

REFERENCE: B-5541

PROJECT: 55041

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND (SOIL AND ROCK)
3	ROADWAY TITLE SHEET
4-6	PLAN SHEETS
7	PAVEMENT DATA SHEETS
8-13	DCP DATA GRAPHS
14-16	PAVEMENT CORE PHOTOS
17	PAVEMENT CORE EVALUATION

APPENDIX A

TITLE	SHEETS
SOIL TEST RESULTS	18

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY HAYWOOD
PROJECT DESCRIPTION REPLACE BRIDGE NO. 236
ON I-40 OVER SR 1513 IN HAYWOOD COUNTY

PAVEMENT & SUBGRADE INVESTIGATION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5541	1	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

Q. HILL

CG2 EXPLORATION

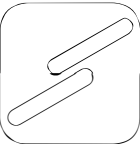
INVESTIGATED BY J. HOLLAND

DRAWN BY J. HOLLAND

CHECKED BY J. CRENSHAW

SUBMITTED BY SCHNABEL ENG.

DATE AUGUST 2024



Schnabel
ENGINEERING



Signed by:

Jason A. Holland

08/20/2024

DF15142D0C83428A

SIGNATURE

DATE

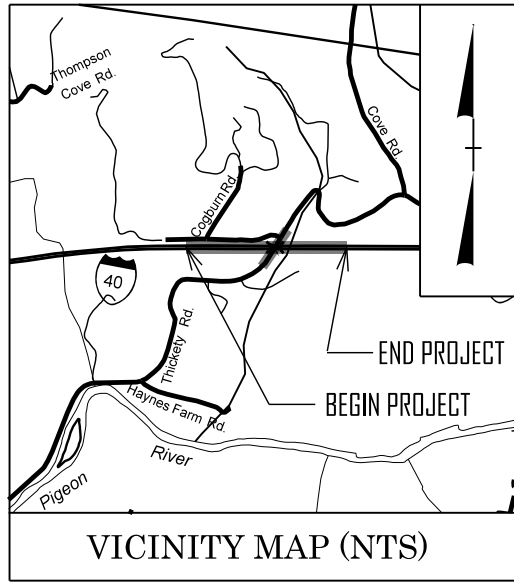
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

09/08/99

TIP PROJECT: B-5541

CONTRACT:

See Sheet 1A For Index of Sheets



STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

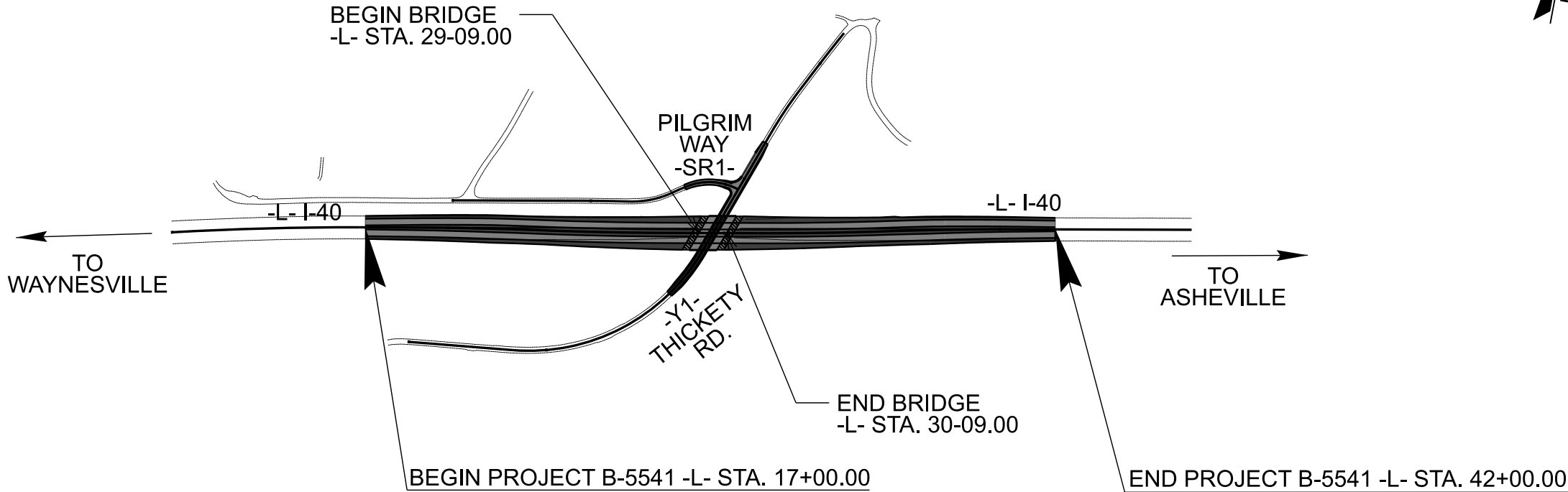
HAYWOOD COUNTY

LOCATION: *BRIDGE NO. 236 OVER SR 1513 ON I-40*

TYPE OF WORK: *GRADING, PAVING, DRAINAGE AND STRUCTURES*

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5541		
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
55041.1.1		P.E.	

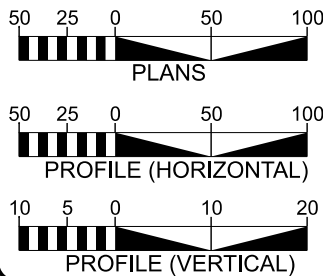
DESIGN RECOMMENDATION PLAN SET



THERE IS FULL CONTROL OF ACCESS ON THIS PROJECT.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ???

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2024 = 60000
ADT 2044 = 77500
K = 8 %
D = 55 %
T = 15 % *
V = 65 MPH
* TTST = 12% DUAL 3%
FUNC CLASS =
INTERSTATE
STATE WIDE TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5541 = 0.471 MILES
LENGTH STRUCTURE TIP PROJECT B-5541 = 0.021 MILES
TOTAL LENGTH TIP PROJECT B-5541 = 0.492 MILES

Prepared in the Office of:
NIVIS
NIVS ENGINEERS & CONSULTANTS, INC.
8514 MCALPINE PARK DRIVE, STE 135
CHARLOTTE, NC 28213
P: 704.537.7300 www.NIVS.com
NC License # F-1333
formerly CALVIN Engineers & Consultants

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
MARCH 15, 2024

LETTING DATE:
JUNE 17, 2025

CHRIS ANDERSON, PE
PROJECT ENGINEER

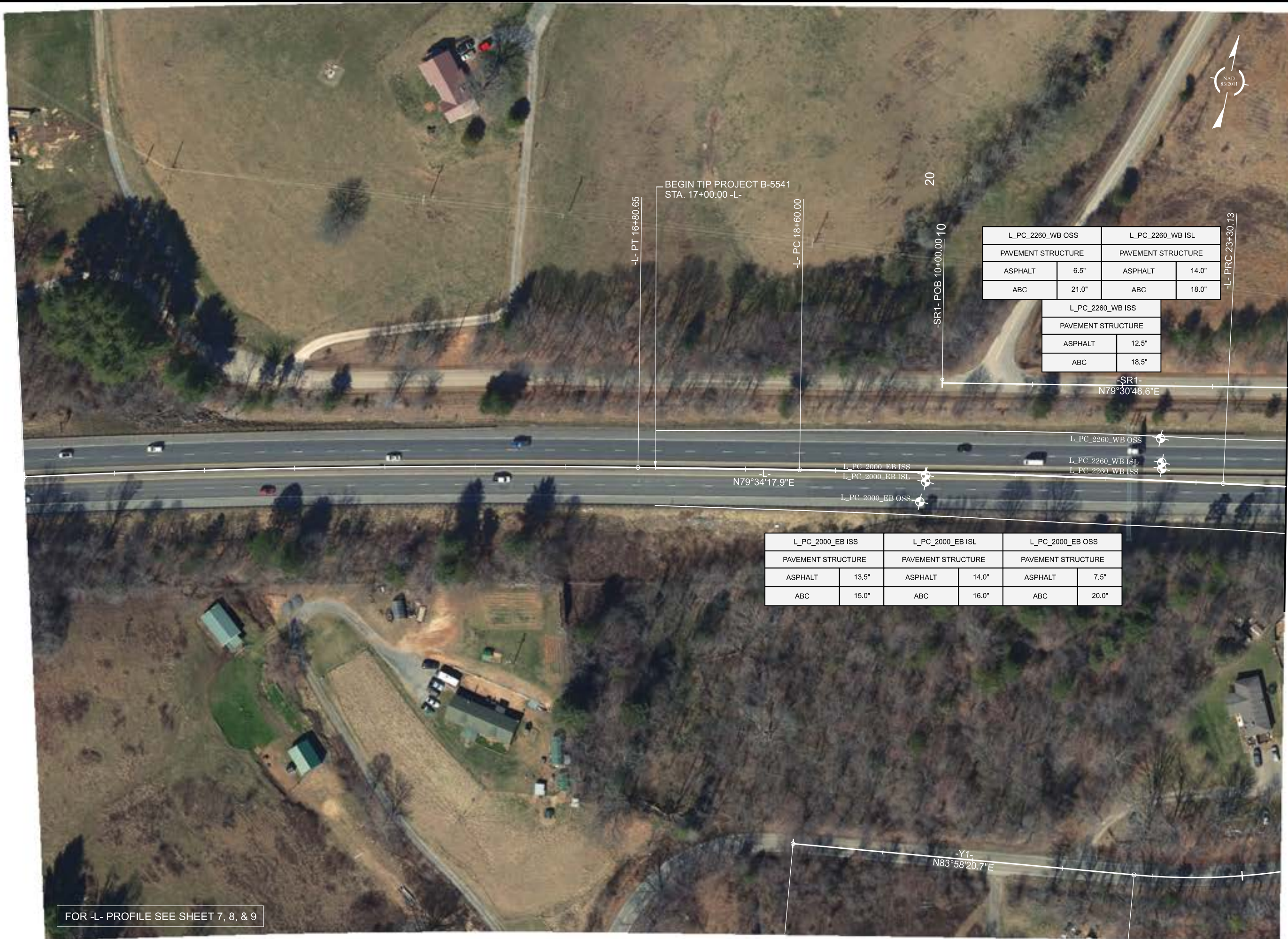
DYLAN MCCANN
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.
ROADWAY DESIGN ENGINEER


SIGNATURE: _____ P.E.





FOR -L- PROFILE SEE SHEET 7, 8, & 9

MATCH LINE - - STA 24+00.00 SEE SHEET PLAN 005

B-5541	
R/W	4
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION HAYWOOD COUNTY	
	
ROADWAY DESIGN UNIT	
ROADWAY DESIGN ENGINEER	

HYDRAULICS
ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

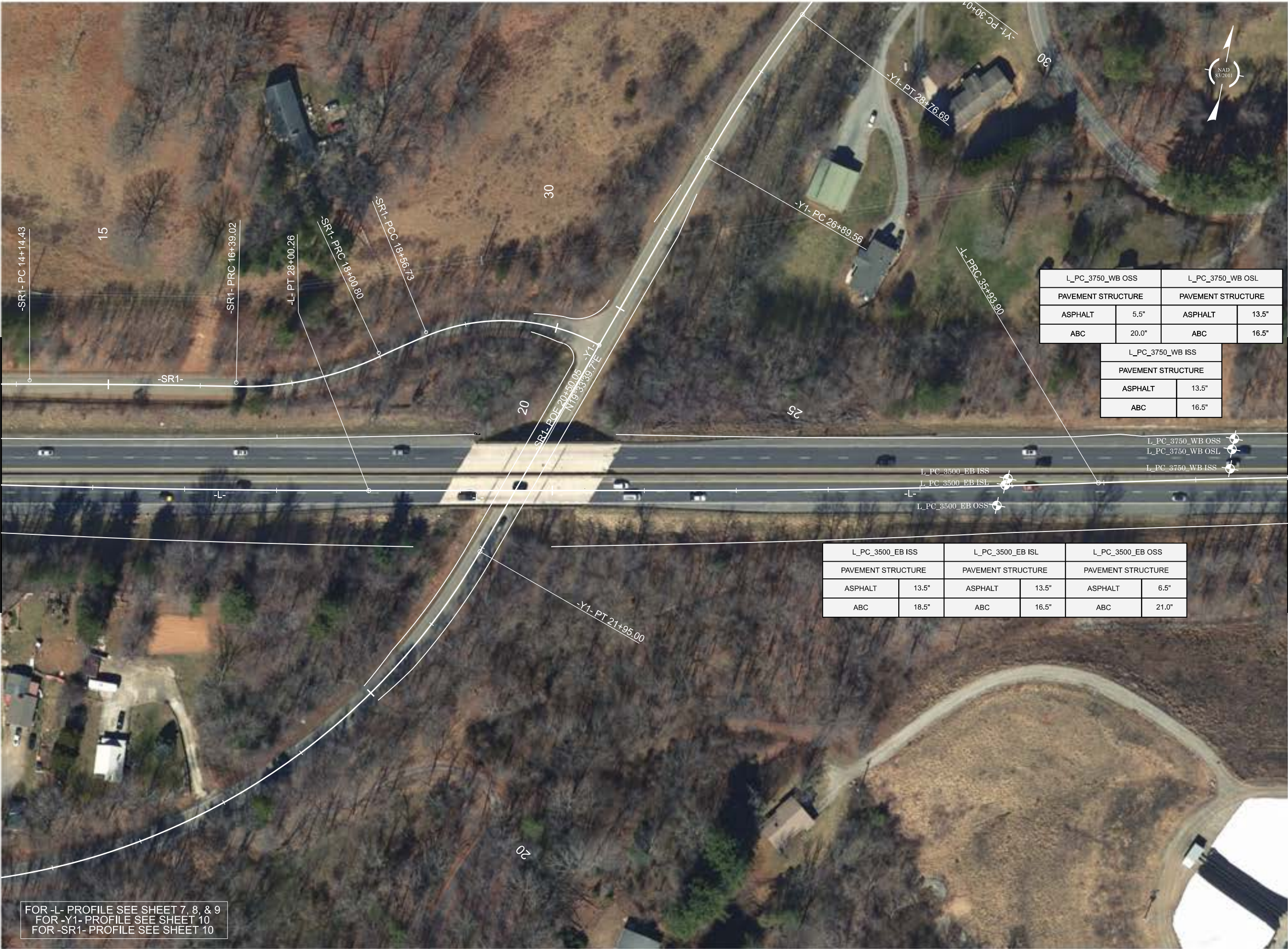
DOCUMENT NOT CONSIDERED FINAL
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CHARLOTTE, NC 28213
P: 704.537.7300 www.NV5.com
NE license #142531

REVIEWS

MATCH LINE - - STA 24+00.00 SEE SHEET PLAN 004

MATCH LINE - - STA 38+00.00 SEE SHEET PLAN 006



L_PC_3750_WB OSS		L_PC_3750_WB OSL	
PAVEMENT STRUCTURE		PAVEMENT STRUCTURE	
ASPHALT	5.5"	ASPHALT	13.5"
ABC	20.0"	ABC	16.5"

