

REFERENCE: BR-0090

PROJECT: 67090

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4	PROFILE(S)
5-6	CROSS SECTION(S)
7-12	BORE LOG(S) & CORE REPORT(S)
13	SOIL TEST RESULTS
14-15	CORE PHOTOGRAPH(S)
16	SITE PHOTOGRAPH(S)

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY NASH
PROJECT DESCRIPTION BRIDGE NO.36 ON NC 561
OVER FISHING CREEK

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0090	1	16

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

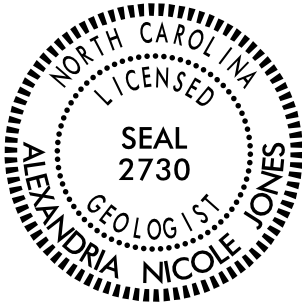
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

A. N. JONES
N. O. MOORE
C. M. WALKER
D. G. PINTER

INVESTIGATED BY A. N. JONES
DRAWN BY A. N. JONES
CHECKED BY N. T. ROBERSON
SUBMITTED BY N. T. ROBERSON
DATE AUGUST 2022



DocuSigned by:
Alexandria Jones 01/10/2023
4349EF40DD60478...
SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.									
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS														NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.															
FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.										FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.																													
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.										WEATHERING																													
FRESH										ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.																													
VERY SLIGHT (V SL.)										ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.																													
SLIGHT (SL.)										ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.																													
MODERATE (MOD.)										SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.																													
MODERATELY SEVERE (MOD. SEV.)										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i>																													
SEVERE (SEV.)										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF</i>																													
VERY SEVERE (V SEV.)										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i>																													
COMPLETE										ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.																													
ROCK HARDNESS										VERY HARD												CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.																	
HARD										CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.												MODERATELY HARD																	
MODERATELY HARD										CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.																													
MEDIUM HARD										CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.																													
SOFT										CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.																													
VERY SOFT										CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																													
FRACTURE SPACING										BEDDING												BENCH MARK: BL-102, REBAR WITH CAP, -L- STA. 15+16 18'RT																	
TERM										SPACING												TERM																	
VERY WIDE										MORE THAN 10 FEET												THICKNESS																	
WIDE										3 TO 10 FEET												4 FEET																	
MODERATELY CLOSE										1 TO 3 FEET												1.5 - 4 FEET																	
CLOSE										0.16 TO 1 FOOT												0.16 - 1.5 FEET																	
VERY CLOSE										LESS THAN 0.16 FEET												VERY THINLY BEDDED																	
																						THICKLY BEDDED																	
																						THICKLY LAMINATED																	
																						THINLY LAMINATED																	
																						< 0.008 FEET																	
INDURATION										FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.												FRIABLE																	
										RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.												MODERATELY INDURATED																	

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

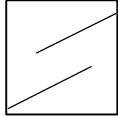
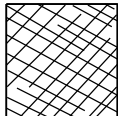
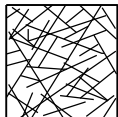

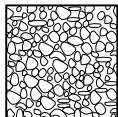
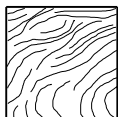
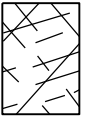


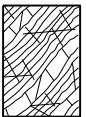
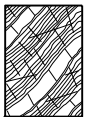



