STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	SF-590165	1	20

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY _MECKLENBURG

SITE DESCRIPTION BRIDGE NO. 165 ON SR 5469

(SHOPTON RD.) OVER COFFEY CREEK

CONTENTS

SHEET NO. **DESCRIPTION** TITLE SHEET 2.2A LEGEND (SOIL & ROCK) SUPPLEMENTAL LEGEND (GSI) 2B, 2C 3 SITE PLAN 4-6 CROSS SECTIONS 7-19 BORE LOG(S), CORE REPORT(S), & CORE PHOTOGRAPH(S) 20 SITE PHOTOGRAPH(S)

PERSONNEL

J.K. STICKNEY

C.L. SMITH

B.E. FOSTER

INVESTIGATED BY J.E. BEVERLY

DRAWN BY <u>J.E</u>. BEVERLY

CHECKED BY C.R. LAVENDER III

SUBMITTED BY K.B. MILLER

DATE _SEPTEMBER 2021

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY DIPPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 199) 707-850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

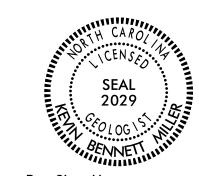
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NDICATED IN THE SUBSURFACE OR INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



DocuSigned by:

11/1/2021

DØ57AMB&AFND710&6RSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED**

PROJECT REFERENCE NO.	SHEET NO.
SF-590165	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 1 OF 2)

														(1 2	IUL	1 OI 2)
						SC)IL	DES	SCR:	<u>IPTI</u>	ON					GRADATION
BE PENE ACCORE IS CONSIST	TRATI DING BASEI ENCY,	D WI O THE O ON COLO	TH A (E STAI THE A R, TEX	CONTIN NDARD ASHTO TURE, I	NUOUS PENE SYS1 MOIST	FLIC TRAT TEM. I	MI-CO OHT P ION 1 BASIC AASH	ONSOL POWER TEST DESI TO CL	IDATE AUGE (AASH CRIPT ASSIF	D, OR R AN ITO T IONS	WEAT O YIEL 206, A GENER	.D LES ASTM (ALLY) D OTH	EARTH MA S THAN 100 01586). SOIL NCLUDE TH ER PERTINE	Ø BLOWS PI _ CLASSIFI IE FOLLOWI ENT FACTOR	ER FOOT CATION ING: RS SUCH	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS
,	as mi VERY	STIFF	.GRAY.S	SILTY C	LAY, MC	NST W	TH II	NTERB	BEDDEL	FINE	SAND	LAYER.	Y, ETC. FOI S.HIGHLY PLA	ASTIC.A-7-6	•	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.
CENEDA		,					<u> D</u>	AA			MATER		CATION	l		MINERALOGICAL COMPOSITION
GENERAL CLASS.				JLAR M							SSING =		OR	GANIC MATER	IALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.
GROUP CLASS.		A-1-b	A-3	_	4 A-2	A-2 -5 A-2	2-6 4-	_	A-4	A-5	A-6	A-7 A-7-5 A-7-6	A-1, A-2 A-3	A-4, A-5 A-6, A-7		ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY
SYMBOL		00000					.	S		N 7		A-7-6				SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50
% PASSING	0000	00000	4:::::	3310/2533	00:00		.75						· · · · · · · · · · · · · · · · · · ·	SILT-		HIGHLY COMPRESSIBLE LL > 50
*10 *40 *200		50 M	K 51 MN K 10 M)	1 35 M	x 35 I	4X 35	MX 35	5 MX 3	36 MN	36 MN	36 MN	36 MN	Granular Soils	CLAY SOILS	MUCK, PEAT	PERCENTAGE OF MATERIAL GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL
MATERIAL PASSING *40 LL PI	6	_ мх	– NP					1 MN 4			40 MX 11 MN	41 MN 11 MN	LITT	S WITH LE OR ERATE	HIGHLY ORGANIC	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE
GROUP INDEX	_	0	0		0		4 MX	: 1	8 MX	12 MX	16 MX	NO MX		nts of Anic	SOILS	GROUND WATER
USUAL TYPES OF MAJOR MATERIALS	GRAV	FRAGS EL, AND AND				OR CL AND			SIL SOI			YEY ILS		TTER		▼ water level in Bore hole immediately after drilling ▼ static water level after 24 hours ▼
GEN. RATING AS SUBGRADE				LENT 1					FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE	<u>▽P₩</u> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA ○
			PI OF									ROUP IS	> LL - 30			MISCELLANEOUS SYMBOLS
PRIMARY	SOIL	TYPE		COMP	ACTN		OR .	Ī	RANG	GE OF	STAN			GE OF UNC PRESSIVE S (TONS/F)	STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES
						E				4 T	4				. ,	SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SPT OMT TEST BORING SLOPE INDICATOR INSTALLATION
MATERI	GENERALLY COOSE COOSE									0 30 0 50 50			N/A		ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	
SILT-C MATERI	MATERIAL MEDIUM DENSE (NON-COHESIVE) DENSE							2 T 4 T 8 T	0 8 0 15 0 30			< 0.25 0.25 TO 0.5 TO 1 TO 2 2 TO 4 > 4	0.5 1.0 2	INFERRED SOIL BOUNDARY CORE BORING SOUNDING ROD INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE TYPE ALLUVIAL SOIL BOUNDARY PIEZOMETER INSTALLATION SPT N-VALUE		
							JRE	OR	R GF		SI.	ZE	1			RECOMMENDATION SYMBOLS
U.S. STD. SI OPENING (M		SIZE				4 .76	10 2.0	00	40 0.42		60 0.25	200 0.07	5 0.053			UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE SHALLOW SHALLOW SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF
BOULDE (BLDR.			OBBLE			AVEL GR.)			COARS SAND CSE. S)		FINE SANI (F SI)	SILT (SL.)	CLAY (CL.)	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNDERCUT UNDER
GRAIN MI SIZE IN		3Ø5 12		75 3			2.0	Ø			0. 25		0.05	0.005	5	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7/- UNIT WEIGHT
			SOIL		DIS					LAT	ION	OF	TERMS			CPT - CONE PENETRATION TEST NP - NON PLASTIC 🦎 - DRY UNIT WEIGHT
			SCAL IMITS			(DESC	MOIST RIPTI	ON				FIELD MOI			DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u> DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK
LL PLASTIC	+	LIQUI	D LIM	ΙT	_	- !	SATU (SA	RATEI	D -		FROM	BELO!	OUID; VERY	OUND WATE	R TABLE	e - VOID RATIO
RANGE < (PI) PL	.	PLAST	IC LI	MIT	_	- '	WET	- (W)					REQUIRES IMUM MOIS		J	FRAGS FRAGMENTS w - MOISTURE CONTENT CBR - CALIFORNIA BEARING HI HIGHLY V - VERY RATIO
				DISTUF LIMIT		- 1	MOIS	T - (1	M)		SOLIC	; AT 0	R NEAR OF	PTIMUM MC	DISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:
						- 1	DRY	- (D)					DDITIONAL IMUM MOIS		0	CME-45C CLAY BITS X AUTOMATIC MANUAL 6° CONTINUOUS FLIGHT AUGER CORE SIZE:
							PL	_AS	TICI	ΤY						X 8'HOLLOW AUGERS
h:::	N Pi 1	CTIC					PLAS	TICIT		DEX (PI)		DI	RY STRENC		CME-550 HARD FACED FINGER BITS X-NO
SL1 MO	DERA'	Y PLA TELY	PLAST	IC				10	0-5 6-15 6-25	DE.				VERY LOW SLIGHT MEDIUM	v	VANE SHEAR TEST
HIL	y⊓L T	PLAS1	110						LOR					HIGH		PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER
								R CO	MBIN	NOITA			YELLOW-B ESCRIBE A			X CME-550X TRICONE TRICONE SOUNDING ROD X CORE BIT VANE SHEAR TEST YANG-CARB. SOUNDING ROD VANE SHEAR TEST

