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\\rsandh.com\files\Transportation\P\1031782004_U-5798 (Gillis HillRoad)_P&D\Design\Structures\CAD\Culvert\410_001_U-5798B_SMU_CU_001_250001.dgn AcostaM

ASSUMED LIVE LOAD ----- HL-93 OR ALTERNATE LOADING DESIGN FILL = 7.17' MAX. AND 2.67' MIN. CONCRETE CULVERTS TO BE POURED IN THE FOLLOWING ORDER FOR EACH STAGE:

1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.

HEADWALLS.

3″∅ WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH SPECIFICATIONS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FEET.LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF THE EXTERIOR WALL ABOVE THE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS.EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN.FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

ENTIRE LENGTH OF THE EXPANSION JOINT. SEE SECTION 414 OF THE STANDARD SPECIFICATIONS FOR CULVERT EXCAVATION AND BACKFILLING. EXCAVATE AT LEAST 1 FOOT BELOW BOTTOM OF CULVERT AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH ARTICLE 414-4 OF THE STANDARD SPECIFICATIONS. FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT OF STRUCTURES, SEE SPECIAL PROVISIONS. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

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cui vert					STAGE II
					CONSTRUCTION S
OTAL STRUC	TURE QUAN	TITIES			(SECTION LOOKING DOV
REINFORCING ST	EEL		FOUNDATION CONDITI	ONING MATERIAL	STAGE I CONSTRUC
STAGE I			STAGE I	90 TONS	
CULVERT	13,526	LBS.	STAGE II	119 TONS	
WING ETC	503	LBS.	STAGE III	86 TONS	
TOTAL	14,029	LBS.	STAGE IV	114 TONS	
STAGE II	35,186		TOTAL	109 TONS	
WINC FIC	503	LDJ.			-
TOTAL	35,689	LBS.	STRUCTURE		
STAGE III			STA.25746.JU -LD-	LUIVIF SUIVI	_
CULVERT	12,950	LBS.	CULVERT EXCAVATION		
WING ETC	503	LBS.	STA.25+46.30 -LB-	LUMP SUM	
TOTAL	13,453	LBS.			
STAGE IV			HORIZON	ITAL CURVE D	ATA -LB-
CULVERT	33,680	LBS.		P.I. STA. = 27+42.62	2
WING ETC	503	LBS.		$\Delta = 47^{\circ} - 45' - 00'' (LT)$ $D = 05^{\circ} - 43' - 46.5''$)
TOTAL	34,183	LBS.		L = 833.39' T = 442.62'	
OTAL STEEL	97,354	LBS.		R= 1000.00' SE = 0.04	DOCUMENT NOT
			,		FINAL UNL SIGNATURES

NUIES

2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE

KXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX		XXXXXXXX FAGE III		STAGE	E IV	
SEQUENCE WNSTREAM) ICTION JCTION		CONST (secti CS STA	RUCTION on looking ge iii cons age iv cons	SEQ downs ⁻ structi tructi	UENCE (ream) Ion on	
		PROJEC <u>CU</u> STATIC	CT NO I <u>MBERL</u> DN:25- f 8	U- AND +46.	<u>-5798</u> co <u>30 -</u> L	B UNTY _B-
	6/19/2025 WR TH CARO/ OF ESS/ON SEAL 052672 WG INE CARO/ Signed by: Matthew J. Acousta RSSS	depa TRI CONC F0 SR 11 SR	state of RTMENT OI PLE 12 RETE R STEWA 02 AND 1934 AN 90 9	NORTH CAR F TRAN RALEIGH 2'-0 BOX BOX RTS SR 14 ND CE P SKE	olina NSPORTA CUL CREEK 120 BE LTIC	TION '-0" VERT ON TWEEN DR.
CONSIDERED LESS ALL COMPLETED	RS&H Architects-Engineers-Planners, Inc. 8521 Six Forks Road, Suite 400 Ralelgh, NC 27615 919-926-4100 FAX 919-846-9080 www.rsandh.com North Carolina License Nos. 50073 * F-0493 * C-28	NO. вү: 1 2	REVISIO DATE: NO. 3 4	NS BY:	DATE:	SHEET NO. C-1 Total Sheets 8