GEOTECHNICAL ENGINEERING UNIT

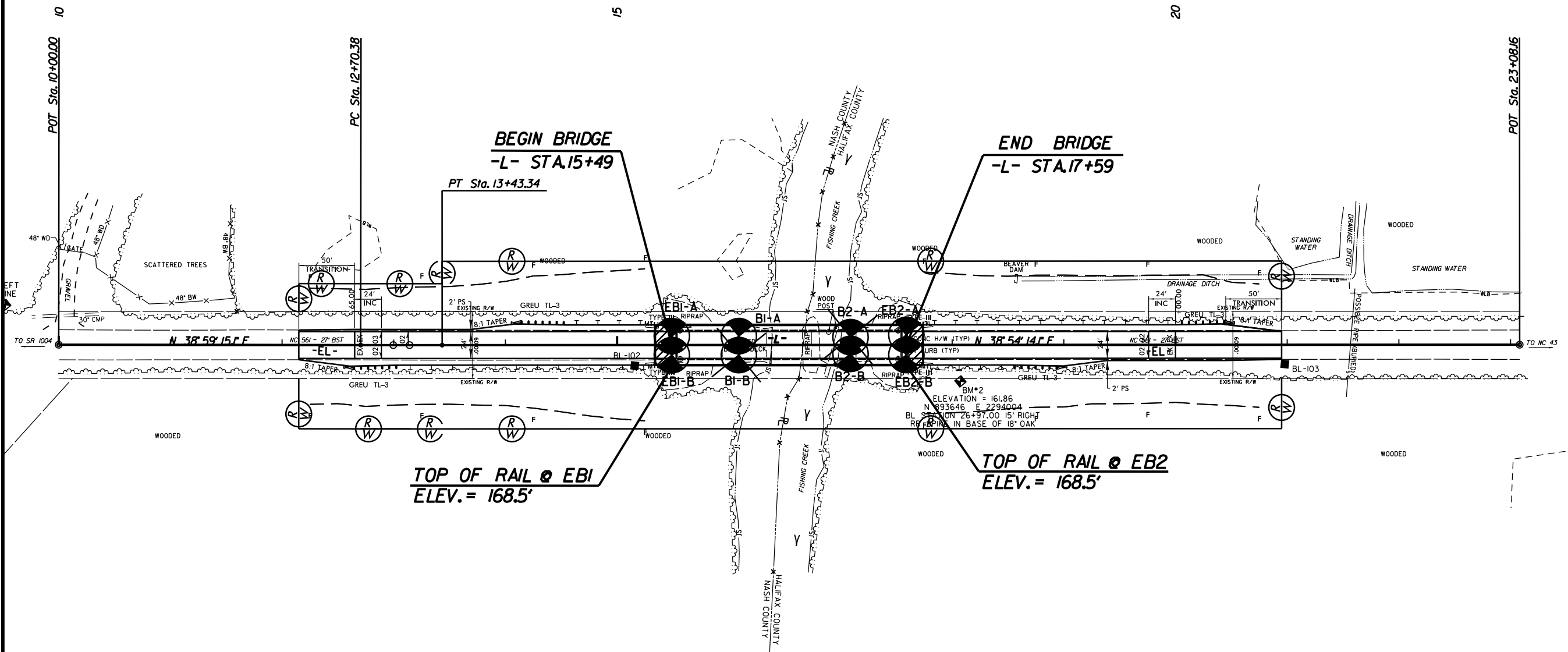
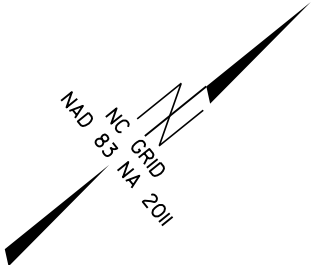
SUBSURFACE INVESTIGATION

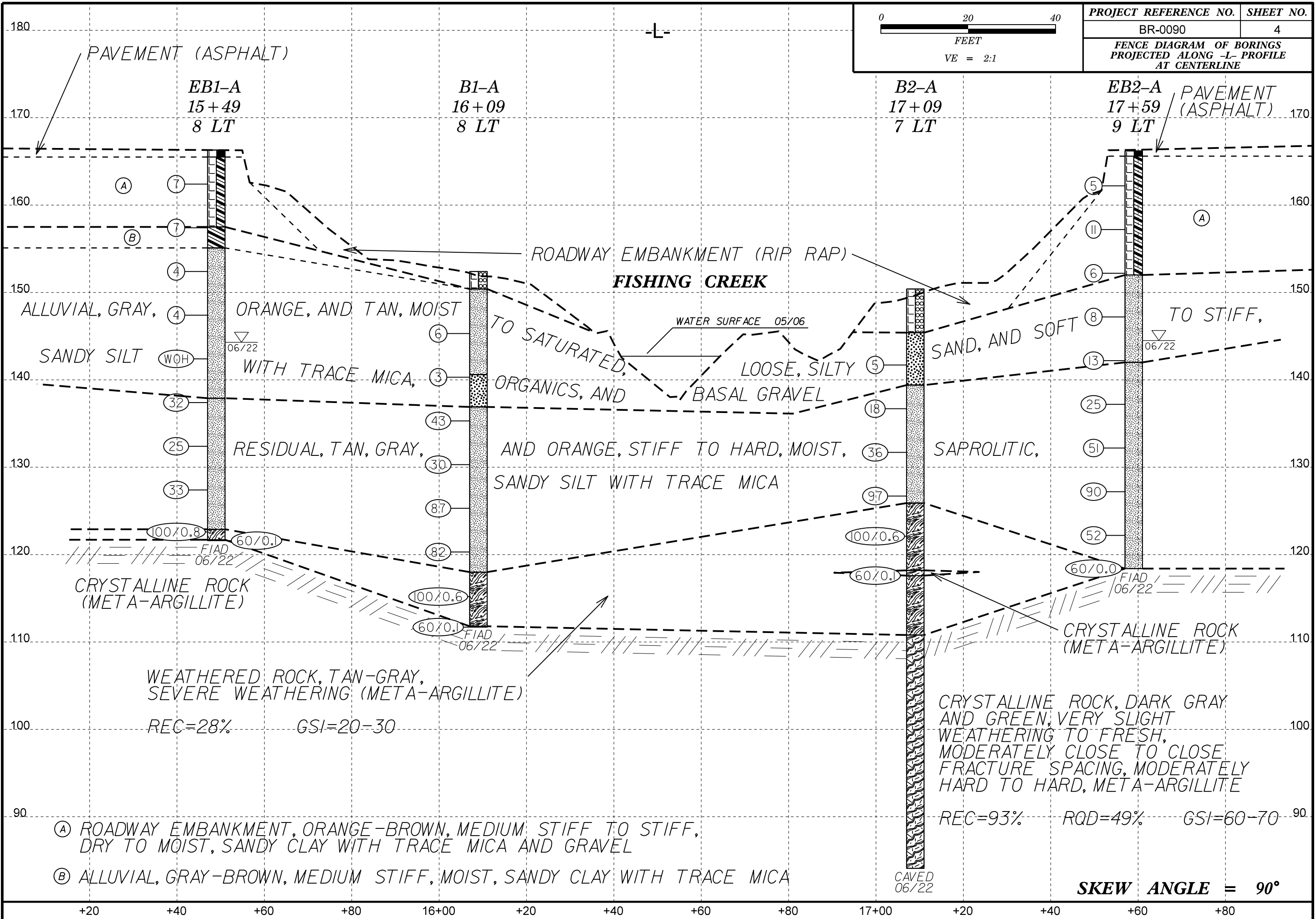
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