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 2 OF 2)

ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN I.FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.

ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: 115115 NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES 3 100 BLOWS PER FOOT IF TESTED. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT FINE TO COARSE GRAIN IONEQUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK WEATHERING **ERESH** ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS II OF A CRYSTALLINE NATURE. (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO SLIGHT 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN MODERATE GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH MODERATELY SEVERE (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT SEVERE REDUCED IN STRENOTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. (SEV.) IF TESTED. WOULD YIELD SPT N VALUES > 100 BPF ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VERY SEVERE (V SEV.) VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED HARD TO DETACH HAND SPECIMEN. CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED MODERATELY

, ILDIOI	CHI DE GROOTED ON GOODED BIDS INCHES BEEF DI FIRM THESSONE OF KINDE ON FICK FORM.
HARD	CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE
	POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN
	PIECES CAN BE BROKEN BY FINGER PRESSURE.
VEDV	CAN BE CARVED WITH KNIFE CAN BE EXCAVATED READILY WITH DOINT OF DICK DIFFEE 1 INCH

CAN BE GROOVED OR COLIGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.

BY MODERATE BLOWS.

MEDILIM

VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTUR	RE SPACING	BEDD:	ING
<u>TERM</u>	SPACING	<u>TERM</u>	THICKNESS
VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	4 FEET
WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
MODERATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
CLOSE	0.16 TO 1 FOOT	VERY THINLY BEDDED	0.03 - 0.16 FEET
VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET
		THINLY LAMINATED	< 0.008 FEET
·	71.00.10	ATION	<u> </u>

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE

MODERATELY INDURATED

GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;
BREAKS EASILY WHEN HIT WITH HAMMER.

INDURATED

GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;
DIFFICULT TO BREAK WITH HAMMER.

EXTREMELY INDURATED

SHAPP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;
SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

<u>ALLUVIUM (ALLUV.)</u> - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. <u>AQUIFER</u> - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

<u>ARTESIAN</u> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUTUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM

 $\underline{\text{CORE}}$ RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

 $\overline{ ext{DIKE}}$ - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

 $\overline{ ext{DIP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

 $\underline{\sf DIP}$ DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

 $\underline{\text{FAULT}}$ - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

 $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

 $\underline{\text{LEDGE}}$ - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

<u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

 $\underline{\textit{RESIDUAL (RES.)} \, \textit{SOIL}} \, \, \textit{-} \, \, \textit{SOIL} \, \, \textit{FORMED IN PLACE BY THE WEATHERING OF ROCK.}$

ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

 $\underline{\text{SLICKENSIDE}}$ - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

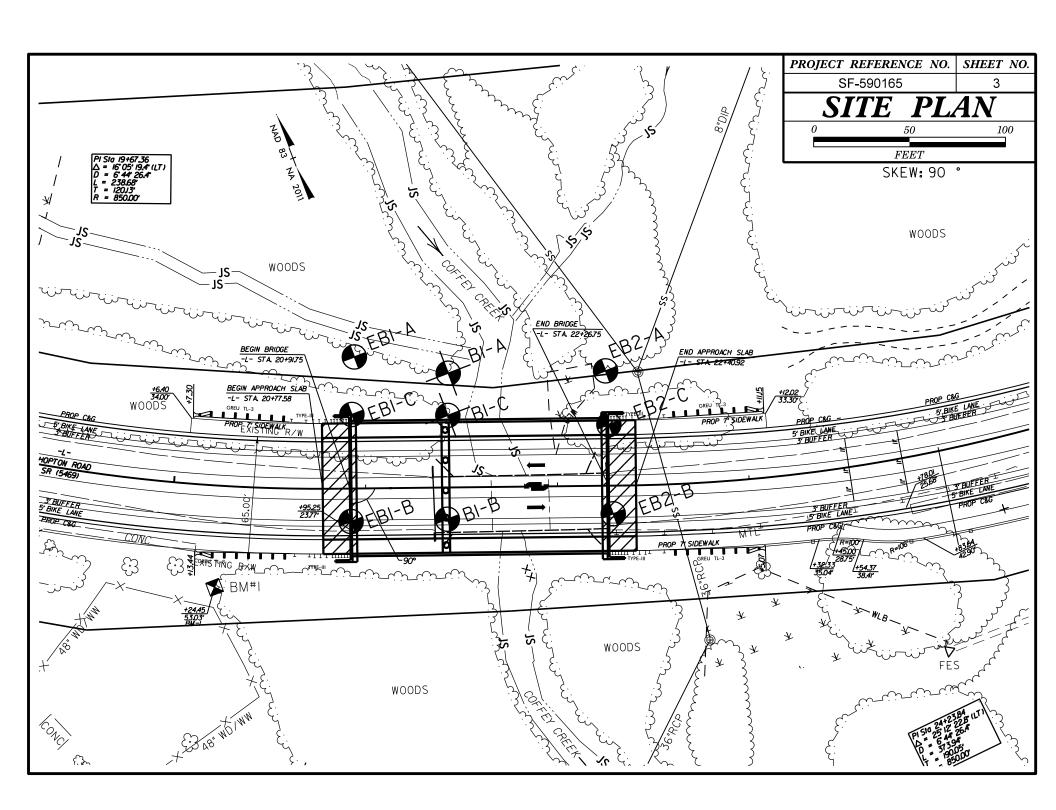
BENCH MARK: BM #1: STA 20+17.29, 81.78' RT, BENCH TIE SPIKE IN 15" PINE N 519944.80, E 1422914.32

