SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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SHEET NO. **DESCRIPTION** TITLE SHEET LEGEND (SOIL AND ROCK) ROADWAY TITLE SHEET PLAN SHEETS PAVEMENT DATA SHEETS 8-13 DCP DATA GRAPHS PAVEMENT CORE PHOTOS PAVEMENT CORE EVALUATION

APPENDIX A

SHEETS SOIL TEST RESULTS 18

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY _**HAYWOOD**

PROJECT DESCRIPTION _REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY

PAVEMENT & SUBGRADE INVESTIGATION

STATE PROJECT REFERENCE NO. B-5541

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6550. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE OR NOL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

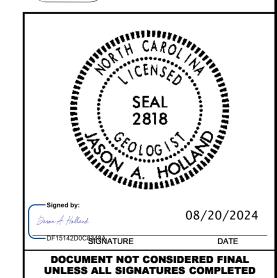
PERSONNEL

	Q. HILL
	CG2 EXPLORATION
ESTIGATED	BY _ <i>J. HOLLAND</i>

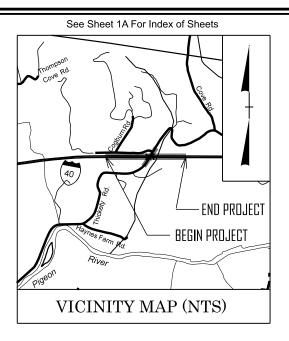


DRAWN BY _J. HOLLAND SUBMITTED BY SCHNABEL ENG.

DATE _AUGUST 2024



B REFERENCE



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

HAYWOOD COUNTY

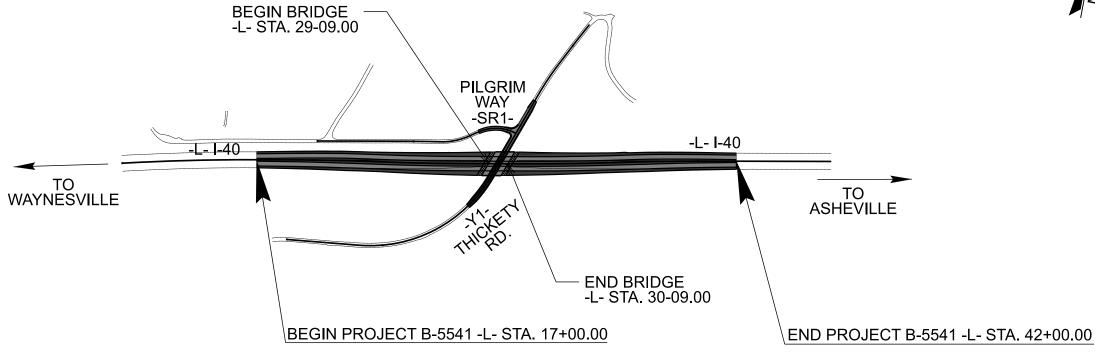
LOCATION: BRIDGE NO. 236 OVER SR 1513 ON I-40

TYPE OF WORK: GRADING, PAVING, DRAINAGE AND STRUCTURES

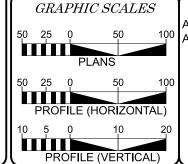
STATE	STATI		SHEET NO.	TOTAL SHEETS			
N.C.	E	B-5541					
STAT	TE PROJ. NO.	F. A. PROJ. NO.		DESCRIPT	TION		
550	041.1.1			P.E.			
		_					
		_					
		_					
		_					
		_					

DESIGN RECOMMENDATION PLAN SET





THERE IS FULL CONTROL OF ACCESS ON THIS PROJECT. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ??? INCOMPLETE PLANS



DESIGN DATA

ADT 2024 = 60000 ADT 2044 = 77500 K = 8 % D = 55 %

T = 15 % *V = 65 MPH* TTST =12% DUAL 3% FUNC CLASS =

INTERSTATE STATE WIDE TIER

PROJECT LENGTH

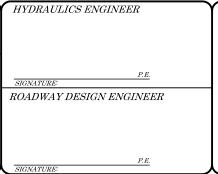
LENGTH ROADWAY TIP PROJECT B-5541 = 0.471 MILES LENGTH STRUCTURE TIP PROJECT B-5541 = 0.021 MILES TOTAL LENGTH TIP PROJECT B-5541 = 0.492 MILES

Prepared in the Office of: 2018 STANDARD SPECIFICATION RIGHT OF WAY DATE: CHRIS ANDERSON, PE MARCH 15, 2024

LETTING DATE:

JUNE 17, 2025

DYLAN MCCANN





ROBERT CAROLINA
DEPARTMENT OF TRANSPORTATION
HAYWOOD COUNTY

ROADWAY DESIGN UNIT
ROADWAY DESIGN
ENGINEER

INCOMPLETE PLANS
SO OF THE PLANS
SO

FWTSTONS

R/W ROADWAY DESIGN ENGINEER

HYDRAULICS ENGINEER

OCUMENT NOT CONSIDERED FINA LESS ALL S**I**GNATURES COMPLET

PREPARED BY
NIVS
NOS ENANCERS & CONSULTATES, FIC.
851 & MOULTHIS PARK DEPK, SLIFE 235
DARGOTTE, KOR251
E 74-653 7/2500 MANUAVIS-2001
d Sense 17-4200

B-5541 R/W 6

HYDRAULICS ENGINEER

PREPARED BY

NAS ENGINEERS & CONSULTANTS, INC.
8514 MCNLPINE PARK DENY, SLITE 135
CHARLOTTE, NC 28211
P. 704,537,7300 www.Wis.com
Mt Green P.4331

PAVEMENT INVESTIGATION DATA SHEET

Project: 55041.1.1 TIP: B-5541

I-40 Route: County: Haywood

Prepared by: J. Holland 4/17/2024 Date: Reviewed by: Date:

		Width	3				Paveme	ent Struct	ure / Thic	kness 3		Subgrade ⁴					State Plane (Coordinates
Position ¹ (StaDirection_Lane,Shldr.)	Cut/Fill ² (Est. of Amount)	Lane(s)	Shoulder(s)	Offset Distance	Crown "C" or Super "S"	Pavement Layering	Total to Subgrade (in.)	Asphalt (in.)	Concrete (in.)	ABC (in.)	CTBC (in.)	Description	Sample ID	AASHTO Classification	Soil Moisture Probe Depth	Asphalt Notes	Northing	Easting
-L- STA 20+00																		
L_PC_2000_EB_ISS	CUT ± 5.0'	ISL 12.0'	ISS 6.0'	1.5' FY	С	Asphalt ABC	28.5	13.5	-	15.0	-	RES: Reddish-brown, silty SAND, little mica (2.4' - 5.0')	-	A-2-4	M 5.0'	No pavement distress observed	677,165	843,489
L_PC_2000_EB_ISL	CUT ± 5.0'	ISL 12.0'	ISS 6.0'	4.0' FY	С	Asphalt ABC	30.0	14.0	-	16.0	-	RES: Brown, orange, silty SAND, little mica (2.5' - 5.0')	S-3	A-2-4(0)	13% 5.0'	Recent overlay. No pavement distress observed	677,158	843,490
L_PC_2000_EB_OSS	CUT ± 5.0'	OSL 12.0'	OSS 11.5'	3.5' FW	С	Asphalt ABC	27.5	7.5	-	20.0	-	RES: Brown, orange, silty SAND, little mica (2.3' - 5.0')	-	A-2-4	M 5.0'	No pavement distress observed	677,135	843,488
-L- STA 22+60			ı			_			ī	1								
L_PC_2260_WB_OSS	CUT ± 12.0'	OSL 12.0'	OSS 11.0'	5.0' FW	С	Asphalt ABC	27.5	6.5	-	21.0	-	RES: White, tan, silty SAND, some mica (2.3' - 5.0')	S-13	A-2-4(0)	18% 5.0'	No pavement distress observed	677,256	843,737
L_PC_2260_WB_ISL	CUT ± 13.0'	ISL 12.5'	ISS 6.0'	4.0' FY	С	Asphalt ABC	32.0	14.0	-	18.0	-	RES: Tan, brown, silty SAND, little mica (2.7' - 5.0')	S-9	A-2-4(0)	8% 5.0'	Recent overlay. No pavement distress observed	677,230	843,743
L_PC_2260_WB_ISS	CUT ± 14.0'	ISL 12.5'	ISS 6.0'	1.5' FY	С	Asphalt ABC	31.0	12.5	-	18.5	-	RES: Tan, white, silty SAND, some mica (2.6' - 5.0')	-	A-2-4	M 5.0'	No pavement distress observed	677,223	843,744
-L- STA 35+00															· ·			
L_PC_3500_EB_ISS	CUT ± 12.0'	ISL 11.5'	ISS 6.0'	1.5' FY	С	Asphalt ABC	32.0	13.5	-	18.5	-	RES: Tan, sandy SILT, trace mica (2.7' - 5.0')	S-6	A-4(0)	10% 5.0'	No pavement distress observed	677,436	844,960
L_PC_3500_EB_ISL	CUT ± 12.0'	ISL 11.5'	ISS 6.0'	4.0' FY	С	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, sandy SILT, trace mica (2.5' - 5.0')	-	A-4	M 5.0'	Recent overlay. No pavement distress observed	677,428	844,958
L_PC_3500_EB_OSS	CUT ± 12.0'	OSL 12.0'	OSS 12.5'	3.5' FW	С	Asphalt ABC	27.5	6.5	-	21.0	-	RES: Brown, orange, silty CLAY, little mica (2.3' - 5.0')	S-2	A-7-6(4)	77% 5.0'	No pavement distress observed	677,405	844,953
-L- STA 37+50				<u> </u>										•				
L_PC_3750_WB_OSS	CUT ± 15.0'	OSL 12.0'	OSS 11.0'	7.0' FW	С	Asphalt ABC	25.5	5.5	-	20.0	-	RES: White, tan, silty SAND, some mica (2.1' - 5.0')	S-10 S-11	A-2-4(0) A-2-4(0)	4% 13% 5.0'	No pavement distress observed	677,523	845,194
L_PC_3750_WB_OSL	CUT ± 13.0'	OSL 12.0'	OSS 11.0'	2.5' FW	С	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, brown, sandy SILT, little mica (2.5' - 5.0')	-	A-4	M 5.0'	Recent overlay. No pavement distress observed	677,511	845,194
L_PC_3750_WB_ISS	CUT ± 10.0'	ISL 12.0'	ISS 6.0'	0.5' FY	С	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, sandy CLAY, trace mica (2.5' - 5.0')	S-7	A-6(1)	15% 5.0'	No pavement distress observed	677,490	845,195

- Notes:

 1. Refer to plan sheets for pavement investigation locations.
- 2. Cut/Fill are rough estimates based on a review of existing topographic features and are for reference only.
- 3. Values are based on field measurements at time of investigation.
- 4. Soil descriptions, classifications, and moisture are based on visual-manual methods, unless otherwise noted.

Abbrevations:
OSL = Outside Lane ISL = Inside Lane CL = Center Lane LTL = Left Turn Lane

CTL = Center Turn Lane RTL = Right Turn Lane DECEL = Deceleration Lane ACCEL = Acceleration Lane

OSS = Outside Shoulder ISS = Inside Shoulder GM = Grass Median OGS = Outside Grass Shoulder

PS = Paved Shoulder RT LN = Right Lane LT LN = Left Lane COL = Collector Lane D = Dry Soil

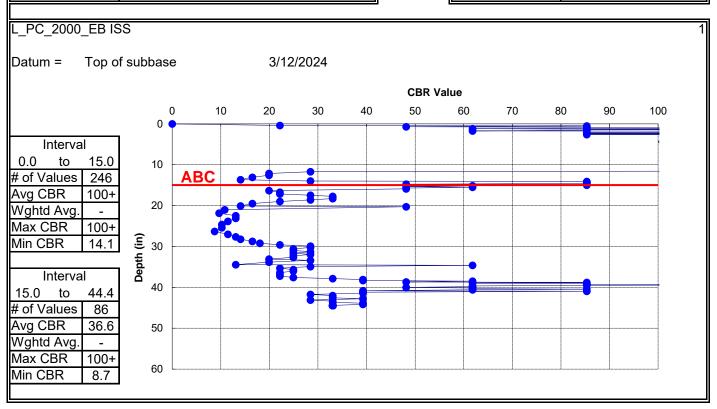
RT = Right LT = Left (I) = Inside (O) = Outside M = Moist Soil NB = Northbound SB = Southbound FW = From White FY = From Yellow W = Wet Soil

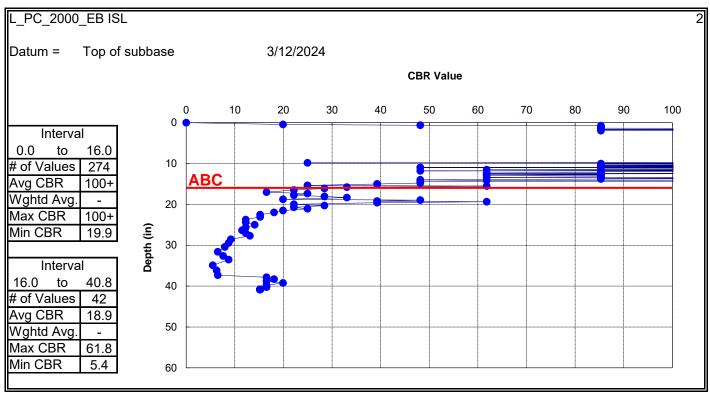
ABC = Aggregate Base Course CTBC = Cement-Treated Base Course RES = Residual Soil R.E. = Roadway Embankment



	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
	Replace Bridge No. 236 on I-40
	Haywood

GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A



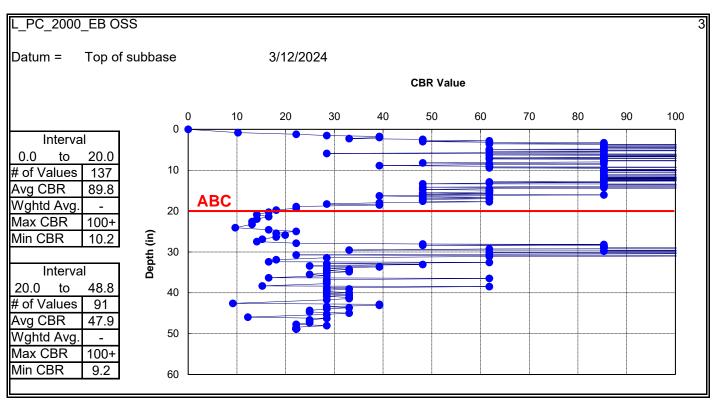


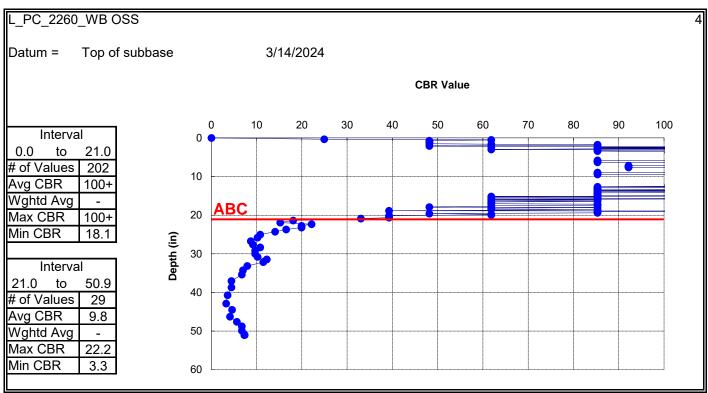
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age	9

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

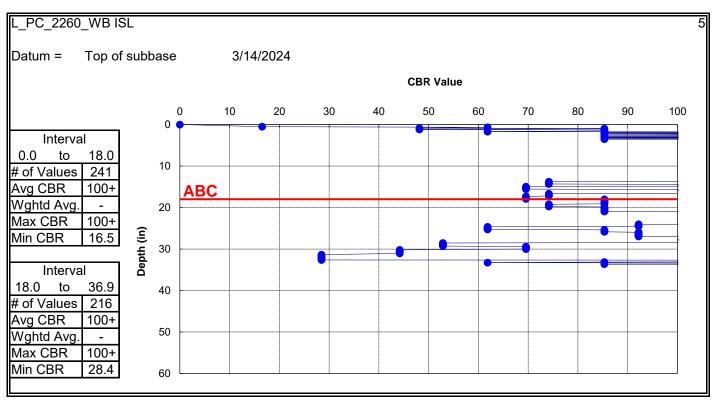
GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A

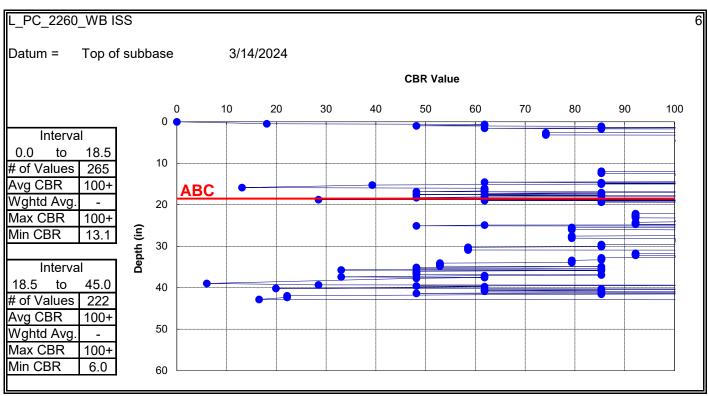




	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
	Replace Bridge No. 236 on I-40
	Havwood

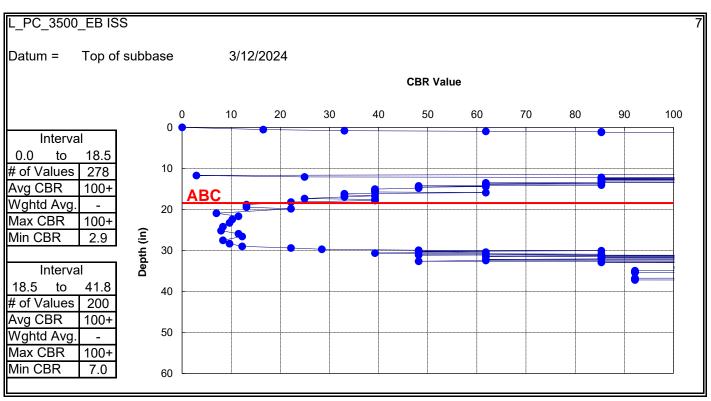
GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A

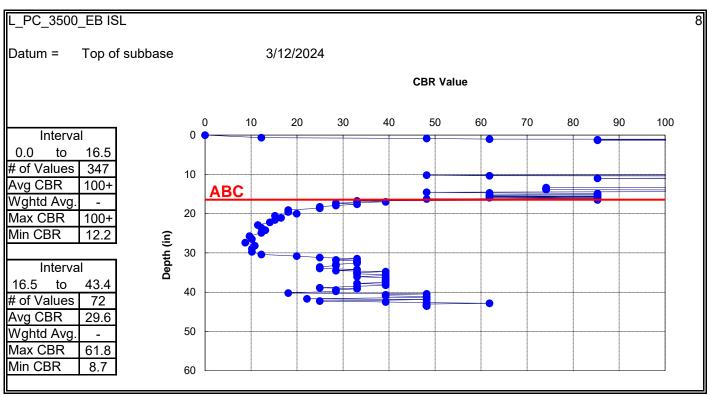




	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
	Haywood

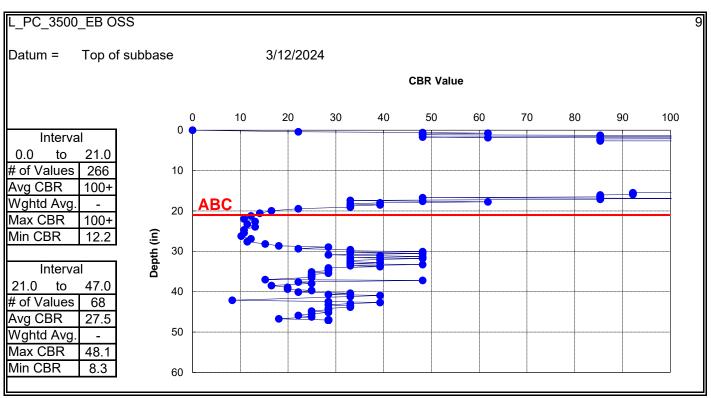
GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A

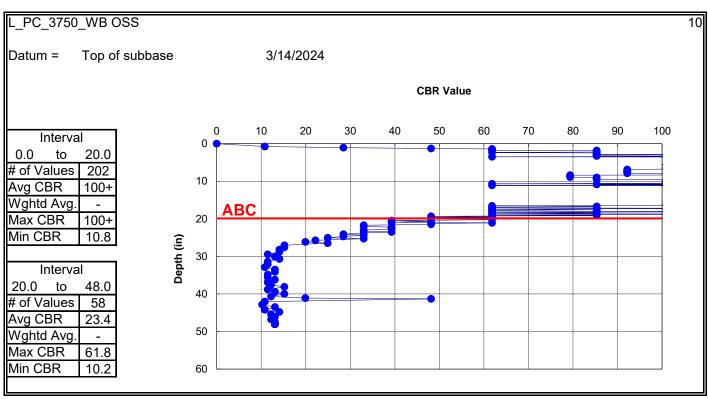




	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
	Replace Bridge No. 236 on I-40
	Haywood

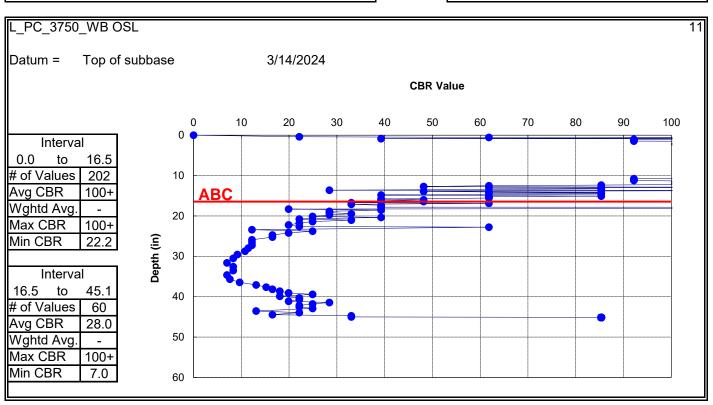
GEOLOGIST	Quinton Hill			
	C. Odom			
GEOTECH(S)	Z. Taylor			
	N/A			

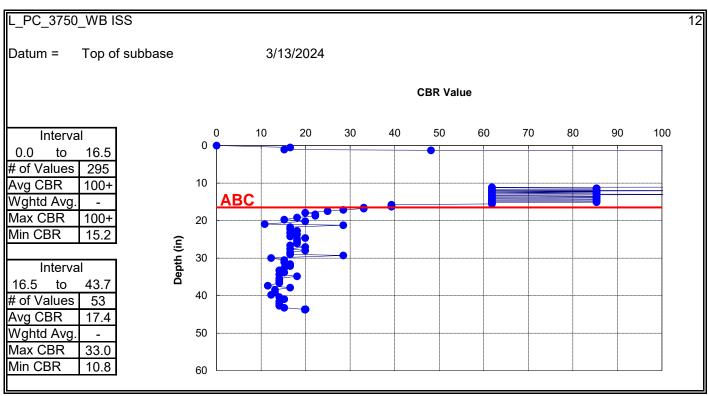




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PROJECT NAME 2-Lane Extension of SR 1630				
	Replace Bridge No. 236 on I-40			
	Haywood			

GEOLOGIST	Quinton Hill			
	C. Odom			
GEOTECH(S)	Z. Taylor			
	N/A			





PAVEMENT CORE PHOTOGRAPHS TIP No. B-5541|Haywood County, NC





PAVEMENT CORE PHOTOGRAPHS TIP No. B-5541|Haywood County, NC



PAVEMENT CORE PHOTOGRAPHS TIP No. B-5541|Haywood County, NC

L_PC_3500_EB_OSS





L_PC_3750_WB_ISS

PAVEMENT CORE EVALUATION FOR I-40 HAYWOOD COUNTY, NC

	CORE	LOCATION		SUB	BASE				PAVEMENT CORE EVALUATION
	Station				Layer		Layer		
	Total				Thick-		Thick-		
Lina	Pavement	Lane Direction	Lane	Layer	ness	Layer	ness	# of Lifts	REMARKS
Line	(in.)	Direction	Location	Type ABC	(in.) 15.0	Type S	(in.) 5.00	_	Low severity oxidation and stripping in 1st two lifts.
-L-	20+00	EB	ISS	ADC	15.0	В	8.50	_	Low severity stripping at boundary of 2nd/3rd lift.
-	13.50	LD	133				0.50	,	Low severity stripping at boundary or 2na/3rd int.
	13.50			ABC	16.0	OGFC	1.00	1	
				7100	10.0	S	1.50	1	
-L-	20+00	EB	ISL			ı	4.00	1	
						S	3.75		Mechanical break at lift interface.
	14.00					В	3.75		Low severity stripping.
				ABC	20.0	S	7.50		Aggregate size ≤12.5mm. Bottom 1" is larger bound aggregate.
-L-	20+00	EB	oss					0	30 0
	7.50							0	
				ABC	21.0	S	6.50	_	Aggregate size ≤12.5mm. Bottom 1" is larger bound aggregate.
-L-	22+60	WB	oss					0	
	6.50							0	
				ABC	18.0	OGFC	1.00	1	
						S	1.50	1	
-L-	22+60	WB	ISL			ı	4.00	1	Mechanical break at lift interface.
						S	3.75	3	
	14.00					В	3.75	1	
	22.60			ABC	18.5	S	5.00	3	Aggregate size ≤12.5mm. Delamination 2.5" from top.
-L-	22+60	WB	ISS			В	7.50	3	
	12.50							0	
	35+00			ABC	18.5	S	5.50	3	Aggregate size ≤12.5mm. Low severity stripping near bottom of 3rd lift.
-L-	33+00	EB	ISS			- 1	5.00	2	Mechanical break within 1st lift.
	13.50					В	3.00	1	
	35+00			ABC	16.5	OGFC	1.00	1	
-L-	33100	EB	ISL			S	9.00	6	Aggregate size ≤12.5mm. 4th lift 0.5", possible OGFC.
	13.50					В	3.50	1	
1	35+00			ABC	21.0	S	6.50		Aggregate size ≤12.5mm.
-L-		EB	OSS					0	
<u></u>	6.50							0	
I	37+50			ABC	20.0	S	6.00		Aggregate size ≤12.5mm.
-L-	37.33	WB	OSS					0	
<u> </u>	6.00							0	
I	37+50			ABC	16.5	OGFC	1.00	1	
-L-		WB	OSL			S	4.00	_	Aggregate size ≤12.5mm.
<u> </u>	13.50					- 1	8.50	_	3rd lift is 1" of surface mix.
I	37+50			ABC	16.5	S	5.50		Aggregate size ≤12.5mm.
-L-		WB	ISS			- 1	5.00	2	
	13.50					В	3.00	1	

Abbrevations:

OSL = Outside Lane
ISL = Inside Lane
CL = Center Lane
DECEL = Deceleration Li OSL = Outside Lane CTL = Center Turn Lane OSS = Outside Shoulder
ISL = Inside Lane RTL = Right Turn Lane ISS = Inside Shoulder
CL = Center Lane DECEL = Deceleration Lane LTL = Left Turn Lane ACCEL = Acceleration Lane OGS = Outside Grass Shoulder NB = Northbound ABC = Aggregate Base Course SB = Southbound RES = Residual Soil FW = From White R.E. = Roadway Embankment PS = Paved Shoulder RT = Right

Pavement Layer Types:
(S) = Surface (I) = Intermediate (B) = Base SD = Sand Asphalt C = Concrete MS/CS = Mat Seal or Chip Seal OGFC = Open Graded Friction Course UT = Ultra-thin Bonded Wearing Course PADC = Permeable Asphalt Drainage Course DS = Drainage Sand CTBC = Cement Treated Base Course PC = Prime Coat S-C = Soil Cement S-L = Soil Lime N.E. = Not Encountered FEA = Flat and Elongated Aggregate



