -				
	-	-								0000	- -				
-55	-	_		N=18				-		000	- -				
	-	-								000	- -				
-60		-		N=22				1		000	- -				
	-	- -								0000	- -				
	-	-		N=16				-		0000	- -				
-65	_	F						1		0000	- -				
	-	<u> </u>								000	- -				
-70				N=15						000	- -				
	-	E								000	- - -				
	-			N=32				-		000	- -				
-75	-	_						1		0000	- -				
	-	-								000	- -				
-80	-	_		N=12				-		000	- -				
	-	 -								000	- 82.5				77.5
-85	-84.4 -	- 79.4		N=100/0.7				1				STAL PLAIN SEDIMEN FRIABLE LIMESTONE		AYERS,	
	-85.4 -	- 80.4 -	1.0	1:12/1.0 N=32	(0.0)	(0.0) _0%_/		1		坩	- -	SATURATED			
	-	<u> </u>		N=100/0.9	9			-		井	- -				
-90	_	-		700,0.0	1			1		耳	_				
	-									H	- - -				
-95				N=32							- -				
	-	-									- -				
100		_		N=34				1			- -				
-100	-	-						1		井	-				
	-	 -									- -				
		-		N=30				<u> </u>	1		-104.9 Boring Terminat	ed at Elevation -104.9	ft in Very Soft I	imestone	99.9
	-	<u> </u>									Other Samples:	ed at Lievation - 104.9	it iii very soit L		
	-	- -									ST-02 (15.0 - 17.0)				
	-	F									- -				
		Ē									- -				
	-	E									- 				
	-	E									- - -				
	-	E									- - -				
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SHEET 15

														.06																	,							1
		5569.1.2					B-56					FY BI		PRT				GEOLOG	GIST Arger	bright, D.				S 4556					B-561					AUFOR	T			GEOLOG
SIT	E DES	SCRIPTION	ON	BRIDG	SE NO). 9 O	N -L- (S	SR 1	112) O\	VER I	BLOU	NTS C	REEK								GROUNE	WTR (ft)	SITE	DESC	RIPTION	I BRID	OGE NO	D. 9 OI	N -L- (SI	R 1112	OVEF	R BLOU	NTS C	REEK				
вог	RING	NO. E	B2-A			ST	ATION	27	+29			OFF	SET	24 ft L	Т			ALIGNM	IENT -L-		0 HR.	N/A	BOR	RING NO	O . EB2	-A		ST	ATION	27+29)		OFF	SET 2	4 ft LT			ALIGNME
COI	LAR	ELEV.	10.8	ft		то	TAL D	EPTI	H 74.3	3 ft		NOF	RTHING	618	3,489			EASTING	G 2,604,98	6	24 HR.	9.9	COL	LAR EI	LEV. 1	0.8 ft		то	TAL DE	PTH	74.3 ft		NOR	THING	618,48	39		EASTING
DRIL	L RIG	/HAMMER	R EFF./[DATE	GFC	00075 (ME-450	2 89%	08/19/20	019				DRILL	MET	HOD	Mu	d Rotary		HAMI	MER TYPE	utomatic	DRILI	L RIG/HA	AMMER E	FF./DATE	GFC	D0075 C	ME-45C	89% 08	19/2019		I		DRILL MI	ETHOD	Mud	d Rotary
DRI	LLER	R Smith	RF			_			06/08			CON	/IP. DA	TE 0				 	E WATER D				_		Smith, F			_	ART DA				CON	IP. DAT				SURFACE
ELE\	, DR	IVE DEE		BLOW	COU				BLOW		R FOC			SAMI		7/	L						ELEV		E DEPTH		w coul					ER FOC			SAMP	$\overline{}$	L	
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SITE	DESCR	PTION	BRII	DGE N	10.90	ON -L-	(SR 1112)	OVER BLOUN	NTS CRI	EEK					GROUND WTR (
3OR	NG NO.	EB2-	A		S ⁻	TATIO	V 27+29		OFFSI	ET 2	4 ft LT			ALIGNMENT -L-	0 HR. N
COLI	LAR ELE	EV . 10).8 ft		T	OTAL I	DEPTH 7	4.3 ft	NORT	HING	618,4	89		EASTING 2,604,986	24 HR . 9
ORILL	. RIG/HAM	MER EF	F./DATE	E GI	FO0075	CME-45	5C 89% 08/1	9/2019			DRILL N	IETHOD	Mu	d Rotary HAM	MER TYPE Automatic
DRIL	LER S	mith, R	. E.		S	TART I	DATE 06	/08/20	СОМР	. DAT	E 06/	08/20		SURFACE WATER DEPTH	
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(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50	75	100	NO.	MOI	O G	SOIL AND ROCK DE	SCRIPTION
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BRIDGE NO. 9 OVER BLOUNTS CREEK ON SR 1112 (MOUTH OF CREEK RD)

