

REFERENCE: R-5963A

PROJECT: 48599

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

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-Y4-	10+00 - 10+73	8
-Y5-	11+25 - 12+75	8
-Y6-	10+00 - 10+73	9
-Y7-	13+25 - 20+30	9
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-Y7B-	10+00 - 10+70	10
-Y8-	11+25 - 12+75	10
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY CHATHAM

PROJECT DESCRIPTION NEW ROUTE FROM CHATHAM
PARKWAY, US 15/US 501 SOUTH OF PITTSBORO
TO US 64 BUSINESS, CONSTRUCT ROADWAY
ON NEW LOCATION

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-5963A	1	

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11/25/2024

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SIGNATURE

DATE

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP)										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOOGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATERIAL.									
SOIL LEGEND AND AASHTO CLASSIFICATION										MINERALOGICAL COMPOSITION										WEATHERING																			
GENERAL CLASS.										MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.										FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.																			
GROUP CLASS.										COMPRESSIBILITY										VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.																			
SYMBOL										PERCENTAGE OF MATERIAL										SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.																			
Z PASSING #10 #40 #200										GROUND WATER										MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.																			
MATERIAL PASSING #40 LL PI										MISCELLANEOUS SYMBOLS										MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i>																			
GROUP INDEX										RECOMMENDATION SYMBOLS										SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF</i>																			
USUAL TYPES OF MAJOR MATERIALS										ABBREVIATIONS										VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i>																			
GEN. RATING AS SUBGRADE										EQUIPMENT USED ON SUBJECT PROJECT										COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.																			
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30										DRILL UNITS:										ROCK HARDNESS																			
CONSISTENCY OR DENSENESS										ADVANCING TOOLS:										VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.																			
PRIMARY SOIL TYPE										HAMMER TYPE:										HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.																			
COMPACTNESS OR CONSISTENCY										CORE SIZE:										MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.																			
RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)										HAND TOOLS:										MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.																			
RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)										INDURATION										SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.																			
GENERALY GRANULAR MATERIAL (NON-COHESIVE)										FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.										VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																			
GENERALY SILT-CLAY MATERIAL (COHESIVE)										FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.																													
TEXTURE OR GRAIN SIZE										MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.																													
U.S. STD. SIEVE SIZE OPENING (MM)										INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.																													
BOULDER (BLDR.)										EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.																													
COBBLE (COB.)																																							
GRAVEL (GR.)																																							
COARSE SAND (CSE. SD.)																																							
FINE SAND (F SD.)																																							
SILT (SL.)																																							
CLAY (CL.)																																							
GRAIN SIZE																																							
SOIL MOISTURE - CORRELATION OF TERMS																																							
SOIL MOISTURE SCALE (ATTERBERG LIMITS)																																							
FIELD MOISTURE DESCRIPTION																																							
GUIDE FOR FIELD MOISTURE DESCRIPTION																																							
LL - LIQUID LIMIT																																							
PLASTIC RANGE (PI)																																							
PL - PLASTIC LIMIT																																							
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT																																							
SL - SHRINKAGE LIMIT																																							
PLASTICITY																																							
NON PLASTIC																																							
SLIGHTLY PLASTIC																																							
MODERATELY PLASTIC																																							
HIGHLY PLASTIC																																							
COLOR																																							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																																							

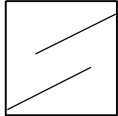
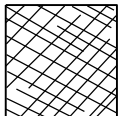
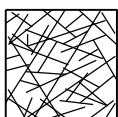
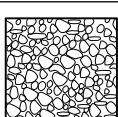
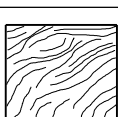
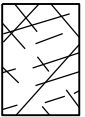
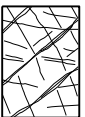
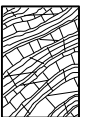
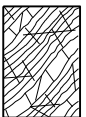
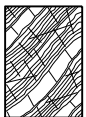
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000) From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS VERY GOOD Very rough, fresh unweathered surfaces GOOD Rough, slightly weathered, iron stained surfaces FAIR Smooth, moderately weathered and altered surfaces POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	STRUCTURE INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	DECREASING SURFACE QUALITY ➡ 90 80 70 60 50 40 30 20 10 N/A N/A	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000) From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.	SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes) VERY GOOD - Very Rough, fresh unweathered surfaces GOOD - Rough, slightly weathered surfaces FAIR - Smooth, moderately weathered and altered surfaces POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings	COMPOSITION AND STRUCTURE A. Thick bedded, very blocky sandstone. The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability. B. Sandstone with thin inter-layers of siltstone C. Sandstone and siltstone in similar amounts D. Siltstone or silty shale with sandstone layers E. Weak siltstone or clayey shale with sandstone layers F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces. ➡ Means deformation after tectonic disturbance	70 60 50 40 30 20 10
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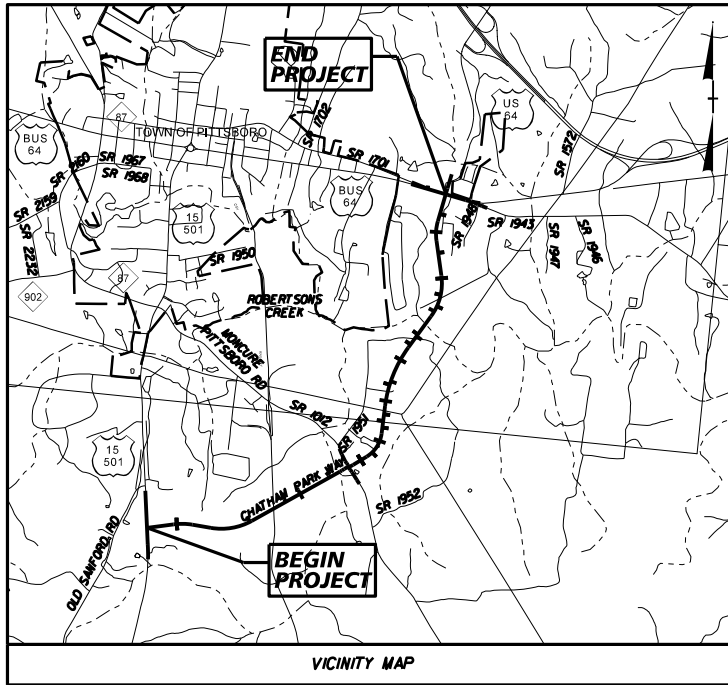
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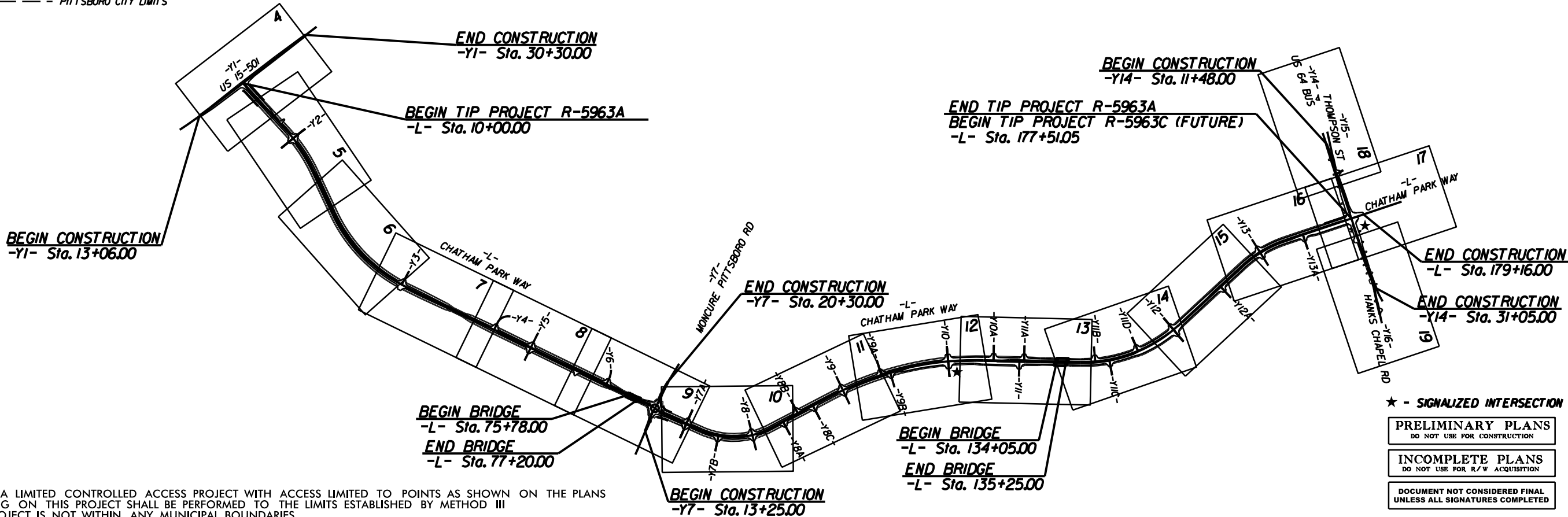
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CONTRACT:

TIP PROJECT: R-5963A

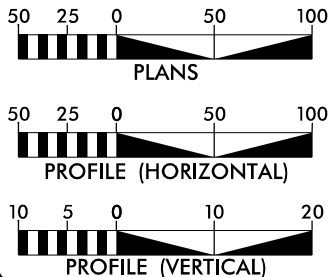


RW PLANS



THIS IS A LIMITED CONTROLLED ACCESS PROJECT WITH ACCESS LIMITED TO POINTS AS SHOWN ON THE PLANS
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III
THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES

GRAPHIC SCALES



R-5963A DESIGN DATA

ADT 2025 = 0
ADT 2045 = 31000
K = 7%
D = 55
T = 5%*
V = 50 MPH
* (TTST 2% + DUAL 3%)
FUNCTIONAL CLASSIFICATION:
RURAL
MAJOR COLLECTOR

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-5963A = 3.114 MILES
LENGTH STRUCTURE TIP PROJECT R-5963A = 0.059 MILES
TOTAL LENGTH TIP PROJECT R-5963A = 3.173 MILES

PLANS PREPARED FOR
THE NCDOT BY:

2024 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JULY 19, 2024

LETTING DATE:
MARCH 18, 2025

Kimley»Horn

VANCE W. BLANTON, P.E.
PROJECT ENGINEER

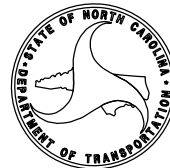
TYLER G. SPRING, P.E.
PROJECT DESIGN ENGINEER

JEFFREY A. STRODER, P.E.
PROJECT MANAGER
NCDOT HIGHWAY DIVISION 8

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.
ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

CHATHAM COUNTY

LOCATION: NEW ROUTE, FROM CHATHAM PARKWAY, US 15/US 501 SOUTH OF PITTSBORO
TO US 64 BUSINESS, CONSTRUCT ROADWAY ON NEW LOCATON

TYPE OF WORK: GRADING, DRAINAGE, BRIDGES, PAVING, SIGNALS, AND RETAINING WALLS

NC GRID
NAD 1983/NA 2011

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-5963A	3	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
48599.1.1		P.E.	

11/15/2024

STATE PROJECT:48599.1.1

TIP NO.:R-5963A

COUNTY:Chatham

DESCRIPTION:New Route from Chatham Parkway, US 15/US 501 South of Pittsboro to US 64 Business, Construct Roadway on New Location

SUBJECT:Geotechnical Roadway Inventory Report

PROJECT DESCRIPTION

The project consists of the proposed approximately 4.0-mile roadway which will extend from US 15-501 at a new intersection approximately 1,300 feet south of the intersection of US 15-501 and Old Sanford Road and extend northeast to tie into the intersection of US 64 Business and Chatham Park Way. The proposed roadway will also include a new roundabout intersection with Moncure Pittsboro Road.

The project will include improvements along US 15-501 (-Y1- alignment), Moncure Pittsboro Road (-Y7- alignment), and US 64 Business (-Y14- alignment) along with improvements to ancillary side roads Hanks Chapel Road (-Y16- alignment) and Thompson Road (-Y15- alignment) where they tie into US 64 Business. Additionally, a new single-span set of dual bridges over Robeson Creek, a new two-span bridge over an Unnamed Tributary to Robeson Creek, and six (6) retaining walls are proposed.

The proposed Chatham Park Way (-L- alignment) primarily traverses undeveloped farmers’ fields that appear to be in operation. Preliminary plans indicate maximum cut and fill depths up to approximately 40 feet and 20 feet, respectively, for the project with a majority of the mass grading along the new -L- alignment.

The following alignments are included as part of this project:

Alignment	Station
-L- (Chatham Park Way)	10+00 to 177+51
-RBT-	10+00 to 13+45
-Y1- (US 15-501)	13+06 to 30+30
-Y2-	11+25 to 12+75
-Y3-	11+25 to 12+75
-Y4-	10+00 to 10+73
-Y5-	11+25 to 12+75
-Y6-	10+00 to 10+73
-Y7- (Moncure Pittsboro Road)	13+25 to 20+30
-Y7A-	10+00 to 10+73
-Y7B-	10+00 to 10+70
-Y8-	11+25 to 12+75

Alignment	Station
-Y8A-	10+00 to 10+74
-Y8B-	11+26 to 12+00
-Y8C-	10+00 to 10+74
-Y9-	10+00 to 10+73
-Y9A-	11+25 to 12+00
-Y9B-	10+00 to 10+75
-Y10-	11+25 to 12+75
-Y10A-	11+25 to 12+00
-Y11-	10+00 to 11+01
-Y11A-	11+12 to 12+00
-Y11B-	11+12 to 12+00
-Y11C-	10+00 to 10+84
-Y11D-	11+12 to 12+00
-Y12-	11+12 to 12+96
-Y12A-	10+00 to 10+85
-Y13-	10+16 to 12+86
-Y13A-	10+00 to 10+86
-Y14- (US 64 Business)	11+48 to 31+05
-Y15- (Thompson Street)	13+40 to 15+00
-Y16- (Hanks Chapel Road)	10+00 to 14+37
-DRW1-	10+00 to 10+56

Bridges were investigated at the following approximate locations:

Structure Location	Station	Structure Type
-L- (Chatham Park Way)	75+78 to 77+20	Two Spans
-L- (Chatham Park Way) NB	134+08 to 135+30	Single Span
-L- (Chatham Park Way) SB	134+00 to 135+22	Single Span

A total of six (6) Retaining Walls were investigated at the following approximate stations:

Wall ID	Alignment	Station
Wall No. 1 (-W1-)	-L-	10+62 LT to 13+50 LT
Wall No. 2 (-W2-)	-L-	10+62 RT to 13+50 RT
Wall No. 3 (-W3-)	-L-	80+39 LT to 81+87 LT
Wall No. 4 (-W4-)	-L-	80+31 RT to 81+87 RT
Wall No. 5 (-W5-)	-L-	174+00 LT to 176+98 LT
Wall No. 6 (-W6-)	-L-	174+00 RT to 176+69 LT

The geotechnical field investigation was conducted by CG2 during the period from July 2024 to September 2024. A truck-mounted Mobile B-29, an ATV-mounted CME 550X, and a Diedrich D50 track rig, each equipped with an automatic hammer, were used to advance a total of 171 soil test borings and rock soundings during this investigation. Standard Penetration Tests (SPT) were performed at selected depths

within each SPT boring. Representative soil samples were collected for visual-manual classification in the field and evaluated in the office by a staff professional under the supervision of a licensed engineer or geologist. Selected soil samples were submitted for laboratory analysis by an approved NCDOT M&T testing facility.

PHYSIOGRAPHY AND GEOLOGY

The project corridor is located within the Piedmont Physiographic Province of North Carolina. The Piedmont Physiographic Province generally consists of hills and ridges which are intertwined with an established system of draws, streams, and valleys. According to the 1985 Geologic Map of North Carolina, the bedrock under the site consists of felsic to intermediate metavolcanic rock. Crystalline rock and weathered rock encountered during this investigation consisted of Metavolcanic Rock.

Within the project alignment, much of the bedrock is overlain by near-surface material consisting of roadway embankment, artificial fill, alluvial, and residual soils. Residual soils are derived from in situ chemical and physical weathering of the rock in the area and vary in thickness. The residual soils in this region are typically finer grained with a higher clay content near the surface due to advanced weathering, and typically become coarser grained with increasing depth as the degree of weathering decreases. As the degree of weathering decreases, the residual soils generally retain the overall appearance and fabric of the parent rock (sometimes referred to as “saprolite”). The boundary between soil and rock is not always sharply defined. A transitional zone termed “weathered rock” is often found overlying the parent bedrock. Weathered rock is defined as material requiring 100 blows with less than one foot of penetration from the SPT hammer.

Alluvial soils are transported and deposited by water and are naturally variable in character, consistency/density, and often contain organic materials. Alluvial soil deposits of varying age were observed within the project alignment in low lying areas adjacent to Robeson Creek and were encountered within borings performed for the investigation. These alluvial materials contain variable amounts of rounded gravels, cobbles, and boulders, typical of stream terrace deposits.

In general, maximum existing grades occur near -L- Station 12+50 and grades generally decrease heading up-station. Along -L- at Stations 35+50, 59+00, 85+00, 115+50, 154+00 and 177+50, there are noted high points along the existing alignment. Generally, the site drains to lower elevation areas near -L- Stations 52+00, 76+50, 90+00, and 134+50. Generally, the existing topography in the cross-slope direction is relatively flat along the existing alignment, with some topographic features draining in both left and right directions.

Soil Properties

A generalized description of the soil and rock encountered is included below. Soils and rock encountered during the roadway investigation include roadway embankment, artificial fill, alluvial soils, residual soils, weathered rock, and crystalline rock.

Roadway Embankment soils are similar in nature to residual soils and may be derived from nearby sources. Roadway Embankment soils were encountered along the project corridor in areas where the existing state-maintained roadways are present. This material generally consists of soft to very stiff silty/sandy clays and

medium dense silty sands with trace amounts of gravel. Laboratory testing of the roadway embankment soils indicated PI's ranging from 32 to 42 for the clayey (A-6, A-7-6) soils.

Artificial Fill soils are materials that have been moved and/or placed by man or mechanical means. Artificial fill soils were encountered along the project corridor within the residential property and farmland along the proposed corridor and adjacent to the existing state-maintained roadways. The artificial fill soils generally consist of medium stiff to stiff silty/sandy clays, sandy silty, and loose clayey sands, with trace amounts of organics, gravel, asphalt, and mica. Laboratory testing of the artificial fill soils indicated a PI of 12 for the sandy (A-2-6) soils and PI's ranging from 15 to 29 for the clayey (A-6, A-7-5, A-7-6) soils.

Alluvial soils were observed in proximity to Robeson Creek and the various drainage features. Alluvial soils were encountered in Borings L_13465_EB1-A, L_13465_EB1-B, L_13465_EB1-C, L_7675_B1-A, L_7675_B1-B, and L_7675_B1-C. The alluvial soils generally consist of very soft to stiff sandy/silty clays, sandy silts, and loose to medium dense silty sands, and sands. Variable amounts of gravel and organics were encountered within the alluvial soils. The silty alluvial soils (A-4) encountered had a PI test result of 8.

Residual soils were encountered across the site at the surface or underlying the artificial fill, roadway embankment, and/or alluvial soils. The residual soils generally consist of soft to hard silty/sandy clays, sandy silts, and loose to very dense silty/clayey sands. Trace mica and gravel-sized rock fragments were encountered within the residual soils. Laboratory testing of the residual soils indicated PI's ranging from 10 to 18 for the sandy (A-2-4, A-2-6, A-2-7) soils; PI's ranging from 4 to 10 for the silty (A-4) soils; and PI's ranging from 11 to 61 for the clayey (A-6, A-7-5, A-7-6) soils.

Weathered rock was encountered extensively along the project corridor from near ground surface (L_7300_RT, L_8400_RT, Y1_1400_RT, and Y1_2600_LT) to beginning depths ranging from 1.5 to 38.5 feet below the ground surface. The weathered rock consisted of Metavolcanic Rock. The top of weathered rock depth corresponds to an elevation ranging from approximately EL 302 to EL 499 ft.

Crystalline rock was encountered along the project corridor at depths from approximately 3.3 to 41.3 feet below existing ground surface. The crystalline rock consisted of Metavolcanic Rock. This top of rock depth corresponds to an elevation ranging from approximately EL 298 to EL 487 ft.

Groundwater

Groundwater measurements were taken during July through September 2024. Groundwater measurements were attempted at the completion of drilling in each boring and rod sounding, at which time groundwater was encountered in eleven (11) of the borings at depths ranging from approximately 4.1 to 34.7 feet below the existing grades. Subsequent groundwater measurements were attempted after at least 24 hours following the completion of drilling where possible. At the time of subsequent water level measurements, groundwater was encountered at depths ranging from 0.2 to 40.4 feet below existing grades in sixty-four (64) of the borings. The remaining borings were either recorded as dry or filled in after drilling due to our demobilization from the project site. The soils encountered were generally described as dry to saturated above and below groundwater elevation.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

Very soft to soft soils were encountered along the project corridor at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	20+75 to 21+25	LT to RT
-L-	26+25 to 27+25	LT to RT
-L-	54+75 to 56+25	LT to RT
-L-	81+20 to 81+75	LT to RT
-L-	91+75 to 92+25	LT to RT
-L-	95+00 to 99+00	LT to RT
-L-	109+00 to 113+00	LT to RT
-L-	133+65 to 134+16	LT to RT
-L-	167+00 to 169+00	LT to RT
-RBT-	10+00 to 12+10	LT to RT
-Y7-	19+00 to 20+30	LT to RT
-Y14-	11+48 to 31+05	LT to RT

Highly plastic soils (PI>25) were encountered throughout the project corridor, and were specifically encountered at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	11+25 to 13+25	LT to RT
-L-	23+75 to 26+25	LT to RT
-L-	29+75 to 51+75	LT to RT
-L-	56+25 to 60+25	LT to RT
-L-	71+75 to 74+75	LT to RT
-L-	82+25 to 89+25	LT to RT
-L-	101+25 to 105+25	LT to RT
-L-	116+75 to 119+75	LT to RT
-L-	126+75 to 129+75	LT to RT
-L-	152+75 to 154+25	LT to RT
-L-	172+75 to 174+25	LT
-RBT-	10+10 to 12+10	LT to RT
-Y14-	16+75 to 18+75	LT to RT

Shallow groundwater was encountered within 6 feet of proposed subgrade at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	10+00 to 18+00	LT to RT
-L-	25+00 to 29+00	LT to RT
-L-	44+00 to 55+00	LT to RT
-L-	80+00 to 95+00	LT to RT
-L-	99+00 to 101+00	LT to RT

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-Y1-	15+00 to 17+00	LT to RT
-Y1-	20+00 to 26+00	LT to RT
-Y14-	15+00 to 19+00	LT to RT
-Y14-	23+00 to 28+00	LT to RT

Alluvial soils were encountered on the project at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets</u>
-L-	64+15 to 68+81	LT
-L-	75+99 to 76+99	LT to RT
-L-	89+58 to 92+67	LT to RT
-L-	133+65 to 137+32	LT to RT
-L-	163+74 to 165+58	LT
-Y7-	19+21 to 20+70	RT

Artificial Fill soils were encountered on the project at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	11+40 to 11+70	LT to RT
-L-	78+89 to 79+42	LT to RT
-L-	80+03 to 81+85	LT to RT
-RBT-	10+00 to 10+28	LT to RT
-RBT-	12+64 to 13+45	LT to RT
-Y7-	14+42 to 16+85	LT to RT
-Y14-	10+00 to 21+26	LT to RT
-Y16-	11+25 to 12+25	LT to RT

Crystalline rock was encountered above or within 6 feet of proposed grade at the following locations.

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	10+00 to 14+00	LT to RT
-L-	33+25 to 35+25	LT
-L-	35+75 to 36+25	LT
-L-	58+75 to 59+25	LT to RT
-L-	81+20 to 81+75	LT to RT
-L-	82+75 to 83+25	LT to RT
-L-	151+25 to 153+25	LT to RT
-L-	153+75 to 154+25	LT to RT
-Y1-	25+00 to 30+30	LT to RT

Rock outcrops were observed at the following locations:

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	75+40 to 76+65	LT to RT
-L-	84+18 to 85+51	RT
-L-	135+00 to 135+60	LT

Water Wells: There are several residences near the project site which could indicate that water wells may be present. Water wells were observed within the proposed construction corridor at the following locations.

<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	15+95	7 LT
-Y7-	21+26	62 LT
-Y14-	18+65	161 RT

Please note, water wells could be encountered at other locations due to the presence of dwellings and businesses.

Several ponds were observed along the project corridor at the following approximate locations:

<u>Alignment</u>	<u>Reported WSE (ft)</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	383.0	86+43 to 92+63	LT
-L-	391.0	89+58 to 92+67	LT to RT
-L-	392.7	90+99 to 91+46	LT to RT
-L-	392.7	90+81 to 91+82	RT

The new alignment will also extend through an existing irrigation pond approximately at -L- station 89+00 to 93+00. We understand plans are to mechanically drain the pond during construction for embankment construction, permanent dewatering of the ponds is not currently planned; therefore, they will be allowed to refill post-embankment construction.

CG2 performed probes within the pond at the following locations:

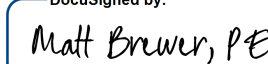
<u>Test Location</u>	<u>North (ft)</u>	<u>East (ft)</u>	<u>Groundline Elevation (ft)</u>	<u>Water Depth (ft)</u>	<u>Measured Muck Thickness (ft)</u>
PO-01	708129	1954230	390.1	3.0	0.7
PO-02	708235	1954306	930.0	2.2	0.3
PO-03	708186	1954246	389.0	4.1	0.4
PO-04	708291	1954297	389.8	2.4	0.8
PO-05	708214	1954403	391.8	2.1	1.9
PO-06	708279	1954376	390.4	2.7	2.0

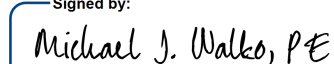
GEOTECHNICAL TESTING

Two (2) thin-wall Shelby tube samples were collected during the investigation and consolidation testing was performed.

<u>Sample No.</u>	<u>Alignment</u>	<u>Stations</u>	<u>Offsets (ft)</u>	<u>Sample Depth (ft)</u>
ST-02	-L-	65+00	CL	4.0-6.0
ST-01	-L-	134+16	47 LT	1.0-3.0

Sincerely,
Carolinus Geotechnical Group, PLLC

DocuSigned by:

 386129C0A4C1462...
 D. Matthew Brewer, PE
 Senior Project Engineer

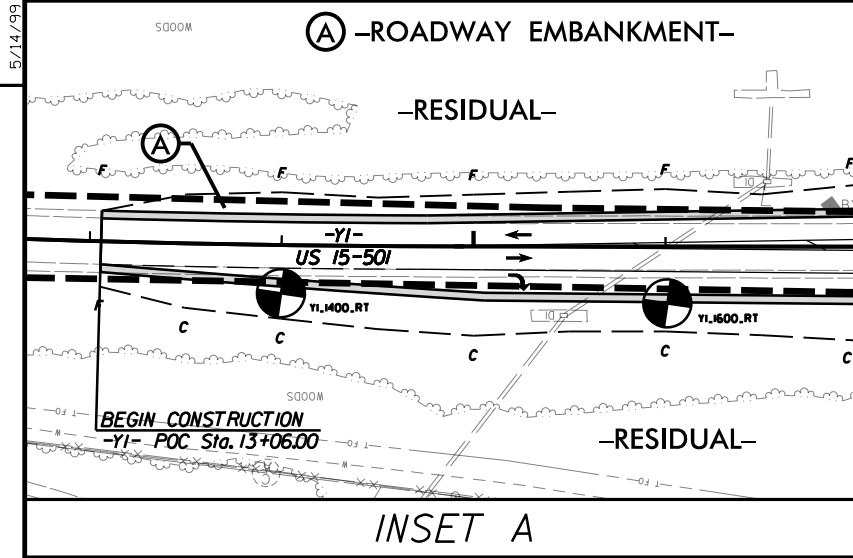
Signed by:

 B0BEC14A5D19492...
 Michael J. Walko, PE
 Principal Engineer

5/14/99

\$DATE\$

\$FILE\$

REVISIONS



-YI-

PI Sta 12+44.06

$\Delta = 4^{\circ}06'40.1''$ (LT)

D = 0'50'33.3"

L = 487.92'

T = 244.06'

R = 6,800.00'

SE = EXIST

RO = NA

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

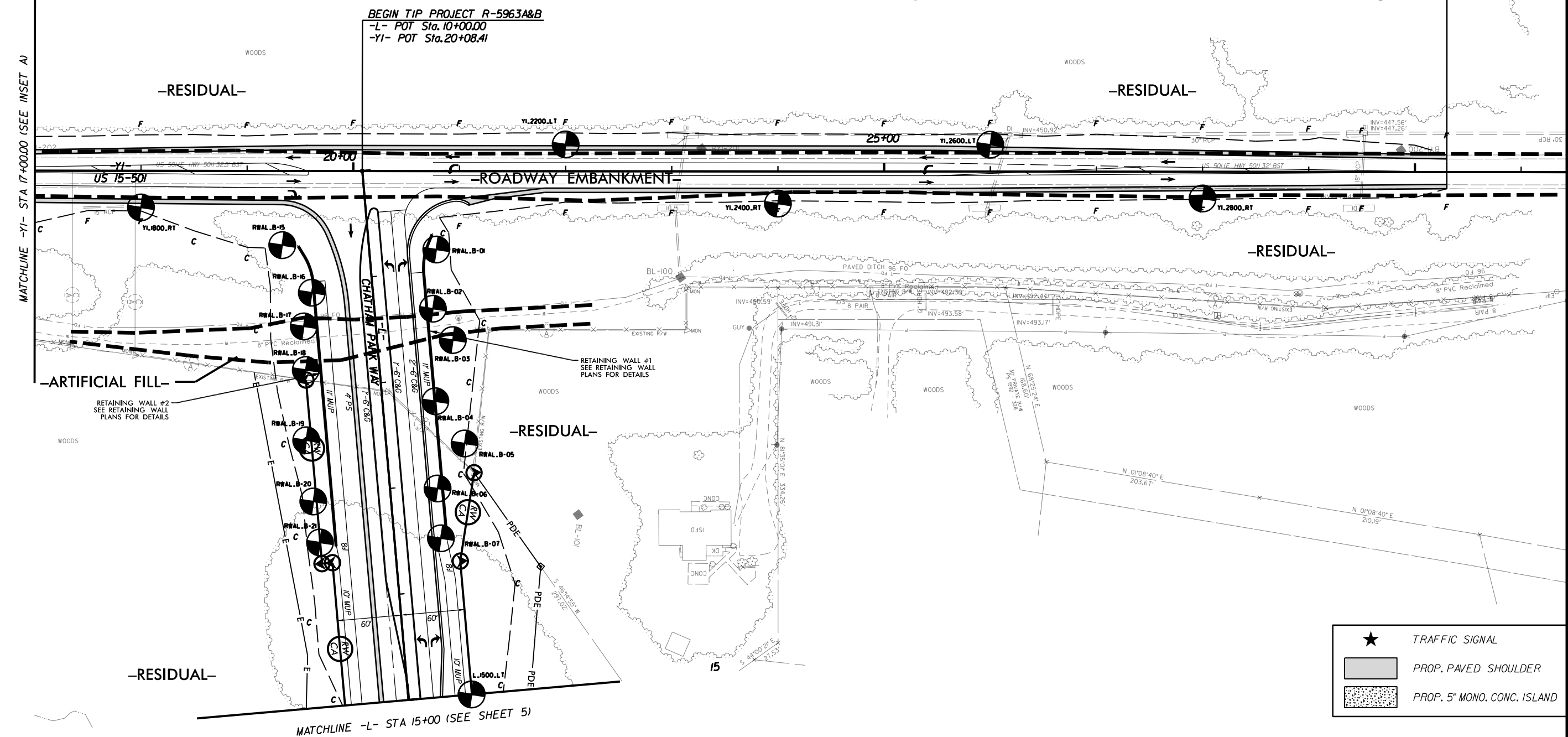
PROJECT REFERENCE NO.		SHEET NO.
R-5963A		4
RW SHEET NO.		
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		