L_PC_3750_WB ISS	
PAVEMENT STRUCTURE	
ASPHALT	13.5"
ABC	16.5"

L_PC_3500_EB ISS		L_PC_3500_EB ISL		L_PC_3500_EB OSS	
PAVEMENT STRUCTURE		PAVEMENT STRUCTURE		PAVEMENT STRUCTURE	
ASPHALT	13.5"	ASPHALT	13.5"	ASPHALT	6.5"
ABC	18.5"	ABC	16.5"	ABC	21.0"

FOR -L- PROFILE SEE SHEET 7, 8, & 9
FOR -Y1- PROFILE SEE SHEET 10
FOR -SR1- PROFILE SEE SHEET 10

B-5541

R/W

5

NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

HAYWOOD COUNTY

ROADWAY DESIGN UNIT

ROADWAY DESIGN ENGINEER

HYDRAULICS ENGINEER

INCOMPLETE PLANS

DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

PREPARED BY

NVI5

NVI ENGINEERING & CONSULTANTS, INC.
8514 HICKORY PARK DRIVE, SUITE 110
CHARLOTTE, NC 28211
P: 704.517.7200 F: 704.517.7201
www.nvi5.com

REVISIONS

MATCH LINE - - STA 38+00.00 SEE SHEET PLAN 005



FOR -L- PROFILE SEE SHEET 7, 8, & 9



B-5541	
R/W	6
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION HAYWOOD COUNTY	
ROADWAY DESIGN UNIT ROADWAY DESIGN ENGINEER	
HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
PREPARED BY NYS ENGINEERING & CONSULTANTS, INC. 8514 HICKORY PARK DRIVE, SUITE 110 CHARLOTTE, NC 28211 P: 704.517.7200 www.NYSeng.com NYS REG. # 4011	

REVISIONS

PAVEMENT INVESTIGATION DATA SHEET

Project:	55041.1.1
TIP:	B-5541

Route:	I-40
County:	Haywood

Prepared by:	J. Holland	Date:	4/17/2024
Reviewed by:		Date:	

		Width ³				Pavement Structure / Thickness ³					Subgrade ⁴							State Plane Coordinates	
Position ¹ (Sta._Direction_Lane,Shldr.)	Cut/Fill ² (Est. of Amount)	Lane(s)	Shoulder(s)	Offset Distance	Crown "C" or Super "S"	Pavement Layering	Total to Subgrade (in.)	Asphalt (in.)	Concrete (in.)	ABC (in.)	CTBC (in.)	Description	Sample ID	AASHTO Classification	Soil Moisture	Probe Depth	Asphalt Notes	Northing	Easting
-L- STA 20+00																			
L_PC_2000_EB_ISS	CUT ± 5.0'	ISL 12.0'	ISS 6.0'	1.5' FY	C	Asphalt ABC	28.5	13.5	-	15.0	-	RES: Reddish-brown, silty SAND, little mica (2.4' - 5.0')	-	A-2-4	M	5.0'	No pavement distress observed	677,165	843,489
L_PC_2000_EB_ISL	CUT ± 5.0'	ISL 12.0'	ISS 6.0'	4.0' FY	C	Asphalt ABC	30.0	14.0	-	16.0	-	RES: Brown, orange, silty SAND, little mica (2.5' - 5.0')	S-3	A-2-4(0)	13%	5.0'	Recent overlay. No pavement distress observed	677,158	843,490
L_PC_2000_EB_OSS	CUT ± 5.0'	OSL 12.0'	OSS 11.5'	3.5' FW	C	Asphalt ABC	27.5	7.5	-	20.0	-	RES: Brown, orange, silty SAND, little mica (2.3' - 5.0')	-	A-2-4	M	5.0'	No pavement distress observed	677,135	843,488
-L- STA 22+60																			
L_PC_2260_WB_OSS	CUT ± 12.0'	OSL 12.0'	OSS 11.0'	5.0' FW	C	Asphalt ABC	27.5	6.5	-	21.0	-	RES: White, tan, silty SAND, some mica (2.3' - 5.0')	S-13	A-2-4(0)	18%	5.0'	No pavement distress observed	677,256	843,737
L_PC_2260_WB_ISL	CUT ± 13.0'	ISL 12.5'	ISS 6.0'	4.0' FY	C	Asphalt ABC	32.0	14.0	-	18.0	-	RES: Tan, brown, silty SAND, little mica (2.7' - 5.0')	S-9	A-2-4(0)	8%	5.0'	Recent overlay. No pavement distress observed	677,230	843,743
L_PC_2260_WB_ISS	CUT ± 14.0'	ISL 12.5'	ISS 6.0'	1.5' FY	C	Asphalt ABC	31.0	12.5	-	18.5	-	RES: Tan, white, silty SAND, some mica (2.6' - 5.0')	-	A-2-4	M	5.0'	No pavement distress observed	677,223	843,744
-L- STA 35+00																			
L_PC_3500_EB_ISS	CUT ± 12.0'	ISL 11.5'	ISS 6.0'	1.5' FY	C	Asphalt ABC	32.0	13.5	-	18.5	-	RES: Tan, sandy SILT, trace mica (2.7' - 5.0')	S-6	A-4(0)	10%	5.0'	No pavement distress observed	677,436	844,960
L_PC_3500_EB_ISL	CUT ± 12.0'	ISL 11.5'	ISS 6.0'	4.0' FY	C	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, sandy SILT, trace mica (2.5' - 5.0')	-	A-4	M	5.0'	Recent overlay. No pavement distress observed	677,428	844,958
L_PC_3500_EB_OSS	CUT ± 12.0'	OSL 12.0'	OSS 12.5'	3.5' FW	C	Asphalt ABC	27.5	6.5	-	21.0	-	RES: Brown, orange, silty CLAY, little mica (2.3' - 5.0')	S-2	A-7-6(4)	77%	5.0'	No pavement distress observed	677,405	844,953
-L- STA 37+50																			
L_PC_3750_WB_OSS	CUT ± 15.0'	OSL 12.0'	OSS 11.0'	7.0' FW	C	Asphalt ABC	25.5	5.5	-	20.0	-	RES: White, tan, silty SAND, some mica (2.1' - 5.0')	S-10 S-11	A-2-4(0) A-2-4(0)	4% 13%	5.0'	No pavement distress observed	677,523	845,194
L_PC_3750_WB_OSL	CUT ± 13.0'	OSL 12.0'	OSS 11.0'	2.5' FW	C	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, brown, sandy SILT, little mica (2.5' - 5.0')	-	A-4	M	5.0'	Recent overlay. No pavement distress observed	677,511	845,194
L_PC_3750_WB_ISS	CUT ± 10.0'	ISL 12.0'	ISS 6.0'	0.5' FY	C	Asphalt ABC	30.0	13.5	-	16.5	-	RES: Tan, sandy CLAY, trace mica (2.5' - 5.0')	S-7	A-6(1)	15%	5.0'	No pavement distress observed	677,490	845,195

- Notes:**
- 1. Refer to plan sheets for pavement investigation locations.
 - 2. Cut/Fill are rough estimates based on a review of existing topographic features and are for reference only.
 - 3. Values are based on field measurements at time of investigation.
 - 4. Soil descriptions, classifications, and moisture are based on visual-manual methods, unless otherwise noted.

- Abbreviations:**
- OSL = Outside Lane
ISL = Inside Lane
CL = Center Lane
LTL = Left Turn Lane

CTL = Center Turn Lane
RTL = Right Turn Lane
DECEL = Deceleration Lane
ACCEL = Acceleration Lane

OSS = Outside Shoulder
ISS = Inside Shoulder
GM = Grass Median
OGS = Outside Grass Shoulder

PS = Paved Shoulder
RT LN = Right Lane
LT LN = Left Lane
COL = Collector Lane
D = Dry Soil

RT = Right
LT = Left
(I) = Inside
(O) = Outside
M = Moist Soil

NB = Northbound
SB = Southbound
FW = From White
FY = From Yellow
W = Wet Soil

ABC = Aggregate Base Course
CTBC = Cement-Treated Base Course
RES = Residual Soil
R.E. = Roadway Embankment

CONE PENETROMETER RESULTS

Page 8

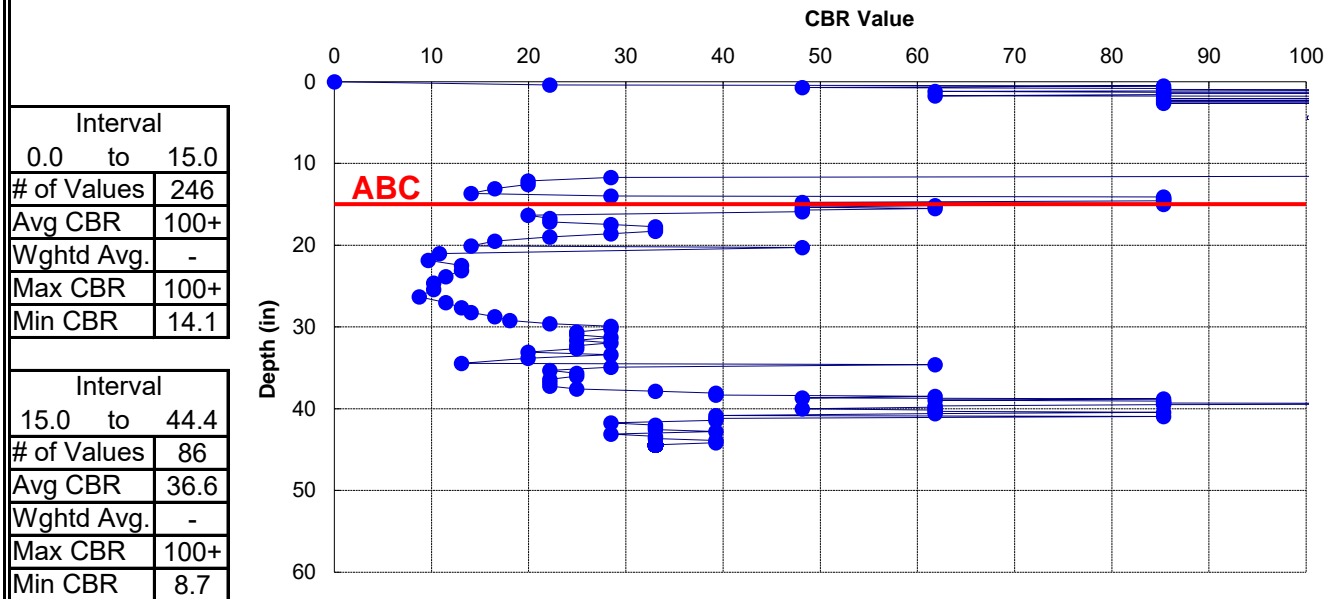
PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_2000_EB ISS

1

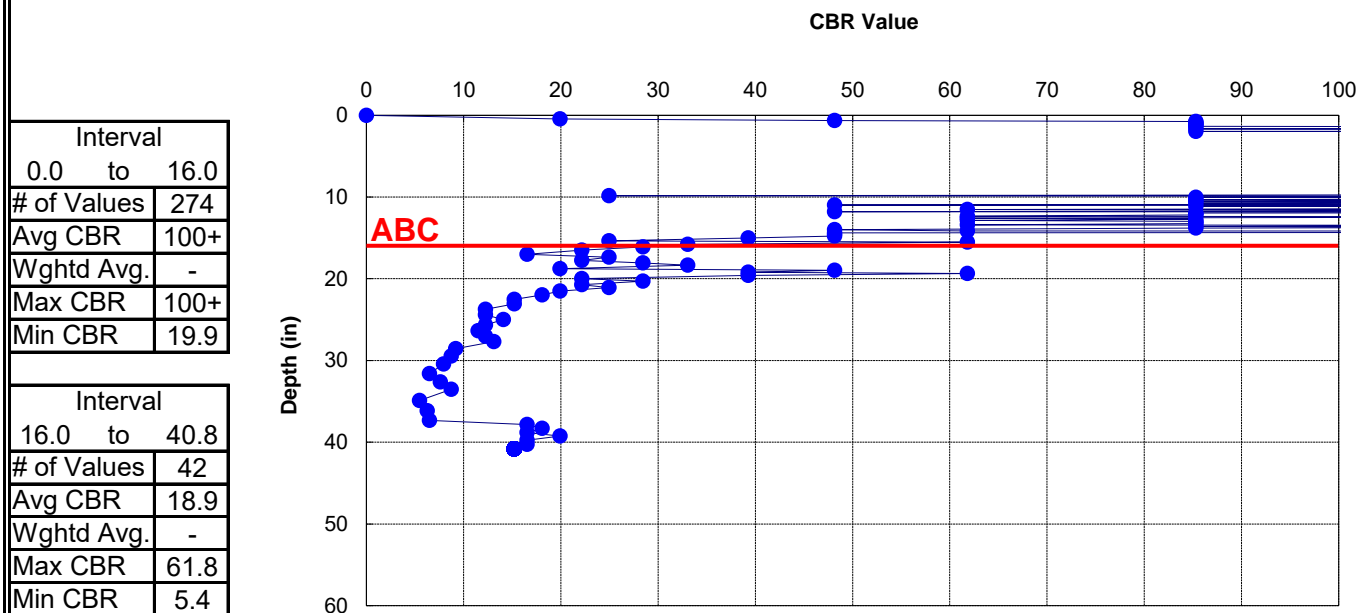
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L_PC_2000_EB ISL

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CONE PENETROMETER RESULTS

Page 9

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_2000_EB OSS

3

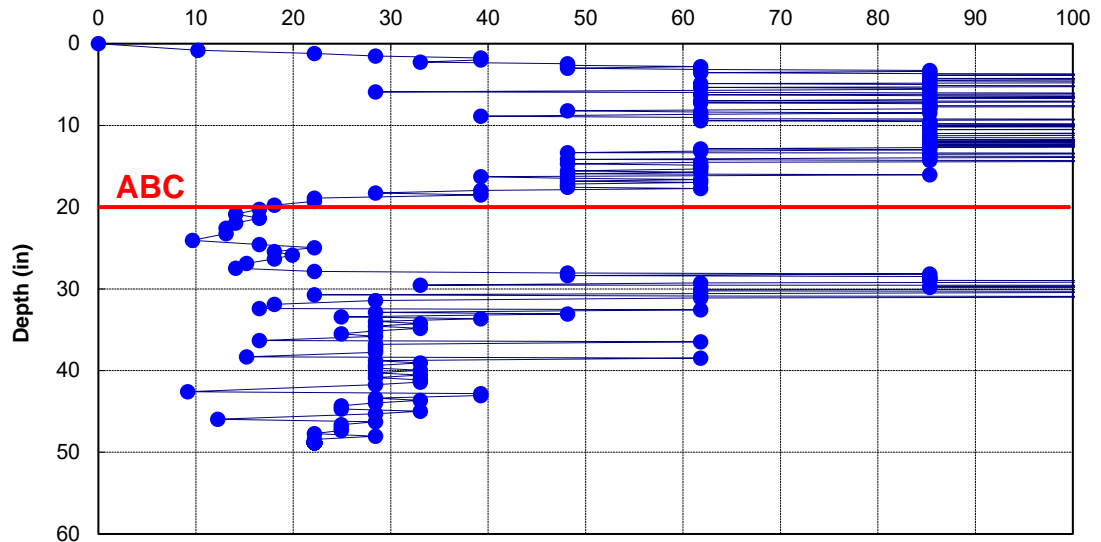
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3/12/2024