		(LO SUMM/	AD A Ary	AND F For	RESIS REIN	STAN For(CE F CED (ACTO Conc)R RATIN Rete box	IG (L X CU	rfr) Lver	TS			
							STRENGTH I LIMIT STATE									
										MOMENT		SHEAR				
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING (#)	MINIMUM Rating factors (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT Type	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT Type	DISTANCE FROM Left end of Element (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.05		1.75	1.82	3	TOP SLAB	4.73	1.05	3	TOP SLAB	11.39	
DESIGN		HL-93 (OPERATING)	NZA		1.36		1.35	2.37	3	TOP SLAB	4.73	1.36	3	TOP SLAB	11.39	
RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	1.21	43.560	1.75	2.04	3	TOP SLAB	4.73	1.21	3	TOP SLAB	11.39	
		HS-20 (OPERATING)	36.000		1.56	56.160	1.35	2.64	3	TOP SLAB	4.73	1.56	3	TOP SLAB	11.39	
		SNSH	13.500		3.36	45.360	1.40	3.74	3	EXTERIOR WALL	8.63	3.36	3	TOP SLAB	11.39	
		SNGARBS2	20.000		2.83	56.600	1.40	3.60	3	EXTERIOR WALL	8.63	2.83	3	TOP SLAB	11.39	
	ICLE	SNAGRIS2	22.000		2.98	65.560	1.40	3.72	3	EXTERIOR WALL	8.63	2.98	3	TOP SLAB	11.39	
	<pre> < </pre>	SNCOTTS3	27.250	$\langle 3 \rangle$	1.45	39.513	1.40	2.44	3	TOP SLAB	4.73	1.45	3	TOP SLAB	11.39	
	С С С	SNAGGRS4	34.925		1.74	60.770	1.40	2.77	3	EXTERIOR WALL	8.63	1.74	3	TOP SLAB	11.39	
	ING	SNS5A	35.550		1.64	58.302	1.40	2.72	3	EXTERIOR WALL	8.63	1.64	3	TOP SLAB	11.39	
		SNS6A	39.950		1.62	64.719	1.40	2.72	3	EXTERIOR WALL	8.63	1.62	3	TOP SLAB	11.39	
LEGAL		SNS7B	42.000		1.61	67.620	1.40	2.84	3	EXTERIOR WALL	8.63	1.61	3	TOP SLAB	11.39	
RATING	ER	TNAGRIT3	33.000		2.49	82.170	1.40	3.78	3	EXTERIOR WALL	8.63	2.49	3	TOP SLAB	11.39	
	RAIL	TNT4A	33.075		1.73	57.220	1.40	2.80	3	EXTERIOR WALL	8.63	1.73	3	TOP SLAB	11.39	
	I – T	TNT6A	41.600		1.59	66.144	1.40	2.89	3	EXTERIOR WALL	8.63	1.59	3	TOP SLAB	11.39	
	ST)	TNT7A	42.000		1.62	68.040	1.40	2.82	3	EXTERIOR WALL	8.63	1.62	3	TOP SLAB	11.39	
	TOR (TT)	TNT7B	42.000		1.71	71.820	1.40	2.78	3	EXTERIOR WALL	8.63	1.71	3	TOP SLAB	11.39	
	TRAC	TNAGRIT4	43.000		1.64	70.520	1.40	2.80	3	EXTERIOR WALL	8.63	1.64	3	TOP SLAB	11.39	
	ICK	TNAGT5A	45.000		1.62	72.900	1.40	3.06	3	TOP SLAB	12.33	1.62	3	TOP SLAB	11.39	
	TRL	TNAGT5B	45.000		1.55	69.750	1.40	2.60	3	TOP SLAB	12.33	1.55	3	TOP SLAB	11.39	
EMERGEN	CY	EV2	28.750		1.95	56.063	1.30	2.87	3	TOP SLAB	4.73	1.95	3	TOP SLAB	11.39	
VEHICLE	(EV)	EV3	43.000	$\langle 4 \rangle$	1.20	51.600	1.30	2.12	3	TOP SLAB	4.73	1.20	3	TOP SLAB	11.39	



DRAWN BY :	NSC	DATE : _	12/2023	
CHECKED BY :	MRA	DATE : _	12/2023	
DESIGN ENGINEER	OF RECORD:	MRA	DATE : _	03/2025

6/19/2025 \\rsandh.com\files\Transportation\P\1031782004_U-5798 (Gillis HillRoad)_P&D\Design\Structures\CAD\Culvert\410_003_U-5798B_SMU_CU_002_250001.dgn AcostaM

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(LOOKING DOWNSTREAM)

DOCUMENT NOT CONSI	DERED
FINAL UNLESS AL	L
SIGNATURES COMPLE	TED

LOAD FACTORS:

DESIGN LUAD	RAIING	FACIURS		
LOAD TYPE	MAX FACTOR	MIN Factor		
DC	1.25	0.90		
DW	1.50	0.65		
ΕV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

DESTAN LAND DATING EACTARS

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE. LOAD RATINGS FOR MAXIMUM AND MINIMUM FILL CONDITIONS HAVE BEEN EVALUATED.MINIMUM FILL CONDITION CONTROL LOAD RATINGS.

COMMENTS:

- 1.
- 2.
- 3.
- 4.