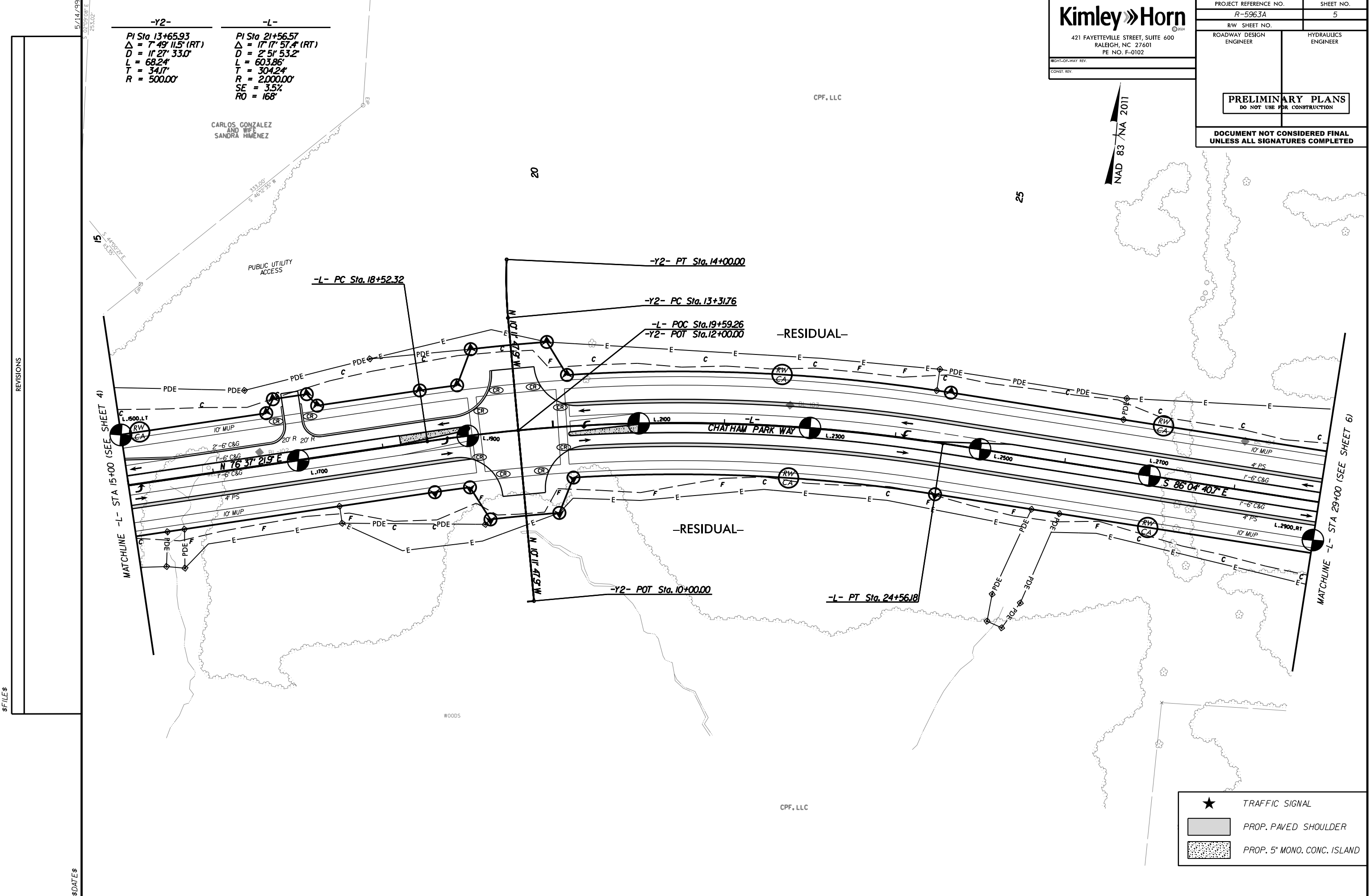
NAD 83 / NA 2011

30

END CONSTRUCTION

-YI- POT Sta. 30+30.00





-Y2-	-L-
PI Sta 13+65.93	PI Sta 21+56.57
$\Delta = 7^{\circ}49'11.5''$ (RT)	$\Delta = 17^{\circ}17'57.4''$ (RT)
D = 11'27" 33.0'	D = 2'51" 53.2'
L = 68.24'	L = 603.86'
T = 34.17'	T = 304.24'
R = 500.00'	R = 2000.00'
	SE = 3.5%
	RO = 168'

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.
CONST. REV.

PROJECT REFERENCE NO.		SHEET NO.
R-5963A		5
RW SHEET NO.		
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		

★ TRAFFIC SIGNAL

PROP. PAVED SHOULDER

PROP. 5" MONO. CONC. ISLAND

REVISIONS

\$FILE\$

\$DATE\$

CPF, LLC
DB 1898 PG 446
PS 2013 - 144

-L- PT Sta. 44+14.26

-Y3- POT Sta. 10+00.00
-L- POC Sta. 43+46.49

—RESIDUAL—

-RESIDUAL-

CPF, LLC
DB 1898 PG 446
PS 2013 - 144

	TRAFFIC SIGNAL
	PROP. PAVED SHOULDER
	PROP. 5' MONO. CONC. ISLAND

MATCHLINE -L- STA 42+00 (SEE SHEET 6)

MATCHLINE -L- STA 56+00 (SEE SHEET 8)

REVISIONS

\$FILE\$

5/14/99

\$DATE\$

5/14/99

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

PROJECT REFERENCE NO.
R-5963A

R/W SHEET NO.

ROADWAY DESIGN ENGINEER

SHEET NO.
8

HYDRAULICS ENGINEER

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

-Y4-	-Y5-
PI Sta 12+18.02	PI Sta 13+62.14
$\Delta = 76^{\circ} 24' 19.1''$ (RT)	$\Delta = 5^{\circ} 35' 01.4''$ (RT)
D = 38' 11" 49.9"	D = 7' 22' 09.2"
L = 200.03'	L = 75.77'
T = 118.05'	T = 37.92'
R = 150.00'	R = 777.50'

REVISIONS

\$FILE\$

\$DATE\$

The diagram shows a plan view of a road layout. Key features include:

- Stationing:** -Y4- PT Sta. 13+00.00, -Y4- PC Sta. 10+99.97, -L- POT Sta. 56+66.24, -Y4- POT Sta. 10+00.00, -Y5- PC Sta. 13+24.23, -Y5- PT Sta. 14+00.00, -L- POT Sta. 61+91.23, -Y5- POT Sta. 11+84.34, -Y5- POT Sta. 10+00.00.
- Curves:** 50°R, 50°R, 10' MUP, 4' PS, 1'-6" C&G, 1'-6" C&G, N 55°06'58.2" E, N 34°53'01.8" W.
- Features:** WATHAM PARK WAY, ALLUVIAL, -RESIDUAL-, -Y4- PT Sta. 13+00.00, -Y5- PT Sta. 14+00.00.
- Other:** CP SOUTH, LLC, NAD 83 / NA 2011.

(A) -CRYSTALLINE ROCK- (OUTCROP)

★

TRAFFIC SIGNAL

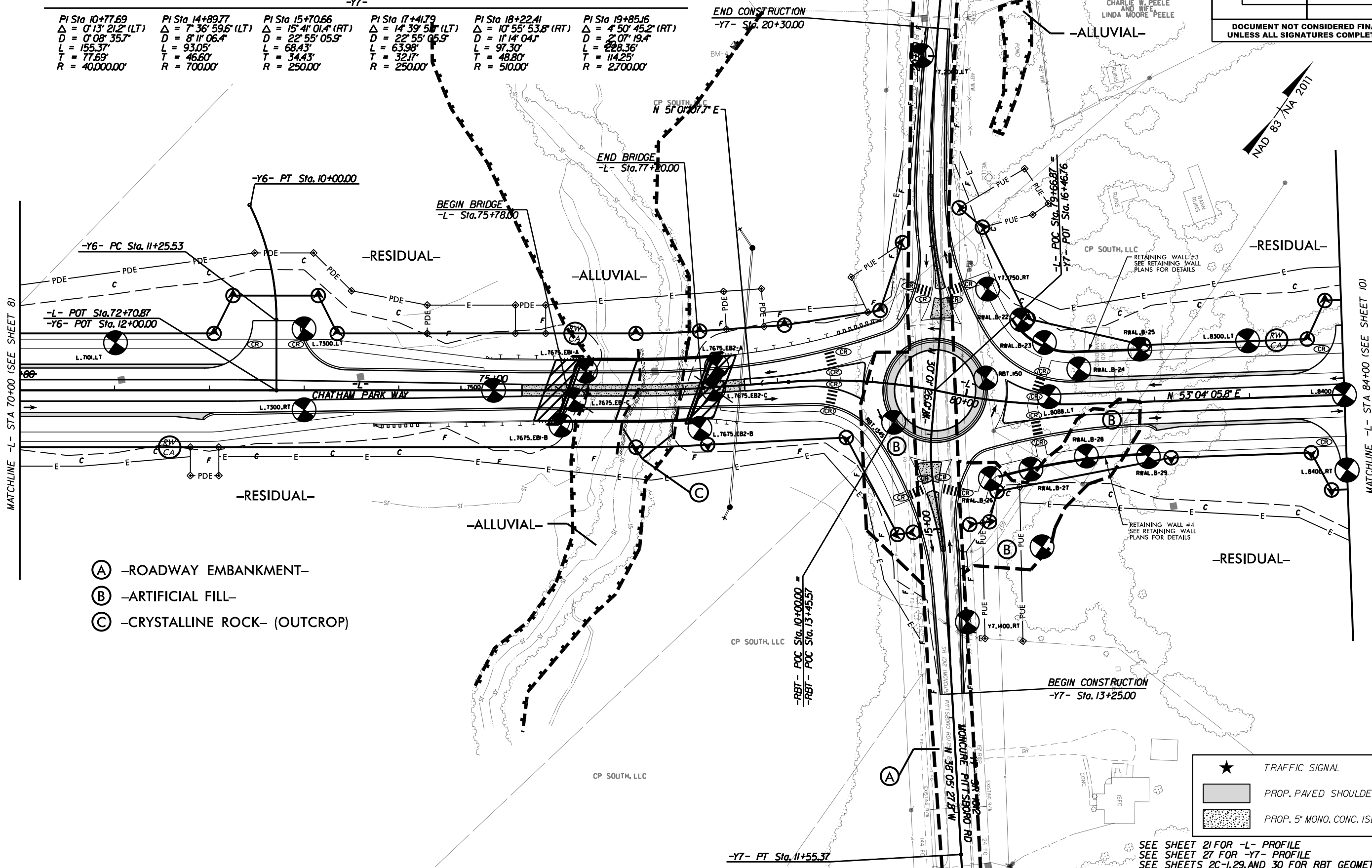
PROP. PAVED SHOULDER

PROP. 5" MONO. CONC. ISLAND

CP SOUTH, LLC

-L-			-Y6-		-RBT-	
PI Sta 76+85.98 Δ = 4° 05' 50.5" (LT) D = 5' 43' 46.5" L = 71.51' T = 35.77' R = 1,000.00'	PI Sta 78+95.29 Δ = 15° 53' 49.5" (RT) D = 9' 32' 57.5" L = 166.47' T = 83.77' R = 600.00'	PI Sta 80+50.85 Δ = 13° 50' 51.4" (LT) D = 9' 32' 57.5" L = 145.01' T = 72.86' R = 600.00'	PI Sta 11+38.37 Δ = 26° 23' 37.7" (LT) D = 21' 01' 33.5" L = 125.53' T = 63.90' R = 272.50'	PI Sta 10+00.00 Δ = 359° 59' 56.1" (LT) D = 104' 10' 26.9" L = 345.57' T = 0.00' R = 55.00'		

-Y7-					
PI Sta 10+77.69 Δ = 0° 13' 21.2" (LT) D = 0' 08' 35.7" L = 155.37' T = 77.69' R = 40,000.00'	PI Sta 14+89.77 Δ = 7° 36' 59.6" (LT) D = 8' 11' 06.4" L = 93.05' T = 46.60' R = 700.00'	PI Sta 15+70.66 Δ = 15° 41' 01.4" (RT) D = 22° 55' 05.9" L = 68.43' T = 34.43' R = 250.00'	PI Sta 17+41.79 Δ = 14° 39' 51.1" (LT) D = 22° 55' 05.9" L = 63.98' T = 32.17' R = 250.00'	PI Sta 18+22.41 Δ = 10° 55' 53.8" (RT) D = 11' 14' 04.1" L = 97.30' T = 48.80' R = 510.00'	PI Sta 19+85.16 Δ = 4° 50' 45.2" (RT) D = 2' 07' 19.4" L = 228.36' T = 114.25' R = 2700.00'



- (A) -ROADWAY EMBANKMENT-
- (B) -ARTIFICIAL FILL-
- (C) -CRYSTALLINE ROCK- (OUTCROP)

- ★ TRAFFIC SIGNAL
- PROP. PAVED SHOULDER
- PROP. 5" MONO. CONC. ISLAND

SEE SHEET 21 FOR -L- PROFILE
SEE SHEET 27 FOR -Y7- PROFILE
SEE SHEETS 2C-1, 29, AND 30 FOR RBT GEOMETRY

REVISIONS

\$FILE\$

\$DATE\$

5/14/99

\$DATE\$

\$FILE\$

REVISIONS

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.
CONST. REV.

PROJECT REFERENCE NO. SHEET NO.

R-5963A 10

R/W SHEET NO.

ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

JASON WALDECK AND
SYLVIA R. GEORGE

NAD 83 / NA 2011

-L-
PI Sta 91+19.24
 $\Delta = 51^{\circ} 40' 19.3" (LT)$
 $D = 5^{\circ} 43' 46.5"$
 $L = 901.85'$
 $T = 484.19'$
 $R = 1,000.00'$
 $SE = 4\%$
 $RO = 192'$

CP SOUTH, LLC

DAVID C. EVERETTE
AND WIFE
ELOISE M. EVERETTE

-ALLUVIAL-

POND
WSE=383.0'

CP SOUTH, LLC

-Y7A- POT Sta. 10+00.00

-L- POC Sta. 84+18.66
-Y7A- POT Sta. 12+00.00

-L- PC Sta. 86+35.05

-RESIDUAL-

-ALLUVIAL-

-Y7B- POT Sta. 10+00.00
-L- POT Sta. 88+59.33

POND
WSE=391.0'

-L- PT Sta. 95+36.89

-Y8- POT Sta. 10+00.00

-RESIDUAL-

-RESIDUAL-

CHATHAM PARK WAY

-Y7A- POT Sta. 14+00.00

-CRYSTALLINE ROCK- (OUTCROP)

CP SOUTH, LLC

-RESIDUAL-

-Y7B- POT Sta. 12+00.00

CP SOUTH, LLC

-ALLUVIAL-

POND
WSE=392.7'

-L- POC Sta. 93+00.00
-Y8- POT Sta. 12+00.00

-Y8- POT Sta. 14+00.00

★ TRAFFIC SIGNAL
PROP. PAVED SHOULDER
PROP. 5" MONO. CONC. ISLAND

5/14/99

\$DATE\$

\$FILE\$

REVISIONS

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

NAD 83 / NA 2011

PROJECT REFERENCE NO.

R-5963A

SHEET NO.

11

R/W SHEET NO.

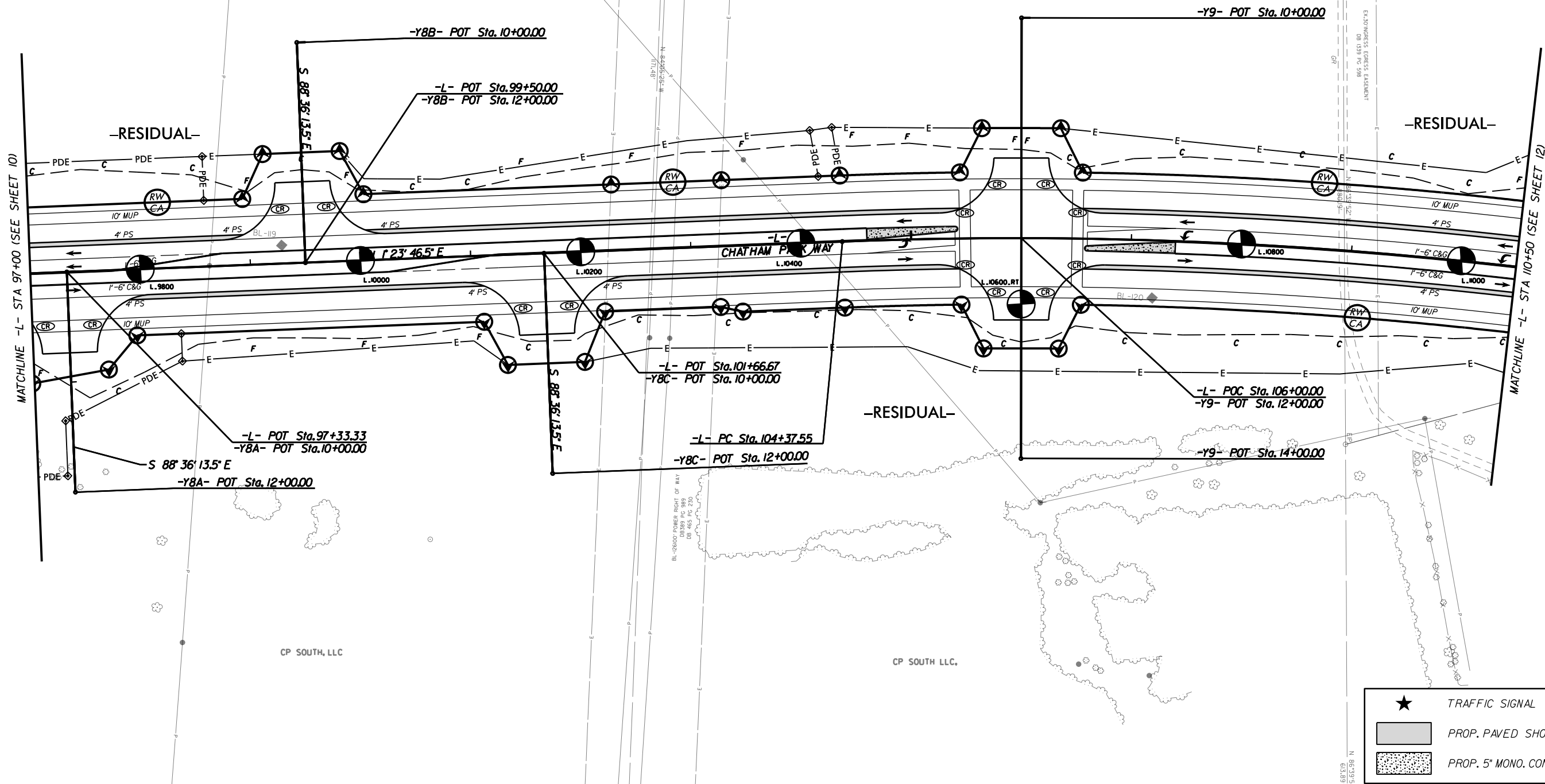
ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

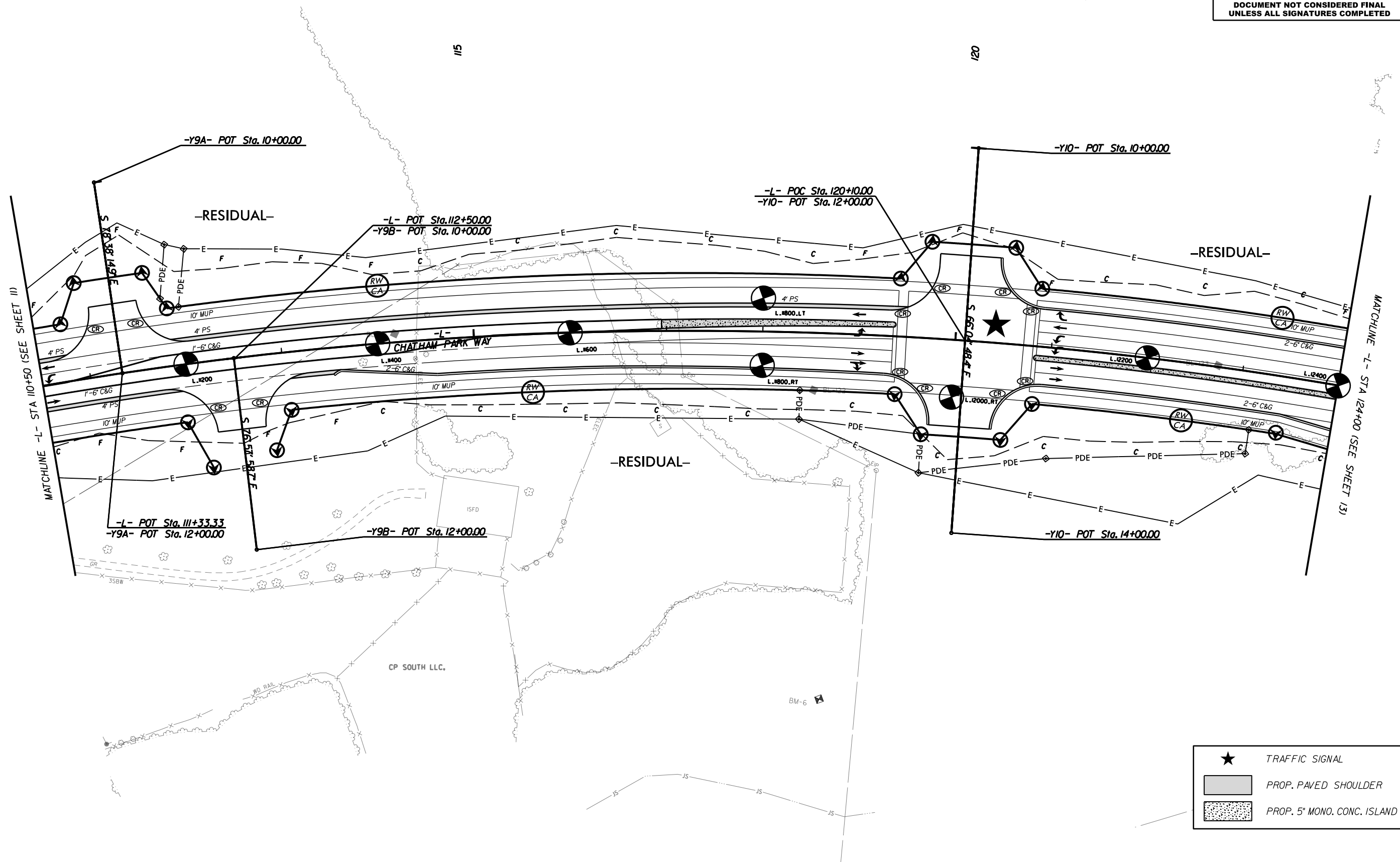


★ TRAFFIC SIGNAL

PROP. PAVED SHOULDER

PROP. 5" MONO. CONC. ISLAND

PI Sta 114+73.90
 $\Delta = 29^\circ 03' 01.4''$ (RT)
 $D = 125^\circ 56.6''$
 $L = 2,028.10'$
 $T = 1,036.35'$
 $R = 4,000.00'$
 $SE = 2.5\%$
 $RO = 120'$



5/14/99

\$DATE\$

\$FILE\$

REVISIONS

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

PROJECT REFERENCE NO.

R-5963A

SHEET NO.

13

R/W SHEET NO.

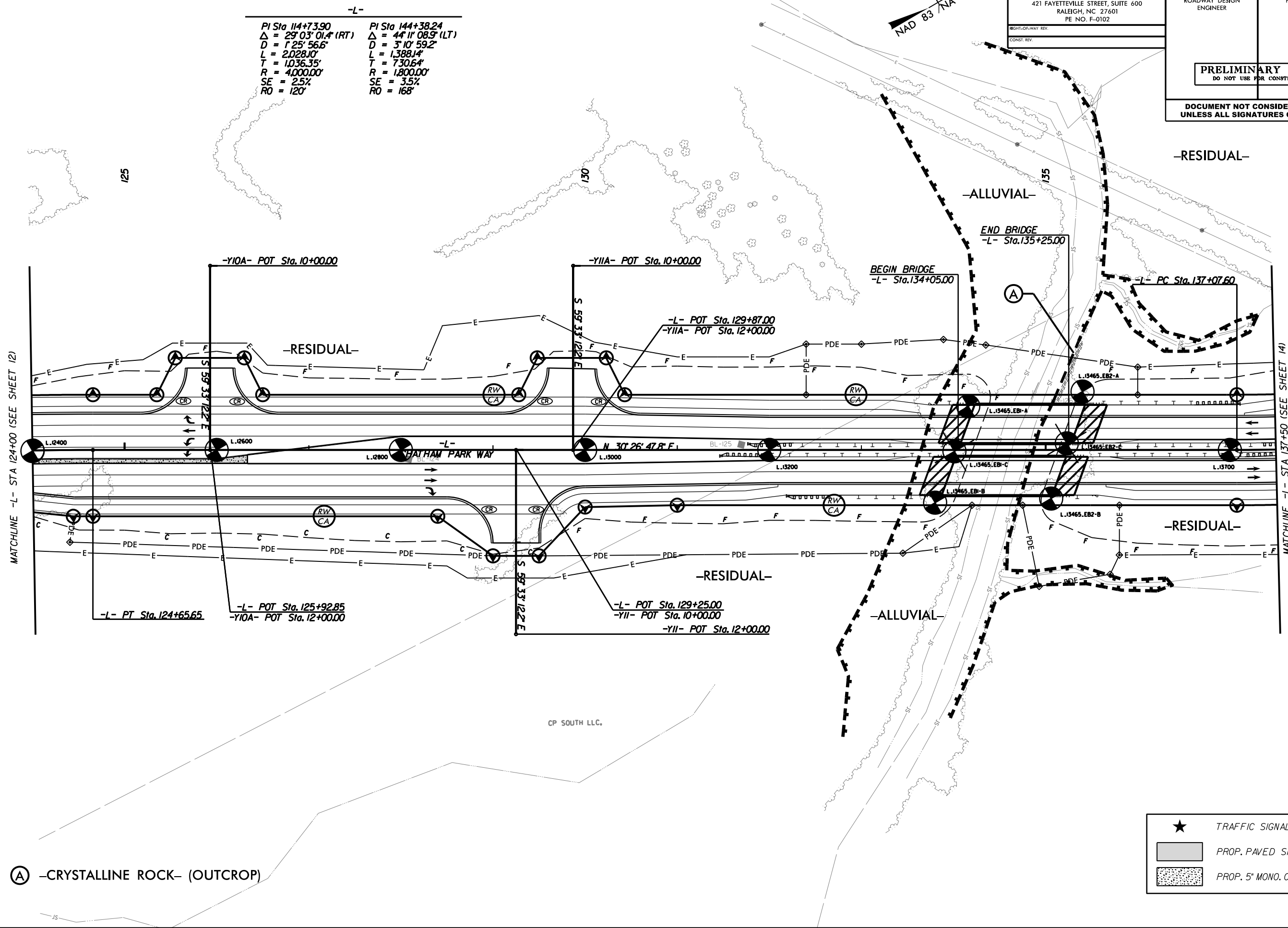
ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



Ⓐ -CRYSTALLINE ROCK- (OUTCROP)

- ★ TRAFFIC SIGNAL
- ▒ PROP. PAVED SHOULDER
- ▒ PROP. 5" MONO. CONC. ISLAND

5/14/99

\$DATE\$

\$FILE\$

REVISIONS

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

NAD 83 / N.A. 2011

PROJECT REFERENCE NO.

R-5963A

SHEET NO.

14

R/W SHEET NO.

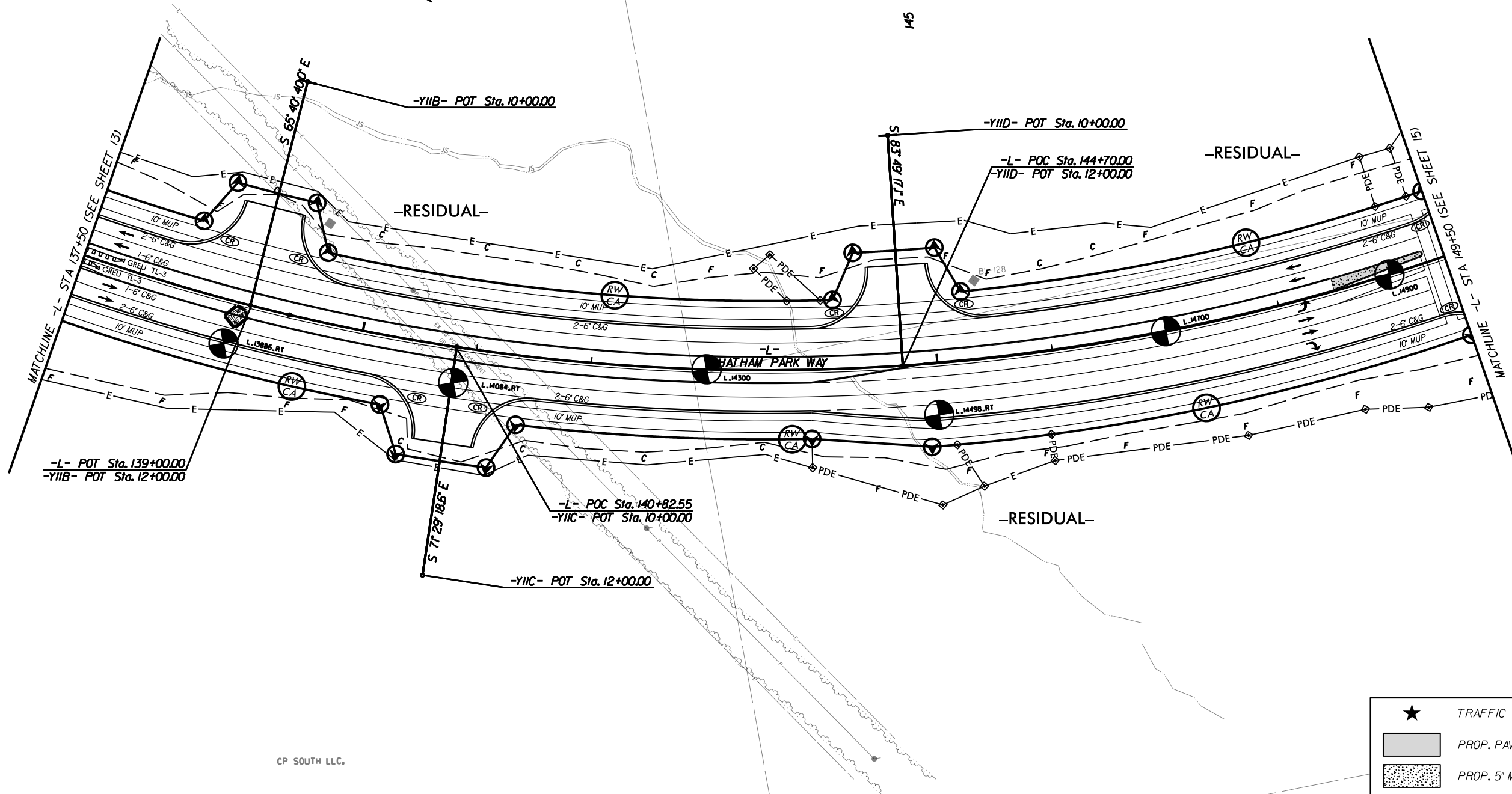
ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

-L-
PI Sta 144+38.24
 $\Delta = 44^{\circ} 11' 08.9''$ (LT)
D = 3' 10' 59.2"
L = 1,388.14'
T = 730.64'
R = 1,800.00'
SE = 3.5%
RO = 168'



- ★ TRAFFIC SIGNAL
- PROP. PAVED SHOULDER
- PROP. 5' MONO. CONC. ISLAND

5/14/99

\$DATE\$

\$FILE\$

REVISIONS

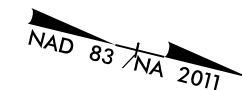
-L-	
PI Sta 144+38.24	PI Sta 165+36.44
$\Delta = 44^{\circ}11'08.9"$ (LT)	$\Delta = 24^{\circ}19'40.4"$ (RT)
D = 3'10"59.2"	D = 3'49"11.0"
L = 1,388.14'	L = 636.90'
T = 730.64'	T = 323.32'
R = 1,800.00'	R = 1,500.00'
SE = 3.5%	SE = 4%
RO = 168'	RO = 190'

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.