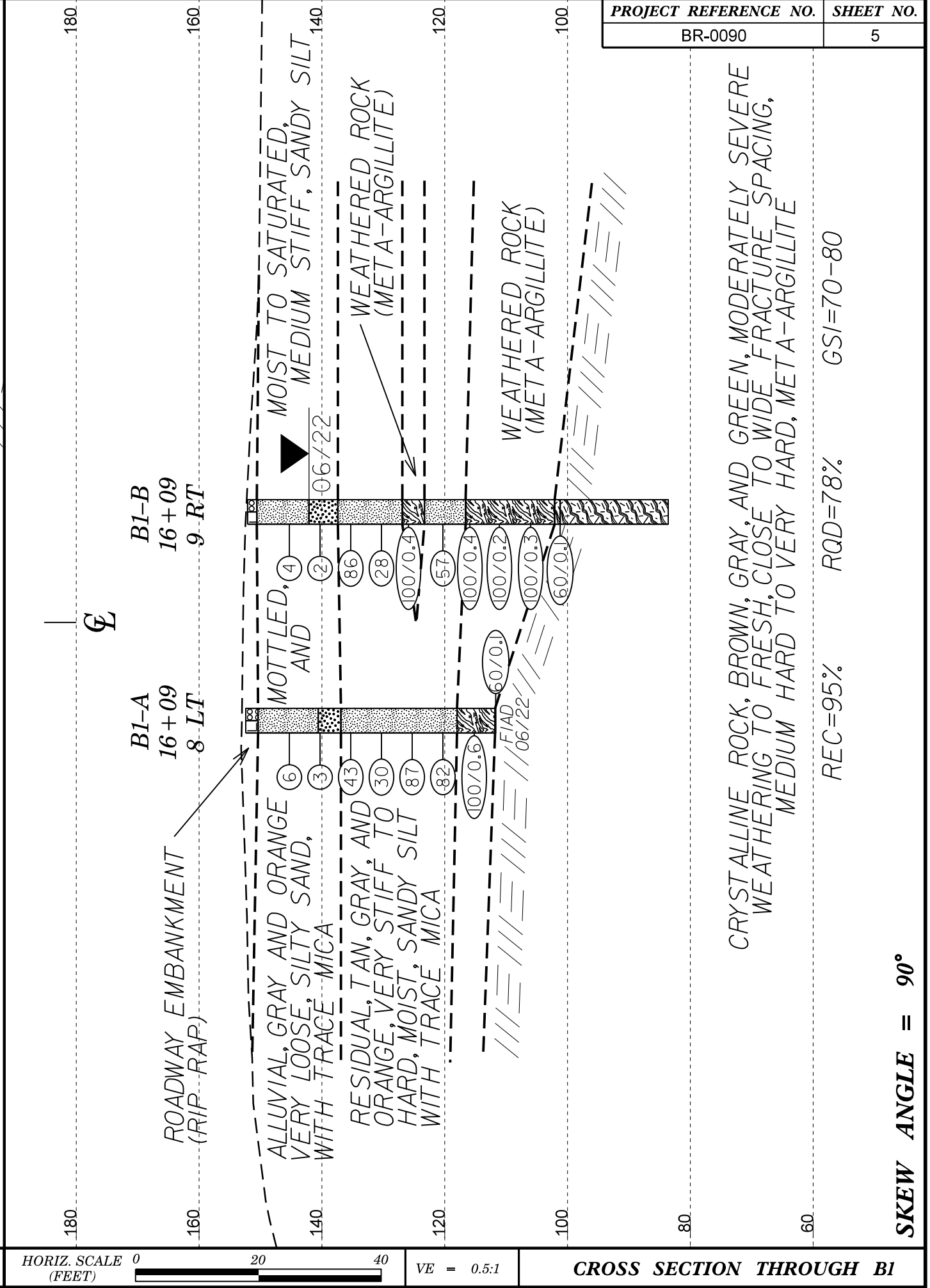
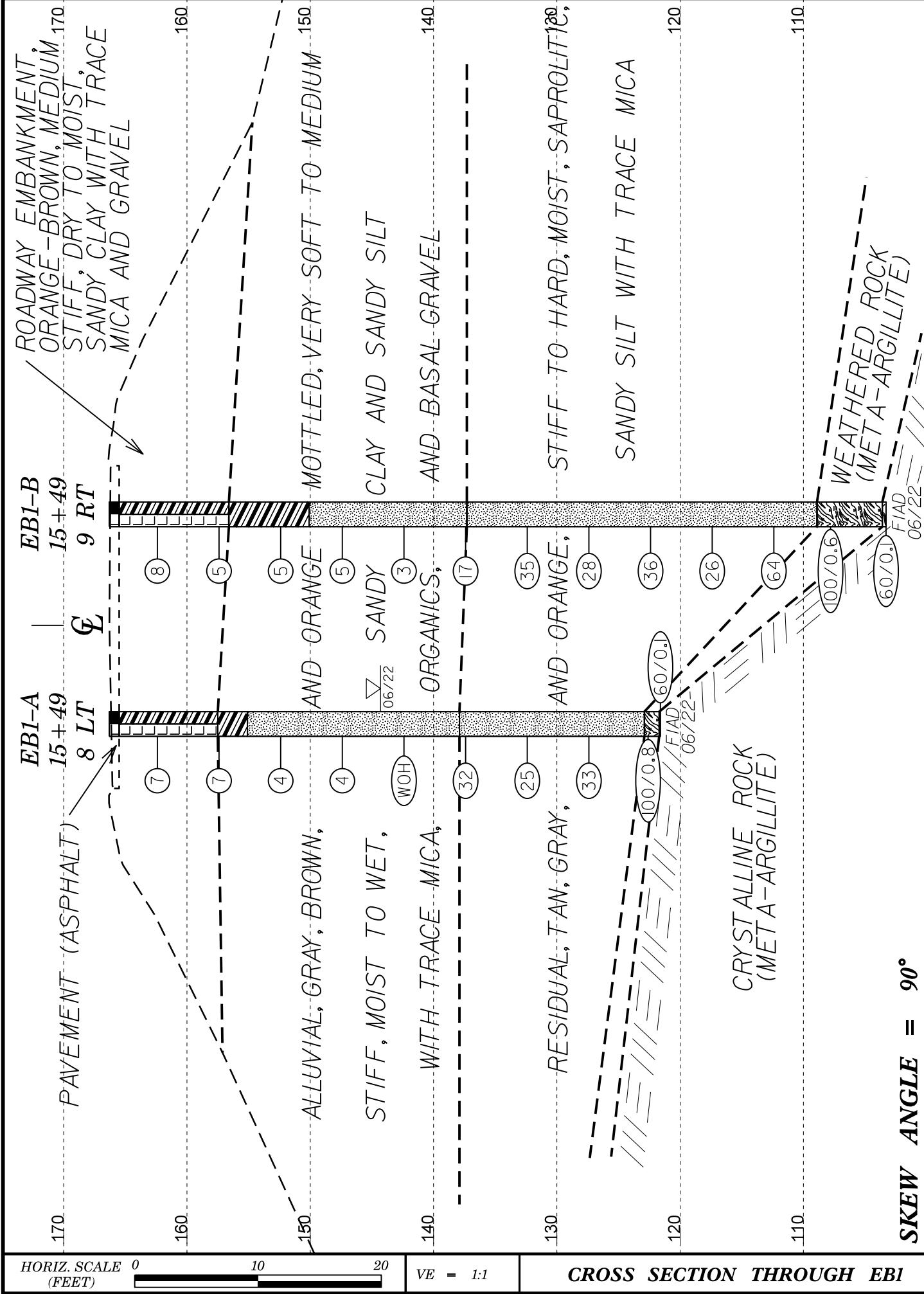
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

