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#### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY NASH

PROJECT DESCRIPTION BRIDGE NO. 36 ON NC 561 **OVER FISHING CREEK** 

# 0600-Ż REFERENCE

# 7090 0 PROJEC

STATE PROJECT REFERENCE NO. STATE SHEETS 16 N.C **BR-0090** 1

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DATE AUGUST 2022



## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT** SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLICHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASITO CLASSIFICATION, AND OTHER PERIMENT FACTORS SUCH CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERIMENT FACTORS SUCH	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPON SAMPLER EQUAL TO R LESS THAN 0.1 FOOT PRE 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REFRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD VIELD SPT N VALUES >
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION	ROCK (WR)
GENERAL         GRANULAR MATERIALS         SILT-CLAY MATERIALS         ORGANIC MATERIALS           CLASS.         ( ≤ 35%, PASSING *200)         ( > 35%, PASSING *200)         0RGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT
GROUP         A-1         A-3         A-2         A-4         A-5         A-6         A-7         A-1, A-2         A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	RUCK (CR) SNEISS, GABBRO, SCHIST, ETC.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-75 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.
SYMBOL BOOCOUNT STATE ST	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN
X PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.
*10 50 MX GLAY MUCK, SOLIS CLAY PEAT		WEATHERING
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN	ORGANIC MATERIAL GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER
MATERIAL PASSING =40	TRACE OF ORGANIC MATTER         2         - 3%         3         - 5%         TRACE         1         - 10%           LITTLE ORGANIC MATTER         3         - 5%         5         - 12%         LITTLE         10         - 20%	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,
LL – – 40 MX 41 MN	MODERATELY ORGANIC         5         10%         12         20%         SOME         20         - 35%           HIGHLY ORGANIC         >         10%         >         20%         HIGHLY         35%         AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF
	GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO
USUAL TYPES STONE FRACS ORGANIC SUILS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOULS SOULS	STATIC WATER LEVEL AFTER <u>24</u> HOURS	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MATERIALS SAND SHID SHITLE HIS SHID SOLD SOLD SHID SOLD SHID SHID SHID SHID SHID SHID SHID SHI	$\nabla$ PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	E 	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
PI OF A-7-5 SUBGROUP IS $\leq$ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL
VERY LOOSE < 4	WITH SOIL DESCRIPTION FOR ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT CHAUGER BORING CONE PENETROMETER TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE
(NON-COHESIVE) VERY DENSE > 50		SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK
VERY SOFT         < 2         < 0.25           GENERALLY         SOFT         2 TO 4         0.25 TO 0.5	I - INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</i>
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	TEST BORING WELL	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND
MATERIAL         STIFF         8 TO 15         1 TO 2           (COHESIVE)         VERY STIFF         15 TO 30         2 TO 4		SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.
		ROCK HARDNESS
TEXTURE OR GRAIN SIZE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.
(BLDR.) (COB,) (GR.) (CSE, SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY $\gamma$ - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{d}$ - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS
	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO         SD SAND, SANDY         SS - SPLIT SPOON           F - FINE         SL SILT, SILTY         ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY
	FOSS FOSSILIFEROUS         SLI SLIGHTLY         RS - ROCK           FRAC FRACTURED, FRACTURES         TCR - TRICONE REFUSAL         RT - RECOMPACTED TRIAXIAL	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.
BANGE - WET - (W) SEMISULID; REQUIRES DRIING TO	FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING
	HI HIGHLY V - VERY RATIO	TERM         SPACING         TERM         THICKNESS           VERY WIDE         MORE THAN 10 FEET         VERY THICKLY BEDDED         4 FEET
OM _ OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT         USED         ON         SUBJECT         PROJECT           DRILL         UNITS:         ADVANCING         TOOLS:         HAMMER         TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET
SL SHRINKAGE LIMIT		MODERATELY CLOSE         1 TO 3 FEET         THINLY BEDDED         0.16 - 1.5 FEET           CLOSE         0.16 TO 1 FOOT         VERY THINLY BEDDED         0.03 - 0.16 FEET
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET
PLASTICITY	X         CME-55         CME-55         CME-55         CME-55         CORE SIZE:	INDURATION
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH	X         CME-550         Image: Arrow of the second	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC
NON PLASTIC 0-5 VERY LOW		FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS;
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM	HAND TOOLS:	GENILE BLOW BY HAMMER DISINIEGRATES SAMPLE.
HIGHLY PLASTIC 26 OR MORE HIGH		MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
COLOR		INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	Image: Construction of the second	DIFFICULI IU BREAK WITH HAMMER.
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

#### PROJECT REFERENCE NO. **BR-0090**



TERMS AND DEFINITIONS

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

SURFACE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT BOCKS OR CUTS MASSIVE ROCK.  $\underline{\text{DIP}}$  - The angle at which a stratum or any planar feature is inclined from the horizontal. OATINGS IF OPEN. <u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. AMMER BLOWS IF FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  $\underline{\mathsf{FLOAT}}$  - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. OSS OF STRENGTH JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. ALUES < 100 BPF RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. ROCK.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT S REQUIRES <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO LOWS REQUIRED THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT EEP CAN BE OR SLIP PLANE. ETACHED STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL OR PICK POINT. WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL BLOWS OF THE TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. FRAGMENTS IT. SMALL, THIN STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. PIECES 1 INCH ED READILY BY TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: BL-102, REBAR WITH CAP, -L- STA. 15+16 18'RT THICKNESS 4 FEET 1.5 - 4 FEET ELEVATION: 165.7 FEET 16 - 1.5 FEET NOTES: - 0.16 FEET PROFILE AND CROSS SECTIONS EXISTING GROUNDLINES TAKEN FROM BR0090 RDY\_PFL\_DGN DATED 01/25/2022 AND BR0090\_RDY\_XSC 25.DGN DATED 01/26/2022. 08 - 0.03 FEET 0.008 FEET TOP OF RAIL @ EBI-L- STA. 15+53, 15'RT ELEV.= 168.5 AT. PRESSURE, ETC. TOP OF RAIL @ EB2 -L- STA. 17+55, 15'RT ELEV.= 168.5 EEL PROBE:

#### DATE: 8-15-1-

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

AASHTO LRFD Figure 10.4.6.4–1 — Determination of GSI for Jointed F	Rock Mass (Marı	nos and Hoek,2	:000)			AASHTO LRFD Figure 10.4.6.4–2 — Determination of GSI for T
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed F GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000) From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	<b>GOOD</b> rough, fresh unweathered surfaces	<b>GOOD</b> Rough, slightly weathered, iron stained surfaces	ch, moderately weathered and ed surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	<b>VERY POOR</b> Slickensided, highly weathered surfaces with soft clay coatings or fillings	AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for T GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000) From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fai poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by
STRUCTURE	V KERY Very DEC		JRFACE QUA		~	using effective stress analysis.
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90			N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets		70 60				B. Sand- stone with stone and or silty shale
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets		5	0			thun inter- layers of siltstone siltstone with sand- stone layers amounts
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity			40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.
discontinuity sets. Persistence of bedding planes or schistosity DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces				20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	N/A	N/A			10	Sandstone are trad into small rock pu → Means deformation after tectonic disturbance

