

August 4, 2023

MEMORANDUM TO: Clark S. Morrison, PhD, PE
State Pavement Design Engineer
North Carolina Department of Transportation

FROM: Joshua D. Fregosi, PE
Program Manager
Kleinfelder, Inc

STATE PROJECT: 67015.1.1 (BR-0015)

COUNTY: Davidson

DESCRIPTION: Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

SUBJECT: Pavement and Subgrade Investigation Report

Kleinfelder, Inc. has completed the evaluation of the pavement and subgrade investigation for this project and presents the following.

This project consists of the widening of US 29/US 70 (-L-) and replacement of Bridge No. 67 and No. 68 over SR 1192 (-Y1-). At the project location, US 29/US 70 is a four-lane highway consisting of two lanes in the northbound and southbound direction with a grass median dividing the highway. Additionally, the project consists of the widening of SR 1192 (-Y1-), Forest Rose Drive (-DRW1-), and US 29/US 70 northbound exit ramps (-RPD-, -RPD_RT-) to National Boulevard as well as a realignment of Murphy Drive (-Y2-), onramp to US 29/US70 southbound (-RPB-), and US 29/US 70 southbound exit ramp (-LPB-) to SR 1192.

The soils encountered beneath the existing roadway consisted of both roadway embankment and residual soils. Predominant soil types encountered consisted of silty clays (A-7) with lesser amounts of sandy silts (A-4).

Anticipated borrow will likely consist of soil types listed above that meet the Piedmont and Western Area criteria for Acceptance of Borrow Material, Table 1018-1 of the 2024 Standard Specification.

The existing pavement was observed to be in good condition on US 29/US 70 (-L-). Surface pavement distress was not observed on US 29/US 70. The existing pavement was observed to be in worse condition on West 5th Avenue (-Y1-) and Murphy Drive (-Y2-). Surface pavement distress on the West 5th Avenue is primarily characterized by low severity transverse and longitudinal cracking, moderate to high severity fatigue cracking, and rutting (with and without spalling). Transverse, longitudinal, and fatigue crack widths ranged from 3 to 10 mm. Rutting was encountered ranging from 3 to 6 mm deep. Surface pavement distress on Murphy Drive is primarily characterized by moderate to high severity fatigue cracking with spalling. Fatigue crack widths ranged from 6 to 13 mm. Rutting was encountered ranging from 3 to 6 mm deep.



The length of the mainline (US 29/US 70, -L-) is approximately 0.71 mile.

Areas of Special Geotechnical Interest

1. Highly Plastic Soils Encountered Beneath the Existing Roadway and Grass Median

Highly plastic soils (PI > 25) were encountered at the following locations based on laboratory test results on the soils:

LINE	STATION AND OFFSET	PI
-L-	18+30 WB ISS	48
-L-	19+60 EB ISS	34
-L-	25+90 WB ISS	35
-L-	31+50 WB OSS	43
-L-	32+75 EB DECEL LN	33
-L-	38+70 WB ISS	39
-L-	42+80 EB ISL	37
-Y1-	15+70 RT LN	35
-Y2-	16+20 LT LN	34

2. Groundwater:

Groundwater was not observed during this investigation.

3. Samples Classified as Wet

The subgrade soils at the following locations were classified as wet based on the visual inspection and laboratory test results on the soils:

LINE	STATION AND OFFSET	MOISTURE (%)
-L-	31+50 WB OSS	35.0
-L-	38+70 WB ISS	25.9
-Y1-	15+70 RT LN	24.8
-Y2-	16+20 LT LN	32.8



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08/30/2023

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Program Manager

Document Not Considered Final Unless All Signatures Are Completed

DHK/JDF:jrs

ATTACHMENT: Pavement and Subgrade Investigation
Pavement Core Evaluation

REFERENCE: BR-0015

PROJECT: 67015

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY DAVIDSON
PROJECT DESCRIPTION BRIDGE NO. 67 AND NO. 68
REPLACEMENTS ON US 29/US 70 NB & SB OVER
SR 1192 (W. 5TH AVENUE)

PAVEMENT AND SUBGRADE INVESTIGATION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0015	1	

CAUTION NOTICE

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GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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PERSONNEL

D. KUBINSKI

M. FOSTER

TRIGON EXPLORATION

INVESTIGATED BY KLEINFELDER, INC

DRAWN BY D. KUBINSKI

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SUBMITTED BY KLEINFELDER, INC

DATE AUGUST 2023

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Daniel H. Kubinski 8/30/2023

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DATE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

***NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS***

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)							ORGANIC MATERIALS																											
GROUP CLASS.	A-1	A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5																														
CLASS.	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7			A-7-5	A-7-6	A-3	A-6, A-7																													
SYMBOL																																										
% PASSING #10	50 MX	30 MX	50 MX	51 MN																																						
% #40	30 MX	30 MX	50 MX	10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN																														
% #200	15 MX	25 MX	10 MX	10 MX	10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN																														
MATERIAL PASSING #40 PI	<table><tr><td>—</td><td>—</td><td>40 MX</td><td>41 MN</td><td>40 MX</td><td>41 MN</td><td>40 MX</td><td>41 MN</td><td>40 MX</td><td>41 MN</td><td></td><td></td><td></td><td></td></tr><tr><td>6 MX</td><td>NP</td><td>10 MX</td><td>10 MX</td><td>11 MN</td><td>11 MN</td><td>10 MX</td><td>10 MX</td><td>11 MN</td><td>11 MN</td><td></td><td></td><td></td><td></td></tr></table>														—	—	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN					6 MX	NP	10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN				
—	—	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN																																	
6 MX	NP	10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN																																	
GROUP INDEX	0	0	0	4 MX				8 MX	12 MX	16 MX	ND MX																															
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS, GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS																															
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE																												
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30																																										

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE	< 4	
	LOOSE	4 TO 10	
	MEDIUM DENSE	10 TO 30	N/A
	DENSE	30 TO 50	
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT	< 2	< 0.25
	SOFT	2 TO 4	0.25 TO 0.5
	MEDIUM STIFF	4 TO 8	0.5 TO 1.0
	STIFF	8 TO 15	1 TO 2
	VERY STIFF	15 TO 30	2 TO 4
HARD	> 30	> 4	

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.75	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE, SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL } LIQUID LIMIT PL }	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM } OPTIMUM MOISTURE SL }	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:
ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING

STATIC WATER LEVEL AFTER 24 HOURS

PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA

SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION

SOIL SYMBOL

ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT

INFERRED SOIL BOUNDARY

INFERRED ROCK LINE

ALLUVIAL SOIL BOUNDARY

DIP & DIP DIRECTION OF ROCK STRUCTURES

TEST BORING

AUGER BORING

CORE BORING

MONITORING WELL

PIEZOMETER INSTALLATION

SLOPE INDICATOR INSTALLATION

CONE PENETROMETER TEST

SOUNDING ROD

TEST BORING WITH CORE

SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT

UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE

UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.

CRYSTALLINE ROCK (CR)

FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

NON-CRYSTALLINE ROCK (NCR)

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTARY ROCK (CP)

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH

ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.

VERY SLIGHT (V SL.)

ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.

SLIGHT (SL.)

ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.

MODERATE (MOD.)

SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW FLAT. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.

MODERATELY SEVERE (MOD. SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KALINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*

SEVERE (SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*

VERY SEVERE (V SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*

COMPLETE

ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD

CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.

HARD

CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.

MODERATELY HARD

CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.

MEDIUM HARD

CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.

SOFT

CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.

VERY SOFT

CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE

RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.

MODERATELY INDURATED

GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.

INDURATED

GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.

EXTREMELY INDURATED

SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
AQUIFER - A WATER BEARING FORMATION OR STRATA.
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGED FROM PARENT MATERIAL.
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIUOUS STRATUM.
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
ROCK QUALITY DESIGNATION (ROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
STRATA ROCK QUALITY DESIGNATION (SROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: N/A

ELEVATION: N/A FEET

NOTES:
⊗ PAVEMENT CORE WITH KESSLER DUAL MASS DCP TEST
⊕ HAND AUGER WITH KESSLER DUAL MASS DCP TEST
⊙ PAVEMENT CORE

DATE: 8-15-14

ABBREVIATIONS

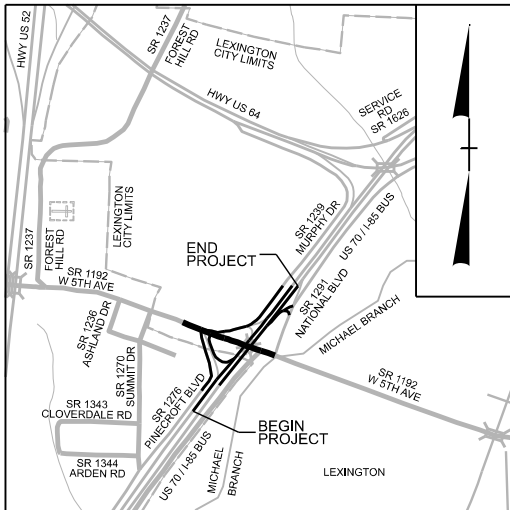
N/A - NOT APPLICABLE
NM - NOT MEASURED
KDCP - KESSLER DUAL MASS DCP
PS - PAVED SHOULDER
WB - WESTBOUND
EB - EASTBOUND
LT - LEFT
RT - RIGHT
OSS - OUTSIDE SHOULDER
ISS - INSIDE SHOULDER
OSL - OUTSIDE LANE
DECEL - DECELERATION
ACCEL - ACCELERATION
LN - LANE
WL - WHITE LINE
YL - YELLOW LINE
AG - AT GRADE
F - FILL
C - CUT
CR - CROWN
SU - SUPERELEVATION

09/08/99

PROJECT: BR-0015

CONTRACT: TBD

See Sheet 1A For Index of Sheets



VICINITY MAP (NTS)



STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

DAVIDSON COUNTY

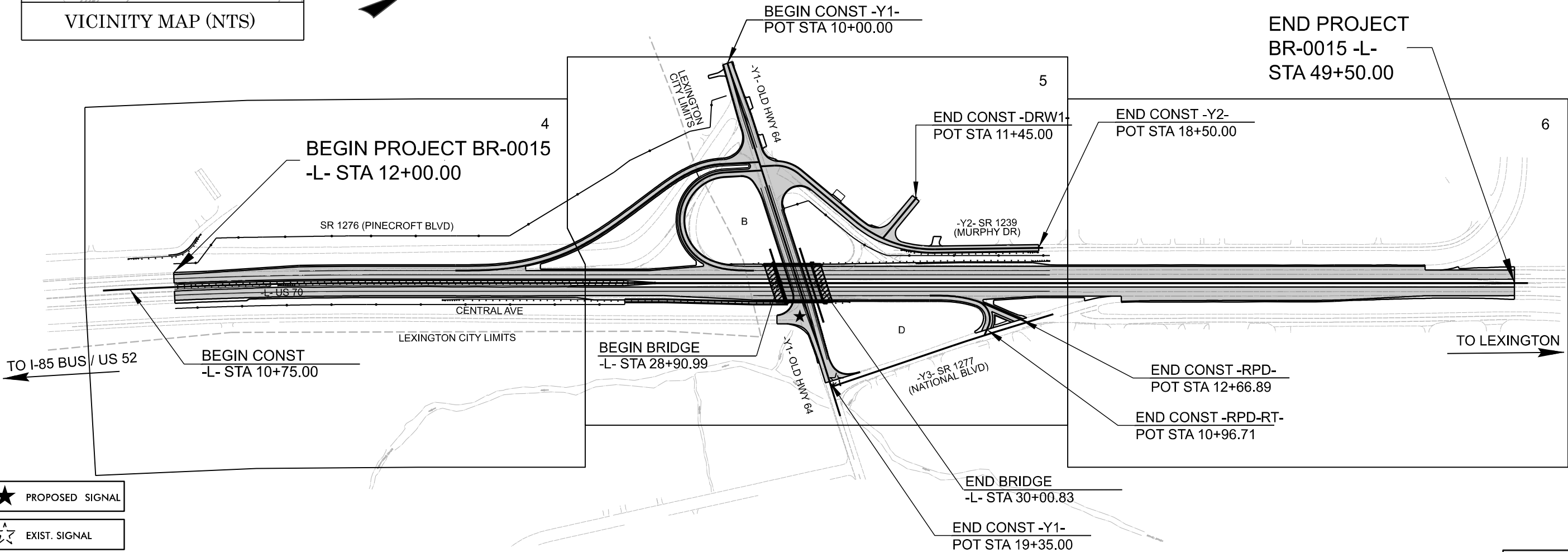
LOCATION: *BRIDGE 280067 AND 280068 REPLACEMENTS ON US 29/US 70.
NB & SB OVER SR 1192 (W. 5TH AVE.)*

TYPE OF WORK: *GRADING, DRAINAGE, PAVING, WIDENING AND STRUCTURES*

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0015	3	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
67015.1.1		PE	
67015.2.1		R/W, UTIL	
67015.3.1		CONST.	

STAGE 2 PLANS
(2RD1)

Plans Developed with
OpenRoads

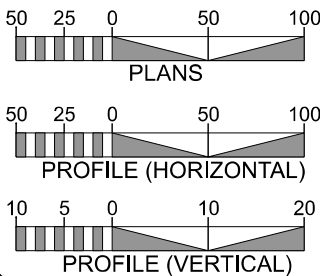


- ★ PROPOSED SIGNAL
- ☆ EXIST. SIGNAL

THIS IS A CONTROL OF ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES.
A PORTION OF THIS PROJECT IS LOCATED WITHIN THE LIMITS OF THE CITY OF LEXINGTON.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.
**A DESIGN EXCEPTION IS REQUIRED FOR -RPD- AND -RPD_RT- MAXIMUM GRADE.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2022 = 14500
ADT 2045 = 18700
K = 8 %
D = 55 %
T = 9 % *
V = 60 MPH
* TTST =5% DUAL 4%
FUNC CLASS =
PRINCIPAL ARTERIAL
STATEWIDE TIER

PROJECT LENGTH

PROJECT LENGTHS FOR PROJECT BR-0015:

LENGTH ROADWAY PROJECT BR-0015	=	0.689 MILES
LENGTH STRUCTURES PROJECT BR-0015	=	0.021 MILES
TOTAL LENGTH PROJECT BR-0015	=	0.710 MILES

NCDOT Contact:

DAVID STUTTS, PE

Prepared in the Office of:



NC FIRM LICENSE No: C-1506
301 Fayetteville St.,
Suite 1500
Raleigh, NC 27601
(919) 882-7839

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
11/24/2023

LETTING DATE:
12/17/2024

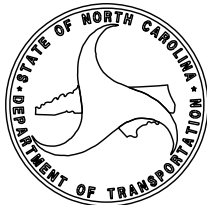
JACOB H. DUKE, P.E.
PROJECT ENGINEER

ANDREA B. GORDON, E.I.
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____
ROADWAY DESIGN ENGINEER

SIGNATURE: _____
P.E.



CORE ID	L_1830_WB_ISS	L_1830_WB_OSL	L_1830_WB_OSS
ASPHALT	6.75"	7.25"	8.50"
CONCRETE	0.00"	9.25"	0.00"
ABC STONE	2.00"	4.50"	0.00"



CORE ID	L_1960_EB_ISS	L_1960_EB_OSL	L_1960_EB_OSS
ASPHALT	7.25"	8.25"	10.25"
CONCRETE	0.00"	9.25"	0.00"
ABC STONE	0.00"	4.75"	11.75"



CORE ID	L_3870_WB_ISS	L_3870_WB_DECEL_LN	L_3870_WB_OSS
ASPHALT	7.00"	11.00"	7.50"
CONCRETE	0.00"	0.00"	0.00"
ABC STONE	0.00"	7.00"	0.00"



CORE ID	L_4280_EB_ISL	L_4280_EB_ACCEL_LN	L_4280_EB_OSS
ASPHALT	8.50"	9.50"	7.00"
CONCRETE	9.50"	0.00"	0.00"
ABC STONE	5.00"	8.50"	0.00"

PAVEMENT INVESTIGATION DATA SHEET

TIP (WBS):	BR-0015 (67015.1.1)	Project:	Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)	Date:	May 2023
County:	Davidson	Route:	I-85 Business, West 5th Avenue, Murphy Drive	Notes By:	Dan Kubinski

		Width (ft)		(ft)		Pavement Section Thickness (in)							Subgrade								
Position (Sta.,Lane,Shldr.)	Cut/Fill	Lane(s)	Shoulder	Offset Distance	Crown or Superelevation	Gross to Top of Soil	Asphalt	Concrete	ABC Stone	PADL	Soil Stabilization	Sand	Moisture Sample No.	Sample No.	Description	AASHTO Classification	Soil Moisture (%)	Probe Depth (ft)			
L_1830_WB_ISS	C	12.0	1.0 PS	0.0 YL	CR	8.75	6.75	0.00	2.00	0.00	0.00	0.00	S-1		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	27.1	5.0	No pavement distress observed	757,186	1,621,293
L_1830_WB_OSL	C	12.0	4.5 PS	3.0 WL		21.00	7.25	9.25	4.50	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,198	1,621,278
L_1830_WB_OSS	C	12.0	4.5 PS	1.0 WL		8.50	8.50	0.00	0.00	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,201	1,621,274
L_1960_EB_ISS	C	12.0	1.5 PS	0.5 YL	CR	7.25	7.25	0.00	0.00	0.00	0.00	0.00	S-2		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	30.1	5.0	No pavement distress observed	757,274	1,621,392
L_1960_EB_OSL	C	12.0	4.0 PS	3.0 WL		22.25	8.25	9.25	4.75	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,260	1,621,409
L_1960_EB_OSS	C	12.0	4.0 PS	2.0 WL		22.00	10.25	0.00	11.75	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,257	1,621,413
L_2590_WB_ISS	AG	12.0	1.0 PS	0.0 YL	CR	8.50	8.50	0.00	0.00	0.00	0.00	0.00	S-3		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	28.4	5.0	No pavement distress observed	757,786	1,621,759
L_3150_WB_OSS	AG	12.0	7.0 PS	4.0 WL	CR	9.25	9.25	0.00	0.00	0.00	0.00	0.00	S-4		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	35.0	5.0	No pavement distress observed	758,245	1,622,081
L_3275_EB_DECEL_LN	AG	12.0	3.5 PS	2.5 WL	CR	17.75	10.75	0.00	7.00	0.00	0.00	0.00	S-5		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	26.7	5.0	No pavement distress observed	758,292	1,622,226
L_3870_WB_ISS	C	12.0	1.0 PS	0.5 YL	CR	7.00	7.00	0.00	0.00	0.00	0.00	0.00	S-6		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-6	25.9	5.0	No pavement distress observed	758,798	1,622,544
L_3870_WB_DECEL_LN	C	12.0	2.0 PS	6.5 WL		18.00	11.00	0.00	7.00	0.00	0.00	0.00	N/A		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	758,817	1,622,519
L_3870_WB_OSS	C	12.0	2.0 PS	1.5 WL		7.50	7.50	0.00	0.00	0.00	0.00	0.00	N/A		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	758,821	1,622,514
L_4000^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	S-18^ N/A		RES: Moist, Olive Gray and Reddish Brown, Moderately Plastic, Silty CLAY (0.0 - 2.2'), Reddish Brown, Slightly Plastic to Non Plastic, Fine Sandy SILT, Trace Mica (2.2 - 6.8')	A-7-5 A-4	33.2 N/A	6.8	Hand auger boring performed for roadway investigation with KDCP test in center grass median	758,893	1,622,633
L_4280_EB_ISL	C	12.0	1.0 PS	1.0' YL	CR	23.00	8.50	9.50	5.00	0.00	0.00	0.00	S-7		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7-5	38.4	5.0	No pavement distress observed	759,107	1,622,815
L_4280_EB_ACCEL_LN	C	12.0	4.0 PS	4.5 WL		18.00	9.50	0.00	8.50	0.00	0.00	0.00	N/A		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	759,088	1,622,839
L_4280_EB_OSS	C	12.0	4.0 PS	2.0 WL		7.00	7.00	0.00	0.00	0.00	0.00	0.00	N/A		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	759,084	1,622,844
L_4600^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	S-20^ N/A		RES: Moist, Reddish Brown, Moderately Plastic, Silty CLAY, Trace Mica (0.0 - 3.0'), Brown, Non Plastic, Coarse to Fine Sandy SILT, Trace Mica (3.0 - 6.5')	A-7-5 A-4	26.8	6.5	Hand auger boring performed for roadway investigation with KDCP test in center grass median	759,367	1,623,000
L_4800^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	N/A N/A		RES: Moist, Yellowish Brown and Reddish Brown, Moderately Plastic, Silty CLAY (0.0 - 1.0'), Brown, Non Plastic, Coarse to Fine Sandy SILT, Trace Mica (1.0 - 6.7')	A-7 A-4	N/A N/A	6.7	Hand auger boring performed for roadway investigation with KDCP test in center grass median	759,525	1,623,123
Y1_1570_LT_PS	F	12.5	4.5 PS	1.0 WL	CR	5.00	5.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	No pavement distress observed	758,098	1,621,920
Y1_1570_LT_LN	F	12.5	4.5 PS	2.5 WL		6.00	6.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	Low transverse and longitudinal cracking (3 - 6 mm crack width); Moderate to high fatigue cracking (3 - 6 mm crack width); Rutting (3 - 6 mm depth)	758,095	1,621,919
Y1_1570_RT_LN	F	12.5	4.0 PS	3.0 WL		7.00	7.00	0.00	0.00	0.00	0.00	0.00	S-8		RE: Wet, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-6	24.8	5.0	Low transverse and longitudinal cracking (3 - 6 mm crack width); Moderate to high fatigue cracking with spalling (3 - 10 mm crack width); Rutting (3 mm depth)	758,077	1,621,912
Y1_1570_RT_PS	F	12.5	4.0 PS	1.0 WL		4.00	4.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	No pavement distress observed	758,073	1,621,911
Y2_1620_LT_LN	C	9.0	0.5 PS	2.0 WL	CR	10.00	3.00	0.00	7.00	0.00	0.00	0.00	S-9		RES: Wet, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	32.8	5.0	Moderate to high fatigue cracking with spalling (6 - 13 mm crack width); Rutting (4 mm depth)	758,472	1,622,178

Note(s):
N/A - Not Applicable
NM - Not Measured
KDCP - Kessler Dual Mass DCP
PS - Paved Shoulder

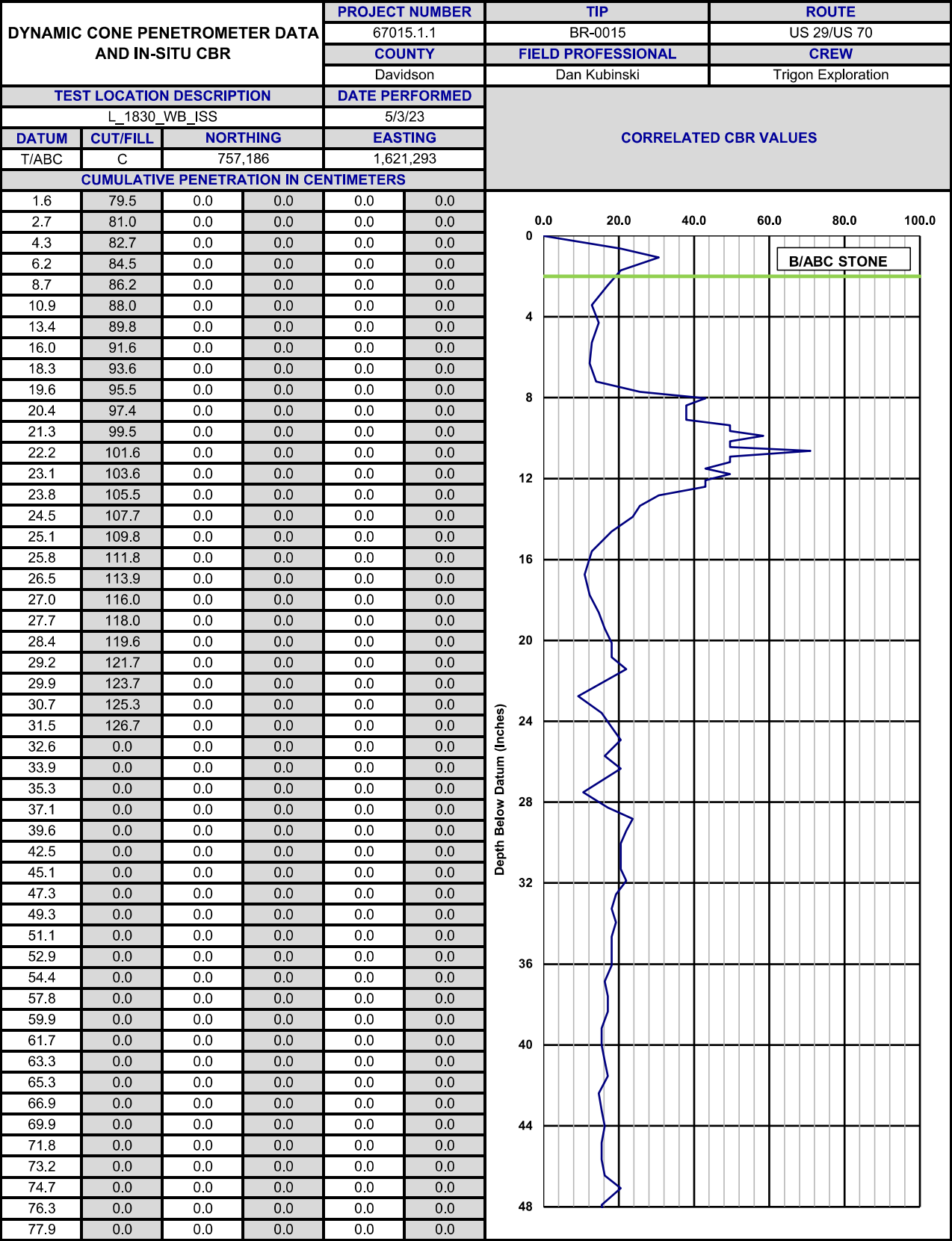
WB - Westbound
EB - Eastbound
LT - Left
RT - Right

OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane
DECEL - Deceleration

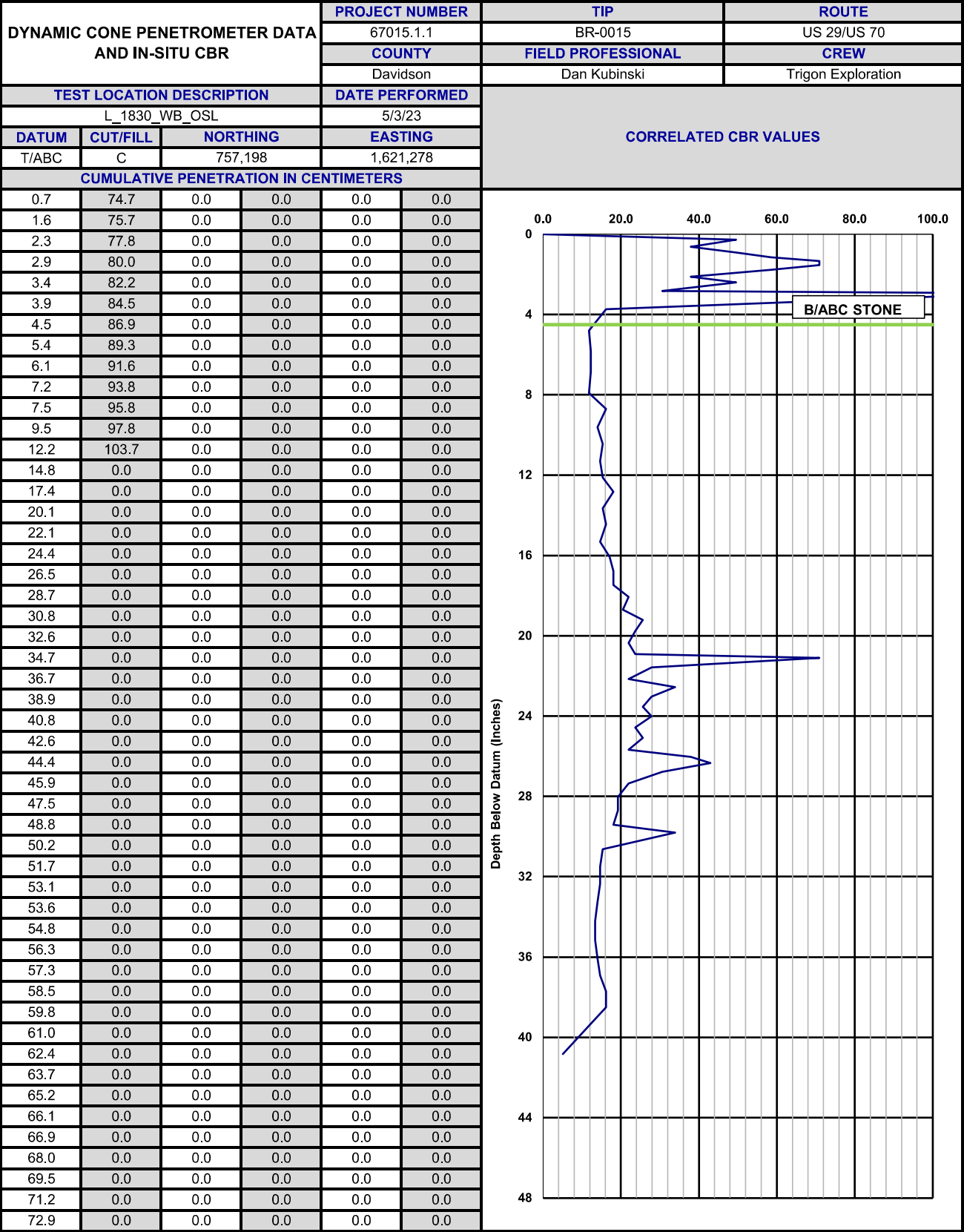
ACCEL - Acceleration
LN - Lane
WL - White Line
YL - Yellow Line

AG - At Grade
F - Fill
C - Cut
CR - Crown

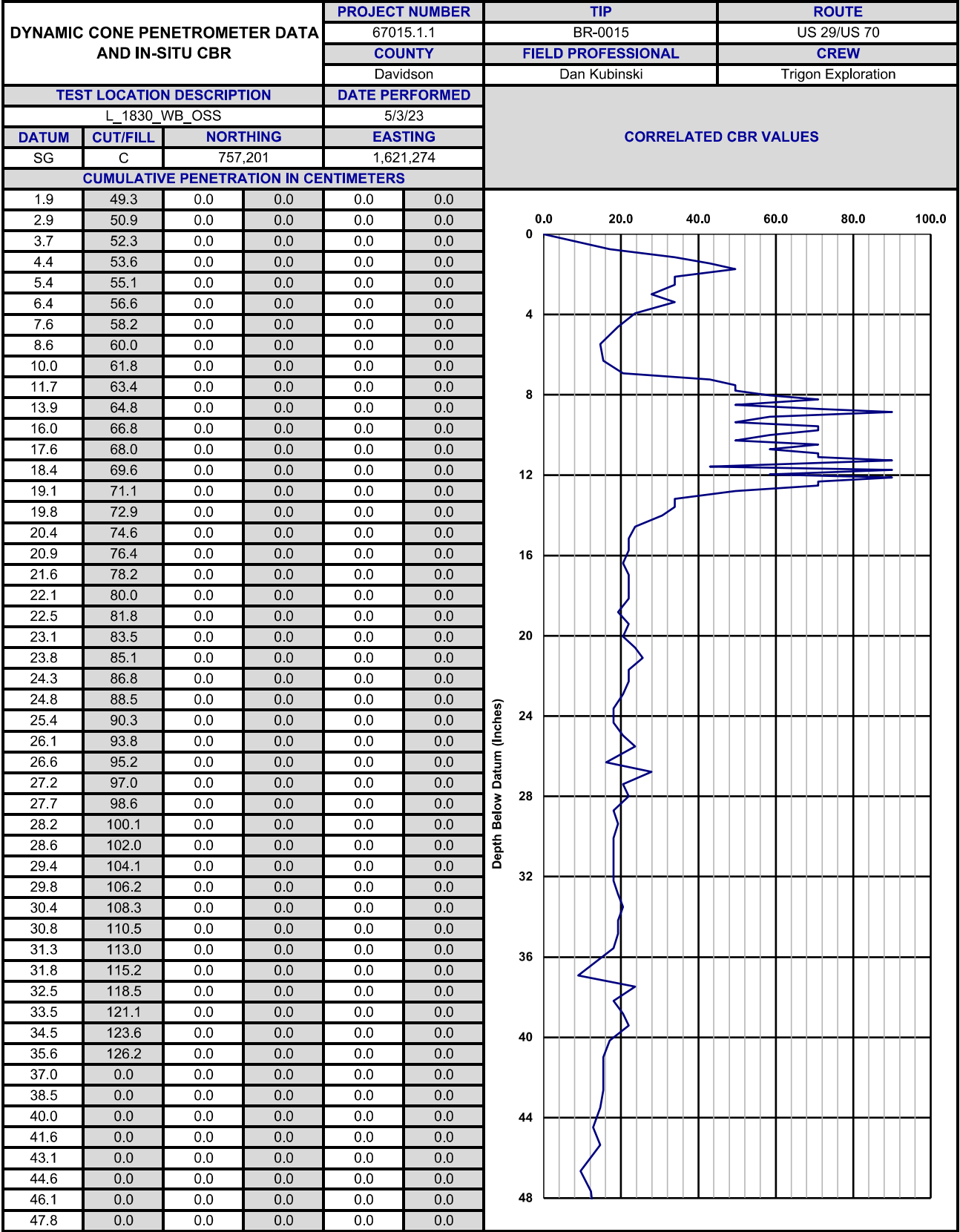
SU - Superelevation
^Based on Roadway Investigation Boring Number
*Based on Roadway Investigation Sample Number



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T/ABC - Top of ABC Stone
B/ABC - Bottom of ABC Stone
Ex. Gr. - Existing Grade



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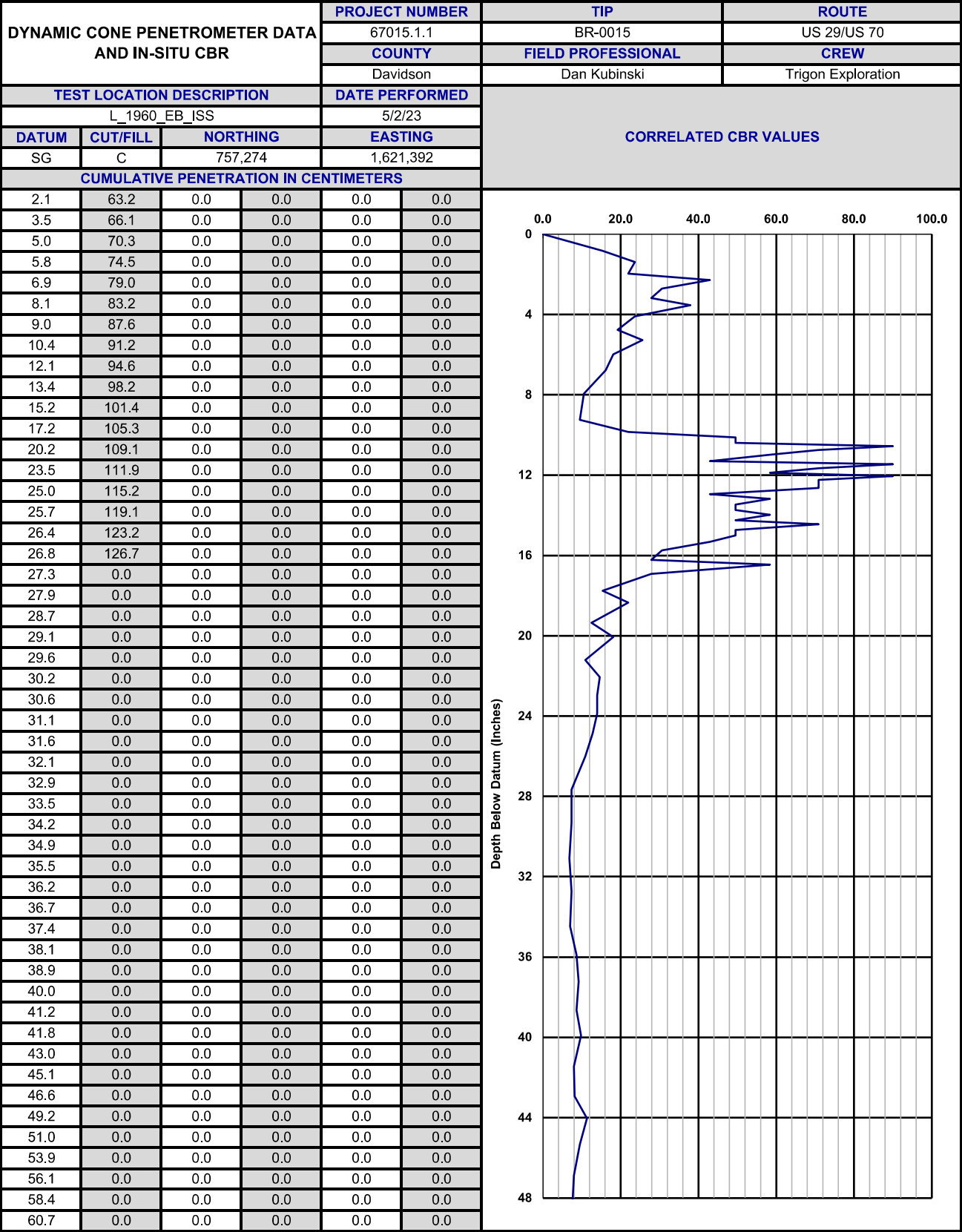


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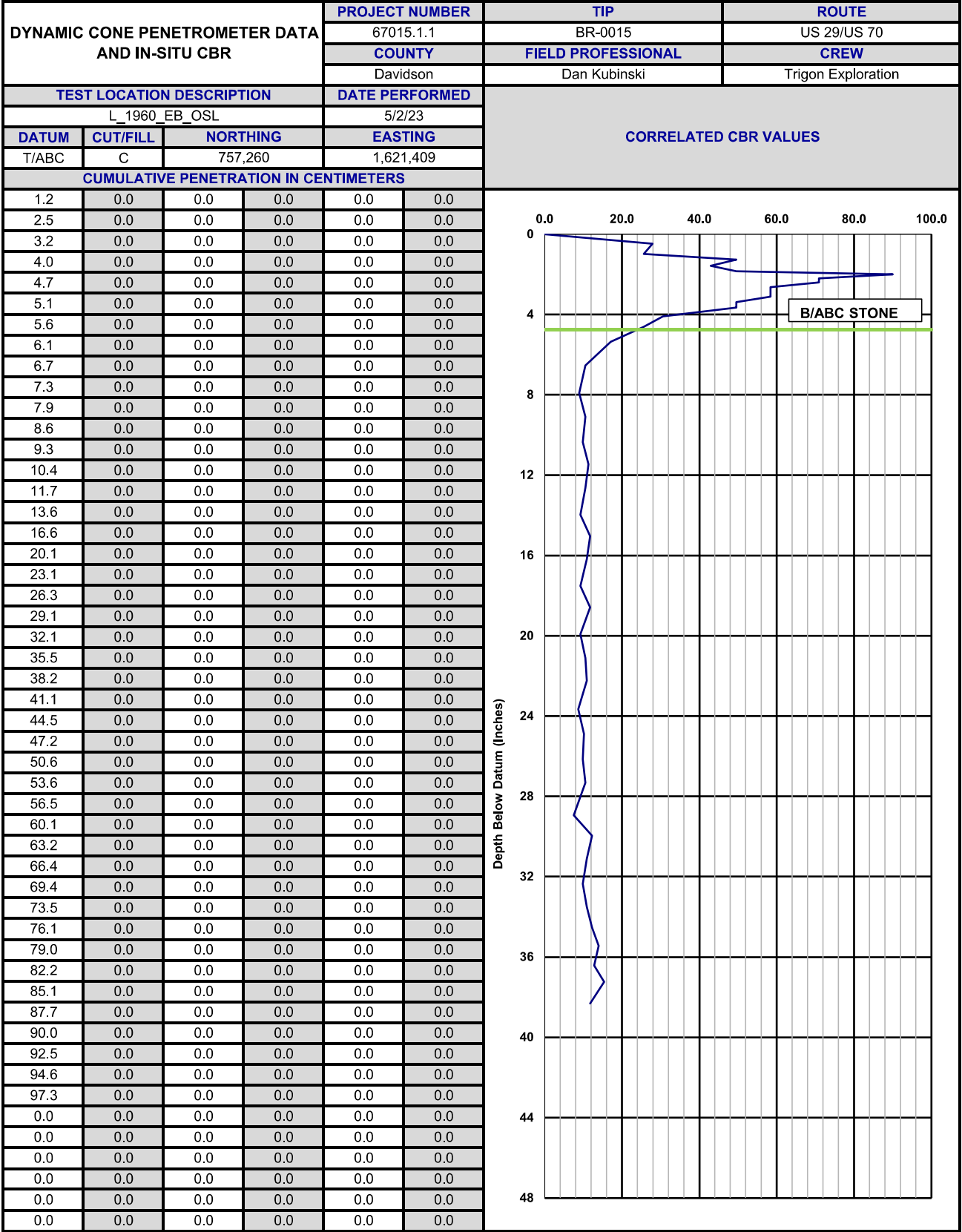