PROJECT REFERENCE NO.

R-5963A

SHEET NO.

15

R/W SHEET NO.

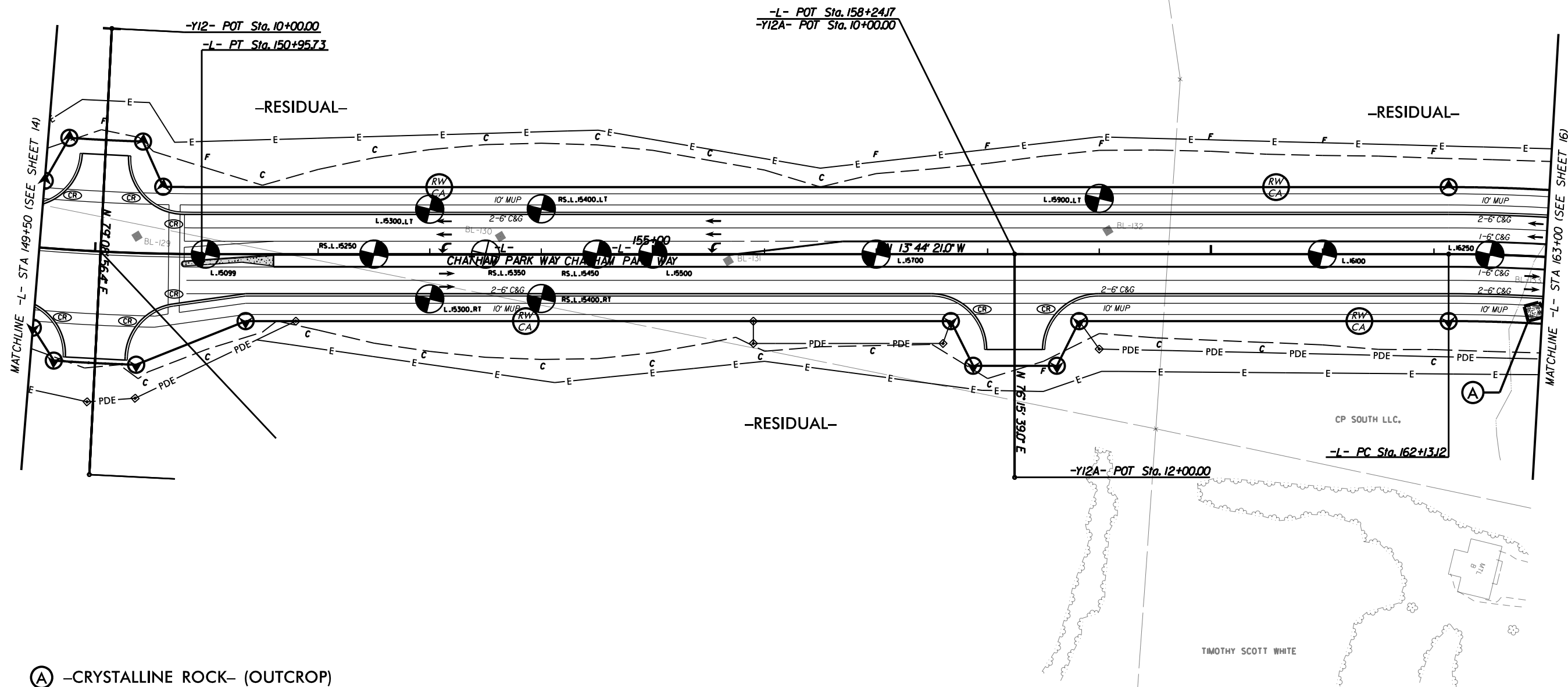
ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



- ★ TRAFFIC SIGNAL
- PROP. PAVED SHOULDER
- PROP. 5" MONO. CONC. ISLAND

5/14/99

\$DATE\$

\$FILE\$

REVISIONS

-L-
PI Sta 165+36.44
 $\Delta = 24' 19" 40.4" (RT)$
 $D = 3' 49" 11.0"$
 $L = 636.90'$
 $T = 323.32'$
 $R = 1500.00'$
 $SE = 4\%$
 $RO = 190'$

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

PROJECT REFERENCE NO.

R-5963A

SHEET NO.

16

R/W SHEET NO.

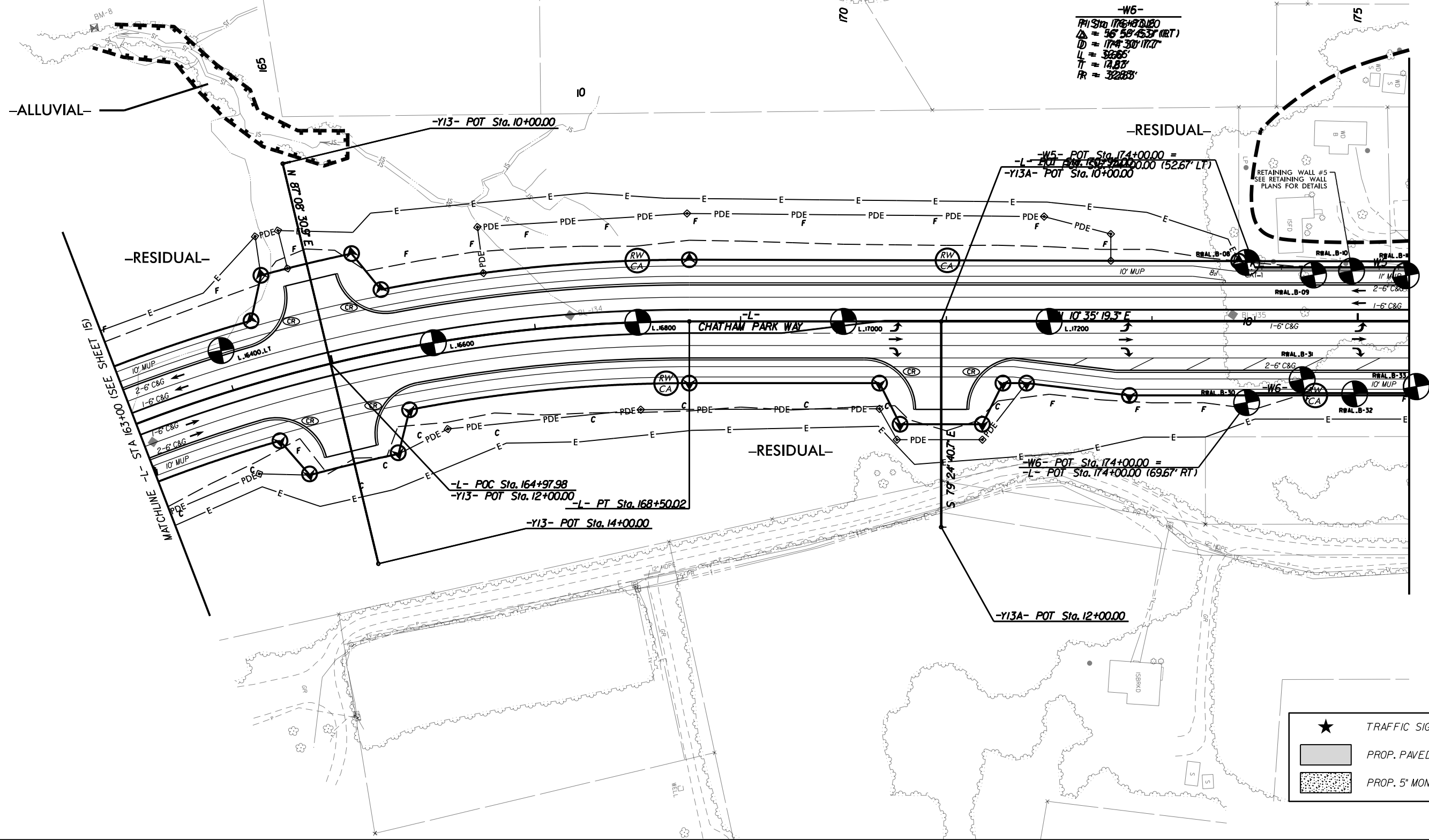
ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**



- ★ TRAFFIC SIGNAL
- PROP. PAVED SHOULDER
- PROP. 5" MONO. CONC. ISLAND

MATCHLINE -L- STA 175+50 (SEE SHEET 17)

5/14/99

Kimley»Horn

421 FAYETTEVILLE STREET, SUITE 600
RALEIGH, NC 27601
PE NO. F-0102

RIGHT-OF-WAY REV.

CONST. REV.

PROJECT REFERENCE NO. SHEET NO.

R-5963A 17

R/W SHEET NO.

ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

-L-
PI Sta 188+24.18
 $\Delta = 12^{\circ} 06' 56.2" (RT)$
 $D = 2^{\circ} 17' 30.6"$
 $L = 528.64'$
 $T = 265.31'$
 $R = 2,500.00'$

NAD 83 / NA 2011

-W5- POC Sta. 176+98.00 =
-L- POT Sta. 176+92.87 (67.62' LT)

MATCHLINE -L- STA 175+50 (SEE SHEET 16)

-W6- POC Sta. 176+75.00 =
-L- POT Sta. 176+75.00 (71.12' RT)

(A) -ROADWAY EMBANKMENT-

MATCHLINE -Y14- STA 16+50 (SEE SHEET 18)

MATCHLINE -Y14- STA 26+00 (SEE SHEET 19)

-ARTIFICIAL FILL-

-RESIDUAL-

END CONSTRUCTION
-L- 179+16.00

-RESIDUAL-

-L- PC Sta. 185+58.87

- ★ TRAFFIC SIGNAL
- PROP. PAVED SHOULDER
- PROP. 5" MONO. CONC. ISLAND

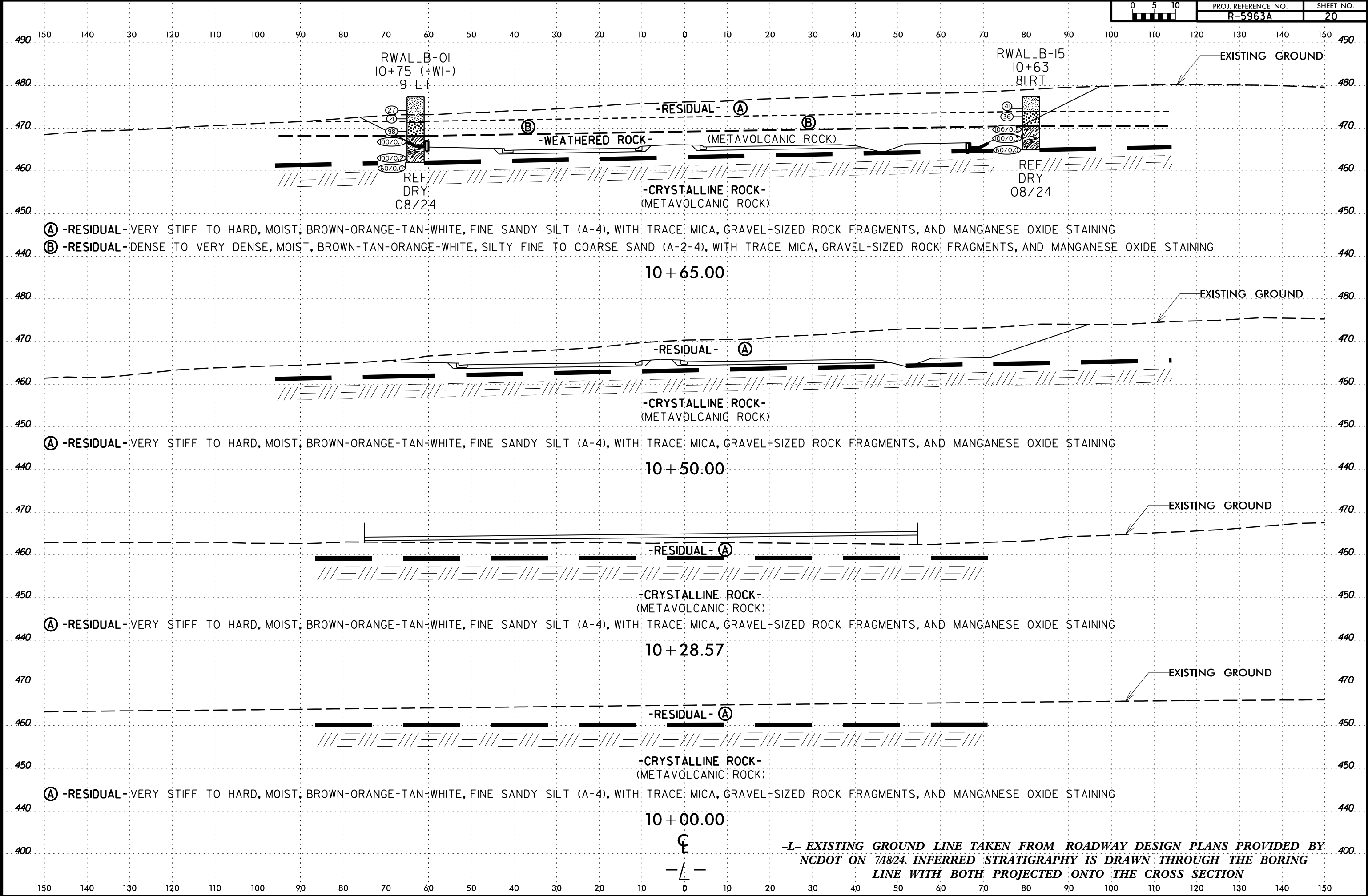
\$FILE\$

\$DATE\$

	TRAFFIC SIGNAL
	PROP. PAVED SHOULDER
	PROP. 5" MONO. CONC. ISLAND

6/23/16

I4-NOV-2024 18:53
C:\Users\jellidmont\brun\Carolinac Geotechnical Group, PLLC\Matt Brewer - Projects\0252 - R-5963A.RDY_PDI_NCDOT_GEO\TECH\ysec-R-5963A_GEO.XSL.dgn
\$\$\$\$\$USERNAME\$\$\$\$\$



6/23/16

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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

540 540

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510 510

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490 490

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470 470

460 460

450 450

440 440

11 + 00.00

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-16	SS-2023	2' RT	11+10 -W2-	1.0-2.5	A-7-5(29)	58	23	1.2%	1.7%	44.0%	53.1%	99.3%	98.5%	96.8%	31.9%	ND
RWAL B-16	SS-2025	2' RT	11+10 -W2-	6.0-7.5	A-7-5(24)	53	23	3.8%	2.2%	45.0%	49.0%	95.9%	93.0%	90.5%	28.7%	ND

RWAL B-02
11+35 (-W1-)
6 LT

RWAL B-16
11+10 (-W2-)
2 RT

SS-2023
SS-2025

EXISTING GROUND

VERY STIFF TO HARD, MOIST,
MICA, GRAVEL-SIZED

ORANGE-BROWN-TAN, FINE TO COARSE SANDY SILT (A-4), WITH TRACE
ROCK FRAGMENTS, AND MANGANESE OXIDE STAINING

REF
DRY
08/24

-CRYSTALLINE ROCK-
(METAVOLCANIC ROCK)

REF
DRY
08/24

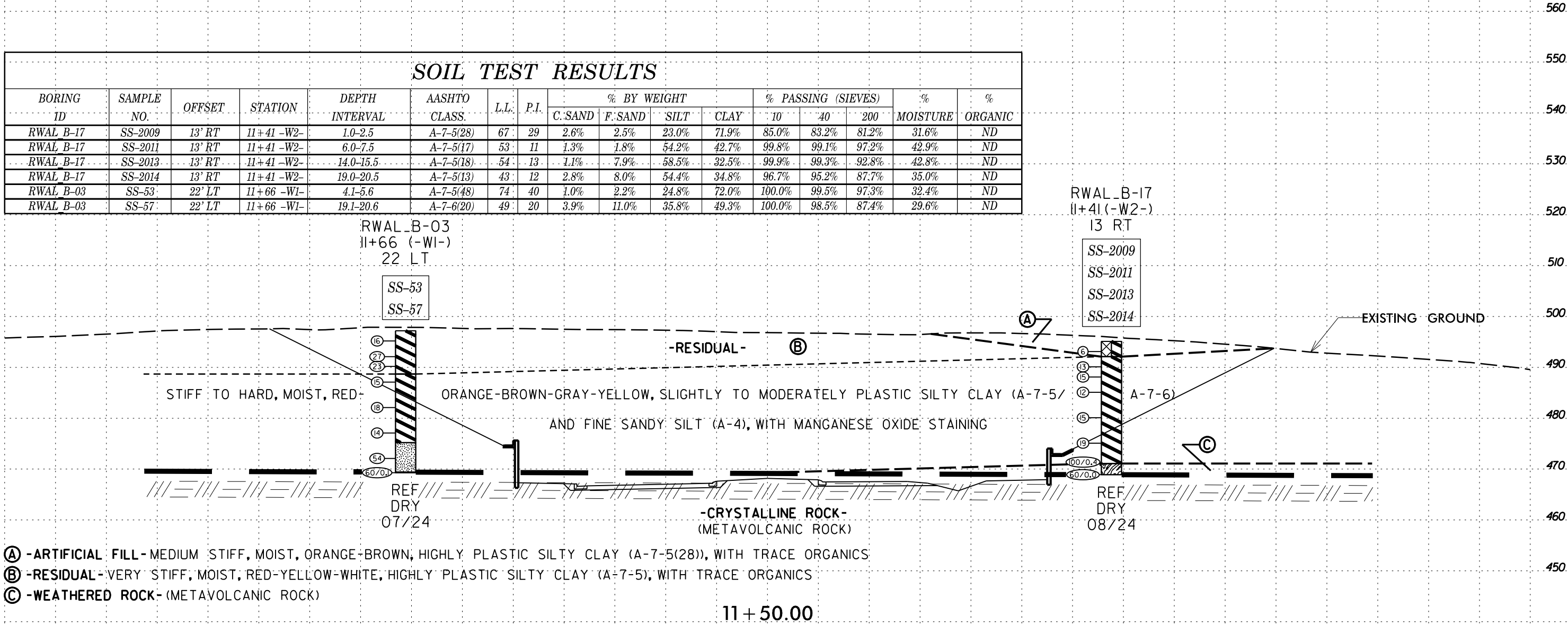
- Ⓐ -RESIDUAL- VERY STIFF, MOIST, TAN-ORANGE-BROWN, SILTY CLAY (A-7), WITH TRACE MICA, GRAVEL-SIZED ROCK FRAGMENTS, AND MANGANESE OXIDE STAINING
Ⓑ -RESIDUAL- VERY STIFF, MOIST, ORANGE-RED-TAN, MODERATELY PLASTIC SILTY CLAY (A-7-5), WITH TRACE MICA, ORGANICS, AND MANGANESE OXIDE STAINING
Ⓒ -WEATHERED ROCK- (METAVOLCANIC ROCK)

-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16

I4-NOV-2024 18:53
C:\Users\KellieMontbrun\Carolina Geotechnical Group, PLLC\Matt Brewer - Projects\0252 - R-5963A.RD\PD\NCDOT GEOTECH\SS-R-5963A_GEO.XSL.dgn
\$\$\$\$\$USERNAME\$\$\$\$\$

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6/23/16

I4-NOV-2024 18:53
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\$\$\$\$\$USERNAME\$\$\$\$\$



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490

490

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470

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460

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450

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-18	SS-2017	14' RT	11+82 -W2-	1.0-2.5	A-7-6(40)	67	41	2.8%	1.3%	28.5%	67.4%	91.6%	89.4%	88.1%	28.9%	ND
RWAL B-04	SS-01	1' LT	12+22 -W1-	1.0-2.5	A-7-5(47)	78	45	5.3%	7.0%	16.6%	71.1%	99.6%	96.0%	89.2%	35.2%	ND
RWAL B-04	SS-04	1' LT	12+22 -W1-	9.3-10.8	A-7-5(33)	63	31	4.5%	7.4%	27.8%	60.3%	100.0%	97.4%	89.8%	35.3%	ND

RWAL B-04
12+22 (-W1-)
1' LT

SS-01
SS-04

RWAL B-18
11+82 (-W2-)
14 RT

SS-2017

STIFF TO VERY STIFF, MOIST, YELLOW
A-7-6) WITH TRACE ORGANICS, MICA,

ORANGE-RED-TAN-BROWN, HIGHLY PLASTIC SILTY CLAY (A-7-5/
AND MANGANESE OXIDE STAINING

VERY STIFF TO HARD, MOIST, YELLOW

ORANGE-BROWN-RED-TAN, FINE SANDY SILT (A-4), WITH TRACE MICA, GRAVEL
SIZED ROCK FRAGMENTS,

AND MANGANESE OXIDE STAINING

-WEATHERED ROCK-
(METAVOLCANIC ROCK)

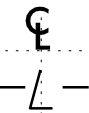
-CRYSTALLINE ROCK-
(METAVOLCANIC ROCK)

REF
DRY

REF
DRY
08/24

12 + 00.00

(A) -WEATHERED ROCK- (METAVOLCANIC ROCK)



-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

6/23/16
14-NOV-2024 18:53
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\$\$\$\$\$USERNAME\$\$\$\$\$

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450 450

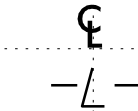
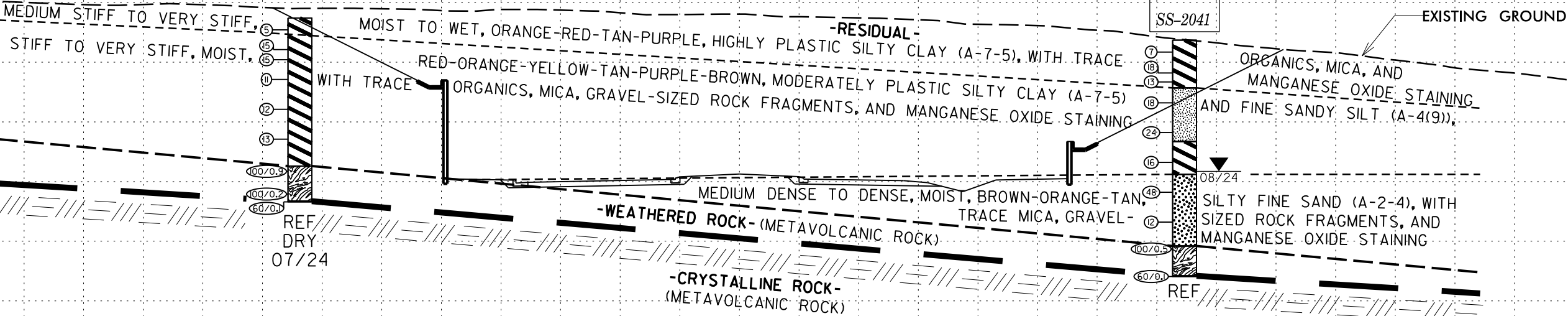
440 440

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-19	SS-2037	20' RT	12+48 -W2-	4.5-6.0	A-7-5(28)	63	30	3.2%	3.5%	38.0%	55.3%	87.5%	85.5%	82.2%	39.6%	ND
RWAL B-19	SS-2040	20' RT	12+48 -W2-	14.5-16.0	A-4(9)	40	10	7.2%	11.6%	58.8%	22.4%	97.6%	93.7%	81.0%	30.0%	ND
RWAL B-19	SS-2041	20' RT	12+48 -W2-	19.5-21.0	A-7-5(13)	41	11	3.0%	3.6%	58.3%	35.1%	99.9%	97.9%	93.8%	35.5%	ND
RWAL B-05	SS-44	25' LT	12+64 -W1-	4.3-5.8	A-7-5(15)	46	16	5.8%	11.7%	35.9%	46.6%	96.9%	93.5%	83.0%	30.2%	ND

RWAL_B-05
12+64 (-W1-)
25 LT
SS-44

RWAL_B-19
12+48 (-W2-)
20 RT
SS-2037
SS-2040
SS-2041

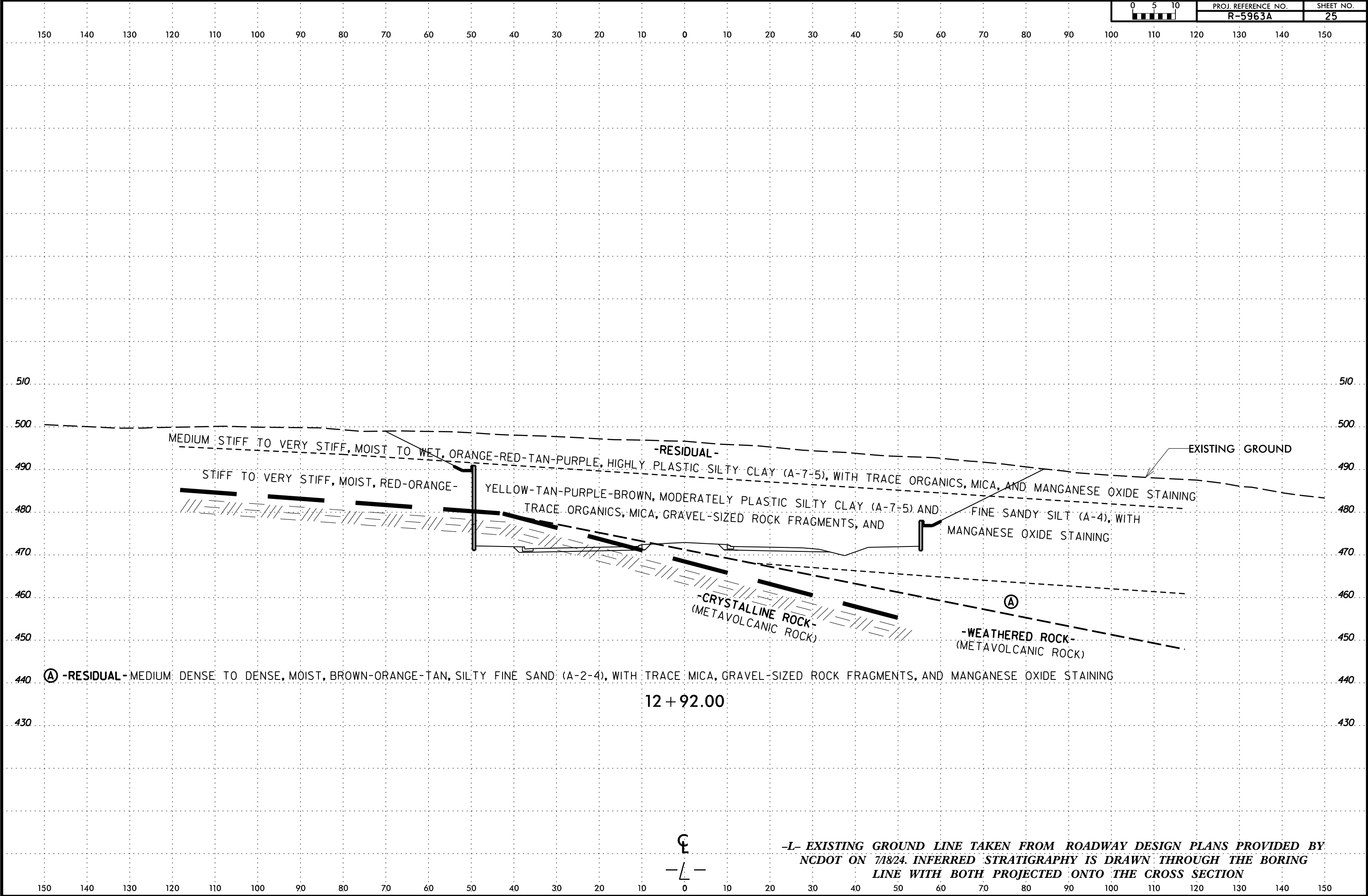


EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16

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0 5 10	PROJ. REFERENCE NO.	SHEET NO.
	R-5963A	25



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550
540
530
520
510
500
490
480
470
460
450
440

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-06	SS-14	4' RT	13+04 -W1-	6.0-7.5	A-7-6(11)	42	13	8.8%	17.2%	32.5%	41.5%	99.9%	95.8%	77.7%	25.3%	ND
RWAL B-20	SS-19	18' RT	13+06 -W2-	1.0-2.5	A-7-5(53)	81	50	3.0%	2.9%	14.7%	79.4%	96.3%	94.2%	91.5%	32.3%	ND
RWAL B-20	SS-23	18' RT	13+06 -W2-	13.5-15.0	A-7-6(11)	41	14	9.4%	15.0%	36.6%	39.0%	100.0%	94.9%	78.4%	34.2%	ND

RWAL B-06
13+04 -W1-
4 RT

SS-14

RWAL B-20
13+06 (-W2-)
18 RT

SS-19
SS-23

STIFF TO
STIFF TO VERY STIFF, MOIST,

11
14
22
16
22
100/0.3
60/0.0

REF
DRY
07/24

-RESIDUAL-
VERY STIFF, MOIST, RED, HIGHLY PLASTIC SILTY CLAY (A-7-5(53)),
RED-ORANGE-YELLOW-TAN, SLIGHTLY PLASTIC SILTY CLAY (A-7-6),

-CRYSTALLINE ROCK-
(METAVOLCANIC ROCK)