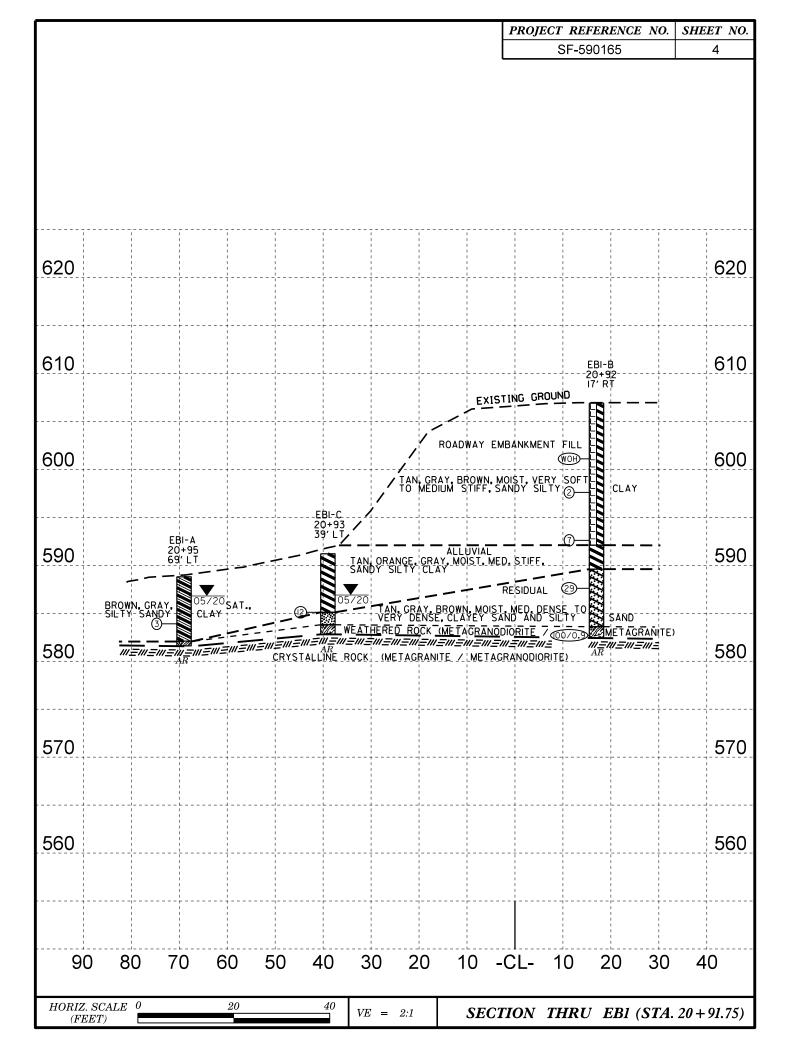
ELEVATION: 613.0 FEET

NOTES:

