

August 4, 2023

**MEMORANDUM TO:** Clark S. Morrison, PhD, PE  
State Pavement Design Engineer  
North Carolina Department of Transportation

**FROM:** Joshua D. Fregosi, PE  
Program Manager  
Kleinfelder, Inc

**STATE PROJECT:** 67015.1.1 (BR-0015)

**COUNTY:** Davidson

**DESCRIPTION:** Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5<sup>th</sup> Avenue)

**SUBJECT:** Pavement and Subgrade Investigation Report

Kleinfelder, Inc. has completed the evaluation of the pavement and subgrade investigation for this project and presents the following.

This project consists of the widening of US 29/US 70 (-L-) and replacement of Bridge No. 67 and No. 68 over SR 1192 (-Y1-). At the project location, US 29/US 70 is a four-lane highway consisting of two lanes in the northbound and southbound direction with a grass median dividing the highway. Additionally, the project consists of the widening of SR 1192 (-Y1-), Forest Rose Drive (-DRW1-), and US 29/US 70 northbound exit ramps (-RPD-, -RPD\_RT-) to National Boulevard as well as a realignment of Murphy Drive (-Y2-), onramp to US 29/US70 southbound (-RPB-), and US 29/US 70 southbound exit ramp (-LPB-) to SR 1192.

The soils encountered beneath the existing roadway consisted of both roadway embankment and residual soils. Predominant soil types encountered consisted of silty clays (A-7) with lesser amounts of sandy silts (A-4).

Anticipated borrow will likely consist of soil types listed above that meet the Piedmont and Western Area criteria for Acceptance of Borrow Material, Table 1018-1 of the 2024 Standard Specification.

The existing pavement was observed to be in good condition on US 29/US 70 (-L-). Surface pavement distress was not observed on US 29/US 70. The existing pavement was observed to be in worse condition on West 5<sup>th</sup> Avenue (-Y1-) and Murphy Drive (-Y2-). Surface pavement distress on the West 5<sup>th</sup> Avenue is primarily characterized by low severity transverse and longitudinal cracking, moderate to high severity fatigue cracking, and rutting (with and without spalling). Transverse, longitudinal, and fatigue crack widths ranged from 3 to 10 mm. Rutting was encountered ranging from 3 to 6 mm deep. Surface pavement distress on Murphy Drive is primarily characterized by moderate to high severity fatigue cracking with spalling. Fatigue crack widths ranged from 6 to 13 mm. Rutting was encountered ranging from 3 to 6 mm deep.



The length of the mainline (US 29/US 70, -L-) is approximately 0.71 mile.

## Areas of Special Geotechnical Interest

### 1. Highly Plastic Soils Encountered Beneath the Existing Roadway and Grass Median

Highly plastic soils (PI > 25) were encountered at the following locations based on laboratory test results on the soils:

LINE	STATION AND OFFSET	PI
-L-	18+30 WB ISS	48
-L-	19+60 EB ISS	34
-L-	25+90 WB ISS	35
-L-	31+50 WB OSS	43
-L-	32+75 EB DECEL LN	33
-L-	38+70 WB ISS	39
-L-	42+80 EB ISL	37
-Y1-	15+70 RT LN	35
-Y2-	16+20 LT LN	34

### 2. Groundwater:

Groundwater was not observed during this investigation.

### 3. Samples Classified as Wet

The subgrade soils at the following locations were classified as wet based on the visual inspection and laboratory test results on the soils:

LINE	STATION AND OFFSET	MOISTURE (%)
-L-	31+50 WB OSS	35.0
-L-	38+70 WB ISS	25.9
-Y1-	15+70 RT LN	24.8
-Y2-	16+20 LT LN	32.8



Prepared by,  
**KLEINFELDER, INC.**  
NC Firm License No. F-1312



DocuSigned by:

*Daniel H. Kubinski*

08/30/2023

AB2F7FFB796A411...

Daniel H. Kubinski, PE  
North Carolina License No. 046042

Joshua D. Fregosi, PE  
Program Manager

***Document Not Considered Final Unless All Signatures Are Completed***

DHK/JDF:jrs

**ATTACHMENT:** Pavement and Subgrade Investigation  
Pavement Core Evaluation

REFERENCE: BR-0015

PROJECT: 67015

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2 - 2A	LEGEND, ABBREVIATIONS
3	ROADWAY TITLE SHEET
4 - 6	PLAN SHEETS
7	PAVEMENT INVESTIGATION DATA SHEET
8 - 17	KESSLER DUAL MASS DCP DATA SHEETS
18 - 19	PAVEMENT CORE PHOTOGRAPHS
20 - 24	LABORATORY TEST RESULTS

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

ROADWAY  
SUBSURFACE INVESTIGATION

COUNTY DAVIDSON  
PROJECT DESCRIPTION BRIDGE NO. 67 AND NO. 68  
REPLACEMENTS ON US 29/US 70 NB & SB OVER  
SR 1192 (W. 5TH AVENUE)

PAVEMENT AND SUBGRADE INVESTIGATION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BR-0015	1	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:  
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.  
2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

D. KUBINSKI

M. FOSTER

TRIGON EXPLORATION

INVESTIGATED BY KLEINFELDER, INC

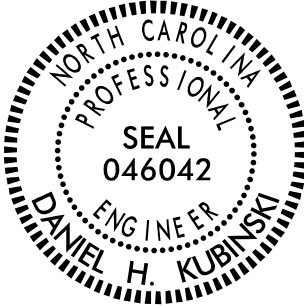
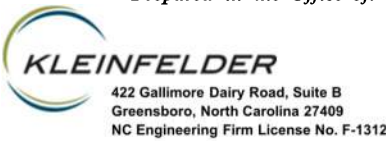
DRAWN BY D. KUBINSKI

CHECKED BY J. FREGOSI

SUBMITTED BY KLEINFELDER, INC

DATE AUGUST 2023

Prepared in the Office of:



DocuSigned by:  
Daniel H. Kubinski 8/30/2023

AB2F7F6B235A11 SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED



***NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT  
SUBSURFACE INVESTIGATION  
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS***

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS ( > 35% PASSING #200)							ORGANIC MATERIALS		
GROUP CLASS.	A-1	A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5					
CLASS.	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7			A-7-5	A-7-6	A-3	A-6, A-7				
SYMBOL																	
% PASSING #10	50 MX	30 MX	50 MX	51 MN													
#40	30 MX	30 MX	50 MX	10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN					
#200	15 MX	25 MX	10 MX	10 MX	10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	10 MX	11 MN					
MATERIAL PASSING #40 PI	-		NP		40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN					
GROUP INDEX	0		0		0		4 MX		8 MX		12 MX		16 MX				
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS, GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS						
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE			
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30																	

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE	< 4	
	LOOSE	4 TO 10	
	MEDIUM DENSE	10 TO 30	N/A
	DENSE	30 TO 50	
VERY DENSE	> 50		
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT	< 2	< 0.25
	SOFT	2 TO 4	0.25 TO 0.5
	MEDIUM STIFF	4 TO 8	0.5 TO 1.0
	STIFF	8 TO 15	1 TO 2
	VERY STIFF	15 TO 30	2 TO 4
HARD	> 30	> 4	

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE, SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
SIZE	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)		FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL PLASTIC RANGE (PI) PL	LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
	PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM SL	OPTIMUM MOISTURE SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
		- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:  
ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE LL < 31  
MODERATELY COMPRESSIBLE LL = 31 - 50  
HIGHLY COMPRESSIBLE LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING

STATIC WATER LEVEL AFTER 24 HOURS

PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA

SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION

SOIL SYMBOL

ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT

INFERRED SOIL BOUNDARY

INFERRED ROCK LINE

ALLUVIAL SOIL BOUNDARY

DIP & DIP DIRECTION OF ROCK STRUCTURES

TEST BORING

AUGER BORING

CORE BORING

MONITORING WELL

PIEZOMETER INSTALLATION

SLOPE INDICATOR INSTALLATION

CONE PENETROMETER TEST

SOUNDING ROD

TEST BORING WITH CORE

SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT

UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE

UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.  
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.

CRYSTALLINE ROCK (CR)

FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

NON-CRYSTALLINE ROCK (NCR)

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTARY ROCK (CP)

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH

ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.

VERY SLIGHT (V SL.)

ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.

SLIGHT (SL.)

ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.

MODERATE (MOD.)

SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW FLAK. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.

MODERATELY SEVERE (MOD. SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KALINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*

SEVERE (SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KALINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*

VERY SEVERE (V SEV.)

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*

COMPLETE

ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD

CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.

HARD

CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.

MODERATELY HARD

CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.

MEDIUM HARD

CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.

SOFT

CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.

VERY SOFT

CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE

RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.

MODERATELY INDURATED

GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.

INDURATED

GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.

EXTREMELY INDURATED

SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  
AQUIFER - A WATER BEARING FORMATION OR STRATA.  
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  
ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.  
CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.  
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.  
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  
DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.  
DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.  
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.  
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.  
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.  
FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.  
JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.  
LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.  
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIUOUS STRATUM.  
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.  
ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.  
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  
SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  
STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  
STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  
STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: N/A

ELEVATION: N/A FEET

NOTES:  

PAVEMENT CORE WITH KESSLER DUAL MASS DCP TEST

HAND AUGER WITH KESSLER DUAL MASS DCP TEST

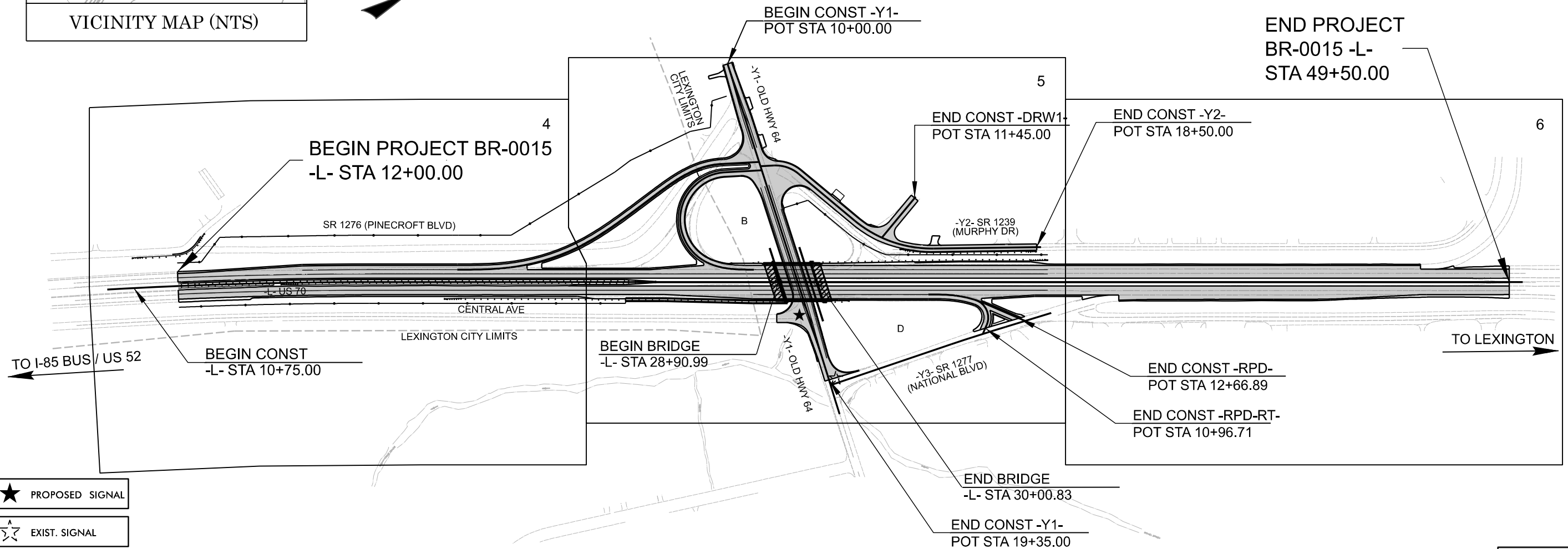
PAVEMENT CORE

DATE: 8-15-14

**ABBREVIATIONS**

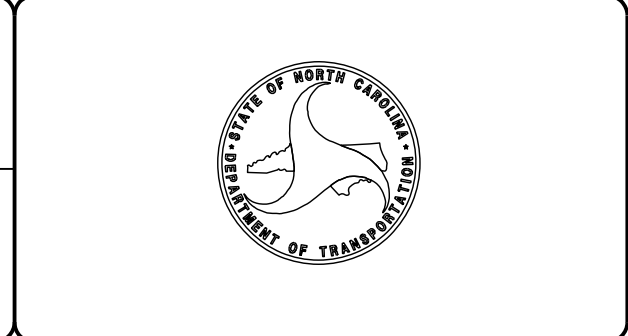
N/A - NOT APPLICABLE  
NM - NOT MEASURED  
KDCP - KESSLER DUAL MASS DCP  
PS - PAVED SHOULDER  
WB - WESTBOUND  
EB - EASTBOUND  
LT - LEFT  
RT - RIGHT  
OSS - OUTSIDE SHOULDER  
ISS - INSIDE SHOULDER  
OSL - OUTSIDE LANE  
DECEL - DECELERATION  
ACCEL - ACCELERATION  
LN - LANE  
WL - WHITE LINE  
YL - YELLOW LINE  
AG - AT GRADE  
F - FILL  
C - CUT  
CR - CROWN  
SU - SUPERELEVATION

## Plans Developed with OpenRoads



**INCOMPLETE PLANS**  
DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED





CORE ID	L_1830_WB_ISS	L_1830_WB_OSL	L_1830_WB_OSS
ASPHALT	6.75"	7.25"	8.50"
CONCRETE	0.00"	9.25"	0.00"
ABC STONE	2.00"	4.50"	0.00"



CORE ID	L_1960_EB_ISS	L_1960_EB_OSL	L_1960_EB_OSS
ASPHALT	7.25"	8.25"	10.25"
CONCRETE	0.00"	9.25"	0.00"
ABC STONE	0.00"	4.75"	11.75"



CORE ID	L_2590_WB_ISS
ASPHALT	8.50"
CONCRETE	0.00"
ABC STONE	0.00"
CORE ID	L_3275_EB_DECEL_LN
ASPHALT	10.75"
CONCRETE	0.00"
ABC STONE	7.00"



CORE ID	L_3870_WB_ISS	L_3870_WB_DECEL_LN	L_3870_WB_OSS
ASPHALT	7.00"	11.00"	7.50"
CONCRETE	0.00"	0.00"	0.00"
ABC STONE	0.00"	7.00"	0.00"



CORE ID	L_4280_EB_ISL	L_4280_EB_ACCEL_LN	L_4280_EB_OSS
ASPHALT	8.50"	9.50"	7.00"
CONCRETE	9.50"	0.00"	0.00"
ABC STONE	5.00"	8.50"	0.00"



PAVEMENT INVESTIGATION DATA SHEET

TIP (WBS):	BR-0015 (67015.1.1)	Project:	Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)	Date:	May 2023
County:	Davidson	Route:	I-85 Business, West 5th Avenue, Murphy Drive	Notes By:	Dan Kubinski