(#) CONTROLLING LOAD RATING

 $\left<1\right>$ design load rating (hl-93)

 $\left< 2 \right>$ design load rating (HS-20)

3 LEGAL LOAD RATING **

 $\langle 4 \rangle$ Emergency verhicle load rating * *

** SEE CHART FOR VEHICLE TYPE

	PROJ <u>C</u> STAT	ECT <u>:UM</u> ION 2 of	- no. <u>1BER</u> n:2! 8	 	<u>-U-</u> <u>AND</u> -46.	<u>-5798</u> co <u>30 -l</u>	B unty _B-		
6/19/2025	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-TNTERSTATE TRAFFIC)								
RS&H Architects-Engineers-Planners, Inc.			REVIS	SION	١S		SHEET NO.		
8521 Six Forks Road, Suite 400 Ralelgh, NC 27615 919-926-4100 FAX 919-846-9080	NO. ВҮ:		DATE:	NO.	BY:	DATE:	C-2 total sheets		
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PLAN

WING FOOTING AB WHEN SLAB HAN FOOTING

U-5798B PROJECT NO.__ CUMBERLAND COUNTY STATION: 25+46.30 -LB-

		SHEET 4 C)F 8					
	6/19/2025 OFESS/0/14 SEAL 052672 MC INE ¹²	DEPA	RTMENT BARRI		NORTH CARG TRAN Aleigh STAI	NSPORTA NDARD	TION	
	Matthew J. Acount Matthew J. Acount Matthew J. Acosta	T C(TRIPLE 12 FT.X 8 FT CONCRETE BOX CULVER 90° SKEW					
	RS&H Architects-Engineers-Planners, Inc.		REVI	SIONS	S		SHEET NO.	
)	8521 Six Forks Road, Suite 400 Ralelgh, NC 27615 919-926-4100 FAX 919-846-9080 www.rsandh.com	NO. ВҮ:	DATE:	NO.	BY:	DATE:	C-4 total sheets	
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STD.NO.CB90_3

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Docusign Envelope ID: A3D9725A-5DA8-4FFD-9238-D8B6AA7CA210

BILL OF MATERIAL

		ST	AGE	I				ST	AGE	II			STAGE III				
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	Τ
A1	138	#4	1	6'-7"	607	A1	138	#4	1	6'-7"	607	A1	132	#4	1	6'-7"	T
A2	138	#4	1	6'-7"	607	A2	138	#4	1	6'-7"	607	A2	132	#4	1	6'-7"	
A3	138	#4	1	6'-7"	607	A3	138	#4	1	6'-7"	607	A 3	132	#4	1	6'-7"	_
A100	138	#6	STR	17'-11"	3714	A300	138	#6	STR	38'-4"	7946	A100	132	#6	STR	17'-11"	+
A200	138	#6	STR	17'-11"	3714	A400	138	#6	STR	38'-4"	7946	A200	132	#6	STR	17'-11"	-
B1	138	#4	STR	10'-1"	930	A500	138	#6	STR	24'-0"	4975	B1	132	#4	STR	10'-1"	_
B2	138	#4	STR	7'-4"	676							B2	132	#4	STR	7'-4"	+
Β3	138	#4	STR	10'-1"	930	A600	138	#6	STR	24'-0"	4975	B3	132	#4	STR	10'-1"	
C1	70	#4	STR	35′-7″	1664	B1	138	#4	STR	10'-1"	930	C1	70	#4	STR	35′-7″	+
						B2	138	#4	STR	7'-4"	676						+
D2	4	#6	STR	2'-10"	17	B3	138	#4	STR	10'-1"	930	D2	4	#6	STR	2'-10"	T
D3	16	#6	STR	2'-6"	60												
						C1	194	#4	STR	35'-7"	4611						
						D1	4	#6	STR	1'-10"	11						+
						D2	4	#6	STR	2'-10"	17						T
						D3	50	#6	STR	2'-6"	188						_
						G1	4	#5	STR	38'-4"	160						_
REIN	NFOR	CING S	STEEL	13,5	526 LBS.	REI	NFOR	CING S	STEEL	35,	186 LBS.	REI	NFOR	CING S	STEEL	12,	9!
SILI	_ FOF	r one	BARRE	L		SILI	_ FOF	r two	BARR	ELS		SILI	_ FOF	r one	BARRE	L	
CLAS	SS A	CONCF	RETE			CLA	SS A	CONCF	RETE			CLAS	SS A	CONCF	RETE		
	LC)W FLC	DW SIL	_L	0.0 C.Y.		LC	W FLC	W SIL	L	0.4 C.Y.	LOW FLOW SILL C					
	ΗI	GH FL(DW SIL	_L	0.9 C.Y.		HI(GH FLC	W SIL	L	0.9 C.Y.		HI(GH FLC	WSIL		(
			τοτα	Ĺ	0.9 C.Y.				TOTA	4L	1.3 C.Y.				TOTA	4 L	(



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GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $2^{1}/_{2}$ ".