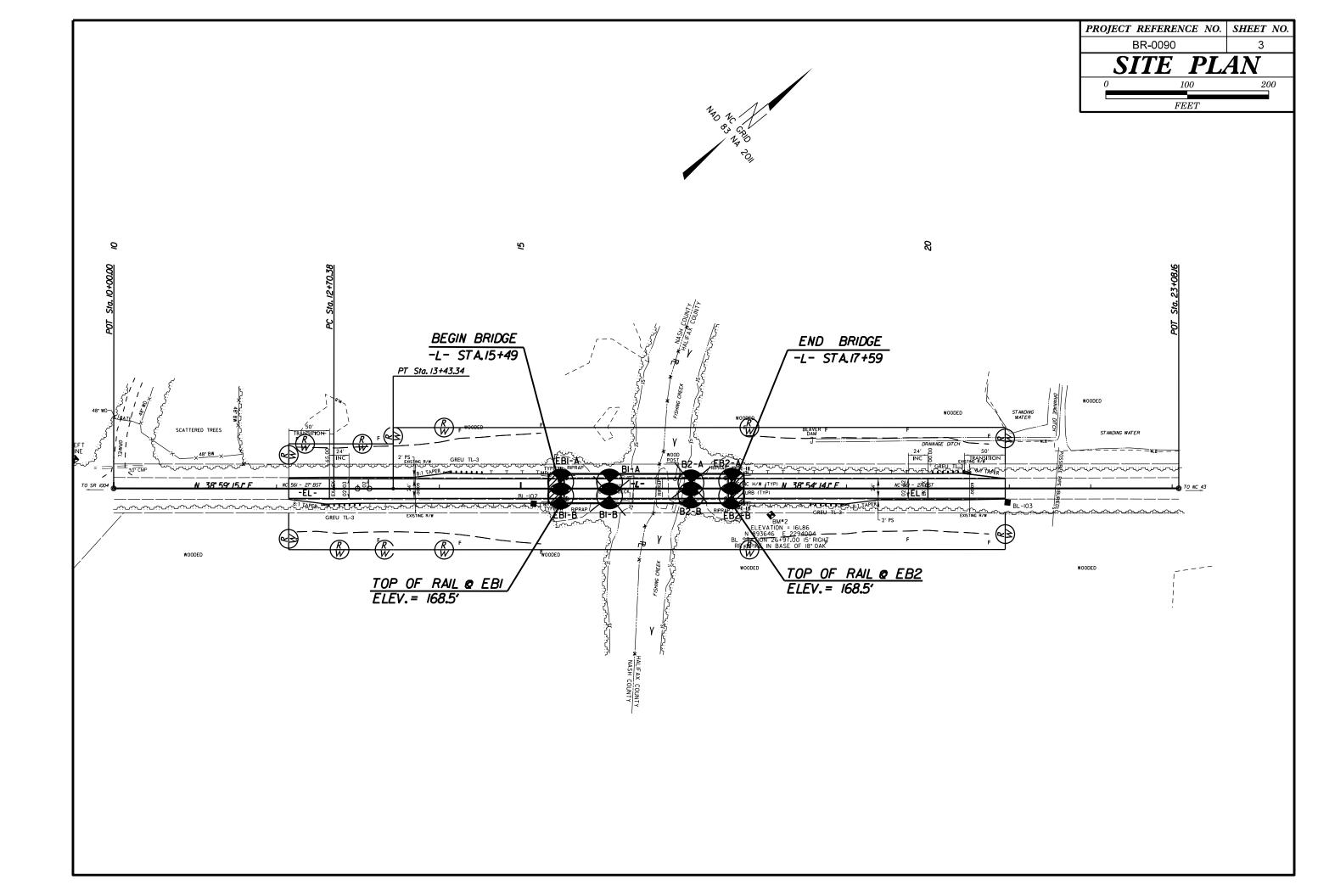
Fectonically Defo	rmed Heterog	geneous Rock	Masses (Marır	nos and Hoek	, 2000)
ی تر بر ه ع SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)	VERY GOOD - Very Rough, fresh unweathered surfaces	<b>GOOD -</b> Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	<b>VERY POOR -</b> Very smooth, slicken- sided or highly weathered surfaces with soft clay coatings or fillings
E. Weak siltstone or clayey shale with sandstone layers	70 60	A 50 B 40	СЦ	E	
oformed, d/faulted, ale or sultstone deformed forming an tructure oformed sulty forming a with pockets ers of			30	F 20	10 1/
ansformed veces.					

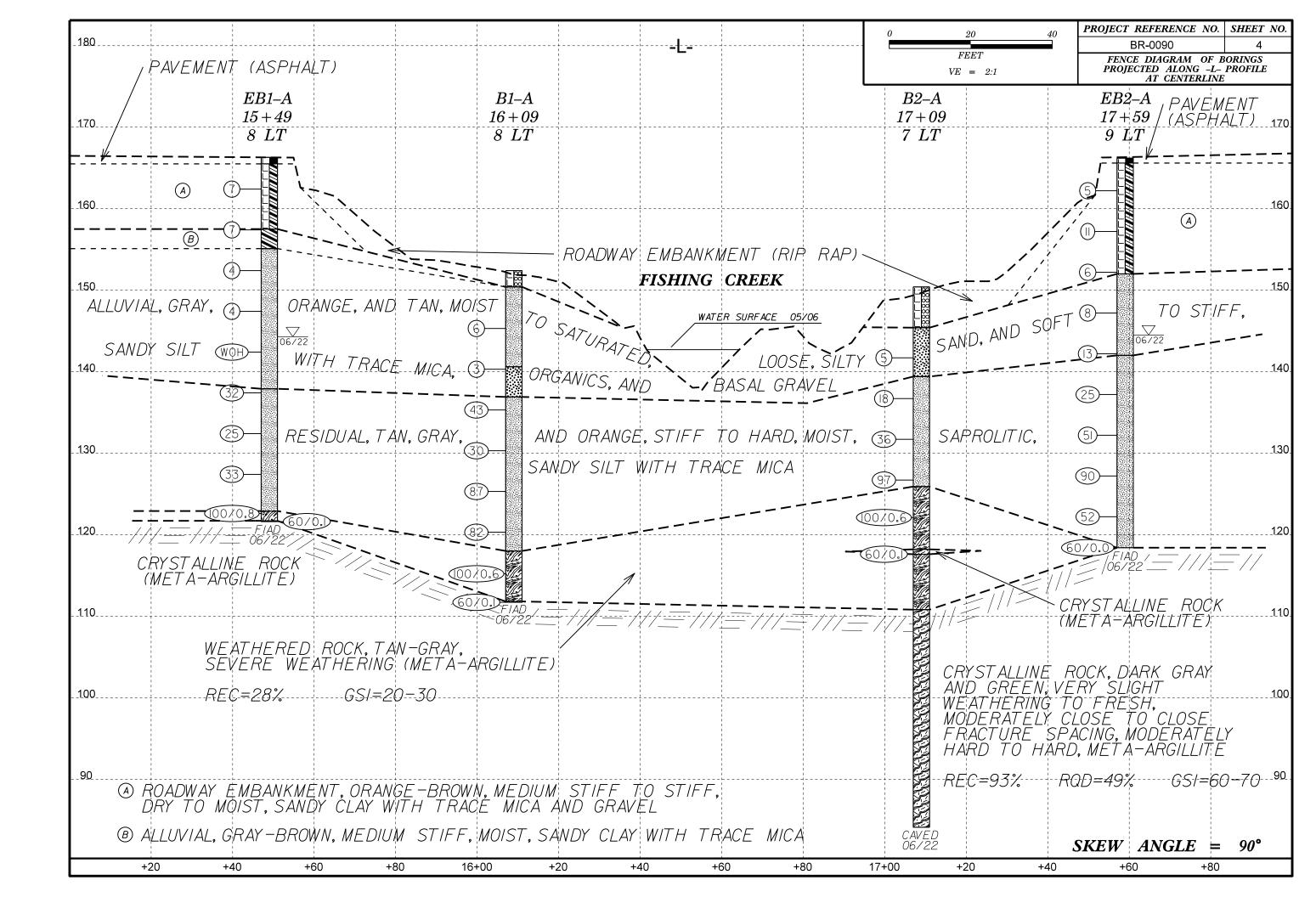
PROJECT REFERENCE NO.