							SOIL TES	T RESU	JLTS							
BORING	SAMPLE			DEPTH INTERVAL	AASHTO	LIQUID	PLASTICITY		% BY	′ WEIGHT		% PAS	SSING (SI	EVES)	%	%
NO.	NO.	STATION	OFFSET	(FEET)	CLASS.	LIMIT	INDEX	GRAVEL	C.SAND	F.SAND	SILT & CLAY	10	40	200	MOISTURE	ORGANIC
В1-В	SS-050	19+06	14' RT	28.6-30.1	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	58.0	-
B1-B	SS-059	19+06	14' RT	73.3-74.8	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	33.0	-
B2-A	SS-026	20+13	12' LT	15.5-17.0	A-4	23	5	0.00	0.20	50.11	49.70	100.00	99.80	49.70	24.7	-
B2-A	SS-028	20+13	12' LT	25.5-27.0	A-1-B	0	0	3.31	50.27	40.19	6.23	96.69	46.42	6.23	38.6	-
В3-В	SS-004	21+45	10' RT	13.5-15.0	A-2-4	0	0	0.00	0.41	82.00	17.58	100.00	99.59	17.58	23.1	-
B4-A	SS-069	22+55	12' LT	17.9-19.4	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	23.8	-
B4-A	SS-071	22+55	12' LT	27.9-29.4	A-1-B	0	0	0.15	55.45	35.15	9.25	99.85	44.40	9.25	26.0	-
В5-В	SS-100	23+84	12' RT	69.1-70.6	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	23.5	-
В6-А	SS-112	25+00	8' LT	23.5-25.0	A-6	30	14	0.00	0.41	37.63	61.96	100.00	99.59	61.96	47.3	-
В6-А	SS-113	25+00	8' LT	26.8-28.3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	24.6	-
В6-А	SS-124	25+00	8' LT	88.4-89.9	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	16.9	-
B7-B	SS-134	26+23	12' RT	32.8-34.3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	19.8	-
B1-B	ST-01	19+06	14' RT	20.1-22.1	A-2-4	0	0	0.00	0.46	73.76	25.78	100.00	99.54	25.78	28.2	-
B7-B	ST-02	26+23	12' RT	15.0-17.0	A-2-6	29	11	0.48	37.86	43.10	18.57	99.52	61.66	18.57	27.7	-

BRIDGE NO. 9 ON SR 1112 OVER BLOUNTS CREEK (MOUTH OF CREEK RD)

						ROCK	TEST RES	JLTS				
				DEPTH	LENGTH	DIAMETER	AREA	VOL	.UME	UNIT WEIGHT	COMPRESSIVE	
BORING	SAMPLE NO.	STATION	OFFSET	INTERVAL (ft)	(in.)	(in.)	(sq. in.)	(in. ³)	(cf)	(pcf)	STRENGTH (psi)	TESTING METHOD
B6-A	RS-01	25+00	8FT LT	86.4 - 87.1	4.94	1.95	2.97	14.7	0.01	106.9	400	ASTM D-7012-14 METHOD C

SITE PHOTOGRAPHS BRIDGE NO. 9 OVER BLOUNTS CREEK ON SR 1112 (MOUTH OF CREEK ROAD)





View of SR 1112 looking northeast.

View of proposed SR 1112 looking east.



View of Blounts Creek looking northwest.



April 5, 2021

Memorandum to: J. L. Pilipchuk, L.G., P.E.

State Geotechnical Engineer

WBS Element: 45569.1.2
TIP: B-5614
County: BEAUFORT

Description: Bridge 9 over Blounts Creek on SR 1112 (Mouth of the Creek Rd)

Subject: Design Scour Elevations

After a review of site flooding history, historical scour depth, and geologic conditions encountered at the site, Schnabel Engineering, has determined the design scour elevations (DSE), and presents the following:

	Theoretical	Historical Scour	Design Scour	Does DSE impact
Location	Scour Elevation	Elevation	Elevation	end bents?
Bent 1	-15.2 feet	-23.4 feet	-15.2 feet	No
Bent 2	-23.9 feet	-24.5 feet	-23.9 feet	No
Bent 3	-21.3 feet	-22.1 feet	-21.3 feet	No
Bent 4	-21.4 feet	-24.6 feet	-21.4 feet	No
Bent 5	-19.0 feet	-25.3 feet	-19.0 feet	No
Bent 6	-17.8 feet	-21.7 feet	-17.8 feet	No
Bent 7	-16.3 feet	-17.0 feet	-16.3 feet	No

The Theoretical Scour Elevation is from the Bridge Survey and Hydraulic Design Report dated 2/5/2021. The Design Scour Elevation is the same as the Theoretical Scour Elevation. The subsurface investigation revealed Alluvial materials consisting of silty/sandy clay and clayey sand generally extending deeper than the Theoretical Scour Elevations at each bent. These materials are not resistant to scour.

Jacob C. Wessell, P.E. Senior Engineer