		Width (ft)		(ft)		Pavement Section Thickness (in)							Subgrade								
Position (Sta.,Lane,Shldr.)	Cut/Fill	Lane(s)	Shoulder	Offset Distance	Crown or Superelevation	Gross to Top of Soil	Asphalt	Concrete	ABC Stone	PADL	Soil Stabilization	Sand	Moisture Sample No.	Sample No.	Description	AASHTO Classification	Soil Moisture (%)	Probe Depth (ft)			
L_1830_WB_ISS	C	12.0	1.0 PS	0.0 YL	CR	8.75	6.75	0.00	2.00	0.00	0.00	0.00	S-1		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	27.1	5.0	No pavement distress observed	757,186	1,621,293
L_1830_WB_OSL	C	12.0	4.5 PS	3.0 WL		21.00	7.25	9.25	4.50	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,198	1,621,278
L_1830_WB_OSS	C	12.0	4.5 PS	1.0 WL		8.50	8.50	0.00	0.00	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,201	1,621,274
L_1960_EB_ISS	C	12.0	1.5 PS	0.5 YL	CR	7.25	7.25	0.00	0.00	0.00	0.00	0.00	S-2		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	30.1	5.0	No pavement distress observed	757,274	1,621,392
L_1960_EB_OSL	C	12.0	4.0 PS	3.0 WL		22.25	8.25	9.25	4.75	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,260	1,621,409
L_1960_EB_OSS	C	12.0	4.0 PS	2.0 WL		22.00	10.25	0.00	11.75	0.00	0.00	0.00	N/A		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	757,257	1,621,413
L_2590_WB_ISS	AG	12.0	1.0 PS	0.0 YL	CR	8.50	8.50	0.00	0.00	0.00	0.00	0.00	S-3		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	28.4	5.0	No pavement distress observed	757,786	1,621,759
L_3150_WB_OSS	AG	12.0	7.0 PS	4.0 WL	CR	9.25	9.25	0.00	0.00	0.00	0.00	0.00	S-4		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	35.0	5.0	No pavement distress observed	758,245	1,622,081
L_3275_EB_DECEL_LN	AG	12.0	3.5 PS	2.5 WL	CR	17.75	10.75	0.00	7.00	0.00	0.00	0.00	S-5		RES: Moist, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	26.7	5.0	No pavement distress observed	758,292	1,622,226
L_3870_WB_ISS	C	12.0	1.0 PS	0.5 YL	CR	7.00	7.00	0.00	0.00	0.00	0.00	0.00	S-6		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-6	25.9	5.0	No pavement distress observed	758,798	1,622,544
L_3870_WB_DECEL_LN	C	12.0	2.0 PS	6.5 WL		18.00	11.00	0.00	7.00	0.00	0.00	0.00	N/A		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	758,817	1,622,519
L_3870_WB_OSS	C	12.0	2.0 PS	1.5 WL		7.50	7.50	0.00	0.00	0.00	0.00	0.00	N/A		RES: Wet, Reddish Brown, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	758,821	1,622,514
L_4000^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	S-18^ N/A		RES: Moist, Olive Gray and Reddish Brown, Moderately Plastic, Silty CLAY (0.0 - 2.2'), Reddish Brown, Slightly Plastic to Non Plastic, Fine Sandy SILT, Trace Mica (2.2 - 6.8')	A-7-5 A-4	33.2 N/A	6.8	Hand auger boring performed for roadway investigation with KDCP test in center grass median	758,893	1,622,633
L_4280_EB_ISL	C	12.0	1.0 PS	1.0' YL	CR	23.00	8.50	9.50	5.00	0.00	0.00	0.00	S-7		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7-5	38.4	5.0	No pavement distress observed	759,107	1,622,815
L_4280_EB_ACCEL_LN	C	12.0	4.0 PS	4.5 WL		18.00	9.50	0.00	8.50	0.00	0.00	0.00	N/A		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	759,088	1,622,839
L_4280_EB_OSS	C	12.0	4.0 PS	2.0 WL		7.00	7.00	0.00	0.00	0.00	0.00	0.00	N/A		RES: Moist, Reddish Brown, Highly Plastic, Silty CLAY , Trace Mica (0.0 - 5.0')	A-7	N/A	5.0	No pavement distress observed	759,084	1,622,844
L_4600^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	S-20^ N/A		RES: Moist, Reddish Brown, Moderately Plastic, Silty CLAY, Trace Mica (0.0 - 3.0'), Brown, Non Plastic, Coarse to Fine Sandy SILT, Trace Mica (3.0 - 6.5')	A-7-5 A-4	26.8	6.5	Hand auger boring performed for roadway investigation with KDCP test in center grass median	759,367	1,623,000
L_4800^	C	N/A	N/A	N/A	N/A	0.00	0.00	0.00	0.00	0.00	0.00	0.00	N/A N/A		RES: Moist, Yellowish Brown and Reddish Brown, Moderately Plastic, Silty CLAY (0.0 - 1.0'), Brown, Non Plastic, Coarse to Fine Sandy SILT, Trace Mica (1.0 - 6.7')	A-7 A-4	N/A N/A	6.7	Hand auger boring performed for roadway investigation with KDCP test in center grass median	759,525	1,623,123
Y1_1570_LT_PS	F	12.5	4.5 PS	1.0 WL	CR	5.00	5.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	No pavement distress observed	758,098	1,621,920
Y1_1570_LT_LN	F	12.5	4.5 PS	2.5 WL		6.00	6.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	Low transverse and longitudinal cracking (3 - 6 mm crack width); Moderate to high fatigue cracking (3 - 6 mm crack width); Rutting (3 - 6 mm depth)	758,095	1,621,919
Y1_1570_RT_LN	F	12.5	4.0 PS	3.0 WL		7.00	7.00	0.00	0.00	0.00	0.00	0.00	S-8		RE: Wet, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-6	24.8	5.0	Low transverse and longitudinal cracking (3 - 6 mm crack width); Moderate to high fatigue cracking with spalling (3 - 10 mm crack width); Rutting (3 mm depth)	758,077	1,621,912
Y1_1570_RT_PS	F	12.5	4.0 PS	1.0 WL		4.00	4.00	0.00	NM	0.00	0.00	0.00	N/A		No auger probe + KDCP test performed; Utility Conflicts	N/A	N/A	N/A	No pavement distress observed	758,073	1,621,911
Y2_1620_LT_LN	C	9.0	0.5 PS	2.0 WL	CR	10.00	3.00	0.00	7.00	0.00	0.00	0.00	S-9		RES: Wet, Red, Highly Plastic, Silty CLAY (0.0 - 5.0')	A-7-5	32.8	5.0	Moderate to high fatigue cracking with spalling (6 - 13 mm crack width ); Rutting (4 mm depth)	758,472	1,622,178

Note(s):  
N/A - Not Applicable  
NM - Not Measured  
KDCP - Kessler Dual Mass DCP  
PS - Paved Shoulder

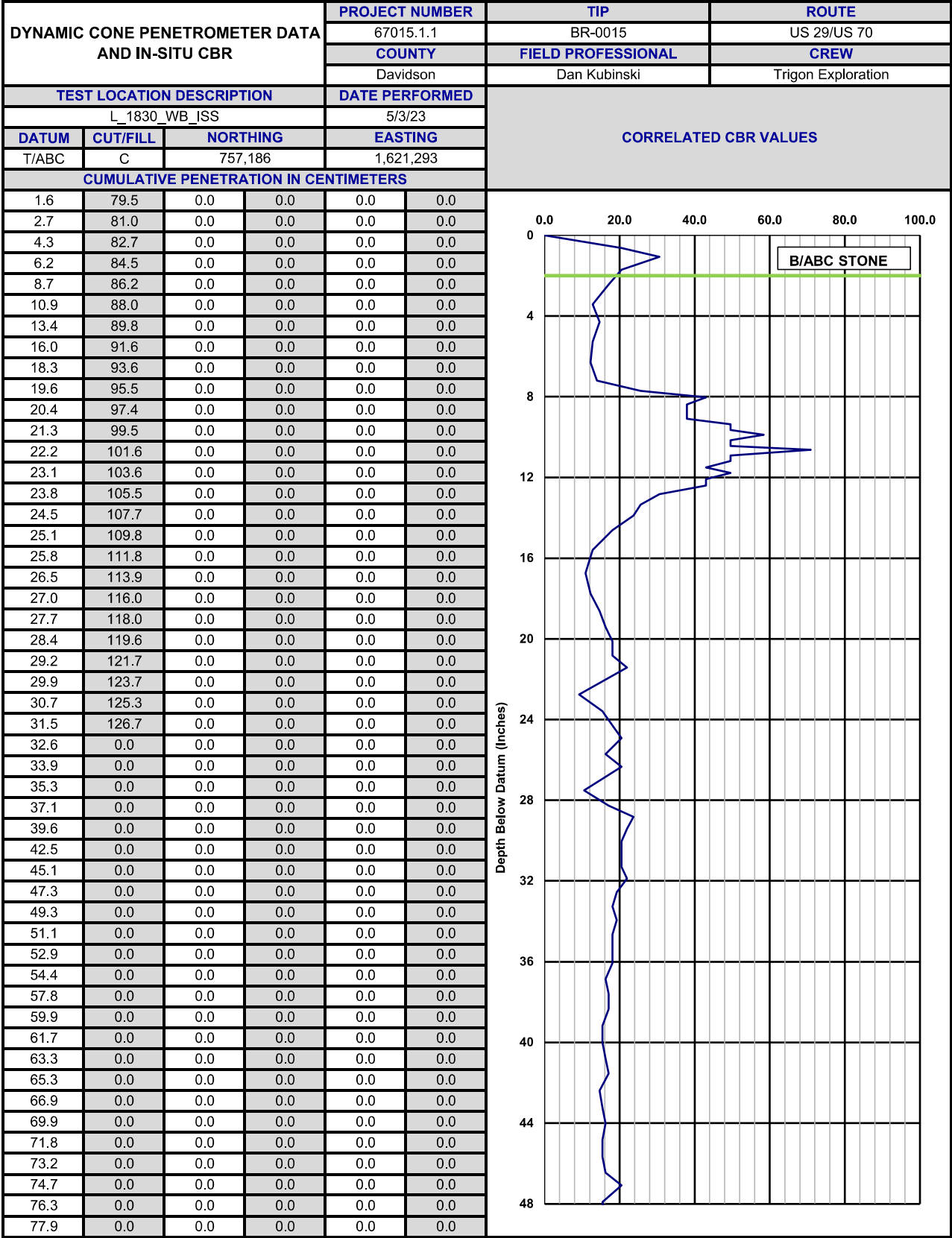
WB - Westbound  
EB - Eastbound  
LT - Left  
RT - Right

OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration

ACCEL - Acceleration  
LN - Lane  
WL - White Line  
YL - Yellow Line

AG - At Grade  
F - Fill  
C - Cut  
CR - Crown

SU - Superelevation  
^Based on Roadway Investigation Boring Number  
\*Based on Roadway Investigation Sample Number



Note(s):

WB - Westbound

EB - Eastbound

OSS - Outside Shoulder

ISS - Inside Shoulder

OSL - Outside Lane

DECEL - Deceleration

ACCEL - Acceleration

LN - Lane

AG - At Grade

F - Fill

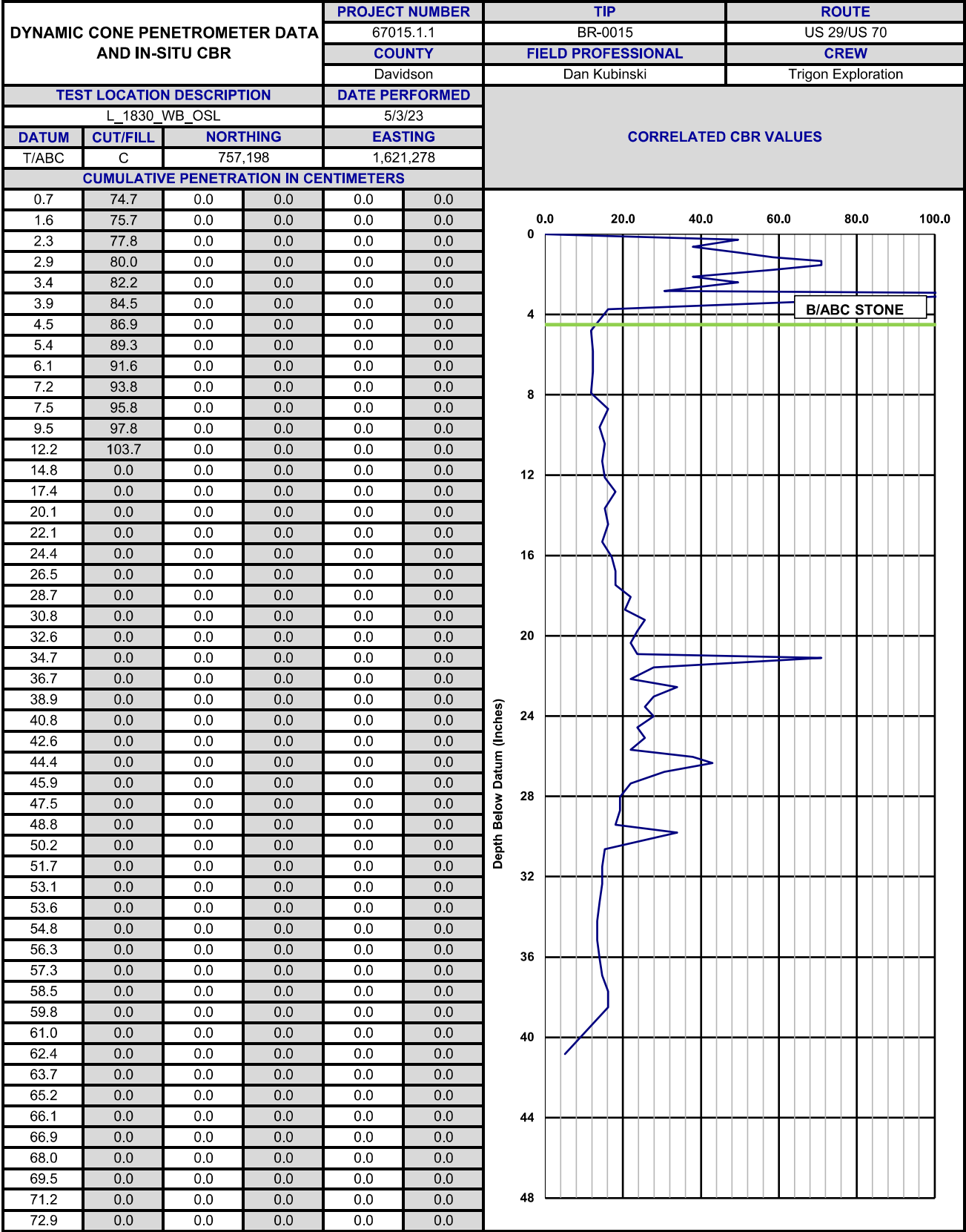
C - Cut

SG - Subgrade

T/ABC - Top of ABC Stone

B/ABC - Bottom of ABC Stone

Ex. Gr. - Existing Grade



Note(s):

WB - Westbound

EB - Eastbound

OSS - Outside Shoulder

ISS - Inside Shoulder

OSL - Outside Lane

DECEL - Deceleration

ACCEL - Acceleration

LN - Lane

AG - At Grade

F - Fill

C - Cut

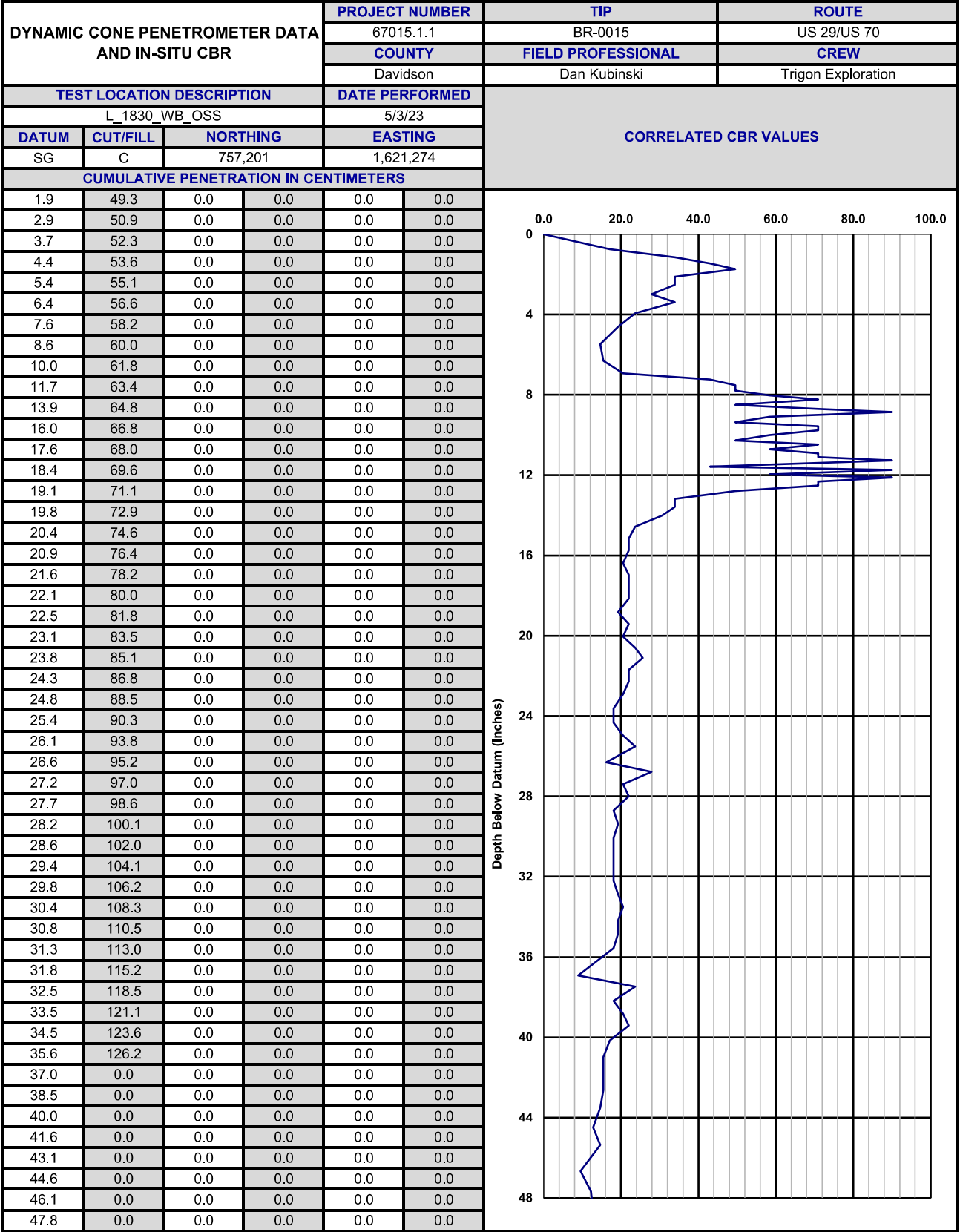
SG - Subgrade

T/ABC - Top of ABC Stone

B/ABC - Bottom of ABC Stone

Ex. Gr. - Existing Grade





Note(s):