FIAD= FILLED IMMEDIATELY AFTER DRILLING

DATE: 8-15-14

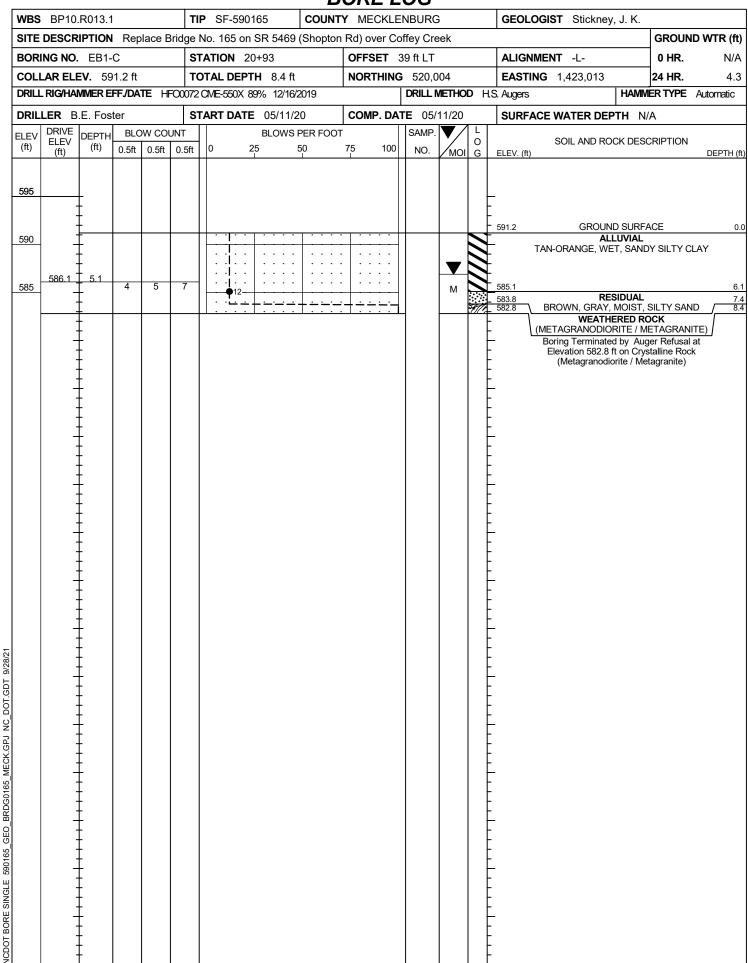


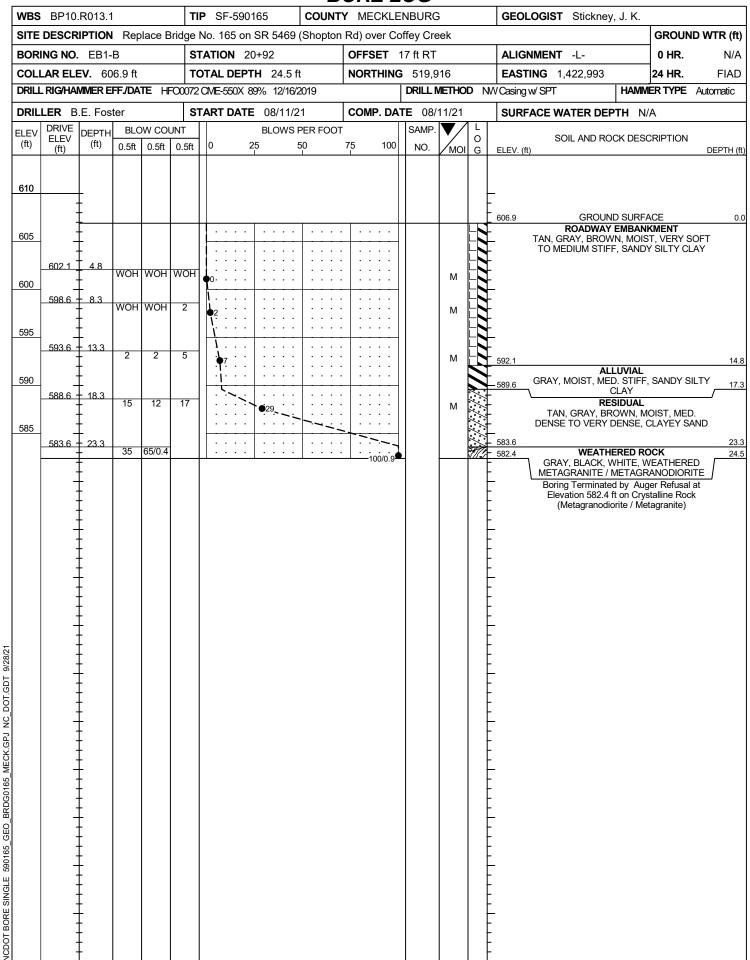


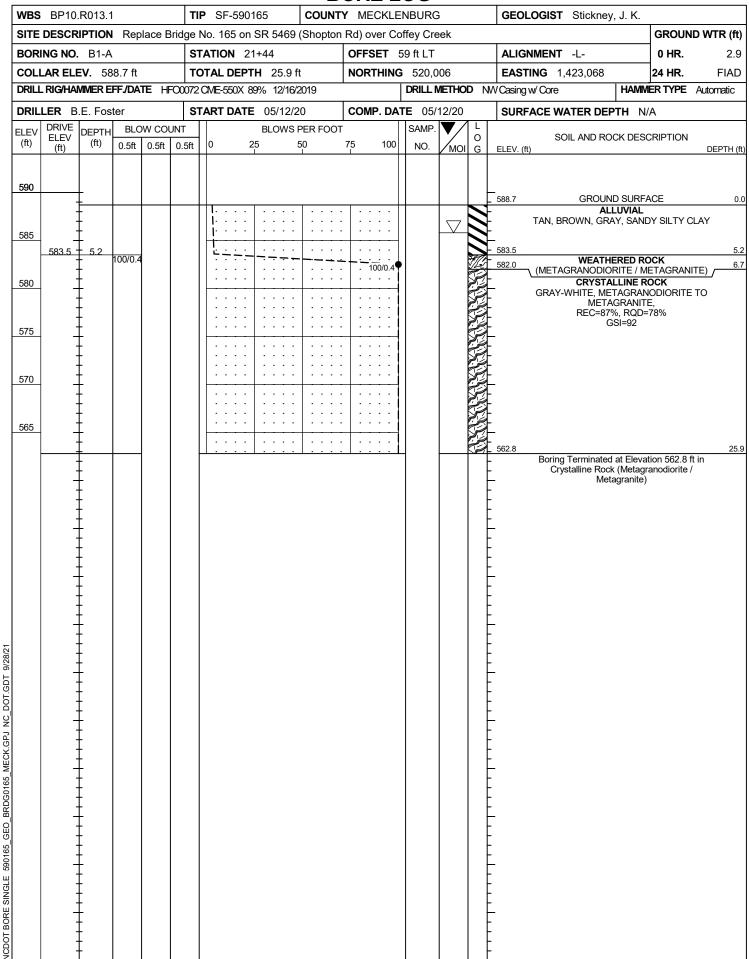
									PROJEC	T REF	ERENCI	E NO.	SHEET	NO.
								L		SF-59	0165		5	
	 		 	 		 			!					
620									¦ 				6	20
1 1 1	 		 	1 1 1	 	 	1		1				 	
		· - - -							<u>+</u>					
610	 		 		 	 	 		1 1 1 1	 			6	10
010									1					10
	 			i ! !	1	!				E	ВІ-В			
-	; 	-;; 	 ! !		-				†	2 I	BI-B I+42 7' RT	· 		
600				 	+	 		<u>^</u> -	<u> </u>	- 	3 -		6	00
1 1 1	 		 	 	!	 	!	 		3			1	
		<u> </u>			EXIST	NG GROUN	ק	Ι ROΔDWΔ	Y EMBAI	J. J				
E00	 	Bi-A 21+44 59' L	,	BI-C 2I+43 38' L T						2—			F	.00
590		59' L'	r 		720		RED	, ORANG	E, MOIST	SOFT,	SILTY	CLAY	<u></u> 5	90
			— + — ·		LLŲVIA	L -	-+	·		4				
	i		' AN. BR	OWN, GF	RAY, MO	DIST TO	VET, VE	RY SOF	T, SAND	SILTY	ÇLAY			
580		<u>(00/0.⊅</u> = =	UZIUZIUZIUZIUZ		- @	` <u> </u>	-		 	_WOB			5	80
	 						ہے ۔ EATHER	ED ROC	<u> </u>				 	
						LLINE ROO HITE, MET AGRANODI 4, ROD=92	MÉ	TAGRANI - ME-T-A	TE / C	100/0.3 DRITE				
F70	REC=87%. GSI=92	RUD= 78%	 	G	RAÝ-W	HITE, MET	AGRANI	TE		100 (0.3)			_	70
570				T R	O MET EC≓97;	AGRANODI ∴ ROD⊧92	ORITE,				6	= <u> </u>	<u>5</u>	70
	 		 		SI=95 :	 	!			(0/0.)	973.3I I	1	RILLED	
				BT					i		CAPAI HARD	BLE OF C CRYSTAL	ORILLÉD SING BIT CUTTING LLINE; RO	CK
560	 	BT	 	BT	 	 	 		1 1 1 1	6 0/0.		JONE		60
						 	!							
	A RESIDUAL	- TAN, YELL	OW, GRAY, ME	D. DENSE	TO VE	RY DENSI	E, CLAY	EY SAN	n 	(0/0.) I	BT			
	 		 	 	 	 	1			1			 	
90	80 7	70 60	20	40	30	20	1() -(::	10	20	30	40	
	3 0 ,	2 00	_0	. •	- -	_0	. `		-	. •	_•	- -	. •	
HORIZ. S	CALE 0	20		40	VE =	2:1	SE	CTIO	N TH	RU	BENT	1 (STA	1 21 + 4	11.7:

610 EXISTING GROUND ROADWAY EMBANKMENT STIFT: SILTY CLAY BED ORANGE MOIST, MEDIUM STIFT: SILTY CLAY ALLUVIAL OST/20 FED DAL FED DAL TAN CRAY BROWN, CRAY MOIST TO WET SILTY AND CLAY METAGRANITE MOIST METAGRANITE METAGRANITE METAGRANITE METAGRANITE METAGRANITE MOIST METAGRANITE METAGR												PRO	ECT	REFE	RENC	E NO.	SHI	EET NO
Second S													S	F-590	165			6
Second S																		
Mode								TE	EST I	RESUL	LTS							
610 ENSTING GROUND ROADWAY EMBANNON FED CRANGE MOIST, MEDIUM STIFT, SLIVE, CLAY ALLUVIAL OS 22. SEAD BROWN, CRAY, MOIST TO WET, SOFT. SEAD BROWN, CRAY, BROWN, CRAY, MOIST TO WET, SOFT. METAGRANDOURIE METAGRANIE METAGRANIE METAGRANIE METAGRANIC		NO.			INTERVAL	CLA	SS.		C. SAN	D F. SAND	SILT		10	40	200	4		
600 EXISTING GROUND ROADWAY EMBANMENT STEP - SELLY CLAY STEP - SELLY CLAY BED - A CO - LT SED - A SED - A CO - LT SED - A SED -	L 	<u>SS-I</u>	5.0′ LT		3.8-5.3′	A-6	·(9) <u>3</u>	<u> </u>	4 9.6	26.1	30.1	34.1	100	96	<u> 71</u> 			
EXISTING GROUND ROADWAY EMBANKMENT GEORGE MOIST, MEDIUM STIFF, SANGE MOIST, MEDIUM STIFF, MEDIUM STIF	620				 											 		620
EXISTING GROUND RED ORANGE MOIST, MEDIUM STIFF, SILYY CLIAY, MEDIUM STIFF, SILYY CLIAY, MEDIUM STIFF, SILYY CLIAY, MEDIUM STIFF, SILYY CLIAY, MEDIUM ALLUVIAL OST/20 RED, BROWN, GRAY, MOIST, TO WET, SILTY AND CLAYGOTO RESPURANT SILTY CLAY, AND CLAYGOTO RESPURANT SILTY SILTY AND CLAYGOTO RESPURANT SILTY AND CLAYGOTO RESPURANT SILTY AND CLAYGOTO RESPURAN	610							+ -		1 1 1				EB2- 22+2	B 19			610
SSC. STIFF, SILTY CLAY, MEDILM STIFF, SILTY CLAY, MEDILM STIFF, SILTY CLAY, MEDILM STIFF, SILTY CLAY, MEDILM ALLUVIAL OST/20 SANDY SILTY CLAY MODISTAND CLAYED SANDY SILTY CLAY MODISTAND CLAYED METAGRANGURITE METAGRANIE METAGRANIE METAGRANIE METAGRANIE METAGRANIE METAGRANIE METAGRANIE METAGRANIE METAGRANODIORITE STORY METAGRANODIORITE METAGRANODIORITE STORY METAGRANODIORITE METAGRANODIORITE STORY METAGRANODIORITE METAG	600						EX	(IST IN	iG GROUI	VD / RO	DADWAY	_	\neg			 		600
ALLUVIAL 3				E	B2-A		- 		, , ,	/		+	MEDIL ⑤			+-		
TANLGRAY, BROWN. RESIDUAL TANLGRAY, BROWN. MOIST TO WET, SILTY AND CLAYEO MEMPHERED METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE METAGRANODIORITE Solution Solution METAGRANODIORITE METAGRA	590				V 05/20	 	7		05/20									590
ME I ALRANULIONIE IN SUI	580			ESIDUAL -	TAN	GRAY. B			TOIST T	WFT.S	L LTY AN	ID CLA	(2) (EV) (1)=		AND W=W=			580
	570	(M		DDIORITE /	METAL METAL METAL METAL	GRANITE)	00/0.3 /=	AR	METAG	EMEME RYSTALL RANITE /	METAC	K GRANODI	ORITE			 		570
90 80 70 60 50 40 30 20 10 -CL- 10 20 30 40	560							1 1		 					 			560
	Ç	90	80	70	60 5	0 4	Ю	30) 2	0 1	0 -	CL-	1(0	20	30) 4	ŀO
HORIZ. SCALE 0 20 40 $VE = 2:1$ SECTION THRU EB2 (STA. 22 + 26.	HORI	Z. SCAL	E 0		20		40	17E	0.1	<u> </u>	O E O T	ION,	TIT	D 7 7	EDO	/ CT 4	22 :	26 751

										B	<u>JR</u>		<u>OG</u>					
WBS BP	10.R013.	1		TI	IP S	F-590′	165		COL	JNTY	ME	CKLE	NBURG	3		GEOLOGIST Stickney, J. K.		
SITE DES	CRIPTION	l Rep	lace B	ridge	No. 1	165 on	SR 54	169 (Shop	oton l	Rd) o	ver Co	ffey Cre	eek			GROUND WT	R (f
BORING N	IO. EB1	-A		S	TATIO	ON 20	0+95				OFFS	ET (69 ft LT			ALIGNMENT -L-	0 HR.	N/A
COLLAR E				- 1		. DEPT					NOR	THING	520,0	031		EASTING 1,423,025	24 HR.	2.
DRILL RIG/I	HAMMER E	FF./DA	TE HF	- O0072	2 CME-	-550X 8	12%	2/16/2	019				DRILLI	METHO	D H	I.S. Augers HAMM	ERTYPE Auton	natic
DRILLER	B.E. Fos	ster		S	TART	DATE	05/	11/20	0		COM	P. DA	TE 05/	11/20		SURFACE WATER DEPTH N	'A	
ELEV DRIN	V DLF 111	-	0.5ft		0	2	BLO\		PER F		75 	100	SAMP. NO.	MOI	L O I G	SOIL AND ROCK DESC ELEV. (ft)		PTH
590 585 584	9 3.9	1	2	1							 			Sat.		588.8 GROUND SURFA ALLUVIAL BROWN, GRAY, SATURA SANDY CLAY	ATED, SILTY	
	+++++++++++++++++++++++++++++++++++++++								_							582.1 S81.6 (METAGRANODIORITE / M Boring Terminated by Aug Elevation 581.6 ft on Crys (Metagranodiorite / Me	ETAGRANITE) Jer Refusal at stalline Rock	