EXISTING GROUND
WITH TRACE ORGANICS
WITH TRACE ORGANICS
AND MANGANESE OXIDE STAINING

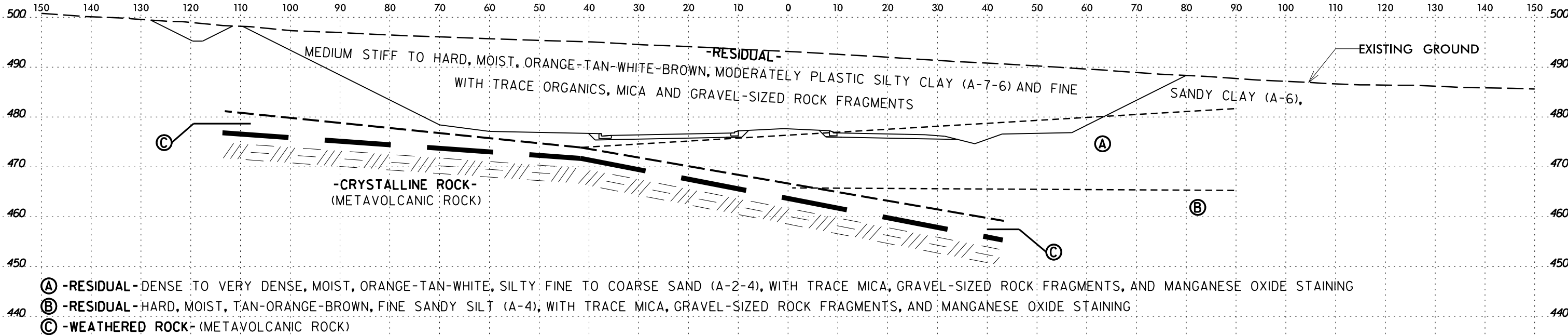
BT -WEATHERED ROCK-
(METAVOLCANIC ROCK)

- (A) -RESIDUAL- VERY DENSE, MOIST, YELLOW-ORANGE, SILTY FINE SAND (A-2-4), WITH TRACE GRAVEL-SIZED ROCK FRAGMENTS
(B) -WEATHERED ROCK- (METAVOLCANIC ROCK)

13 + 00.00

-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

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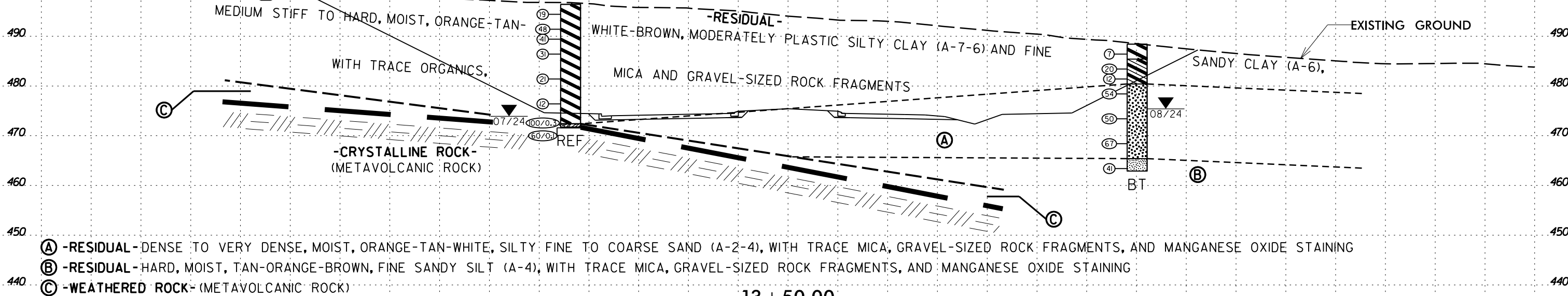
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SOIL TEST RESULTS

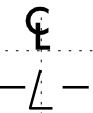
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								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL_B-07	SS-28	44' LT	13+51 -L-	1.0-2.5	A-7-6(11)	41	19	11.3%	16.8%	32.9%	39.0%	85.8%	79.9%	65.0%	20.2%	ND
RWAL_B-07	SS-33	44' LT	13+51 -L-	19.0-20.5	A-7-6(18)	46	18	2.3%	10.3%	46.4%	41.0%	98.3%	96.8%	89.3%	39.1%	ND

RWAL_B-07
13+51
44' LT
SS-28
SS-33

RWAL_B-21
13+45 (-W2-)
15' RT



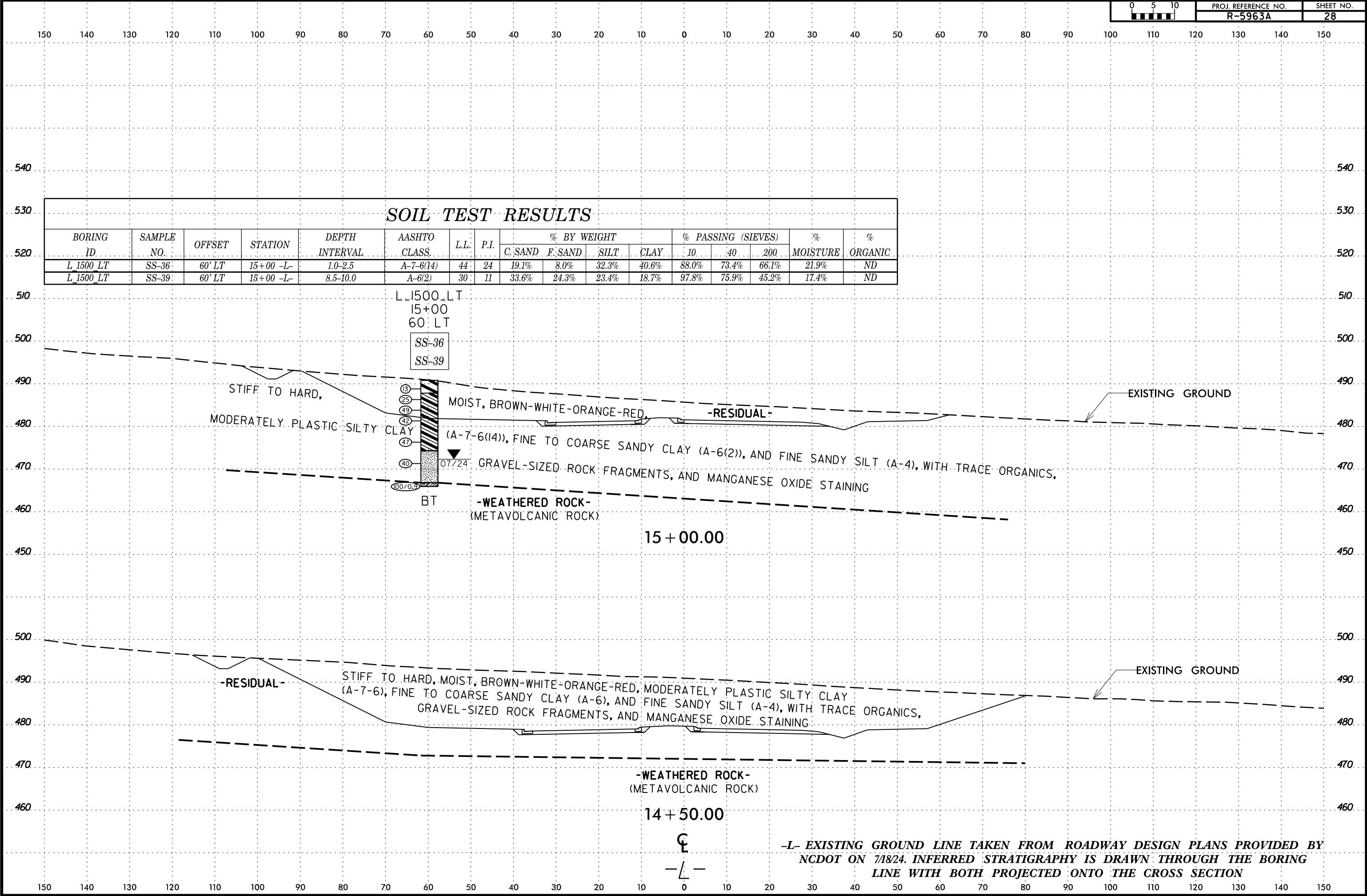
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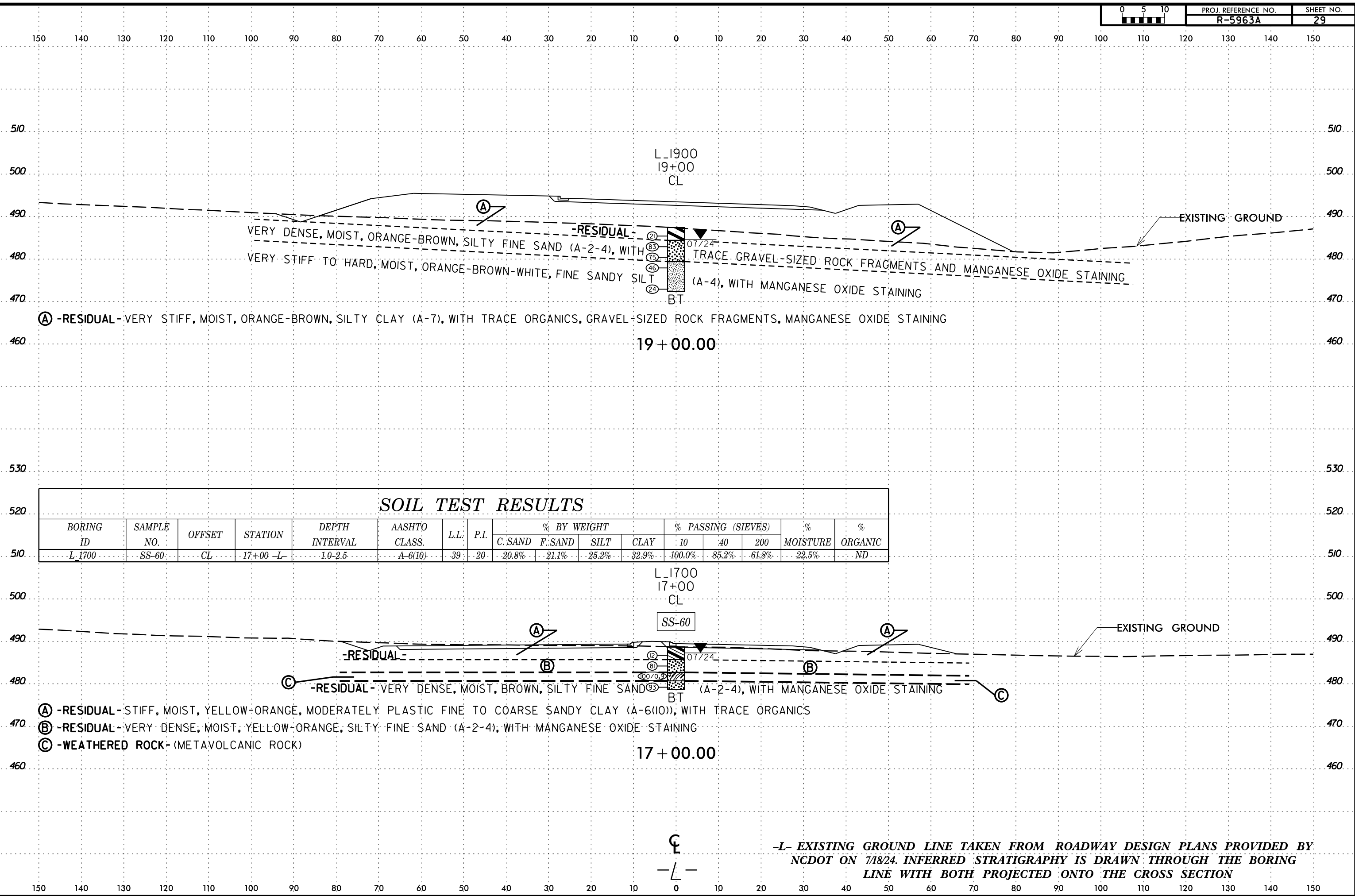


-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

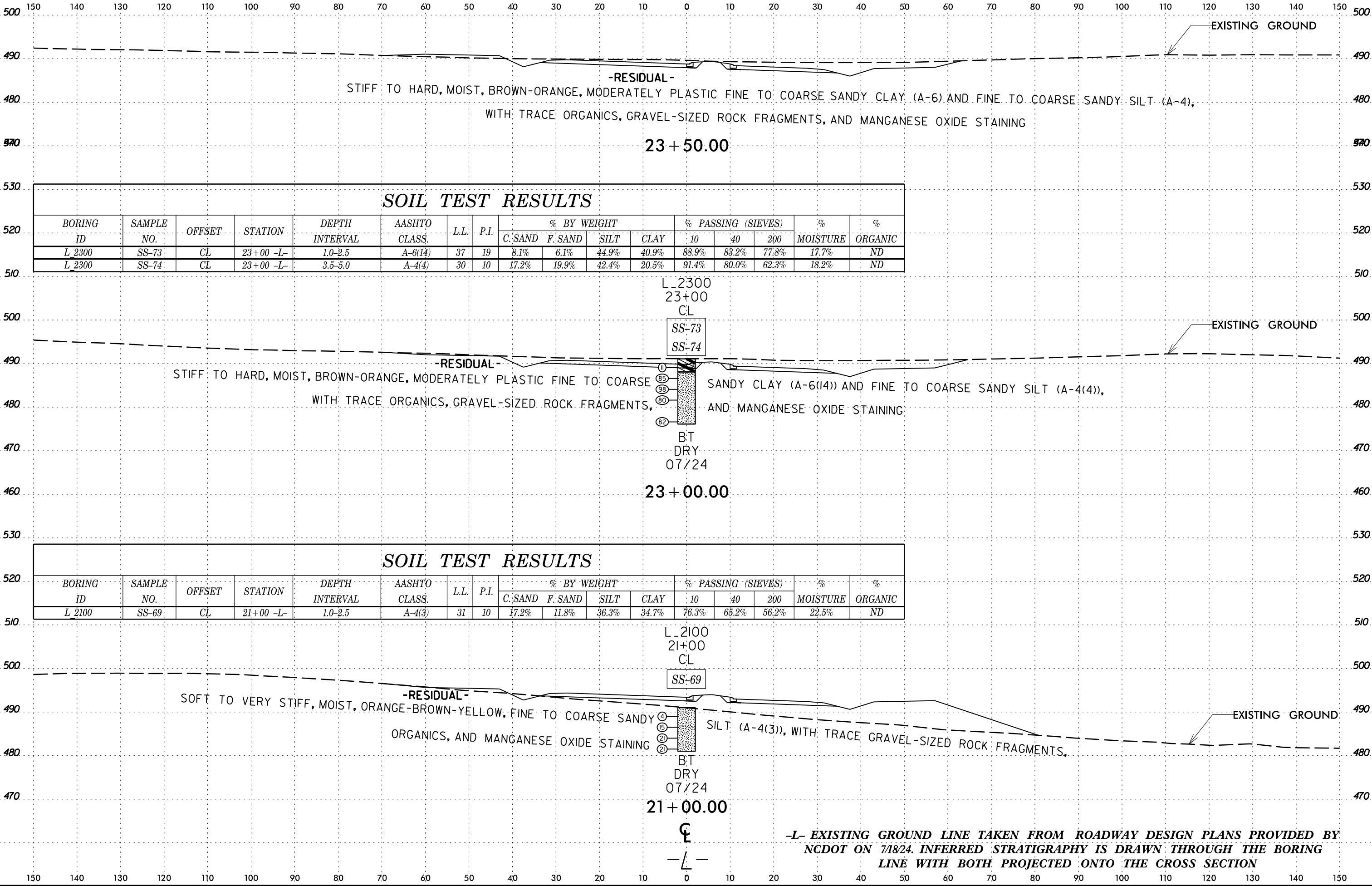
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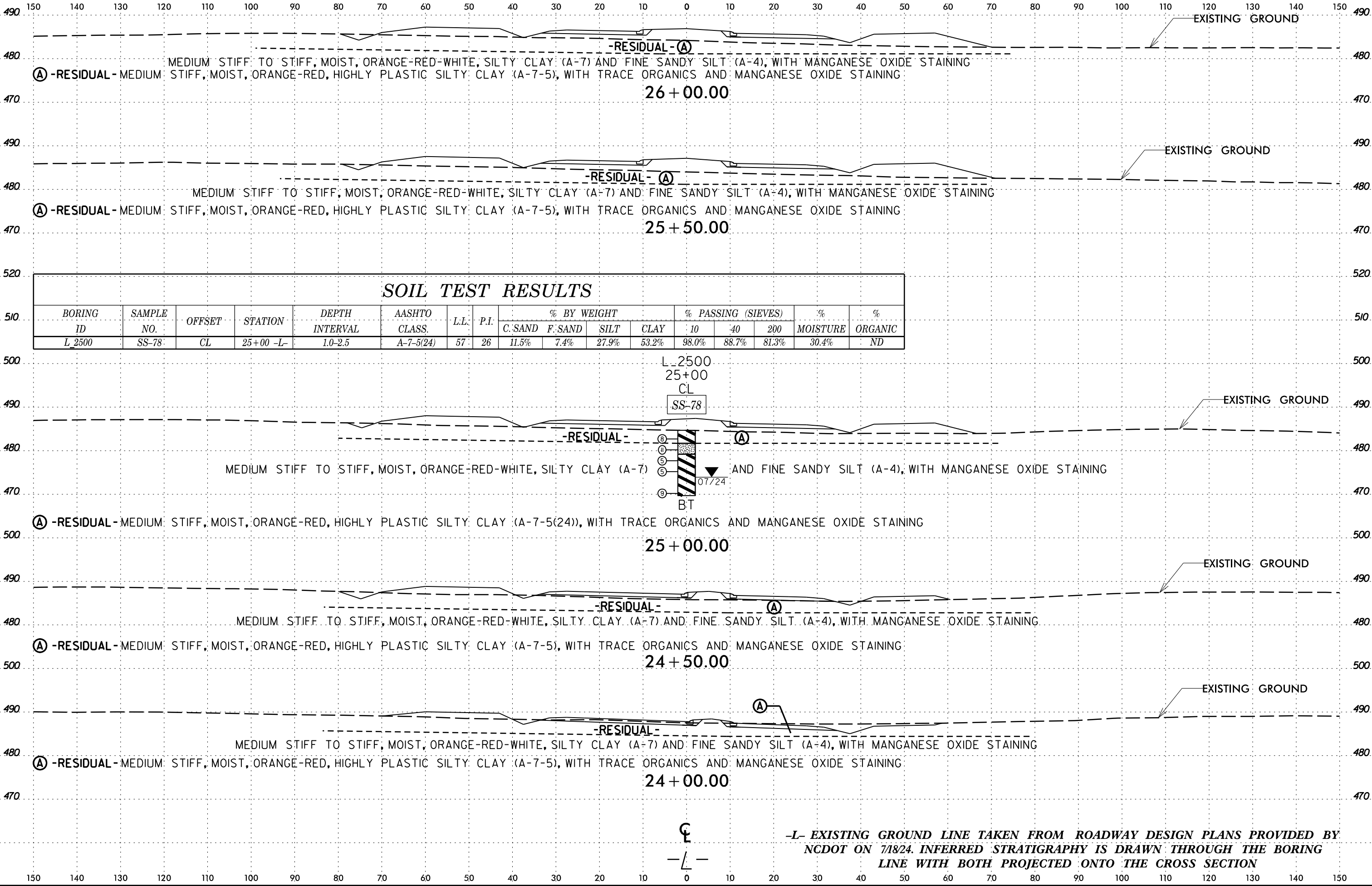


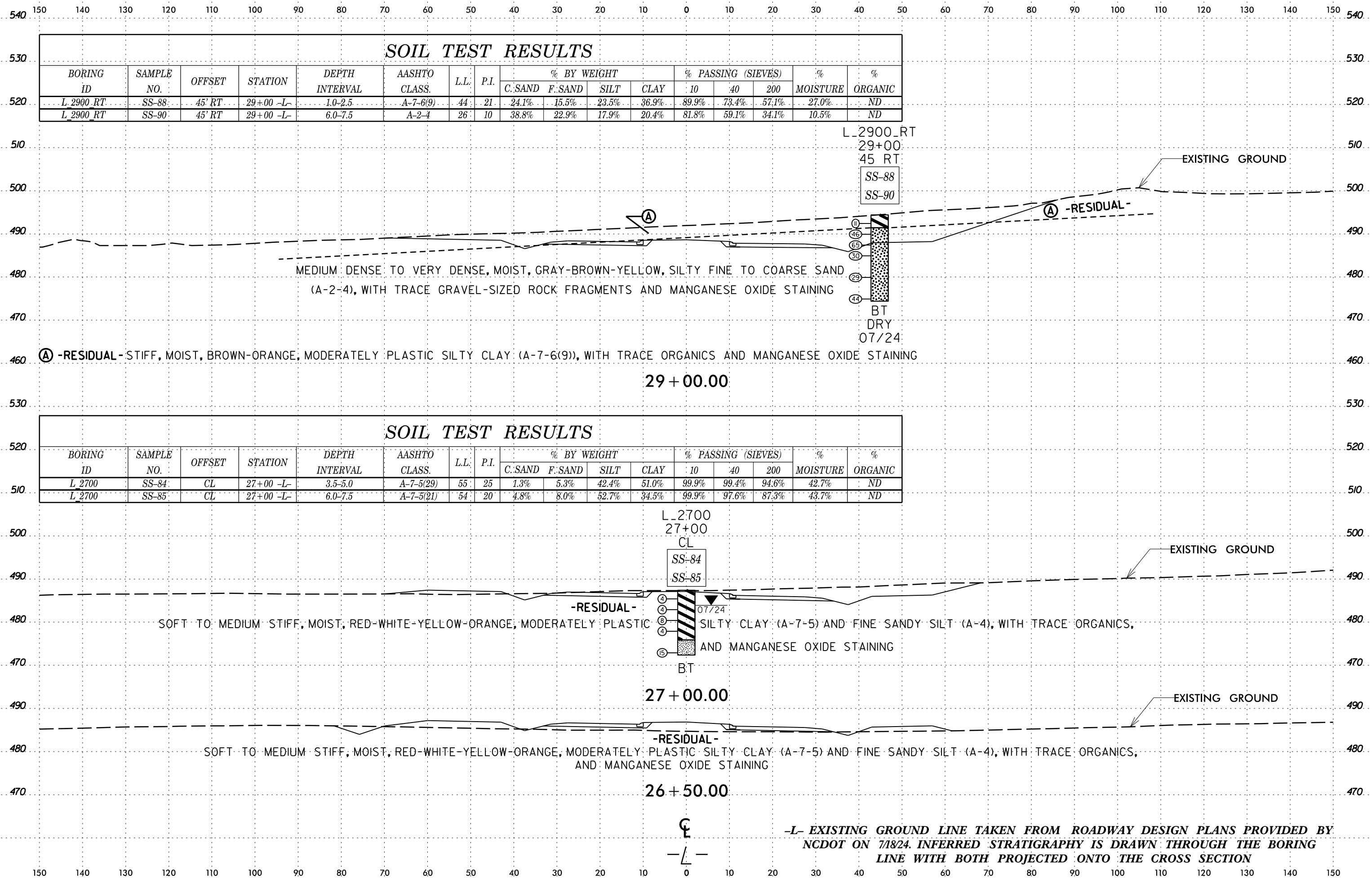
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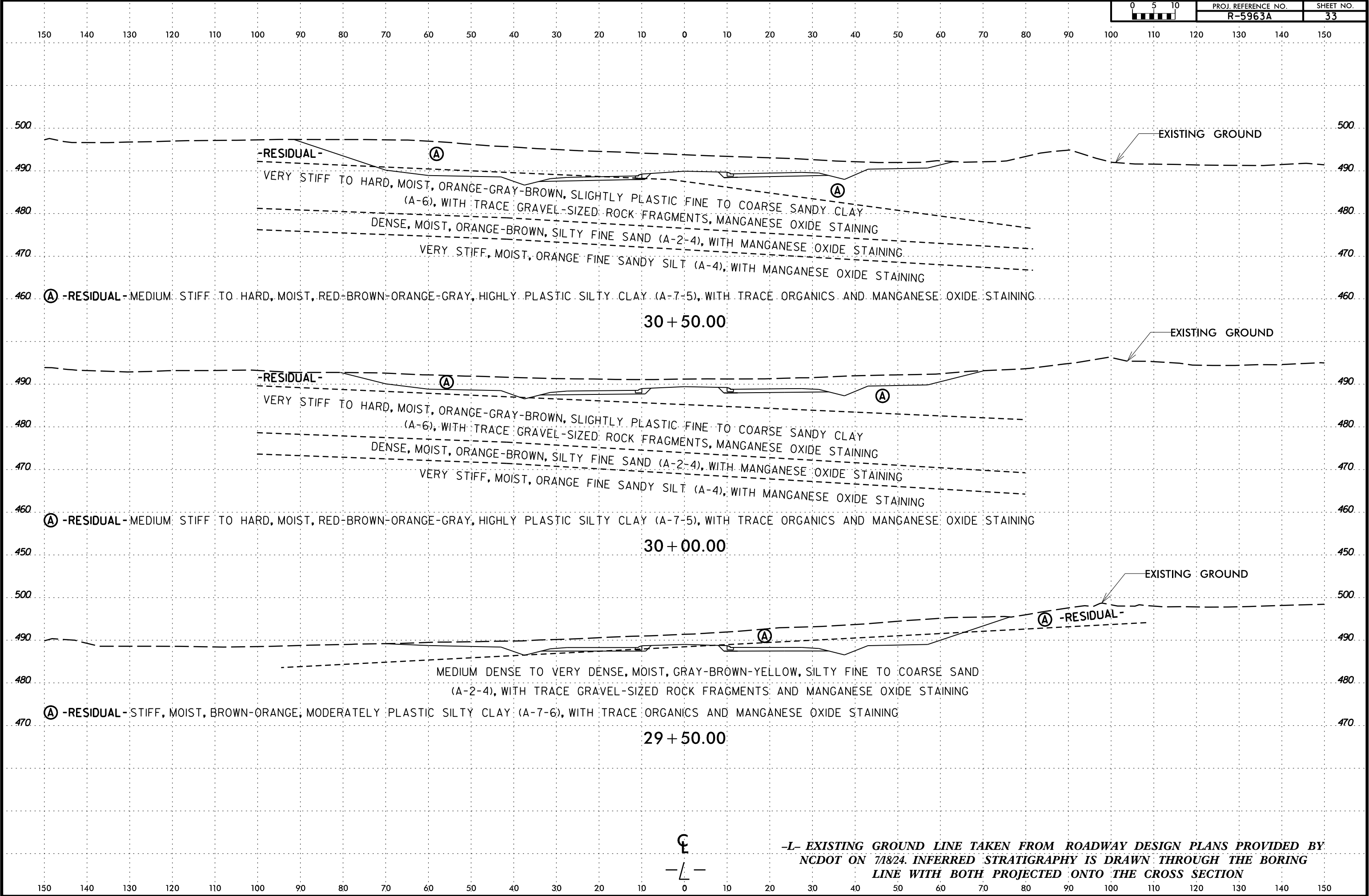
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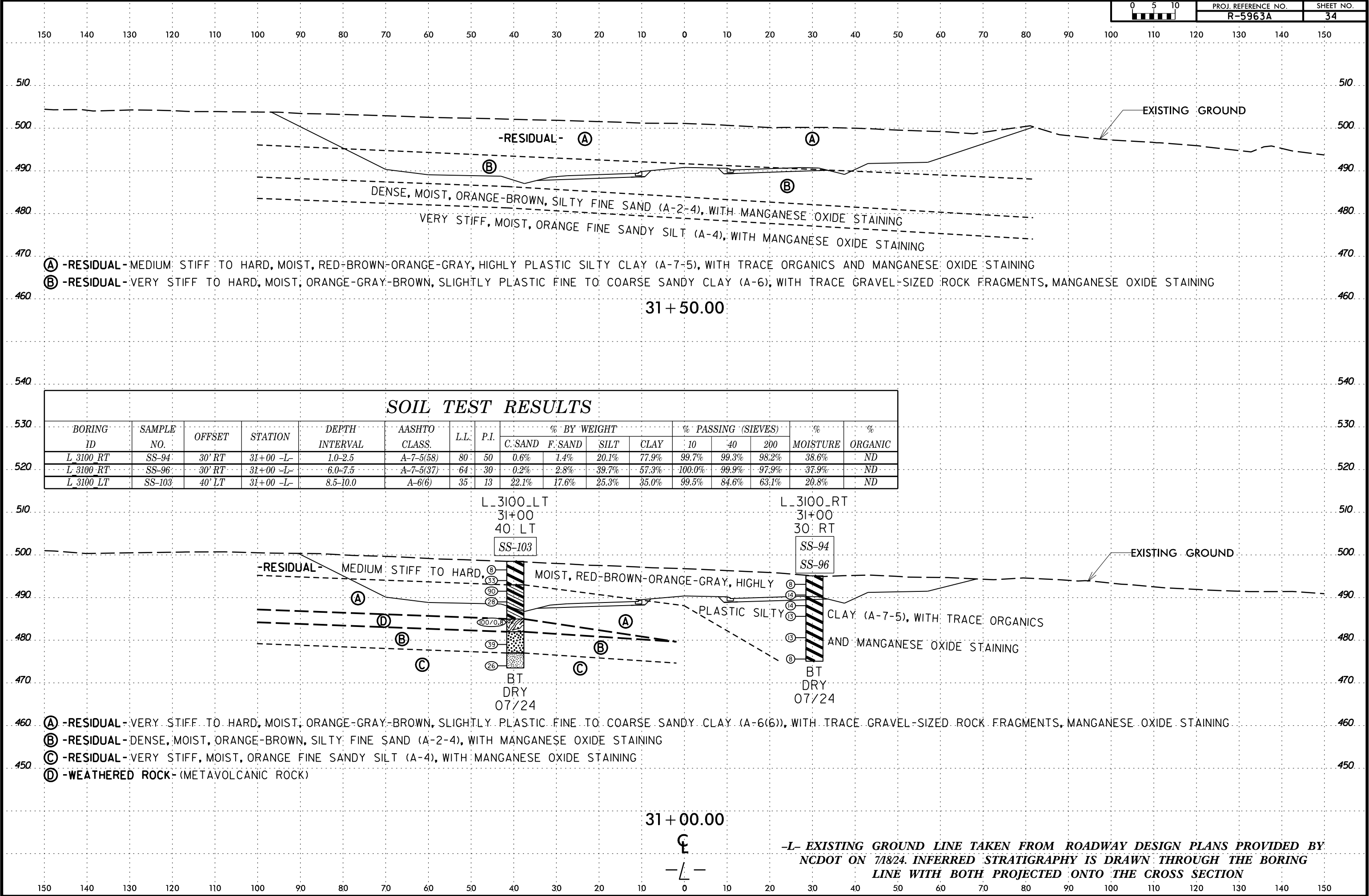