<div><div>GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)</div><div>From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.</div></div>	<div>SURFACE CONDITIONS</div> <div>VERY GOOD Very rough, fresh unweathered surfaces</div> <div>GOOD Rough, slightly weathered, iron stained surfaces</div> <div>FAIR Smooth, moderately weathered and altered surfaces</div> <div>POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments</div> <div>VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings</div>	<div>STRUCTURE</div> <div><div>INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities</div><div>BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets</div><div>VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets</div><div>BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity</div><div>DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces</div><div>LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes</div></div>	<div>DECREASING SURFACE QUALITY ➡</div> <div>90</div> <div>80</div> <div>70</div> <div>60</div> <div>50</div> <div>40</div> <div>30</div> <div>20</div> <div>10</div> <div>N/A</div> <div>N/A</div>	<div>GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)</div> <div>From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.</div>	<div>SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)</div> <div>VERY GOOD - Very Rough, fresh unweathered surfaces</div> <div>GOOD - Rough, slightly weathered surfaces</div> <div>FAIR - Smooth, moderately weathered and altered surfaces</div> <div>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</div> <div>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</div>	<div>COMPOSITION AND STRUCTURE</div> <div><div>A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.</div><div>B. Sandstone with thin inter-layers of siltstone</div><div>C. Sandstone and siltstone in similar amounts</div><div>D. Siltstone or silty shale with sandstone layers</div><div>E. Weak siltstone or clayey shale with sandstone layers</div><div><div>C, D, E, and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.</div>F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure</div><div>G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers</div><div>H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.</div></div> <div>➡ Means deformation after tectonic disturbance</div>	<div>70</div> <div>60</div> <div>50</div> <div>40</div> <div>30</div> <div>20</div> <div>10</div>
---	--	---	---	---	---	---	---