CBR Value

Interval	0.0 to 20.0
# of Values	137
Avg CBR	89.8
Wghtd Avg.	-
Max CBR	100+
Min CBR	10.2

Interval	20.0 to 48.8
# of Values	91
Avg CBR	47.9
Wghtd Avg.	-
Max CBR	100+
Min CBR	9.2



L_PC_2260_WB OSS

4

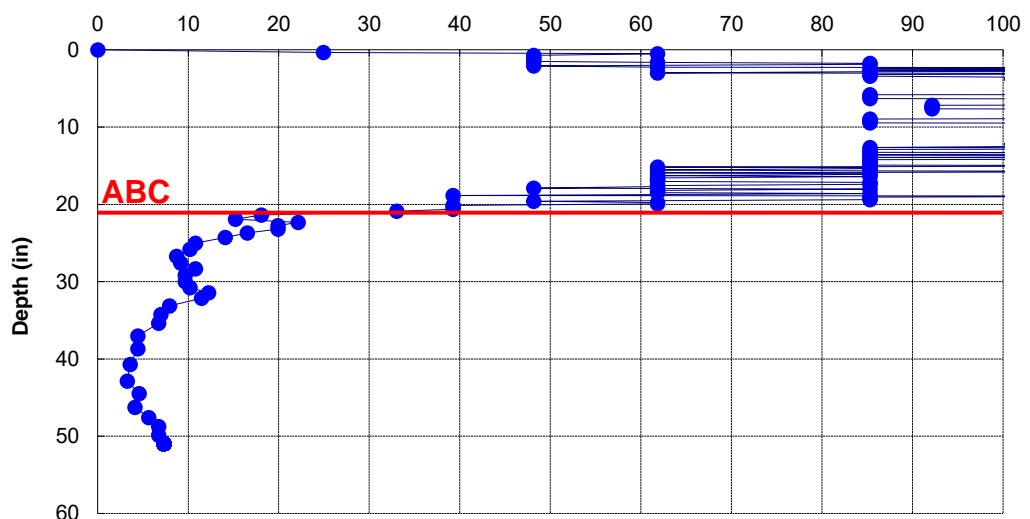
Datum = Top of subbase

3/14/2024

CBR Value

Interval	0.0 to 21.0
# of Values	202
Avg CBR	100+
Wghtd Avg	-
Max CBR	100+
Min CBR	18.1

Interval	21.0 to 50.9
# of Values	29
Avg CBR	9.8
Wghtd Avg	-
Max CBR	22.2
Min CBR	3.3



CONE PENETROMETER RESULTS

Page 10

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_2260_WB ISL

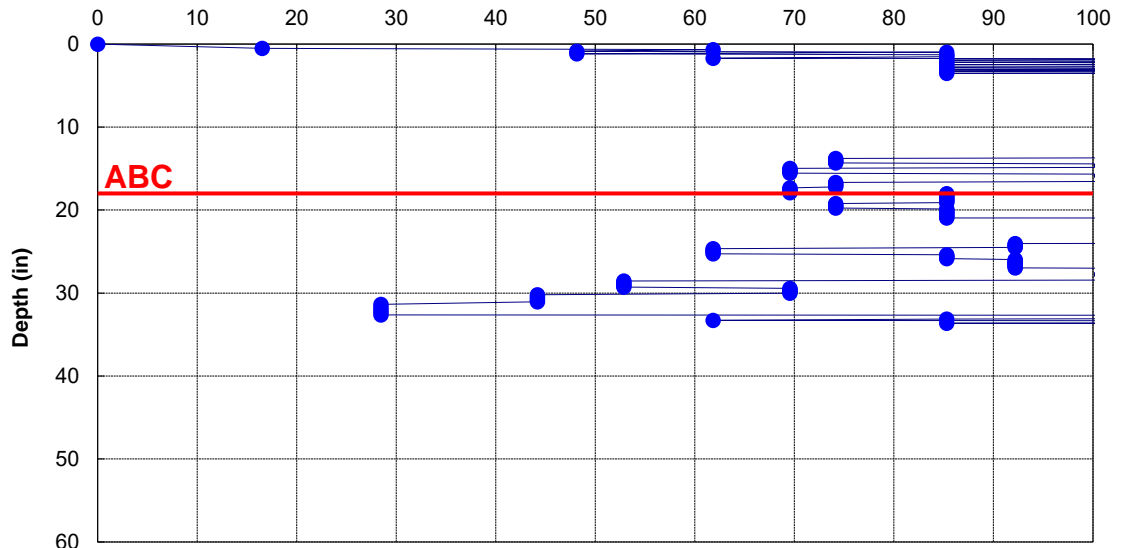
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Datum = Top of subbase 3/14/2024

CBR Value

Interval	0.0 to 18.0
# of Values	241
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	16.5

Interval	18.0 to 36.9
# of Values	216
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	28.4



L_PC_2260_WB ISS

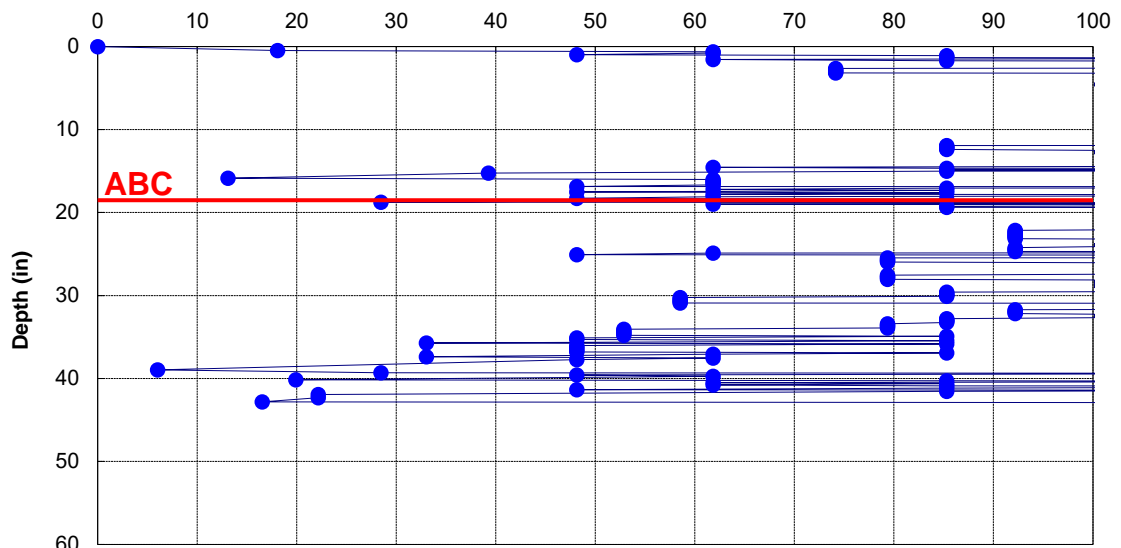
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Datum = Top of subbase 3/14/2024

CBR Value

Interval	0.0 to 18.5
# of Values	265
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	13.1

Interval	18.5 to 45.0
# of Values	222
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	6.0



CONE PENETROMETER RESULTS

Page 11

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_3500_EB ISS

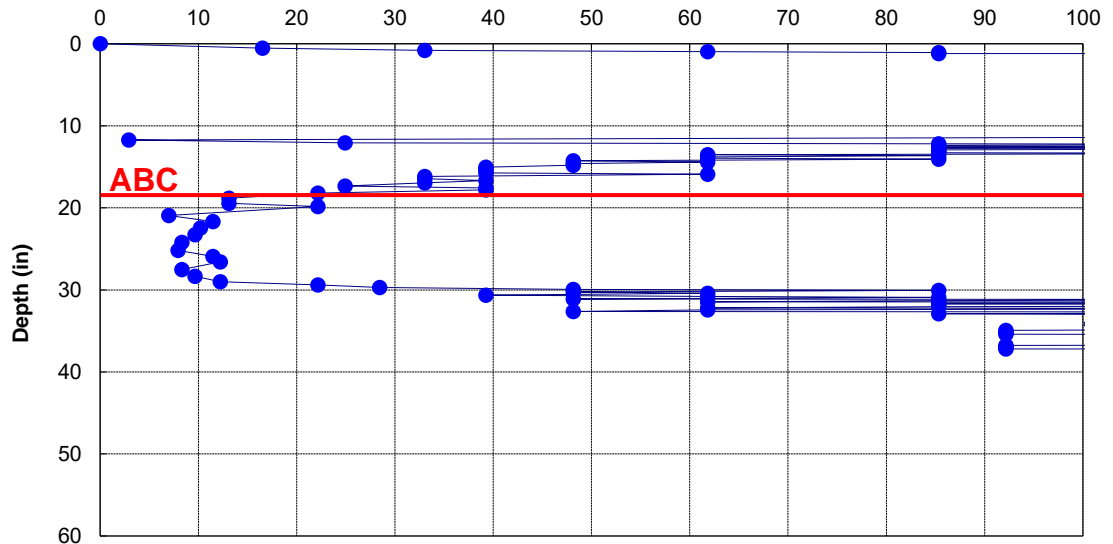
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Datum = Top of subbase 3/12/2024

CBR Value

Interval	0.0 to 18.5
# of Values	278
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	2.9

Interval	18.5 to 41.8
# of Values	200
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	7.0



L_PC_3500_EB ISL

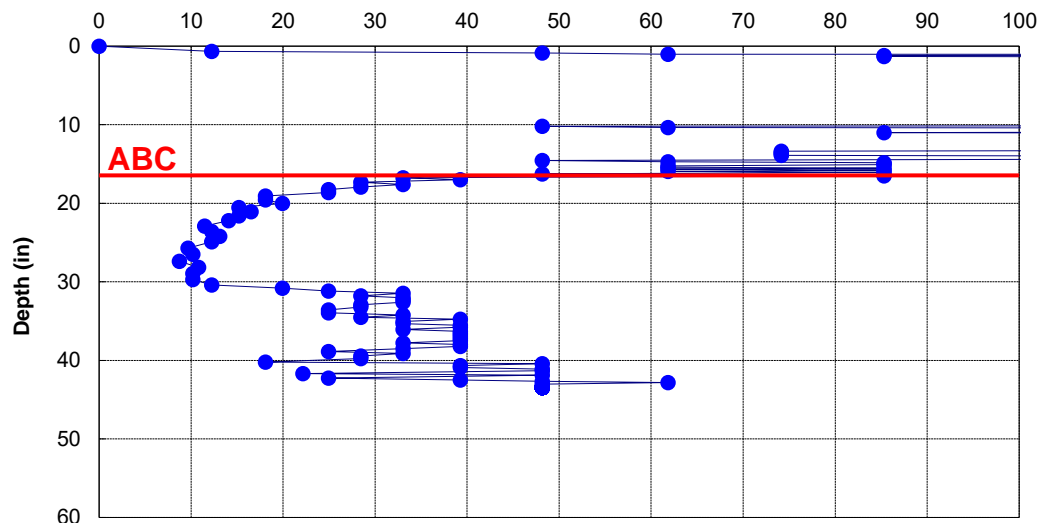
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Datum = Top of subbase 3/12/2024

CBR Value

Interval	0.0 to 16.5
# of Values	347
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	12.2

Interval	16.5 to 43.4
# of Values	72
Avg CBR	29.6
Wghtd Avg.	-
Max CBR	61.8
Min CBR	8.7



CONE PENETROMETER RESULTS

Page 12

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_3500_EB OSS

9

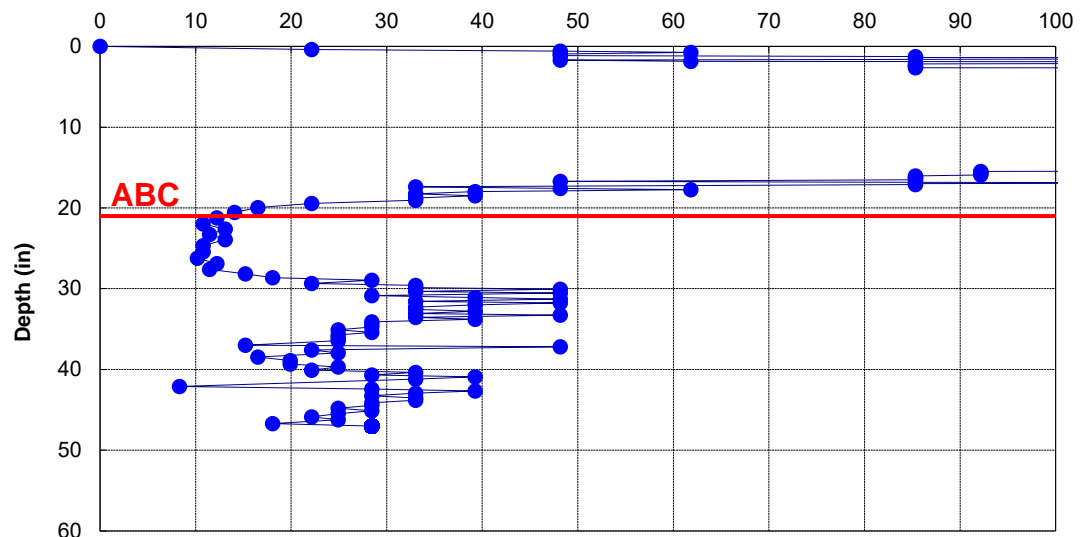
Datum = Top of subbase

3/12/2024

CBR Value

Interval	
0.0 to 21.0	
# of Values	266
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	12.2

Interval	
21.0 to 47.0	
# of Values	68
Avg CBR	27.5
Wghtd Avg.	-
Max CBR	48.1
Min CBR	8.3



L_PC_3750_WB OSS

10

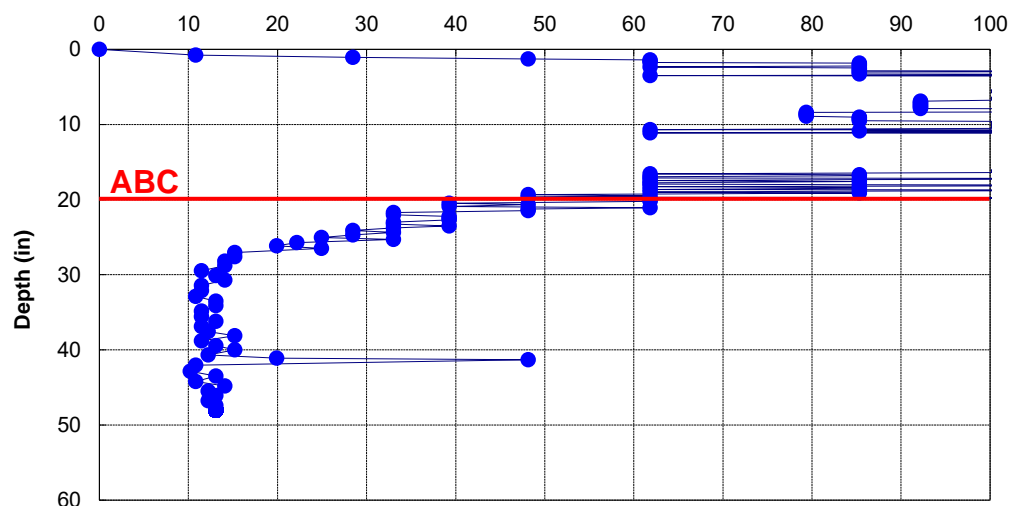
Datum = Top of subbase

3/14/2024

CBR Value

Interval	
0.0 to 20.0	
# of Values	202
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	10.8