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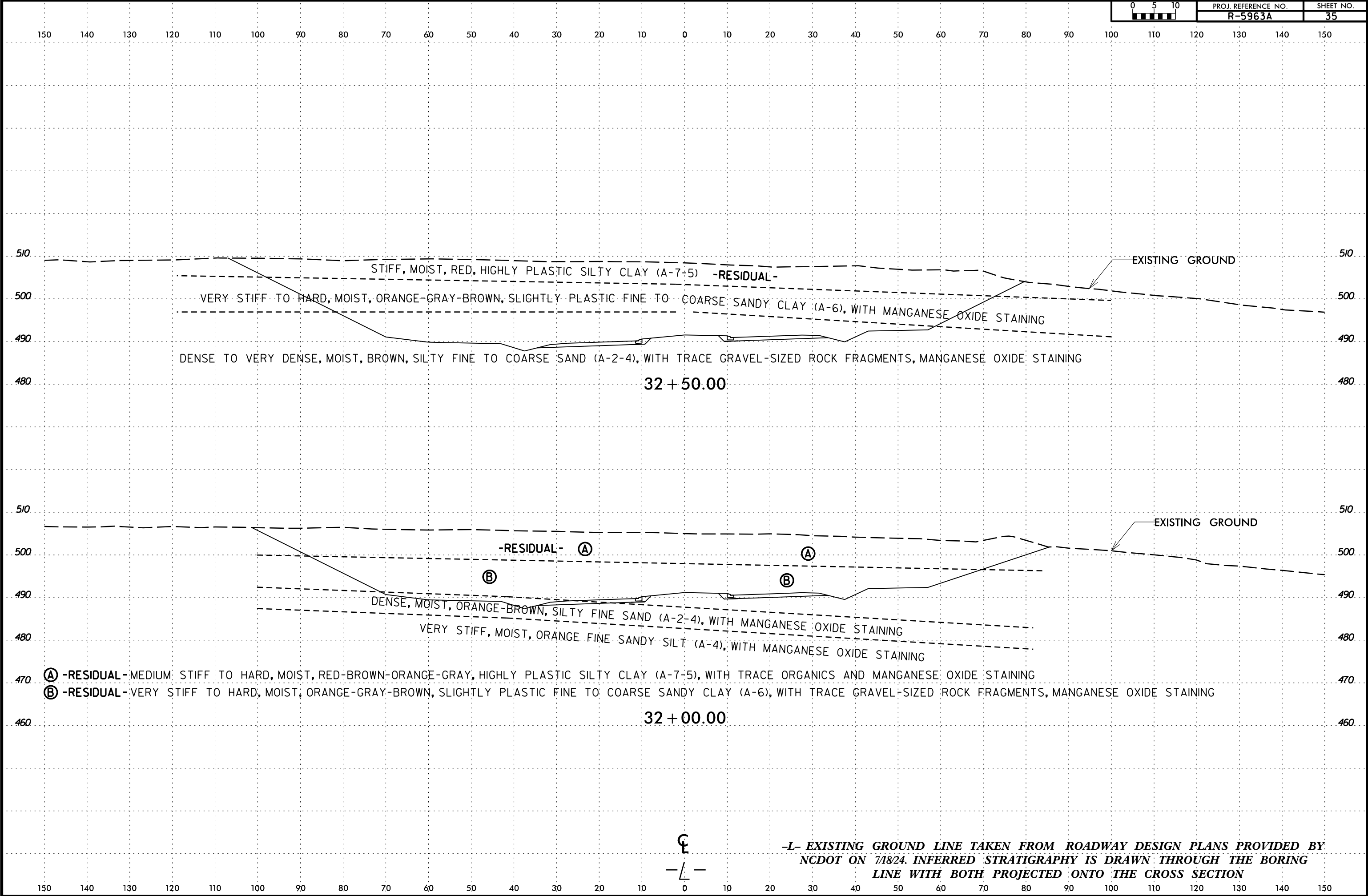


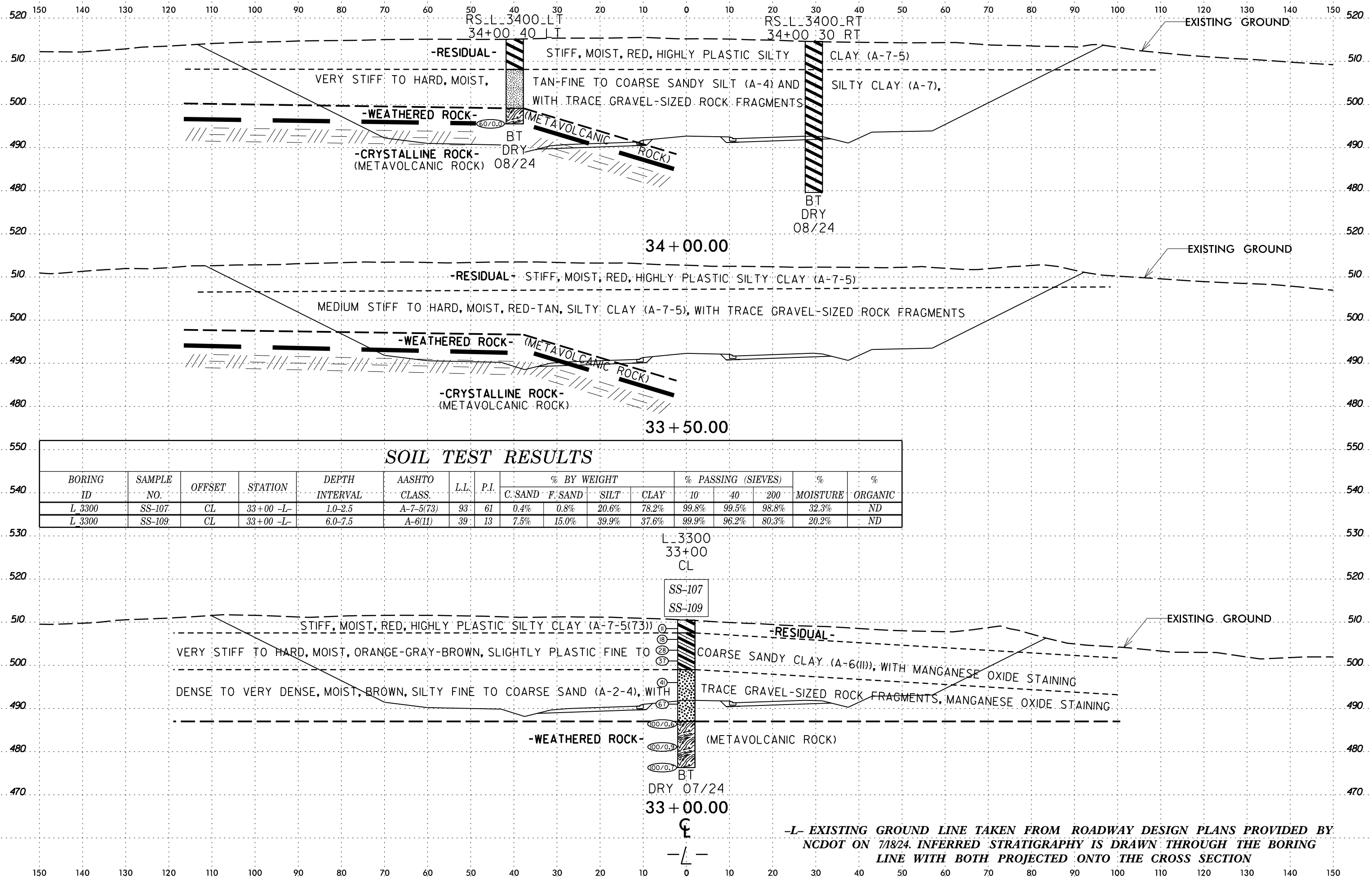
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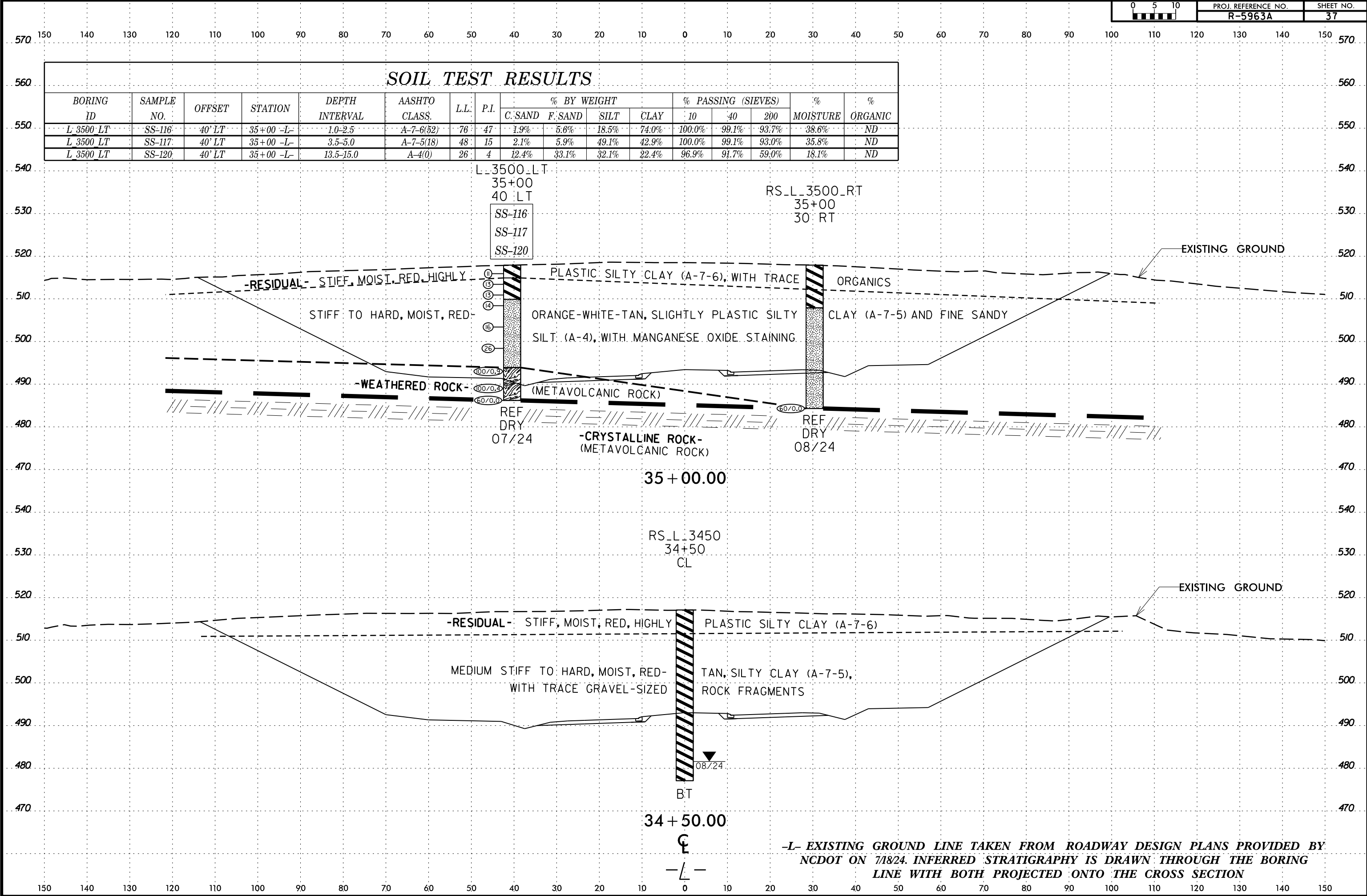
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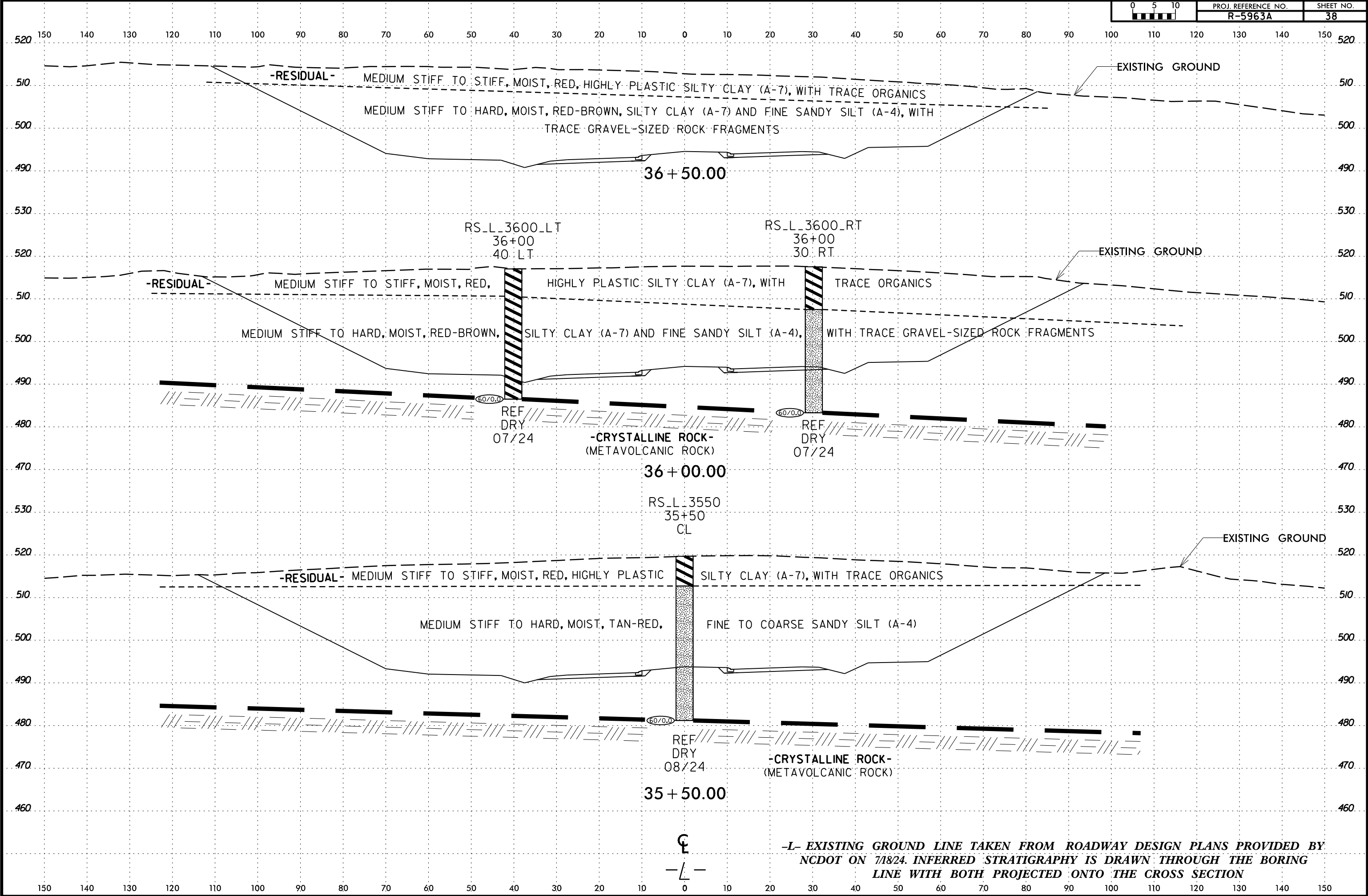


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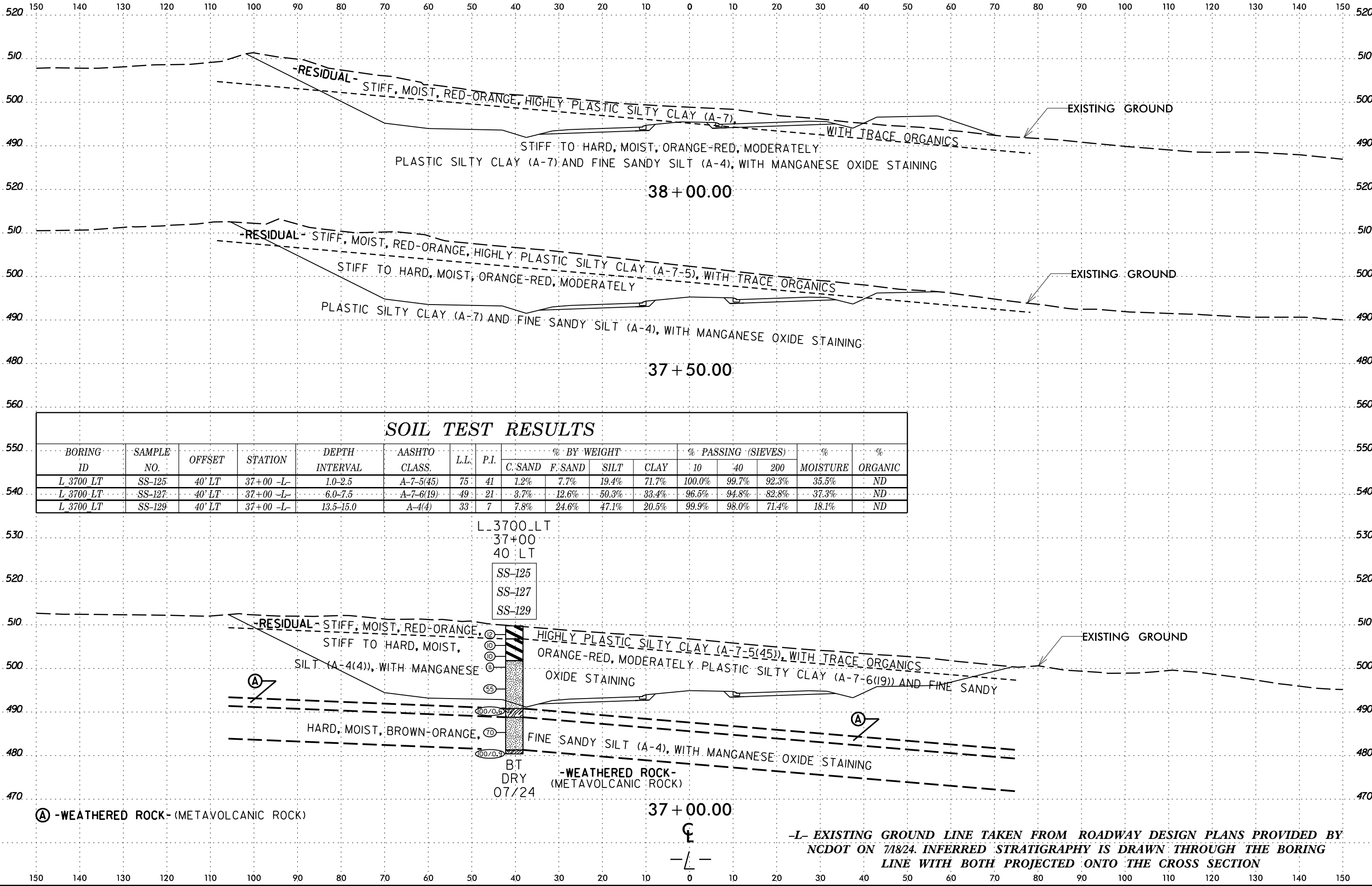
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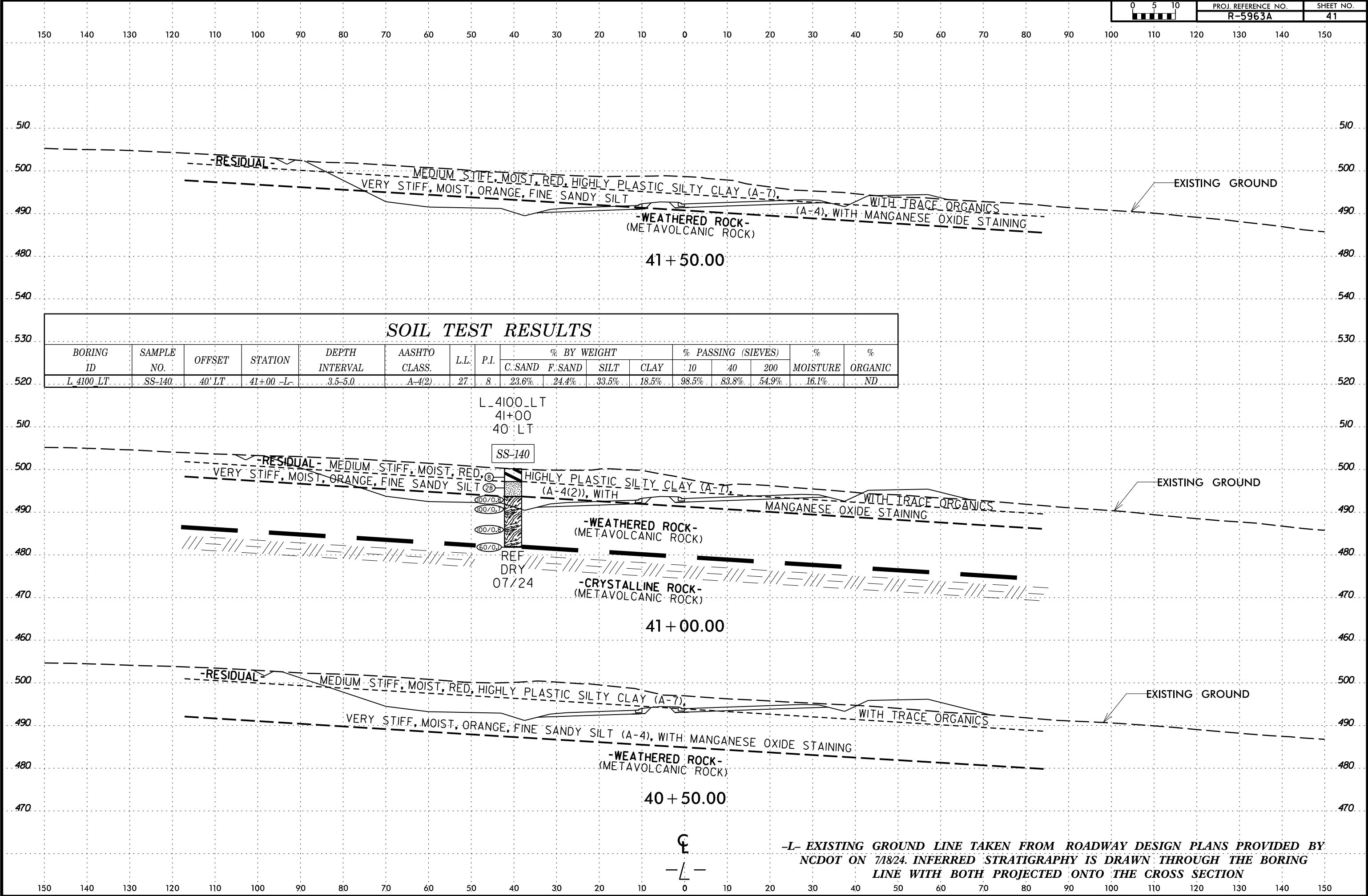
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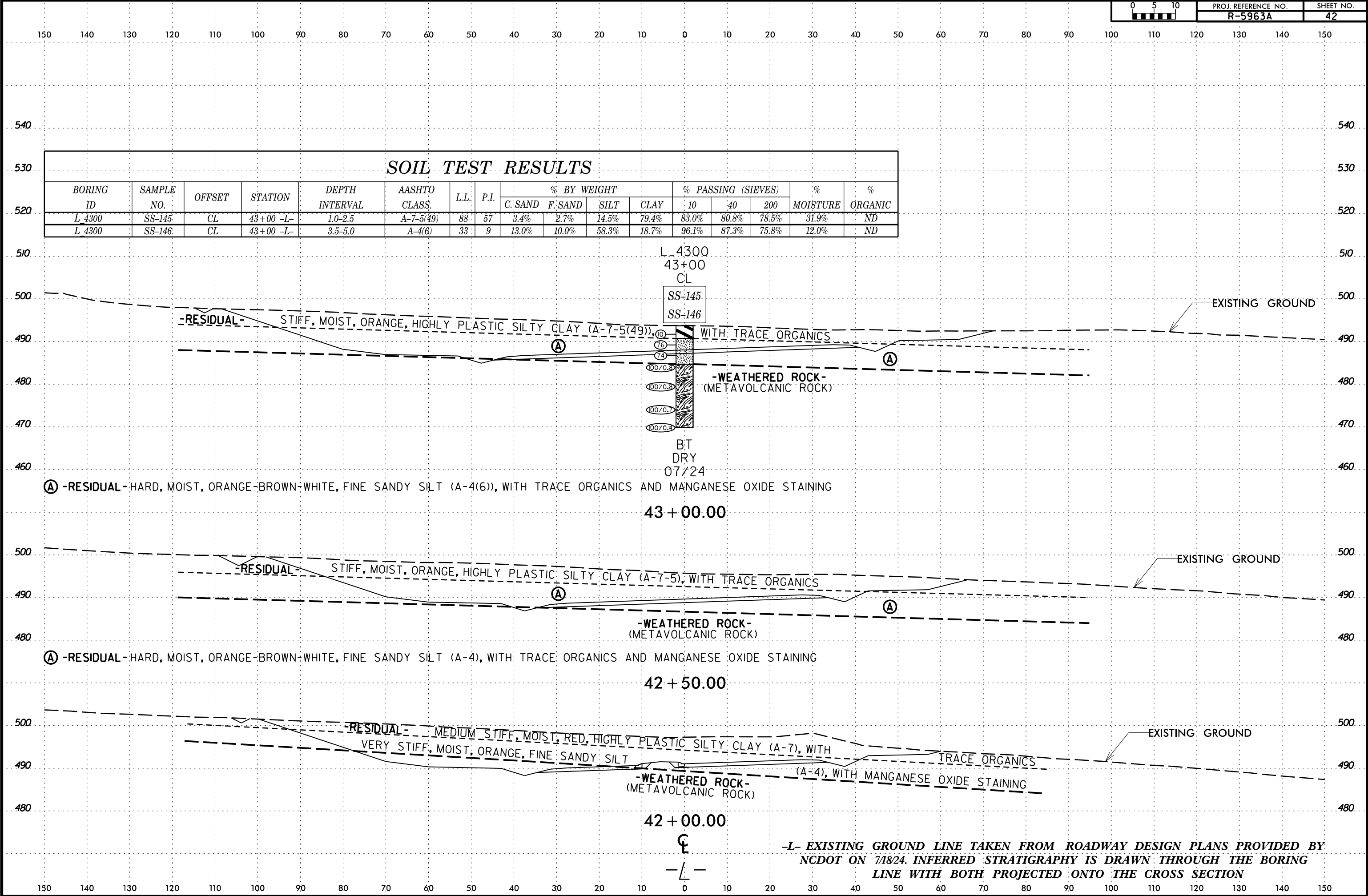
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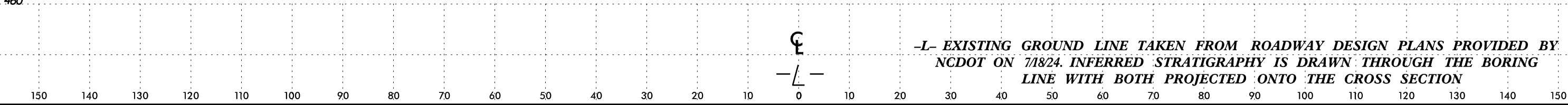
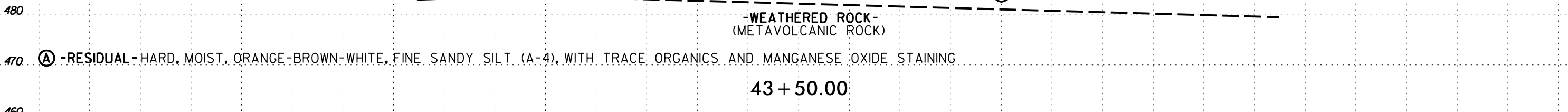
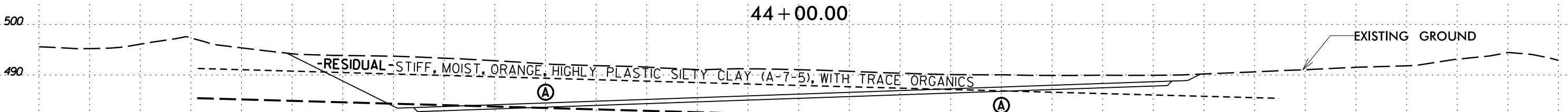
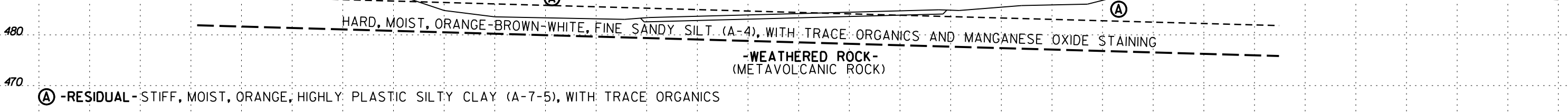
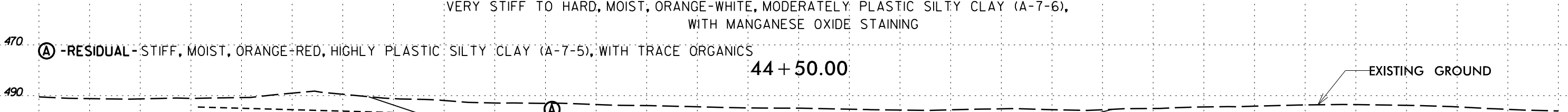
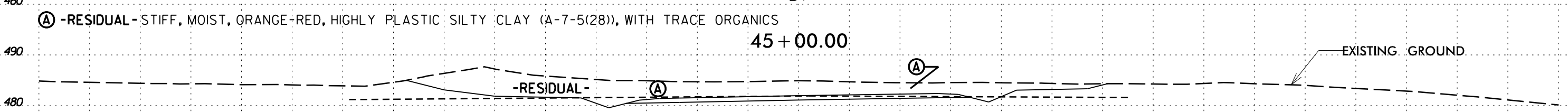
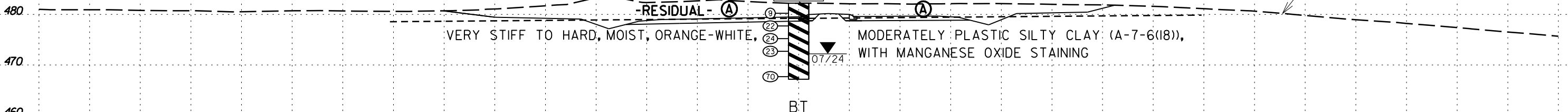
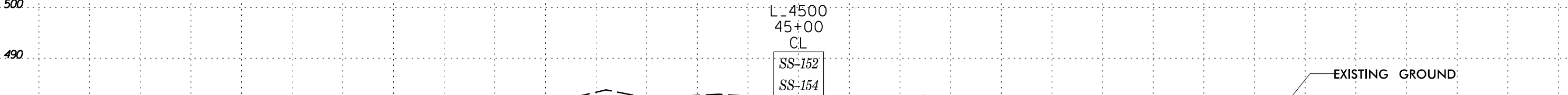


