

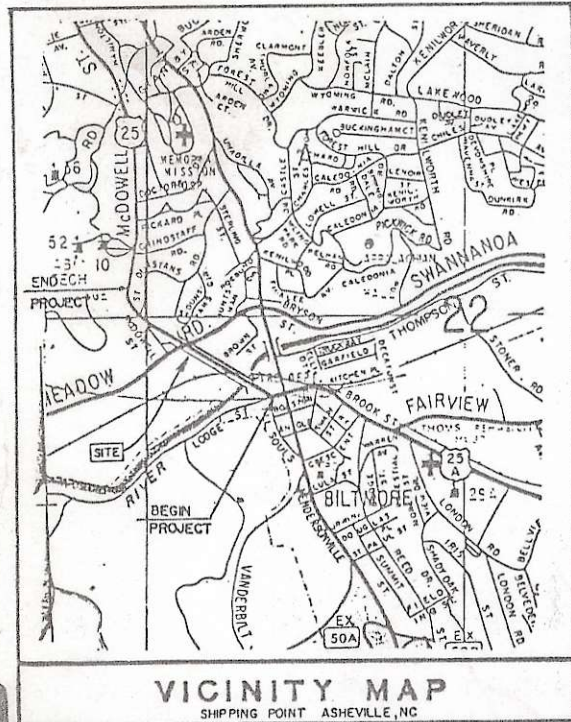
B-1070

PROJECT: 81841002

STRUCTURES

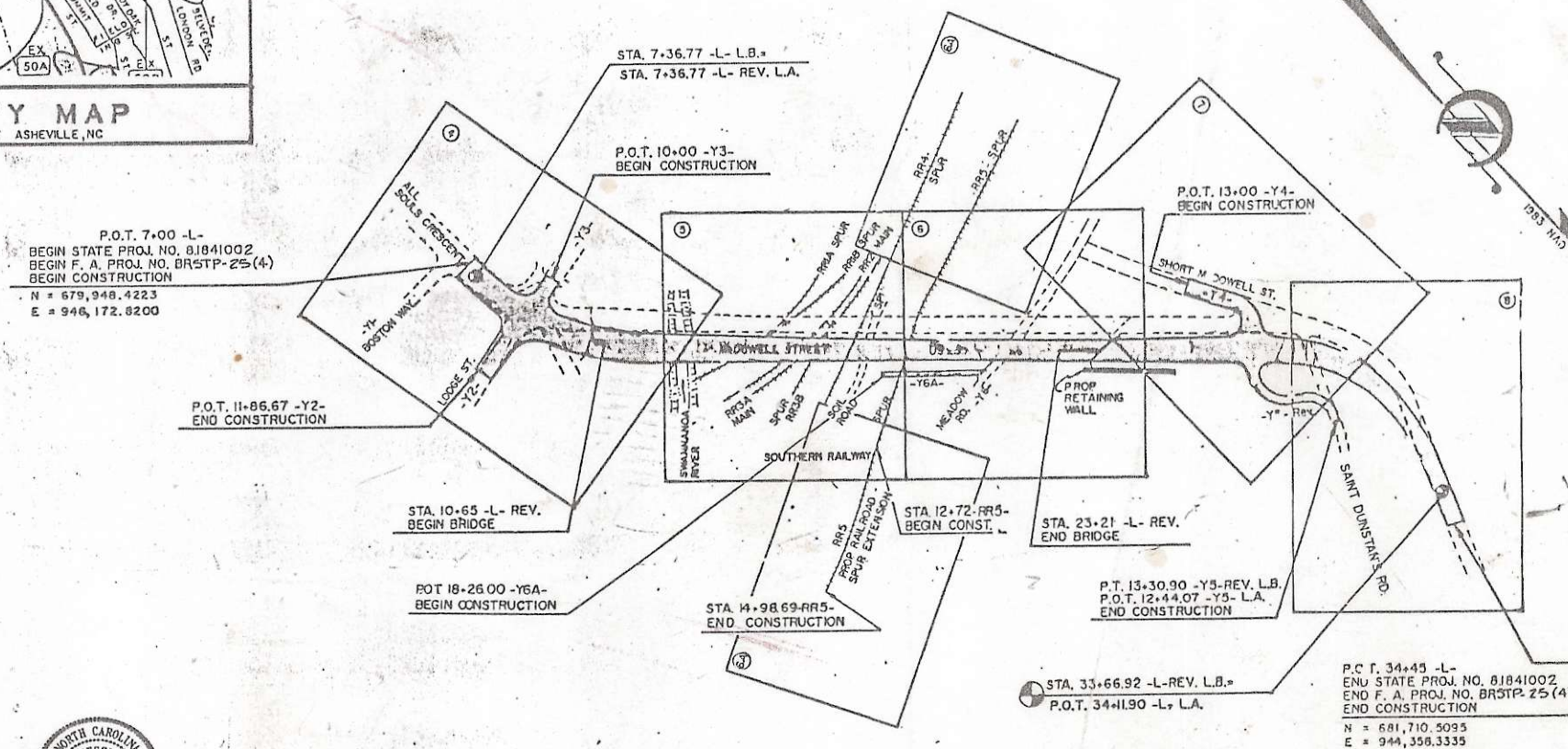
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
PLAN AND PROFILE OF PROPOSED
STATE HIGHWAY
BUNCOMBE CO.

Brq. #76



LOCATION: BRIDGE NO. 76 OVER SWANNANOA RIVER,
SOUTHERN RAILWAY AND SR 3556 AND
APPROACHES ON McDOWELL STREET
(US-25) IN CITY OF ASHEVILLE.

TYPE OF WORK: GRADING, PAVING, DRAINAGE, STRUCTURE & LIGHTING

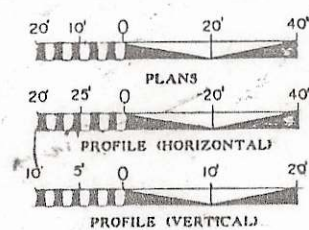


STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N. C.	81841002		
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
81841001	BRM-5000(6)	P.E.	
81841002	BRSTP-25(4)	Const.	

CONVENTIONAL SYMBOLS

County Line	
City or Town Line	
Exist. Right of Way Line & Marker	
Prop. Right of Way Line & Marker	
Survey Line	
Property Line	
Exist. Fence	
Proposed Road	
Existing Road	
Railroad	
Control of Access Line	
Slope Stake Line	
Bridge	
Culvert	
Woods	
Telephone or Telegraph Pole	
Tower Pole and Line	
Power Pole	
Easement Line	
Temporary Silt Fence	
Guard Rail	
Sanitary Sewer Line	
Water Line	
Gas Line	
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Prop. Wheelchair Ramp	
TEMP. DRAINAGE EASEMENT	
PERM. DRAINAGE EASEMENT	
PERM. EASEMENT	

GRAPHIC SCALES



DESIGN DATA

ADT 1995 = 25,300
ADT 2015 = 44,700
DHV = 10 %
D = 60 %
T = 3 %
V = 35 MPH

PLANS PREPARED BY:



TGS ENGINEERS
CONTACT: MR. R. E. PEARSON, P.E., NCDOT COORDINATION ENGINEER

LETTING DATE: OCT. 15, 1996

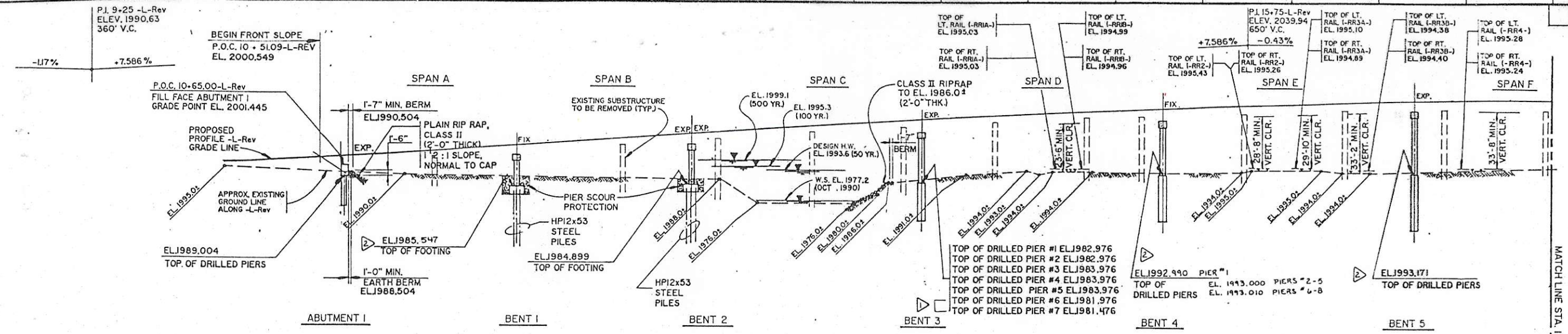
LENGTH OF ROADWAY F. A. PROJECT BRM-5000 (6) = 0.273 MILE
LENGTH OF STRUCTURE F. A. PROJECT BRM-5000 (6) = 0.238 MILE
TOTAL LENGTH OF STATE PROJECT 81841001 = 0.511 MILES

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

DR Martin P.E.
STATE HIGHWAY ENGINEER - DESIGN

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED _____
DIVISION ADMINISTRATOR DATE



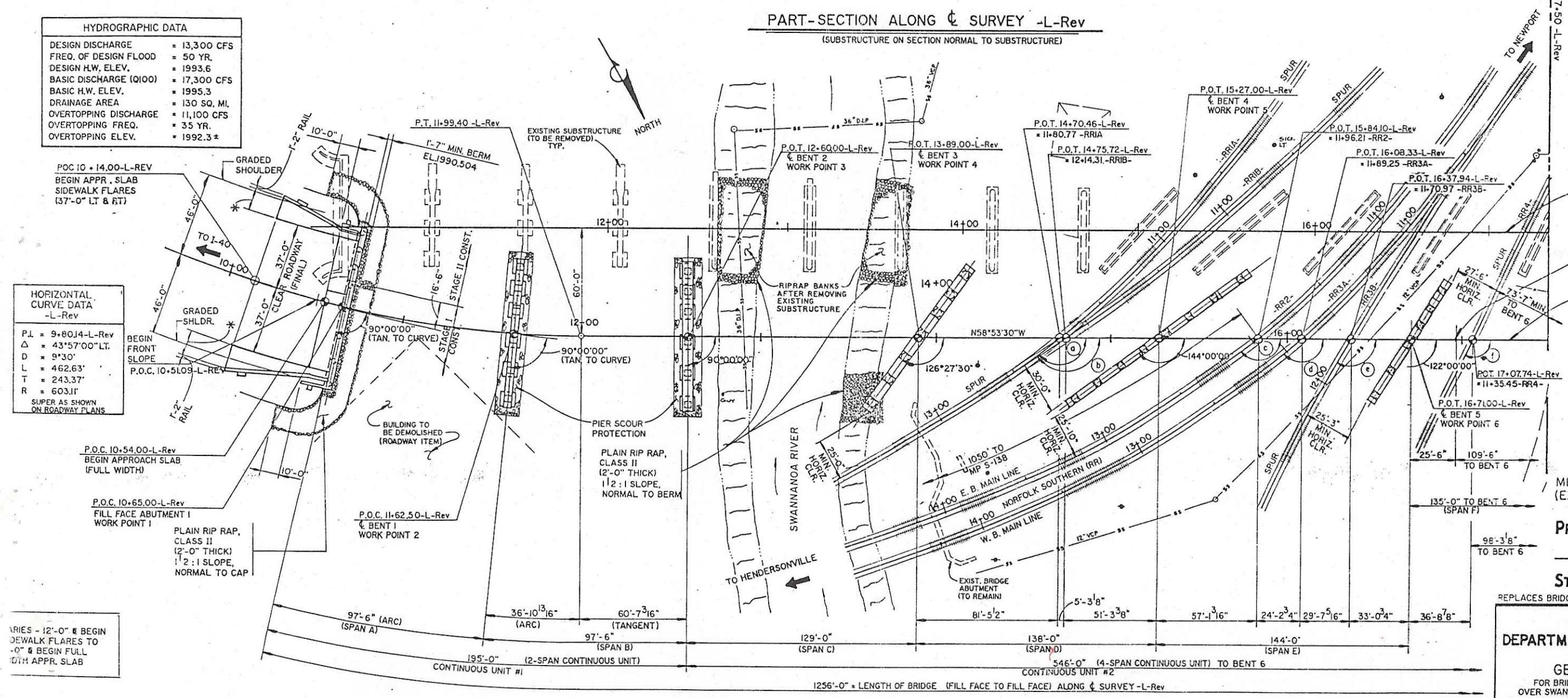
HYDROGRAPHIC DATA

DESIGN DISCHARGE	= 13,300 CFS
FREQ. OF DESIGN FLOOD	= 50 YR.
DESIGN H.W. ELEV.	= 1993.6
BASIC DISCHARGE (Q100)	= 17,300 CFS
BASIC H.W. ELEV.	= 1995.3
DRAINAGE AREA	= 130 SQ. MI.
OVERTOPPING DISCHARGE	= 11,100 CFS
OVERTOPPING FREQ.	= 35 YR.
OVERTOPPING ELEV.	= 1992.3*

HORIZONTAL CURVE DATA -L-Rev

P.L.	= 9+80.14-L-Rev
Δ	= 43°57'00" LT.
D	= 9°30'
L	= 462.63'
T	= 243.37'
R	= 603.11'

SUPER AS SHOWN ON ROADWAY PLANS



ANGLES

(a)	141°24'36"
(b)	146°39'59"
(c)	136°50'34"
(d)	137°23'04"
(e)	119°30'29"
(f)	116°05'04"

MILE POST S-139.2 (E.B. MAIN LINE)

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev

REPLACES BRIDGE NO. 76

SHEET 1 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNAHOA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JULY 1995

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1	ROR	2/1/97	3		
2	ROR	3/1/97	4		

SHEET NO. 5-1
TOTAL SHEETS 274

VARIES - 12'-0" & BEGIN DEWALK FLARES TO -0" & BEGIN FULL 10'W APPR. SLAB

DATE 7/95
DATE 7/95

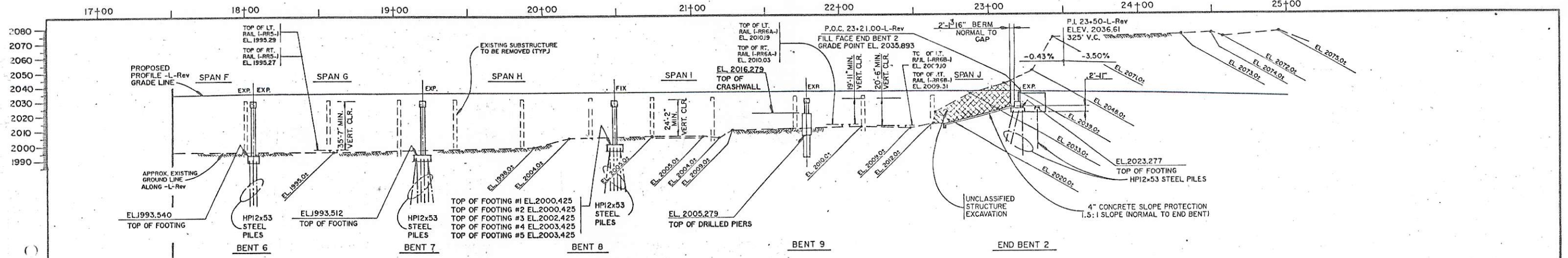
PART-PLAN

REVISED TOP OF PIER ELEVATIONS & BENT 3, PIERS 6 & 7. BY: FCJ

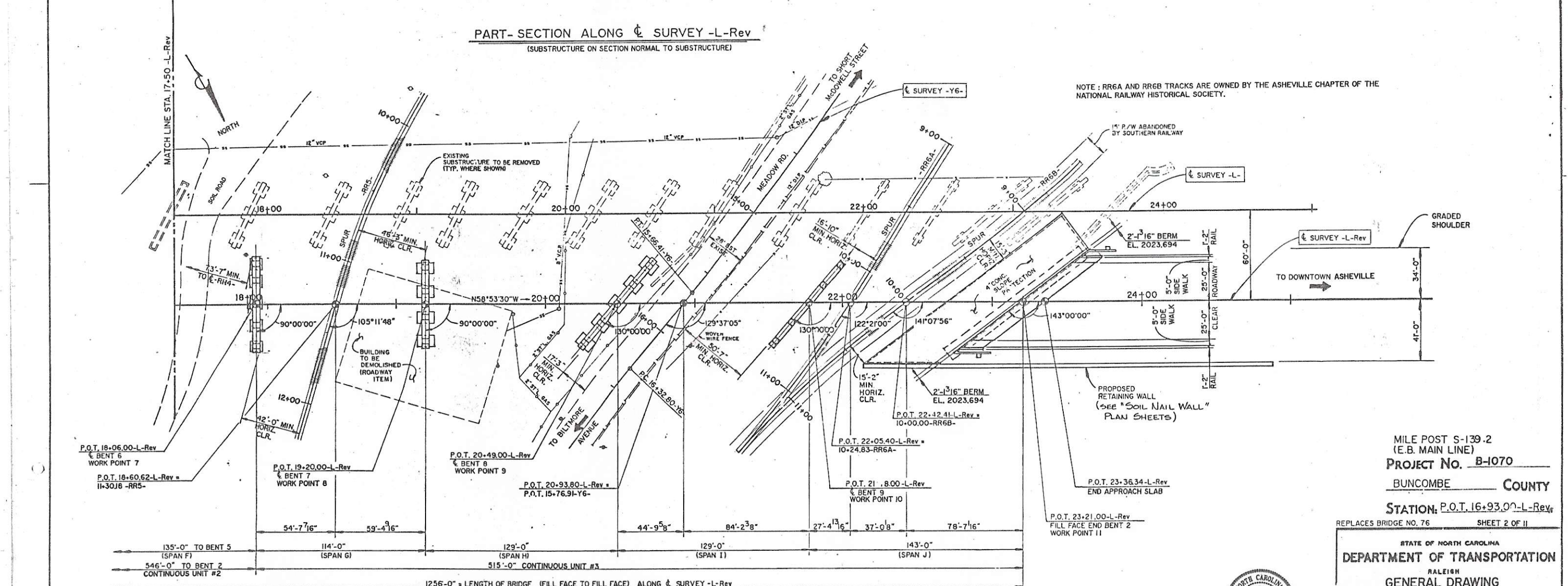
REVISED TOP OF PIER ELEVATIONS & BENTS 1, 4, 5. BY: CEP



PREPARED BY
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED



PART-SECTION ALONG & SURVEY -L-Rev
(SUBSTRUCTURE ON SECTION NORMAL TO SUBSTRUCTURE)



PART-PAN

NOTE: RR6A AND RR6B TRACKS ARE OWNED BY THE ASHEVILLE CHAPTER OF THE NATIONAL RAILWAY HISTORICAL SOCIETY.

MILE POST S-139.2
(E.B. MAIN LINE)
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev

REPLACES BRIDGE NO. 76 SHEET 2 OF 11

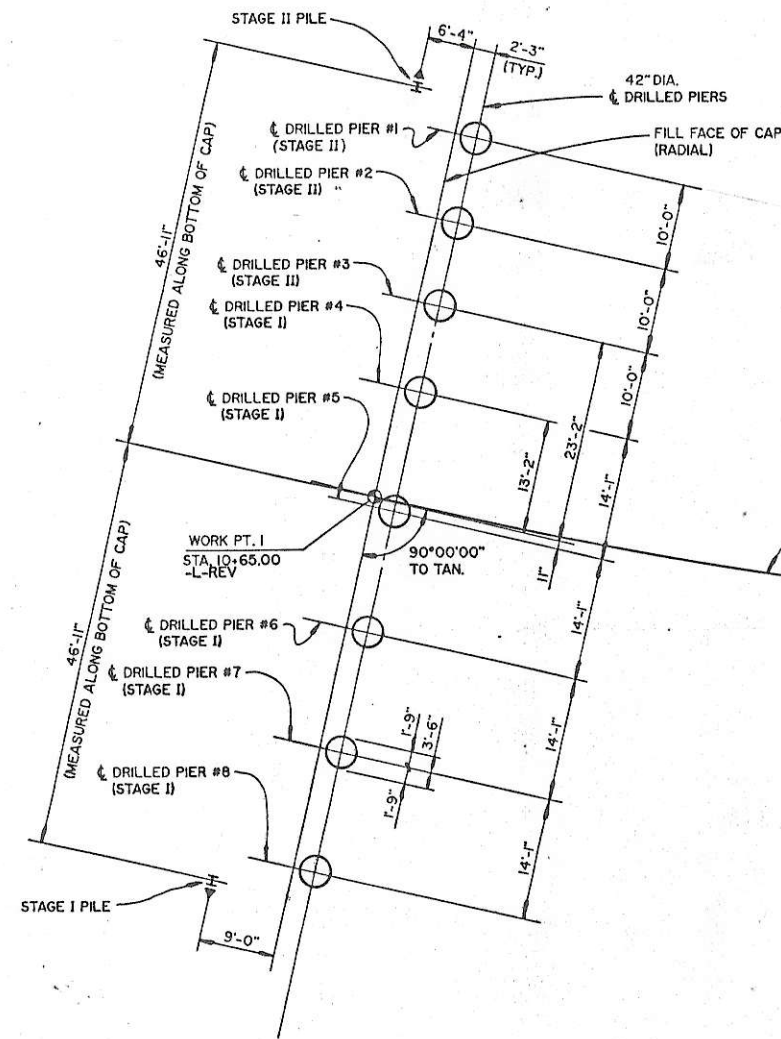
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOVA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JULY 1993



PREPARED BY
THOMPSON - GORDON - HICKS
ENGINEERS, INCORPORATED

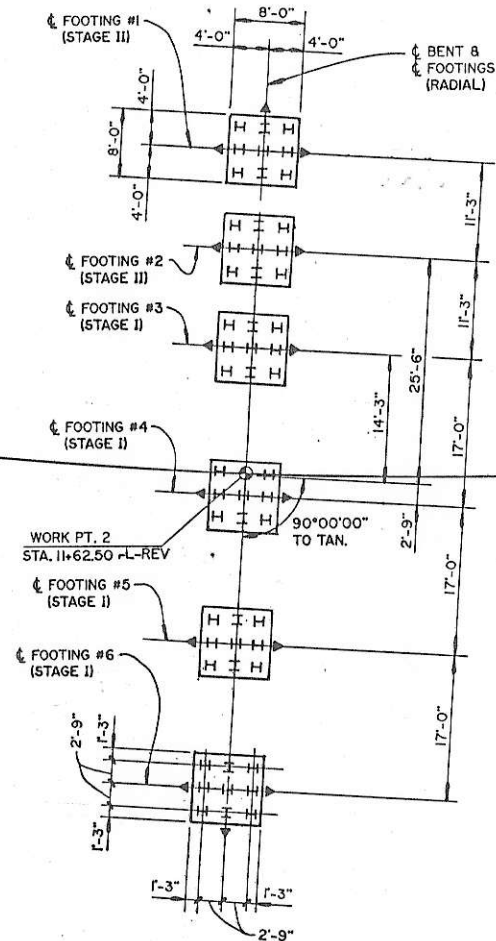
REVISIONS						SHEET NO. 5-2
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			

DRAWN BY RTJ DATE 6/11/93
CHECKED BY CCM DATE 7/1/93



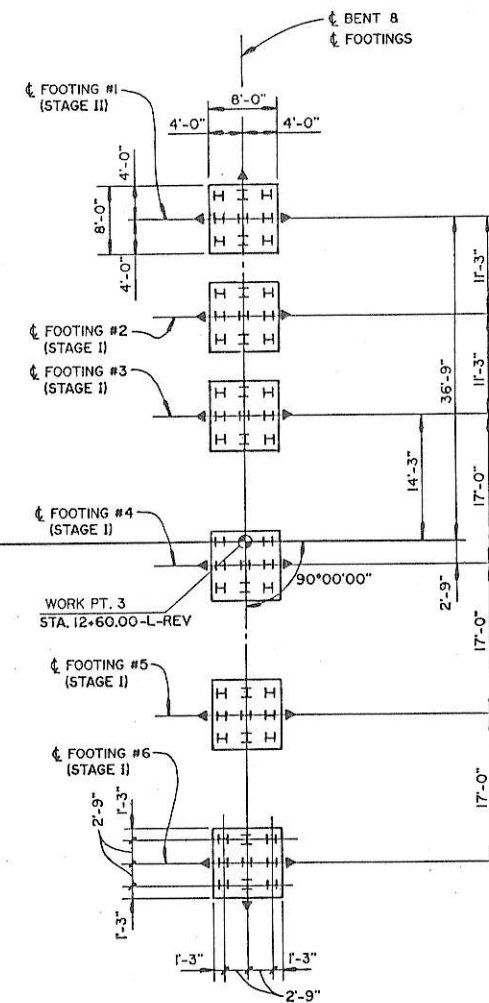
ABUTMENT I

▲ BATTER THESE PILES 3:12
IN DIRECTION SHOWN AT
ABUTMENT I.



BENT I

▲ BATTER THESE PILES 1 1/2:12
IN DIRECTION SHOWN AT
BENTS I & 2.



BENT 2

PART-FOUNDATION LAYOUT - STAGE I & STAGE II CONSTRUCTION

NOTES:
FOR DRILLED PIERS, SEE SPECIAL PROVISIONS.
DIMENSIONS LOCATING PILES ARE SHOWN TO THE
PILE CENTERLINE.
ALL PILES ARE HP 12x53
STEEL PILES.



PREPARED BY
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED

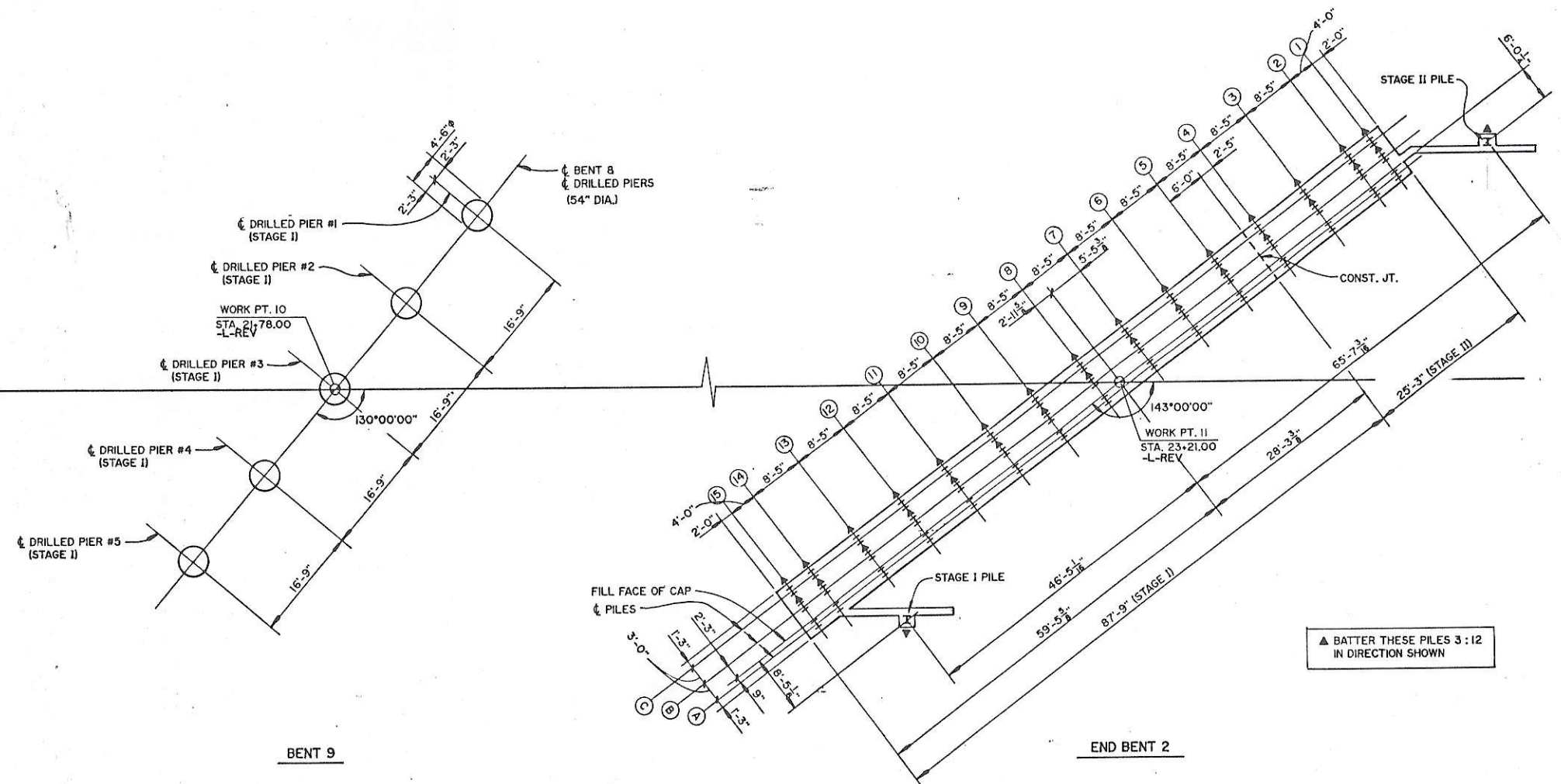
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOVA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JUNE 1995

REVISIONS						SHEET NO. 3 TOTAL SHEETS 224
NO.	BY	DATE	NO.	BY	DATE	
1			3			
2			4			

DRAWN BY: RTJ DATE: 6-3-95
CHECKED BY: ECM DATE: 7-9-95



PART-FOUNDATION LAYOUT - STAGE I & STAGE II CONSTRUCTION

NOTES:

FOR DRILLED PIERS, SEE SPECIAL PROVISIONS.

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE.

ALL PILES ARE HP 12x53 STEEL PILES.



PREPARED BY
THOMPSON-BORDON-SHOOK
ENGINEERS, INCORPORATED

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

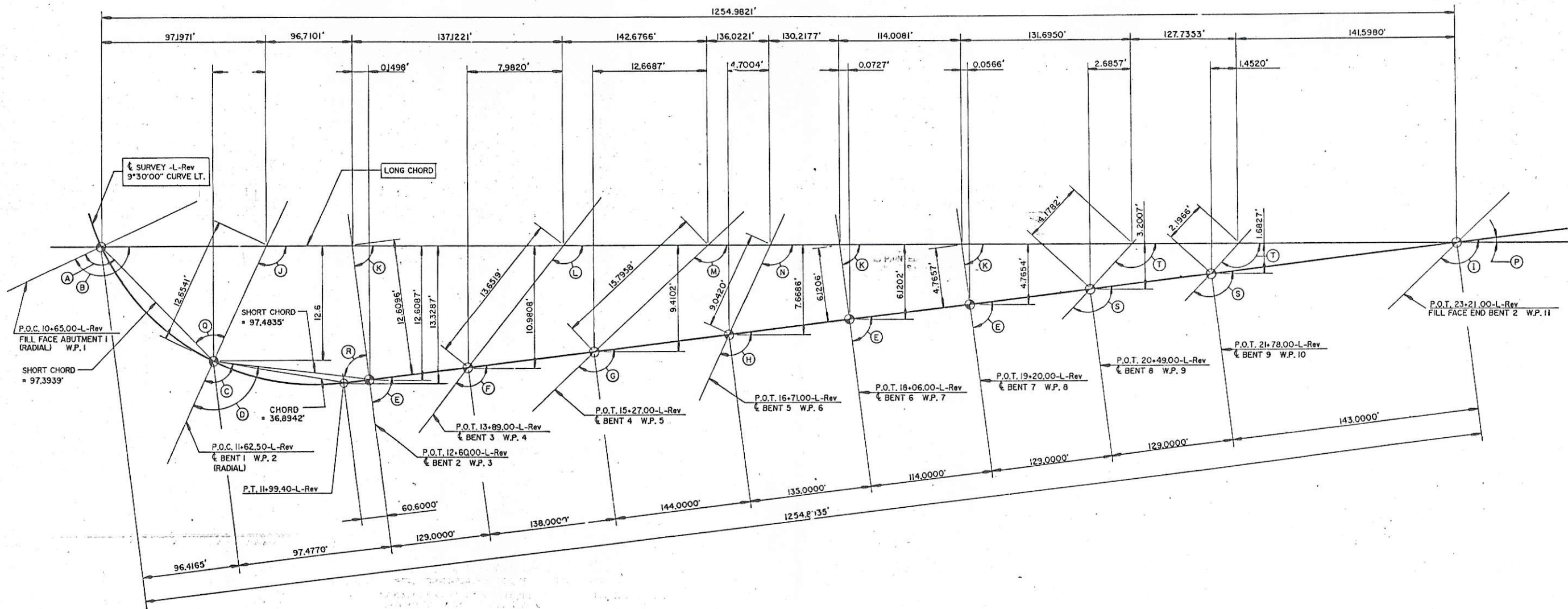
SHEET 7 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOVA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JUNE 1995

REVISIONS						SHEET NO. 5-7
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			

DRAWN BY: RTH DATE: 6-95

CHECKED BY: CCM DATE: 7/95



LONG CHORD LAYOUT

HORIZONTAL CURVE DATA -L-Rev	
P.I.	= 9+80.14-L-Rev
Δ	= 43°57'00\"L.T.
D	= 9°30'
L	= 462.63'
T	= 243.37'
R	= 603.11'
S.E.	= .03'/'

ANGLES	
(A)	94°37'53"
(B)	102°05'15"
(C)	91°45'10"
(D)	92°50'32"
(E)	90°00'00" TO TAN.
(F)	126°27'30" TO TAN.
(G)	144°00'00" TO TAN.
(H)	122°00'00" TO TAN.
(I)	143°00'00" TO TAN.
(J)	92°49'29"
(K)	89°19'09"
(L)	125°46'39"
(M)	143°19'09"
(N)	121°19'09"
(P)	00°40'51"
(Q)	85°22'07"
(R)	89°20'12"
(S)	130°00'00" TO TAN.
(T)	129°19'09"

RELATIVE COORDINATE SYSTEM		
WORK PT. NO.	NORTHS	EASTS
1	37381.096	9985.0867
2	3834.5230	9998.8715
P.T. OF CURVE	3871.40110	10000.0000
3	3932.0000	10000.0000
4	4061.0000	10000.0000
5	4199.0000	10000.0000
6	4343.0000	10000.0000
7	4478.0000	10000.0000
8	4592.0000	10000.0000
9	4721.0000	10000.0000
10	4850.0000	10000.0000
11	4993.0000	10000.0000

MILE POST S-139.2 (EASTBOUND MAINLINE)

PROJECT NO. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00 -L-Rev

REPLACES BRIDGE NO. 76 SHEET 8 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON I-25 (McDOWELL STREET)
OVER SWANNANOVA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JULY 1995



PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		

5-8
224

DRAWN BY: RTH DATE: AUGUST 1991
CHECKED BY: CCM DATE: 7/95

MINIMUM TIP ELEVATIONS FOR DRILLED PIERS		
LOCATION	DRILLED PIER NO.	MINIMUM TIP ELEVATION
ABUTMENT 1	1	1973.004
	2	1973.004
	3	1973.004
	4	1973.004
	5	1973.004
	6	1973.004
	7	1973.004
	8	1973.004
BENT 3	1	1964.976
	2	1964.976
	3	1964.976
	4	1964.976
	5	1964.976
	6	1963.976
BENT 4	1	1970.990
	2	1966.9.000
	3	1968.000
	4	1967.000
	5	1966.000
	6	1965.010
	7	1965.010
	8	1964.010
BENT 5	1	1962.171
	2	1962.171
	3	1961.171
	4	1961.171
	5	1961.171
	6	1961.171
BENT 9	1	1993.279
	2	1991.279
	3	1991.279
	4	1990.279
	5	1990.279

THESE MINIMUM TIP ELEVATIONS ARE PROVIDED FOR USE BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

GENERAL DRAWING NOTES

ASSUMED LIVE LOAD = HS 20-44 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES, FOR SEISMIC PERFORMANCE CATEGORY B.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET S-N.

TO PROVIDE PROTECTION FROM POSSIBLE SCOUR, THE FOOTINGS SHALL NOT BE CONSTRUCTED AT ELEVATIONS HIGHER THAN THOSE SHOWN IN THE PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", APRIL 1993.

THE SCOUR CRITICAL ELEVATION FOR BENT 1 IS 1982.00 FT.

THE SCOUR CRITICAL ELEVATION FOR BENT 2 IS 1981.00 FT.

THE SCOUR CRITICAL ELEVATIONS FOR BENT 3 IS 1970.00 FT.

STEEL CASINGS FOR THE DRILLED PIERS AT ABUTMENT 1, BENT 5 AND BENT 9 SHALL NOT BE LEFT IN PLACE.

PILE INTEGRITY TESTING (P.I.T.) WILL BE REQUIRED ON EACH DRILLED PIER. FOR PILE INTEGRITY TESTING, SEE SPECIAL PROVISIONS.

PILES FOR BENTS 1, 2, 6, 7 & 8 AND END BENT 2 SHALL BE DRIVEN TO A MINIMUM BEARING CAPACITY OF 50 TONS PER PILE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA AT END BENT 2 SHALL BE EXCAVATED FOR A DISTANCE OF THIRTY-FOUR FEET LEFT AND FORTY-ONE FEET RIGHT OF THE CENTERLINE OF SURVEY -L-REV. THIS WORK WILL BE MEASURED AND PAID FOR AS "UNCLASSIFIED STRUCTURE EXCAVATION".

FOR MAINTENANCE AND PROTECTION OF HIGHWAY TRAFFIC BENEATH THE PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

FOR FABRICATED METAL STAY-IN-PLACE FORMS, SEE SPECIAL PROVISIONS.

REQUIRED TIP BEARING FOR ALL DRILLED PIERS IS 25 TSF.

WORK ON THE APPROACH SLABS SHALL NOT BE STARTED UNTIL THE APPROACH FILL HAS BEEN IN PLACE FOR ONE MONTH.

DRILLED PIERS, CAPS AND BACKWALL AT ABUTMENT 1 SHALL BE CONSTRUCTED PRIOR TO PLACING THE REQUIRED 11 FEET OF EMBANKMENT FILL.

THE 42 INCH AND 54 INCH DRILLED PIERS SHALL HAVE A MINIMUM PENETRATION OF THREE FEET INTO HARD ROCK. HARD ROCK SHALL BE DEFINED AS ROCK WHICH CANNOT BE DRILLED WITH CONVENTIONAL EARTH AUGERS AND/OR UNDERREAMING TOOLS, BUT INSTEAD REQUIRES THE USE OF SPECIAL ROCK AUGERS, CORE BARRELS AND/OR BLASTING. THE DRILLED PIERS SHALL HAVE MINIMUM DIAMETERS OF 40 INCHES AND 52 INCHES RESPECTIVELY, IN THE HARD ROCK ZONE.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE LOCATION OF THE U.S. SPRINT FIBER OPTIC CABLES IN THE VICINITY OF BENT 5.

FOR FALSEWORK AND FORMS OVER OR ADJACENT TO TRAFFIC, SEE SPECIAL PROVISIONS.

ALL STRUCTURAL STEEL FOR THIS STRUCTURE SHALL BE AASHTO M270, GRADE 50W, AND PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED ON THE PLANS.

THE SUBSTRUCTURE OF THE EXISTING STRUCTURE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION AND THOMPSON-GORDON-SHOOK ENGINEERS, INCORPORATED FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE STRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF A 27-SPAN (OF VARYING LENGTHS) REINFORCED CONCRETE SUPERSTRUCTURE WITH A 5'-0" SIDEWALK ON EACH SIDE, A 40'-0" CLEAR ROADWAY WIDTH FLARING TO A 50'-0" CLEAR ROADWAY WIDTH TO PROVIDE LEFT TURNING LANES AT THE SOUTH END FOR AN OVERALL LENGTH OF 1,390 FEET, SUPPORTED BY REINFORCED CONCRETE SUBSTRUCTURE AND LOCATED AT THE SITE OF THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

ALL CONTINUOUS GIRDERS SPAN UNITS ARE COMPOSITE.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE THIRTY INCH SAMPLE OF EACH BAR SIZE USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO THIRTY INCH SAMPLES OF EACH BAR SIZE USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

HP 12x53 STEEL PILES AT END BENT 2 SHALL BE EMBEDDED TO THE MINIMUM TIP ELEVATION OF EL.2011.00, WITH 50 TONS BEARING. EIGHTEEN (18) INCH DIAMETER SHAFT EXCAVATION WILL BE REQUIRED TO ACHIEVE THIS. SEE THE SPECIAL PROVISION ENTITLED "SHAFT EXCAVATION FOR STEEL PILES" FOR DETAILS.

THE RETAINING WALL AT STA.23+00 ±, 41 FT. RIGHT OF -L-REV NEEDS TO BE CONSTRUCTED PRIOR TO CONSTRUCTING END BENT 2, IN ORDER TO FACILITATE THE REQUIRED FIFTY (50) FEET OF EXCAVATION.

INASMUCH AS THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR REMOVAL OF EXISTING STRUCTURE AT STATION 16+93.00-L-REV.

FOR REINFORCED CONCRETE DECK SLAB, SEE SPECIAL PROVISIONS.

ALL PAVEMENT MARKING WILL BE IN ACCORDANCE WITH THE PAVEMENT MARKING PLANS AND PROVIDE FOR BICYCLES.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO SPECIFICATIONS.

FOR POT BEARINGS, SEE SPECIAL PROVISIONS.

PLAIN RIP RAP SHALL BE PLACED ON THE EXISTING BANKS AT THE EXISTING SUBSTRUCTURE UNITS IMMEDIATELY AFTER REMOVAL OF THESE SUBSTRUCTURE UNITS.

PLAIN RIP RAP SHALL BE PLACED ON THE EXISTING SLOPE AT BENT 3 IMMEDIATELY AFTER BACKFILLING EXCAVATION OF BENT 3.

FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

FOR MODULAR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

FOR CURING BRIDGE DECK SLABS, SEE THE SPECIAL PROVISION ENTITLED "REINFORCED CONCRETE DECK SLAB".

FOR MECHANICAL BUTT SPLICING OF REINFORCING STEEL, SEE SPECIAL PROVISIONS.

ELECTRICAL CONDUIT SYSTEM SHALL BE PROVIDED BY THE CONTRACTOR. SEE SPECIAL PROVISIONS AND DETAILS ENTITLED "ELECTRICAL CONDUIT SYSTEM". TRAFFIC SIGNAL CIRCUITRY SHALL BE PROVIDED BY OTHERS.

FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

FOR "INSTALL PLASTIC TELEPHONE CONDUITS", SEE SPECIAL PROVISIONS.

FOR SELECT BACKFILL MATERIAL, SEE SPECIAL PROVISIONS.

MILE POST S-139.2
(E.B. MAIN LINE)

PROJECT NO. B-1070
BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

REPLACES BRIDGE NO. 76 SHEET 9 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JULY 1995

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	TOTAL SHEETS
1	AR	3/7/97	2			5-9
2			4			224



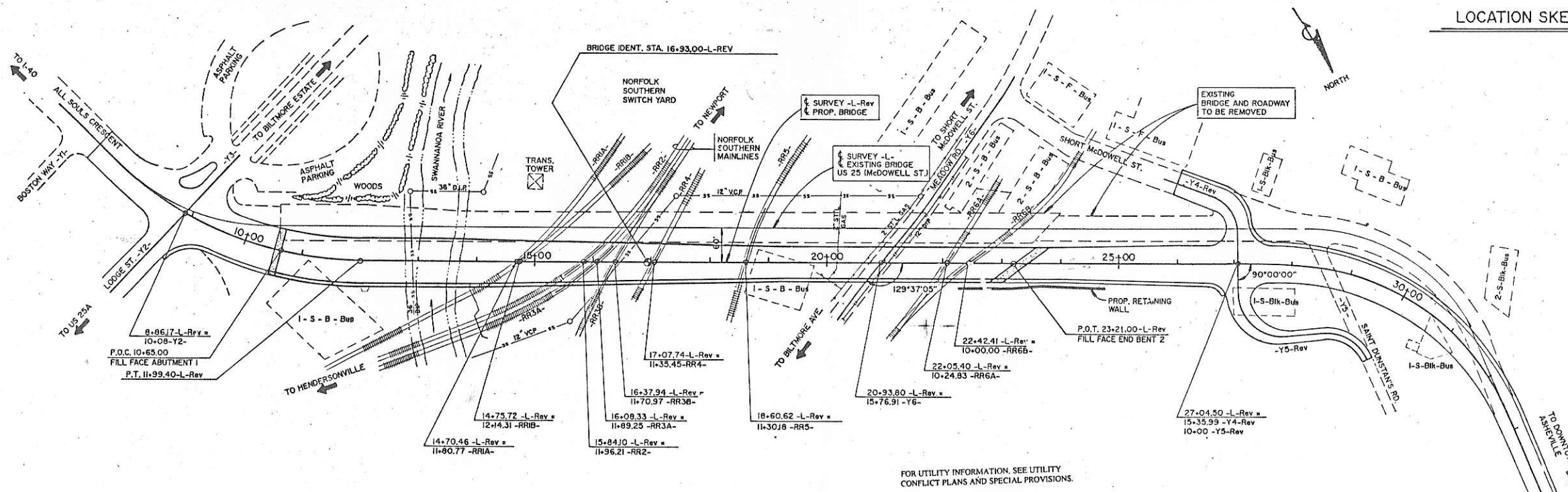
PREPARED BY
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

▶ REVISED MINIMUM TIP ELEVATIONS FOR BENTS 4 & 5. BY: CEP

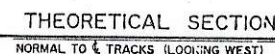
SCM DATE 7/95
N.M.W. DATE 7/95

#1 BRONZE DISK IN TOP OF WEST
ABUTMENT WALL OF RR BRIDGE,
LOCATED 17' SOUTH OF SOUTH RR
RAIL. STAMPED E-39, 1932 EL. = 1994.159
CORRECTED EL. 1993.956

#5 CHISELED SQUARE IN TOP RAIL OF EXISTING
BRIDGE ON EAST SIDE LOCATED 26' EAST
OF 16+32-L- EL. 2027.396



FOR UTILITY INFORMATION, SEE UTILITY
CONFLICT PLANS AND SPECIAL PROVISIONS



TOP OF RAIL ELEVATIONS		
--RR6A- STATIONS	LEFT RAIL	RIGHT RAIL
9+00	2009.08	2009.02
9+50	2009.10	2009.73
10+00	2010.25	2010.16
10+50	2009.91	2009.85
11+00	2009.87	2009.85

TOP OF RAIL ELEVATIONS		
STATIONS	LEFT RAIL	RIGHT RAIL
9+00	2008.41	2008.5
9+50	2008.88	2009.0
10+00	2009.10	2009.3
10+50	2009.29	2009.6
11+00	2009.89	2009.8

MILE POST S-139.2 (EASTBOUND
 MAINLINE)

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev

REPLACES BRIDGE NO. 76 SHEET 10 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE

REV. 11-10-85						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	TOTAL SHEETS 224
1			3			
2			4			

DRAWN BY RTJ DATE Aug 1991
CHECKED BY ECM DATE 7/95

— PREPARED BY —
THOMPSON • GORDON • SHOOTER
ENGINEERS, INCORPORATED

TOTAL BILL OF MATERIAL

ITEM	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION	3'-6" DIA. DRILLED PIERS IN ROCK	3'-6" DIA. DRILLED PIERS NOT IN ROCK	4'-6" DIA. DRILLED PIERS IN ROCK	4'-6" DIA. DRILLED PIERS NOT IN ROCK	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	REINFORCING STEEL	BRIDGE APPROACH SLABS	SPIRAL COLUMN REINFORCING STEEL	STRUCTURAL STEEL	
UNIT	LUMP SUM	CUBIC YARDS	LINEAR FEET	LINEAR FEET	LINEAR FEET	LINEAR FEET	CUBIC YARDS	SQUARE FEET	SQUARE FEET	CUBIC YARDS	LBS	LUMP SUM	LBS	APPROX. LBS	
CONSTRUCTION STAGE	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II
SUPERSTRUCTURE															
CONTINUOUS UNIT #1								11,377	4,878	381	10,047	3,597	380		314,500
CONTINUOUS UNIT #2								34,105	7,828	1,092	30,443	4,179	1,089		1,170,500
CONTINUOUS UNIT #3								24,522	6,072	1,078	21,940	2,395	1,074		921,000
ABUTMENT I				24.0	14.4	56.0	33.6							1,834	1,100
BENT 1		160	80											1,055	528
BENT 2		150	30											2,536	507
BENT 3						52.5	7.2	58.0	10.8					7,985	1,353
BENT 4						52.5	8.8	134.5	13.2					9,420	1,408
BENT 5						23.9	4.7	135.2	26.4					8,820	1,741
BENT 6		100	25											3,724	933
BENT 7		120	0											3,774	0
BENT 8		180	0											3,333	0
BENT 9		100	0			39.0	0	31.0	0					3,163	0
END BENT 2		880	275							1,840	520				
APPROACH SLABS												LS	LS		
SUB-TOTALS	LS	1,690	410	24.0	14.4	56.0	33.6	167.9	20.7	358.7	50.4	1,840	520	70,004	18,778
TOTALS	LS	2,100		38.4		89.6		188.6		409.1		2,360		91,333	

ITEM	HP 12 x 53 STEEL PILES	CLASSIC BRIDGE RAILING	4" SLOPE PROTECTION	POT BEARINGS	FILTER FABRIC FOR DRAINAGE	PLAIN RIP RAP CLASS II (2'-0" THICK)	EXPANSION JOINT SEALS	MODULAR EXPANSION JOINT SEALS	ELECTRICAL CONDUIT SYSTEM	SHAFT EXCAVATION FOR STEEL PILES	SELECT BACKFILL MATERIAL, CLASS I	INSTALL PLASTIC TELEPHONE CONDUITS	
UNIT	NUMBER	LF	NUMBER	LF	LINEAR FEET	SQUARE YARDS	LUMP SUM	SQUARE YARDS	TONS	LUMP SUM	LUMP SUM	LUMP SUM	LINEAR FEET
CONSTRUCTION STAGE	STAGE I	STAGE I	STAGE II	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I	STAGE II	STAGE I
SUPERSTRUCTURE													
CONTINUOUS UNIT #1					259.69	233.53	LS	LS		LS	LS	LS	LS
CONTINUOUS UNIT #2					546.10	546.10	LS	LS		LS	LS	LS	LS
CONTINUOUS UNIT #3					490.55	570.18	LS	LS		LS	LS	LS	LS
ABUTMENT I	1	17	1	17					119	84	107	76	
BENT 1	36	360	18	180									
BENT 2	45	450	9	90									
BENT 3									76	306	69	275	
BENT 4													
BENT 5													
BENT 6	36	900	9	225									
BENT 7	48	960	0	0									
BENT 8	60	960	0	0									
BENT 9													
END BENT 2	34	215	13	130			485	120					374
APPROACH SLABS													143
SUB-TOTALS	260	3,862	50	642	1,296.34	1,349.81	485	120	LS	LS	195	390	176
TOTALS									527				351

DRAWN BY: CCM DATE: 7/15
CHECKED BY: N.M.W. DATE: 7-15

REVISD BENT 3, STAGE I, AND TOTAL QUANTITIES FOR DRILLED PIER NOT IN ROCK, CLASS A CONCRETE, REINFORCING STEEL, AND SPIRAL COLUMN REINFORCING STEEL. ✓ BY: FCJ



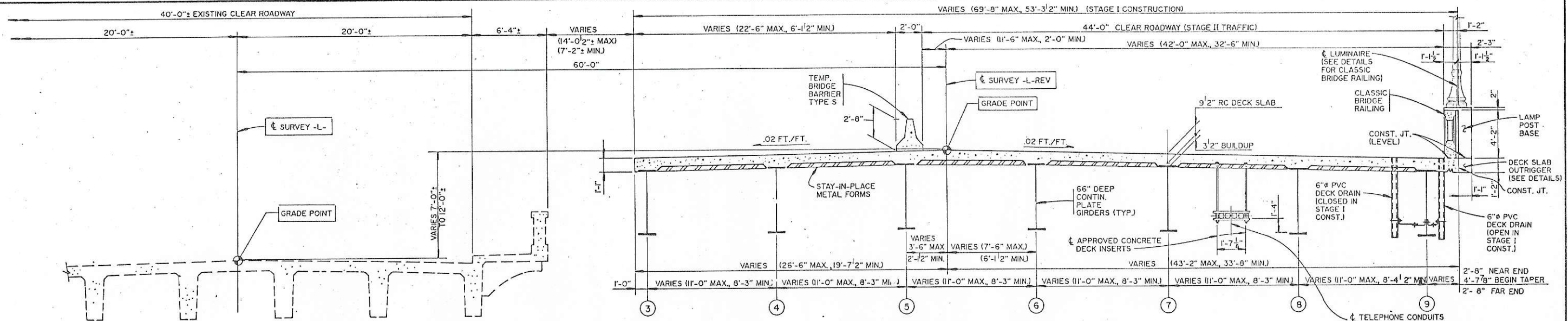
PREPARED BY: THOMPSON - GORDON - SHOOK ENGINEERS, INCORPORATED

MILE POST S-139.2 (E.B. MAIN LINE)

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev
REPLACES BRIDGE NO. 76 SHEET II OF II

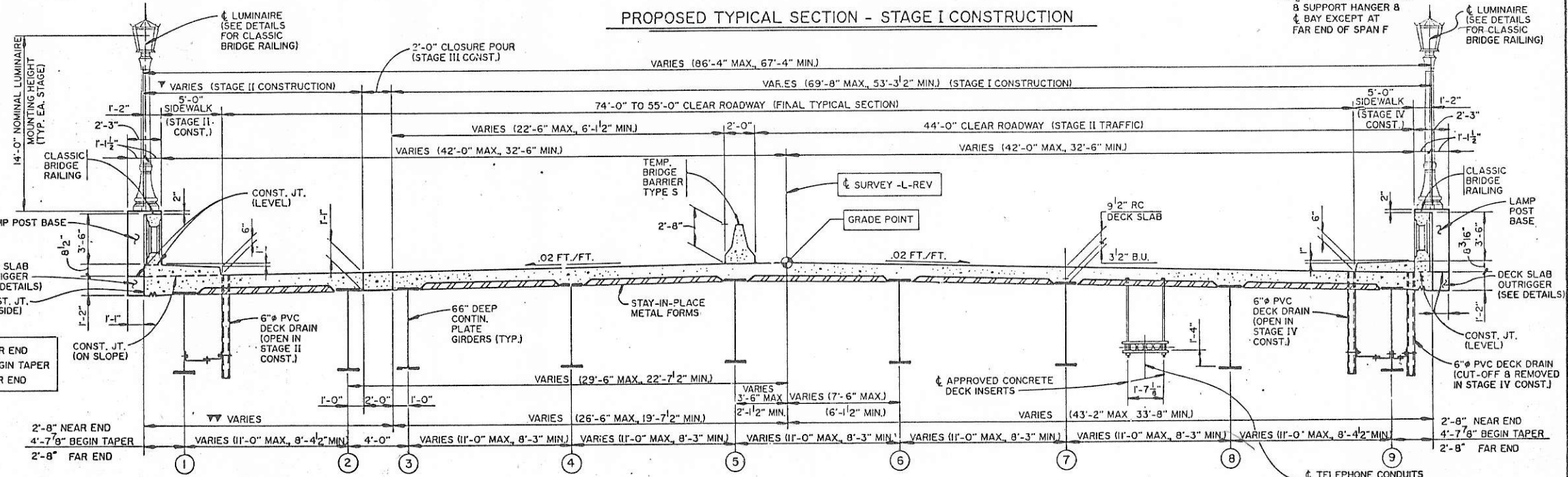
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON US 25 (McDOWELL STREET)
OVER SWANNANOVA RIVER, NORFOLK SOUTHERN RR,
AND SR 3556 (MEADOW RD.)
BETWEEN I-40 AND DOWNTOWN ASHEVILLE
IN THE CITY OF ASHEVILLE
JULY 1995

REVISIONS						SHEET NO. 5-11 TOTAL SHEETS 224
NO.	BY	DATE	NO.	BY	DATE	
1	ROB	2/6/97	3			
2			4			



EXISTING TYPICAL SECTION

▽ 14'-8" NEAR END
16'-1 1/16" BEGIN TAPER
12'-0 1/2" FAR END



PROPOSED TYPICAL SECTION STAGES II & III CONSTRUCTION

PROPOSED TYPICAL SECTION STAGES I & IV CONSTRUCTION

UTILITY NOTES:
THE DIMENSIONS AND DETAILS SHOWN FOR CONCRETE INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM AND ARE NOT TO BE CONSTRUED TO BE AN APPROVAL FOR THE ATTACHMENT OF THE UTILITY TO THE SUPERSTRUCTURE.
FOR RESPONSIBILITIES FOR FURNISHING AND PLACING STEEL PLATE SLEEVE, SEE UTILITY SPECIAL PROVISIONS.

CONTINUOUS UNIT #2
TYPICAL SECTIONS - SPANS C, D, E & F
(ONE CONTINUOUS UNIT OF 4 SPANS - 1 @ 129'-0", 1 @ 138'-0", 1 @ 144'-0" & 1 @ 135'-0")

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev

SHEET 2 OF 3

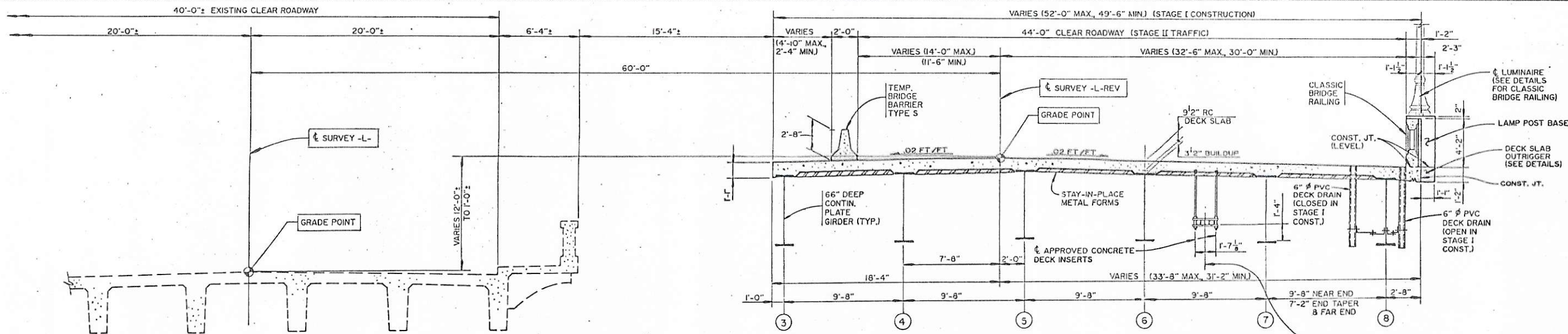
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
CONSTRUCTION SEQUENCE FOR BRIDGE ON US 25 (McDOWELL STREET) OVER SWANNANOVA RIVER, HIOFOLK SOUTHERN RR, AND SR 3556 (MADDOX RD.) BETWEEN I-40 AND DOWNTOWN ASHEVILLE IN THE CITY OF ASHEVILLE JULY 1995					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

SHEET NO. 5-13
TOT. SHEETS 274



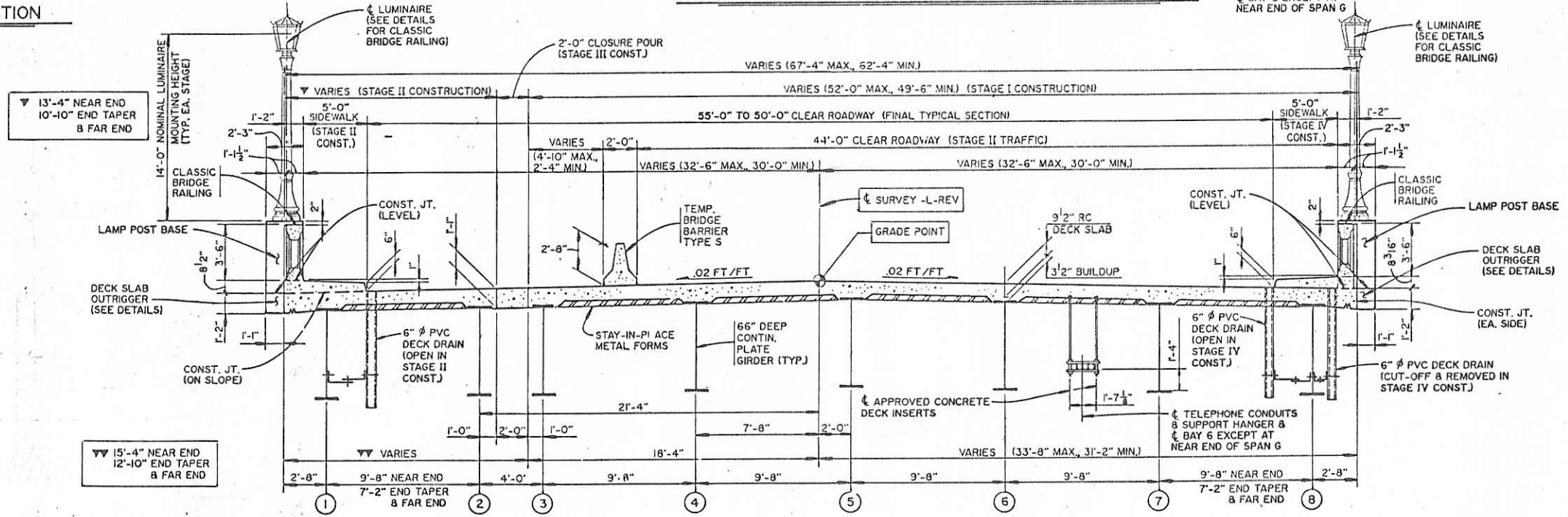
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

DRAWN BY: RTH
CHECKED BY: ECM
DATE: 7/95



EXISTING TYPICAL SECTION

PROPOSED TYPICAL SECTION - STAGE I CONSTRUCTION



PROPOSED TYPICAL SECTION STAGES II & III CONSTRUCTION

PROPOSED TYPICAL SECTION STAGES I & IV CONSTRUCTION

UTILITY NOTES:
 THE DIMENSIONS AND DETAILS SHOWN FOR THE CONCRETE INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM AND ARE NOT TO BE CONSTRUED TO BE AN APPROVAL FOR THE ATTACHMENT OF THE UTILITY TO THE SUPERSTRUCTURE.
 FOR RESPONSIBILITIES FOR FURNISHING AND PLACING STEEL PLATE SLEEVE, SEE UTILITY SPECIAL PROVISIONS.

CONTINUOUS UNIT #3
 TYPICAL SECTIONS - SPANS G, H, I & J
 (ONE CONTINUOUS UNIT OF 4 SPANS - 1 @ 114'-0", 2 @ 129'-0", 1 @ 143'-0")

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev

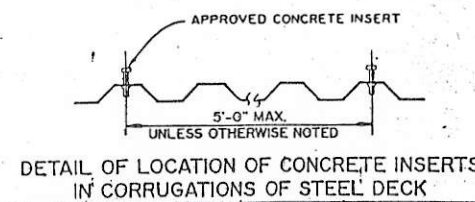
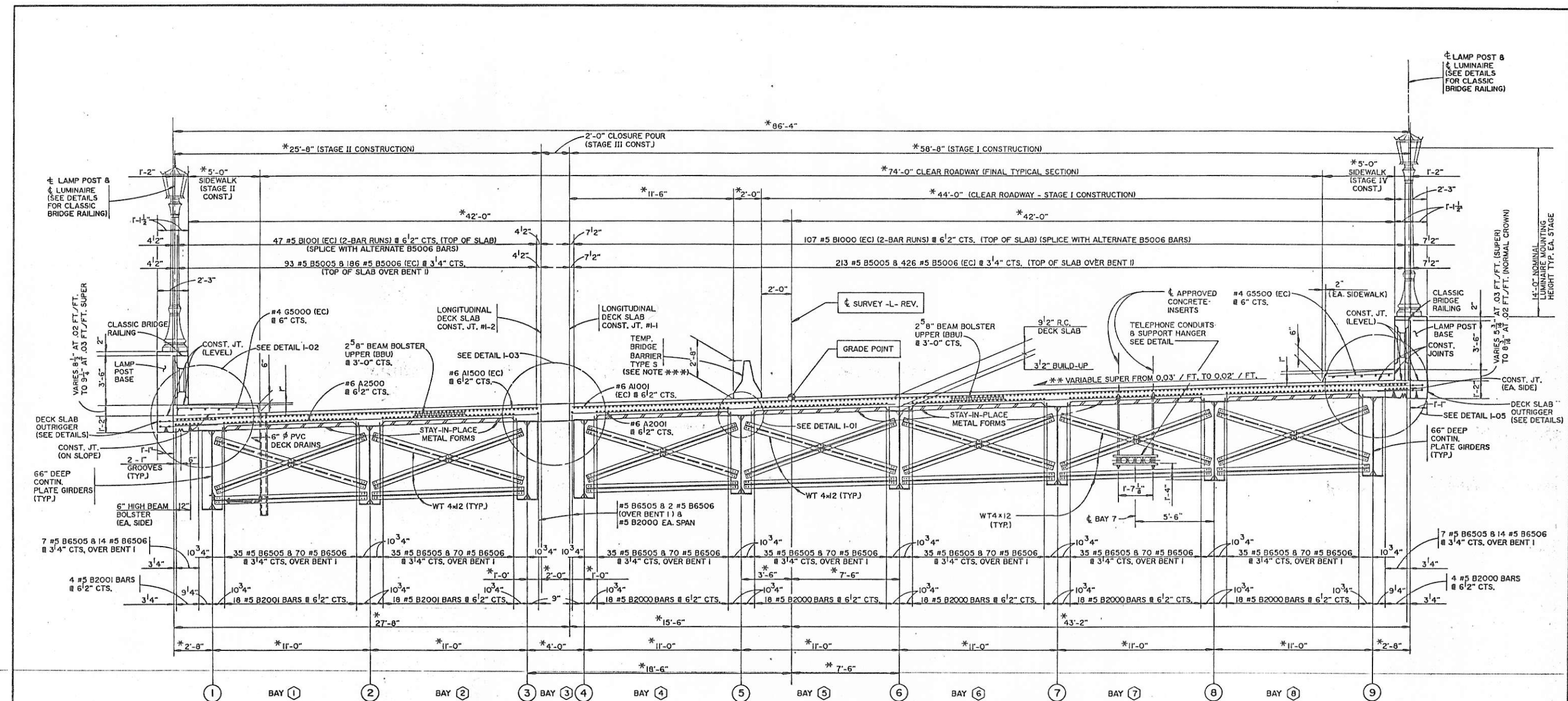
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
CONSTRUCTION SEQUENCE FOR BRIDGE ON US 25 (McDOWELL STREET) OVER SWANNANOA RIVER, NORFOLK SOUTHERN RR, AND SR 3556 (MEADOW RD.) BETWEEN I-40 AND DOWNTOWN ASHEVILLE IN THE CITY OF ASHEVILLE JULY 1995					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

PREPARED BY
 THOMPSON-GORDON-SHOOK
 ENGINEERS, INCORPORATED

4/12/96

SHEET NO. 5-14
 TOTAL SHEETS 224



INSERTS MUST BE LOCATED IN HIGH PART OF "STAY-IN-PLACE" FORMS. EXACT LOCATION AS SHOWN IN INSERT SPACING PLAN MAY VARY SLIGHTLY IN ORDER TO AVOID LOCATING INSERTS ON SLOPING OR LOW SURFACE OF FORMS AND TO AVOID INTERMEDIATE CROSSFRAMES. IF POSSIBLE, DO NOT EXCEED 5'-0" SPACING.

TYPICAL SECTION AT INTERMEDIATE DIAPHRAGMS SPANS A AND B

NOTES: THE DIMENSIONS AND DETAILS SHOWN FOR CONCRETE INSERTS ARE FOR THE CONTRACTORS BENEFIT IN PLACING THE INSERTS AND ARE NOT TO BE CONSTRUED AS AN APPROVAL FOR THE ATTACHMENT OF A UTILITY LINE.

FOR RESPONSIBILITIES FOR FURNISHING AND PLACING CONCRETE INSERTS, SEE UTILITY SPECIAL PROVISIONS.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 2



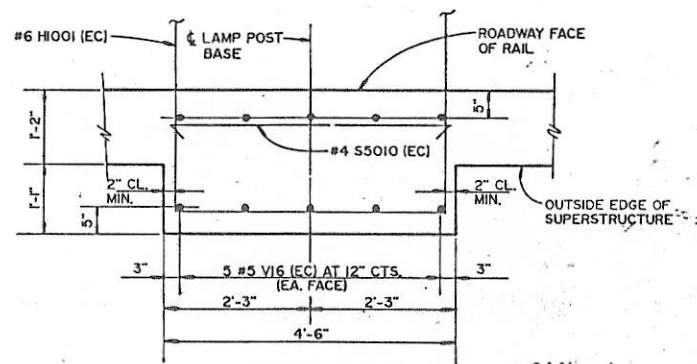
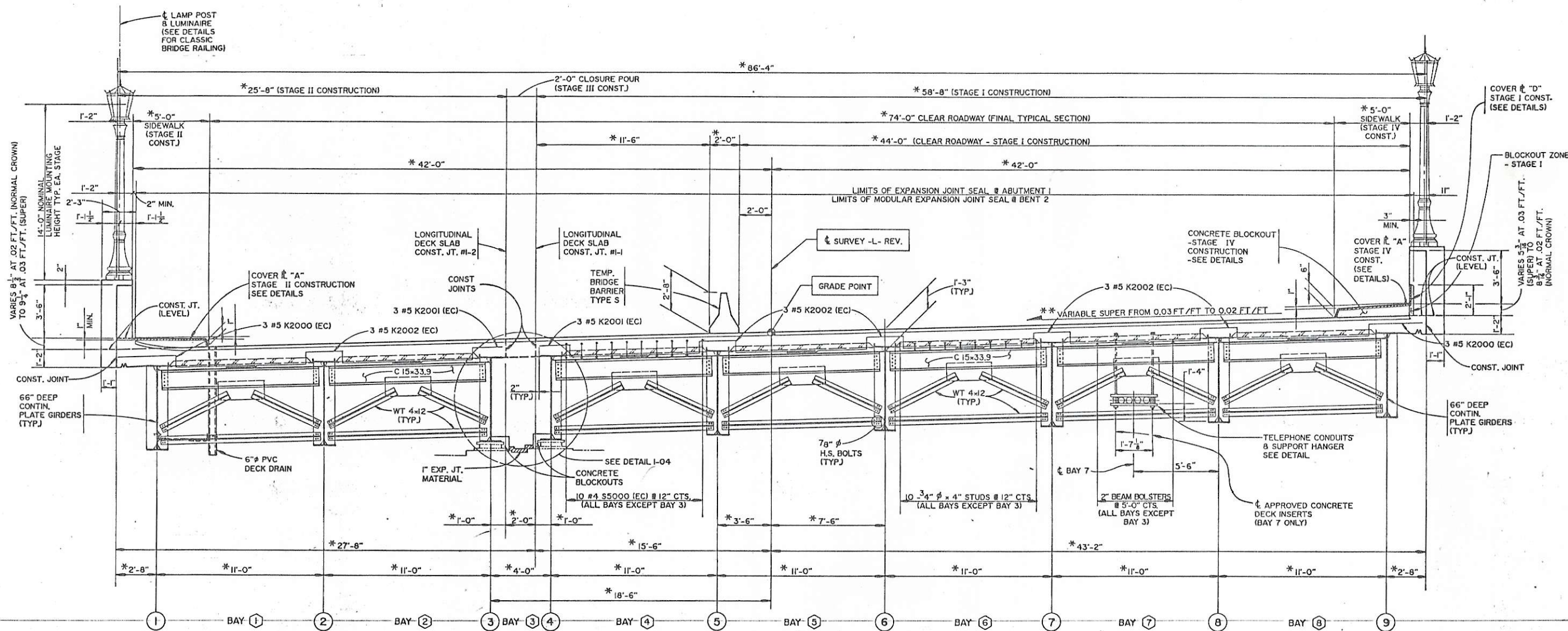
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION				
RAILROAD SUPERSTRUCTURE TYPICAL SECTIONS CONTINUOUS UNIT #1 (SPANS A AND B) SEPT. 1994				
REVISIONS				
NO.	BY	DATE	NO.	DATE
1			2	
3			4	

DRAWN BY: N.M. WILSON
CHECKED BY: G.M.
DATE: SEPT. 94
DATE: 12/95

* RADIAL DIMENSIONS, MEASURED THRU THE WORK POINTS FROM P.O.C. 10+65.00-L-REV TO P.T. 11+99.40-L-REV TO P.O.T. 12+60.00-L-REV ARE MEASURED ALONG TANGENT PORTION OF THE FIRST CONTINUOUS UNIT.	** SEE DETAIL I-07 SHOWING VARIABLE SUPERELEVATION INFORMATION.	(EC) DENOTES EPOXY - COATED REINFORCING STEEL.	O = OPEN CIRCLE = ADDITIONAL #5 B5005, B5006, B6505 & B6506 BARS OVER BENT 1.
			● = CLOSED CIRCLE = MAIN #5 B1000 & B2000 BARS (STAGE I) & MAIN #5 B1001 & B2001 BARS (STAGE II)

*** TEMP. BRIDGE BARRIER, TYPE S SHALL BE RELOCATED TO FACILITATE STAGE IV SIDEWALK CONSTRUCTION. SEE TRAFFIC CONTROL PLANS.

PREPARED BY:
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED



DETAIL I-08
PLAN OF DECK SLAB OUTRIGGER
AT LAMP POST BASE
SEE SECTION G-6 - CLASSIC BRIDGE RAILING DETAILS,
SHEET 15 OF 17, FOR ADDITIONAL DETAILS.

* RADIAL DIMENSIONS, MEASURED THRU THE WORK POINTS FROM P.O.C. 10+63.00-L-REV TO P.T. 11+99.40-L-REV. DIMENSIONS FROM P.T. 11+99.40-L-REV TO P.O.T. 12+60.00-L-REV ARE MEASURED ALONG TANGENT PORTION OF THE FIRST CONTINUOUS UNIT.	** SEE DETAIL I-07 SHOWING VARIABLE SUPERELEVATION INFORMATION.	(EC) DENOTES EPOXY - COATED REINFORCING STEEL.
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NOTES FOR DECK SLAB - CONTINUOUS UNIT #1

THE DIMENSIONS AND DETAILS SHOWN FOR THE CONCRETE DECK INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM AND SHOULD NOT BE CONSTRUED TO BE AN APPROVAL FOR THE ATTACHMENT OF THE UTILITY TO THE STRUCTURE.

FOR RESPONSIBILITIES FOR FURNISHING AND PLACING THE CONCRETE DECK INSERTS, SEE UTILITY SPECIAL PROVISIONS.

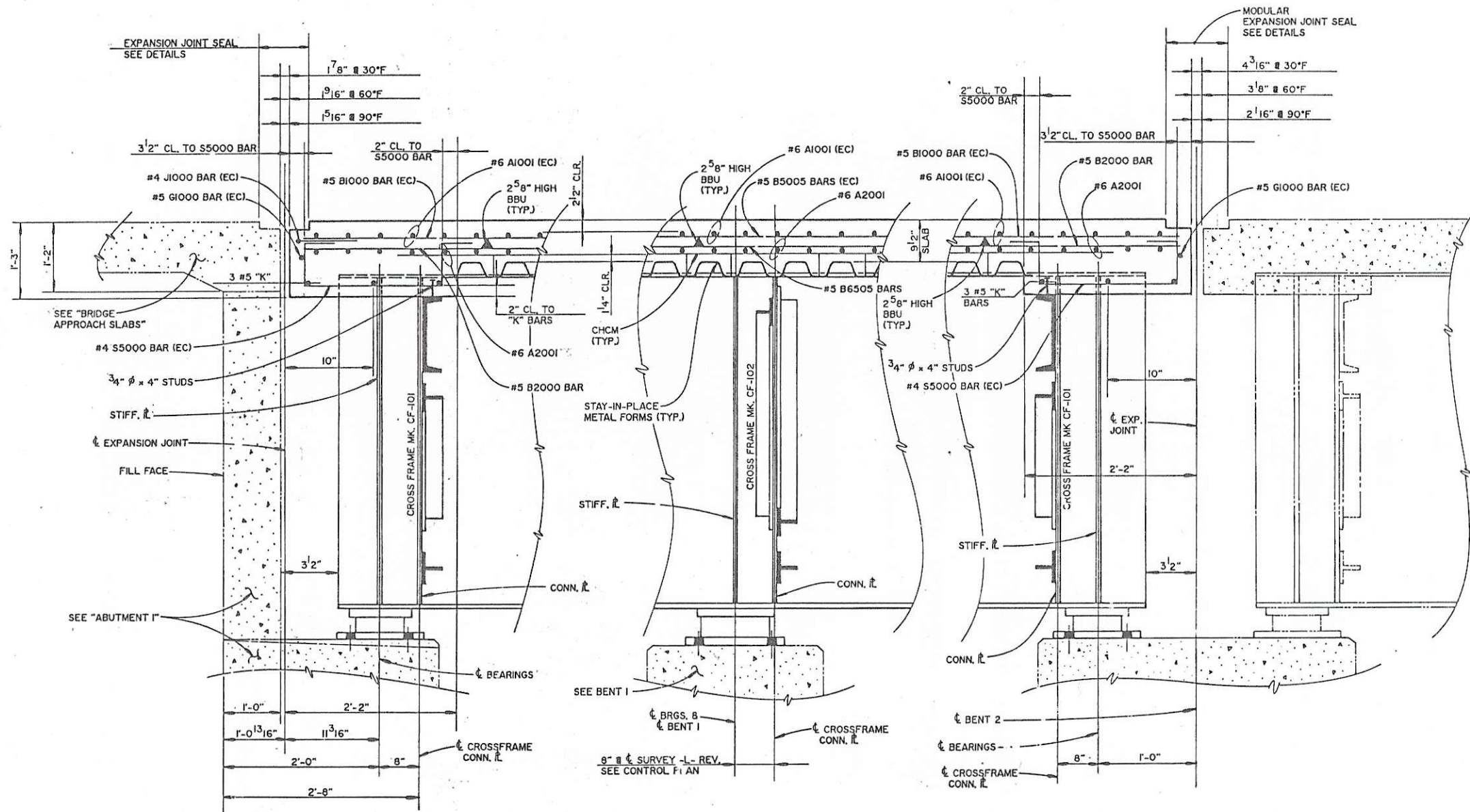
FOR CLASSIC BRIDGE RAILS, SEE SPECIAL PROVISIONS.



PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV.

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE TYPICAL SECTIONS CONTINUOUS UNIT #1 (SPANS A AND B) SEPT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
SHEET NO. 5-16					TOTAL SHEETS 224



NOTES FOR DECK SLAB - CONTINUOUS UNIT #1

PROVIDE CONTINUOUS HIGH CHAIR FOR METAL DECK (CHCM) @ 4'-0" CTS. WITH LEG SPACING TO MATCH THE PITCH OF THE FORMS AND WITH A HEIGHT TO SUPPORT THE BOTTOM LAYER OF SLAB REINFORCEMENT A CLEAR DISTANCE OF 1-1/4" ABOVE THE TOP OF THE STAY-IN-PLACE FORM.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE TRANSVERSE JOINTS IN THE DECK SLAB (WHERE APPLICABLE) ARE CONTINUOUS IN A STRAIGHT LINE THROUGH THE CLASSIC BRIDGE RAIL. THE CONTRACTOR'S ATTENTION IS ALSO CALLED TO THE FACT THAT THE LONGITUDINAL JOINTS IN THE DECK SLAB ARE CONTINUOUS IN A STRAIGHT LINE FROM BEGINNING OF CONTINUOUS UNIT TO ENDING OF CONTINUOUS UNIT.

THE EXPANSION JOINT SEAL AT ABUTMENT 1 SHALL BE CAPABLE OF HANDLING A TOTAL THERMAL MOVEMENT, MEASURED PARALLEL TO THE CENTERLINE OF ROADWAY, OF 1-1/8" (9/16" EXPANSION AND 9/16" CONTRACTION FROM A MIDPOINT TEMPERATURE OF 60° F).

THE MODULAR EXPANSION JOINT SEAL AT BENT 2 SHALL BE CAPABLE OF HANDLING A TOTAL THERMAL MOVEMENT, MEASURED PARALLEL TO THE CENTERLINE OF ROADWAY, OF 4-1/4" (2-1/8" EXPANSION AND 2-1/8" CONTRACTION FROM A MIDPOINT TEMPERATURE OF 60° F).

THE TOP OF EXPANSION JOINT SEALS AND MODULAR EXPANSION JOINT SEALS SHALL BE SET A MINIMUM OF 1/8" AND A MAXIMUM OF 1/4" BELOW THE TOP OF SLAB.

FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

FOR MODULAR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

CLASSIC BRIDGE RAILS AND SIDEWALKS IN THE CONTINUOUS UNIT SHALL NOT BE CAST UNTIL THE DECK SLAB CONCRETE IN EACH STAGE OF CONSTRUCTION HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

PREVIOUSLY CAST CONCRETE IN THE CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

STAY-IN-PLACE FORMS SHALL HAVE CLOSED TAPERED ENDS.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND GIRDER STIFFENER OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED AND INCLUDED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR ON THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

STRUCTURAL STEEL ERECTION IN EACH STAGE OF CONSTRUCTION FOR THE CONTINUOUS UNIT SHALL BE COMPLETE BEFORE FALSEWORK OR FORMS ARE PLACED IN EACH STAGE OF THE UNIT.

LAP SPLICES, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED IN THE B5000 BARS OVER BENT 1.