Interval	
20.0 to 48.0	
# of Values	58
Avg CBR	23.4
Wghtd Avg.	-
Max CBR	61.8
Min CBR	10.2



CONE PENETROMETER RESULTS

Page 13

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L_PC_3750_WB OSL

11

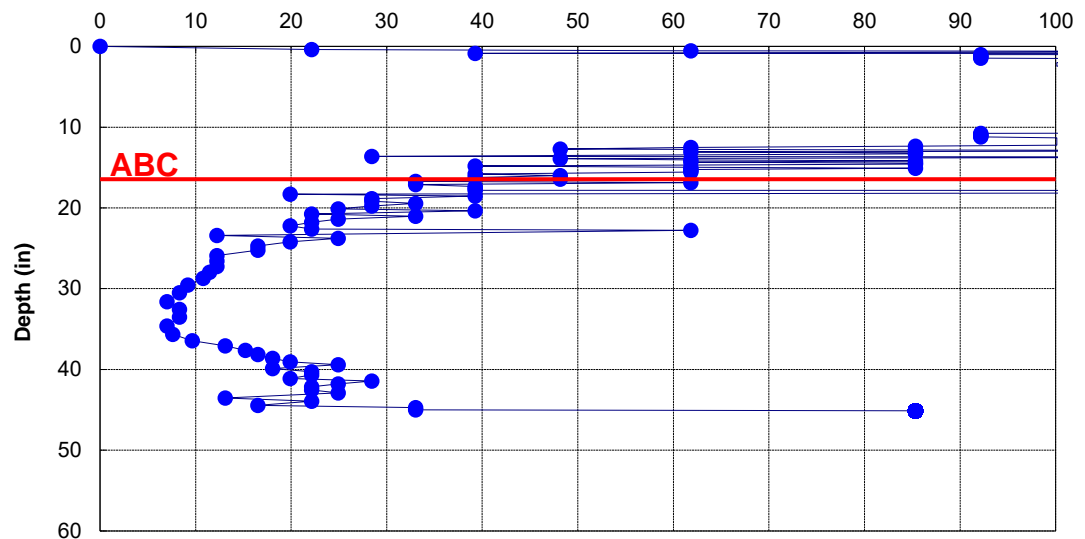
Datum = Top of subbase

3/14/2024

CBR Value

Interval	0.0 to 16.5
# of Values	202
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	22.2

Interval	16.5 to 45.1
# of Values	60
Avg CBR	28.0
Wghtd Avg.	-
Max CBR	100+
Min CBR	7.0



L_PC_3750_WB ISS

12

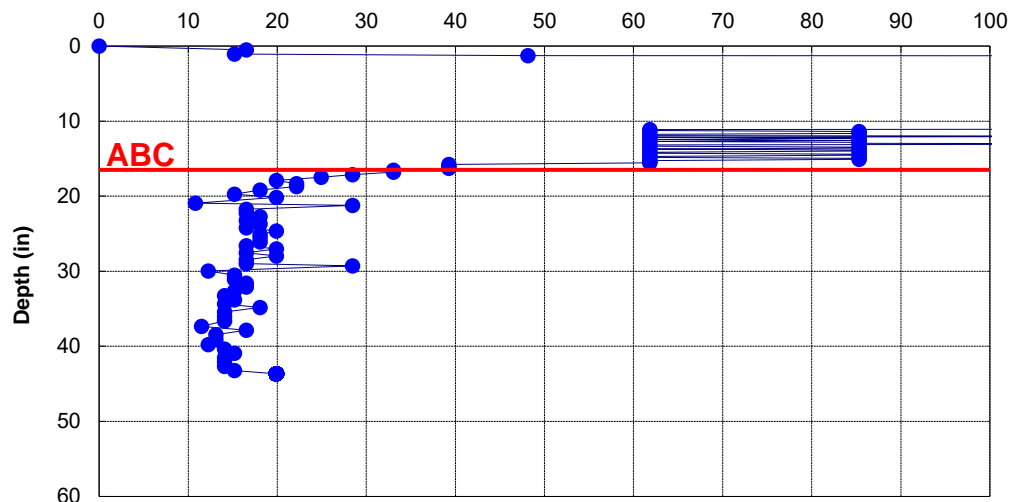
Datum = Top of subbase

3/13/2024

CBR Value

Interval	0.0 to 16.5
# of Values	295
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	15.2

Interval	16.5 to 43.7
# of Values	53
Avg CBR	17.4
Wghtd Avg.	-
Max CBR	33.0
Min CBR	10.8



PAVEMENT CORE PHOTOGRAPHS
TIP No. B-5541|Haywood County, NC

L_PC_2000_EB_ISS



L_PC_2000_EB_ISL



L_PC_2000_EB_OSS



L_PC_2260_WB_OSS



PAVEMENT CORE PHOTOGRAPHS
TIP No. B-5541|Haywood County, NC

L_PC_2260_WB_ISL

TOP



BOTTOM

TOP



BOTTOM

L_PC_2260_WB_ISS

L_PC_3500_EB_ISS

TOP



BOTTOM

TOP



BOTTOM

L_PC_3500_EB_ISL

PAVEMENT CORE PHOTOGRAPHS
TIP No. B-5541|Haywood County, NC

L_PC_3500_EB_OSS



L_PC_3750_WB_OSS



L_PC_3750_WB_ISS

**PAVEMENT CORE EVALUATION FOR I-40
HAYWOOD COUNTY, NC**

CORE LOCATION				SUBBASE		PAVEMENT CORE EVALUATION				
Line	Station Total Pavement (in.)	Lane Direction	Lane Location	Layer Type	Layer Thick- ness (in.)	Layer Type	Layer Thick- ness (in.)	# of Lifts	REMARKS	
-L-	20+00	EB	ISS	ABC	15.0	S	5.00	3	Low severity oxidation and stripping in 1st two lifts.	
					B	8.50	3	Low severity stripping at boundary of 2nd/3rd lift.		
	13.50									
-L-	20+00	EB	ISL	ABC	16.0	OGFC	1.00	1		
						S	1.50	1		
						I	4.00	1		
						S	3.75	3	Mechanical break at lift interface.	
				14.00		B	3.75	1	Low severity stripping.	
-L-	20+00	EB	OSS	ABC	20.0	S	7.50	4	Aggregate size ≤12.5mm. Bottom 1" is larger bound aggregate.	
							0			
	7.50						0			
-L-	22+60	WB	OSS	ABC	21.0	S	6.50	3	Aggregate size ≤12.5mm. Bottom 1" is larger bound aggregate.	
							0			
	6.50						0			
-L-	22+60	WB	ISL	ABC	18.0	OGFC	1.00	1		
						S	1.50	1		
						I	4.00	1	Mechanical break at lift interface.	
						S	3.75	3		
				14.00		B	3.75	1		
-L-	22+60	WB	ISS	ABC	18.5	S	5.00	3	Aggregate size ≤12.5mm. Delamination 2.5" from top.	
					B	7.50	3			
	12.50						0			
-L-	35+00	EB	ISS	ABC	18.5	S	5.50	3	Aggregate size ≤12.5mm. Low severity stripping near bottom of 3rd lift.	
					I	5.00	2	Mechanical break within 1st lift.		
	13.50				B	3.00	1			
-L-	35+00	EB	ISL	ABC	16.5	OGFC	1.00	1		
					S	9.00	6	Aggregate size ≤12.5mm. 4th lift 0.5", possible OGFC.		
	13.50				B	3.50	1			
-L-	35+00	EB	OSS	ABC	21.0	S	6.50	3	Aggregate size ≤12.5mm.	
							0			
	6.50						0			
-L-	37+50	WB	OSS	ABC	20.0	S	6.00	3	Aggregate size ≤12.5mm.	
							0			
	6.00						0			
-L-	37+50	WB	OSL	ABC	16.5	OGFC	1.00	1		
					S	4.00	2	Aggregate size ≤12.5mm.		
	13.50				I	8.50	4	3rd lift is 1" of surface mix.		
-L-	37+50	WB	ISS	ABC	16.5	S	5.50	3	Aggregate size ≤12.5mm.	
					I	5.00	2			
	13.50				B	3.00	1			

Abbreviations:

OSL = Outside Lane CTL = Center Turn Lane OSS = Outside Shoulder PS = Paved Shoulder RT = Right NB = Northbound ABC = Aggregate Base Course
ISL = Inside Lane RTL = Right Turn Lane ISS = Inside Shoulder RT LN = Right Lane LT = Left SB = Southbound RES = Residual Soil
CL = Center Lane DECEL = Deceleration Lane GM = Grass Median LT LN = Left Lane (I) = Inside FW = From White R.E. = Roadway Embankment
LTL = Left Turn Lane ACCEL = Acceleration Lane OGS = Outside Grass Shoulder COL = Collector Lane (O) = Outside FY = From Yellow

Pavement Layer Types:

(S) = Surface (I) = Intermediate (B) = Base SD = Sand Asphalt C = Concrete MS/CS = Mat Seal or Chip Seal OGFC = Open Graded Friction Course UT = Ultra-thin Bonded Wearing Course
PADC = Permeable Asphalt Drainage Course DS = Drainage Sand CTBC = Cement Treated Base Course PC = Prime Coat S-C = Soil Cement S-L = Soil Lime
N.E. = Not Encountered FEA = Flat and Elongated Aggregate

REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY															
PAVEMENT INVESTIGATION SOIL TEST RESULTS -L-															
SAMPLE NO.	BORING ID	OFFSET DISTANCE	DEPTH INTERVAL (ft)	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-3	L_PC_2000_EB ISL	4' FY	4.0-5.0	A-2-4 (0)	31	8	37.6	30.3	6.5	25.6	87.2	62.5	33.1	13	-
S-13	L_PC_2260_WB OSS	5' FW	4.0-5.0	A-2-4 (0)	NP	NP	41.6	29.3	7.5	21.6	84	55.3	28.9	18	-
S-9	L_PC_2260_WB ISL	4' FY	4.0-5.0	A-2-4 (0)	NP	NP	39.2	38	3.5	19.4	90.6	68.7	25.4	8	-
S-2	L_PC_3500_EB OSS	3.5' FW	4.0-5.0	A-7-6 (4)	45	18	27.5	31.7	18.1	22.6	93.9	75.1	43.8	77	-
S-6	L_PC_3500_EB ISS	1.5' FW	4.0-5.0	A-4 (0)	29	6	31.8	35.5	12.8	19.9	90.8	70.2	36.1	10	-
S-10	L_PC_3750_WB OSS	7' FW	2.1-3.0	A-2-4 (0)	NP	NP	50.2	26.2	11	12.6	80.2	50.8	24.1	4	-
S-11	L_PC_3750_WB OSS	7' FW	4.0-5.0	A-2-4 (0)	NP	NP	41.2	32.1	5.7	21	84.4	55.9	27.7	13	-
S-7	L_PC_3750_WB ISS	0.5' FY	4.0-5.0	A-6 (1)	39	11	30.2	33.2	14.3	22.2	92.7	72.2	40	15	-

Notes:

1. Refer to Pavement Investigation Data Sheet for station abbreviations

BULK SAMPLE SOIL TESTS SUMMARY FOR -L-														
Station	Offset (ft)	Sample ID	Depth Interval (ft)	AASHTO Soil Class. (Group Index)	L.L.	P.I.	% Passing (Sieves)			Max. Dry Density (pcf)	Optimum Moisture Content (%)	CBR Value		CBR Swell (%)
							10	40	200			0.1"	0.2"	
25+00	80' RT	BS-2	0.0 - 10.0	A-6 (5)	40	16	89.3	67.3	50.3	107.1	17.3	3.4	3.8	2.5
34+50	80' RT	BS-1	0.0 -10.0	A-7-5 (4)	52	15	96.2	75.7	46	107.2	18.8	2.7	3.4	4.7

Notes: 1. Refer to laboratory transmittal for additional information

BULK SAMPLE SOIL TESTS SUMMARY FOR -Y1-														
Station	Offset (ft)	Sample ID	Depth Interval (ft)	AASHTO Soil Class. (Group Index)	L.L.	P.I.	% Passing (Sieves)			Max. Dry Density (pcf)	Optimum Moisture Content (%)	CBR Value (Corrected)		CBR Swell (%)
							10	40	200			0.1"	0.2"	
26+00	15' LT	BS-3	0.0 - 10.0	A-2-6 (0)	37	12	64.9	40.3	20.5	119.6	14	12	13.3	4.1

Notes: 1. Refer to laboratory transmittal for additional information

REFERENCE: B-5541

PROJECT: 55041

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN
-L-	17+00 - 42+00	4-6
-YI-	20+00 - 26+50	5
SRI	18+50 - 20+50	5

APPENDIX A

TITLE	SHEETS
BORING LOGS	7-20
DCP RAW DATA LOGS	21-22
SOIL TEST RESULTS	23-24

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY HAYWOOD
PROJECT DESCRIPTION REPLACE BRIDGE NO. 236
ON I-40 OVER SR 1513 IN HAYWOOD COUNTY
INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5541	1	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

Q. HILL

CG2 EXPLORATION

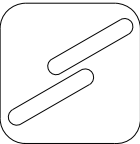
INVESTIGATED BY J. HOLLAND

DRAWN BY J. HOLLAND

CHECKED BY J. CRENSHAW

SUBMITTED BY SCHNABEL ENG.