Prepared by: J. Jenkins

Reviewed by: J. Crenshaw

	REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY														
	PAVEMENT INVESTIGATION SOIL TEST RESULTS -L-														
SAMPLE	MPLE BORING ID OFFSET DEPTH INTERVAL AASHTO L.L P.I. % BY WEIGHT % PASSING (SIEVES) %									%					
NO.	BUKING ID	DISTANCE	(ft)	CLASS.	L.L	P.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
S-3	L_PC_2000_EB ISL	4' FY	4.0-5.0	A-2-4 (0)	31	8	37.6	30.3	6.5	25.6	87.2	62.5	33.1	13	-
S-13	L_PC_2260_WB OSS	5' FW	4.0-5.0	A-2-4 (0)	NP	NP	41.6	29.3	7.5	21.6	84	55.3	28.9	18	-
S-9	L_PC_2260_WB ISL	4' FY	4.0-5.0	A-2-4 (0)	NP	NP	39.2	38	3.5	19.4	90.6	68.7	25.4	8	-
S-2	L_PC_3500_EB OSS	3.5' FW	4.0-5.0	A-7-6 (4)	45	18	27.5	31.7	18.1	22.6	93.9	75.1	43.8	77	-
S-6	L_PC_3500 EB ISS	1.5' FW	4.0-5.0	A-4 (0)	29	6	31.8	35.5	12.8	19.9	90.8	70.2	36.1	10	-
S-10	L_PC_3750_WB OSS	7' FW	2.1-3.0	A-2-4 (0)	NP	NP	50.2	26.2	11	12.6	80.2	50.8	24.1	4	-
S-11	L_PC_3750_WB OSS	7' FW	4.0-5.0	A-2-4 (0)	NP	NP	41.2	32.1	5.7	21	84.4	55.9	27.7	13	-
S-7	L_PC_3750_WB ISS	0.5' FY	4.0-5.0	A-6 (1)	39	11	30.2	33.2	14.3	22.2	92.7	72.2	40	15	-

Notes:

^{1.} Refer to Pavement Investigation Data Sheet for station abbreviations

	BULK SAMPLE SOIL TESTS SUMMARY FOR -L-													
Station	Offset (ft)	Sample ID	Depth Interval (ft)	AASHTO Soil Class. (Group Index)	L.L. P.I.		% Passing (Sieves)		Max. Dry Density (pcf)	Optimum Moisture Content	CBR Value		CBR Swell (%)	
							10	40	200		(%)	0.1"	0.2"	
25+00	80' RT	BS-2	0.0 - 10.0	A-6 (5)	40	16	89.3	67.3	50.3	107.1	17.3	3.4	3.8	2.5
34+50	80' RT	BS-1	0.0 -10.0	A-7-5 (4)	52	15	96.2	75.7	46	107.2	18.8	2.7	3.4	4.7

Notes: 1. Refer to laboratory transmittal for additional information

	BULK SAMPLE SOIL TESTS SUMMARY FOR -Y1-															
Station	Offset (ft)	Sample ID	Depth Interval (ft)	AASHTO Soil Class. (Group Index)	L.L.	L.L. P.I.		% Passing (Sieves)		P.I. % Passing (Sieves)		Max. Dry Density (pcf)	Optimum Moisture Content	CBR (Corre	Value ected)	CBR Swell (%)
							10	40	200		(%)	0.1"	0.2"			
26+00	15' LT	BS-3	0.0 - 10.0	A-2-6 (0)	37	12	64.9	40.3	20.5	119.6	14	12	13.3	4.1		

Notes: 1. Refer to laboratory transmittal for additional information

CONTENTS

APPENDIX A

DCP RAW DATA LOGS SOIL TEST RESULTS

BORING LOGS

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REFEREN

STATION 17+00 - 42+00 4-6 -L--YI-20+00 - 26+50 18+50 - 20+50

SHEETS

7-20

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY **HAYWOOD**

PROJECT DESCRIPTION REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY **INVENTORY**

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

<u>LINE</u> <u>PLAN</u>

STATE PROJECT REFERENCE NO. STATE OF NORTH CAROLINA B-5541

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES, THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6550. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IM-PELACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

Q. HILL
CG2 EXPLORATION

INVESTIGATED BY <u>J.</u>HOLLAND

DRAWN BY _ **J. HOLLAND**

CHECKED BY J. CRENSHAW

SUBMITTED BY _SCHNABEL ENG.

DATE APRIL 2024





DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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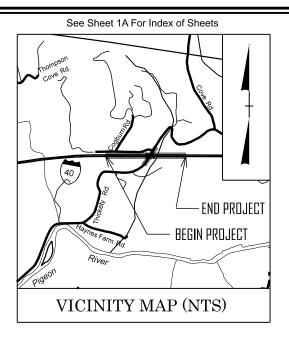
PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	<u>AQUIFER</u> - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	NI//SI//A	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
ULASS. (\$ 35% PASSING *200) (> 35% PASSING *200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-2-5 A-3 A-3 A-5 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 0000 d00000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7. PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
■10 50 MX GRANULAR SIL1- MUCK,	PERCENTAGE OF MATERIAL	CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 30 MX 50 MX 51 MN PEAT *200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITILL M HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN.RATING EXCELLENT TO COOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBLIKADE POUR	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTIVES OF PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-VALUE) (TUNS/FT-)	WITH SOIL DESCRIPTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4 CONTROL OF CONTROL	SOIL SYMBOL OPT DWT TEST BORING SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL MEDIUM DENSE 30 10 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT THOUGH BURING TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING ● SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i>	OF AN INTERVENING IMPERVIOUS STRATUM,
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MW MONITORING WELL TEST BORING	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	TTTTT ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY SPT N-VALUE	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAV	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	LICED IN THE TOP 2 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY γ - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOU MOISTURE SCALE FIELD MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE LL _ LIQUID LIMIT	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE CONTROL OF THE PROPERTY	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING	BENCH MARK: N/A
	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: N/A FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: N/A FEET
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS FLIGHT AUGER CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY	CME-55 X 8" HOLLOW AUGERS CORE SIZE:	INDURATION	
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM	VANE SHEAR TEST CASING W/ ADVANCER HAND TOOLS:	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH X HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
1	10cm CONE PENETROMETER VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN. RED. YELLOW-BROWN, BLUE-GRAY).			1
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X DIEDRICH D-50 X 4" THIN-WALL CORE BARREL X K-100 KESSLER DCP	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

HAYWOOD COUNTY

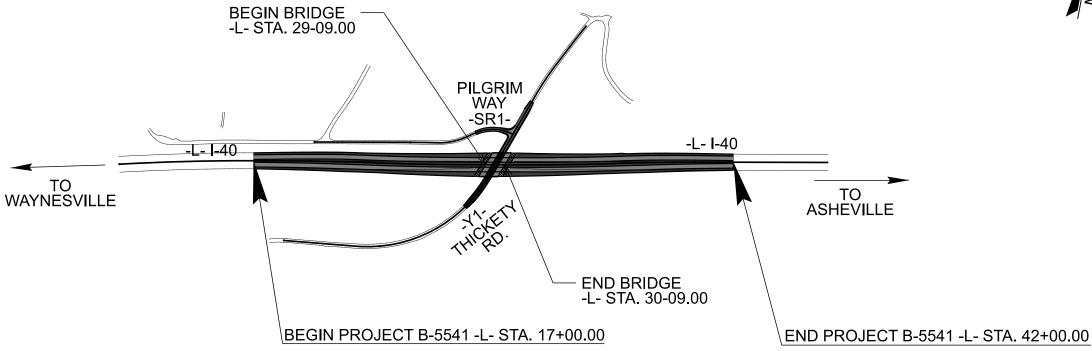
LOCATION: BRIDGE NO. 236 OVER SR 1513 ON I-40

TYPE OF WORK: GRADING, PAVING, DRAINAGE AND STRUCTURES

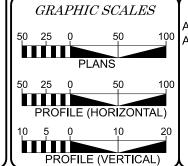
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DESIGN RECOMMENDATION PLAN SET





THERE IS FULL CONTROL OF ACCESS ON THIS PROJECT. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ??? INCOMPLETE PLANS



DESIGN DATA ADT 2024 = 60000

ADT 2044 = 77500 K = 8 %D = 55 %

T = 15 %*V = 65 MPH* TTST =12% DUAL 3% FUNC CLASS = INTERSTATE

STATE WIDE TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5541 = 0.471 MILES LENGTH STRUCTURE TIP PROJECT B-5541 = 0.021 MILES TOTAL LENGTH TIP PROJECT B-5541 = 0.492 MILES

Prepared in the Office of: 2018 STANDARD SPECIFICATION RIGHT OF WAY DATE: CHRIS ANDERSON, PE

MARCH 15, 2024

LETTING DATE:

JUNE 17, 2025

DYLAN MCCANN

HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER





Schnabel Engineering South, P.C.
1133 Military Cutoff Road, Suite 210 / Wilmington, NC 28405

T 910.769.1621 schnabel-eng.com

Sheet 3A

April 23, 2024

STATE PROJECT: 55041.1.1

TIP NUMBER: B-5541

COUNTY: HAYWOOD

DESCRIPTION: REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY

SUBJECT: Geotechnical Roadway Inventory Report

Project Description

The project consists of the replacement of the existing four span Bridge (No. 236) over SR-1513 (Thickety Rd.) and widening improvements to the 0.45-mile roadway bridge approach along I-40 in the town of Clyde, North Carolina. The newly constructed bridge will be replaced on the existing alignment, with symmetrical widening to the outside, and accommodate two lanes of traffic in each direction during construction. We also investigated an existing 72-inch diameter corrugated metal pipe (CMP) culvert, located about 100 feet from the end of the new bridge, at approximately -L- STA. 31+00, which drains surface waters from a tributary of Pigeon River. The provided plans showed anticipated cut excavations as deep as $20\pm$ feet and proposed embankment construction with fill heights up to $23\pm$ feet max fill over the culvert. Slope inclinations are 2:1 or flatter, except at culvert steepened fill slopes up to 1.35:1 are proposed. Boring logs associated with the existing culvert are included in this report.

The field investigation was conducted in February and March of 2024 utilizing a track-mounted Diedrich D-50 drill machine with an automatic hammer, and hand tools. Standard Penetration Tests (SPT) were performed at selected locations. Borings were advanced with hollow stem augers, polycrystalline diamond compact bit (PDC) for mud rotary methods, and hand tools along the project corridor. Hand augers were performed at locations the drill rig could not access, or where concentrated underground utilities warranted. Dynamic Cone Penetration Tests (DCP) were conducted at all hand auger locations. Representative soil samples were collected and forwarded to an approved testing facility for soil quality analysis, moisture content, California Bearing Ratio, specific gravity, unconsolidated undrained triaxial shear, one-dimensional consolidation, and AASHTO classification.

The Following Alignments were Investigated

Line		Statio	n	Length (ft)
-L-	17+00	to	42+00	2,500
-Y1-	20+00	to	26+50	650
-SR1-	18+50	to	20+50	200
			Total=	3,350 feet (~0.634 miles)

Physiography and Geology

Based on a review of the Geologic map of North Carolina (1985) and the Geologic map of the Western Half of the Asheville 1:100,000-Scale 30 x 60 minute quadrangle, North Carolina and Tennessee (2008), the project is located in the Blue Ridge Physiographic Province. According to the 30 x 60 quadrangle map, the Hot Springs thrust fault appears to intersect the project corridor, trending from northeast to southwest, exposing Grenville-aged granulite facies metamorphic rocks of Earlies Gap (Ye). Soils in the area generally consist of residual sands, silts, and clays. Weathered rock and Late Proterozoic crystalline rock of the Ashe Metamorphic Suite Complex (Zas) and Earlies Gap (Ye) primarily consisted of schist and biotite gneiss. Topography along the project corridor is undulating, traversing along I-40 and bounded by some heavily wooded areas, a few residences, and a paper processing facility to the southeast. Natural ground surface elevations range from 2617± feet above sea level at the beginning of the -Y1-alignment to 2724± feet above sea level at the end of the -L- alignment construction limits.

Soil Properties

Soil and rock encountered along the project corridor are divided into five categories based on origin: roadway embankment soils, alluvial soils, residual soils, weathered rock, and crystalline rock.

Roadway embankment soils consisting of medium dense SAND and GRAVEL (A-1-b), very loose to loose silty and clayey SAND (A-2-7), and soft to medium stiff, sandy SILT (A-4) were encountered along the -L- alignment. Soils moistures were typically moist and varied in thickness from the ground surface to a maximum of 16 feet. Within the cohesive roadway embankment soil, moisture contents were reported at 21%. The plasticity index (PI) within the cohesive soil was 8.

Alluvial soils consisting of very loose SAND and GRAVEL (A-1-b), very loose to medium dense silty SAND and clayey SAND (A-2-4, A-2-6), medium stiff sandy SILT (A-4), and very soft to soft silty CLAY (A-7-6) were encountered along the -L- and -Y1- alignments near the Culvert Outfall. Soils moistures were typically moist to wet, and varied in thickness from 3 to 12 feet. Within the cohesive alluvial soils, moisture contents ranged from 27.0% to 35.0%. Plasticity indices (PI) within the cohesive sediments range from 15 to 16.

Residual soils consisting of very loose to very dense silty SAND and clayey SAND (A-2-4, A-2-6, A-2-7), soft to hard, sandy SILT and clayey SILT (A-4, A-5), and very soft to hard, sandy CLAY, silty CLAY, and sandy and silty CLAY (A-6, A-7-5, A-7-6) were encountered throughout the project corridor. Soil moistures were typically moist to saturated and varied in thickness from the ground surface to a maximum of 79.4 feet. Within the cohesive residual soils, moisture contents ranged from 4.0% to 77.0%. Plasticity indices (PI) within the cohesive sediments range from 5 to 20.

Weathered rock consisting of tan, orange, brown, and white, INJECTION GNEISS, was encountered interlaid and underlying residual soils at several locations along the project corridor. Weathered rock elevations in these borings varied from 2579± feet above sea level to 2655± feet above sea level. Auger and split spoon refusal were noted beneath some of these layers on crystalline rock (INJECTION GNEISS).

Crystalline rock was identified by split spoon refusal, underlying residual soils and weathered rock at several locations along the corridor. USGS geologic maps and the historical Structure Subsurface Investigation confirm bedrock consists of INJECTION GNEISS. Top of rock elevations in these borings varied from 2567± feet above sea level to 2609± feet above sea level. No rock core samples were collected as part of this investigation.