WBS 67090.1.1		TIP BR-0090		COUNTY NASH		GEOLOGIST Jones, A. N.							
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK							GROUND WTR (ft)						
BORING NO. B1-A		STATION 16+09		OFFSET 8 ft LT		ALIGNMENT -L-		0 HR. N/A					
COLLAR ELEV. 152.4 ft		TOTAL DEPTH 40.7 ft		NORTHING 893,517		EASTING 2,293,847		24 HR. FIAD					
DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550X 87% 02/23/2021				DRILL METHOD NW Casing w/ Advancer			HAMMER TYPE Automatic						
DRILLER Pinter, D. G.		START DATE 06/30/22		COMP. DATE 06/30/22		SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			
155													
150													152.4 GROUND SURFACE 0.0
													150.4 ROADWAY EMBANKMENT 2.0
													ROADWAY EMBANKMENT RIP RAP
145	146.3	6.1	2	2	4								ALLUVIAL GRAY AND ORANGE MOTTLED, SANDY SILT WITH TRACE MICA
140	141.3	11.1	1	1	2								140.6 GRAY, SILTY SAND 11.8
	136.3	16.1	11	19	24								136.9 RESIDUAL 15.5
135													TAN, GRAY, AND ORANGE, SANDY SILT WITH TRACE MICA
130	131.3	21.1	10	13	17								
125	126.3	26.1	16	48	39								
120	121.3	31.1	31	35	47								
	116.3	36.1	27	58	42/0.1								118.0 WEATHERED ROCK (META-ARGILLITE) 34.4
115													
	111.8	40.6	60/0.1										111.8 CRYSTALLINE ROCK (META-ARGILLITE) 40.6
													111.7 40.7
													Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 111.7 ft IN CRYSTALLINE ROCK (META-ARGILLITE)