DATE APRIL 2024



Schnabel
ENGINEERING



Jason A. Holland

SIGNATURE

05/23/2024

DATE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)				SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS			
GROUP CLASS.	A-1	A-3	A-2		A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5		
SYMBOL												
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	
MATERIAL PASSING #40 LL PI	— 6 MX		— NP		40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 11 MN	40 MX 11 MN	
GROUP INDEX	0		0		4 MX		8 MX		12 MX		16 MX	
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND		SILTY SOILS		CLAYEY SOILS		SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER	
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD				FAIR TO POOR				FAIR TO POOR		POOR	
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS ≥ LL - 30												

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE OPENING (MM)	4	10	40	60	200	270
	4.75	2.00	0.42	0.25	0.075	0.053
	COARSE SAND (CSE. SD.)		FINE SAND (F SD.)		SILT (SL.)	CLAY (CL.)
GRAIN SIZE	MM 305 IN. 12	75 3	2.0		0.25	0.05

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL PLASTIC RANGE (PI) PL	- LIQUID LIMIT (- SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
OM SL	OPTIMUM MOISTURE SHRINKAGE LIMIT	
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC., ARE USED TO DESCRIBE APPEARANCE.
--

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:
ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.
ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE LL < 31
MODERATELY COMPRESSIBLE LL = 31 - 50
HIGHLY COMPRESSIBLE LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
 STATIC WATER LEVEL AFTER 24 HOURS
 PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
 SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE)
WITH SOIL DESCRIPTION
 SOIL SYMBOL
 ARTIFICIAL FILL (AF) OTHER
THAN ROADWAY EMBANKMENT
 INFERRED SOIL BOUNDARY
 INFERRED ROCK LINE
 ALLUVIAL SOIL BOUNDARY
 DIP & DIP DIRECTION
OF ROCK STRUCTURES
 TEST BORING
 AUGER BORING
 CORE BORING
 MONITORING WELL
 PIEZOMETER
INSTALLATION
 SLOPE INDICATOR
INSTALLATION
 CONE PENETROMETER
TEST
 SOUNDING ROD
 TEST BORING
WITH CORE
 SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT
 SHALLOW
UNDERCUT
 UNCLASSIFIED EXCAVATION -
UNSUITABLE WASTE
 UNCLASSIFIED EXCAVATION -
ACCEPTABLE DEGRADABLE ROCK
 UNCLASSIFIED EXCAVATION -
ACCEPTABLE, BUT NOT TO BE
USED IN THE TOP 3 FEET OF
EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HL - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT
SAMPLE ABBREVIATIONS
S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING
RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:

☐ CME-45C
☐ CME-55
☐ CME-550
☐ VANE SHEAR TEST
☐ PORTABLE HOIST
☐ _____
☒ DIEDRICH D-50

ADVANCING TOOLS:

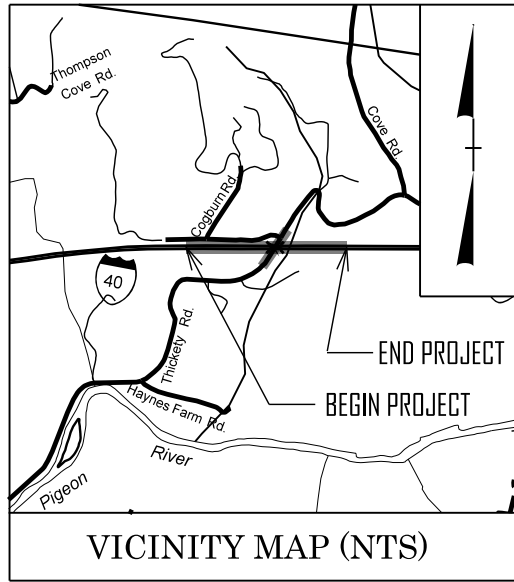
☐ CLAY BITS
☐ 6" CONTINUOUS FLIGHT AUGER
☒ 8" HOLLOW AUGERS
☐ HARD FACED FINGER BITS
☐ TUNG.-CARBIDE INSERTS
☐ CASING ☐ W/ ADVANCER
☐ TRICONE _____ ' STEEL TEETH
☐ TRICONE _____ ' T

09/08/99

TIP PROJECT: B-5541

CONTRACT:

See Sheet 1A For Index of Sheets



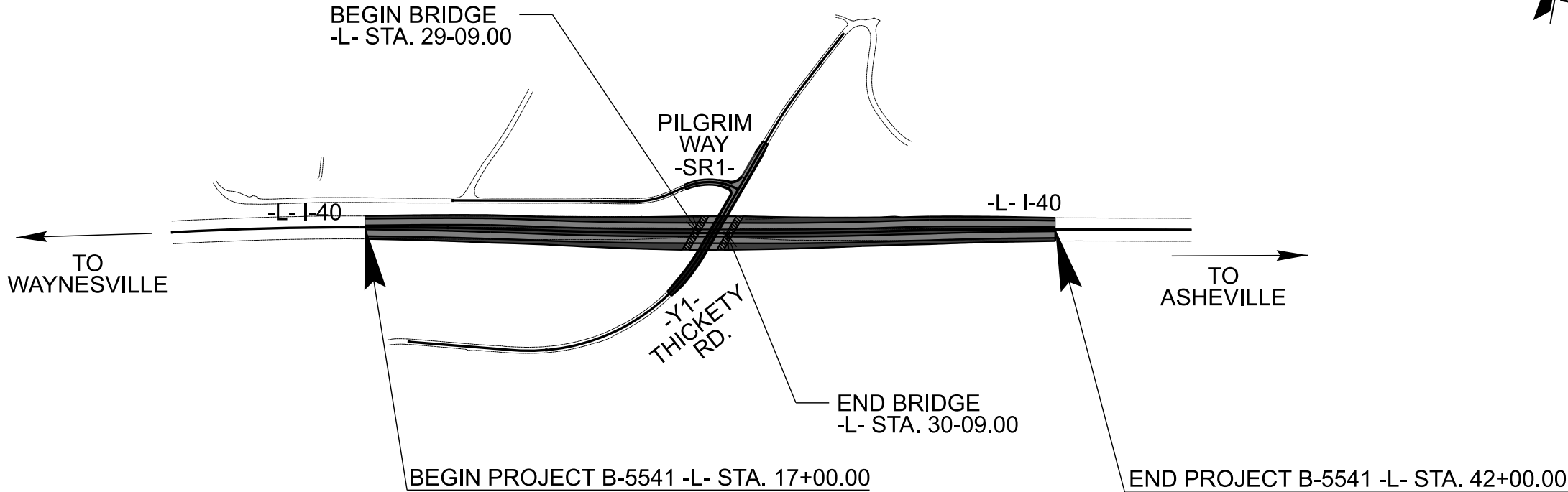
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS
HAYWOOD COUNTY

LOCATION: *BRIDGE NO. 236 OVER SR 1513 ON I-40*

TYPE OF WORK: *GRADING, PAVING, DRAINAGE AND STRUCTURES*

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5541	3	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
55041.1.1		P.E.	

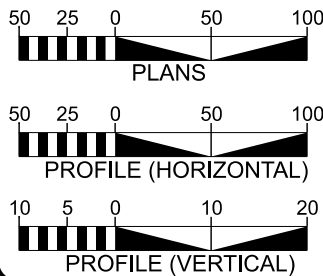
DESIGN RECOMMENDATION PLAN SET



THERE IS FULL CONTROL OF ACCESS ON THIS PROJECT.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ???

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2024 = 60000
ADT 2044 = 77500
K = 8 %
D = 55 %
T = 15 % *
V = 65 MPH
* TTST = 12% DUAL 3%
FUNC CLASS =
INTERSTATE
STATE WIDE TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5541 = 0.471 MILES
LENGTH STRUCTURE TIP PROJECT B-5541 = 0.021 MILES
TOTAL LENGTH TIP PROJECT B-5541 = 0.492 MILES

Prepared in the Office of:

NIVIS

NIVS ENGINEERS & CONSULTANTS, INC.
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CHARLOTTE, NC 28213
P: 704.537.7300 www.NIVS.com
NC License # F-1333
formerly CALVIN Engineers & Consultants

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
MARCH 15, 2024

LETTING DATE:
JUNE 17, 2025

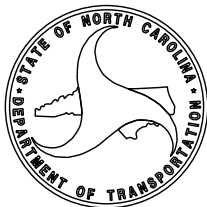
CHRIS ANDERSON, PE
PROJECT ENGINEER

DYLAN MCCANN
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.
ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



April 23, 2024

STATE PROJECT: 55041.1.1

TIP NUMBER: B-5541

COUNTY: HAYWOOD

DESCRIPTION: REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY

SUBJECT: Geotechnical Roadway Inventory Report

Project Description

The project consists of the replacement of the existing four span Bridge (No. 236) over SR-1513 (Thickety Rd.) and widening improvements to the 0.45-mile roadway bridge approach along I-40 in the town of Clyde, North Carolina. The newly constructed bridge will be replaced on the existing alignment, with symmetrical widening to the outside, and accommodate two lanes of traffic in each direction during construction. We also investigated an existing 72-inch diameter corrugated metal pipe (CMP) culvert, located about 100 feet from the end of the new bridge, at approximately - L- STA. 31+00, which drains surface waters from a tributary of Pigeon River. The provided plans showed anticipated cut excavations as deep as 20± feet and proposed embankment construction with fill heights up to 23± feet max fill over the culvert. Slope inclinations are 2:1 or flatter, except at culvert steepened fill slopes up to 1.35:1 are proposed. Boring logs associated with the existing culvert are included in this report.

The field investigation was conducted in February and March of 2024 utilizing a track-mounted Diedrich D-50 drill machine with an automatic hammer, and hand tools. Standard Penetration Tests (SPT) were performed at selected locations. Borings were advanced with hollow stem augers, polycrystalline diamond compact bit (PDC) for mud rotary methods, and hand tools along the project corridor. Hand augers were performed at locations the drill rig could not access, or where concentrated underground utilities warranted. Dynamic Cone Penetration Tests (DCP) were conducted at all hand auger locations. Representative soil samples were collected and forwarded to an approved testing facility for soil quality analysis, moisture content, California Bearing Ratio, specific gravity, unconsolidated undrained triaxial shear, one-dimensional consolidation, and AASHTO classification.

The Following Alignments were Investigated

Line	Station			Length (ft)
-L-	17+00	to	42+00	2,500
-Y1-	20+00	to	26+50	650
-SR1-	18+50	to	20+50	200
			Total=	3,350 feet (~0.634 miles)

Physiography and Geology

Based on a review of the Geologic map of North Carolina (1985) and the Geologic map of the Western Half of the Asheville 1:100,000-Scale 30 x 60 minute quadrangle, North Carolina and Tennessee (2008), the project is located in the Blue Ridge Physiographic Province. According to the 30 x 60 quadrangle map, the Hot Springs thrust fault appears to intersect the project corridor, trending from northeast to southwest, exposing Grenville-aged granulite facies metamorphic rocks of Earlies Gap (Ye). Soils in the area generally consist of residual sands, silts, and clays. Weathered rock and Late Proterozoic crystalline rock of the Ashe Metamorphic Suite Complex (Zas) and Earlies Gap (Ye) primarily consisted of schist and biotite gneiss. Topography along the project corridor is undulating, traversing along I-40 and bounded by some heavily wooded areas, a few residences, and a paper processing facility to the southeast. Natural ground surface elevations range from 2617± feet above sea level at the beginning of the -Y1- alignment to 2724± feet above sea level at the end of the -L- alignment construction limits.

Soil Properties

Soil and rock encountered along the project corridor are divided into five categories based on origin: roadway embankment soils, alluvial soils, residual soils, weathered rock, and crystalline rock.

Roadway embankment soils consisting of medium dense SAND and GRAVEL (A-1-b), very loose to loose silty and clayey SAND (A-2-7), and soft to medium stiff, sandy SILT (A-4) were encountered along the -L- alignment. Soils moistures were typically moist and varied in thickness from the ground surface to a maximum of 16 feet. Within the cohesive roadway embankment soil, moisture contents were reported at 21%. The plasticity index (PI) within the cohesive soil was 8.

Alluvial soils consisting of very loose SAND and GRAVEL (A-1-b), very loose to medium dense silty SAND and clayey SAND (A-2-4, A-2-6), medium stiff sandy SILT (A-4), and very soft to soft silty CLAY (A-7-6) were encountered along the -L- and -Y1- alignments near the Culvert Outfall. Soils moistures were typically moist to wet, and varied in thickness from 3 to 12 feet. Within the cohesive alluvial soils, moisture contents ranged from 27.0% to 35.0%. Plasticity indices (PI) within the cohesive sediments range from 15 to 16.

Residual soils consisting of very loose to very dense silty SAND and clayey SAND (A-2-4, A-2-6, A-2-7), soft to hard, sandy SILT and clayey SILT (A-4, A-5), and very soft to hard, sandy CLAY, silty CLAY, and sandy and silty CLAY (A-6, A-7-5, A-7-6) were encountered throughout the project corridor. Soil moistures were typically moist to saturated and varied in thickness from the ground surface to a maximum of 79.4 feet. Within the cohesive residual soils, moisture contents ranged from 4.0% to 77.0%. Plasticity indices (PI) within the cohesive sediments range from 5 to 20.

Weathered rock consisting of tan, orange, brown, and white, INJECTION GNEISS, was encountered interlaid and underlying residual soils at several locations along the project corridor. Weathered rock elevations in these borings varied from 2579± feet above sea level to 2655± feet above sea level. Auger and split spoon refusal were noted beneath some of these layers on crystalline rock (INJECTION GNEISS).

Crystalline rock was identified by split spoon refusal, underlying residual soils and weathered rock at several locations along the corridor. USGS geologic maps and the historical Structure Subsurface Investigation confirm bedrock consists of INJECTION GNEISS. Top of rock elevations in these borings varied from 2567± feet above sea level to 2609± feet above sea level. No rock core samples were collected as part of this investigation.

Groundwater

Borings were left open for a minimum of 24 hours to equilibrate with the surrounding conditions, when conditions permitted. Groundwater data was collected in February and March of 2024. Groundwater elevations generally varied with topography and ranged from 2614± to 2674± feet above sea level.

Areas of Special Geotechnical Interest

A. Alluvial Soils were encountered in the following sections.

Alignment	Begin Station	End Station	Offset
-L-	30+00	30+75	± 117 RT
-L-	31+25	31+50	± 170 LT
-Y1-	20+25	20+75	± 75 RT

In addition to the alluvial soils found in the areas listed above, a culvert was identified at approximately -L- Sta. 30+50. The plans that were available did not indicate any proposed construction related to this culvert, however, after discussing with NCDOT we were granted authorization to investigate both north and south outfall locations.