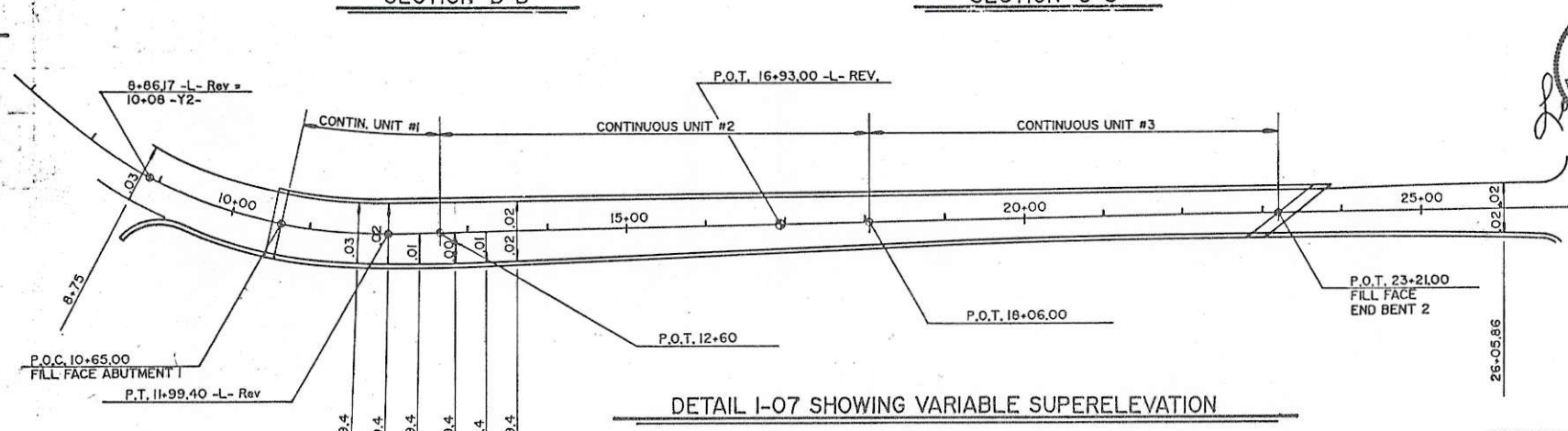
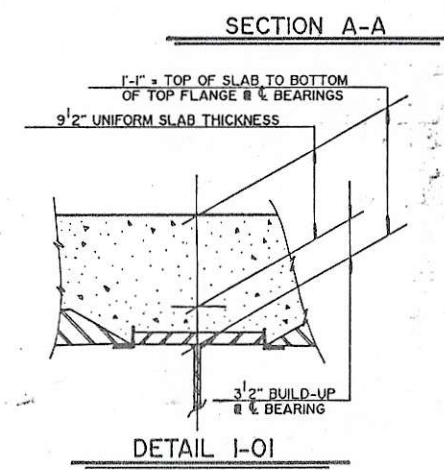
DIRECTION OF CASTING DECK CONCRETE SHALL BE FROM THE FIXED BEARING AT BENT 1 TOWARD EACH EXPANSION BEARING AT ABUTMENT 1 AND BENT 2.

SEE THE TRAFFIC CONTROL PLANS FOR STAGE I, II, III & IV LOCATIONS AND PAY LIMITS OF THE TEMPORARY BRIDGE BARRIERS - TYPE S.

THE TOPS OF DECK DRAINS SHALL BE SET 3/8" BELOW SURFACE OF DECK SLAB.

FALSEWORK AND METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TESTS. SEE STRUCTURAL STEEL GIRDER ELEVATION SHEETS.

FOR BARS SHOWN AND NO BAR MARKS AND/OR BAR-RUNS SHOWN, SEE PLANS OF SPANS.



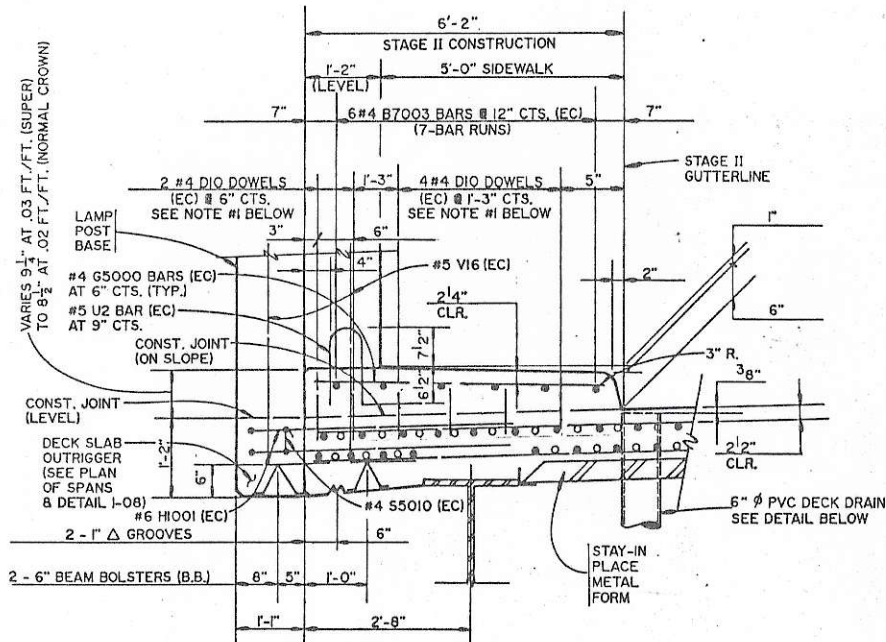
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00 -L- Rev.

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE DETAILS (CONTINUOUS UNIT #1) OCT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
SHEET NO. 5-17 TOTAL SHEETS 224					

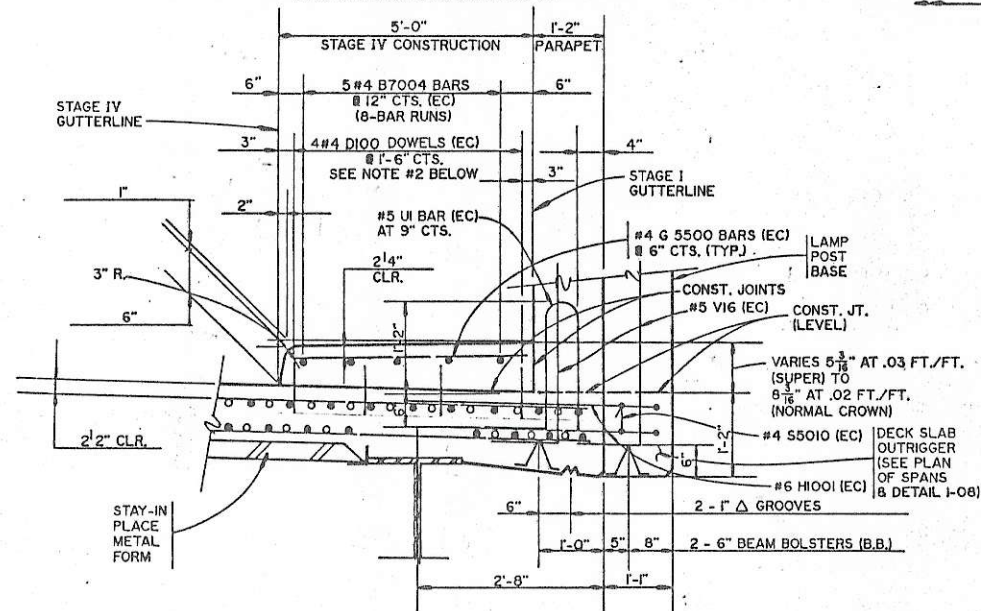
DRAWN BY: N. M. WILSON
 CHECKED BY: ECM
 DATE: OCT. 94
 DATE: 6/95

U1-003
 PREPARED BY:
 THOMPSON-BORDON-SHOOK
 ENGINEERS, INCORPORATED



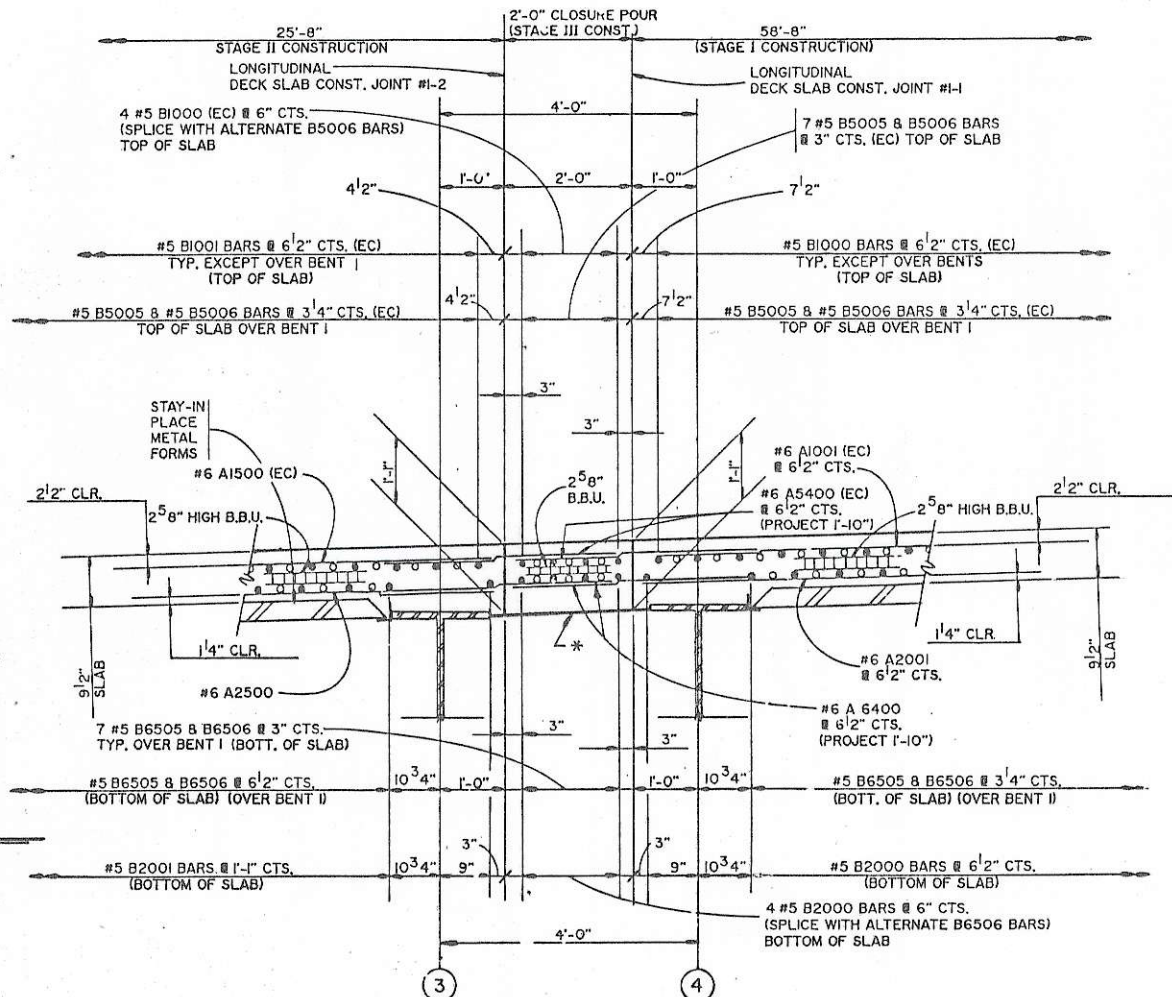
DETAIL I-02 - TYPICAL SIDEWALK SECTION - STAGE II CONSTRUCTION

NOTE #1: DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER DECK SLAB HAS BEEN SCAFFOLD OFF



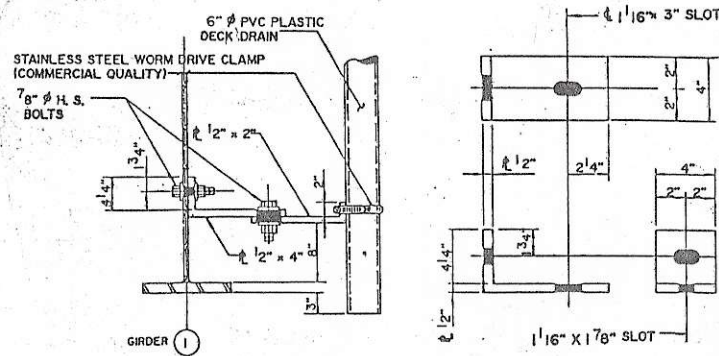
DETAIL I-05 - TYPICAL SIDEWALK SECTION - STAGE IV CONSTRUCTION

NOTE #2: DOWELS SHALL BE ADHESIVELY ANCHORED IN DECK SLAB DURING STAGE IV SIDEWALK CONSTRUCTION. SEE SPECIAL PROVISIONS.

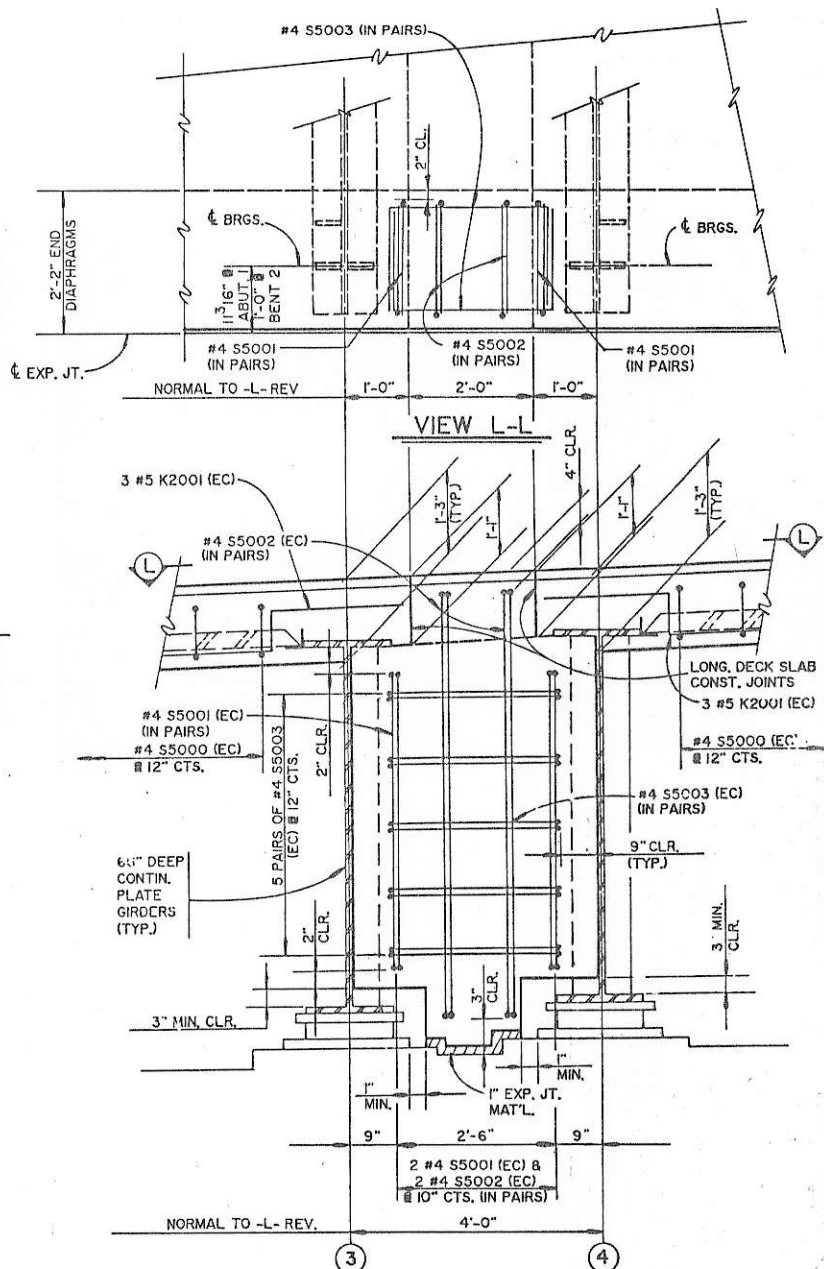


DETAIL I-03 - TYPICAL CLOSURE POUR SECTION

* FORM BOTTOM OF OVERHANGS & CLOSURE POUR FLUSH WITH BOTTOM OF TOP FLANGES.



DETAIL I-06 - DRAIN CONNECTION



DETAIL I-04 - TYPICAL CLOSURE POUR END DIAPHRAGM



NOTES:

COUPLING IN DRAIN PIPE WILL BE PERMITTED AS APPROVED BY THE ENGINEER.
TOP OF FLOOR DRAIN TO BE SET $\frac{3}{8}$ " BELOW SURFACE OF SLAB.
4-1/2" SQUARE LUGS TO BE GLUED TO THE PVC PLASTIC PIPE AT EQUAL SPACES AROUND THE PIPE DRAIN APPROXIMATELY 4" FROM THE TOP OF THE PIPE.
BOLT SIZE TO BE SAME AS DIAPHRAGMS AND CROSS FRAME CONNECTIONS. STAINLESS STEEL WORM DRIVE HOSE CLAMP SHALL BE COMMERCIAL QUALITY.
PVC PLASTIC DECK DRAINS SHALL BE PAINTED WITH AN APPROVED VINYL PAINT. SEE SPECIAL PROVISIONS.

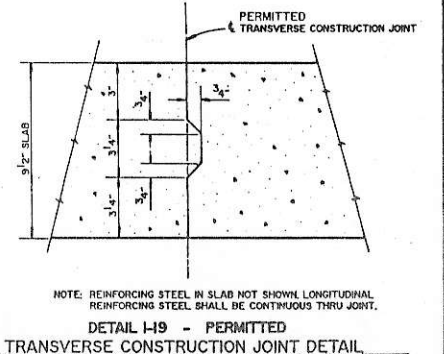
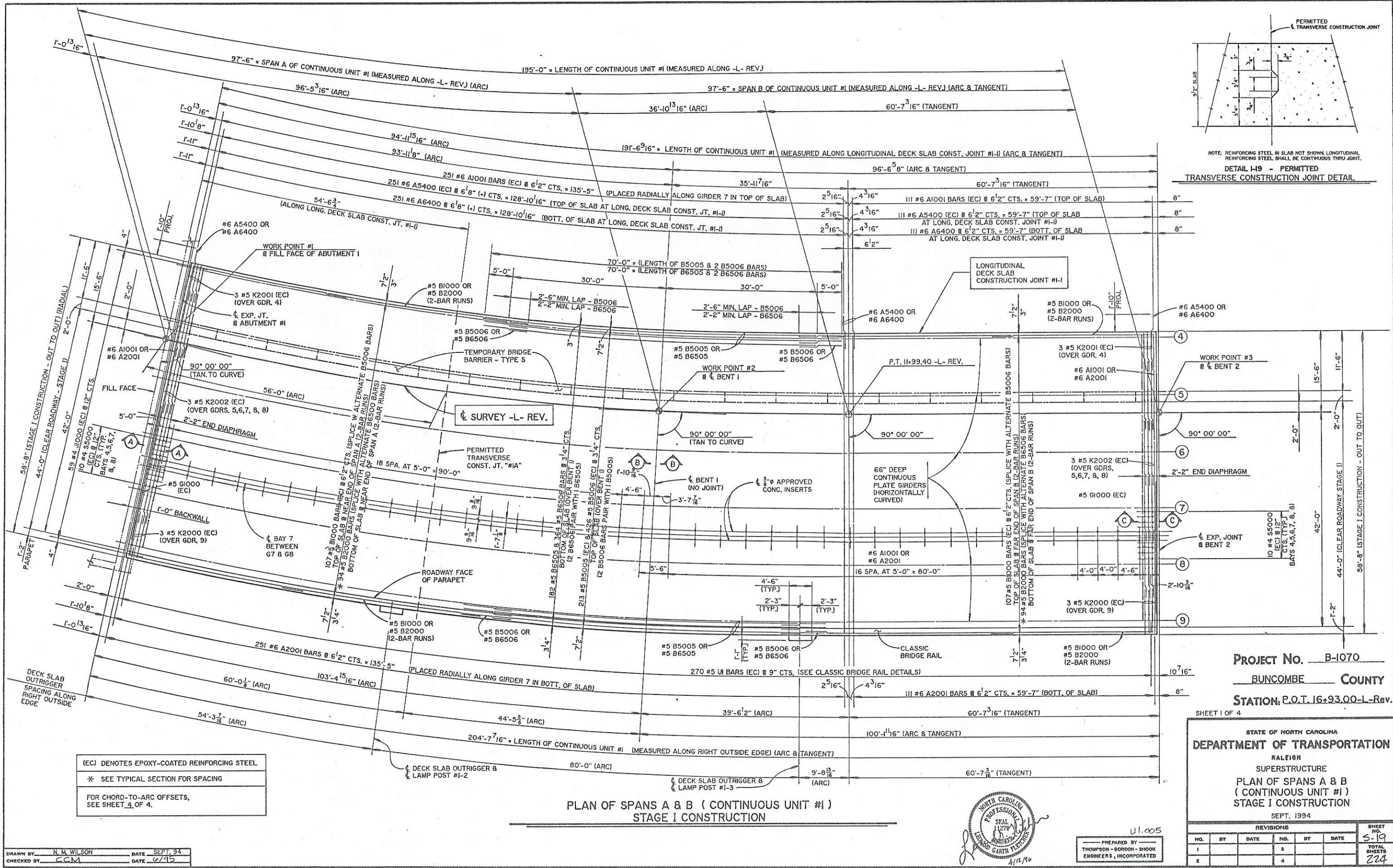
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE DETAILS (CONTINUOUS UNIT #1) JAN. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
SHEET NO. 5-18 TOTAL SHEETS 224					

DRAWN BY: N.M. WILSON
CHECKED BY: CEM
DATE: JAN 94
DATE: 6/95

PREPARED BY:
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
* SEE TYPICAL SECTION FOR SPACING
FOR CHORD-TO-ARC OFFSETS, SEE SHEET 4 OF 4.

DRAWN BY: N. M. WILSON
CHECKED BY: CCM
DATE: SEPT. 94
DATE: 6/95

PLAN OF SPANS A & B (CONTINUOUS UNIT #1)
STAGE I CONSTRUCTION



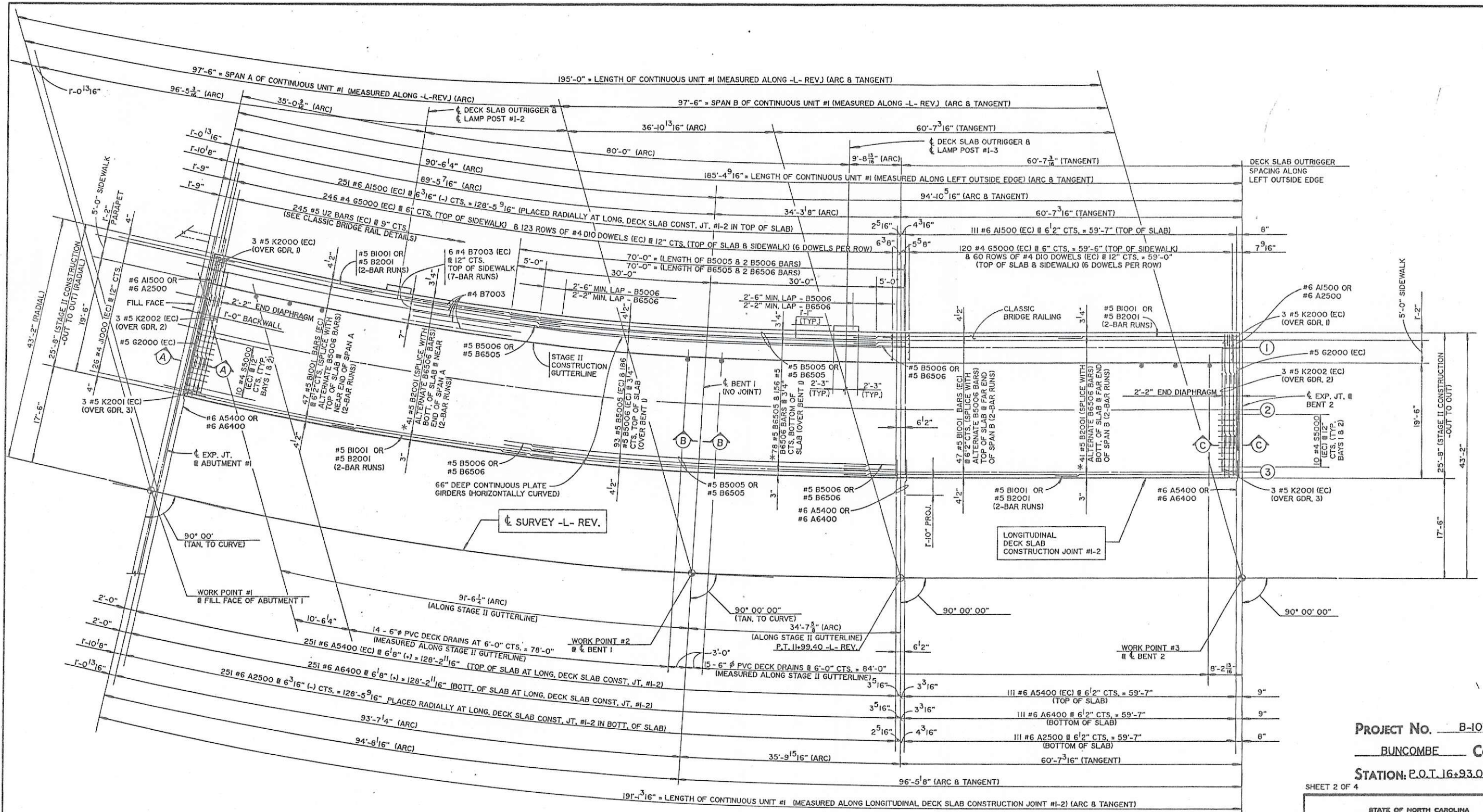
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED
4.1.005

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.
SHEET 1 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPANS A & B
(CONTINUOUS UNIT #1)
STAGE I CONSTRUCTION
SEPT. 1994

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

SHEET NO. 5-19
TOTAL SHEETS 224



PLAN OF SPANS A & B (CONTINUOUS UNIT #1)
STAGE II CONSTRUCTION

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

SHEET 2 OF 4

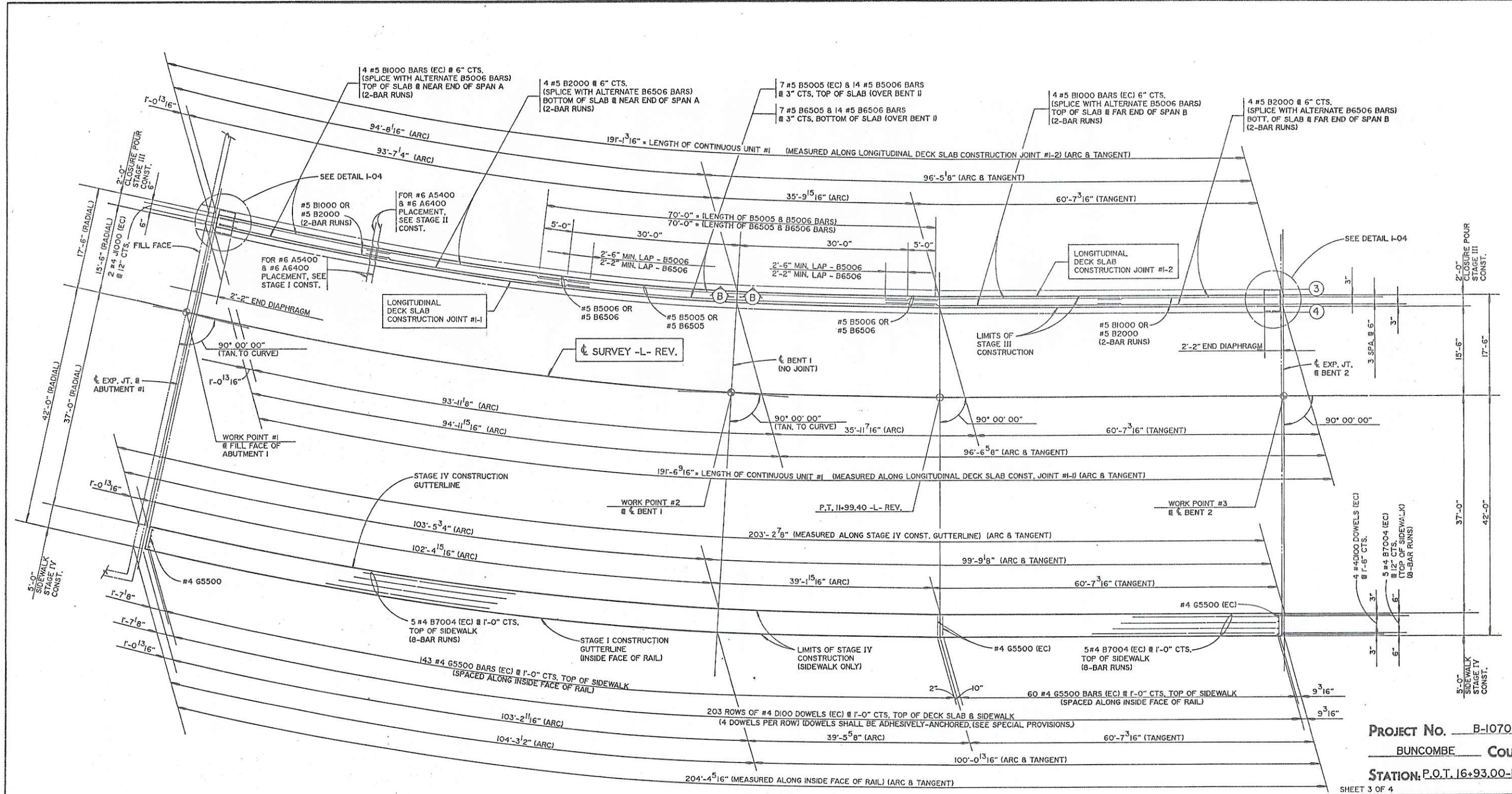
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE PLAN OF SPANS A & B (CONTINUOUS UNIT #1) STAGE II CONSTRUCTION					
OCT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			4		
SHEET NO. 5-20					TOTAL SHEETS 224



PREPARED BY
THOMPSON-BORDON-SHOCK
ENGINEERS, INCORPORATED

(EC) DENOTES EPOXY-COATED REINFORCING STEEL
* SEE TYPICAL SECTION FOR SPACING
FOR CHORD-TO-ARC OFFSETS,
SEE SHEET 4 OF 4.

DRAWN BY N.M. WILSON DATE OCT. 94
CHECKED BY C.M. DATE 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL

PLAN OF SPANS A & B (CONTINUOUS UNIT #1)
STAGES III & IV CONSTRUCTION

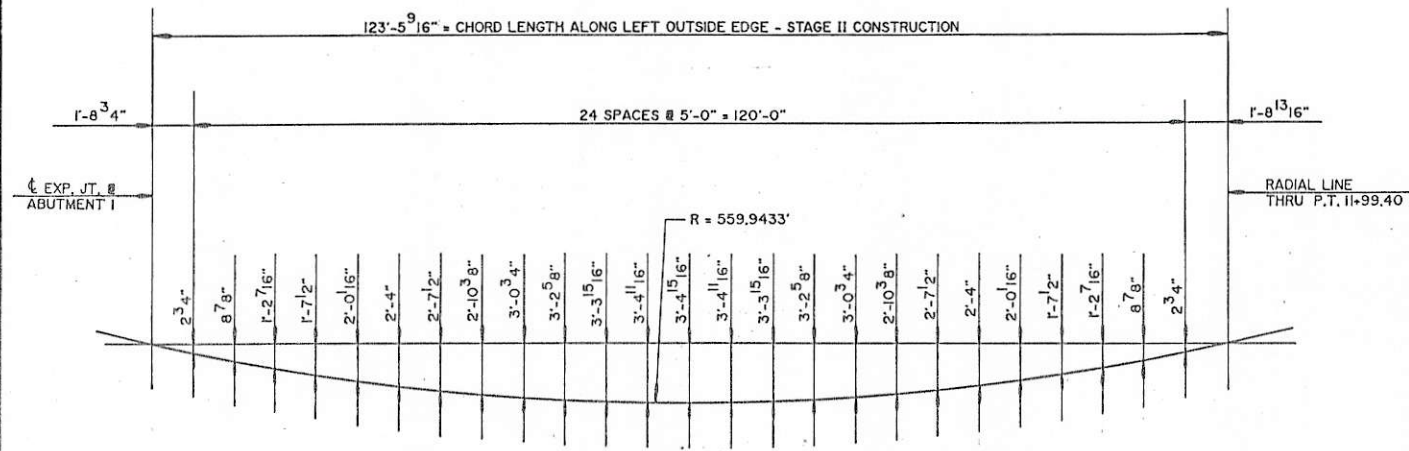
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.
SHEET 3 OF 4



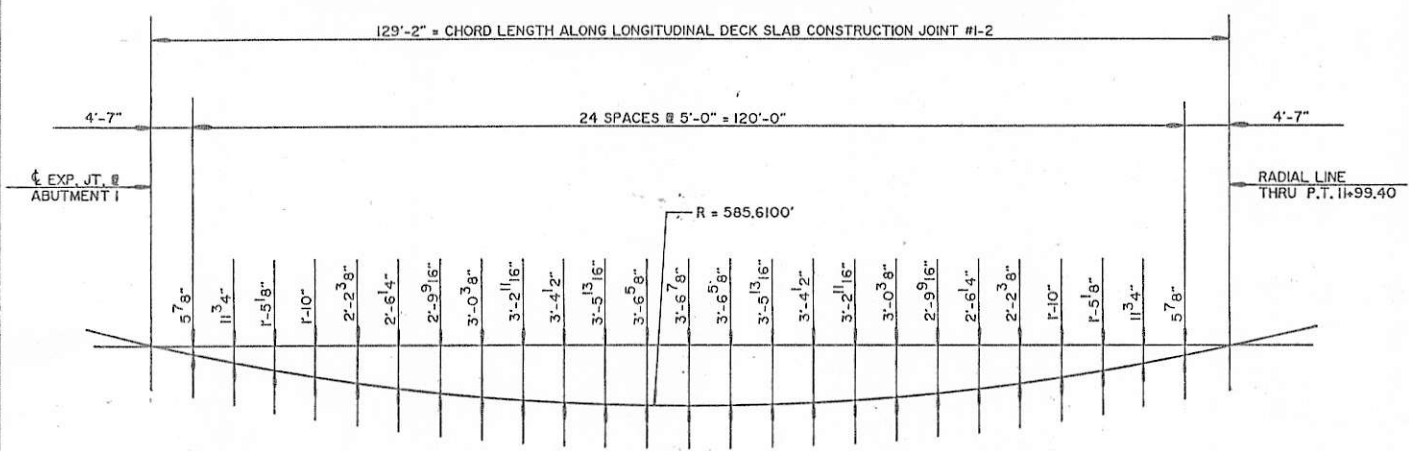
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE PLAN OF SPANS A & B (CONTINUOUS UNIT #1) STAGES III & IV CONSTRUCTION OCT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			4		
					SHEET NO. 5-21
					TOTAL SHEETS 224

DRAWN BY: N.M. WILSON DATE: OCT. 94
CHECKED BY: CCM DATE: 10/15

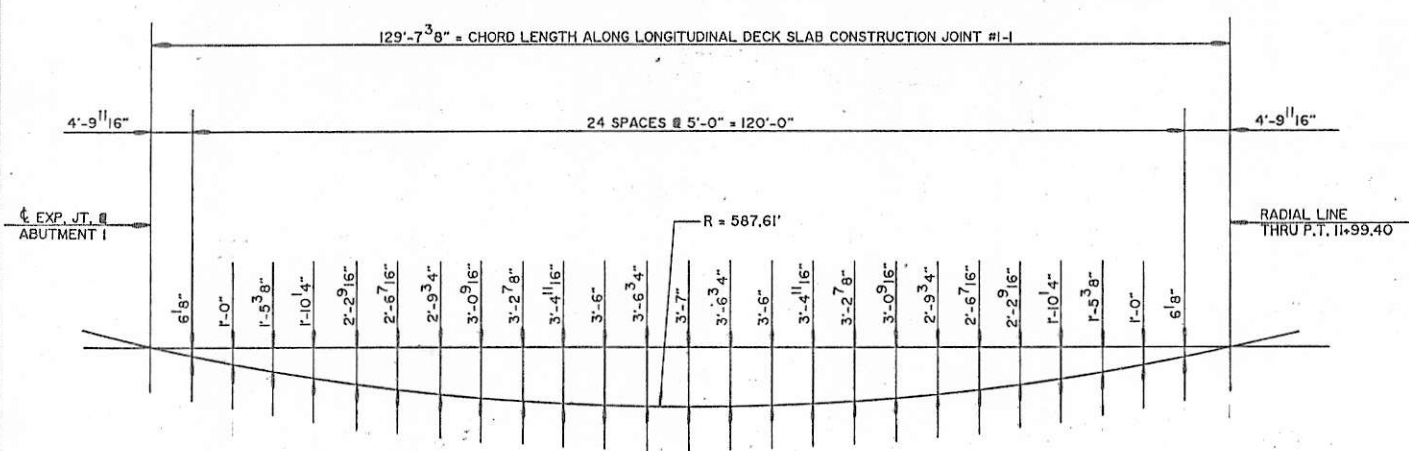
PREPARED BY:
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED



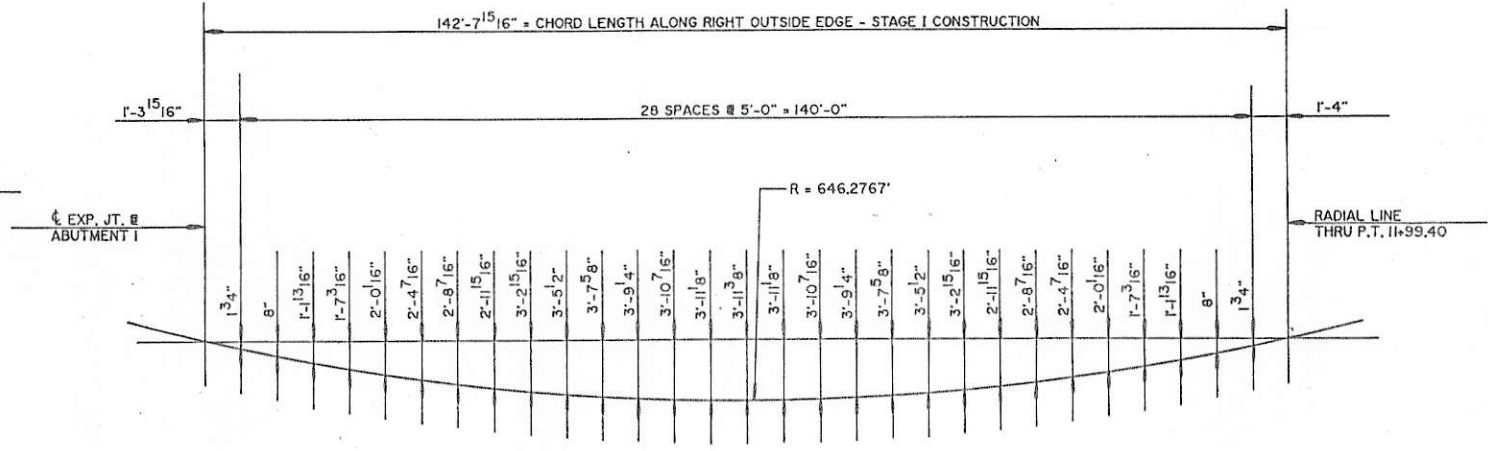
CHORD-TO-ARC OFFSETS @ LEFT OUTSIDE EDGE



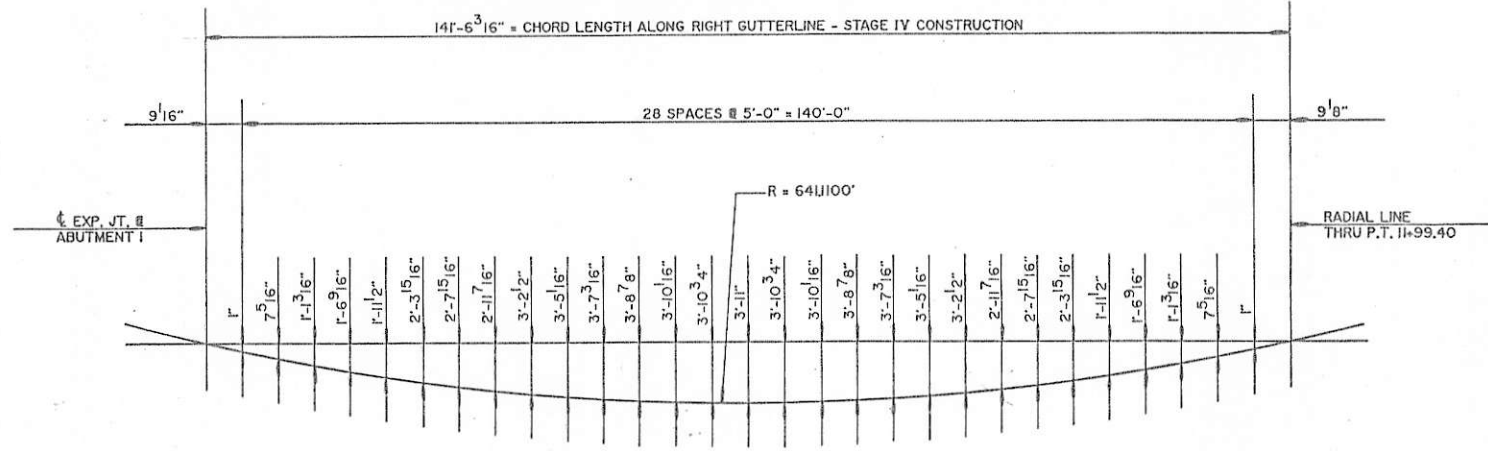
CHORD-TO-ARC OFFSETS @ LONGITUDINAL DECK SLAB CONSTRUCTION JOINT #1-2



CHORD-TO-ARC OFFSETS @ LONGITUDINAL DECK SLAB CONSTRUCTION JOINT #1-1



CHORD-TO-ARC OFFSETS @ RIGHT OUTSIDE EDGE



CHORD-TO-ARC OFFSETS @ RIGHT GUTTERLINE

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

SHEET 4 OF 4

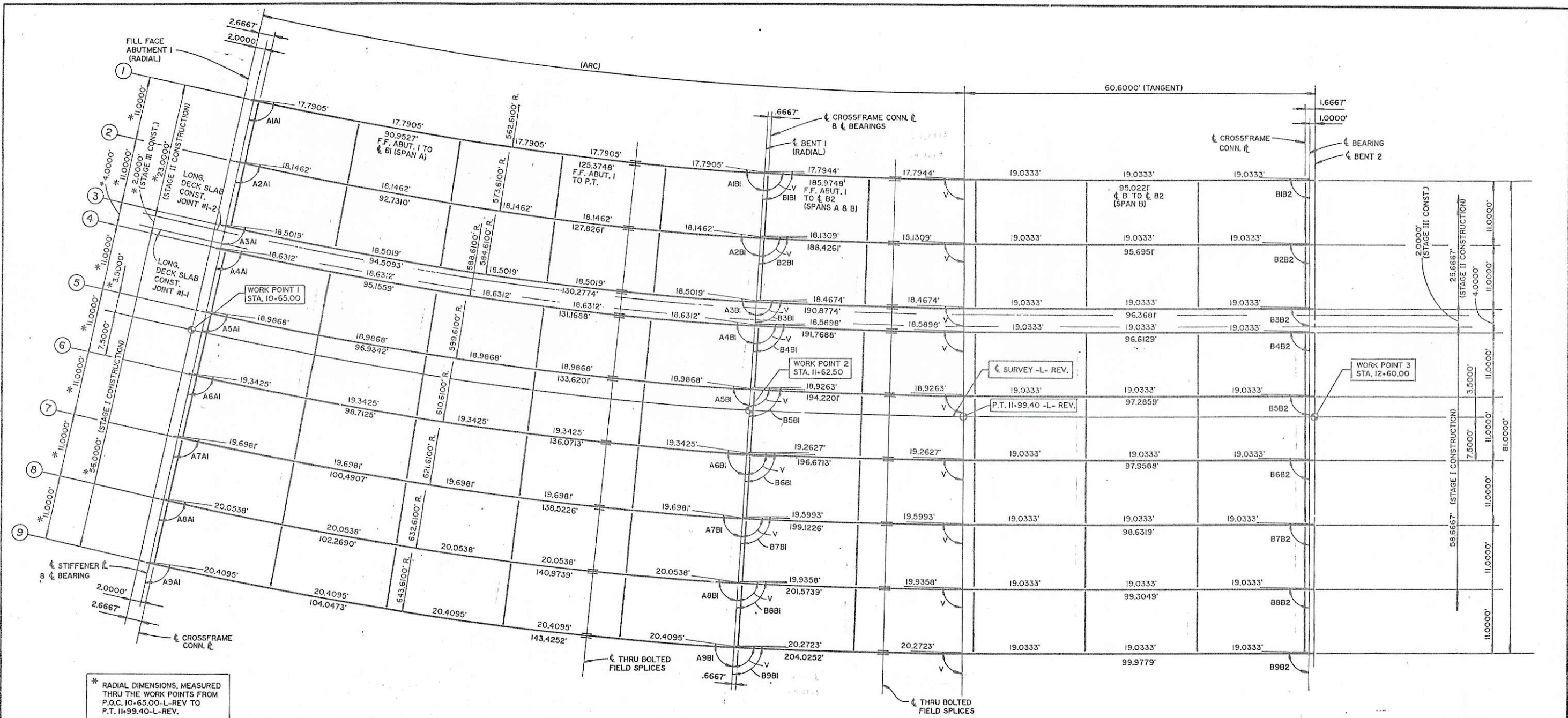


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
CHORD-TO-ARC OFFSETS
CONTINUOUS UNIT #1
FEBRUARY 1995

REVISIONS					SHEET NO.
NO.	BY	DATE	NO.	BY	
1			3		5.22
2			4		TOTAL SHEETS
					224

DRAWN BY: N.M. WILSON
CHECKED BY: CCM
DATE: FEB. 95
DATE: 6/95

PREPARED BY:
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED



* RADIAL DIMENSIONS, MEASURED THROUGH THE WORK POINTS FROM P.O.C. 10+65.00-L-REV TO P.T. 11+99.40-L-REV.

CONTROL PLAN FOR SPANS A & B
(CONTINUOUS UNIT #1)

CHORD ANGLES									
SPAN A CHORD FROM CL BEARING AT ABUTMENT I TO CL BENT I				SPAN B CHORD FROM CL BENT I TO P.T.		SPAN B CHORD FROM CL BENT I TO CL BEARING AT BENT 2			
A1A1	94°43'59"	A1B1	94°31'46"	V	91°45'10"	B1B1	92°51'48"	B1B2	90°38'32"
A2A1	94°43'52"	A2B1	94°31'53"			B2B1	92°51'22"	B2B2	90°38'58"
A3A1	94°43'45"	A3B1	94°32'00"			B3B1	92°50'54"	B3B2	90°39'26"
A4A1	94°43'43"	A4B1	94°32'02"			B4B1	92°50'43"	B4B2	90°39'36"
A5A1	94°43'36"	A5B1	94°32'09"			B5B1	92°50'16"	B5B2	90°40'04"
A6A1	94°43'30"	A6B1	94°32'15"			B6B1	92°49'49"	B6B2	90°40'31"
A7A1	94°43'24"	A7B1	94°32'21"			B7B1	92°49'22"	B7B2	90°40'58"
A8A1	94°43'19"	A8B1	94°32'26"			B8B1	92°48'56"	B8B2	90°41'24"
A9A1	94°43'13"	A9B1	94°32'32"			B9B1	92°48'30"	B9B2	90°41'50"

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
CONTROL PLAN
CONTINUOUS UNIT #1
OCT. 1994

DRAWN BY: N.M. WILSON / RTJ DATE: OCT. 94
CHECKED BY: CCM DATE: 11/95

PREPARED BY:
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO. 5-23 TOTAL SHEETS 224
NO.	BY	DATE	NO.	BY	DATE	
1			2			
3			4			



* RADIAL DIMENSIONS, MEASURED THRU THE WORK POINTS FROM P.O.C. 10+65.00-L-REV TO P.T. 11+99.40-L-REV.
DIMENSIONS FROM P.T. 11+99.40-L-REV TO P.O.C. 12+60.00-L-REV ARE MEASURED ALONG TANGENT PORTION OF THE FIRST CONTINUOUS UNIT.

FRAMING PLAN FOR SPANS A & B (CONTINUOUS UNIT #1)

PROJECT NO. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

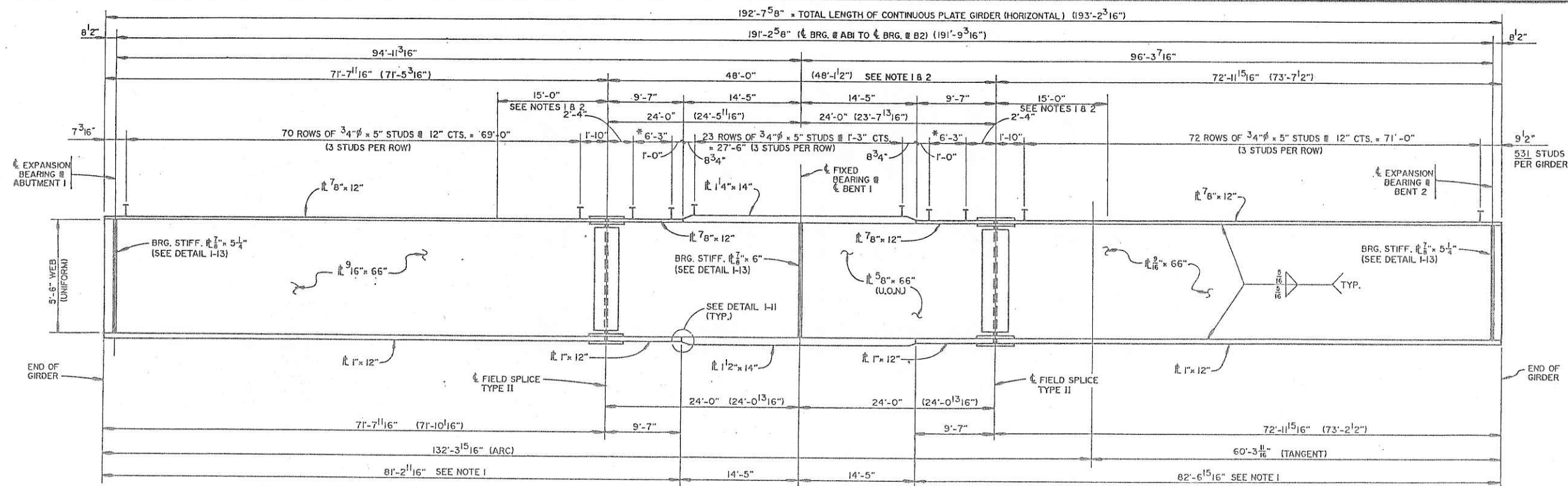


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
FRAMING PLAN
CONTINUOUS UNIT #1
OCT. 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-24
2			4			TOTAL SHEETS
						224

PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

DRAWN BY: N.M. WILSON DATE: OCT. 94
CHECKED BY: CEM DATE: 12/95

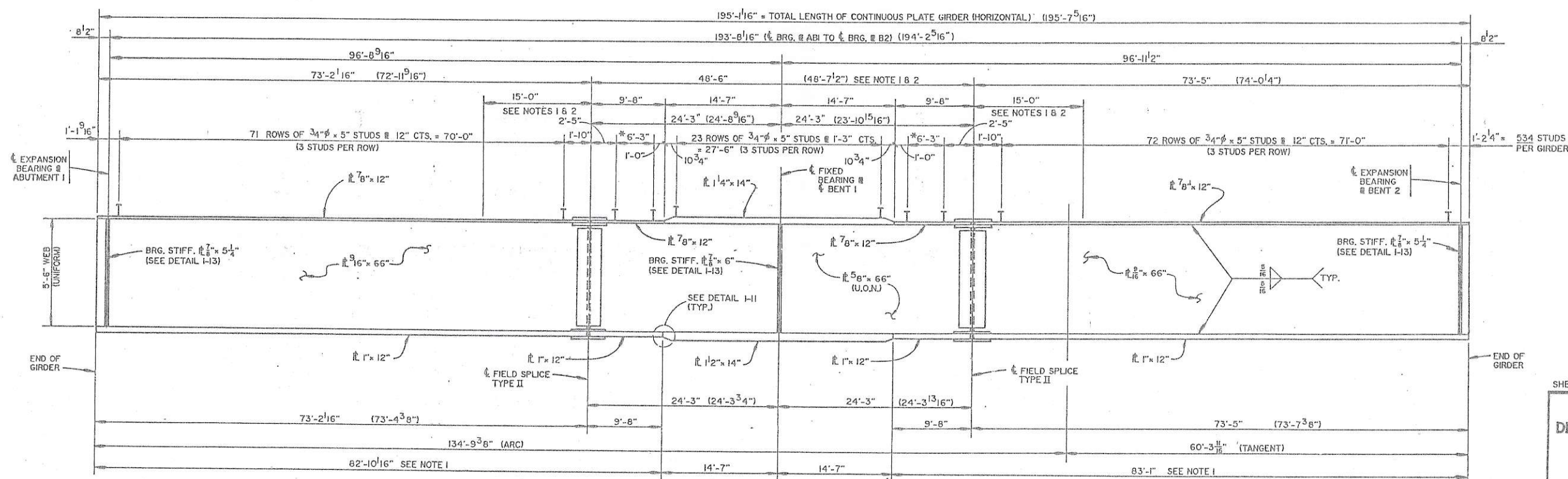


GIRDER NO. 5 ELEVATION

* 6 ROWS OF 3/4" x 5" STUDS @ 1'-3" CTS. = 6'-3" (3 STUDS PER ROW)

- NOTES :
1. CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS. IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.
 2. NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.
 3. ALL DIMENSIONS SHOWN IN PARENTHESIS ARE SLOPED ALONG THE GRADE LINE.

NOTE : ALL SHEAR STUD SPACINGS SHOWN ON THIS SHEET ARE BASED ON THE SLOPED DISTANCE ALONG THE TOP FLANGE OF EACH GIRDER SECTION.



GIRDER NO. 6 ELEVATION

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 5

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER ELEVATION FOR
GIRDERS NO. 5 AND NO. 6
CONTINUOUS UNIT #1
DECEMBER 1992

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-27
2			4			TOTAL SHEETS 224

DRAWN BY: RKL DATE: 12/92
CHECKED BY: CCM DATE: 6/95



PREPARED BY:
THOMPSON - BORDON - WOOD
ENGINEERS, INCORPORATED

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED.

CAMBERED GIRDER LENGTHS SHALL BE ADJUSTED AND BEARINGS ARE TO BE PLACED ON THE CAMBERED GIRDER SO AS TO BE ALIGNED WITH THE ANCHORS AFTER DEAD LOAD DEFLECTION HAS OCCURRED, SHOP PLANS SHALL BE PREPARED ACCORDINGLY.

FOR POT BEARINGS, SEE SPECIAL PROVISIONS.

AT ALL POINTS OF SUPPORT IN SPANS A & B, NUTS FOR ANCHOR BOLTS SHALL BE TIGHTENED FINGER TIGHT AND GIVEN AN ADDITIONAL 1/4 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURKED WITH A SHARP TOOL.

SHEAR STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPICE WELDS.

ALL FIELD CONNECTIONS TO BE 7/8" DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED. SEE ALSO BOLTED FIELD SPICE DETAILS.

SHOP SPICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED TOP FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION OR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS. KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPICES. NO SHOP SPICES WILL BE PERMITTED IN THE BOTTOM FLANGE PLATES.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

END OF GIRDERS SHALL BE PLUMB.

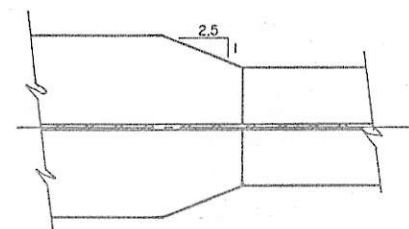
WHEN WELDING THE SOLE PLATE TO THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE BEARING DOES NOT EXCEED 250° F. TEMPERATURES ABOVE THIS MAY DAMAGE THE TPE OR ELASTOMER.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

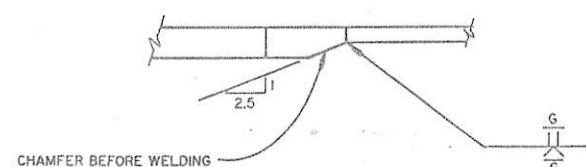
IN LIEU OF THE TURN OF THE NUT METHOD, TENSION ON A325 BOLTS MAY BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS. FOR DIRECT TENSION INDICATORS, SEE SPECIAL PROVISIONS.

FOR RADIOGRAPHIC TESTING OF BUTT WELDS, SEE SPECIAL PROVISIONS.

FOR SURFACE PREPARATION OF UNPAINTED STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

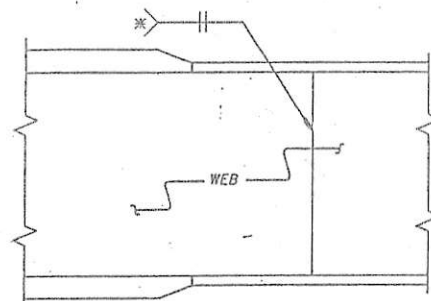


PLAN OF FLANGE

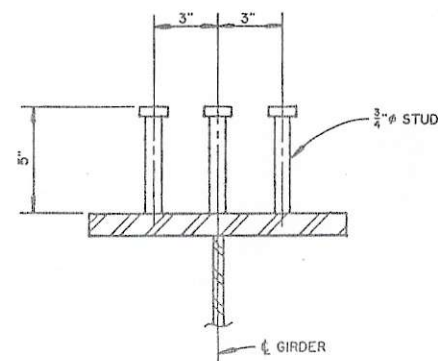
ELEVATION OF BOTTOM FLANGE
(TOP FLANGE SIMILAR)

DETAIL I-11

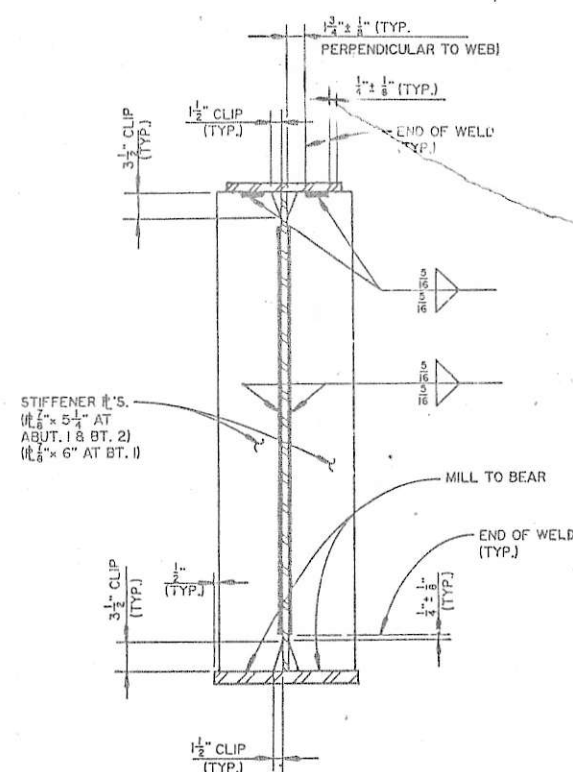
* GRIND SMOOTH AND FLUSH ON
OUTER FACE OF EXTERIOR
GIRDERS

ELEVATION
TYPICAL WEB BUTT JOINT

DETAIL I-16



DETAIL I-12



DETAIL I-13

BEARING STIFFENER
(NORMAL TO WEB -
TYP. ALL GIRDERS)

PROJECT 3-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16-S-100-L-REV

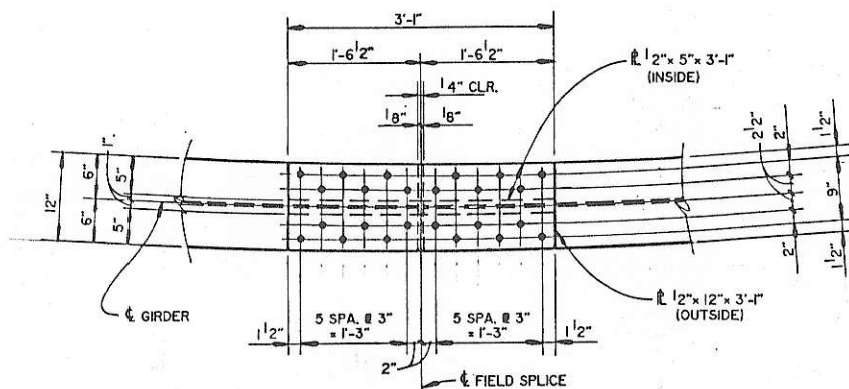


U1.017

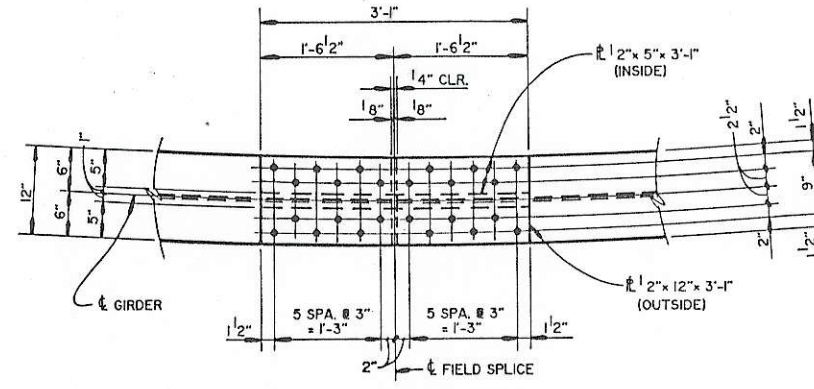
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL DETAILS CONTINUOUS UNIT #1					
AUGUST 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. 5-3
					TOTAL SHEETS 224

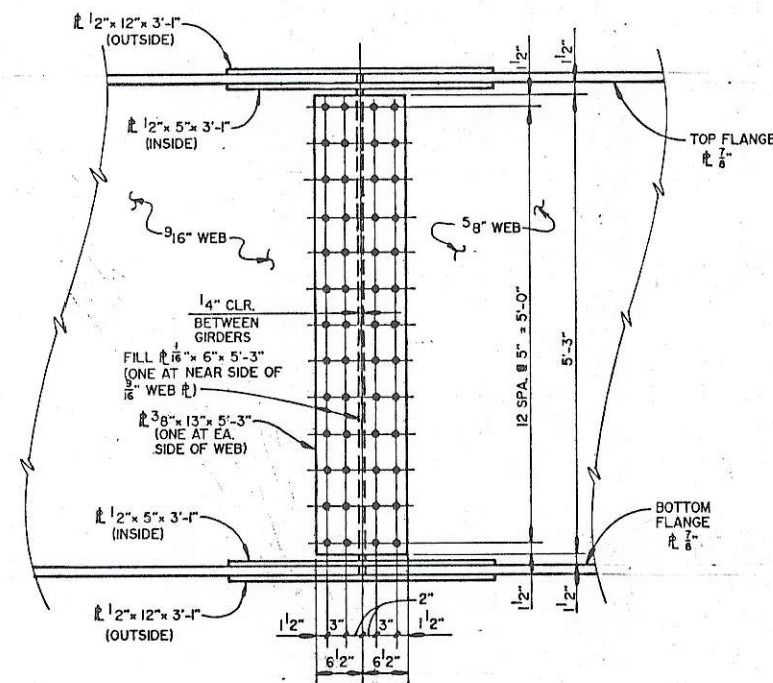
DRAWN BY: RTM DATE: 8-24
CHECKED BY: GCM DATE: 6/95



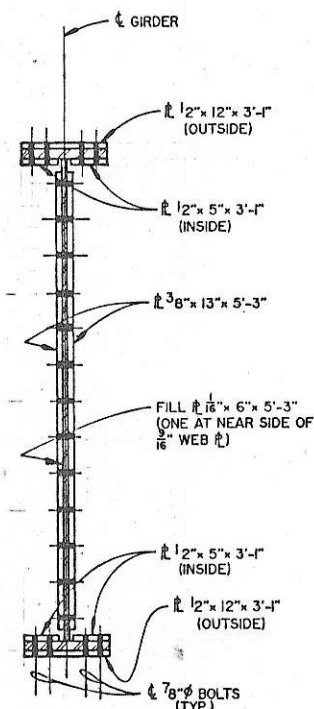
PLAN OF TOP FLANGE



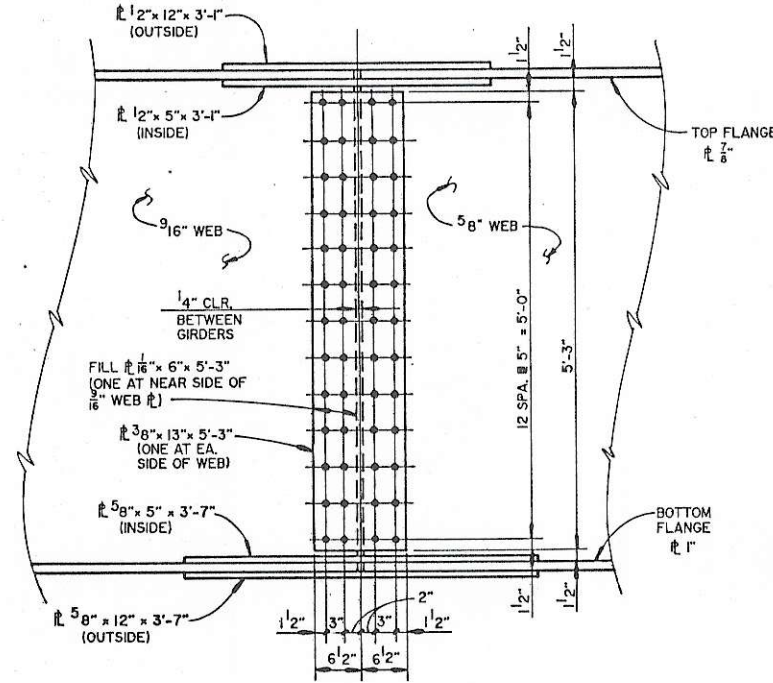
PLAN OF TOP FLANGE



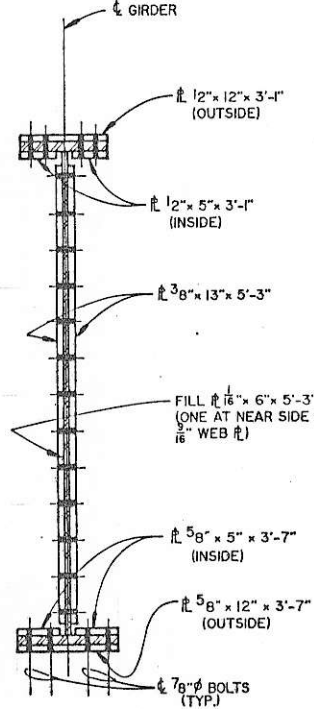
ELEVATION



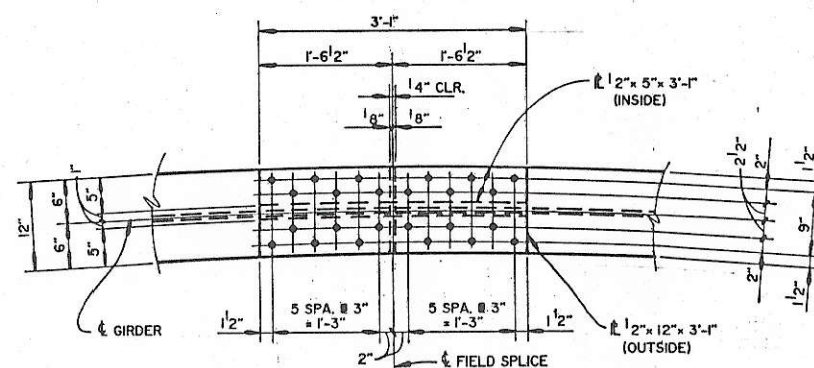
SECTION THRU SPLICE



ELEVATION



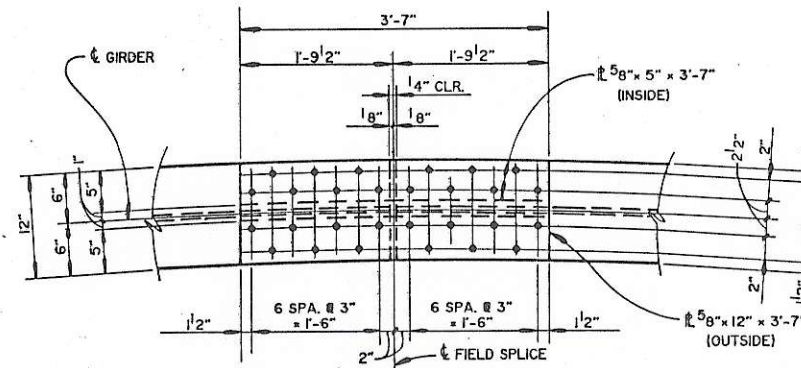
SECTION THRU SPLICE



PLAN OF BOTTOM FLANGE

FIELD SPLICE - TYPE I

(100 - 7/8" H.S. BOLTS REQ'D. THIS SPLICE)



PLAN OF BOTTOM FLANGE

FIELD SPLICE - TYPE II

(104 - 7/8" H.S. BOLTS REQ'D. THIS SPLICE)

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

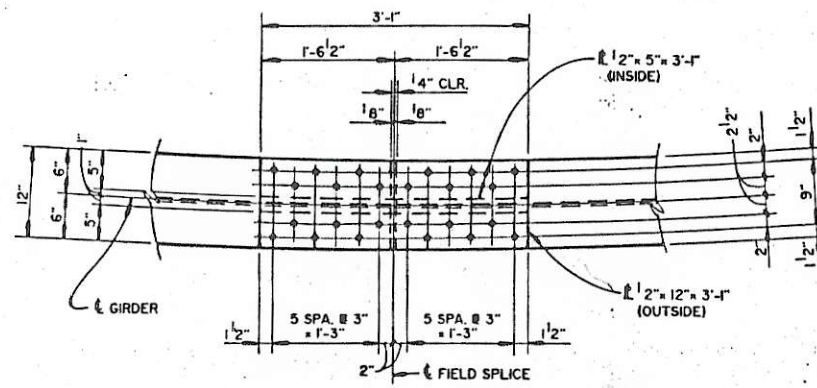
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
SPlice DETAILS
CONTINUOUS UNIT #1
DECEMBER 1992



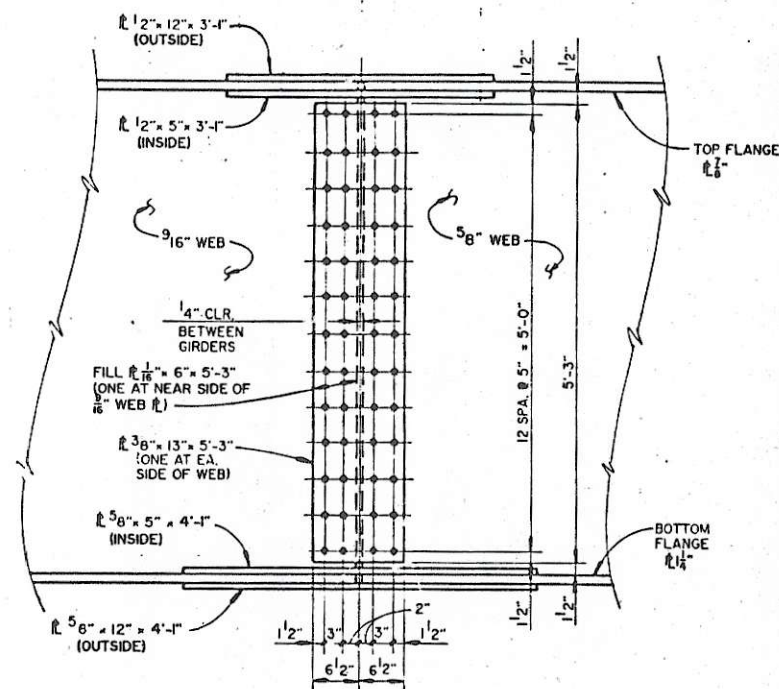
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-32
2			4			224

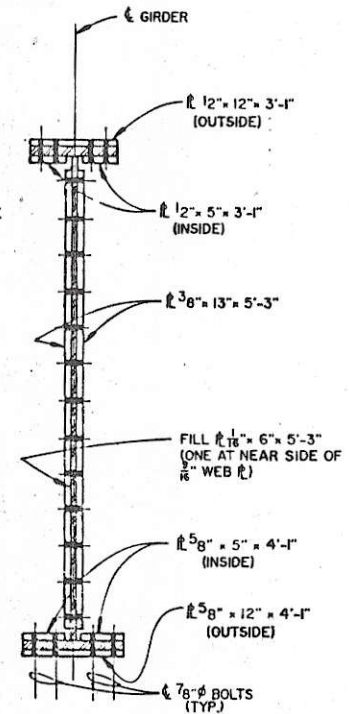
DRAWN BY: RCL
CHECKED BY: GCM
DATE: 12/12/95
DATE: 6/9/95



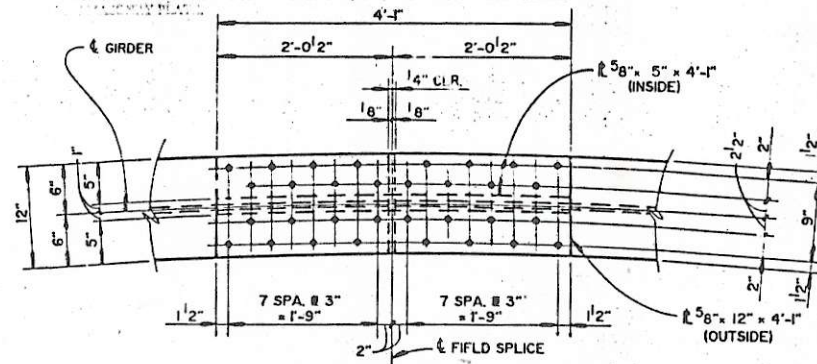
PLAN OF TOP FLANGE



ELEVATION



SECTION THRU SPLICE



PLAN OF BOTTOM FLANGE

FIELD SPLICE - TYPE III

(108 - 7/8" H.S. BOLTS REQ'D. THIS SPLICE)

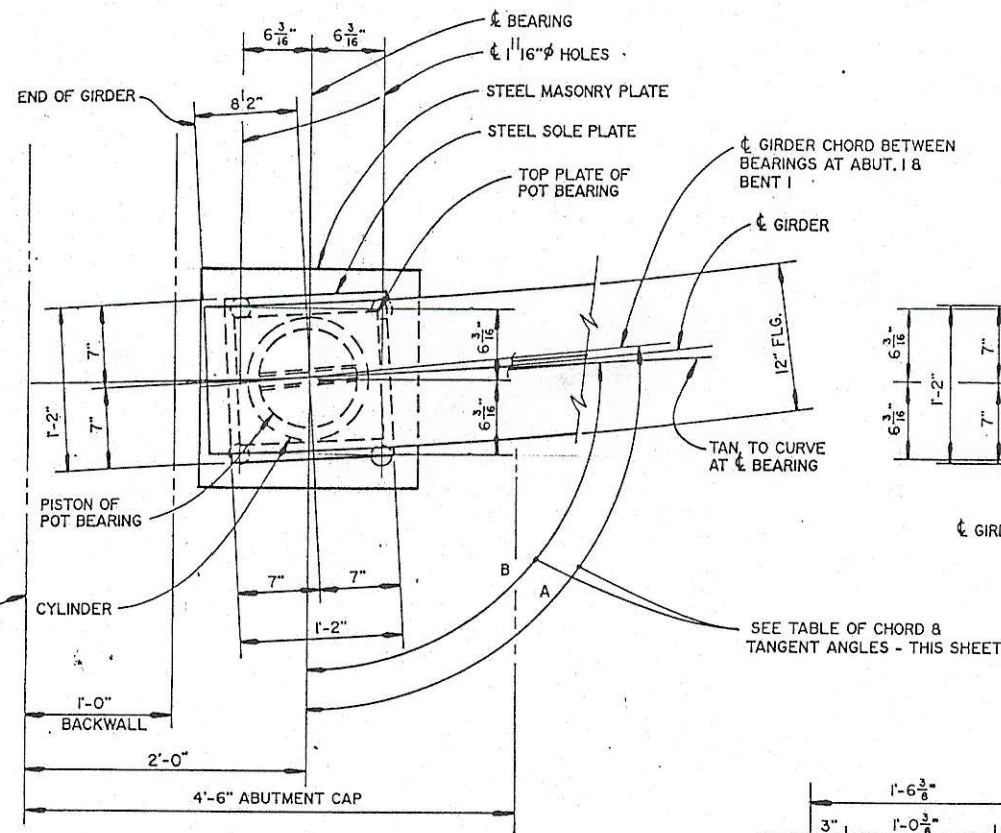
DRAWN BY: RL DATE: 12/32
CHECKED BY: SEM DATE: 6/95



U1.019
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PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

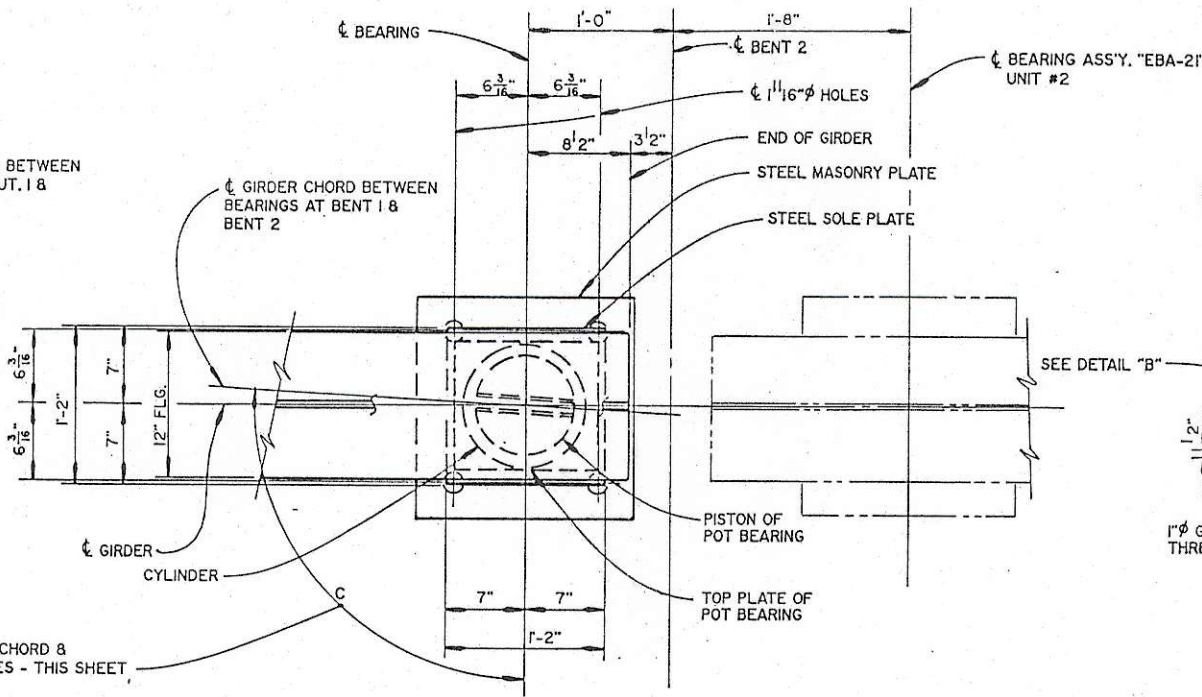
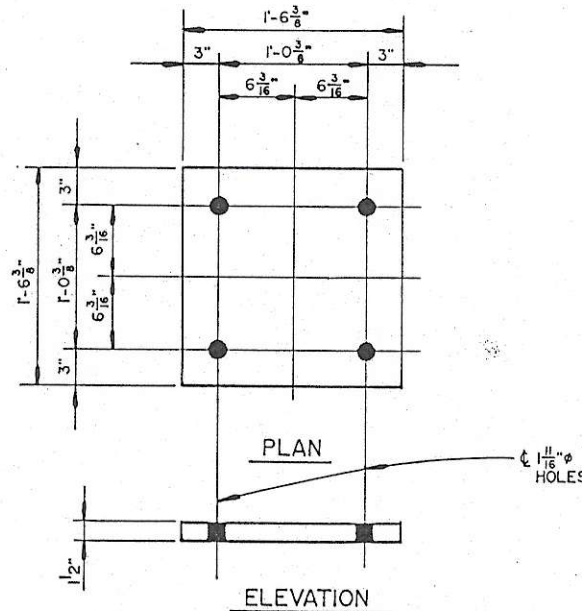
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL SPLICE DETAILS CONTINUOUS UNIT #1 DECEMBER 1992					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. 5-33 TOTAL SHEETS 224



PLAN - "EBA-II" AT ABUTMENT 1

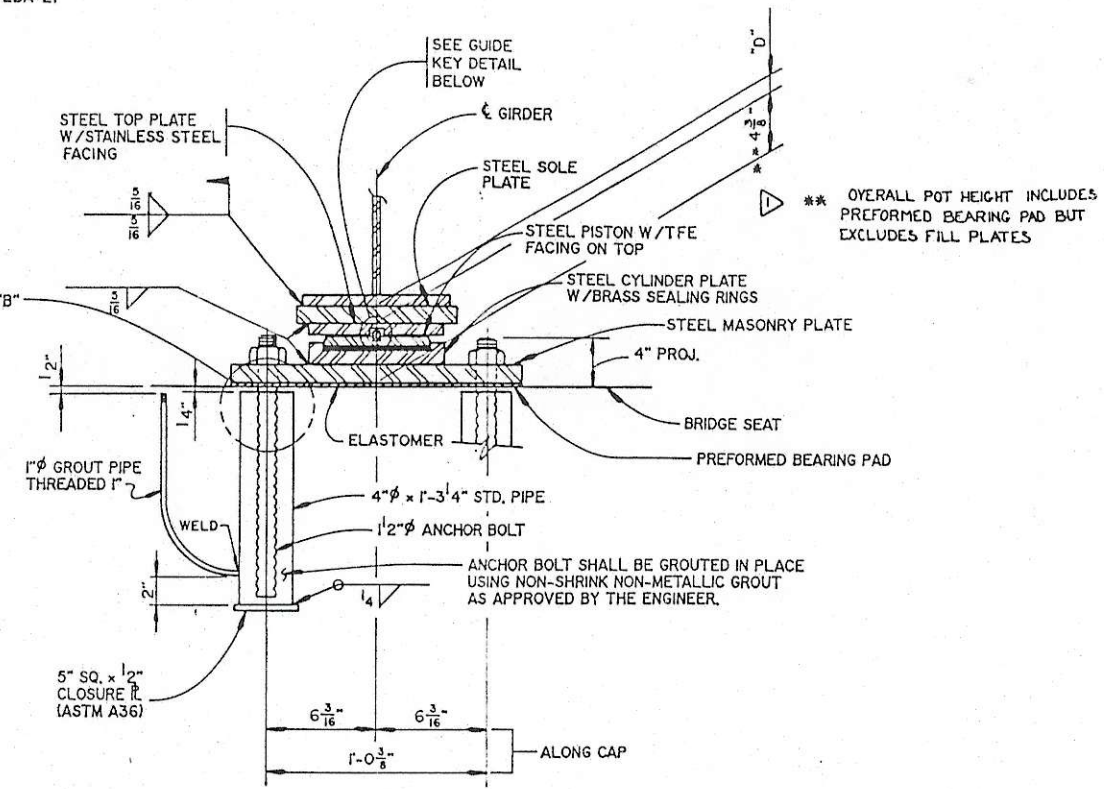
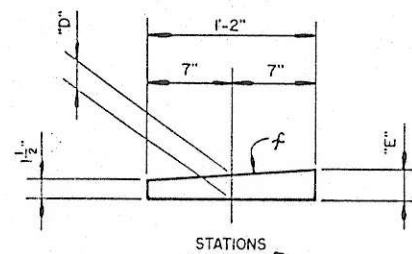
TABLE FOR FILL PLATES			
MARK	LOCATION	THICKNESS	NO. REQ'D.
FP-101	GDR. 4, BT. 2 (NS)	1/8"	1
FP-102	GDR. 7, BT. 2 (NS)	1/8"	1
FP-103	GDR. 9, BT. 2 (NS)	3/4"	1

* AT THE CONTRACTOR'S OPTION, HE MAY COMBINE FILL PLATES, FP-101, ETC., WITH THE MASONRY PLATE AT THE LOCATION SHOWN ON THE FRAMING PLAN. FILL PLATES NOT COMBINED SHALL HAVE THE THICKNESS AS SHOWN ON THE PLANS AND SHALL HAVE THE SAME LENGTH, WIDTH AND HOLE PATTERN AS THE MASONRY PLATE.