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 67090.1.1			TIP BR-0090			COUNTY NASH			GEOLOGIST Jones, A. N.							
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK									GROUND WTR (ft)							
BORING NO. B1-B			STATION 16+09			OFFSET 9 ft RT			ALIGNMENT -L-							
COLLAR ELEV. 152.1 ft			TOTAL DEPTH 68.5 ft			NORTHING 893,506			EASTING 2,293,861							
DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550X 87% 02/23/2021						DRILL METHOD NW Casing w/ Core			HAMMER TYPE Automatic							
DRILLER Pinter, D. G.			START DATE 06/28/22			COMP. DATE 06/29/22			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
155																
															152.1	GROUND SURFACE
150															150.6	ROADWAY EMBANKMENT RIP RAP
	146.3	5.8														ALLUVIAL GRAY AND ORANGE MOTTLED, SANDY SILT WITH TRACE MICA
145			2	2	2											
	141.3	10.8													142.1	GRAY, SILTY SAND WITH TRACE MICA
140			1	1	1											
	136.3	15.8													137.4	RESIDUAL TAN AND ORANGE, SANDY SILT WITH TRACE MICA
135			16	38	48											
	131.3	20.8														
130			12	17	11											
	126.3	25.8													126.9	WEATHERED ROCK (META-ARGILLITE)
125			100/0.4													
	121.3	30.8													123.3	RESIDUAL TAN AND ORANGE, SANDY SILT WITH TRACE MICA
120			22	28	29											
	116.3	35.8													116.6	WEATHERED ROCK (META-ARGILLITE)
115			100/0.4													
	111.3	40.8														
110			100/0.2													
	106.3	45.8														
105			100/0.3													
	101.3	50.8													102.1	CRYSTALLINE ROCK (META-ARGILLITE)
100			60/0.1												100.6	BROWN, GRAY, AND GREEN, SEVERE WEATHERING TO FRESH, CLOSE TO WIDE FRACTURE SPACING, MEDIUM HARD TO VERY HARD, META-ARGILLITE
																REC=95% RQD=78% GSI=70-80
95																
90																
85																
															83.6	Boring Terminated at Elevation 83.6 ft IN CRYSTALLINE ROCK (META-ARGILLITE)

GEOTECHNICAL BORING REPORT

CORE LOG

[illegible]

GEOTECHNICAL BORING REPORT
BORE LOG

GEOTECHNICAL BORING REPORT
CORE LOG

WBS 67090.1.1				TIP BR-0090		COUNTY NASH		GEOLOGIST Jones, A. N.								
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK												GROUND WTR (ft)				
BORING NO. B2-A				STATION 17+09			OFFSET 7 ft LT			ALIGNMENT -L-		0 HR. N/A				
COLLAR ELEV. 150.4 ft				TOTAL DEPTH 66.3 ft			NORTHING 893,594			EASTING 2,293,911		24 HR. Caved 7.0				
DRILL RIGHAMMER EFF./DATE HFC0072 CME-550X 87% 02/23/2021							DRILL METHOD NW Casing w/ Core				HAMMER TYPE Automatic					
DRILLER Pinter, D. G.				START DATE 06/22/22			COMP. DATE 06/27/22			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
155																
150														150.4	GROUND SURFACE	0.0
145															ROADWAY EMBANKMENT RIP RAP	
140	142.7	7.7												145.4		5.0
135	137.7	12.7													ALLUVIAL GRAY, SILTY SAND WITH TRACE ORGANICS	
130	132.7	17.7														
125	127.7	22.7													RESIDUAL GRAY AND TAN, SAPROLITIC, SANDY SILT	
120	122.7	27.7														
115	117.7	32.7														
110																
105																
100																
95																
90																
85																