WB - Westbound

EB - Eastbound

OSS - Outside Shoulder

ISS - Inside Shoulder

OSL - Outside Lane

DECEL - Deceleration

ACCEL - Acceleration

LN - Lane

AG - At Grade

F - Fill

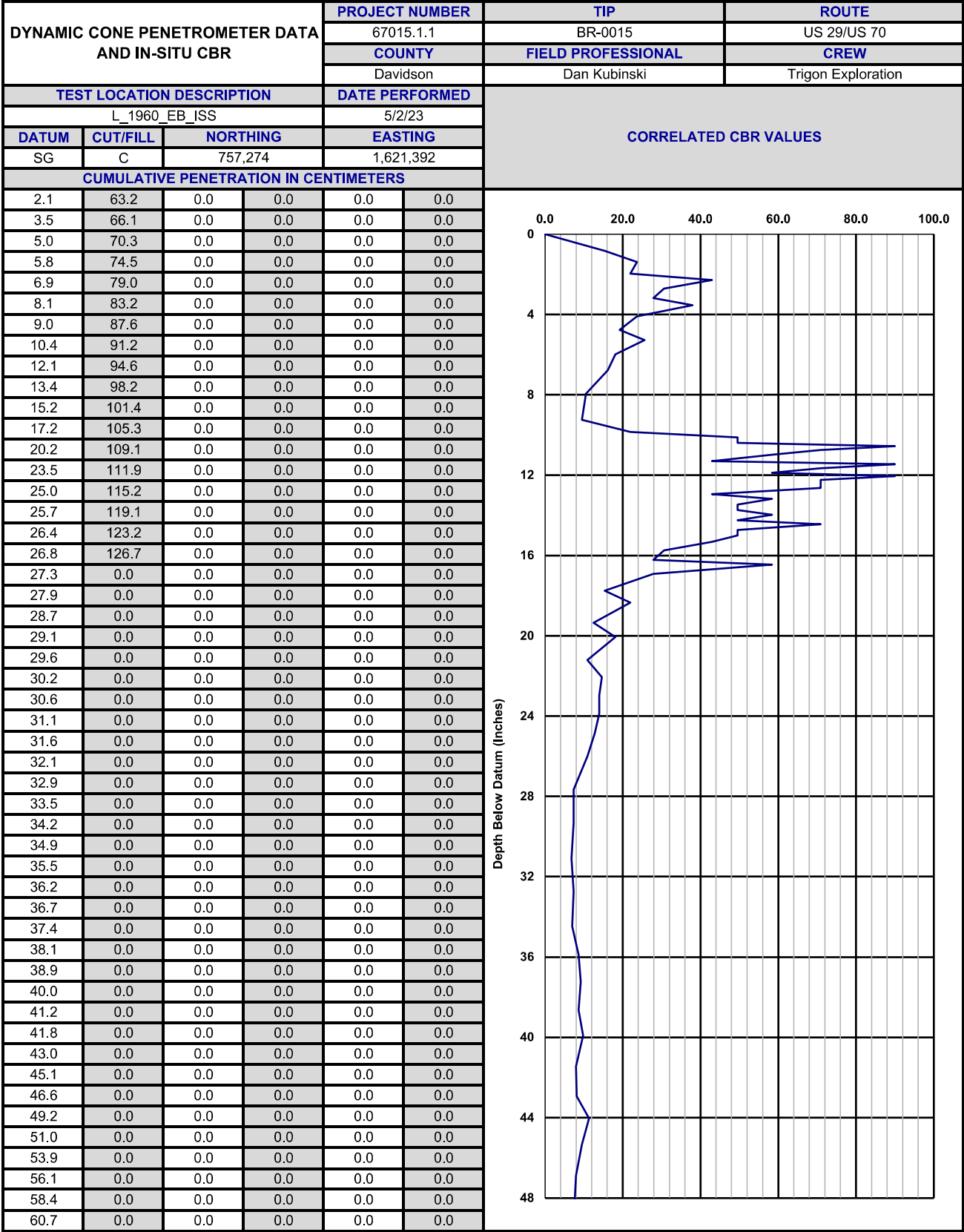
C - Cut

SG - Subgrade

T/ABC - Top of ABC Stone

B/ABC - Bottom of ABC Stone

Ex. Gr. - Existing Grade



Note(s):

WB - Westbound

EB - Eastbound

OSS - Outside Shoulder

ISS - Inside Shoulder

OSL - Outside Lane

DECEL - Deceleration

ACCEL - Acceleration

LN - Lane

AG - At Grade

F - Fill

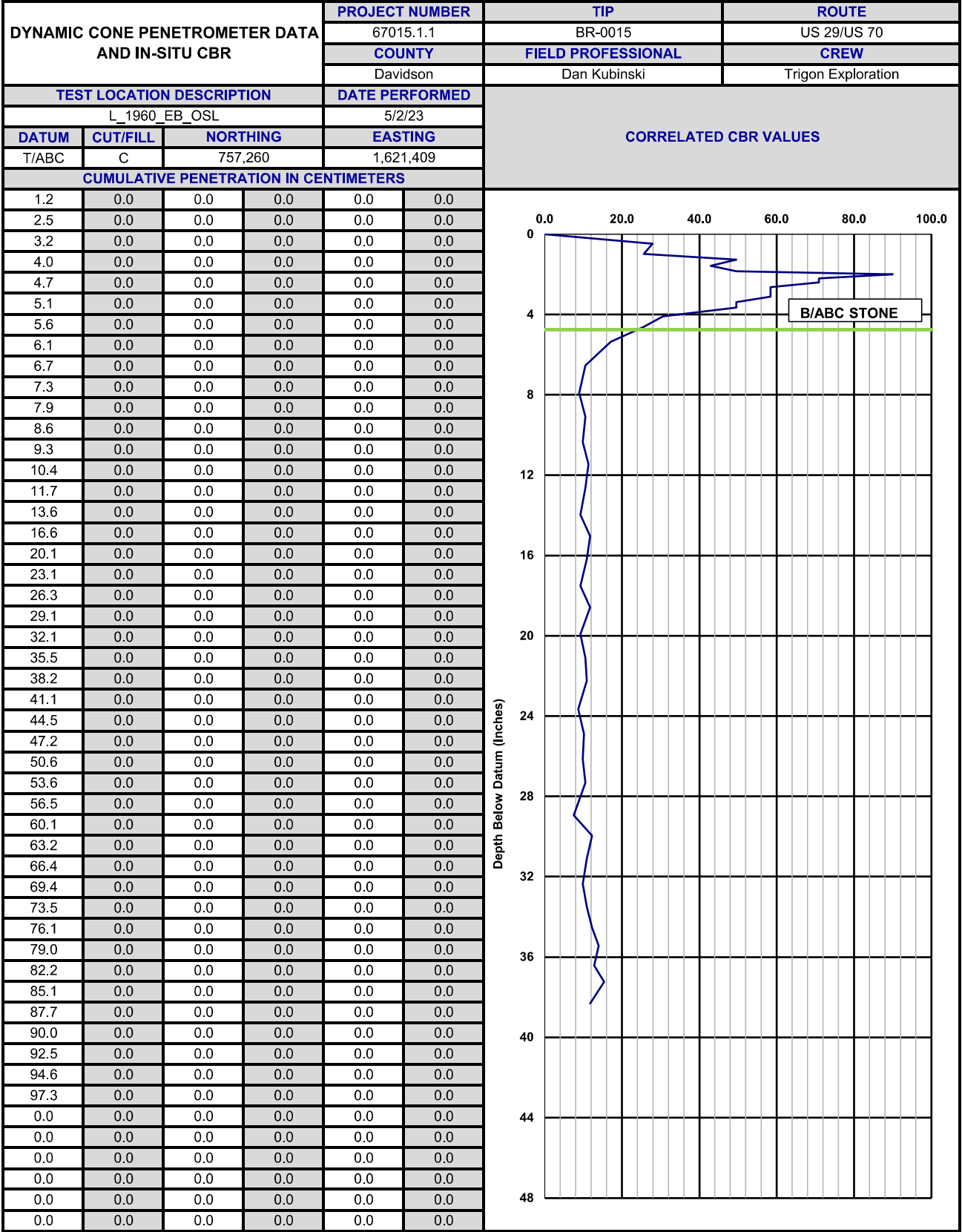
C - Cut

SG - Subgrade

T/ABC - Top of ABC Stone

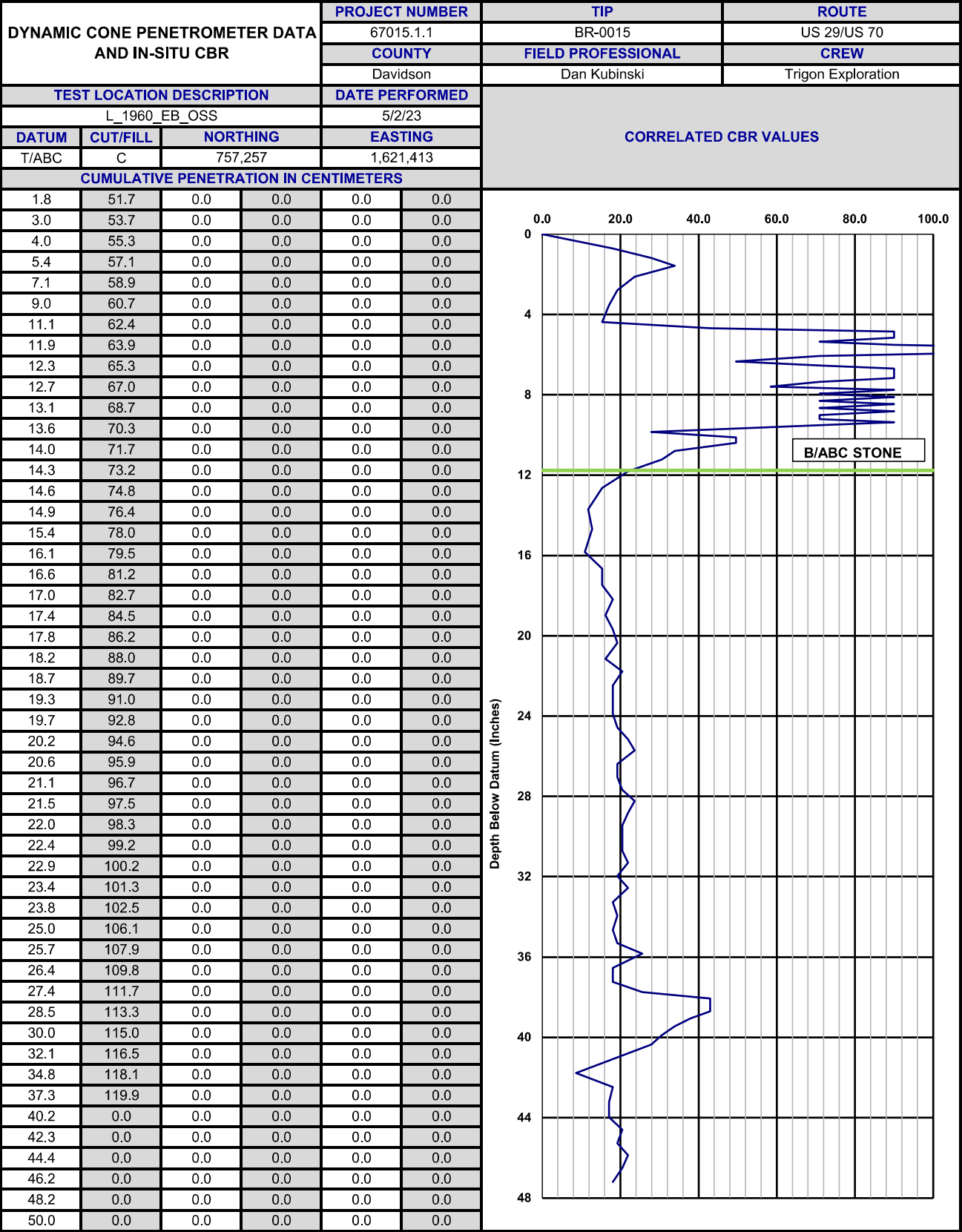
B/ABC - Bottom of ABC Stone

Ex. Gr. - Existing Grade



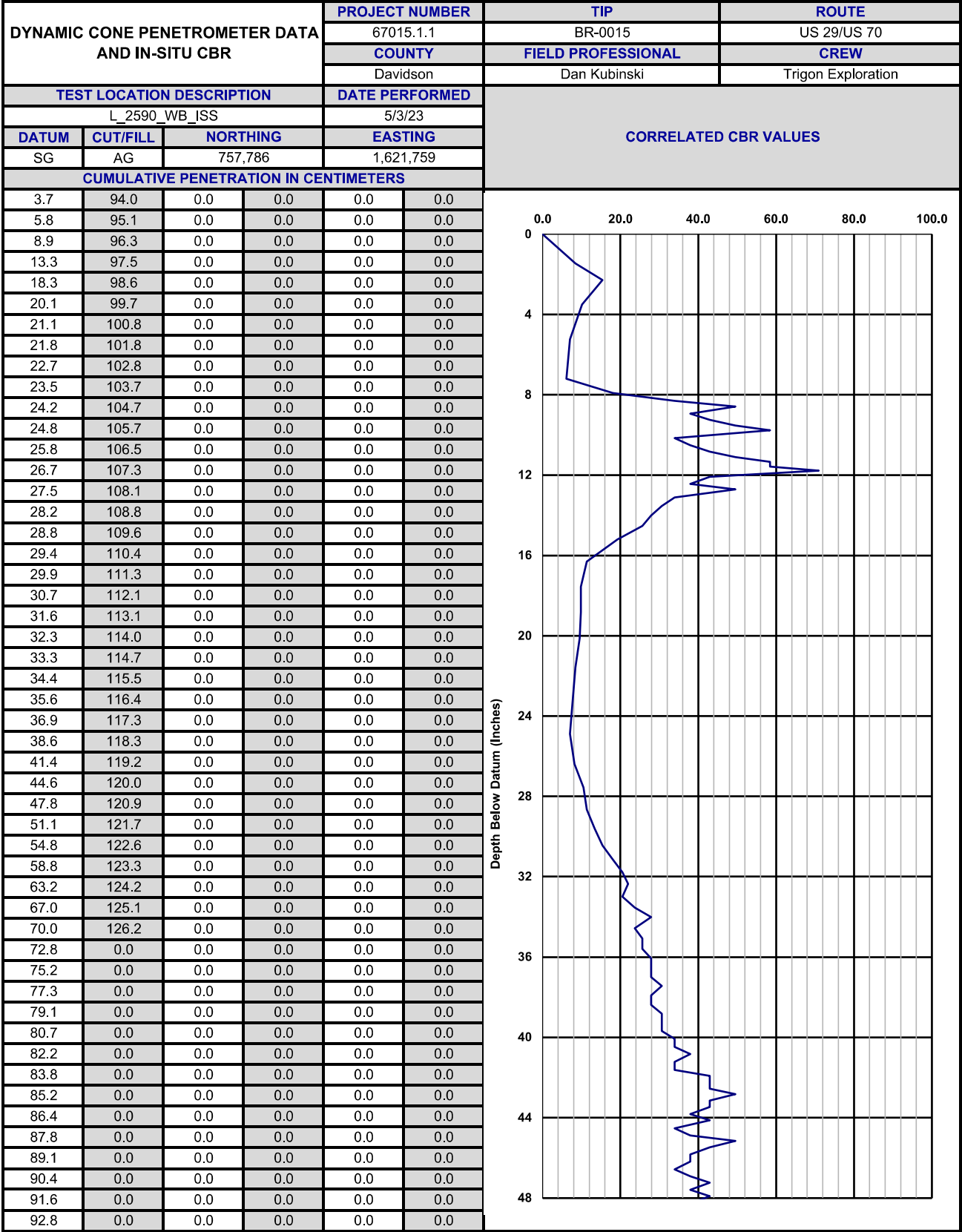
Note(s):

WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade



Note(s):

WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade

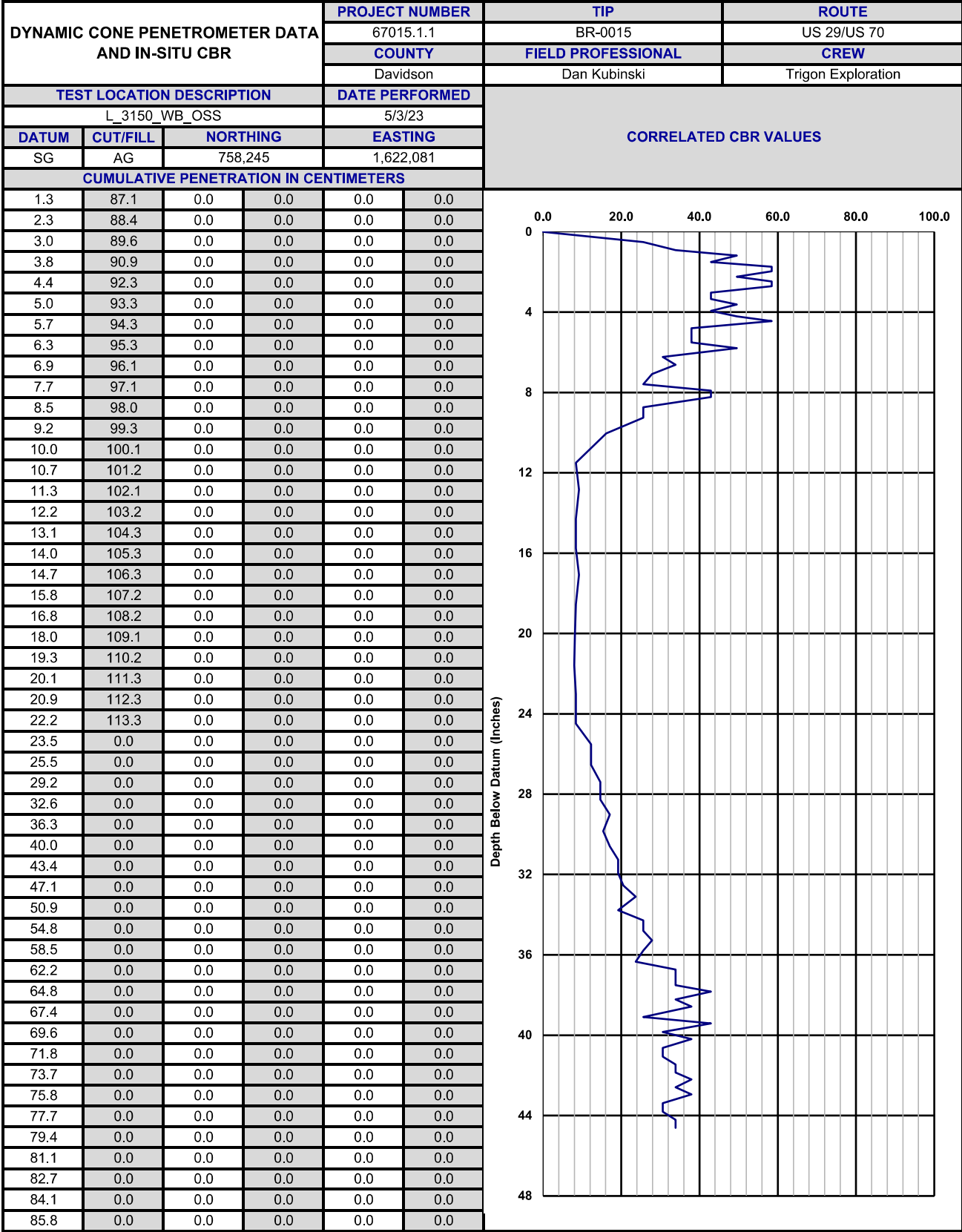


**Note(s):**

WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane

DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill

C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade

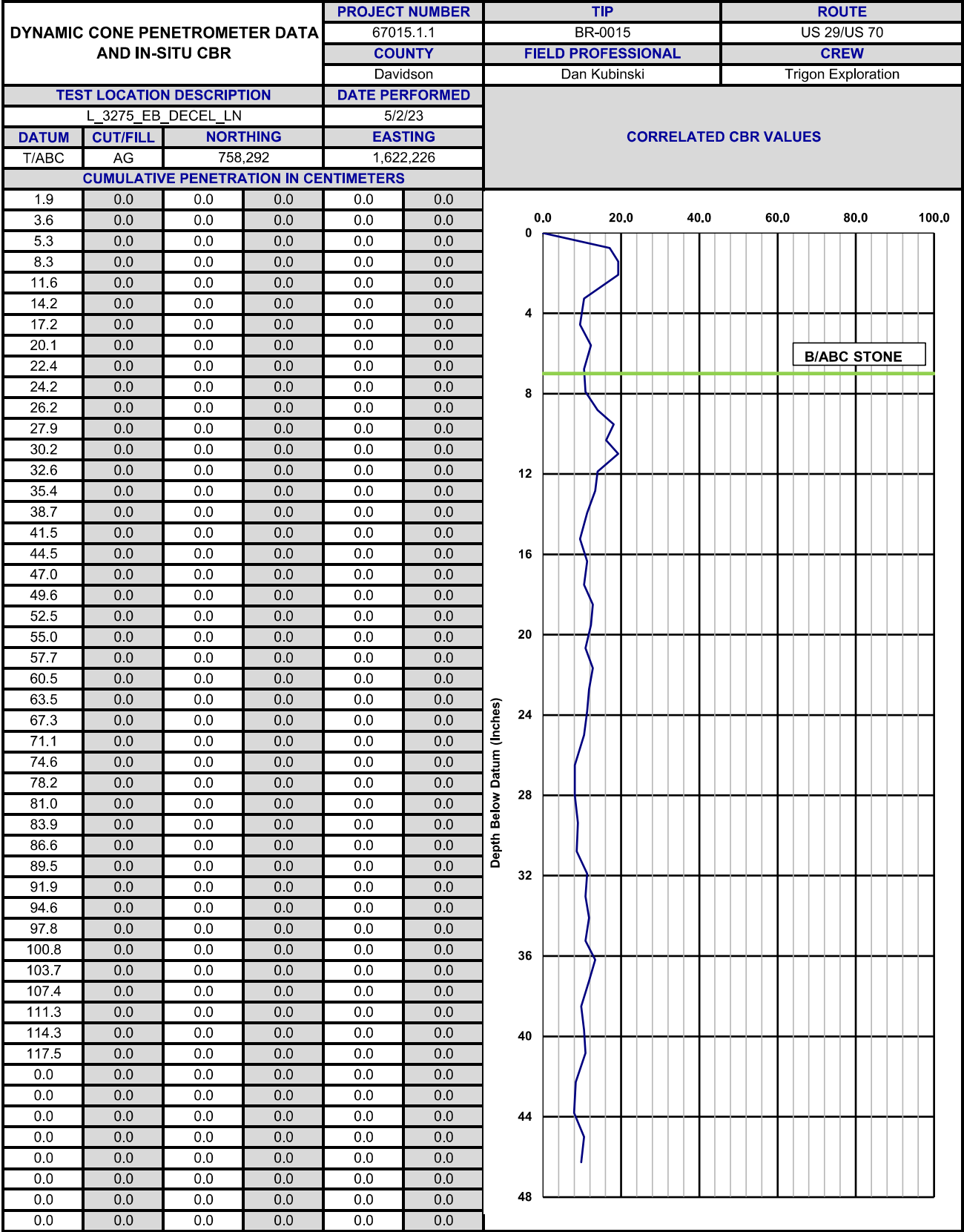


**Note(s):**

WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane

DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill

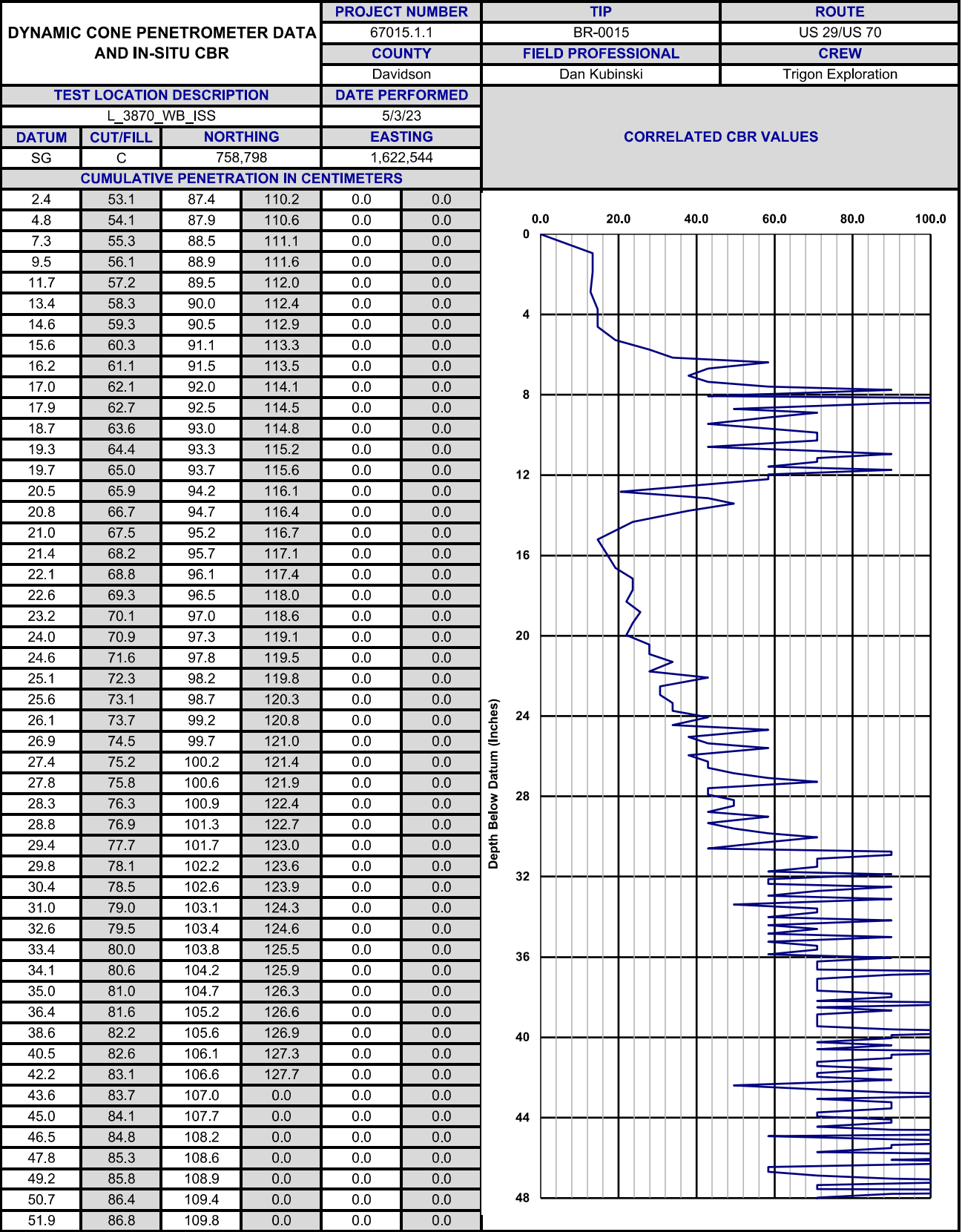
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade



Note(s):  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane

DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill

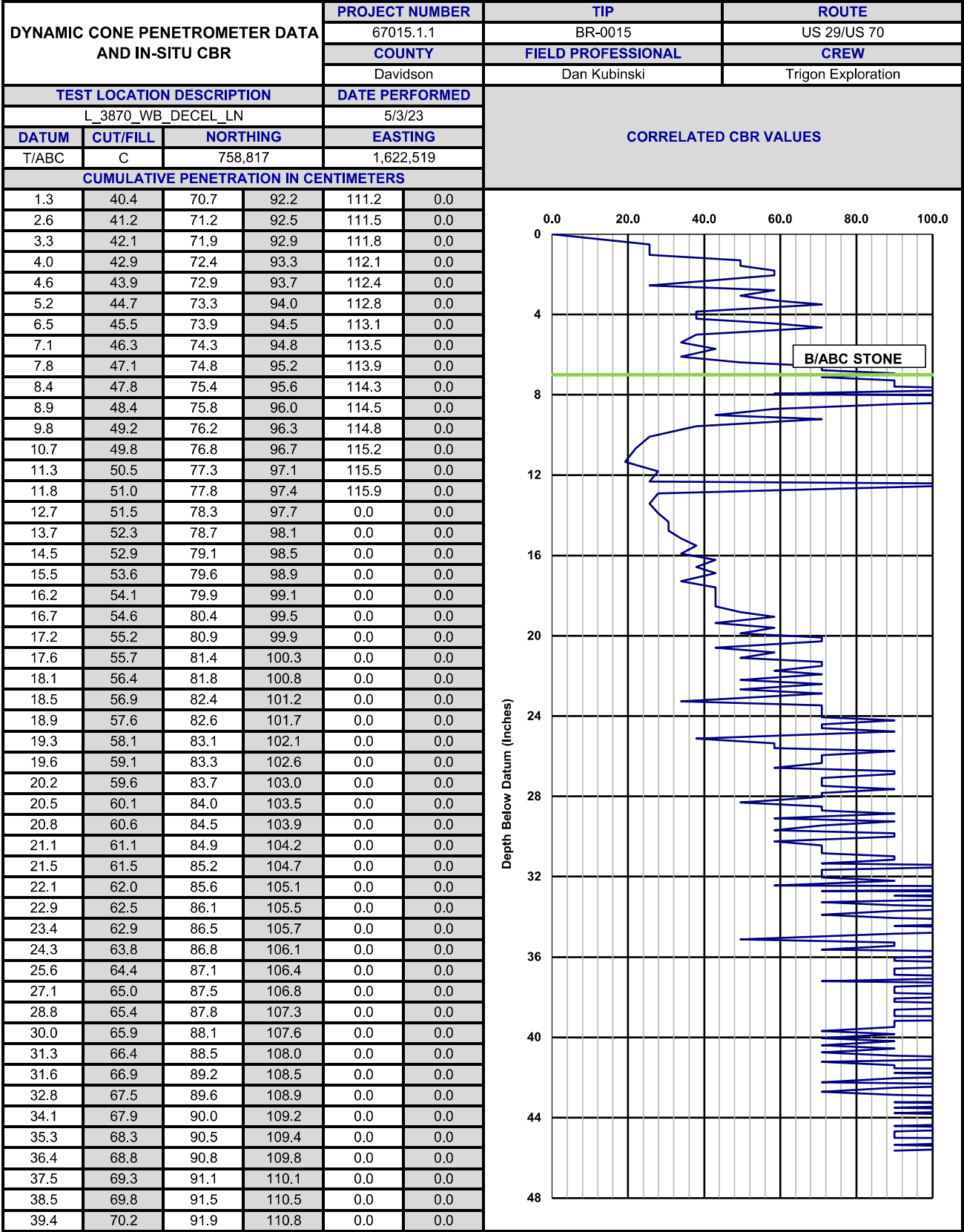
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade