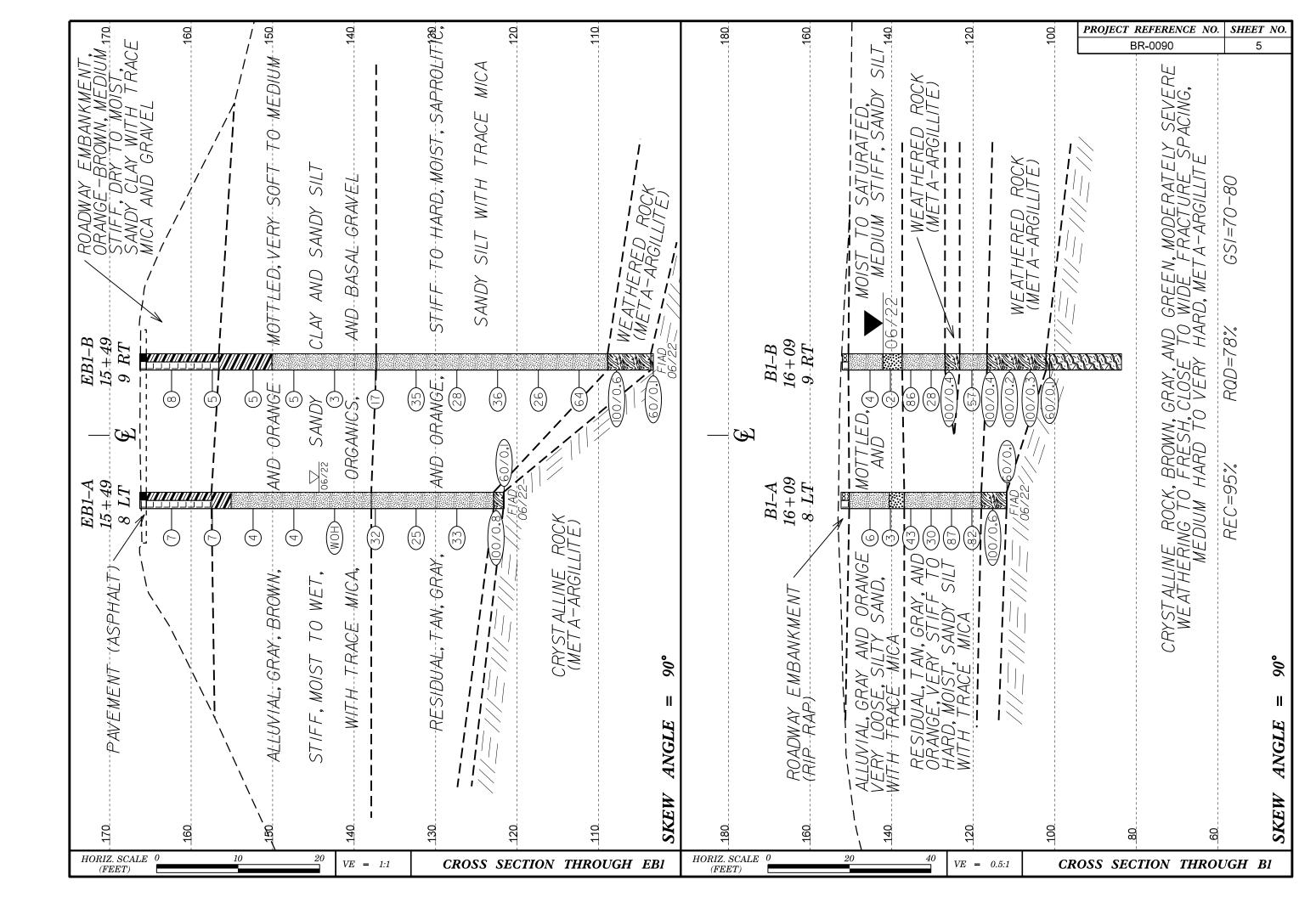
BR-0090

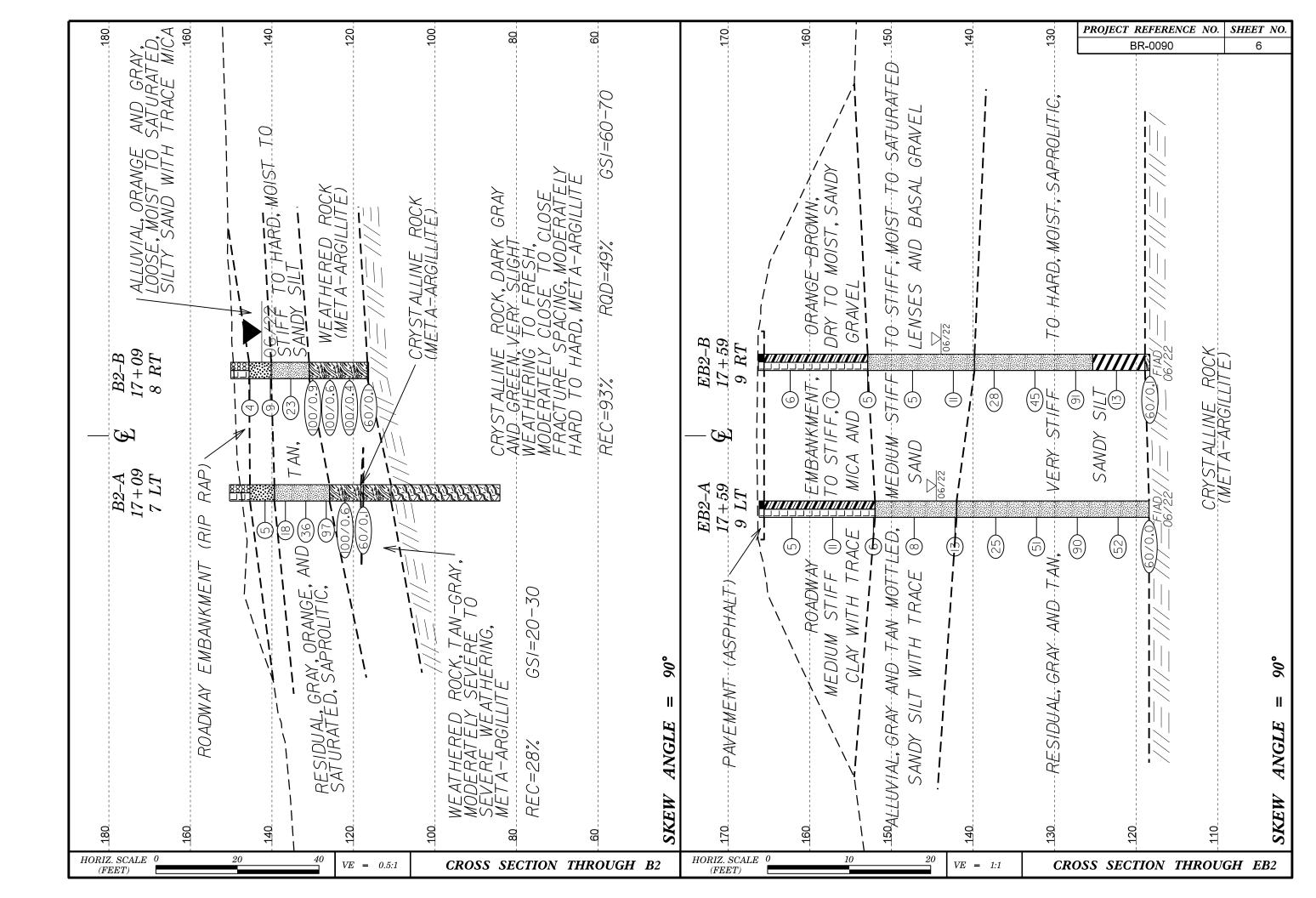
SHEET NO.