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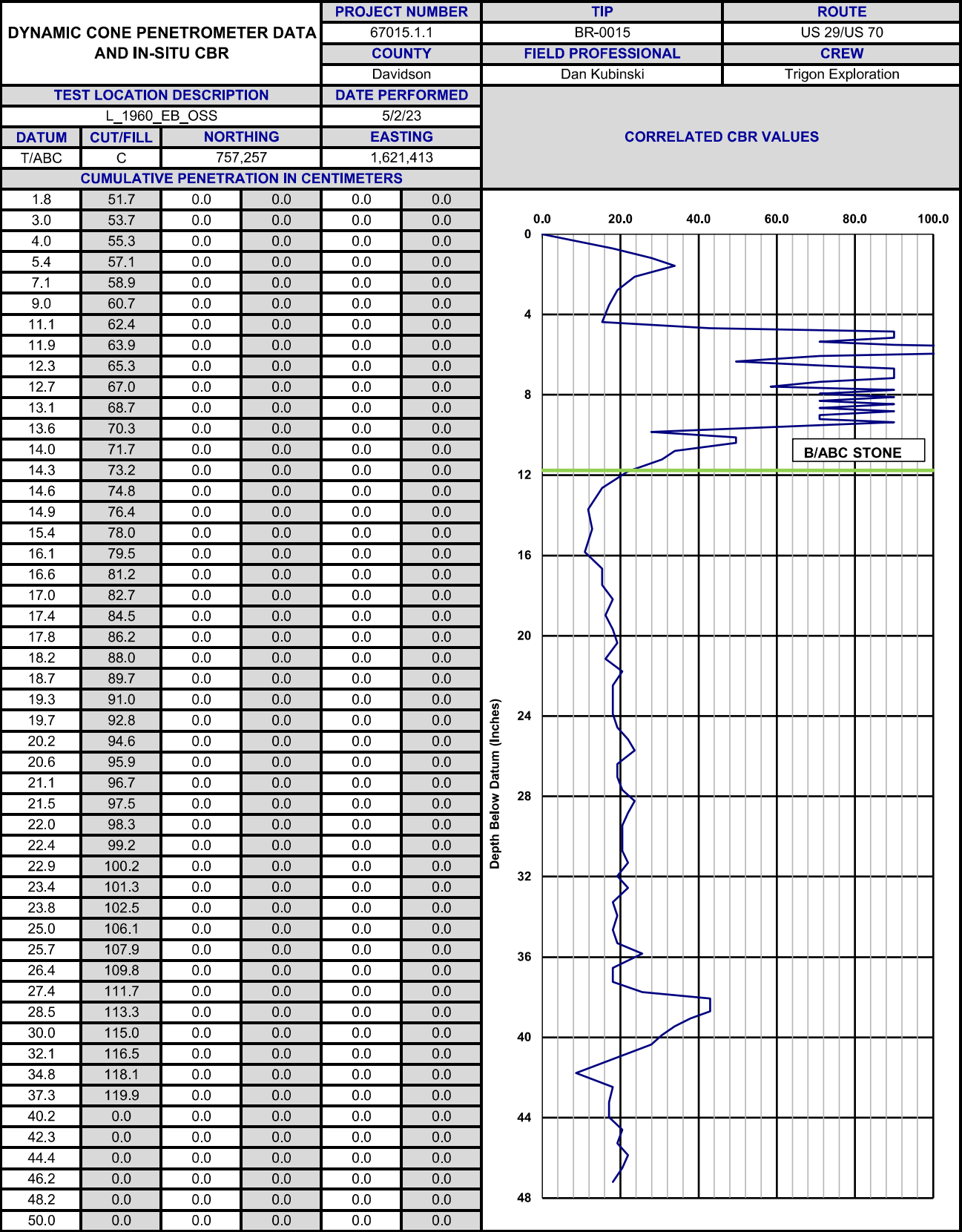
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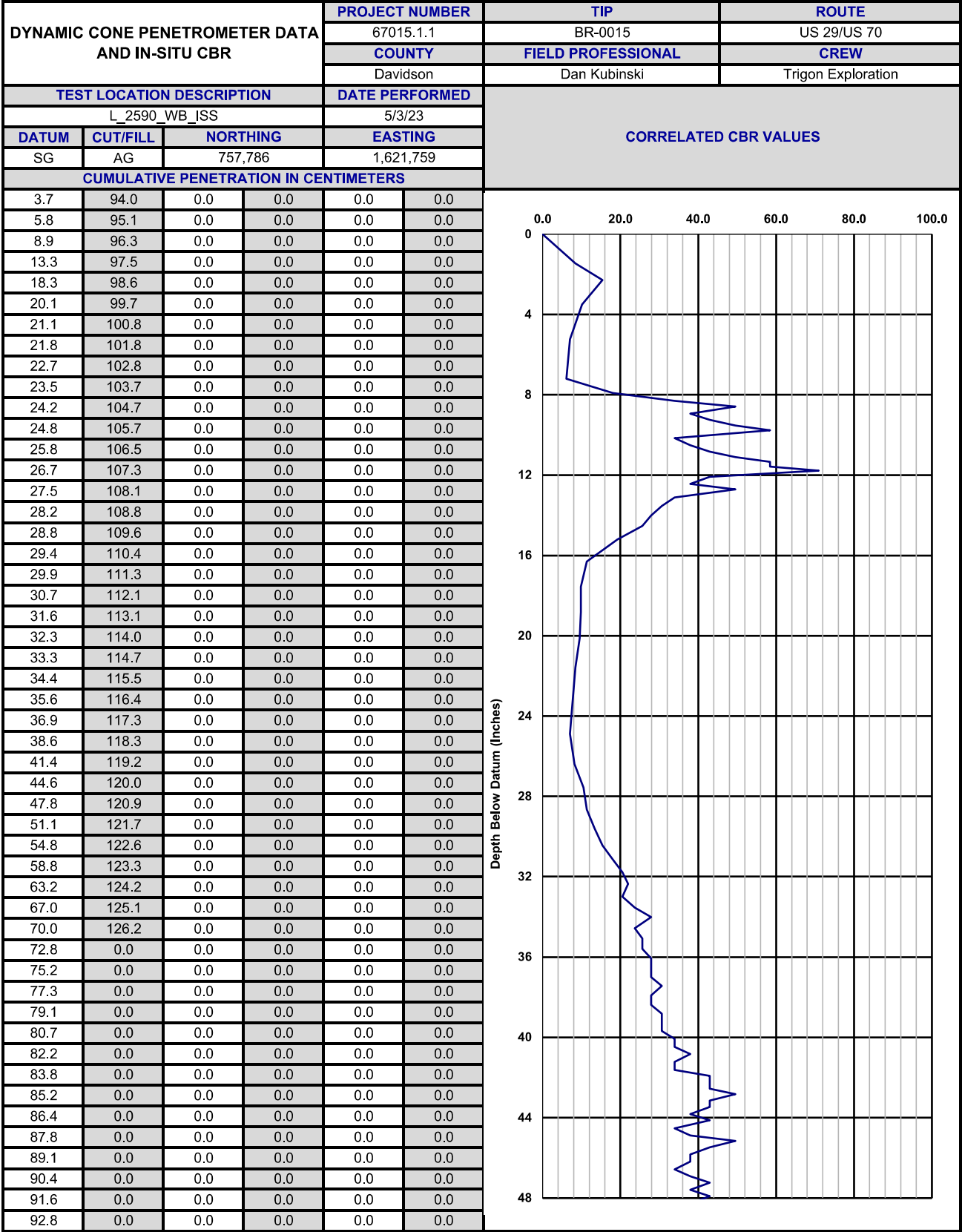
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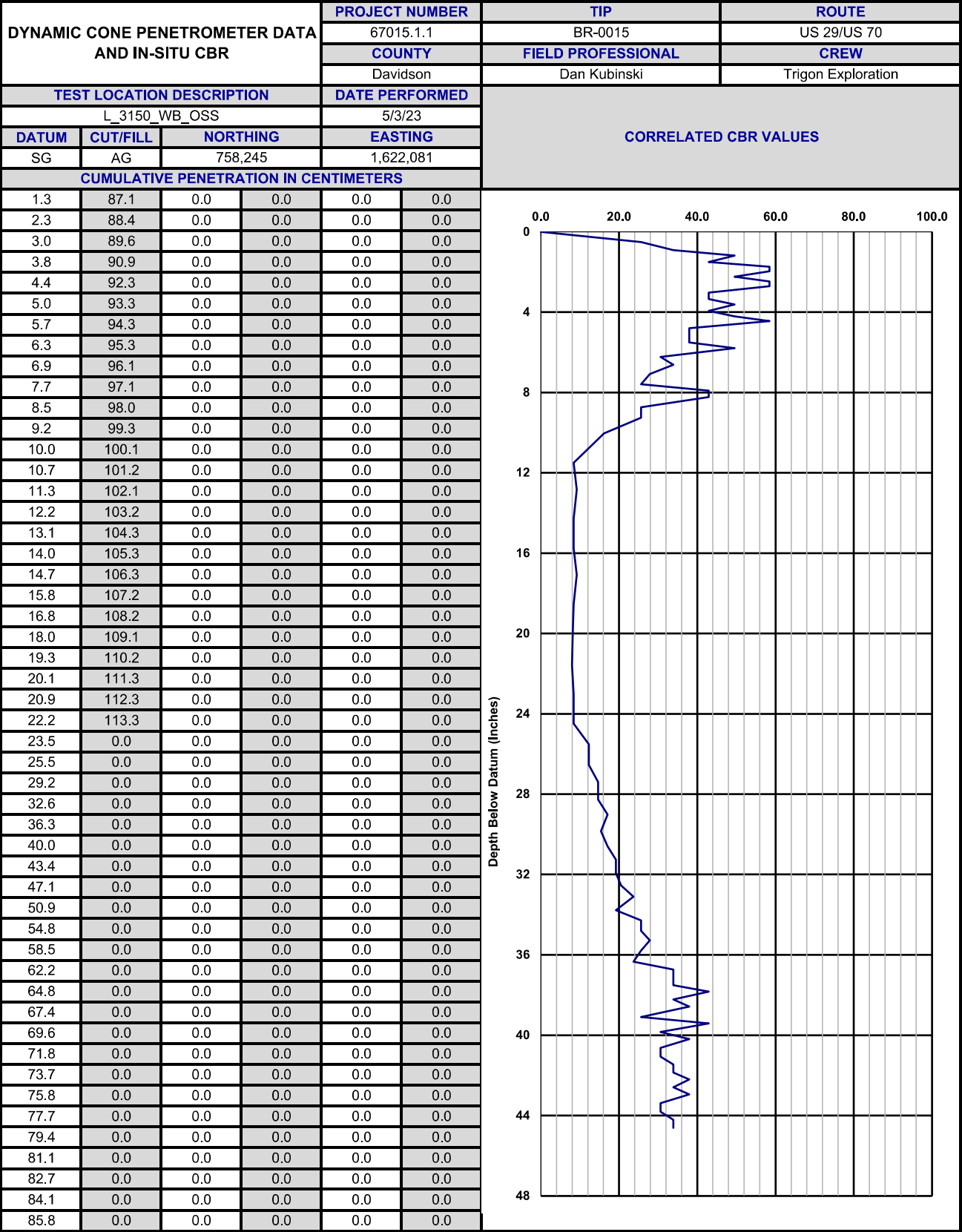
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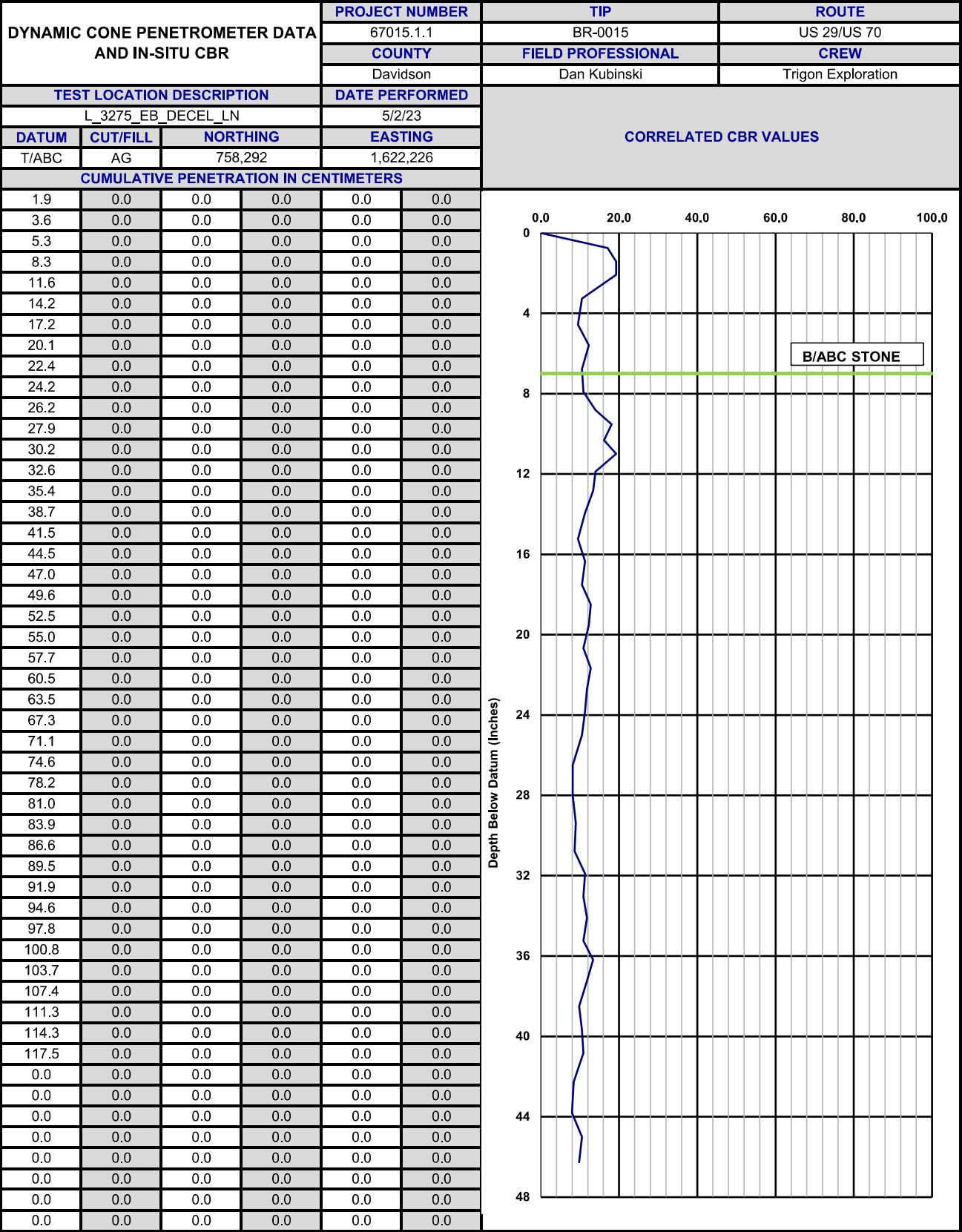
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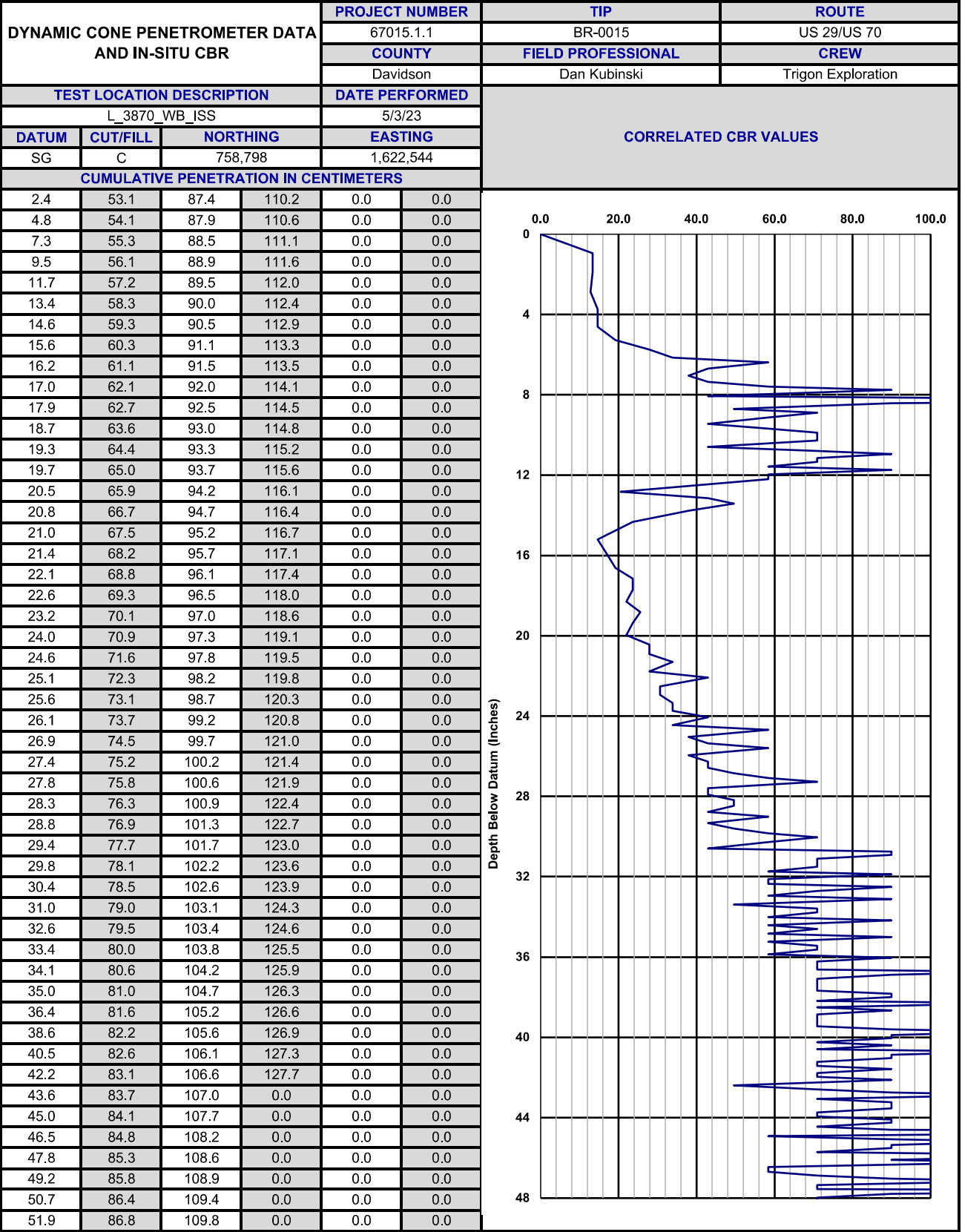
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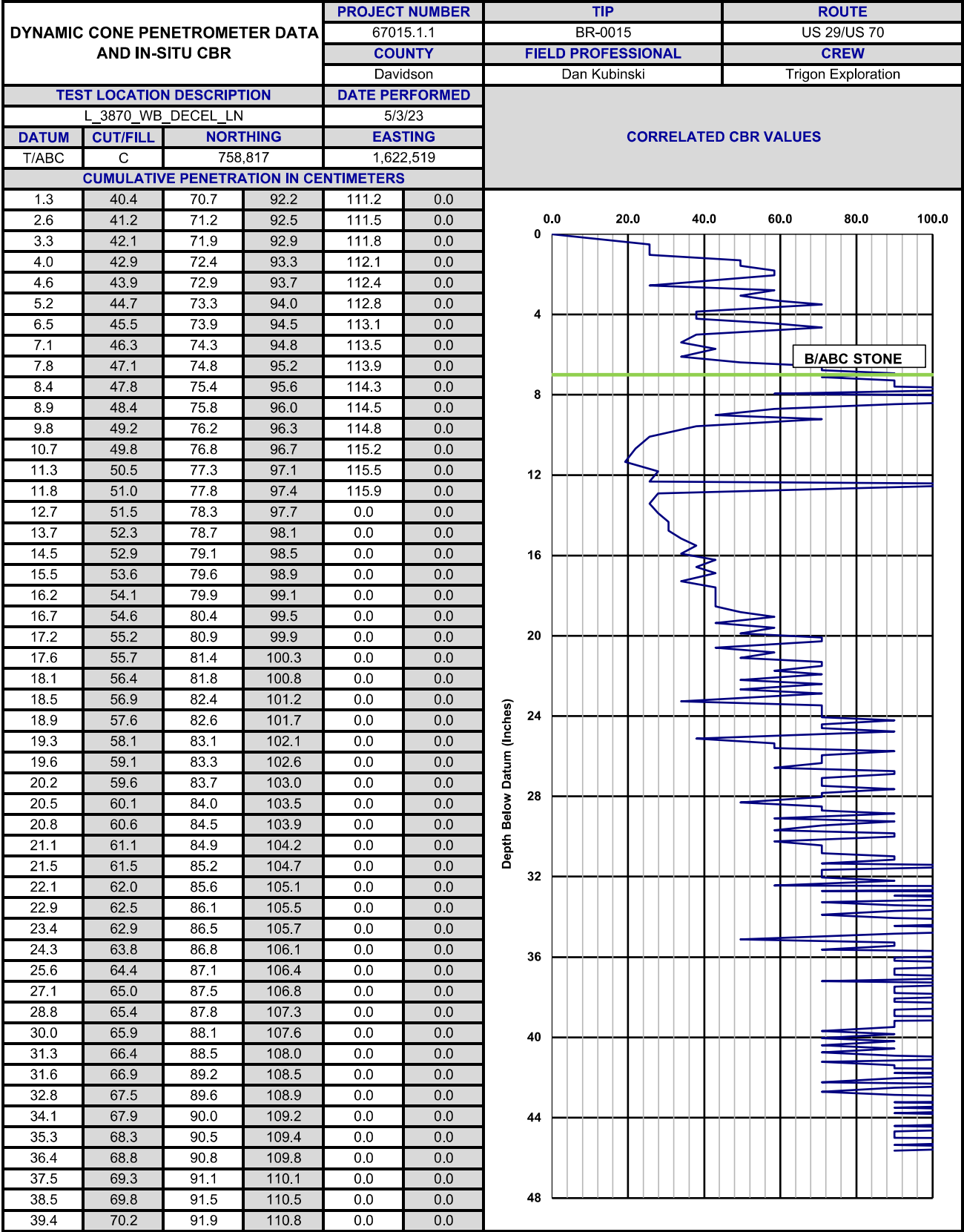
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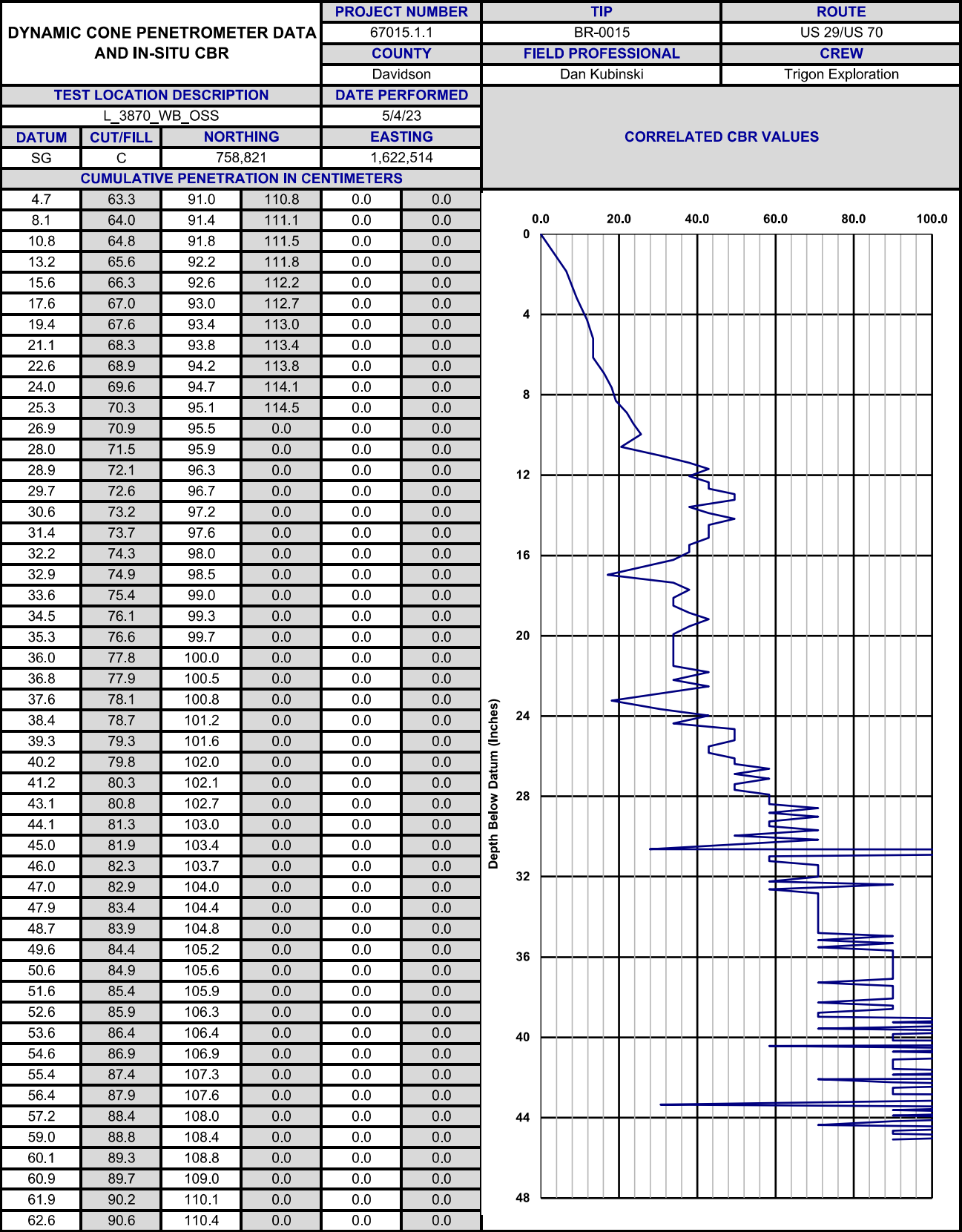
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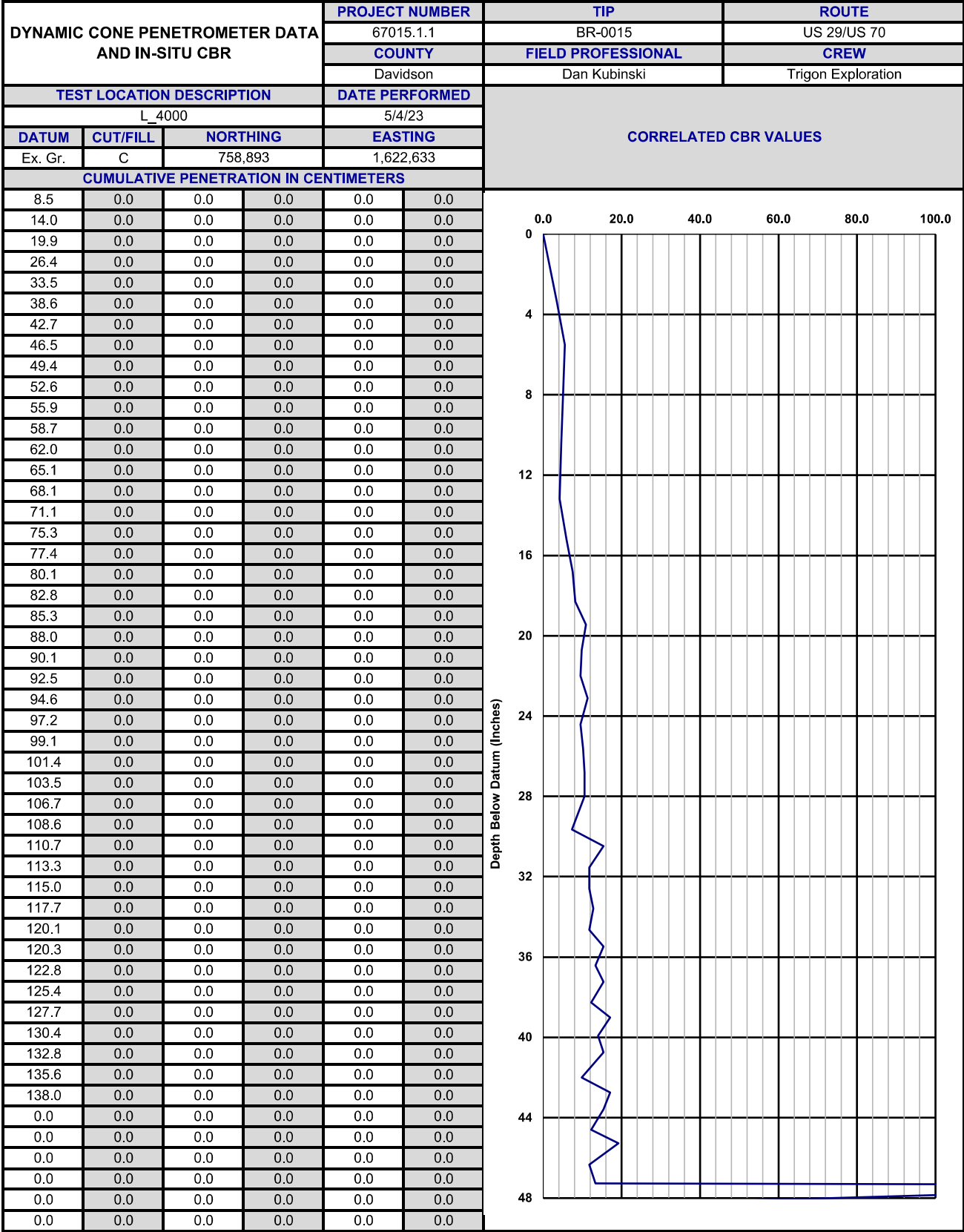
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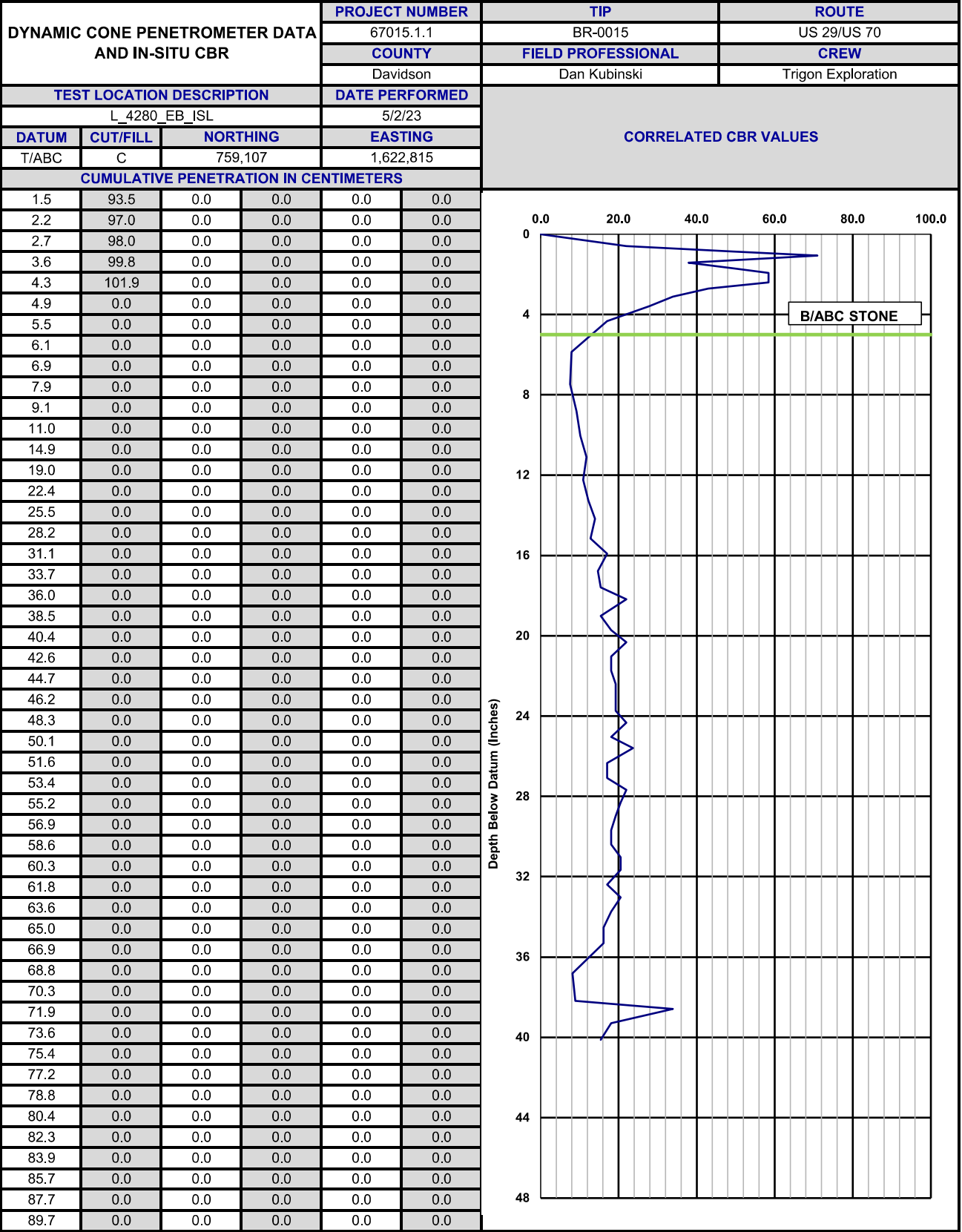
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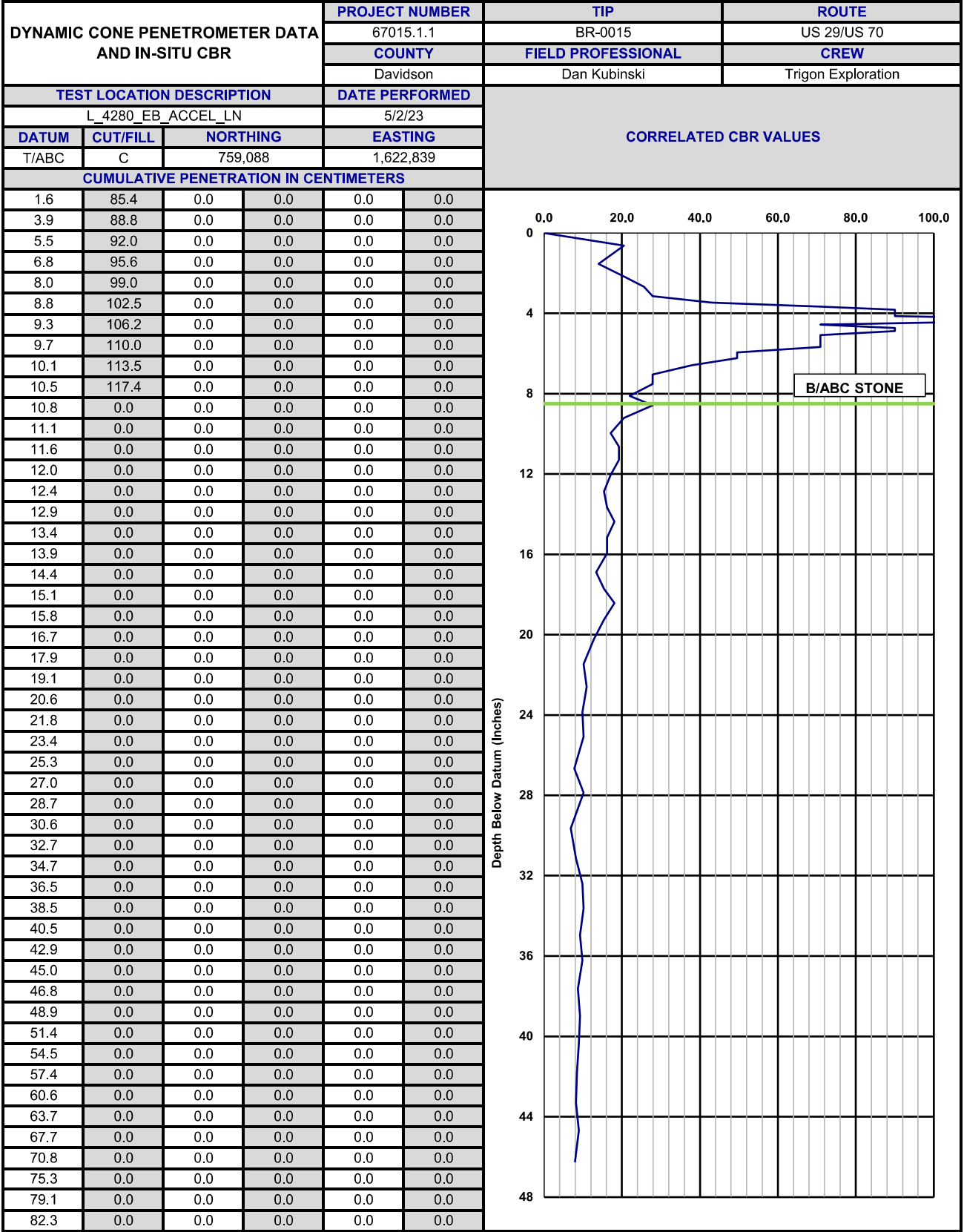
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B/ABC - Bottom of ABC Stone
Ex. Gr. - Existing Grade



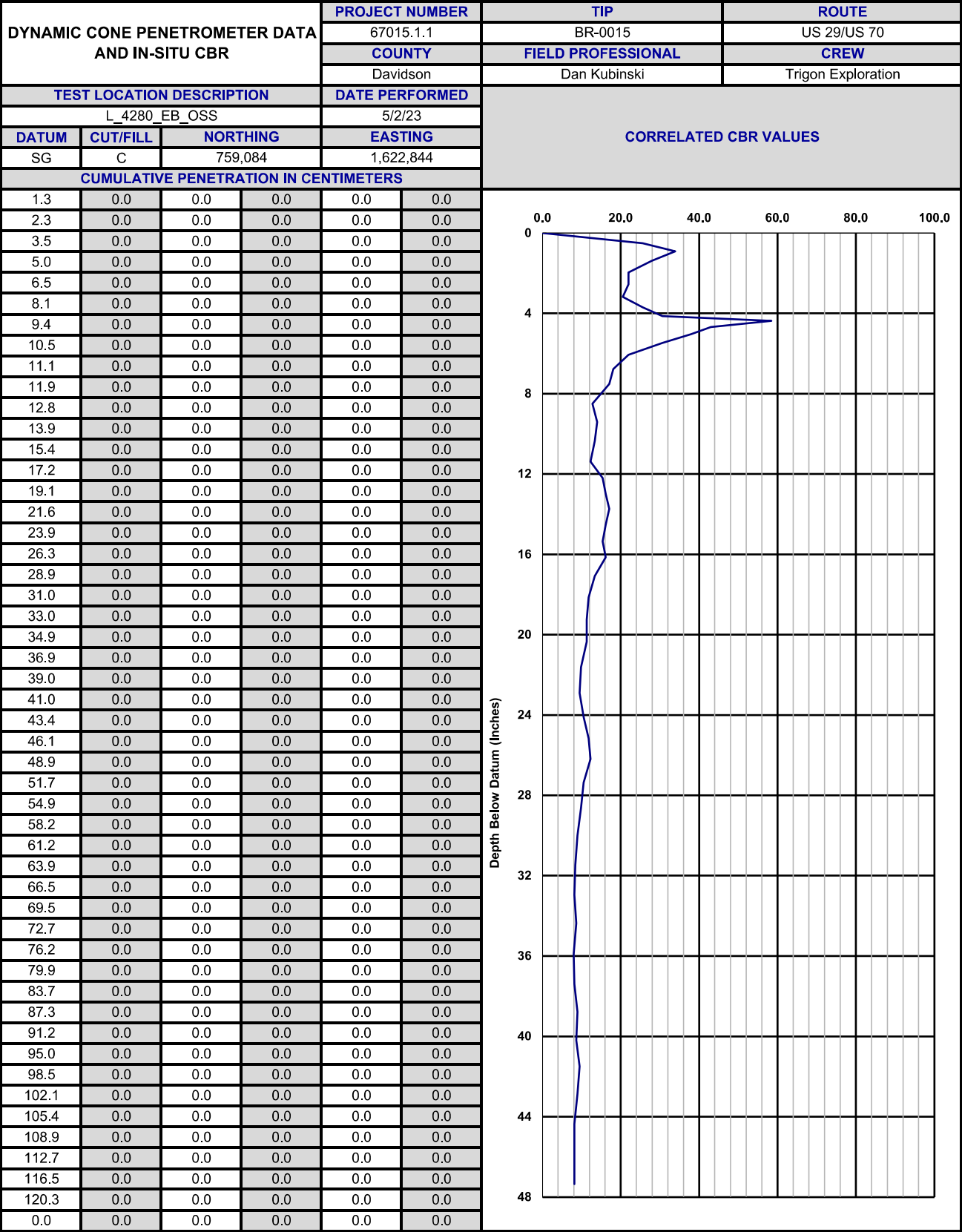
Note(s):
WB - Westbound
EB - Eastbound
OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane

DECEL - Deceleration
ACCEL - Acceleration
LN - Lane
AG - At Grade
F - Fill

C - Cut
SG - Subgrade
T/ABC - Top of ABC Stone
B/ABC - Bottom of ABC Stone
Ex. Gr. - Existing Grade



Note(s):
WB - Westbound
EB - Eastbound
OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane
DECEL - Deceleration
ACCEL - Acceleration
LN - Lane
AG - At Grade
F - Fill
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T/ABC - Top of ABC Stone
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Ex. Gr. - Existing Grade



Note(s):
WB - Westbound
EB - Eastbound
OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane
DECEL - Deceleration
ACCEL - Acceleration
LN - Lane
AG - At Grade
F - Fill
C - Cut
SG - Subgrade
T/ABC - Top of ABC Stone
B/ABC - Bottom of ABC Stone
Ex. Gr. - Existing Grade

[illegible]

Note(s):		
WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade

[illegible]

Note(s):		
WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade

[illegible]

Note(s):	
RT - Right	PS - Paved Shoulder
LN - Lane	SG - Subgrade
AG - At Grade	T/ABC - Top of ABC Stone
F - Fill	B/ABC - Bottom of ABC Stone
C - Cut	Ex. Gr. - Existing Grade

DYNAMIC CONE PENETROMETER DATA AND IN-SITU CBR				PROJECT NUMBER		TIP		ROUTE			
				67015.1.1		BR-0015		Murphy Drive			
				COUNTY		FIELD PROFESSIONAL		CREW			
Davidson				Dan Kubinski		Trigon Exploration					
TEST LOCATION DESCRIPTION				DATE PERFORMED		CORRELATED CBR VALUES					
Y2_1620_LT_LN				5/4/23							
DATUM	CUT/FILL	NORTHING		EASTING							
T/ABC	C	758,472		1,622,178							
CUMULATIVE PENETRATION IN CENTIMETERS											
1.0	66.9	0.0	0.0	0.0	0.0						
1.6	68.4	0.0	0.0	0.0	0.0						
2.1	69.9	0.0	0.0	0.0	0.0						
2.6	71.7	0.0	0.0	0.0	0.0						
3.0	73.4	0.0	0.0	0.0	0.0						
3.3	75.1	0.0	0.0	0.0	0.0						
3.7	76.9	0.0	0.0	0.0	0.0						
4.2	78.7	0.0	0.0	0.0	0.0						
4.6	80.3	0.0	0.0	0.0	0.0						
5.1	81.9	0.0	0.0	0.0	0.0						
5.7	83.4	0.0	0.0	0.0	0.0						
6.3	84.9	0.0	0.0	0.0	0.0						
6.8	86.2	0.0	0.0	0.0	0.0						
7.1	87.6	0.0	0.0	0.0	0.0						
7.8	88.9	0.0	0.0	0.0	0.0						
8.1	90.2	0.0	0.0	0.0	0.0						
8.8	91.5	0.0	0.0	0.0	0.0						
9.3	93.0	0.0	0.0	0.0	0.0						
9.7	94.4	0.0	0.0	0.0	0.0						
10.4	95.6	0.0	0.0	0.0	0.0						
11.0	96.9	0.0	0.0	0.0	0.0						
11.5	97.9	0.0	0.0	0.0	0.0						
12.5	99.1	0.0	0.0	0.0	0.0						
13.1	100.4	0.0	0.0	0.0	0.0						
13.8	101.6	0.0	0.0	0.0	0.0						
15.1	102.6	0.0	0.0	0.0	0.0						
17.9	103.7	0.0	0.0	0.0	0.0						
21.2	104.8	0.0	0.0	0.0	0.0						
24.2	105.8	0.0	0.0	0.0	0.0						
27.2	106.9	0.0	0.0	0.0	0.0						
30.5	108.1	0.0	0.0	0.0	0.0						
32.8	109.3	0.0	0.0	0.0	0.0						
34.8	110.4	0.0	0.0	0.0	0.0						
36.7	111.5	0.0	0.0	0.0	0.0						
38.5	112.6	0.0	0.0	0.0	0.0						
40.3	113.9	0.0	0.0	0.0	0.0						
41.9	114.9	0.0	0.0	0.0	0.0						
43.8	115.9	0.0	0.0	0.0	0.0						
45.6	117.0	0.0	0.0	0.0	0.0						
47.4	117.5	0.0	0.0	0.0	0.0						
49.3	119.1	0.0	0.0	0.0	0.0						
51.2	120.0	0.0	0.0	0.0	0.0						
53.2	121.3	0.0	0.0	0.0	0.0						
55.0	122.4	0.0	0.0	0.0	0.0						
57.0	123.4	0.0	0.0	0.0	0.0						
58.6	124.4	0.0	0.0	0.0	0.0						
60.3	125.6	0.0	0.0	0.0	0.0						
62.1	0.0	0.0	0.0	0.0	0.0						
63.7	0.0	0.0	0.0	0.0	0.0						
65.4	0.0	0.0	0.0	0.0	0.0						

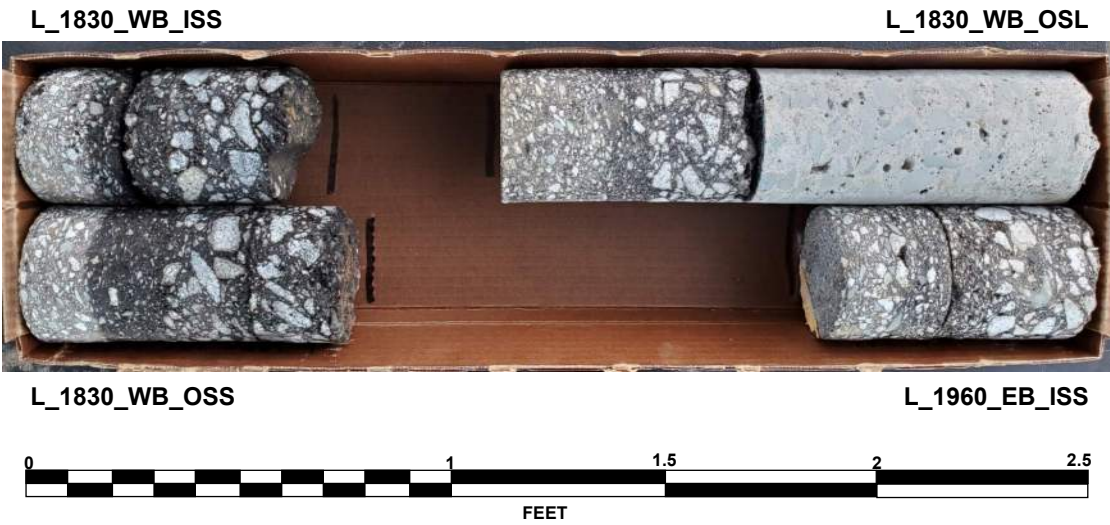
Note(s):	
RT - Right	PS - Paved Shoulder
LN - Lane	SG - Subgrade
AG - At Grade	T/ABC - Top of ABC Stone
F - Fill	B/ABC - Bottom of ABC Stone
C - Cut	Ex. Gr. - Existing Grade

PAVEMENT CORE PHOTOGRAPHS

BR-0015 (67015.1.1)

Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

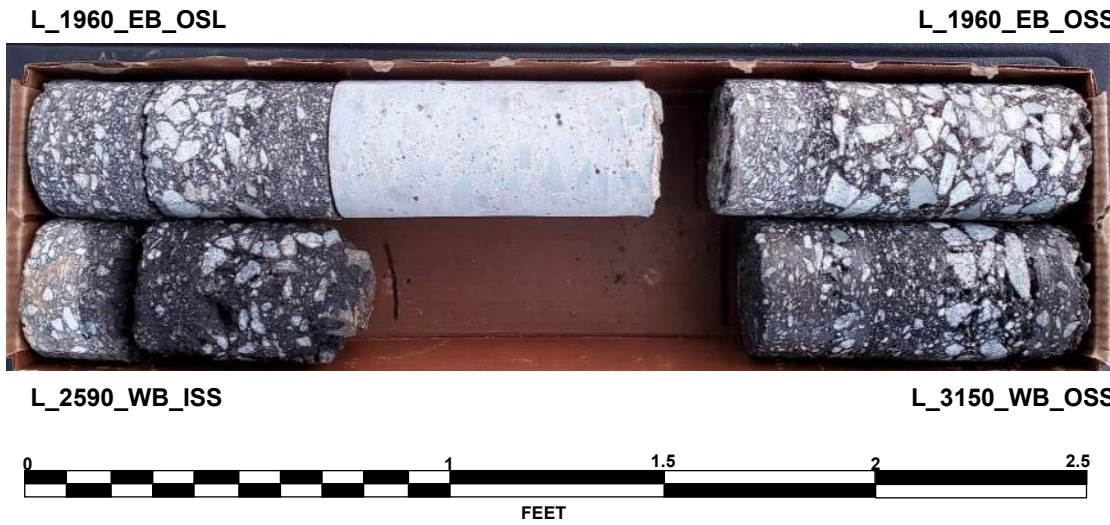
BOX 1



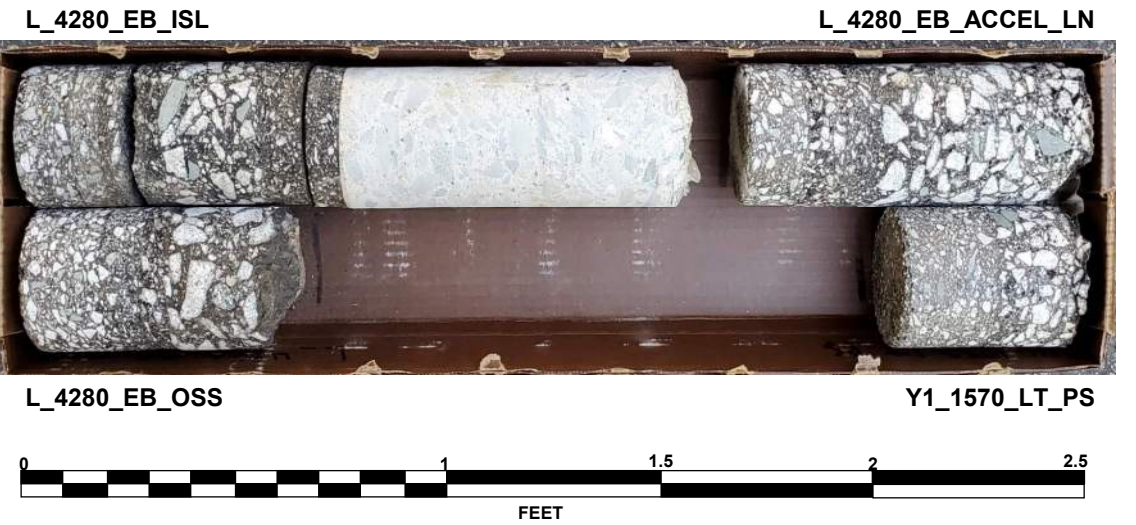
BOX 3



BOX 2



BOX 4

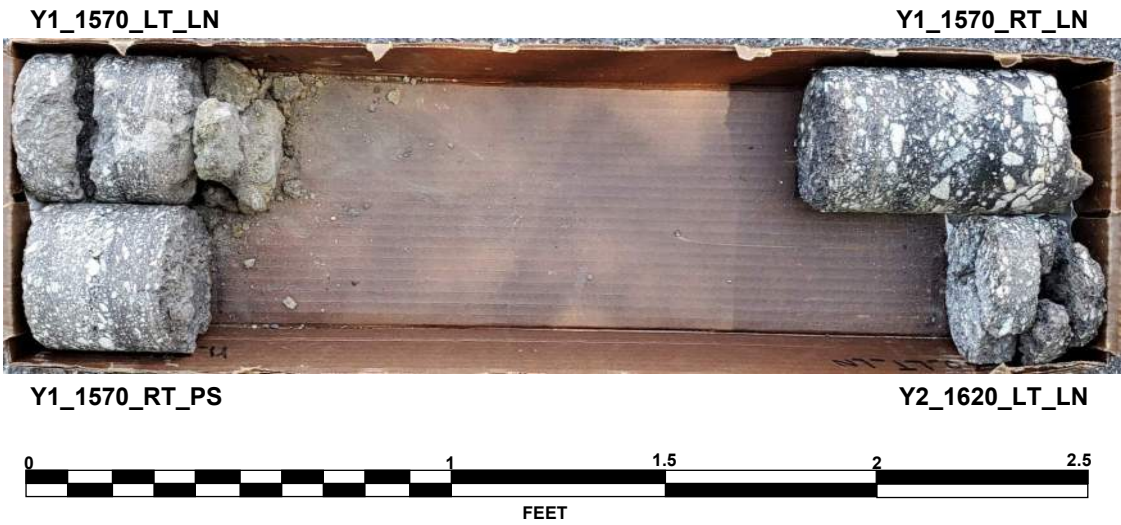


PAVEMENT CORE PHOTOGRAPHS

BR-0015 (67015.1.1)

Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

BOX 5



LABORATORY SUMMARY SHEET FOR SOIL SAMPLES

WBS NO. (TIP NO.): 67015.1.1 (BR-0015)
PROJECT ID: 41620
COUNTY: DAVIDSON
DESCRIPTION: BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)

									Atterberg Limits			Gradation Results							
Sample No.	Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	N-Value (blows/ft)	L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
S-1	L_1830_WB_ISS	-L-	18+30	12' LT	0.0 - 5.0	27.1	A-7-5	-	82	34	48	3.0	95.0	84.0	59.0	18.7	22.2	18.6	40.5
S-2	L_1960_EB_ISS	-L-	19+60	12' RT	0.0 - 5.0	30.1	A-7-5	--	68	34	34	0.0	99.0	93.0	64.0	13.6	26.7	29.1	30.6
S-3	L_2590_WB_ISS	-L-	25+90	12' LT	0.0 - 5.0	28.4	A-7-5	--	69	34	35	1.0	98.0	92.0	66.0	11.3	26.7	21.6	40.4
S-4	L_3150_WB_OSS	-L-	31+50	39' LT	0.0 - 5.0	35.0	A-7-5	--	76	33	43	1.0	98.0	94.0	76.0	6.1	23.4	21.6	48.9
S-5	L_3275_EB_DECEL_LN	-L-	32+75	47' RT	0.0 - 5.0	26.7	A-7-5	--	64	31	33	1.0	99.0	96.0	75.0	5.8	26.0	31.9	36.3
S-6	L_3870_WB_ISS	-L-	38+70	12' LT	0.0 - 5.0	25.9	A-7-6	--	59	20	39	2.0	96.0	89.0	72.0	12.8	15.7	15.0	56.5
S-18*	L_4000^	-L-	40+00	0' CL	1.0 - 1.5	33.2	A-7-5	--	65	40	25	0.0	99.1	94.7	73.2	10.1	20.9	23.9	45.1
S-7	L_4280_EB_ISL	-L-	42+80	13' RT	0.0 - 5.0	38.4	A-7-5	--	79	42	37	0.0	100.0	98.0	73.0	5.2	26.5	15.6	52.7
S-20*	L_4600^	-L-	46+00	0' CL	1.0 - 2.0	26.8	A-7-5	--	51	33	18	0.0	99.0	96.0	51.0	13.0	41.3	13.6	32.1
S-8	Y1_1570_RT_LN	-Y1-	15+70	10' RT	0.0 - 5.0	24.8	A-7-6	--	59	24	35	1.0	99.0	93.0	69.0	13.2	21.2	19.4	46.2
S-9	Y2_1620_LT_LN	-Y2-	16+20	6' LT	0.0 - 5.0	32.8	A-7-5	--	65	31	34	1.0	99.0	97.0	80.0	5.1	19.5	20.3	55.1
CBR-1*	L_1450^	-L-	14+50	88' LT	8.5 - 18.5	27.0	A-7-5	--	60	44	16	0.0	100.0	93.1	56.6	14.7	35.3	23.4	26.6
CBR-2*	L_1850^	-L-	18+50	89' LT	0.0 - 10.0	30.6	A-7-5	--	60	35	25	0.0	100.0	95.2	75.9	9.9	19.4	26.1	44.7
CBR-3*	L_4200^	-L-	42+00	0' CL	0.0 - 2.5	37.3	A-7-5	--	73	37	36	8.0	91.0	96.6	78.4	7.2	18.6	28.9	45.2

*Based on Roadway Investigation sample number

^Based on Roadway Investigation boring number

Michelle Stadel, P.E.
Lab Manager, NCDOT Certification No.: 111-02-1203

Victoria Siebert
Lab Technician, NCDOT Certification No.: 109-02-1003



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

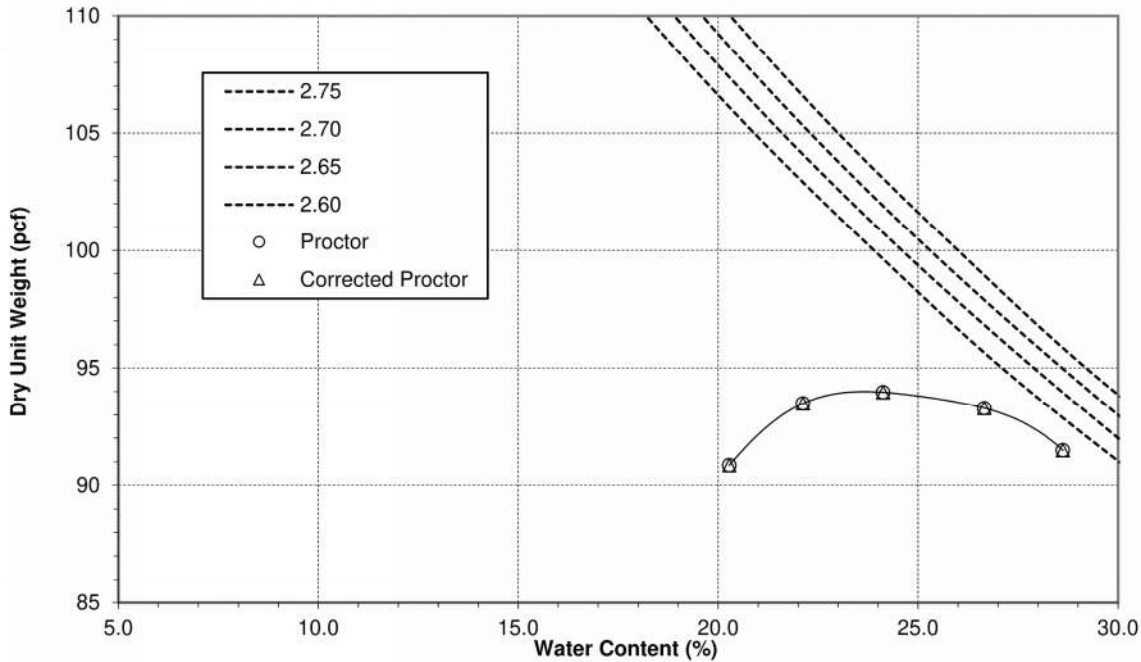
NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/24/2023 by C. Blalock

Material Description: Brown Sandy Silt (A-7-5)

Location: Boring No. L_1450, Alignment -L-, STA 14+50, 88' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	94.0	na
Optimum Water Content (%)	23.6	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Moist

Remarks:

AASHTO T-100, Soil Specific Gravity @ 20°C: 2.747

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Sample Source: Boring No. L_1450, Alignment -L-, STA 14+50, 88' LT

Sample ID: CBR-1

Sample Description: Brown Sandy Silt (A-7-5)

Material Used:

Surcharge Weight: 10 lbs

Date Tested: 5/30/2023

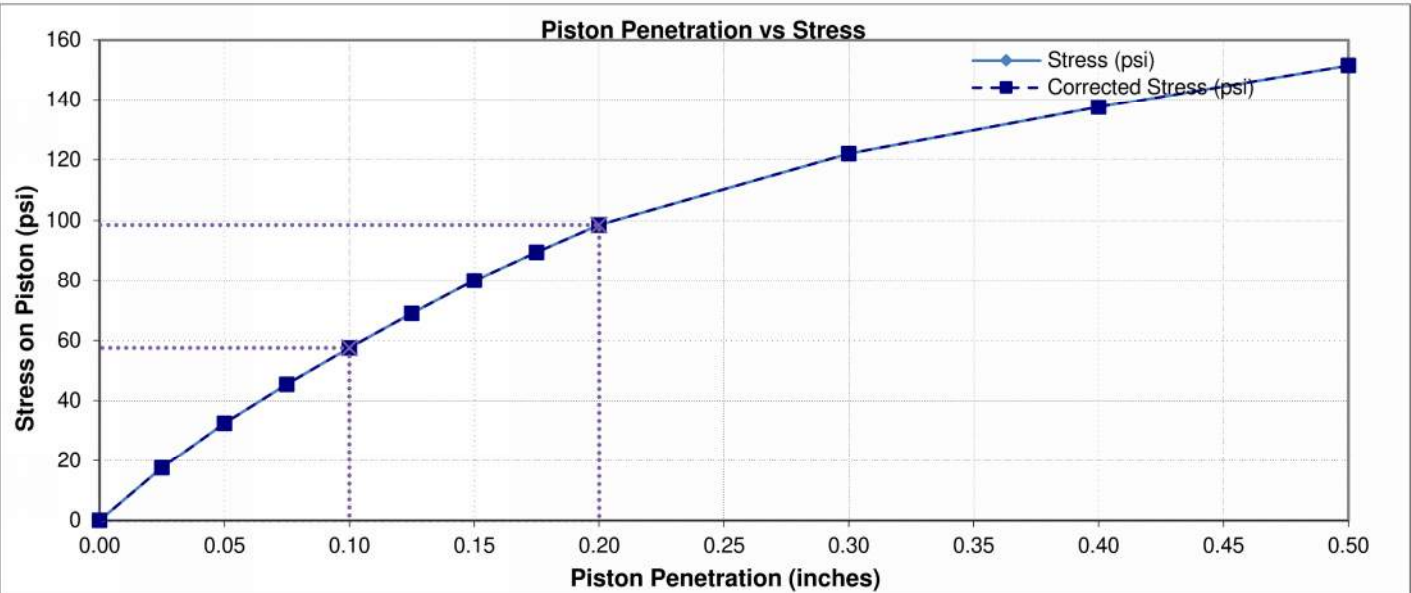
Tested By: C. Blalock

Condition of Sample: Soaked

Time Soaked: 96 hrs

AASHTO T193 - Standard Test Method for The California Bearing Ratio (CBR)

Dry Unit Wgt Before Soaking (pcf):	90.8	Compaction Method:	Manual
Water Content Before Soaking (%):	21.2	Max. Dry Unit Weight:	94.0 pcf
Dry Unit Wgt After Soaking (pcf):	101.1	Optimum Water Content:	23.6 %
Water Content After Soak, Top in. (%):	30.3		
Swell (%):	5.65		
CBR (Corrected CBR) @ 0.1 in. Penetration:	5.8 (5.8)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.6 (6.6)		



Remarks:

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

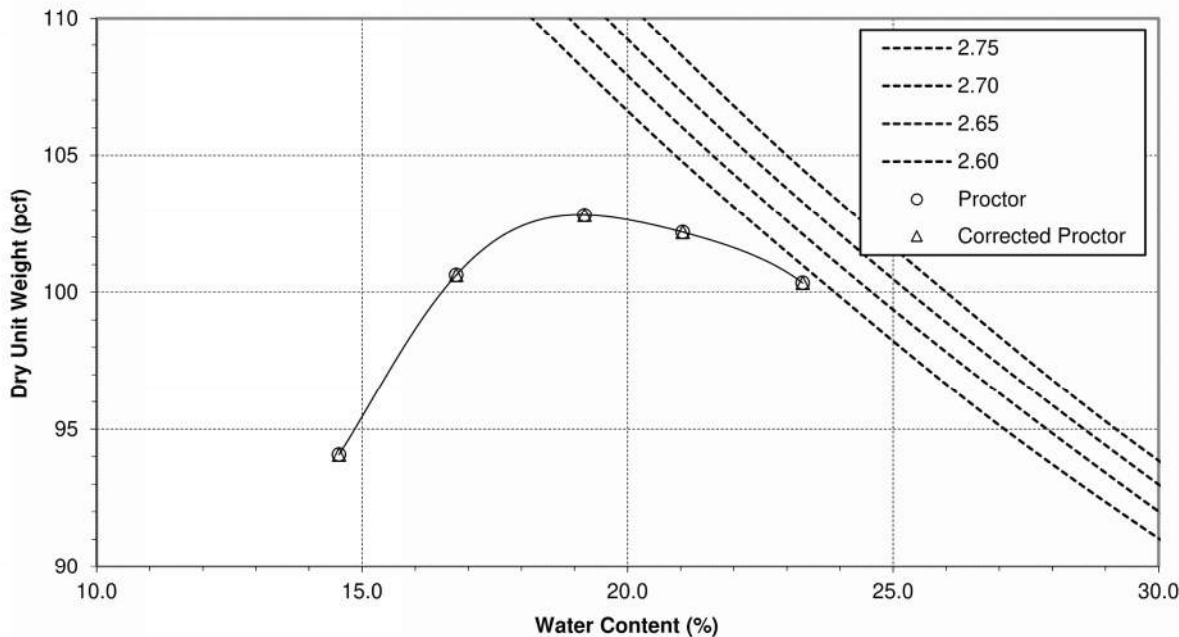
NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/24/2023 by C. Blalock

Material Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Location: Boring No. L_1850, -L-, STA 18+50, 89' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	102.8	na
Optimum Water Content (%)	19.1	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Moist

Remarks:

AASHTO T-100, Soil Specific Gravity at 20°C: 2.678

SEED

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Sample Source: Boring No. L_1850, -L-, STA 18+50, 89' LT

Sample ID: CBR-2

Sample Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Material Used:

Surcharge Weight: 10 lbs

Date Tested: 5/30/2023

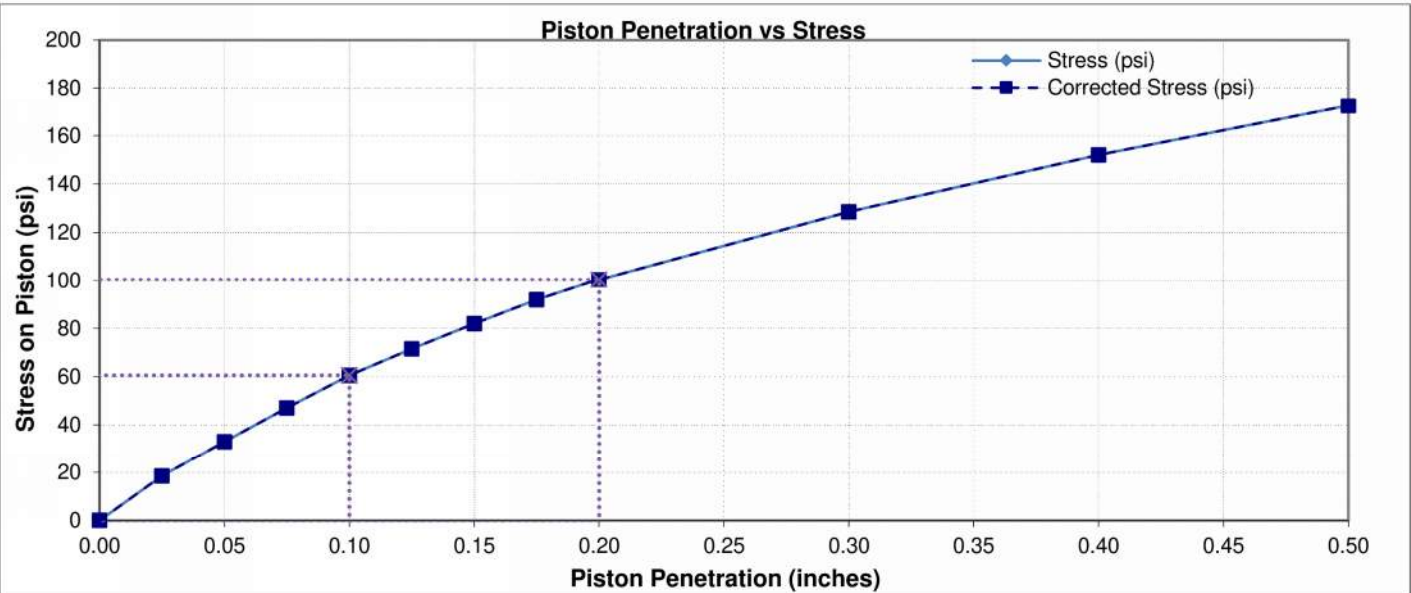
Tested By: C. Blalock

Condition of Sample: Soaked

Time Soaked: 96 hrs

AASHTO T-193 - Standard Test Method for The California Bearing Ratio

Dry Unit Wgt Before Soaking (pcf):	100.6	Compaction Method:	Manual
Water Content Before Soaking (%):	17.0	Max. Dry Unit Weight:	102.8 pcf
Dry Unit Wgt After Soaking (pcf):	106.1	Optimum Water Content:	19.1 %
Water Content After Soak, Top in. (%):	27.1		
Swell (%):	4.1		
CBR (Corrected CBR) @ 0.1 in. Penetration:	6.1 (6.1)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.7 (6.7)		



Remarks:

SEED

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

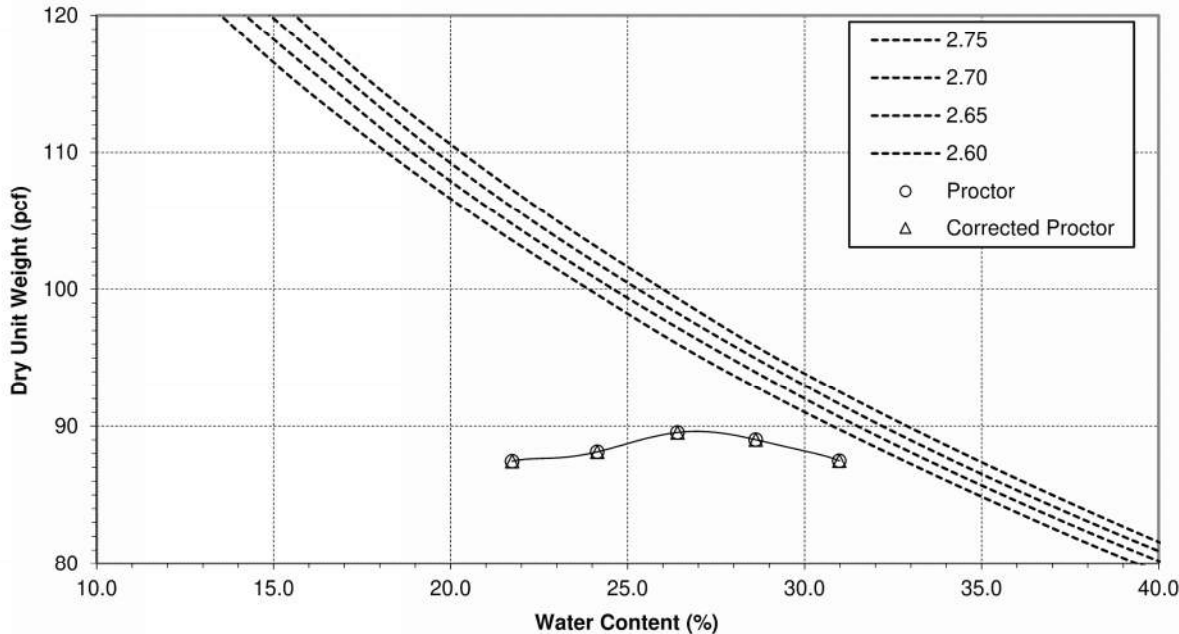
NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/26/2023 by C. Blalock

Material Description: Brown Sandy Silt (A-7-5)

Location: Boring No. L-1450, Alignment -L-, STA 14+50, 88' LT, CBR-1, 8.5' - 18.5'



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	89.6	na
Optimum Water Content (%)	26.9	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Dry

Remarks:

+4% Lime

Reviewed on 6/9/2023 by Michelle Stadel,

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Tested By : C. Blalock

Date Molded: 5/31/2023

Sample Location: Boring No. L-1450, CBR-1, 8.5' - 18.5'

Alignment -L-, STA 14+50, 88' LT

ASTM D5102 - Modified, Unconfined Compressive Strength of Compacted Soil-Lime Mixtures

	A	B	C	D
Sample Preparation:	AASHTO T99	AASHTO T99	AASHTO T99	AASHTO T99
Water Content (%):	25.7	26	25.9	26
Height (in):	4.632	4.619	4.644	4.62
Diameter (in):	4.00	4.00	3.999	4.002
Cross-Sectional Area (in ²):	12.57	12.59	12.56	12.58
Test Date:	6/7/2023	6/7/2023	6/7/2023	6/7/2023
Age (days):	7	7	7	7
Maximum Load (lbf):	860	830	850	850
h/d Conversion Factor	none	none	none	none
Compressive Strength (psi):	70	65	70	70
Dry Unit Weight (pcf):	89.6	89.2	88.4	88.5

Sample Preparation:

Water Content (%):

Height (in):

Diameter (in):

Cross-Sectional Area (in²):

Test Date:

Age (days):

Maximum Load (lbf):

h/d Conversion Factor

Compressive Strength (psi):

Curing Details: Samples extruded, placed in plastic bags, and cured for 7 days at 73°F ± 4°.

Specification: Per NCDOT, cast according to AASHTO T99 and broke per AASHTO T208.

Remarks:

(A) 3.0% Lime; Percent Strain = 2.0. (B) 3.0% Lime; Percent Strain = 2.4.

(C) 5% Lime; Percent Strain = 2.5. (D) 5% Lime; Percent Strain = 2.9.

Reviewed on 6/9/2023 by Michelle Stadel,

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00651 Rev. 0

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L-4200, CBR-3

NCDOT BR-0015 Roadway

Sampled by: Dan Kubinski

Date: 5/5/2023

02-000L - Lab

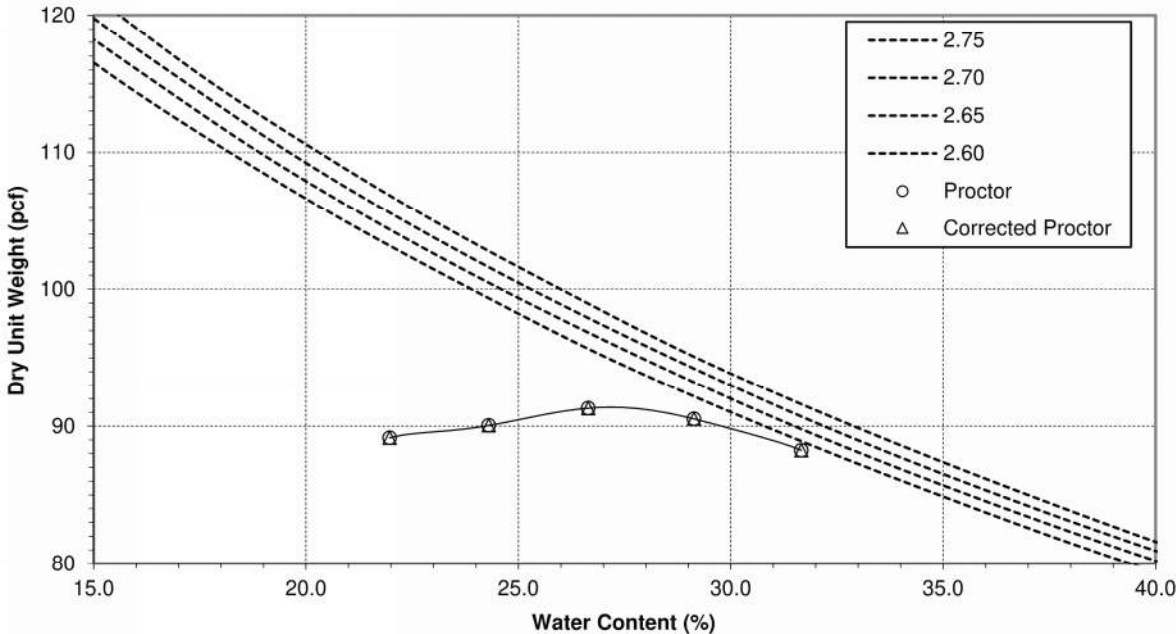
Submitted by: Dan Kubinski

Date: 5/15/2023

Tested on 5/25/2023 by C. Blalock

Material Description: Reddish Brown Elastic Silt with Sand (A-7-5)

Location: Boring No. L-4200, Alignment -L-, STA 42+00, 0' CL, CBR-3, 0' - 2.5'



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	91.4	na
Optimum Water Content (%)	27.2	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Dry

Remarks:

+4% Lime

AASHTO T100, Soil Specific Gravity = 2.727

Reviewed on 6/9/2023 by Michelle Stadel,

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00651 Rev. 0

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L-4200, CBR-3

NCDOT BR-0015 Roadway

Sampled by: Dan Kubinski

Date: 5/5/2023

02-000L - Lab

Submitted by: Dan Kubinski

Date: 5/15/2023

Tested By : C. Blalock

Date Molded: 5/31/2023

Sample Location: Boring No. L-4200, CBR-3, 0' - 2.5'

Alignment -L-, STA 42+00, 0' CL

ASTM D5102 - Modified, Unconfined Compressive Strength of Compacted Soil-Lime Mixtures

	A	B	C	D
Sample Preparation:	AASHTO T208	AASHTO T208	AASHTO T208	AASHTO T208
Water Content (%):	25.4	25.5	26.2	25.9
Height (in):	4.641	4.623	4.628	4.615
Diameter (in):	3.99	4.01	4	4
Cross-Sectional Area (in ²):	12.53	12.6	12.57	12.57
Test Date:	6/7/2023	6/7/2023	6/7/2023	6/7/2023
Age (days):	7	7	7	7
Maximum Load (lbf):	760	770	820	910
h/d Conversion Factor	none	none	none	none
Compressive Strength (psi):	60	60	65	70
Dry Unit Weight (pcf):	91.9	91.6	90.1	90.5

Sample Preparation:

Water Content (%):

Height (in):

Diameter (in):

Cross-Sectional Area (in²):

Test Date:

Age (days):

Maximum Load (lbf):

h/d Conversion Factor

Compressive Strength (psi):

Curing Details: Samples extruded, placed in plastic bags, and cured for 7 days at 73°F ± 4°.

Specification: Per NCDOT, cast according to AASHTO T99 and broke per AASHTO T208.

Remarks:

(A) 3.0% Lime; Percent Strain = 2.2. (B) 3.0% Lime; Percent Strain = 2.1.

(C) 5% Lime; Percent Strain = 2.1. (D) 5% Lime; Percent Strain = 2.4.

Reviewed on 6/9/2023 by Michelle Stadel,

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Michelle M. Stadel

PAVEMENT CORE EVALUATION

BR-0015 (67015.1.1)

BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)

LINE	STATION	ABC STONE (in)	LAYER THICKNESS (in)	PAVEMENT LAYERS	REMARKS
-L-	1830_WB_ISS 6.75" Asphalt	2.00	4.00	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			2.75	B	1 Lift; Low oxidation
-L-	1830_WB_OSL 7.25" Asphalt 9.25" Concrete	4.50	4.25	S	3 Lifts; Low oxidation
			3.00	B	1 Lift; Delamination between base asphalt and concrete layer; Low oxidation
			9.25	C	1 Lift
-L-	1830_WB_OSS 8.50" Asphalt	0.00	3.75	S	3 Lifts; Low oxidation
			4.75	B	2 Lifts; Low oxidation
-L-	1960_EB_ISS 7.25" Asphalt	0.00	4.25	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			3.00	B	1 Lift; Low oxidation
-L-	1960_EB_OSL 8.25" Asphalt 9.25" Concrete	4.75	3.00	S	2 Lifts; Delamination between surface asphalt and base asphalt layers; Low oxidation
			3.25	B	1 Lift; Low oxidation
			2.00	S	1 Lift; Delamination between surface asphalt and concrete layer; Low oxidation
			9.25	C	1 Lift
-L-	1960_EB_OSS 10.25" Asphalt	11.75	3.00	S	2 Lifts; Low oxidation
			7.25	B	2 Lifts; Low oxidation
-L-	2590_WB_ISS 8.50" Asphalt	0.00	4.50	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			4.00	B	1 Lift; Low oxidation
-L-	3150_WB_OSS 9.25" Asphalt	0.00	5.25	S	3 Lifts; Low oxidation
			2.50	B	1 Lift; Low oxidation
			1.50	S	1 Lift; Low oxidation
-L-	3275_EB_DECEL_LN 10.75" Asphalt	7.00	5.00	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			5.75	B	1 Lift; Low oxidation
-L-	3870_WB_ISS 7.00" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.00	B	1 Lift; Low oxidation
-L-	3870_WB_DECEL_LN 11.00" Asphalt	7.00	4.00	S	2 Lifts; Low oxidation
			7.00	B	2 Lifts; Low oxidation
-L-	3870_WB_OSS 7.50" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.50	B	1 Lift; Low oxidation

Note(s):

NM - Not Measured
PS - Paved Shoulder
WB - Westbound
EB - Eastbound
LT - Left
RT - Right

OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane
DECEL - Deceleration
ACCEL - Acceleration
LN - Lane

S - Asphalt Surface Course
I - Asphalt Intermediate Course
B - Asphalt Base Course
C - Concrete

PAVEMENT CORE EVALUATION

BR-0015 (67015.1.1)

BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)

LINE	STATION	ABC STONE (in)	LAYER THICKNESS (in)	PAVEMENT LAYERS	REMARKS
-L-	4280_EB_ISL 8.50" Asphalt 9.50" Concrete	5.00	3.75	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			2.75	B	1 Lift; Low oxidation
			2.00	S	2 Lifts; Delamination between 1st and 2nd lift; Low oxidation
			9.50	C	
-L-	4280_EB_ACCEL_LN 9.50" Asphalt	8.50	3.75	S	3 Lifts; Low oxidation
			5.75	B	1 Lift; Low oxidation
-L-	4280_EB_OSS 7.00" Asphalt	0.00	3.50	S	3 Lifts; Low oxidation
			3.50	B	1 Lift; Low oxidation
-Y1-	1570_LT_PS 5.00" Asphalt	NM	2.50	S	1 Lift; Low oxidation
			2.50	I	1 Lift; Low oxidation
-Y1-	1570_LT_LN 6.00" Asphalt	NM	6.00	S	2 Lifts; Low oxidation; Cracked completely through 1st surface asphalt layer
-Y1-	1570_RT_LN 7.00" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.00	I	1 Lift; Low oxidation
-Y1-	1570_RT_PS 4.00" Asphalt	NM	4.00	S	2 Lifts; Low oxidation
-Y2-	1620_LT_LN 3.00" Asphalt	7.00	3.00	S	2 Lifts; Low oxidation; Cracked completely through surface asphalt layer

Note(s):

NM - Not Measured
PS - Paved Shoulder
WB - Westbound
EB - Eastbound
LT - Left
RT - Right

OSS - Outside Shoulder
ISS - Inside Shoulder
OSL - Outside Lane
DECEL - Deceleration
ACCEL - Acceleration
LN - Lane

S - Asphalt Surface Course
I - Asphalt Intermediate Course
B - Asphalt Base Course
C - Concrete

REFERENCE: BR-0015

PROJECT: 67015

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	I2+00.00 - 49+50.00	4 - 6	N/A
-YI-	I0+00.00 - I9+35.00	5	N/A
-Y2-	I0+00.00 - I8+50.00	5	N/A
-RPB-	I0+00.00 - I9+28.28	4 - 5	N/A
-RPD-	I0+00.00 - I2+66.89	5	N/A
-RPD-RT-	I0+00.00 - I0+96.7I	5	N/A
-LPB-	I0+00.00 - I5+92.00	5	N/A
-DRWI-	I0+00.00 - II+45.00	5	N/A

APPENDICES

APPENDIX	TITLE	SHEETS
A	BORE LOGS	7 - 33
B	LABORATORY RESULTS	34 - 37

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY DAVIDSON
PROJECT DESCRIPTION BRIDGE NO. 67 AND NO. 68
REPLACEMENTS ON US 29/US 70 NB & SB OVER
SR 1192 (W. 5TH AVENUE)

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0015	1	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

M. FOSTER

J. KARDON

D. KUBINSKI

TRIGON EXPLORATION

INVESTIGATED BY KLEINFELDER, INC

DRAWN BY D. KUBINSKI

CHECKED BY J. FREGOSI

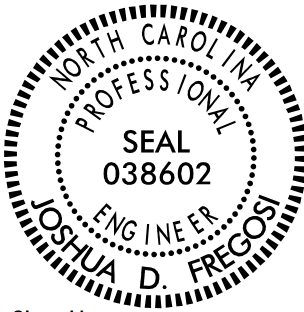
SUBMITTED BY KLEINFELDER, INC

DATE AUGUST 2023

Prepared in the Office of:



422 Gallimore Dairy Road, Suite B
Greensboro, North Carolina 27409
NC Engineering Firm License No. F-1312



DocuSigned by:

Joshua D. Fregosi

08/08/2023

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SIGNATURE

DATE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

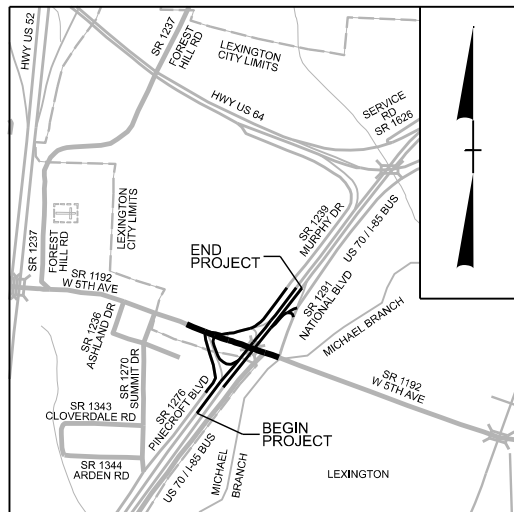
SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																																																																																																																																																																																																																																																																																																																																																																																																																																													
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:										<u>ALLUVIUM (ALLUV.)</u> - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. <u>AQUIFER</u> - A WATER BEARING FORMATION OR STRATA. <u>ARENACEOUS</u> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <u>ARGILLACEOUS</u> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. <u>ARTESIAN</u> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. <u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <u>COLLUVIUM</u> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. <u>CORE RECOVERY (REC.)</u> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. <u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. <u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. <u>FAULT</u> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. <u>FISSILE</u> - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. <u>FLOAT</u> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. <u>FLOOD PLAIN (FP)</u> - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. <u>FORMATION (FM)</u> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. <u>JOINT</u> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. <u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. <u>LENS</u> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. <u>MOTTLED (MOT.)</u> - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. <u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. <u>RESIDUAL (RES.) SOIL</u> - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. <u>ROCK QUALITY DESIGNATION (RQD)</u> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <u>SAPROLITE (SAP.)</u> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. <u>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)</u> - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. <u>STRATA CORE RECOVERY (ISRC.)</u> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. <u>STRATA ROCK QUALITY DESIGNATION (SRQD)</u> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. <u>TOPSOIL (TS.)</u> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.																																																																																																																																																																																																																																																																																																																																																																																																																																													
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS										WEATHERED ROCK (WR)										CRISTALLINE ROCK (CR)																																																																																																																																																																																																																																																																																																																																																																																																																																													
GENERAL CLASS.		GRANULAR MATERIALS (≤ 35% PASSING #200)				SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS				A-1, A-2		A-4, A-5				A-1, A-2		A-4, A-5																																																																																																																																																																																																																																																																																																																																																																																																																																																					
GROUP CLASS.	A-1	A-3	A-2		A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7																																																																																																																																																																																																																																																																																																																																																																																																																																																															
SYMBOL	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
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USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND			SILTY SOILS			CLAYEY SOILS																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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CONSISTENCY OR DENSENESS										GROUND WATER										WEATHERING																																																																																																																																																																																																																																																																																																																																																																																																																																																							
PRIMARY SOIL TYPE		COMPACTNESS OR CONSISTENCY		RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)				RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION		SOIL SYMBOL		ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT		INFERRED SOIL BOUNDARY		INFERRED ROCK LINE		ALLUVIAL SOIL BOUNDARY		DIP & DIP DIRECTION OF ROCK STRUCTURES		TEST BORING		SLOPE INDICATOR INSTALLATION		CONE PENETROMETER TEST		SOUNDING ROD		TEST BORING WITH CORE		SPT N-VALUE																																																																																																																																																																																																																																																																																																																																																																																																																																							
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)		VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE		< 4 4 TO 10 10 TO 30 30 TO 50 > 50				N/A																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
GENERALLY SILT-CLAY MATERIAL (COHESIVE)		VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD		< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30				< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
TEXTURE OR GRAIN SIZE										RECOMMENDATION SYMBOLS										ROCK HARDNESS																																																																																																																																																																																																																																																																																																																																																																																																																																																							
U.S. STD. SIEVE SIZE OPENING (MM)		4		10		40		60		200		270		0.075		0.053																																																																																																																																																																																																																																																																																																																																																																																																																																																											
BOULDER (BLDR.)		COBBLE (COB.)		GRAVEL (GR.)		COARSE SAND (CSE. SD.)		FINE SAND (F SD.)		SILT (SL.)		CLAY (CL.)																																																																																																																																																																																																																																																																																																																																																																																																																																																															
GRAIN SIZE		MM IN.		305 12		75 3		2.0		0.25		0.05		0.005																																																																																																																																																																																																																																																																																																																																																																																																																																																													
SOIL MOISTURE - CORRELATION OF TERMS										MISCELLANEOUS SYMBOLS										ROCK HARDNESS																																																																																																																																																																																																																																																																																																																																																																																																																																																							
SOIL MOISTURE SCALE (ATTERBERG LIMITS)		FIELD MOISTURE DESCRIPTION				GUIDE FOR FIELD MOISTURE DESCRIPTION				AR - AUGER REFUSAL		BT - BORING TERMINATED		CL - CLAY		CPT - CONE PENETRATION TEST		CSE. - COARSE		DPT - DILATOMETER TEST		DPM - DYNAMIC PENETRATION TEST		e - VOID RATIO		f - FINE		FOSS. - FOSSILIFEROUS		FRAC. - FRACTURED, FRACTURES		FRAGS. - FRAGMENTS		HI. - HIGHLY		MED. - MEDIUM		MICA - MICACEOUS		MOD. - MODERATELY		NP - NON PLASTIC		ORG. - ORGANIC		PMT - PRESSUREMETER TEST		SAP. - SAPROLITIC		SD. - SAND, SANDY		SL. - SILT, SILTY		SLI. - SLIGHTLY		TCR - TRICONE REFUSAL		w - MOISTURE CONTENT		V - VERY		VST - VANE SHEAR TEST		WEA. - WEATHERED		γ - UNIT WEIGHT		γ _d - DRY UNIT WEIGHT																																																																																																																																																																																																																																																																																																																																																																																																							
LL		LIQUID LIMIT				USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE																																																																																																																																																																																																																																																																																																																																																																																																																																																																				</	

09/08/99

PROJECT: BR-0015

CONTRACT: TBD

See Sheet 1A For Index of Sheets



VICINITY MAP (NTS)



STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

DAVIDSON COUNTY

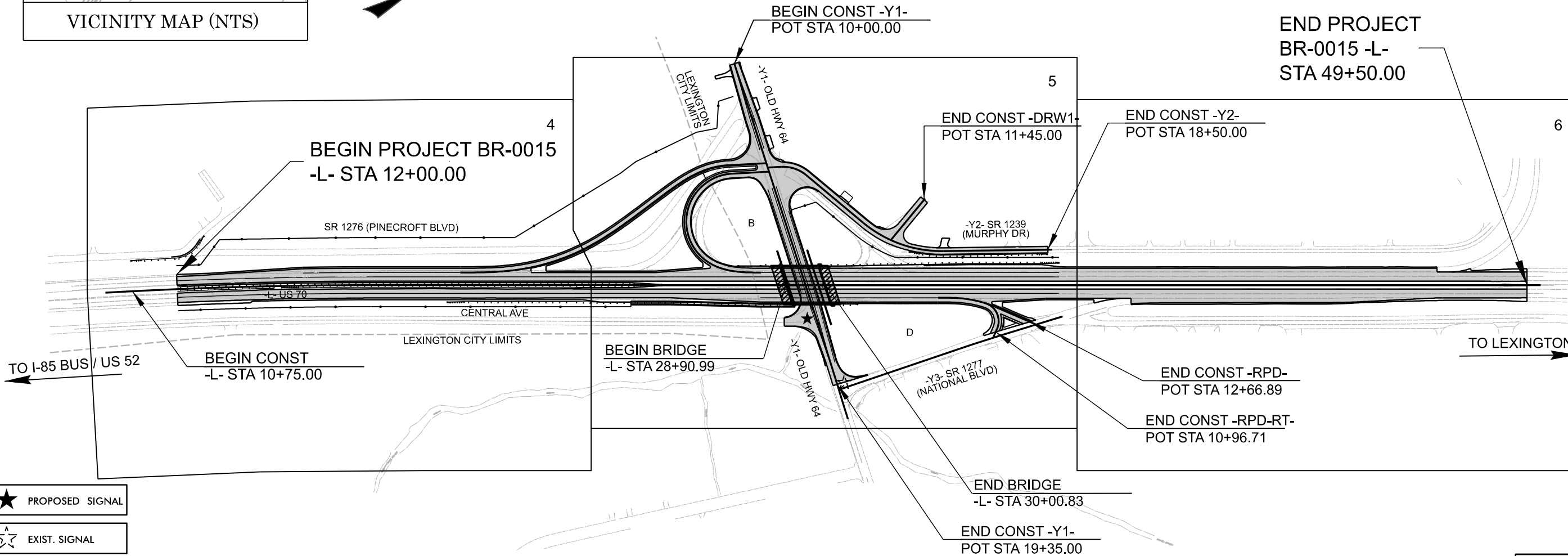
LOCATION: *BRIDGE 280067 AND 280068 REPLACEMENTS ON US 29/US 70.
NB & SB OVER SR 1192 (W. 5TH AVE.)*



TYPE OF WORK: *GRADING, DRAINAGE, PAVING, WIDENING AND STRUCTURES*

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0015	3	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
67015.1.1		PE	
67015.2.1		R/W, UTIL	
67015.3.1		CONST.	