WBS 67090.1.1				TIP BR-0090				COUNTY NASH				GEOLOGIST Jones, A. N.							
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK												GROUND WTR (ft)							
BORING NO. B2-A				STATION 17+09				OFFSET 7 ft LT				ALIGNMENT -L-				0 HR. N/A			
COLLAR ELEV. 150.4 ft				TOTAL DEPTH 66.3 ft				NORTHING 893,594				EASTING 2,293,911				24 HR. Caved 7.0			
DRILL RIGHAMMER EFF./DATE HFC0072 CME-550X 87% 02/23/2021								DRILL METHOD NW Casing w/ Core				HAMMER TYPE Automatic							
DRILLER Pinter, D. G.				START DATE 06/22/22				COMP. DATE 06/27/22				SURFACE WATER DEPTH N/A							
CORE SIZE NXWL				TOTAL RUN 33.5 ft															
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		L O G	DESCRIPTION AND REMARKS			DEPTH (ft)					
					REC. (ft) %	RQD (ft) %		REC. (ft) %	RQD (ft) %										
117.6											Begin Coring @ 32.8 ft								
	117.6	32.8	2.7	0:40/0.7 0:32/1.0 0:22/1.0	(0.8) 30%	(0.0) 0%		(1.9) 28%	(0.0) 0%					117.6	WEATHERED ROCK	32.8			
115	114.9	35.5									TAN-GRAY, SEVERE WEATHERING, META-ARGILLITE								
			5.0	0:28/1.0 0:29/1.0 0:26/1.0 1:03/1.0 1:04/1.0	(2.1) 42%	(0.4) 8%													
											GSI=20-30								
110	109.9	40.5									110.8			39.6					
			4.6	0:45/1.0 0:58/1.0 0:54/1.0 0:56/1.0 0:37/0.6	(4.0) 87%	(3.3) 72%		(24.9) 93%	(13.0) 49%										
105	105.3	45.1					RS-2				CRYSTALLINE ROCK								
			3.9	1:21/1.0 1:56/1.0 1:37/1.0 2:03/0.9	(2.6) 67%	(1.6) 41%													
											DARK GRAY AND GREEN, VERY SLIGHT WEATHERING TO FRESH, MODERATELY CLOSE TO CLOSE FRACTURE SPACING, MODERATELY HARD TO HARD, META-ARGILLITE								
			2.3	0:52/0.3 1:34/1.0 2:04/1.0	(2.3) 100%	(0.9) 39%													
100	101.4	49.0									GSI=60-70								
	99.1	51.3	5.0	1:07/1.0 1:11/1.0 1:12/1.0 0:54/1.0 1:02/1.0	(5.0) 100%	(2.3) 46%													
							RS-3												
95	94.1	56.3																	
			5.0	1:17/1.0 1:01/1.0 1:09/1.0 1:01/1.0 1:05/1.0	(5.0) 100%	(0.5) 10%													
90	89.1	61.3																	
			5.0	1:21/1.0 1:20/1.0 1:32/1.0 1:07/1.0 1:29/1.0	(5.0) 100%	(4.0) 80%													
85	84.1	66.3									Boring Terminated at Elevation 84.1 ft IN CRYSTALLINE ROCK (META-ARGILLITE)			66.3					

NCDOT BORE DOUBLE BR0090_GEO_BRDG_BH.GPJ NC_DOT.GDT 8/8/22

NCDOT BORE DOUBLE BR0090_GEO_BRDG_BH.GPJ NC_DOT.GDT 8/8/22

WBS 67090.1.1				TIP BR-0090				COUNTY NASH				GEOLOGIST Jones, A. N.					
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK												GROUND WTR (ft)					
BORING NO. B2-B				STATION 17+09				OFFSET 8 ft RT				ALIGNMENT -L-				0 HR. N/A	
COLLAR ELEV. 150.1 ft				TOTAL DEPTH 33.8 ft				NORTHING 893,585				EASTING 2,293,923				24 HR. 7.6	
DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550X 87% 02/23/2021								DRILL METHOD NW Casing w/ Advancer				HAMMER TYPE Automatic					
DRILLER Pinter, D. G.				START DATE 06/29/22				COMP. DATE 06/29/22				SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION				
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
155																	
150																	
145	146.4	3.7	5	2	2									150.1 GROUND SURFACE 0.0			
														ROADWAY EMBANKMENT RIP RAP			
140	141.4	8.7	6	6	3									145.6 4.5			
														ALLUVIAL ORANGE AND GRAY, SILTY SAND			
135	136.4	13.7	8	10	13									140.2 9.9			
														RESIDUAL GRAY, ORANGE, AND TAN, SAPROLITIC, SANDY SILT			
130	131.4	18.7	13	32	68/0.4									130.9 19.2			
														WEATHERED ROCK (META-ARGILLITE)			
125	126.4	23.7	67	33/0.1										100/0.9 100/0.6 100/0.4			
120	121.4	28.7	100/0.4											116.6 116.3 33.5 33.8			
														CRYSTALLINE ROCK (META-ARGILLITE)			
	116.4	33.7	60/0.1											Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 116.3 ft IN CRYSTALLINE ROCK (META-ARGILLITE)			