2A



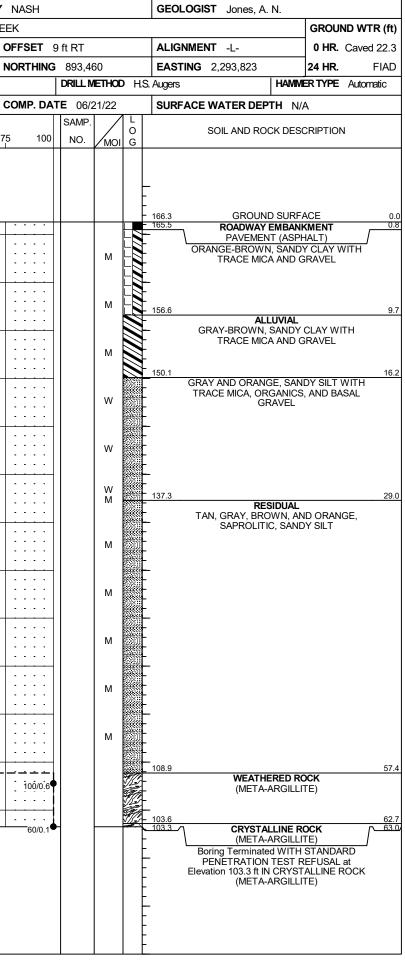




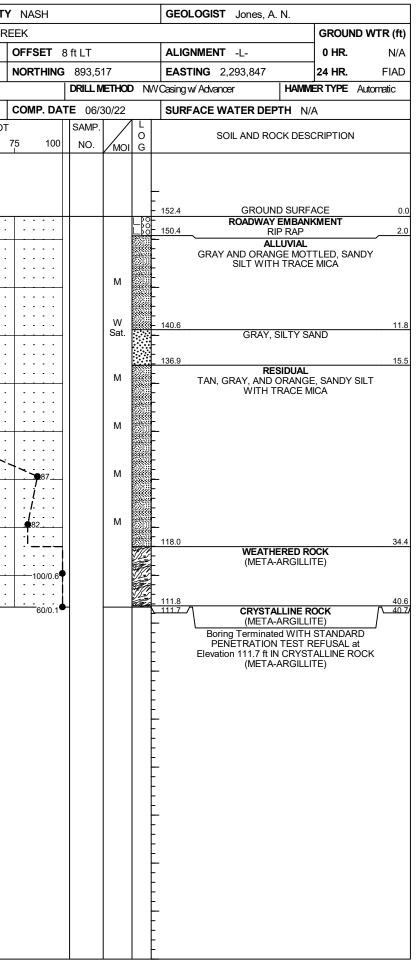


#### GEOTECHNICAL BORING REPORT BORE LOG

				- 1																					1					
67090.1.						R-009			COUN						GEC	LOGIST Jones, A. N					67090					P BR-			COUN	
DESCRIPT	TION	BRID	GE NO	-					HING									GROUND W	rr (ft)					DGE N	O. 36	ON NC	561 C	OVER FI	SHING C	
<b>ING NO.</b> E	EB1-A			S	TATIO	<b>ON</b> 1	15+49	9		OF	FSET 8	ft LT			ALIC	INMENT -L-		0 HR.	22.0	BOR	NG NO.	EB1-I	3		S	TATION	<b>1</b> 5+	+49		0
LAR ELEV.	. 166	6.3 ft		Т	OTAL	_ DEP	тн	44.7 ft		NO	rthing					<b>TING</b> 2,293,810	1	24 HR.	FIAD	COLI	LAR ELI	<b>EV.</b> 16	6.3 ft		Т	OTAL C	DEPTH	<b>i</b> 63.0 f	ft	N
RIG/HAMME	ER EFF	JDATE	RFO	0074 C	ME-5	5 92%	5 10/12	2/2020				DRILL	/IETHC	DD⊢	I.S. Augers	;	HAMME	RTYPE Auton	natic	DRILL	. RIG/HAN	/IMER EF	F./DAT	E RFO	20074 C	ME-55 9	32% 10	)/12/2020		
LER Pinte				S	TART	T DAT	<b>E</b> 0	6/20/2	2	СО	MP. DAT	<b>E</b> 06/2	20/22	2	SUR	FACE WATER DEPT	H N/A			DRIL	LER P				S	TART C	JATE	06/21/2	22	C
DRIVE ELEV DE	ЕРТН	BLO	w col	JNT			BL	OWSI	PER FO	от		SAMP.	▼⁄			SOIL AND ROC				ELEV	DRIVE ELEV	DEPTH	BLC	w co	UNT			BLOWS	PER FOO	JT
(ft)	(ft)	0.5ft	0.5ft	0.5ft	0		25		50	75	100	NO.	Имс	DI G	ELEV.		IT DECC		EPTH (ft)	(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	5	50	75
															L					170		Ļ								
<u>†</u>															E						-	ŧ								
					 		<b>.</b>	<u></u>		• • •					- 166.3 - 165.5	GROUND			0.0 0.8			<u> </u>					• • • •			<u>.</u> т
					∣⊢i		+ :								-	- ROADWAY E PAVEMENT	(ASPH/	ALT)	0.0	165	-	+							+	-+
163.4 7	2.9	2	3	4		7						SS-1	D			ORANGE-BROWN, TRACE MICA					163.4	2.9	3	4	4					•
I Ŧ					.								1		F					160		Ŧ				.j.				-
158.4	7.9	-													-						158.4	7.9				1.				•
Ŧ		2	3	4	.	7	:					SS-2	M		- 157.5 -		UVIAL		8.8			ŧ	2	2	3	<b>•</b> 5				:
+					∔										155.1	GRAY-BROWN, S → TRAC	Sandy C E Mica	LAY WITH	11.2	155	-	ŧ				╽│╷╴				÷
153.4 + 1	12.9	1	2	2		· · ·		· · · · · ·		· · ·	· · · ·		м		-	GRAY AND ORANG					153.4	<u>† 12.9</u>	2	2	3			· · · · ·		:
‡					¶4	'		:::							F		AVEL			150	-	ŧ				¶°:	::	· · · · ·		
	17 0				<del>   </del> - 										F					150	- 148.4	+ + 17 0								
	17.5	2	2	2	4		:	· · · · · ·			· · · ·		м		-							+	2	2	3	<b>6</b> 5		· · · · ·		:
					<u>    .</u>										L					145		ŧ				<u> :</u> :				·
143.4 2	22.9	WOH	WOH	WOH		· · ·		· · ·	· · ·	· ·											143.4	22.9	1	2	1		· ·	· · · ·		:
<u>†</u>		won	WOII	WO	H∳0. I		:			-   -		SS-3	W		Ł							ŧ	'		'	•3 -	::			:
{ +						~									F					140	_	Ŧ								-+-
138.4 + 2	27.9	10	14	18				32					W M		137.9	PESI	IDUAL		28.4		138.4	<u>† 27.9</u> †	10	9	8		17	· · · · ·		
Ŧ							1								F	TAN-GRAY, SAPRO		SANDY SILT		135	-	ŧ					$\mathbb{N}$			:
	32.9						1.								-					100	133.4	32.9						<u> </u>		-
T T		8	11	14	:	· · ·	<b>4</b> 25	· · · ·				SS-4	М		F							Ŧ	16	14	21			35		:
							<u>i</u> v.								-					130	_	‡					· · ·	<u>. /</u>		÷
128.4 3	37.9	8	8	25		· · ·	] ]	· · ·							L						128.4	37.9	7	11	17			1 		:
		Ŭ	Ŭ	20				33					М									ł	'					Q28		
+								<u>.</u>							F					125	-	+						<u>\.</u>	<u> </u>	-+
123.4 + 4		18	82/0.3		-	· · ·		<u> </u> 	. <u></u> _		100/0.8				122.9	WEATHER	RED RO	ск	43.4 44.6		123.4	1 42.9 1	11	18	18					•
+		60/0.1									<b>−</b> 60/0.1●				121.6	(META-Al CRYSTALI			44.7	120		+						. /		
-															-	(META-AI	RGILLIT	E)			118.4	47.9			47		• •	<i>j</i>		•
I Ŧ															F	Boring Terminated PENETRATION T	TEST RE	FUSAL at				Ŧ	8	9	17		-	26		
+															F	Elevation 121.6 ft IN ( (META-Al				115	-	Ŧ								<u> </u>
1															F	Υ Υ		,			113.4	<u>† 52.9</u>	11	17	47			· · · · ·		-
‡															F					110	-	ŧ							<b>.</b>	
‡															-						108.4	+ + 57.9								
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‡															È.					105	-	‡								÷
‡															È						103.4	62.9	60/0.1	<u> </u>			• •		• • • •	·
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	67090							-0090		_	-	UNT
	DESCR		BRID	DGE N						FIS	HING	g CF
	NG NO.	B1-A	0.4.7						+09	7 ^		
	.ar ele .rig/han										1	
	<b>LER</b> Pi DRIVE	1		W CO		T			BLOV			-00
ELEV (ft)	ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	0.5ft		0	25			60	00
155												
	-	Ē										
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150	-	F					<u> </u>				<u> </u>	
	-						] .	· ·	· ·	 	:	 
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4.45	141.3	11.1	1	1	2		<u> </u>   <u> </u>	::	•••		:	
140	-	F			2		• <u>3</u>		 		<u>.</u>	
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135	136.3	16.1	11	19	24	1		•••		43	<u>-</u>	
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	131.3	21.1	10	13	17		· · ·	::	./.		:	
130	-	F		15			 		<b>4</b> 30_	~	<u>-</u>	
	-						· · ·	::	· · · ·		1-	
125	126.3	26.1	16	48	39	1		•••		 	-	
	-	L					· ·	· ·	•••		:	
	121.3	31.1	21	25	47		· ·	: :	•••		:	
120		F	31	35	47		<u></u>				:	
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115	116.3	36.1	27	58	42/0.1	1	· ·		••		.	
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	111.8	40.6	60/0.1			┞				<u>.</u> .	.	
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#### **GEOTECHNICAL BORING REPORT** BORF I OG