Groundwater

Borings were left open for a minimum of 24 hours to equilibrate with the surrounding conditions, when conditions permitted. Groundwater data was collected in February and March of 2024. Groundwater elevations generally varied with topography and ranged from 2614± to 2674± feet above sea level.

Areas of Special Geotechnical Interest

A. Alluvial Soils were encountered in the following sections.

Alignment	Begin Station	End Station	Offset
-L-	30+00	30+75	± 117 RT
-L-	31+25	31+50	± 170 LT
-Y1-	20+25	20+75	± 75 RT

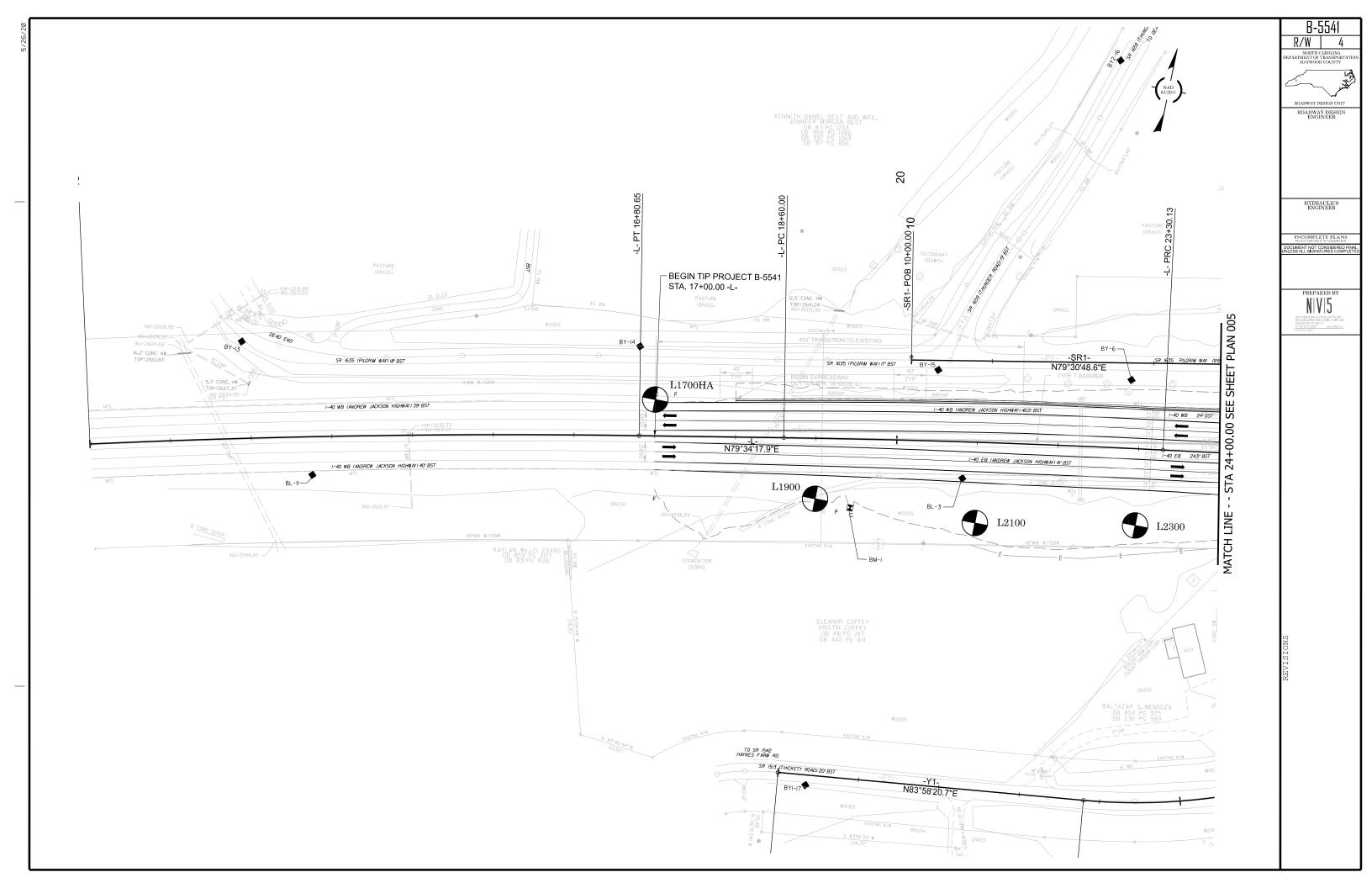
In addition to the alluvial soils found in the areas listed above, a culvert was identified at approximately -L- Sta. 30+50. The plans that were available did not indicate any proposed construction related to this culvert, however, after discussing with NCDOT we were granted authorization to investigate both north and south outfall locations.

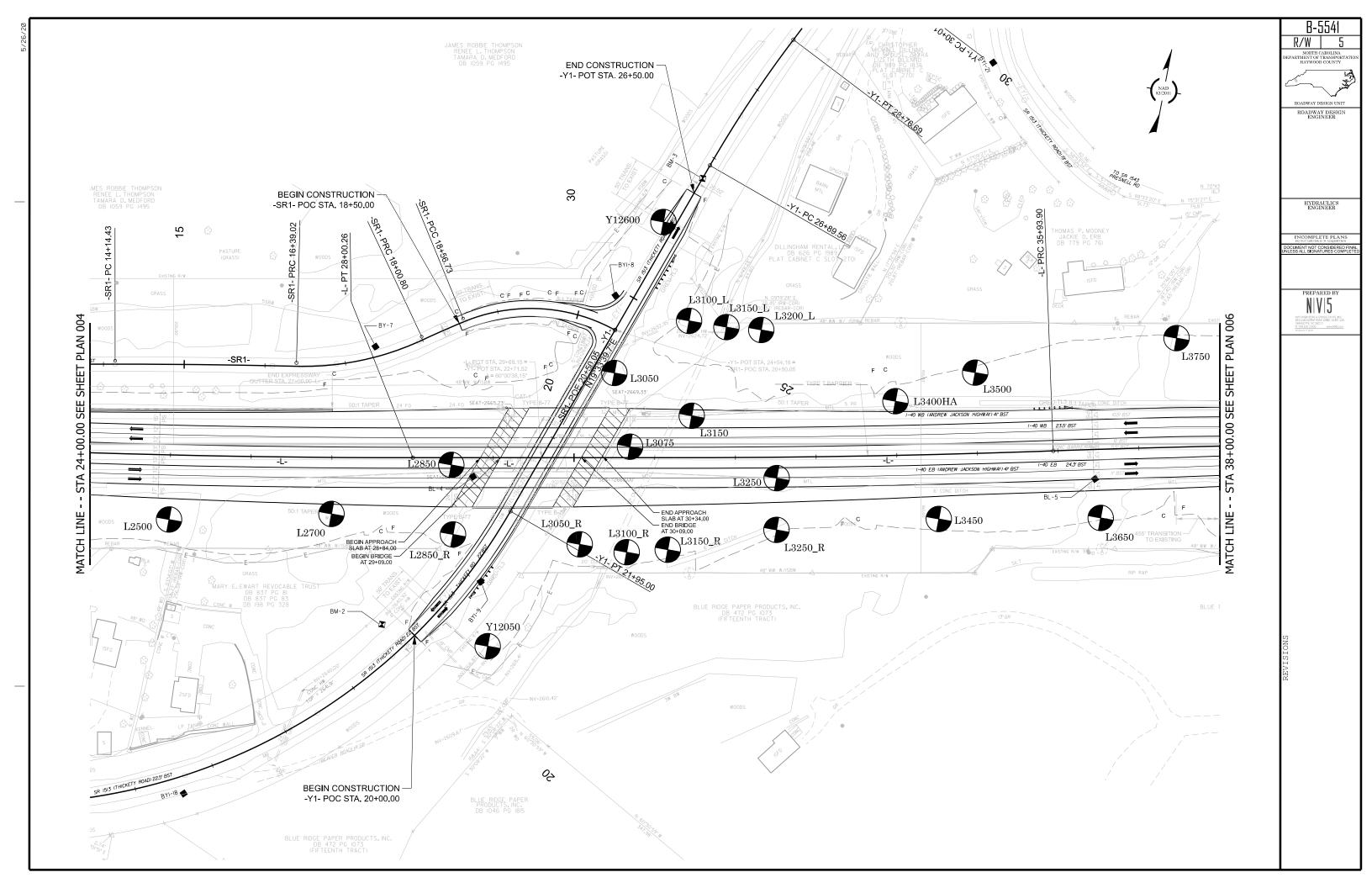
B. Fill soils were encountered in the following sections.

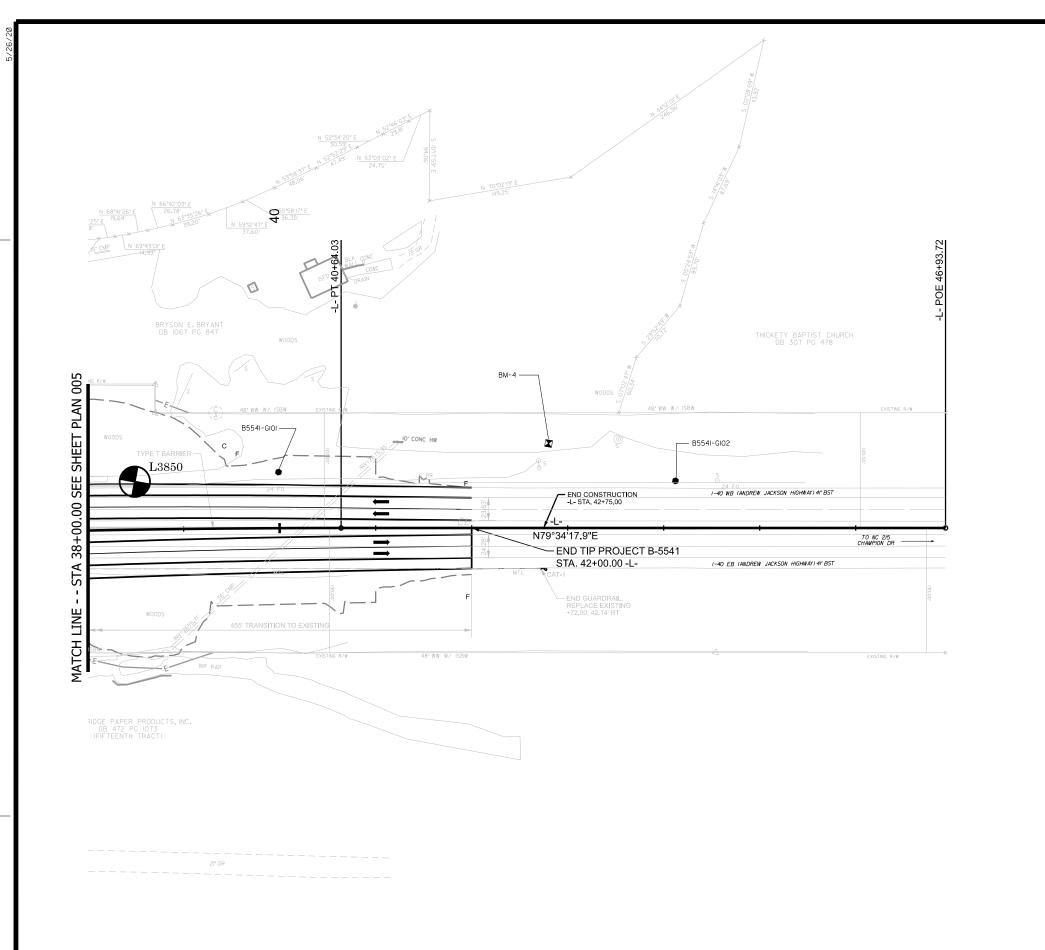
Alignment	Begin Station	End Station
-L-	28+25	31+00