STAGE 2 PLANS
(2RD1)

Plans Developed with
OpenRoads

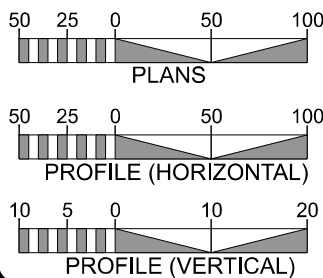


-  PROPOSED SIGNAL
-  EXIST. SIGNAL

THIS IS A CONTROL OF ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES.
A PORTION OF THIS PROJECT IS LOCATED WITHIN THE LIMITS OF THE CITY OF LEXINGTON.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.
**A DESIGN EXCEPTION IS REQUIRED FOR -RPD- AND -RPD_RT- MAXIMUM GRADE.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2022 = 14500
ADT 2045 = 18700
K = 8 %
D = 55 %
T = 9 % *
V = 60 MPH
* TTST =5% DUAL 4%
FUNC CLASS =
PRINCIPAL ARTERIAL
STATEWIDE TIER

PROJECT LENGTH

PROJECT LENGTHS FOR PROJECT BR-0015:

LENGTH ROADWAY PROJECT BR-0015	=	0.689 MILES
LENGTH STRUCTURES PROJECT BR-0015	=	0.021 MILES
TOTAL LENGTH PROJECT BR-0015	=	0.710 MILES

NCDOT Contact:

DAVID STUTTS, PE

Prepared in the Office of:



NC FIRM LICENSE No: C-1506
301 Fayetteville St.,
Suite 1500
Raleigh, NC 27601
(919) 882-7839

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
11/24/2023

LETTING DATE:
12/17/2024

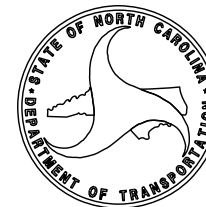
JACOB H. DUKE, P.E.
PROJECT ENGINEER

ANDREA B. GORDON, E.I.
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____
ROADWAY DESIGN ENGINEER

SIGNATURE: _____
P.E.





August 4, 2023
Kleinfelder File No. GSO23L155050

STATE PROJECT: 67015.1.1 (BR-0015)
COUNTY: Davidson
DESCRIPTION: Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

SUBJECT: **GEOTECHNICAL REPORT - INVENTORY**

PROJECT DESCRIPTION

This project consists of the widening of US 29/US 70 (-L-) and replacement of Bridge No. 67 and No. 68 over SR 1192 (-Y1-). At the project location, US 29/US 70 is a four-lane highway consisting of two lanes in the northbound and southbound direction with a grass median dividing the highway. Additionally, the project consists of the widening of SR 1192 (-Y1-), Forest Rose Drive (-DRW1-), and US 29/US 70 northbound exit ramps (-RPD-, -RPD_RT-) to National Boulevard as well as a realignment of Murphy Drive (-Y2-), onramp to US 29/US 70 southbound (-RPB-), and US 29/US 70 southbound exit ramp (-LPB-) to SR 1192.

Bridge No. 67 and No. 68 will be approximately 110 feet long (W.P. # 1 to W.P. #2) and 109.3 feet wide (out to out). The bridge will consist of a concrete deck with precast concrete girders. Abutment MSE retaining walls are proposed at end bent no. 1 and end bent no. 2. At end bent no. 1, a retaining wall will extend along US 29/US 70 northbound from station 24+60.37 -L- (54' RT) to 29+16.75 -L- (60' RT) and turn to run parallel along SR 1192 from station 29+16.75 -L- (60' RT) to 15+30.00 -Y1- (46' RT). At end bent no. 2, a retaining wall will extend parallel to SR 1192 from station 15+53.00 -Y1- (34' LT) to 17+73.00 -Y1- (34' LT). Maximum cut and fill heights to achieve finished grade are anticipated to be approximately 31 feet and 24 feet, respectively.

The geotechnical investigation was conducted in April and May 2023. Standard Penetration Test borings were advanced with a CME-55 drill rig with an automatic hammer. Hand augers were also performed in areas where the use of a drill rig was restricted, or overhead and underground utility conflicts were observed. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by Kleinfelder, Inc.

The following alignments, totaling 1.43 miles, were investigated. Plan sheets of these alignments are included in this report.

<u>LINE</u>	<u>STATIONS</u>
-L-	12+00.00 – 49+50.00
-Y1-	10+00.00 – 19+35.00
-Y2-	10+00.00 – 18+50.00
-RPB-	10+00.00 – 19+28.28
-RPD-	10+00.00 – 12+66.89
-RPD-RT-	10+00.00 – 10+96.71
-LPB-	10+00.00 – 15+92.00
-DRW1-	10+00.00 – 11+45.00

PHYSIOGRAPHY AND GEOLOGY

According to the 1985 Geologic Map of North Carolina, the project is located in the Charlotte Belt of the Piedmont Physiographic Province. The Charlotte Belt typically consists of metamorphosed igneous rocks and the bedrock unit encompassing the site is Metamorphosed Mafic Rock (PzZm). The Piedmont is characterized by gently rolling topography, well rounded hills, and long low ridges with a few hundred feet of elevation difference between the hills and valleys. It contains deeply weathered bedrock and relatively few rock outcrops. Erratic weathering of the rock can often result in varying depths of the residual soil profile. Additionally, it is not uncommon to encounter lenses or boulders of bedrock and zones of weathered rock within the residual soil, well above the general bedrock level.

The project corridor is comprised primarily of residential and rural properties. The general topography along the project is flat to gently sloping.

Surface water is drained from the corridor by the existing roadway ditches and culverts. Michael Branch runs along the east side of the project parallel to US 29/US 70.

SOIL PROPERTIES

Soils encountered during this investigation are separated into three categories based on origin. They consist of roadway embankment, alluvial soil, and residual soil.

Roadway embankment is defined as any material placed to raise grade for roadway construction and is present along the existing roadways on the project and inside exit/entry ramps. The roadway embankment encountered generally consist of moist to wet, soft to very stiff, fine sandy silts (A-4), clayey silts (A-5), and silty clays (A-7) with trace mica, gravel, and concrete fragments. The plasticity index of the roadway embankment clayey silt (A-5) tested is 9. The plasticity index of the roadway embankment silty clays (A-7) tested ranged from 22 to 38.

Alluvial soils are formed by deposition and are present underlying roadway embankment at the existing bridge abutments on US 29/US 70 (-L-), SR 1192 (-Y1-), and Central Avenue (-Y3-). The alluvial soils encountered generally consist of moist to saturated, very soft to stiff, silty clays (A-7) with trace mica and organic matter, and moist, loose, clayey fine to coarse sands (A-2-6).

The onsite residual soils are the product of the in-place chemical and mechanical weathering of the parent bedrock, and oftentimes maintain the same layering and lineation of the parent bedrock. Residual soils are derived from the weathering of underlying mafic metamorphic rock. The majority of the residual soil encountered consist of moist to saturated, soft to hard, fine and coarse to fine sandy silts (A-4), clayey silts (A-5), silty clays (A-7) with trace mica, rock fragments, and quartz fragments and wet, very dense, silty coarse to fine sands (A-2-4). The plasticity index of the residual silty clays (A-7) tested ranged from 12 to 44.

ROCK PROPERTIES

Weathered rock was encountered on and along US 29/US 70 (-L-) at the existing bridge abutments and Central Avenue (-Y3-) at elevations ranging from 666.4 to 691.5 feet Mean Sea Level (MSL). The weathered rock consists of Metagabbro. Crystalline rock was not encountered during the field investigation.

GROUNDWATER

Groundwater was encountered at elevations ranging from 694.9 to 722.1 feet. Typically, the groundwater depth ranges from 11.2 to 21.1 feet below the existing ground surface, where encountered.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

1) Highly Plastic Clays: Highly plastic clays (PI > 25) were encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	12+25 to 25+75	LT to RT
-L-	30+75 to 32+75	LT to RT
-L-	33+25 to 33+75	LT to RT
-L-	35+75 to 39+75	LT to RT
-L-	41+75 to 45+75	LT to RT
-Y1-	10+25 to 12+25	LT to RT
-Y1-	13+25 to 14+75	LT to RT
-Y2-	12+25 to 16+06	LT to RT
-RPB-	10+00 to 16+75	LT to RT
-RPB-	18+75 to 19+14	LT to RT
-LPB-	12+34 to 13+54	LT
-DRW1-	10+00 to 11+45	LT to RT

2) Groundwater: The following areas exhibit a high-water table, seasonal high groundwater, or the potential for groundwater related construction problems:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-Y1-	15+75 to 17+50	LT to RT

3) Alluvial Soil: Alluvial soil was encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	27+25 to 28+25	RT
-L-	28+25 to 29+25	LT to RT
-Y1-	14+50 to 17+50	RT

4) Wet Samples: Soils classified as wet based on visual inspection and laboratory test results were encountered at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	20+50 to 22+25	LT
-L-	26+00 to 27+25	RT
-L-	42+00 to 45+75	LT to RT
-LPB-	12+84 to 15+25	RT

Prepared by,
KLEINFELDER, INC.
NC License No. F-1312



Daniel H. Kubinski, PE
Senior Professional

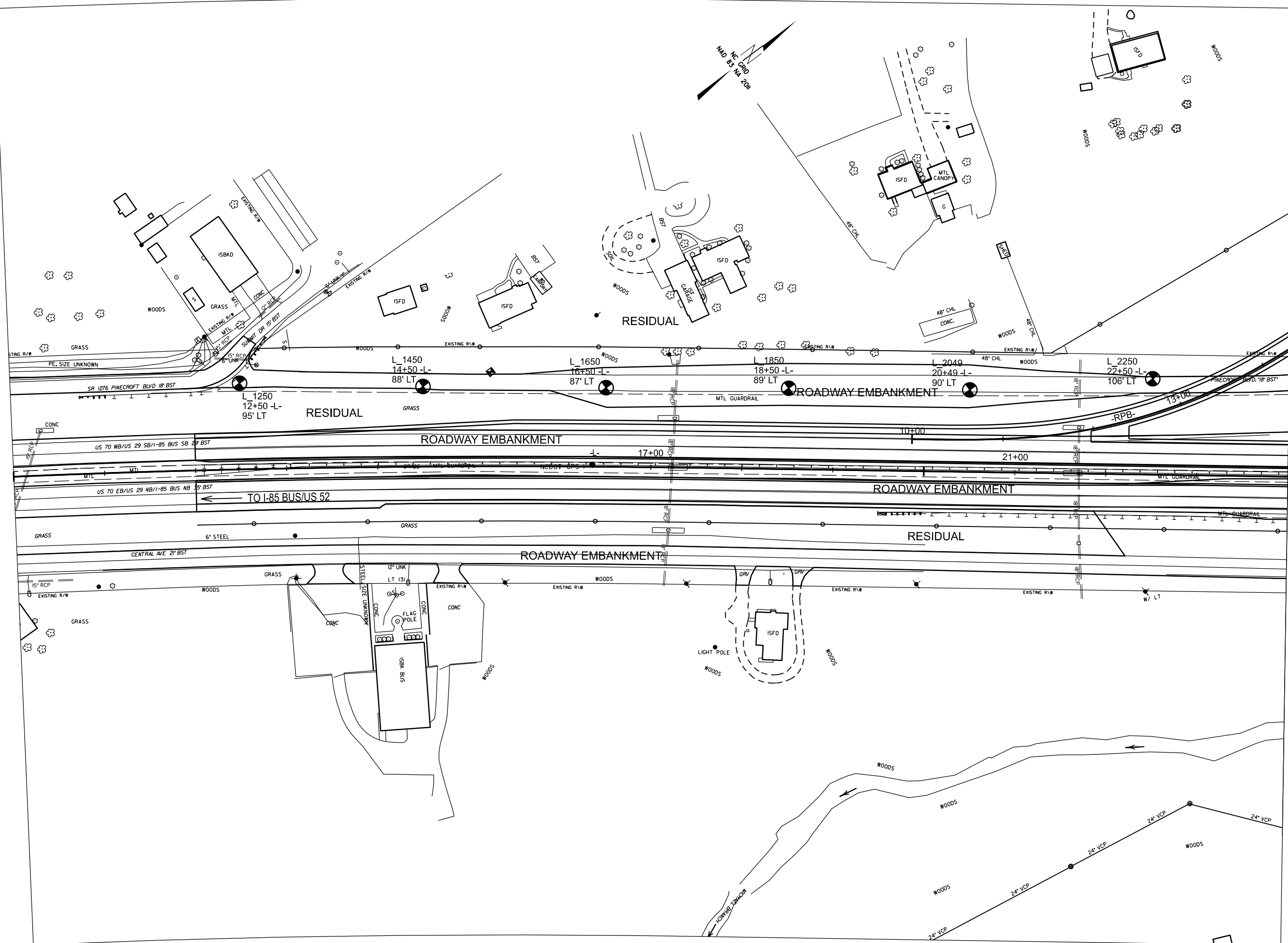
DHK/JDF:jrs

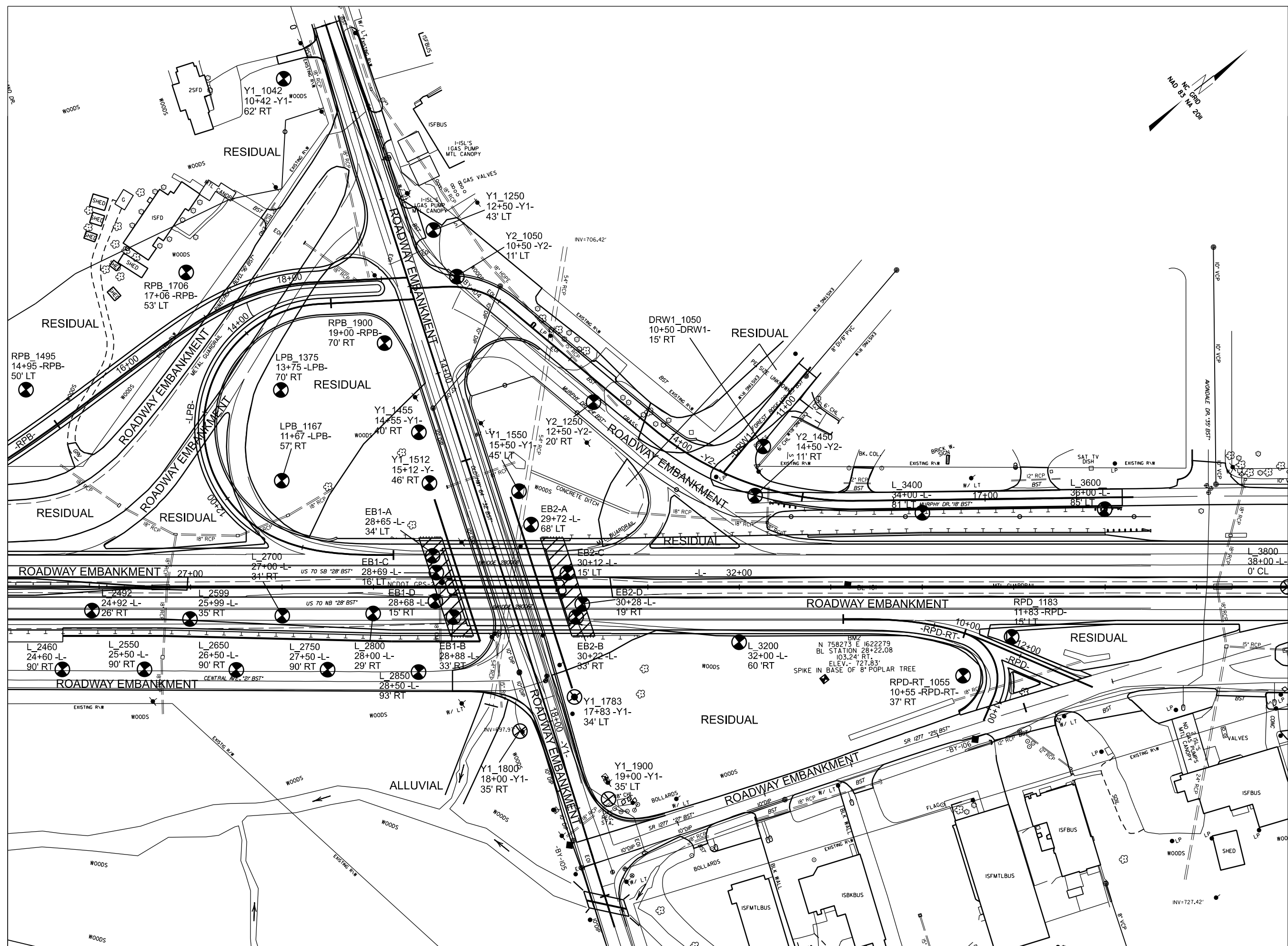
Bulk Samples

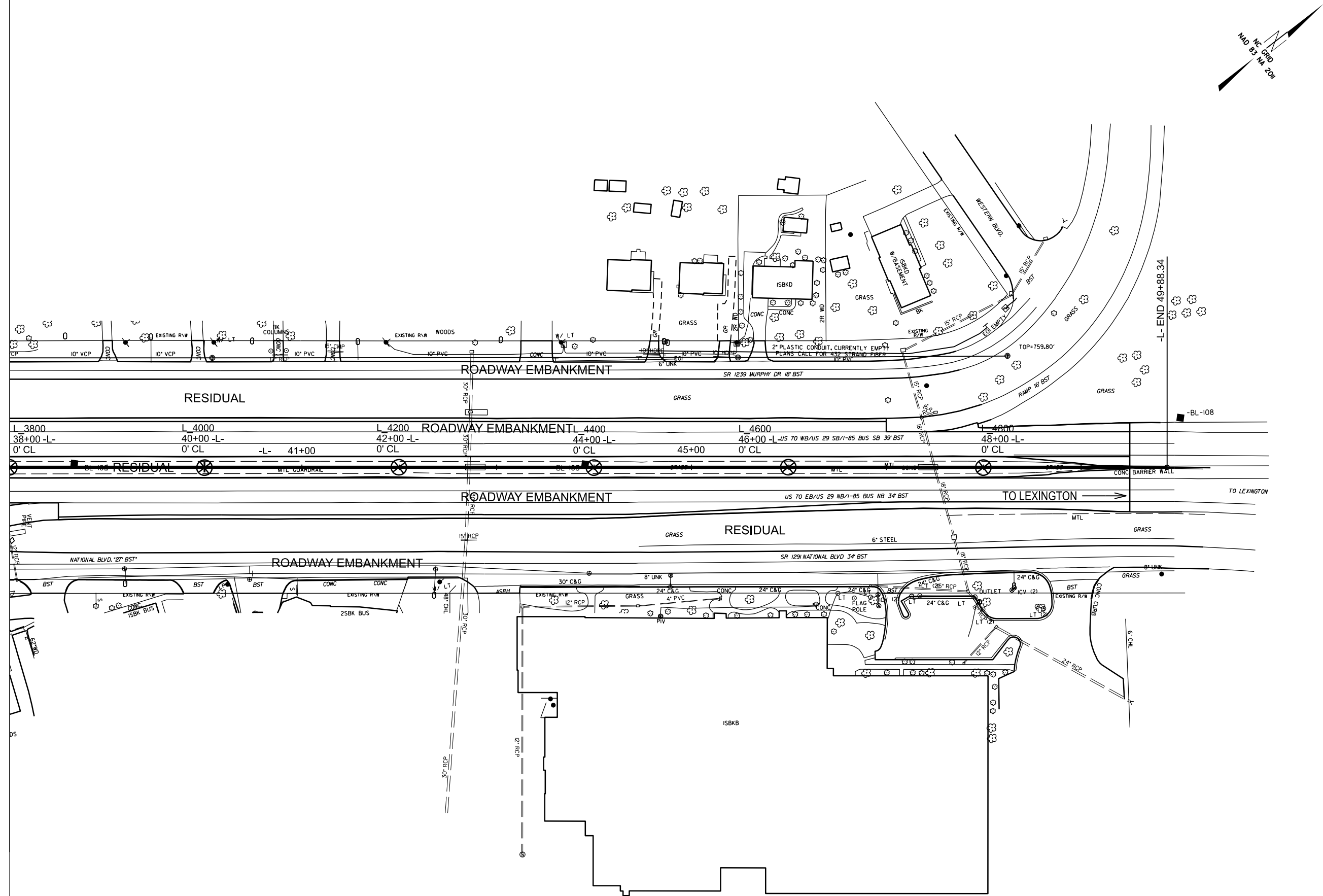
<u>Sample No.</u>	<u>Alignment</u>	<u>Station No.</u>	<u>Offset</u>	<u>Depth (ft)</u>	<u>Tests Performed</u>
CBR-1	-L-	14+50	88' LT	8.5 – 18.5	CBR
CBR-2	-L-	18+50	89' LT	0.0 – 10.0	CBR
CBR-3	-L-	42+00	0' CL	0.0 – 2.5	No Test



Joshua D. Fregosi, PE
Program Manager







PROJECT: 67015 REFERENCE: BR-0015

PROJECT: 67015 REFERENCE: BR-0015

PROJECT REFERENCE NO.	SHEET NO.
BR-0015	7

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
APPENDIX A
BORE LOGS

GEOTECHNICAL BORING REPORT
BORE LOG

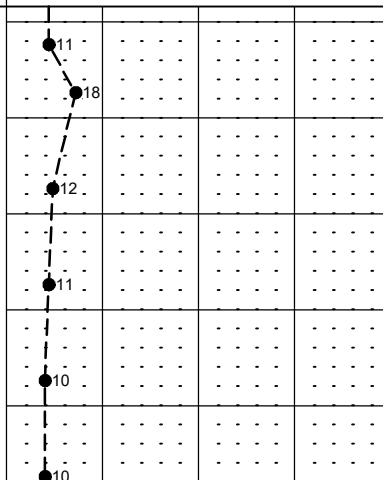
WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster							
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)					
BORING NO. L_1650			STATION 16+50			OFFSET 87 ft LT			ALIGNMENT -L-		0 HR. Dry				
COLLAR ELEV. 753.2 ft			TOTAL DEPTH 20.1 ft			NORTHING 757,089			EASTING 1,621,124		24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/08/23			COMP. DATE 05/08/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
755															
750	752.2	1.0	4	10	8									753.2 GROUND SURFACE 0.0	
	749.6	3.6	4	7	11							M		752.2 ROADWAY EMBANKMENT 1.0	
745	744.6	8.6	4	5	5									750.2 Asphalt and ABC Stone (0.0 - 1.0 Foot) 3.0	
	739.6	13.6	2	3	4							SS-3	27%	Very Stiff, Highly Plastic, Dark Reddish Brown and Brown, Silty CLAY, Trace Gravel	
735	734.6	18.6	2	3	4									RESIDUAL	
														Very Stiff to Stiff, Highly Plastic, Reddish Brown to Reddish Yellow, Silty CLAY	
														741.5 Medium Stiff, Yellowish Brown, Fine Sandy SILT, Trace Mica 11.7	


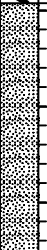
WBS 67015.1.1				TIP BR-0015				COUNTY DAVIDSON				GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)												GROUND WTR (ft)					
BORING NO. L_1850				STATION 18+50				OFFSET 89 ft LT				ALIGNMENT -L-				0 HR. Dry	
COLLAR ELEV. 753.2 ft				TOTAL DEPTH 20.0 ft				NORTHING 757,249				EASTING 1,621,245				24 HR. FIAD	
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022								DRILL METHOD H.S. Augers				HAMMER TYPE Automatic					
DRILLER R. Toothman				START DATE 05/08/23				COMP. DATE 05/08/23				SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
755																	
750	752.2	1.0		3	8	5	13							753.2 GROUND SURFACE 0.0			
	749.7	3.5		2	3	4	7							752.2 ROADWAY EMBANKMENT 1.0			
745														750.2 Asphalt and ABC Stone (0.0 - 1.0 Foot) 3.0			
	744.7	8.5		3	4	6	10							750.2 Stiff, Moderately Plastic, Dark Brown and Dark Reddish Brown, Silty CLAY, Trace Gravel 3.0			
740														RESIDUAL			
	739.7	13.5		2	5	5	10							Medium Stiff to Stiff, Moderately Plastic, Reddish Brown, Silty CLAY			
735														741.4 Stiff to Medium Stiff, Reddish Yellow to Yellowish Brown, Fine Sandy SILT, Trace Mica 11.8			
	734.7	18.5		2	3	4	7							733.2 Boring Terminated at Elevation 733.2 ft in RESIDUAL (Sandy SILT) 20.0			
														Other Samples: CBR-2 (0.0 - 10.0)			

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)						
BORING NO. L_2049			STATION 20+49			OFFSET 90 ft LT			ALIGNMENT -L-			0 HR. Dry			
COLLAR ELEV. 755.8 ft			TOTAL DEPTH 25.0 ft			NORTHING 757,407			EASTING 1,621,366			24 HR. FIAD			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 05/08/23			COMP. DATE 05/08/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
760															
755	754.8	1.0	6	5	6							M		755.8	0.0
750	752.3	3.5	5	8	10						SS-4	27%		754.8	1.0
	747.3	8.5	4	5	7									752.8	3.0
745											SS-5	43%		749.0	6.8
740	742.3	13.5	3	5	6							W			
735	737.3	18.5	3	4	6							W			
	732.3	23.5	2	4	6							M		733.8	22.0
														730.8	25.0
Boring Terminated at Elevation 730.8 ft in RESIDUAL (Sandy SILT)															

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. L_2250			STATION 22+50			OFFSET 106 ft LT			ALIGNMENT -L-			0 HR. Dry		
COLLAR ELEV. 757.2 ft			TOTAL DEPTH 35.0 ft			NORTHING 757,575			EASTING 1,621,477			24 HR. FIAD		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/08/23			COMP. DATE 05/08/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
760														
	757.2	0.0	3	8	9						SS-6	16%		757.2 GROUND SURFACE 0.0
755	753.7	3.5	3	3	4							M		RESIDUAL Very Stiff to Medium Stiff, Highly Plastic, Dark Reddish Brown to Reddish Yellow to Reddish Brown, Silty CLAY, Trace Mica and Rock Fragments
750	748.7	8.5	5	8	10						SS-7	32%		
745	743.7	13.5	2	2	3							M		
740	738.7	18.5	2	3	3						SS-8	43%		740.2 Medium Stiff, Slightly Plastic, Reddish Brown, Silty CLAY 17.0
735	733.7	23.5	2	2	3							M		735.4 Medium Stiff to Stiff, Yellowish Brown to Olive Brown and Pink, Coarse to Fine Sandy SILT, Trace Mica and Rock Fragments 21.8
730	728.7	28.5	2	3	5							M		
725	723.7	33.5	11	7	6							M		722.2 Boring Terminated at Elevation 722.2 ft in RESIDUAL (Sandy SILT) 35.0