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R-5963A	43

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520 520

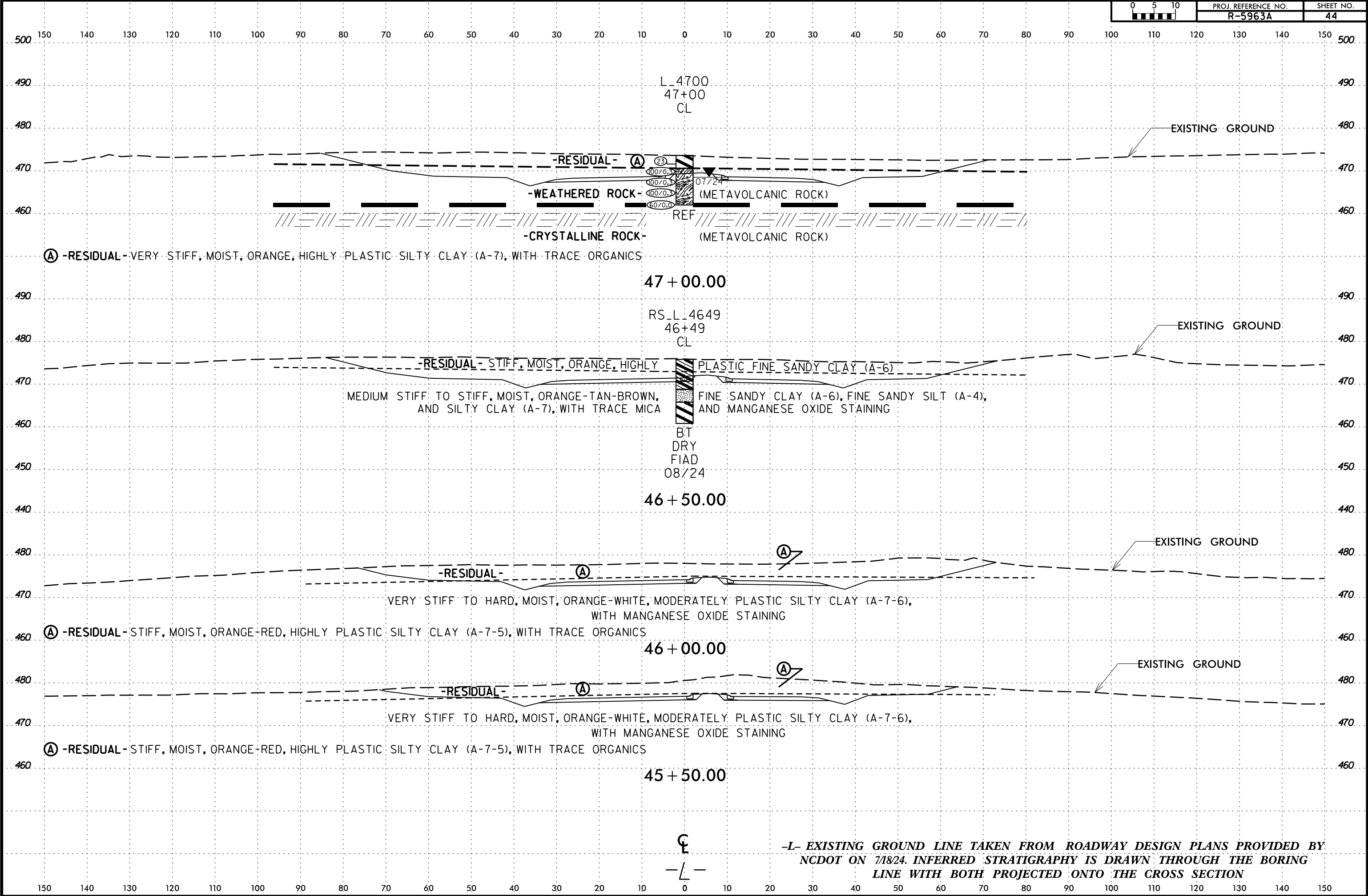
SOIL TEST RESULTS																
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								C. SAND	F. SAND	SILT	CLAY	10	40	200		
L 4500	SS-152	CL	45+00 -L-	1.0-2.5	A-7-5(28)	71	38	1.0%	1.8%	16.1%	81.1%	71.9%	71.3%	70.2%	30.7%	ND
L 4500	SS-154	CL	45+00 -L-	6.0-7.5	A-7-6(18)	43	19	2.8%	15.7%	46.3%	35.2%	100.0%	98.6%	88.0%	24.9%	ND

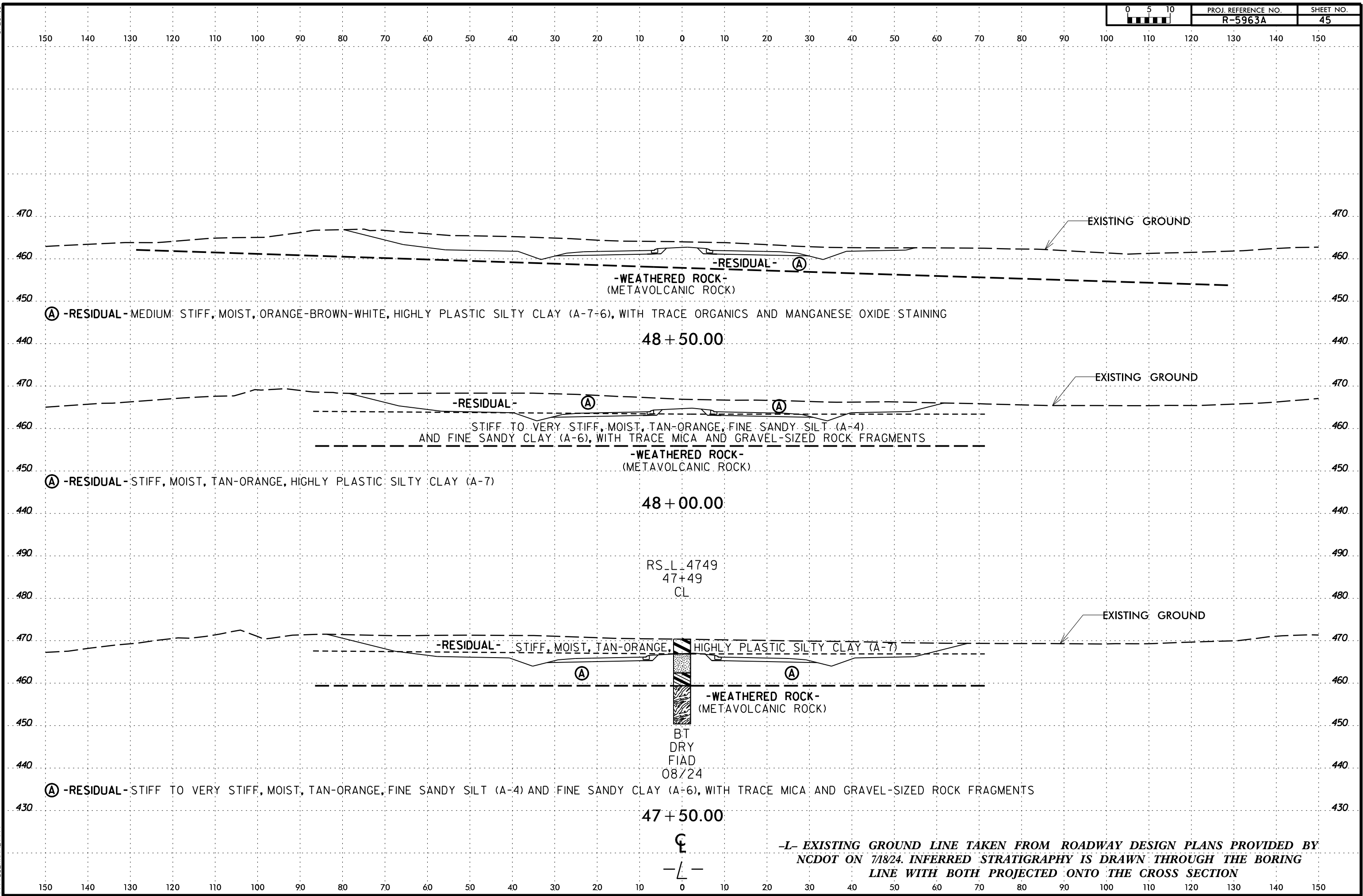


-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

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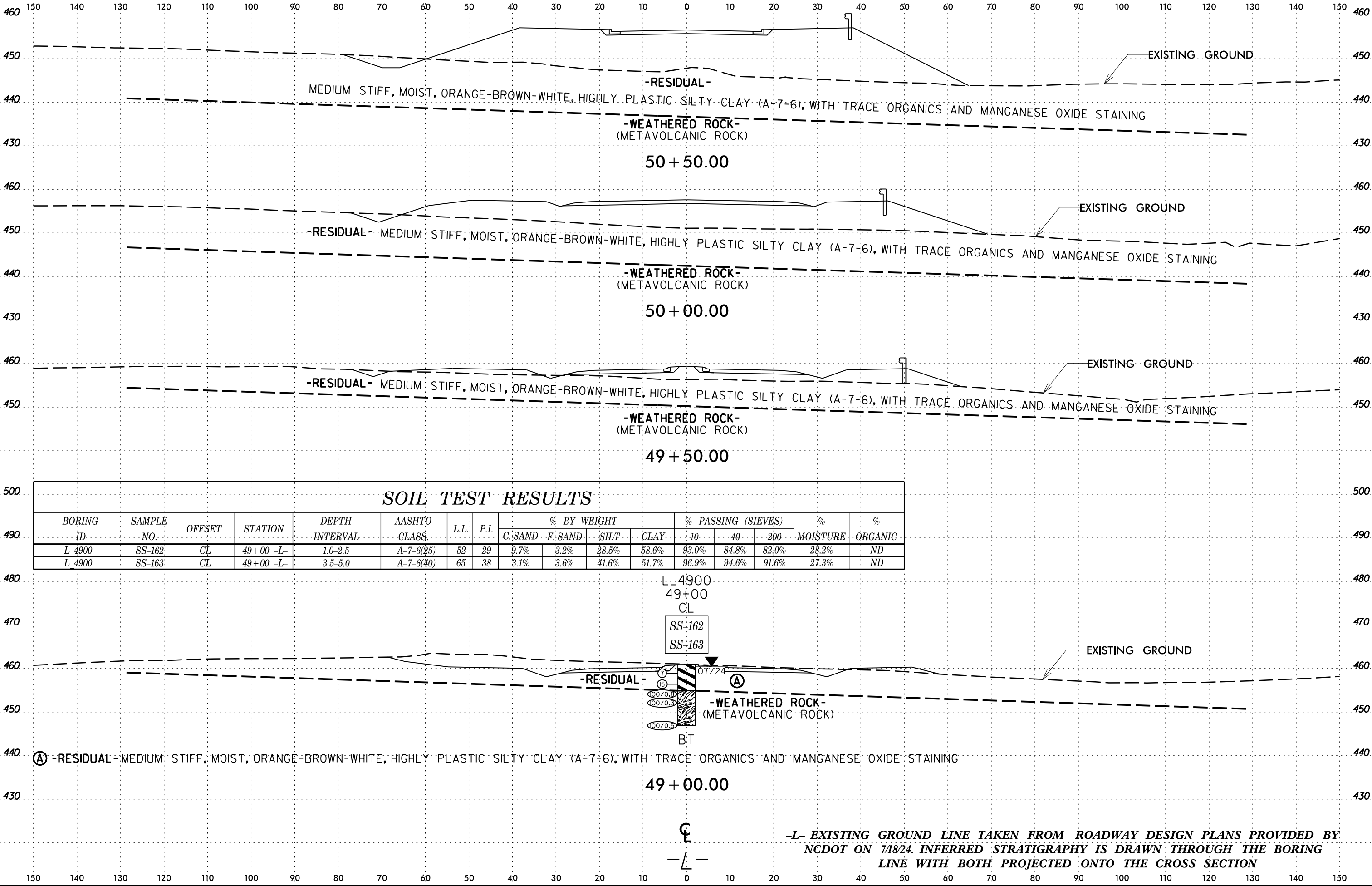
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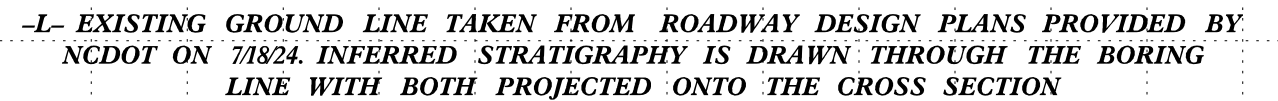


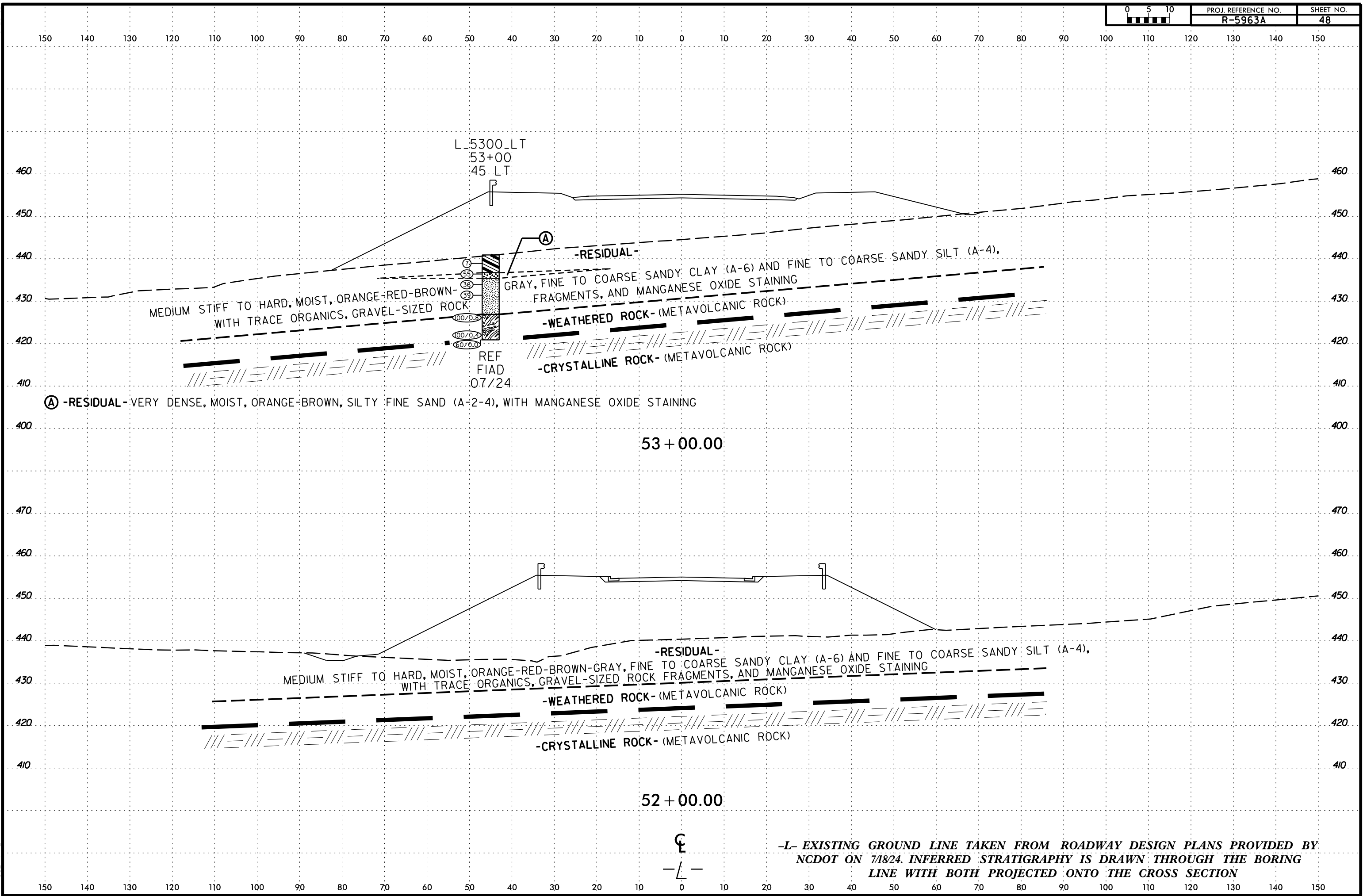


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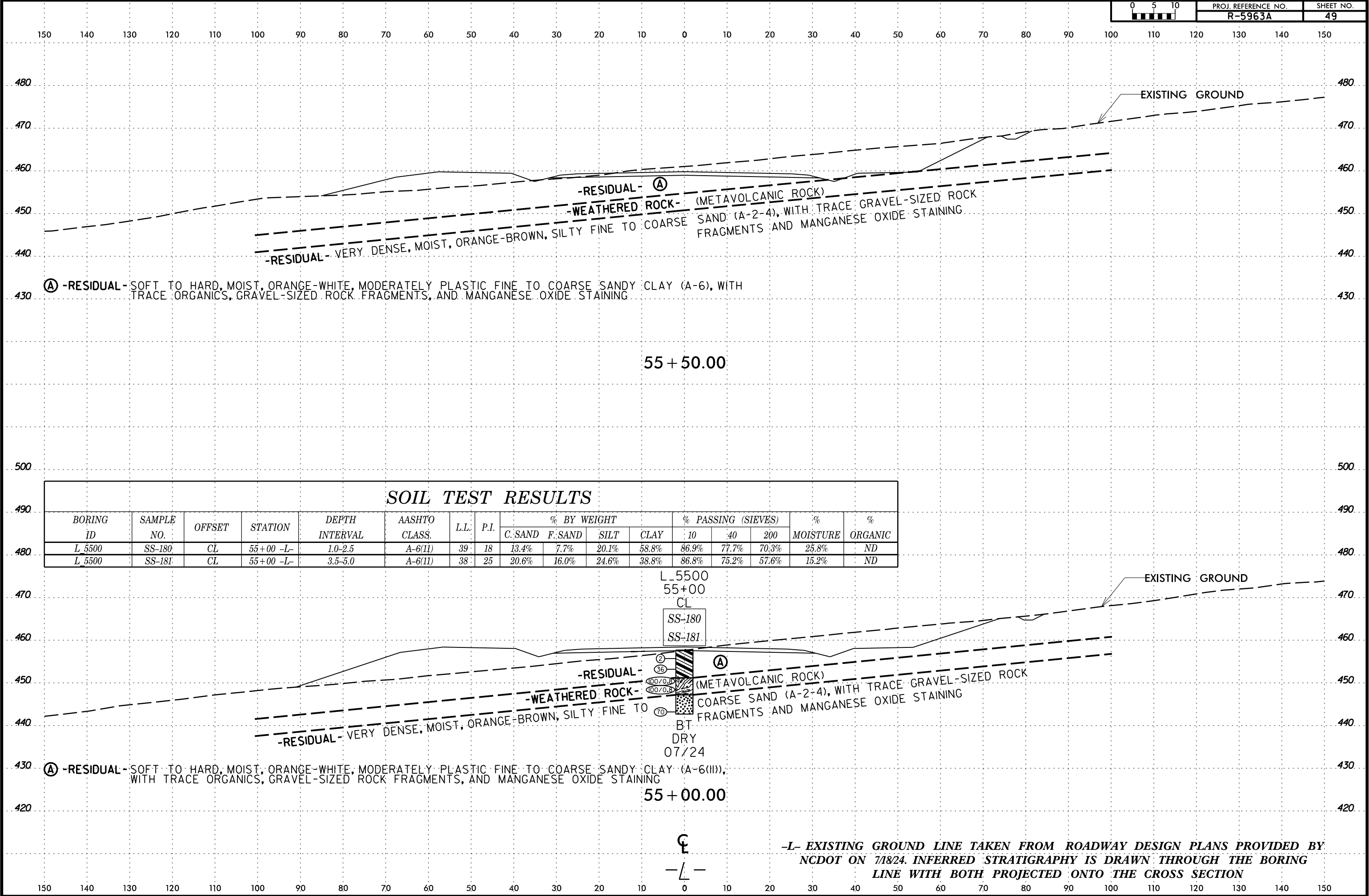
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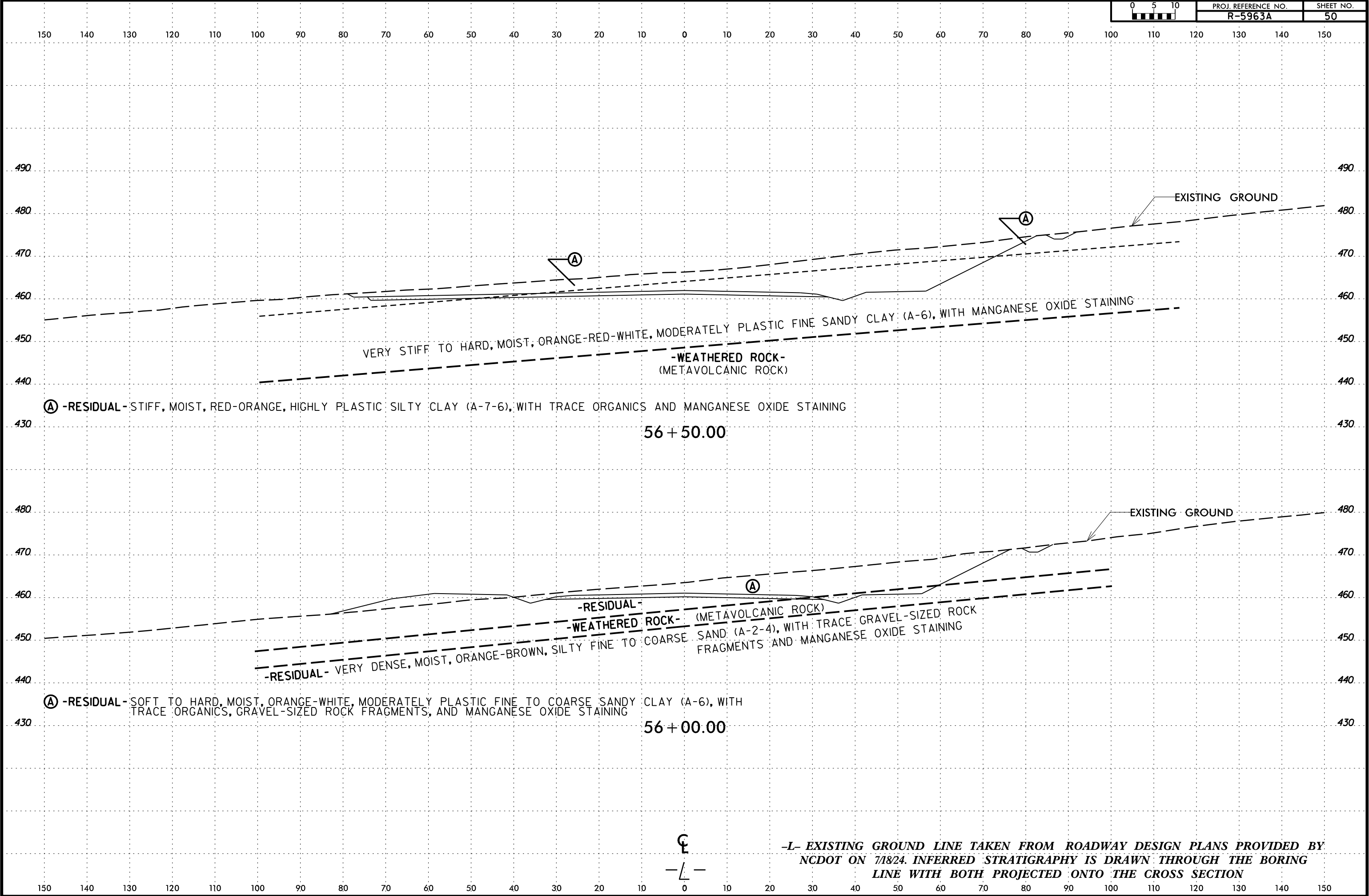
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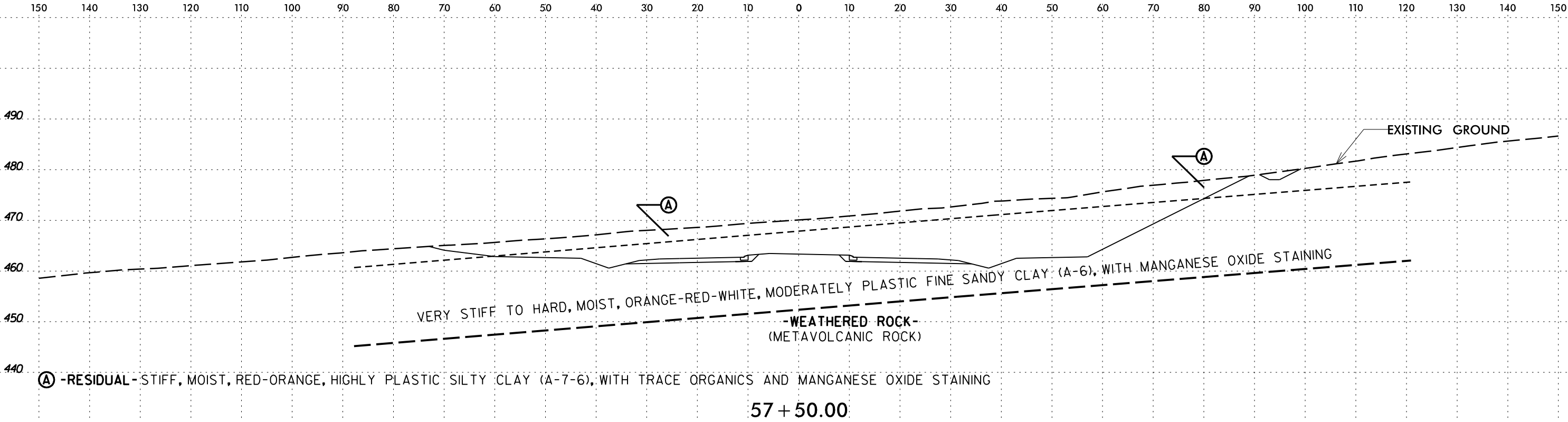
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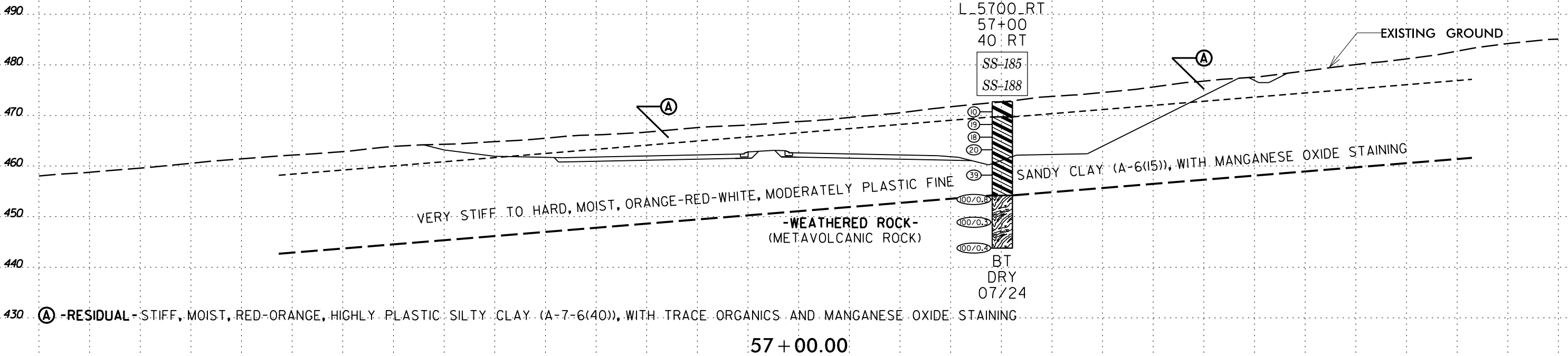


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SOIL TEST RESULTS

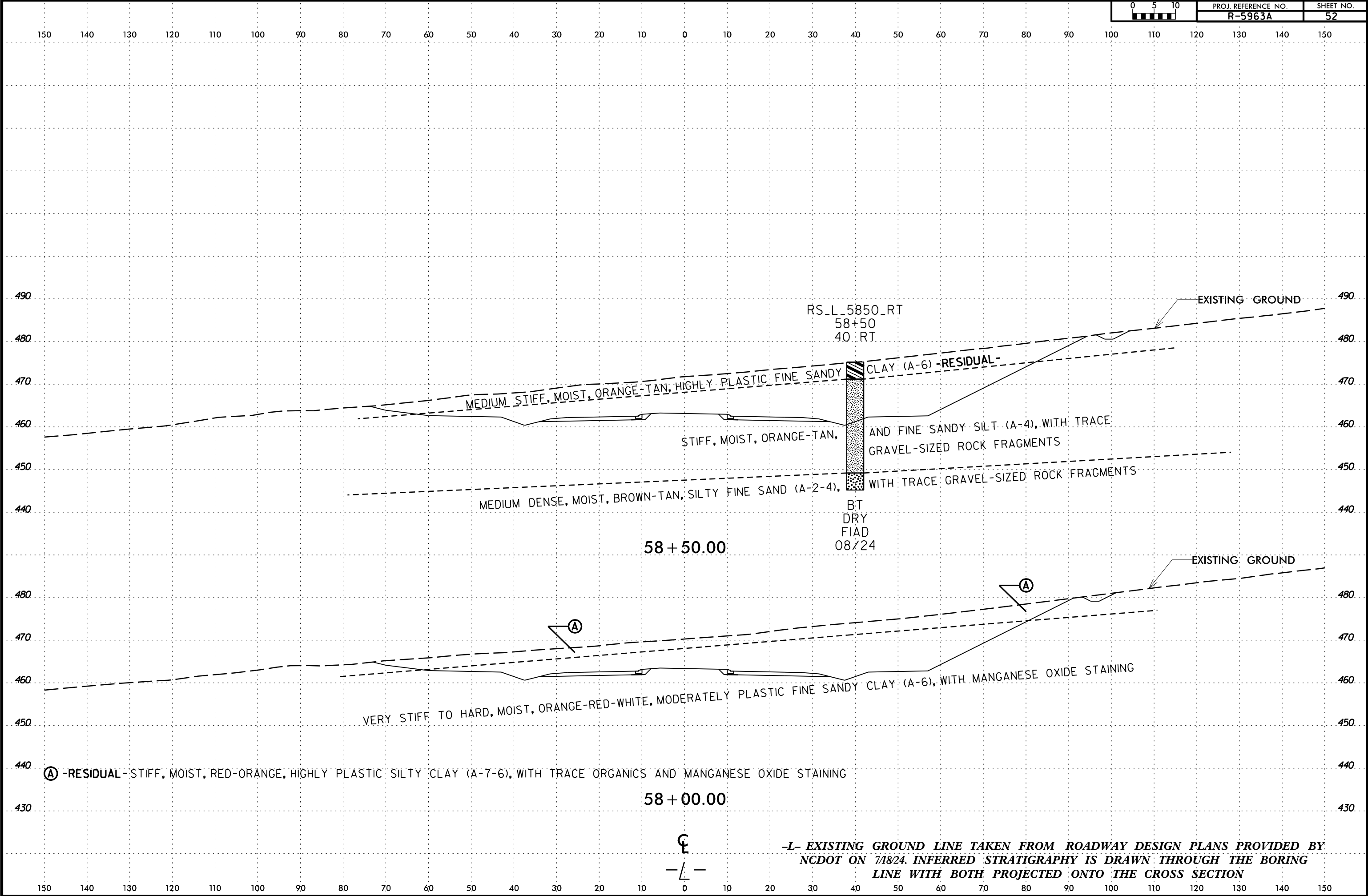
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								C. SAND	F. SAND	SILT	CLAY	10	40	200		
L 5700 RT	SS-185	40' RT	57+00 -L-	1.0-2.5	A-7-6(40)	67	40	0.4%	13.1%	25.0%	61.5%	96.8%	96.6%	88.4%	25.6%	ND
L 5700 RT	SS-188	40' RT	57+00 -L-	8.5-10.0	A-6(15)	40	21	1.5%	9.3%	32.3%	56.9%	82.2%	81.7%	76.4%	24.5%	ND