GEOTECHNICAL BORING REPORT

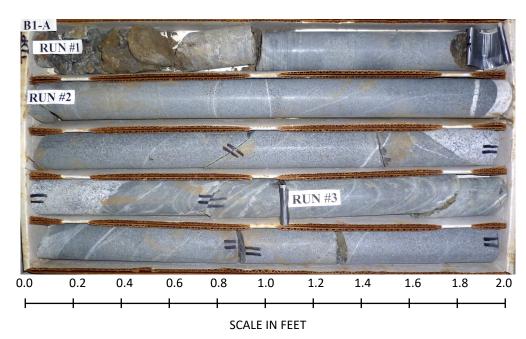
WBc	17PD	.10.R.1	38		TIP	SF-59	20165				RE L	NBURG	GEOLOGIS	ST Stickne	v K		
				Jaco Brid								offey Creek	GEOLOGIA	JI SUCKILE	y, J. IX.	GPOLIN	D WTR (ft
		. B1-A		naut DIIC	ř –		21+44	10) 60	ισρισί	_	FSET		ALIGNMEN	NT -l-		0 HR.	2.9
		EV. 58					PTH 25	Q ff		+		520,006	+	1,423,068		24 HR.	FIAD
				TF HECY			(89% 12/		 9	INC	KIHING	DRILL METHOD N			HAMM		Automatic
											MD DA	TE 05/12/20					7 MIOITEMO
	E SIZE	B.E. Fos	ter				TE 05/1			+	INIP. DA	IE 05/12/20	SURFACE	WATER DE	PIH N	/A	
	RUN	1		DRILL	RI	JN	N 19.2 f	STF	RATA	1							
(ft)	ELEV (ft)	DEPTH (ft)	RUN (ft)	RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STF REC. (ft) %	RQD (ft) %	L O G	ELEV. (1		DESCRIPTION		KS		DEPTH (
582	582.0	6.7	4.2		(1.9)	(0.8)		(16.7)	(14.9)		_ 582.0		Begin Cori	ing @ 6.7 ft LLINE ROCK			6
580	_	Ŧ			45%	19%		`87%	78%		_	GRAY-WHITE, VER' METAGRANODIC	Y SLIGHTLY W	EATHERED T			
-	577.8	10.9	5.0		(5.0)	(4.9)					Ē	WEI/ (CIV # 10 B)	WIDE FRAC	TURE SPACIN SI=92	iG	. 02002 .	O
575		Ŧ	3.0		100%	98%					-		G	31-92			
	572.8	15.9									_						
	312.0	15.9	5.0		(5.0)	(4.5)					-						
570	-	‡			100%	90%					_						
	567.8	20.9									-						
565		‡	5.0		(4.8) 96%	(4.7) 94%					_						
303		‡															
ŀ	562.8	25.9									_ 562.8	Boring Term	ninated at Eleva	tion 562.8 ft in	Crystallin	e Rock	25
	_	‡									_		(Metagranodio	rite / Metagrar	nite)		
	-	Ŧ									_						
		‡															
		+									_						
		Ŧ															
		İ									_						
		Ŧ															
	-	‡									_						
	•	t									_						
		Ŧ															
	-	‡									_						
	•	ł									_						
		Ŧ															
	-	‡									_						
		╁									_						
		‡															
	-	‡															
	,	+									_						
	,	‡									_						
	_	†									_						
		+									_						
		‡									_						
	-	+									_						
		Ŧ									Γ						
		‡									_						
	-	+									-						
		‡															
		t									_						
	-	+									H						
		‡									<u> </u>						
	•	t									L						
	-	Ŧ									<u></u>						
		‡									ţ						
		+									-						
	-	‡									<u> </u>						
		†									-						
		Ť															

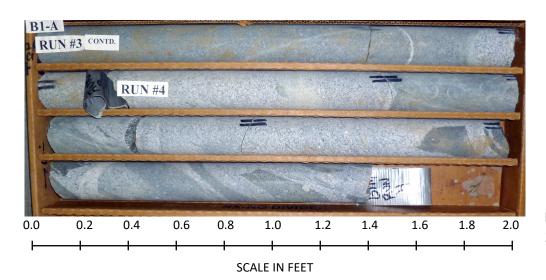
CORE PHOTOGRAPHS:

Bridge No. 165 on SR 5469 (Shopton Rd.) over

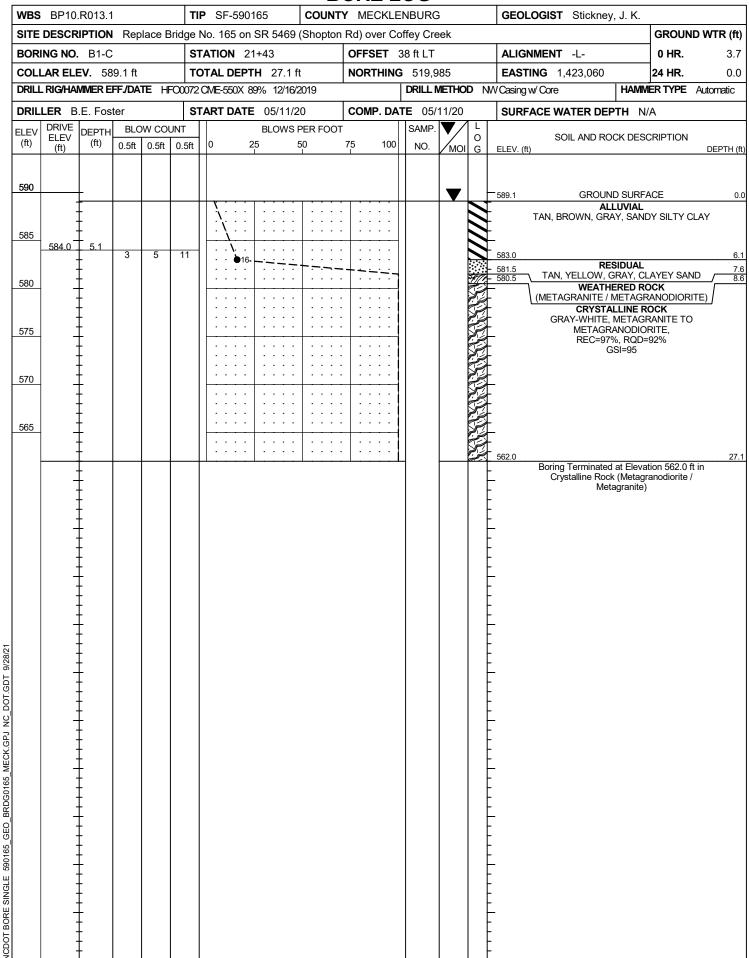
Coffey Creek B1-A: -L- Station 21+44, 59' LT