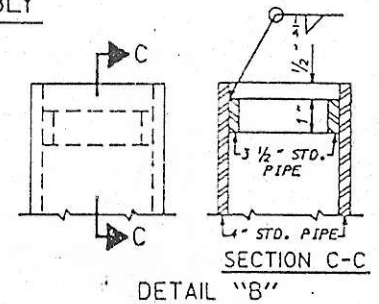
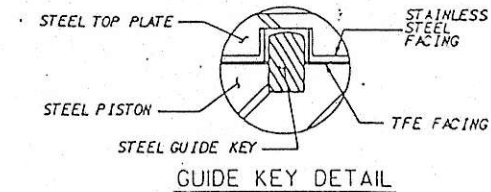


PLAN - "EBA-II" AT BENT 2

GIRDER	CHORD & TANGENT ANGLES		
	AT ABUTMENT 1	AT BENT 2	
	A (CHORD)	B (TANGENT)	C (CHORD)
1	94°43'59"	90°12'13"	90°38'32"
2	94°43'52"	90°11'59"	90°38'58"
3	94°43'45"	90°11'46"	90°39'26"
4	94°43'43"	90°11'41"	90°39'36"
5	94°43'36"	90°11'28"	90°40'04"
6	94°43'30"	90°11'16"	90°40'31"
7	94°43'24"	90°11'04"	90°40'58"
8	94°43'19"	90°10'52"	90°41'24"
9	94°43'13"	90°10'41"	90°41'50"



SECTION THRU EXPANSION BEARING ASSEMBLY



FOR ADDITIONAL POT BEARING DETAILS, SEE APPROVED SHOP DRAWINGS.

SOLE PLATE TABLE		
MARK	"D"	"E"
SP-101	2 1/16"	2 5/8"
SP-102	2 1/32"	2 5/16"
SP-103	2"	2 1/2"
SP-104	1 31/32"	2 7/16"
SP-105	1 15/16"	2 3/8"
SP-109	2 1/16"	2 5/8"
SP-110	2 1/16"	2 5/8"
SP-111	2 1/16"	2 5/8"
SP-112	2 1/16"	2 5/8"
SP-113	2"	2 1/2"
SP-114	2"	2 1/2"



PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

SHEET 1 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL

BEARING DETAILS FOR
CONTINUOUS UNIT #1
JANUARY 1993

REVISIONS				SHEET NO.
NO.	BY	DATE	NO.	
1	RR	3/7/97	1	5-34
2			2	TOTAL SHEETS
				224

TABLE FOR PLATE SETTING DATA (EXPANSION POT BEARINGS)				
TEMPERATURE AT TIME OF SETTING	30°F	60°F	90°F	*
EBA-II	- 1/4"	0	+ 1/4"	1/8"

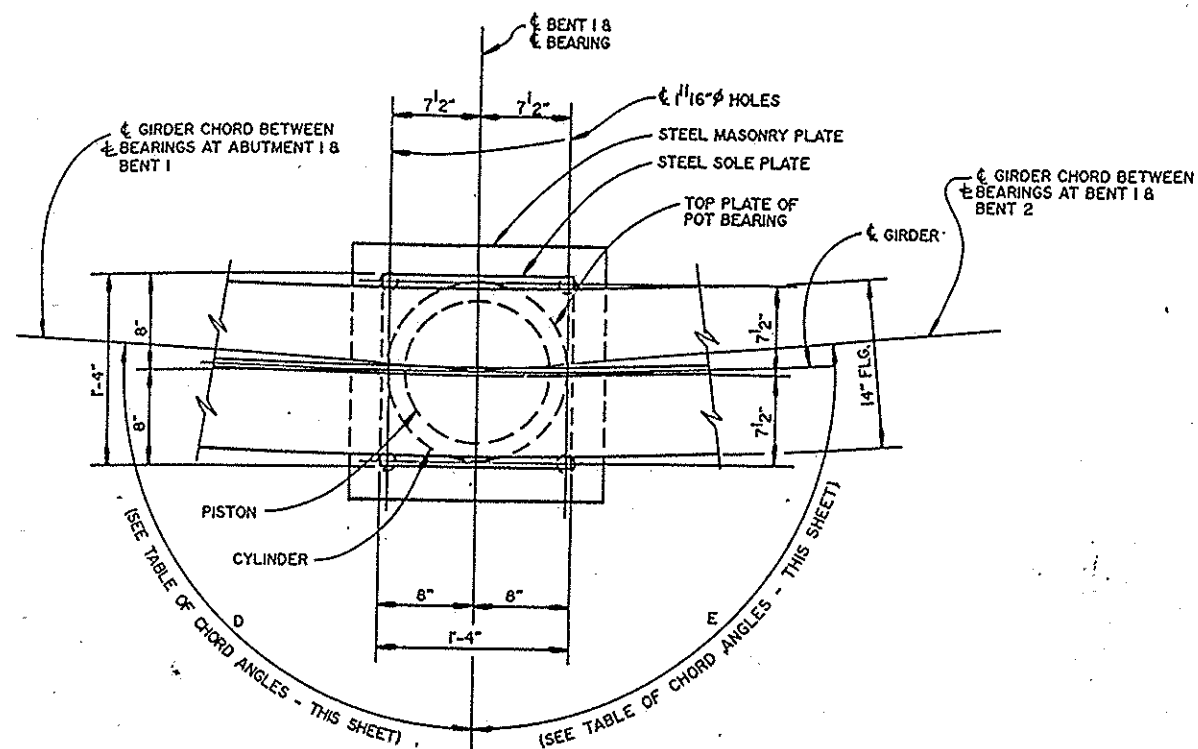
* CORRECTION FOR END ROTATION DUE TO WEIGHT OF SLAB AND COMPOSITE DEAD LOAD.

REVISED SOLE PLATE THICKNESSES, OVERALL POT HEIGHTS, AND ADDED REFERRAL TO APPROVED SHOP DRAWINGS. ✓ BY: CEP

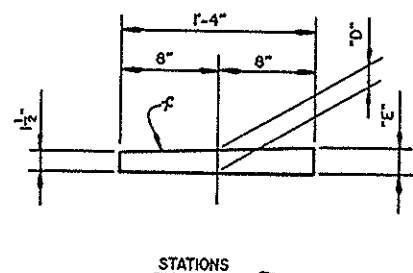
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

01.020

DATE 1-23
DATE 6/96
BY CCM



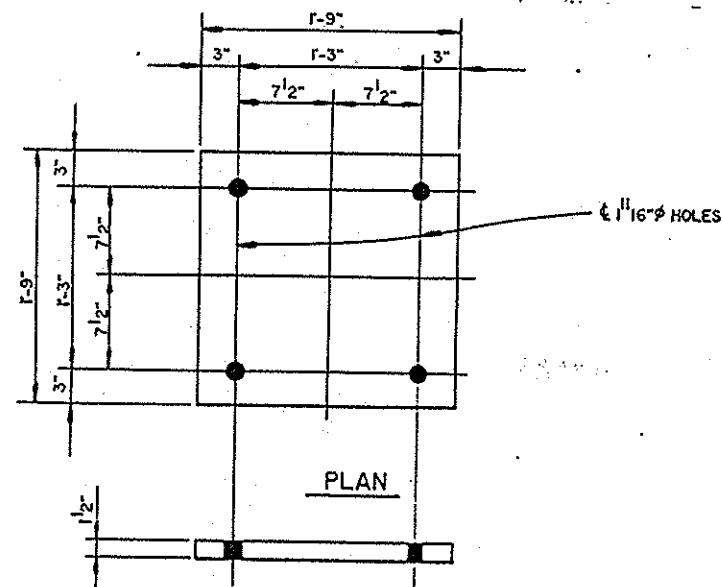
PLAN - "FBA-12" AT BENT 1



ELEVATION OF BEVELED SOLE PLATE

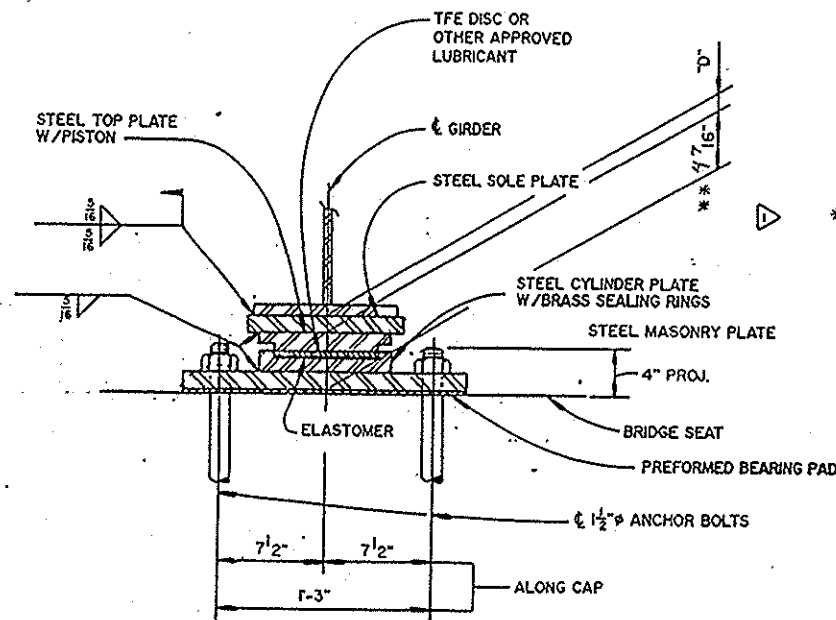
MARK SP-106 THRU SP-108

SOLE PLATE TABLE		
MARK	"D"	"E"
SP-106	2 1/8"	2 3/4"
SP-107	2 1/8"	2 5/8"
SP-108	2"	2 1/2"



ELEVATION

MASONRY PLATE - FBA-12



SECTION THRU FIXED BEARING ASSEMBLY

TABLE OF CHORD ANGLES		
GIRDER	AT BENT 1	
	D	E
1	94°31'46"	92°51'48"
2	94°31'53"	92°51'22"
3	94°32'00"	92°50'54"
4	94°32'02"	92°50'43"
5	94°32'09"	92°50'16"
6	94°32'15"	92°49'49"
7	94°32'21"	92°49'22"
8	94°32'26"	92°48'56"
9	94°32'32"	92°48'30"

TABLE FOR LOADS AND MOVEMENTS						
BEARING	LOCATION	VERTICAL LOAD (K)			LATERAL LOAD (K)	TOTAL MOVEMENT (INCHES)
		DEAD	LIVE	TOTAL		
EBA-11	ABUTMENT 1	90	60	150	15	2"
FBA-12	BENT 1	250	100	350	35	0"
EBA-11	BENT 2	90	60	150	15	2"

NOTES:

FOR POT BEARINGS, SEE SPECIAL PROVISIONS.

FOR THERMAL SPRAYED COATINGS (METALIZATION), SEE SPECIAL PROVISIONS.

AT ALL POINTS OF SUPPORTS IN THE CONTINUOUS UNIT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT ON THE MASONRY PLATE AND GIVEN AN ADDITIONAL 1/4 TURN, THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

SOLE PLATES SHOULD BE WELDED TO GIRDER FLANGES AND ANCHOR BOLTS SHOULD BE GROUTED BEFORE FALSEWORK IS PLACED.

WHEN WELDING THE SOLE PLATE TO THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE BEARING DOES NOT EXCEED 250°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE TFE OR ELASTOMER.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE CLOSURE PLATE AND THE STANDARD PIPE FOR THESE EXPANSION ASSEMBLIES SHALL NOT BE GALVANIZED.

FOR ADDITIONAL POT BEARING DETAILS, SEE APPROVED SHOP DRAWINGS.

** OVERALL POT BEARING HEIGHT INCLUDES PREFORMED BEARING PAD

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
BEARING DETAILS FOR
CONTINUOUS UNIT #1

JANUARY 1993.

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1	AR	3/7/97	3			5-35
2			4			TOTAL SHEETS 224



UL 021

PREPARED BY
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

REVISED SOLE THICKNESS, OVERALL POT HEIGHT, AND ADDED REFERRAL TO APPROVED SHOP DRAWINGS. BY: CEP

DATE 1-28
DATE 6/95

GIRDER 1

DEFLECTION DUE TO WEIGHT OF STEEL	.003	.006	.007	.008	.007	.006	.004	.002	0		.001	.004	.007	.010	.011	.012	.011	.008	.004
DEFLECTION DUE TO WEIGHT OF SLAB	.017	.031	.041	.044	.041	.033	.021	.010	.002		.007	.021	.038	.053	.064	.066	.060	.046	.024
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.002	.004	.005	.005	.005	.004	.003	.001	0		.002	.004	.005	.005	.005	.004	.003	.001	0
TOTAL DEAD LOAD DEFLECTION	.022	.041	.053	.057	.053	.043	.028	.013	.002		.010	.029	.050	.068	.080	.082	.074	.055	.028
VERTICAL CURVE ORDINATE	-.063	-.103	-.122	-.118	-.104	-.089	-.075	-.061	-.046		.061	.120	.180	.224	.186	.149	.111	.074	.037
SUPERELEVATION ORDINATE	-.123	-.114	-.102	-.091	-.075	-.060	-.045	-.029	-.015		.002	.006	.009	.012	.024	.034	.046	.057	.069
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)	.008	.015	.019	.021	.019	.015	.010	.005	.001		.001	0	.001	-.031	-.036	-.037	-.034	-.025	-.013
REQUIRED CAMBER	$-2\frac{1}{8}$	$-2\frac{7}{16}$	$-2\frac{7}{16}$	$-2\frac{1}{4}$	$-1\frac{5}{16}$	$-1\frac{5}{16}$	$-1\frac{5}{16}$	$-1"$	$-\frac{3}{4}$		$\frac{3}{4}$	$1\frac{1}{2}$	$2\frac{1}{4}$	$2\frac{7}{16}$	$2\frac{1}{16}$	$1\frac{3}{4}$	$1\frac{1}{2}$	$1\frac{1}{4}$	$1\frac{1}{8}$

GIRDER 2

GIRDER 2										.1	.2	.3	.4	.5	.6	.7	.8	.9		.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL										.003	.006	.008	.009	.008	.007	.004	.002	0		.001	.004	.007	.010	.011	.012	.011	.008	.005
DEFLECTION DUE TO WEIGHT OF SLAB										.024	.044	.057	.062	.058	.047	.031	.015	.003		.009	.027	.049	.068	.082	.085	.078	.059	.032
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK										.002	.004	.005	.005	.005	.004	.003	.001	0		.001	.002	.004	.006	.007	.007	.007	.005	.003
TOTAL DEAD LOAD DEFLECTION										.029	.055	.070	.076	.071	.058	.038	.018	.003		.011	.033	.060	.084	.100	.104	.096	.072	.040
VERTICAL CURVE ORDINATE										-.062	-.102	-.119	-.115	-.099	-.084	-.069	-.054	-.038		.045	.012	.133	.165	.138	.111	.084	.058	.031
SUPERELEVATION ORDINATE										-.124	-.113	-.102	-.091	-.076	-.060	-.045	-.030	-.016		.001	.081	.006	.009	.021	.032	.043	.053	.065
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS) 										.009	.015	.021	.022	.021	.017	.012	.006	.001		.001	.001	.001	-.039	-.047	-.049	-.045	-.034	-.019
REQUIRED CAMBER 										$-2\frac{1}{8}$	$-2\frac{3}{8}$	$-2\frac{3}{8}$	$-2\frac{3}{16}$	$-1\frac{7}{8}$	$-1\frac{1}{2}$	$-1\frac{1}{4}$	$-\frac{15}{16}$	$-\frac{5}{8}$		$\frac{9}{16}$	$1\frac{1}{8}$	$1\frac{11}{16}$	$1\frac{5}{8}$	$1\frac{3}{8}$	$1\frac{1}{8}$	1"	$1\frac{15}{16}$	$1\frac{5}{16}$

GIRDER 3

GIRDER 3	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.004	.007	.009	.010	.009	.007	.005	.002	.001	.001	.004	.007	.010	.012	.012	.011	.008	.005
DEFLECTION DUE TO WEIGHT OF SLAB	.019	.035	.045	.049	.046	.037	.025	.012	.003	.006	.019	.035	.049	.059	.062	.056	.043	.023
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.003	.005	.006	.007	.007	.005	.004	.002	0	.001	.003	.005	.007	.008	.009	.008	.006	.003
TOTAL DEAD LOAD DEFLECTION	.026	.047	.060	.066	.062	.049	.034	.016	.004	.008	.026	.047	.066	.079	.083	.075	.057	.031
VERTICAL CURVE ORDINATE	-.061	-.1010	-.117	-.111	-.095	-.079	-.063	-.047	-.031	.028	.056	.084	.104	.087	.071	.054	.037	.021
SUPERELEVATION ORDINATE	-.124	-.205	-.104	-.094	-.079	-.063	-.049	-.034	-.020	.013	.025	.037	.048	.053	.057	.062	.066	.071
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS) 	.010	.018	.024	.025	.024	.020	.013	.007	.001	.001	.001	.001	-.030	-.034	-.037	-.034	-.026	-.014
REQUIRED CAMBER 	$-2\frac{1}{8}$	$-2\frac{3}{8}$	$-2\frac{3}{8}$	$-2\frac{3}{16}$	$-1\frac{13}{16}$	$-1\frac{7}{16}$	$-1\frac{3}{16}$	$-\frac{7}{8}$	$-\frac{5}{8}$	$\frac{1}{2}$	1"	$1\frac{7}{16}$	$1\frac{1}{2}$	$1\frac{1}{4}$	$1\frac{1}{16}$	1"	$1\frac{15}{16}$	$1\frac{5}{16}$

NOTE:
DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN
DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS
GIVEN IN FRACTIONS OF AN INCH.

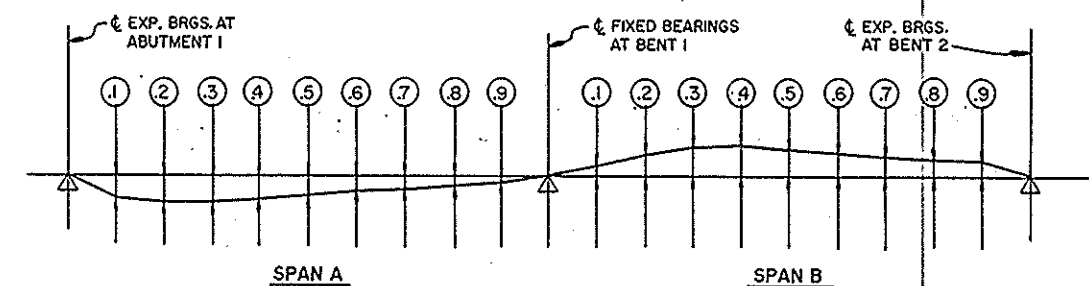
DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS

REVISION NO. 1: REVISED TO CHANGE "CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING" & "REQUIRED CAMBER" FOR GIRDERS 1, 2 & 3; AND TO CHANGE "SCHEMATIC CAMBER ORDINATES - GIRDERS 1 & 2".

BY: NMW 1/30/97
CHECKED BY: CCM 1/30/97

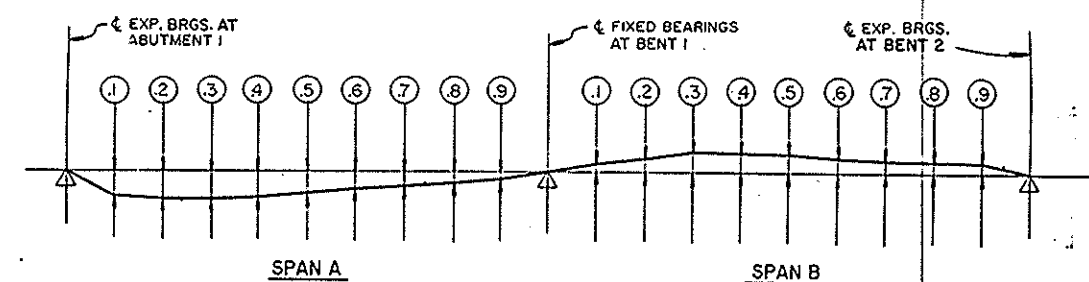
REVISION NO. 2: REVISED INCORRECT NUMBERS IN REVISION NO. 1.

BY: NMW 2/26/97
CHECKED BY: CCM 2/27/97



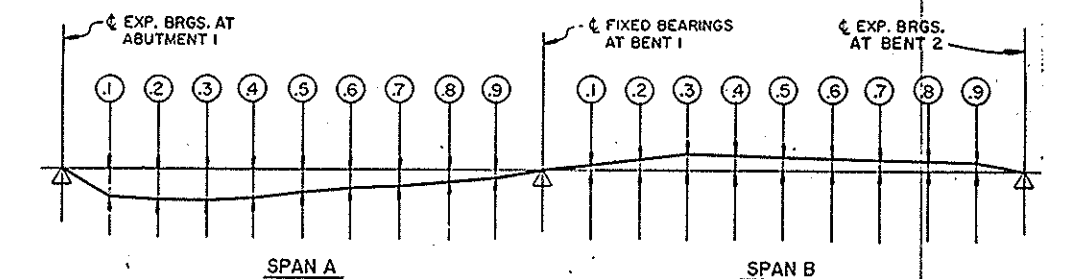
SCHEMATIC CAMBER ORDINATES - GIRDER 1

NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 2

NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 3

NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
DEAD LOAD DEFLECTIONS
CONTINUOUS UNIT # 1
MAY 1995

REVISIONS:						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1	NMW	1/30/97	3			5-36
2	CCM	2/27/97	4			224



PREPARED BY:
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED

DRAWN BY: BZL
CHECKED BY: CCM
DATE: 5-95
DATE: 6/95

GIRDER 4

	EXPANSION BEARINGS AT ABUTMENT 1									FIXED BEARINGS AT BENT 1								
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.004	.007	.009	.010	.009	.008	.005	.003	.001	.001	.004	.007	.010	.012	.012	.011	.008	.005
DEFLECTION DUE TO WEIGHT OF SLAB	.020	.036	.047	.051	.048	.039	.027	.013	.003	.006	.018	.033	.047	.057	.059	.054	.042	.023
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.001	.003	.003	.004	.003	.003	.002	.001	0	0	.001	.003	.004	.004	.004	.004	.003	.002
TOTAL DEAD LOAD DEFLECTION	.025	.046	.059	.065	.060	.050	.034	.017	.004	.007	.023	.043	.061	.073	.075	.069	.053	.030
VERTICAL CURVE ORDINATE	-.061	-.099	-.116	-.110	-.094	-.077	-.061	-.044	-.028	.022	.044	.066	.082	.069	.056	.043	.030	.017
SUPERELEVATION ORDINATE	-.123	-.114	-.102	-.091	-.075	-.060	-.045	-.030	-.015	.002	.004	.007	.010	.021	.031	.042	.053	.065
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)	.010	.019	.025	.027	.026	.021	.014	.007	.001	.001	.001	.002	-.029	-.035	-.036	-.033	-.025	-.014
REQUIRED CAMBER	$-2\frac{1}{16}$	$-2\frac{5}{16}$	$-2\frac{5}{16}$	$-2\frac{1}{16}$	$-1\frac{11}{16}$	$-1\frac{3}{8}$	$-1\frac{1}{8}$	$-1\frac{13}{16}$	$-1\frac{1}{2}$	$\frac{5}{16}$	$\frac{9}{16}$	$\frac{3}{16}$	$\frac{3}{4}$	$\frac{11}{16}$	$\frac{5}{8}$	$\frac{5}{8}$	$\frac{11}{16}$	$\frac{13}{16}$

GIRDER 5

	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.004	.007	.010	.011	.010	.008	.006	.003	.001	.001	.004	.007	.009	.011	.012	.011	.008	.004
DEFLECTION DUE TO WEIGHT OF SLAB	.028	.052	.068	.074	.070	.057	.039	.020	.005	.008	.024	.046	.066	.079	.083	.076	.058	.005
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.001	.002	.003	.003	.003	.003	.002	.001	0	.001	.002	.003	.003	.003	.003	.002	.001	0
TOTAL DEAD LOAD DEFLECTION	.033	.061	.081	.088	.083	.068	.047	.024	.006	.010	.030	.056	.078	.093	.098	.089	.067	.009
VERTICAL CURVE ORDINATE	-.060	-.098	-.113	-.106	-.089	-.072	-.055	-.037	-.020	.006	.011	.016	.021	.018	.015	.013	.010	.006
SUPERELEVATION ORDINATE	-.123	-.113	-.103	-.092	-.076	-.060	-.045	-.031	-.016	.001	.004	.006	.009	.021	.032	.042	.053	.066
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)	.011	.020	.025	.028	.026	.021	.014	.007	.001	0	0	.001	-.038	-.045	-.047	-.043	-.032	-.004
REQUIRED CAMBER	$-2\frac{1}{16}$	$-2\frac{5}{16}$	$-2\frac{5}{16}$	$-2\frac{1}{16}$	$-1\frac{11}{16}$	$-1\frac{5}{16}$	$-1\frac{1}{16}$	$-\frac{3}{4}$	$-\frac{7}{16}$	$\frac{1}{16}$	$\frac{1}{8}$	$\frac{1}{4}$	$-\frac{1}{16}$	$-\frac{1}{16}$	0	$\frac{1}{8}$	$\frac{3}{8}$	$\frac{13}{16}$

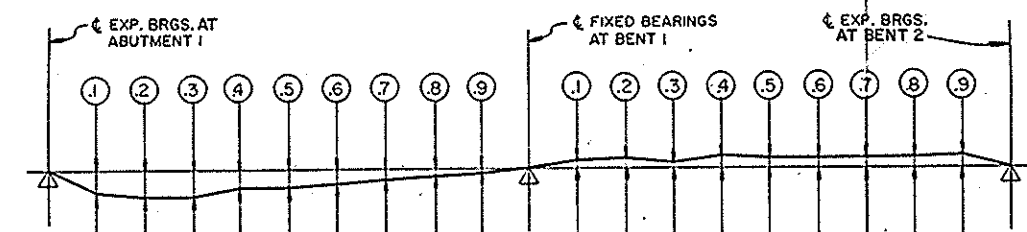
GIRDER 6

	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.004	.008	.011	.012	.011	.009	.006	.003	.001	.001	.003	.007	.009	.011	.012	.011	.008	.005
DEFLECTION DUE TO WEIGHT OF SLAB	.031	.057	.075	.081	.077	.063	.043	.022	.006	.007	.024	.045	.066	.080	.084	.077	.059	.032
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.001	.003	.003	.004	.003	.003	.02	.001	0	0	.001	.002	.003	.004	.004	.003	.003	.001
TOTAL DEAD LOAD DEFLECTION	.036	.068	.089	.097	.091	.075	.051	.026	.007	.008	.028	.054	.078	.095	.100	.091	.070	.038
VERTICAL CURVE ORDINATE	-.059	-.096	-.111	-.103	-.085	-.067	-.049	-.031	-.012	0	.001	.001	.002	.002	.003	.002	.003	.003
SUPERELEVATION ORDINATE	-.123	-.114	-.102	-.091	-.076	-.060	-.045	-.029	-.016	.002	.004	.006	.009	.020	.030	.043	.052	.065
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)	.012	.021	.028	.030	.029	.024	.007	.009	.001	.001	.001	.002	-.038	-.046	-.048	-.044	-.034	-.019
REQUIRED CAMBER	$-2\frac{1}{16}$	$-2\frac{1}{4}$	$-2\frac{1}{4}$	$-1\frac{15}{16}$	$-1\frac{9}{16}$	$-1\frac{1}{2}$	$-1\frac{1}{16}$	$-\frac{5}{8}$	$-\frac{5}{16}$	$\frac{1}{16}$	$\frac{1}{16}$	$\frac{1}{8}$	$-\frac{3}{16}$	$-\frac{5}{16}$	$-\frac{3}{16}$	$\frac{1}{16}$	$\frac{1}{4}$	$\frac{9}{16}$

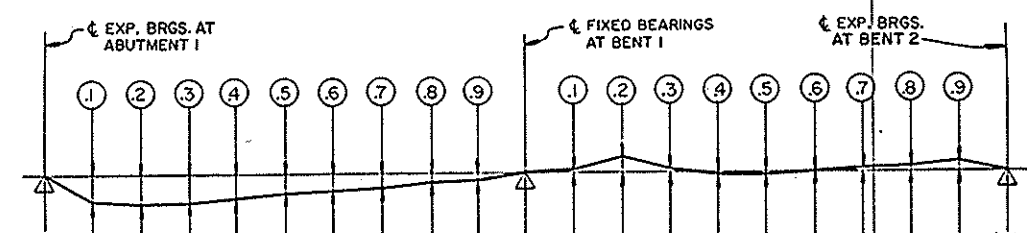
NOTE: DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS GIVEN IN FRACTIONS OF AN INCH.

DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS

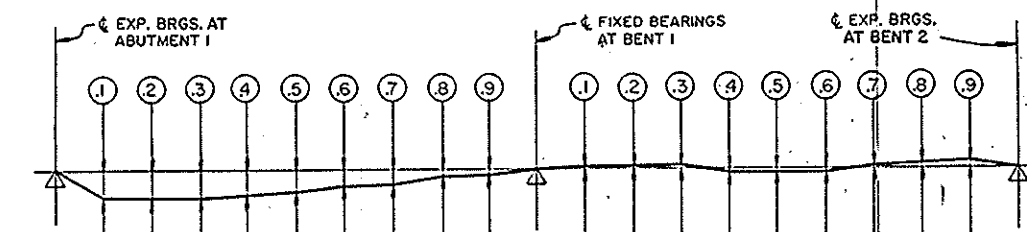
EXPANSION BEARINGS AT BENT 2



SCHEMATIC CAMBER ORDINATES - GIRDER 4
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 5
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 6
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.

PROJECT NO. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 2 OF 3

REVISION NO. 2: REVISED INCORRECT NUMBERS IN REVISION NO. 1.
BY: NMW 2/26/97
CHECKED BY: CCM 2/27/97

REVISION NO. 1: REVISED TO CHANGE "CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING" & "REQUIRED CAMBER" FOR GIRDERS 4, 5 & 6; AND TO CHANGE "SCHEMATIC CAMBER ORDINATES - GIRDERS 4, 5 & 6".

BY: NMW 1/30/97
CHECKED BY: CCM 1/30/97



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
DEAD LOAD DEFLECTIONS
CONTINUOUS UNIT #1
MAY 1995

REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1	NMW	1/30/97	3		
2	CCM	2/27/97	4		
				TOTAL SHEETS	224

PREPARED BY:
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED

DRAWN BY: RTJ
CHECKED BY: CCM
DATE: 5-95
DATE: 6/95

GIRDER 7	EXPANSION BEARINGS AT ABUTMENT 1									FIXED BEARINGS AT BENT 1									SPAN B								
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9									
DEFLECTION DUE TO WEIGHT OF STEEL	.005	.009	.011	.012	.012	.010	.007	.004	.001	.001	.003	.006	.009	.011	.012	.011	.008	.005									
DEFLECTION DUE TO WEIGHT OF SLAB	.031	.058	.075	.082	.078	.065	.045	.023	.007	.006	.022	.042	.061	.074	.079	.072	.055	.030									
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.002	.003	.004	.004	.004	.003	.002	.001	0	0	.001	.002	.003	.004	.004	.003	.003	.001									
TOTAL DEAD LOAD DEFLECTION	.038	.070	.090	.098	.094	.078	.054	.028	.008	.007	.026	.050	.073	.089	.095	.086	.066	.036									
VERTICAL CURVE ORDINATE	-.058	-.094	-.108	-.100	-.081	-.062	-.043	-.024	-.005	0	.001	.001	.002	.003	.003	.002	.003	.003									
SUPERELEVATION ORDINATE	-.124	-.114	-.103	-.091	-.076	-.060	-.046	-.030	-.016	.002	.003	.005	.008	.018	.029	.041	.051	.062									
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)  	.012	.021	.028	.031	.029	.024	.017	.009	.001	.001	.001	.002	-.035	-.043	-.046	-.042	-.032	-.018									
REQUIRED CAMBER  	$-2\frac{1}{16}$	$-2\frac{1}{8}$	$-2\frac{3}{16}$	$-1\frac{15}{16}$	$-1\frac{1}{2}$	$-1\frac{3}{16}$	$-\frac{7}{8}$	$-\frac{1}{2}$	$-\frac{1}{4}$	$\frac{1}{16}$	$\frac{1}{16}$	$\frac{1}{8}$	$\frac{5}{16}$	$\frac{1}{4}$	$\frac{3}{16}$	$\frac{1}{16}$	$\frac{1}{4}$	$\frac{9}{16}$									

GIRDER 8	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.005	.009	.012	.014	.013	.012	.007	.004	.001	.001	.003	.006	.009	.011	.012	.011	.008	.005
DEFLECTION DUE TO WEIGHT OF SLAB	.034	.063	.082	.090	.086	.071	.050	.026	.008	.006	.021	.042	.061	.075	.079	.073	.056	.031
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.002	.003	.004	.004	.004	.003	.002	.001	0	0	.001	.002	.003	.004	.004	.003	.003	.001
TOTAL DEAD LOAD DEFLECTION	.041	.075	.098	.108	.103	.086	.059	.031	.009	.007	.025	.050	.073	.090	.095	.087	.067	.037
VERTICAL CURVE ORDINATE	-.057	-.092	-.106	-.096	-.076	-.057	-.037	-.017	.003	0	.001	.002	.002	.003	.004	.002	.003	.003
SUPERELEVATION ORDINATE	-.124	-.114	-.102	-.091	-.076	-.060	-.045	-.030	-.015	.003	.003	.005	.008	.018	.027	.040	.049	.061
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS)  	.012	.022	.028	.031	.029	.024	.017	.010	.001	0	.001	.001	-.035	-.043	-.046	-.042	-.032	-.018
REQUIRED CAMBER  	$-2\frac{1}{16}$	$-2\frac{3}{16}$	$-2\frac{3}{16}$	$-1\frac{7}{8}$	$-1\frac{1}{2}$	$-1\frac{1}{8}$	$-\frac{3}{4}$	$-\frac{7}{16}$	$-\frac{1}{8}$	$\frac{1}{16}$	$\frac{1}{16}$	$\frac{1}{8}$	$-\frac{5}{16}$	$-\frac{1}{4}$	$-\frac{3}{16}$	0	$\frac{1}{4}$	$\frac{9}{16}$

GIRDER 9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.006	.010	.013	.015	.014	.012	.008	.004	.001	.001	.003	.006	.009	.011	.012	.011	.009	.005
DEFLECTION DUE TO WEIGHT OF SLAB	.029	.054	.070	.077	.073	.061	.043	.023	.007	.004	.016	.032	.048	.059	.063	.058	.045	.024
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & SIDEWALK	.002	.004	.005	.005	.005	.004	.003	.002	.001	0	.001	.002	.003	.004	.004	.004	.003	.002
TOTAL DEAD LOAD DEFLECTION	.037	.068	.088	.097	.092	.077	.054	.029	.009	.005	.020	.040	.060	.074	.079	.073	.057	.031
VERTICAL CURVE ORDINATE	-.057	-.091	-.103	-.093	-.072	-.051	-.031	-.010	-.012	0	.001	.002	.003	.004	.004	.001	.002	.003
SUPERELEVATION ORDINATE	-.123	-.114	-.102	-.091	-.076	-.061	-.046	-.030	-.017	.002	.003	.004	.006	.016	.026	.039	.048	.058
CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING (CURVED GIRDERS) 	.012	.022	.029	.032	.031	.025	.018	.010	.001	.001	.001	.002	-.029	-.035	-.038	-.035	-.027	-.015
REQUIRED CAMBER 	$-2\frac{1}{16}$	$-2\frac{3}{16}$	$-2\frac{1}{8}$	$-1\frac{13}{16}$	$-1\frac{3}{8}$	$-1\frac{1}{16}$	$-\frac{11}{16}$	$-\frac{3}{8}$	$-\frac{5}{16}$	$\frac{1}{16}$	$\frac{1}{16}$	$\frac{1}{8}$	$-\frac{1}{4}$	$-\frac{3}{16}$	$-\frac{1}{8}$	$\frac{1}{16}$	$\frac{1}{4}$	$\frac{9}{16}$

NOTE:
DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN
DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS
GIVEN IN FRACTIONS OF AN INCH.

DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS

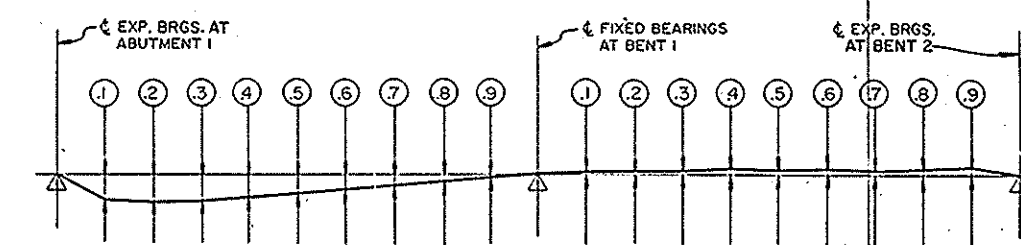
DRAWN BY: RTJ
CHECKED BY: CCM
DATE: 2-95
DATE: 6/95

REVISION NO. 1: REVISED TO CHANGE "CAMBER DUE TO DISSIPATION RESULTING FROM HEAT CURVING" & "REQUIRED CAMBER" FOR GIRDERS 7, 8 & 9; AND TO CHANGE "SCHEMATIC CAMBER ORDINATES - GIRDERS 7, 8 & 9".
BY: NMW 1/30/97
CHECKED BY: CCM 1/30/97

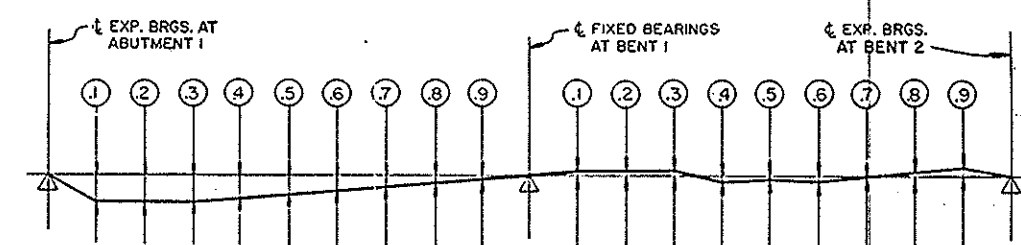
REVISION NO. 2: REVISED INCORRECT NUMBERS IN REVISION NO. 1.
BY: NMW 2/26/97
CHECKED BY: CCM 2/27/97



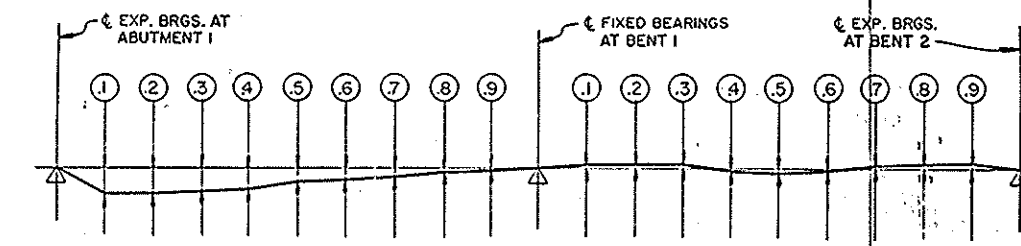
U1.024
PREPARED BY:
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED



SCHEMATIC CAMBER ORDINATES - GIRDER 7
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 8
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



SCHEMATIC CAMBER ORDINATES - GIRDER 9
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL DEAD LOAD DEFLECTIONS CONTINUOUS UNIT # 1 MAY 1995					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1	NMW	1/30/97	3		
2	CCM	2/27/97	4		
SHEET NO. 5-38					TOTAL SHEETS 224

EPOXY-COATED REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE I CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1001	362	#6	STR.	58'-4"	31,717
A5400	362	#6	STR.	5'-0"	2,719
B1000	428	#5	STR.	36'-0"	16,071
B5005	213	#5	STR.	60'-0"	13,330
B5006	426	#5	STR.	7'-6"	3,332
K2000	6	#5	2	9'-11"	62
K2001	6	#5	2	8'-3"	52
K2002	24	#5	3	14'-10"	371
S5000	100	#4	1	4'-1"	273
S5010	2	#4	7	5'-9"	8
G1000	2	#4	STR.	58'-4"	122
H001	4	#6	6	14'-2"	85
J1000	59	#4	4	1'-5"	56
U1	270	#5	5	4'-6"	1,267
V13	20	#5	8	5'-7"	116
TOTAL					69,581

EPOXY-COATED REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE II CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1500	362	#6	STR.	25'-4"	13,774
A5400	362	#6	STR.	5'-0"	2,719
B1001	188	#5	STR.	32'-6"	6,373
B5005	93	#5	STR.	60'-0"	5,820
B5006	186	#5	STR.	7'-6"	1,455
K2000	6	#5	2	9'-11"	62
K2001	6	#5	2	8'-3"	52
K2002	6	#5	3	14'-10"	93
G2000	2	#5	STR.	25'-4"	53
H001	4	#6	6	14'-2"	85
S5000	40	#4	1	4'-1"	109
S5010	2	#4	7	5'-9"	8
J1000	26	#4	4	1'-5"	25
U2	245	#5	5	3'-6"	894
V15	10	#5	8	5'-11"	62
V16	10	#5	8	5'-10"	61
TOTAL					31,645

REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE I CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A2001	362	#6	STR.	58'-4"	31,717
A6400	362	#6	STR.	4'-7"	2,492
B2000	376	#5	STR.	35'-7"	13,955
B6505	182	#5	STR.	60'-0"	11,390
B6506	364	#5	STR.	7'-2"	2,721
TOTAL					62,275

REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE II CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A2500	362	#6	STR.	25'-4"	13,774
A6400	362	#6	STR.	4'-7"	2,492
B2001	164	#5	STR.	32'-3"	5,516
B6505	78	#5	STR.	60'-0"	4,881
B6506	156	#5	STR.	7'-2"	1,166
TOTAL					27,829

CLASS AA CONCRETE SUMMARY (BY STAGE)		
STAGE I (DECK SLAB - POUR 1)	108.7 CY	
(DECK SLAB - POUR 2)	267.2 CY	
STAGE II (DECK SLAB)	166.8 CY	
STAGE II (SIDEWALK)	21.5 CY	
STAGE III (CLOSURE POUR)	15.6 CY	
STAGE IV (SIDEWALK)	17.2 CY	
TOTAL	597.0 CY	

CLASSIC BRIDGE RAIL QUANTITY SUMMARY (BY STAGE)			
	EPOXY - COATED REINFORCING STEEL (LBS)	CLASS AA CONCRETE (CY)	LINEAR FOOTAGE (LF)
STAGE I	3,871	28.2	203.29
STAGE II	3,541	20.2	184.57
STAGE III	N/A	N/A	N/A
STAGE IV	N/A	N/A	N/A
TOTALS	7,412	48.4	387.86
CLASSIC BRIDGE RAIL			387.86 LF

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS					
BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	DEVELOPMENT LENGTH	WELDED	DEVELOPMENT LENGTH	WELDED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#8	6'-0"	4'-0"	6'-0"	4'-0"	6'-10"

GROOVING BRIDGE FLOORS SUMMARY (BY STAGE)				
DECK SLAB	STAGE			TOTAL
	I	II	III	
	10,047	3,597	380	14,024
GROOVING BRIDGE FLOORS (UNIT #1 ONLY) = 14,024 SF				

CONTINUOUS UNIT #1 SUPERSTRUCTURE QUANTITIES	
REINFORCED CONCRETE DECK SLAB	16,636 SF
GROOVING BRIDGE FLOORS	14,024 SF
STRUCTURAL STEEL*	APPROX. 455,700 LBS
CLASSIC BRIDGE RAIL	387.86 LF
POT BEARINGS	LS
EXPANSION JOINT SEALS	LS
MODULAR EXPANSION JOINT SEALS	LS

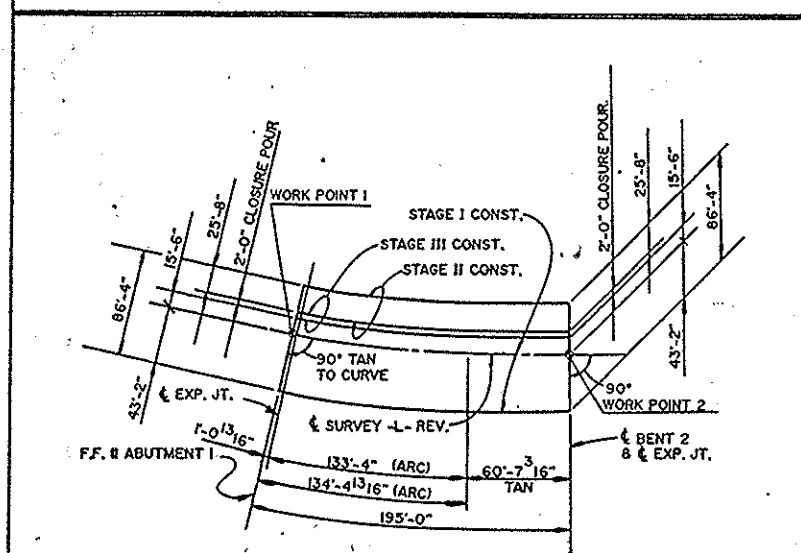
STRUCTURAL STEEL QUANTITY BREAKDOWN*	
STAGE I CONSTRUCTION	APPROX. 314,500 LBS.
STAGE II CONSTRUCTION	APPROX. 141,200 LBS.
TOTAL (UNIT #1 ONLY)	APPROX. 455,700 LBS.

EPOXY-COATED REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE III CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1000	16	#5	STR.	32'-8"	545
B5005	7	#5	STR.	60'-0"	438
B5006	14	#5	STR.	7'-6"	110
S5001	8	#4	6	8'-3"	44
S5002	8	#4	6	10'-0"	53
S5003	20	#4	6	6'-0"	80
TOTAL					1,270

EPOXY-COATED REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE IV CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B7004	40	#4	STR.	27'-3"	728
G5500	203	#4	STR.	4'-6"	610
D100	812	#4	STR.	0'-8"	362
J1000	2	#4	5	1'-5"	2
TOTAL					1,702

REINFORCING STEEL SCHEDULE FOR DECK SLAB CONTINUOUS UNIT #1					
STAGE III CONSTRUCTION					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B2000	16	#5	STR.	32'-4"	540
B6505	7	#5	STR.	60'-0"	438
B6506	14	#5	STR.	7'-2"	105
TOTAL					1,083

LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB - CONTINUOUS UNIT #1



SUMMARY	
STAGE I	11,377 SF
STAGE II	4,878 SF
STAGE III	381 SF
STAGE IV (SIDEWALK)	NA
TOTAL	16,636 SF

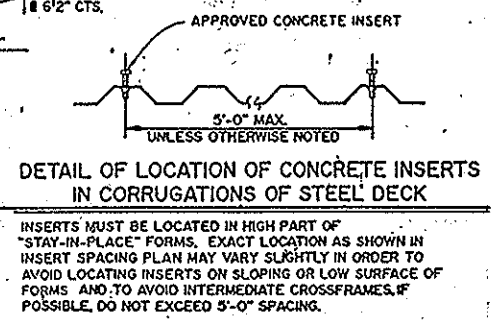
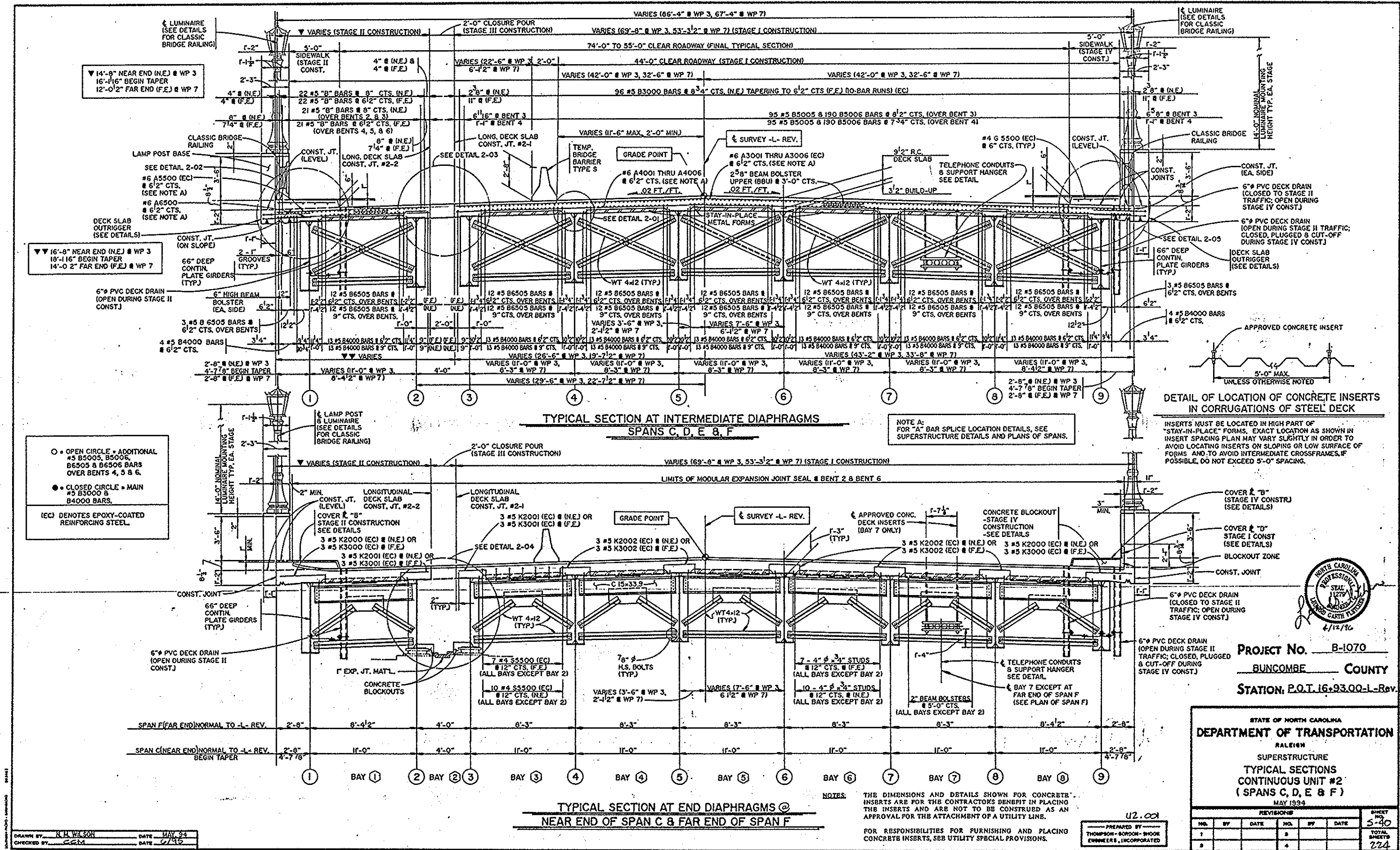


PREPARED BY
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PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
REINFORCING STEEL SCHEDULES & SUMMARIES OF QUANTITIES FOR CONTINUOUS UNIT #1					
MARCH 1995					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
SHEET NO. 5-39					TOTAL SHEETS 224

DRAWN BY: N.M. WALSH DATE: MAR 95
CHECKED BY: GCM DATE: 6/9/95



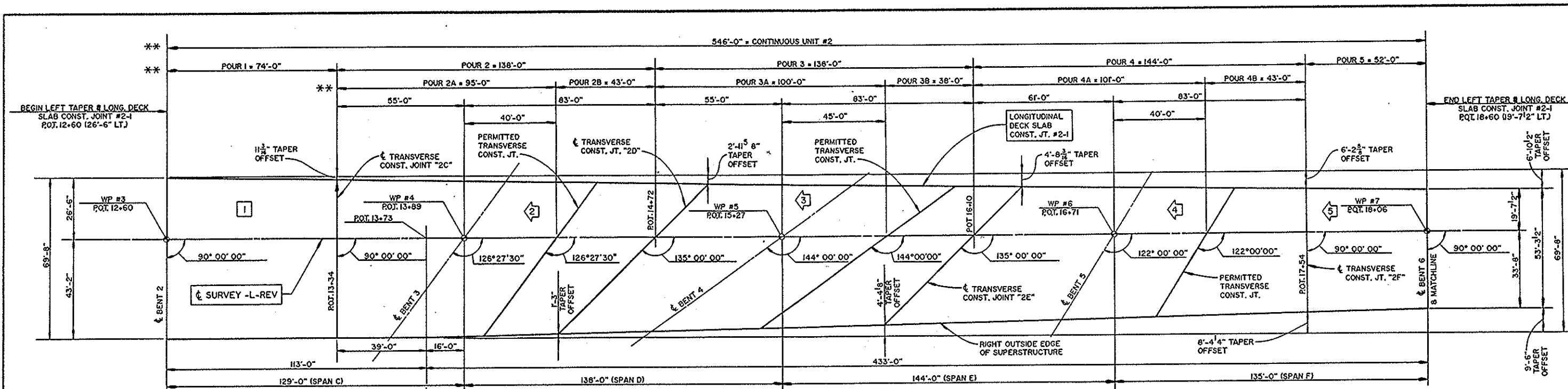
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-1-Rev.

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
TYPICAL SECTIONS
CONTINUOUS UNIT #2
(SPANS C, D, E & F)
MAY 1994

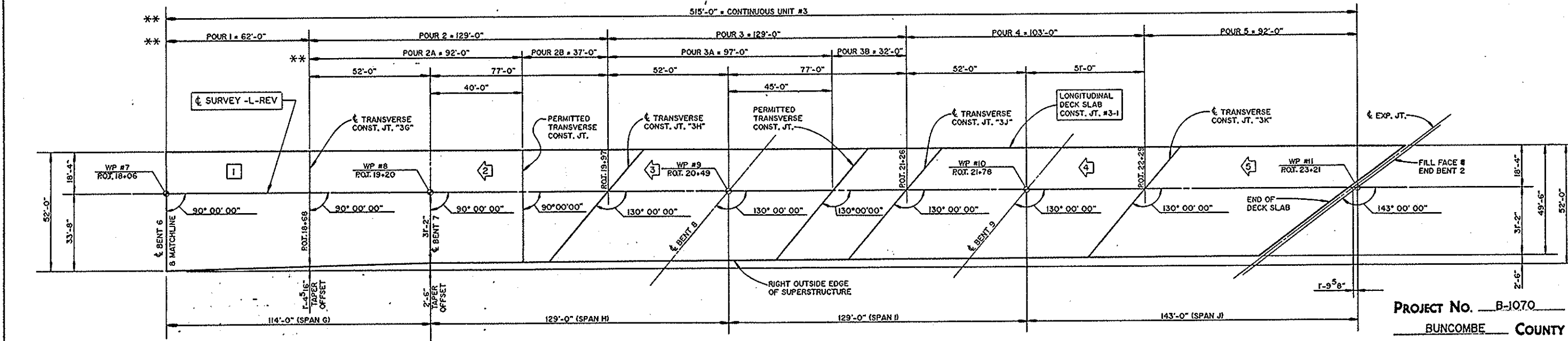
DRAWN BY: N.M. WILSON
CHECKED BY: G.M.
DATE: MAY 94
DATE: 6/94

U2.001
PREPARED BY:
THOMPSON & BROWN-SHOWN
ENGINEERS, INCORPORATED

DRAWN BY: N.M. WILSON DATE: JAN 94
CHECKED BY: COM DATE: 6/95



CONTINUOUS UNIT #2 - STAGE I CONSTRUCTION
DECK SLAB POURING SEQUENCE



CONTINUOUS UNIT #3 - STAGE I CONSTRUCTION
DECK SLAB POURING SEQUENCE

INDICATES POUR NUMBER AND DIRECTION OF POUR (WHERE APPLICABLE)
** MEASURED ALONG & SURVEY -L- REV.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.

SHEET 3 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE DETAILS
DECK SLAB POURING SEQUENCE
CONTINUOUS UNITS #2 & #3
STAGE I CONSTRUCTION
MAY 1994



U7.004
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ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-43
2			4			224

DRAWN BY: N. M. WILSON
CHECKED BY: [Signature]
DATE: MAY 94

PROVIDE CONTINUOUS HIGH CHAIR FOR METAL DECK (CHCM) @ 4'-0" CTS. WITH LEG SPACING TO MATCH THE PITCH OF THE FORMS AND WITH A HEIGHT TO SUPPORT THE BOTTOM LAYER OF SLAB REINFORCEMENT A CLEAR DISTANCE OF 1-1/4" ABOVE THE TOP OF THE STAY-IN-PLACE FORM.

THE MODULAR EXPANSION JOINT SEAL AT BENT 2 SHALL BE CAPABLE OF HANDLING A TOTAL THERMAL MOVEMENT, MEASURED PARALLEL TO THE CENTERLINE OF ROADWAY, OF 4-1/4" (2-1/8" EXPANSION AND 2-1/8" CONTRACTION FROM A MIDPOINT TEMPERATURE OF 60° F).

THE TOPS OF THE MODULAR EXPANSION JOINT SEALS SHALL BE SET A MINIMUM OF 1/8" AND A MAXIMUM OF 1/4" BELOW THE TOP OF SLAB.

CLASSIC BRIDGE RAILS AND SIDEWALKS IN THE CONTINUOUS UNIT SHALL NOT BE CAST UNTIL THE DECK SLAB CONCRETE IN EACH STAGE OF CONSTRUCTION HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

STAY-IN-PLACE FORMS SHALL HAVE CLOSED TAPERED ENDS.

STRUCTURAL STEEL ERECTION IN EACH STAGE OF CONSTRUCTION FOR THE CONTINUOUS UNIT SHALL BE COMPLETE BEFORE FALSEWORK OR FORMS ARE PLACED IN EACH STAGE ON THE UNIT.

SEE THE TRAFFIC CONTROL PLANS FOR STAGE I, II, III & IV
LOCATIONS AND PAY LIMITS OF THE TEMPORARY BRIDGE BARRIERS
- TYPE S.

FALSEWORK AND METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TESTS. SEE STRUCTURAL STEEL GIRDER ELEVATION SHEETS.

THE DIMENSIONS AND DETAILS SHOWN FOR THE CONCRETE DECK
INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM
AND SHOULD NOT BE CONSTRUED TO BE AN APPROVAL FOR THE
ATTACHMENT OF THE UTILITY TO THE STRUCTURE.

DIRECTION OF CASTING DECK CONCRETE SHALL BE AS SHOWN ON THE DECK SLAB POURING SEQUENCE SHEET FOR STAGE 1 CONSTRUCTION. DIRECTION OF CASTING DECK CONCRETE IN SUBSEQUENT STAGES SHALL FOLLOW THE SAME PATTERN.

#6 HOOT (EC)

LAMP POST & LUMINAIRE

ROADWAY FACE OF RAIL

#4 S5010 (EC)

2" CL MIN.

OUTSIDE EDGE OF SUPERSTRUCTURE

#5 V16 (EC) AT 12" CTS. (EA. FACE)

3"

2'-3"

2'-3"

4'-6"

DETAIL 2-09
PLAN OF DECK SLAB OUTRIGGER
AT LAMP POST BASE

SEE SECTION G-G - CLASSIC BRIDGE RAILING DETAILS,
SHEET 15 OF 17, FOR ADDITIONAL DETAILS.

DRAWN BY: _____ RTJ _____ DATE: MAY 94
CHECKED BY: _____ CCM _____ DATE: 6/95



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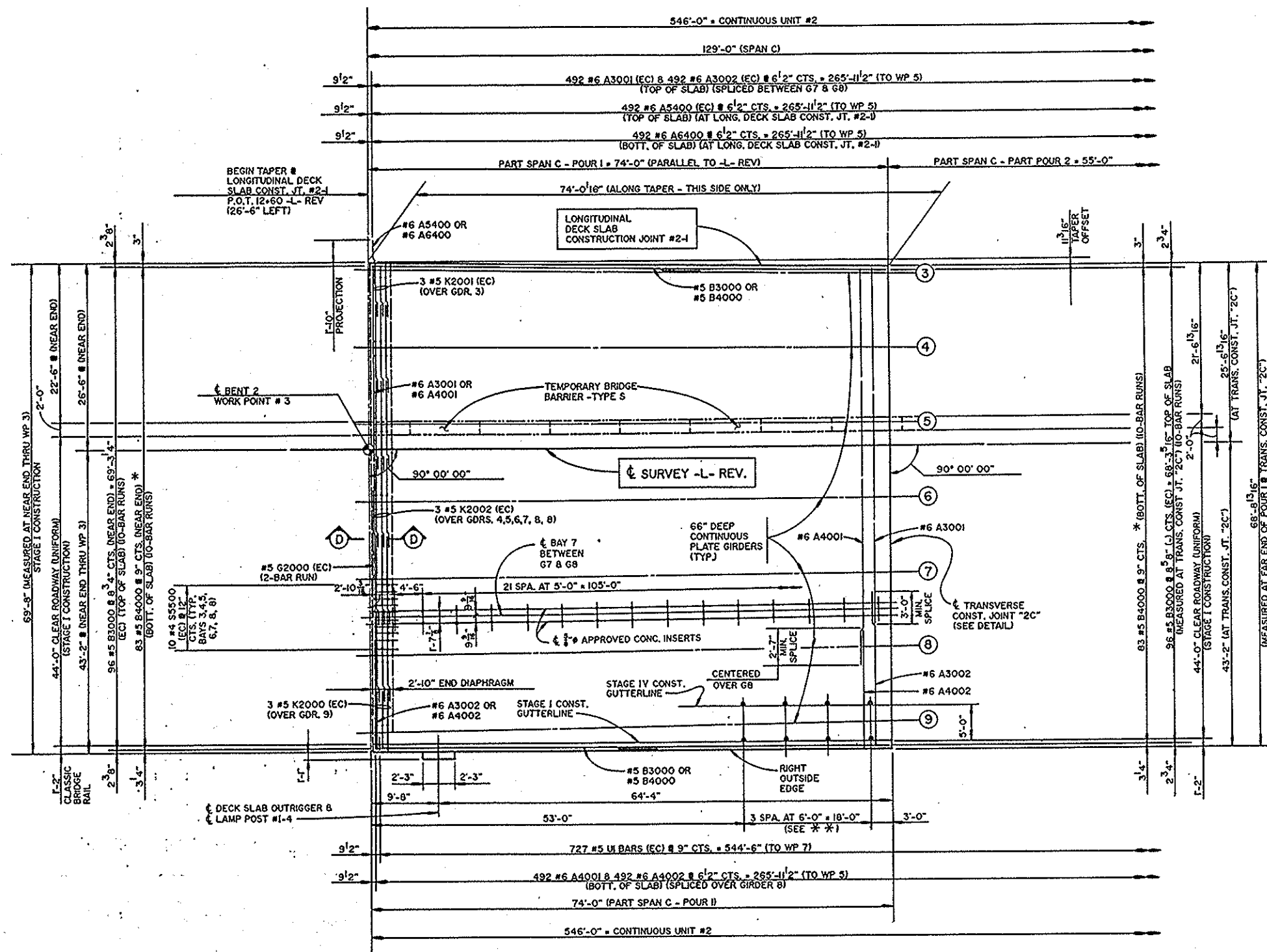
PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 4 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RAILROAD
SUPERSTRUCTURE
DETAILS AND NOTES
(CONTINUOUS UNIT #2)

MAY 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS
2			4			224



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING
 * 4 - 6" PVC DECK DRAINS AT STAGE IV CONST. GUTTERLINE &
 4 - 6" PVC DECK DRAINS AT STAGE I CONST. GUTTERLINE

PART-PLAN OF SPAN C (CONTINUOUS UNIT #2)
 STAGE I CONSTRUCTION
 (POUR I)



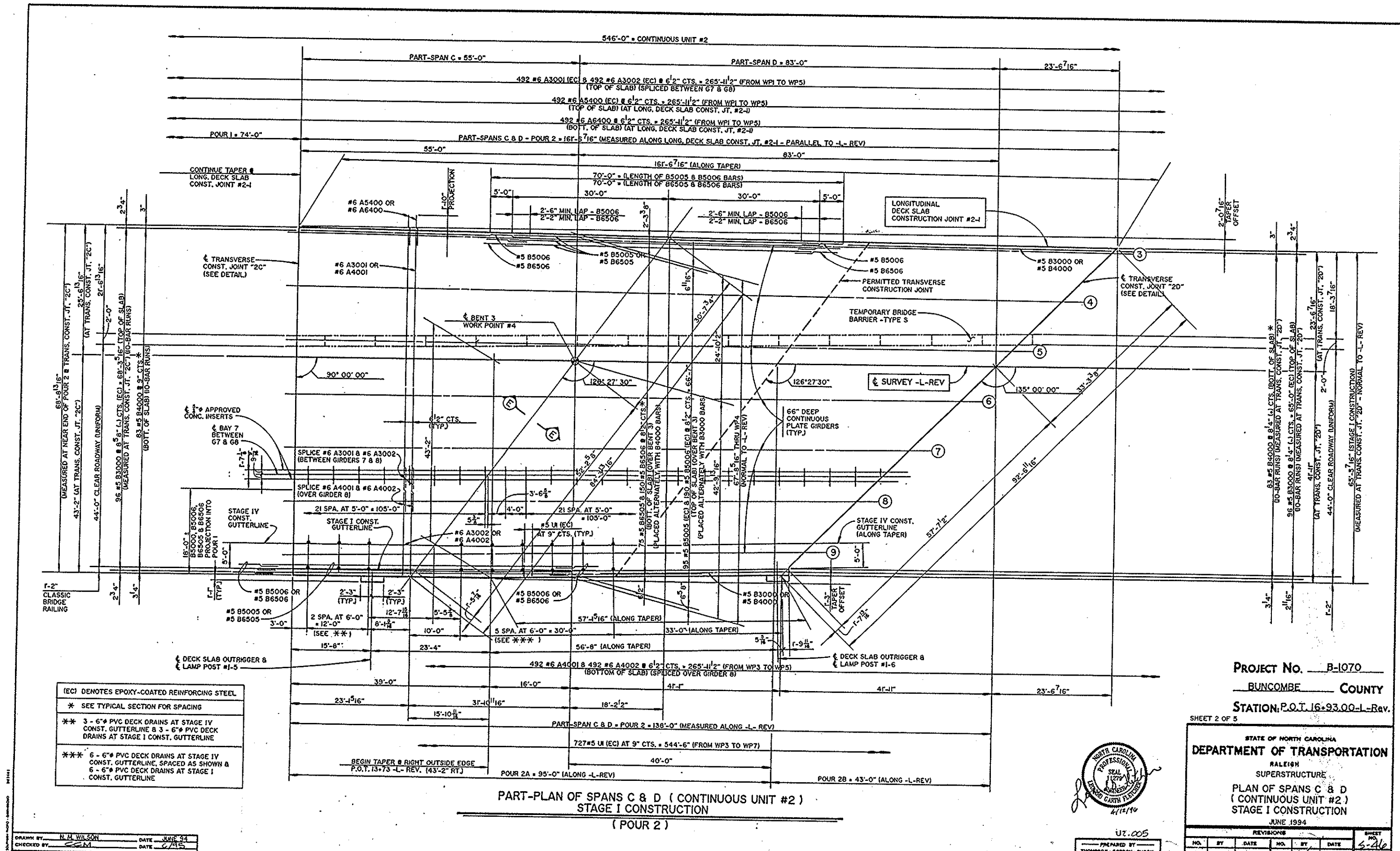
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PROJECT NO. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE PLAN OF SPAN C (CONTINUOUS UNIT #2) STAGE I CONSTRUCTION JUNE 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. 5-45
					TOTAL SHEETS 224

DRAWN BY: N.W. WILSON DATE: JUNE 94
 CHECKED BY: DATE: 6/95

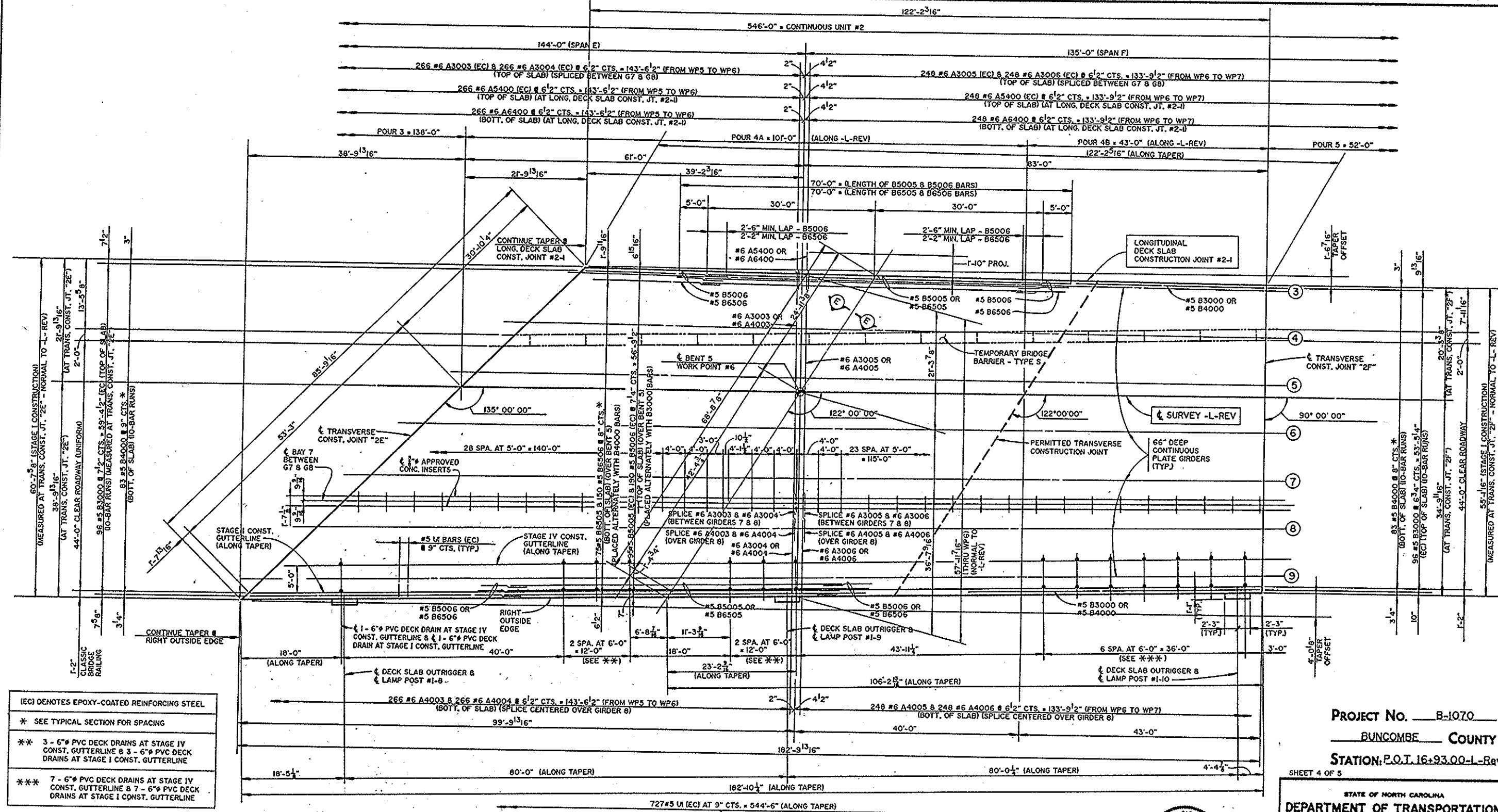


PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16-93.00-1 -Rev.

SHEET 2 OF 5

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPANS C & D
(CONTINUOUS UNIT #2)
STAGE I CONSTRUCTION
JUNE 1994

REVISIONS						SHEET NO. 5-46
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 274
2			4			



(EC) DENOTES EPOXY-COATED REINFORCING STEEL

* SEE TYPICAL SECTION FOR SPACING

** 3 - 6" PVC DECK DRAINS AT STAGE IV CONST. GUTTERLINE & 3 - 6" PVC DECK DRAINS AT STAGE I CONST. GUTTERLINE

*** 7 - 6" PVC DECK DRAINS AT STAGE IV CONST. GUTTERLINE & 7 - 6" PVC DECK DRAINS AT STAGE I CONST. GUTTERLINE

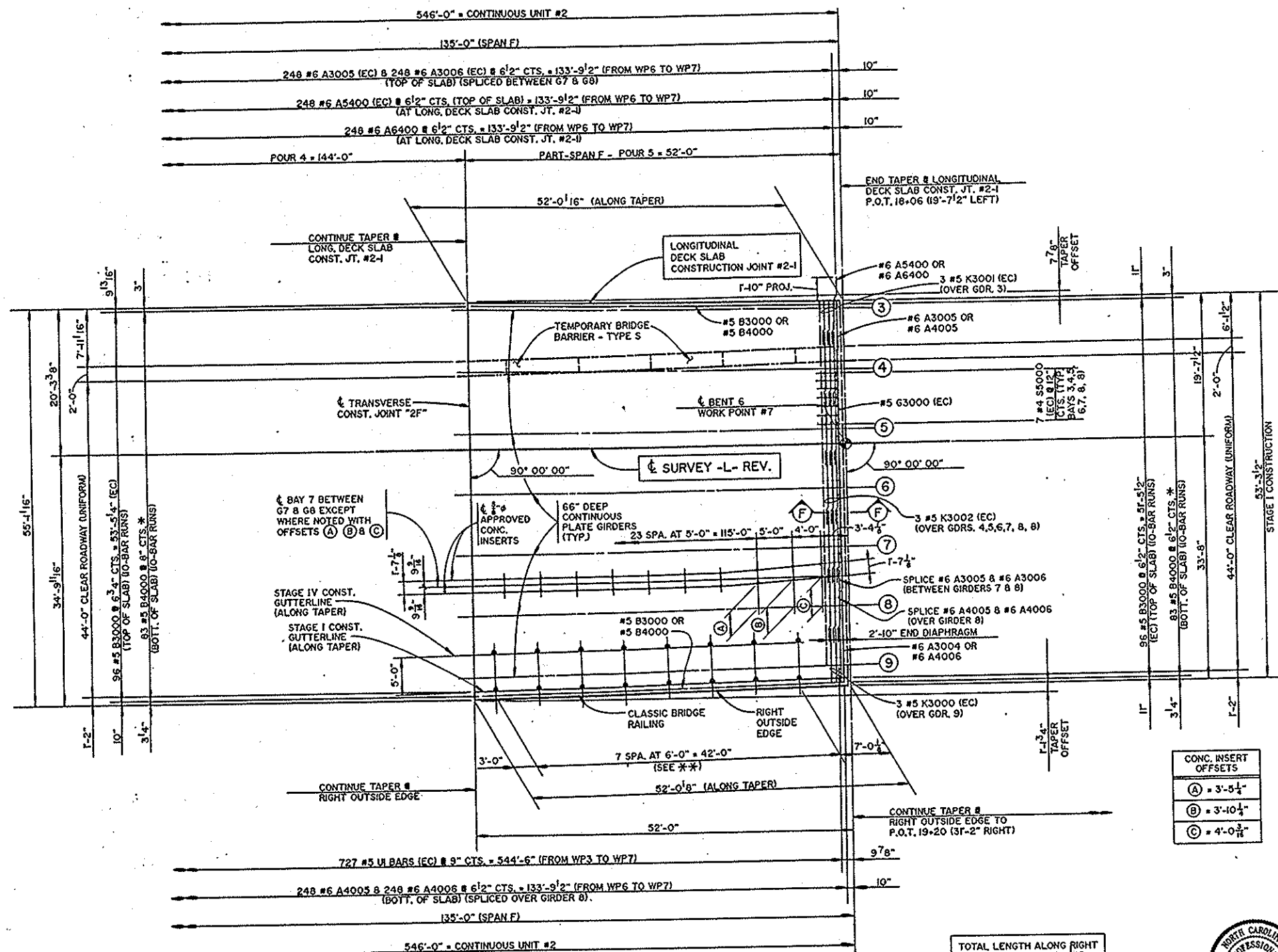
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-Rev.
SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE PLAN OF SPANS E & F (CONTINUOUS UNIT #2) STAGE I CONSTRUCTION JUNE 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
SHEET NO. 5-48					TOTAL SHEETS 224



U2.007
PREPARED BY
THOMPSON-BORDON-SHOCK
ENGINEERS, INCORPORATED

DRAWN BY: N.M. WILSON
CHECKED BY: CCM
DATE: 8/19/94
DATE: 6/7/94



(EC) DENOTES EPOXY-COATED REINFORCING STEEL

* SEE TYPICAL SECTION FOR SPACING

** 8 - 6" PVC DECK DRAINS AT STAGE IV CONST. GUTTERLINE & 8 - 6" PVC DECK DRAINS AT STAGE I CONST. GUTTERLINE

PART-PLAN OF SPAN F (CONTINUOUS UNIT #2)
STAGE I CONSTRUCTION
(POUR 5)

TOTAL LENGTH ALONG RIGHT OUTSIDE EDGE = 546'-14" (ALONG TAPER)

CONC. INSERT OFFSETS	
(A)	3'-5 1/4"
(B)	3'-10 1/4"
(C)	4'-0 3/4"



02.008
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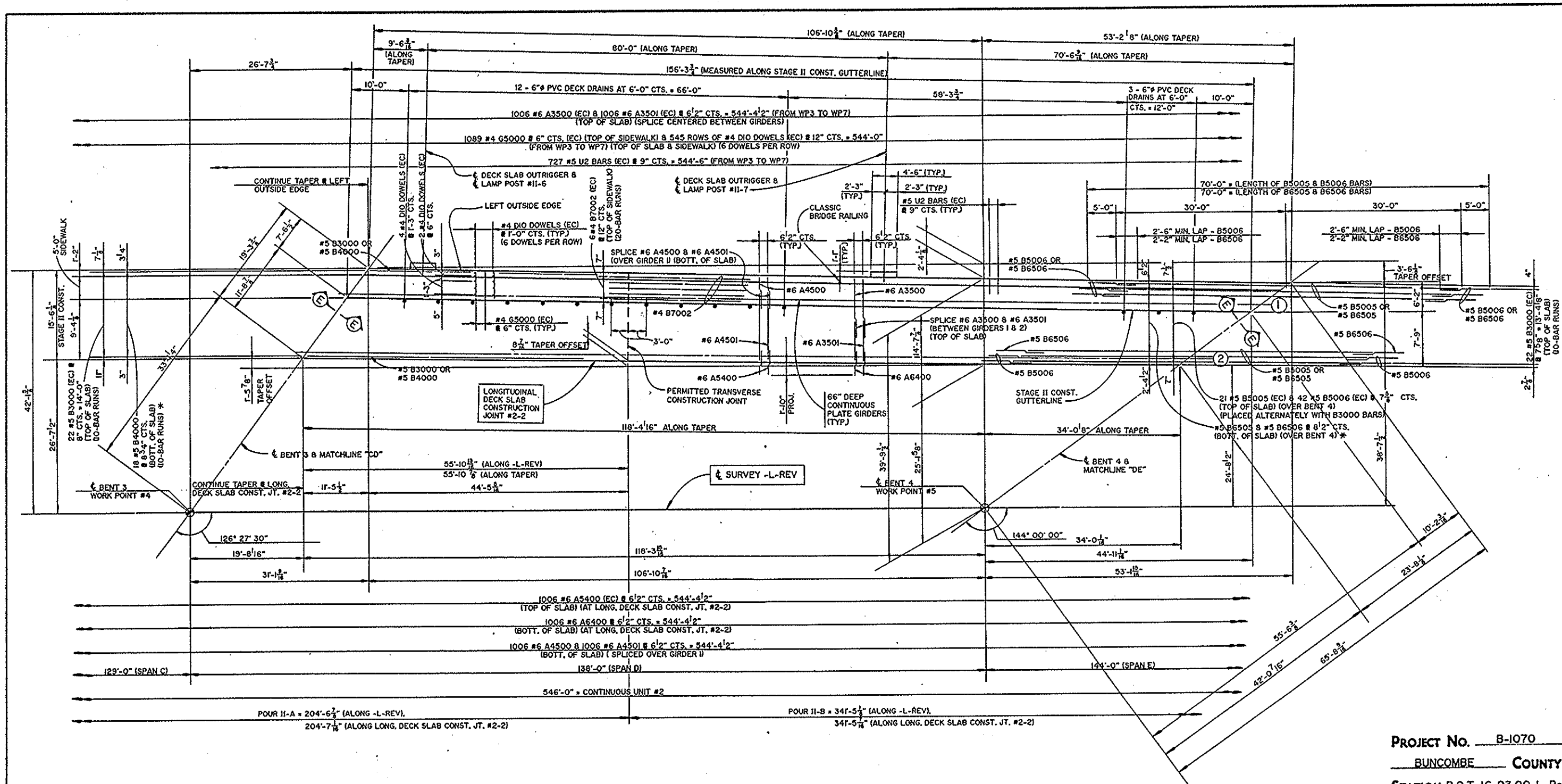
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-1-Rev.

SHEET 5 OF 5

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPAN F
(CONTINUOUS UNIT #2)
STAGE I CONSTRUCTION
JUNE 1994

REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					TOTAL SHEETS
					274

DRAWN BY: N.M. WILSON
CHECKED BY: JEM
DATE: JUNE 94
DATE: 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPAN D (CONTINUOUS UNIT #2)
 STAGE II CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-1-Rev.

SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPAN D
 (CONTINUOUS UNIT #2)
 STAGE II CONSTRUCTION
 JUNE 1994

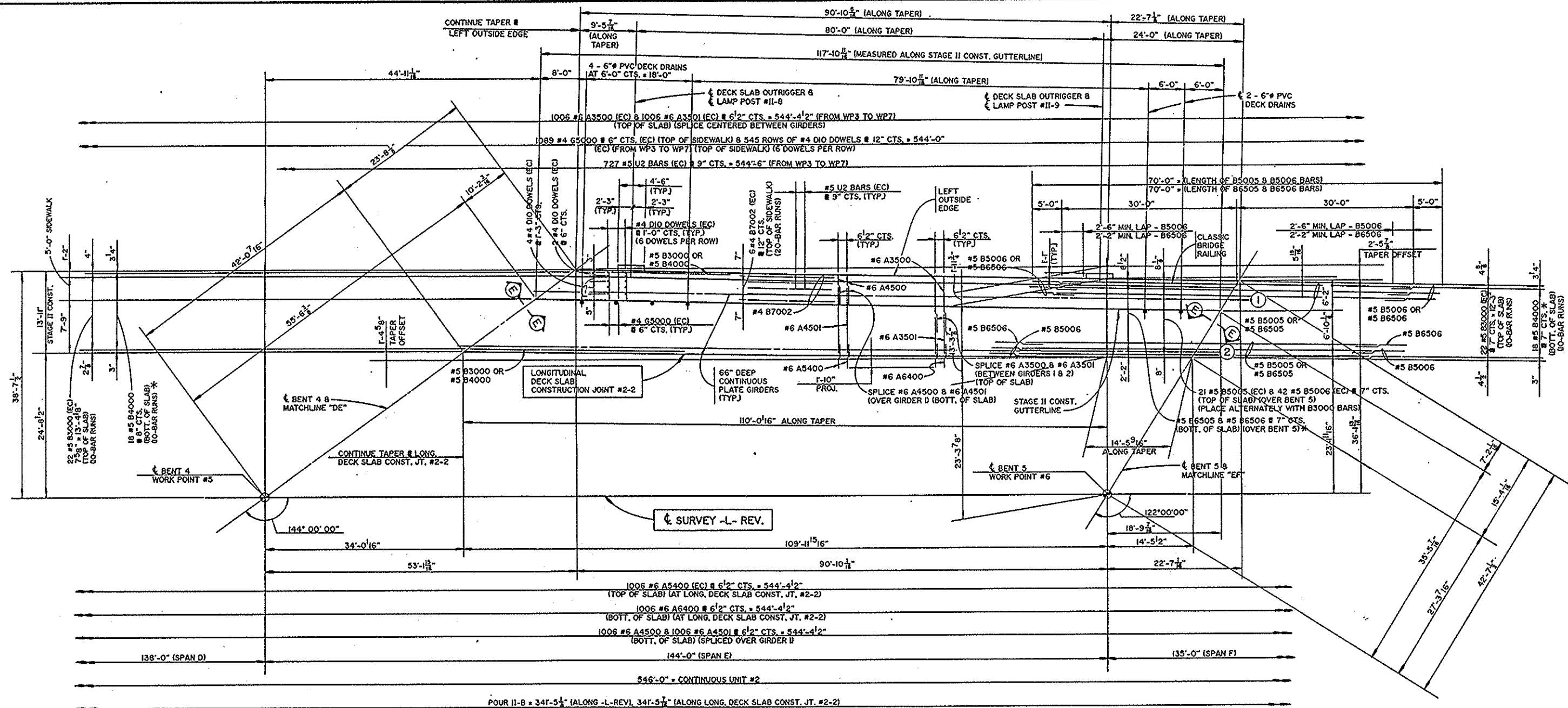


U2.010
 PREPARED BY
 THOMPSON - GORDON - BHOOK
 ENGINEERS, INCORPORATED

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

SHEET NO.
 2-51
 TOTAL SHEETS
 224

DRAWN BY: N.H. WILSON
 CHECKED BY: CCM
 DATE: JUNE 94
 DATE: 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPAN E (CONTINUOUS UNIT #2)
 STAGE II CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

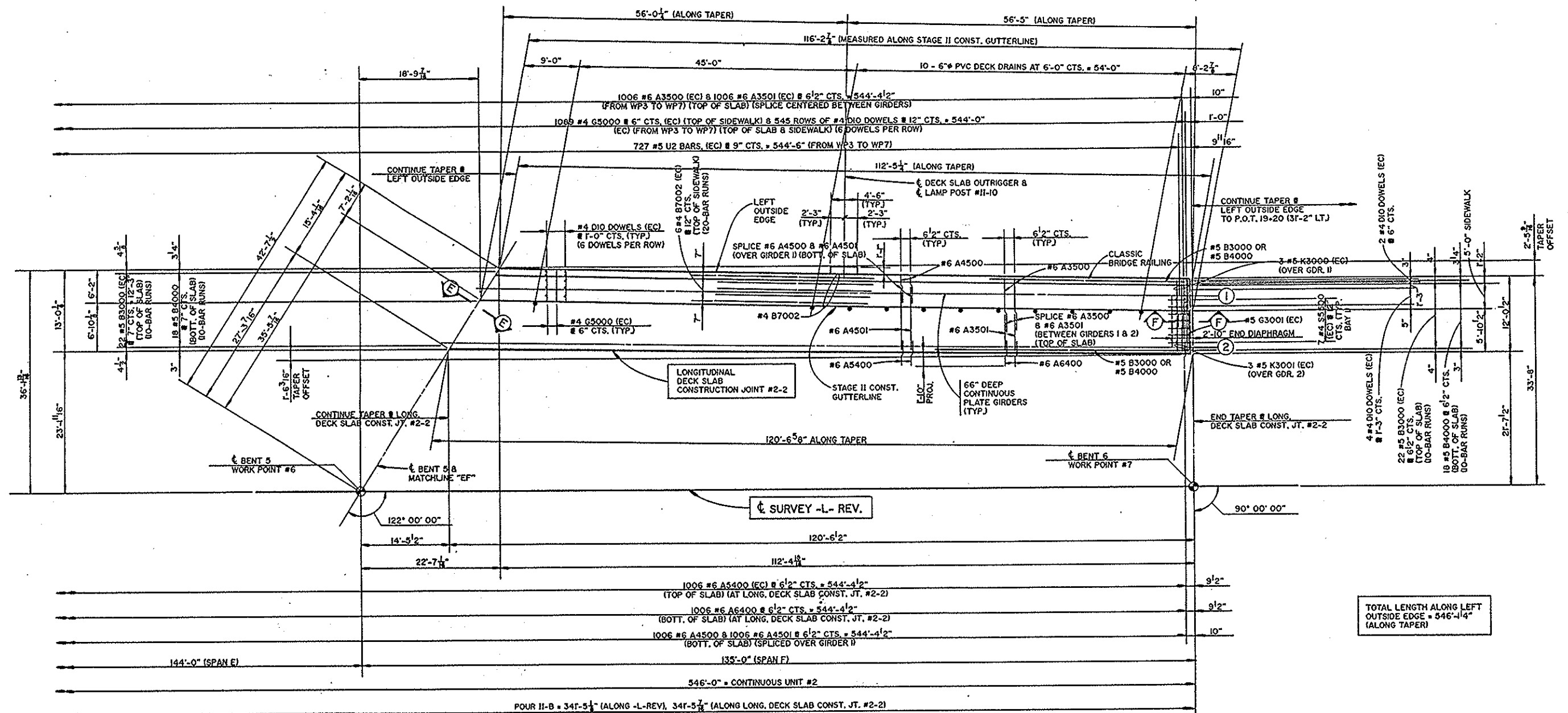
SHEET 3 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE PLAN OF SPAN E (CONTINUOUS UNIT #2) STAGE II CONSTRUCTION JUNE 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
SHEET NO. 5-52 TOTAL SHEETS 224					



PREPARED BY
 THOMPSON-GORDON-SHOOK
 ENGINEERS, INCORPORATED

DRAWN BY: N.M. WILSON
 CHECKED BY: [Signature]
 DATE: JUNE 94
 DATE: 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPAN F (CONTINUOUS UNIT #2) STAGE II CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 4 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPAN F
 (CONTINUOUS UNIT #2)
 STAGE II CONSTRUCTION
 JUNE 1994

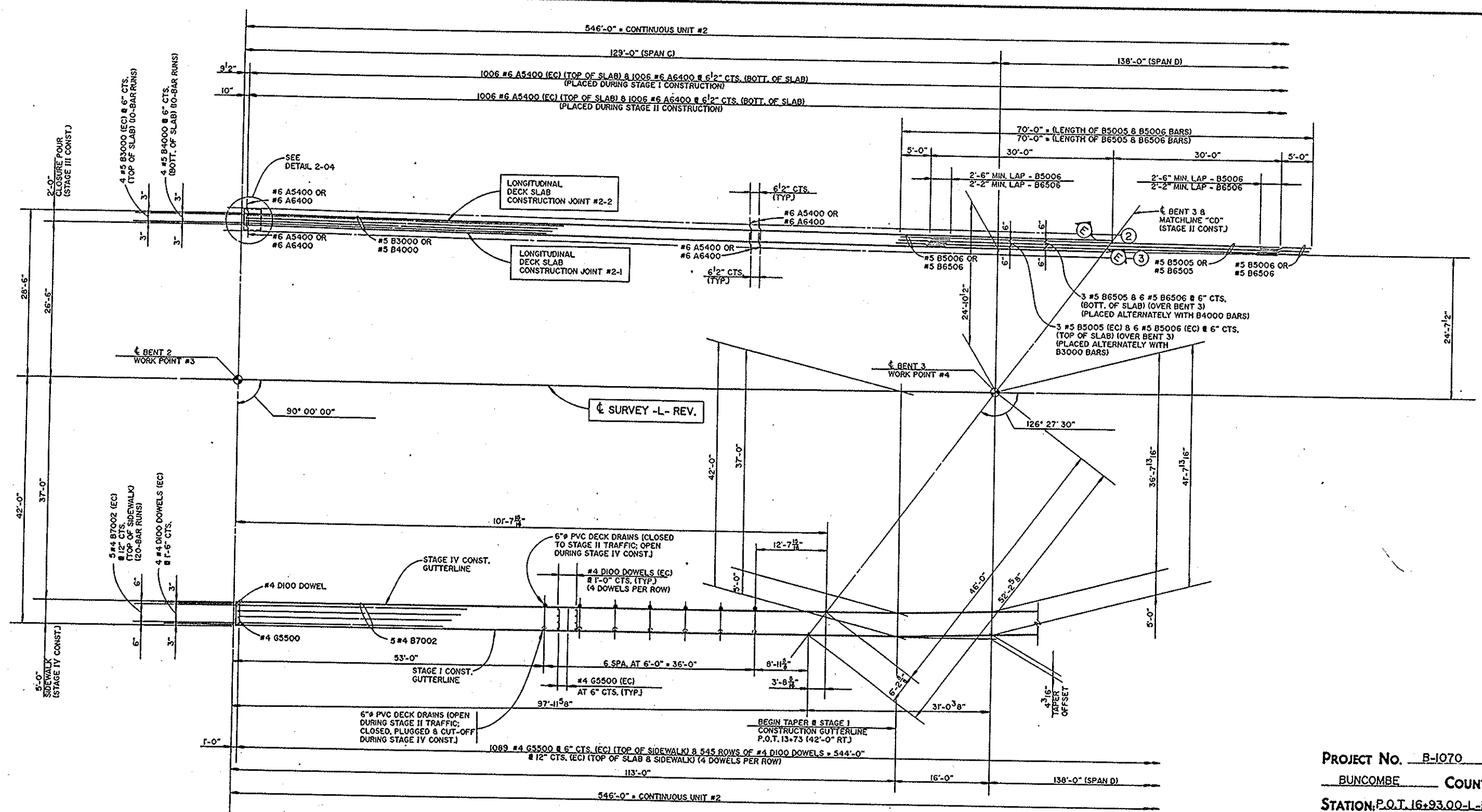


U2.012

PREPARED BY
 THOMPSON & GORDON - SNOOK
 ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			553
2			4			224

DRAWN BY: N.M. WILSON
 CHECKED BY: GCM
 DATE: JUNE 94
 DATE: 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL

PLAN OF SPAN C (CONTINUOUS UNIT #2)
STAGES III & IV CONSTRUCTION

DRAWN BY: N. M. WILSON DATE: JUNE 94
CHECKED BY: EEM DATE: 6/95

U2.013

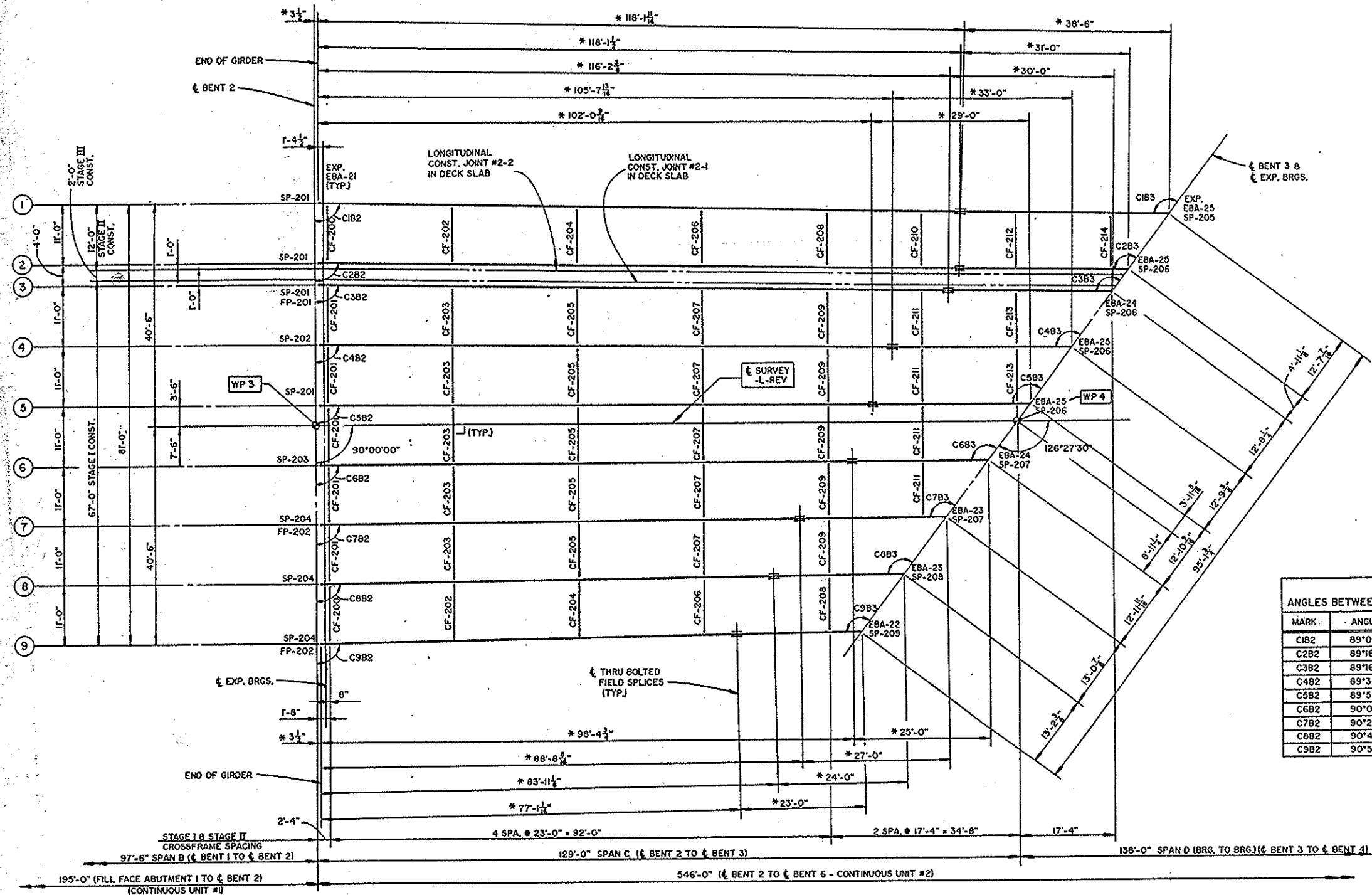
— PREPARED BY —
THOMPSON-GORDON-SHOOF
ENGINEERS, INCORPORATED

PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16-93.00-1-Rev.

SHEET 1 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPAN C
(CONTINUOUS UNIT #2)
STAGES III & IV CONSTRUCTION
JUNE 1994

REVISIONS						SHEET NO. 5-54
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			



SPAN C ANGLES BETWEEN GIRDERS AND BEARINGS			
MARK	ANGLE	MARK	ANGLE
CIB2	89°00'12"	CIB3	125°27'42"
C2B2	89°16'43"	C2B3	125°44'13"
C3B2	89°16'43"	C3B3	125°44'13"
C4B2	89°34'02"	C4B3	126°01'32"
C5B2	89°51'21"	C5B3	126°18'51"
C6B2	90°08'39"	C6B3	126°36'09"
C7B2	90°25'58"	C7B3	126°53'28"
C8B2	90°43'17"	C8B3	127°10'47"
C9B2	90°59'48"	C9B3	127°27'18"

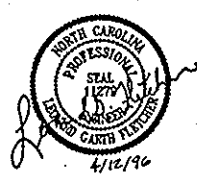
GIRDER LENGTHS FOR UNIT #2	
① ⑨	* 545'-6"
② ③ ⑧	* 545'-5 1/4"
④ ⑦	* 545'-5 3/16"
⑤ ⑥	* 545'-5"

* THESE DIMENSIONS ARE MEASURED ALONG GIRDERS HORIZONTALLY.

FRAMING PLAN - SPAN C - STAGES I, II & III CONST.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

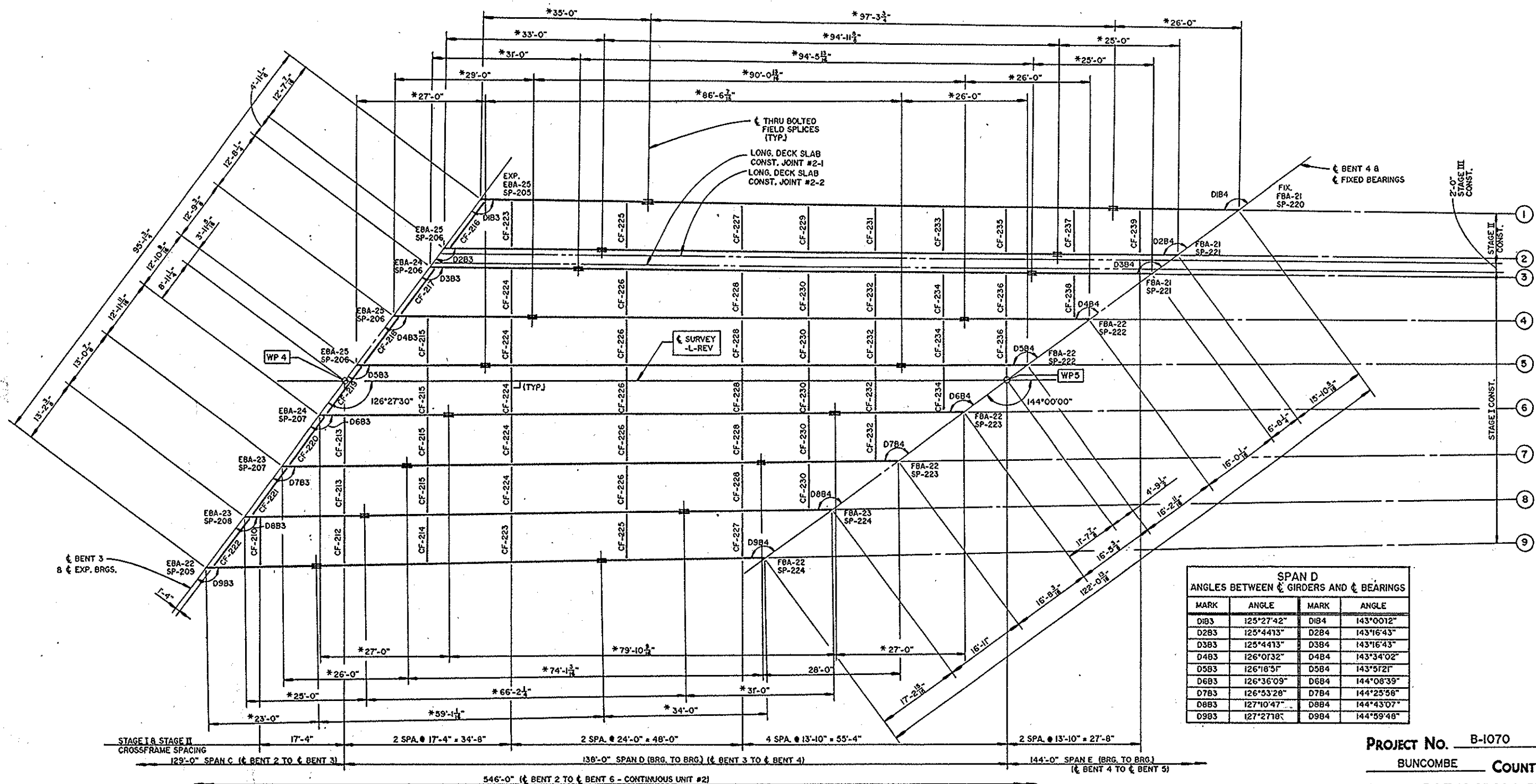
SHEET 1 OF 4



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL					
FRAMING PLAN - SPAN C (CONTINUOUS UNIT #2) STAGES I, II & III CONST. JULY 1994					
REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					TOTAL SHEETS 224

DRAWN BY: RTH DATE: 7-74
CHECKED BY: GCM DATE: 6-75

PREPARED BY:
THOMPSON - GORDON - BHOOK
ENGINEERS, INCORPORATED



SPAN D
ANGLES BETWEEN GIRDERS AND BEARINGS

MARK	ANGLE	MARK	ANGLE
D1B3	125°27'42"	D1B4	143°00'12"
D2B3	125°44'13"	D2B4	143°16'43"
D3B3	125°44'13"	D3B4	143°16'43"
D4B3	126°01'32"	D4B4	143°34'02"
D5B3	126°18'51"	D5B4	143°51'21"
D6B3	126°36'09"	D6B4	144°08'39"
D7B3	126°53'28"	D7B4	144°25'58"
D8B3	127°10'47"	D8B4	144°43'07"
D9B3	127°27'18"	D9B4	144°59'48"

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV.

* THESE DIMENSIONS ARE MEASURED ALONG G GIRDER HORIZONTALLY.

FRAMING PLAN - SPAN D - STAGES I, II & III CONST.



SHEET 2 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
FRAMING PLAN - SPAN D
(CONTINUOUS UNIT #2)
STAGES I, II & III CONST.
JULY 1994

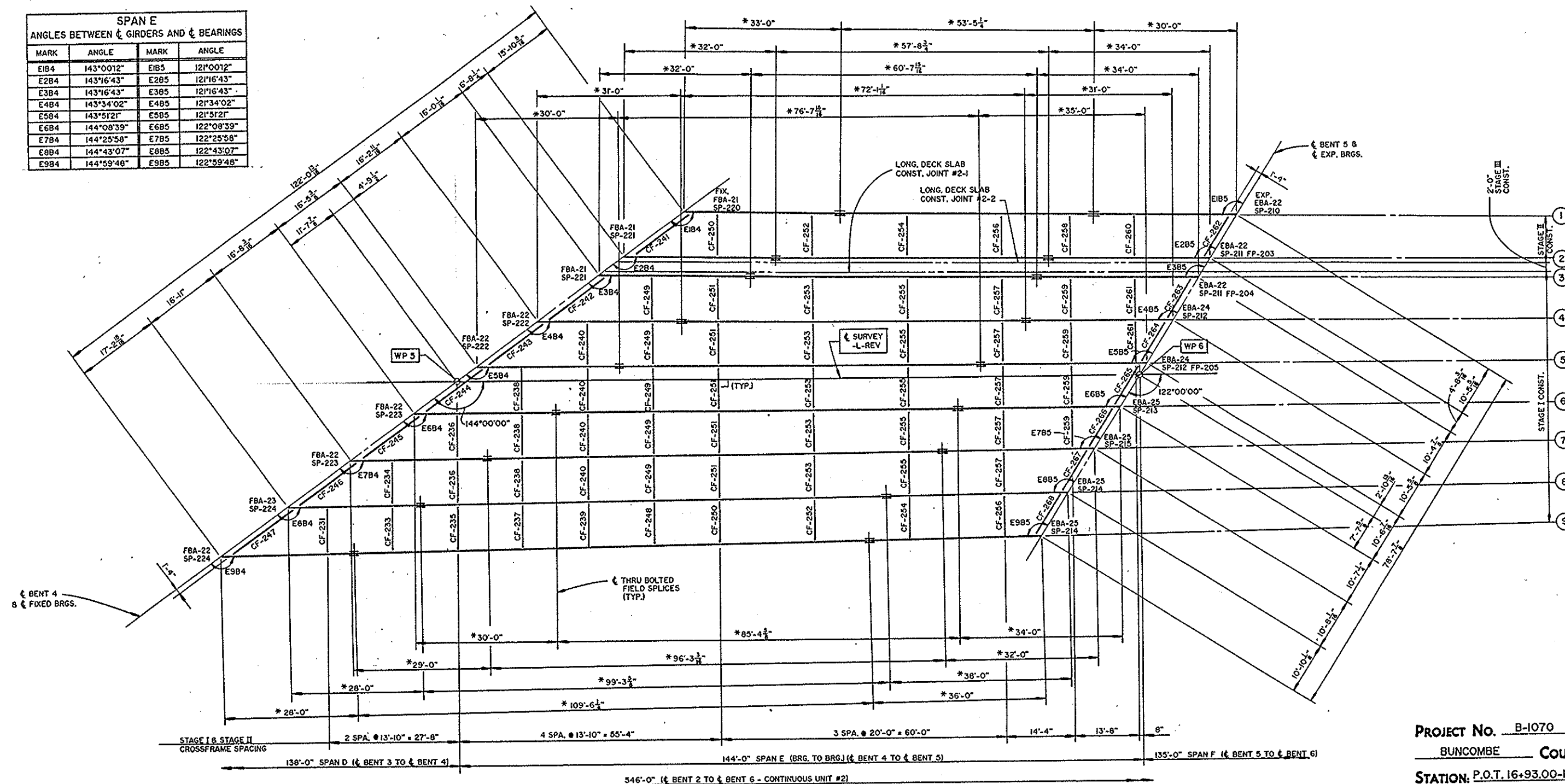
REVISIONS			
NO.	BY	DATE	DESCRIPTION
1			
2			

PREPARED BY
THOMPSON-GORDON-BROOK
ENGINEERS, INCORPORATED

5-59
TOTAL SHEETS
224

DRAWN BY: ATL DATE: 6-24
CHECKED BY: SEM DATE: 6/25

SPAN E ANGLES BETWEEN GIRDERS AND BEARINGS			
MARK	ANGLE	MARK	ANGLE
E1B4	143°00'12"	E1B5	121°00'12"
E2B4	143°16'43"	E2B5	121°16'43"
E3B4	143°16'43"	E3B5	121°16'43"
E4B4	143°34'02"	E4B5	121°34'02"
E5B4	143°51'21"	E5B5	121°51'21"
E6B4	144°08'39"	E6B5	122°08'39"
E7B4	144°25'58"	E7B5	122°25'58"
E8B4	144°43'07"	E8B5	122°43'07"
E9B4	144°59'46"	E9B5	122°59'46"



* THESE DIMENSIONS ARE MEASURED ALONG GIRDERS HORIZONTALLY.

FRAMING PLAN - SPAN E - STAGES I, II & III CONST.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
FRAMING PLAN - SPAN E
(CONTINUOUS UNIT #2)
STAGES I, II & III CONST.
JULY 1994



12.019
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THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO. 5100
NO.	BY	DATE	NO.	BY	DATE	
1			2			TOTAL 224
2			4			

DRAWN BY: RLV
CHECKED BY: ECM
DATE: 6-28-94
DATE: 6/95



NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

STATION: P.O.T. 16+93.00-L-REV

SHEET 1 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 1 ELEVATION
CONTINUOUS UNIT #2

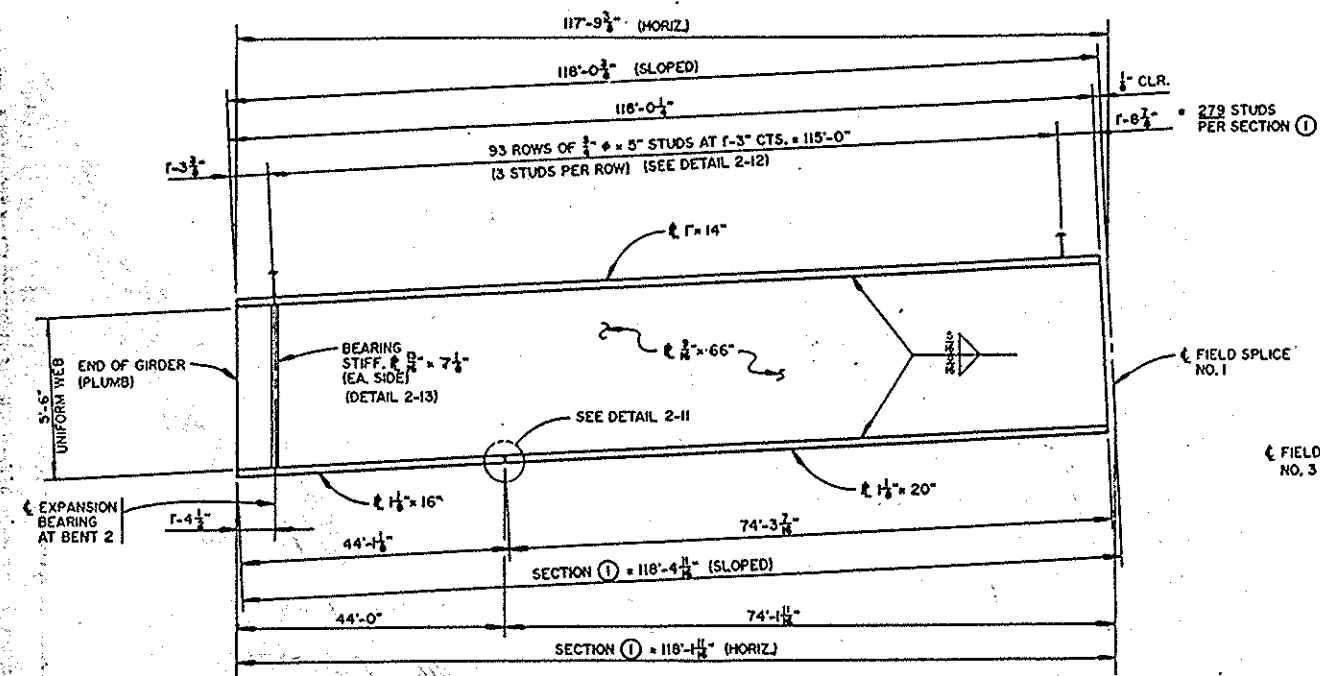
JULY 1994

REVISIONS						SHEET NO. 5-62 TOTAL SHEETS 224
NO.	BY	DATE	NO.	BY	DATE	
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2			4			

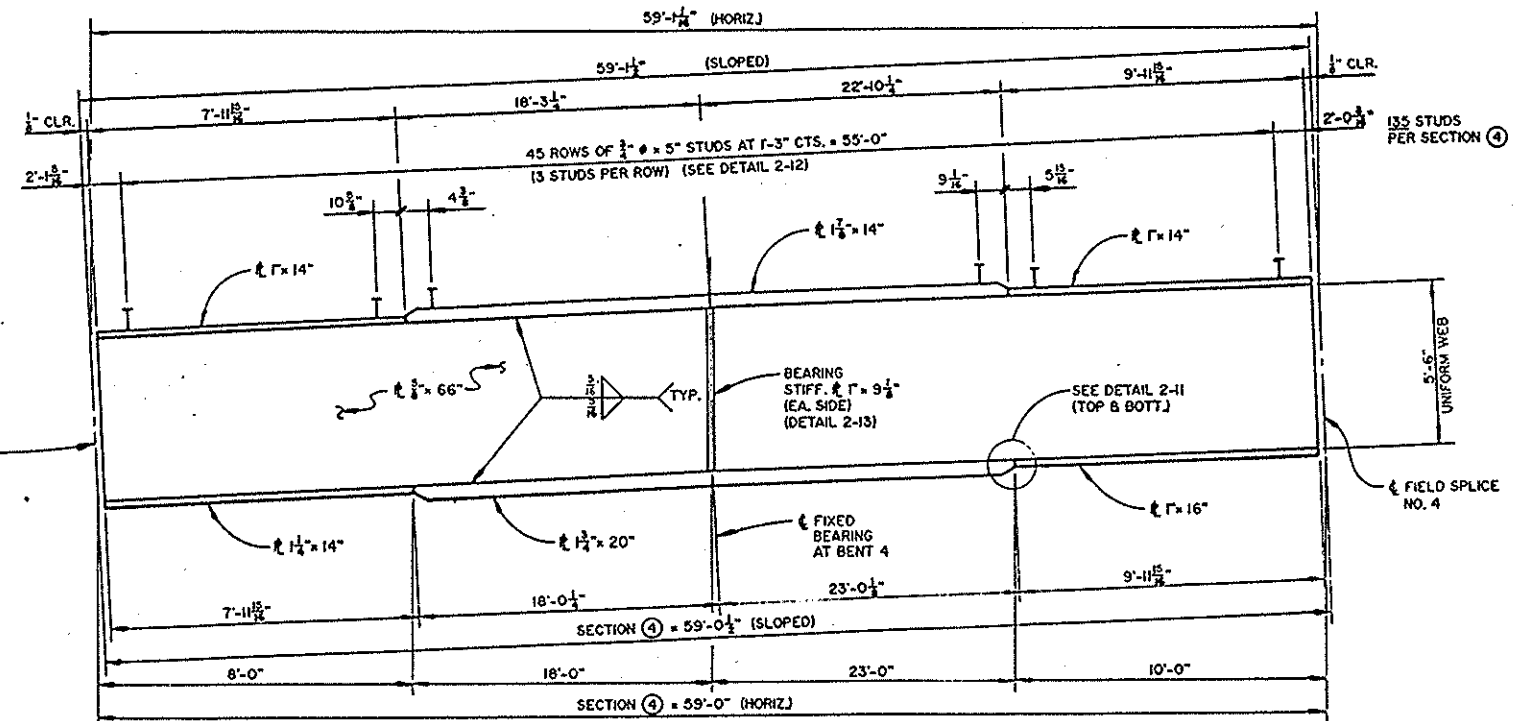
42.021

PREPARED BY
THOMPSON-GORDON-AND
ENGINEERS, INCORPORAT

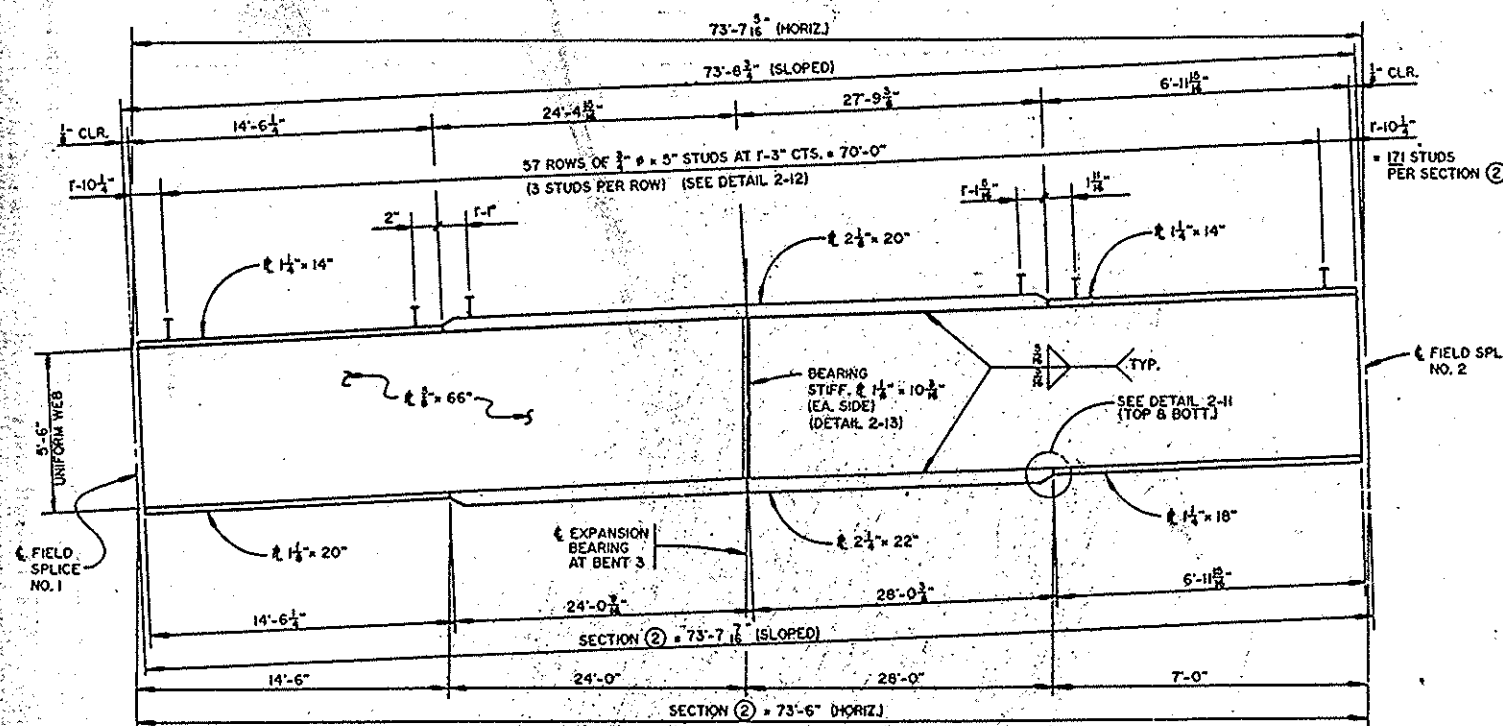
DRAWN BY RTJ DATE 7-74
CHECKED BY CEM DATE 6/95



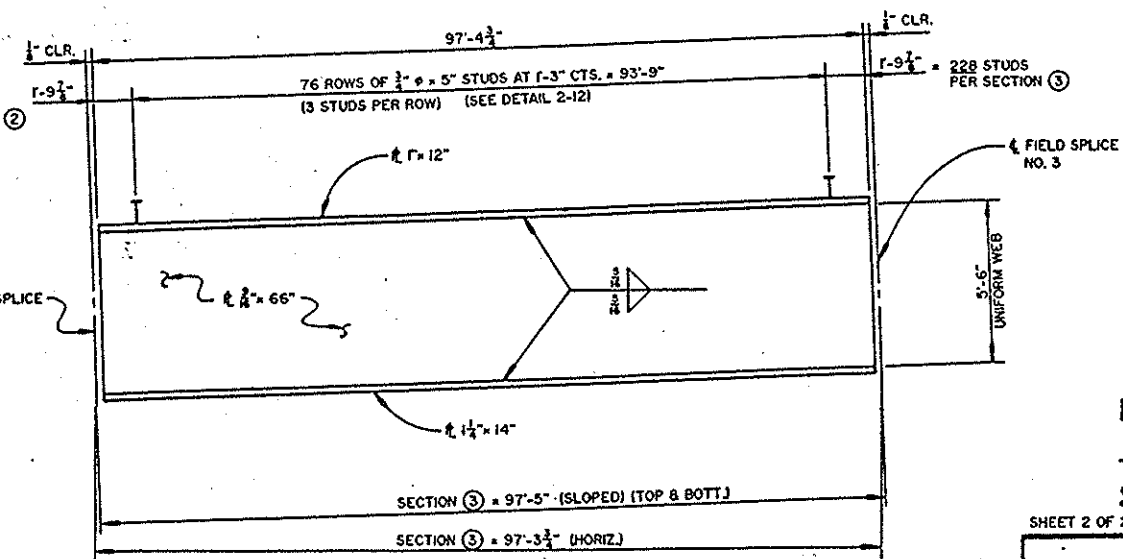
GIRDER SECTION ① - 1 REQ'D.



GIRDER SECTION (4) - 1 REQ'D.



GIRDER SECTION (2) - 1 REQ'D.



GIRDER SECTION (3) - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV

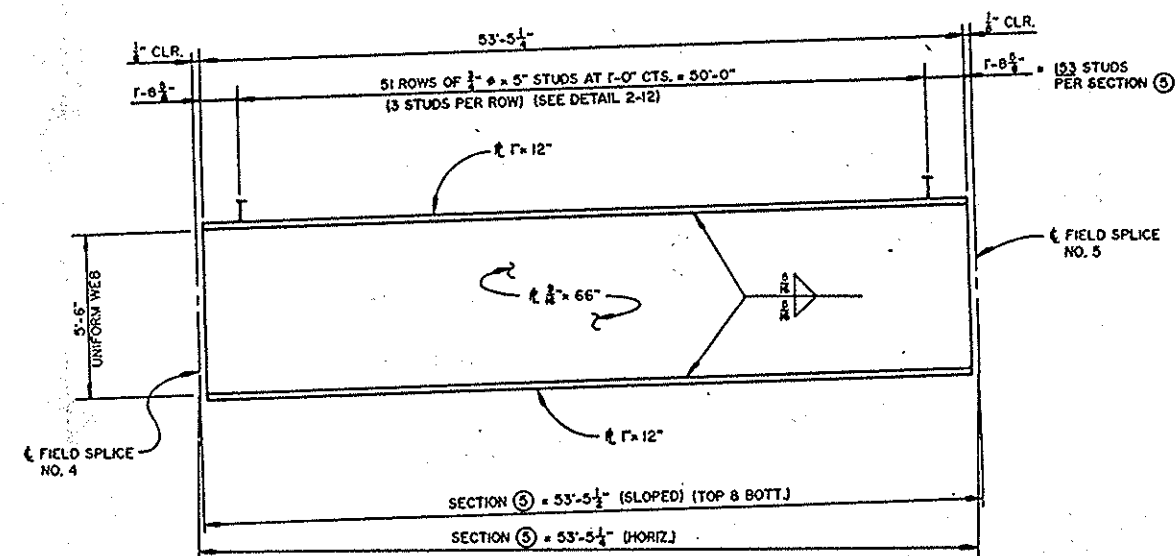
SHEET 2 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER I DETAILS
CONTINUOUS UNIT #2

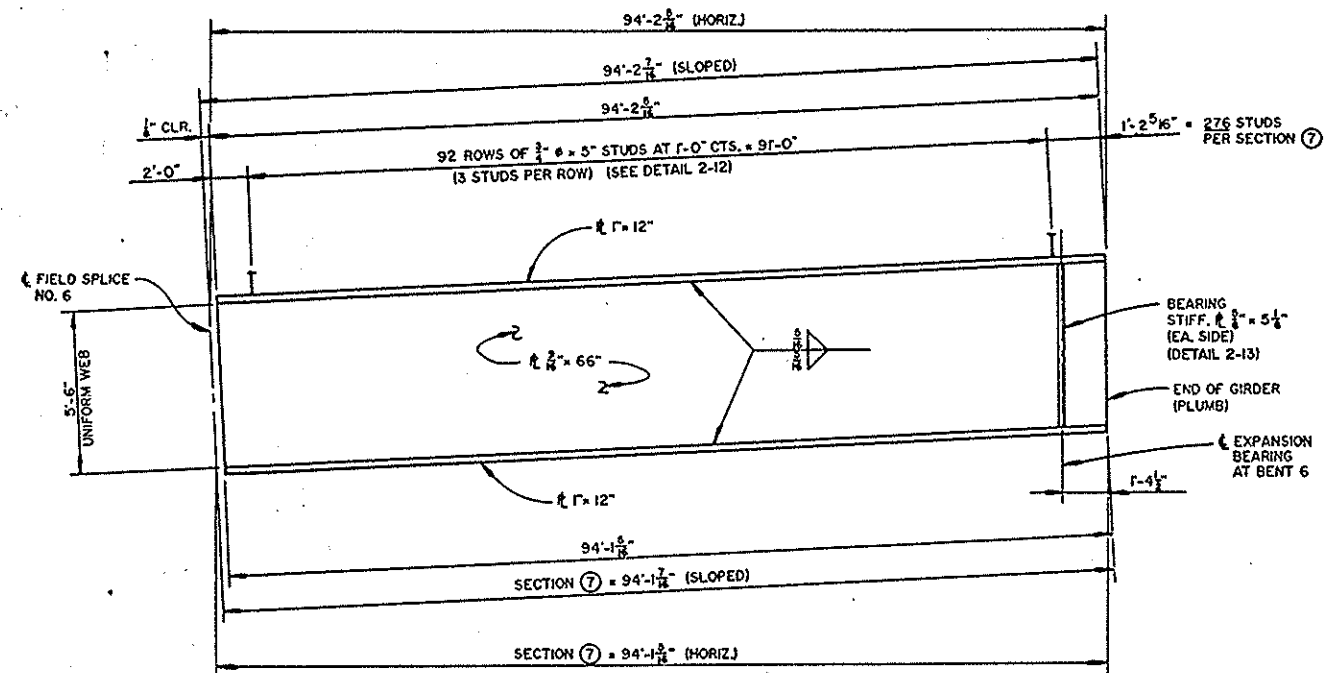
U2.022

— PREPARED BY —
THOMPSON - GORDON - SMOON
ENGINEERS, INCORPORATED

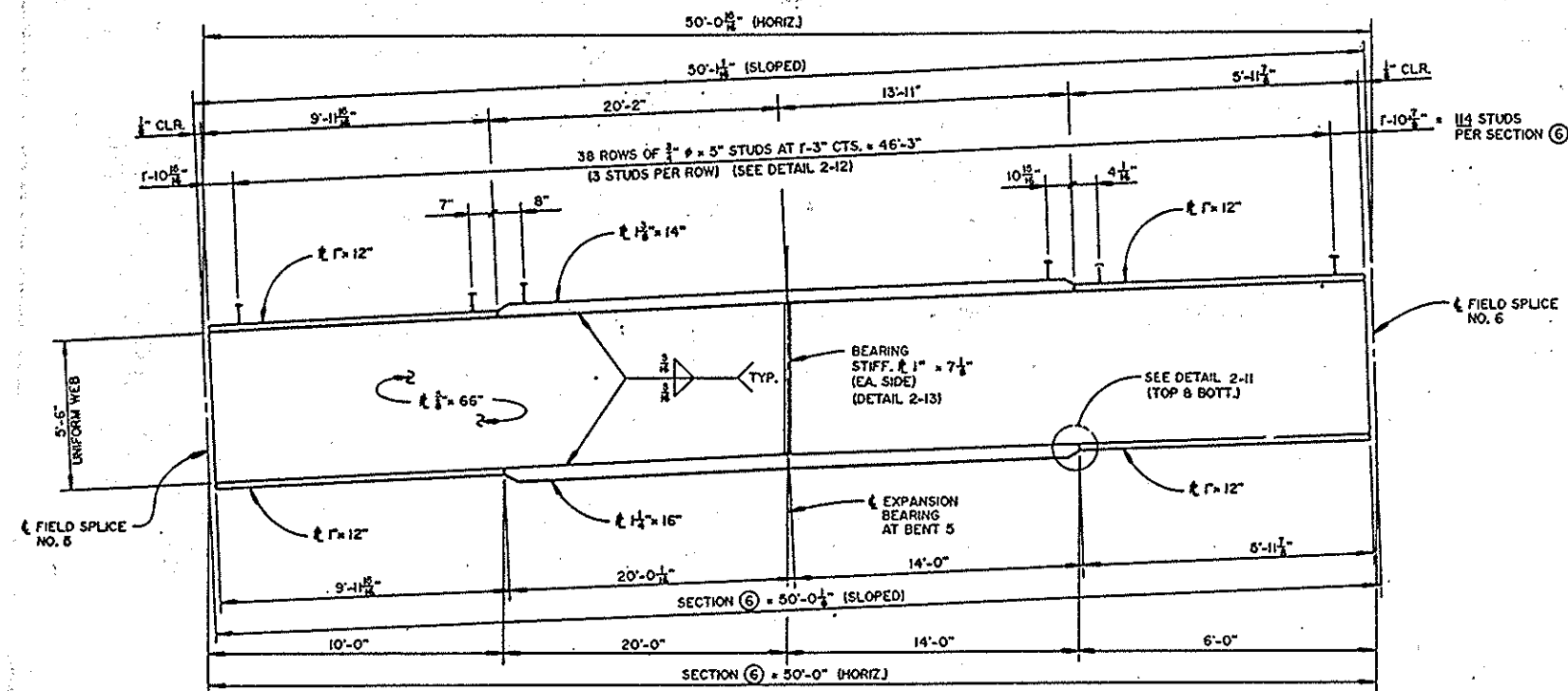
REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			2			TOTAL SHEETS
			3			774



GIRDER SECTION ⑤ - 1 REQ'D.



GIRDER SECTION ⑦ - 1 REQ'D.



GIRDER SECTION ⑥ - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 27



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 1 DETAILS
CONTINUOUS UNIT #2
AUGUST 1994

U2.023
PREPARED BY
THOMPSON-BOROUGH-SHOCK
ENGINEERS, INCORPORATED

NO.	BY	DATE	NO.	BY	DATE	SHEET NO.
1			2			564
2			3			224

DRAWN BY: RFL
CHECKED BY: CEM
DATE: 8/94



NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

SHEET 4 OF 27

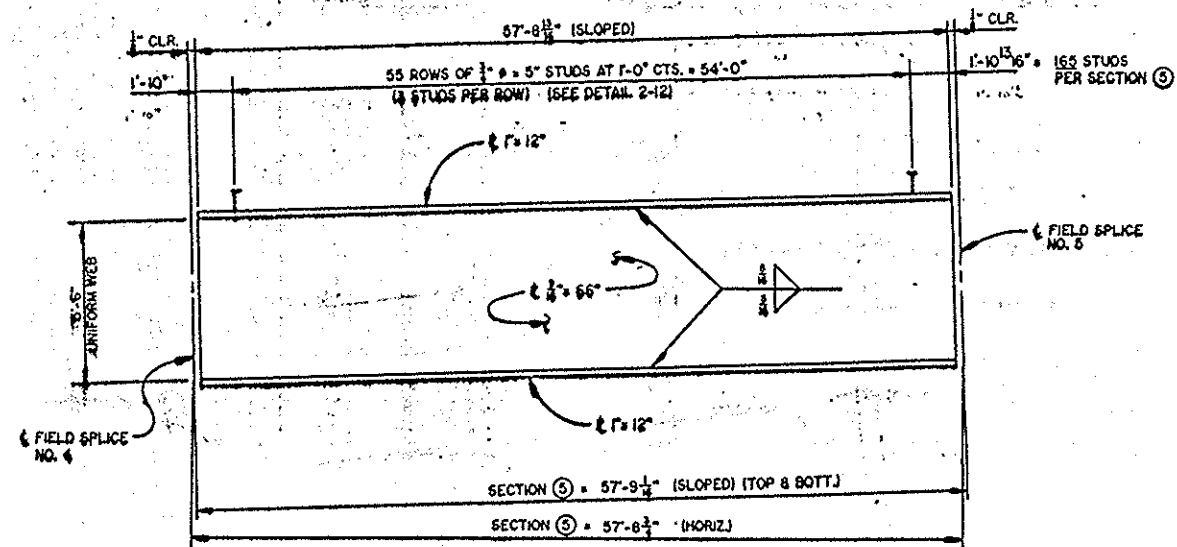
JULY 1994

REVISIONS						SHEET NO. 5-65
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 774
2			4			

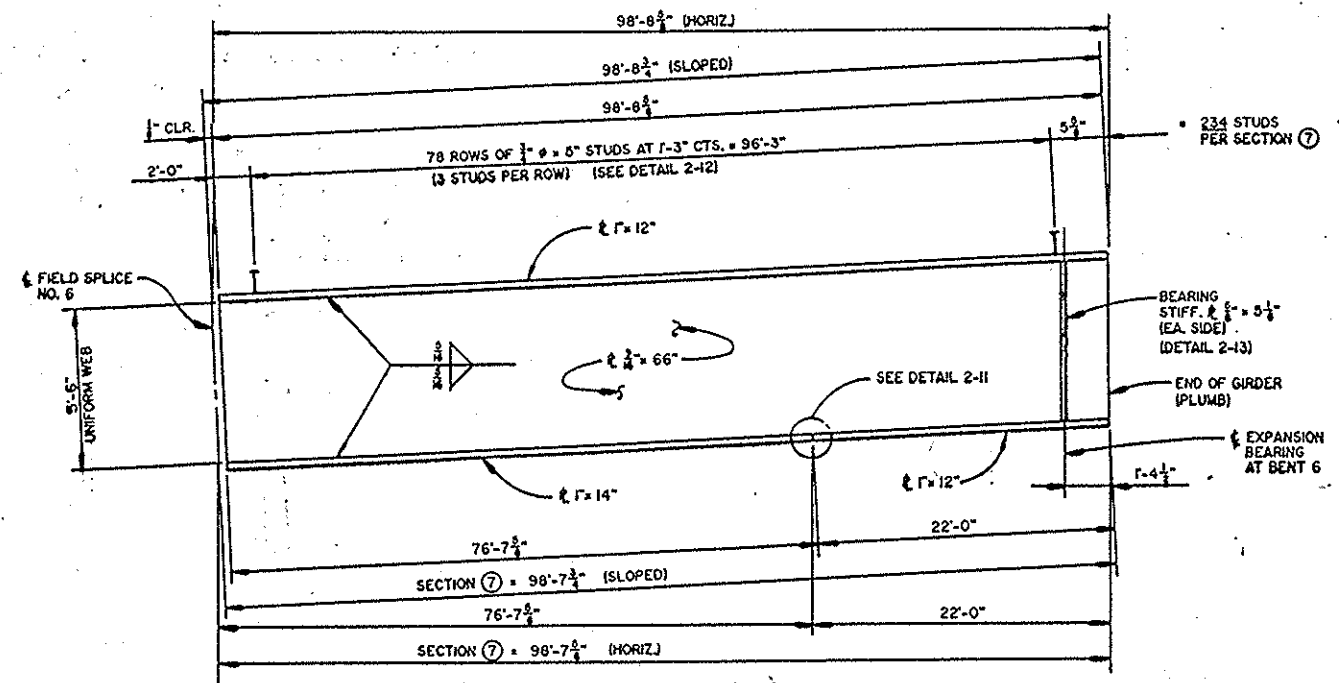
DRAWN BY RTJ DATE 7-94
CHECKED BY ECM DATE 6/95


 NORTH CAROLINA
 PROFESSIONAL
 SEAL
 11279
 LICENSED ARCHITECT
 4/12/90

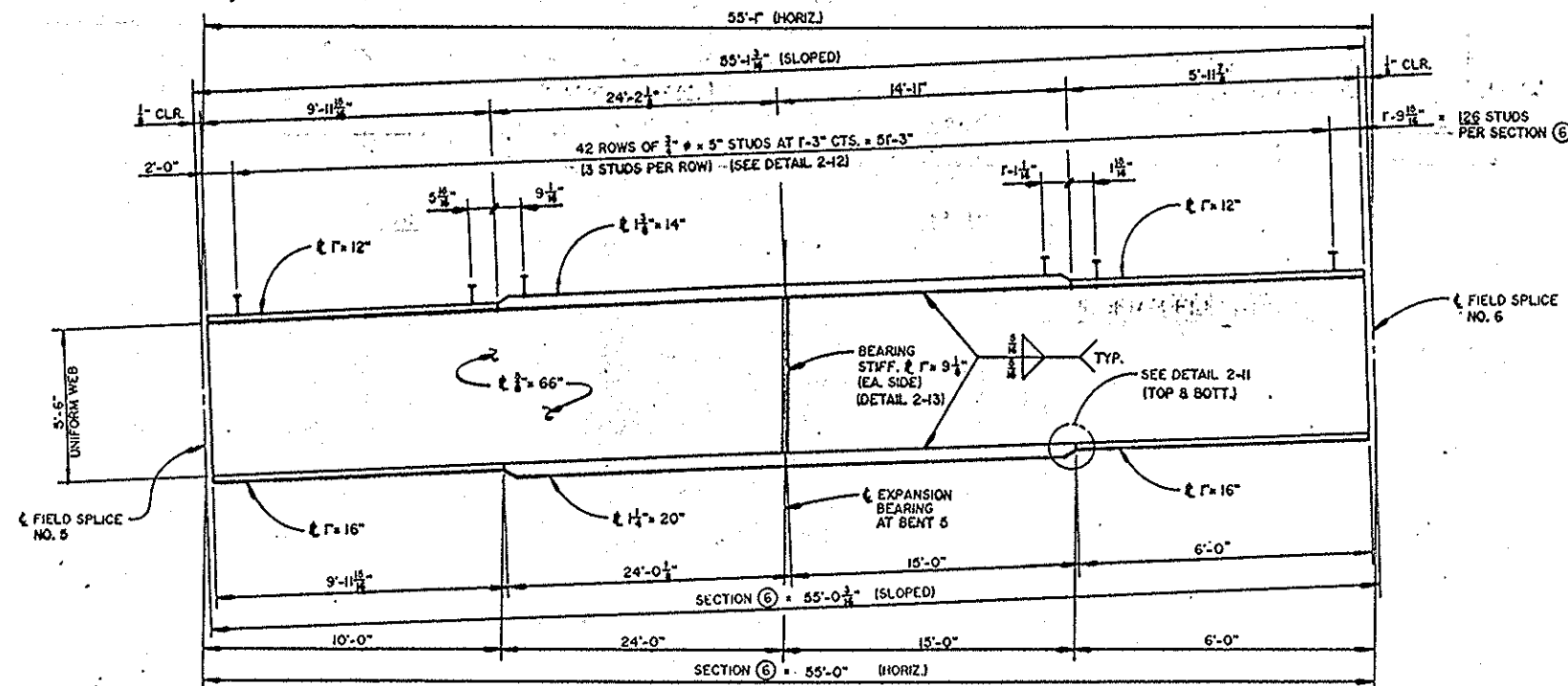
42.02
PREPARED BY
THOMPSON-GORDON-AND
ENGINEERS, INCORPORATED



GIRDER SECTION (5) - 1 REQ'D.



GIRDER SECTION (7) - 1 REQ'D.



GIRDER SECTION (6) - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 6 OF 27

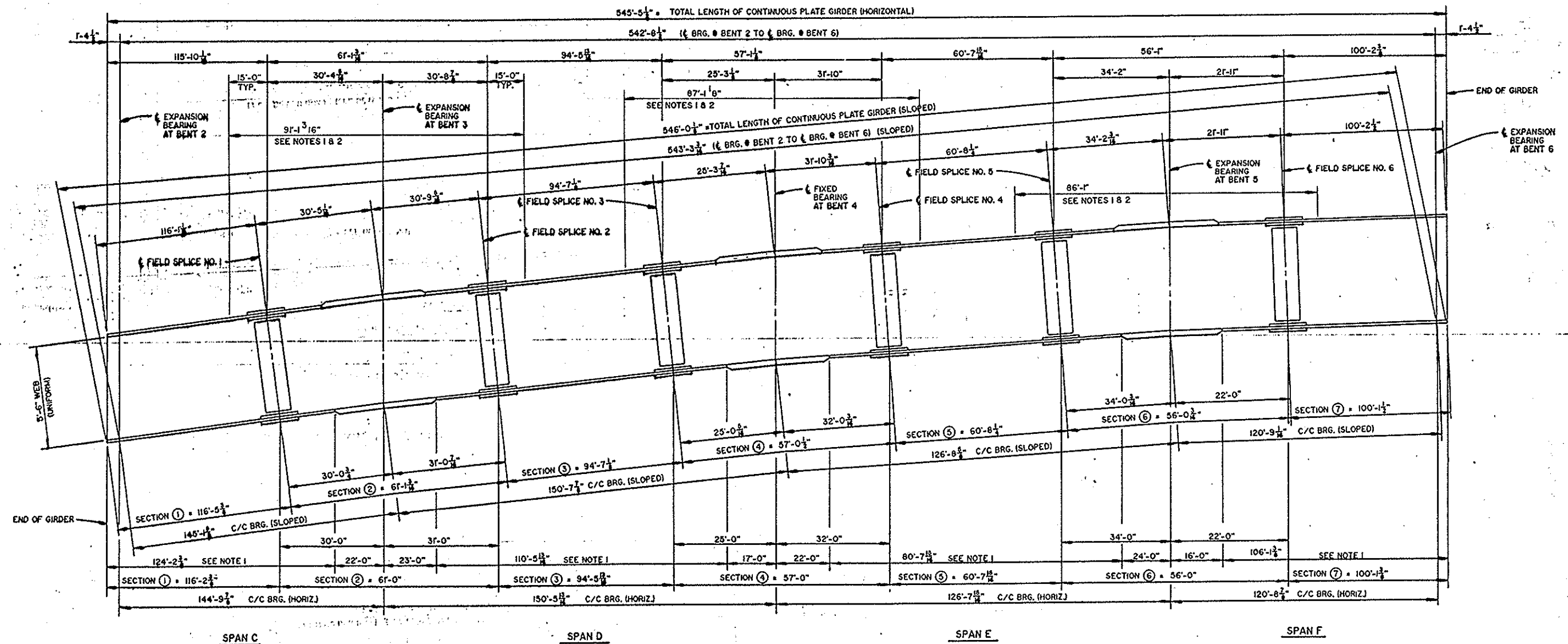
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 2 DETAILS
CONTINUOUS UNIT #2

JULY 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			2			567
2			3			224



U2.026
PREPARED BY
THOMPSON-BORDON-SHOCK
ENGINEERS, INCORPORATED



GIRDER 3 ELEVATION

- NOTE 1: CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.
- IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.
- NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV
SHEET 7 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 3 ELEVATION
CONTINUOUS UNIT #2
JULY 1994

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			5		
2			6		
3			7		
4			8		

SHEET NO. 5-68
TOTAL SHEETS 224



U2.027
PREPARED BY
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

DATE 7-94
GCM
DATE 6/95

[illegible]

PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV

SHEET 8 OF 27

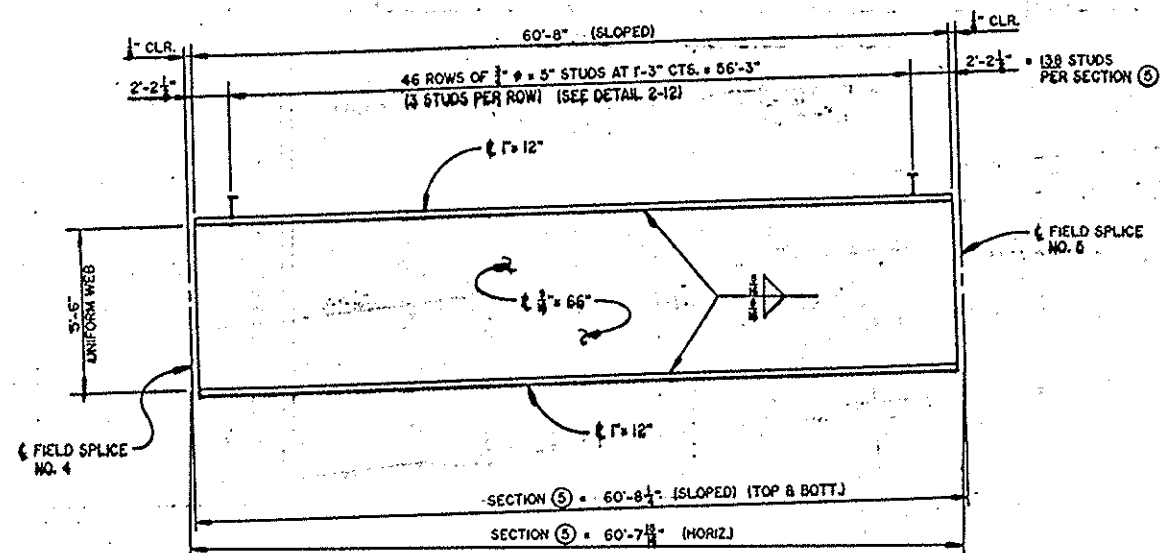
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 3 DETAILS
CONTINUOUS UNIT #2
AUGUST 1994

REVISIONS						NO. 5-69
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			

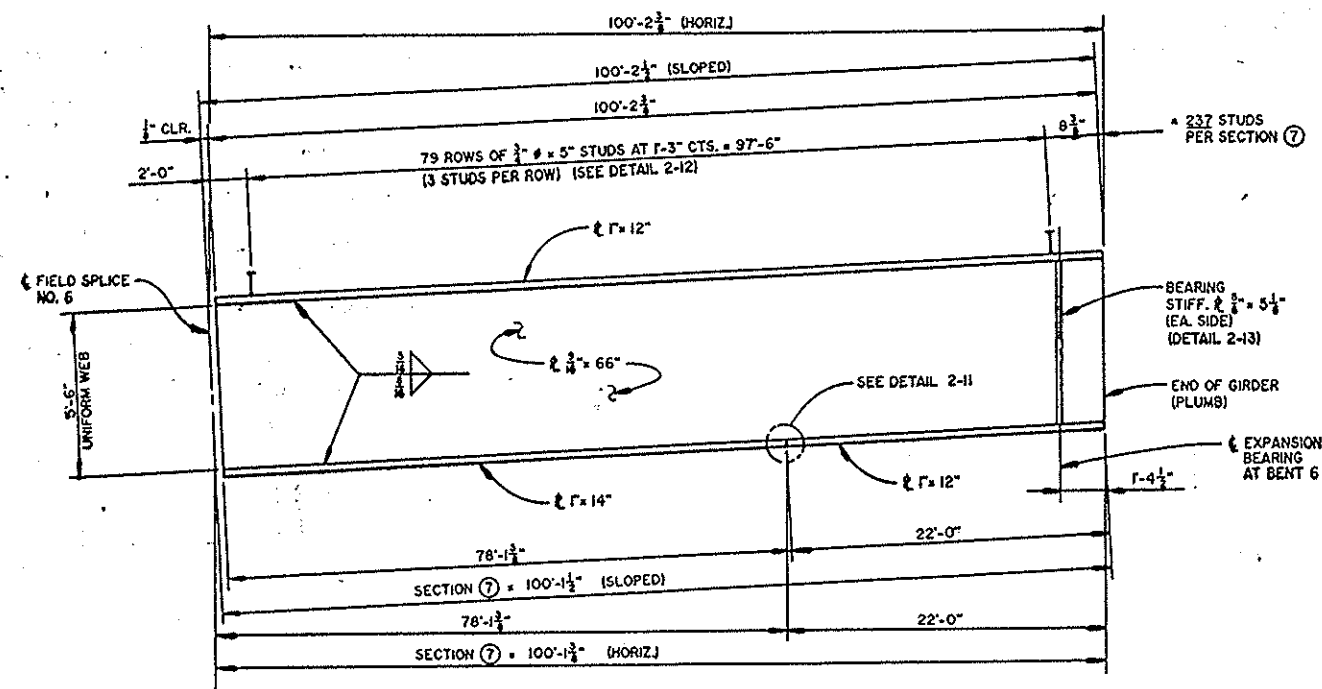
U2.028

— PREPARED BY —
THOMPSON-GORDON-SHOX
ENGINEERS, INCORPORATED

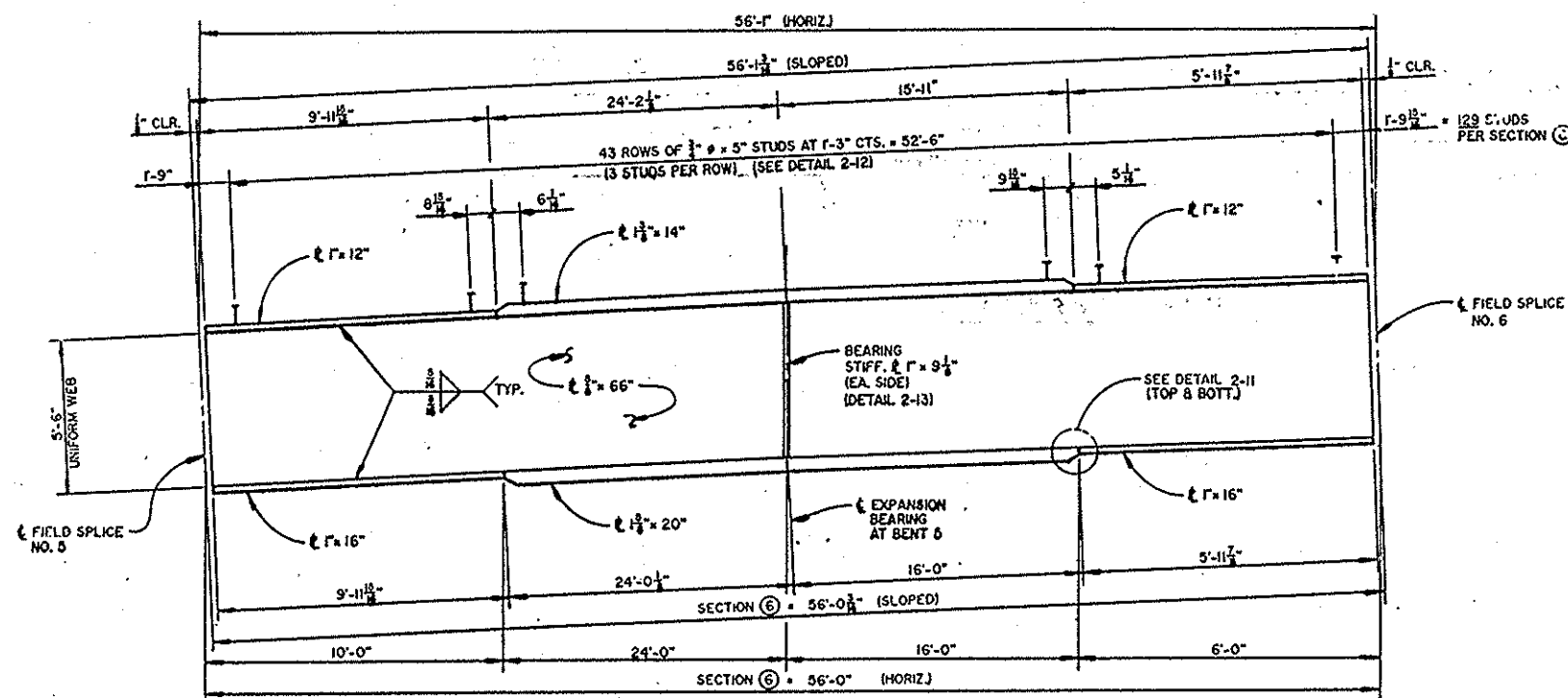




GIRDER SECTION 5 - 1 REQ'D.



GIRDER SECTION 7 - 1 REQ'D.



GIRDER SECTION 6 - 1 REQ'D.

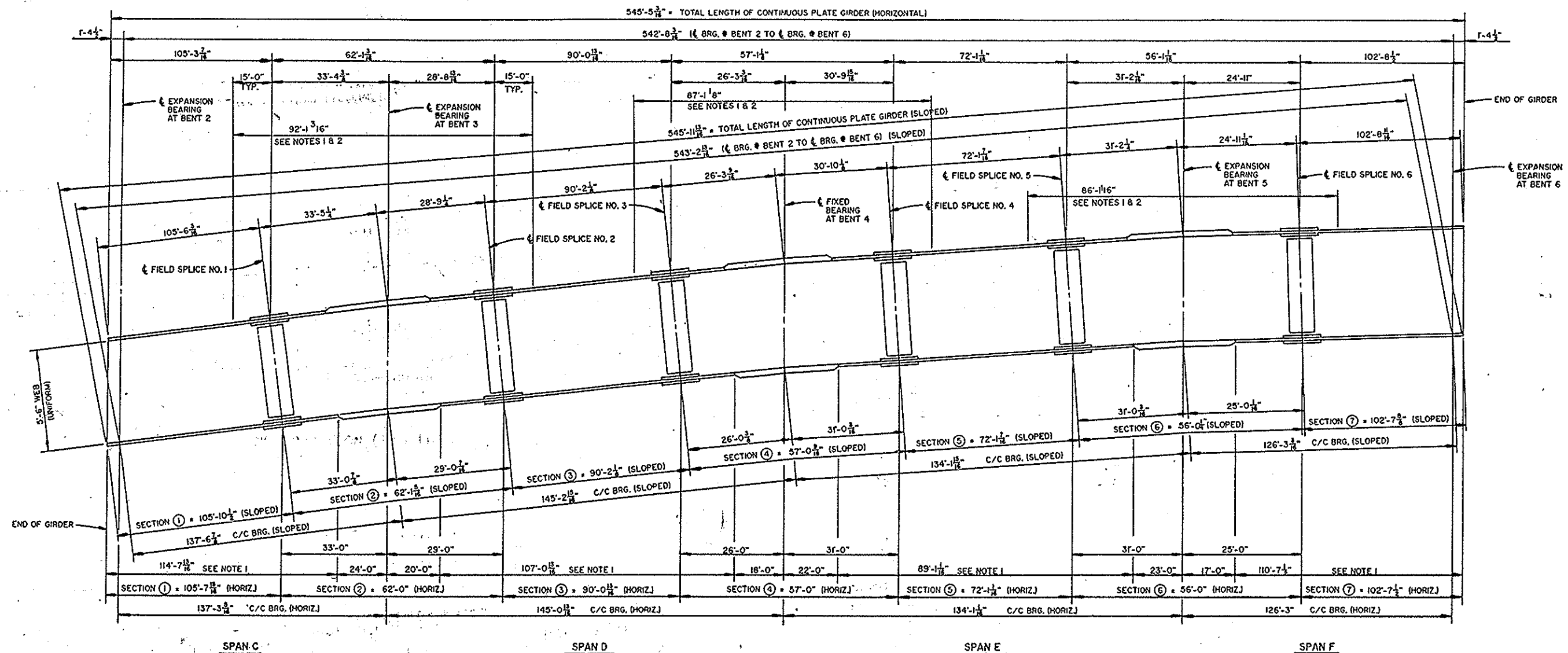
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV
SHEET 9 OF 27

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL GIRDER 3 DETAILS CONTINUOUS UNIT #2 JULY 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
TOTAL SHEETS					224



U2.029
PREPARED BY
THOMPSON-BORDON-SHOOR
ENGINEERS, INCORPORATED

DRAWN BY: [Signature]
CHECKED BY: [Signature]
DATE: 7/94
DATE: 6/95



GIRDER 4 ELEVATION

- NOTE 1: CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.
- IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.
- NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

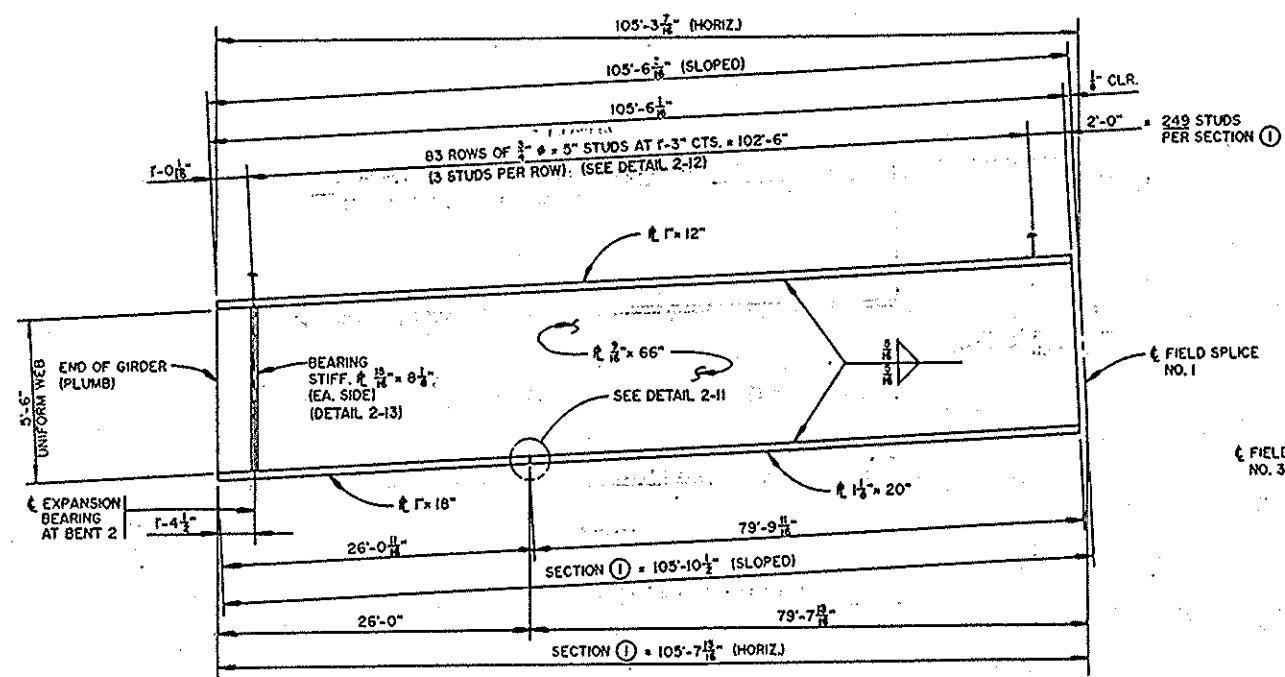
PROJECT No. 8-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV
SHEET 10 OF 27



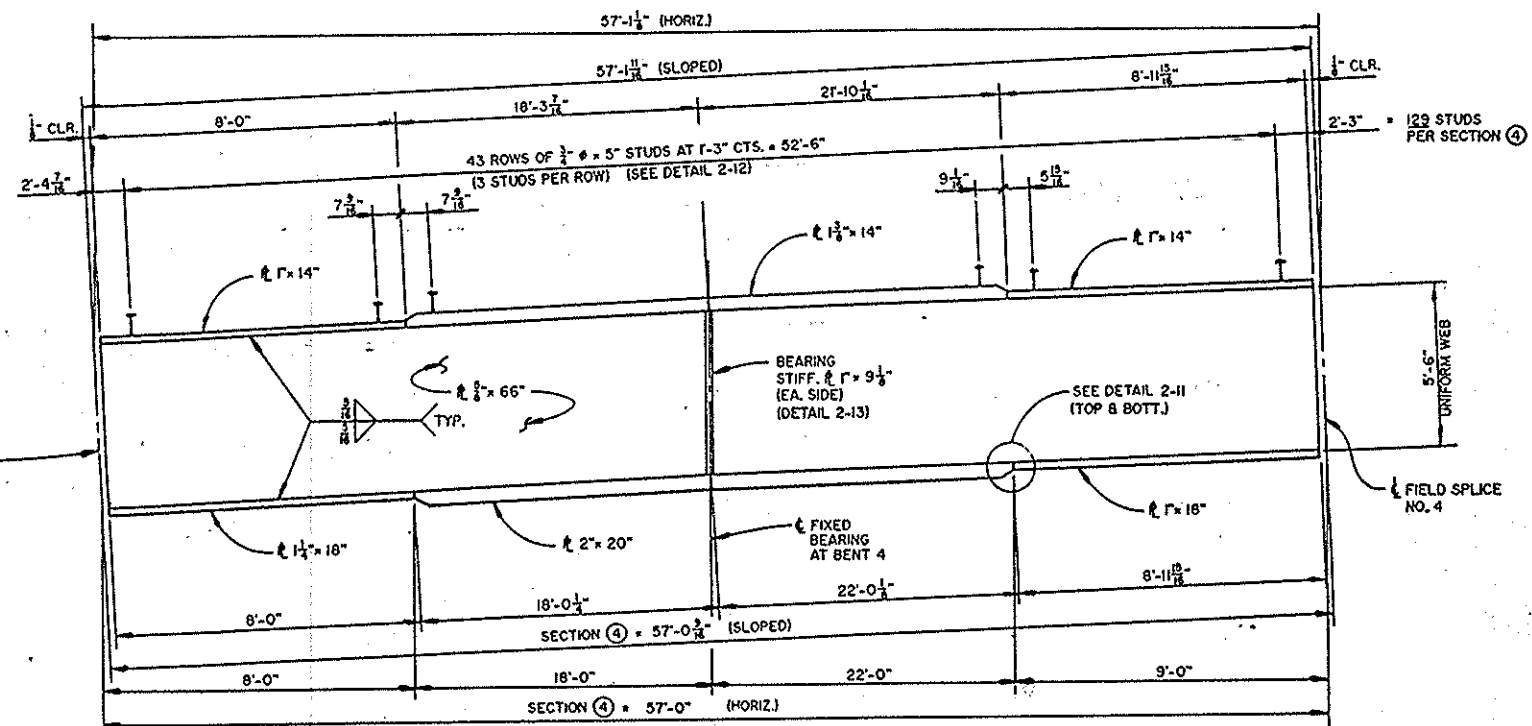
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL GIRDER 4 ELEVATION CONTINUOUS UNIT #2 JULY 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
TOTAL SHEETS					224

DRAWN BY: RTH DATE: 7-74
CHECKED BY: EEM DATE: 6/85

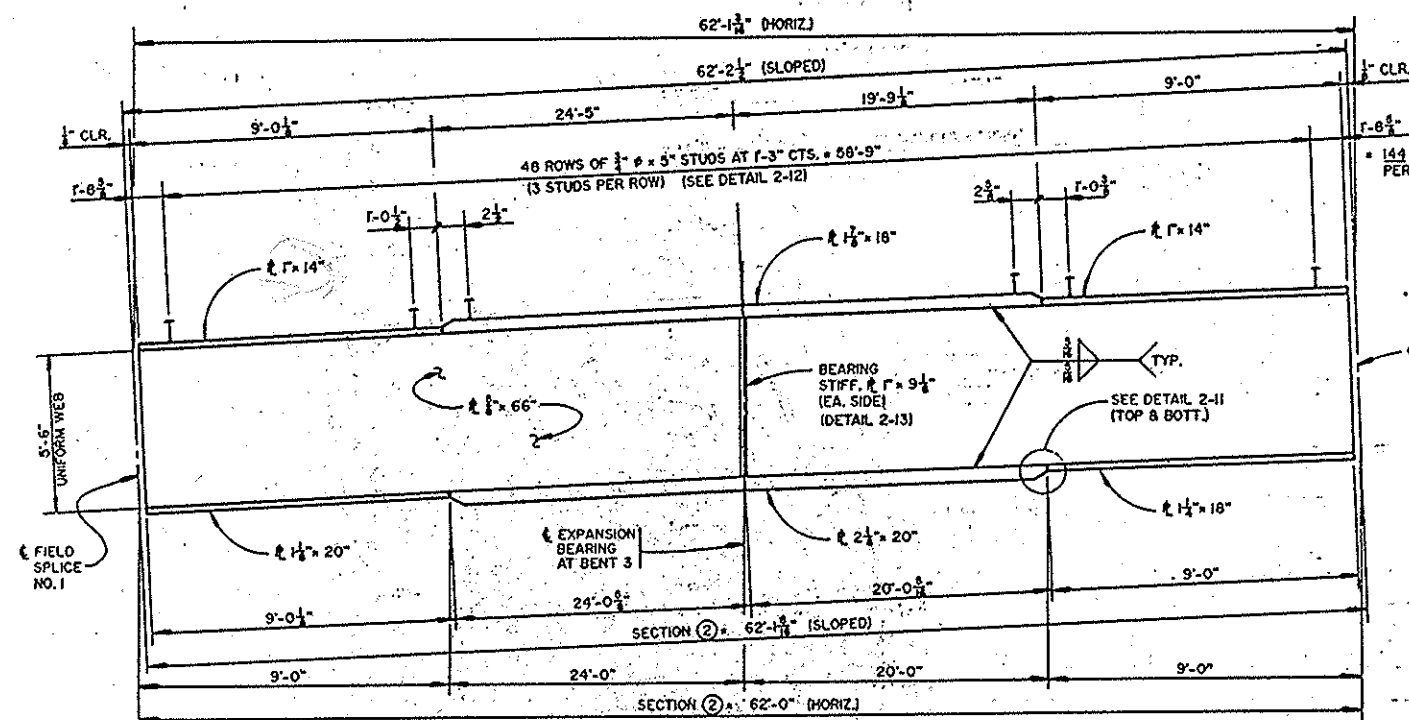
U2.050
PREPARED BY:
THOMPSON-BORDON-SHOCK
ENGINEERS, INCORPORATED



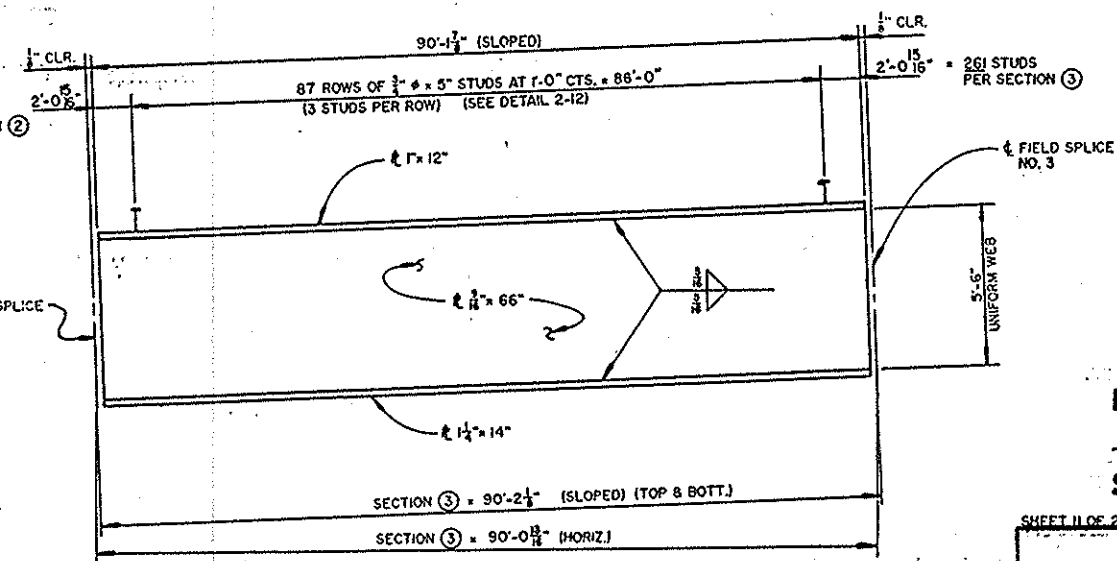
GIRDER SECTION ① - 1 REQ'D.