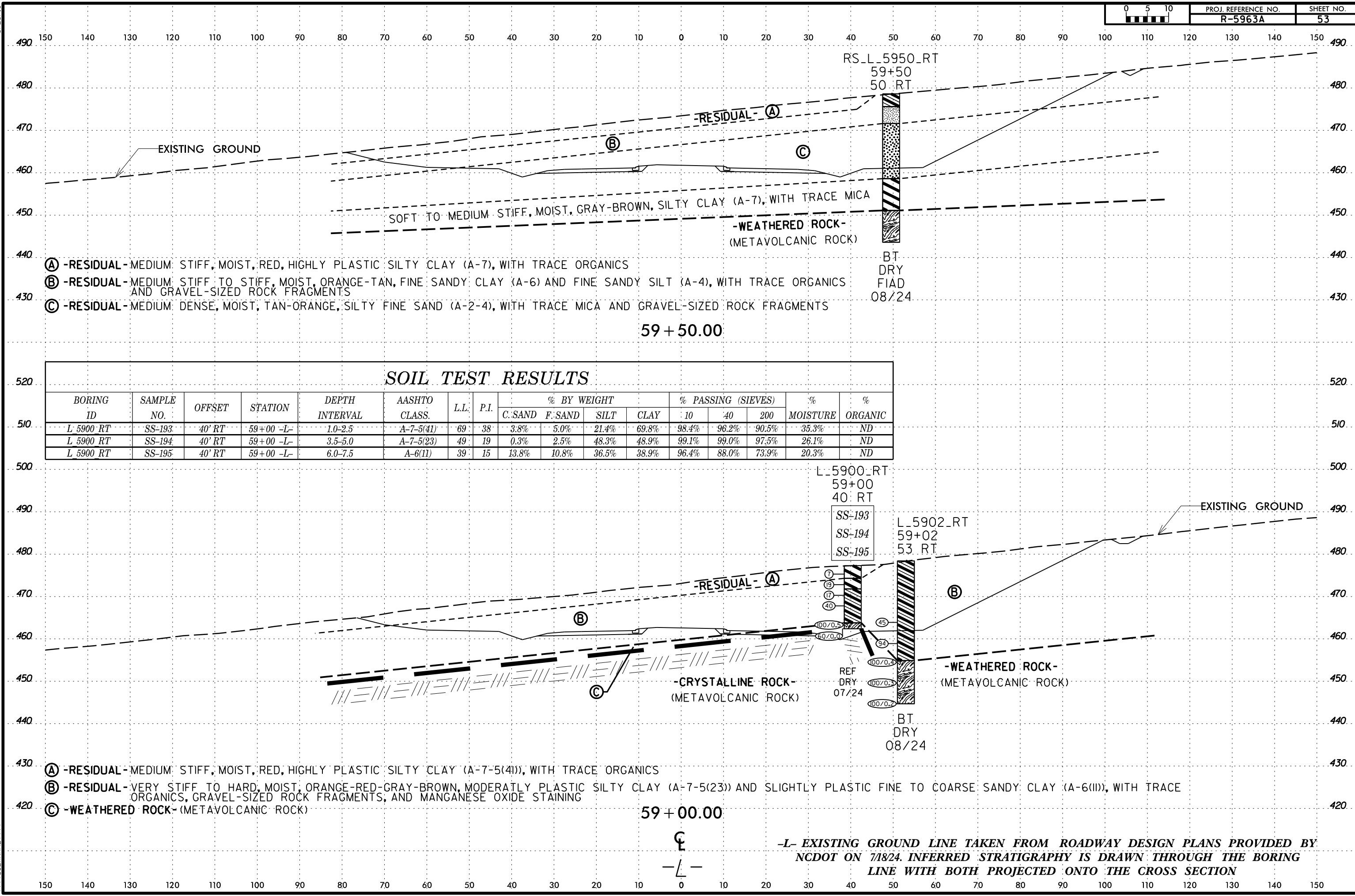
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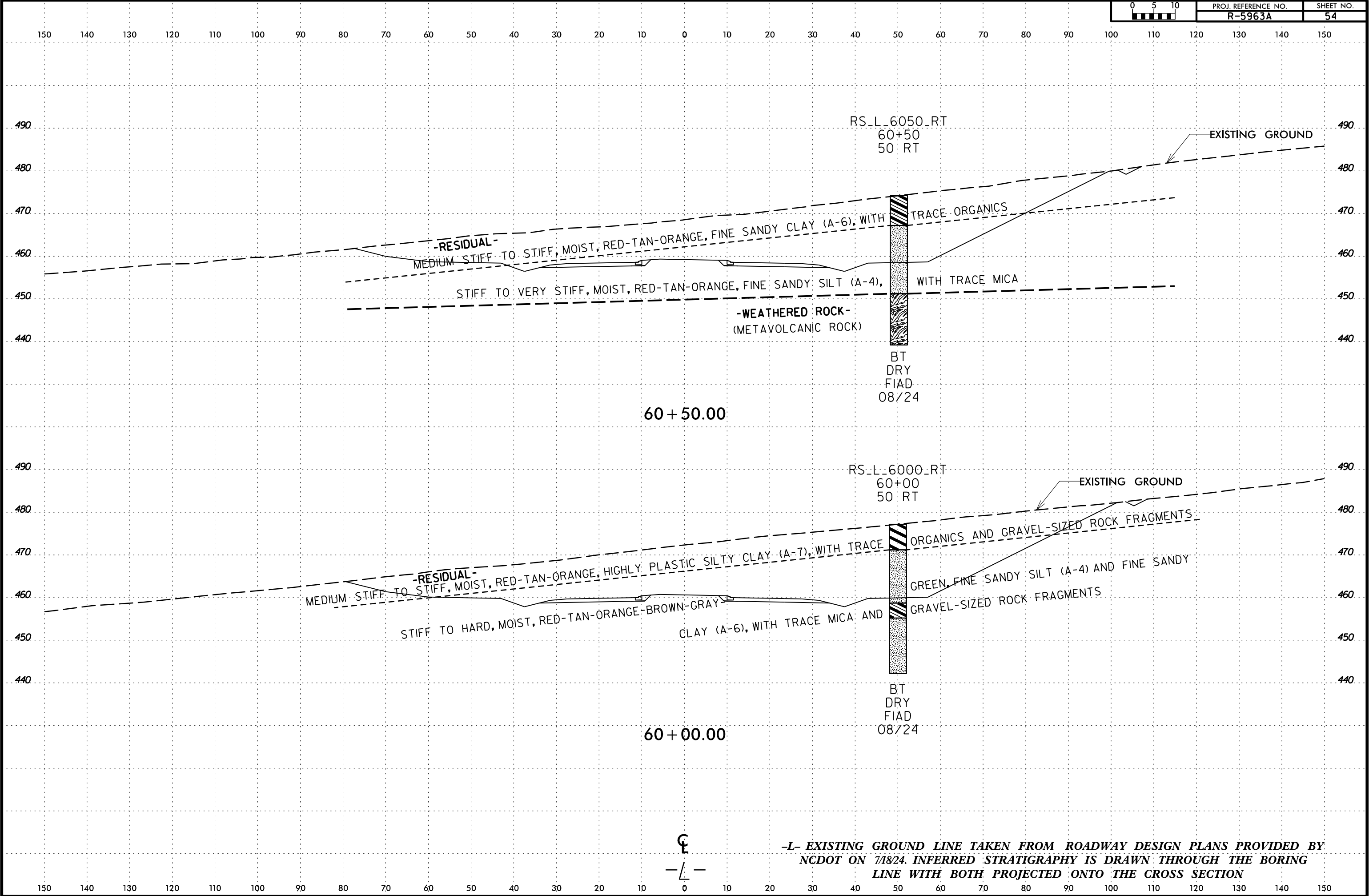


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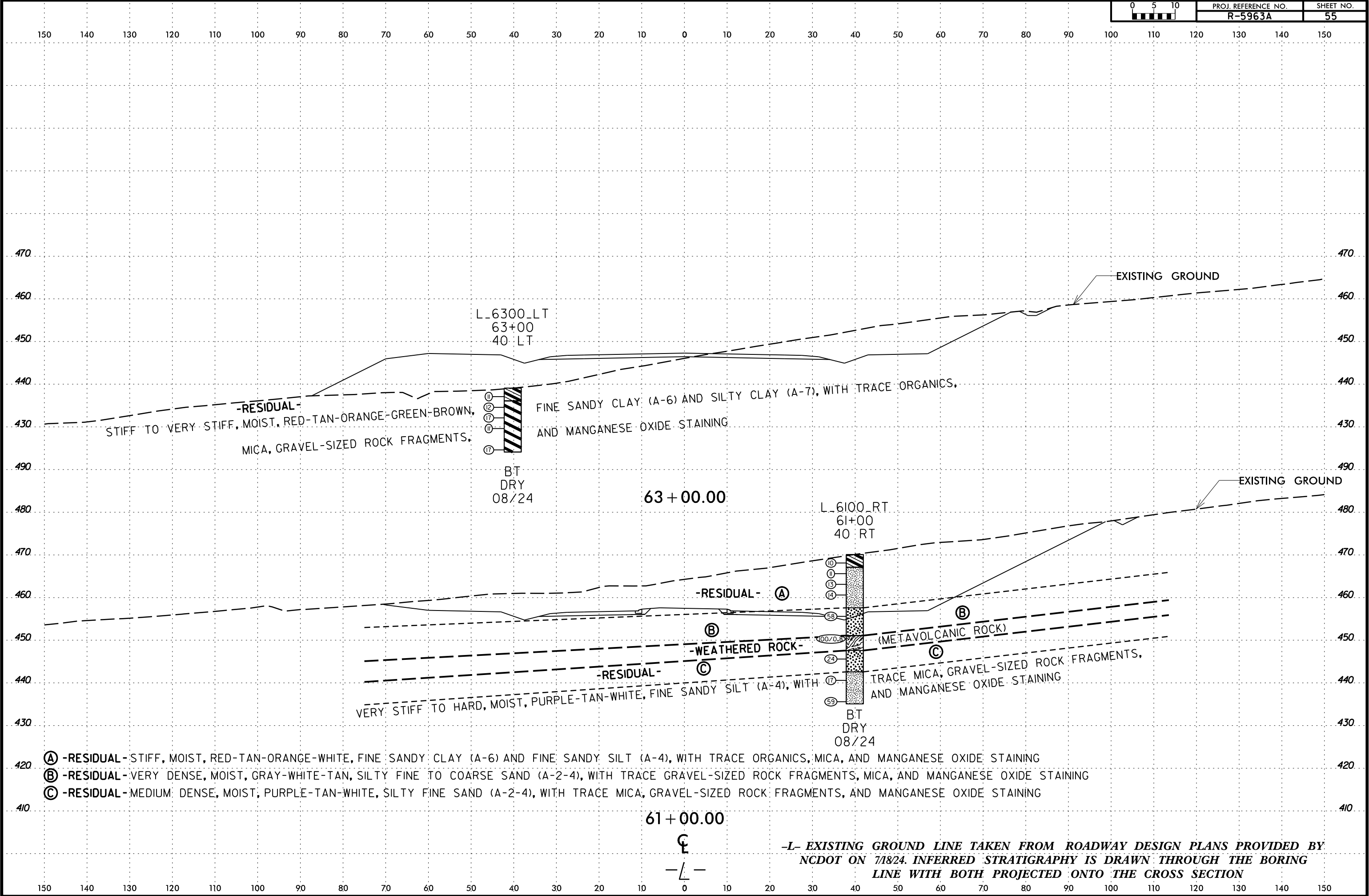


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R-5963A	54



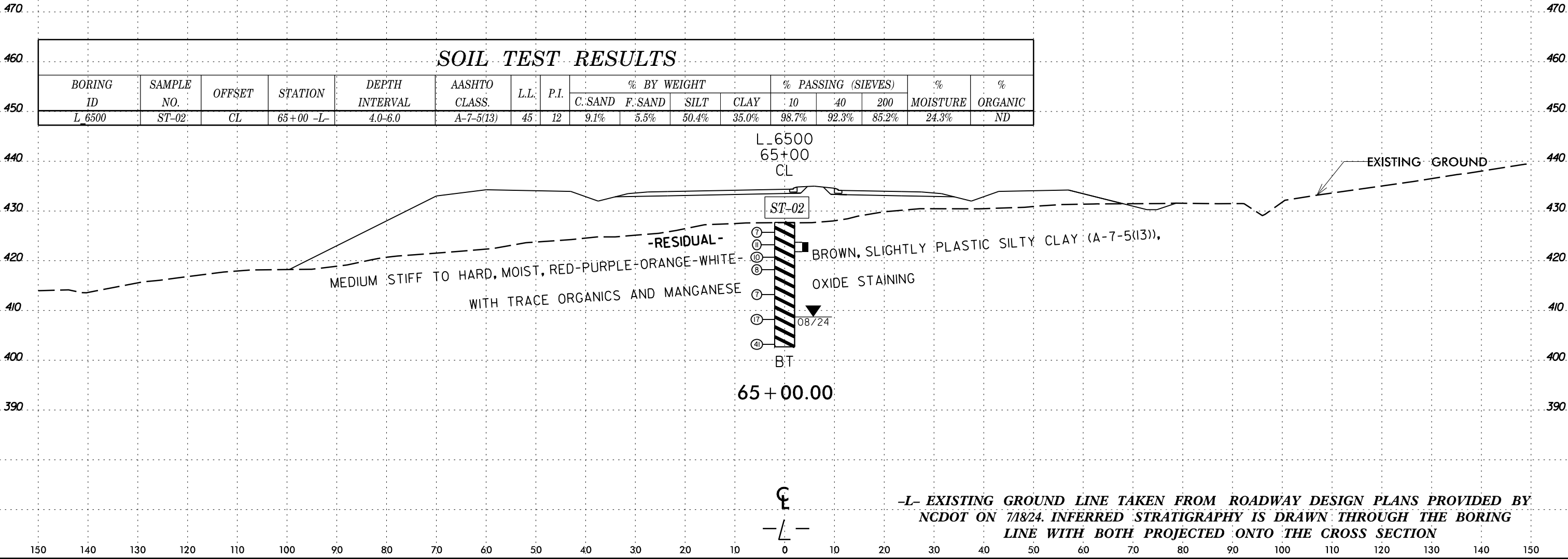
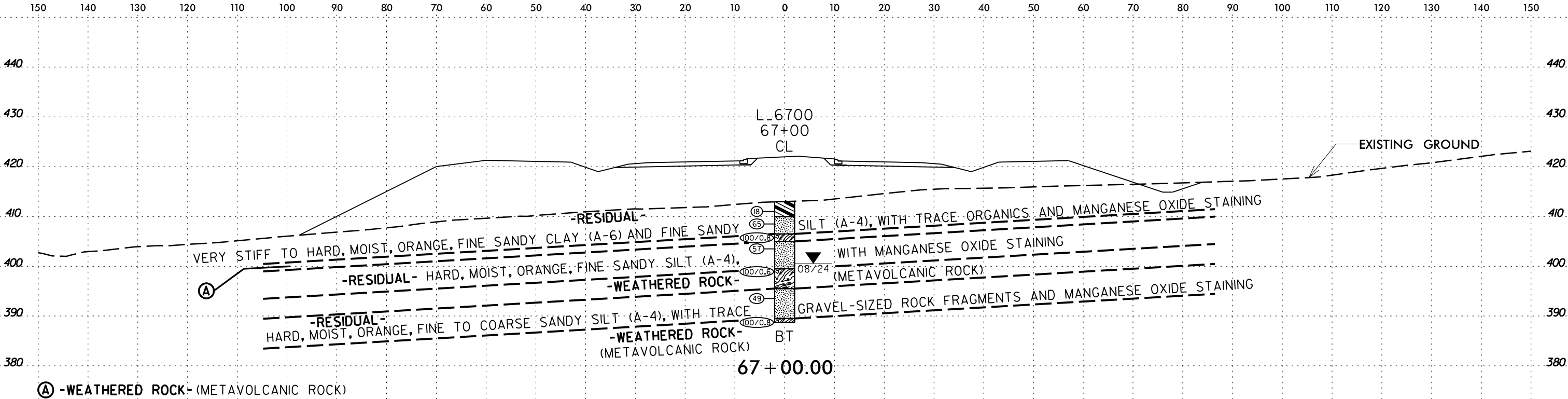
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6/23/16

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PROJ. REFERENCE NO.
R-5963A

SHEET NO.
57

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470 470

460 460

450 450

440 440

430 430

420 420

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71 + 00.00

420 420

410 410

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390 390

380 380

69 + 00.00

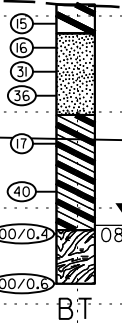
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SOIL TEST RESULTS

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								C. SAND	F. SAND	SILT	CLAY	10	40	200		
L 7101 LT	SS-407	50' LT	71+01 -L-	1.0-2.5	A-7-5(21)	50	20	6.8%	3.8%	44.7%	44.7%	98.7%	93.5%	88.9%	31.3%	ND
L 7101 LT	SS-409	50' LT	71+01 -L-	6.0-7.5	A-4(9)	37	9	6.4%	7.5%	57.7%	28.4%	99.5%	96.0%	86.8%	23.8%	ND
L 7101 LT	SS-411	50' LT	71+01 -L-	13.5-15.0	A-6(7)	37	11	14.9%	20.8%	27.4%	36.9%	96.0%	85.2%	67.7%	27.4%	ND

L 7101 LT
71+01
50' LT

SS-407
SS-409
SS-411



-RESIDUAL-

(A)

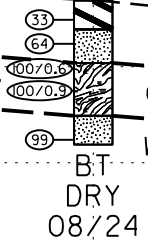
-WEATHERED ROCK-
(METAVOLCANIC ROCK)

EXISTING GROUND

(A) -RESIDUAL- STIFF TO HARD, MOIST, RED-GRAY-BROWN-ORANGE, MODERATELY PLASTIC SILTY CLAY (A-7-5(21)), FINE SANDY SILT (A-4(9)), AND SLIGHTLY PLASTIC FINE TO COARSE SANDY CLAY (A-6(7)), WITH TRACE ORGANICS, GRAVEL-SIZED ROCK FRAGMENTS, AND MANGANESE OXIDE STAINING

71 + 00.00

L 6900
69+00
CL



-RESIDUAL-

(A)

-WEATHERED ROCK-
(METAVOLCANIC ROCK)

WITH MANGANESE OXIDE STAINING

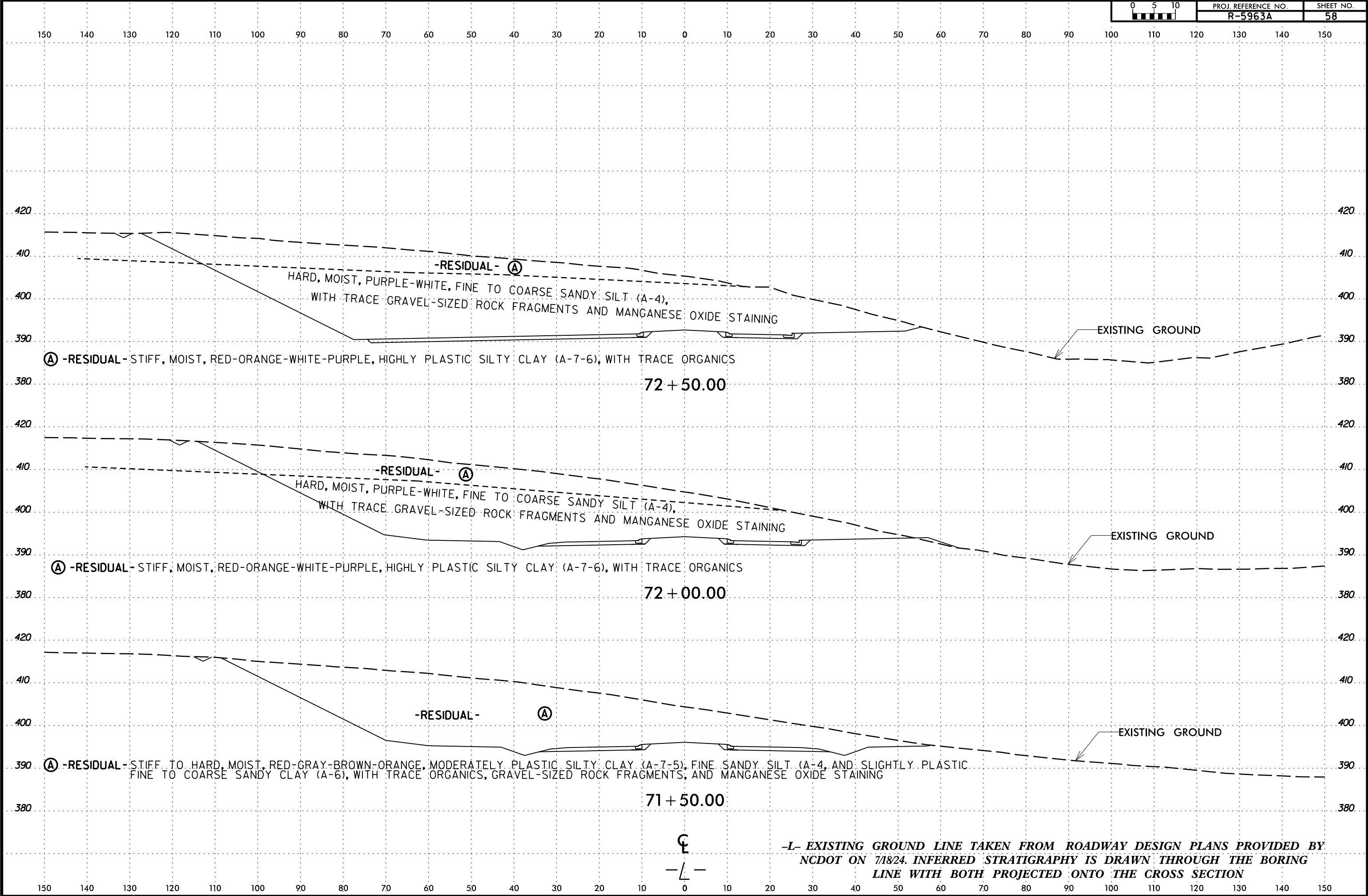
EXISTING GROUND

(A) -RESIDUAL- HARD, MOIST, ORANGE-WHITE, SILTY CLAY (A-7) AND FINE TO COARSE SANDY SILT (A-4), WITH TRACE GRAVEL-SIZED ROCK FRAGMENTS AND MANGANESE OXIDE STAINING

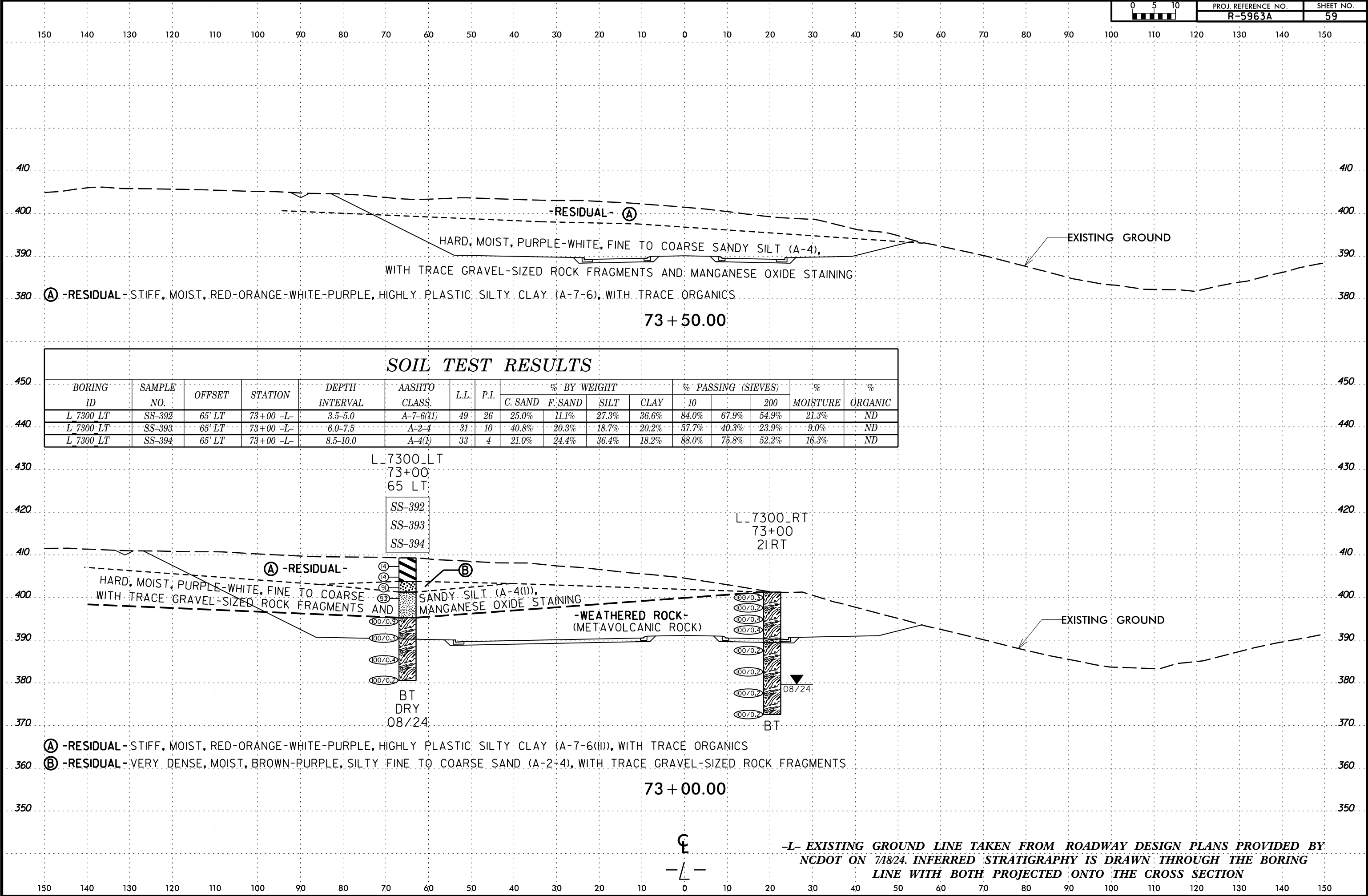
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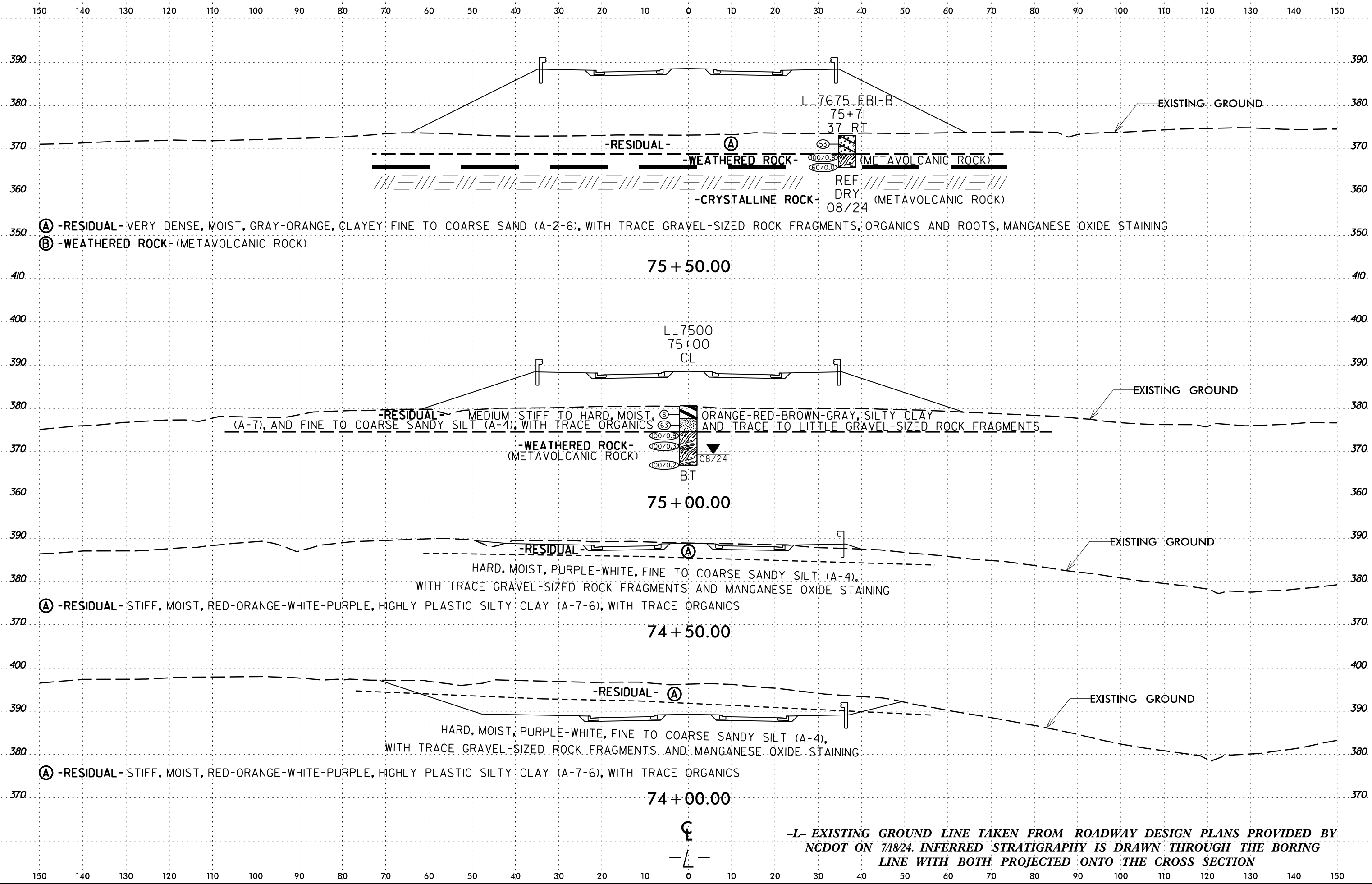
CL
08/24