B. Fill soils were encountered in the following sections.

Alignment	Begin Station	End Station
-L-	28+25	31+00

Jason A. Holland

Jason A. Holland, LG
Project Geologist
Schnabel Engineering

B-5541

R/W4

NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

HAYWOOD COUNTY

ROADWAY DESIGN UNIT

ROADWAY DESIGN ENGINEER

HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

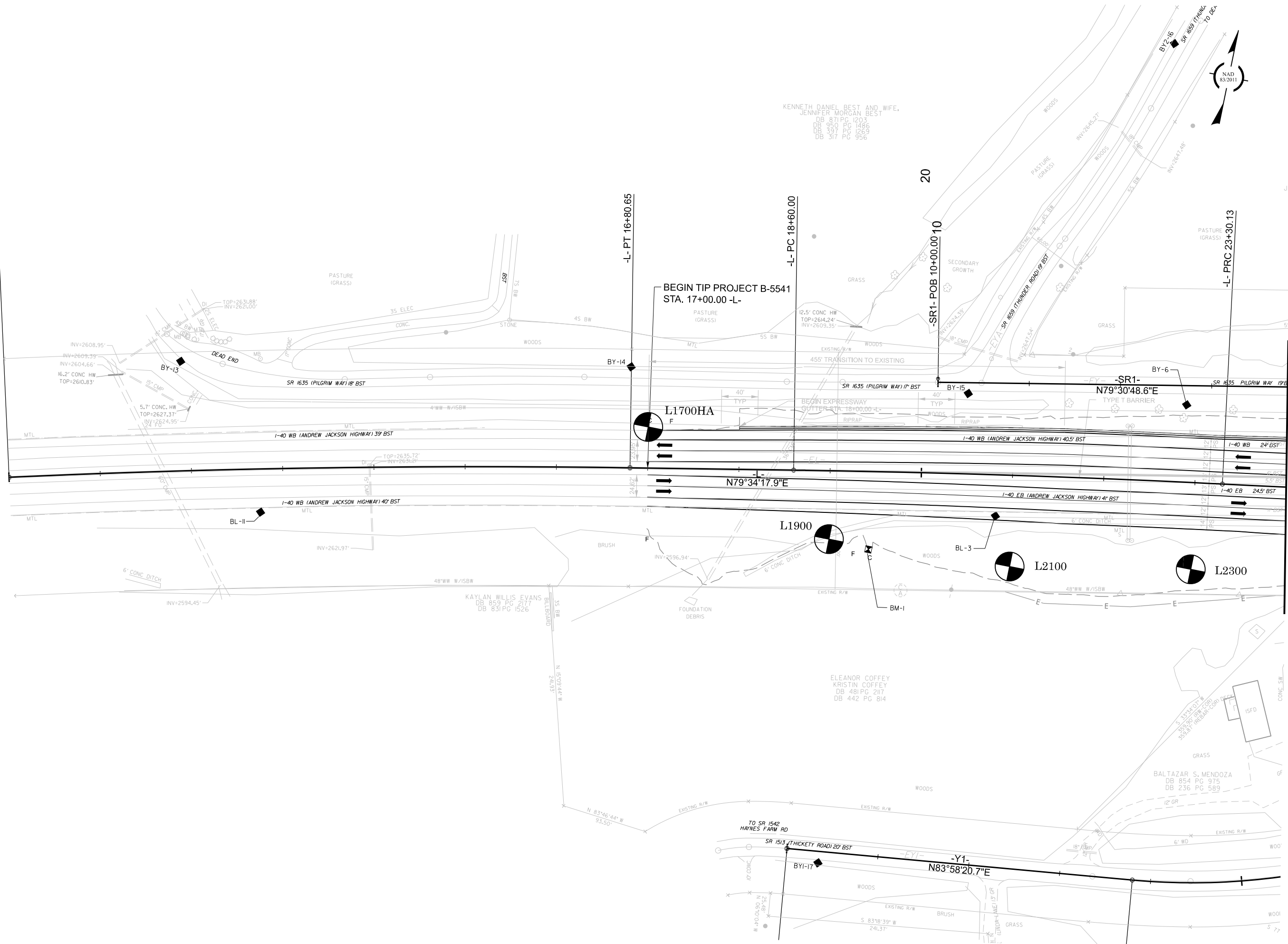
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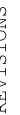
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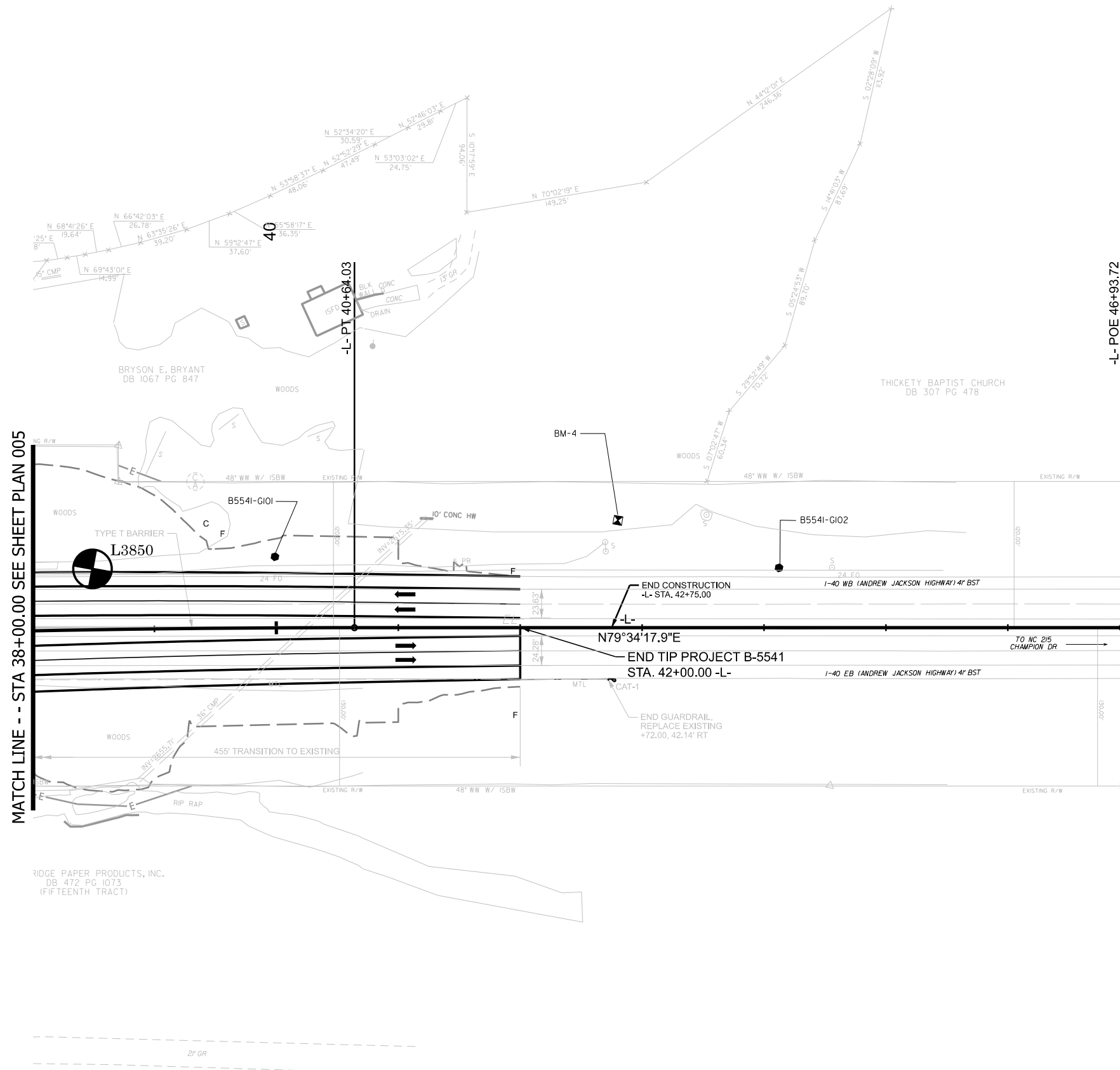
NVI ENGINEERING & CONSULTANTS, INC.
2814 HAZLEHURST PARK DRIVE, SUITE 130
CHARLOTTE, NC 28211
P: 704.517.7200 www.nvi.com
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REVISIONS



MATCH LINE - - STA 24+00.00 SEE SHEET PLAN 005





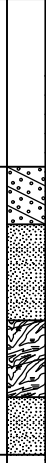
REVISIONS

FOR -L- PROFILE SEE SHEET 7, 8, & 9

GEOTECHNICAL BORING REPORT

BORE LOG

[illegible]




WBS 55041.1.1		TIP B-5541		COUNTY HAYWOOD		GEOLOGIST Q. Hill								
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513							GROUND WTR (ft)							
BORING NO. L1900		STATION 19+00		OFFSET 75 ft RT		ALIGNMENT -L-		0 HR. Dry						
COLLAR ELEV. 2,630.3 ft		TOTAL DEPTH 15.0 ft		NORTHING 677,077		EASTING 843,403		24 HR. Dry						
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic								
DRILLER C. Odom		START DATE 02/29/24		COMP. DATE 03/01/24		SURFACE WATER DEPTH N/A								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
2635														
2630	2,630.3	0.0												2,630.3 GROUND SURFACE 0.0
2625	2,626.8	3.5	1	1	2						SS-57	22%		RESIDUAL Orange and tan, clayey SAND (A-2-6(0)), with some rock fragments, micaceous
	2,624.3	6.0	5	4	5							M		2,627.3 3.0 Tan and white, sandy SILT (A-4), micaceous, low plasticity
	2,621.8	8.5	14	29	41							M		2,622.3 8.0
2620	2,616.8	13.5	68	32/0.3										WEATHERED ROCK Tan, and orange, INJECTION GNEISS
														2,618.3 12.0
			3	3	7							M		RESIDUAL Tan, orange, and white, sandy SILT (A-4), micaceous, saprolitic
														2,615.3 15.0 Boring Terminated at Elevation 2,615.3 ft in sandy SILT (RESIDUAL) 2-inches Topsoil

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24


NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

WBS		55041.1.1		TIP		B-5541		COUNTY		HAYWOOD		GEOLOGIST		Q. Hill																																																																																																																																																																																																																																																																																																																																																																																																																																																																											
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BORING NO.			L2700			STATION			27+00			OFFSET			70 ft RT			ALIGNMENT			-L-			0 HR.		Dry																																																																																																																																																																																																																																																																																																																																																																																																																																																															
COLLAR ELEV.			2,674.2 ft			TOTAL DEPTH			25.0 ft			NORTHING			677,210			EASTING			844,192			24 HR.		Dry																																																																																																																																																																																																																																																																																																																																																																																																																																																															
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DRILLER					C. Odom					START DATE					02/28/24					COMP. DATE					02/29/24					SURFACE WATER DEPTH										N/A																																																																																																																																																																																																																																																																																																																																																																																																																																																	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	<div><div></div><div>MOI</div></div>	<div>L O G</div>	SOIL AND ROCK DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																											
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2675	2,674.2	0.0				1	2	3	<div><div><div><div></div><div>5</div></div><div><div></div><div>11</div></div><div><div></div><div>14</div></div><div><div></div><div>19</div></div><div><div></div><div>22</div></div><div><div></div><div>15</div></div><div><div></div><div>26</div></div></div></div>																																																																																																																																																																																																																																																																																																																																																																																																																																																																																

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill					
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513										GROUND WTR (ft)				
BORING NO. L2850_R			STATION 28+50			OFFSET 95 ft RT			ALIGNMENT -L-		0 HR. 30.5			
COLLAR ELEV. 2,646.7 ft			TOTAL DEPTH 35.0 ft			NORTHING 677,212			EASTING 844,344		24 HR. 26.0			
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER C. Odom			START DATE 02/28/24			COMP. DATE 02/29/24			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
2650														
	2,646.7	0.0												2,646.7 GROUND SURFACE 0.0
2645			1	1	2						SS-34	17%		ROADWAY EMBANKMENT Brown and orange, silty and clayey SAND (A-2-7(0)), with trace rock fragments, contains root fragments, micaceous
	2,643.2	3.5		2	3							M		
2640			2	3	5									2,641.2 5.5
	2,640.7	6.0		9	12							M		RESIDUAL Orange, brown, and red, sandy SILT (A-4(0)), contains manganese oxides, micaceous, saprolitic
	2,638.2	8.5		9	12							M		
2635				9	12									
	2,633.2	13.5		4	5							M		
2630				4	5									
	2,628.2	18.5		5	5							M		
2625				5	5									
	2,623.2	23.5		3	2						SS-40	29%		
2620				3	2							M		
	2,618.2	28.5		3	5									
2615				3	5									
	2,613.2	33.5		4	6							M		2,611.7 35.0
				4	6									Boring Terminated at Elevation 2,611.7 ft in sandy SILT (RESIDUAL)

NC DOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill					
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513									GROUND WTR (ft)					
BORING NO. L3050_R			STATION 30+07			OFFSET 107 ft RT			ALIGNMENT -L-		0 HR. 9.1			
COLLAR ELEV. 2,632.3 ft			TOTAL DEPTH 30.0 ft			NORTHING 677,229			EASTING 844,501		24 HR. 5.5			
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER C. Odom			START DATE 03/04/24			COMP. DATE 03/05/24			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
2635														
	2,632.3	0.0	1	0	1	1					SS-97	35%		2,632.3 GROUND SURFACE 0.0
2630	2,628.8	3.5	8	4	3	7								ALLUVIAL Brown, silty CLAY (A-7-5(8)), micaceous, moderate plasticity
	2,626.3	6.0	1	1	3	4								2,629.3 3.0
2625	2,623.8	8.5	1	1	2	3						M		RESIDUAL Tan, brown, and orange, sandy SILT (A-4(0)), with trace rock fragments and little clay, micaceous, saprolitic
	2,618.8	13.5	6	4	9	13					SS-101	26%		
2615	2,613.8	18.5	4	5	8	13						M		
	2,608.8	23.5	10	9	4	13						M		
2605	2,603.8	28.5	5	7	10	17						M		
														Boring Terminated at Elevation 2,602.3 ft in sandy SILT (RESIDUAL)
														Other Samples: ST-2 (8.0 - 10.0)

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill						
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513									GROUND WTR (ft)						
BORING NO. L3100_L			STATION 31+43			OFFSET 170 ft LT			ALIGNMENT -L-			0 HR. Dry			
COLLAR ELEV. 2,631.7 ft			TOTAL DEPTH 30.0 ft			NORTHING 677,526			EASTING 844,584			24 HR. Dry			
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER C. Odom			START DATE 03/05/24			COMP. DATE 03/06/24			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2635															
	2,631.7	0.0													
2630			2	1	3										
	2,628.2	3.5	WOH	1	1										
2625	2,625.7	6.0													
	2,623.2	8.5	12	17	8										
2620			2	2	4										
	2,618.2	13.5													
2615			3	4	6										
	2,613.2	18.5													
2610			7	10	12										
	2,608.2	23.5													
2605			8	11	11										
	2,603.2	28.5	4	4	8										

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NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

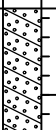
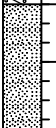
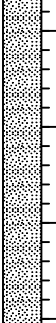
WBS 55041.1.1		TIP B-5541		COUNTY HAYWOOD		GEOLOGIST Q. Hill									
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513								GROUND WTR (ft)							
BORING NO. L3150_R		STATION 31+16		OFFSET 114 ft RT		ALIGNMENT -L-		0 HR.	20.3						
COLLAR ELEV. 2,635.2 ft		TOTAL DEPTH 30.0 ft		NORTHING 677,242		EASTING 844,609		24 HR.	21.0						
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER C. Odom		START DATE 03/05/24		COMP. DATE 03/06/24		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2640															
2635	2,635.2	0.0												2,635.2 GROUND SURFACE 0.0	
2630	2,631.7	3.5	1	1	2							M		RESIDUAL Orange and brown, silty CLAY (A-7-5), with little rock fragments, micaceous, low to moderate plasticity	
	2,629.2	6.0	5	7	9							M			
2625	2,626.7	8.5	6	7	9						SS-114	49%		Orange, tan, and brown, silty SAND (A-2-4(0)), with little to some clay and trace rock fragments, micaceous, saprolitic	
	2,621.7	13.5	6	9	12							Sat.			
2620	2,616.7	18.5	6	6	9								Sat.		
	2,611.7	23.5	7	8	9							Sat.			
2610	2,606.7	28.5	2	2	5								Sat.		
			7	11	15							Sat.			
															Boring Terminated at Elevation 2,605.2 ft in silty SAND (RESIDUAL)