GIRDER SECTION ④ - 1 REQ'D.



GIRDER SECTION ② - 1 REQ'D.



GIRDER SECTION ③ - 1 REQ'D.

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION, P.O.T. 16+93.00-L-REV

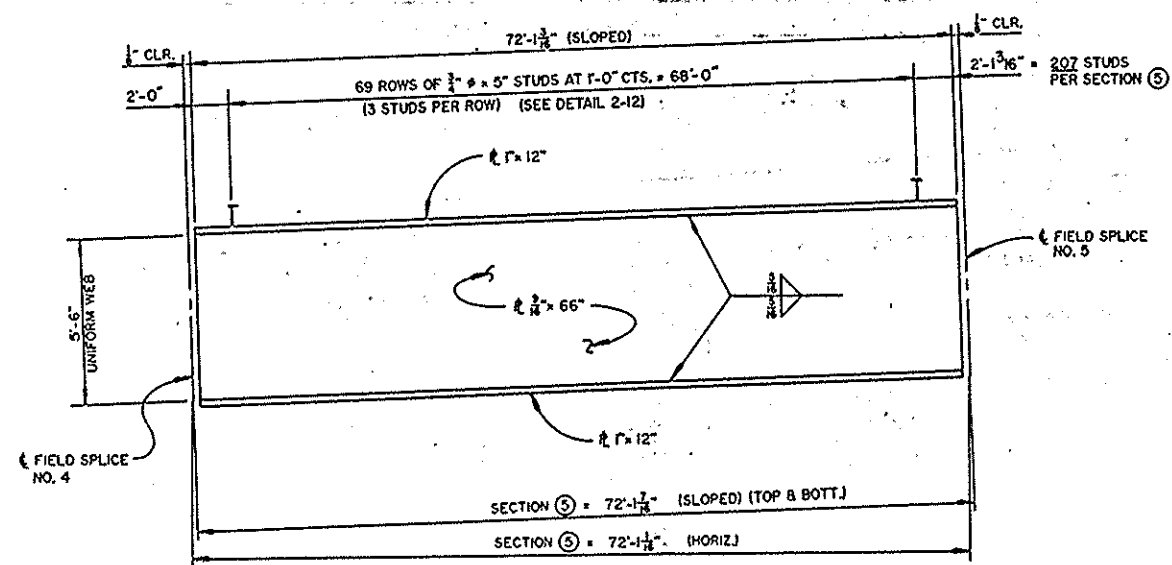
SHEET 11 OF 27

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
 GIRDER 4 DETAILS
 CONTINUOUS UNIT #2
 AUGUST 1994

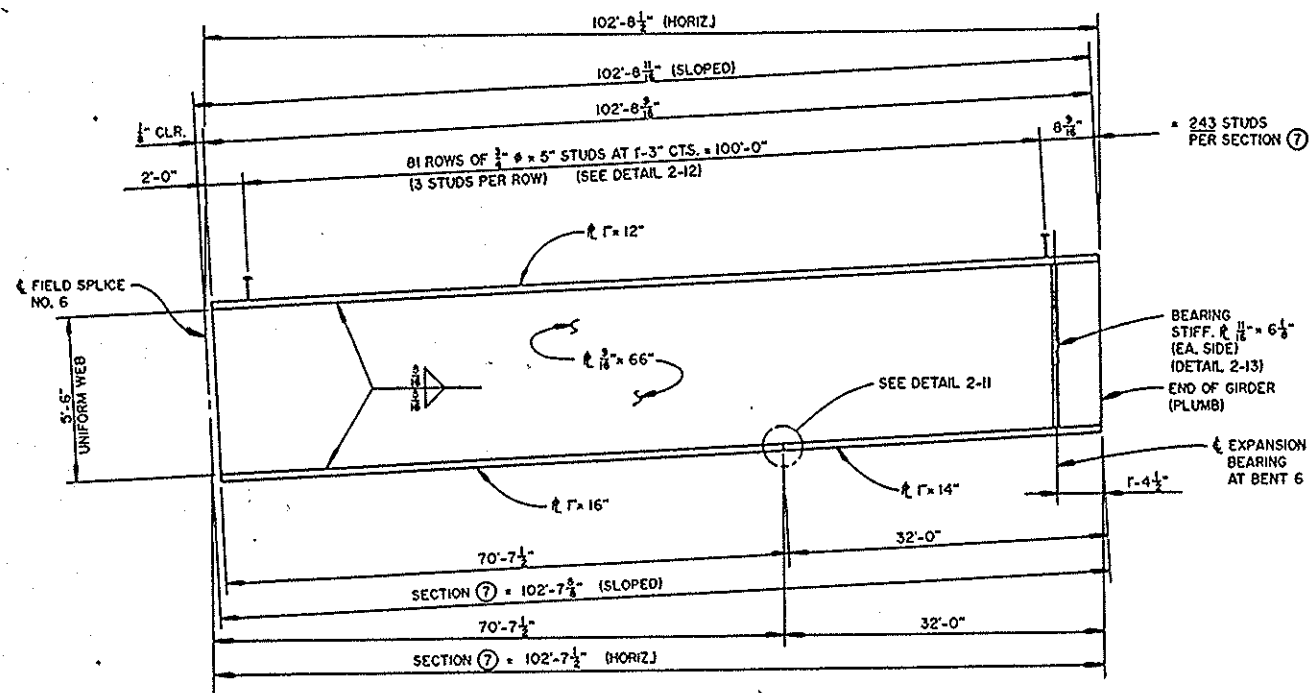
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

PREPARED BY
 THOMPSON-GORDON-SHOCK
 ENGINEERS, INCORPORATED

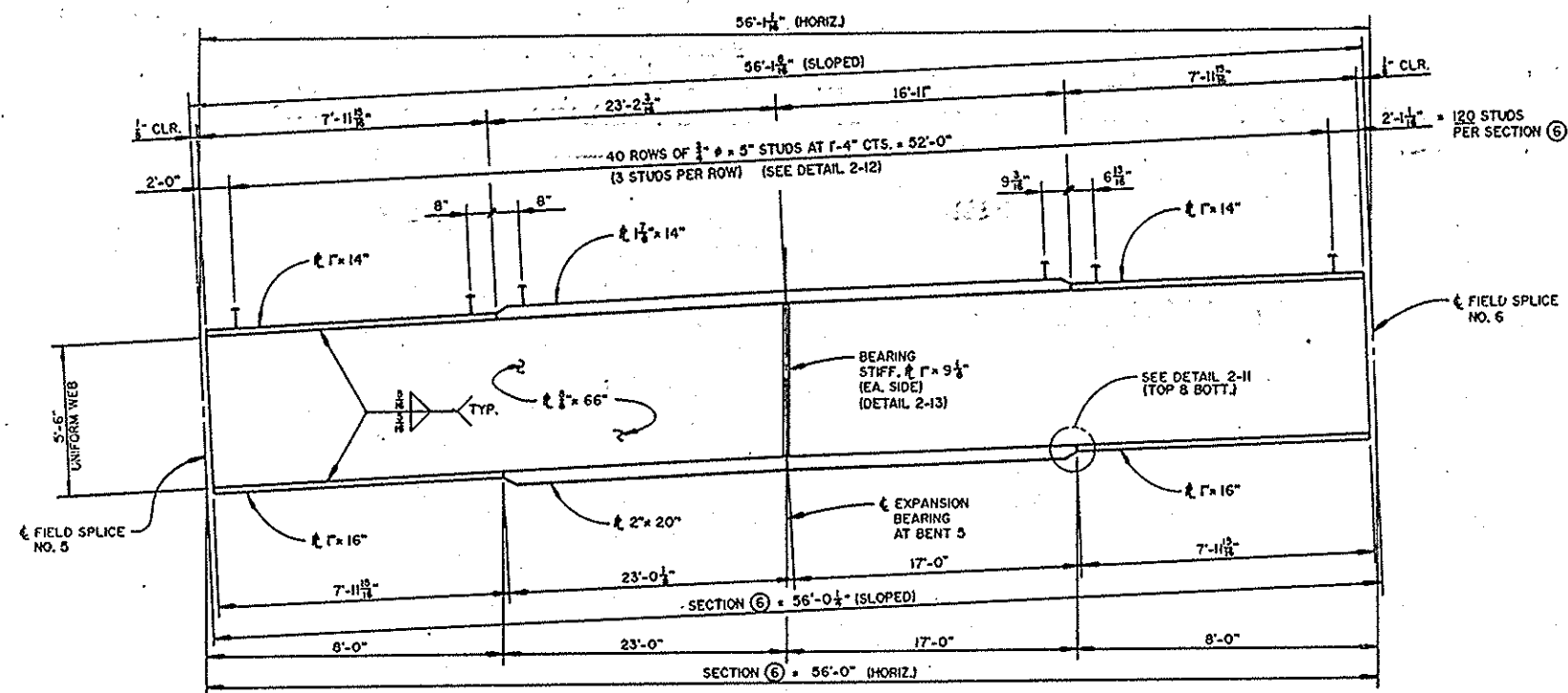
SHEET NO. 572
 TOTAL SHEETS 224



GIRDER SECTION 5 - 1 REQ'D.



GIRDER SECTION 7 - 1 REQ'D.



GIRDER SECTION 6 - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-1-REV

SHEET 12 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 4 DETAILS
CONTINUOUS UNIT #2
AUGUST 1994

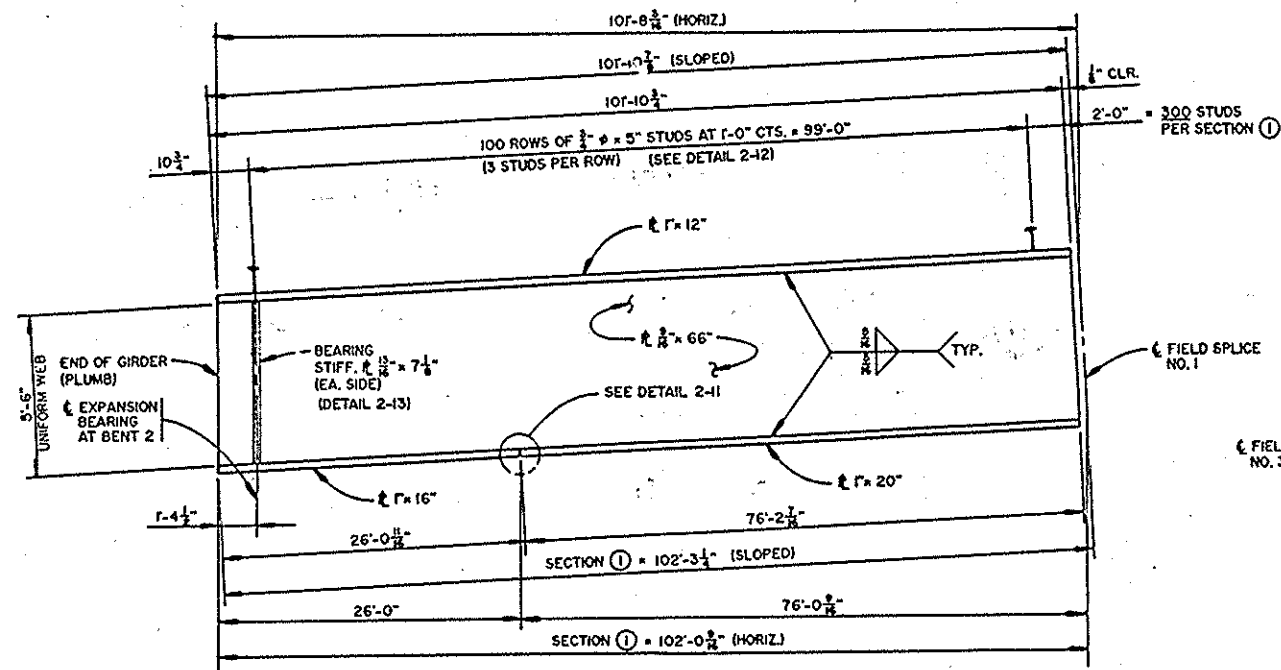


U2.032
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

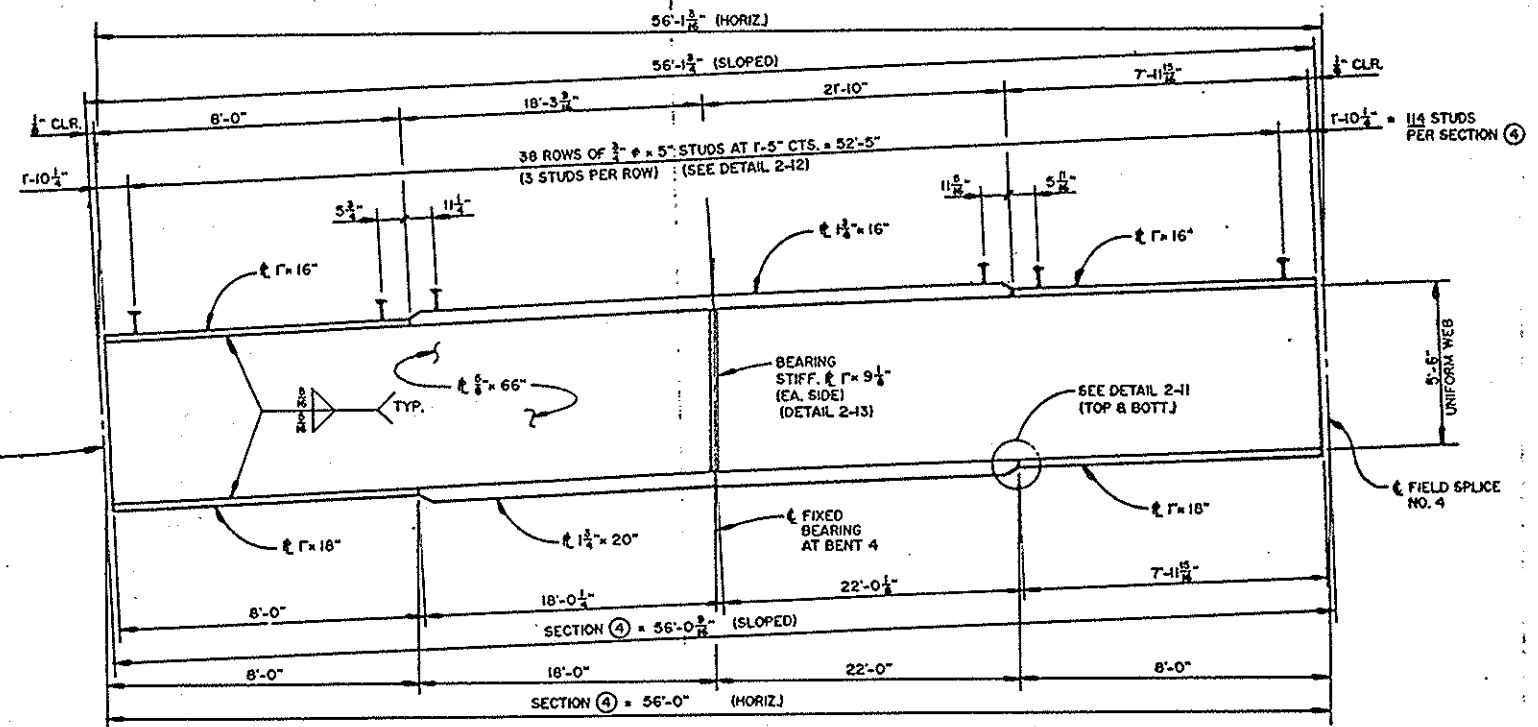
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

5-73
224

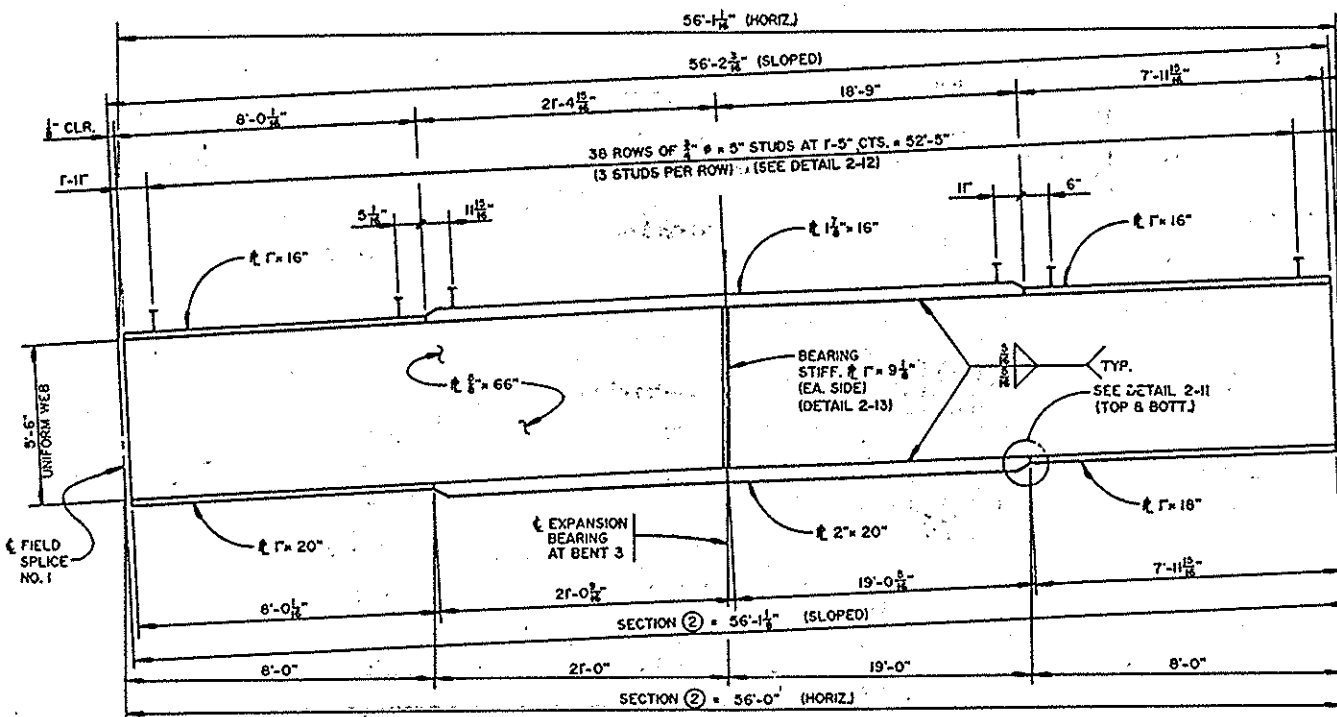
DRAWN BY: RTH DATE: 8-24
CHECKED BY: EEM DATE: 8/28



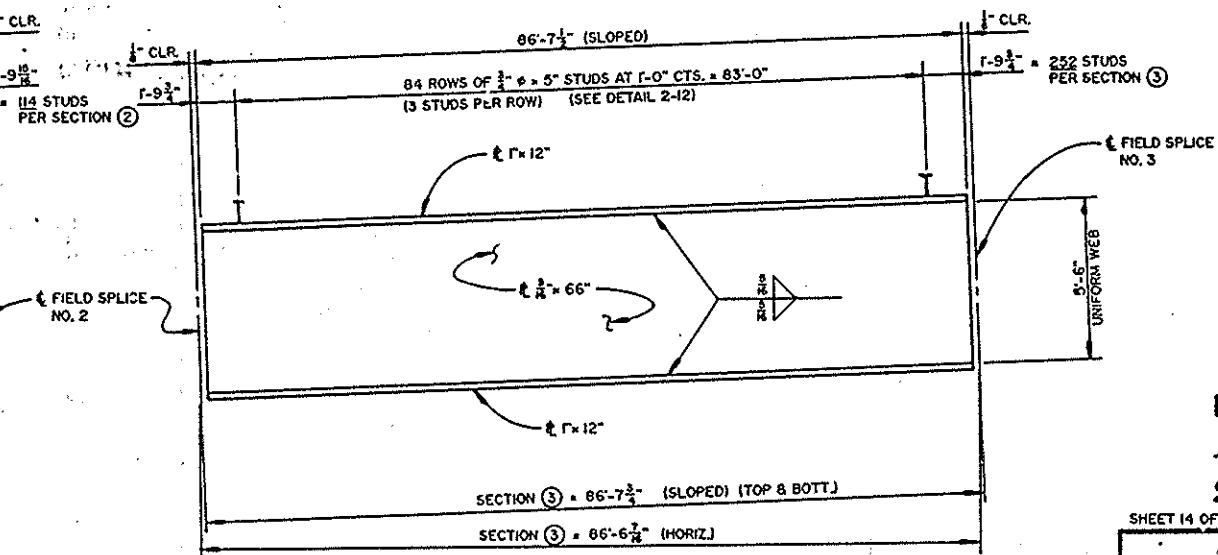
GIRDER SECTION ① - 1 REQ'D.



GIRDER SECTION ④ - 1 REQ'D.



GIRDER SECTION ② - 1 REQ'D.



GIRDER SECTION ③ - 1 REQ'D.

PROJECT No. 8-1070
BUNCOMBE COUNTY
STATION P.O.T. 16+93.00-L-REV

SHEET 14 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 5 DETAILS
CONTINUOUS UNIT #2
AUGUST 1994



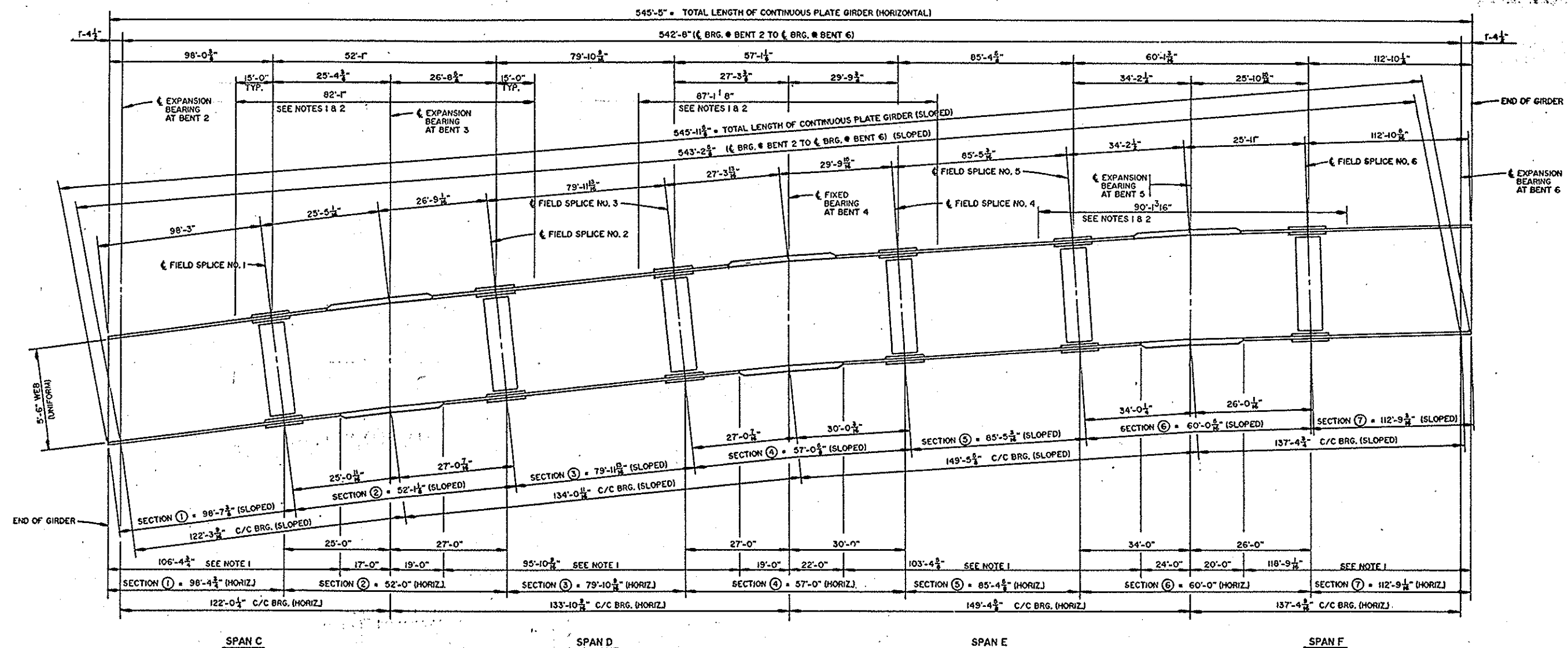
U2.034
PREPARED BY
THOMPSON-ORRISON-SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-75
2			4			TOTAL SHEETS 224

DRAWN BY: RJS
CHECKED BY: CEM
DATE: 8-24-94
DATE: 8/95



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
GIRDER 5 DETAILS
CONTINUOUS UNIT #2
 AUGUST 1994



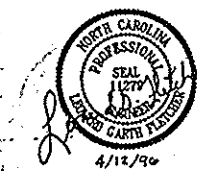
GIRDER 6 ELEVATION

- NOTE 1: CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPICE PLATES AND FLANGE SPICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.
- IF THE PERMITTED SHOP FLANGE SPICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.
- NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 16 OF 27

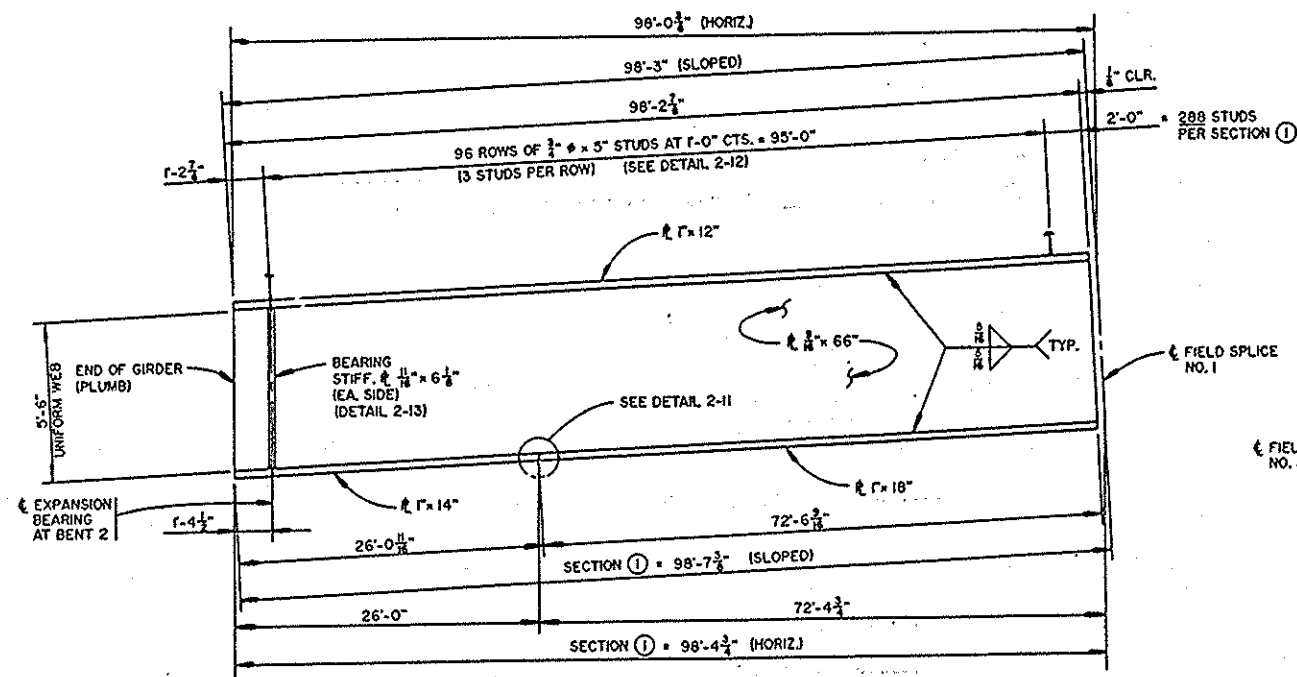
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 6 ELEVATION
CONTINUOUS UNIT #2
JULY 1994



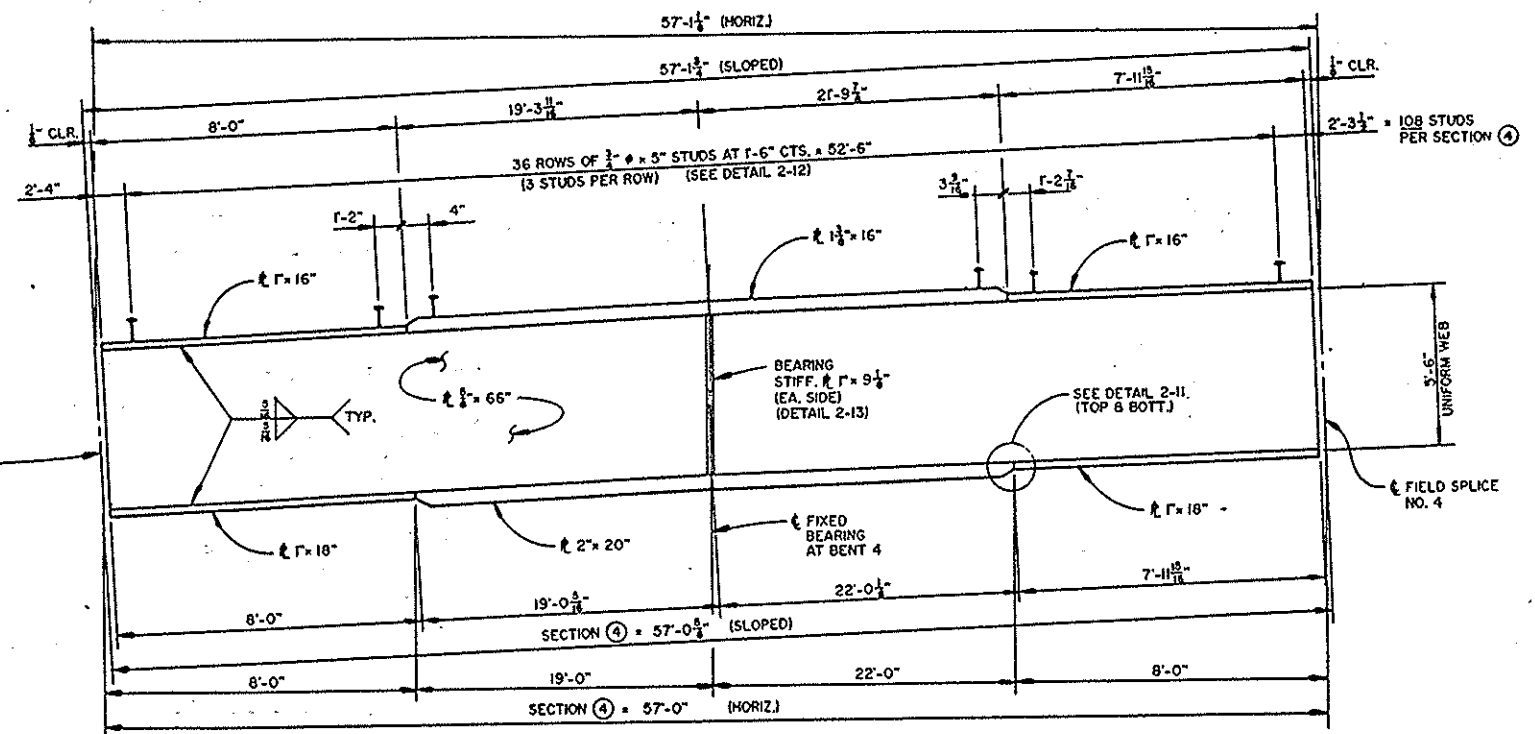
U2.056
PREPARED BY
THOMPSON-BORDON-SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			5			5-77
2			4			224

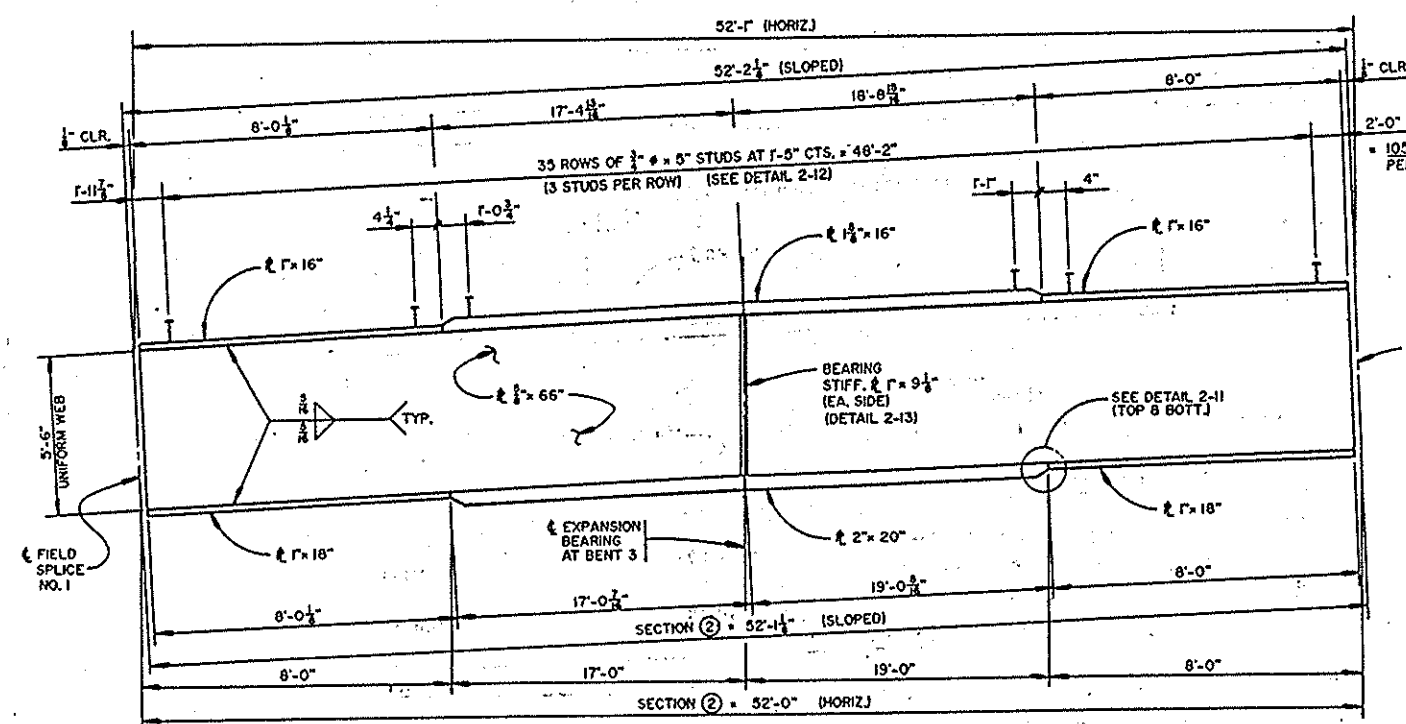
DRAWN BY: BTD
CHECKED BY: ECM
DATE: 1-94
DATE: 6-1-95



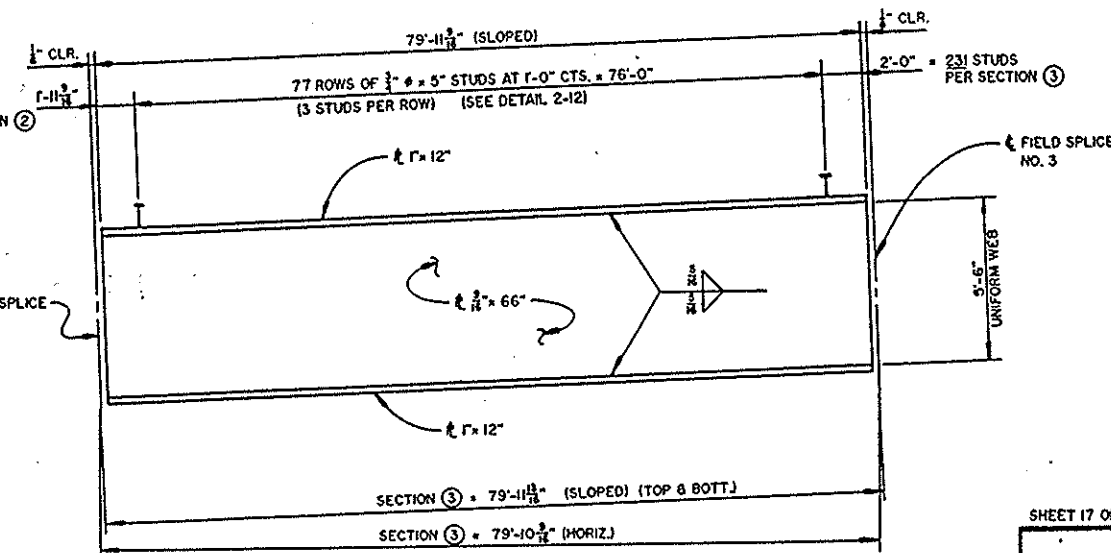
GIRDER SECTION 1 - 1 REQ'D.



GIRDER SECTION 4 - 1 REQ'D.



GIRDER SECTION 2 - 1 REQ'D.



GIRDER SECTION 3 - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION, P.O.T. 16+93.00-L-REV

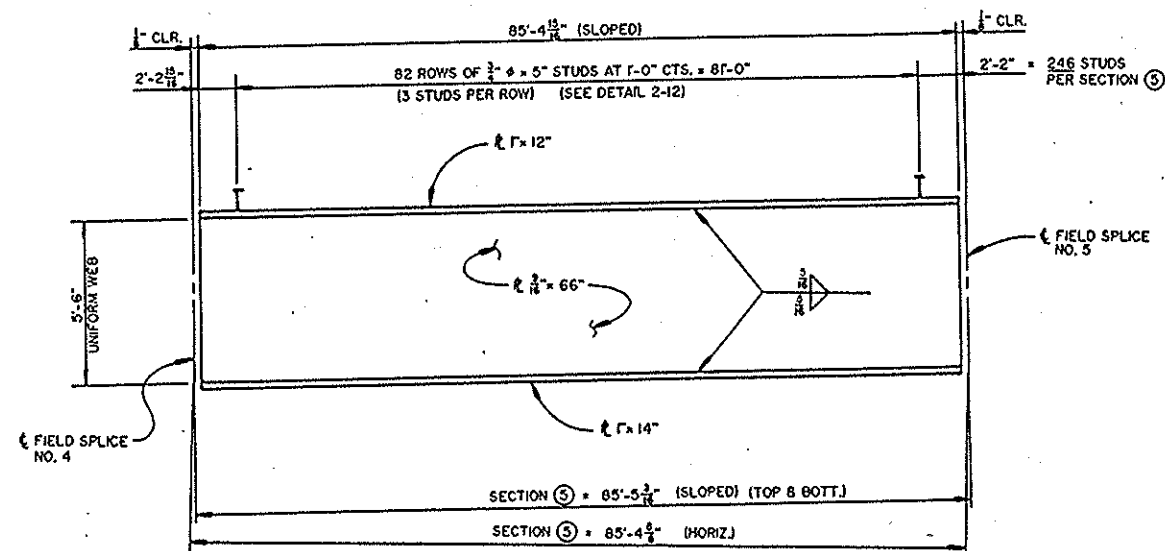
SHEET 17 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 6 DETAILS
CONTINUOUS UNIT #2
AUGUST 1994

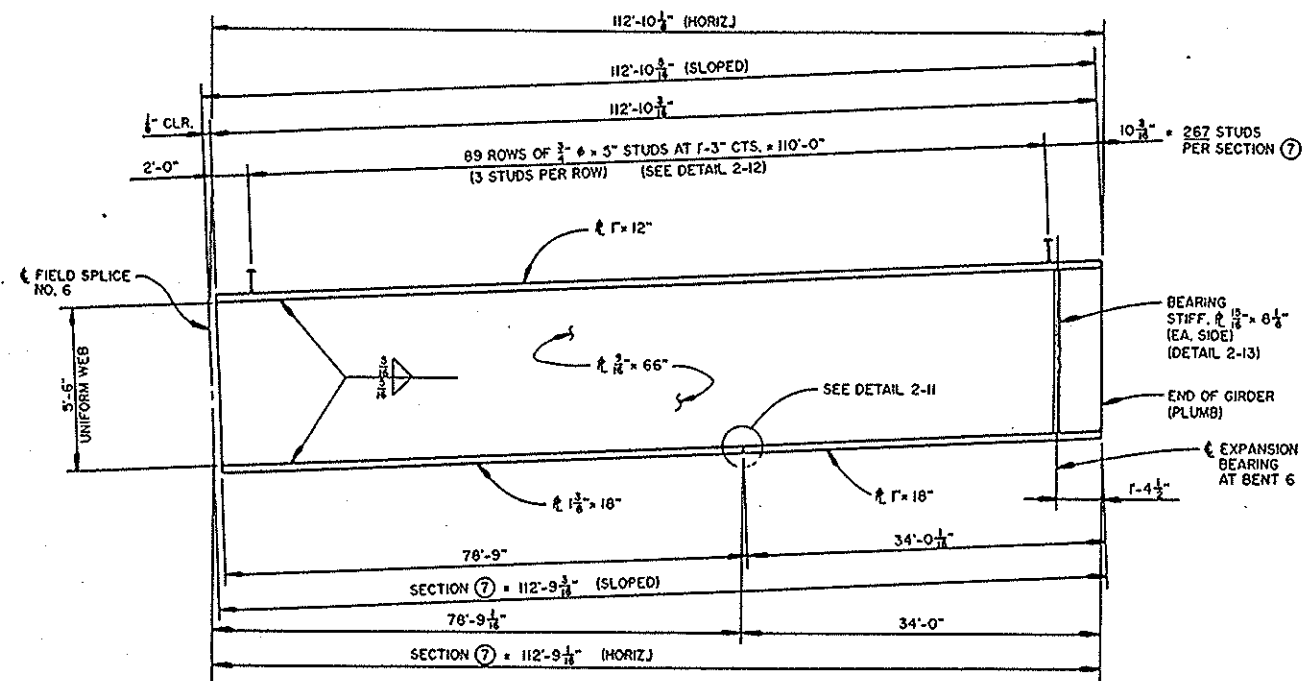
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			5		
2			6		

42.037
PREPARED BY
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED

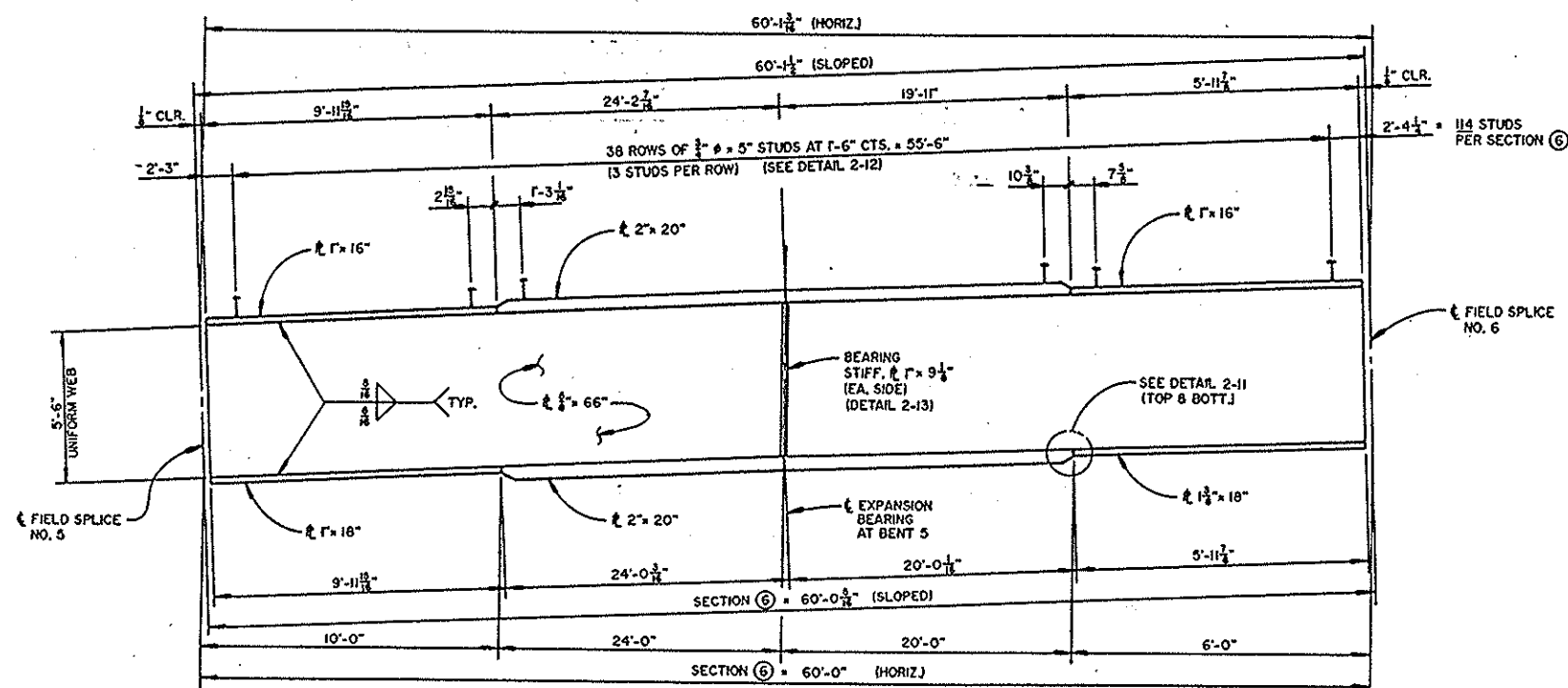
SHEET NO. 5-78
TOTAL SHEETS 224



GIRDER SECTION (5) - I REQ'D.



GIRDER SECTION (7) - I REQ'D.



GIRDER SECTION (6) - I REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION, P.O.T. 16+93.00-L-REV

SHEET 18 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 6 DETAILS
CONTINUOUS UNIT #2

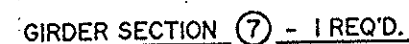
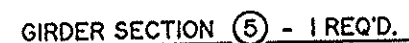
AUGUST 1994



412.038
PREPARED BY
THOMPSON - BORDON - SHOOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			579
2			4			224

DRAWN BY: RFL
CHECKED BY: RFL
DATE: 8-24-94
DATE: 8-24-94



SHEET 21 OF 27

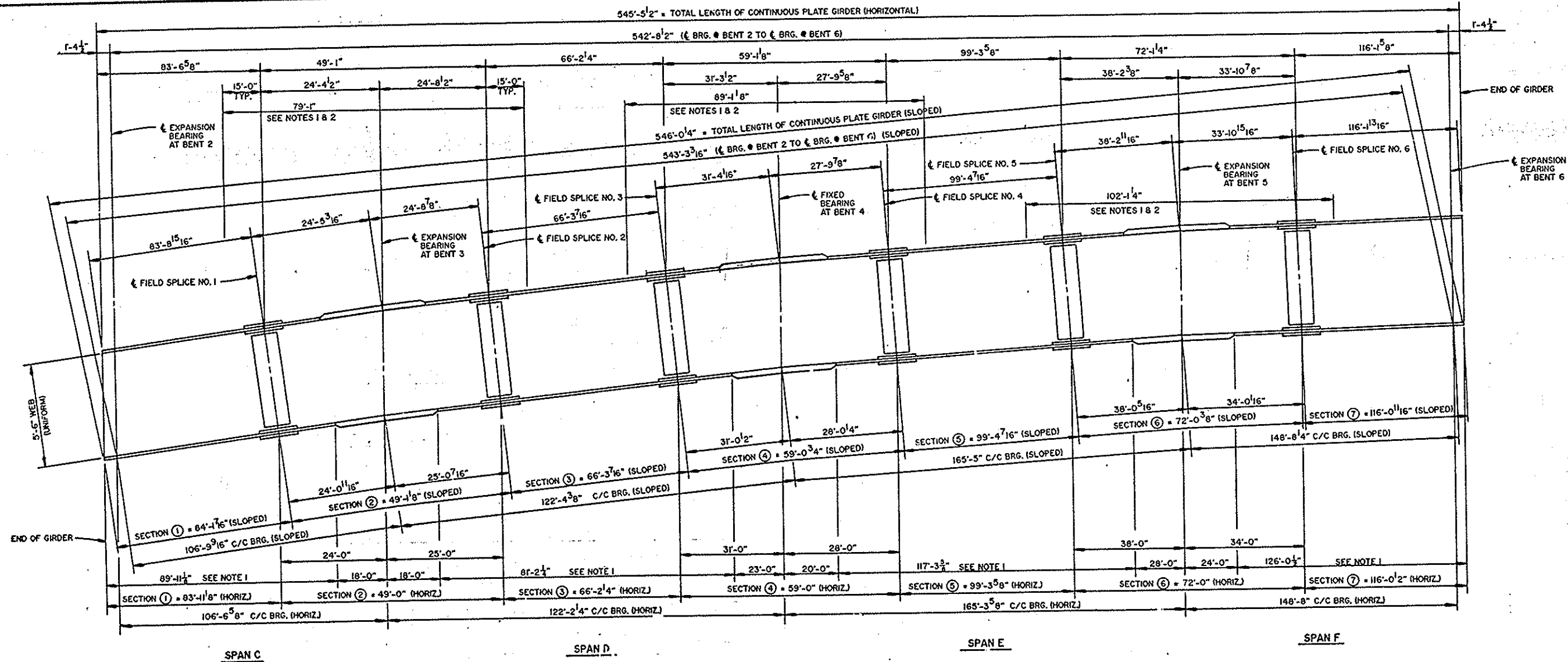
AUGUST 1994

REVISIONS						SHEET NO. 5-82
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			



U2.04
— PREPARED BY —
THOMPSON-BORDON-SHOON
ENGINEERS, INCORPORATED

DRAWN BY: RTJ DATE: 8-94
CHECKED BY: CCM DATE: 6/95



GIRDER 8 ELEVATION

- NOTE 1: CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPlice PLATES AND FLANGE SPlice PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.
- IF THE PERMITTED SHOP FLANGE SPlice, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.
- NOTE 2: NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION, P.O.T. 16+93.00-L-REV

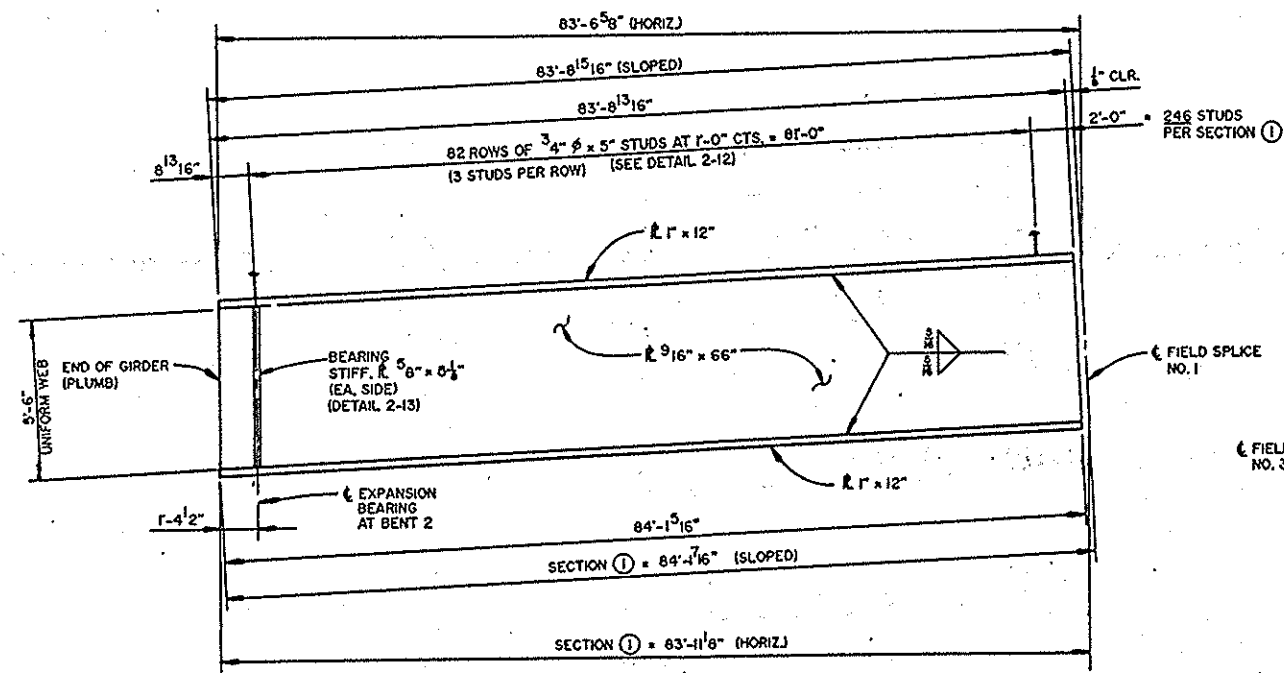
SHEET 22 OF 27

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALPH H. SUPERSTRUCTURE STRUCTURAL STEEL GIRDER 8 ELEVATION CONTINUOUS UNIT #2 JULY 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			4		
TOTAL SHEETS					274

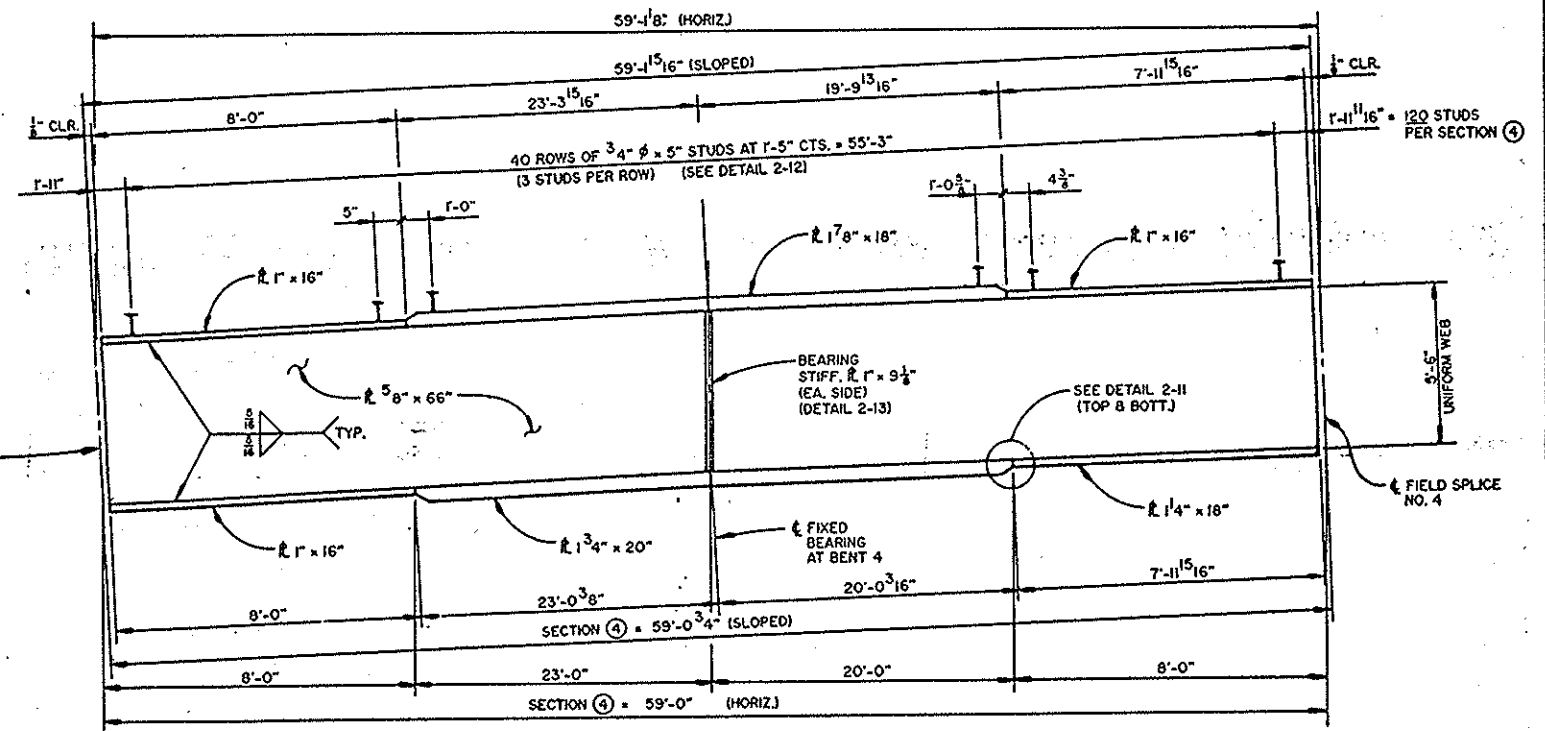


U2.042
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

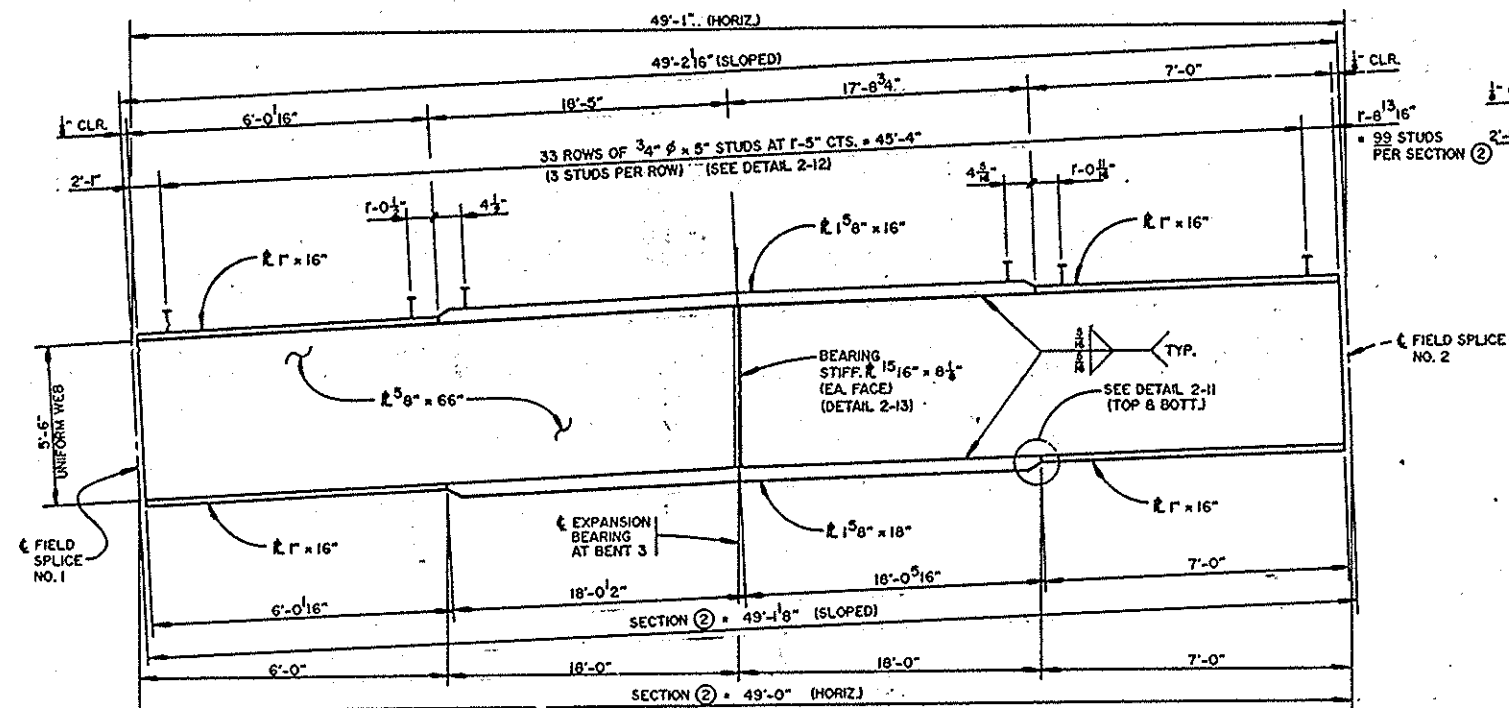
DRAWN BY: BTH DATE: 8/28
CHECKED BY: BTH DATE: 6/95



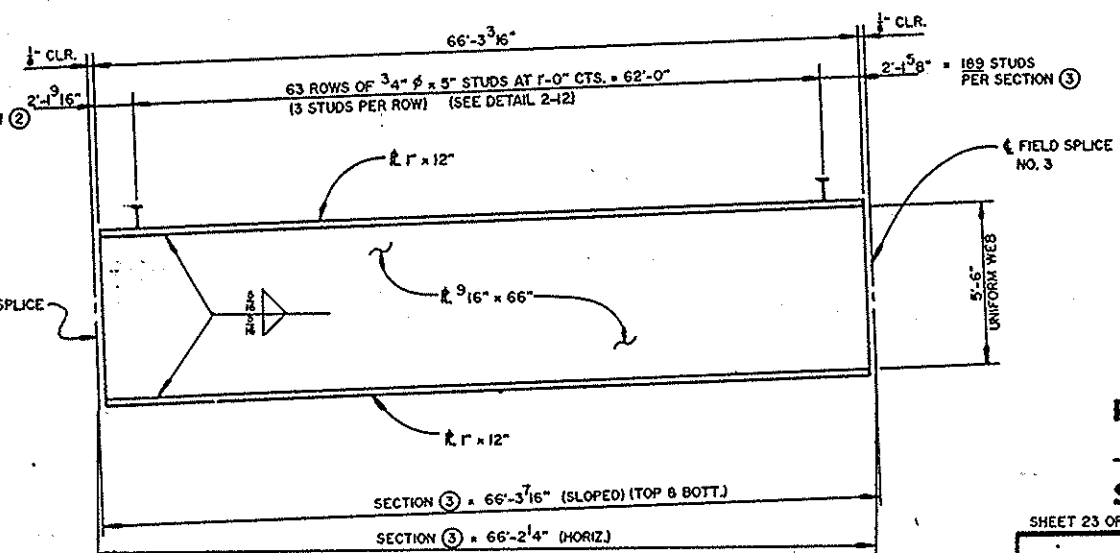
GIRDER SECTION ① - 1 REQ'D.



GIRDER SECTION ④ - 1 REQ'D.



GIRDER SECTION ② - 1 REQ'D.



GIRDER SECTION ③ - 1 REQ'D.

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV
 SHEET 23 OF 27

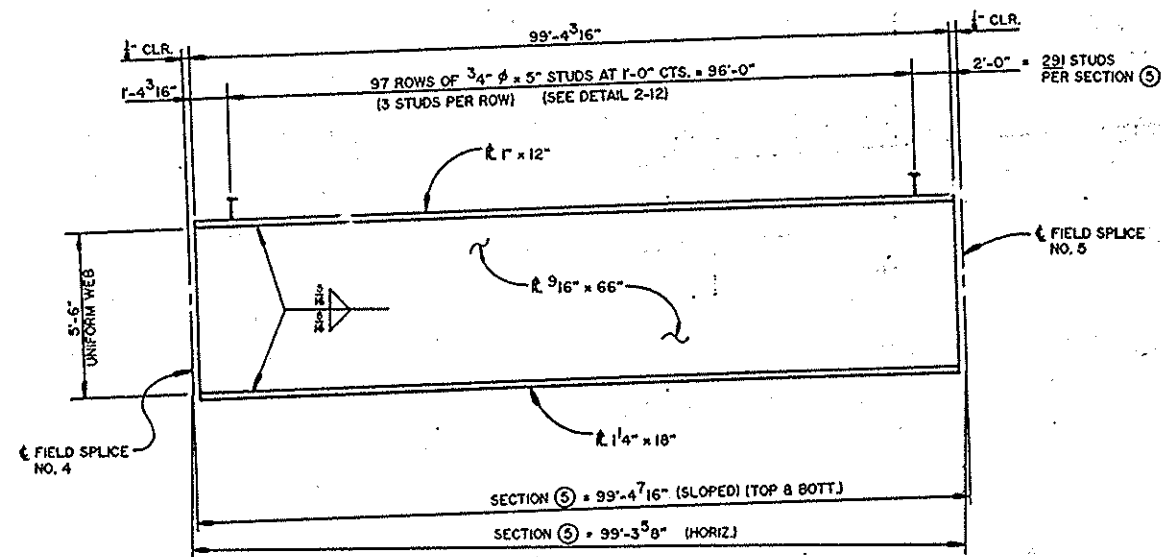
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
 GIRDER 8 DETAILS
 CONTINUOUS UNIT #2
 AUGUST 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			2			584
2			3			224

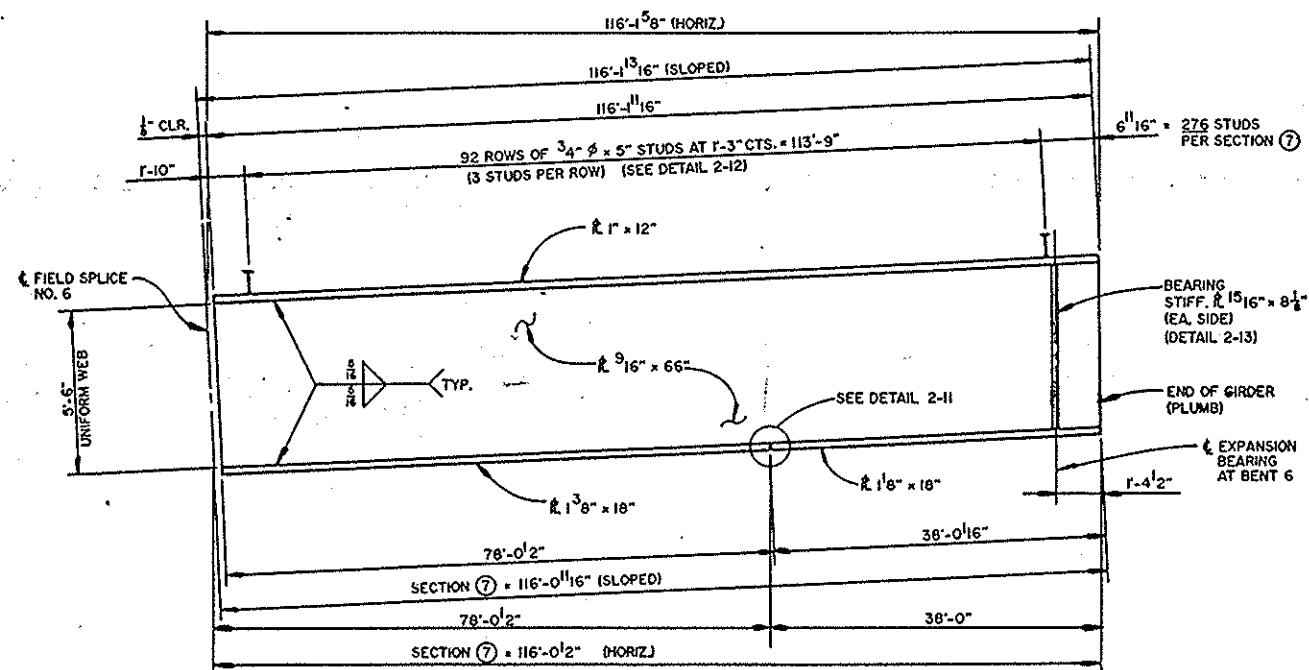


4/12/94
 U2.0A3
 PREPARED BY
 THOMPSON - BORDEN - SHOOK
 ENGINEERS, INCORPORATED

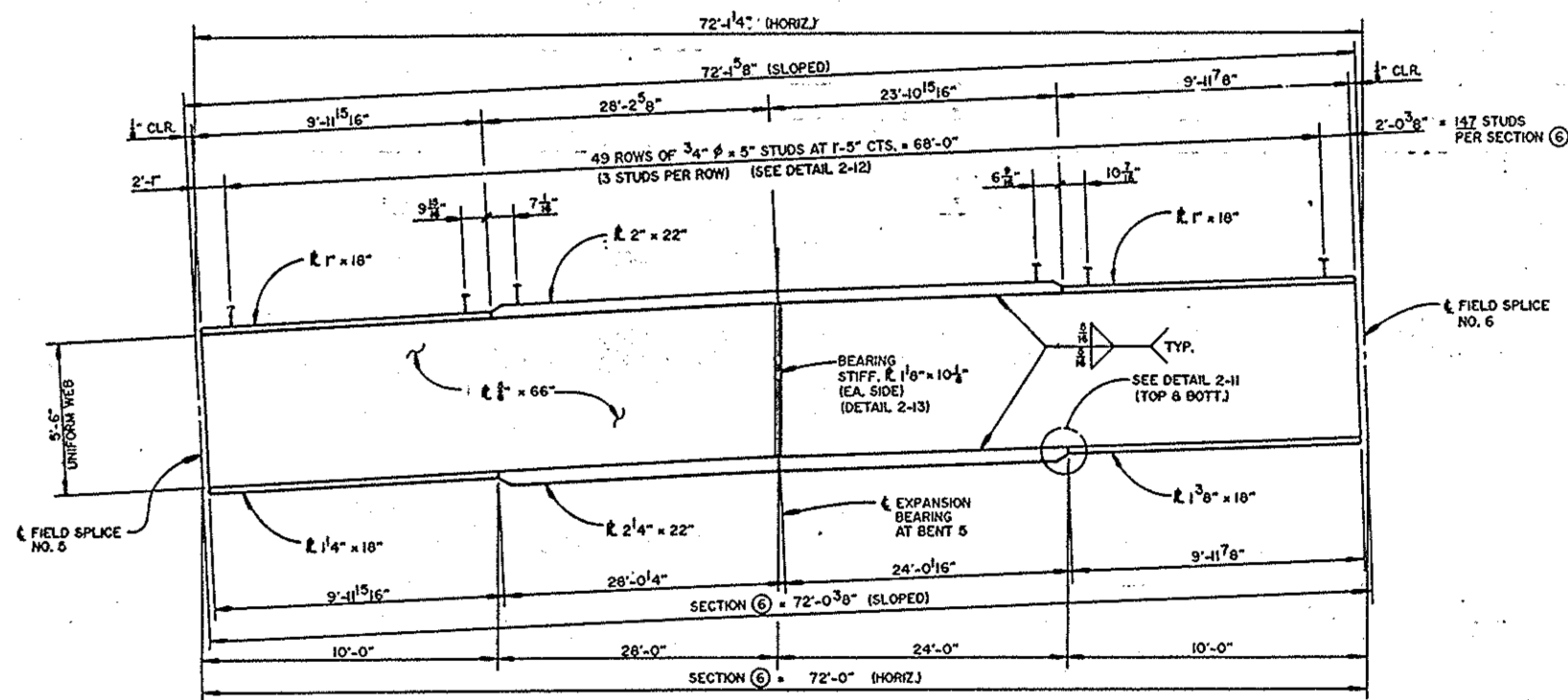
DRAWN BY: RJS
 CHECKED BY: GEM
 DATE: 8-24
 DATE: 4/95



GIRDER SECTION ⑤ - 1 REQ'D.



GIRDER SECTION ⑦ - 1 REQ'D.



GIRDER SECTION ⑥ - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION, P.O.T. 16+93.00-L-REV

SHEET 24 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER & DETAILS
CONTINUOUS UNIT #2

AUGUST 1994

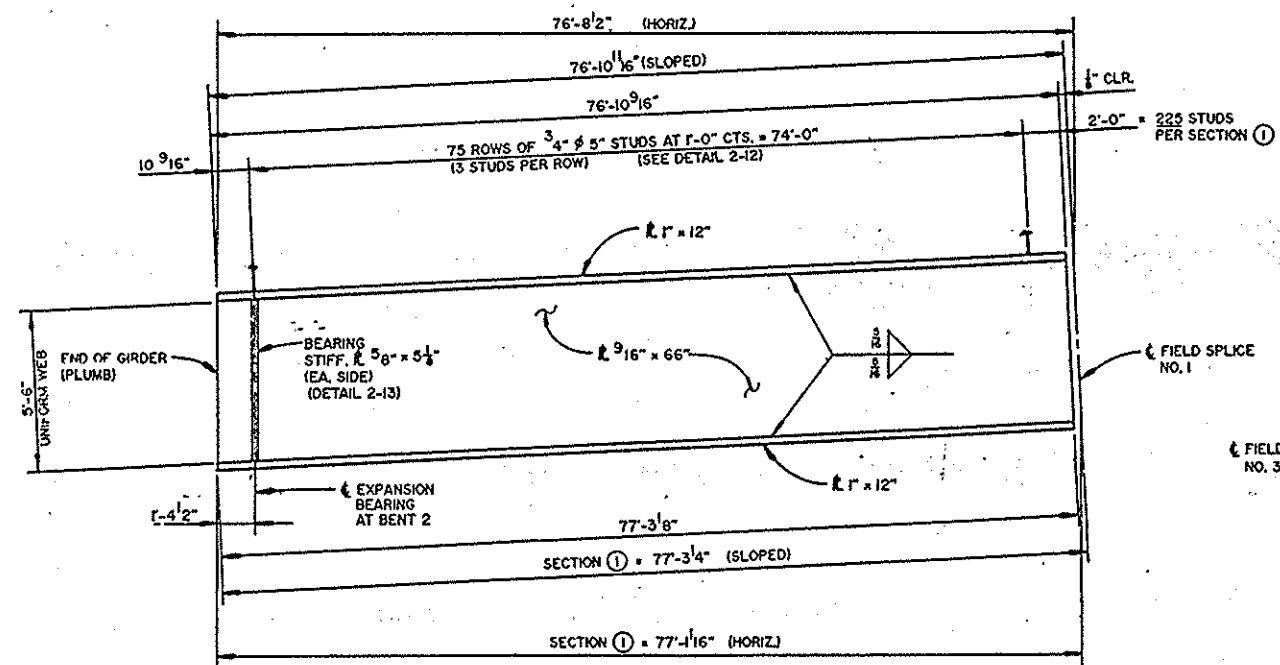
REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
				TOTAL SHEETS	224



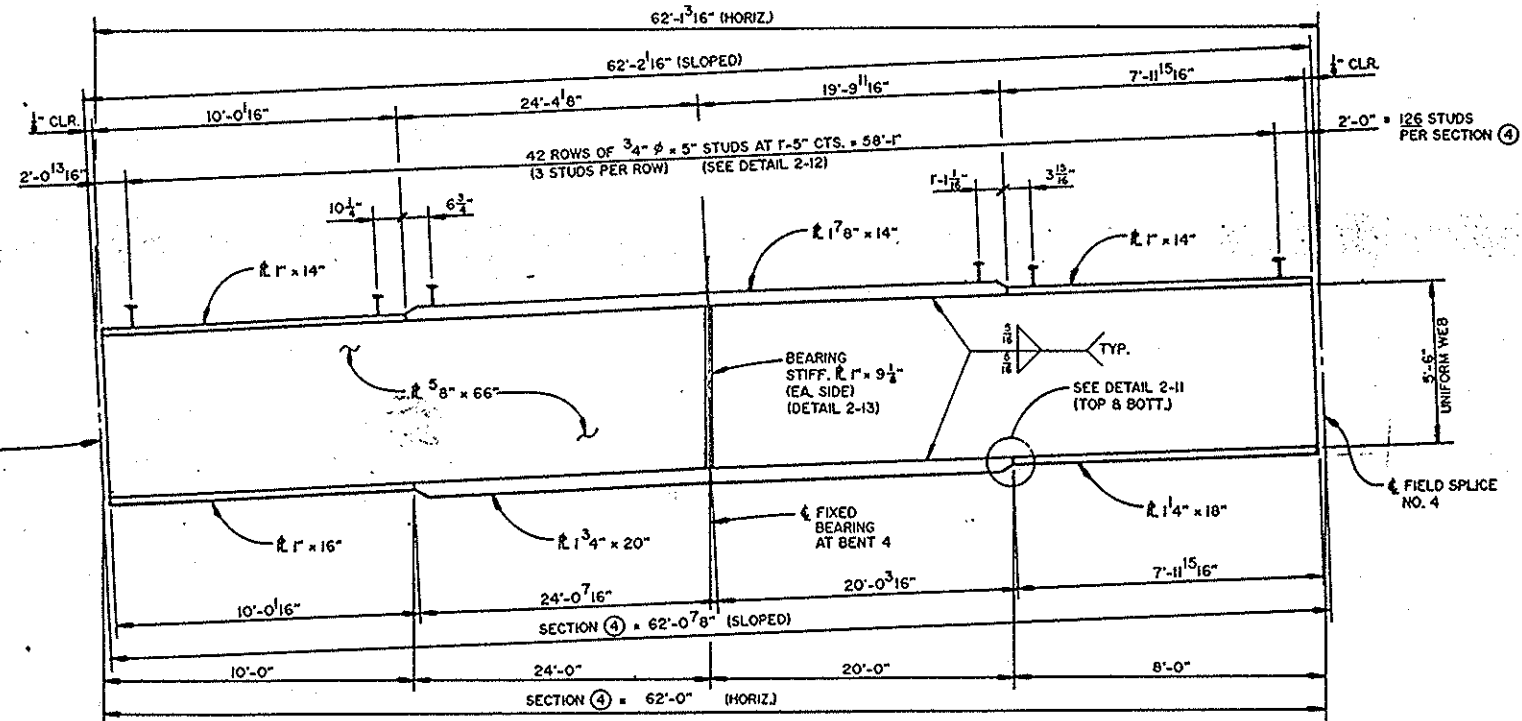
U2.044

PREPARED BY
THOMPSON-BORON-SHOCK
ENGINEERS, INCORPORATED

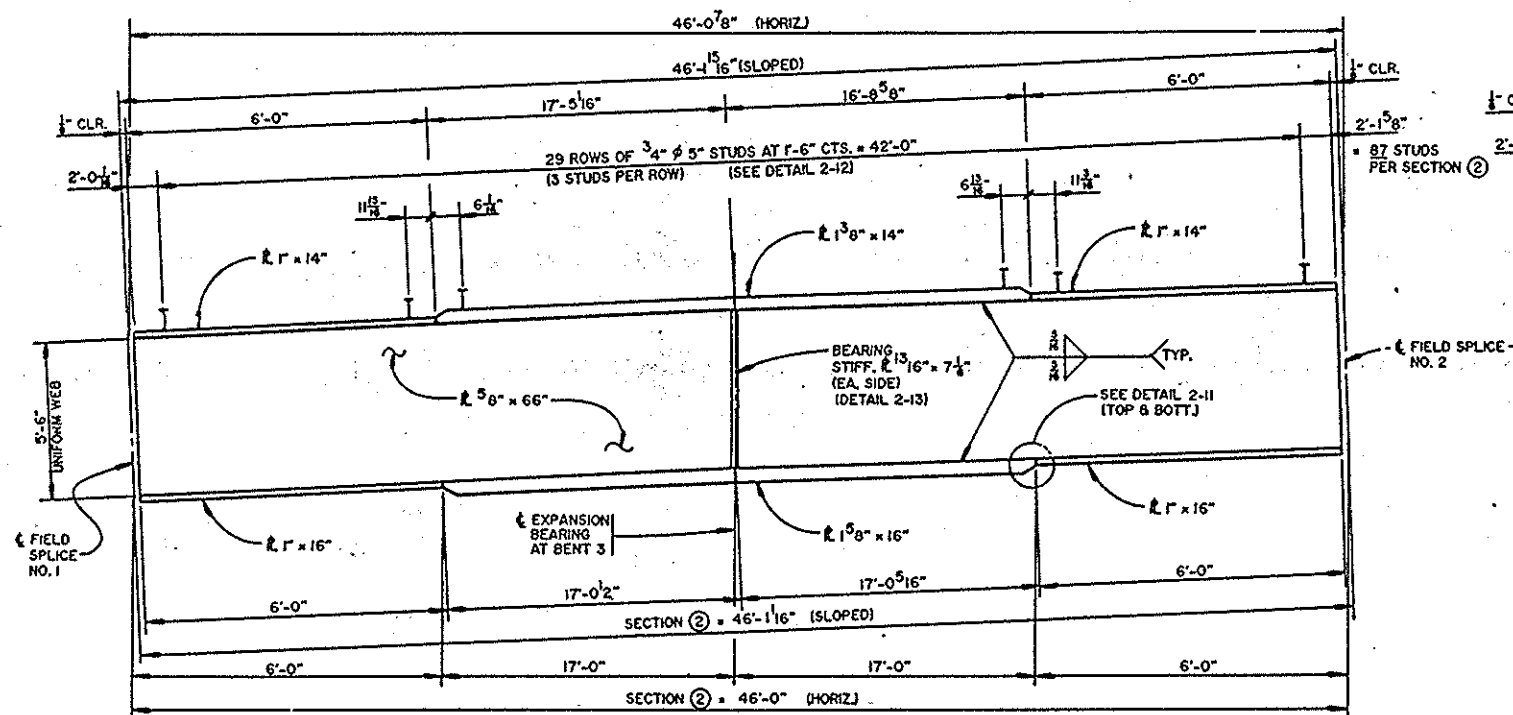
DRAWN BY: RSM DATE: 8-24
CHECKED BY: RSM DATE: 8/25



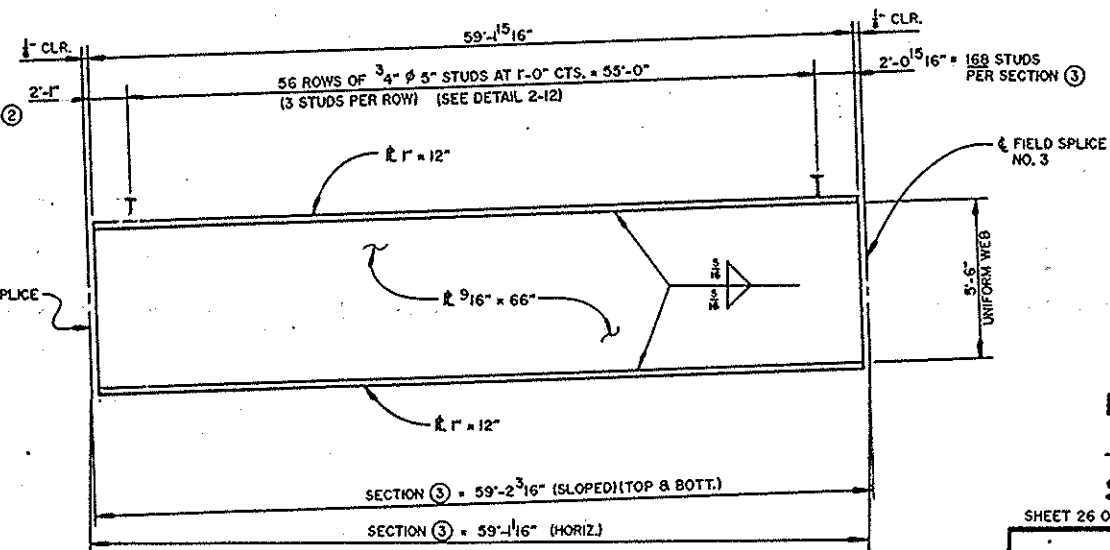
GIRDER SECTION ① - 1 REQ'D.