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WBS	67090	0.1.1			Т	IP BR-00	90	COUNT	Y NASH			GEO	LOGIST Jones, A.	N.		WB	<b>S</b> 6709	0.1.1			TIP	BR-00	)90	С	OUN
SITE	DESCR	RIPTION	BRI	DGEN	IO. 36	ON NC 56	1 OVER FIS	SHING CF	REEK						GROUND WTR (ft)	SITE	E DESCF	RIPTION	BRID	DGE NO.	36 ON	NC 56	61 OVER	FISHI	NG C
BORI	ing no.	. B1-B			s	TATION	16+09		OFFSET	9 ft RT		ALIG	NMENT -L-		0 HR. N/A	BOF	ring no	. B1-B			STA	TION	16+09		
COLI	LAR ELI	<b>EV.</b> 15	52.1 ft		Т	OTAL DE	<b>PTH</b> 68.5 f	ť	NORTHING	893,5	06	EAS	<b>FING</b> 2,293,861		<b>24 HR.</b> 10.0	COL	LAR EL	<b>EV.</b> 15	52.1 ft		тот	AL DE	<b>PTH</b> 68	.5 ft	
DRILL	RIG/HAN	VIMER EF	FF./DAT	EHF	00072 (	CME-550X 8	7% 02/23/20	21	•	DRILL	/IETHOD N	WCasing	w/ Core	HAMME	RTYPE Automatic	DRIL	l Rig/Hai	MMER EF	F./DATI	E HFC00	72 CME	-550X 8	87% 02/2	3/2021	
DRIL	<b>LER</b> P	Pinter, D	. G.		s	TART DA	<b>FE</b> 06/28/2	22	COMP. DA	TE 06/	29/22	SUR	ACE WATER DEP	TH N/A		DRI	LER F	Pinter, D	. G.		STA	RT DA	TE 06/2	28/22	
ELEV	DRIVE ELEV		BLC	ow co	UNT		BLOWS	PER FOOT	r	SAMP.		·	SOIL AND ROC		RIPTION	COF	RE SIZE	NXWL			ТОТ	AL RUI	<b>N</b> 17.01	ft	
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50	75 100	NO.	MOI Ğ				DEPTH (ft)	ELE\	, RUN ELEV	DEPTH	RUN	DRILL RATE	REC.	UN RQD	SAMP.	REC.	RATA
																(ft)	(ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %
155		Ļ										L				100 e 100	100.6	51.5		4.40/4.0	(4.7)	(1.0)		(4.0.0)	
	-	‡										- 152.1	GROUNE		CE 0.0		98.6	53.5	2.0	1:18/1.0 1:09/1.0	85%	50%		95%	) (13.2 78%
150		<u>†</u>				<u>   ! : :</u>						0- 150.6	ROADWAY B	EMBANK				Ŧ	5.0	0:45/1.0 0:47/1.0	(4.5) 90%	(2.2) 44%			
150	-	‡										-	ALL	P RAP		95		Ŧ		0:30/1.0 0:44/1.0		lan (			
		±											GRAY AND ORANG SILT WITH				93.6	<u>- 58.5</u> -	5.0	0:54/1.0	(5.0)	(5.0)			
145	146.3	<u> </u>	2	2	2	-   <b>↓</b> 4					м	L				90		ŧ		0:52/1.0 0:54/1.0	100%	100%	RS-1		
		‡				:::	.				🕳 🎆						88.6	63.5		1:05/1.0		L			
	141.3	10.8	1	1	1	:::	.			00.15		142.1	GRAY, SILTY SAND	D WITH T	10.0 TRACE MICA			‡	5.0	1:12/1.0		(5.0) 100%			
140	-	ŧ	'	'	'	¢2.	<b>_</b>			SS-10	Sat.					85		‡		2:42/1.0 1:46/1.0		lan (			
		+					· [	· <b> </b> · · · · ·				137.4	DEO		14.7		83.6	<u>+ 68.5</u> +		2:08/1.0		+			+
135	136.3	<u> </u>	16	38	48	1		```	<b>→</b>	SS-11	м	L	TAN AND ORANGE	E, SAND	Y SILT WITH			‡							
		ŧ									1	L	TRAC	CE MICA			-	ŧ				ľ			
	131.3	20.8	10	47								L						‡				lan (			
130		ŧ	12	17	11		<b>P</b> 28	+			M	F						‡				lan (			
		Ŧ										126.9			25.2			‡				lan (			
125	126.3	<u>† 25.8</u> 1	100/0.4	4				<u> </u>	100/0.4				WEATHE (META-A					‡				ľ			
		Ŧ										123.3					-	‡				lan (			
	121.3	30.8		00									TAN AND ORANG		Y SILT WITH			‡				lan (			
120	-	Ŧ	22	28	29			<b>.............</b>			M	F	TRAC	CE MICA				‡				ľ			
		Ŧ										_ _ 116.6			35.5			‡				lan (			
115	116.3	<u>T 35.8</u> T	100/0.4	4				<u>`</u>	100/0.4			<u>- 110.0</u>	WEATHE		CK			ŧ				lan (			
	] .	Ŧ											(META-A	ARGILLII			-	ŧ				lan (			
	111.3	T 40.8																ŧ				lan (			
110		Ŧ	100/0.2	2				+ • • • •	100/0.2			<b>F</b>						ŧ				lan (			
	.	Ŧ																‡							
105	106.3	<u> </u>	100/0.3	3						<b> </b>								‡							
	-	Ŧ										Ŧ				8/8/22	-	‡							
	101.3	50.8					· · · · · · ·					<u>102.1</u> 100.6	CRYSTAL			GDT 8		‡							
100	-	Ŧ	60/0.1						- 60/0.1	[]			META-A BROWN, GRAY, A	ARGILLIT	E) / 51.5	DOT.0	.	‡							
		Ŧ										Ŧ	WEATHERING TO WIDE FRACTURE	FRESH	, CLOSE TO	NC		‡							
95		Ŧ										£	HARD TO VERY HA			BH.GPJ		‡							
	] .	Ŧ										Ŧ	REC=95% RQE	D=78%	GSI=70-80	BH	-	‡							
		Ŧ					· · · · · · ·			RS-1		Ŧ				BRDG		‡							
90	-	Ŧ								<u></u>	1	ł				GEO_B	.	‡							
	.	Ŧ										Ŧ						‡							
85	.	Ŧ										Ŧ				BR0090		‡							
-	<u> </u>	Ŧ	<u> </u>	<u> </u>	<u> </u>	· · · ·						83.6	Daving Tage in the	at Elson	68.5		.	‡							
		Ŧ										F	Boring Terminated CRYSTALLINE ROC			DOUE		‡							
	-	Ŧ										F				CORE DOUBLE	.	‡							
		Ŧ										F				01 C(		‡							
	.	Ŧ			1							F				NCDOT		t				ſ			
		<u> </u>				1										<u> </u>						<u>ل</u> ــــــــــــــــــــــــــــــــــــ		-	4