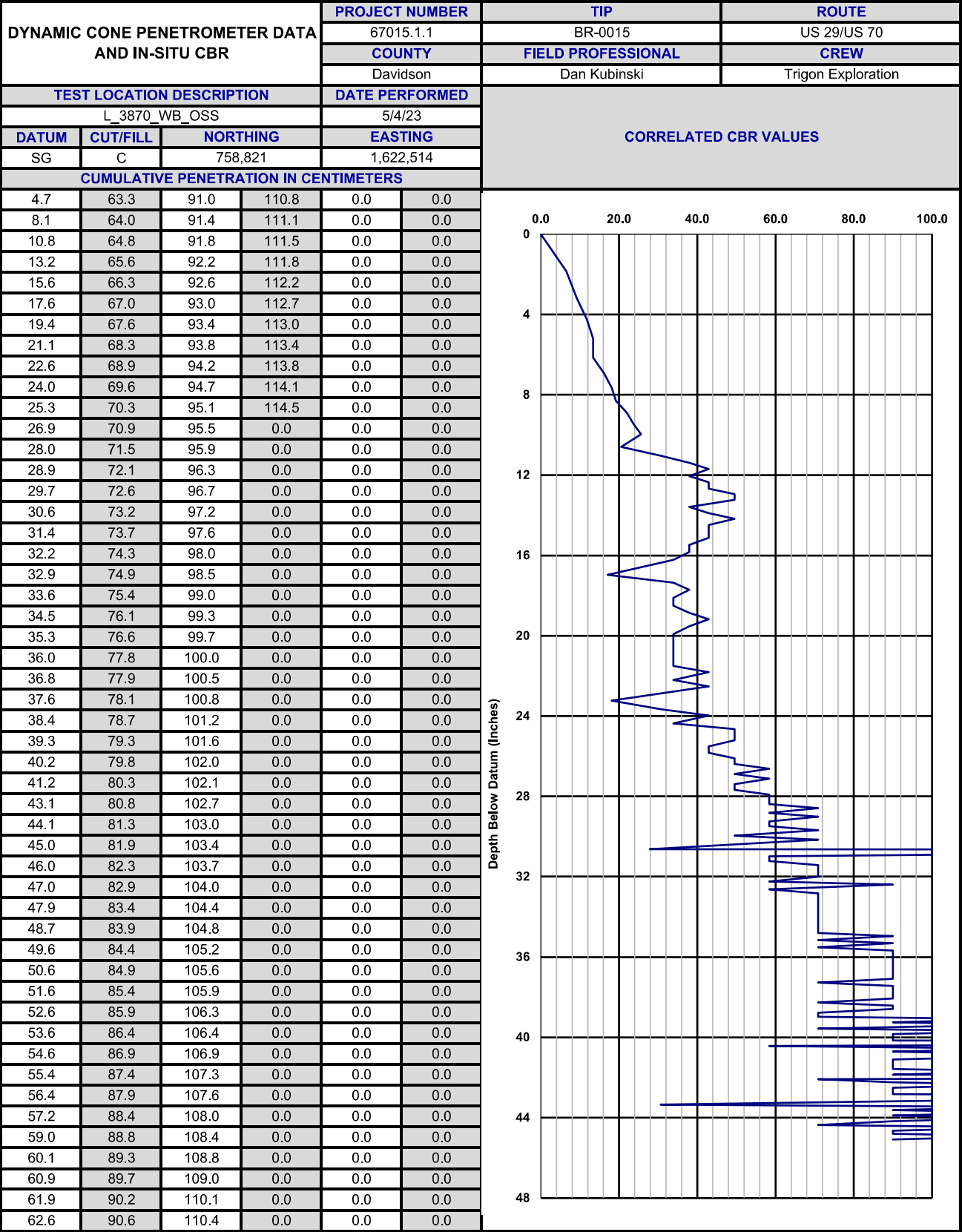
Note(s):  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane

DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill

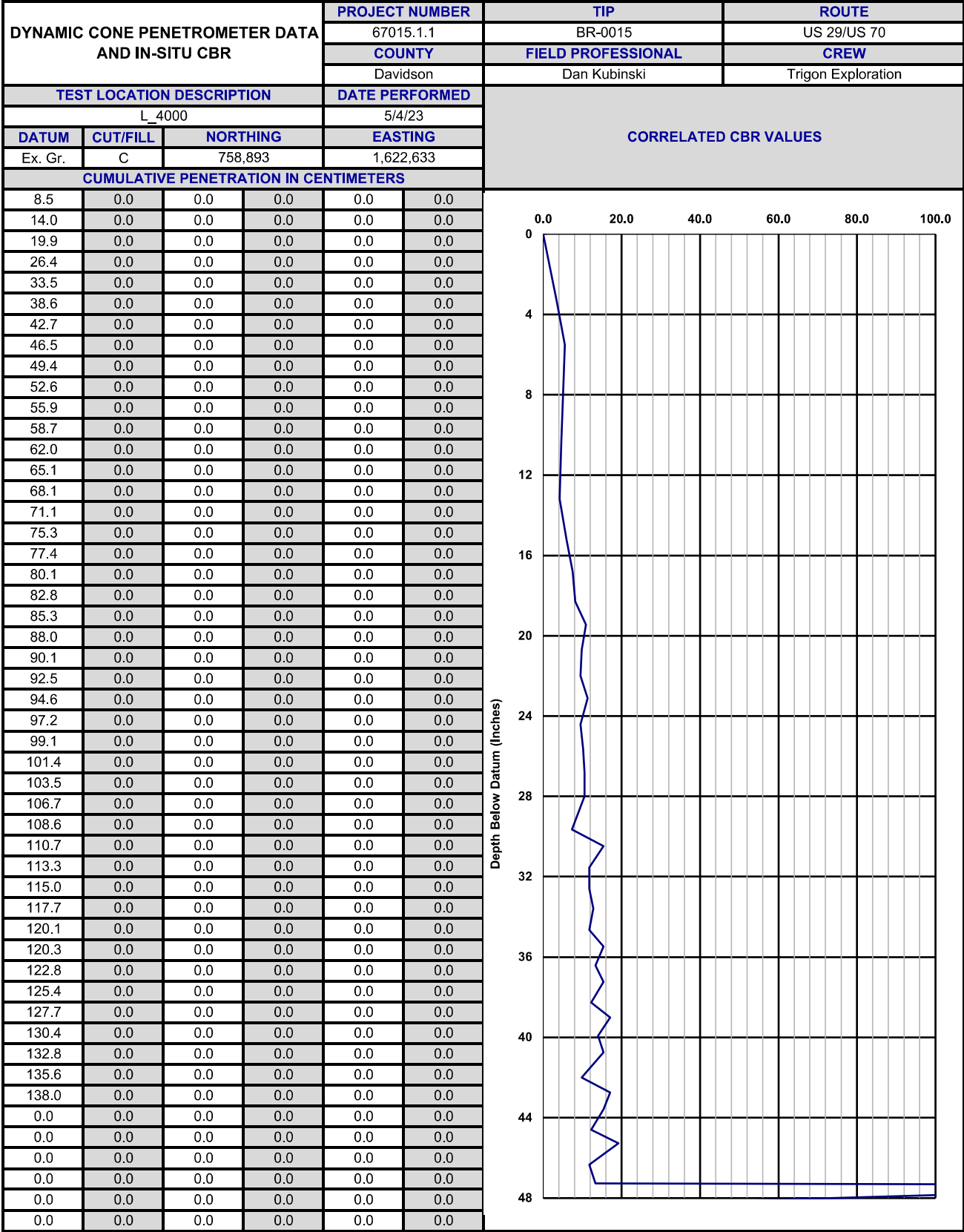
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade



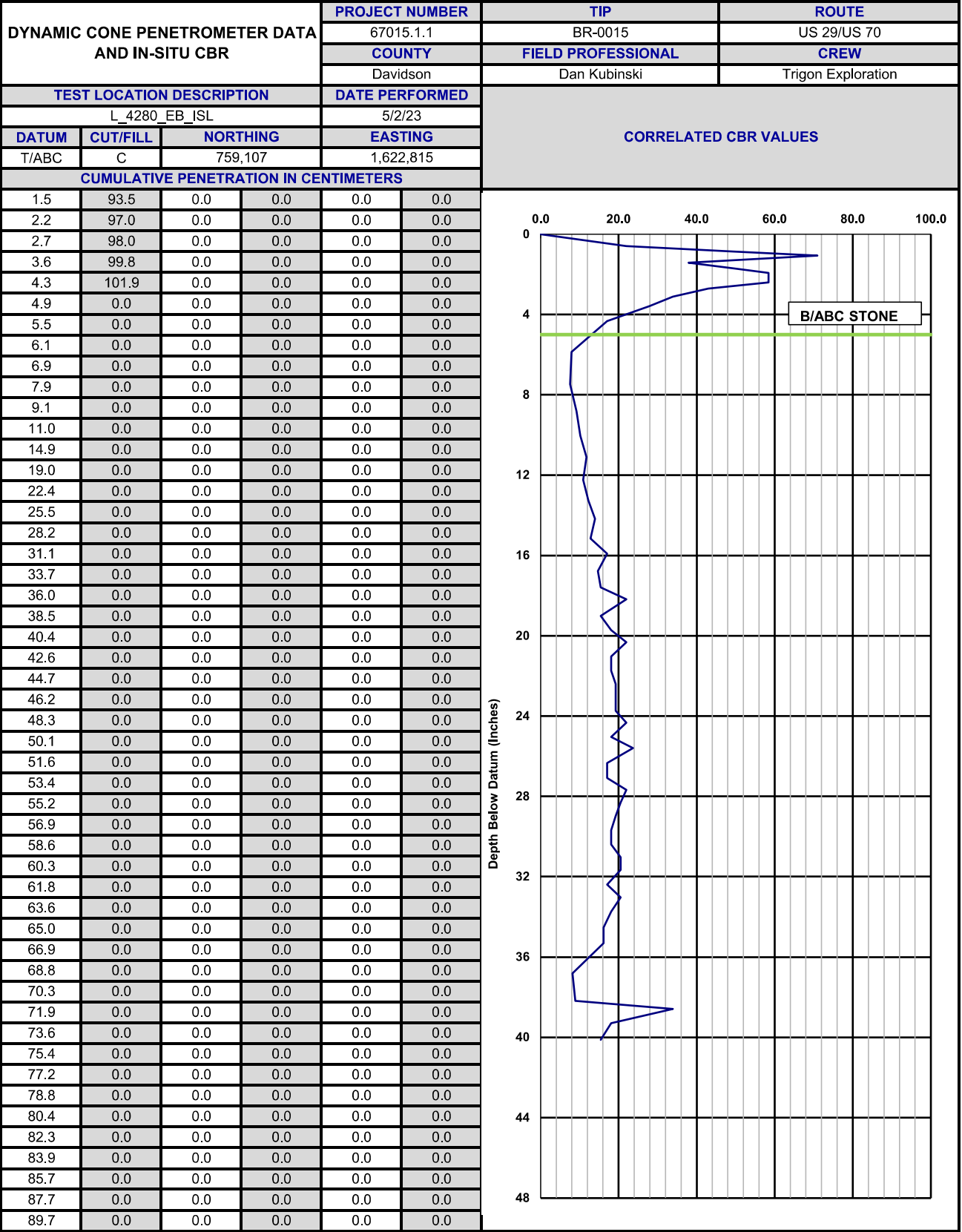
**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade



**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade

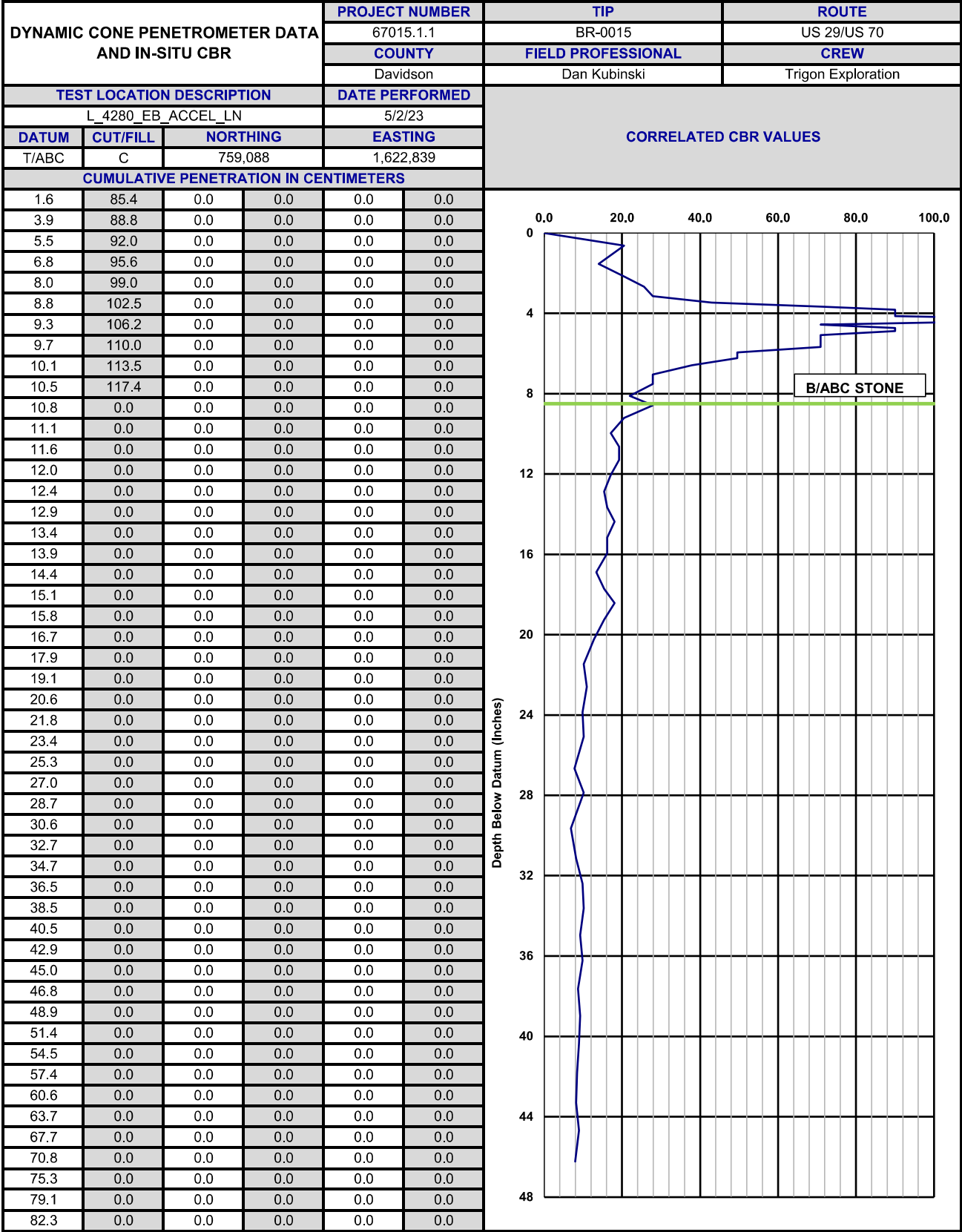


**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade

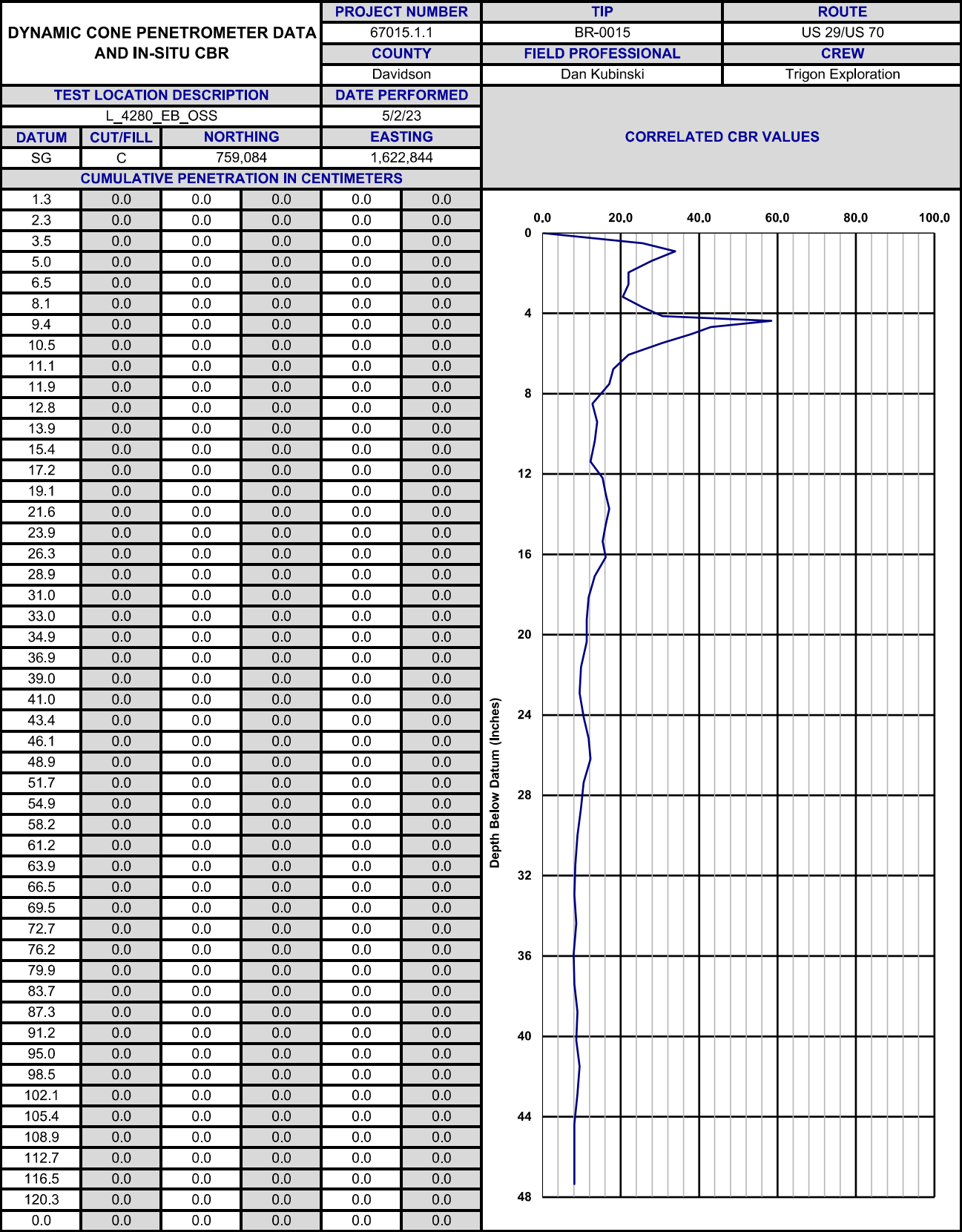


**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade





**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade



**Note(s):**  
WB - Westbound  
EB - Eastbound  
OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane  
AG - At Grade  
F - Fill  
C - Cut  
SG - Subgrade  
T/ABC - Top of ABC Stone  
B/ABC - Bottom of ABC Stone  
Ex. Gr. - Existing Grade

[illegible]

<b>Note(s):</b>		
WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade

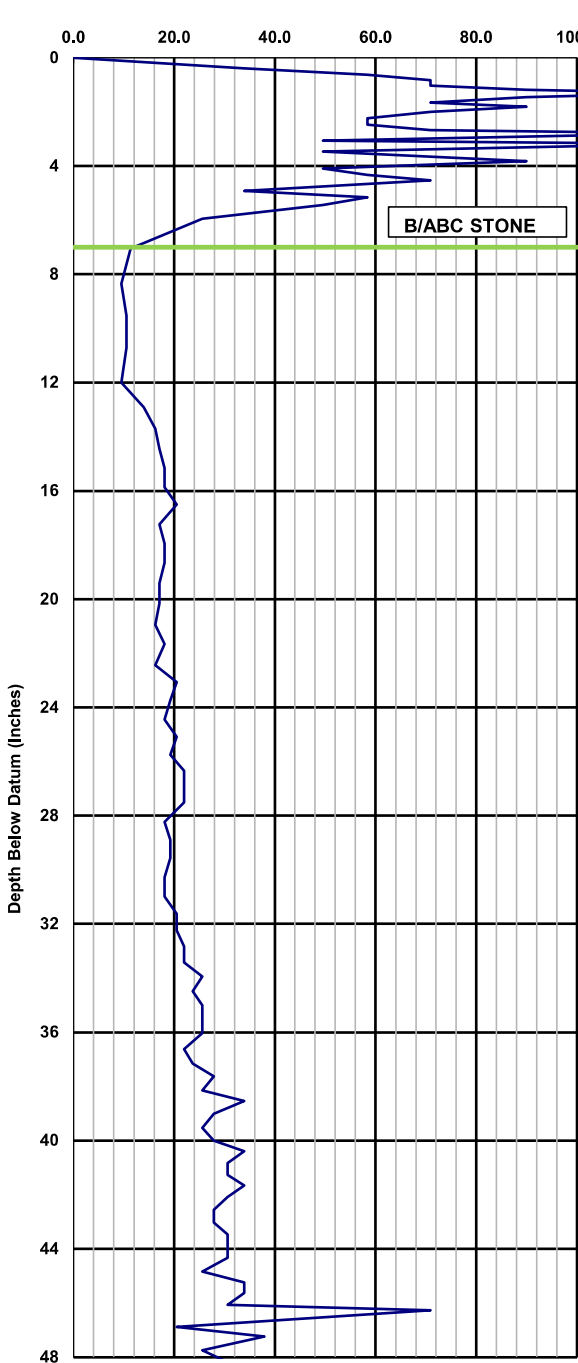
[illegible]

<b>Note(s):</b>		
WB - Westbound	DECEL - Deceleration	C - Cut
EB - Eastbound	ACCEL - Acceleration	SG - Subgrade
OSS - Outside Shoulder	LN - Lane	T/ABC - Top of ABC Stone
ISS - Inside Shoulder	AG - At Grade	B/ABC - Bottom of ABC Stone
OSL - Outside Lane	F - Fill	Ex. Gr. - Existing Grade



[illegible]

<b>Note(s):</b>	
RT - Right	PS - Paved Shoulder
LN - Lane	SG - Subgrade
AG - At Grade	T/ABC - Top of ABC Stone
F - Fill	B/ABC - Bottom of ABC Stone
C - Cut	Ex. Gr. - Existing Grade

DYNAMIC CONE PENETROMETER DATA AND IN-SITU CBR				PROJECT NUMBER		TIP		ROUTE	
				67015.1.1		BR-0015		Murphy Drive	
				COUNTY		FIELD PROFESSIONAL		CREW	
				Davidson		Dan Kubinski		Trigon Exploration	
TEST LOCATION DESCRIPTION				DATE PERFORMED		CORRELATED CBR VALUES			
Y2_1620_LT_LN				5/4/23					
DATUM	CUT/FILL	NORTHING		EASTING					
T/ABC	C	758,472		1,622,178					
CUMULATIVE PENETRATION IN CENTIMETERS									
1.0	66.9	0.0	0.0	0.0	0.0				
1.6	68.4	0.0	0.0	0.0	0.0				
2.1	69.9	0.0	0.0	0.0	0.0				
2.6	71.7	0.0	0.0	0.0	0.0				
3.0	73.4	0.0	0.0	0.0	0.0				
3.3	75.1	0.0	0.0	0.0	0.0				
3.7	76.9	0.0	0.0	0.0	0.0				
4.2	78.7	0.0	0.0	0.0	0.0				
4.6	80.3	0.0	0.0	0.0	0.0				
5.1	81.9	0.0	0.0	0.0	0.0				
5.7	83.4	0.0	0.0	0.0	0.0				
6.3	84.9	0.0	0.0	0.0	0.0				
6.8	86.2	0.0	0.0	0.0	0.0				
7.1	87.6	0.0	0.0	0.0	0.0				
7.8	88.9	0.0	0.0	0.0	0.0				
8.1	90.2	0.0	0.0	0.0	0.0				
8.8	91.5	0.0	0.0	0.0	0.0				
9.3	93.0	0.0	0.0	0.0	0.0				
9.7	94.4	0.0	0.0	0.0	0.0				
10.4	95.6	0.0	0.0	0.0	0.0				
11.0	96.9	0.0	0.0	0.0	0.0				
11.5	97.9	0.0	0.0	0.0	0.0				
12.5	99.1	0.0	0.0	0.0	0.0				
13.1	100.4	0.0	0.0	0.0	0.0				
13.8	101.6	0.0	0.0	0.0	0.0				
15.1	102.6	0.0	0.0	0.0	0.0				
17.9	103.7	0.0	0.0	0.0	0.0				
21.2	104.8	0.0	0.0	0.0	0.0				
24.2	105.8	0.0	0.0	0.0	0.0				
27.2	106.9	0.0	0.0	0.0	0.0				
30.5	108.1	0.0	0.0	0.0	0.0				
32.8	109.3	0.0	0.0	0.0	0.0				
34.8	110.4	0.0	0.0	0.0	0.0				
36.7	111.5	0.0	0.0	0.0	0.0				
38.5	112.6	0.0	0.0	0.0	0.0				
40.3	113.9	0.0	0.0	0.0	0.0				
41.9	114.9	0.0	0.0	0.0	0.0				
43.8	115.9	0.0	0.0	0.0	0.0				
45.6	117.0	0.0	0.0	0.0	0.0				
47.4	117.5	0.0	0.0	0.0	0.0				
49.3	119.1	0.0	0.0	0.0	0.0				
51.2	120.0	0.0	0.0	0.0	0.0				
53.2	121.3	0.0	0.0	0.0	0.0				
55.0	122.4	0.0	0.0	0.0	0.0				
57.0	123.4	0.0	0.0	0.0	0.0				
58.6	124.4	0.0	0.0	0.0	0.0				
60.3	125.6	0.0	0.0	0.0	0.0				
62.1	0.0	0.0	0.0	0.0	0.0				
63.7	0.0	0.0	0.0	0.0	0.0				
65.4	0.0	0.0	0.0	0.0	0.0				

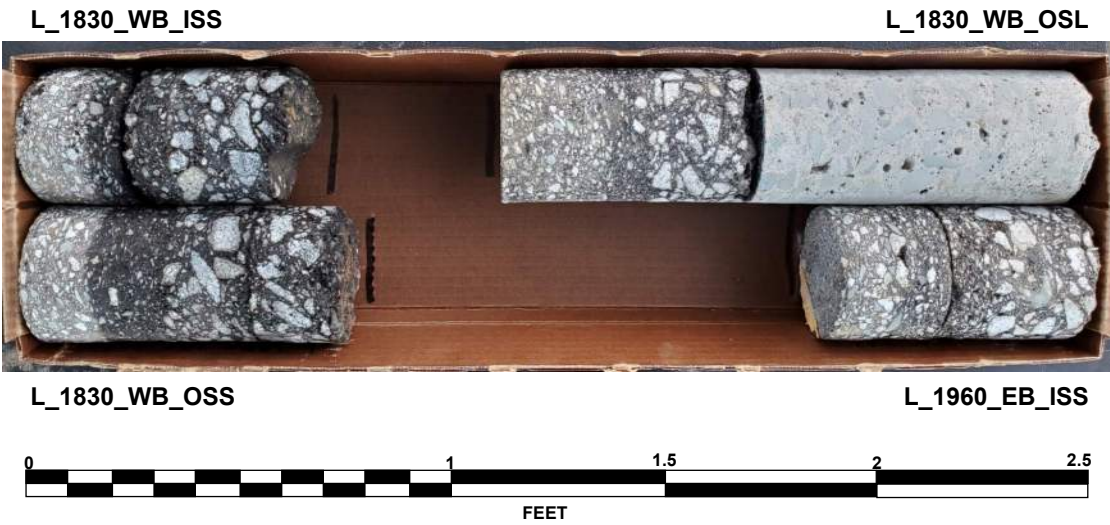
<b>Note(s):</b>	
RT - Right	PS - Paved Shoulder
LN - Lane	SG - Subgrade
AG - At Grade	T/ABC - Top of ABC Stone
F - Fill	B/ABC - Bottom of ABC Stone
C - Cut	Ex. Gr. - Existing Grade

PAVEMENT CORE PHOTOGRAPHS

BR-0015 (67015.1.1)

Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

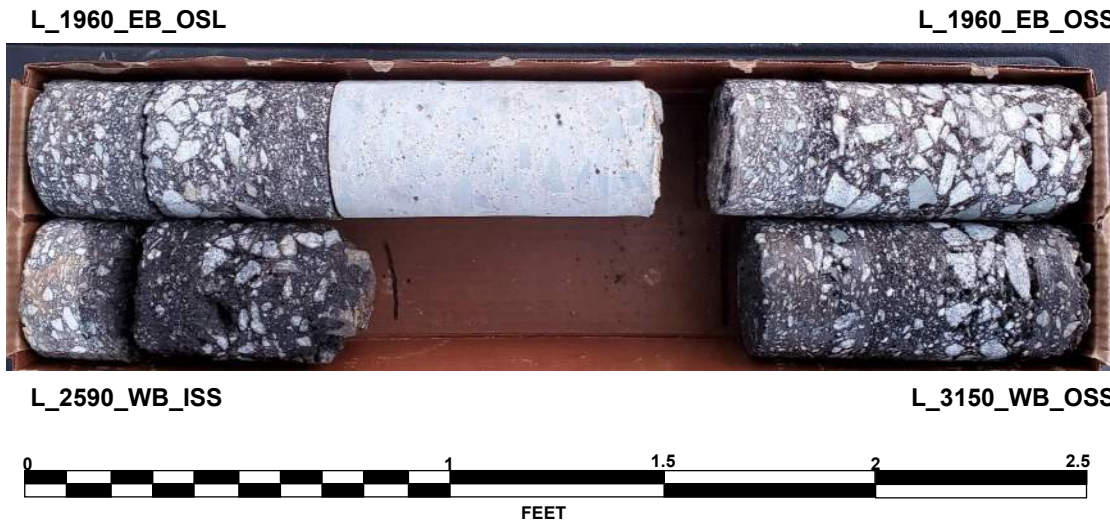
BOX 1



BOX 3



BOX 2



BOX 4

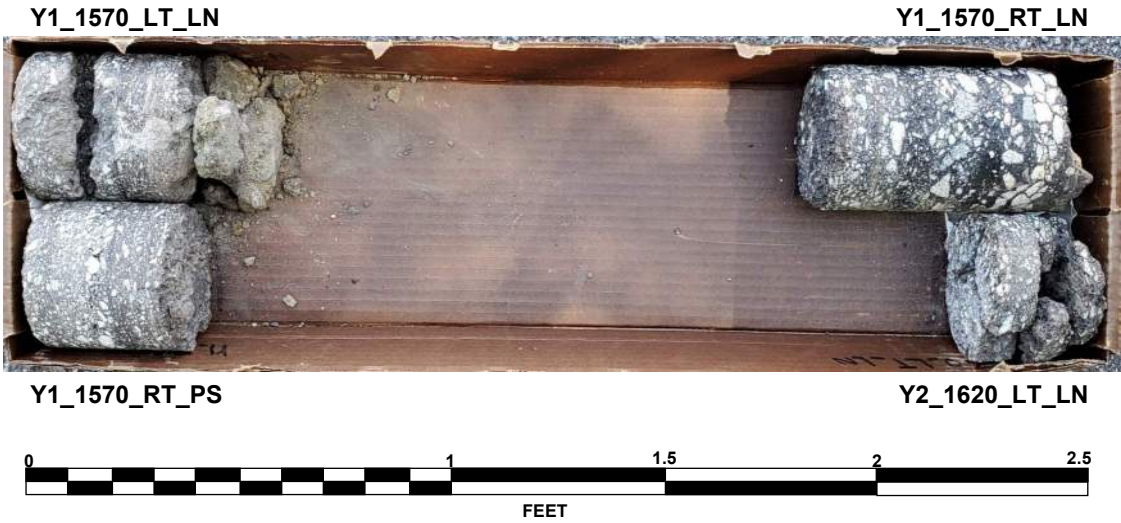


PAVEMENT CORE PHOTOGRAPHS

BR-0015 (67015.1.1)

Bridge No. 67 and No. 68 Replacements on US 29/US 70 NB & SB over SR 1192 (W. 5th Avenue)

BOX 5



LABORATORY SUMMARY SHEET FOR SOIL SAMPLES

WBS NO. (TIP NO.): 67015.1.1 (BR-0015)  
PROJECT ID: 41620  
COUNTY: DAVIDSON  
DESCRIPTION: BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)