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST J. Kardon						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)					
BORING NO. L_2550			STATION 25+50			OFFSET 90 ft RT			ALIGNMENT -L-		0 HR. Dry				
COLLAR ELEV. 718.8 ft			TOTAL DEPTH 15.0 ft			NORTHING 757,692			EASTING 1,621,815		24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/09/23			COMP. DATE 05/09/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
720															
715	717.8	1.0	5	2	3	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><di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WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. L_2599			STATION 25+99			OFFSET 35 ft RT			ALIGNMENT -L-			0 HR. Dry		
COLLAR ELEV. 735.6 ft			TOTAL DEPTH 25.0 ft			NORTHING 757,765			EASTING 1,621,802			24 HR. FIAD		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/05/23			COMP. DATE 05/05/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
740														
735														
	734.6	1.0	3	2	3	5						M		735.6 GROUND SURFACE 0.0
														734.9 ROADWAY EMBANKMENT 0.7
	732.0	3.6	2	2	3	5						M		733.3 Asphalt and ABC Stone (0.0 - 0.7 Foot) 2.3
730														Medium Stiff, Highly Plastic, Reddish Brown, Silty CLAY, Trace Mica
														Medium Stiff, Reddish Brown, Fine Sandy SILT, Trace Mica
	727.1	8.5	3	3	5	5								728.1 RESIDUAL 7.5
725										SS-10	39%			Medium Stiff, Highly Plastic, Reddish Brown to Reddish Yellow, Silty CLAY
	722.1	13.5	1	3	4	4						W		
720														
	717.1	18.5	2	4	4	4						M		718.6 Medium Stiff, Reddish Yellow and Light Brownish Gray to Yellowish Brown and Light Brownish Gray, Coarse to Fine Sandy SILT, Trace Mica and Rock Fragments 17.0
715														
	712.1	23.5	2	2	4	4						M		710.6 Boring Terminated at Elevation 710.6 ft in RESIDUAL (Sandy SILT) 25.0

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

[illegible]

NC DOT BORE DOUBLE BR0015 GEO RDWY_GINT.GPJ NC DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. L_2800			STATION 28+00			OFFSET 29 ft RT			ALIGNMENT -L-			0 HR. Dry		
COLLAR ELEV. 733.4 ft			TOTAL DEPTH 35.0 ft			NORTHING 757,927			EASTING 1,621,920			24 HR. FIAD		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/05/23			COMP. DATE 05/05/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
735														
730	729.9	3.5	2	2	3	5								733.4 GROUND SURFACE 0.0
														731.6 ROADWAY EMBANKMENT 1.8
														Asphalt and ABC Stone (0.0 - 1.8 Feet)
														Medium Stiff, Reddish Brown, Fine Sandy SILT, Little Mica
725	724.9	8.5	2	2	3	5								726.6 6.8
														Medium Stiff, Moderately Plastic, Reddish Brown and Olive Gray to Reddish Brown to Olive Brown, Silty CLAY, Trace Mica
720	719.9	13.5	2	3	4	7					SS-14	30%		
715	714.9	18.5	2	4	4	8						M		
710	709.9	23.5	2	2	3	5						M		
705	704.9	28.5	1	3	6	9						M		706.4 27.0
														ALLUVIAL
														Stiff, Highly Plastic, Dark Brown and Gray, Silty CLAY
700	699.9	33.5	2	4	5	9						M		701.6 31.8
														RESIDUAL
														Stiff, Light Brownish Gray, Silty CLAY
														698.4 35.0
														Boring Terminated at Elevation 698.4 ft in RESIDUAL (Silty CLAY)

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster				
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)				
BORING NO. EB1-A			STATION 28+65			OFFSET 34 ft LT			ALIGNMENT -L-			0 HR. N/A	
COLLAR ELEV. 733.4 ft			TOTAL DEPTH 54.3 ft			NORTHING 758,017			EASTING 1,621,910			24 HR. FIAD	
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD Mud Rotary			HAMMER TYPE Automatic				
DRILLER R. Toothman			START DATE 04/19/23			COMP. DATE 04/19/23			SURFACE WATER DEPTH N/A				
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			
735													
730	730.8	2.6	3	3	3								733.4 GROUND SURFACE 0.0
	728.9	4.5	2	2	2								732.4 ROADWAY EMBANKMENT 1.0
725													
	724.7	8.7	2	3	3								
720													
	719.8	13.6	WOH	1	2								722.4 Soft to Medium Stiff, Yellowish Brown, Fine Sandy SILT, Trace Mica 11.0
715													
	714.8	18.6	1	1	2								
710													
	709.8	23.6	1	2	2								
705													
	704.8	28.6	1	3	4								706.9 ALLUVIAL 26.5
700													
	699.8	33.6	WOH	2	3								
695													
	694.8	38.6	32	35	50								695.4 RESIDUAL 38.0
690													
	689.8	43.6	16	38	62/0.4								
685													
	684.8	48.6	62	38/0.1									688.8 WEATHERED ROCK 44.6
680													
	679.8	53.6	57	43/0.2									
													679.1 Boring Terminated at Elevation 679.1 ft in Weathered Rock (METAGABBRO) 54.3

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

[illegible]

NC DOT BORE DOUBLE BR0015 GEO RDWY_GINT.GPJ NC DOT GDT 7/20/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. EB2-A			STATION 29+72			OFFSET 68 ft LT			ALIGNMENT -L-		0 HR. 18.8			
COLLAR ELEV. 718.8 ft			TOTAL DEPTH 54.1 ft			NORTHING 758,123			EASTING 1,621,949		24 HR. 12.4			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 04/18/23			COMP. DATE 04/18/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
720														
	717.8	1.0	2	2	3	5						M		718.8 GROUND SURFACE 0.0
715	715.0	3.8	2	3	2	5						M		ROADWAY EMBANKMENT Medium Stiff, Reddish Brown, Silty CLAY
710	710.0	8.8	1	2	2	4						M		712.3 RESIDUAL 6.5 Medium Stiff, Reddish Brown to Dark Reddish Brown, Silty CLAY
705	705.0	13.8	1	2	4	6								
700	700.0	18.8	2	3	4	7						W		702.3 16.5 Medium Stiff to Hard, Reddish Brown, Gray, Brown, and Light Brownish Gray, Coarse to Fine Sandy SILT, Trace Mica
695	695.0	23.8	5	8	9	17						Sat.		
690	690.0	28.8	4	7	8	15						Sat.		
685	685.0	33.8	3	5	4	9						Sat.		
680	680.0	38.8	9	19	34	53						Sat.		
675	675.0	43.8	46	54/0.3						100/0.8				677.3 41.5 WEATHERED ROCK Olive Brown, METAGABBRO
670	670.0	48.8	65	35/0.2						100/0.7				
665	665.0	53.8	100/0.3							100/0.3				664.7 54.1 Boring Terminated at Elevation 664.7 ft in Weathered Rock (METAGABBRO)

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

[illegible]



GEOTECHNICAL BORING REPORT
BORE LOG


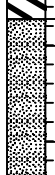

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)					
BORING NO. EB2-D			STATION 30+28			OFFSET 19 ft RT			ALIGNMENT -L-			0 HR. N/A			
COLLAR ELEV. 733.3 ft			TOTAL DEPTH 64.2 ft			NORTHING 758,114			EASTING 1,622,052			24 HR. 11.2			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD Mud Rotary				HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 04/26/23			COMP. DATE 04/26/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
735															
														733.3	0.0
														732.3	1.0
730	730.0	3.3	2	3	3	6						M		ROADWAY EMBANKMENT	
														Asphalt & ABC Stone (0.0 - 1.0 Foot)	
														Medium Stiff, Reddish Brown, Silty CLAY, Trace Mica	
725	724.8	8.5	3	3	3	6						M			
														721.4	11.9
720	720.0	13.3	3	4	4	8						M		RESIDUAL	
														Medium Stiff to Stiff, Reddish Brown and Yellowish Brown, Silty CLAY, Trace Mica	
715	715.0	18.3	3	5	6	11						W			
														711.7	21.6
710	710.0	23.3	3	3	4	7						W		Medium Stiff to Hard, Reddish Yellow, Light Brownish Gray, Brown, and Pale Red, Coarse to Fine Sandy SILT, Trace Mica	
705	705.0	28.3	2	2	3	5						Sat.			
700	700.0	33.3	6	7	17	24						Sat.			
695	695.0	38.3	10	8	7	15						Sat.			
690	690.0	43.3	6	11	15	26						Sat.			
685	685.0	48.3	10	19	28	47									
680	680.0	53.3	18	27	38	65						Sat.			
675	675.0	58.3	30	60	40/0.2									674.0	59.3
														WEATHERED ROCK	
670	670.0	63.3	16	84/0.4										Olive Gray and Light Brownish Gray, METAGABBRO	
														669.1	64.2
														Boring Terminated at Elevation 669.1 ft in Weathered Rock (METAGABBRO)	

NCDOT BORE DOUBLE BR0015_GEO ROWY_GINT.GPJ NC DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. L_3200			STATION 32+00			OFFSET 60 ft RT			ALIGNMENT -L-			0 HR. Dry		
COLLAR ELEV. 731.9 ft			TOTAL DEPTH 9.9 ft			NORTHING 758,224			EASTING 1,622,190			24 HR. Dry		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 04/24/23			COMP. DATE 04/24/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
735														
	731.9	0.0												731.9 GROUND SURFACE 0.0
730			2	11	11							M		RESIDUAL Very Stiff to Stiff, Yellowish Brown and Olive Gray to Reddish Brown and Reddish Yellow, Coarse to Fine Sandy SILT, Trace Mica and Rock Fragments
	728.5	3.4										M		
			3	4	6							M		
725												M		
	723.5	8.4	2	4	5									722.0 9.9
Boring Terminated at Elevation 722.0 ft in RESIDUAL (Sandy SILT)														

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster							
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)							
BORING NO. L_3400			STATION 34+00			OFFSET 81 ft LT			ALIGNMENT -L-			0 HR. Dry				
COLLAR ELEV. 742.6 ft			TOTAL DEPTH 20.0 ft			NORTHING 758,469			EASTING 1,622,201			24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic							
DRILLER R. Toothman			START DATE 05/04/23			COMP. DATE 05/04/23			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
745																
740	742.6	0.0	4	2	4						SS-15	26%		742.6	GROUND SURFACE 0.0	
	739.1	3.5	3	5	6							M			735.8	6.8
735	734.1	8.5	2	4	3							M	Medium Stiff, Yellowish Brown, Fine Sandy SILT, Trace Mica			
730	729.1	13.5	2	3	4							M				
	724.1	18.5	2	3	5							M				
725												M		722.6	20.0	
														Boring Terminated at Elevation 722.6 ft in RESIDUAL (Sandy SILT)		

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)				
BORING NO. L_3600			STATION 36+00			OFFSET 85 ft LT			ALIGNMENT -L-		0 HR. Dry			
COLLAR ELEV. 748.1 ft			TOTAL DEPTH 20.0 ft			NORTHING 758,629			EASTING 1,622,320		24 HR. Dry			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic				
DRILLER R. Toothman			START DATE 05/04/23			COMP. DATE 05/04/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
750														
745	748.1	0.0	3	4	5							M		748.1 GROUND SURFACE 0.0
	744.6	3.5	4	5	8						SS-16	32%		RESIDUAL Stiff, Highly Plastic, Dark Reddish Brown to Red to Reddish Yellow, Silty CLAY
740	739.6	8.5	5	7	8							M		736.4 11.7 Stiff to Medium Stiff, Reddish Yellow, Fine Sandy SILT
	734.6	13.5	3	4	6							M		
730	729.6	18.5	3	4	4							M		728.1 20.0 Boring Terminated at Elevation 728.1 ft in RESIDUAL (Sandy SILT)
					</									

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster				
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)								GROUND WTR (ft)				
BORING NO. L_3800		STATION 38+00		OFFSET CL		ALIGNMENT -L-		0 HR. Dry				
COLLAR ELEV. 741.1 ft		TOTAL DEPTH 7.0 ft		NORTHING 758,735		EASTING 1,622,510		24 HR. Dry				
DRILL RIG/HAMMER EFF./DATE N/A				DRILL METHOD Hand Auger			HAMMER TYPE N/A					
DRILLER M. Foster		START DATE 04/26/23		COMP. DATE 04/26/23		SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100		
745												
740												741.1 GROUND SURFACE 0.0
												RESIDUAL Medium Stiff to Stiff, Highly Plastic, Reddish Brown to Yellowish Brown, Silty CLAY
735												734.1 Boring Terminated at Elevation 734.1 ft in RESIDUAL (Silty CLAY) 7.0

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster				
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)								GROUND WTR (ft)				
BORING NO. L_4000		STATION 40+00		OFFSET CL		ALIGNMENT -L-		0 HR. Dry				
COLLAR ELEV. 744.6 ft		TOTAL DEPTH 6.8 ft		NORTHING 758,893		EASTING 1,622,633		24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE N/A				DRILL METHOD Hand Auger			HAMMER TYPE N/A					
DRILLER M. Foster		START DATE 04/27/23		COMP. DATE 04/27/23		SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100		
745												744.6 GROUND SURFACE 0.0
740												742.4 RESIDUAL Soft to Medium Stiff, Moderately Plastic, Olive Gray to Reddish Brown, Silty CLAY, Trace Mica 2.2
												737.8 Medium Stiff, Reddish Brown, Fine Sandy SILT, Trace Mica 6.8
Boring Terminated at Elevation 737.8 ft in RESIDUAL (Sandy SILT)												
NOTE: Kessler dual mass dynamic cone penetrometer test performed on 5/4/2023 adjacent to hand auger boring.												

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23



NCDOT BORE DOUBLE BR0015 GEO RDWY GINT.GPJ NC DOT.GDT 6/15/23

WBS 67015.1.1				TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST D. Kubinski							
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)							
BORING NO. L_4400				STATION 44+00			OFFSET CL			ALIGNMENT -L-			0 HR. Dry				
COLLAR ELEV. 747.3 ft				TOTAL DEPTH 6.5 ft			NORTHING 759,209			EASTING 1,622,878			24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE N/A							DRILL METHOD Hand Auger				HAMMER TYPE N/A						
DRILLER D. Kubinski				START DATE 05/02/23			COMP. DATE 05/02/23			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	<div><div>L O G</div></div>	SOIL AND ROCK DESCRIPTION				
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					MOI		
750																	
														747.3	GROUND SURFACE 0.0		
745														745.8	RESIDUAL Soft, Highly Plastic, Reddish Brown, Silty CLAY, Trace Mica		-1.5
															Medium Stiff, Olive, Coarse to Fine Sandy SILT, Trace Mica		
														740.8	Boring Terminated at Elevation 740.8 ft in RESIDUAL (Sandy SILT)		6.5

NCDOT BORE DOUBLE BR0015 GEO RDWY GINT.GPJ NC DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST D. Kubinski				
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)				
BORING NO. L_4800			STATION 48+00			OFFSET CL			ALIGNMENT -L-			0 HR. Dry	
COLLAR ELEV. 744.6 ft			TOTAL DEPTH 6.7 ft			NORTHING 759,525			EASTING 1,623,123			24 HR. FIAD	
DRILL RIG/HAMMER EFF./DATE N/A						DRILL METHOD Hand Auger				HAMMER TYPE N/A			
DRILLER D. Kubinski			START DATE 05/03/23			COMP. DATE 05/03/23			SURFACE WATER DEPTH N/A				
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100	MOI		
745													
740											M		744.6 GROUND SURFACE 0.0 743.6 RESIDUAL 1.0 Soft, Yellowish Brown and Reddish Brown, Silty CLAY Soft to Medium Stiff, Brown, Coarse to Fine Sandy SILT, Trace Mica 737.9 Boring Terminated at Elevation 737.9 ft in RESIDUAL (Sandy SILT) 6.7 NOTE: Kessler dual mass dynamic cone penetrometer test performed on 5/4/2023 adjacent to hand auger boring.



GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)				
BORING NO. Y1_1042			STATION 10+42			OFFSET 62 ft RT			ALIGNMENT -Y1-		0 HR. Dry			
COLLAR ELEV. 753.3 ft			TOTAL DEPTH 24.8 ft			NORTHING 758,208			EASTING 1,621,398		24 HR. FIAD			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic				
DRILLER R. Toothman			START DATE 05/10/23			COMP. DATE 05/10/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	▼ MOI	L O G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
755														
	753.3	0.0												753.3 GROUND SURFACE 0.0
750	750.0	3.3	3	5	7	12					SS-21	28%		RESIDUAL Stiff, Highly Plastic, Reddish Brown and Reddish Yellow, Silty CLAY, Trace Mica
745	745.0	8.3	4	6	6	12						M		745.8 Stiff, Yellowish Brown, Coarse to Fine Sandy SILT, Trace Mica 7.5
740	740.0	13.3	5	7	6	13						M		
735	735.0	18.3	4	6	7	13						M		
730	730.0	23.3	3	4	5	9						M		
			3	4	5	9						M		728.5 Boring Terminated at Elevation 728.5 ft in RESIDUAL (Sandy SILT) 24.8

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)						
BORING NO. Y1_1250			STATION 12+50			OFFSET 43 ft LT			ALIGNMENT -Y1-			0 HR. Dry			
COLLAR ELEV. 732.2 ft			TOTAL DEPTH 10.0 ft			NORTHING 758,236			EASTING 1,621,629			24 HR. FIAD			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 05/08/23			COMP. DATE 05/08/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
735															
730	731.2	1.0	4	5	6	11	SS-22	31%	731.8	0.4
	728.7	3.5	3	4	5	9		M	728.7	3.5
725	723.7	8.5	2	3	3	6		M	723.7	8.5

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster								
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)						
BORING NO. Y1_1455			STATION 14+55			OFFSET 40 ft RT			ALIGNMENT -Y1-		0 HR. Dry					
COLLAR ELEV. 718.8 ft			TOTAL DEPTH 10.0 ft			NORTHING 758,088			EASTING 1,621,794		24 HR. FIAD					
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 05/08/23			COMP. DATE 05/08/23			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
720																
715	718.8	0.0	1	2	3	5						M		718.8	GROUND SURFACE	0.0
	715.3	3.5	2	3	3	6						M		ROADWAY EMBANKMENT Medium Stiff, Highly Plastic, Reddish Brown and Olive Brown to Reddish Brown and Olive Gray, Silty CLAY, Trace Mica		
710	710.3	8.5	1	3	3	6						M		711.3	ALLUVIAL	7.5
														708.8	Medium Stiff, Gray, Silty CLAY, Trace Mica Boring Terminated at Elevation 708.8 ft in ALLUVIAL (Silty CLAY)	10.0

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)											GROUND WTR (ft)			
BORING NO. Y1_1512			STATION 15+12			OFFSET 46 ft RT			ALIGNMENT -Y1-			0 HR.	Dry	
COLLAR ELEV. 715.1 ft			TOTAL DEPTH 19.9 ft			NORTHING 758,063			EASTING 1,621,845			24 HR.	FIAD	
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic				
DRILLER R. Toothman			START DATE 05/10/23			COMP. DATE 05/10/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
720														
715	715.1	0.0												715.1 GROUND SURFACE 0.0
710	711.7	3.4	2	3	2									ROADWAY EMBANKMENT
			2	2	2									Medium Stiff, Reddish Brown, Silty CLAY, Trace Mica and Gravel
705	706.7	8.4	WOH	1	3						SS-23	39%		Medium Stiff, Slightly Plastic, Reddish Yellow, Clayey SILT, Trace Mica
														ALLUVIAL
700	701.7	13.4	WOH	2	2									Medium Stiff to Stiff, Highly Plastic, Dark Gray and Dark Brown to Dark Reddish Brown and Dark Brown, Silty CLAY, Trace Mica and Organic Matter (Topsoil, Wood Fragments)
	696.7	18.4	5	7	8									

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)											GROUND WTR (ft)				
BORING NO. Y1_1550			STATION 15+50			OFFSET 45 ft LT			ALIGNMENT -Y1-			0 HR.	Dry		
COLLAR ELEV. 717.8 ft			TOTAL DEPTH 13.5 ft			NORTHING 758,135			EASTING 1,621,912			24 HR.	Dry		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers				HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 04/18/23			COMP. DATE 04/18/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
720															
715	716.8	1.0	4	4	3	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><di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WBS 67015.1.1			TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST D. Kubinski								
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)					
BORING NO. Y1_1783			STATION 17+83			OFFSET 34 ft LT			ALIGNMENT -Y1-		0 HR. Dry				
COLLAR ELEV. 715.2 ft			TOTAL DEPTH 10.3 ft			NORTHING 758,045			EASTING 1,622,127		24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE N/A						DRILL METHOD Hand Auger				HAMMER TYPE N/A					
DRILLER D. Kubinski			START DATE 05/02/23			COMP. DATE 05/02/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER INCREMENT			SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION			
			1.75in	1.75in	1.75in	0	25								
720															
715															
710	714.2	1.0	11	12	12										
	713.2	2.0	7	8	13										
	712.2	3.0	9	9	12										
	711.2	4.0	22	25+											
	710.2	5.0	19	23	25										
705	709.2	6.0	13	25	25+										
	708.2	7.0	15	25+											
	707.2	8.0	25+												
	706.2	9.0	24	25+											
	705.2	10.0	24	25+											
715.2 GROUND SURFACE 0.0															
RESIDUAL Medium Stiff to Stiff, Olive, Coarse to Fine Sandy SILT, Trace Mica															
704.9 10.3															
Boring Terminated at Elevation 704.9 ft in RESIDUAL (Sandy SILT)															
NOTE: Conventional dynamic cone penetrometer test performed at 1 foot intervals.															

NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/14/23

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST D. Kubinski					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)								GROUND WTR (ft)					
BORING NO. Y1_1800		STATION 18+00		OFFSET 35 ft RT		ALIGNMENT -Y1-		0 HR. Dry					
COLLAR ELEV. 713.9 ft		TOTAL DEPTH 7.0 ft		NORTHING 757,975		EASTING 1,622,120		24 HR. FIAD					
DRILL RIG/HAMMER EFF./DATE N/A				DRILL METHOD Hand Auger		HAMMER TYPE N/A							
DRILLER D. Kubinski		START DATE 05/04/23		COMP. DATE 05/04/23		SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft) DEPTH (ft)
715													713.9 GROUND SURFACE 0.0
710											M		RESIDUAL Medium Stiff to Stiff, Brown to Light Brownish Gray, Coarse to Fine Sandy SILT
											M		706.9 7.0
													Boring Terminated at Elevation 706.9 ft in RESIDUAL (Sandy SILT)

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST D. Kubinski					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)								GROUND WTR (ft)					
BORING NO. Y1_1900		STATION 19+00		OFFSET 35 ft LT		ALIGNMENT -Y1-		0 HR. Dry					
COLLAR ELEV. 705.9 ft		TOTAL DEPTH 6.7 ft		NORTHING 758,006		EASTING 1,622,238		24 HR. FIAD					
DRILL RIG/HAMMER EFF./DATE N/A				DRILL METHOD Hand Auger		HAMMER TYPE N/A							
DRILLER D. Kubinski		START DATE 05/04/23		COMP. DATE 05/04/23		SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			SAMP. NO.	MOI	L O G	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft) DEPTH (ft)
710													705.9 GROUND SURFACE 0.0
705											M		RESIDUAL Soft to Medium Stiff, Light Brownish Gray, Coarse to Fine Sandy SILT
700											M		699.2 6.7
													Boring Terminated at Elevation 699.2 ft in RESIDUAL (Sandy SILT)

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23


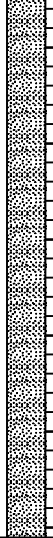
GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)						
BORING NO. Y2_1050			STATION 10+50			OFFSET 11 ft LT			ALIGNMENT -Y2-			0 HR. Dry			
COLLAR ELEV. 729.5 ft			TOTAL DEPTH 9.9 ft			NORTHING 758,225			EASTING 1,621,685			24 HR. FIAD			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 04/28/23			COMP. DATE 04/28/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
730															
	729.5	0.0				1	3	4						729.5	0.0
						1									
725	726.0	3.5				21	3	4							
						1									
						1									
720	721.1	8.4				3	2	2						722.8	6.7

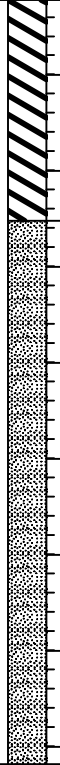
WBS 67015.1.1				TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST J. Kardon				
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)										GROUND WTR (ft)				
BORING NO. Y2_1250				STATION 12+50			OFFSET 20 ft RT			ALIGNMENT -Y2-			0 HR. Dry	
COLLAR ELEV. 730.5 ft				TOTAL DEPTH 15.0 ft			NORTHING 758,259			EASTING 1,621,885			24 HR. FIAD	
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022							DRILL METHOD H.S. Augers				HAMMER TYPE Automatic			
DRILLER R. Toothman				START DATE 05/09/23			COMP. DATE 05/09/23			SURFACE WATER DEPTH N/A				
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
735														
730														
	729.5	1.0		3	3	3								
725	727.0	3.5		2	4	4								
720	722.0	8.5		3	3	3								
	717.0	13.5		2	2	3								

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23




NCDOT BORE DOUBLE BR0015_GEO_RDWY_GINT.GPJ NC_DOT.GDT 6/15/23

WBS 67015.1.1				TIP BR-0015		COUNTY DAVIDSON		GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)								GROUND WTR (ft)						
BORING NO. RPB_1495		STATION 14+95		OFFSET 50 ft LT		ALIGNMENT -RPB-		0 HR. Dry						
COLLAR ELEV. 764.4 ft		TOTAL DEPTH 39.9 ft		NORTHING 757,777		EASTING 1,621,494		24 HR. Dry						
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers		HAMMER TYPE Automatic						
DRILLER R. Toothman		START DATE 04/28/23		COMP. DATE 04/28/23		SURFACE WATER DEPTH N/A								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
765														
	764.4	0.0												GROUND SURFACE 0.0
760	760.9	3.5	1	3	6							M		RESIDUAL Stiff to Very Stiff, Highly Plastic, Red, Silty CLAY
			5	9	10							M		
755	756.0	8.4												757.4 Stiff, Moderately Plastic, Red, Silty CLAY 7.0
			3	5	7						SS-26	34%		
750	751.0	13.4												752.7 Medium Stiff to Stiff, Red to Reddish Brown to Yellowish Brown to Light Brownish Gray, Fine Sandy SILT, Trace Mica 11.7
			2	3	4							M		
745	746.0	18.4										M		
			3	4	4							M		
740	741.0	23.4										M		
			2	5	6							M		
735	736.0	28.4										M		
			2	4	6							M		
730	731.0	33.4										M		
			2	4	5							M		
725	726.0	38.4										M		
			2	3	5							M		
														Boring Terminated at Elevation 724.5 ft in RESIDUAL (Sandy SILT)


GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)						
BORING NO. RPB_1706			STATION 17+06			OFFSET 53 ft LT			ALIGNMENT -RPB-			0 HR.	Dry		
COLLAR ELEV. 758.9 ft			TOTAL DEPTH 39.8 ft			NORTHING 757,994			EASTING 1,621,500			24 HR.	Dry		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 04/28/23			COMP. DATE 04/28/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
760															
	758.9	0.0												758.9	0.0
			WOH	WOH	2							M		RESIDUAL Soft to Medium Stiff, Moderately Plastic, Reddish Brown, Silty CLAY, Trace Mica	
755	755.6	3.3	4	6	2						SS-27	33%			
750	750.4	8.5	2	3	4							M			
745	745.4	13.5	4	5	6							M			
740	740.4	18.5	4	7	7							M			
735	735.4	23.5	4	6	8							M			
730	730.4	28.5	5	6	8							M			
725	725.4	33.5	5	7	8							M			
720	720.6	38.3	3	6	7							M			
													Boring Terminated at Elevation 719.1 ft in RESIDUAL (Sandy SILT)		

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster							
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)							
BORING NO. RPB_1900			STATION 19+00			OFFSET 70 ft RT			ALIGNMENT -RPB-			0 HR.	Dry			
COLLAR ELEV. 729.7 ft			TOTAL DEPTH 19.5 ft			NORTHING 758,118			EASTING 1,621,694			24 HR.	Dry			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic							
DRILLER R. Toothman			START DATE 04/19/23			COMP. DATE 04/19/23			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
730																
725	728.7	1.0									SS-28	38%		729.7	GROUND SURFACE	0.0
	726.7	3.0	2	3	3									M	RESIDUAL Medium Stiff, Highly Plastic, Reddish Brown, Silty CLAY, Trace Mica	
720	721.7	8.0												724.2	5.5	Medium Stif to Soft, Reddish Yellow to Reddish Yellow and Light Brownish Gray to Reddish Yellow and Gray, Fine Sandy SILT, Trace Mica
	716.7	13.0	2	3	3									M		
715	711.7	18.0	1	1	2											Boring Terminated at Elevation 710.2 ft in RESIDUAL (Sandy SILT)
														M	710.2	






GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster						
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)						
BORING NO. RPD_1183			STATION 11+83			OFFSET 15 ft LT			ALIGNMENT -RPD-			0 HR. Dry			
COLLAR ELEV. 734.7 ft			TOTAL DEPTH 10.0 ft			NORTHING 758,463			EASTING 1,622,368			24 HR. FIAD			
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic						
DRILLER R. Toothman			START DATE 05/04/23			COMP. DATE 05/04/23			SURFACE WATER DEPTH N/A						
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)
735														734.7	GROUND SURFACE 0.0
730	734.7	0.0	2	2	3	5		M		RESIDUAL Medium Stiff, Brown, Coarse to Fine Sandy SILT, Trace Mica	
	731.2	3.5	2	2	3	5					
725	726.2	8.5	2	3	4	7		M			724.7
													Boring Terminated at Elevation 724.7 ft in RESIDUAL (Sandy SILT)		



WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. RPD-RT_1055			STATION 10+55			OFFSET 37 ft RT			ALIGNMENT -RPD-RT-			0 HR. Dry		
COLLAR ELEV. 730.3 ft			TOTAL DEPTH 9.8 ft			NORTHING 758,395			EASTING 1,622,369			24 HR. FIAD		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 05/01/23			COMP. DATE 05/01/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
735														
730	730.3	0.0	1	2	2									730.3 GROUND SURFACE 0.0
725	727.0	3.3	1	2	3						SS-29	31%		RESIDUAL Medium Stiff, Moderately Plastic, Reddish Brown, Silty CLAY, Trace Mica Medium Stiff, Yellowish Brown, Fine Sandy SILT, Trace Mica
			1	2	3									
	722.0	8.3	2	3	4									
														720.5 Boring Terminated at Elevation 720.5 ft in RESIDUAL (Sandy SILT) 9.8

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

NCDOT BORE DOUBLE BR0015 GEO RDWY GINT.GPJ NC DOT.GDT 6/15/23

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster					
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)					
BORING NO. LPB_1375			STATION 13+75			OFFSET 70 ft RT			ALIGNMENT -LPB-			0 HR. Dry		
COLLAR ELEV. 737.5 ft			TOTAL DEPTH 20.0 ft			NORTHING 757,997			EASTING 1,621,665			24 HR. Dry		
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic					
DRILLER R. Toothman			START DATE 04/19/23			COMP. DATE 04/19/23			SURFACE WATER DEPTH N/A					
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
740														
735	736.4	1.1	2	3	3						SS-30	40%		737.5 GROUND SURFACE 0.0
	734.0	3.5	2	4	4									731.5 RESIDUAL Medium Stiff, Moderately Plastic, Reddish Yellow, Silty CLAY 6.0
730												W		731.5 Stiff to Medium Stiff, Reddish Yellow to Light Brownish Gray to Yellowish Brown, Fine Sandy SILT 6.0
725	729.0	8.5	2	4	5							M		
720	724.0	13.5	3	3	5							M		
	719.0	18.5	2	3	5							M		717.5 Boring Terminated at Elevation 717.5 ft in RESIDUAL (Sandy SILT) 20.0

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 67015.1.1			TIP BR-0015			COUNTY DAVIDSON			GEOLOGIST M. Foster							
SITE DESCRIPTION Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)									GROUND WTR (ft)							
BORING NO. DRW1_1050			STATION 10+50			OFFSET 15 ft RT			ALIGNMENT -DRW1-			0 HR. Dry				
COLLAR ELEV. 737.5 ft			TOTAL DEPTH 20.0 ft			NORTHING 758,376			EASTING 1,622,037			24 HR. FIAD				
DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 83% 05/09/2022						DRILL METHOD H.S. Augers			HAMMER TYPE Automatic							
DRILLER R. Toothman			START DATE 04/25/23			COMP. DATE 04/25/23			SURFACE WATER DEPTH N/A							
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
740																
735	737.5	0.0	3	7	5						SS-31	31%		737.5	0.0	
	734.0	3.5	3	5	6							M				
730												M			730.7	6.8
	729.0	8.5	2	4	4							M			Medium Stiff to Stiff, Reddish Brown to Yellowish Brown to Light Brown, Fine Sandy SILT, Trace Mica	
725												M				
	724.0	13.5	2	4	5							M				
720												M				
	719.0	18.5	2	4	5							M		717.5	20.0	
														Boring Terminated at Elevation 717.5 ft in RESIDUAL (Sandy SILT)		

NCDOT BORE DOUBLE BR0015_GEO_ROWY_GINT.GPJ NC_DOT.GDT 6/15/23

PROJECT: 67015 REFERENCE: BR-0015

PROJECT REFERENCE NO.	SHEET NO.
BR-0015	34

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
APPENDIX B
LABORATORY RESULTS

LABORATORY SUMMARY SHEET FOR SOIL SAMPLES

WBS NO. (TIP NO.): 67015.1.1 (BR-0015)
PROJECT ID: 41620
COUNTY: DAVIDSON
DESCRIPTION: BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)

									Atterberg Limits			Gradation Results							
Sample No.	Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	N-Value (blows/ft)	L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
SS-1	L_1250	-L-	12+50	95' LT	0.4 - 1.9	30.3	A-7-6	10	74	34	40	0.0	98.8	85.5	68.7	19.9	14.4	11.9	53.8
SS-2	L_1450	-L-	14+50	88' LT	3.5 - 5.0	24.1	A-7-6	17	54	28	26	0.0	99.0	84.0	64.0	21.3	20.7	25.1	33.0
SS-3	L_1650	-L-	16+50	87' LT	3.6 - 5.1	26.7	A-7-5	18	76	33	43	0.0	99.0	93.8	79.2	11.5	11.5	36.4	40.6
SS-4	L_2049	-L-	20+49	90' LT	3.5 - 5.0	27.3	A-7-5	18	68	32	36	0.0	100.0	97.2	77.8	8.5	16.5	42.4	32.6
SS-5	L_2049	-L-	20+49	90' LT	8.5 - 10.0	42.8	A-7-5	12	58	40	18	0.0	100.0	97.3	65.0	12.2	29.2	26.3	32.3
SS-6	L_2250	-L-	22+50	106' LT	0.0 - 1.5	16.2	A-7-6	17	48	21	27	2.0	96.0	89.9	66.3	17.4	20.1	20.0	42.5
SS-7	L_2250	-L-	22+50	106' LT	8.5 - 10.0	31.5	A-7-5	18	86	42	44	16.0	100.0	95.5	81.8	9.0	11.7	19.0	60.3
SS-8	L_2250	-L-	22+50	106' LT	18.5 - 20.0	42.6	A-7-5	6	62	50	12	0.0	100.0	96.0	68.5	7.2	32.9	27.6	32.2
SS-9	L_2460	-L-	24+60	90' RT	1.0 - 2.5	37.1	A-7-5	6	78	45	33	0.0	100.0	97.8	86.5	4.2	13.2	23.4	59.2
SS-10	L_2599	-L-	25+99	35' RT	8.5 - 10.0	39.4	A-7-5	8	65	34	31	0.0	100.0	99.4	80.3	2.4	25.4	34.6	37.7
SS-11	L_2650	-L-	26+50	90' RT	1.0 - 2.5	32.1	A-7-5	7	73	35	38	2.0	97.0	94.5	79.0	8.8	16.4	21.4	53.4
SS-12	L_2700	-L-	27+00	31' RT	3.7 - 5.2	32.5	A-7-5	6	52	30	22	0.0	100.0	90.1	64.0	16.3	25.7	25.6	32.5
SS-13	L_2750	-L-	27+50	90' RT	1.0 - 2.5	21.7	A-7-5	5	56	31	25	3.0	92.0	88.3	63.3	18.1	24.2	23.0	34.6
SS-14	L_2800	-L-	28+00	29' RT	13.5 - 15.0	29.7	A-7-5	7	58	34	24	0.0	100.0	96.0	72.4	9.4	23.7	24.9	41.9
SS-15	L_3400	-L-	34+00	81' LT	0.0 - 1.5	25.8	A-7-5	6	64	44	20	14.0	94.0	85.0	60.0	14.4	28.8	23.9	32.9
SS-16	L_3600	-L-	36+00	85' LT	3.5 - 5.0	32.2	A-7-5	13	72	35	37	0.0	100.0	97.3	86.0	5.9	10.6	61.7	97.3
S-17	L_3800	-L-	38+00	0' CL	0.5 - 1.0	24.7	A-7-6	--	55	25	30	0.0	99.2	92.5	69.9	14.0	19.8	13.2	53.0
S-18	L_4000	-L-	40+00	0' CL	1.0 - 1.5	33.2	A-7-5	--	65	40	25	0.0	99.1	94.7	73.2	10.1	20.9	23.9	45.1
S-19	L_4400	-L-	44+00	0' CL	1.0 - 1.5	30.4	A-7-6	--	56	29	27	4.0	96.0	91.0	60.0	11.3	32.6	19.6	36.5
S-20	L_4600	-L-	46+00	0' CL	1.0 - 2.0	26.8	A-7-5	--	51	33	18	0.0	99.0	96.0	51.0	13.0	41.3	13.6	32.1
SS-21	Y1_1042	-Y1-	10+42	62' RT	0.0 - 1.5	27.7	A-7-5	12	69	37	32	0.0	100.0	95.2	76.1	10.9	16.2	20.3	52.6
SS-22	Y1_1250	-Y1-	12+50	43' LT	1.0 - 2.5	31.1	A-7-5	11	60	36	24	0.0	99.0	93.3	73.9	11.2	20.5	24.9	43.4
SS-23	Y1_1512	-Y1-	15+12	46' RT	3.4 - 4.1	38.7	A-5	4	50	41	9	0.0	99.0	94.3	66.6	11.8	28.5	25.9	33.8
SS-24	Y1_1550	-Y1-	15+50	45' LT	3.5 - 5.0	28.6	A-7-5	9	59	35	24	0.0	98.0	94.3	76.3	10.3	15.9	25.7	48.1
SS-25	Y2_1250	-Y2-	12+50	20' RT	1.0 - 2.5	31.3	A-7-5	6	67	33	34	0.0	100.0	96.8	77.3	7.2	20.7	23.3	48.8
SS-26	RPB_1495	-RPB-	14+95	50' LT	8.4 - 9.9	34.2	A-7-5	12	66	49	17	0.0	100.0	97.3	75.2	7.8	21.9	27.7	42.6
SS-27	RPB_1706	-RPB-	17+06	53' LT	3.3 - 4.8	32.7	A-7-5	13	64	43	21	0.0	99.9	96.1	73.2	9.4	22.0	16.1	52.5
SS-28	RPB_1900	-RPB-	19+00	70' RT	1.0 - 2.5	37.8	A-7-5	6	74	46	28	0.0	100.0	99.3	76.1	2.3	28.4	25.3	44.0
SS-29	RPD-RT_1055	-RPD-RT-	10+55	37' RT	0.0 - 1.5	30.7	A-7-5	4	51	31	20	1.0	97.0	89.7	64.7	16.0	25.8	33.5	24.7
SS-30	LPB_1375	-LPB-	13+75	70' RT	1.1 - 2.6	39.5	A-7-5	6	62	37	25	0.0	100.0	98.4	85.5	3.6	17.6	36.7	42.1
SS-31	DRW1_1050	-DRW1-	10+50	15' RT	0.0 - 1.5	30.7	A-7-5	12	62	34	28	0.0	99.4	92.2	72.4	13.8	17.3	21.8	47.1
CBR-1	L_1450	-L-	14+50	88' LT	8.5 - 18.5	27.0	A-7-5	--	60	44	16	0.0	100.0	93.1	56.6	14.7	35.3	23.4	26.6
CBR-2	L_1850	-L-	18+50	89' LT	0.0 - 10.0	30.6	A-7-5	--	60	35	25	0.0	100.0	95.2	75.9	9.9	19.4	26.1	44.7
CBR-3	L_4200	-L-	42+00	0' CL	0.0 - 2.5	37.3	A-7-5	--	73	37	36	8.0	91.0	96.6	78.4	7.2	18.6	28.9	45.2

Michelle Stadel, P.E.
Lab Manager, NCDOT Certification No.: 111-02-1203

Clifford Blalock
Lab Technician, NCDOT Certification No.: 111-03-1203

Matt Johnson
Lab Technician, NCDOT Certification No.: 111-01-1203

Victoria Siebert
Lab Technician, NCDOT Certification No.: 109-02-1003

Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

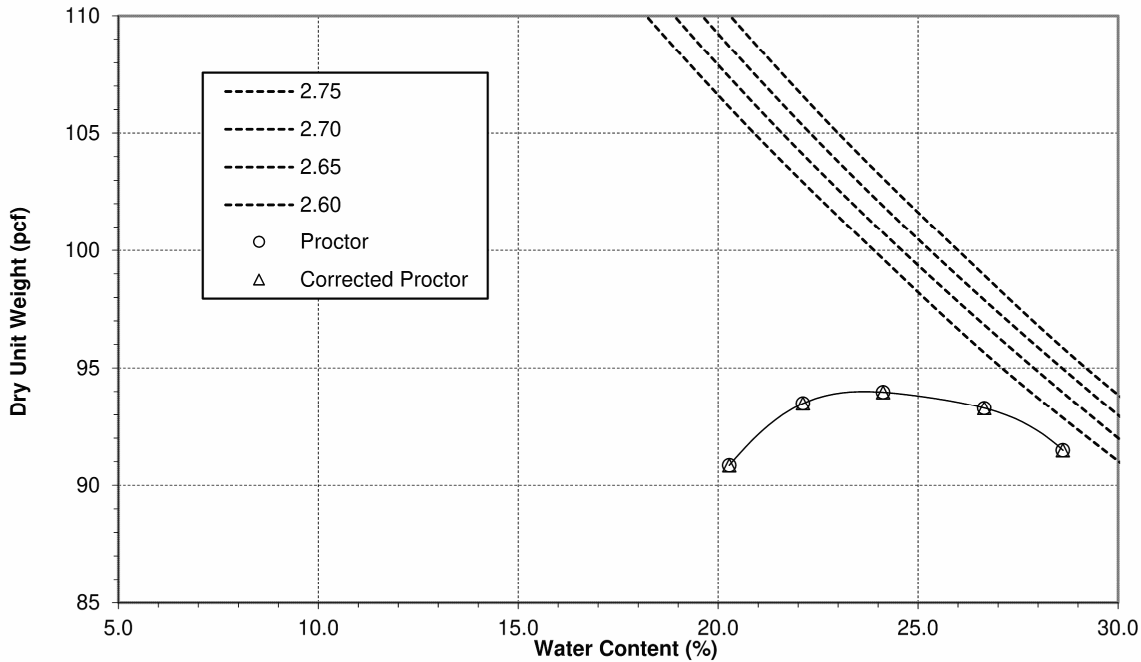
Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/24/2023 by C. Blalock
Material Description: Brown Sandy Silt (A-7-5)
Location: Boring No. L_1450, Alignment -L-, STA 14+50, 88' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	94.0	na
Optimum Water Content (%)	23.6	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual
Specimen Preparation: Moist

Remarks:
AASHTO T-100, Soil Specific Gravity @ 20°C: 2.747

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel

Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

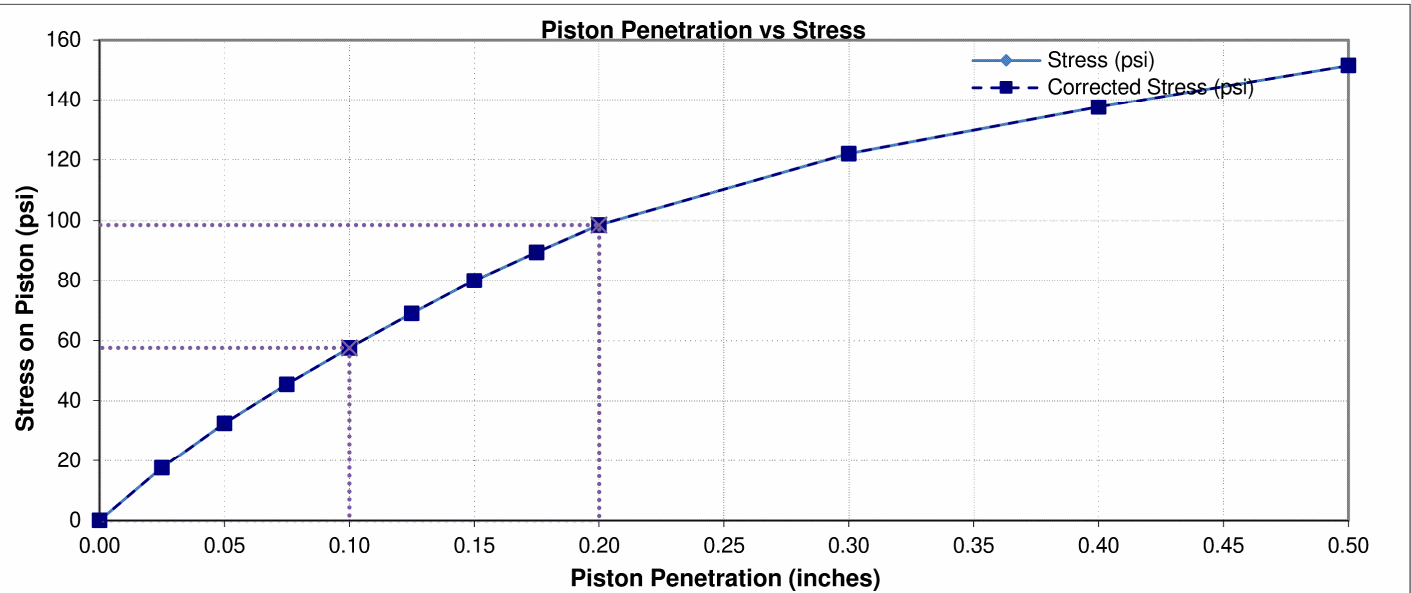
02-000L - Lab

Sample Source: Boring No. L_1450, Alignment -L-, STA 14+50, 88' LT
Sample ID: CBR-1
Sample Description: Brown Sandy Silt (A-7-5)
Material Used:
Surcharge Weight: 10 lbs

Date Tested: 5/30/2023
Tested By: C. Blalock
Condition of Sample: Soaked
Time Soaked: 96 hrs

AASHTO T193 - Standard Test Method for The California Bearing Ratio (CBR)

Dry Unit Wgt Before Soaking (pcf):	90.8	Compaction Method:	Manual
Water Content Before Soaking (%):	21.2	Max. Dry Unit Weight:	94.0 pcf
Dry Unit Wgt After Soaking (pcf):	101.1	Optimum Water Content:	23.6 %
Water Content After Soak, Top in. (%):	30.3		
Swell (%):	5.65		
CBR (Corrected CBR) @ 0.1 in. Penetration:	5.8 (5.8)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.6 (6.6)		



Remarks:

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

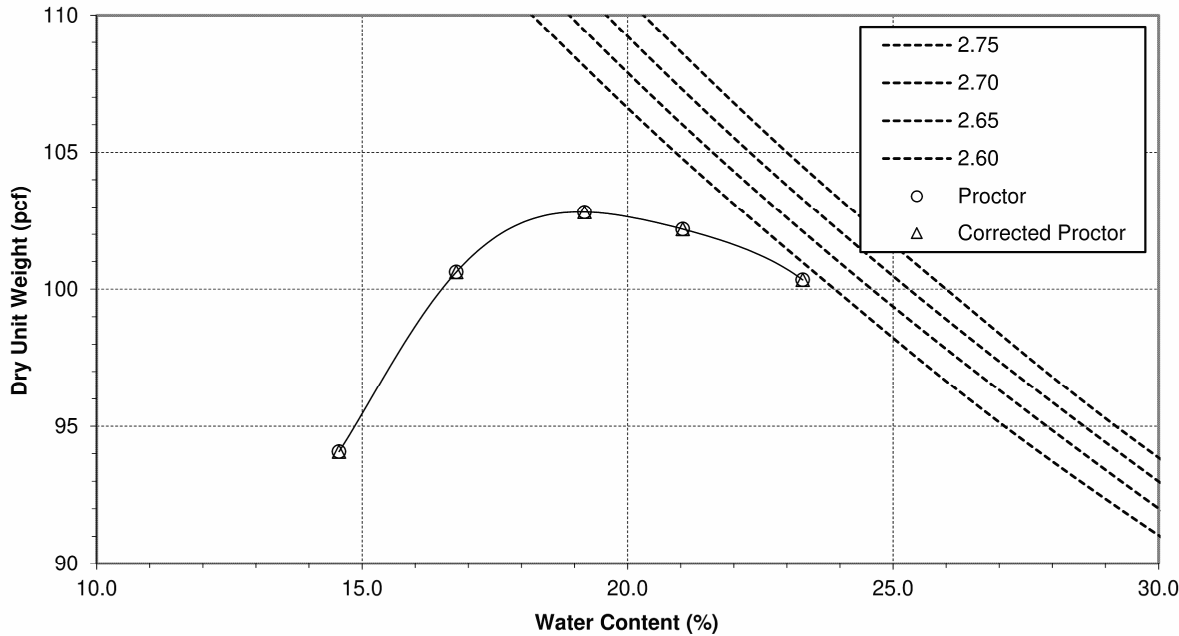
NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/24/2023 by C. Blalock

Material Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Location: Boring No. L_1850, -L-, STA 18+50, 89' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	102.8	na
Optimum Water Content (%)	19.1	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Moist

Remarks:

AASHTO T-100, Soil Specific Gravity at 20°C: 2.678

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Sample Source: Boring No. L_1850, -L-, STA 18+50, 89' LT

Sample ID: CBR-2

Sample Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Material Used:

Surcharge Weight: 10 lbs

Date Tested: 5/30/2023

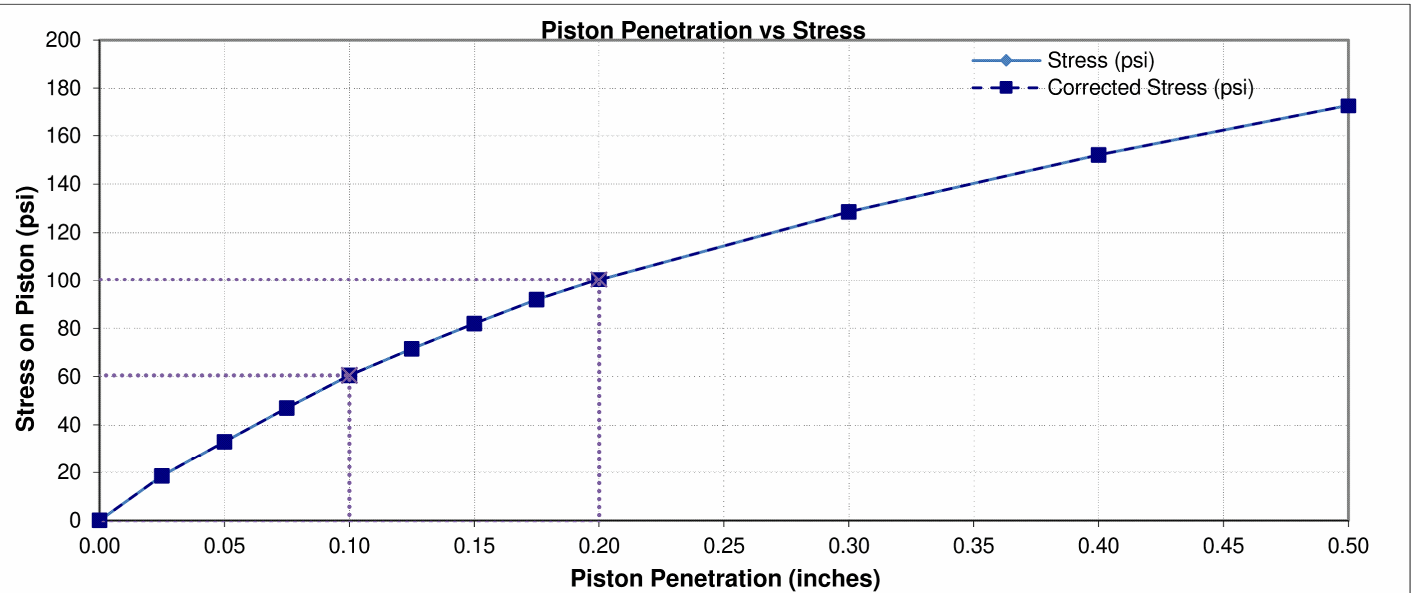
Tested By: C. Blalock

Condition of Sample: Soaked

Time Soaked: 96 hrs

AASHTO T-193 - Standard Test Method for The California Bearing Ratio

Dry Unit Wgt Before Soaking (pcf):	100.6	Compaction Method:	Manual
Water Content Before Soaking (%):	17.0	Max. Dry Unit Weight:	102.8 pcf
Dry Unit Wgt After Soaking (pcf):	106.1	Optimum Water Content:	19.1 %
Water Content After Soak, Top in. (%):	27.1		
Swell (%):	4.1		
CBR (Corrected CBR) @ 0.1 in. Penetration:	6.1 (6.1)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.7 (6.7)		



Remarks:

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.