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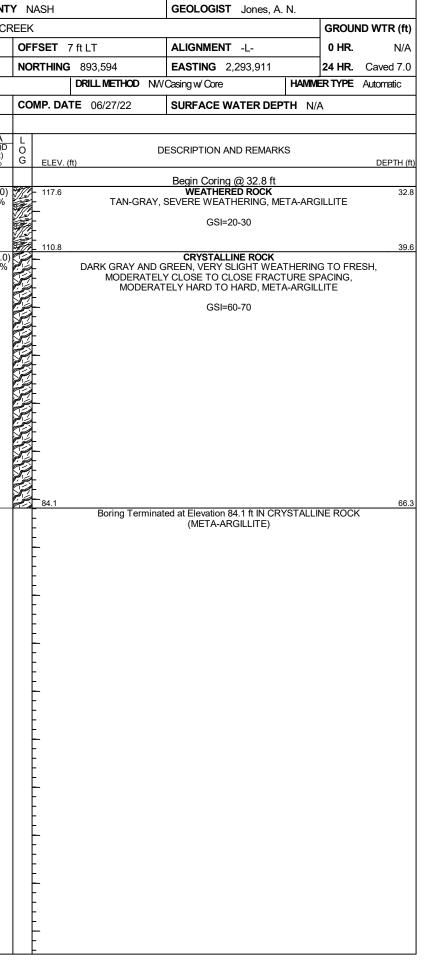
### **GEOTECHNICAL BORING REPORT** CORE LOG

	C	OUNT	YN	ASH			GEOLOGIST	Jones, A.	N.		
2	FISHIN	NG CR	EEK	2						GROUN	ND WTR (ft)
			OF	FSET 9	9 ft RT		ALIGNMENT			0 HR.	N/A
	5 ft		NO	RTHING	893,506		EASTING 2,2	293,861		24 HR.	10.0
	/2021				DRILL METHOD	NW	-				Automatic
-	8/22		со	MP. DA1	TE 06/29/22		SURFACE WA	TER DEP	TH N/A	4	
ft	STR	ATA	L								
	REC. (ft) %	RQD (ft) %	0 G		(4)	D	ESCRIPTION ANI	D REMARK	S		
	70	70		ELEV. (f	(it)		Begin Coring @	ন্স 51 5 ft			DEPTH (ft)
	(16.2) 95%	(13.2) 78%	P	- 100.6			ND GREEN, SEVE FRACTURE SPAC	ERE WEAT			
	0070	10%	R	-			HARD, META-A	ARGILLITE	01111/0		
			R	-			GSI=70	-80			
				-							
_				-							
				-							
				_							
				- 83.6							68.5
				-	Boring Ter	minat	ed at Elevation 83 (META-ARG)	.6 ft IN CRY GILLITE)	'STALLII	NE ROCK	(
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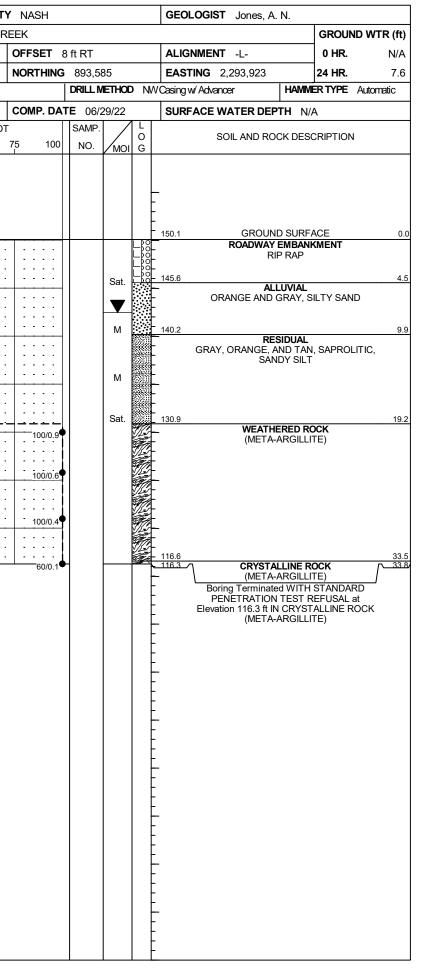
#### **GEOTECHNICAL BORING REPORT** BORE LOG