Dason A. Holland

Jason A. Holland, LG Project Geologist Schnabel Engineering









R/W 6

NORTH CAROLINA
DEPARTMENT OF THANSPORTATION
HAVWOOD COUNTY

ROADWAY DESIGN UNIT
ROADWAY DESIGN UNIT
ENGINEER

HYDRAULICS ENGINEER

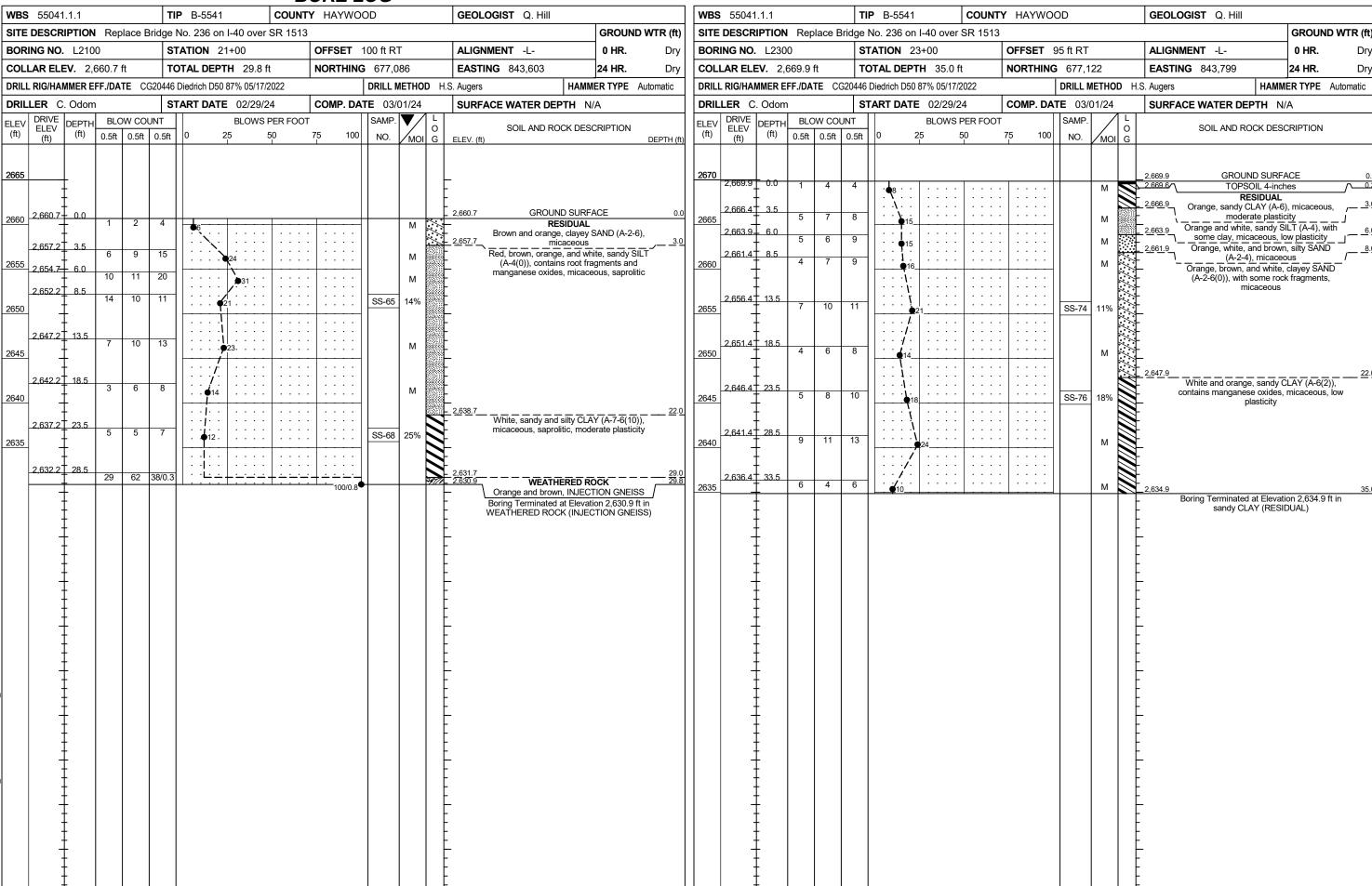
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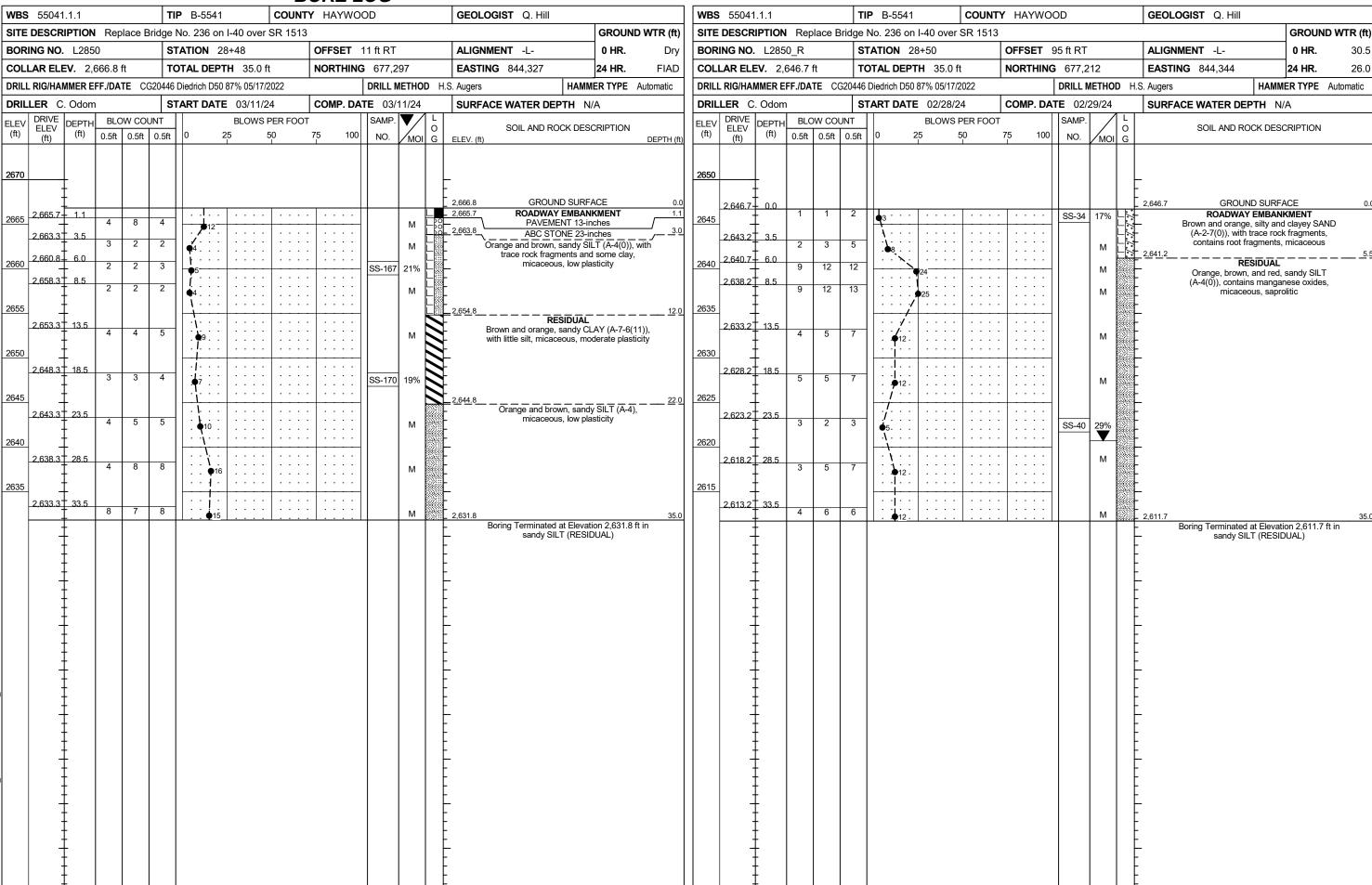
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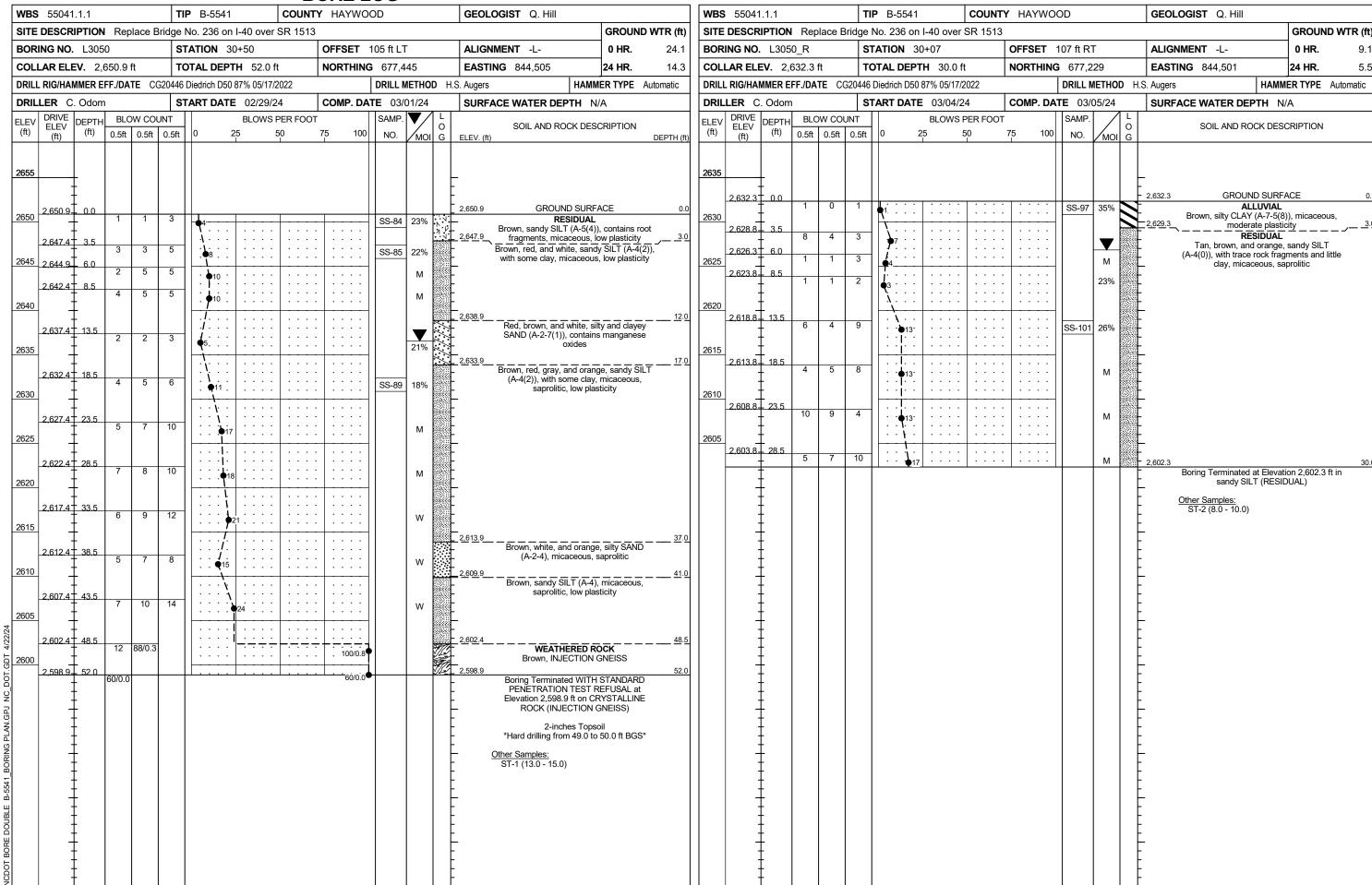
FOR -L- PROFILE SEE SHEET 7, 8, & 9

	<i></i>	BORE LOG																
WBS 55041.1.1		TY HAYWOOD	GEOLOGIST Q. Hill			3S 5504				TIP B-5541		TY HAYWO	OD		GI	EOLOGIST Q. Hill		
	Bridge No. 236 on I-40 over SR 151	1	T	GROUND WTR (ft)						e No. 236 on I-40 ove	r SR 1513						GROUND	
BORING NO. L1700HA	STATION 17+00	OFFSET 45 ft LT	ALIGNMENT -L-	0 HR . Dry	l	RING NO				STATION 19+00		OFFSET 7				LIGNMENT -L-	0 HR.	Dry
COLLAR ELEV. 2,636.9 ft	TOTAL DEPTH 10.0 ft	NORTHING 677,159	EASTING 843,185	24 HR. FIAD		LLAR EL				TOTAL DEPTH 15.0		NORTHING				ASTING 843,403	24 HR.	Dry
DRILL RIG/HAMMER EFF./DATE N/		DRILL METHOD H	, ,	MER TYPE N/A						16 Diedrich D50 87% 05/17		, 	DRILL M			-	HAMMER TYPE AL	utomatic
DRILLER Q. HIII ELEV DRIVE DEPTH BLOW CO	START DATE 03/14/24 UNT BLOWS PER FOO	COMP. DATE 03/14/24 OT SAMP. ▼	SURFACE WATER DEPTH N		ELE	DRIVE	J. Odori	I BLO	W COUNT	START DATE 02/29	PER FOO	COMP. DA	SAMP.	71/24	St	URFACE WATER DEP		
ELEV Cft) DRIVE ELEV (ft) DEPTH BLOW COME (ft) 0.5ft 0.5ft		75 100 NO. MOI G	SOIL AND ROCK DES	CRIPTION DEPTH (ft)	(ft)		(ft)	0.5ft	0.5ft 0.5ft		50	75 100	1 1	моі	O G	SOIL AND ROC	CK DESCRIPTION	
2640			_		263	35	<u> </u> 								-			
			- - _ 2,636.9 GROUND SURF.	ACE 0.0	263	2,630.3	0.0	1	1 2	4 3			SS-57	22%	- - 2,63	RES) SURFACE IDUAL ayey SAND (A-2-6(0)),	0
2635			TOPSOIL 4-INCI RESIDUAL Medium stiff, brown and ora	HES 0.3	262	2,626.8 2,624.3	+	5	4 5	9				M	2,62 - -	27.3 with some rock fra Tan and white, sandy low p	agments, micaceous	<u>, 3</u>
-		S-15 21%	(A-7-5(7)), micaceous, k	ow plasticity	262	2,621.8	I	14 68	29 41 32/0.3		.	70		М	2,62	22.3 WEATHE Tan, and orange.	RED ROCK INJECTION GNEISS	8
						2,616.8	13.5	3	3 7		.			M	2,61	18.3 RES Tan, orange, and w	IDUAL hite, sandy SILT (A-4),	<u>12</u>),
2630		. 2		gments and little		-			0 7		.			IVI	2,61 - - - -		is, saprolitic t Elevation 2,615.3 ft i (RESIDUAL)	15
		C 46 450/	-				<u> </u>								-	2-inche	es Topsoil	
			Boring Terminated at Elevat sandy SILT (RESII	10.0 tion 2,626.9 ft in DUAL)			† †								-			
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NODOI BORE DOUBLE B-5541_BURING PLANGFU NC_DOI.GDI 4/22/24			-				‡								-			
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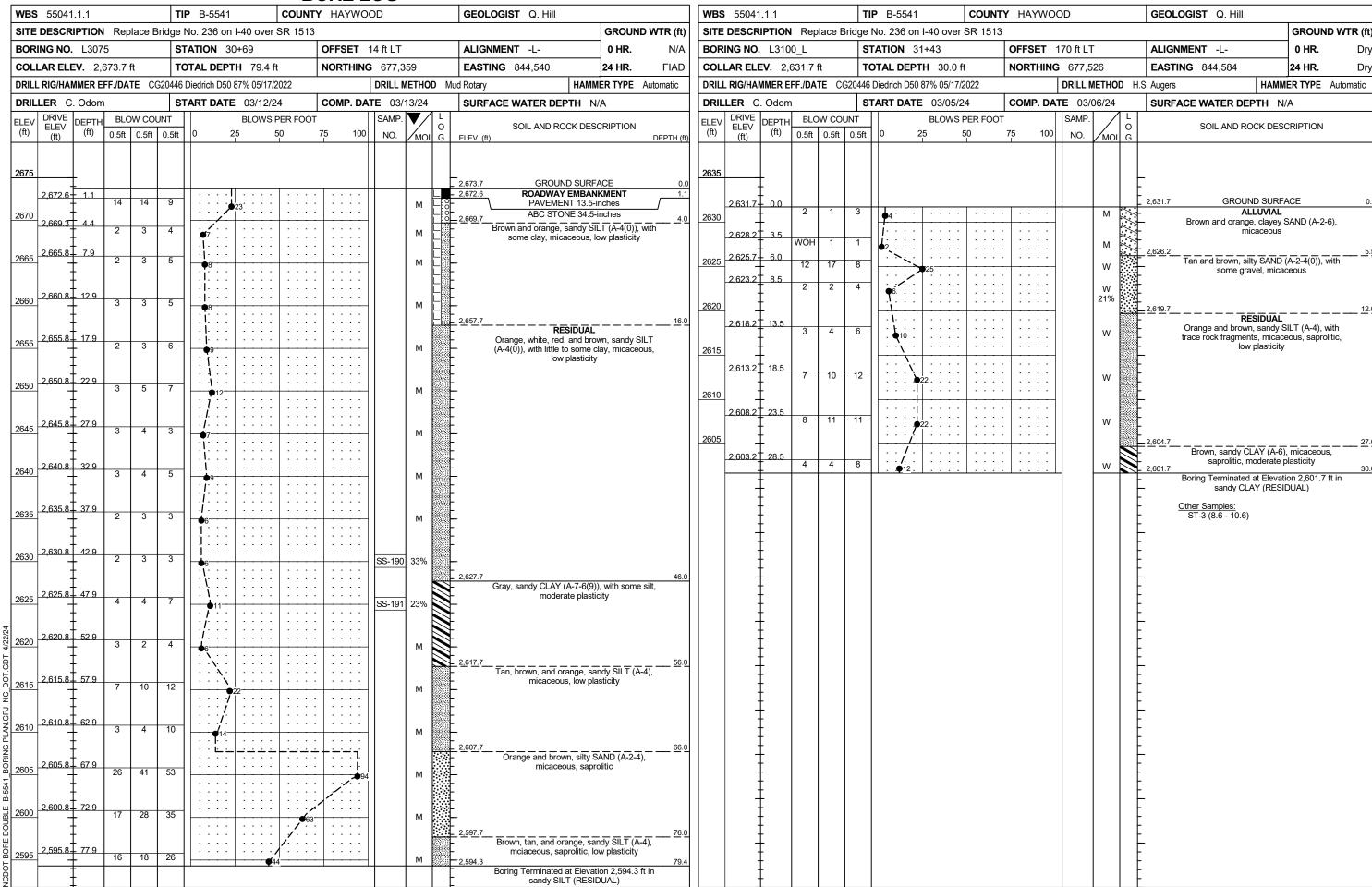
		BORE LOG	1									
WBS 55041.1.1		NTY HAYWOOD	GEOLOGIST Q. Hill		WBS 55041.		TIP B		TY HAYWOO	OD	GEOLOGIST Q. Hill	
SITE DESCRIPTION Replace Br	Ť		T	GROUND WTR (ft)		·		236 on I-40 over SR 151			T	GROUND WTR (ft)
BORING NO. L2500	STATION 25+00	OFFSET 80 ft RT	ALIGNMENT -L-	0 HR. Dry	BORING NO.			ION 27+00	OFFSET 7		ALIGNMENT -L-	0 HR. Dry
			<u> </u>									
		 										