Begin Core 6.7 feet





End Core 25.9 feet



WBS	17BP.	10.R.1	38		TIP	SF-59	90165	С			RE L					GEOLO	GIST	Sti	cknev	v, J. K			
				lace Brid			on SR 54											2.1		, ,	-	ND WT	۲ (ft)
	NG NO.						21+43	- (5)	,		FSET38				7	ALIGNN	IENT	-L-			0 HR.		3.7
	AR ELE				 		PTH 27	.1 ft		_	RTHING				-	ASTIN					24 HR.		0.0
				TE HFOO			(89% 12/)	1				HOD N				,		HAM	MER TYPE		
DRIL	LER B.	E. Fos	ter		STAF	RT DA	TE 05/1	1/20		СО	MP. DA	TE (05/11/	20	-	SURFAC	CE W	ATE	R DEI	PTH N	N/A		
	SIZE						N 18.5 f														***		
ELEV	DUN	DEPTH	RUN	DRILL	RI	JN L BOD	SAMP.		ATA RQD	Ļ													
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	REC. (ft)	RQD (ft) %	NO.	(ft) %	(ft) %	O G	ELEV. ((ft)			DES	SCRIPTIO	ON AN	ND RE	MARK	KS		DEF	PTH (f
5805															ı	Begin C	oring	g @ 8	3.6 ft				
300	580.5	- 8.6 -	3.2		(2.9) 91%	(2.9) 91%		(18.0) 97%	(17.1) 92%		580.5 -	GR	AY-WH	ITE, VEF	RY S	CRYS LIGHTLY				O FRES	SH, VERY I	HARD,	8.0
	577.3	11.8	5.0		(4.8)	(4.4)					-	N	/IETAG	RANODI	IORIT	E TO M FRAC	ETAG TURE	RANI SPA	TE, W CING	ITH CLC	OSE TO W	IDE	
575	4	-			96%	(4.4) 88%					<u>-</u>						GSI=						
	572.3	- 16.8									- -												
570	‡	- -	5.0		(5.0) 100%	(4.6) 92%					- -												
0.0	7	-									-												
	567.3	21.8	5.3		(5.3)	(5.2)					- -												
565	- ‡	- -			100%	98%					-												
	562.0	- 27.1									- - 562.0												27.
		-									-		Во	ring Terr	mina (M	ted at Ele etagrand	evatio	n 562.	.0 ft in	Crystalli	ine Rock		
	7	-									-				(171	ctagranc	Jaiorite	C / IVIC	itagran	iite)			
	‡	-									-												
		-									-												
	1	_									- -												
	}	_								1	-												
	-	-									_												
	1	-									-												
	1	_									- -												
	†	_									-												
	‡	_									- -												
		_									-												
	1	_									<u>-</u>												
	- - - - - -	_									-												
	- 1	_									-												
	1	-									-												
	‡	-									- -												
	†	-									-												
	1	_									- -												
		_									-												
	7	_									-												
	- 1	-									-												
	- 1	_									- -												
	- - - -	_									- -												
											-												
	-	_									-												
	+	-									-												
	1	-									-												
	7	-									-												
	‡	-									- -												
	£	-									- -												
	7	_									-												
	7	-									-												
		-	l					l	l		_												

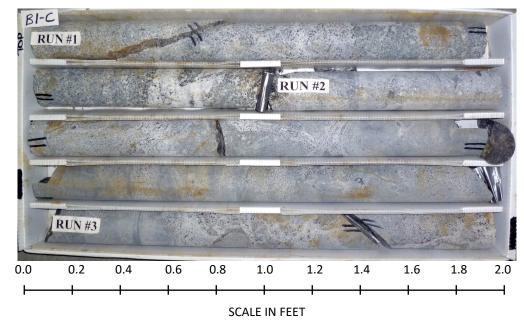
CORE PHOTOGRAPHS:

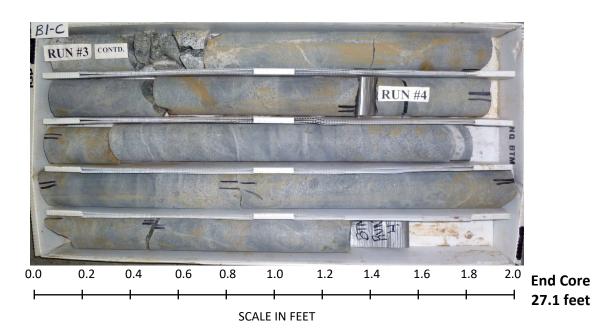
Bridge No. 165 on SR 5469 (Shopton Rd.) over