[illegible]

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

[illegible]

NC DOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill					
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513									GROUND WTR (ft) 0 HR. Dry 24 HR. Dry					
BORING NO. L3500			STATION 35+00			OFFSET 100 ft LT					ALIGNMENT -L-			
COLLAR ELEV. 2,699.5 ft			TOTAL DEPTH 30.0 ft			NORTHING 677,527					EASTING 844,944			
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER D. Underwood			START DATE 03/07/24			COMP. DATE 03/08/24			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
2700	2,699.5	0.0	2	2	2									GROUND SURFACE 0.0
2695	2,696.0	3.5	2	3	5						SS-142	20%		RESIDUAL Orange and brown, silty and clayey SAND (A-2-7(1)), micaceous
	2,693.5	6.0	8	12	11									
2690	2,691.0	8.5	10	10	10									Orange and brown, sandy SILT (A-4(0)), with trace to little clay and rock fragments, micaceous
	2,686.0	13.5	11	13	15									
2685	2,681.0	18.5	12	27	26									
2680	2,676.0	23.5	16	20	27									
2675	2,671.0	28.5	16	27	41									
2670														Boring Terminated at Elevation 2,669.5 ft in sandy SILT (RESIDUAL)

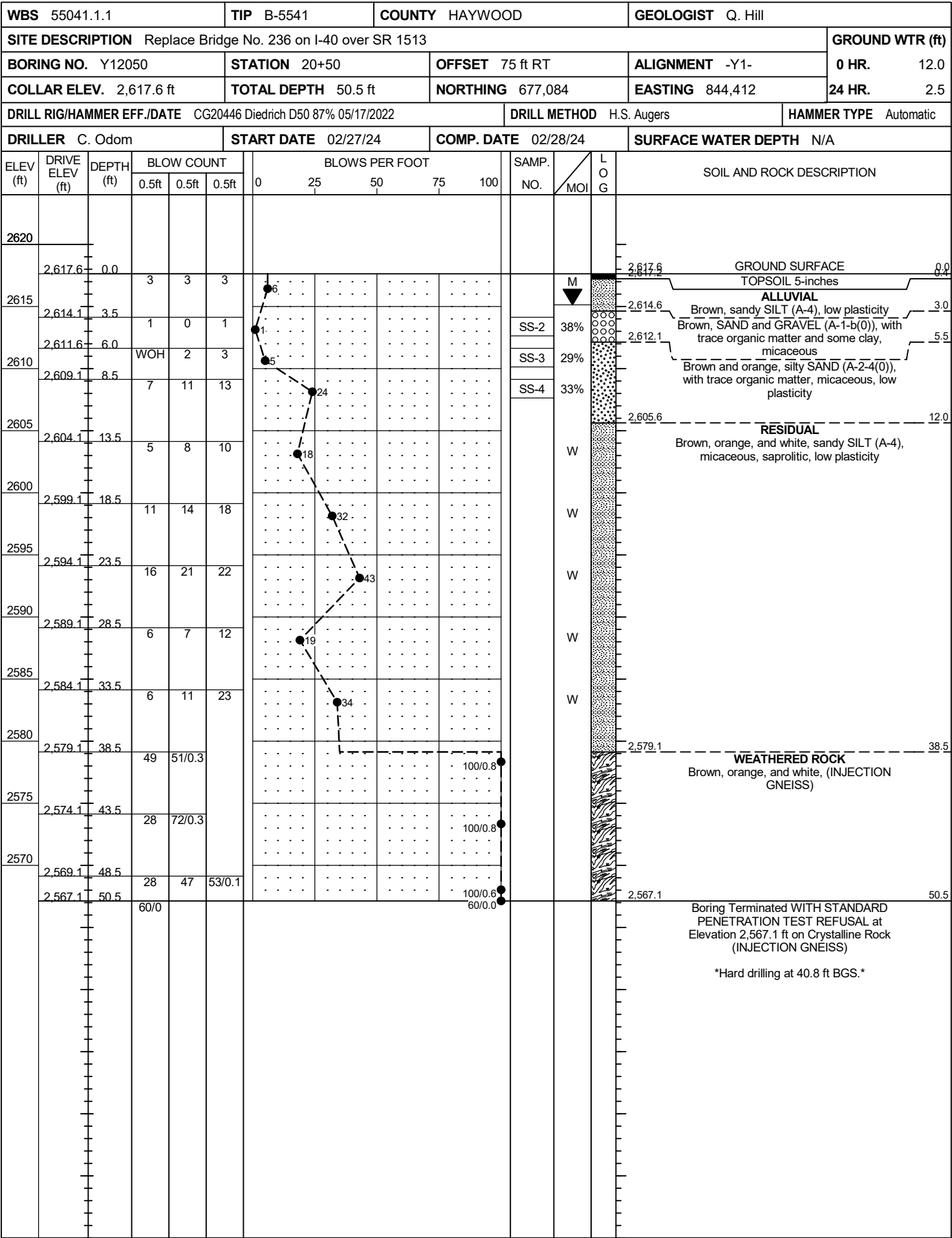
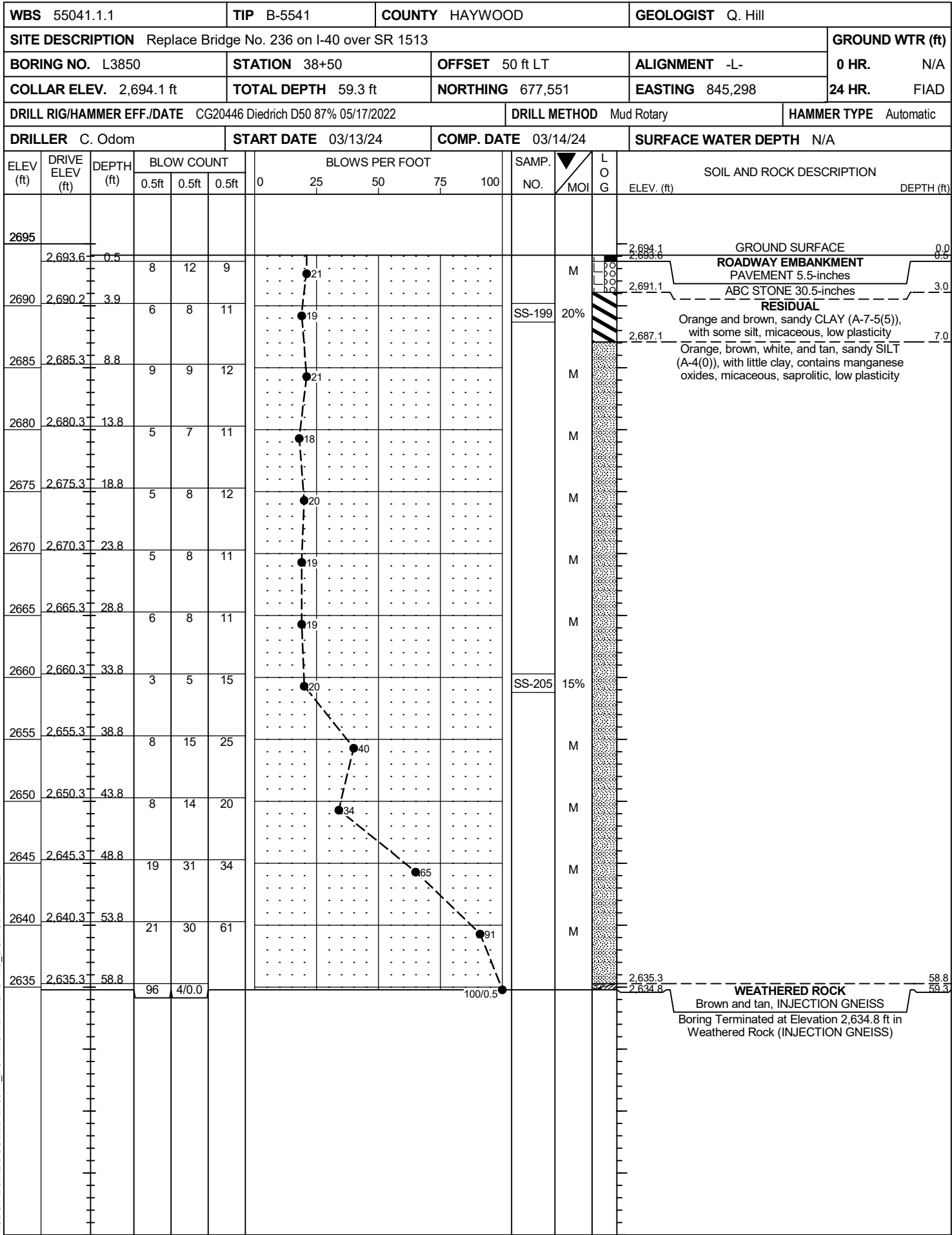
GEOTECHNICAL BORING REPORT
BORE LOG

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill												
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513											GROUND WTR (ft)										
BORING NO. L3650			STATION 36+50			OFFSET 85 ft RT			ALIGNMENT -L-			0 HR. Dry									
COLLAR ELEV. 2,694.8 ft			TOTAL DEPTH 20.0 ft			NORTHING 677,378			EASTING 845,130			24 HR. Dry									
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic												
DRILLER C. Odom			START DATE 02/28/24			COMP. DATE 02/29/24			SURFACE WATER DEPTH N/A												
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION							
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)						
2695	2,694.8	0.0	1	2	2									2,694.8	0.0						
2690	2,691.3	3.5	6	7	9											M				2,691.8	3.0
2685	2,688.8	6.0	2	5	7											M					
2685	2,686.3	8.5	4	7	9						SS-16	23%									
2680	2,681.3	13.5	4	7	8								M								
2675	2,676.3	18.5	5	7	11								M								
														Boring Terminated at Elevation 2,674.8 ft in sandy SILT (RESIDUAL)							

WBS 55041.1.1			TIP B-5541			COUNTY HAYWOOD			GEOLOGIST Q. Hill						
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513									GROUND WTR (ft)						
BORING NO. L3750			STATION 37+50			OFFSET 135 ft LT			ALIGNMENT -L-			0 HR. 57.1			
COLLAR ELEV. 2,724.5 ft			TOTAL DEPTH 70.0 ft			NORTHING 677,614			EASTING 845,182			24 HR. 50.3			
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER D. Underwood			START DATE 03/07/24			COMP. DATE 03/08/24			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2725	2,724.5	0.0	2	3	5									GROUND SURFACE 0.0	
2720	2,721.0	3.5	3	5	8									RESIDUAL Orange and brown, silty and clayey SAND (A-2-7(0)), micaceous	
	2,718.5	6.0	15	18	17										
2715	2,716.0	8.5	17	14	17						SS-151	12%		Orange, tan, white, and brown, sandy SILT (A-4(0)), with trace rock fragments and little clay, micaceous	6.0
	2,711.0	13.5	18	14	16										
2710	2,706.0	18.5	38	25	26										
	2,701.0	23.5	10	11	13										
2695	2,696.0	28.5	16	9	12									Tan and brown, silty SAND (A-2-4(0)), with trace rock fragments and little clay, micaceous	32.0
	2,691.0	33.5	15	16	18						SS-157	14%			
2685	2,686.0	38.5	21	11	14										
	2,681.0	43.5	3	9	8										
2675	2,676.0	48.5	13	21	24									Brown, orange, tan, and white, sandy SILT (A-4), with little clay and rock fragments, micaceous, saprolitic, low plasticity	47.0
	2,671.0	53.5	7	14	20										
2665	2,666.0	58.5	13	17	19										
	2,661.0	63.5	16	27	45										
2655	2,656.0	68.5	18	26	74									2,654.5	70.0
Boring Terminated at Elevation 2,654.5 ft in sandy SILT (RESIDUAL)															

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

GEOTECHNICAL BORING REPORT
BORE LOG



NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 55041.1.1		TIP B-5541		COUNTY HAYWOOD		GEOLOGIST Q. Hill								
SITE DESCRIPTION Replace Bridge No. 236 on I-40 over SR 1513						GROUND WTR (ft)								
BORING NO. Y12600		STATION 26+00		OFFSET 15 ft LT		ALIGNMENT -Y1-								
COLLAR ELEV. 2,653.5 ft		TOTAL DEPTH 15.0 ft		NORTHING 677,640		EASTING 844,531								
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/17/2022				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic								
DRILLER C. Odom		START DATE 02/29/24		COMP. DATE 03/01/24		SURFACE WATER DEPTH N/A								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft) DEPTH (ft)	
2655														
	2,653.5	0.0	2	2	3									2,653.5 GROUND SURFACE 0.0
2650	2,650.0	3.5	7	16	23							SS-79	21%	2,649.6 RESIDUAL Orange and brown, silty SAND (A-2-4(0)), micaceous 3.9
	2,647.5	6.0	10	14	30							M		2,645.5 Red and brown, clayey SAND (A-2-6(0)), with little rock fragments, micaceous 8.0
2645	2,645.0	8.5	21	25	35							M		2,638.5 Brown, sandy SILT (A-4), with little rock fragments, micaceous, low plasticity 15.0
2640	2,640.0	13.5	7	9	9							M		
														Boring Terminated at Elevation 2,638.5 ft in sandy SILT (RESIDUAL)
														2-inches Topsoil
														Other Samples: BS-3 (0.0 - 10.0)