B. 4 - 1″ØX 2¹/4″BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED.(AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" \varnothing X 2 $^{1}/_{4}$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A $7_{16}'' \varnothing$ wire strut with a minimum tensile STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS ``A'' CONCRETE. FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1"Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE

-.375″ØWIRE STRUT

6/19/2025

SEAL 052672

RSSH

Matthew B. Acosta

└── NO.6 GAGE WIRE

PROJECT NO. <u>U-5798B</u> CUMBERLAND COUNTY STATION: 25+46.30 -LB-

SHEET 7 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

ANCHORAGE DETAILS FOR GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

RS&H Architects-Engineers-Planners, Inc.			SHEET NO.				
8521 Six Forks Road, Suite 400 Raleigh, NC 27615	NO.	BY:	DATE:	NO.	BY:	DATE:	C – 7
919-926-4100 FAX 919-846-9080	1			\Im			TOTAL SHEETS
www.rsandh.com North Carolina License Nos. 50073 * F-0493 * C-28	2			4			8

SID. NO. GRA1 (SHT 1)

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BILL OF MATERIAL						BILL OF MATERIAL					
STAGE T AND STAGE TTT						STAGE TT AND STAGE TV					
(FACH STAGE)						(FACH STAGE)					
7	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	6	#4	STR	16'-7"	66	H1	6	#4	STR	16'-7"	66
	2	#4	STR	12'-10"	17	H2	2	#4	STR	12'-10"	17
	2	#4	STR	7'-9"	10	H3	2	#4	STR	7'-9"	10
	2	#4	STR	2'-9"	4	H4	2	#4	STR	2'-9"	4
	12	#4	1	3'-3"	26	Н5	12	#4	1	3'-3"	26
	2	#4	STR	17′-4″	23	H6	2	#4	STR	17′-4″	23
	2	#5	2	10'-7"	22	N1	2	#5	2	10'-7"	22
	2	#5	2	10'-0"	21	N2	2	#5	2	10'-0"	21
	3	#5	2	9'-1"	28	N3	3	#5	2	9'-1"	28
	3	#4	2	8'-3"	1 (N4	3	# 4	2	8'-3"	1 (
	3	# 4 # 4	2	('-4''	15	N5	3	# 4	2	('-4''	17
	3	# 4	2	6'-6"	15	N6	<u> </u>	# 4 # 4	2	<u>б'-б"</u>	13
				5 - I					2	5 - 1	11
	3	#6	STR	6'-0"	27	< 1	7	#6	STR	6'-0"	27
		0	<u> </u>	0 0	<u> </u>	51		0	<u> </u>	0 0	<u> </u>
	3	#5	STR	18′-1″	57	Τ1	3	#5	STR	18'-1"	57
			0.111	10 1					0.111	10 1	
	2	#4	STR	8'-6"	11	V1	2	#4	STR	8'-6"	11
	2	#4	STR	7'-11"	11	V2	2	#4	STR	7′-11″	11
1	3	#4	STR	7'-0"	14	V 3	3	#4	STR	7'-0"	14
	3	#4	STR	6'-2"	12	V 4	3	#4	STR	6'-2"	12
	3	#4	STR	5′-3″	11	٧5	3	#4	STR	5′-3″	11
	3	#4	STR	4'-5"	9	V6	3	#4	STR	4'-5"	9
	3	#4	STR	3'-6"	7	ν7	3	#4	STR	3'-6"	7
	2	#5	3	6'-0"	13	Z1	2	#5	3	6'-0"	13
	2	#5	3	5'-9"	12	Z2	2	#5	3	5'-9"	12
	3	#5	3	5'-3"	16	23	3	#5	3	5'-3"	16
	2 7	++ 4 ++ 1	5 7	4'-8''	<u>у</u>	24	<u>う</u> マ	#4 #7	う マ	4'-8"	у
	ר) ד	т4 #л) ד	4 -1 7/_0″	7 7	20 76	ے ح	т4 #л) ד	4 -1 	0 7
	ך א ר	— […] 4 # Л	ך א	J =0 3′_1″	<u>г</u>	20 77	ך א	— ··· 4 # Л	ך א	J =0 	۱ ۲
				J 1	0	<u> </u>				J _ I	0
NFORCING STEEL ? 1 WING 503 LBS.				REINFORCING STEEL FOR 1 WING 503 LBS.							
ASS A CONCRETE 1 WING 1 HEADWALL 1 END CURTAIN WALL TOTAL 8.3 CY					CLASS A CONCRETE 1 WING 1 HEADWALL 1 END CURTAIN WALL 10.4 CY						

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	see plans
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/2" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ radius WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

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STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FÁLSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 - $\frac{7}{8}$ " Ø studs for 4 - $\frac{3}{4}$ " Ø studs, and stud spacing changes SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø studs based on the ratio of 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAÍNTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