DDIV/E	<u> </u>		SURFACE WATER DEPTH N	I/A						1 1 1	SURFACE WATER DEP	TH N/A
COLLAR ELEV. 2,668.3 ft DRILL RIG/HAMMER EFF./DATE CG DRILLER C. Odom ELEV (ft)	START DATE 02/28/24 NT BLOWS PER FO	75 100 NO. MOI G M M M M M M M M M M M M M M M M M M	SURFACE WATER DEPTH N	ECRIPTION DEPTH (ft) FACE and tan, sandy s, saprolitic, low sticity 25.0	COLLAR ELE DRILL RIG/HAN DRILLER C. ELEV CRIVE ELEV CRIVE 2675 2,674 2 2670 2,665 7 2660 2,665 7 2650 2,650 7 2650	MER EFF./DATE	CG20446 Diedri START COUNT .5ft 0.5ft 0 2 3 5 6 8 7 12 7 8	L DEPTH 25.0 ft Irich D50 87% 05/17/2022 T DATE 02/28/24 BLOWS PER FOO 25 50 65	75 100	DRILL METHOD TE 02/29/24 SAMP.	SURFACE WATER DEP SOIL AND ROG G SOIL AND ROG RES Orange and brow micaceous, m Red, brown, orange (A-4(0)), with trace to clay, contains r micaceo	PAMMER TYPE Automatic TH N/A CK DESCRIPTION D SURFACE SIDUAL In, sandy CLAY (A-6), loaderate plasticity In and white, sandy SILT orack fragments and little nanganese oxides, us, saprolitic at Elevation 2,649.2 ft in (RESIDUAL)
NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24												