NCDOT BORE DOUBLE B-5541_BORING PLAN.GPJ NC_DOT.GDT 4/22/24

CONE PENETROMETER RESULTS

Page 21

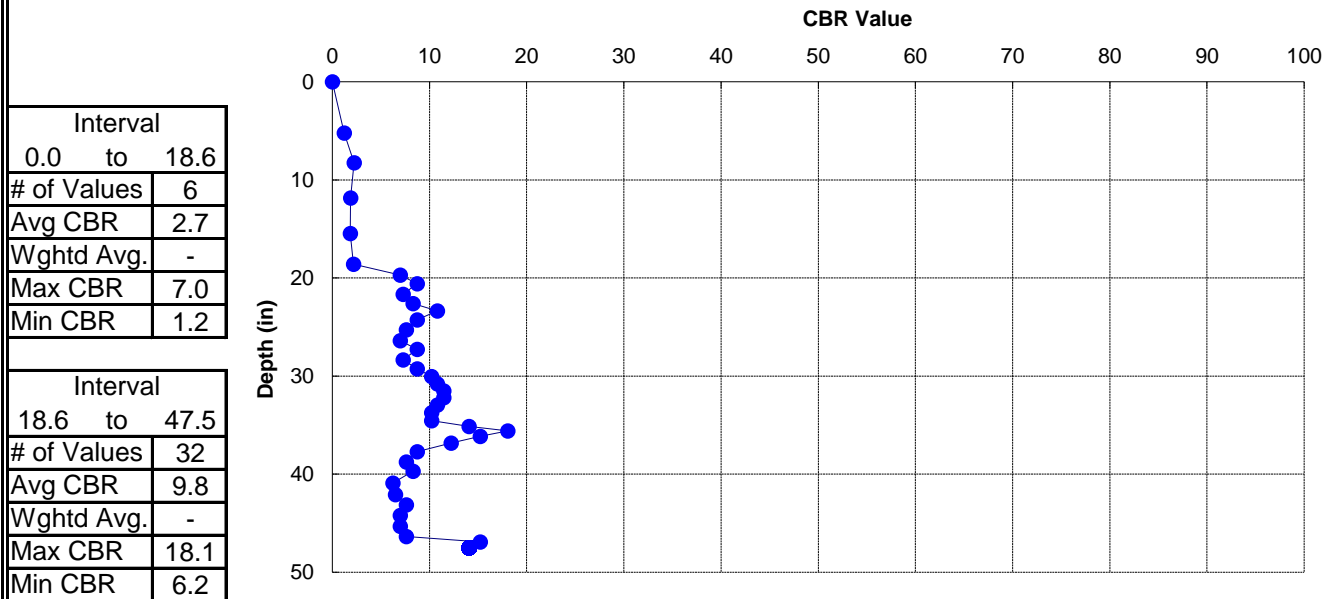
PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L1700HA

1

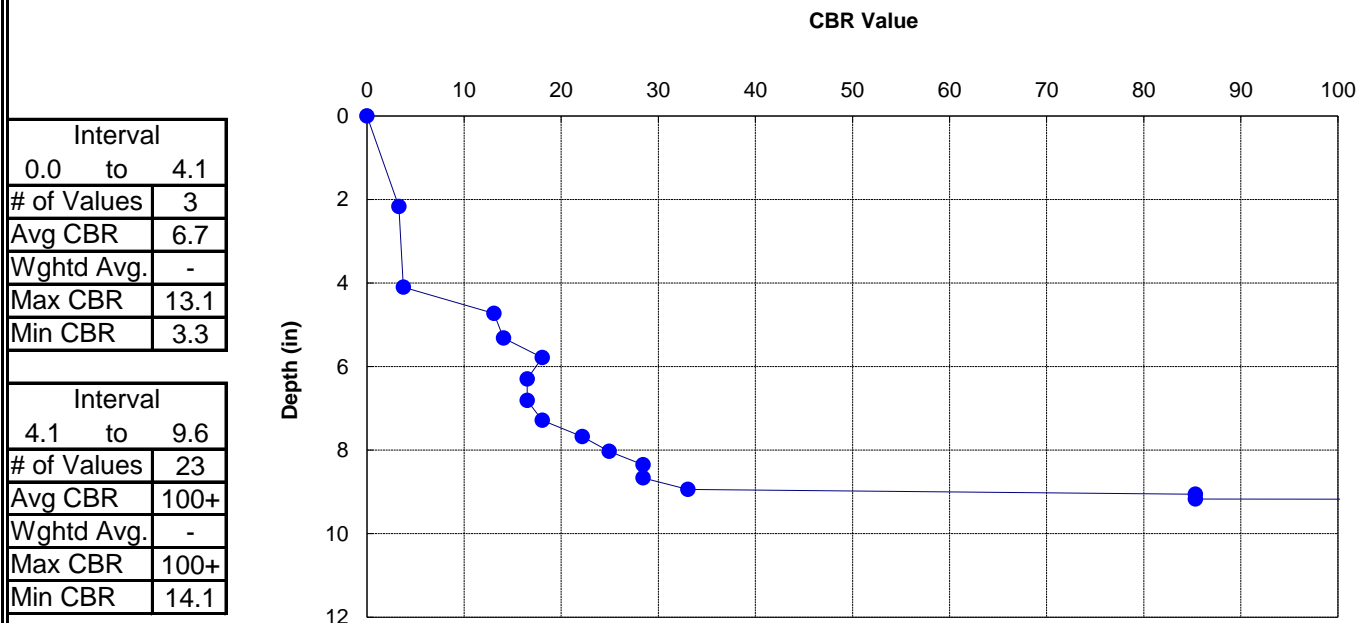
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L3500HA-A

2

Datum = Exstg Grd Lvl 3/14/24



CONE PENETROMETER RESULTS

Page 22

PROJECT NO.	B-5541
PROJECT NAME	2-Lane Extension of SR 1630
ROUTE	Replace Bridge No. 236 on I-40
COUNTY	Haywood

GEOLOGIST	Quinton Hill
GEOTECH(S)	C. Odom
	Z. Taylor
	N/A

L3500HA-B

3

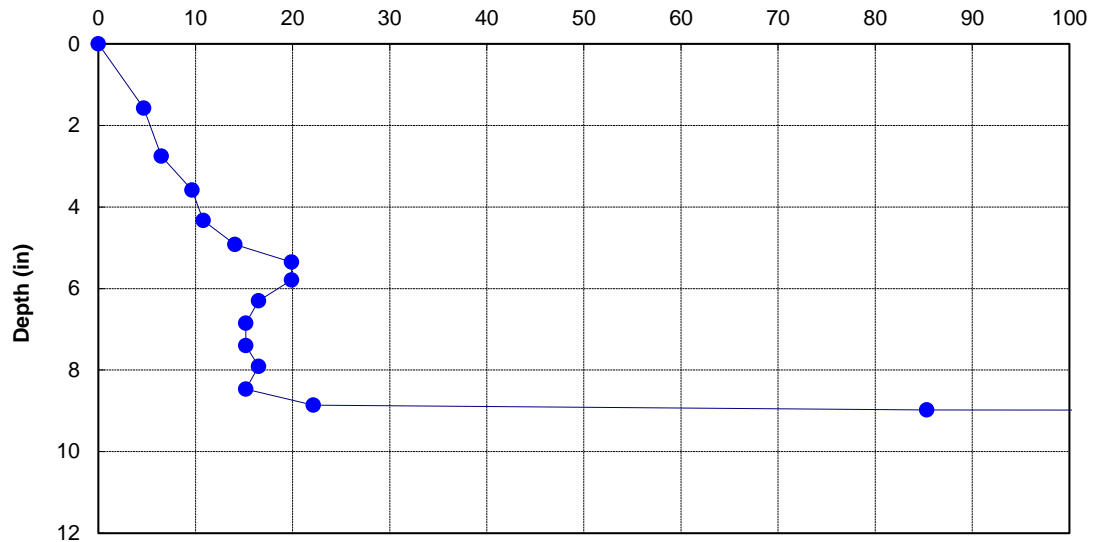
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3/14/24

CBR Value

Interval	
0.0	to 4.9
# of Values	5
Avg CBR	9.1
Wghtd Avg.	-
Max CBR	14.1
Min CBR	4.7

Interval	
4.9	to 9.5
# of Values	50
Avg CBR	100+
Wghtd Avg.	-
Max CBR	100+
Min CBR	15.2



REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY															
SOIL TEST RESULTS -L-															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-15	17+00	45' LT	1.5-2.5	A-7-5 (7)	43	13	17.2	29	24.4	29.5	99	87.9	59.7	21	-
S-16	17+00	45' LT	6.5-7.5	A-4 (0)	NP	NP	24.6	28.7	27.2	19.5	99.8	81.2	53.3	15	-
SS-57	19+00	75' RT	0.0-1.5	A-2-6 (0)	40	11	47.9	21.4	1.9	28.8	74.3	43.3	26.4	22	-
SS-65	21+00	100' RT	8.5-10.0	A-4 (0)	NP	NP	19.2	36.2	30.1	14.6	95.8	85.2	51.1	14	-
SS-68	21+00	100' RT	23.5-25.0	A-7-6 (10)	44	19	19.9	19.5	29.9	30.7	94.2	85	55.3	25	-
SS-74	23+00	95' RT	13.5-15.0	A-2-6 (0)	40	15	52.2	24.9	2.8	20.1	69.5	36.8	21	11	-
SS-76	23+00	95' RT	23.5-25.0	A-6 (2)	39	14	36.9	20.8	12	30.2	84.8	56.7	41.1	18	-
BS-2	25+00	80' RT	0.0-10.0	A-6 (5)	40	16	30.6	17.5	15.8	36.1	89.3	67.3	50.3	20	-
SS-54	25+00	80' RT	13.5-15.0	A-6 (5)	36	13	25.5	20.7	22	31.8	92.4	72.2	56.1	19	-
SS-47	27+00	70' RT	13.5-15.0	A-4 (0)	NP	NP	33	33	18.5	15.6	98.9	77.5	40	12	-
SS-167	28+50	11' RT	6.0-7.5	A-4 (0)	35	8	14.5	28.9	24.4	24.4	92.3	77.8	48.9	21	-
SS-170	28+50	11' RT	18.5-20.0	A-7-6 (11)	43	20	17.2	25.1	18.1	39.6	99.2	87.3	62.7	19	-
SS-34	28+50_R	95' RT	0.0-1.5	A-2-7 (0)	46	16	40.4	29.6	14.6	15.4	90.1	62.3	22	17	-
SS-40	28+50_R	95' RT	23.5-25.0	A-4 (0)	NP	NP	33.6	30.9	13.1	22.3	94.8	74.4	38.5	29	-
SS-84	30+50	105' LT	0.0-1.5	A-5 (4)	45	10	18.8	33.6	23.6	24.1	99.6	89.6	54.3	23	-
SS-85	30+50	105' LT	3.5-5.0	A-4 (2)	39	7	25.4	30.2	21.7	22.7	97.9	80.8	49.2	22	-
ST-1	30+50	105' LT	13.0-15.0	A-2-7 (1)	41	20	51.2	14.7	4.2	29.9	88.4	48.5	32.5	21	-
SS-89	30+50	105' LT	18.5-20.0	A-4 (2)	34	10	20.9	31.3	15.6	32.1	92.1	80.9	49.1	18	-
SS-97	30+50_R	107' RT	0.0-1.5	A-7-5 (8)	52	16	13.5	38.8	22.2	25.5	99.8	92.8	56.3	35	-
ST-2	30+50_R	107' RT	8.0-10.0	A-4 (0)	NP	NP	33.2	42	10.9	13.9	98.9	80.5	36.4	23	-
SS-101	30+50_R	107' RT	13.5-15.0	A-4 (0)	NP	NP	26.3	35.7	17.4	20.6	97.8	81.6	43.6	26	-
SS-190	30+75	14' LT	42.9-44.4	A-4 (0)	45	11	19.6	32	23.3	23.3	98.2	78.6	46.6	33	-
SS-191	30+75	14' LT	47.9-49.4	A-7-6 (9)	43	17	8.3	35.7	20.3	35.8	99.6	96.5	62.7	23	-
ST-3	31+00_L	158' LT	8.6-10.6	A-2-4 (0)	NP	NP	44.4	39.4	5.9	10.4	85.4	57.5	19.1	21	-
SS-105	31+00_R	117' RT	0.0-1.5	A-7-5 (7)	46	15	18.6	26.6	23	31.8	95.2	83.4	58	27	-
SS-212	31+50	52' LT	3.5-5.0	A-4 (0)	NP	NP	17.6	34.9	21.3	26.3	96.9	87.1	53.4	23	-
SS-221	31+50	52' LT	47.5-49.0	A-4 (1)	34	8	28.9	29.8	15.2	26.1	92.3	71	43.5	22	-
SS-114	31+50_R	114' RT	6.0-7.5	A-2-4 (0)	NP	NP	36.3	32.9	8	22.8	89.6	64	33.4	49	-
SS-129	31+50_L	202' LT	3.5-5.0	A-2-4 (0)	NP	NP	34.2	38.2	13.3	14.4	96.5	79.7	31.6	20	-
SS-130	31+50_L	202' LT	6.0-7.5	A-4 (0)	NP	NP	23.9	27.8	20.3	28	94.1	77.6	50.2	46	-
SS-134	32+00_L	158' LT	3.5-5.0	A-2-6 (0)	40	12	34.5	33.3	1.5	30.6	90	66.7	34.2	21	-
SS-135	32+00_L	158' LT	6.0-7.5	A-2-4 (0)	NP	NP	37.9	35.1	16.4	10.7	98.1	76.1	31.8	23	-
SS-26	32+50_R	90' RT	0.0-1.5	A-4 (0)	NP	NP	35	27.8	9.2	28	93.1	71.2	39.1	21	-
SS-23	34+50	80' RT	13.5-15.0	A-4 (0)	NP	NP	23.6	30	32.6	13.8	97.4	85.9	44.3	12	-
BS-1	34+50	80' RT	0.0-10.0	A-7-5 (4)	52	15	30.4	27	18.3	24.3	96.2	75.7	46	19	-
SS-142	35+00	100' LT	3.5-5.0	A-2-7 (1)	49	19	37.5	26.8	5.3	30.4	84.9	61.5	33.6	20	-
SS-145	35+00	100' LT	13.5-15.0	A-4 (0)	NP	NP	37.9	28.7	24.2	9.2	92.8	67.4	37.1	12	-
SS-16	36+50	85' RT	8.5-10.0	A-4 (0)	NP	NP	19.8	39.3	26.8	14.1	99.5	88.2	48.3	23	-
SS-151	37+50	135' LT	6.0-7.5	A-4 (0)	NP	NP	19.7	35.4	31.2	13.7	98.5	88.5	50.5	12	-
SS-157	37+50	135' LT	33.5-35.0	A-2-4 (0)	NP	NP	40	39.5	5.6	14.9	96.6	77.4	26.4	14	-
SS-199	38+50	50' LT	3.9-5.4	A-7-5 (5)	41	11	16.4	34.5	24.4	24.8	99.7	89.2	57.3	20	-
SS-205	38+50	50' LT	33.8-35.3	A-4 (0)	NP	NP	24.4	29.8	30.6	15.1	98.7	81.7	51.9	24	-

REPLACE BRIDGE NO. 236 ON I-40 OVER SR 1513 IN HAYWOOD COUNTY															
SOIL TEST RESULTS -Y1-															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-2	20+50	75' RT	3.5-5.0	A-1-b (0)	NP	NP	66	9.1	4.5	20.4	54.1	21.8	14.1	38	-
SS-3	20+50	75' RT	6.0-7.5	A-2-4 (0)	NP	NP	38.5	35.7	16.6	9.2	99	79.3	30.8	29	-
SS-4	20+50	75' RT	8.5-10.0	A-2-4 (0)	39	7	40.5	32.5	4	23	86.4	62.2	28.3	33	-
SS-79	26+00	15' LT	0.0-1.5	A-2-4 (0)	35	5	35.9	30.4	8.9	24.9	89.6	66.7	35.2	21	-
BS-3	26+00	15' LT	0.0-10.0	A-2-6 (0)	37	12	48.6	24	5	22.4	64.9	40.3	20.5	14	-