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	67090					TIP BR-009			TY NASH				GEOLOGIST Jones, A. N	l				67090					BR-00			OUNT	
				DGE N		ON NC 56		ISHING C	-						GROUND WT	R (ft)				BRID	DGE NO.				FISHI	NG C	RE
BOR	NG NO.	. B2-A				STATION	17+09		OFFSET	7 ft LT			ALIGNMENT -L-		0 HR.	N/A	BOF	ing no.	B2-A			STAT	ION	17+09			C
	LAR EL					TOTAL DEF			NORTHING	,			EASTING 2,293,911		24 HR. Cave			LAR ELE						<b>PTH</b> 66	-		N
DRILL	. RIG/HAI	VIMER EF	F./DAT	EHF	20072	CME-550X 8	7% 02/23/2	021		DRILL	/IETHO	<b>D</b> N	W Casing w/ Core	HAMIME	RTYPE Autom	atic	DRIL	L RIG/HAN	MER EF	F./DATI	E HFOOD	72 CME-	550X 8	37% 02/2	3/2021		
DRIL	LER P				:	START DAT	<b>E</b> 06/22	22	COMP. DA	<b>TE</b> 06/	27/22		SURFACE WATER DEPT	H N/A	\		DRI	<b>.LER</b> Pi	nter, D	. G.		STAF	RT DA	TE 06/2	22/22		С
ELEV	DRIVE ELEV	DEPTH		ow co				PER FOC		SAMP.	▼⁄		SOIL AND ROCH	( DESC	RIPTION		COF	E SIZE	NXWL					<b>N</b> 33.5 f			
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5f	t 0	25	50	75 100	NO.	Имо	I G	ELEV. (ft)			PTH (ft)	ELE	RUN	DEPTH		DRILL RATE	REC.	RQD	SAMP.	REC.		576
																	(ft)	(ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	G
155		ł											_				117.6	117.6 .	32.8	2.7	0.40/0.7	(0.8)	(0.0)		(1.9)	(0.0)	
		ŧ											-				115		35.5	2.1	0:40/0.7 0:32/1.0 0:22/1.0	30%	0%		28%	0%	
150		ŧ											- 150.4 GROUND	SURFA	CE	0.0		-	-	5.0	0:28/1.0 0:29/1.0	(2.1)	(0.4) 8%				
100	-	Ŧ											ROADWAY EI		MENT			-			0:26/1.0 1:03/1.0						
		‡							· · · · · ·				- -				110	109.9	40.5	4.6	1:04/1.0 0:45/1.0	(4.0)	(3.3)		(24.9)	) (13.0 49%	1)
145		‡											- 145.4 	VIAL		5.0		-	-		0:58/1.0	87%	72%			-070	
	142.7	+						·   · · · ·   · · ·					GRAY, SILTY SA	ND WIT	H TRACE		105	105.3	45.1		0:56/1.0			RS-2	-		j.
140		‡	3	3	2	<b>4</b> 5°°°		.   .	·   · · · · ·	SS-8	М							1 -		3.9	1:21/1.0 1:56/1.0 1:37/1.0	(2.6) 67%		·	1		5
140	-	‡								11				ואטכ		11.0		101.4 -	49.0		2:03/0.9						5
	137.7	+ 12.7 +	5	6	12	-  ::`\			· · · · · ·		М		GRAY AND TAN, S	APROL	TIC, SANDY		100	99.1	- 51.3	2.3	0:52/0.3	(2.3) 100%	(0.9) 39%				j.
135	-	ŧ												_ 1					-	5.0	2:04/1.0 1:07/1.0 1:11/1.0	(5.0) 100%	(2.3)				
	132 7	+ + 17.7							· · · · ·								95	-	-		1:12/1.0	100 /0	4070	RS-3			
		1	10	14	22	7 ::::	•36			SS-9	М		-					94.1	56.3	5.0	1:02/1.0	(5.0)	(0.5)				
130	-	ŧ											<b>k</b>					-	-	0.0	1:01/1.0	100%	10%				R
	127.7	22.7	43	37	60	_  ::::					м						90	89.1	-		1:01/1.0						
125		ł							<b>.</b>	97 <b>1</b>		17	_ 125.9 WEATHER	ED RO	СК	24.5			-	5.0	1:21/1.0	(5.0)	(4.0)				
	100 7	F 27.7								1			(META-AF				0.5	-	-		1:20/1.0	100%	80%				
		<u>+ 2/./</u> +	48	52/0.1				.	- 100/0.6	<b> </b>							85	84.1	66.3		1:07/1.0 1:29/1.0						ن میشیخ میشیخ
120	-	Ŧ								{			-					-	-								
	117.7	32.7	60/0.1	4				· · · · ·		i l			118.2 117.6 CRYSTALL	INE RC	CK -	32.2 32.8		-	_								
115		ŧ	60/0.1							T			(META-AF WEATHER	RGILLIT	E)			-									
110	-	ŧ								1			TAN-GRAY, SEVE	RE WE	ATHERING,			-	-								
		‡							· · · · · ·				META-AF					-	-								
110	-	‡											<u>110.8</u> REC=28%			39.6		-	-								
		‡						· · · · ·	· · · · · ·				DARK GRAY AND GI	REEN, ۱	/ERY SLIGHT			-	-								
105		‡						·   · · ·	· · · · · ·	RS-2	4		MODERATELY C FRACTURE SPACE	LOSE T	O CLOSE				-								
105	-	‡									1		HARD TO HARD,				8/8/22		F								
		‡						·   · · · ·   · · ·	· · · · · ·	!			REC=93% RQD	=49%	GSI=60-70		GDT 8/	-	F								
100	-	‡											₽- 				DOT.GI	-	F								
		<b>‡</b>						.   .	·   · · · · ·	!			4_ 1_					-	-								
05		<b>‡</b>								RS-3	-		4				GPJ NC	-	-								
95	-	ŧ									1		4 1_				BH.G		-								
		ŧ								!		Ê	4 4-				BRDG	-	Ļ								
90	_	t								]			<u> </u>						ŀ								
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85		£					<u> </u>						84.1					-	Ļ								
		Ŧ											Boring Terminated a				DOUBLE	-	L L								
		Ŧ												<u> </u>			8		-								
	-	Ŧ											F				CORE	-	Ļ								
		‡											F				NCDOT	-	F								
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#### **GEOTECHNICAL BORING REPORT** CORE LOG

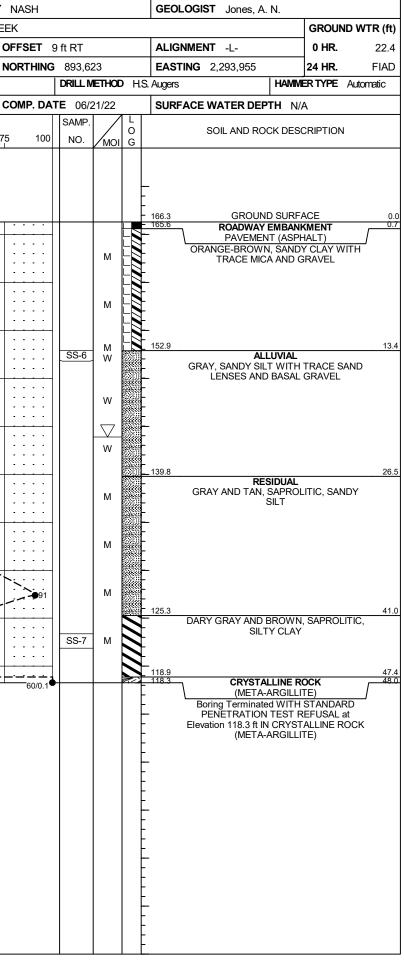


WBS	67090	.1.1			Т	IP	B	R-0090		COUNT
SITE	DESCR	IPTION	BRID	GE N	0. 36	0	N N	C 561 (	OVER FIS	SHING CR
BORI	NG NO.	B2-B			s	T/		<b>DN</b> 17	+09	
COLI	AR ELE	<b>EV.</b> 15	0.1 ft		Т	0	TAL	DEPT	H 33.8 f	t
DRILL	. RIG/HAN	IMER EF	F./DATI	E HFC	0072 0	21	Æ-58	50X 87%	6 02/23/20	21
DRIL	LER Pi	nter. D.	G.		s	Т/	ART	DATE	06/29/2	2
ELEV	DRIVE	DEPTH		W CO		Ι				PER FOOT
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5ft	1	0	2	5	50
										•
155										
	-	F								
	-	F								
150	-	<u> </u>					-			_ · · · ·
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145	146.4	3.7	5	2	2	$\left  \right $	L.		· · · · ·	
1 <del>4</del> J	-	ŧ								+
	- 141.4	8.7					'	 	· · · · ·	
140		L	6	6	3	1	Ŀ	9		
	-	ŧ					:	ix :	· · · ·	
	136.4	13.7	8	10	13		:	$\sum_{i=1}^{n}$		
135	-	+		10	13		<u> </u>		23	+ • • • •
	-	F						j		
130	131.4	18.7	13	32	68/0.4		:	· · · [	· · · · ·	
100	-	ŧ								
	- 126.4 <sup>-</sup>	23.7					:	· · · · · ·	· · · · ·	
125		-	67	33/0.1			·			
	-	ŧ					·	· · ·	· · · ·	
	121.4	28.7	100/0.4				:		· · · ·	
120	-	Ł	100/0.4				-			+
	-	F						• • •		
	116.4	33.7	60/0.1			╀				
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#### GEOTECHNICAL BORING REPORT BORE LOG