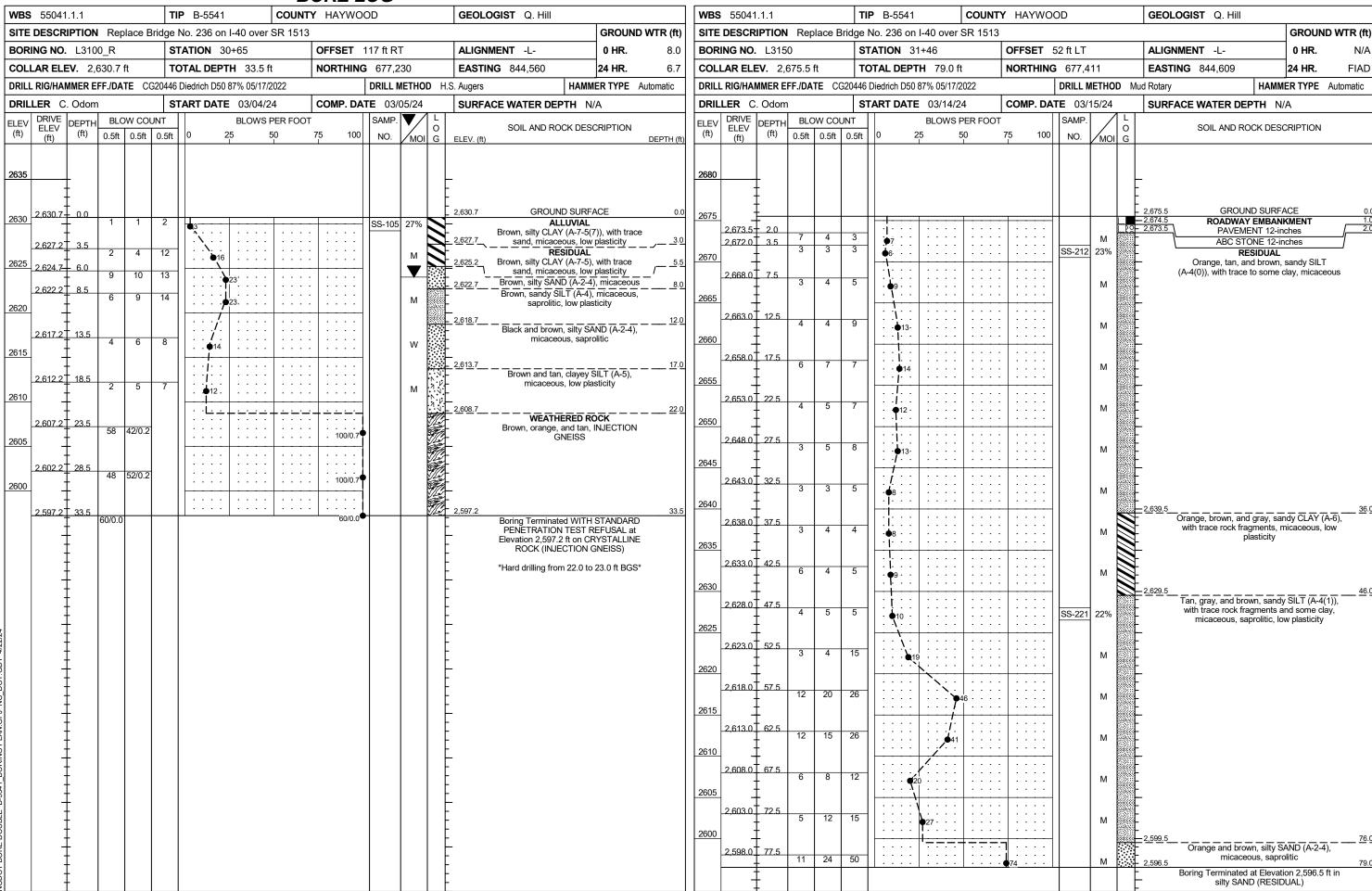


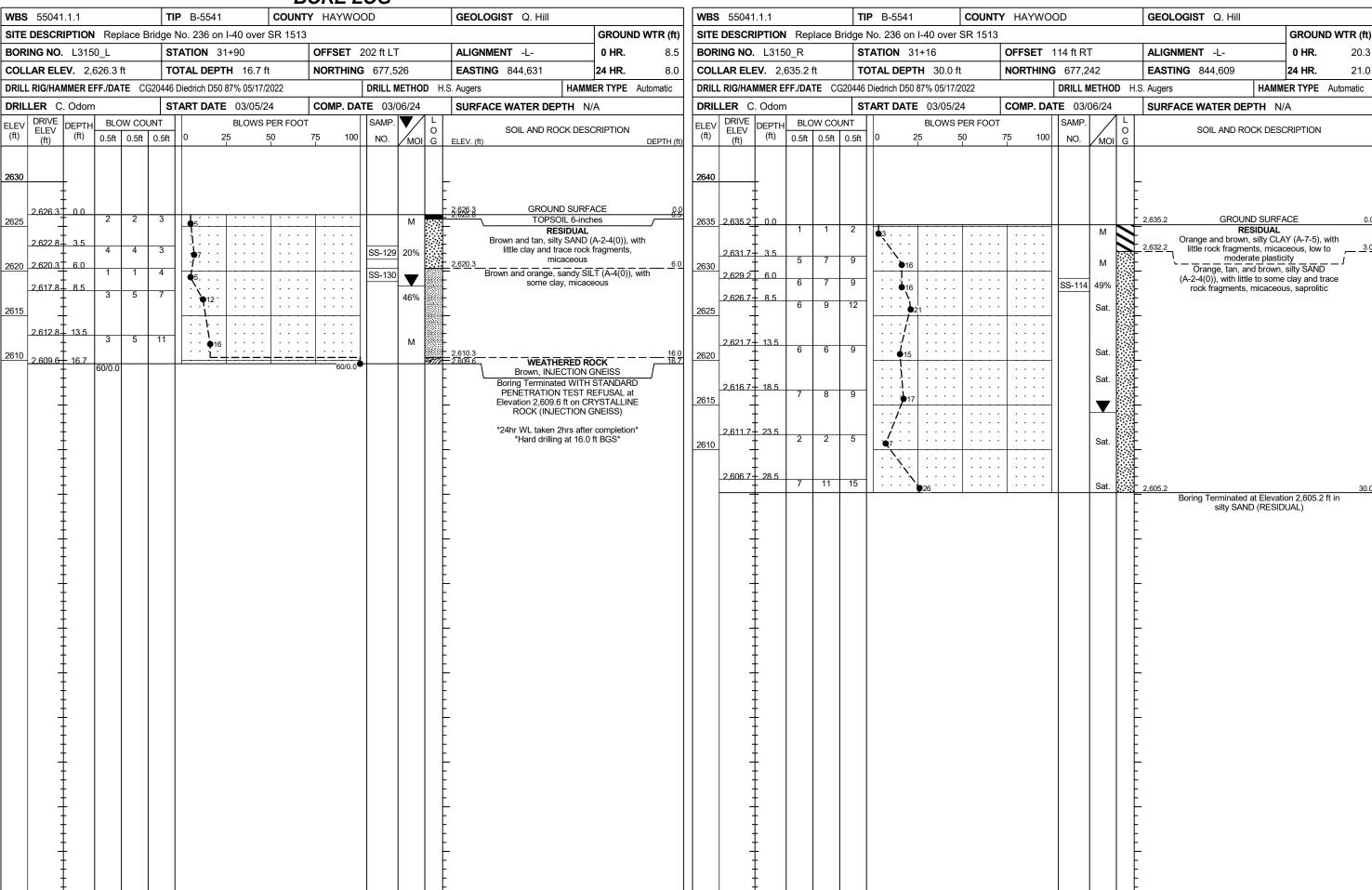


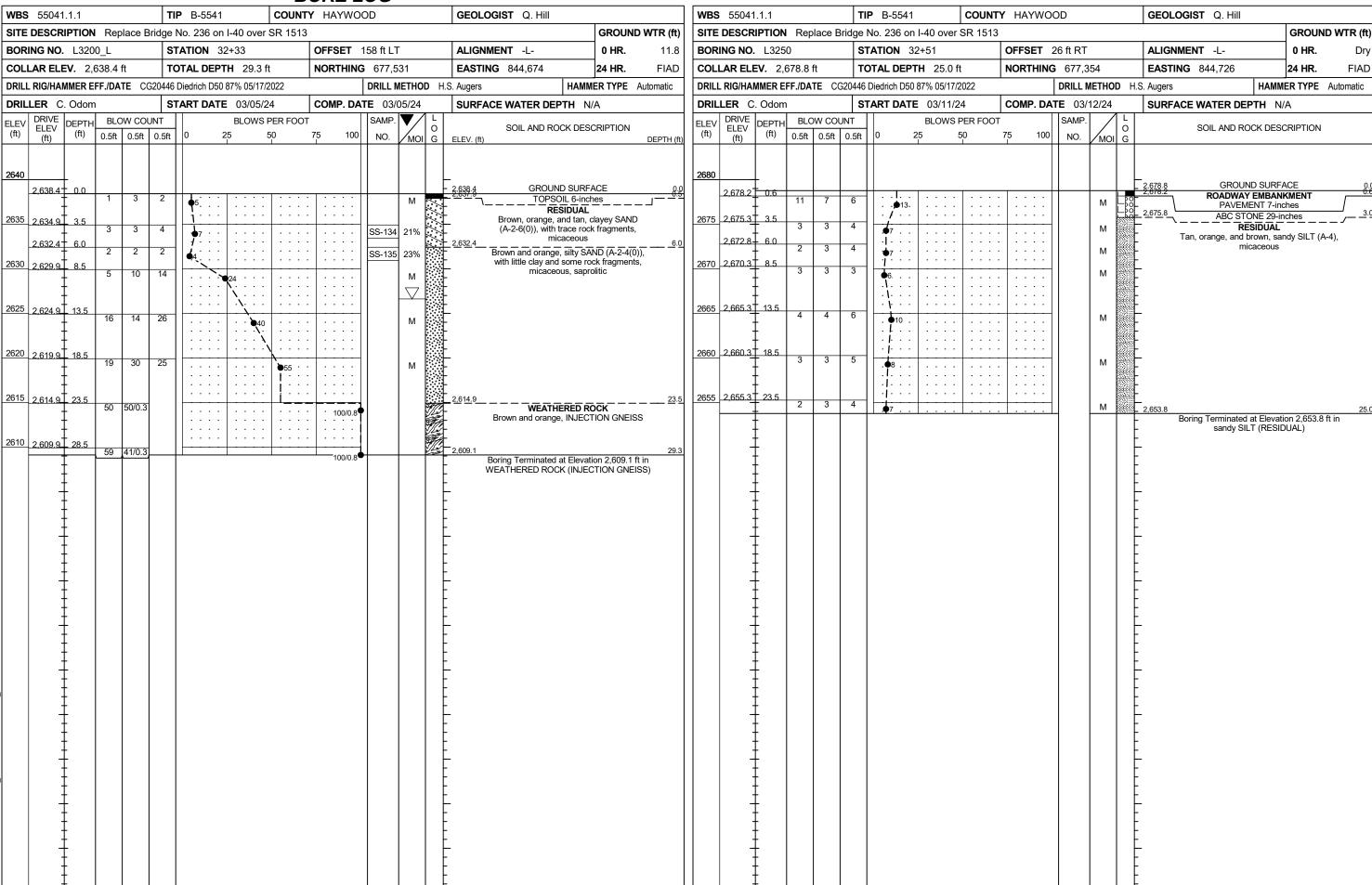
Dry

Dry





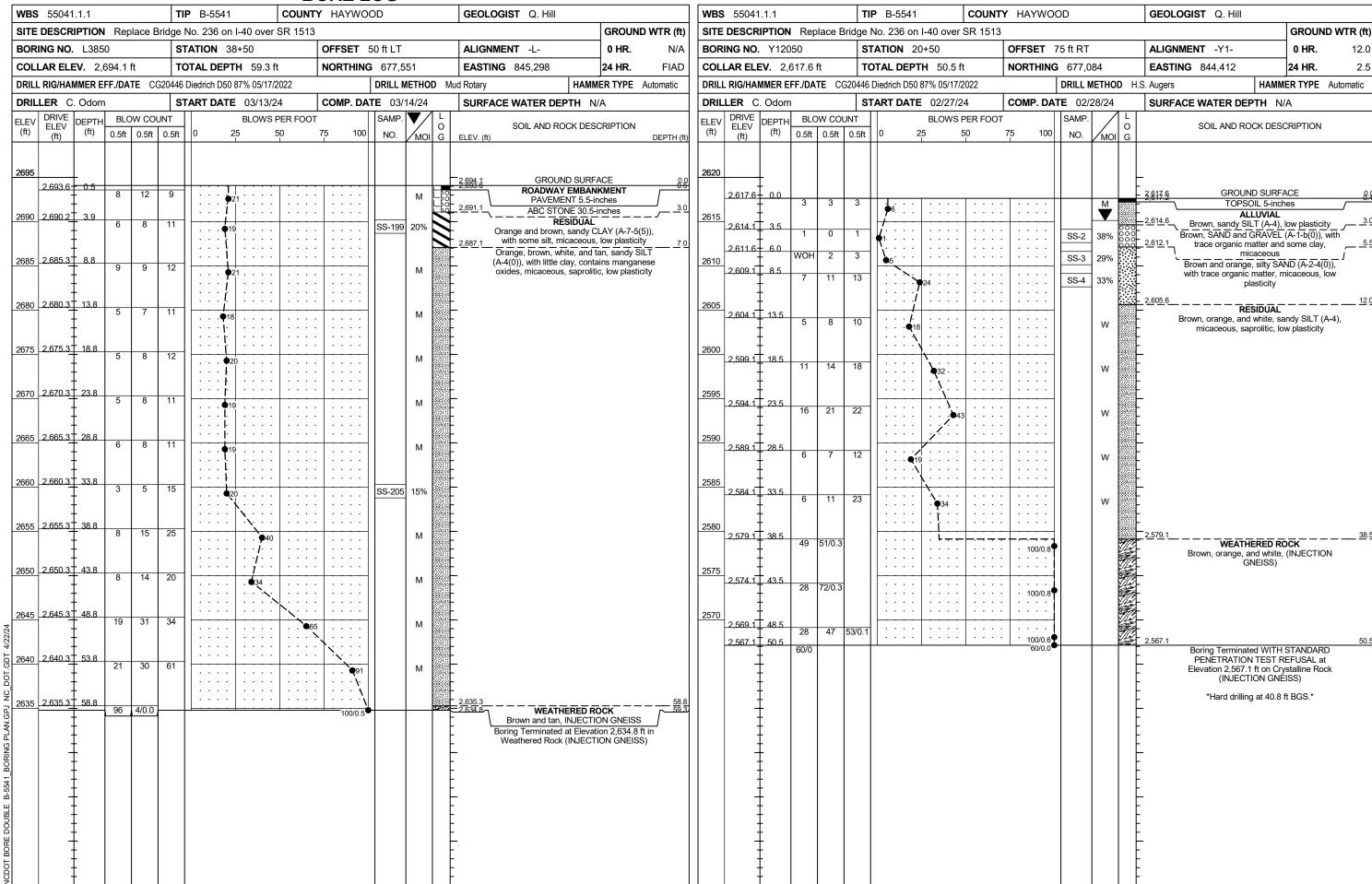




	BORE LOG	T				1
	TY HAYWOOD	GEOLOGIST Q. Hill	WBS 55041.1.1		ITY HAYWOOD	GEOLOGIST Q. Hill
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 151		GROUND WTR (ft)	SITE DESCRIPTION Replace Br	_ <u>ř</u>		GROUND WTR
BORING NO. L3250_R STATION 32+50	OFFSET 90 ft RT	ALIGNMENT -L- 0 HR. Dry	BORING NO. L3400HA	STATION 34+00	OFFSET 67 ft LT	ALIGNMENT -L- 0 HR. D
COLLAR ELEV. 2,660.7 ft TOTAL DEPTH 30.0 ft	NORTHING 677,291	EASTING 844,738 24 HR. Dry	COLLAR ELEV. 2,682.0 ft	TOTAL DEPTH 1.0 ft	NORTHING 677,474	EASTING 844,854 24 HR. FIA
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022	DRILL METHOD H.S		DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD	-
DRILLER C. Odom START DATE 02/28/24	COMP. DATE 02/29/24	SURFACE WATER DEPTH N/A	DRILLER Q. Hill	START DATE 03/14/24	COMP. DATE 03/14/24	SURFACE WATER DEPTH N/A
ELEV (ft)	75 100 100 / 0	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV (ft) DRIVE LEV (ft) DEPTH BLOW COUL	I	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION
2665			2685			2,682.0 GROUND SURFACE
2,660.7 + 0.0 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 2 3 2 3 2 3 2 3 3	SS-26 21%	2,660.7 GROUND SURFACE 0.0 RESIDUAL				2,681.0 ROADWAY EMBANKMENT Medium dense, brown, SAND AND GRAVEL
2,657.2 3.5 10 12 18 30	M M	Orange, brown, and white, sandy SILT (A-4)(0)), with trace to little rock fragments, micaceous, saprolitic				- (A-1-b) Boring Terminated by Auger Refusal at Elevation 2,681.0 ft in SAND AND GRAVEL (ROADWAY EMBANKMENT)
2,647.2 13.5 5 6 11 17	· · · · · · · · · · · · · · · · · · ·					
2,642.2 18.5 4 6 12 18	M M					- - - - - -
2635 17 22 35 557.		2,630.7 30.0				- - - - - -
NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24		Boring Terminated at Elevation 2,630.7 ft in sandy SILT (RESIDUAL)				

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		041.1.1				IP B-5541			TY HAYW	OOD			GEOLOGIST Q. Hill	T		S 55041				IP B-554			ry Haywo	OOD			GEOLOGIST Q. Hill	1
				eplace	Ť	No. 236 or		SR 1513		00.5.5			4.10.045.	GROUND WTR (ft)	-					No. 236 o		er SR 1513		400 5 1			44.100/22-22-2	GROUND WTR
		NO. L3				TATION 3			OFFSET				ALIGNMENT -L-	0 HR. Dry	-	RING NO.				STATION (OFFSET				ALIGNMENT -L-	0 HR.
		ELEV.				OTAL DEP			NORTHII		-		EASTING 844,933	24 HR. Dry		LLAR ELE				OTAL DEP			NORTHIN				EASTING 844,944	24 HR. D
				DAIL		Diedrich D50			1					MER TYPE Automatic						6 Diedrich D5							H.S. Augers	HAMMER TYPE Automatic
		C. Od		1 000/ 0/		TART DAT			COMP. D		2/29/24 D. V		SURFACE WATER DEPTH	N/A		ILLER D. DRIVE				TART DAT			COMP. DA	SAMP		4 71 L	SURFACE WATER DE	PTH N/A
ELE (ft)	V ⊏ □	EV DL;	…⊢	tow co		0		PER FOO	75 10		17	0	SOIL AND ROCK DES	SCRIPTION DEPTH (ft)	ELE\ (ft)	* ELEV/	DEPTH_ (ft)		COUNT 0.5ft 0.5ft	0	25 	S PER FOOT	75 100		1 /	O OI G		OCK DESCRIPTION
269		2.7 - 0.0												FACE 0.0	2700	0 2,699.5	- 0.0	2	2 2	4					M	- NO 0	RE	ID SURFACE
269		9.2 3.5	3		8	: : •13'.					М		RESIDUAL Brown, black, and orange (A-7-5(4)), with trace rock free silt, contains manganese ox	e, sandy CLAY	2695		-	2	3 5	8				SS-142	2 20%	× . ` ·	(A-2-7(1	n, silty and clayey SAND)), micaceous
268	5	6.7 - 6.0 4.2 - 8.5	5	6		15	7				M M M		saprolitic, low pla	asticity	2690	2,693.5 2,691.0			12 11 10 10		• 23			-	M M		trace to little cla	sandy SILT (A-4(0)), with y and rock fragments, caceous
268	2,67	9.2 13.	5 6	9	10	-								gments and little	2685	2,686.0	13.5	11	13 15		28			SS-145	5 12%	6	- - - - -	
267	5 2,67	4.2 18.	5 10	11	13	-	24				М		- - - -		2680	2,681.0	_ 18.5 - -	12 2	27 26			• 53 • 53		-	М		_ _ _ -	
267	2,66	9.2 23.	5 7	15	15		30				М		- - - - 2,667.7	25.0	2675	2,676.0	23.5	16 2	20 27				I	_	М		- - - - -	
		+											- Boring Terminated at Eleva - sandy SILT (RESI - <u>Other Samples:</u>	ation 2,667.7 ft in IDUAL)	2670	2,671.0	28.5 -	16 2	27 41			: :	68	-	M		- - - 2,669.5 - Boring Terminated	at Elevation 2,669.5 ft in
NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDI 4/22/24		+++++++++++++++++++++++++++++++++++++++											BS-1 (0.0 - 10.0)														sandy SIL	T (RESIDUAL)

WD0 55044.4.4		TY HAVANCOR	0501 0010T 0 1177	MIDD FFOALA
WBS 55041.1.1		ry haywood	GEOLOGIST Q. Hill	WBS 55041.1.1 TIP B-5541 COUNTY HAYWOOD GEOLOGIST Q. Hill
<u> </u>	ridge No. 236 on I-40 over SR 1513	OFFSET 85 ft RT	GROUND WTR (ft)	SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513 BORING NO. L3750 STATION 37+50 OFFSET 135 ft LT ALIGNMENT -L- 0 HR. 57
BORING NO. L3650 COLLAR ELEV. 2,694.8 ft	STATION 36+50	NORTHING 677,378	ALIGNMENT -L- 0 HR. Dry	
DRILL RIG/HAMMER EFF./DATE CG	TOTAL DEPTH 20.0 ft	DRILL METHOD H	EASTING 845,130 24 HR. Dry S. Augers HAMMER TYPE Automatic	COLLAR ELEV. 2,724.5 ft TOTAL DEPTH 70.0 ft NORTHING 677,614 EASTING 845,182 24 HR. 50 DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022 DRILL METHOD H.S. Augers HAMMER TYPE Automati
DRILLER C. Odom	START DATE 02/28/24	COMP. DATE 02/29/24	SURFACE WATER DEPTH N/A	·
DDIVE		· · · · · · · · · · · · · · · · · · ·	SURFACE WATER DEPTH N/A	DDW/F
ELEV (ft) (ft) DEPTH BLOW COL		75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV O. SOIL AND ROCK DESCRIPTION SAMP. O. SOIL AND ROCK DESCRIPTION O. SOIL AND ROCK DESC
2695 0.0 1 2	2 4	· · · · · M	_2,694.8 GROUND SURFACE 0.0 RESIDUAL	2725
2,691.3 3.5 6 7	9		Orange and brown, sandy CLAY (A-7-5), 2,691.8 contains manganese oxides, moderate plasticity Orange, white, and brown, sandy SILT	2720 3.5 3 5 8 Orange and brown, silty and clayey SAND (A-2-7(0)), micaceous
2,688.8 6.0 2 5 2,686.3 8.5	7		Orange, white, and brown, sandy SILT (A-4(0)), with little clay, contains manganese oxides, micaceous, saprolitic	2,718.5 6.0
2685 4 7	9 16	SS-16 23%	· - -	2715 2,710.0 0.3 17 14 17 M M clay, micaceous
2680 2,681.3 13.5 4 7	8 15	M	- - -	2710 2,711.0 13.5 18 14 16 30 M
2,676.3 18.5 5 7	11 18	· · · · · · · · · · · · · · · · · · ·		2,706.0 18.5 38 25 26
			Boring Terminated at Elevation 2,674.8 ft in sandy SILT (RESIDUAL)	2,701.0 23.5
			<u>-</u> :	2700
			-	2695 2,696.0 28.5 16 9 12 M
			: -	2690 2.691.0 33.5 15 16 18 SS-157 14% SS-157 14%
			· ·	2,686.0 38.5
			- :	
			· - -	2680 2,681.0 43.5 3 9 8 / M M
			- -	2675 2676.0 48.5 Brown, orange, tan, and white, sandy SILT (A-4), with little clay and rock fragments, micaceous, saprolitic, low plasticity
, ‡				2,671.0 53.5 7 14 20
			- - -	
			- - -	2665 13 17 19 36 1 1 1 M M
			· -	2660 2,661.0 63.5 16 27 45
			: -	2.656.0 68.5 18 26 74 W
				Boring Terminated at Elevation 2,654.5 ft in sandy SILT (RESIDUAL)
			- :	
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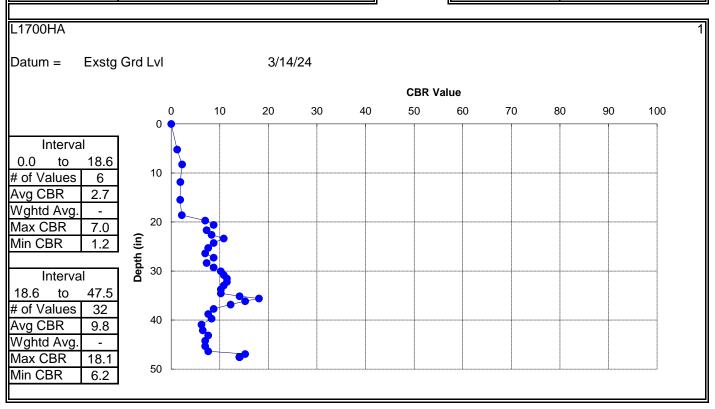