GIRDER SECTION ④ - 1 REQ'D.



GIRDER SECTION ② - 1 REQ'D.



GIRDER SECTION ③ - 1 REQ'D.

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 26 OF 27

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 9 DETAILS
CONTINUOUS UNIT #2

AUGUST 1994

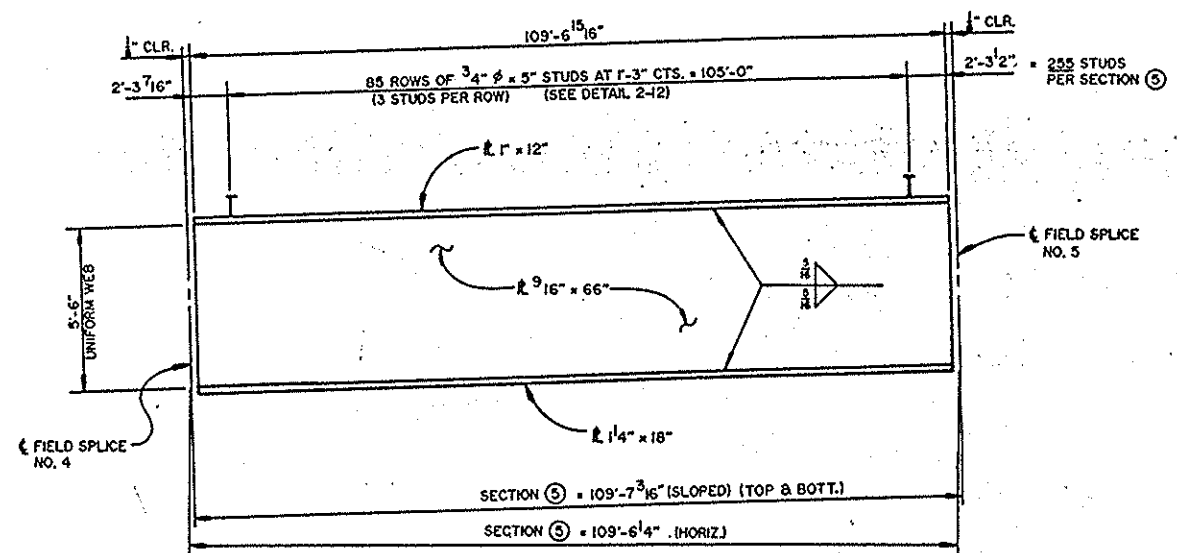
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

SHEET
NO. 5-87
TOTAL
SHEETS 224

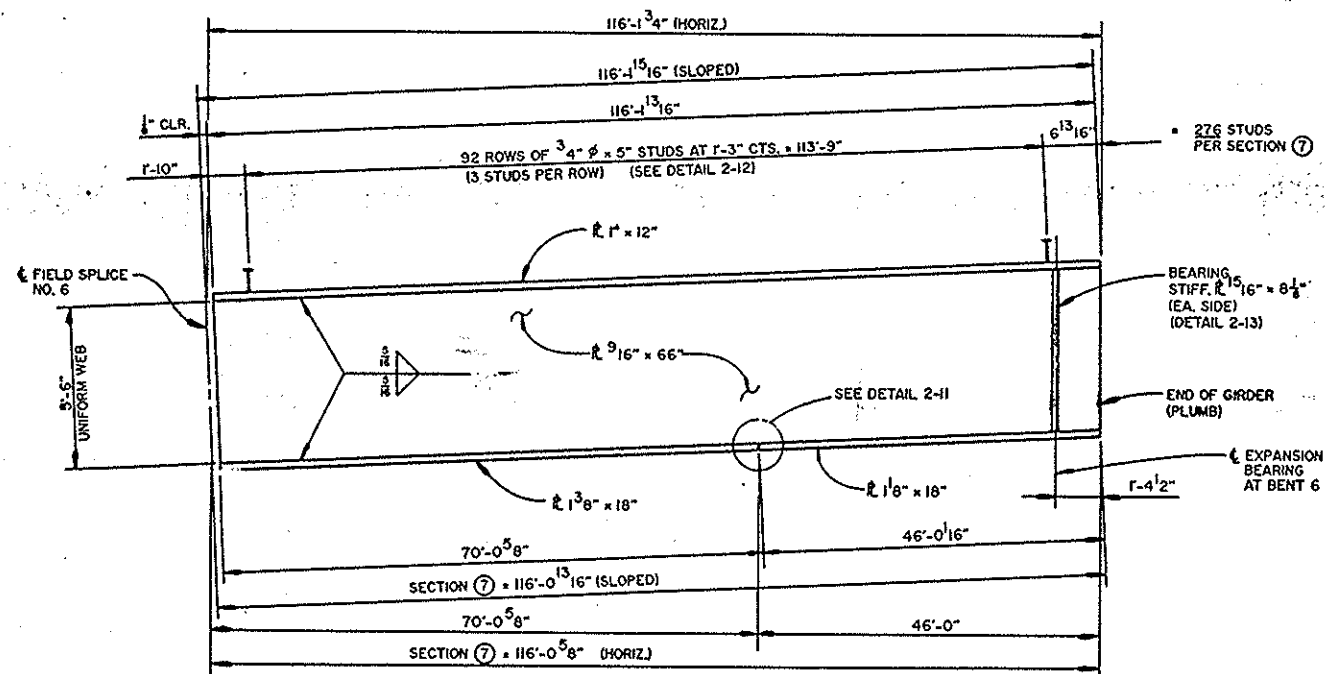


U2.046
PREPARED BY
THOMPSON - BORDON - SNOOK
ENGINEERS, INCORPORATED

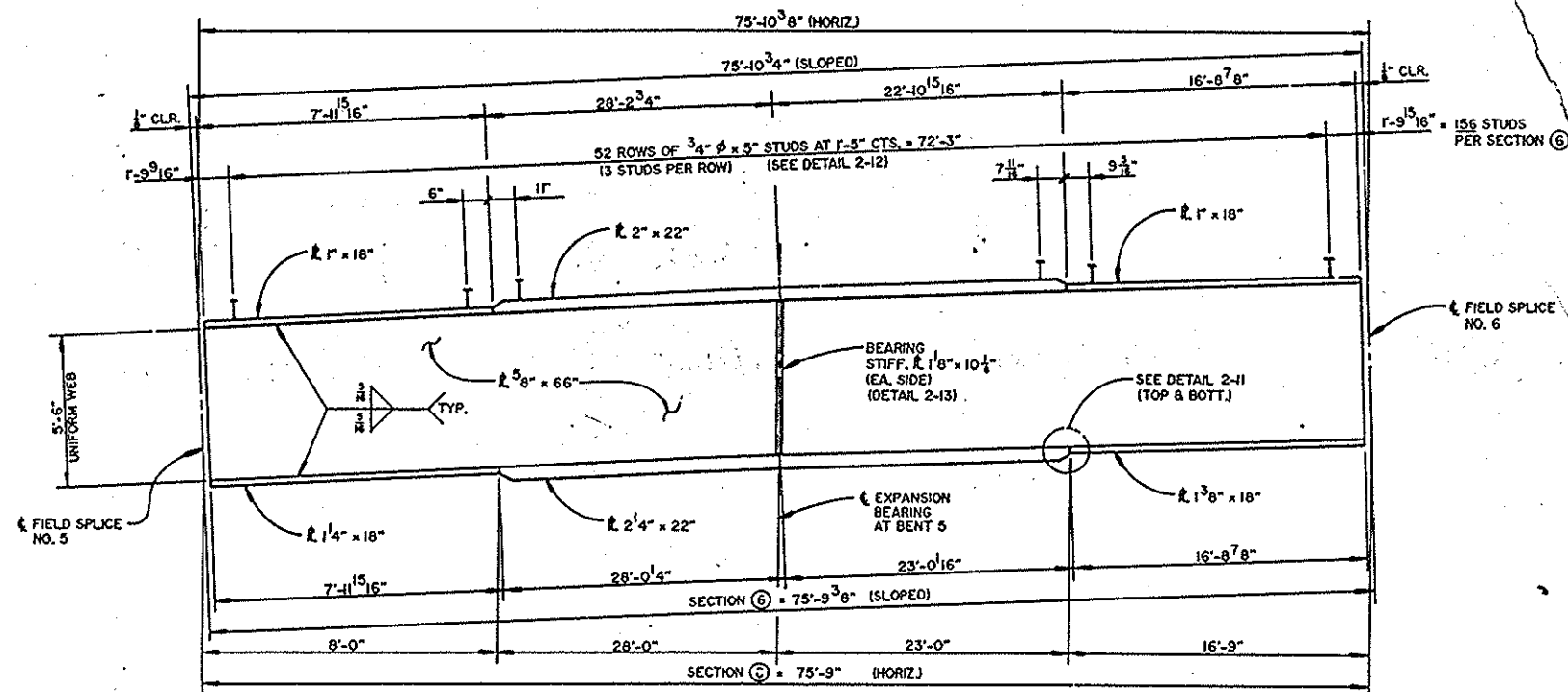
DRAWN BY: RFL DATE: 6-74
CHECKED BY: JCS DATE: 6-95



GIRDER SECTION ⑤ - 1 REQ'D.



GIRDER SECTION ⑦ - 1 REQ'D.



GIRDER SECTION ⑥ - 1 REQ'D.

DRAWN BY: RLW DATE: 8-26
 CHECKED BY: CEM DATE: 6/95

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV

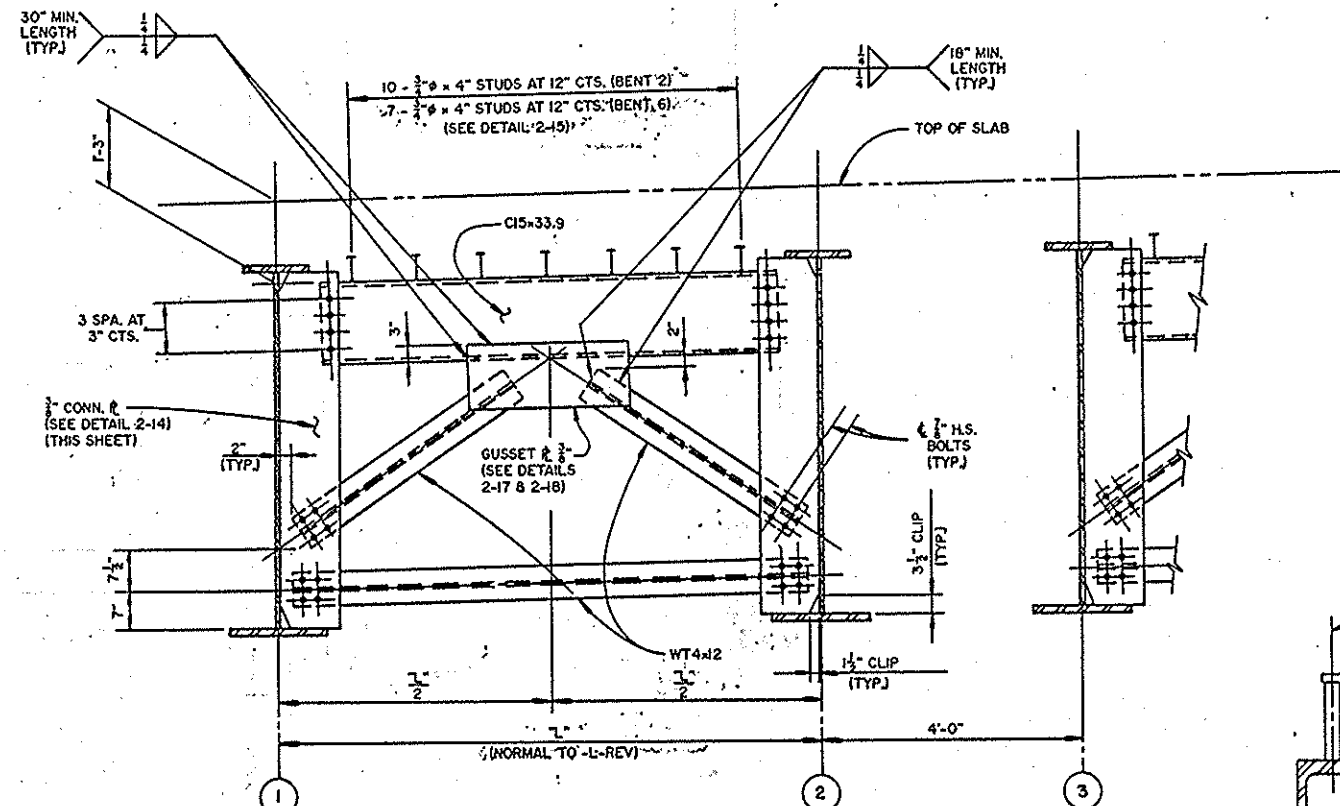
SHEET 27 OF 27

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
 GIRDER 9 DETAILS
 CONTINUOUS UNIT #2
 AUGUST 1994

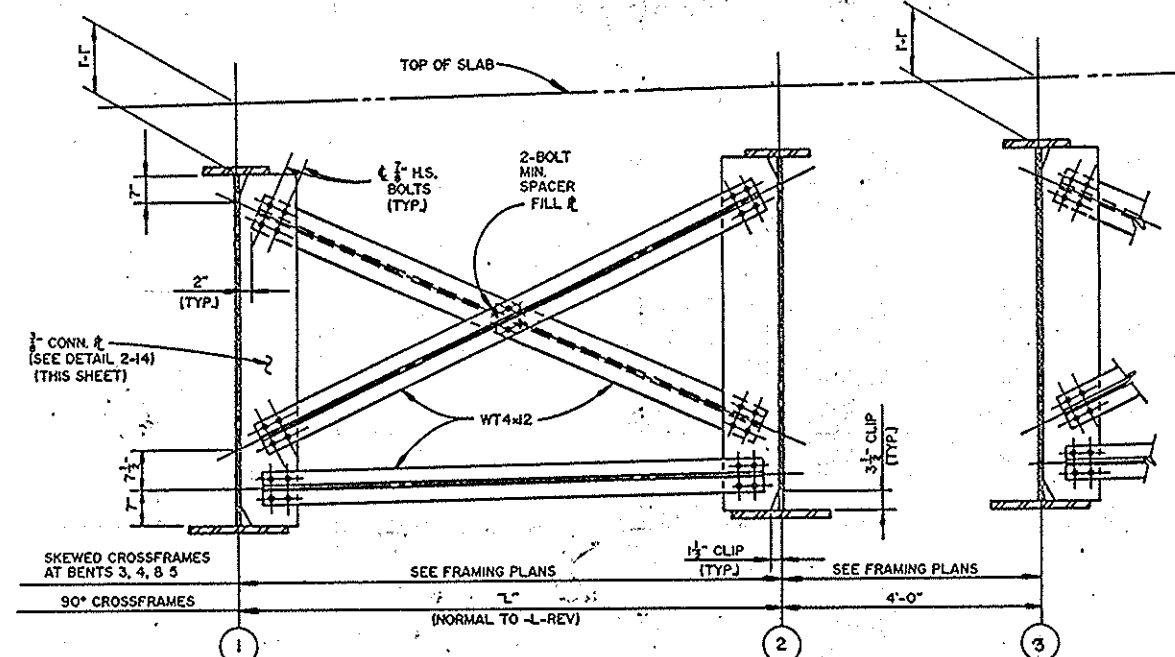


U2.047
 PREPARED BY
 THOMPSON-BORDON-SHOOK
 ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			5			5-88
2			4			TOTAL SHEETS 224



BENT 2 & BENT 6 CROSSFRAME
MARK CF-200, CF-201, CF-281 & CF-282

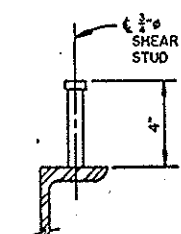


INTERMEDIATE CROSSFRAME

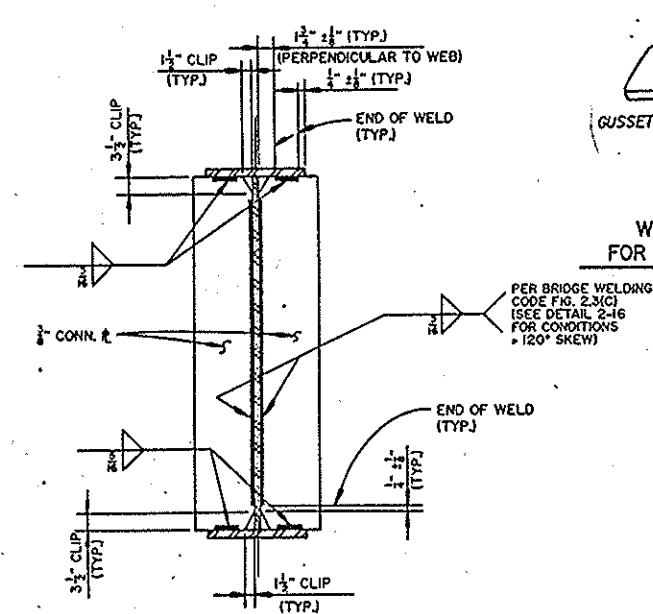
MARK CF-202 THRU CF-215, MARK CF-223 THRU CF-240, MARK CF-246 THRU CF-261 & MARK CF-269 THRU CF-280 — 90° CROSSFRAMES
MARK CF-216 THRU CF-222, MARK CF-241 THRU CF-247 & MARK CF-262 THRU CF-268 — SKEWED CROSSFRAMES AT BENTS 3, 4, & 5

DRAWN BY: RFV DATE: 2-24
CHECKED BY: ECM DATE: 6/95

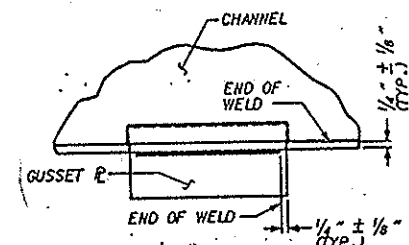
CROSSFRAME LENGTHS "L" (90° CROSSFRAMES ONLY)									
BENT 2 & BENT 6 CROSSFRAME					INTERMEDIATE CROSSFRAME				
	"L"					"L"			
CF-200	10.9888'	CF-202	10.8782'	CF-228	9.9339'	CF-254	9.2580'		
CF-201	10.9882'	CF-203	10.8724'	CF-229	9.9159'	CF-255	9.1751'		
CF-281	8.3862'	CF-204	10.7676'	CF-230	9.8642'	CF-256	9.1619'		
CF-282	8.2618'	CF-205	10.7566'	CF-231	9.8494'	CF-257	9.0743'		
		CF-206	10.6571'	CF-232	9.7946'	CF-258	9.0929'		
		CF-207	10.6407'	CF-233	9.7829'	CF-259	9.0021'		
		CF-208	10.5465'	CF-234	9.7249'	CF-260	9.0272'		
		CF-209	10.5249'	CF-235	9.7163'	CF-261	8.9333'		
		CF-210	10.4631'	CF-236	9.6552'	CF-269	8.9567'		
		CF-211	10.4376'	CF-237	9.6498'	CF-270	8.8594'		
		CF-212	10.3798'	CF-238	9.5855'	CF-271	8.8894'		
		CF-213	10.3503'	CF-239	9.5833'	CF-272	8.7889'		
		CF-214	10.2965'	CF-240	9.5159'	CF-273	8.7889'		
		CF-215	10.2630'	CF-248	9.5168'	CF-274	8.6836'		
		CF-223	10.2131'	CF-249	9.4462'	CF-275	8.6883'		
		CF-224	10.1757'	CF-250	9.4503'	CF-276	8.5782'		
		CF-225	10.0978'	CF-251	9.3765'	CF-277	8.5877'		
		CF-226	10.0548'	CF-252	9.3542'	CF-278	8.4729'		
		CF-227	9.9824'	CF-253	9.2758'	CF-279	8.4572'		
						CF-280	8.3675'		



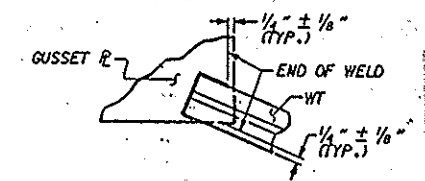
DETAIL 2-15



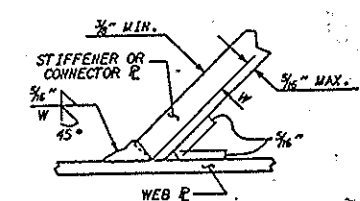
DETAIL 2-14



WELD TERMINATION DETAIL FOR GUSSET PLATE CONNECTION
DETAIL 2-17



WELD TERMINATION DETAIL FOR GUSSET PLATE/WT CONNECTION
DETAIL 2-18



DETAIL 2-16
120° SKEW

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL					
CROSSFRAME DETAILS CONTINUOUS UNIT #2					
SEPT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. <u>5-89</u>
					TOTAL SHEETS <u>224</u>



02.048
PREPARED BY
THOMPSON-BORDON-BROOK
ENGINEERS, INCORPORATED

STRUCTURAL STEEL NOTES - CONTINUOUS UNIT NO. 2

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED.

CAMBERED GIRDER LENGTHS SHALL BE ADJUSTED AND BEARINGS ARE TO BE PLACED ON THE CAMBERED GIRDER SO AS TO BE ALIGNED WITH THE ANCHORS AFTER DEAD LOAD DEFLECTION HAS OCCURRED, SHOP PLANS SHALL BE PREPARED ACCORDINGLY.

FOR POT BEARINGS, SEE SPECIAL PROVISIONS.

AT ALL POINTS OF SUPPORT IN SPANS C, D, E & F, NUTS FOR ANCHOR BOLTS SHALL BE TIGHTENED FINGER TIGHT AND GIVEN AN ADDITIONAL 1/4 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP TOOL.

SHEAR STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELDS.

ALL FIELD CONNECTIONS TO BE 7/8" DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED. SEE ALSO BOLTED FIELD SPLICE DETAILS.

SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED TOP FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION OR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS. KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. NO SHOP SPLICES WILL BE PERMITTED IN THE BOTTOM FLANGE PLATES.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

END OF GIRDERS SHALL BE PLUMB.

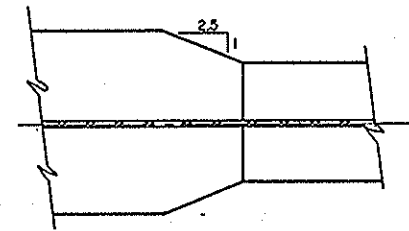
WHEN WELDING THE SOLE PLATE TO THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE BEARING DOES NOT EXCEED 250° F. TEMPERATURES ABOVE THIS MAY DAMAGE THE TFE OR ELASTOMER.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

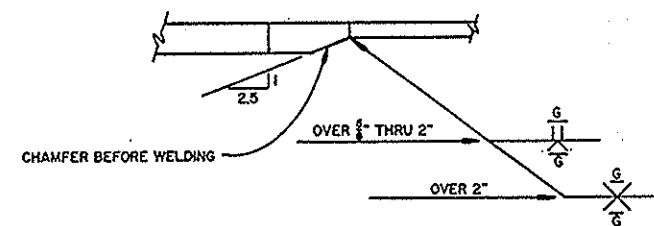
IN LIEU OF THE TURN OF THE NUT METHOD, TENSION ON A325 BOLTS MAY BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS. FOR DIRECT TENSION INDICATORS, SEE SPECIAL PROVISIONS.

FOR RADIOGRAPHIC TESTING OF BUTT WELDS, SEE SPECIAL PROVISIONS.

FOR SURFACE PREPARATION OF UNPAINTED STRUCTURAL STEEL,
SEE SPECIAL PROVISIONS.

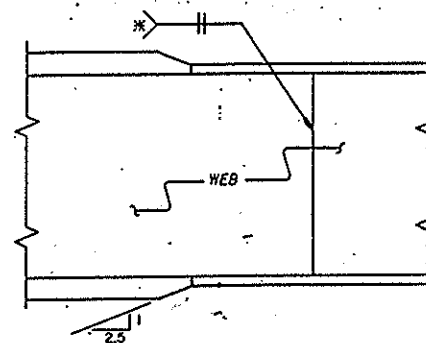


PLAN OF FLANGE



ELEVATION OF BOTTOM FLANGE
(TOP FLANGE SIMILAR)

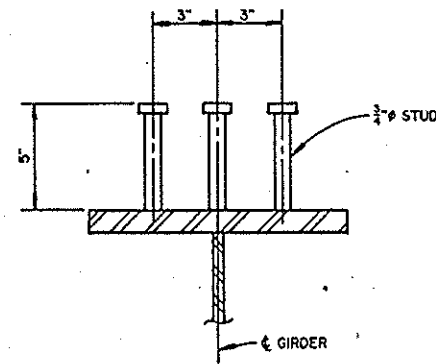
DETAIL 2-11



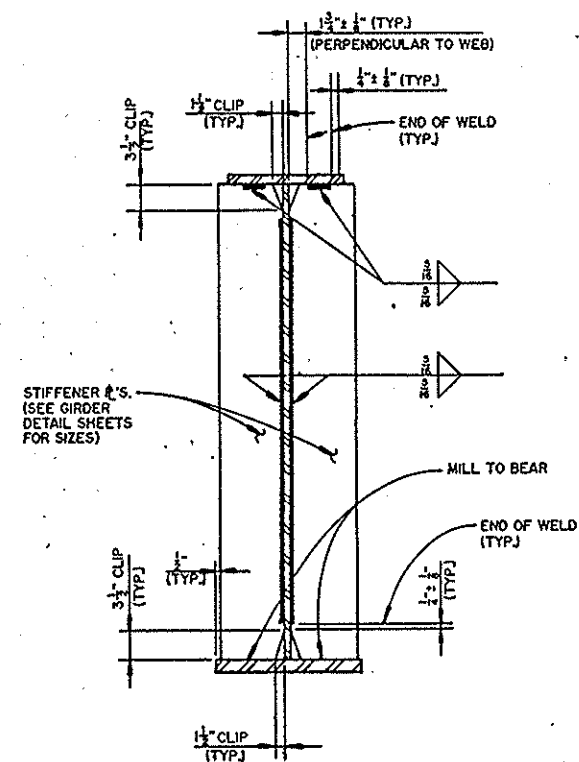
ELEVATION

TYPICAL WEB AND FLANGE BUTT JOINT

DETAIL 2-16



DETAIL 2-12



DETAIL 2-13

BEARING STIFFENER
(NORMAL TO WEB -
TYP. ALL GIRDERS)

* GRIND SMOOTH AND FLUSH ON
OUTER FACE OF EXTERIOR
GIRDERS

STIFFENER $\angle$ 'S.
(SEE GIRDER
DETAIL SHEETS
FOR SIZES)

—MILL TO BEAR

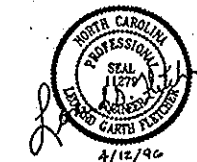
— END OF WELD
(TYP)

1 1/2" CLIP
(TYP.)

DRAWN BY: RTJ DATE: 8-24
CHECKED BY: CGM DATE: 6/15

U2.049

PREPARED BY ———
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED



PROJECT NO. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

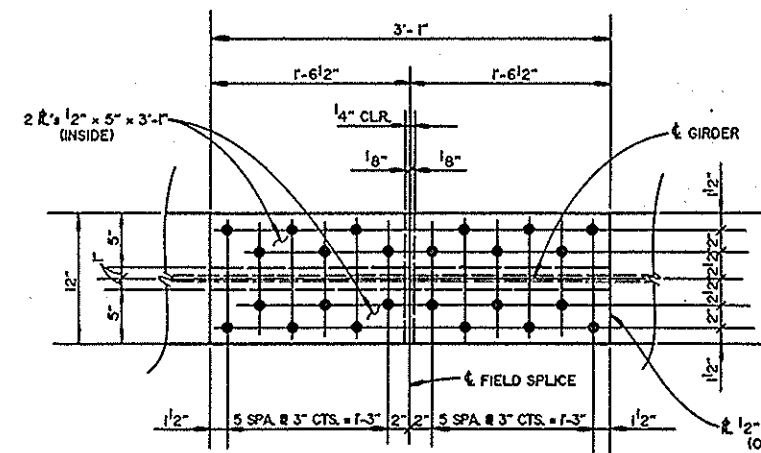
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
DETAILS
CONTINUOUS UNIT #2

AUGUST 1994

REVISIONS						SHEET NO. 590
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			

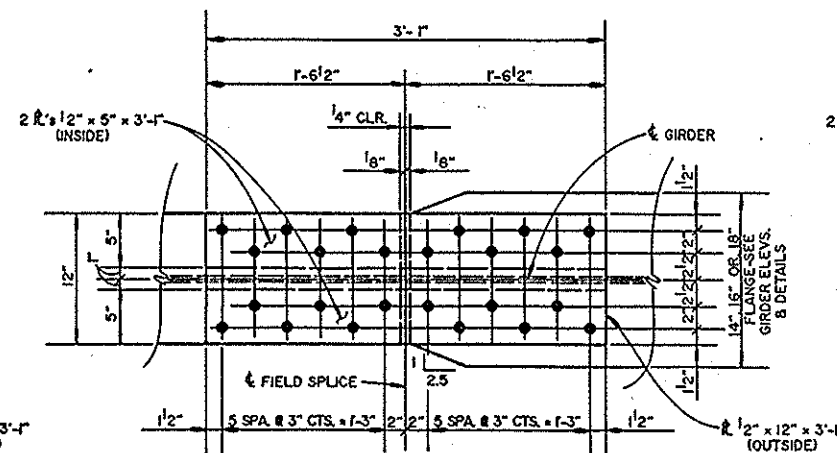
SECRET
NO.
5-90

TOTAL
SHEETS
221



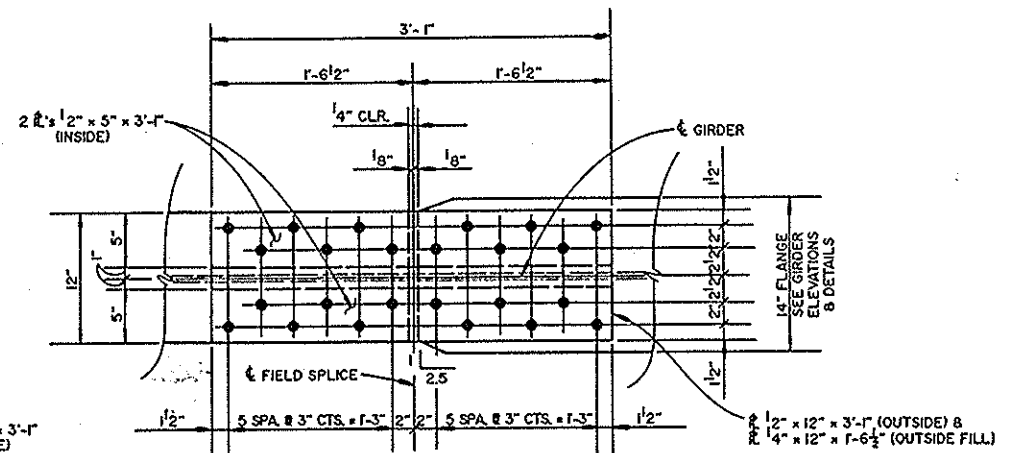
TYPE A FLANGE SPLICE

24 - 7/8" H.S. BOLTS REQ'D.



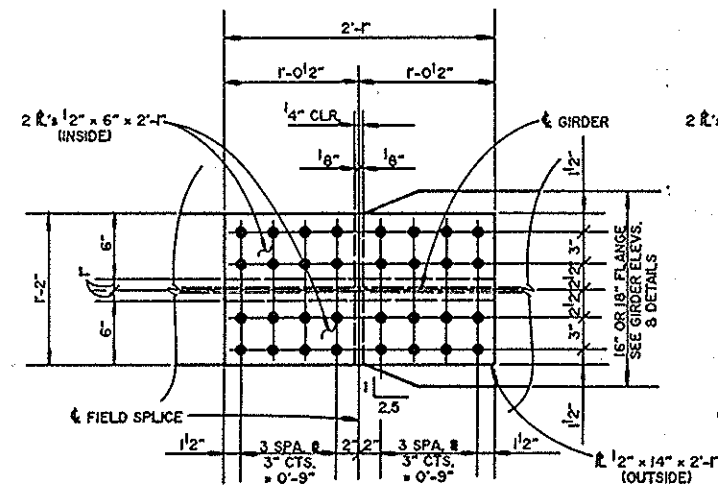
TYPE B FLANGE SPLICE

24 - 7/8" H.S. BOLTS REQ'D.



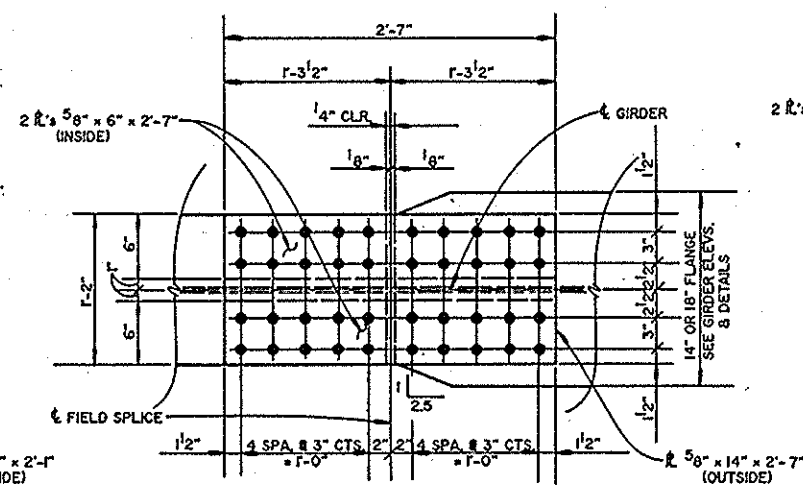
TYPE C FLANGE SPLICE

24 - 7/8" H.S. BOLTS REQ'D.



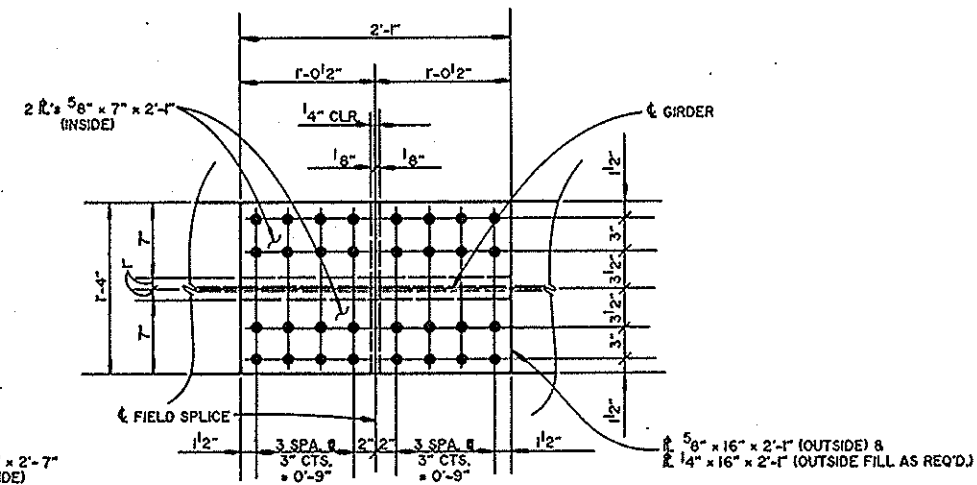
TYPE D FLANGE SPLICE

32 - 7/8" H.S. BOLTS REQ'D.



TYPE E FLANGE SPLICE

40 - 7/8" H.S. BOLTS REQ'D.



TYPE F FLANGE SPLICE

32 - 7/8" H.S. BOLTS REQ'D.

FLANGE SPLICE DETAILS

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
SPlice DETAILS
CONTINUOUS UNIT #2
AUG. 1994



42.050
PREPARED BY
THOMPSON-BOROON-SHOOK
ENGINEERS, INCORPORATED

REVISIONS				SHEET	
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

TOTAL
SHEETS
224

DRAWN BY: N.M. WILSON DATE: AUG. 94
CHECKED BY: CEM DATE: 6/95

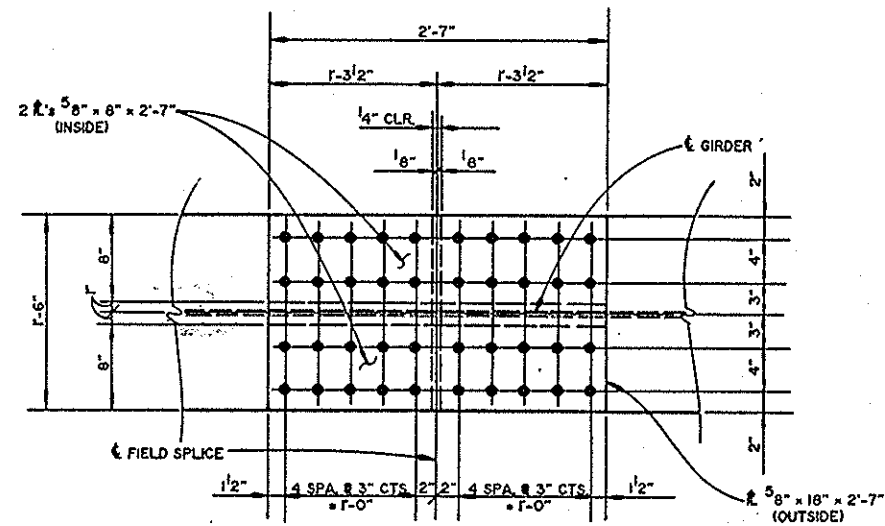
SUMMARY OF FIELD SPLICES

(BY TYPE, SIZE AND LOCATION)

GIRDER NO.	SPLICE NO.	WEB SPLICE TYPE	TOP FLANGE PLATE			BOTTOM FLANGE PLATE		
			LT.	RT.	SPLICE TYPE	LT.	RT.	SPLICE TYPE
1	1	I	1" x 14"	1 1/2" x 14"	J	1 1/2" x 20"	1 1/2" x 20"	H
	2	I	1 1/2" x 14"	1" x 12"	C	1 1/2" x 18"	1 1/2" x 14"	E
	3	II	1" x 12"	1" x 14"	B	1 1/2" x 14"	1 1/2" x 14"	E
	4	II	1" x 14"	1" x 12"	B	1" x 16"	1" x 12"	B
	5	II	1" x 12"	1" x 12"	A	1" x 12"	1" x 12"	A
	6	II	1" x 12"	1" x 12"	A	1" x 12"	1" x 12"	A
2	1	I	1" x 12"	1" x 16"	B	1 1/2" x 20"	1 1/2" x 20"	H
	2	I	1" x 16"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 14"	E
	3	II	1" x 12"	1" x 14"	B	1 1/2" x 14"	1 1/2" x 14"	E
	4	II	1" x 14"	1" x 12"	B	1" x 16"	1" x 12"	B
	5	II	1" x 12"	1" x 12"	A	1" x 12"	1" x 16"	B
	6	I	1" x 12"	1" x 12"	A	1" x 16"	1" x 14"	D
3	1	I	1" x 12"	1" x 16"	B	1 1/2" x 20"	1 1/2" x 20"	H
	2	I	1" x 16"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 14"	E
	3	II	1" x 12"	1" x 14"	B	1 1/2" x 14"	1 1/2" x 14"	E
	4	II	1" x 14"	1" x 12"	B	1" x 16"	1" x 12"	B
	5	II	1" x 12"	1" x 12"	A	1" x 12"	1" x 16"	B
	6	II	1" x 12"	1" x 12"	A	1" x 16"	1" x 14"	D

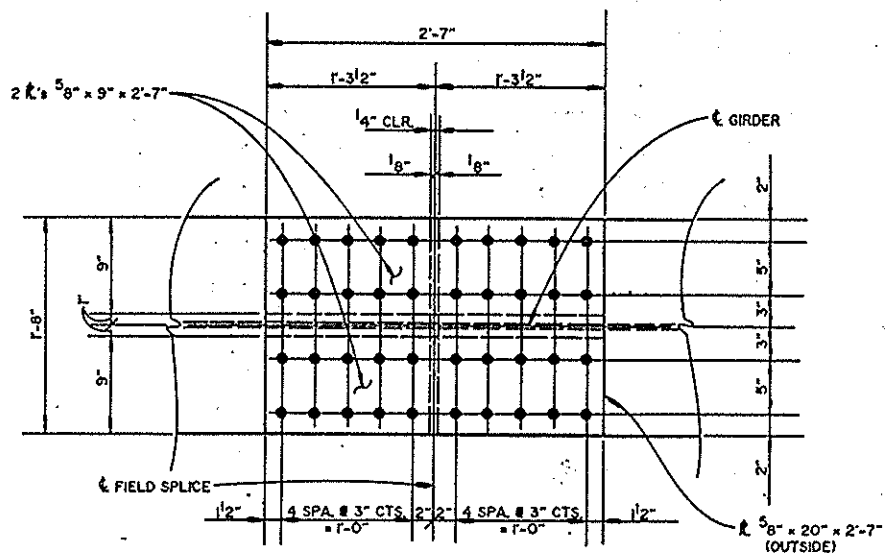
GIRDER NO.	SPLICE NO.	WEB SPLICE TYPE	TOP FLANGE PLATE			BOTTOM FLANGE PLATE		
			LT.	RT.	SPLICE TYPE	LT.	RT.	SPLICE TYPE
4	1	II	1" x 12"	1" x 14"	B	1 1/2" x 20"	1 1/2" x 20"	H
	2	I	1" x 14"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 14"	E
	3	II	1" x 12"	1" x 14"	B	1 1/2" x 14"	1 1/2" x 18"	E
	4	II	1" x 14"	1" x 12"	B	1" x 18"	1" x 12"	B
	5	I	1" x 12"	1" x 14"	B	1" x 12"	1" x 16"	B
	6	I	1" x 14"	1" x 12"	B	1" x 16"	1" x 16"	F
5	1	I	1" x 12"	1" x 16"	B	1" x 20"	1" x 20"	H
	2	I	1" x 16"	1" x 12"	B	1" x 18"	1" x 12"	B
	3	II	1" x 12"	1" x 16"	B	1" x 12"	1" x 18"	B
	4	II	1" x 16"	1" x 12"	B	1" x 18"	1" x 12"	B
	5	II	1" x 12"	1" x 16"	B	1" x 12"	1" x 18"	B
	6	I	1" x 16"	1" x 12"	B	1" x 18"	1" x 18"	G
6	1	I	1" x 12"	1" x 16"	B	1" x 18"	1" x 18"	G
	2	I	1" x 16"	1" x 12"	B	1" x 18"	1" x 12"	B
	3	I	1" x 12"	1" x 16"	D	1" x 12"	1" x 18"	B
	4	II	1" x 16"	1" x 12"	B	1" x 18"	1" x 14"	E
	5	I	1" x 12"	1" x 16"	B	1" x 14"	1" x 18"	E
	6	I	1" x 16"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G

GIRDER NO.	SPLICE NO.	WEB SPLICE TYPE	TOP FLANGE PLATE			BOTTOM FLANGE PLATE		
			LT.	RT.	SPLICE TYPE	LT.	RT.	SPLICE TYPE
7	1	II	1" x 12"	1" x 16"	B	1" x 16"	1" x 16"	F
	2	II	1" x 16"	1" x 12"	B	1" x 16"	1" x 12"	B
	3	I	1" x 12"	1" x 16"	B	1" x 12"	1" x 16"	B
	4	II	1" x 16"	1" x 12"	B	1" x 16"	1" x 16"	F
	5	I	1" x 12"	1" x 18"	B	1" x 16"	1 1/2" x 16"	F
	6	I	1" x 18"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G
8	1	II	1" x 12"	1" x 16"	B	1" x 16"	1" x 16"	B
	2	II	1" x 16"	1" x 12"	B	1" x 16"	1" x 12"	B
	3	I	1" x 12"	1" x 16"	B	1" x 12"	1" x 16"	B
	4	II	1" x 16"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G
	5	I	1" x 12"	1" x 18"	B	1 1/2" x 18"	1 1/2" x 18"	G
	6	I	1" x 18"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G
9	1	II	1" x 12"	1" x 14"	B	1" x 12"	1" x 16"	B
	2	II	1" x 14"	1" x 12"	B	1" x 16"	1" x 12"	B
	3	I	1" x 12"	1" x 14"	B	1" x 12"	1" x 16"	B
	4	II	1" x 14"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G
	5	I	1" x 12"	1" x 18"	B	1 1/2" x 18"	1 1/2" x 18"	G
	6	I	1" x 18"	1" x 12"	B	1 1/2" x 18"	1 1/2" x 18"	G



TYPE G FLANGE SPLICE

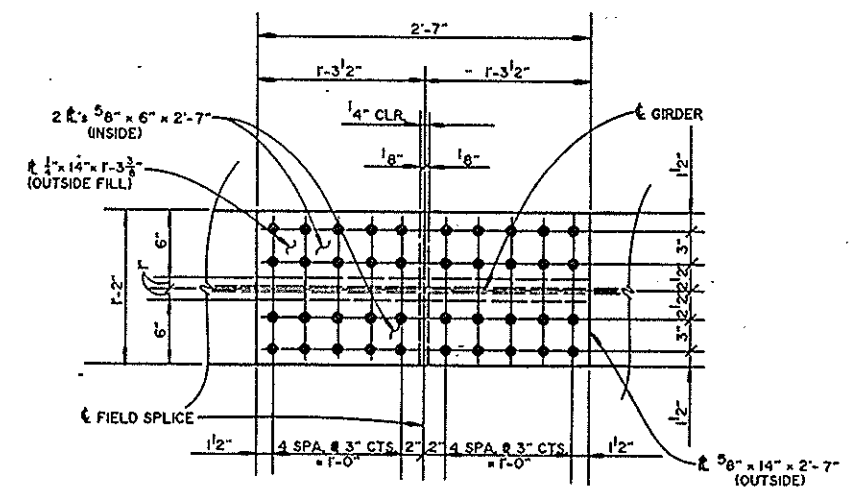
40 - 7/8" H.S. BOLTS REQ'D.



TYPE H FLANGE SPLICE

40 - 7/8" H.S. BOLTS REQ'D.

FLANGE SPLICE DETAILS



TYPE J FLANGE SPLICE

40 - 7/8" H.S. BOLTS REQ'D.

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev.

SHEET 2 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
SPLICE DETAILS
CONTINUOUS UNIT #2

AUG. 1994

REVISIONS

NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		

SHEET NO.

5.92

TOTAL

224

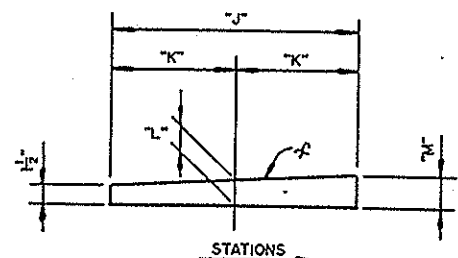
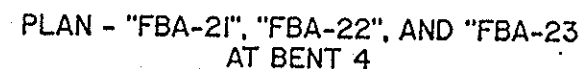


U2.051

PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

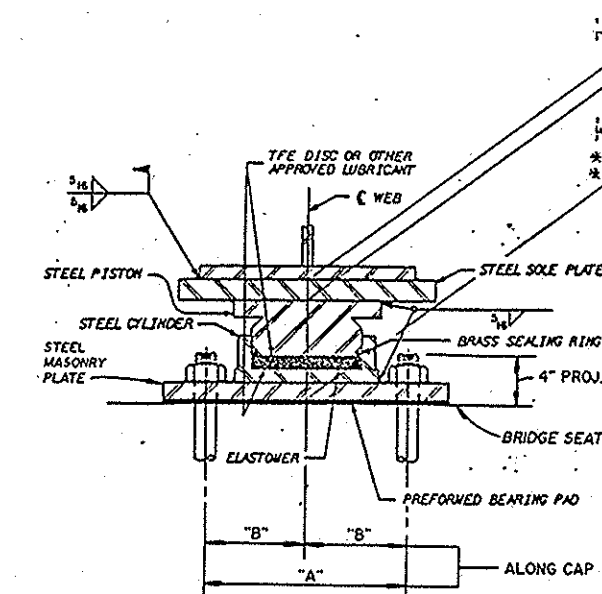
DRAWN BY: N.M. WILSON
CHECKED BY: GCM
DATE: AUG 94
DATE: 6/95





ELEVATION OF BEVELED SOLE PLATE
MARK SP-220 THRU SP-224

MARK	"G"	"H"	"I"	"J"	"K"	"L"	"M"	NO. REQ'D.
SP-220	1'-8"	11"	1'-10"	1'-8"	10"	1 $\frac{13}{16}$ "	2 $\frac{1}{8}$ "	1
SP-221	1'-8"	11"	1'-10"	1'-8"	10"	1 $\frac{3}{4}$ "	2"	2
SP-222	1'-8"	11"	1'-10"	1'-8"	10"	1 $\frac{15}{16}$ "	2 $\frac{1}{4}$ "	2
SP-223	1'-8"	11"	1'-10"	1'-8"	10"	1 $\frac{7}{8}$ "	2 $\frac{1}{4}$ "	2
SP-224	1'-8"	11"	1'-10"	1'-8"	10"	1 $\frac{7}{8}$ "	2 $\frac{1}{4}$ "	2



SECTION THRU FIXED BEARING ASSEMBLY
"FBA-21" THRU "FBA-23"

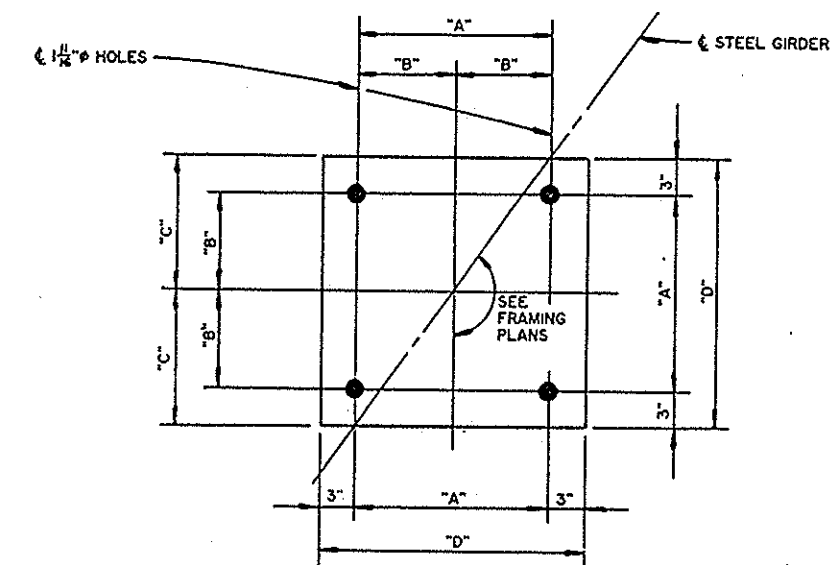
BEARING	LOCATION	VERTICAL LOAD (K)			LATERAL LOAD (K)	TOTAL MOVEMENT (INCHES)
		DEAD	LIVE	TOTAL		
EBA-21	BT.2 & 6	133	67	200	20	2"
EBA-22	BT.3 & 5	280	120	400	40	2"
EBA-23	BT.3	304	146	450	45	2"
EBA-24	BT.3 & 5	347	153	500	50	2"
EBA-25	BT.3 & 5	438	162	600	60	2"
FBA-21	BT.4	254	146	400	40	NA
FBA-22	BT.4	300	150	450	45	NA
FBA-23	BT.4	350	150	500	50	NA

TEMPERATURE AT TIME OF SETTING	30° F	60° F	90° F	※
EBA-21	- 3/8"	0"	+ 3/8"	3/16"
EBA-22	- 5/8"	0"	+ 5/8"	5/16"
EBA-23	- 5/8"	0"	+ 5/8"	5/16"
EBA-24	- 3/4"	0"	+ 3/4"	3/8"
EBA-25	- 3/4"	0"	+ 3/4"	3/8"

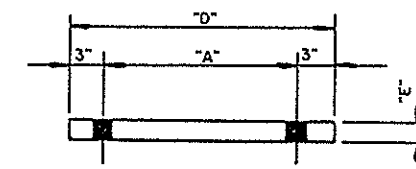
* CORRECTION FOR END ROTATION DUE TO WEIGHT OF SLAB AND COMPOSITE DEAD LOAD.

OVERALL POT HEIGHTS
INCLUDE PREFORMED BEARING PAD

MARK	"A"	"B"	"C"	"D"	"E"	"F"	NO. REQ.
FBA-21	$7-4\frac{1}{2}"$	$8\frac{1}{4}"$	$11\frac{1}{4}"$	$7-10\frac{1}{2}"$	$1\frac{1}{2}"$	$4\frac{13}{16}"$	3
FBA-22	$7-4\frac{1}{2}"$	$8\frac{1}{4}"$	$11\frac{1}{4}"$	$7-10\frac{1}{2}"$	$1\frac{1}{2}"$	$4\frac{21}{16}"$	5
FBA-23	$7-4\frac{1}{2}"$	$8\frac{1}{4}"$	$11\frac{1}{4}"$	$7-10\frac{1}{2}"$	$1\frac{1}{2}"$	$4\frac{15}{16}"$	1

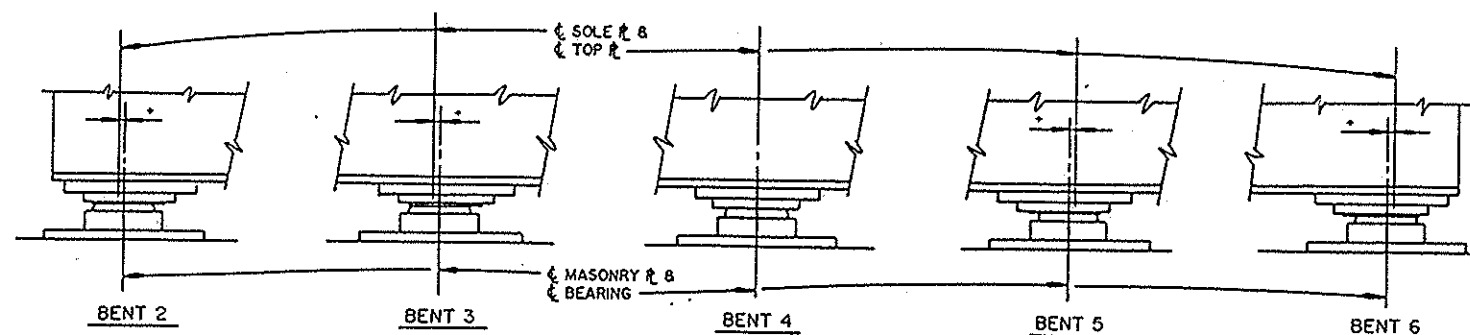


PLAN



ELEVATION

MASONRY PLATE "FBA"



TEMPERATURE SETTING DETAILS

▷ REVISED SOLE PLATE THICKNESSES, OVERALL POT HEIGHTS,
AND ADDED REFERRAL TO APPROVED SHOP DRAWINGS. ✓ BY: CCF



U2.C54

THOMPSON-BORDON-SHOOK
ENGINEERS, INCORPORATED

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-REV

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
BEARING DETAILS
CONTINUOUS UNIT #2

SEPT. 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1	ROB	3/7/77	2			TOTAL SHEETS
3			4			776

SHEET NO. 5-95
TOTAL SHEETS 734

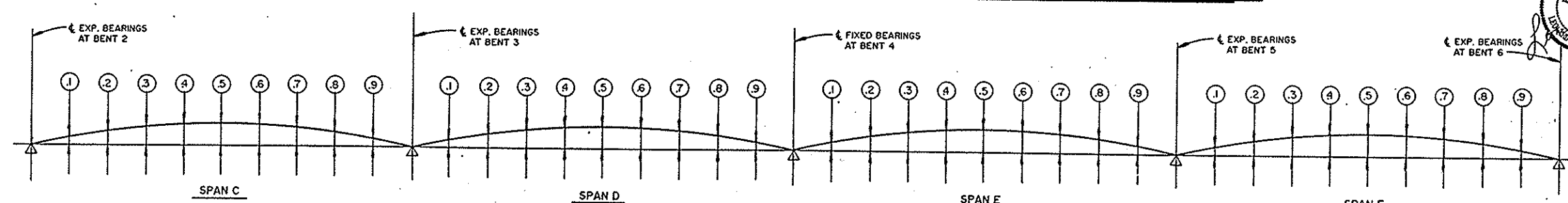
	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6								
GIRDER I	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9									
DEFLECTION DUE TO WEIGHT OF STEEL	.064	.118	.153	.166	.158	.132	.093	.052	.020	-.002	.010	.028	.049	.063	.066	.057	.038	.017	-.006	-.007	-.006	-.004	-.002	-.001	-.002	-.003	-.003	.008	.019	.030	.039	.045	.045	.040	.030	.016									
DEFLECTION DUE TO WEIGHT OF SLAB	.116	.212	.275	.299	.285	.237	.168	.094	.036	-.003	.018	.051	.087	.112	.116	.098	.064	.027	-.007	-.005	0	.005	.007	.005	-.001	-.007	-.009	.022	.054	.087	.114	.131	.132	.118	.089	.048									
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.026	.047	.062	.068	.065	.055	.040	.023	.008	-.001	.005	.013	.022	.027	.028	.024	.015	.006	-.002	-.002	-.001	0	.001	0	.001	.002	.002	.005	.012	.020	.026	.030	.030	.027	.020	.011									
TOTAL DEAD LOAD DEFLECTION	.206	.377	.490	.533	.508	.424	.301	.169	.064	-.006	.033	.092	.158	.202	.210	.179	.117	.050	-.015	-.014	-.007	.001	.006	.004	-.002	-.008	-.010	.035	.085	.137	.179	.206	.207	.185	.139	.075									
VERTICAL CURVE ORDINATE	.134	.238	.312	.357	.372	.356	.312	.238	.134	.139	.247	.325	.371	.386	.371	.324	.247	.139	.075	.134	.176	.201	.210	.201	.176	.134	.076	.071	.125	.165	.189	.196	.188	.164	.126	.071									
REQUIRED CAMBER	4 ¹ / ₁₆ "	7 ³ / ₈ "	9 ⁵ / ₈ "	10 ¹¹ / ₁₆ "	10 ⁹ / ₁₆ "	9 ³ / ₈ "	7 ³ / ₈ "	4 ⁷ / ₈ "	2 ³ / ₈ "	1 ⁵ / ₈ "	3 ³ / ₈ "	5"	6 ³ / ₈ "	7 ¹ / ₁₆ "	7"	6 ¹ / ₁₆ "	4 ³ / ₈ "	2 ¹ / ₄ "	3 ¹ / ₄ "	1 ⁷ / ₁₆ "	2"	2 ⁷ / ₁₆ "	2 ⁹ / ₁₆ "	2 ⁷ / ₁₆ "	2 ¹ / ₁₆ "	1 ¹ / ₂ "	1 ¹ / ₁₆ "	2 ¹ / ₂ "	3 ⁵ / ₈ "	4 ⁷ / ₁₆ "	4 ¹³ / ₁₆ "	4 ³ / ₄ "	4 ³ / ₁₆ "	3 ³ / ₁₆ "	1 ³ / ₄ "										

GIRDER 2	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.021	.038	.048	.050	.044	.032	.018	.006	-.001	.012	.032	.052	.070	.078	.074	.059	.037	.016	-.005	-.005	-.002	.001	.003	.002	-.001	-.004	-.005	.012	.054	.047	.062	.071	.072	.064	.049	.029
DEFLECTION DUE TO WEIGHT OF SLAB	.113	.207	.272	.300	.292	.252	.189	.116	.049	-.021	-.020	-.004	.018	.037	.045	.041	.026	.010	.001	.007	.016	.023	.024	.020	.012	.002	-.003	.016	.040	.066	.089	.103	.105	.094	.072	.053
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.022	.041	.053	.058	.057	.048	.035	.021	.008	.001	.005	.013	.021	.026	.026	.022	.014	.006	-.001	0	.002	.004	.004	.003	.001	-.001	-.002	.006	.014	.024	.031	.036	.036	.033	.025	.013
TOTAL DEAD LOAD DEFLECTION	.156	.286	.373	.408	.393	.332	.242	.143	.056	-.008	.017	.061	.109	.141	.145	.122	.077	.032	-.005	.002	.016	.028	.031	.025	.012	-.003	-.010	.034	.108	.137	.182	.210	.213	.191	.146	.092
VERTICAL CURVE ORDINATE	.121	.215	.282	.323	.336	.323	.283	.215	.122	.130	.231	.303	.346	.361	.347	.304	.231	.130	.085	.152	.199	.277	.237	.227	.198	.151	.086	.078	.138	.181	.207	.216	.207	.181	.138	.078
REQUIRED CAMBER	3 ³ / ₁₆ "	6"	7 ⁷ / ₈ "	8 ³ / ₄ "	8 ³ / ₄ "	7 ⁷ / ₈ "	6 ⁵ / ₁₆ "	4 ⁵ / ₁₆ "	2 ¹ / ₈ "	1 ⁷ / ₁₆ "	3"	4 ³ / ₈ "	5 ⁷ / ₁₆ "	6"	5 ⁷ / ₈ "	5 ¹ / ₈ "	3 ¹¹ / ₁₆ "	1 ¹³ / ₁₆ "	1 ¹ / ₁₆ "	1 ⁷ / ₈ "	2 ³ / ₁₆ "	3 ¹¹ / ₁₆ "	3 ³ / ₁₆ "	3"	2 ¹ / ₂ "	1 ³ / ₄ "	1 ¹ / ₁₆ "	2 ¹³ / ₁₆ "	3 ¹³ / ₁₆ "	4 ¹¹ / ₁₆ "	5 ¹ / ₈ "	5 ¹ / ₁₆ "	4 ⁷ / ₁₆ "	3 ⁷ / ₁₆ "	2 ¹ / ₁₆ "	

GIRDER 3	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6									
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	
DEFLECTION DUE TO WEIGHT OF STEEL	.020	.036	.046	.047	.042	.032	.019	.008	0	.011	.028	.048	.063	.070	.066	.052	.033	.014	-.004	-.003	0	.004	.006	.004	0	-.003	-.005	.012	.030	.048	.064	.074	.075	.067	.051	.027										
DEFLECTION DUE TO WEIGHT OF SLAB	.106	.196	.258	.284	.276	.238	.179	.111	.048	-.020	-.019	-.005	.016	.033	.041	.036	.023	.008	.002	.011	.020	.028	.054	.024	.014	.004	-.003	.016	.039	.067	.090	.106	.109	.099	.075	.041										
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.021	.039	.050	.055	.053	.046	.034	.020	.008	0	.004	.012	.011	.024	.002	.020	.012	.005	-.001	.001	.003	.005	.006	.005	.002	-.001	-.002	.006	.014	.024	.032	.037	.038	.034	.026	.014										
TOTAL DEAD LOAD DEFLECTION	.147	.271	.354	.386	.371	.316	.232	.139	.056	-.009	.013	.055	.090	.127	.109	.108	.068	.027	-.003	.009	.023	.037	.066	.033	.016	0	-.010	.034	.083	.139	.186	.217	.222	.200	.152	.082										
VERTICAL CURVE ORDINATE	.116	.207	.272	.310	.323	.310	.272	.207	.117	.126	.223	.293	.336	.350	.335	.294	.224	.126	.089	.159	.208	.237	.248	.237	.207	.158	.089	.081	.143	.189	.215	.224	.216	.188	.144	.080										
REQUIRED CAMBER	3 ¹ / ₈ "	5 ³ / ₄ "	7 ¹ / ₂ "	8 ³ / ₈ "	8 ⁵ / ₁₆ "	7 ¹ / ₂ "	6 ¹ / ₁₆ "	4 ¹ / ₈ "	2 ¹ / ₁₆ "	1 ⁵ / ₈ "	2 ¹³ / ₁₆ "	4 ³ / ₁₆ "	5 ¹ / ₈ "	5 ³ / ₄ "	5 ⁵ / ₁₆ "	4 ¹³ / ₁₆ "	3 ¹ / ₂ "	1 ¹³ / ₁₆ "	1 ¹ / ₁₆ "	2"	2 ³ / ₄ "	3 ³ / ₁₆ "	3 ³ / ₄ "	3 ¹ / ₄ "	2 ¹¹ / ₁₆ "	1 ⁷ / ₈ "	1 ⁵ / ₁₆ "	2 ¹¹ / ₁₆ "	3 ¹³ / ₁₆ "	4 ¹³ / ₁₆ "	5 ⁵ / ₁₆ "	5 ¹ / ₄ "	4 ³ / ₈ "	3 ⁹ / ₁₆ "	1 ¹³ / ₁₆ "											

NOTE: DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS GIVEN IN FRACTIONS OF AN INCH.

DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS



SCHEMATIC CAMBER ORDINATES - ALL GIRDERS
NOTE: SLOPE FOR THE ZERO CAMBER BASE LINE VARIES.



PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
DEAD LOAD DEFLECTIONS
CONTINUOUS UNIT #2
MARCH 1995

REVISIONS			
NO.	BY	DATE	DESCRIPTION
1			
2			

U2.055
PREPARED BY
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

SHEET NO. 5-96
TOTAL SHEETS 224

DRAWN BY: RKL
CHECKED BY: BEN
DATE: 8-95
DATE: 6-95

GIRDER 4	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6								
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9									
DEFLECTION DUE TO WEIGHT OF STEEL	.051	.094	.124	.136	.132	.114	.084	.051	.021	-.008	-.004	.008	.022	.032	.034	.029	.018	.006	.001	.008	.015	.022	.023	.020	.013	.005	0	.008	.021	.037	.051	.061	.063	.057	.044	.024									
DEFLECTION DUE TO WEIGHT OF SLAB	.079	.144	.188	.206	.199	.168	.122	.072	.028	-.005	.008	.033	.060	.074	.075	.060	.036	.013	.004	.019	.035	.048	.051	.042	.026	.009	-.002	.021	.053	.092	.128	.152	.158	.144	.110	.060									
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.017	.030	.040	.044	.042	.036	.027	.016	.006	-.001	.003	.009	.014	.017	.017	.014	.008	.003	0	.003	.007	.009	.010	.008	.004	.001	-.001	.005	.013	.022	.030	.035	.036	.033	.025	.014									
TOTAL DEAD LOAD DEFLECTION	.147	.268	.352	.386	.373	.318	.233	.139	.055	-.014	.007	.050	.096	.123	.126	.103	.062	.022	.005	.030	.057	.079	.084	.070	.043	.015	-.003	.034	.087	.151	.209	.248	.257	.234	.179	.098									
VERTICAL CURVE ORDINATE	.105	.186	.244	.278	.291	.279	.244	.186	.105	.118	.208	.273	.312	.325	.312	.273	.208	.117	.100	.178	.232	.266	.277	.266	.233	.177	.099	.088	.157	.207	.236	.246	.236	.206	.158	.089									
REQUIRED CAMBER	3"	5 $\frac{7}{16}$ "	7 $\frac{1}{8}$ "	7 $\frac{15}{16}$ "	7 $\frac{15}{16}$ "	7 $\frac{3}{16}$ "	5 $\frac{3}{4}$ "	3 $\frac{7}{8}$ "	1 $\frac{15}{16}$ "	1 $\frac{1}{4}$ "	2 $\frac{3}{16}$ "	3 $\frac{7}{8}$ "	4 $\frac{7}{8}$ "	5 $\frac{3}{8}$ "	5 $\frac{1}{4}$ "	4 $\frac{1}{2}$ "	3 $\frac{1}{4}$ "	1 $\frac{11}{16}$ "	1 $\frac{1}{8}$ "	2 $\frac{1}{2}$ "	3 $\frac{7}{16}$ "	4 $\frac{1}{8}$ "	4 $\frac{5}{16}$ "	4 $\frac{1}{16}$ "	3 $\frac{5}{16}$ "	2 $\frac{5}{16}$ "	1 $\frac{1}{8}$ "	1 $\frac{7}{16}$ "	2 $\frac{15}{16}$ "	4 $\frac{5}{16}$ "	5 $\frac{5}{16}$ "	5 $\frac{15}{16}$ "	5 $\frac{15}{16}$ "	5 $\frac{1}{4}$ "	4 $\frac{1}{16}$ "	2 $\frac{1}{4}$ "									

GIRDER 5	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.043	.079	.104	.115	.112	.096	.072	.044	.018	-.006	-.002	.008	.021	.029	.030	.024	.039	.011	.005	.015	.026	.035	.037	.033	.022	.011	.002	.007	.020	.037	-.053	.063	.067	.061	.047	.025
DEFLECTION DUE TO WEIGHT OF SLAB	.066	.121	.158	.174	.168	.143	.105	.063	.025	-.004	.008	.030	.051	.063	.062	.047	.021	.007	.011	.033	.057	.074	.077	.064	.041	.017	.001	.021	.057	.100	.141	.168	.175	.160	.123	.067
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.014	.025	.033	.037	.035	.030	.022	.013	.005	-.001	.002	.007	.012	.015	.014	.011	.006	.002	.002	.007	.012	.016	.016	.014	.009	.003	0	.005	.014	.024	.033	.038	.040	.036	.028	.015
TOTAL DEAD LOAD DEFLECTION	.123	.225	.295	.326	.315	.269	.199	.120	.048	.011	.008	.045	.084	.107	.106	.082	.066	.020	.018	.055	.095	.125	.130	.111	.072	.031	.003	.033	.091	.161	.227	.269	.282	.257	.198	.107
VERTICAL CURVE ORDINATE	.093	.165	.217	.248	.259	.249	.217	.166	.093	.108	.192	.252	.288	.301	.289	.253	.192	.108	.112	.198	.260	.297	.310	.298	.261	.199	.112	.096	.172	.225	.258	.268	.257	.225	.171	.097
REQUIRED CAMBER	2 $\frac{9}{16}$ "	4 $\frac{11}{16}$ "	6 $\frac{1}{8}$ "	6 $\frac{7}{8}$ "	6 $\frac{7}{8}$ "	6 $\frac{3}{16}$ "	5"	3 $\frac{7}{16}$ "	1 $\frac{11}{16}$ "	1 $\frac{7}{16}$ "	2 $\frac{3}{8}$ "	3 $\frac{5}{16}$ "	4 $\frac{7}{16}$ "	4 $\frac{7}{8}$ "	4 $\frac{3}{4}$ "	4"	3 $\frac{1}{8}$ "	1 $\frac{9}{16}$ "	1 $\frac{9}{16}$ "	3 $\frac{1}{16}$ "	4 $\frac{1}{4}$ "	5 $\frac{1}{16}$ "	5 $\frac{1}{4}$ "	4 $\frac{15}{16}$ "	4"	2 $\frac{3}{4}$ "	1 $\frac{3}{8}$ "	1 $\frac{9}{16}$ "	3 $\frac{1}{8}$ "	4 $\frac{5}{8}$ "	5 $\frac{13}{16}$ "	6 $\frac{7}{16}$ "	6 $\frac{7}{16}$ "	5 $\frac{13}{16}$ "	4 $\frac{7}{16}$ "	2 $\frac{7}{16}$ "

GIRDER 6	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9
DEFLECTION DUE TO WEIGHT OF STEEL	.036	.066	.086	.096	.093	.080	.060	.037	.015	-.005	-.002	.006	.015	.021	.020	.015	.007	.001	.008	.023	.039	.050	.052	.046	.033	.018	.005	.005	.017	.033	.005	.061	.065	.060	.047	.026
DEFLECTION DUE TO WEIGHT OF SLAB	.055	.101	.132	.146	.141	.121	.089	.053	.022	-.026	.004	.021	.036	.043	.040	.027	.011	0	.019	.050	.082	.103	.106	.090	.060	.029	.006	.019	.053	.097	.139	.168	.178	.164	.127	.070
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.012	.021	.028	.031	.030	.026	.019	.011	.005	-.001	.001	.005	.008	.009	.009	.006	.002	0	.004	.011	.018	.023	.023	.020	.013	.006	.001	.005	.013	.024	.033	.040	.041	.037	.029	.016
TOTAL DEAD LOAD DEFLECTION	.103	.188	.246	.273	.264	.227	.168	.101	.042	-.032	.003	.032	.059	.073	.069	.048	.020	.001	.031	.084	.139	.176	.181	.156	.106	.053	.012	.029	.083	.154	.177	.269	.284	.261	.203	.112
VERTICAL CURVE ORDINATE	.077	.137	.177	.201	.205	.191	.160	.110	.063	.099	.177	.232	.265	.276	.265	.232	.177	.100	.124	.220	.289	.330	.344	.330	.289	.220	.124	.104	.186	.244	.279	.291	.279	.244	.186	.105
REQUIRED CAMBER	2 $\frac{3}{16}$ "	3 $\frac{7}{8}$ "	5 $\frac{1}{16}$ "	5 $\frac{11}{16}$ "	5 $\frac{5}{8}$ "	5"	3 $\frac{15}{16}$ "	2 $\frac{9}{16}$ "	1 $\frac{1}{4}$ "	1 $\frac{13}{16}$ "	2 $\frac{3}{16}$ "	3 $\frac{3}{16}$ "	3 $\frac{7}{8}$ "	4 $\frac{3}{16}$ "	4"	3 $\frac{3}{8}$ "	2 $\frac{3}{8}$ "	1 $\frac{3}{16}$ "	1 $\frac{7}{8}$ "	3 $\frac{3}{8}$ "	5 $\frac{1}{8}$ "	6 $\frac{1}{16}$ "	6 $\frac{5}{16}$ "	5 $\frac{13}{16}$ "	4 $\frac{3}{4}$ "	3 $\frac{1}{4}$ "	1 $\frac{5}{8}$ "	1 $\frac{5}{8}$ "	3 $\frac{1}{4}$ "	4 $\frac{3}{4}$ "	5 $\frac{1}{2}$ "	6 $\frac{3}{4}$ "	6 $\frac{3}{4}$ "	6 $\frac{1}{16}$ "	4 $\frac{11}{16}$ "	2 $\frac{5}{8}$ "

NOTE:
DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN
DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS
GIVEN IN FRACTIONS OF AN INCH.

DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS

PROJECT NO. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 2 OF 3



U2.050
PREPARED BY
THOMPSON - GORDON - BHOOK
ENGINEERS, INCORPORATED

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL DEAD LOAD DEFLECTIONS CONTINUOUS UNIT #2 MARCH 1995					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
					SHEET 547 TOTAL SHEETS 224

DRAWN BY: RTJ
CHECKED BY: GEM
DATE: 3-25
DATE: 4-9-92

GIRDER 7	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	
DEFLECTION DUE TO WEIGHT OF STEEL	.030	.054	.071	.079	.077	.067	.050	.031	.013	-.006	-.004	.001	.007	.010	.008	.004	-.001	-.003	.013	.031	.051	.065	.068	.059	.042	.022	.006	.006	.019	.038	.056	.069	.074	.069	.053	.029	
DEFLECTION DUE TO WEIGHT OF SLAB	.046	.085	.111	.122	.119	.102	.076	.047	.019	-.007	-.003	.007	.016	.019	.015	.005	-.004	-.007	.027	.065	.104	.128	.130	.109	.070	.030	.004	.024	.066	.118	.167	.201	.212	.196	.151	.082	
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.010	.018	.023	.026	.025	.022	.016	.010	.004	-.001	0	.002	.004	.005	.004	.002	-.001	-.002	.006	.015	.024	.029	.030	.025	.017	.008	.001	.006	.016	.028	.038	.046	.048	.044	.034	.018	
TOTAL DEAD LOAD DEFLECTION	.086	.157	.205	.227	.221	.191	.142	.088	.036	-.014	-.007	.010	.027	.034	.027	.011	-.006	-.012	.046	.111	.179	.222	.228	.193	.129	.060	.011	.036	.101	.184	.261	.316	.334	.309	.238	.129	
VERTICAL CURVE ORDINATE	.063	.110	.143	.159	.159	.143	.112	.067	.027	.091	.161	.212	.242	.253	.243	.212	.162	.091	.138	.245	.321	.367	.382	.366	.320	.244	.138	.114	.202	.266	.303	.316	.303	.265	.202	.114	
REQUIRED CAMBER	$1\frac{13}{16}$ "	$3\frac{3}{16}$ "	$4\frac{3}{16}$ "	$4\frac{5}{8}$ "	$4\frac{9}{16}$ "	4"	$3\frac{1}{16}$ "	$1\frac{7}{8}$ "	$\frac{3}{4}$ "	$1\frac{5}{16}$ "	$1\frac{7}{8}$ "	$2\frac{11}{16}$ "	$3\frac{1}{4}$ "	$3\frac{5}{8}$ "	$3\frac{1}{4}$ "	$2\frac{11}{16}$ "	$1\frac{7}{8}$ "	$1\frac{5}{16}$ "	$2\frac{3}{16}$ "	$4\frac{1}{4}$ "	6"	$7\frac{1}{16}$ "	$7\frac{5}{16}$ "	$6\frac{11}{16}$ "	$5\frac{3}{8}$ "	$3\frac{5}{8}$ "	$1\frac{13}{16}$ "	$1\frac{13}{16}$ "	$3\frac{5}{8}$ "	$5\frac{5}{8}$ "	$6\frac{3}{4}$ "	$7\frac{9}{16}$ "	$7\frac{5}{8}$ "	$6\frac{7}{8}$ "	$5\frac{1}{4}$ "	$2\frac{15}{16}$ "	

GIRDER 8	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	
DEFLECTION DUE TO WEIGHT OF STEEL	.021	.038	.051	.057	.056	.049	.037	.023	.010	-.004	-.003	.001	.005	.006	.004	0	-.003	-.004	.016	.040	.064	.080	.083	.072	.052	.028	.009	.005	.019	.036	.055	.070	.075	.070	.054	.030	
DEFLECTION DUE TO WEIGHT OF SLAB	.032	.060	.079	.088	.086	.075	.056	.034	.014	-.005	-.002	.005	.010	.011	.006	-.002	-.008	-.009	.033	.080	.124	.150	.150	.124	.080	.034	.004	.029	.076	.132	.186	.225	.237	.219	.253	.093	
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.007	.012	.016	.018	.019	.016	.012	.007	.003	-.001	0	.002	.003	.003	.002	0	-.002	-.002	.008	.019	.029	.035	.036	.030	.020	.009	.002	.006	.017	.030	.042	.050	.052	.048	.037	.020	
TOTAL DEAD LOAD DEFLECTION	.060	.110	.146	.163	.161	.140	.105	.064	.027	-.010	-.005	.008	.018	.020	.012	-.002	-.013	-.015	.057	.139	.217	.265	.269	.226	.152	.071	.015	.040	.112	.198	.278	.345	.364	.337	.344	.143	
VERTICAL CURVE ORDINATE	.054	.094	.121	.135	.135	.122	.096	.056	.004	.083	.147	.194	.221	.230	.220	.193	.147	.083	.152	.269	.354	.404	.422	.404	.354	.270	.152	.123	.218	.287	.327	.341	.327	.287	.218	.123	
REQUIRED CAMBER	$1\frac{3}{8}$ "	$2\frac{7}{16}$ "	$3\frac{3}{16}$ "	$3\frac{9}{16}$ "	$3\frac{9}{16}$ "	$3\frac{1}{8}$ "	$2\frac{7}{16}$ "	$1\frac{7}{16}$ "	$\frac{3}{8}$ "	$1\frac{7}{8}$ "	$1\frac{11}{16}$ "	$2\frac{7}{16}$ "	$2\frac{7}{8}$ "	3"	$2\frac{13}{16}$ "	$2\frac{5}{16}$ "	$1\frac{5}{8}$ "	$1\frac{13}{16}$ "	$2\frac{1}{2}$ "	$4\frac{7}{8}$ "	$6\frac{7}{8}$ "	8"	$8\frac{5}{16}$ "	$7\frac{3}{16}$ "	$6\frac{1}{16}$ "	$4\frac{1}{16}$ "	2"	$1\frac{15}{16}$ "	$3\frac{15}{16}$ "	$5\frac{13}{16}$ "	$7\frac{1}{4}$ "	$8\frac{1}{4}$ "	$8\frac{5}{16}$ "	$7\frac{1}{2}$ "	$6\frac{3}{4}$ "	$3\frac{3}{16}$ "	

GIRDER 9	EXPANSION BEARINGS AT BENT 2 SPAN C									EXPANSION BEARINGS AT BENT 3 SPAN D									FIXED BEARINGS AT BENT 4 SPAN E									EXPANSION BEARINGS AT BENT 5 SPAN F									EXPANSION BEARINGS AT BENT 6
	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	.1	.2	.3	.4	.5	.6	.7	.8	.9	
DEFLECTION DUE TO WEIGHT OF STEEL	.014	.026	.034	.038	.037	.031	.023	.014	.005	-.001	.002	.006	.009	.008	.004	-.001	-.005	-.006	.024	.058	.093	.117	.124	.113	.086	.052	.021	-.005	-.001	.013	.031	.045	.053	.053	.042	.023	
DEFLECTION DUE TO WEIGHT OF SLAB	.030	.055	.073	.082	.081	.071	.054	.034	.015	-.007	-.008	-.005	-.004	-.007	-.012	-.018	-.020	-.002	.049	.113	.172	.207	.208	.174	.116	.055	.013	.026	.075	.143	.209	.257	.274	.255	.198	.108	
DEFLECTION DUE TO WEIGHT OF CLASSIC RAIL & S'WALK	.006	.012	.015	.017	.017	.015	.011	.007	.003	-.002	-.002	-.001	-.001	-.002	-.003	-.004	-.005	-.004	.011	.026	.039	.047	.048	.040	.028	.014	.003	.006	.017	.033	.047	.057	.060	.056	.043	.023	
TOTAL DEAD LOAD DEFLECTION	.050	.093	.122	.137	.135	.117	.088	.055	.023	-.010	-.008	0	.004	-.001	-.011	-.023	-.030	-.012	.084	.197	.304	.371	.380	.327	.230	.121	.037	.027	.091	.189	.287	.359	.387	.364	.283	.154	
VERTICAL CURVE ORDINATE	.049	.087	.114	.130	.135	.128	.111	.082	.042	.075	.133	.175	.200	.208	.200	.175	.133	.075	.167	.297	.390	.446	.465	.445	.389	.297	.167	.132	.235	.309	.353	.368	.353	.309	.235	.132	
REQUIRED CAMBER	$1\frac{3}{16}$ "	$2\frac{3}{16}$ "	$2\frac{13}{16}$ "	$3\frac{3}{16}$ "	$3\frac{1}{4}$ "	$2\frac{15}{16}$ "	$2\frac{3}{8}$ "	$1\frac{5}{8}$ "	$\frac{3}{4}$ "	$\frac{3}{4}$ "	$1\frac{1}{2}$ "	$2\frac{1}{8}$ "	$2\frac{7}{16}$ "	$2\frac{1}{2}$ "	$2\frac{1}{4}$ "	$1\frac{13}{16}$ "	$1\frac{1}{4}$ "	$\frac{3}{4}$ "	3"	$5\frac{15}{16}$ "	$8\frac{5}{16}$ "	$9\frac{13}{16}$ "	$10\frac{1}{8}$ "	$9\frac{1}{4}$ "	$7\frac{7}{16}$ "	5"	$2\frac{7}{16}$ "	$1\frac{15}{16}$ "	$3\frac{15}{16}$ "	6"	$7\frac{11}{16}$ "	$8\frac{3}{4}$ "	$8\frac{7}{8}$ "	$8\frac{1}{16}$ "	$6\frac{3}{16}$ "	$3\frac{7}{16}$ "	

NOTE:
DEFLECTIONS ARE GIVEN AT TENTH POINTS BETWEEN BEARINGS IN DECIMALS OF A FOOT, EXCEPT FINAL REQUIRED CAMBER WHICH IS GIVEN IN FRACTIONS OF AN INCH.