NC DOT BORE DOUBLE BR0090_GEO_BRDG_BH.GPJ NC_DOT.GDT 8/8/22

WBS 67090.1.1		TIP BR-0090		COUNTY NASH		GEOLOGIST Jones, A. N.									
SITE DESCRIPTION BRIDGE NO. 36 ON NC 561 OVER FISHING CREEK							GROUND WTR (ft)								
BORING NO. EB2-B		STATION 17+59		OFFSET 9 ft RT		ALIGNMENT -L-		0 HR. 22.4							
COLLAR ELEV. 166.3 ft		TOTAL DEPTH 48.0 ft		NORTHING 893,623		EASTING 2,293,955		24 HR. FIAD							
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 92% 10/12/2020				DRILL METHOD H.S. Augers			HAMMER TYPE Automatic								
DRILLER Pinter, D. G.		START DATE 06/21/22		COMP. DATE 06/21/22		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
170															
165															
	163.4	2.9	2	3	3										GROUND SURFACE 0.0 ROADWAY EMBANKMENT 0.7 PAVEMENT (ASPHALT) ORANGE-BROWN, SANDY CLAY WITH TRACE MICA AND GRAVEL
160															
	158.4	7.9	2	3	4										
155															
	153.4	12.9	2	3	2										
150															
	148.4	17.9	2	2	3										
145															
	143.4	22.9	3	4	7										
140															
	138.4	27.9	7	10	18										
135															
	133.4	32.9	9	18	27										
130															
	128.4	37.9	23	36	55										
125															
	123.4	42.9	7	5	8										
120															
	118.4	47.9	60/0.1			60/0.1									

EB1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	8' LT	15+49	2.9-4.4	A-6(6)	39	15	19.8	32.3	23.8	24.2	100	88	55	-	-
SS-2	8' LT	15+49	8.8-9.4	A-6(4)	31	14	18.3	36.5	19.0	26.2	100	91	52	-	-
SS-3	8' LT	15+49	17.9-19.4	A-4(0)	28	5	5.4	59.2	17.2	18.1	100	99	46	-	-
SS-4	8' LT	15+49	32.9-34.4	A-4(0)	-	NP	19.1	42.1	32.7	6.0	100	93	49	-	-

BL-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-10	9' RT	16+09	10.8-12.3	A-2-4(0)	-	NP	9.5	67.1	13.4	10.1	100	99	33	-	-
SS-11	9' RT	16+09	15.8-17.3	A-4(0)	-	NP	4.4	25.2	64.4	6.0	100	98	82	-	-

B2-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-8	7' LT	17+09	7.7-9.2	A-2-4(0)	-	NP	5.2	73.5	13.2	8.1	100	98	35	-	-
SS-9	7' LT	17+09	17.7-19.2	A-4(0)	-	NP	6.8	35.6	49.4	8.1	100	98	68	-	-

EB2-A

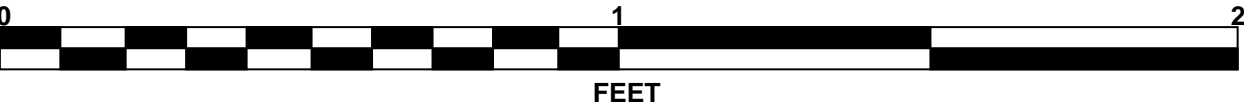
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-5	9' LT	17+59	38.0-39.5	A-4(2)	33	3	6.2	31.8	55.9	6.0	90	87	68	-	-

EB2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	% BY WEIGHT				% PASSING (SIEVES)				%	%
						P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-6	9' RT	17+59	13.4-14.4	A-4(4)	30	8	6.4	36.7	32.7	24.2	100	97	67	-	-
SS-7	9' RT	17+59	42.9-44.4	A-7-5(7)	47	14	17.1	33.8	34.9	14.1	100	91	57	-	-

CORE PHOTOGRAPHS

B1-B
BOXES 1 & 2: 51.5 - 68.5 FEET



CORE PHOTOGRAPHS

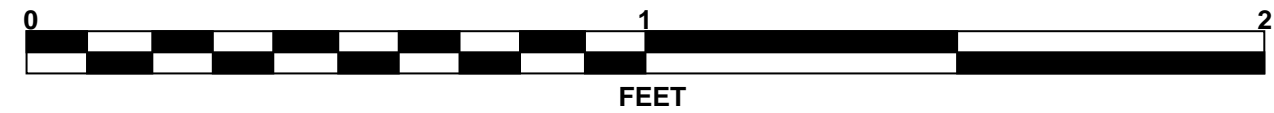
B2-A

BOXES 1 & 2: 32.8 - 54.7 FEET



B2-A

BOXES 3 & 4: 54.7 - 66.3 FEET



SITE PHOTOGRAPH

Bridge No. 36 on -L- (NC 561) over Fishing Creek



Looking Southwest towards End Bent 1