									Atterberg Limits			Gradation Results							
Sample No.	Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	N-Value (blows/ft)	L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
S-1	L_1830_WB_ISS	-L-	18+30	12' LT	0.0 - 5.0	27.1	A-7-5	-	82	34	48	3.0	95.0	84.0	59.0	18.7	22.2	18.6	40.5
S-2	L_1960_EB_ISS	-L-	19+60	12' RT	0.0 - 5.0	30.1	A-7-5	--	68	34	34	0.0	99.0	93.0	64.0	13.6	26.7	29.1	30.6
S-3	L_2590_WB_ISS	-L-	25+90	12' LT	0.0 - 5.0	28.4	A-7-5	--	69	34	35	1.0	98.0	92.0	66.0	11.3	26.7	21.6	40.4
S-4	L_3150_WB_OSS	-L-	31+50	39' LT	0.0 - 5.0	35.0	A-7-5	--	76	33	43	1.0	98.0	94.0	76.0	6.1	23.4	21.6	48.9
S-5	L_3275_EB_DECEL_LN	-L-	32+75	47' RT	0.0 - 5.0	26.7	A-7-5	--	64	31	33	1.0	99.0	96.0	75.0	5.8	26.0	31.9	36.3
S-6	L_3870_WB_ISS	-L-	38+70	12' LT	0.0 - 5.0	25.9	A-7-6	--	59	20	39	2.0	96.0	89.0	72.0	12.8	15.7	15.0	56.5
S-18*	L_4000^	-L-	40+00	0' CL	1.0 - 1.5	33.2	A-7-5	--	65	40	25	0.0	99.1	94.7	73.2	10.1	20.9	23.9	45.1
S-7	L_4280_EB_ISL	-L-	42+80	13' RT	0.0 - 5.0	38.4	A-7-5	--	79	42	37	0.0	100.0	98.0	73.0	5.2	26.5	15.6	52.7
S-20*	L_4600^	-L-	46+00	0' CL	1.0 - 2.0	26.8	A-7-5	--	51	33	18	0.0	99.0	96.0	51.0	13.0	41.3	13.6	32.1
S-8	Y1_1570_RT_LN	-Y1-	15+70	10' RT	0.0 - 5.0	24.8	A-7-6	--	59	24	35	1.0	99.0	93.0	69.0	13.2	21.2	19.4	46.2
S-9	Y2_1620_LT_LN	-Y2-	16+20	6' LT	0.0 - 5.0	32.8	A-7-5	--	65	31	34	1.0	99.0	97.0	80.0	5.1	19.5	20.3	55.1
CBR-1*	L_1450^	-L-	14+50	88' LT	8.5 - 18.5	27.0	A-7-5	--	60	44	16	0.0	100.0	93.1	56.6	14.7	35.3	23.4	26.6
CBR-2*	L_1850^	-L-	18+50	89' LT	0.0 - 10.0	30.6	A-7-5	--	60	35	25	0.0	100.0	95.2	75.9	9.9	19.4	26.1	44.7
CBR-3*	L_4200^	-L-	42+00	0' CL	0.0 - 2.5	37.3	A-7-5	--	73	37	36	8.0	91.0	96.6	78.4	7.2	18.6	28.9	45.2

\*Based on Roadway Investigation sample number

^Based on Roadway Investigation boring number

Michelle Stadel, P.E.  
Lab Manager, NCDOT Certification No.: 111-02-1203

Victoria Siebert  
Lab Technician, NCDOT Certification No.: 109-02-1003





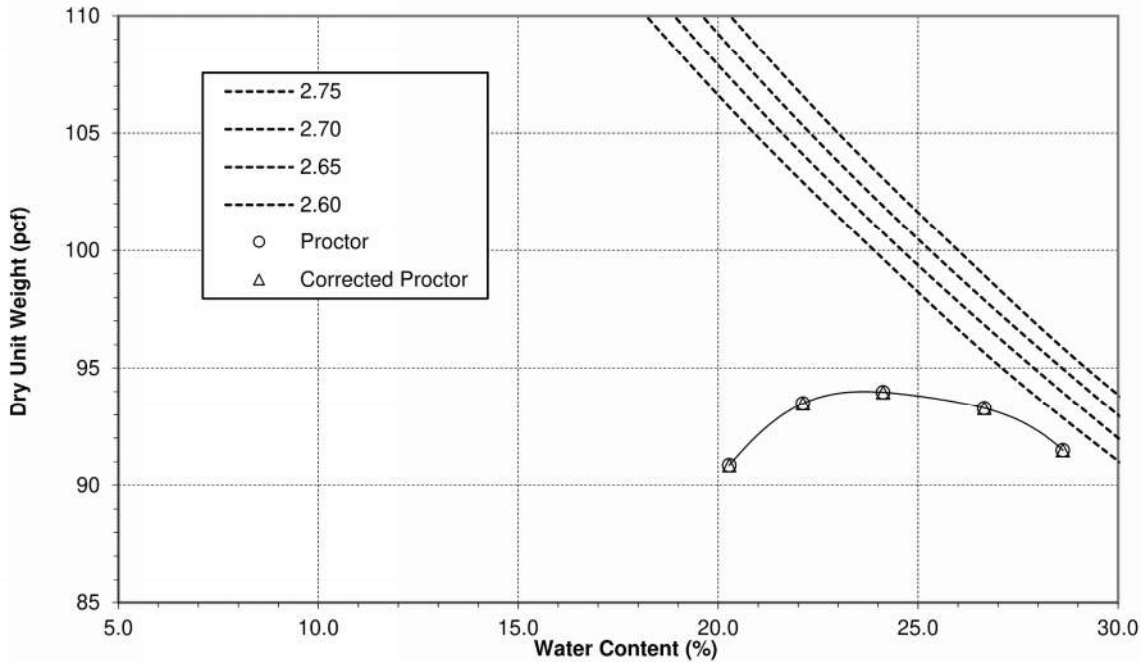
Laboratory Test Report

Client: North Carolina Dept. of Transportation  
Project: 20235702.001A  
NCDOT BR-0015 Roadway  
02-000L - Lab

Report No.: 23-CLT-00649 Rev. 1  
Sampled by: Mayson Foster  
Submitted by: Dan Kubinski

Issued: 6/9/2023  
Field ID: L\_1450, CBR-1  
Date: 5/8/2023  
Date: 5/15/2023

Tested on 5/24/2023 by C. Blalock  
Material Description: Brown Sandy Silt (A-7-5)  
Location: Boring No. L\_1450, Alignment -L-, STA 14+50, 88' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	94.0	na
Optimum Water Content (%)	23.6	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual  
Specimen Preparation: Moist

Remarks:  
AASHTO T-100, Soil Specific Gravity @ 20°C: 2.747

Signature area for review.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation  
Project: 20235702.001A  
NCDOT BR-0015 Roadway  
02-000L - Lab

Report No.: 23-CLT-00649 Rev. 1  
Sampled by: Mayson Foster  
Submitted by: Dan Kubinski

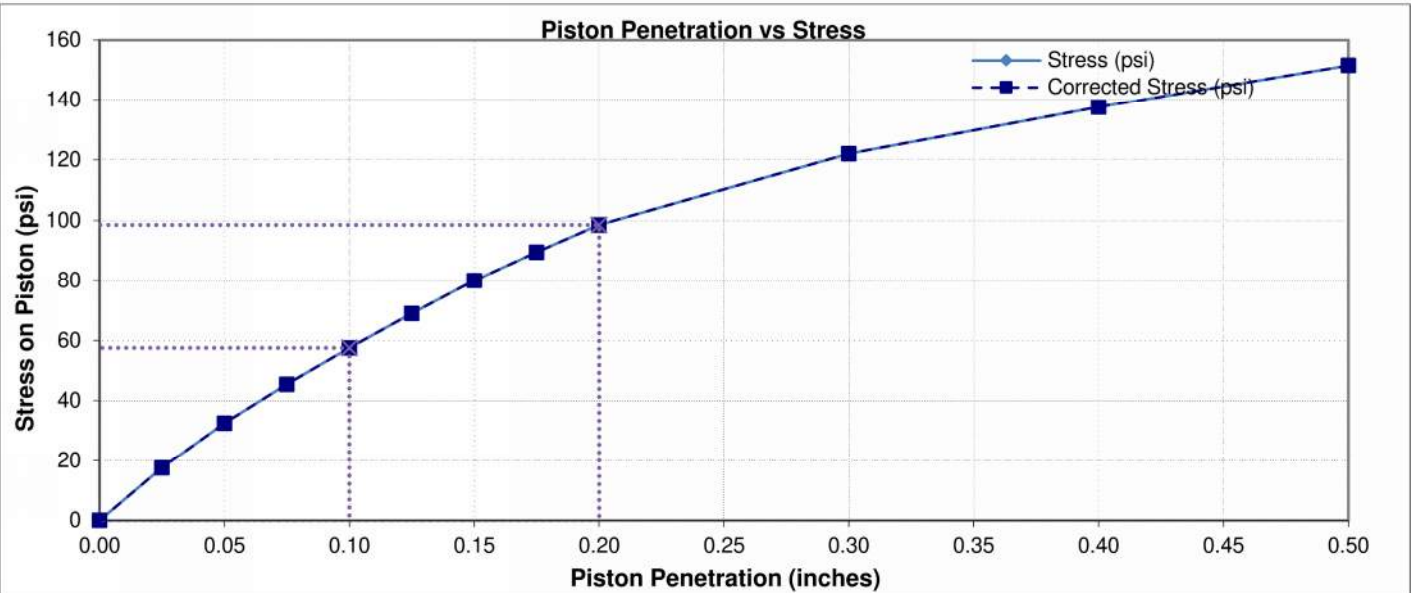
Issued: 6/9/2023  
Field ID: L\_1450, CBR-1  
Date: 5/8/2023  
Date: 5/15/2023

Sample Source: Boring No. L\_1450, Alignment -L-, STA 14+50, 88' LT  
Sample ID: CBR-1  
Sample Description: Brown Sandy Silt (A-7-5)  
Material Used:  
Surcharge Weight: 10 lbs

Date Tested: 5/30/2023  
Tested By: C. Blalock  
Condition of Sample: Soaked  
Time Soaked: 96 hrs

AASHTO T193 - Standard Test Method for The California Bearing Ratio (CBR)

Dry Unit Wgt Before Soaking (pcf):	90.8	Compaction Method:	Manual
Water Content Before Soaking (%):	21.2	Max. Dry Unit Weight:	94.0 pcf
Dry Unit Wgt After Soaking (pcf):	101.1	Optimum Water Content:	23.6 %
Water Content After Soak, Top in. (%):	30.3		
Swell (%):	5.65		
CBR (Corrected CBR) @ 0.1 in. Penetration:	5.8 (5.8)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.6 (6.6)		



Remarks:

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L\_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

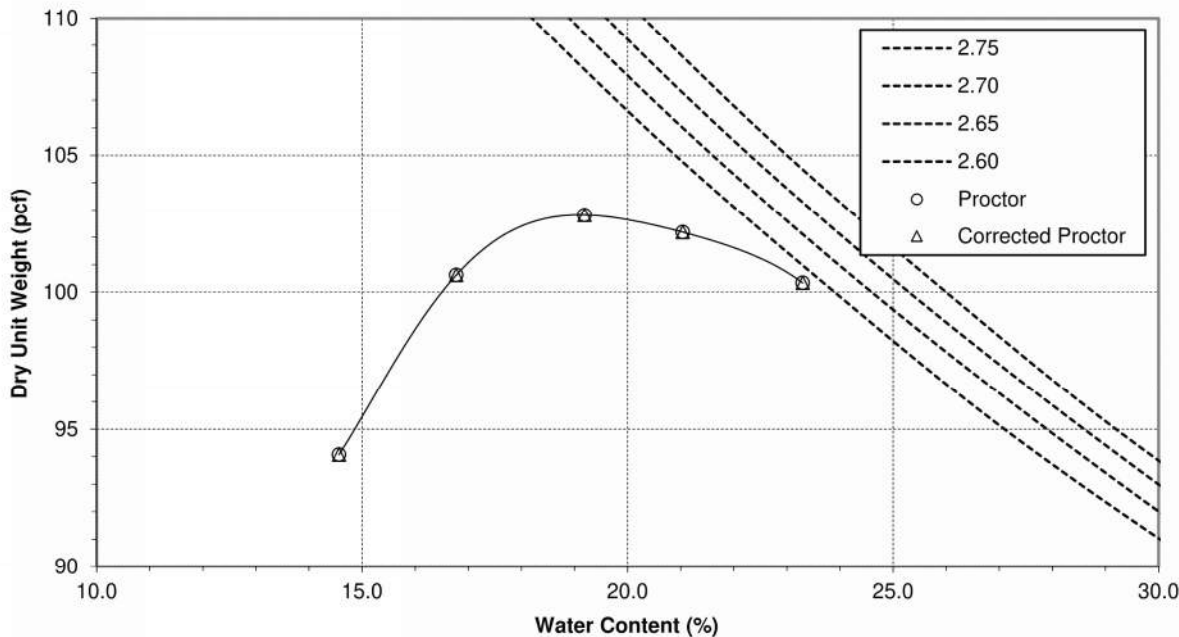
NCDOT BR-0015 Roadway

02-000L - Lab

Tested on 5/24/2023 by C. Blalock

Material Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Location: Boring No. L\_1850, -L-, STA 18+50, 89' LT



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	102.8	na
Optimum Water Content (%)	19.1	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Moist

Remarks:

AASHTO T-100, Soil Specific Gravity at 20°C: 2.678

SEED

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00650 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L\_1850, CBR-2

Sampled by: Mayson Foster

Date: 5/9/2023

Submitted by: Dan Kubinski

Date: 5/15/2023

NCDOT BR-0015 Roadway

02-000L - Lab

Sample Source: Boring No. L\_1850, -L-, STA 18+50, 89' LT

Sample ID: CBR-2

Sample Description: Reddish Yellow Elastic Silt with Sand (A-7-5)

Material Used:

Surcharge Weight: 10 lbs

Date Tested: 5/30/2023

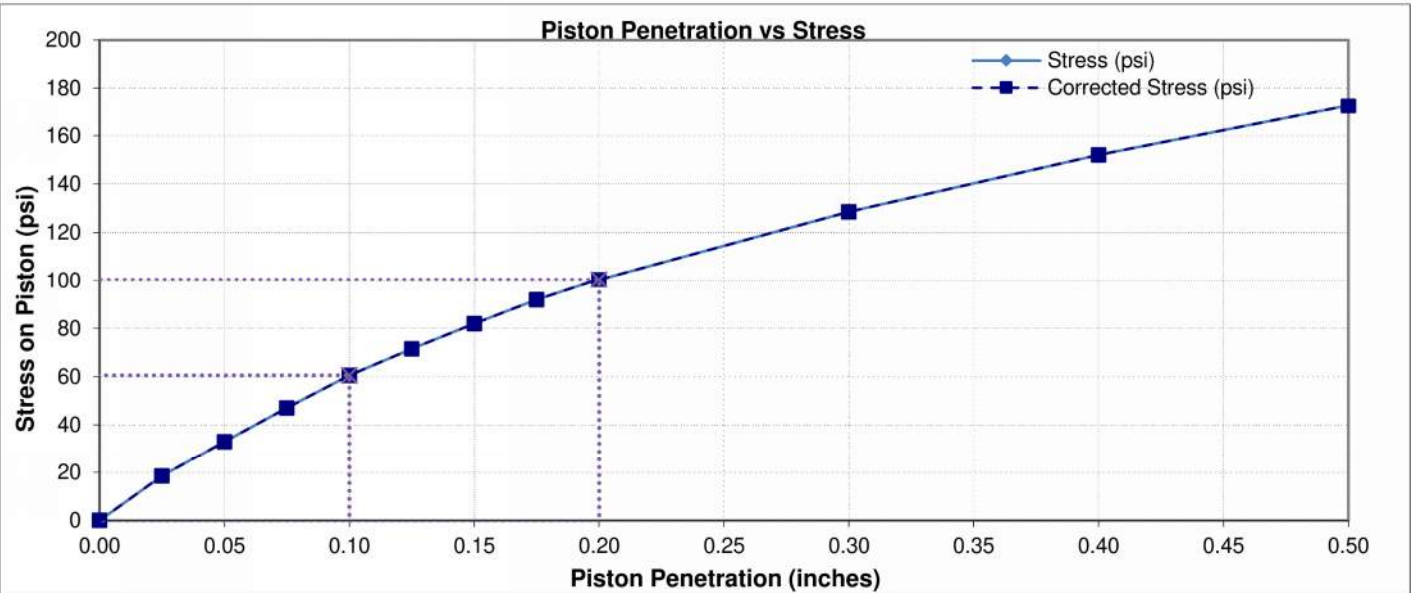
Tested By: C. Blalock

Condition of Sample: Soaked

Time Soaked: 96 hrs

AASHTO T-193 - Standard Test Method for The California Bearing Ratio

Dry Unit Wgt Before Soaking (pcf):	100.6	Compaction Method:	Manual
Water Content Before Soaking (%):	17.0	Max. Dry Unit Weight:	102.8 pcf
Dry Unit Wgt After Soaking (pcf):	106.1	Optimum Water Content:	19.1 %
Water Content After Soak, Top in. (%):	27.1		
Swell (%):	4.1		
CBR (Corrected CBR) @ 0.1 in. Penetration:	6.1 (6.1)		
CBR (Corrected CBR) @ 0.2 in. Penetration:	6.7 (6.7)		



Remarks:

SEED

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.





Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L\_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

NCDOT BR-0015 Roadway

Submitted by: Dan Kubinski

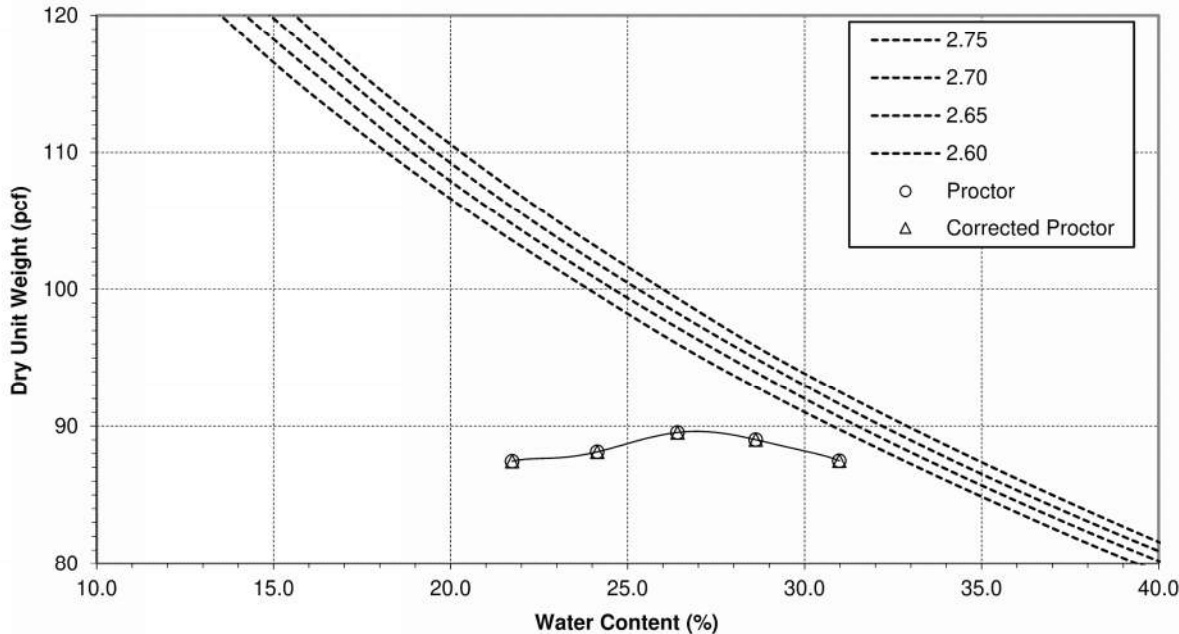
Date: 5/15/2023

02-000L - Lab

Tested on 5/26/2023 by C. Blalock

Material Description: Brown Sandy Silt (A-7-5)

Location: Boring No. L-1450, Alignment -L-, STA 14+50, 88' LT, CBR-1, 8.5' - 18.5'



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	89.6	na
Optimum Water Content (%)	26.9	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Dry

Remarks:

+4% Lime

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00649 Rev. 1

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L\_1450, CBR-1

Sampled by: Mayson Foster

Date: 5/8/2023

NCDOT BR-0015 Roadway

Submitted by: Dan Kubinski

Date: 5/15/2023

02-000L - Lab

Tested By : C. Blalock

Date Molded: 5/31/2023

Sample Location: Boring No. L-1450, CBR-1, 8.5' - 18.5'

Alignment -L-, STA 14+50, 88' LT

ASTM D5102 - Modified, Unconfined Compressive Strength of Compacted Soil-Lime Mixtures

	A	B	C	D
Sample Preparation:	AASHTO T99	AASHTO T99	AASHTO T99	AASHTO T99
Water Content (%):	25.7	26	25.9	26
Height (in):	4.632	4.619	4.644	4.62
Diameter (in):	4.00	4.00	3.999	4.002
Cross-Sectional Area (in <sup>2</sup> ):	12.57	12.59	12.56	12.58
Test Date:	6/7/2023	6/7/2023	6/7/2023	6/7/2023
Age (days):	7	7	7	7
Maximum Load (lbf):	860	830	850	850
h/d Conversion Factor	none	none	none	none
Compressive Strength (psi):	70	65	70	70
Dry Unit Weight (pcf):	89.6	89.2	88.4	88.5

Sample Preparation:

Water Content (%):

Height (in):

Diameter (in):

Cross-Sectional Area (in<sup>2</sup>):

Test Date:

Age (days):

Maximum Load (lbf):

h/d Conversion Factor

Compressive Strength (psi):

Curing Details: Samples extruded, placed in plastic bags, and cured for 7 days at 73°F ± 4°.

Specification: Per NCDOT, cast according to AASHTO T99 and broke per AASHTO T208.

Remarks:

(A) 3.0% Lime; Percent Strain = 2.0. (B) 3.0% Lime; Percent Strain = 2.4.

(C) 5% Lime; Percent Strain = 2.5. (D) 5% Lime; Percent Strain = 2.9.

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M. Stadel



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00651 Rev. 0

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L-4200, CBR-3

NCDOT BR-0015 Roadway

Sampled by: Dan Kubinski

Date: 5/5/2023

02-000L - Lab

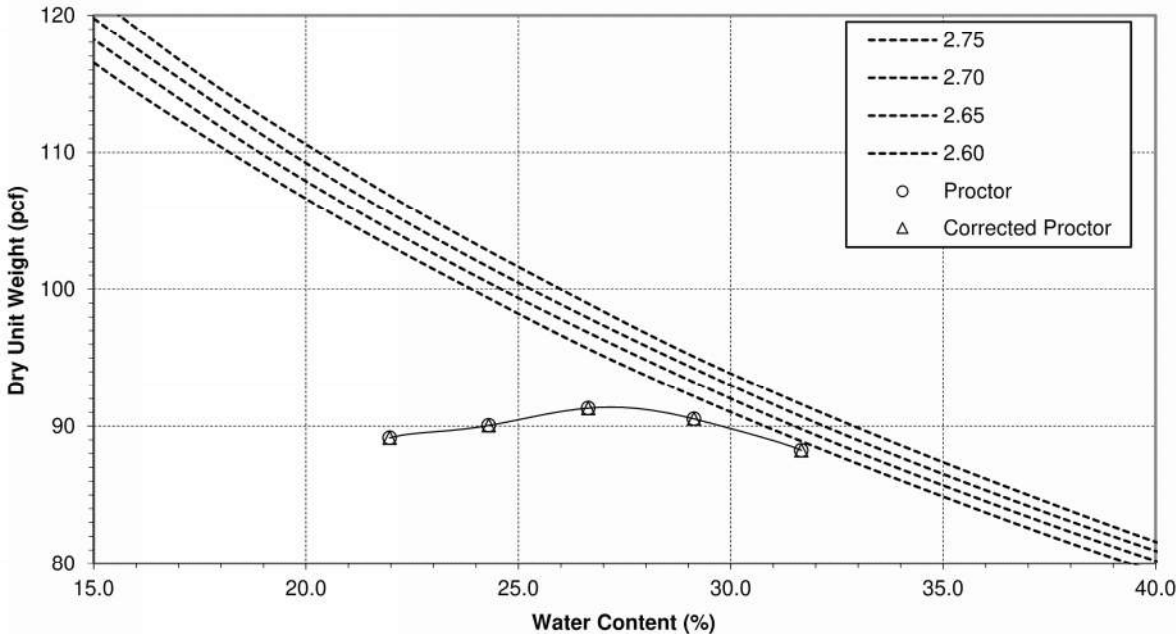
Submitted by: Dan Kubinski

Date: 5/15/2023

Tested on 5/25/2023 by C. Blalock

Material Description: Reddish Brown Elastic Silt with Sand (A-7-5)

Location: Boring No. L-4200, Alignment -L-, STA 42+00, 0' CL, CBR-3, 0' - 2.5'



Test Method: AASHTO T99 A	Uncorrected	Corrected
Maximum Dry Unit Weight (pcf)	91.4	na
Optimum Water Content (%)	27.2	na
Oversize Fraction, retained on 3/4 (%)		<5
Bulk Specific Gravity of Oversize Fraction		na

Rammer Type: Manual

Specimen Preparation: Dry

Remarks:

+4% Lime

AASHTO T100, Soil Specific Gravity = 2.727

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



Laboratory Test Report

Client: North Carolina Dept. of Transportation

Report No.: 23-CLT-00651 Rev. 0

Issued: 6/9/2023

Project: 20235702.001A

Field ID: L-4200, CBR-3

NCDOT BR-0015 Roadway

Sampled by: Dan Kubinski

Date: 5/5/2023

02-000L - Lab

Submitted by: Dan Kubinski

Date: 5/15/2023

Tested By : C. Blalock

Date Molded: 5/31/2023

Sample Location: Boring No. L-4200, CBR-3, 0' - 2.5'

Alignment -L-, STA 42+00, 0' CL

ASTM D5102 - Modified, Unconfined Compressive Strength of Compacted Soil-Lime Mixtures

	A	B	C	D
Sample Preparation:	AASHTO T208	AASHTO T208	AASHTO T208	AASHTO T208
Water Content (%):	25.4	25.5	26.2	25.9
Height (in):	4.641	4.623	4.628	4.615
Diameter (in):	3.99	4.01	4	4
Cross-Sectional Area (in <sup>2</sup> ):	12.53	12.6	12.57	12.57
Test Date:	6/7/2023	6/7/2023	6/7/2023	6/7/2023
Age (days):	7	7	7	7
Maximum Load (lbf):	760	770	820	910
h/d Conversion Factor	none	none	none	none
Compressive Strength (psi):	60	60	65	70
Dry Unit Weight (pcf):	91.9	91.6	90.1	90.5

Sample Preparation:

Water Content (%):

Height (in):

Diameter (in):

Cross-Sectional Area (in<sup>2</sup>):

Test Date:

Age (days):

Maximum Load (lbf):

h/d Conversion Factor

Compressive Strength (psi):

Curing Details: Samples extruded, placed in plastic bags, and cured for 7 days at 73°F ± 4°.

Specification: Per NCDOT, cast according to AASHTO T99 and broke per AASHTO T208.

Remarks:

(A) 3.0% Lime; Percent Strain = 2.2. (B) 3.0% Lime; Percent Strain = 2.1.

(C) 5% Lime; Percent Strain = 2.1. (D) 5% Lime; Percent Strain = 2.4.

Reviewed on 6/9/2023 by Michelle Stadel,

Michelle M Stadel

Limitations: Pursuant to applicable building codes, the results presented in this report are for the exclusive use of the client and the registered design professional in responsible charge. The results apply only to the samples tested. If changes to the specifications were made and not communicated to Kleinfelder, Kleinfelder assumes no responsibility for pass/fail statements (meets/did not meet), if provided. This report may not be reproduced, except in full, without written approval of Kleinfelder.



**PAVEMENT CORE EVALUATION**

**BR-0015 (67015.1.1)**

**BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)**

LINE	STATION	ABC STONE (in)	LAYER THICKNESS (in)	PAVEMENT LAYERS	REMARKS
-L-	1830_WB_ISS 6.75" Asphalt	2.00	4.00	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			2.75	B	1 Lift; Low oxidation
-L-	1830_WB_OSL 7.25" Asphalt 9.25" Concrete	4.50	4.25	S	3 Lifts; Low oxidation
			3.00	B	1 Lift; Delamination between base asphalt and concrete layer; Low oxidation
			9.25	C	1 Lift
-L-	1830_WB_OSS 8.50" Asphalt	0.00	3.75	S	3 Lifts; Low oxidation
			4.75	B	2 Lifts; Low oxidation
-L-	1960_EB_ISS 7.25" Asphalt	0.00	4.25	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			3.00	B	1 Lift; Low oxidation
-L-	1960_EB_OSL 8.25" Asphalt 9.25" Concrete	4.75	3.00	S	2 Lifts; Delamination between surface asphalt and base asphalt layers; Low oxidation
			3.25	B	1 Lift; Low oxidation
			2.00	S	1 Lift; Delamination between surface asphalt and concrete layer; Low oxidation
			9.25	C	1 Lift
-L-	1960_EB_OSS 10.25" Asphalt	11.75	3.00	S	2 Lifts; Low oxidation
			7.25	B	2 Lifts; Low oxidation
-L-	2590_WB_ISS 8.50" Asphalt	0.00	4.50	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			4.00	B	1 Lift; Low oxidation
-L-	3150_WB_OSS 9.25" Asphalt	0.00	5.25	S	3 Lifts; Low oxidation
			2.50	B	1 Lift; Low oxidation
			1.50	S	1 Lift; Low oxidation
-L-	3275_EB_DECEL_LN 10.75" Asphalt	7.00	5.00	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			5.75	B	1 Lift; Low oxidation
-L-	3870_WB_ISS 7.00" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.00	B	1 Lift; Low oxidation
-L-	3870_WB_DECEL_LN 11.00" Asphalt	7.00	4.00	S	2 Lifts; Low oxidation
			7.00	B	2 Lifts; Low oxidation
-L-	3870_WB_OSS 7.50" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.50	B	1 Lift; Low oxidation

**Note(s):**

NM - Not Measured  
PS - Paved Shoulder  
WB - Westbound  
EB - Eastbound  
LT - Left  
RT - Right

OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane

S - Asphalt Surface Course  
I - Asphalt Intermediate Course  
B - Asphalt Base Course  
C - Concrete

**PAVEMENT CORE EVALUATION**

**BR-0015 (67015.1.1)**

**BRIDGE NO. 67 AND NO. 68 REPLACEMENTS ON US 29/US 70 NB & SB OVER SR 1192 (W. 5TH AVENUE)**

LINE	STATION	ABC STONE (in)	LAYER THICKNESS (in)	PAVEMENT LAYERS	REMARKS
-L-	4280_EB_ISL 8.50" Asphalt 9.50" Concrete	5.00	3.75	S	3 Lifts; Delamination between 2nd and 3rd lift; Low oxidation
			2.75	B	1 Lift; Low oxidation
			2.00	S	2 Lifts; Delamination between 1st and 2nd lift; Low oxidation
			9.50	C	
-L-	4280_EB_ACCEL_LN 9.50" Asphalt	8.50	3.75	S	3 Lifts; Low oxidation
			5.75	B	1 Lift; Low oxidation
-L-	4280_EB_OSS 7.00" Asphalt	0.00	3.50	S	3 Lifts; Low oxidation
			3.50	B	1 Lift; Low oxidation
-Y1-	1570_LT_PS 5.00" Asphalt	NM	2.50	S	1 Lift; Low oxidation
			2.50	I	1 Lift; Low oxidation
-Y1-	1570_LT_LN 6.00" Asphalt	NM	6.00	S	2 Lifts; Low oxidation; Cracked completely through 1st surface asphalt layer
-Y1-	1570_RT_LN 7.00" Asphalt	0.00	4.00	S	2 Lifts; Low oxidation
			3.00	I	1 Lift; Low oxidation
-Y1-	1570_RT_PS 4.00" Asphalt	NM	4.00	S	2 Lifts; Low oxidation
-Y2-	1620_LT_LN 3.00" Asphalt	7.00	3.00	S	2 Lifts; Low oxidation; Cracked completely through surface asphalt layer

**Note(s):**

NM - Not Measured  
PS - Paved Shoulder  
WB - Westbound  
EB - Eastbound  
LT - Left  
RT - Right

OSS - Outside Shoulder  
ISS - Inside Shoulder  
OSL - Outside Lane  
DECEL - Deceleration  
ACCEL - Acceleration  
LN - Lane

S - Asphalt Surface Course  
I - Asphalt Intermediate Course  
B - Asphalt Base Course  
C - Concrete