										JKE L	00																
	67090					<b>P</b> BR-00				NASH				GEO	DLOGIST Jones, A. N.				67090					<b>P</b> BR-00		COUN	
				DGE N		ON NC 56		FISHIN								-	JND WTR (ft)		-	-		DGE N			61 OVER F	ISHING C	-
	NG NO.					TATION				OFFSET	-			_	GNMENT -L-	0 HR			ing no.					TATION			OF
	AR ELE					OTAL DE				NORTHING					STING 2,293,941	24 HR			LAR ELE						<b>PTH</b> 48.0		NC
DRILL	rig/Han	IMER EF	F./DAT	E RFO		ME-55 92%					DRILL	METHO	DDH	I.S. Auger	rs <b>HAMM</b>	IER TYPE	Automatic	DRILL	_ RIG/HAN	MER EF	-F./DATI	E RFC			% 10/12/2020		
	<b>.ER</b> Pi	,				TART DA				COMP. DA					RFACE WATER DEPTH N/	/A		DRIL	<b>LER</b> Pi	nter, D					TE 06/21/		CC
ELEV (ft)	DRIVE ELEV	DEPTH (ft)			-			VS PER I		·	SAMP.	17			SOIL AND ROCK DES	CRIPTIC	N	ELEV (ft)	ELEV		' <u> </u>		1			S PER FOO	
(11)	(ft)	(11)	0.5ft	0.5ft	0.5ft	0	25	50	1	75 100 I	NO.	/мс	) G	ELEV.	(ft)		DEPTH (ft)	(11)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50	75
170		+												-				170		_							
	-	F												F					-	-							
165	-	<u> </u>				• • •		• •   •				-		- 166.2 - 165.6	GROUND SURF.		0.0 	165	-	-				1	· · · · ·	· · · · ·	·
	163.2	3.0												-	PAVEMENT (ASPI ORANGE-BROWN, SAND				163.4	- 2.9							•
	-		2	2	3	•5.		.	· · ·	· · · · ·		D		-	TRACE MICA AND C				-	-	2	3	3	<b>•</b> 6			•
160	-	F				-+		.						<b>F</b>				160	-	-				-1			<u> </u>
-	158.2	8.0	5	7	4		 	.	· · ·	· · · · ·		м		Ţ					158.4	7.9	2	3	4				
155	-	t -								· · · · ·				ļ.				155	-	-							
	153.2	- 13.0												-					153.4	- 12.9							
		-	3	3	3	6.		.	· · ·	· · · · ·		M		152.0	ALLUVIAL		14.2		-	-	2	3	2	<b>●</b> <sup>5</sup> ···	· · · · ·		-
150	-	F						.						-	GRAY AND TAN MOTTLE			150		-					· · · · ·		·
-	148.2	18.0	2	4	4			.	· · ·	••••		м		F		GIVAVL	L		148.4	- 17.9 -	2	2	3		· · · · ·		
145	-	ŧ								· · · · ·				F				145	-	-					· · · · ·		-
140	143.2	23.0				<del>- 1 -</del>		.						F				140	143.4	- 22.9				<del>- 1 -</del>			•
Γ		- 20.0	3	5	8	<b>.</b>			· · ·	· · · · ·		Sat. M		142.0	RESIDUAL		24.2		-	-	3	4	7	: •11	· · · · ·		:
140	-	+				···\	· · · ·	.						-	GRAY AND TAN, SAPRO	LITIC, SA	ANDY	140	-	-					<u> </u>		·
-	138.2	28.0	7	12	13		N II		· · ·	· · · · ·		м		-	SILT				138.4	27.9	7	10	18				
135	-	ł	·				. •25 .			· · · · ·				F				135	-	-					• • • • • • • • • • • • • • • • • • • •	:	:
135	133.2	33.0					· · · ·							-				135	133.4	- 32.9					$\frac{1}{1}$		-+-
F		- 33.0	8	16	35		-   -	. <b>. .</b> 51	· · ·	· · · · ·		м		Ļ					-	-	9	18	27			↓45 · · · ·	
130	-	÷					·   · ·		<u>```</u>					L				130		-						· · · · · ·	·
-	128.2	38.0	30	37	53		-   -	· ·   ·	· · · ·		00.5	-		F					128.4	37.9	23	36	55		· · · · ·		1
125	-	ł		57				.	· · ·	•90	SS-5	M		F				405	-	-						· · · · ·	
125	123.2	-							. /.	••••••••••••••••••••••••••••••••••••••				-				125	123.4	- 120							
F		43.0	11	17	35				<' 	· · · · ·		м		F						-	7	5	8	: : <b>•</b> 13		:	:
120	-	ŧ.					·   · ·	· ·   ·						L				120	-	-					· · · ·		÷
F	118.4	47.8	60/0.0					· ·   ·_		60/0.0				<u>- 118.4</u>	Boring Terminated WITH		47.8 ARD		118.4	<u>47.9</u>	60/0.1			· · · ' <u>-</u>	·	<u> </u>	·
	-	ł												F	PENETRATION TEST F Elevation 118.4 ft ON CR	REFUSAI	Lat		-	-	00/011	1					
	-	+												-	ROCK (META-ARG				-	-							
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# PROJ. NO. - 67090.1.1 ID NO. - BR-0090 COUNTY - Nash

EB1-A

			S	OIL 1	TE:	ST	<b>RE</b>	SUL	<b>LTS</b>						
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-1	8' LT	15+49	2.9-4.4	A-6(6)	39	15	19.8	32.3	23.8	24.2	100	88	55	-	-
SS-2	8' LT	15+49	8.8-9.4	A-6(4)	31	14	18.3	36.5	19.0	26.2	100	91	52	-	-
SS-3	8' LT	15+49	17.9-19.4	A-4(0)	28	5	5.4	59.2	17.2	18.1	100	99	46	-	-
SS-4	8' LT	15+49	32.9-34.4	A-4(0)	•	NP	19.1	42.1	32.7	6.0	100	93	49	-	-

### <u>B1-B</u>

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W		% PASSING (SIEVES)			%	%	
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-10	9'RT	16+09	10.8-12.3	A-2-4(0)	-	NP	9.5	67.1	13.4	10.1	100	99	33	-	-
SS-11	9'RT	16+09	15.8-17.3	A-4(0)	-	NP	4.4	25.2	64.4	6.0	100	98	82	-	-

#### **B2-**A

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO					% PASSING (SIEVES)			%	%		
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-8	7' LT	17+09	7.7-9.2	A-2-4(0)	-	NP	5.2	73.5	13.2	8.1	100	98	35	-	-
SS-9	7' LT	17+09	17.7-19.2	A-4(0)	-	NP	6.8	35.6	49.4	8.1	100	98	68	-	-

#### EB2-A

SOIL TEST RESULTS															
SAMPLE			DEPTH	AASHTO	AASHTO % BY WEIGHT % PASSING (SIEVES			% BY WEIGHT				SIEVES)	%	%	
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-5	9' LT	17+59	38.0-39.5	A-4(2)	33	3	6.2	31.8	55.9	6.0	90	87	68	-	-

#### EB2-B

SOIL TEST RESULTS															
SAMPLE			DEPTH	AASHTO		%	BY WEIG	HT		% PAS	SING (S	IEVES)		%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-6	9' RT	17+59	13.4-14.4	A-4(4)	30	8	6.4	36.7	32.7	24.2	100	97	67	-	-
SS-7	9' RT	17+59	42.9-44.4	A-7-5(7)	47	14	17.1	33.8	34.9	14.1	100	91	57	-	-

## **CORE PHOTOGRAPHS**

**B1-B** 





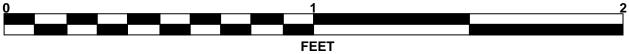
SHEET 14 67090.1.1 (BR-0090)/BRIDGE NO. 36

## BOXES 1 & 2: 51.5 - 68.5 FEET

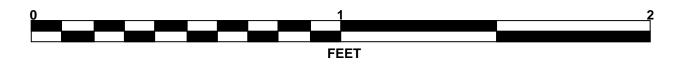
## **CORE PHOTOGRAPHS**

### **B2-A** BOXES 1 & 2: 32.8 - 54.7 FEET









SHEET 15 67090.1.1 (BR-0090)/BRIDGE NO. 36

## **B2-A** BOXES 3 & 4: 54.7 - 66.3 FEET

## SITE PHOTOGRAPH

Bridge No. 36 on –L– (NC 561) over Fishing Creek



SHEET 16 67090.1.1 (BR-0090) Nash Co.