DEAD LOAD DEFLECTIONS AND REQUIRED CAMBER FOR GIRDERS

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV



SHEET 3 OF 3

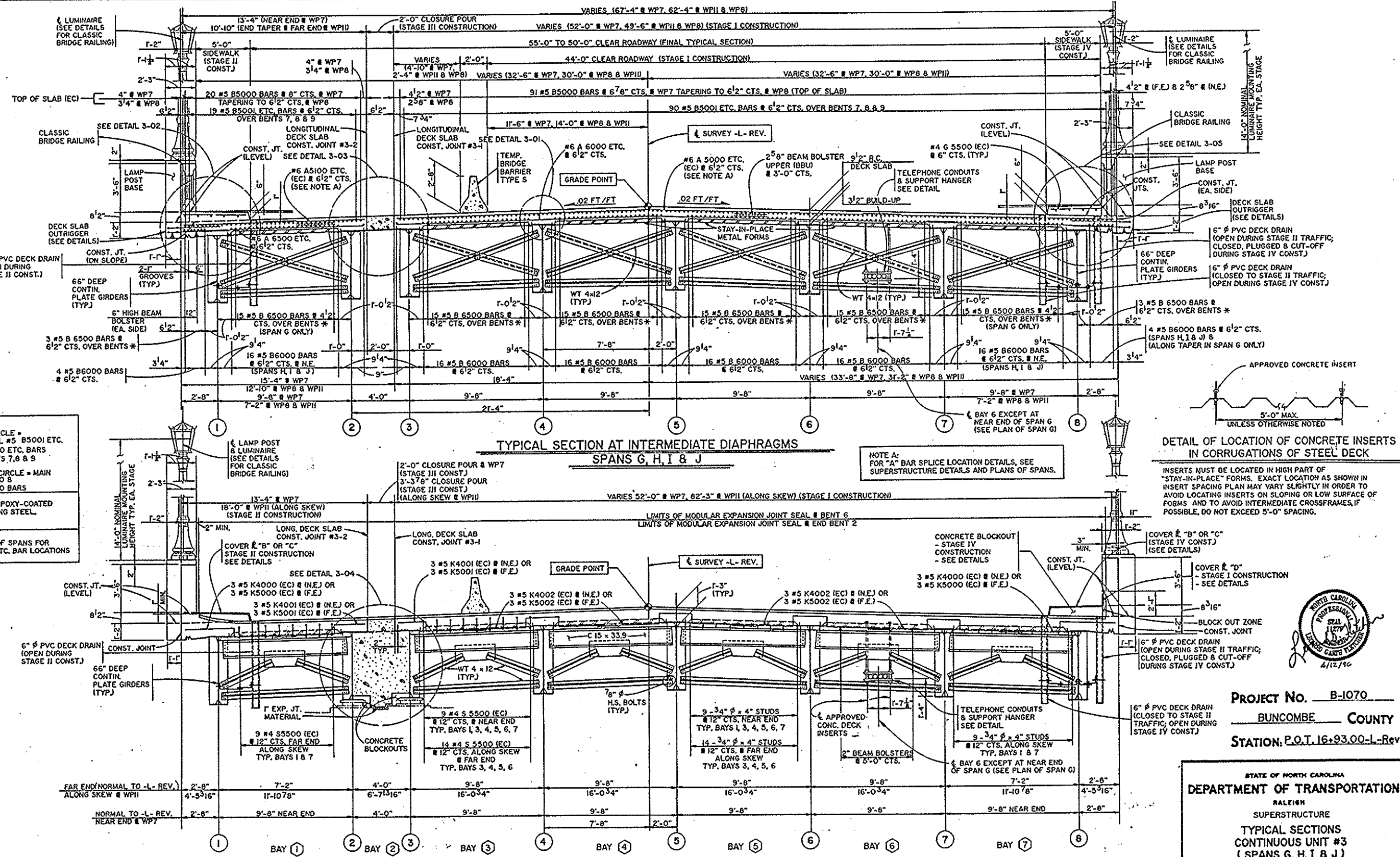
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
DEAD LOAD DEFLECTIONS
CONTINUOUS UNIT #2
MARCH 1995

NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		

SHEET NO. 5-98
TOTAL SHEETS 224

DRAWN BY: RVL
CHECKED BY: CEM
DATE: 8-98
DATE: 6/95

U2.057
PREPARED BY:
THOMPSON-GORDON-SHOCK
ENGINEERS, INCORPORATED

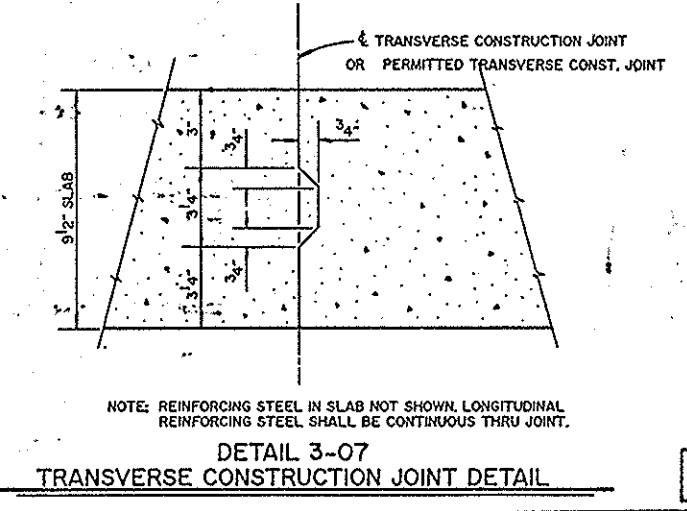
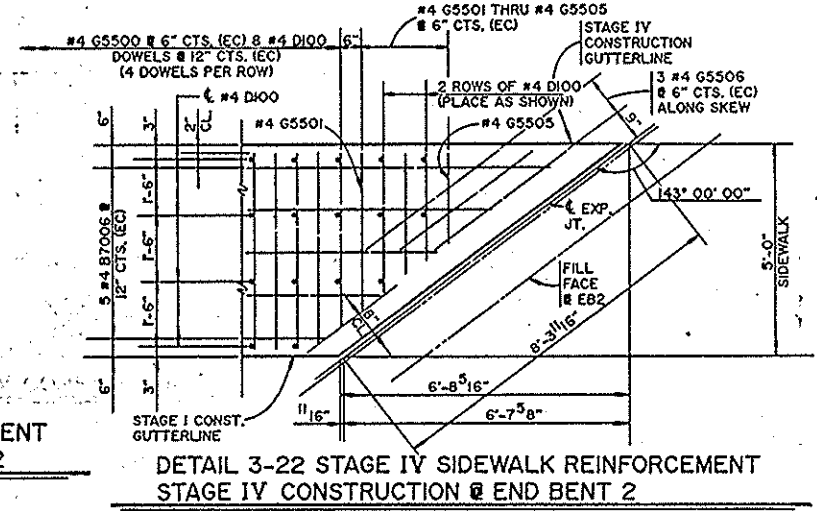
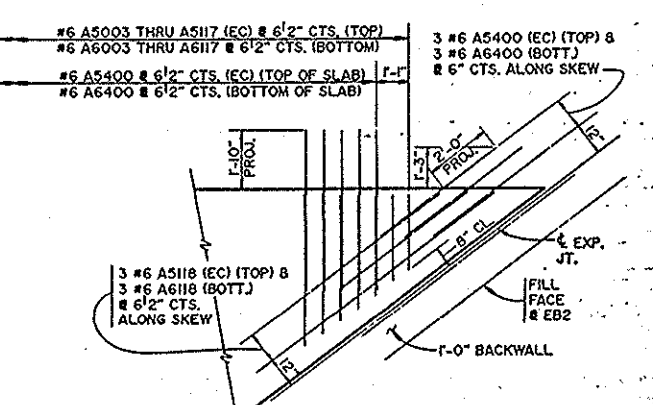
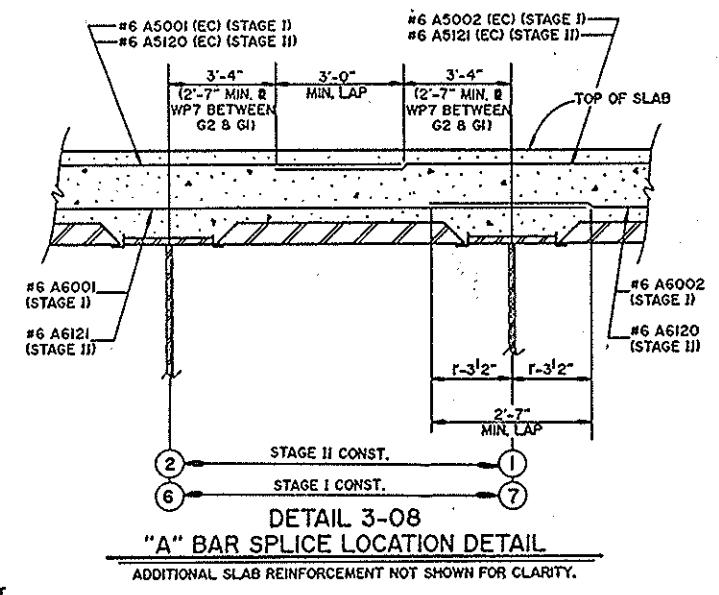
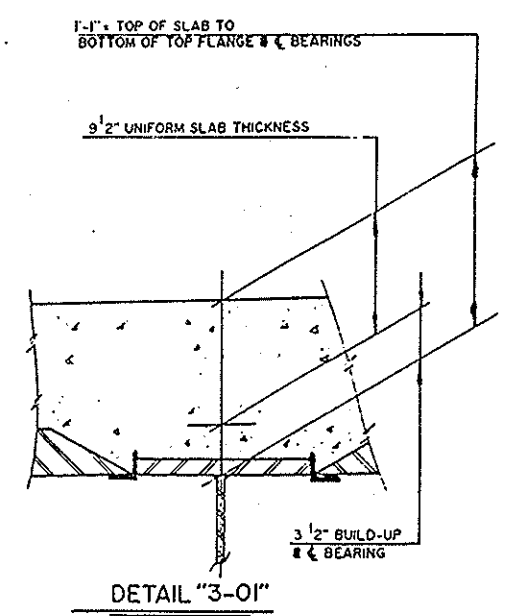
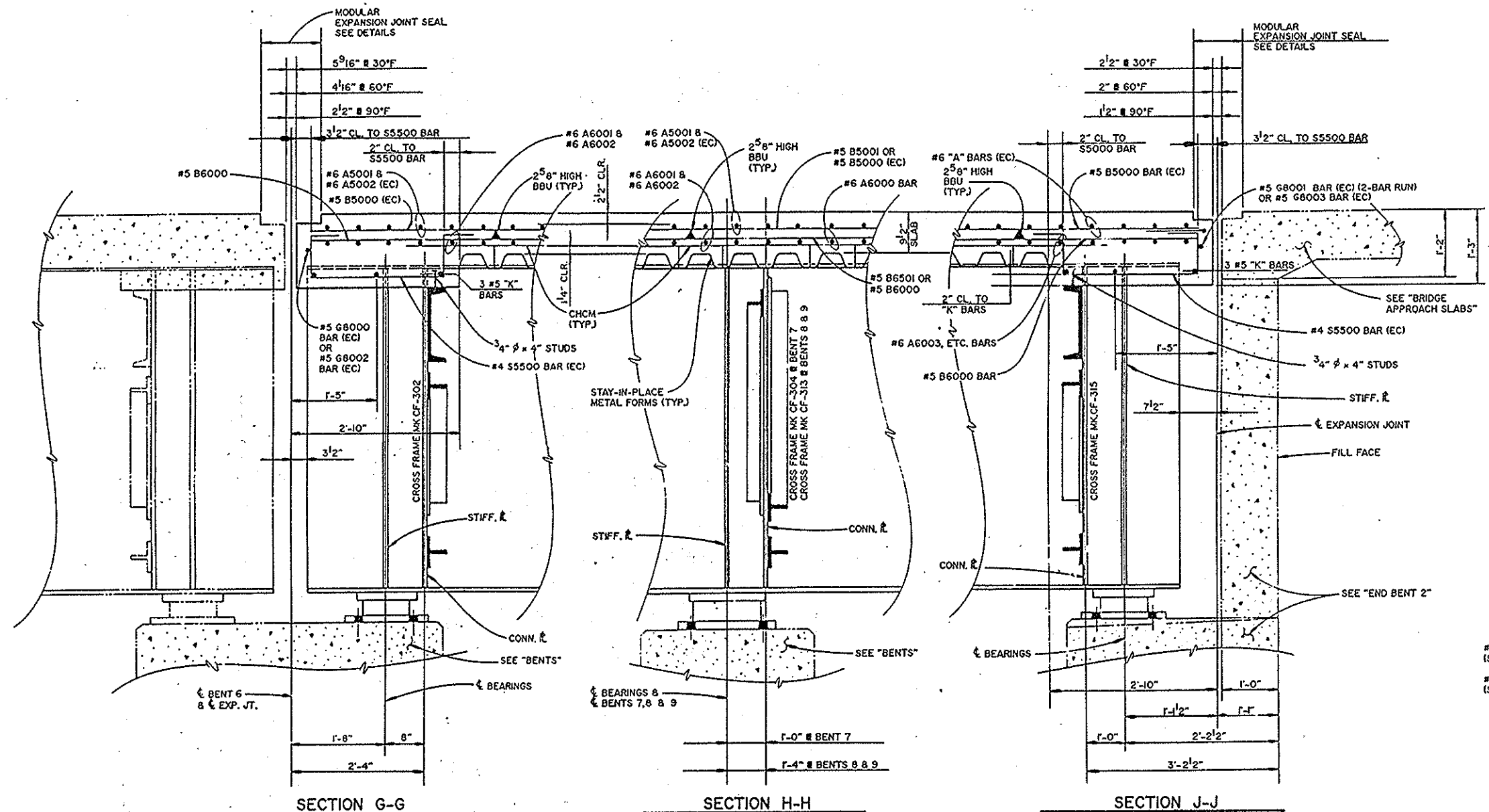


PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RAILROAD SUPERSTRUCTURE TYPICAL SECTIONS CONTINUOUS UNIT #3 (SPANS G, H, I & J) JAN. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
					SHEET NO. 5-10D
					TOTAL SHEETS 224

DRAWN BY: N.M. WILSON
 CHECKED BY: CEM
 DATE: JAN 94
 DATE: 6/95

U3.001
 PREPARED BY:
 THOMPSON-BORRUM-SHOCK
 ENGINEERS, INCORPORATED



PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RALEIGH SUPERSTRUCTURE DETAILS (CONTINUOUS UNIT #3)					
OCT. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

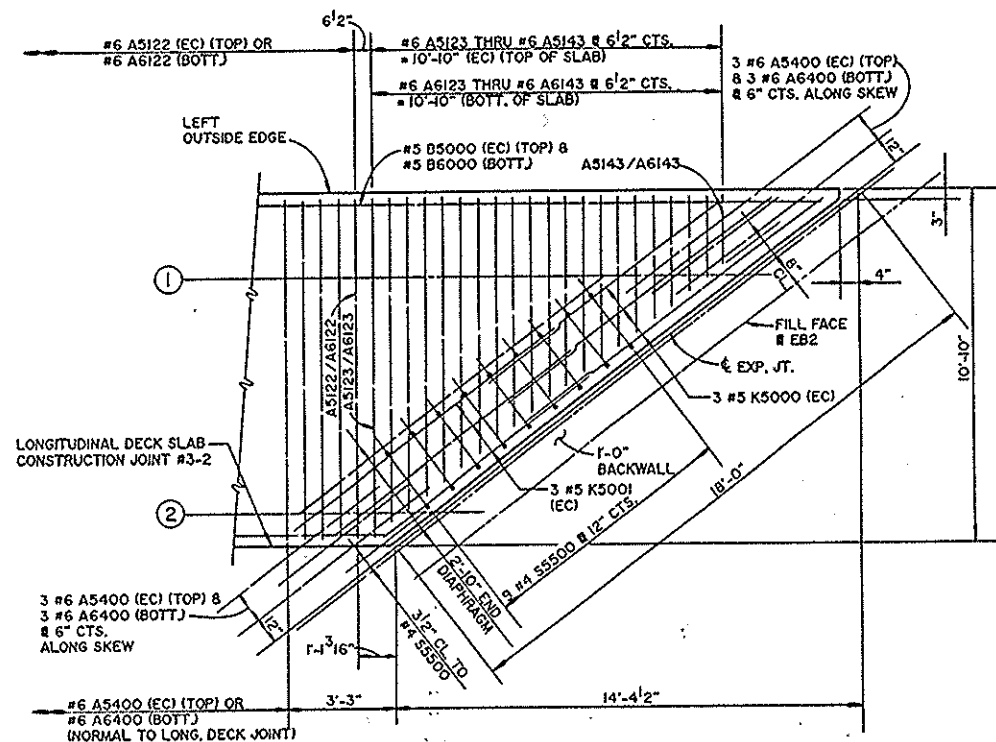
43-002
 PREPARED BY
 THOMPSON-GORDON-SHOCK
 ENGINEERS, INCORPORATED

SHEET NO. 5-101
 TOTAL SHEETS 224

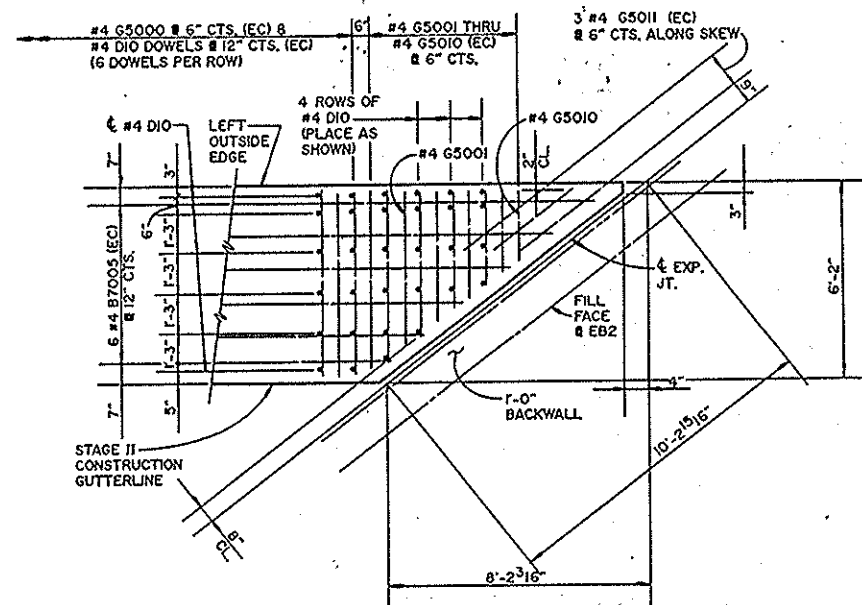


DRAWN BY: N.M. WILSON
 CHECKED BY: CEM
 DATE: OCT. 94
 DATE: 6/95

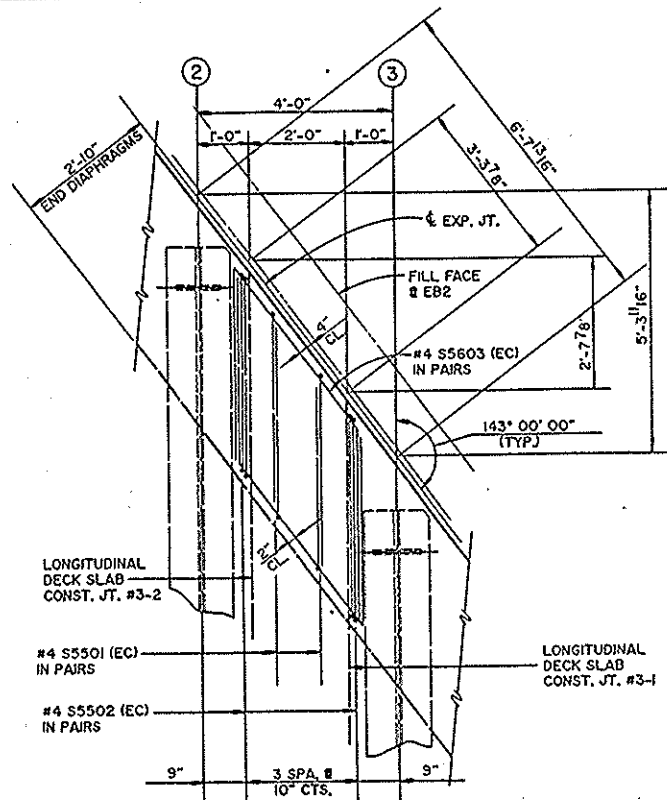
— PREPARED BY —
THOMPSON-BORDON-SHOCK
ENGINEERS, INCORPORATED



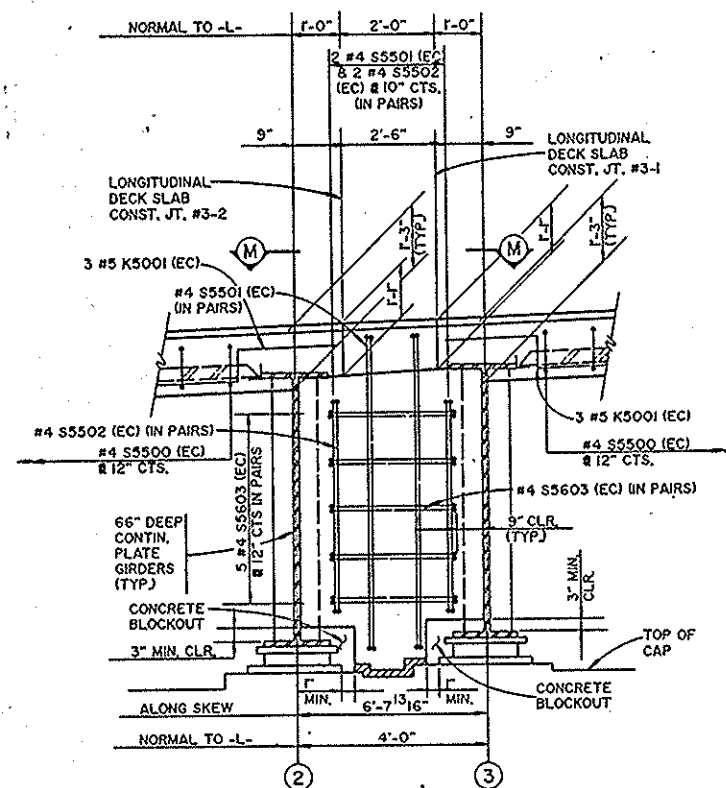
DETAIL 3-09 - DECK SLAB REINFORCEMENT
STAGE II CONSTRUCTION @ END BENT 2



DETAIL 3-10 - TOP OF SIDEWALK REINFORCEMENT
STAGE II CONSTRUCTION @ END BENT 2



VIEW "M-M"



DETAIL 3-23 - CLOSURE POUR END
DIAPHRAGM @ END BENT 2

NOTES FOR DECK SLAB - CONTINUOUS UNIT #1

PROVIDE CONTINUOUS HIGH CHAIR FOR METAL DECK (CHCM) @ 4'-0" CTS. WITH LEG SPACING TO MATCH THE PITCH OF THE FORMS AND WITH A HEIGHT TO SUPPORT THE BOTTOM LAYER OF SLAB REINFORCEMENT A CLEAR DISTANCE OF 1-1/4" ABOVE THE TOP OF THE STAY-IN-PLACE FORM.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE TRANSVERSE JOINTS IN THE DECK SLAB (WHERE APPLICABLE) ARE CONTINUOUS IN A STRAIGHT LINE THROUGH THE CLASSIC BRIDGE RAIL. THE CONTRACTOR'S ATTENTION IS ALSO CALLED TO THE FACT THAT THE LONGITUDINAL JOINTS IN THE DECK SLAB ARE CONTINUOUS IN A STRAIGHT LINE FROM BEGINNING OF CONTINUOUS UNIT TO ENDING OF CONTINUOUS UNIT.

THE MODULAR EXPANSION JOINT SEAL AT END BENT 2 SHALL BE CAPABLE OF HANDLING A TOTAL THERMAL MOVEMENT, MEASURED PARALLEL TO THE CENTERLINE OF ROADWAY, OF 3-1/4" (1-5/8" EXPANSION AND 1-5/8" CONTRACTION FROM A MIDPOINT TEMPERATURE OF 60° F).

THE MODULAR EXPANSION JOINT SEAL AT BENT 6 SHALL BE CAPABLE OF HANDLING A TOTAL THERMAL MOVEMENT, MEASURED PARALLEL TO THE CENTERLINE OF ROADWAY, OF 6-1/8" (3-1/16" EXPANSION AND 3-1/16" CONTRACTION FROM A MIDPOINT TEMPERATURE OF 60° F).

THE TOPS OF THE MODULAR EXPANSION JOINT SEALS SHALL BE SET A MINIMUM OF 1/8" AND A MAXIMUM OF 1/4" BELOW THE TOP OF SLAB.

FOR MODULAR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

CLASSIC BRIDGE RAILS AND SIDEWALKS IN THE CONTINUOUS UNIT SHALL NOT BE CAST UNTIL THE DECK SLAB CONCRETE IN EACH STAGE OF CONSTRUCTION HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

PREVIOUSLY CAST CONCRETE IN THE CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

STAY-IN-PLACE FORMS SHALL HAVE CLOSED TAPERED ENDS.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND GIRDER STIFFENER OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED AND INCLUDED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR ON THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

STRUCTURAL STEEL ERECTION IN EACH STAGE OF CONSTRUCTION FOR THE CONTINUOUS UNIT SHALL BE COMPLETE BEFORE FALSEWORK OR FORMS ARE PLACED IN EACH STAGE OF THE UNIT.

LAP SPLICES, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED IN THE B5000 BARS OVER BENTS 7, 8 & 9.

SEE THE TRAFFIC CONTROL PLANS FOR STAGE I, II, III & IV LOCATIONS AND PAY LIMITS OF THE TEMPORARY BRIDGE BARRIERS - TYPE S.

THE TOPS OF DECK DRAINS SHALL BE SET 3/8" BELOW SURFACE OF DECK SLAB.

FALSEWORK AND METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TESTS. SEE STRUCTURAL STEEL GIRDER ELEVATION SHEETS.

FOR BARS SHOWN AND NO BAR MARKS AND/OR BAR-RUNS SHOWN, SEE PLANS OF SPANS.

THE DIMENSIONS AND DETAILS SHOWN FOR THE CONCRETE DECK INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM AND SHOULD NOT BE CONSTRUED TO BE AN APPROVAL FOR THE ATTACHMENT OF THE UTILITY TO THE STRUCTURE.

FOR RESPONSIBILITIES FOR FURNISHING AND PLACING THE CONCRETE DECK INSERTS, SEE UTILITY SPECIAL PROVISIONS.

FOR CLASSIC BRIDGE RAILS, SEE SPECIAL PROVISIONS.

DIRECTION OF CASTING DECK CONCRETE SHALL BE AS SHOWN ON THE DECK SLAB POURING SEQUENCE SHEET FOR STAGE I CONSTRUCTION. DIRECTION OF CASTING DECK CONCRETE IN SUBSEQUENT STAGES SHALL FOLLOW THE SAME PATTERN.

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev.

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
DETAILS
(CONTINUOUS UNIT #3)
MARCH 1995

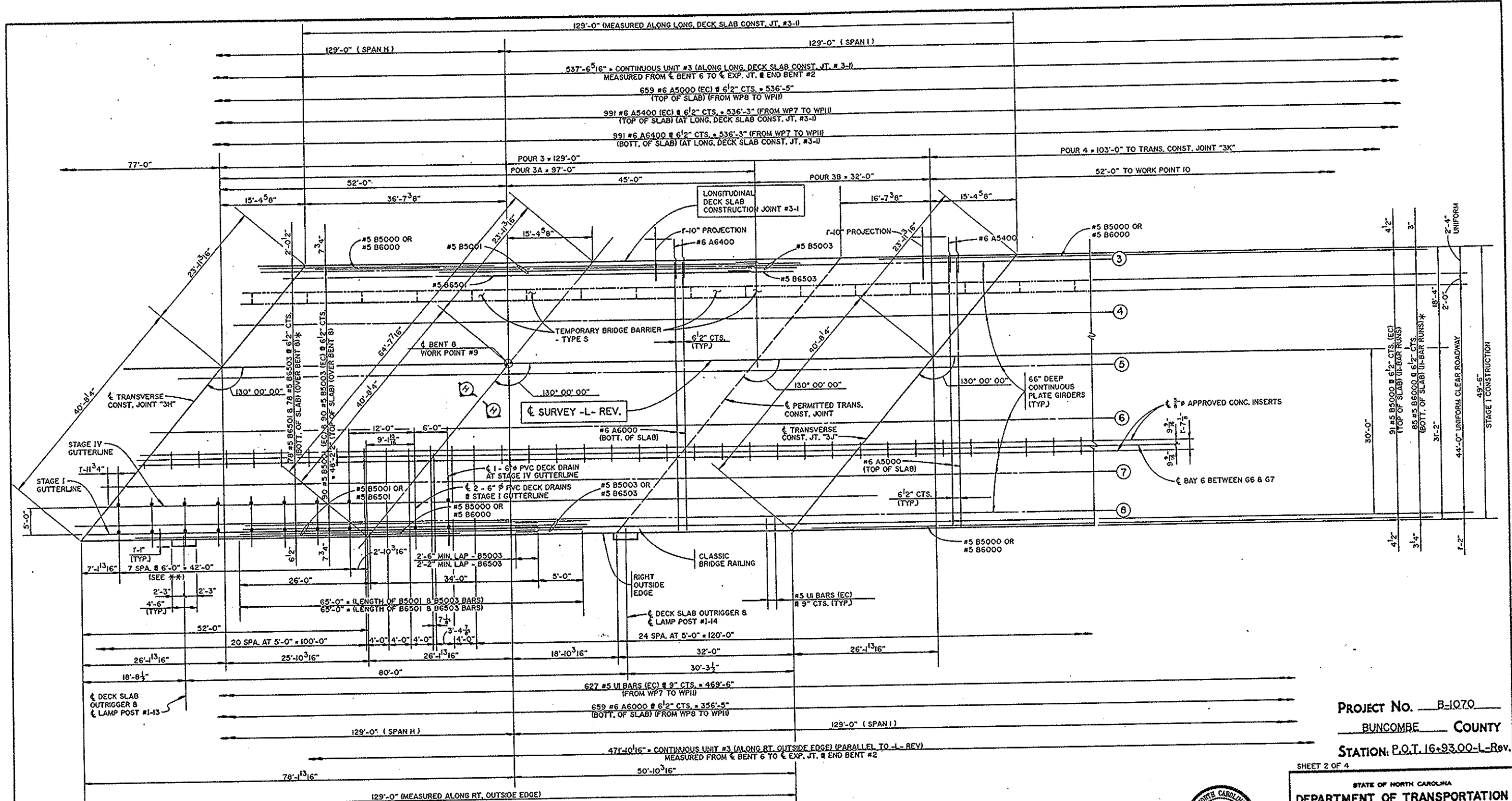


43.004
PREPARED BY
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

REVISIONS				SHEET	
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

TOTAL
SHEETS
224

DRAWN BY: N. H. WILSON
CHECKED BY: CCM
DATE: MAR 95
DATE: 6/95



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING
 FOR DECK SLAB POURING SEQUENCE PLAN,
 SEE CONTIN. UNIT #2 - STAGE I CONSTRUCTION
 DETAIL SHEET 3 OF 3.
 ** 8 - 6" PVC DECK DRAINS @
 STAGE IV GUTTERLINE &
 8 - 6" PVC DECK DRAINS @
 STAGE I GUTTERLINE.

PLAN OF SPANS H & I (CONTINUOUS UNIT #3)
 STAGE I CONSTRUCTION
 (POUR 3)

DRAWN BY: N. H. WILSON DATE: JAN 94
 CHECKED BY: G. M. DATE: 2/15



U3.000
 PREPARED BY:
 THOMPSON-GORDON-SHOOK
 ENGINEERS, INCORPORATED

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-R9v.

SHEET 2 OF 4

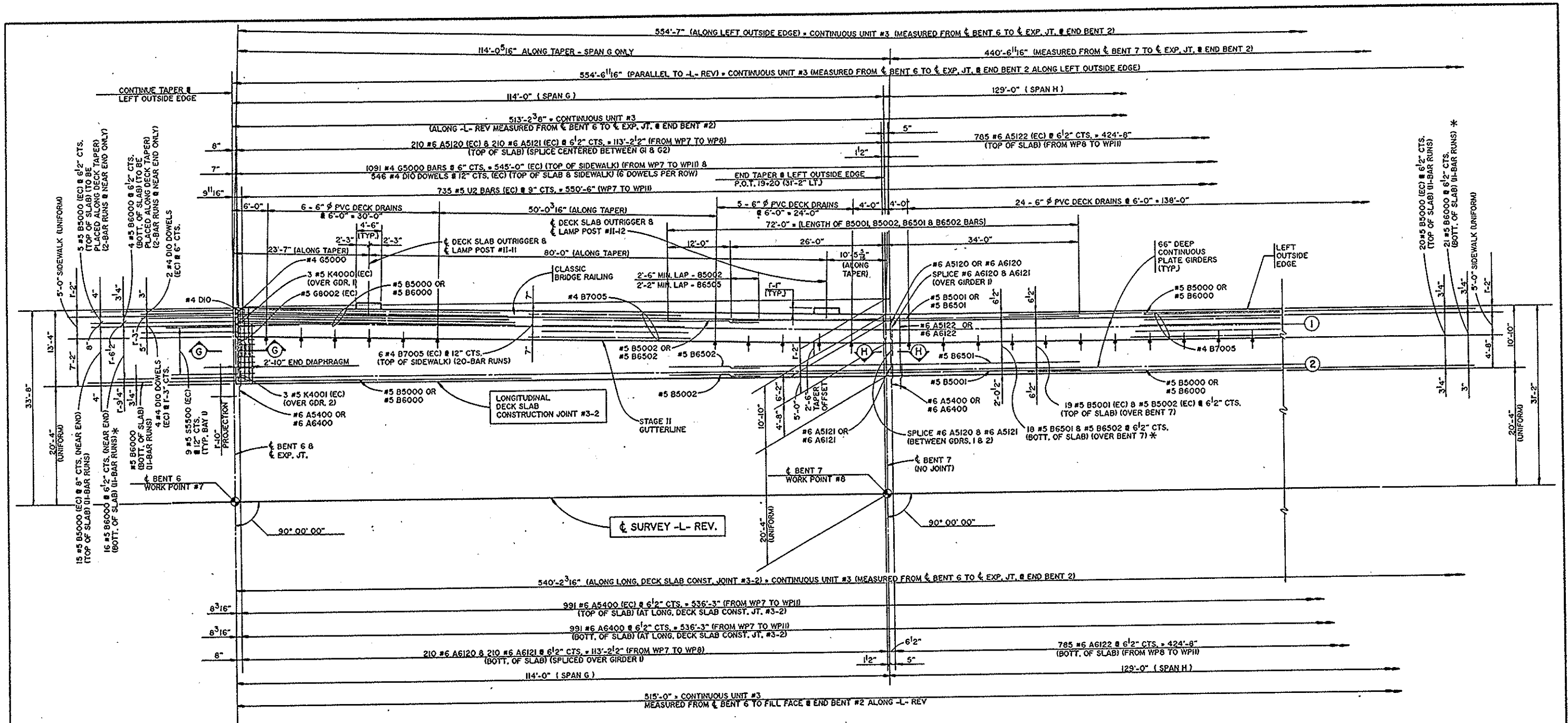
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION					
RAILROAD SUPERSTRUCTURE					
PLAN OF SPANS H & I (CONTINUOUS UNIT #3) STAGE I CONSTRUCTION					
JAN. 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. 2105
					TOTAL SHEETS 224



FOR DECK SLAB POURING SEQUENCE PLAN,
SEE CONTIN. UNIT #2 - STAGE 1 CONSTRUCTION
DETAIL SHEET 3 OF 3.

REVISIONS						SHEET NO. 5-107
NO.	BY	DATE	NO.	BY	DATE	
1			3			TOTAL SHEETS 224
2			4			

PROJECT No. B-1079
BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-1-Rev.



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPANS G & H (CONTINUOUS UNIT #3)
 STAGE II CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 3

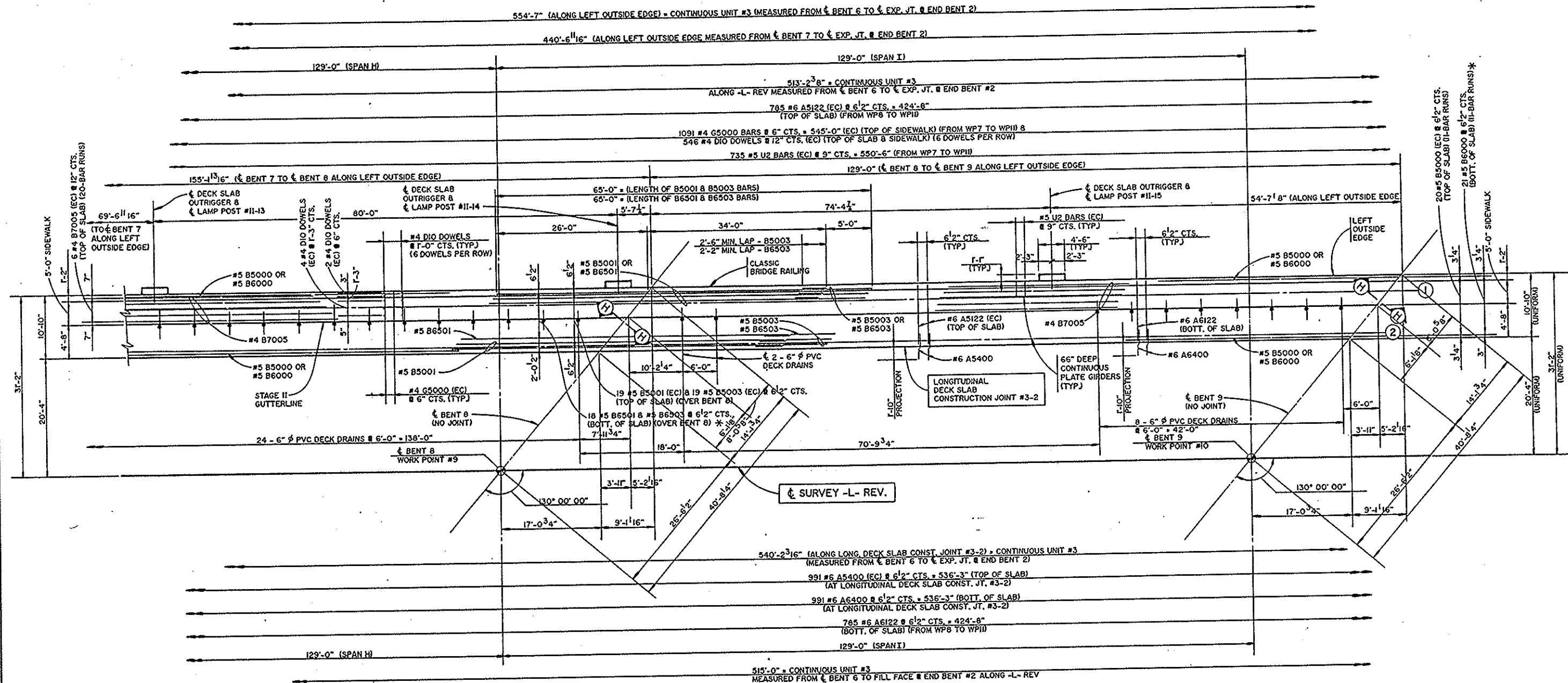
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPANS G & H
 (CONTINUOUS UNIT #3)
 STAGE II CONSTRUCTION
 JAN. 1994

REVISIONS				SHEET NO.			
NO.	BY	DATE	NO.	BY	DATE	TOTAL SHEETS	
1			3			5-108	
2			4			224	



113.009
 PREPARED BY
 THOMPSON-BORCH-SHOCK
 ENGINEERS, INCORPORATED

DRAWN BY: M. H. WILSON
 CHECKED BY: J. H. WILSON
 DATE: JAN 94
 DATE: 1/12/94



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPANS H & I (CONTINUOUS UNIT #3) STAGE II CONSTRUCTION

DRAWN BY: N. M. WILSON DATE: MAR 94
 CHECKED BY: J. M. WILSON DATE: 6/19

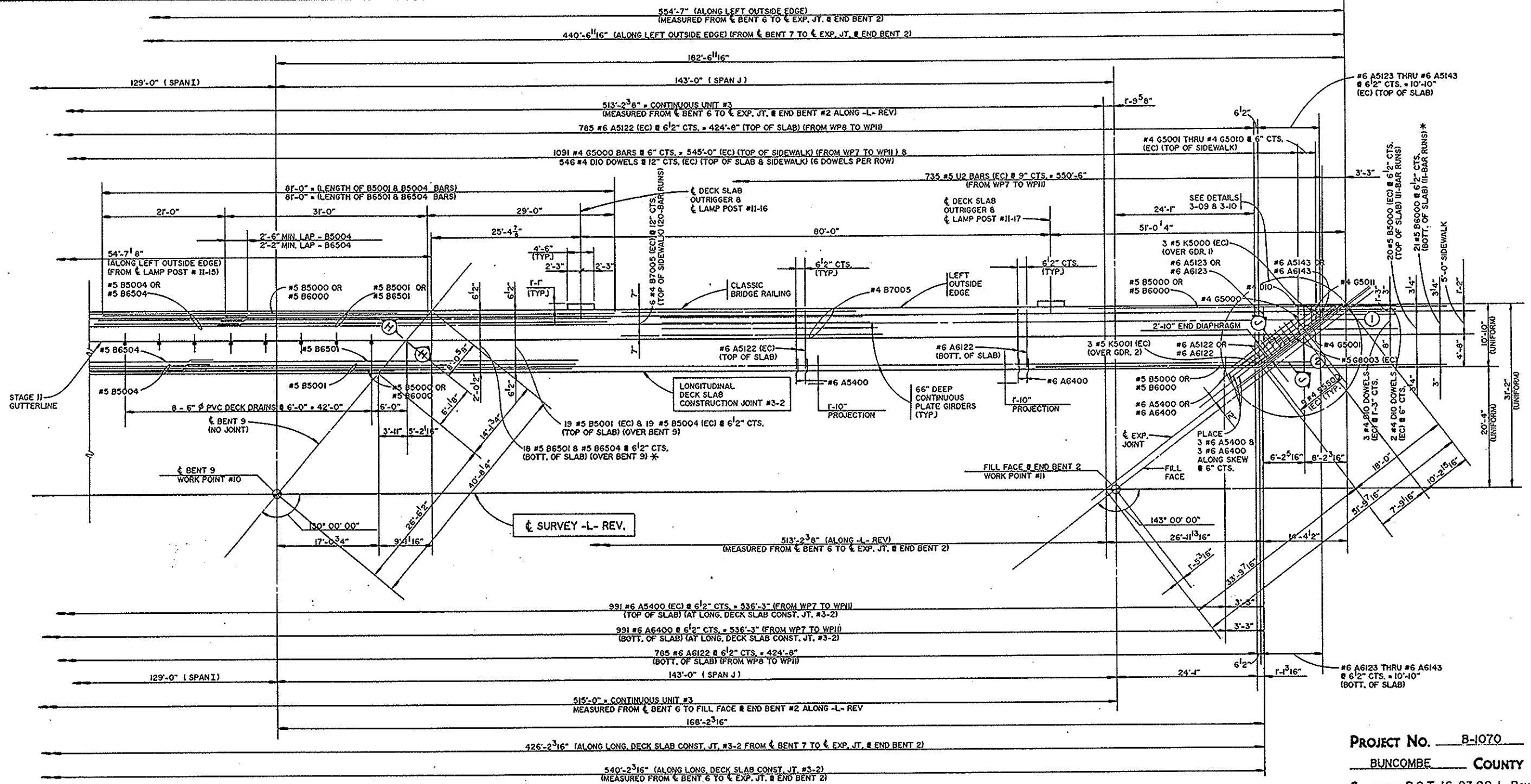


U.S. 010
 PREPARED BY:
 THOMPSON-GORDON-SHOOK
 ENGINEERS, INCORPORATED

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.
 SHEET 2 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPANS H & I
 (CONTINUOUS UNIT #3)
 STAGE II CONSTRUCTION
 MARCH 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-109
2			4			224



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 * SEE TYPICAL SECTION FOR SPACING

PLAN OF SPANS I & J (CONTINUOUS UNIT #3)
 STAGE II CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

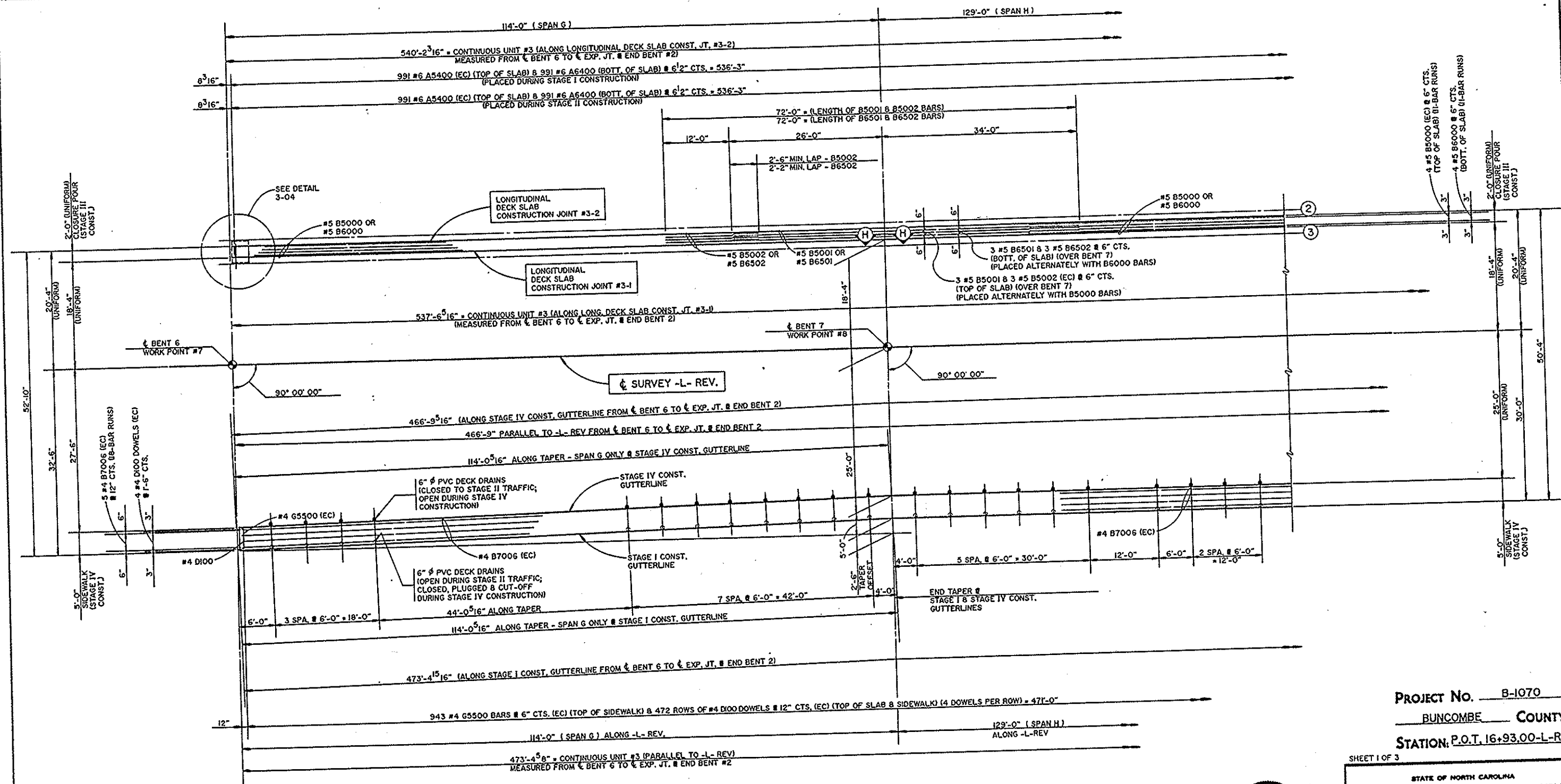
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE					
PLAN OF SPANS I & J (CONTINUOUS UNIT #3) STAGE II CONSTRUCTION MARCH 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

U.S. 211
 THOMPSON - GORDON - SHOOK
 ENGINEERS, INCORPORATED

4/12/96

SHEET NO. 5-110
 TOTAL SHEETS 224



(EC) DENOTES EPOXY-COATED REINFORCING STEEL
 #4 D100 DOWELS (STAGE IV CONST.) SHALL BE ADHESIVELY ANCHORED. SEE SPECIAL PROVISIONS.

PLAN OF SPANS G & H (CONTINUOUS UNIT #3)
 STAGE III & IV CONSTRUCTION

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-Rev.

SHEET 1 OF 3

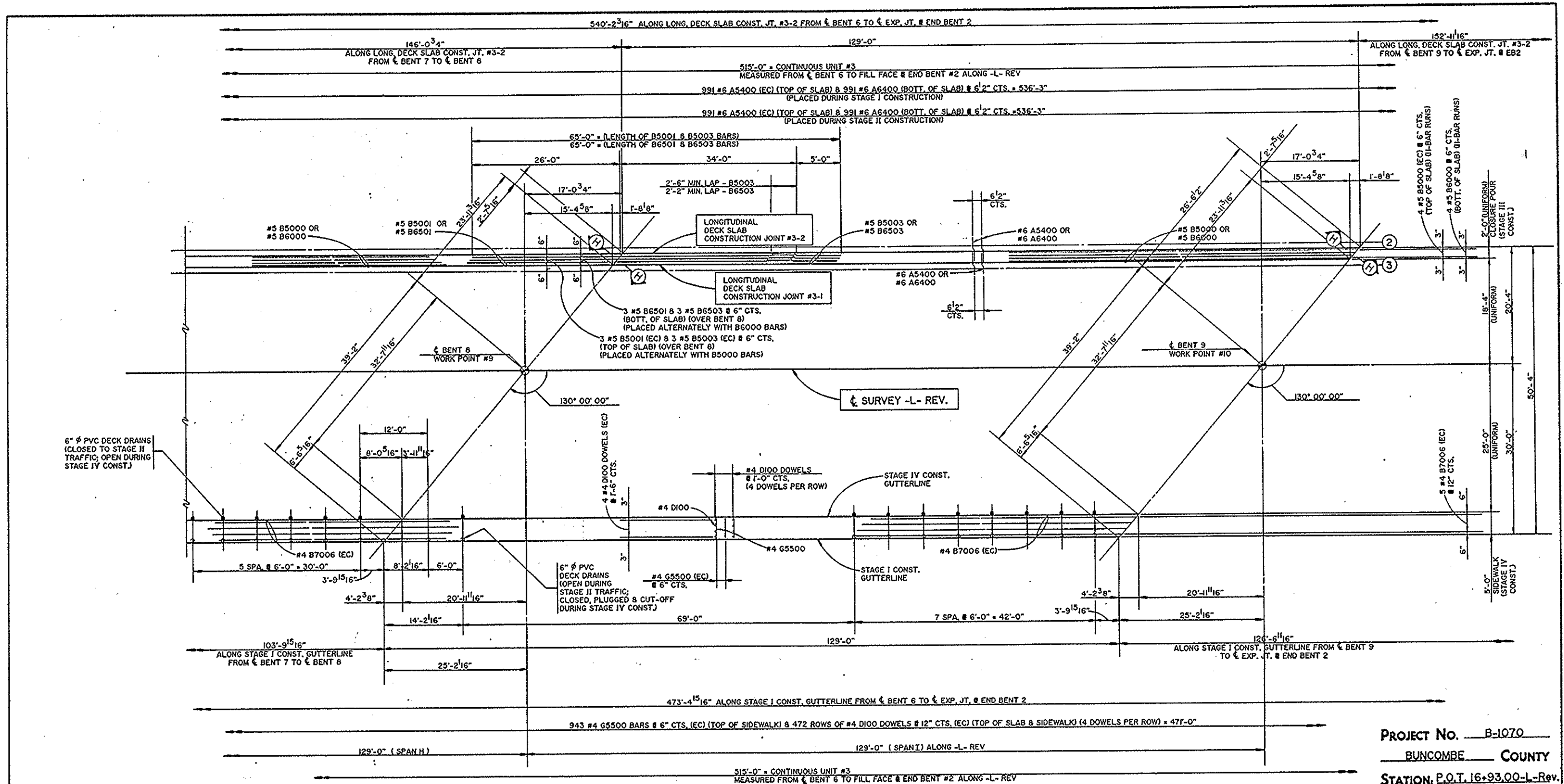
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPANS G & H
 (CONTINUOUS UNIT #3)
 STAGE III & IV CONSTRUCTION
 MARCH 1994

REVISIONS					SHEET NO. 5-111
NO.	BY	DATE	NO.	DATE	
1			2		TOTAL SHEETS 224
2			4		



U3.012
 PREPARED BY
 THOMPSON-GORDON-SHOCK
 ENGINEERS, INCORPORATED

DRAWN BY: N. M. WILSON DATE: MAR 94
 CHECKED BY: GEM DATE: 2/12



(EC) DENOTES EPOXY-COATED REINFORCING STEEL

#4 D100 DOWELS (STAGE IV CONST.) SHALL BE ADHESIVELY ANCHORED. SEE SPECIAL PROVISIONS.

PLAN OF SPANS H & I (CONTINUOUS UNIT #3) STAGE III & IV CONSTRUCTION

PROJECT No. B-1070

BUNCOMBE COUNTY

STATION: P.O.T. 16+93.00-L-Rev.

SHEET 2 OF 3

STATE OF NORTH CAROLINA					
DEPARTMENT OF TRANSPORTATION					
RALEIGH					
SUPERSTRUCTURE					
PLAN OF SPANS H & I					
(CONTINUOUS UNIT #3)					
STAGE III & IV CONSTRUCTION					
MARCH 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
SHEET NO.					5-112
TOTAL SHEETS					224



U3.013

PREPARED BY

THOMPSON - GORDON - SHOOK

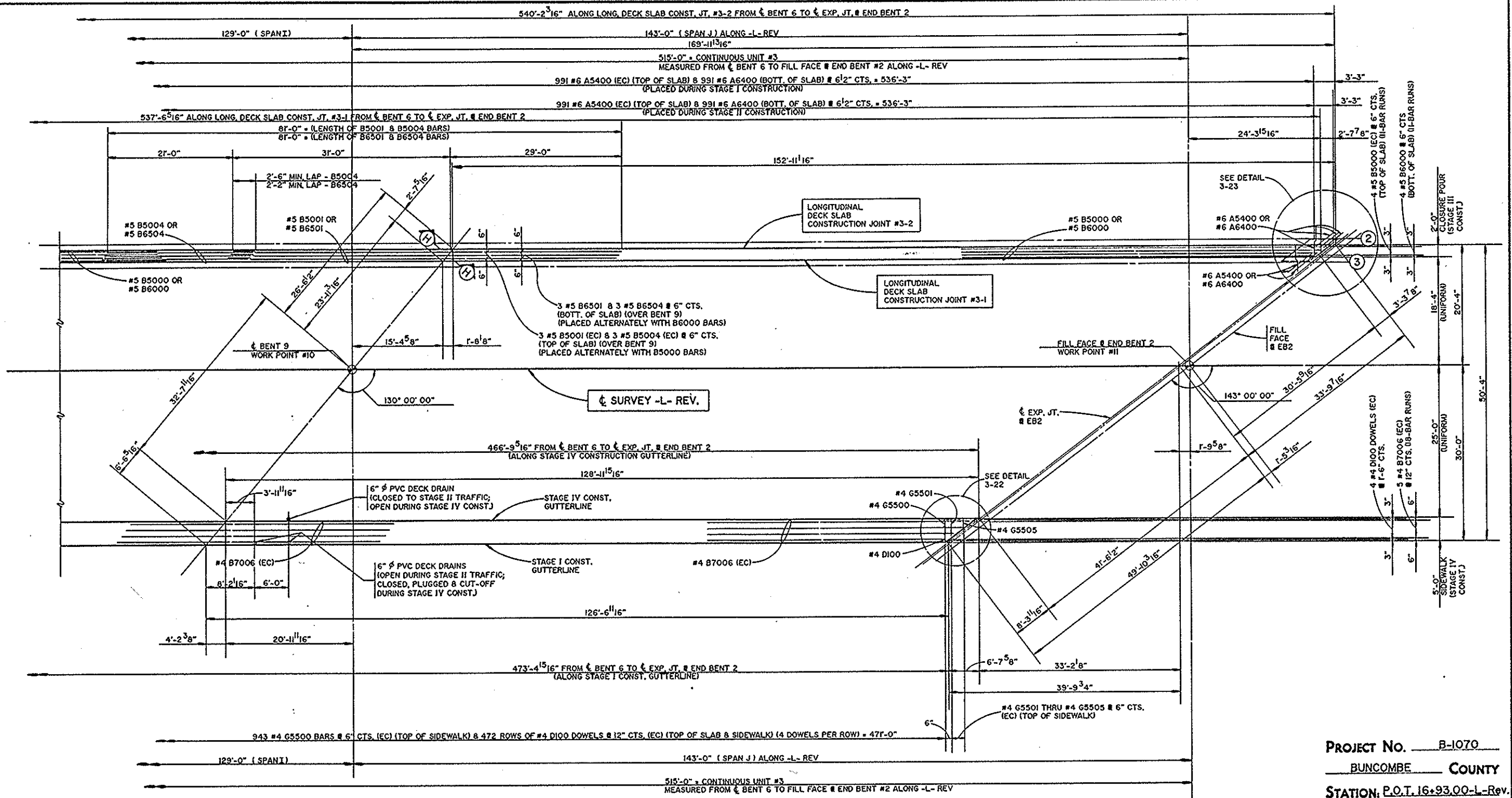
ENGINEERS, INCORPORATED

DRAWN BY: N.M. WILSON

CHECKED BY: ECM

DATE: MAR 94

DATE: 4/15



PLAN OF SPANS I & J (CONTINUOUS UNIT #3)
STAGE III & IV CONSTRUCTION

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV.

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPANS I & J
(CONTINUOUS UNIT #3)
STAGE III & IV CONSTRUCTION
MARCH 1994

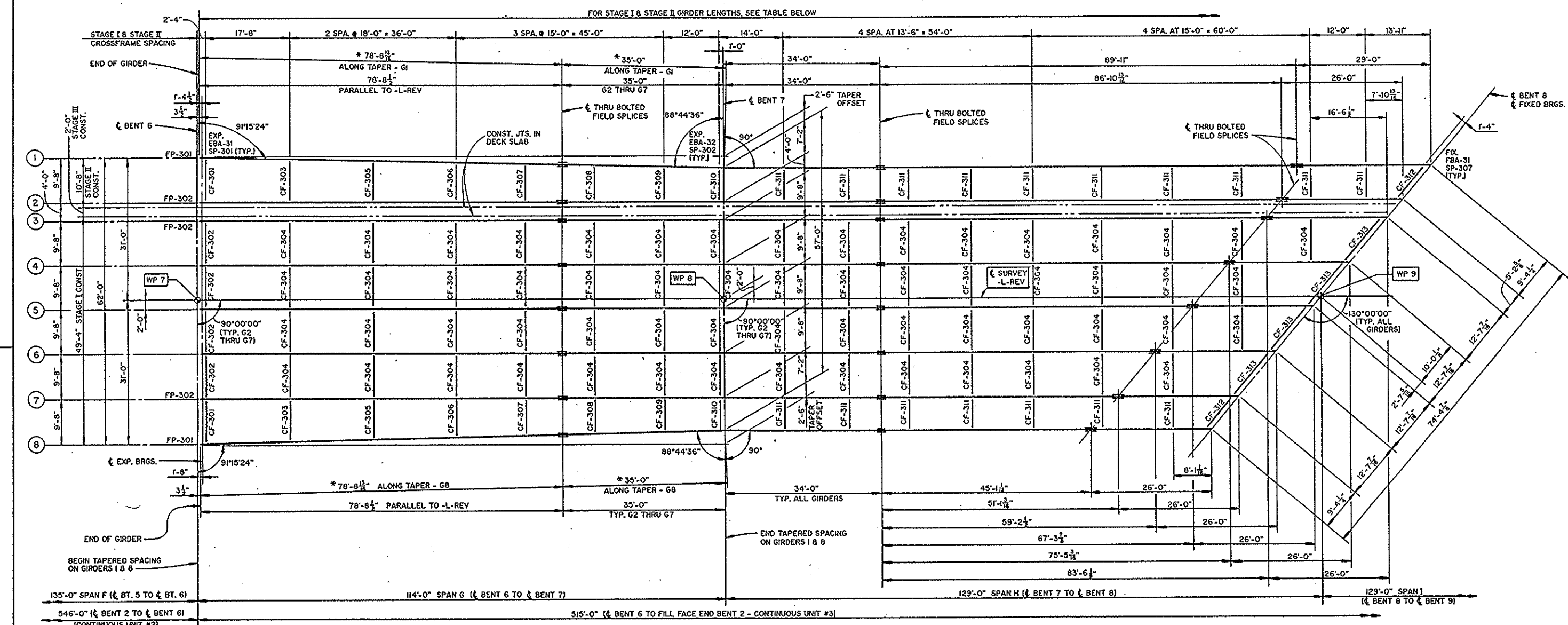
REVISIONS				SHEET			
NO.	BY	DATE	NO.	BY	DATE	TOTAL	SHEETS
1			2			5-113	
2			3			224	



03.014

PREPARED BY
THOMPSON-BOROUGH-BROOK
ENGINEERS, INCORPORATED

DRAWN BY: N. M. WILSON DATE: MAR 94
CHECKED BY: GCM DATE: 4/15



* THESE DIMENSIONS ARE MEASURED ALONG & GIRDER HORIZONTALLY.

FRAMING PLAN - SPANS G & H - STAGES I, II & III CONSTRUCTION

GIRDER LENGTHS FOR UNIT #3	
①	* 549'-8 3/4"
②	540'-2 1/8"
③	634'-10 1/8"
④	522'-0 1/2"
⑤	509'-2 1/8"
⑥	496'-4 1/8"
⑦	483'-6 1/8"
⑧	* 474'-0 1/8"



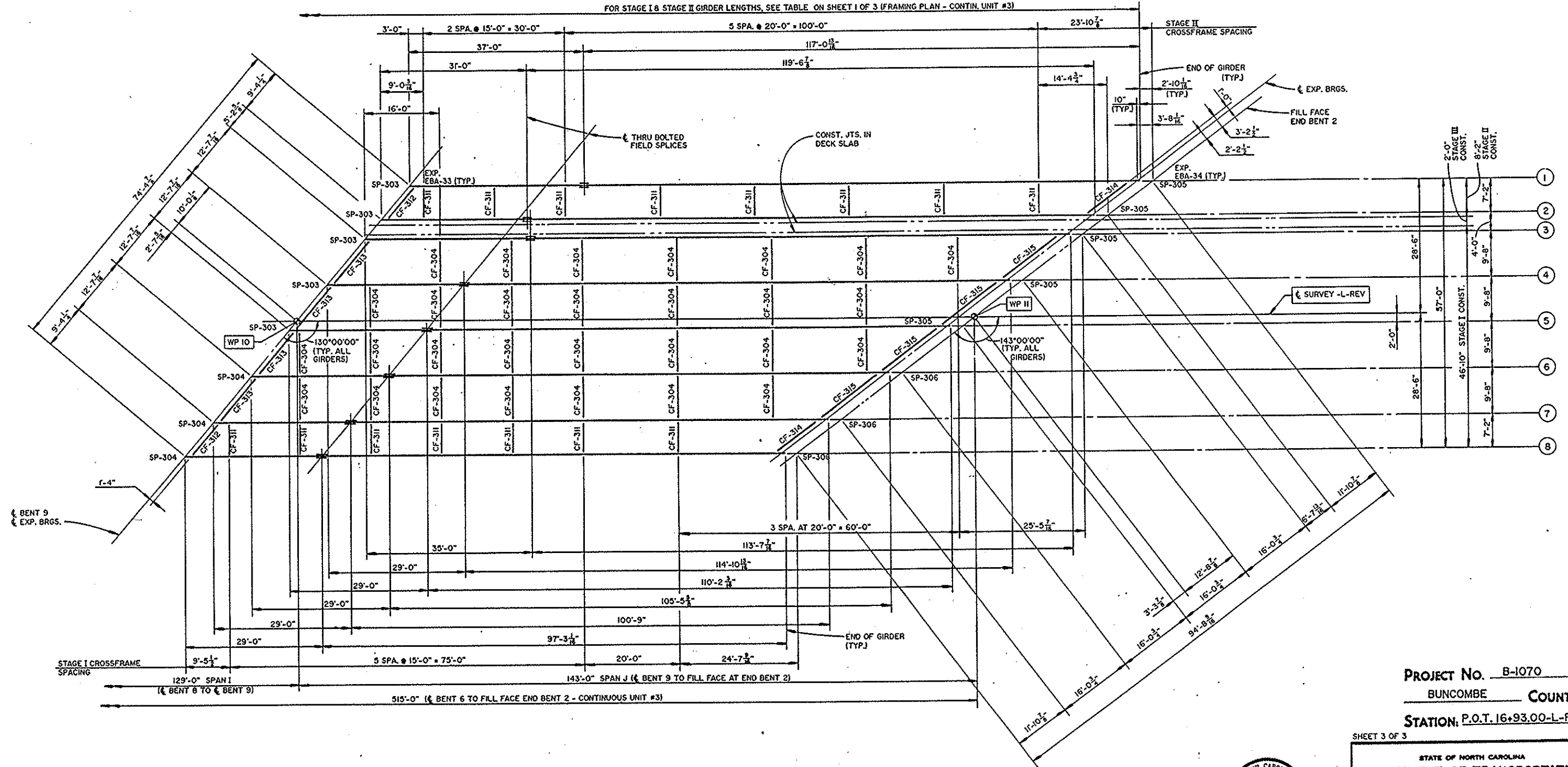
43.015
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL FRAMING PLAN - SPANS G & H (CONTINUOUS UNIT #3) STAGES I, II & III CONST. MARCH 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
					SHEET NO. 5-114 TOTAL SHEETS 224

DRAWN BY: BTL DATE: 12/74
CHECKED BY: GCM DATE: 4/85

FOR STAGE I & STAGE II GIRDER LENGTHS, SEE TABLE ON SHEET 1 OF 3 (FRAMING PLAN - CONTIN. UNIT #3)



FRAMING PLAN - SPAN J - STAGES I, II & III CONSTRUCTION

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 3

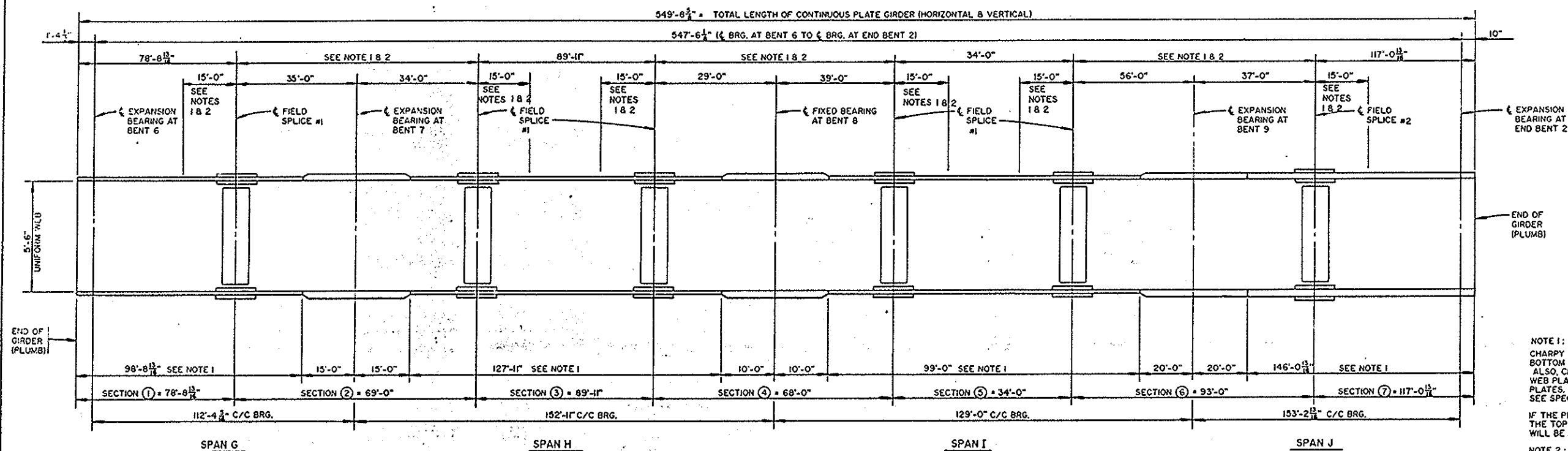
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
FRAMING PLAN - SPAN J
(CONTINUOUS UNIT #3)
STAGES I, II & III CONST.
MARCH 1994



U3.017
PREPARED BY
THOMPSON-GORDON-BROOK
ENGINEERS, INCORPORATED

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-116
2			4			224

DRAWN BY: RTH DATE: 3/74
CHECKED BY: ECM DATE: 6/95

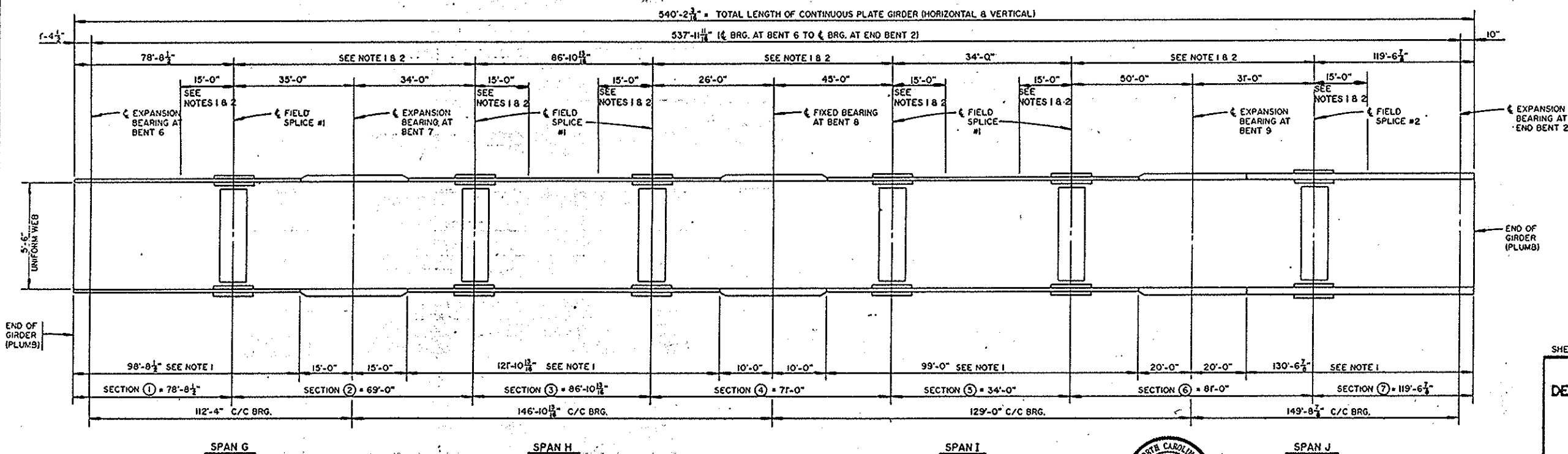


ELEVATION OF GIRDER 1

NOTE 1:
CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.

IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.

NOTE 2:
NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.



ELEVATION OF GIRDER 2

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 1 OF 12

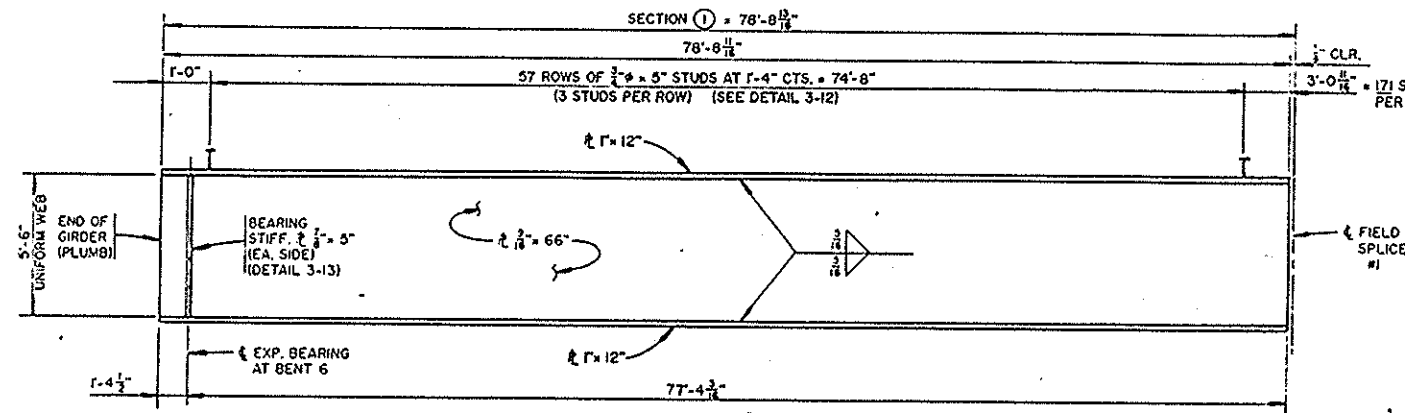
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER ELEVATIONS 1 AND 2
CONTINUOUS UNIT #3
OCTOBER 1994

REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-117
2			4			224

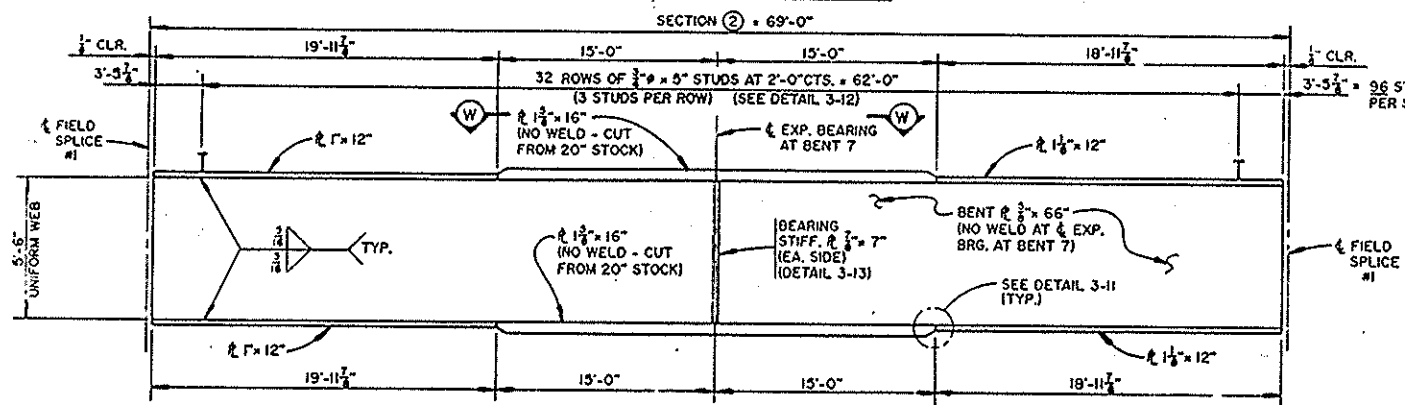


03.018
PREPARED BY
THOMPSON - GORDON - SHOOK
ENGINEERS, INCORPORATED

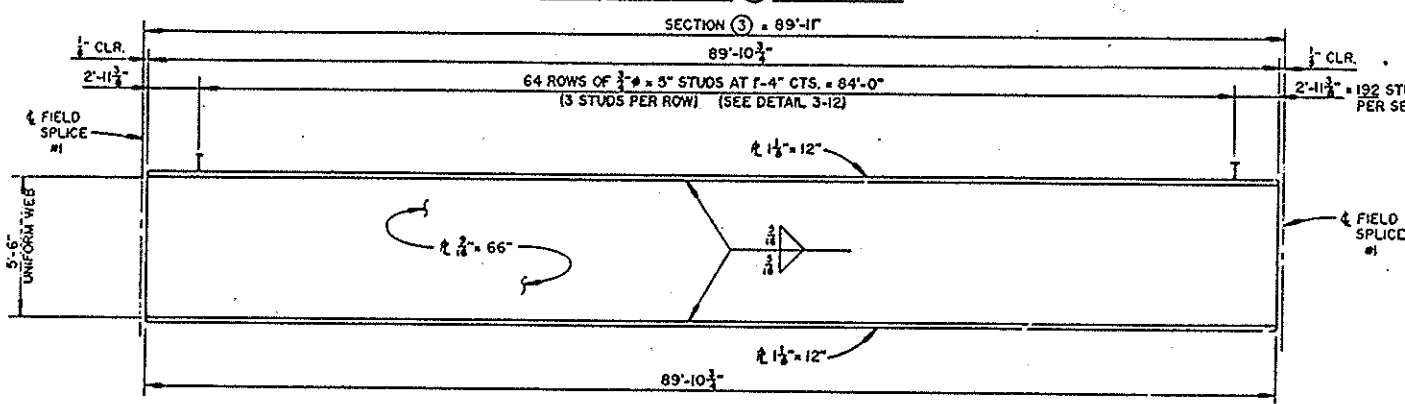
DRAWN BY: RTJ
CHECKED BY: ZEM
DATE: 12-94
DATE: 6/95



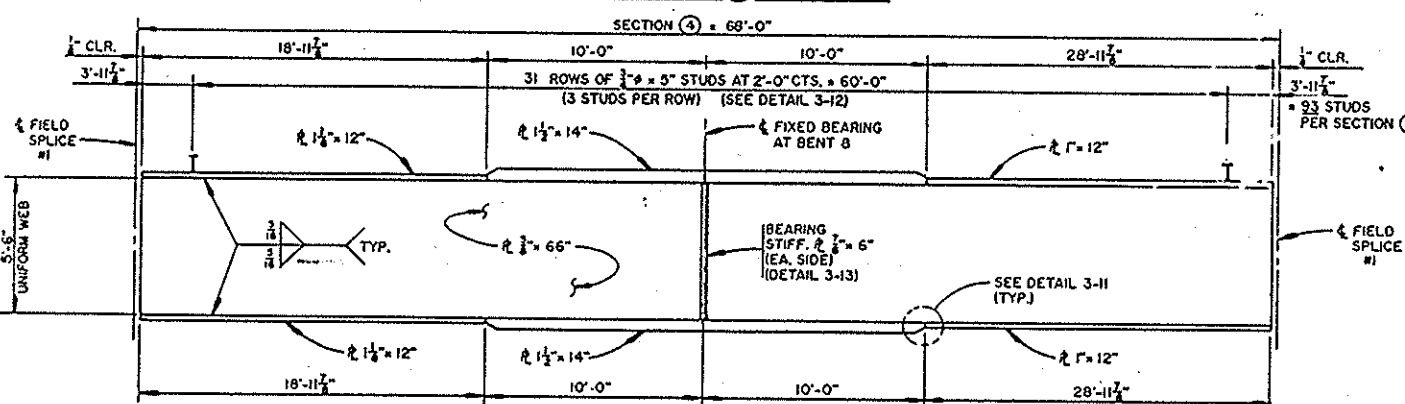
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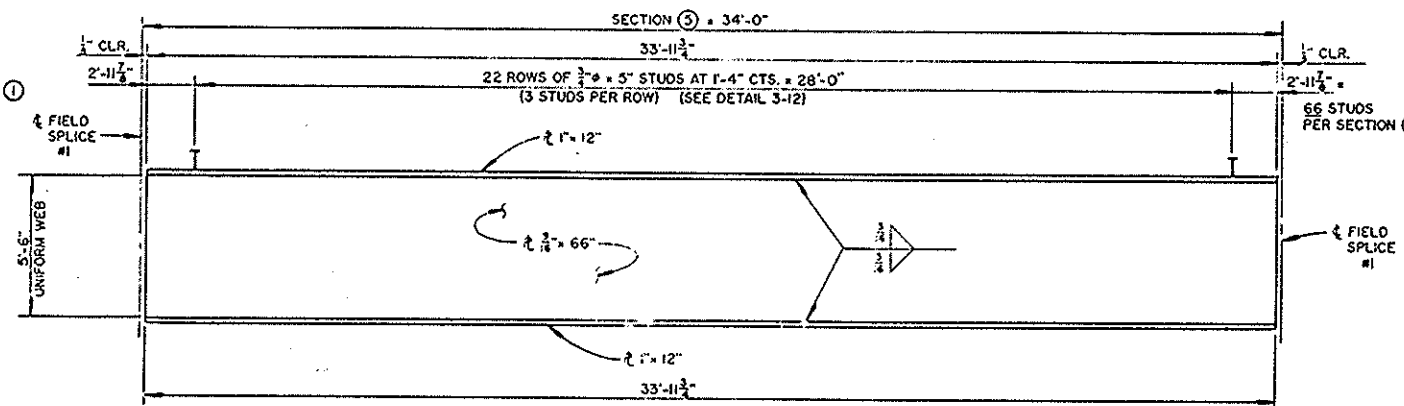
GIRDER SECTION (2) - 1 REQ'D.