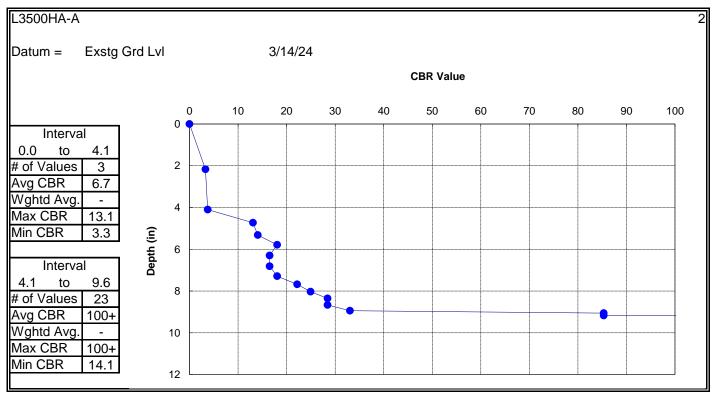
										<u> </u>						
WBS 55041					P B-55			COUNT		YWO	DD			GEOLOGIST Q. Hill		
SITE DESCR	IPTION	Rep	lace B	ridge I	No. 236	on I-4	0 over	SR 1513							GROUN	ID WTR (ft)
BORING NO	. Y126	00		ST	TATION	26+0	00		OFF	SET 1	5 ft LT			ALIGNMENT -Y1-	0 HR.	Dry
COLLAR ELI	EV . 2,6	553.5 f	t	TC	OTAL DE	PTH	15.0 ft	t	NOR	THING	677,6	40		EASTING 844,531	24 HR.	Dry
DRILL RIG/HA	MMER EI	FF./DA	re co	320446	Diedrich D	50 87%	6 05/17/2	2022	•		DRILL N	1ETHO	D H.S	. Augers HAMME	R TYPE	Automatic
DRILLER C	. Odom			ST	TART DA	TE (02/29/2	4	CON	IP. DA	Γ E 03/0			SURFACE WATER DEPTH N/A	A	
ELEV DRIVE ELEV (ft)	DEPTH (ft)		W COL		0		BLOWS F	PER FOO ⁻		100	SAMP. NO.		L O G	SOIL AND ROCK DESC ELEV. (ft)		DEPTH (ft)
655 2,653.5 650 2,650.0 2,647.5 645 2,645.0	3.5	7 10 21	2 16 14 25	3 23 30 35	5		39		.		SS-79	21% M M M		2,653.5 GROUND SURFAL RESIDUAL Orange and brown, silty SAN micaceous Red and brown, clayey SAND little rock fragments, mic 2,645.5 Brown, sandy SILT (A-4), w fragments, micaceous, love	(A-2-4(0)) (A-2-6(0)) (A-2-6(0)) (A-2-6(0))	3.9 , with 8.0 ock
2,640.0	13.5	7	9	9		18)/. 					М		2,638.5 Boring Terminated at Elevatio	ın 2 638 5	15.0
														2-inches Topsoi Other Samples: BS-3 (0.0 - 10.0)	JAL)	

SHEET 20

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

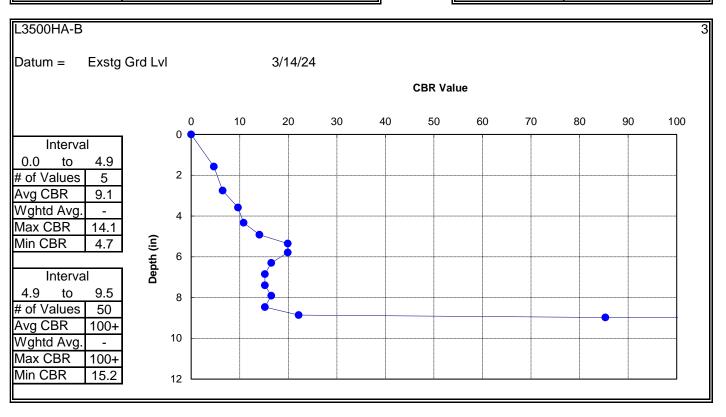
GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A





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	2-Lane Extension of SR 1630
	Replace Bridge No. 236 on I-40
	Haywood

GEOLOGIST	Quinton Hill
	C. Odom
GEOTECH(S)	Z. Taylor
	N/A



REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY SOIL TEST RESULTS -L-															
														SAMPLE	STATION
NO.				CLASS.			C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
S-15	17+00	45' LT	1.5-2.5	A-7-5 (7)	43	13	17.2	29	24.4	29.5	99	87.9	59.7	21	-
S-16	17+00	45' LT	6.5-7.5	A-4 (0)	NP	NP	24.6	28.7	27.2	19.5	99.8	81.2	53.3	15	-
SS-57	19+00	75' RT	0.0-1.5	A-2-6 (0)	40	11	47.9	21.4	1.9	28.8	74.3	43.3	26.4	22	-
SS-65	21+00	100' RT	8.5-10.0	A-4 (0)	NP	NP	19.2	36.2	30.1	14.6	95.8	85.2	51.1	14	-
SS-68	21+00	100' RT	23.5-25.0	A-7-6 (10)	44	19	19.9	19.5	29.9	30.7	94.2	85	55.3	25	-
SS-74	23+00	95' RT	13.5-15.0	A-2-6 (0)	40	15	52.2	24.9	2.8	20.1	69.5	36.8	21	11	-
SS-76	23+00	95' RT	23.5-25.0	A-6 (2)	39	14	36.9	20.8	12	30.2	84.8	56.7	41.1	18	-
BS-2	25+00	80' RT	0.0-10.0	A-6 (5)	40	16	30.6	17.5	15.8	36.1	89.3	67.3	50.3	20	-
SS-54	25+00	80' RT	13.5-15.0	A-6 (5)	36	13	25.5	20.7	22	31.8	92.4	72.2	56.1	19	-
SS-47	27+00	70' RT	13.5-15.0	A-4 (0)	NP	NP	33	33	18.5	15.6	98.9	77.5	40	12	-
SS-167	28+50	11' RT	6.0-7.5	A-4 (0)	35	8	14.5	28.9	24.4	24.4	92.3	77.8	48.9	21	-
SS-170	28+50	11' RT	18.5-20.0	A-7-6 (11)	43	20	17.2	25.1	18.1	39.6	99.2	87.3	62.7	19	-
SS-34	28+50_R	95' RT	0.0-1.5	A-2-7 (0)	46	16	40.4	29.6	14.6	15.4	90.1	62.3	22	17	-
SS-40	28+50_R	95' RT	23.5-25.0	A-4 (0)	NP	NP	33.6	30.9	13.1	22.3	94.8	74.4	38.5	29	-
SS-84	30+50	105' LT	0.0-1.5	A-5 (4)	45	10	18.8	33.6	23.6	24.1	99.6	89.6	54.3	23	-
SS-85	30+50	105' LT	3.5-5.0	A-4 (2)	39	7	25.4	30.2	21.7	22.7	97.9	80.8	49.2	22	-
ST-1	30+50	105' LT	13.0-15.0	A-2-7 (1)	41	20	51.2	14.7	4.2	29.9	88.4	48.5	32.5	21	_
SS-89	30+50	105' LT	18.5-20.0	A-4 (2)	34	10	20.9	31.3	15.6	32.1	92.1	80.9	49.1	18	_
SS-97	30+50 R	103 LT	0.0-1.5	A-7-5 (8)	52	16	13.5	38.8	22.2	25.5	99.8	92.8	56.3	35	-
ST-2	30+50_R	107 RT	8.0-10.0	A-4 (0)	NP	NP	33.2	42	10.9	13.9	98.9	80.5	36.4	23	-
SS-101	30+50 R	107 RT	13.5-15.0	A-4 (0)	NP	NP	26.3	35.7	17.4	20.6	97.8	81.6	43.6	26	_
SS-190	30+75	14' LT	42.9-44.4	A-4 (0)	45	11	19.6	32	23.3	23.3	98.2	78.6	46.6	33	-
SS-191	30+75	14' LT	47.9-49.4	A-7-6 (9)	43	17	8.3	35.7	20.3	35.8	99.6	96.5	62.7	23	
ST-3	31+00 L	158' LT	8.6-10.6	A-2-4 (0)	NP	NP	44.4	39.4	5.9	10.4	85.4	57.5	19.1	21	-
SS-105	31+00 R	117' RT	0.0-1.5	A-7-5 (7)	46	15	18.6	26.6	23	31.8	95.2	83.4	58	27	_
SS-212	31+50	52' LT	3.5-5.0	A-4 (0)	NP	NP	17.6	34.9	21.3	26.3	96.9	87.1	53.4	23	_
SS-212	31+50	52' LT	47.5-49.0	A-4 (0)	34	8	28.9	29.8	15.2	26.1	92.3	71	43.5	22	_
SS-114	31+50 R	114' RT	6.0-7.5	A-2-4 (0)	NP	NP	36.3	32.9	8	22.8	89.6	64	33.4	49	
SS-114 SS-129	31+50_K 31+50_L	202' LT	3.5-5.0	\ /	NP	NP	34.2	38.2	13.3	14.4	96.5	79.7	31.6	20	
SS-129 SS-130	31+50_L 31+50 L	202 LT 202' LT	6.0-7.5	A-2-4 (0) A-4 (0)	NP NP	NP NP	23.9	27.8	20.3	28	96.5	77.6	50.2	46	-
		158' LT									94.1	66.7	34.2	21	-
SS-134	32+00_L 32+00_L		3.5-5.0	A-2-6 (0)	40 NP	12 NP	34.5 37.9	33.3 35.1	1.5 16.4	30.6 10.7	98.1	76.1	34.2	23	
SS-135		158' LT	6.0-7.5	A-2-4 (0)										23	-
SS-26	32+50_R	90' RT	0.0-1.5	A-4 (0)	NP	NP	35	27.8	9.2	28	93.1	71.2	39.1		-
SS-23	34+50	80' RT	13.5-15.0	A-4 (0)	NP	NP 45	23.6	30	32.6	13.8	97.4	85.9	44.3	12 19	-
BS-1	34+50	80' RT	0.0-10.0	A-7-5 (4)	52	15	30.4	27	18.3	24.3	96.2	75.7	46		-
SS-142	35+00	100' LT	3.5-5.0	A-2-7 (1)	49 ND	19 ND	37.5	26.8	5.3	30.4	84.9	61.5	33.6	20	-
SS-145	35+00	100' LT	13.5-15.0	A-4 (0)	NP	NP	37.9	28.7	24.2	9.2	92.8	67.4	37.1	12	-
SS-16	36+50	85' RT	8.5-10.0	A-4 (0)	NP	NP	19.8	39.3	26.8	14.1	99.5	88.2	48.3	23	-
SS-151	37+50	135' LT	6.0-7.5	A-4 (0)	NP	NP	19.7	35.4	31.2	13.7	98.5	88.5	50.5	12	-
SS-157	37+50	135' LT	33.5-35.0	A-2-4 (0)	NP	NP	40	39.5	5.6	14.9	96.6	77.4	26.4	14	-
SS-199	38+50	50' LT	3.9-5.4	A-7-5 (5)	41	11	16.4	34.5	24.4	24.8	99.7	89.2	57.3	20	-
SS-205	38+50	50' LT	33.8-35.3	A-4 (0)	NP	NP	24.4	29.8	30.6	15.1	98.7	81.7	51.9	24	-

	REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY														
SOIL TEST RESULTS -Y1-															
SAMPLE	STATION	OFFSET	DEPTH INTERVAL	AASHTO	L.L	P.I.		% BY WEIGHT				SSING (S	IEVES)	%	%
NO.	STATION	OFFSET	DEPININIERVAL	CLASS.	L.L	F.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-2	20+50	75' RT	3.5-5.0	A-1-b (0)	NP	NP	66	9.1	4.5	20.4	54.1	21.8	14.1	38	-
SS-3	20+50	75' RT	6.0-7.5	A-2-4 (0)	NP	NP	38.5	35.7	16.6	9.2	99	79.3	30.8	29	-
SS-4	20+50	75' RT	8.5-10.0	A-2-4 (0)	39	7	40.5	32.5	4	23	86.4	62.2	28.3	33	-
SS-79	26+00	15' LT	0.0-1.5	A-2-4 (0)	35	5	35.9	30.4	8.9	24.9	89.6	66.7	35.2	21	-
BS-3	26+00	15' LT	0.0-10.0	A-2-6 (0)	37	12	48.6	24	5	22.4	64.9	40.3	20.5	14	-