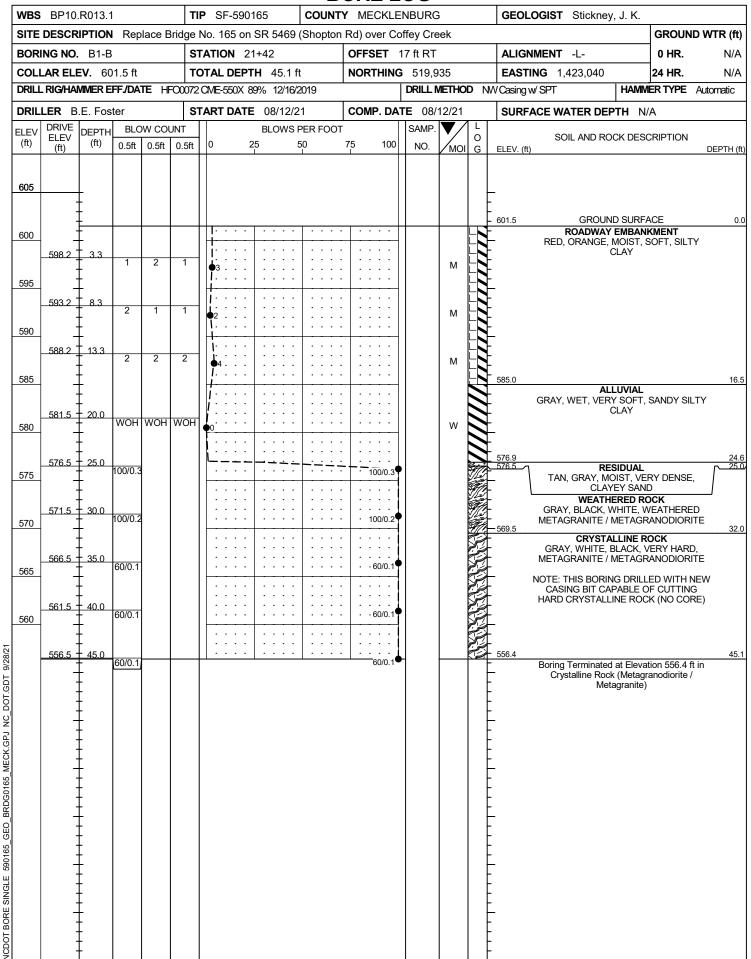
Coffey Creek

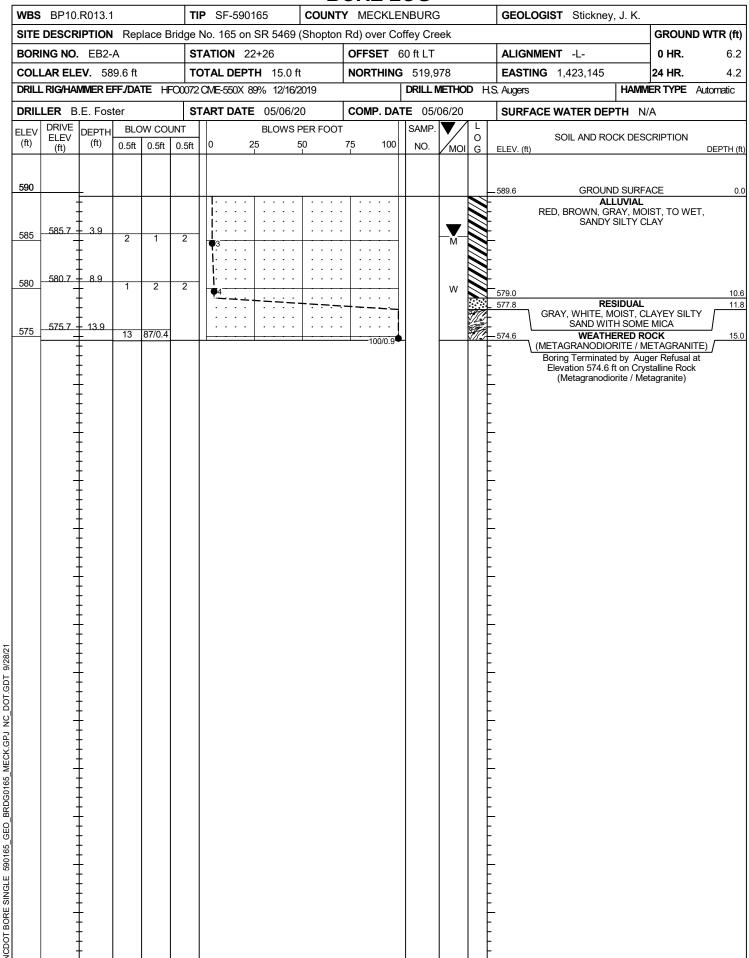
B1-C: -L- Station 21+43, 38 ft LT

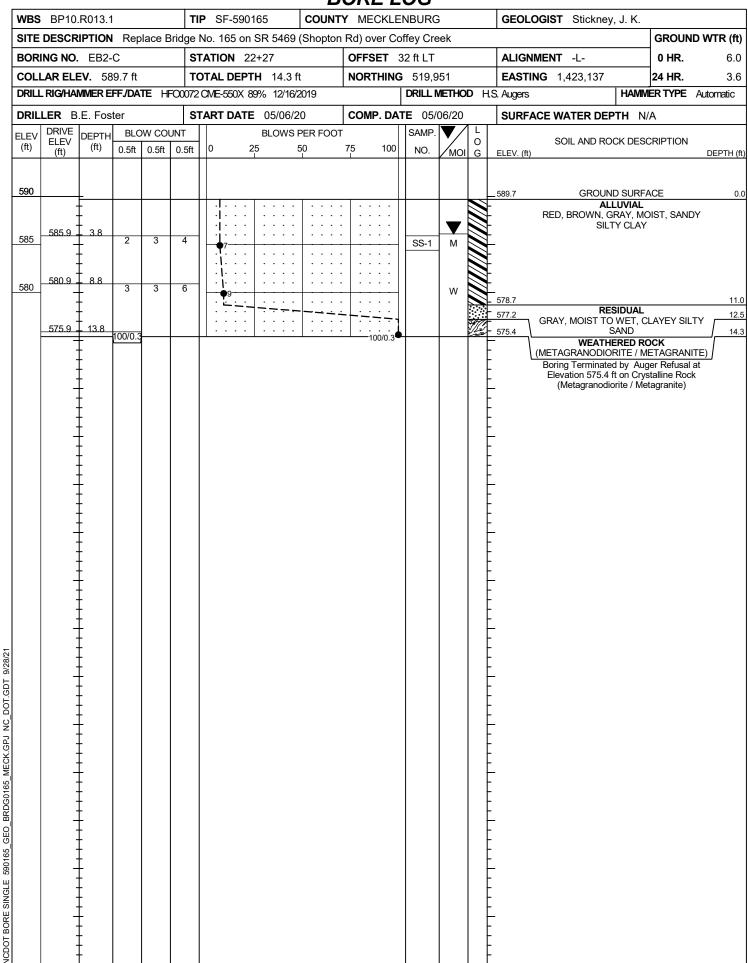
Begin Core 8.6 feet

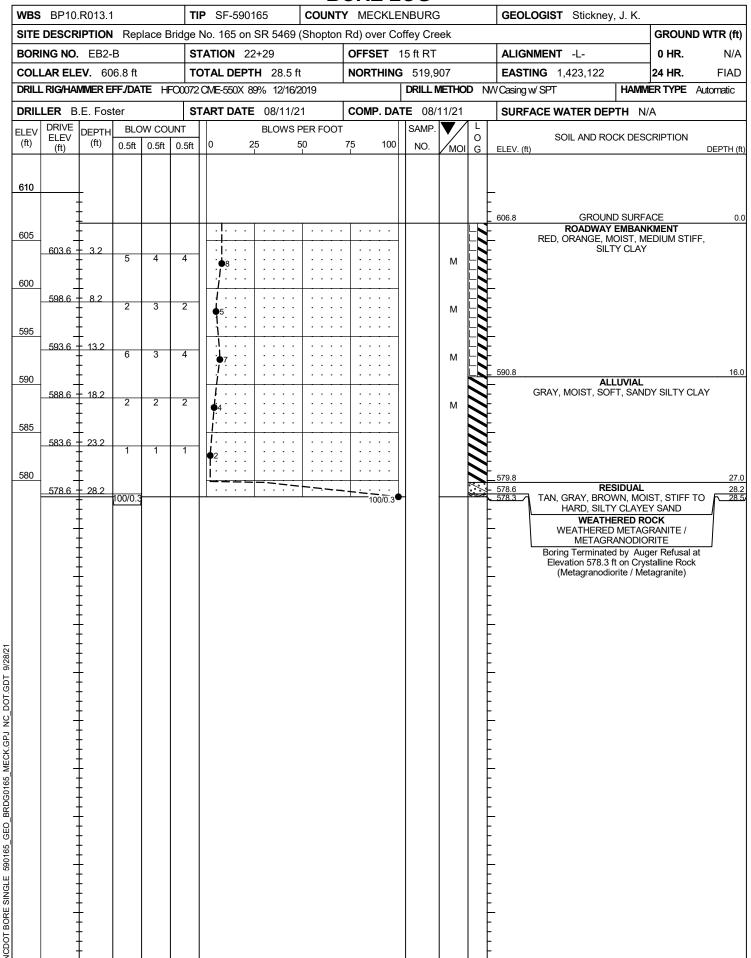












Bridge No. 165 on SR 5469 (Shopton Rd.) over Coffey Creek SITE PHOTOGRAPHS



Photograph No. 1: View looking EB1 to EB2



Photograph No. 2: View looking downstream