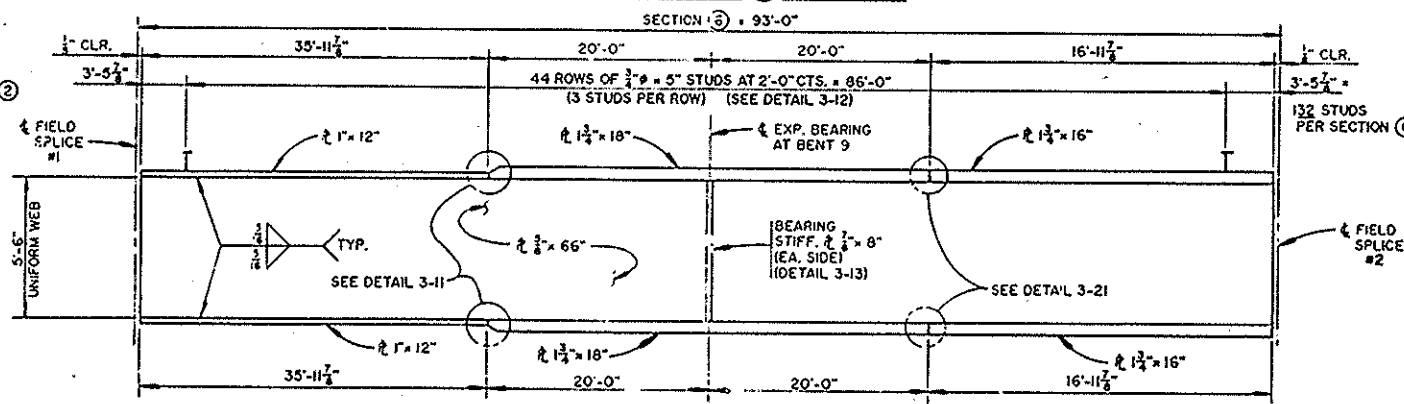
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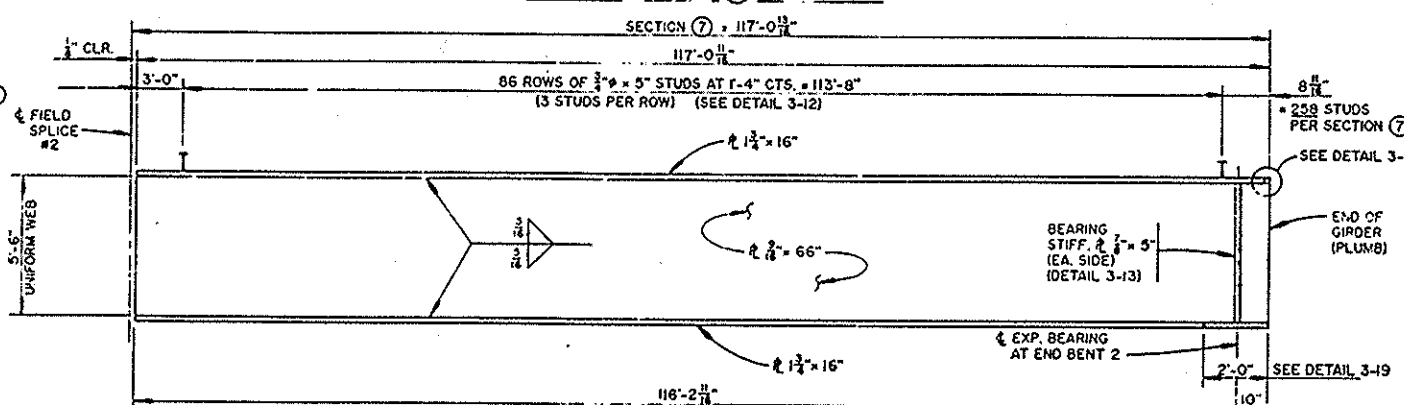
GIRDER SECTION (4) - 1 REQ'D.



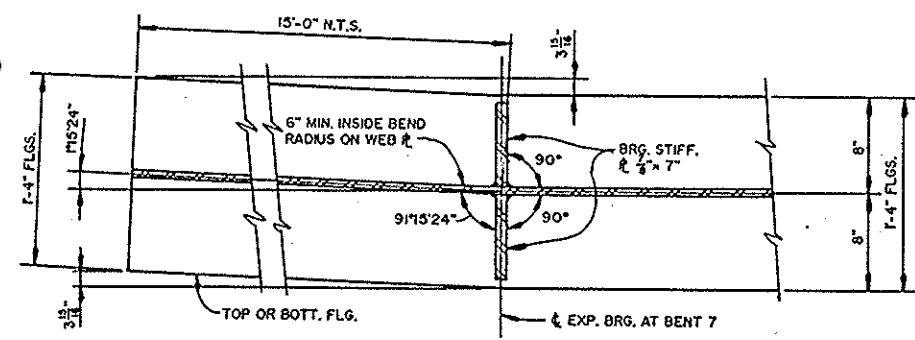
GIRDER SECTION (5) - 1 REQ'D.



GIRDER SECTION (6) - 1 REQ'D.



GIRDER SECTION (7) - 1 REQ'D.



PART - SECTION W-W
GIRDER 1 - SECTION (2) ONLY



PROJECT No. B-1070
BUNCOMBE COUNTY
STATION P.O.T. 16+93.00-L-REV

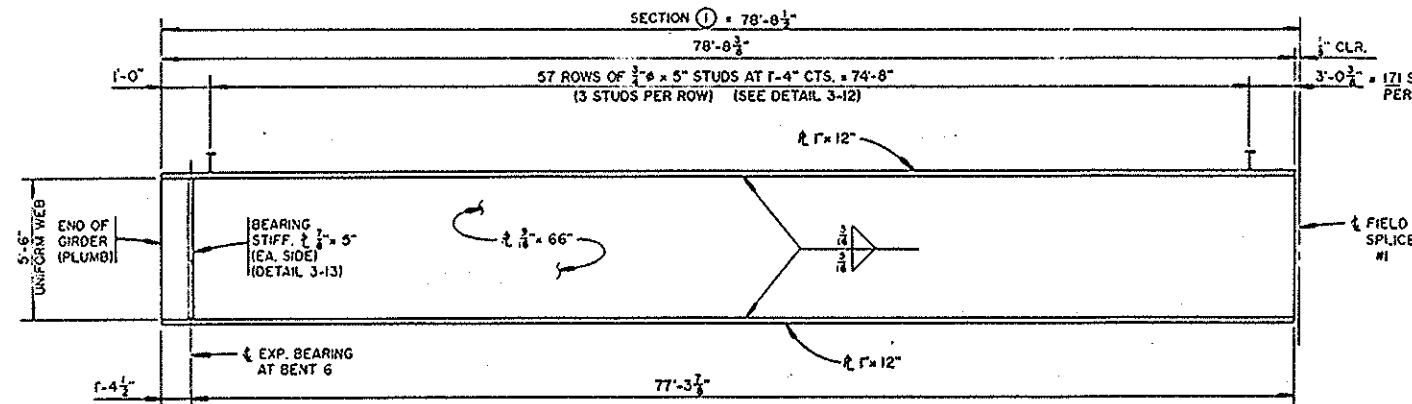
SHEET 2 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 1 DETAILS
CONTINUOUS UNIT #3
OCTOBER 1994

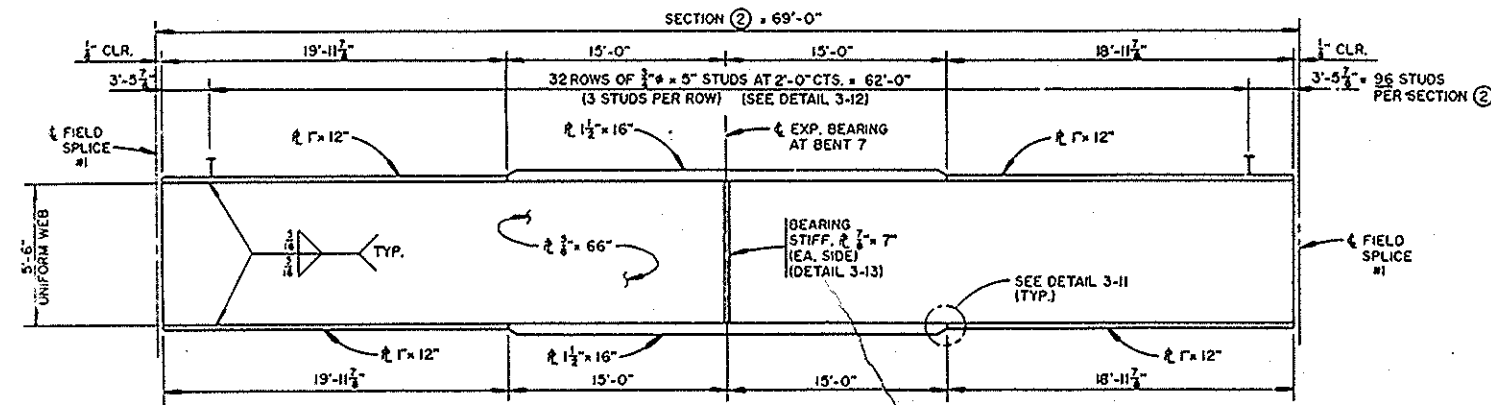
REVISIONS						SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE	5-118	
1			3			TOTAL SHEETS 224	
2			4				

DRAWN BY: KTL
CHECKED BY: GCM
DATE: 10/94
DATE: 9/95

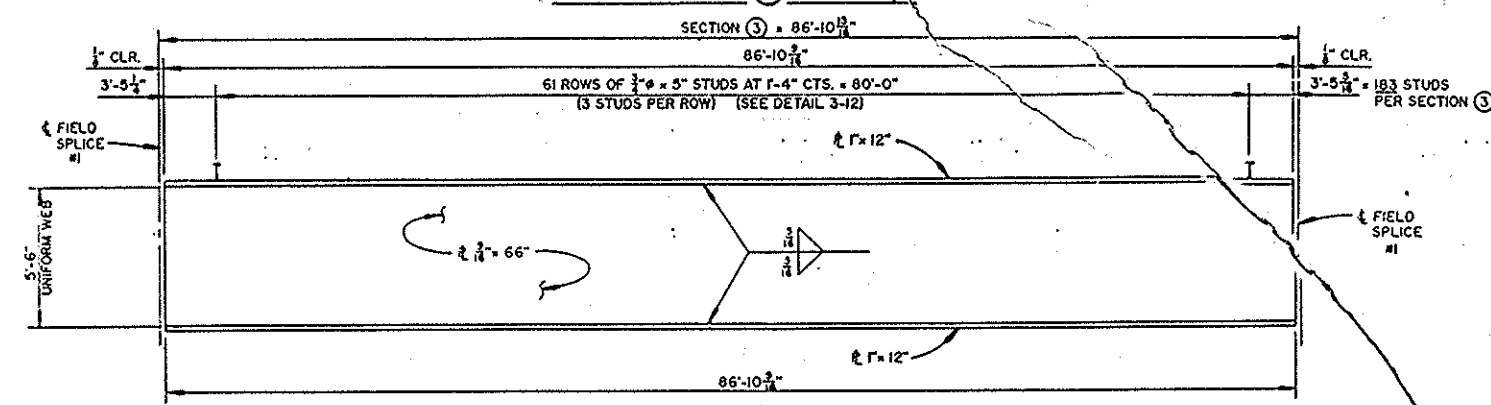
U3.019
PREPARED BY
THOMPSON - GORDON - BROS
ENGINEERS, INCORPORATED



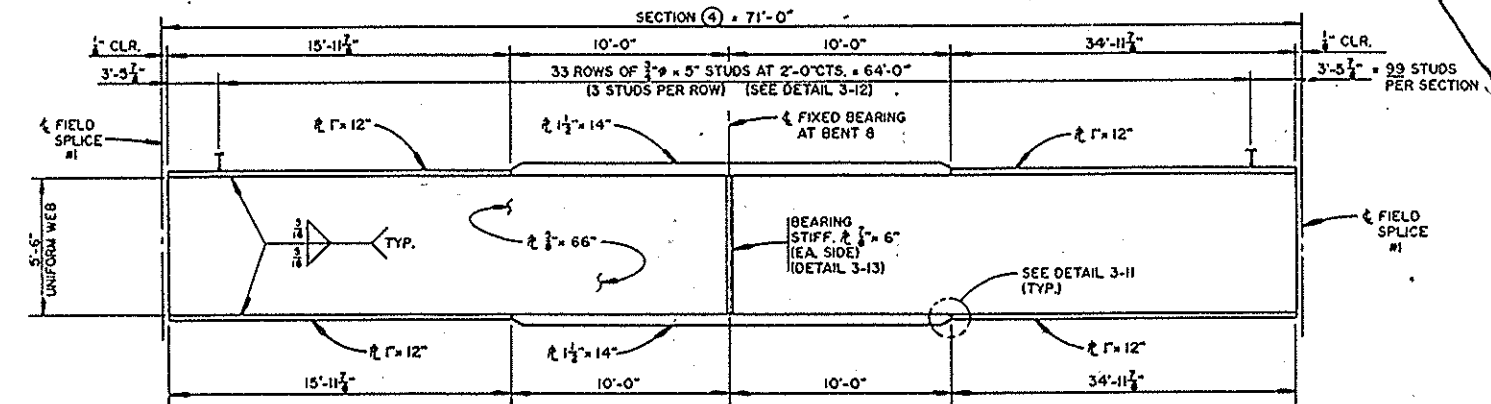
GIRDER SECTION 1 - I REQ'D.



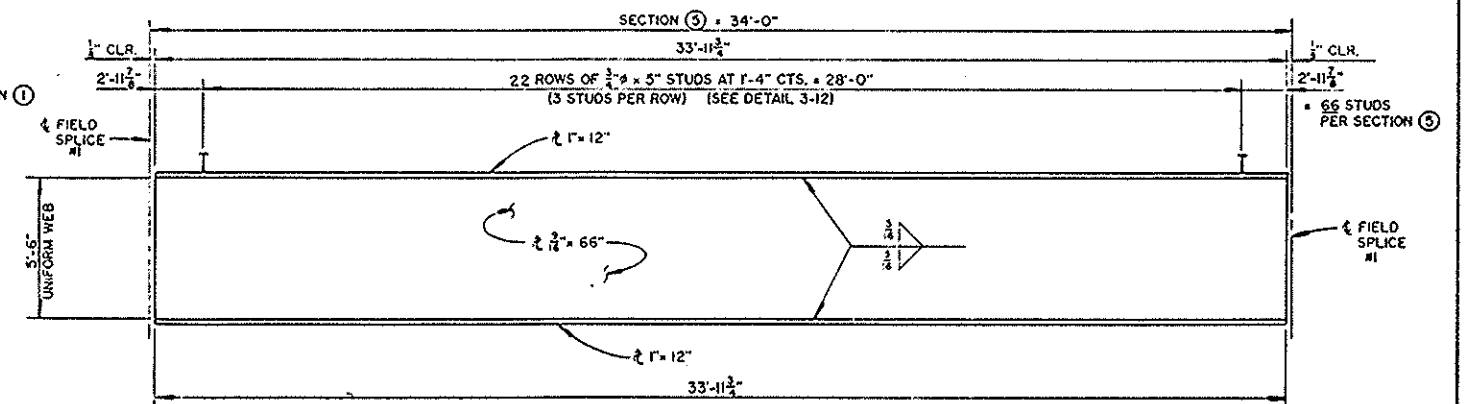
GIRDER SECTION 2 - I REQ'D.



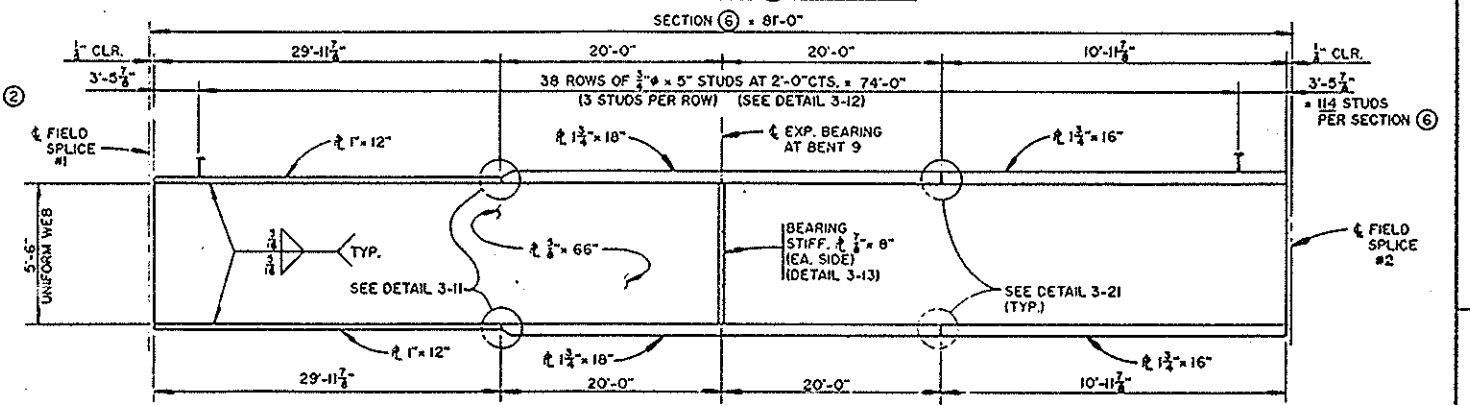
GIRDER SECTION 3 - I REQ'D.



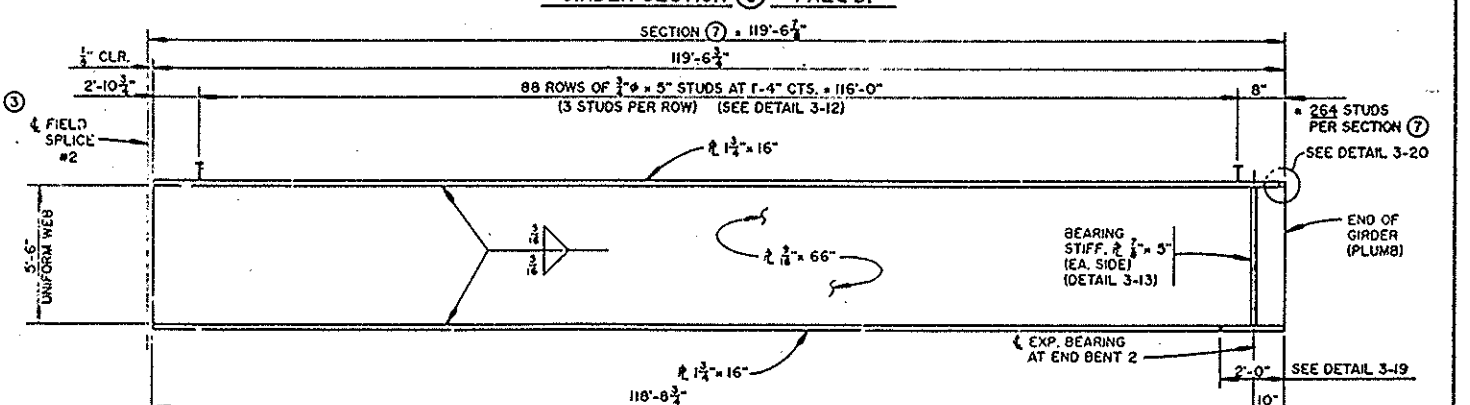
GIRDER SECTION 4 - I REQ'D.



GIRDER SECTION 5 - I REQ'D.



GIRDER SECTION 6 - I REQ'D.



GIRDER SECTION 7 - I REQ'D.

DRAWN BY: KEN
CHECKED BY: GEM
DATE: 10-24
DATE: 6/95



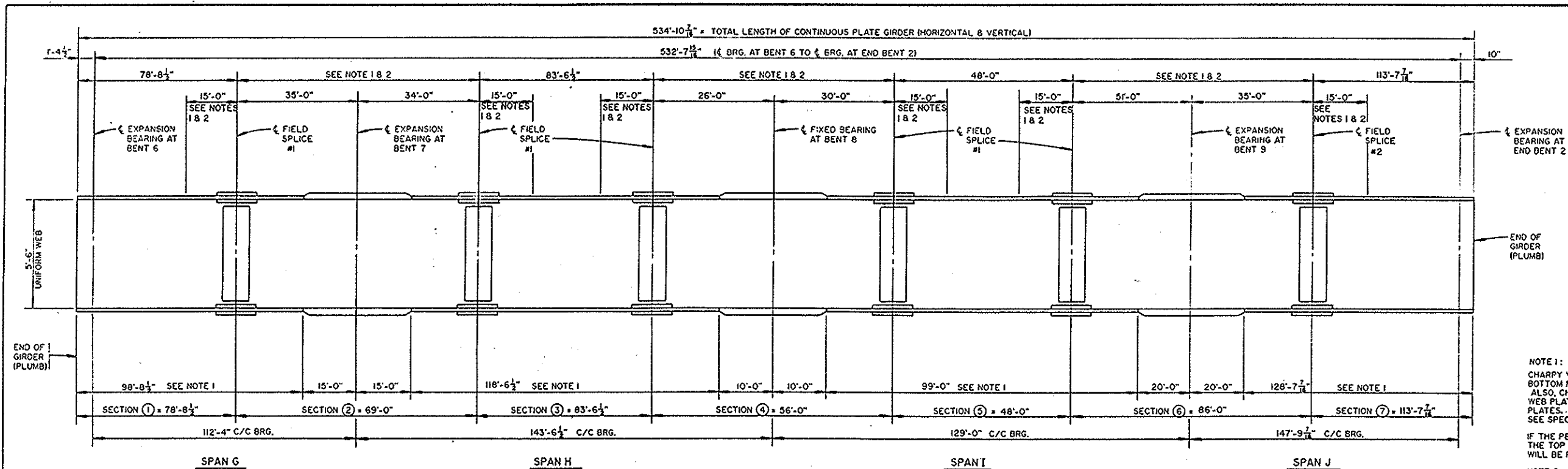
U3.020
PREPARED BY: THOMPSON - 900004 - SNC 14
ENGINEERS, INCORPORATED

PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV

SHEET 3 OF 12

STATE OF NORTH CAROLINA					
DEPARTMENT OF TRANSPORTATION					
RALEIGH					
SUPERSTRUCTURE					
STRUCTURAL STEEL					
GIRDER 2 DETAILS					
CONTINUOUS UNIT #3					
OCTOBER 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			5		
2			6		

SHEET NO. 5-119
TOTAL SHEETS 224

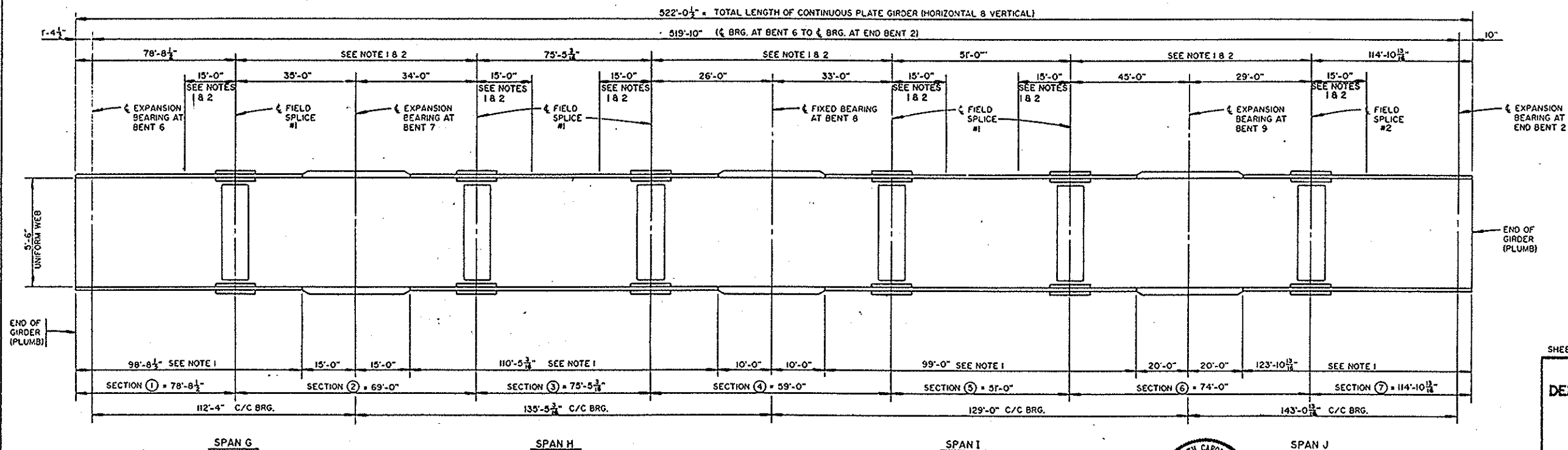


ELEVATION OF GIRDER 3

NOTE 1:
 CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.

IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.

NOTE 2:
 NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.



ELEVATION OF GIRDER 4

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV

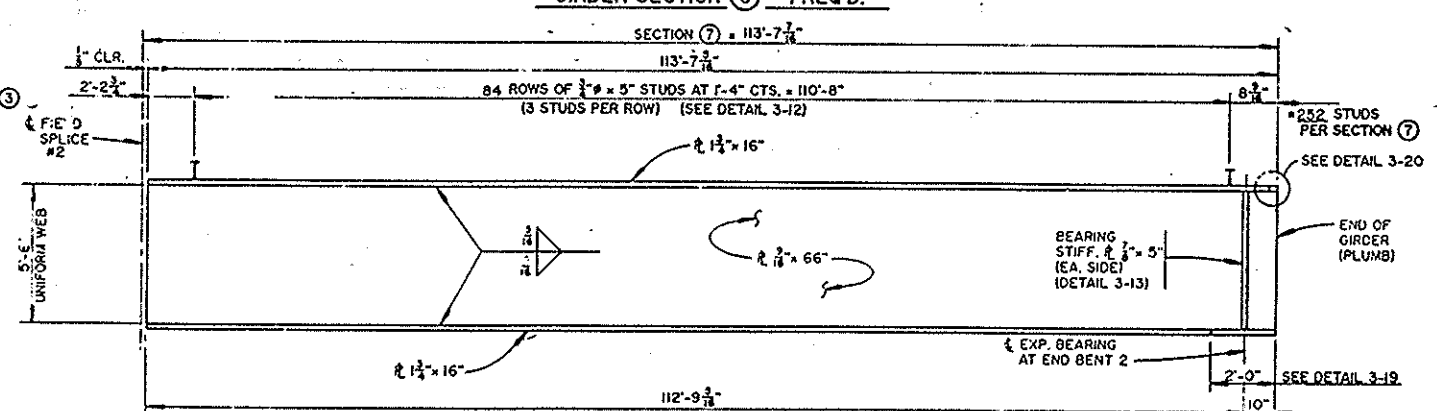
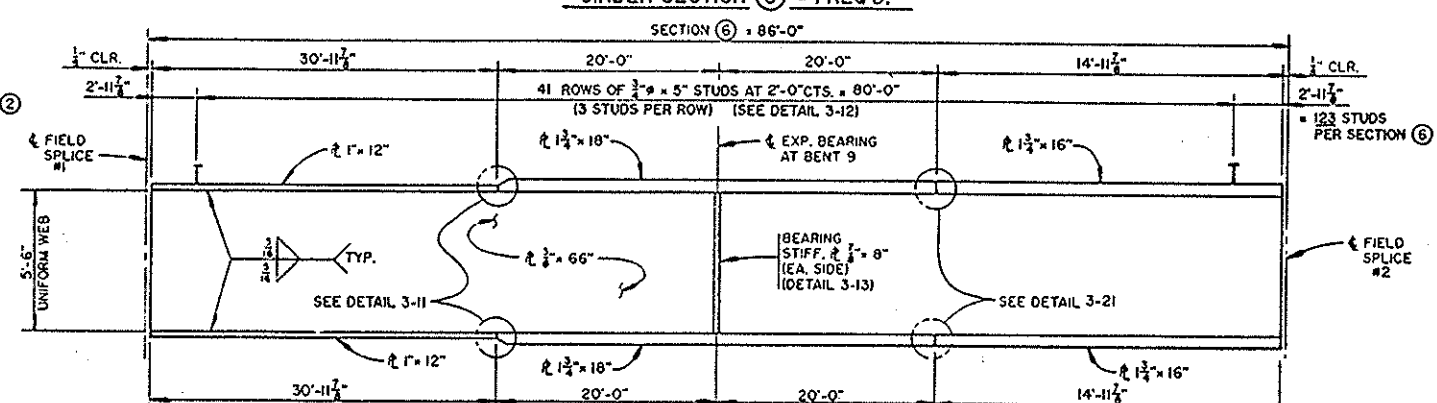
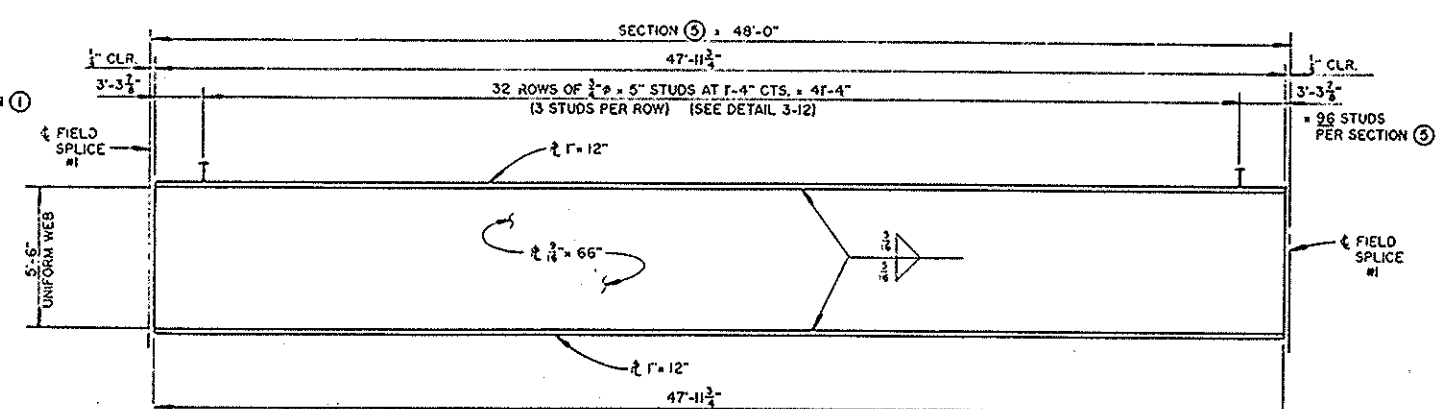
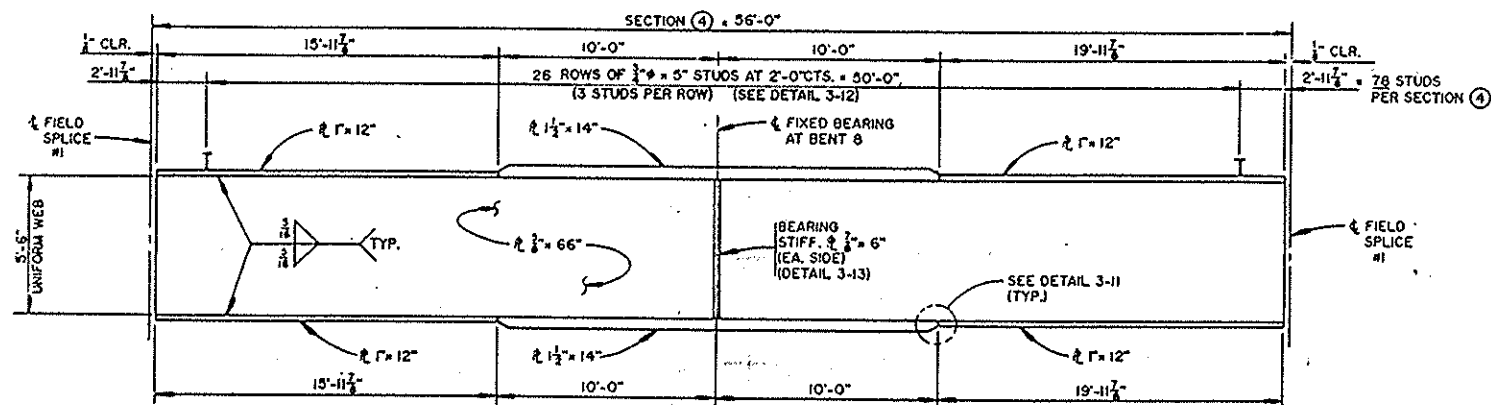
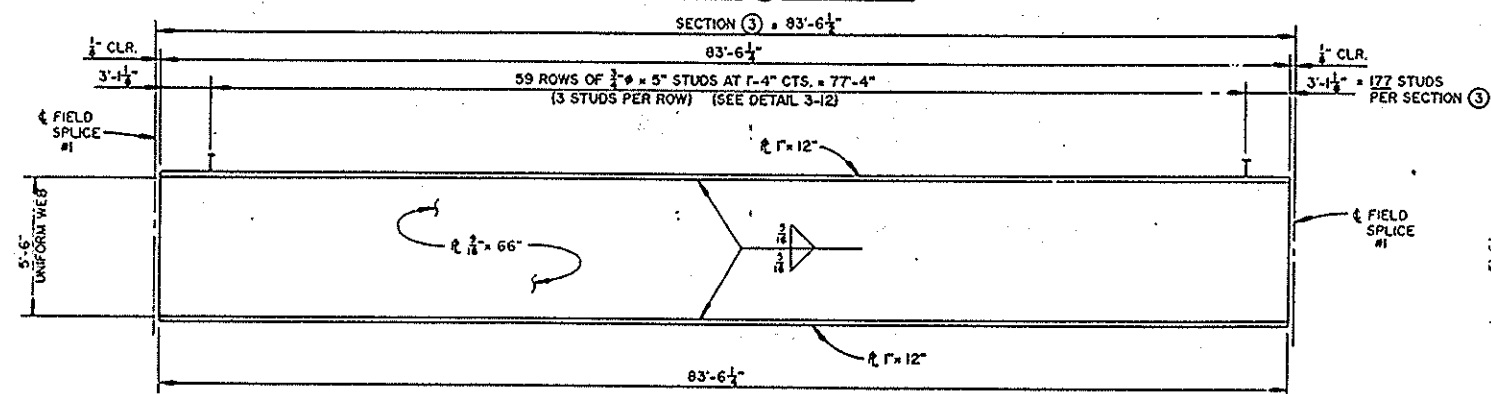
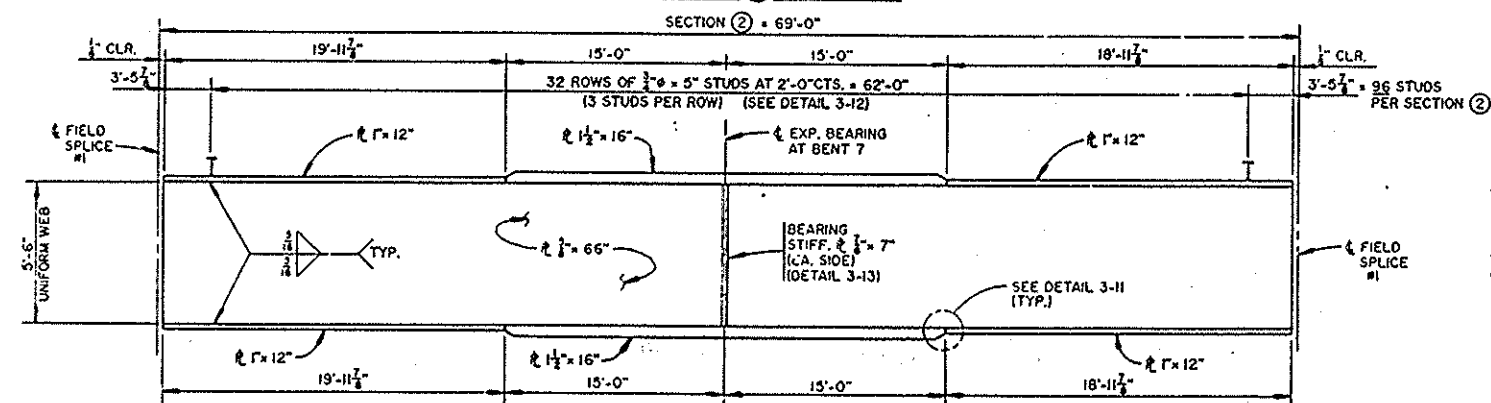
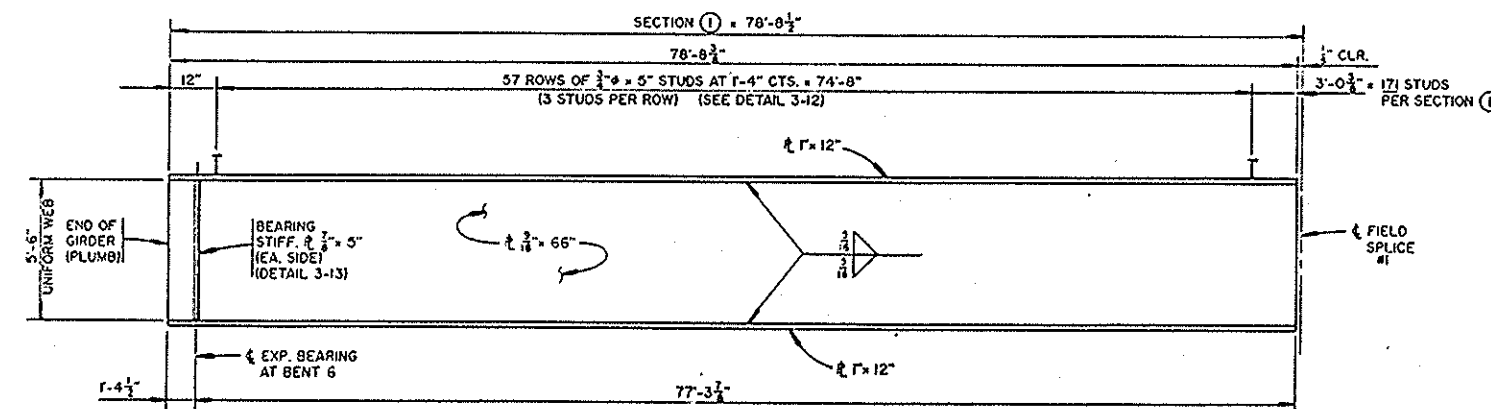
SHEET 4 OF 12

STATE OF NORTH CAROLINA					
DEPARTMENT OF TRANSPORTATION					
RALEIGH					
SUPERSTRUCTURE					
STRUCTURAL STEEL					
GIRDER ELEVATIONS 3 AND 4					
CONTINUOUS UNIT #3					
OCTOBER 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
3			4		
SHEET NO. 5-120					TOTAL SHEETS 274



U3.021
 PREPARED BY
 THOMPSON-BOROUGH-BROOK
 ENGINEERS, INCORPORATED

DRAWN BY: BTJ
 CHECKED BY: EEM
 DATE: 12/94
 DATE: 6/95



PROJECT No. B-1070
BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV

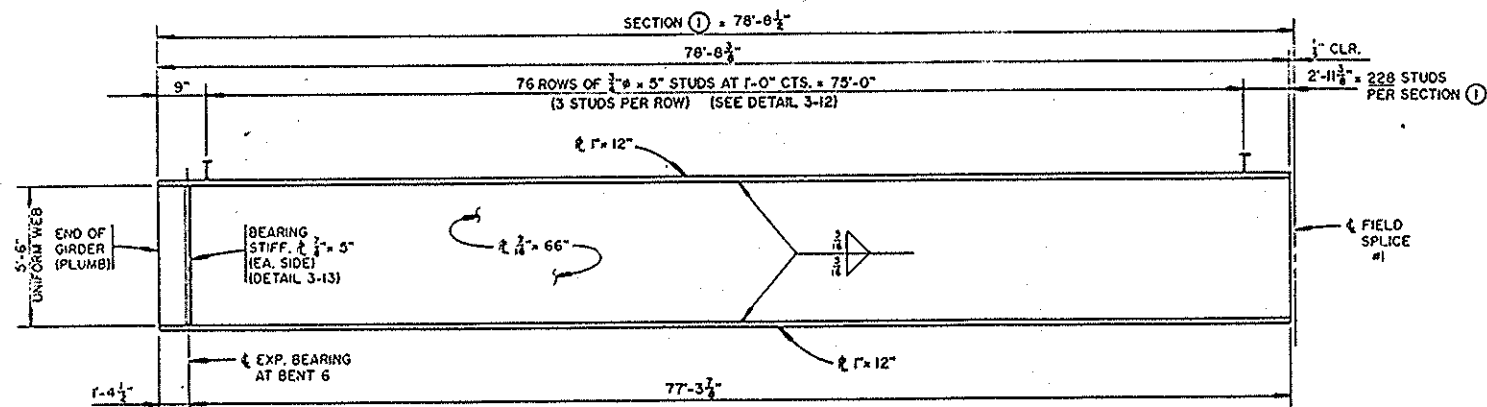
SHEET 3 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
HAZELHORN
SUPERSTRUCTURE
STRUCTURAL STEEL
GIRDER 3 DETAILS
CONTINUOUS UNIT #3

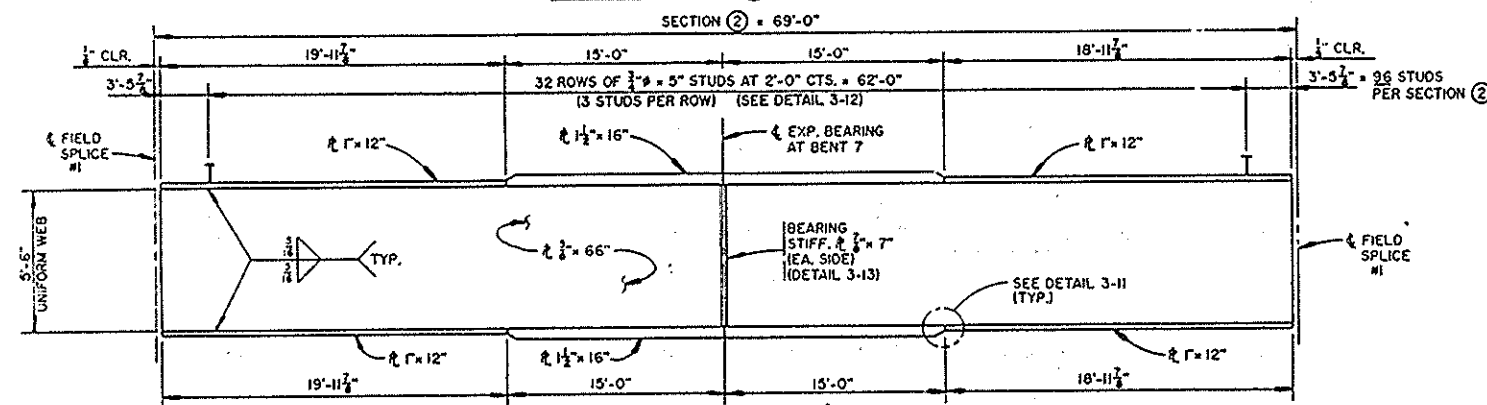
43.022

— PREPARED BY —
THOMPSON-GORDON-SHOOK
ENGINEERS, INCORPORATED

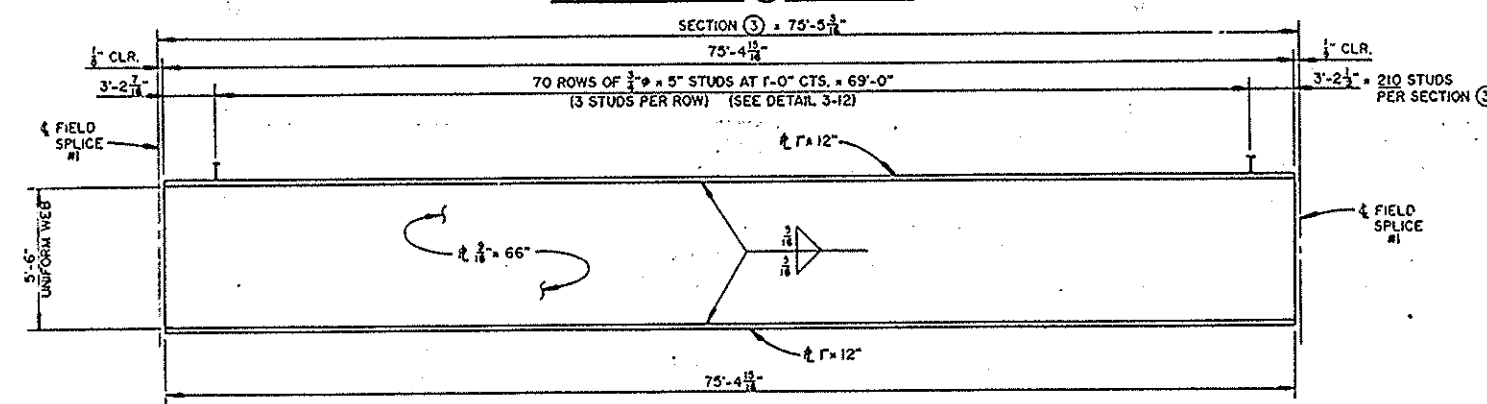
REVISIONS						SHEET NO.
NO.	BY	DATE	NO.	BY	DATE	
1			3			5-121
2			4			TOTAL SHEETS 224



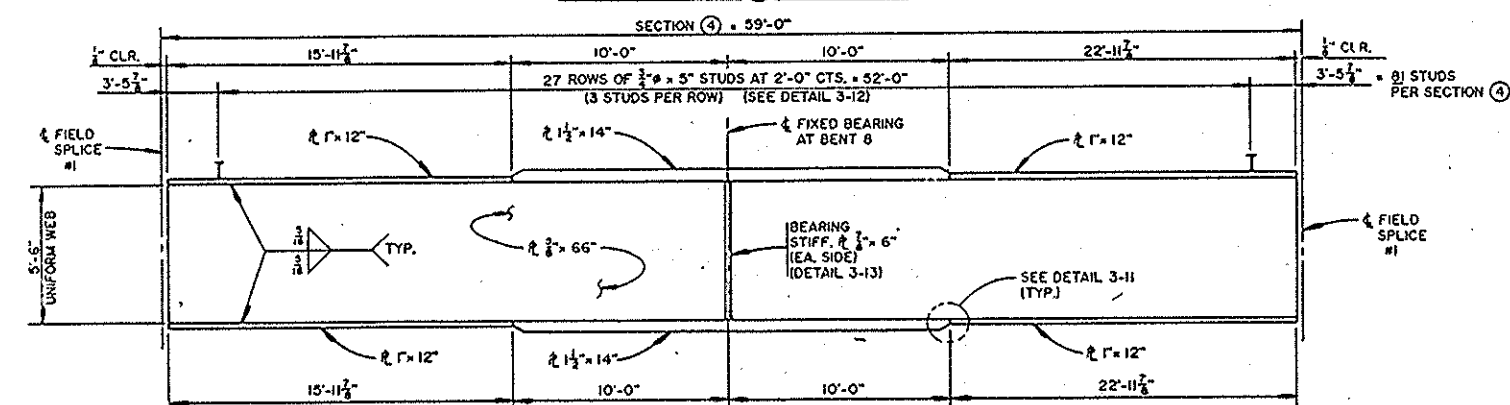
GIRDER SECTION 1 - I REQ'D.



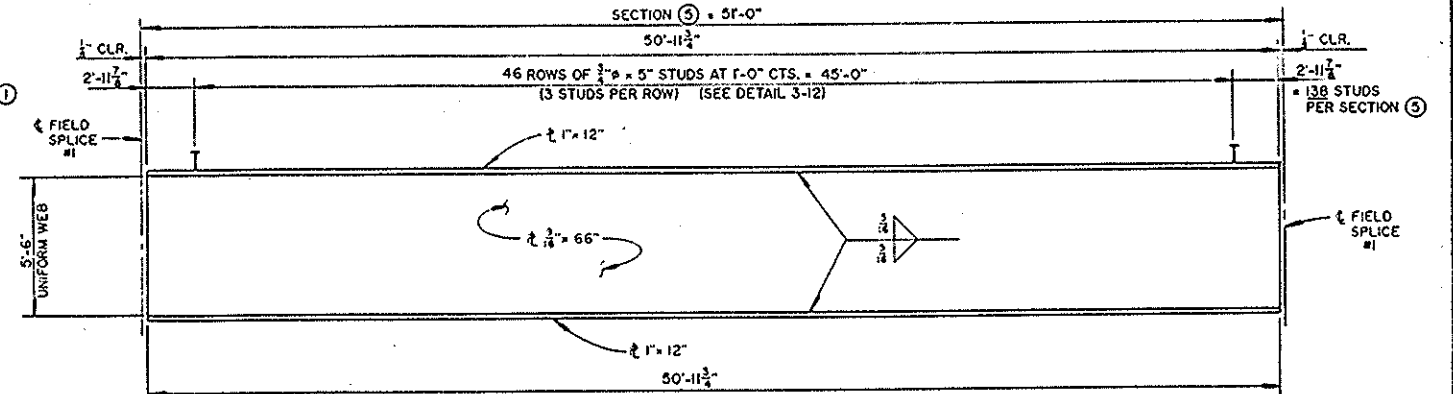
GIRDER SECTION 2 - I REQ'D.



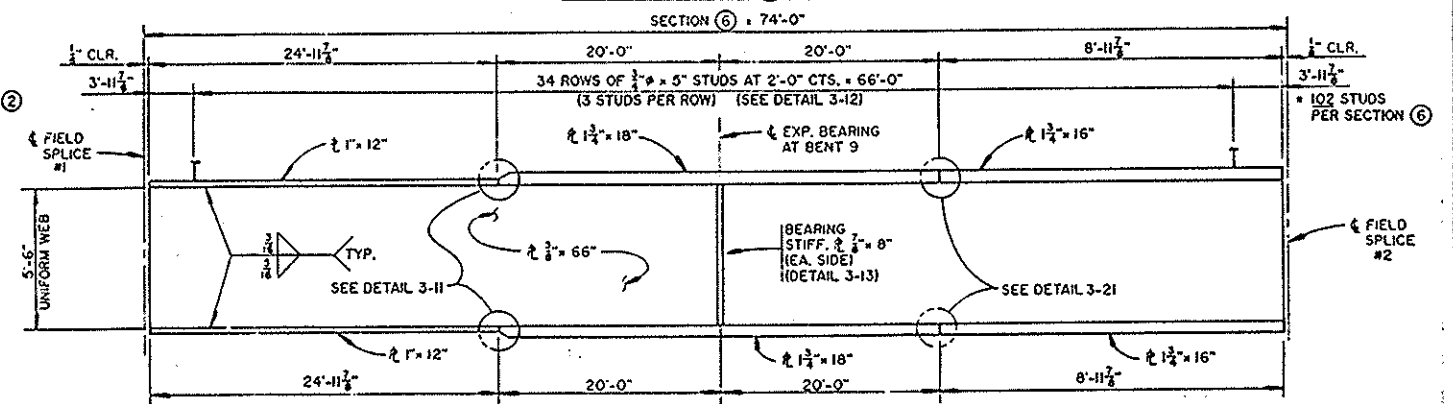
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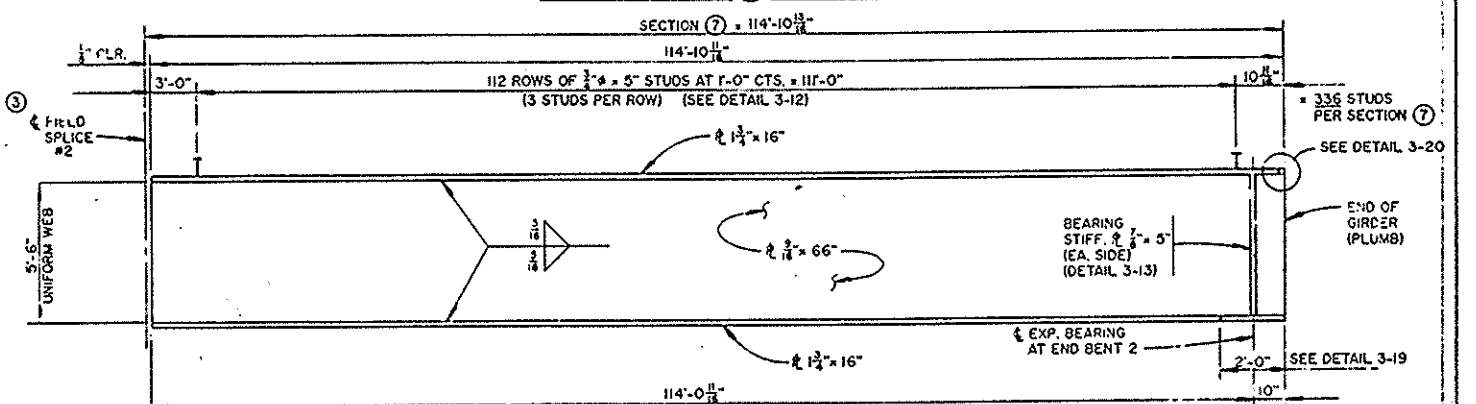
GIRDER SECTION 4 - I REQ'D.



GIRDER SECTION 5 - I REQ'D.



GIRDER SECTION 6 - I REQ'D.



GIRDER SECTION 7 - I REQ'D.

PROJECT NO. B-1070
BUNCOMBE COUNTY
STATION, P.O.T. 16-93.00-L-REV

SHEET 6 OF 12

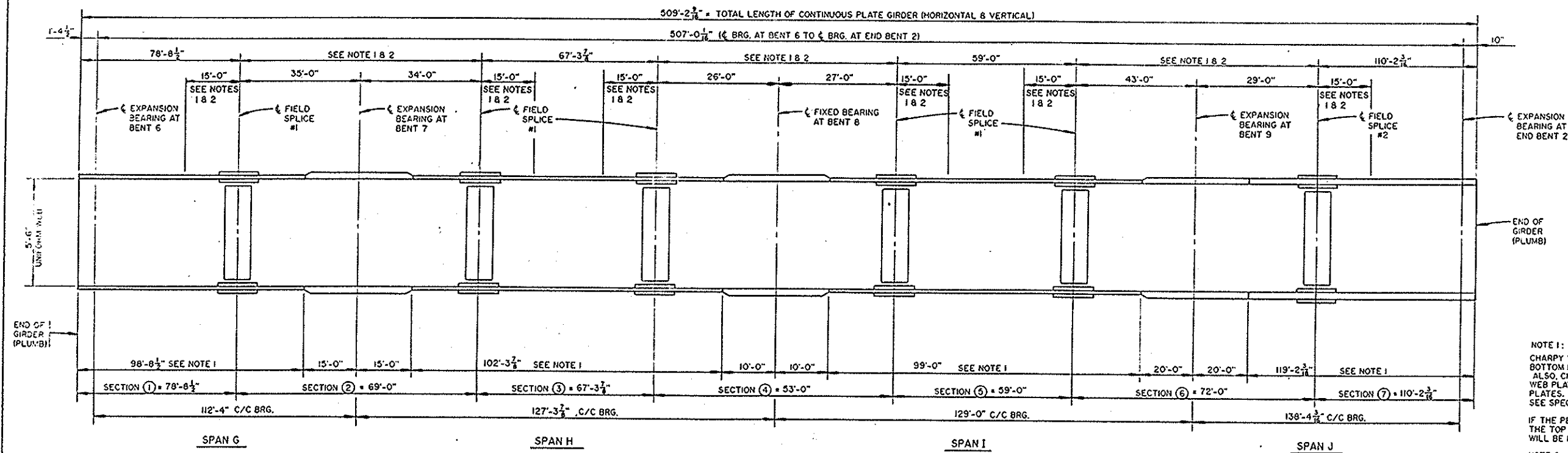


U3.023
PREPARED BY
THOMPSON - BROWN - SNOOK
ENGINEERS, INCORPORATED

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL GIRDER 4 DETAILS CONTINUOUS UNIT #3 OCTOBER 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
3			4		
					SHEET NO. 5-122
					TOTAL SHEETS 274

DRAWN BY: KTL
CHECKED BY: GMA
DATE: 10-24-94
DATE: 10-24-94

1198
2229

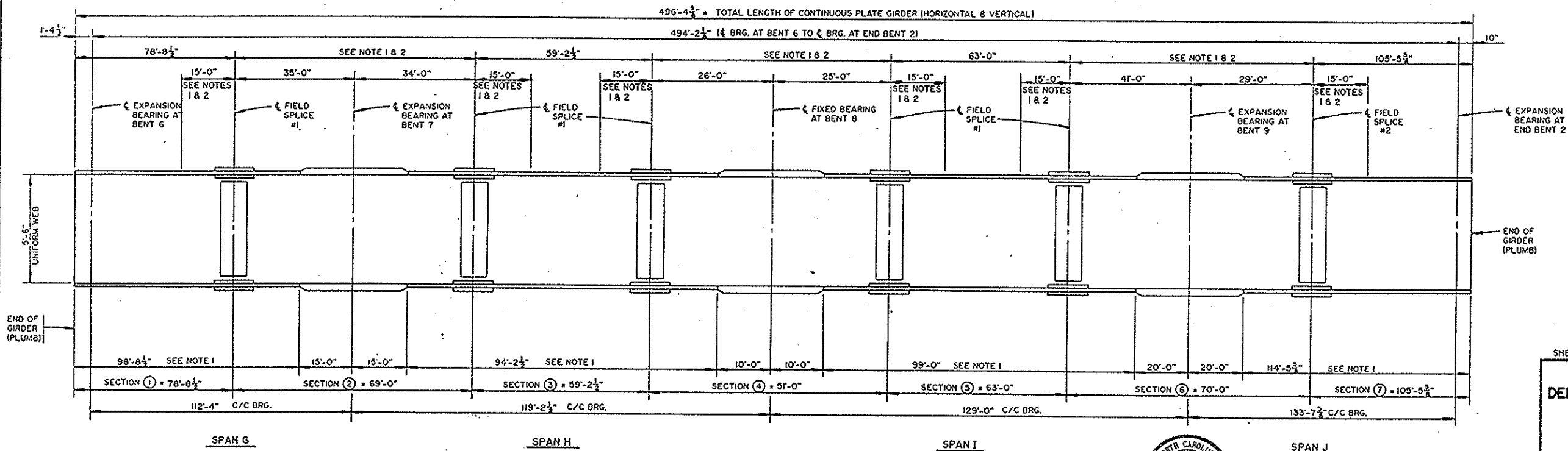


ELEVATION OF GIRDER 5

NOTE 1:
CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR TOP OR BOTTOM FLANGE PLATE WHICH FALLS WITHIN THESE LIMITS. ALSO, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR ALL WEB PLATES, WEB SPLICE PLATES AND FLANGE SPLICE PLATES. FOR CHARPY V-NOTCH TESTS, SEE SPECIAL PROVISIONS.

IF THE PERMITTED SHOP FLANGE SPLICE, AS INDICATED FOR THE TOP FLANGE, IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE TOP FLANGE PLATE.

NOTE 2:
NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.



ELEVATION OF GIRDER 6

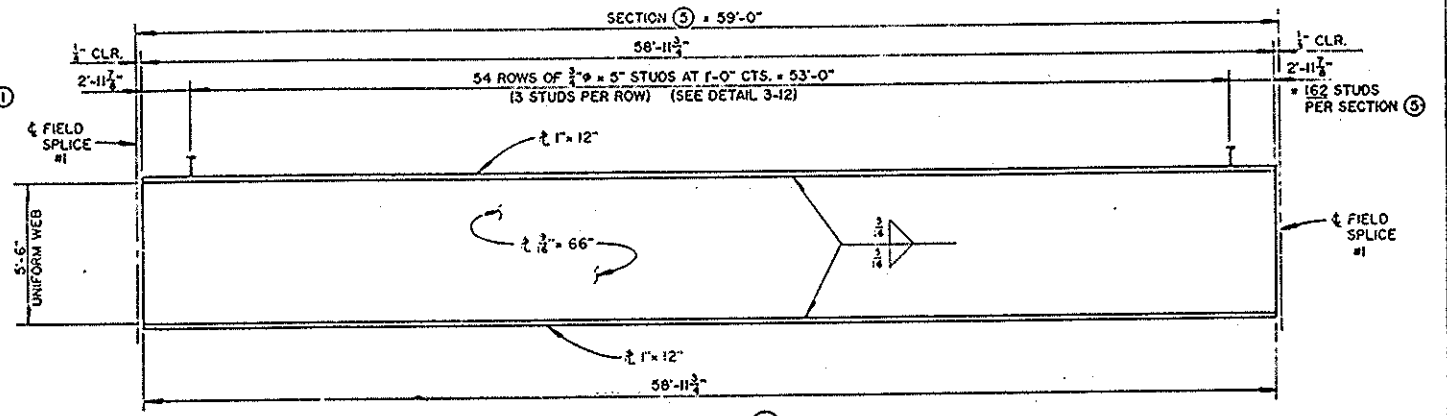
PROJECT No. B-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L-REV
SHEET 7 OF 12

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE STRUCTURAL STEEL GIRDER ELEVATIONS 5 AND 6 CONTINUOUS UNIT #3 OCTOBER 1994					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		
SHEET NO. 5-123 TOTAL SHEETS 224					

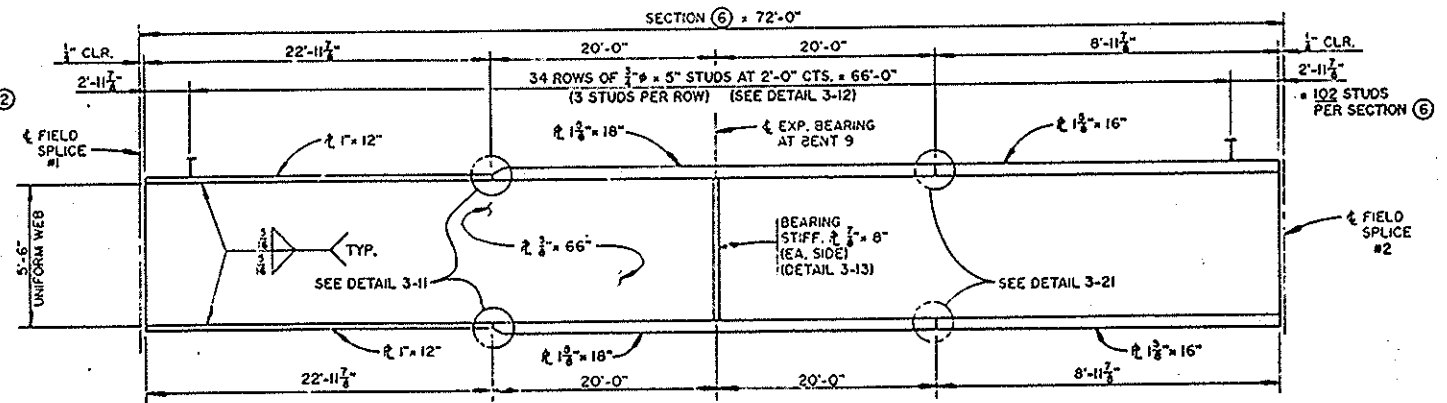


U3.024
PREPARED BY
THOMPSON - BORDEN - BHOOK
ENGINEERS, INCORPORATED

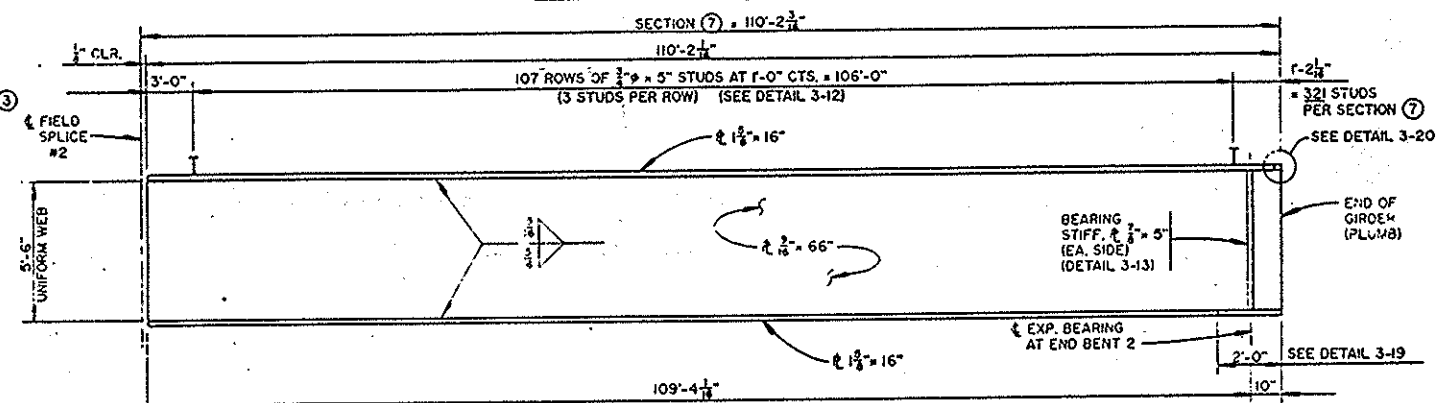
DRAWN BY: RTJ
CHECKED BY: GCM
DATE: 10/94
DATE: 10/94



GIRDER SECTION (5) - 1 REQ'D.



GIRDER SECTION (6) - 1 REQ'D.



GIRDER SECTION (7) - 1 REQ'D.

PROJECT NO. 8-1070
BUNCOMBE COUNTY
STATION: P.O.T. 16+93.00-L- Rev.

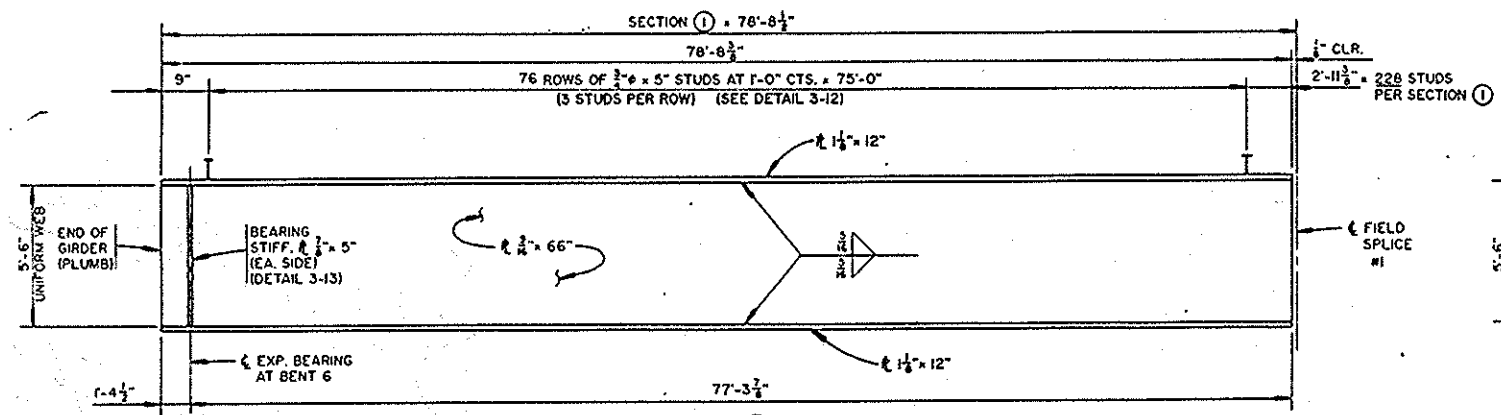


43.025

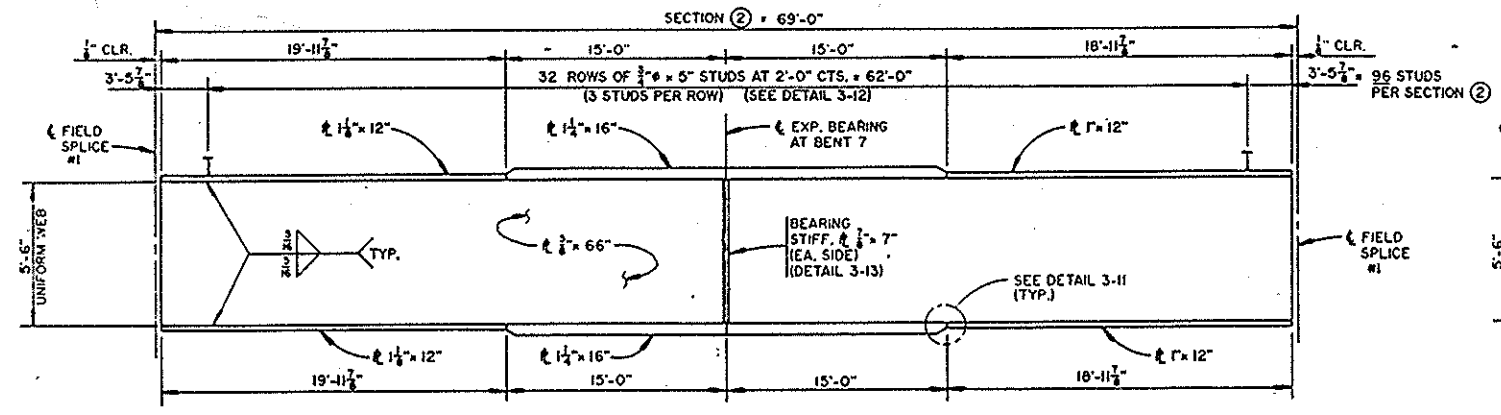
— PREPARED BY —
THOMPSON - BOROON - SHOOT
ENGINEERS, INCORPORATE

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
 GIRDER 5 DETAILS
 CONTINUOUS UNIT #3
 OCTOBER 1994

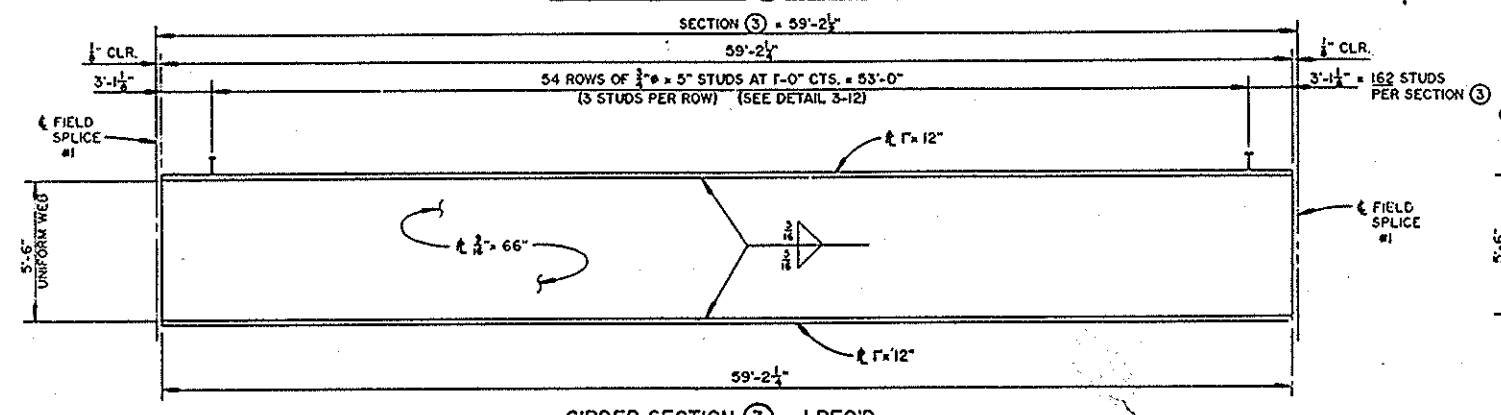
REVISIONS							SHEET NO. 5-124
NO.	BY	DATE	NO.	BY	DATE		TOTAL SHEETS 776
1			5				
			4				



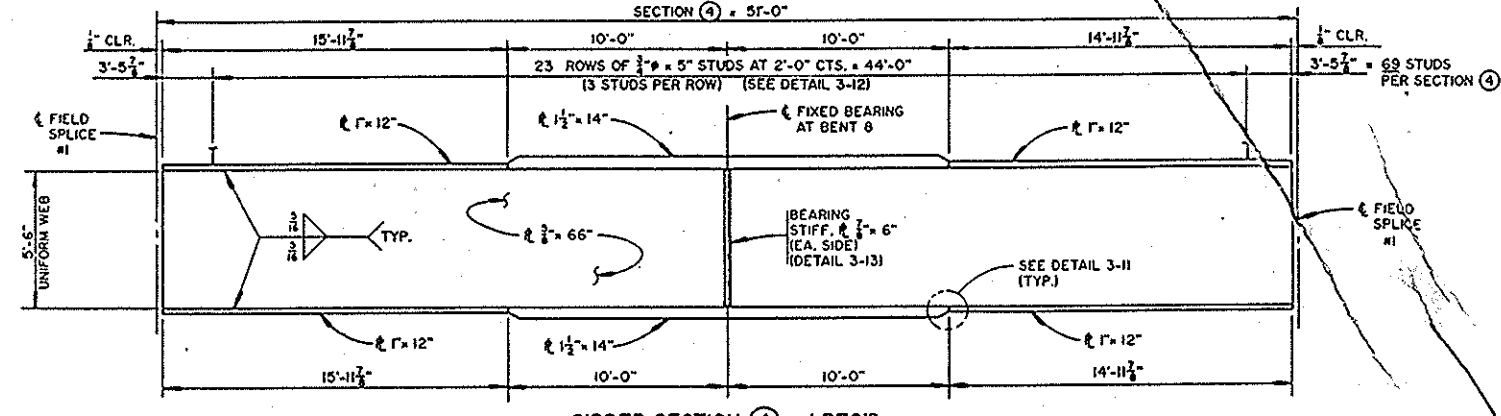
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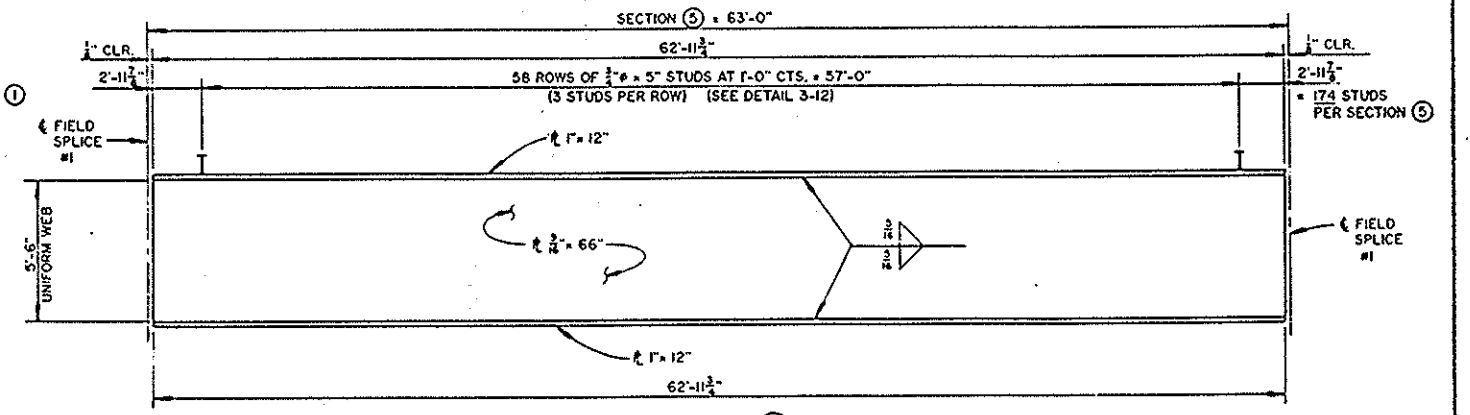
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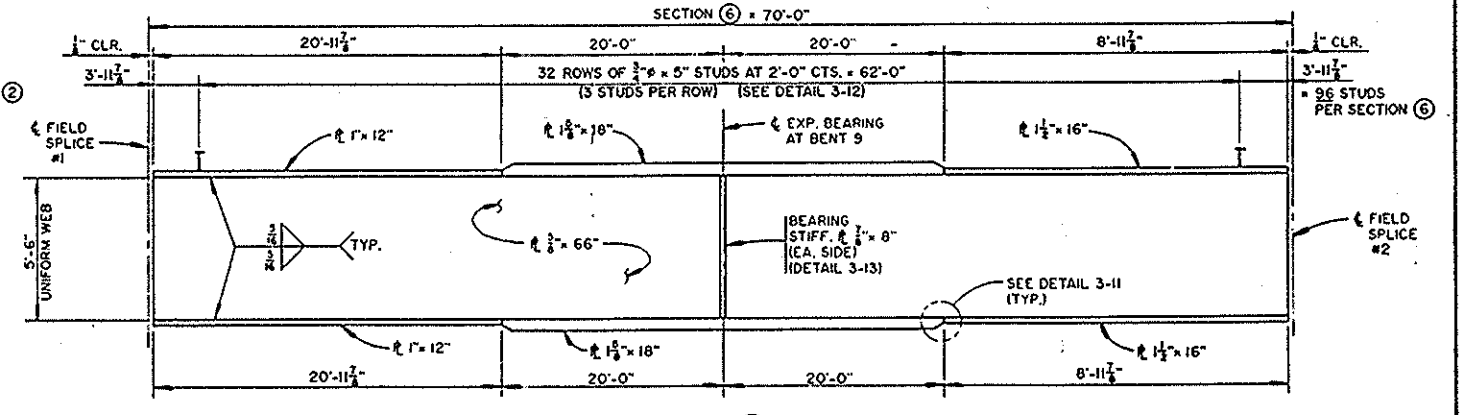
GIRDER SECTION ③ - I REQ'D.



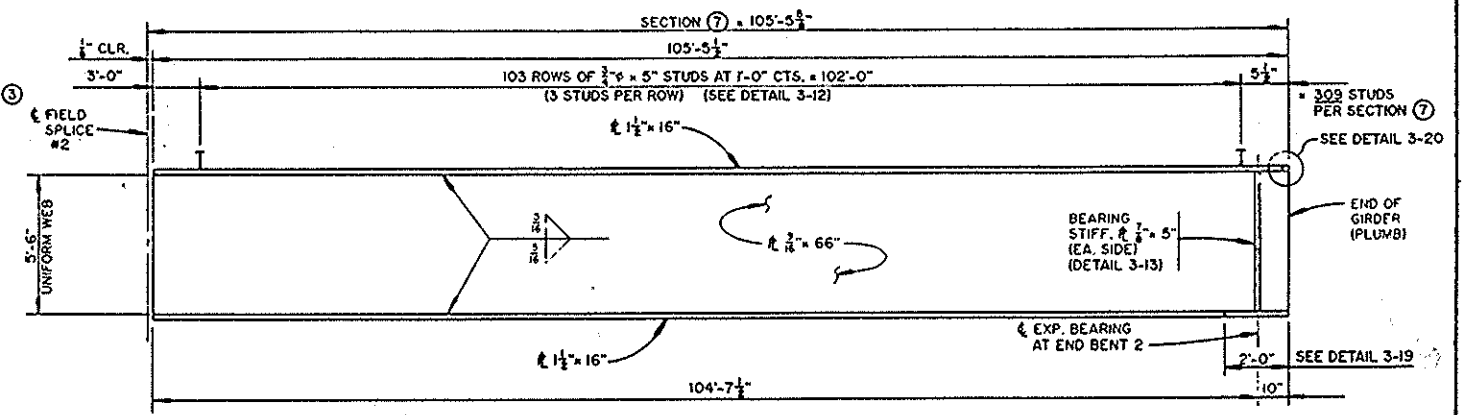
GIRDER SECTION ④ - I REQ'D.



GIRDER SECTION ⑤ - I REQ'D.



GIRDER SECTION ⑥ - I REQ'D.



GIRDER SECTION ⑦ - I REQ'D.

DRAWN BY: SEM DATE: 10-23
 CHECKED BY: SEM DATE: 6/95



U3.026
 PREPARED BY
 THOMPSON-GORDON-SHOOK
 ENGINEERS, INCORPORATED

PROJECT No. B-1070
 BUNCOMBE COUNTY
 STATION: P.O.T. 16+93.00-L-REV
 SHEET 9 OF 12

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 STRUCTURAL STEEL
 GIRDER 6 DETAILS
 CONTINUOUS UNIT #3
 OCTOBER 1994

REVISIONS						SHEET NO. 5-125
NO.	BY	DATE	NO.	BY	DATE	
1			2			TOTAL SHEETS 224
3			4			