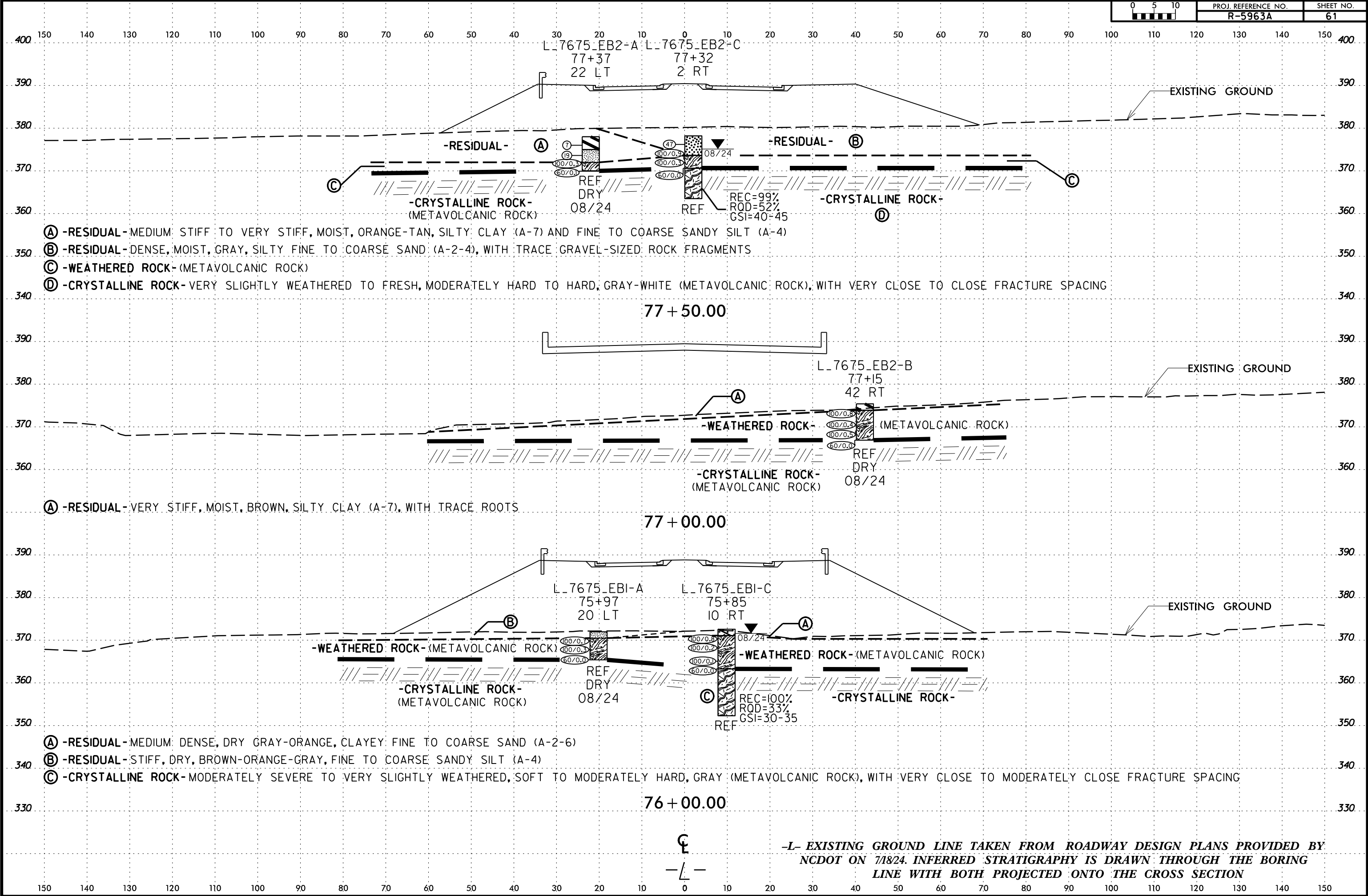
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NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION



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440 440

430 430

420 420

410 410

400 400

390 390

380 380

370 370

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-23	SS-522	4' LT	80+83 -W3-	1.0-2.5	A-7-6(14)	47	20	10.1%	4.5%	32.3%	53.1%	81.0%	74.4%	69.9%	18.9%	ND
RWAL B-23	SS-523	4' LT	80+83 -W3-	3.9-5.4	A-7-5(21)	55	14	0.3%	0.6%	66.3%	32.8%	100.0%	99.9%	99.2%	45.4%	ND
RWAL B-23	SS-525	4' LT	80+83 -W3-	8.9-10.4	A-4(8)	38	6	1.9%	7.0%	62.3%	28.8%	100.0%	98.9%	94.0%	34.5%	ND

RWAL_B-23
80+83 (-W3-)
4 LT

SS-522
SS-523
SS-525

L-8088-LT
80+88
8 LT

-RESIDUAL-

MEDIUM STIFF TO HARD, MOIST, PLASTIC SILTY CLAY (A-7-5) ORGANICS, MANGANESE OXIDE

BROWN-RED-ORANGE-GRAY, SLIGHTLY TO MODERATELY A-7-6), AND FINE SANDY SILT (A-4), WITH TRACE STAINING, AND GRAVEL-SIZED ROCK FRAGMENTS

-WEATHERED ROCK-
(METAVOLCANIC ROCK)

-WEATHERED ROCK-
(METAVOLCANIC ROCK)

81+00.00

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
RWAL B-27	SS-473	CL	80+66 -W4-	3.7-5.2	A-7-5(24)	53	21	2.7%	4.1%	50.2%	43.0%	99.0%	97.1%	93.3%	36.2%	ND
RWAL B-27	SS-476	CL	80+66 -W4-	13.7-15.2	A-4(7)	33	8	4.0%	9.9%	69.7%	16.4%	99.4%	96.6%	89.2%	16.9%	ND

RWAL_B-22
80+46 (-W3-)
8 LT

RWAL_B-27
80+66 (-W4-)
CL

SS-473
SS-476

-RESIDUAL-

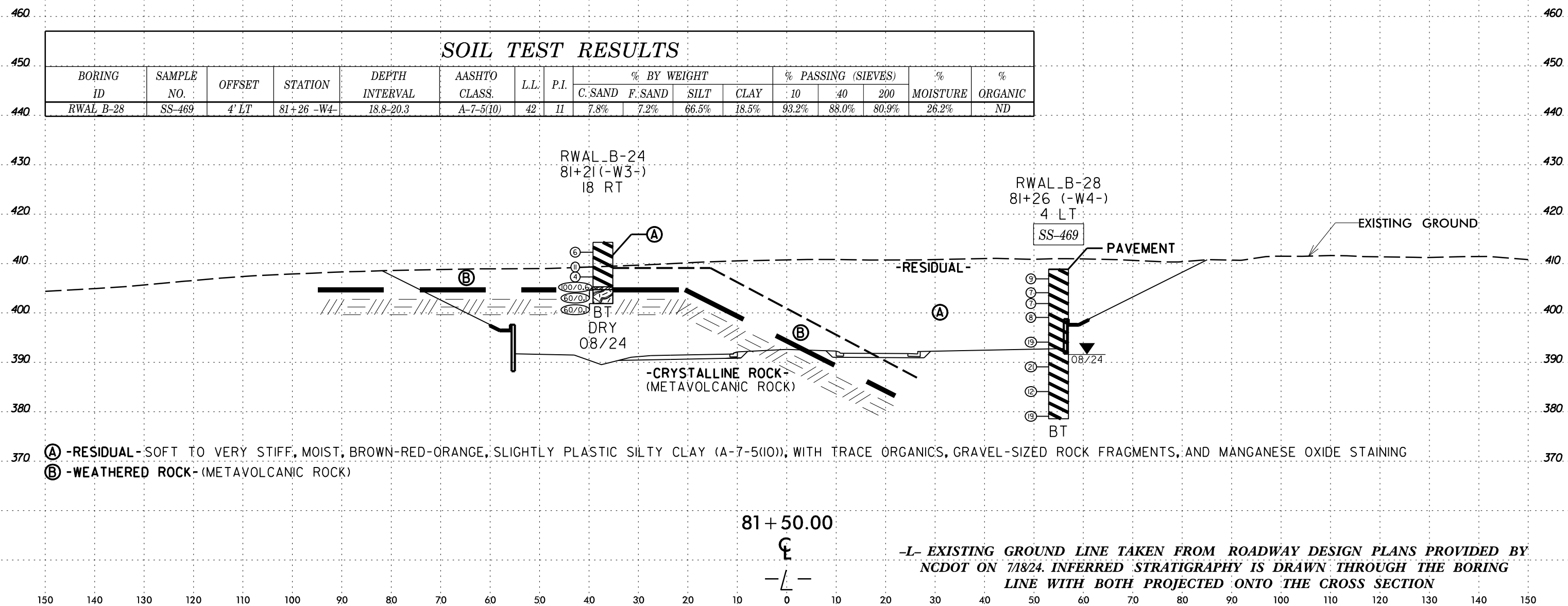
MEDIUM STIFF TO HARD, MOIST, RED-BROWN-ORANGE-TAN, MODERATELY PLASTIC SILTY CLAY (A-7-5), FINE SANDY CLAY (A-6) AND FINE SANDY SILT (A-4), WITH TRACE ORGANICS, GRAVEL-SIZED ROCK

FRAGMENTS, AND MANGANESE OXIDE STAINING

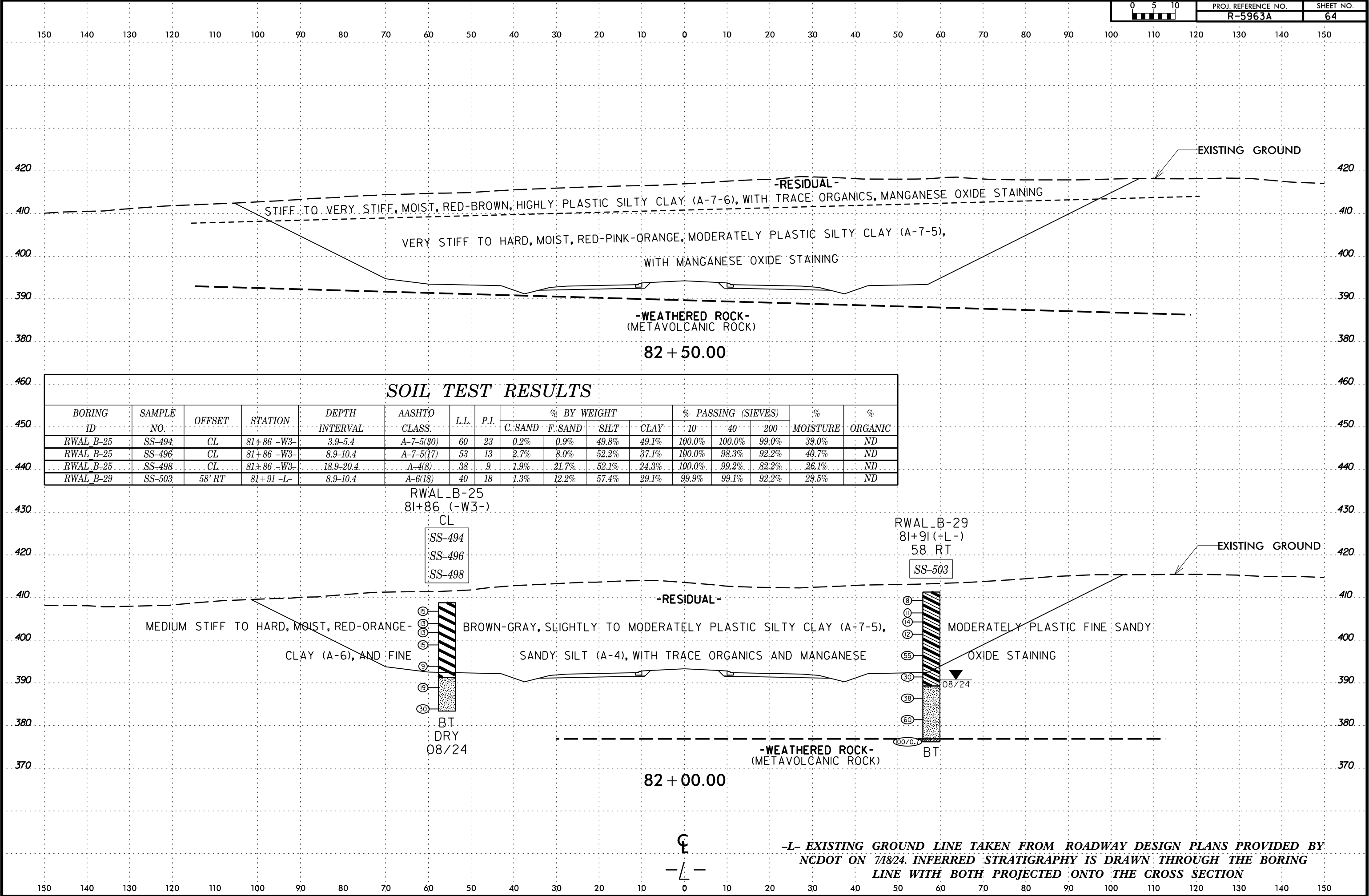
-WEATHERED ROCK- (METAVOLCANIC ROCK)

80+50.00

-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION



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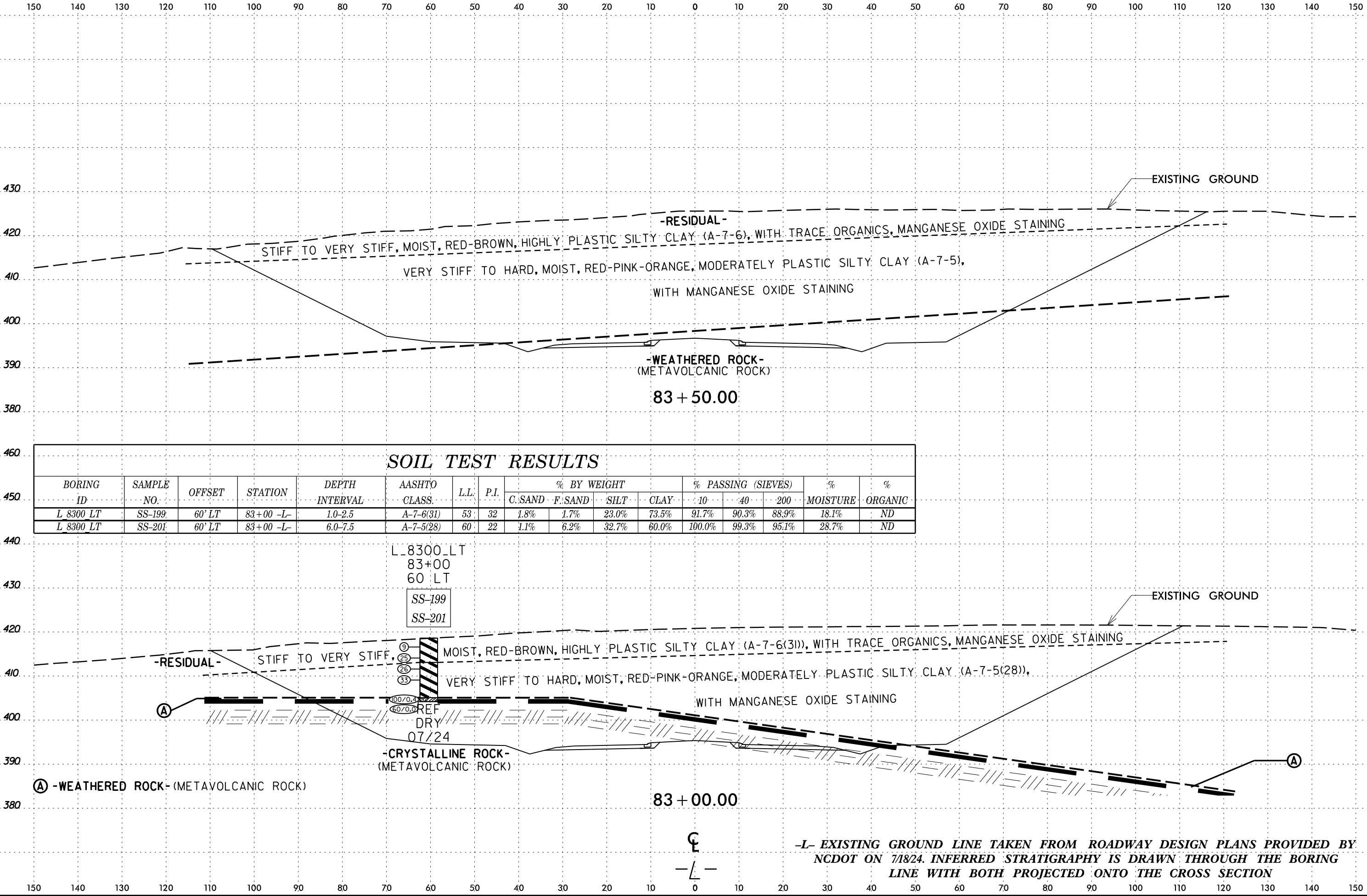
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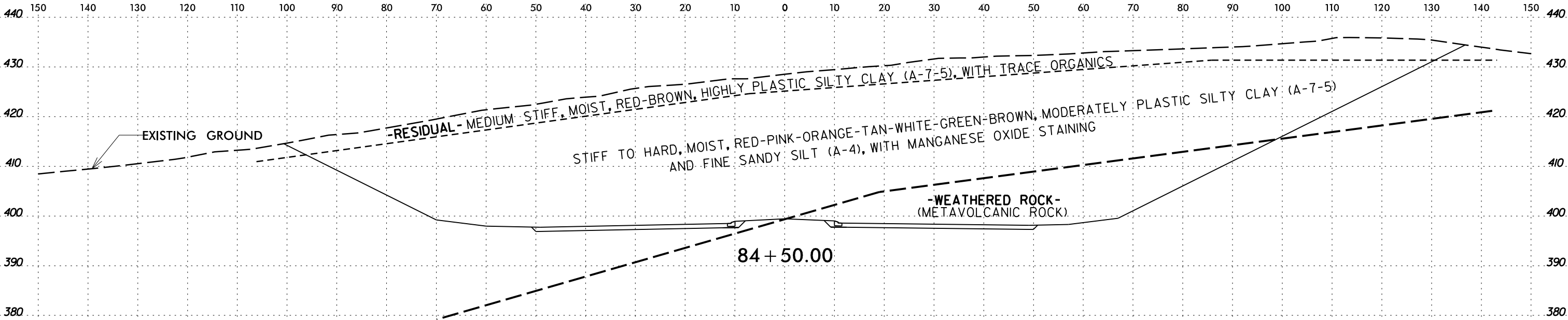
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R-5963A

SHEET NO.
65



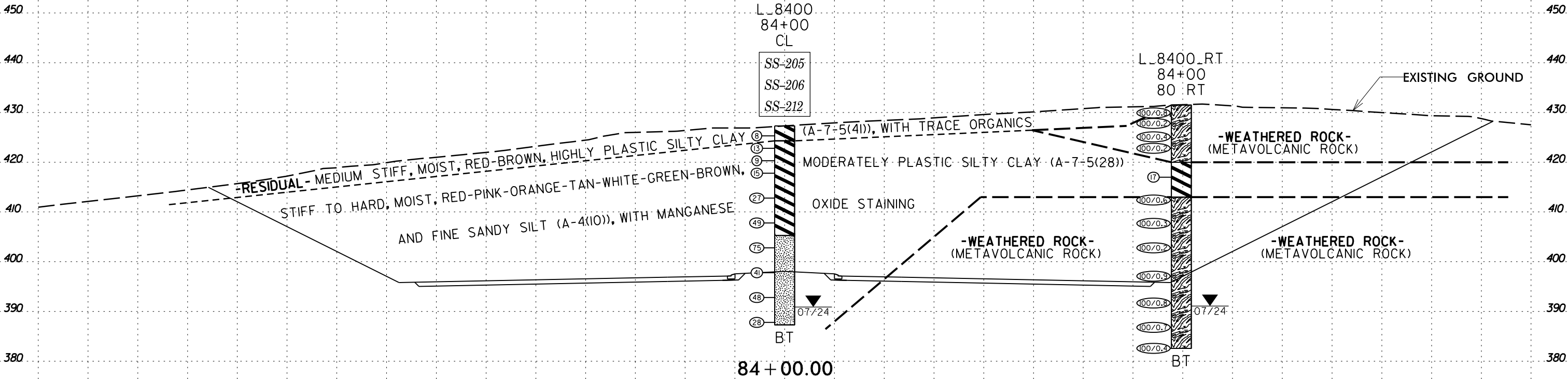
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SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C.SAND	F.SAND	SILT	CLAY	10	40	200		
L 8400	SS-205	CL	84+00 -L-	1.0-2.5	A-7-5(42)	66	35	0.3%	0.8%	21.1%	77.8%	99.6%	99.4%	98.7%	33.0%	ND
L 8400	SS-206	CL	84+00 -L-	3.5-5.0	A-7-5(28)	57	22	0.3%	1.5%	41.2%	57.0%	100.0%	99.8%	98.5%	36.5%	ND
L 8400	SS-212	CL	84+00 -L-	28.5-30.0	A-4(10)	35	10	0.5%	6.6%	72.4%	20.5%	99.9%	99.6%	94.3%	21.5%	ND

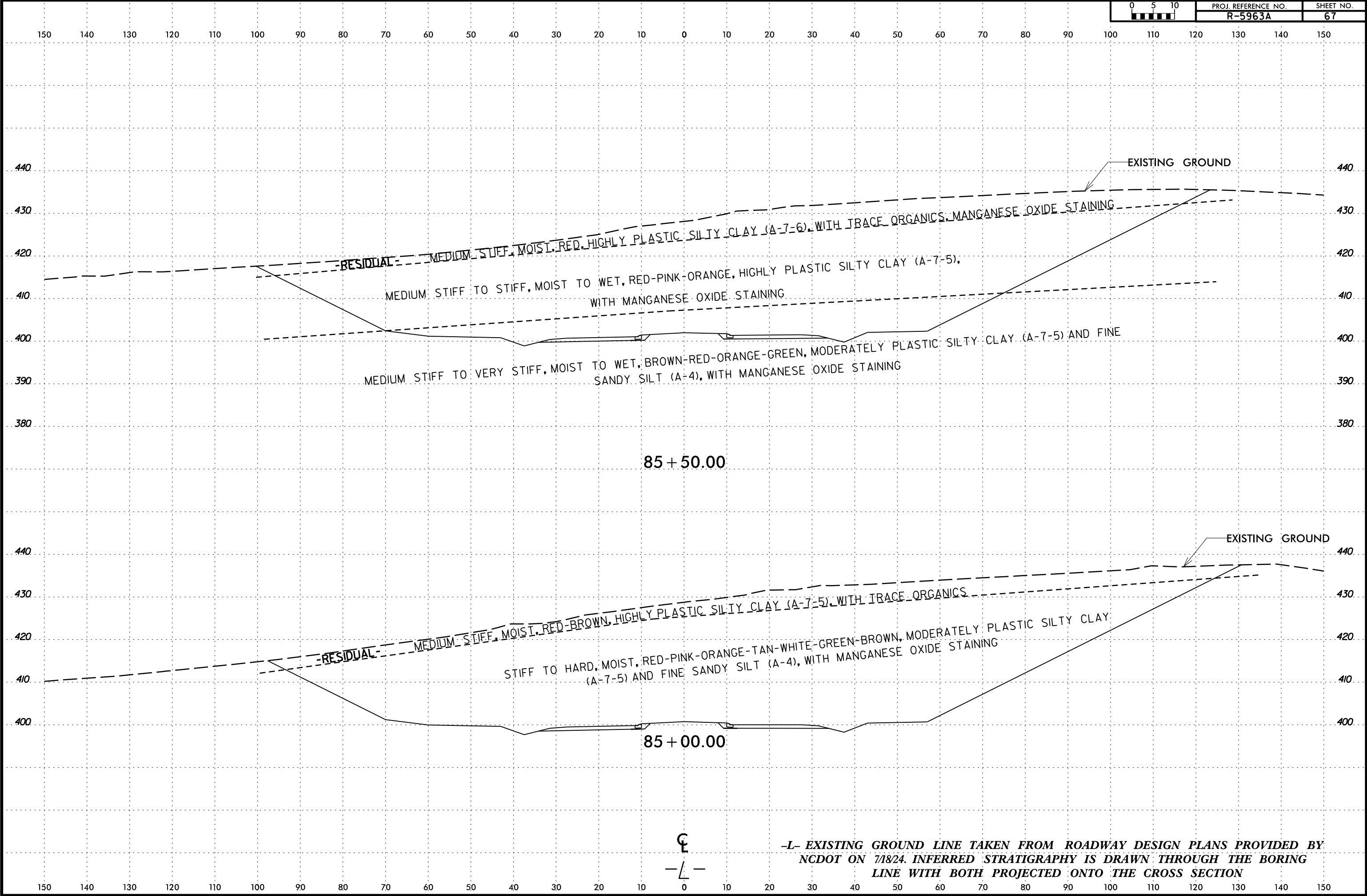


-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

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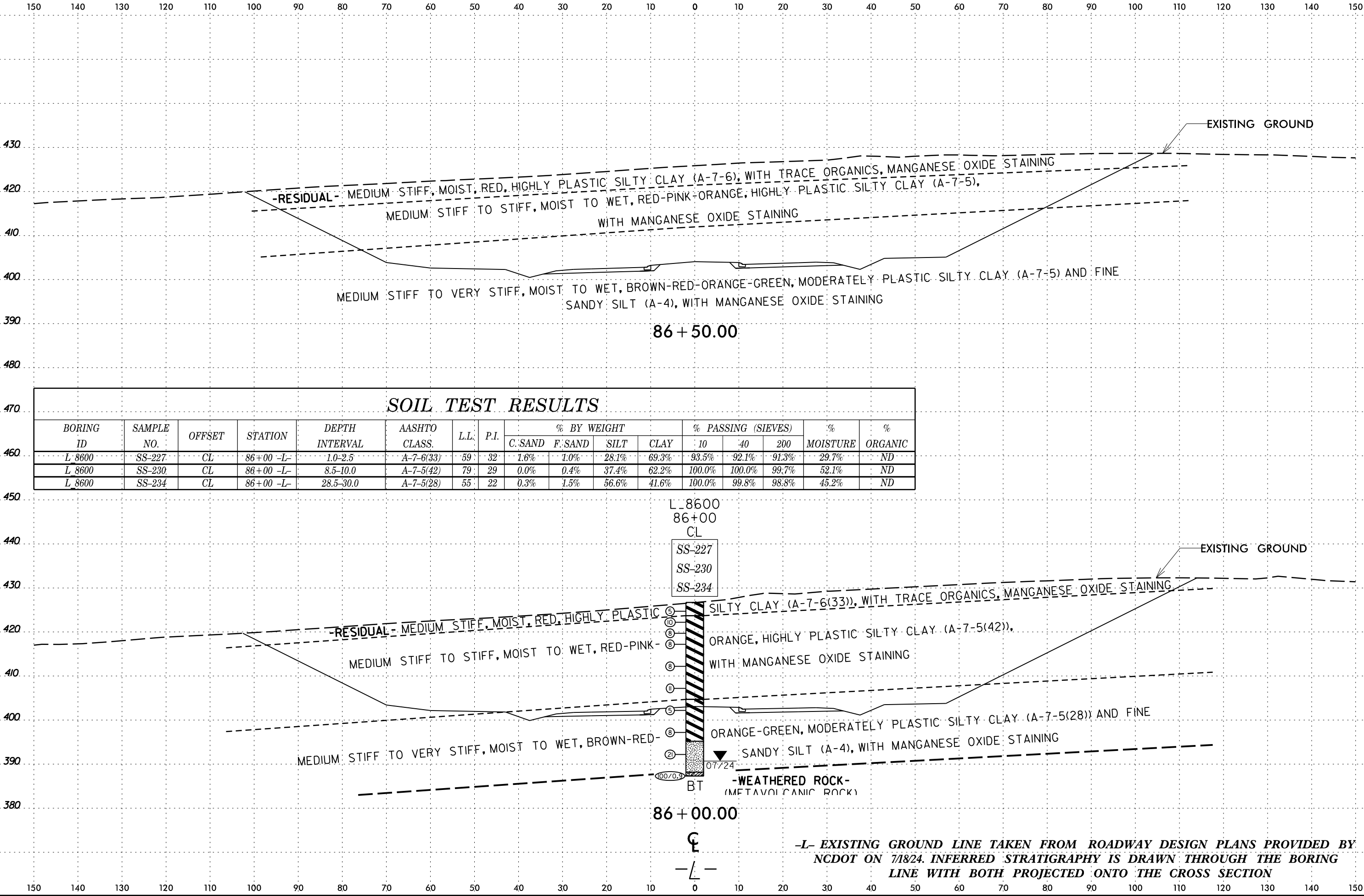
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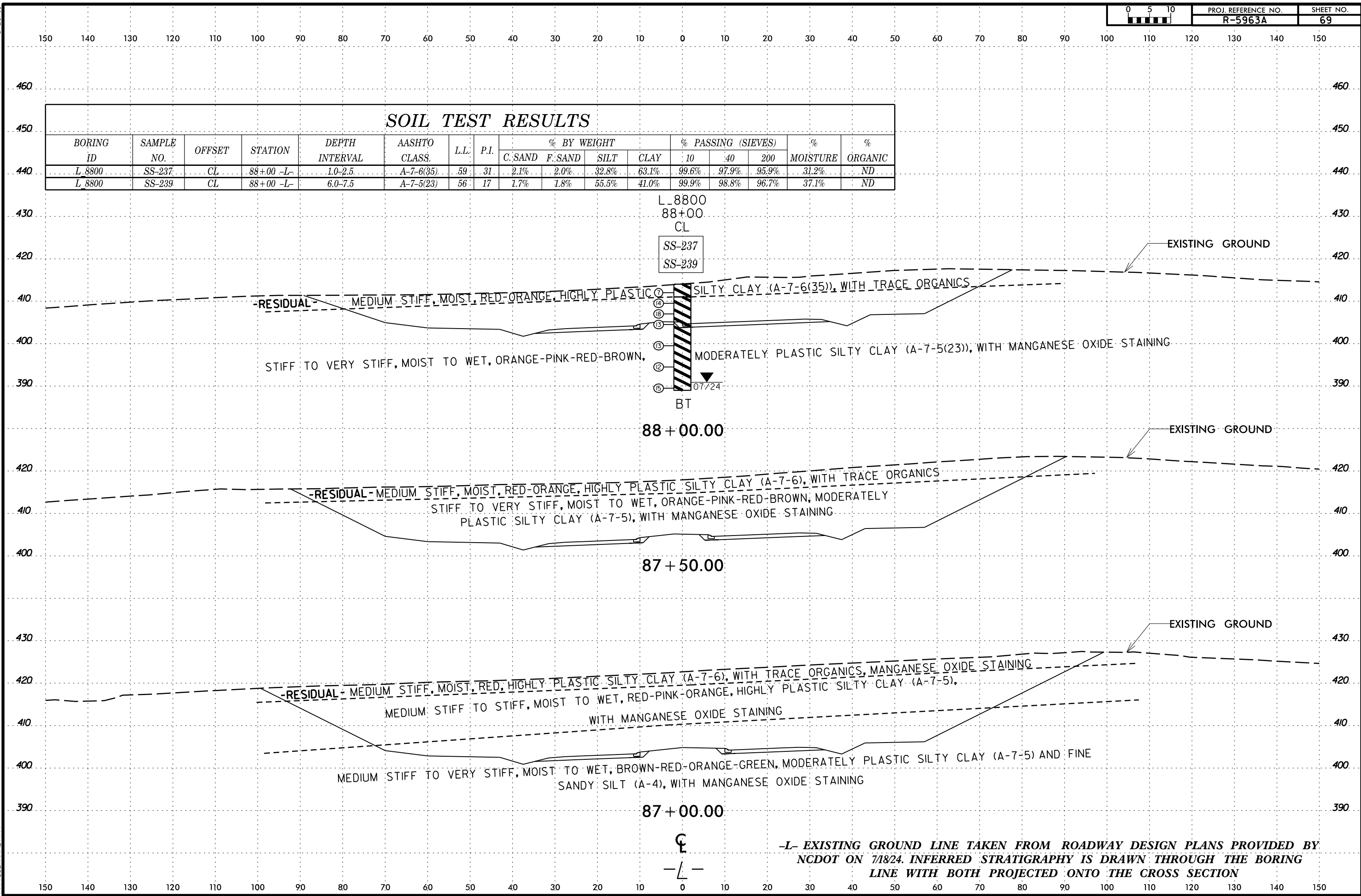
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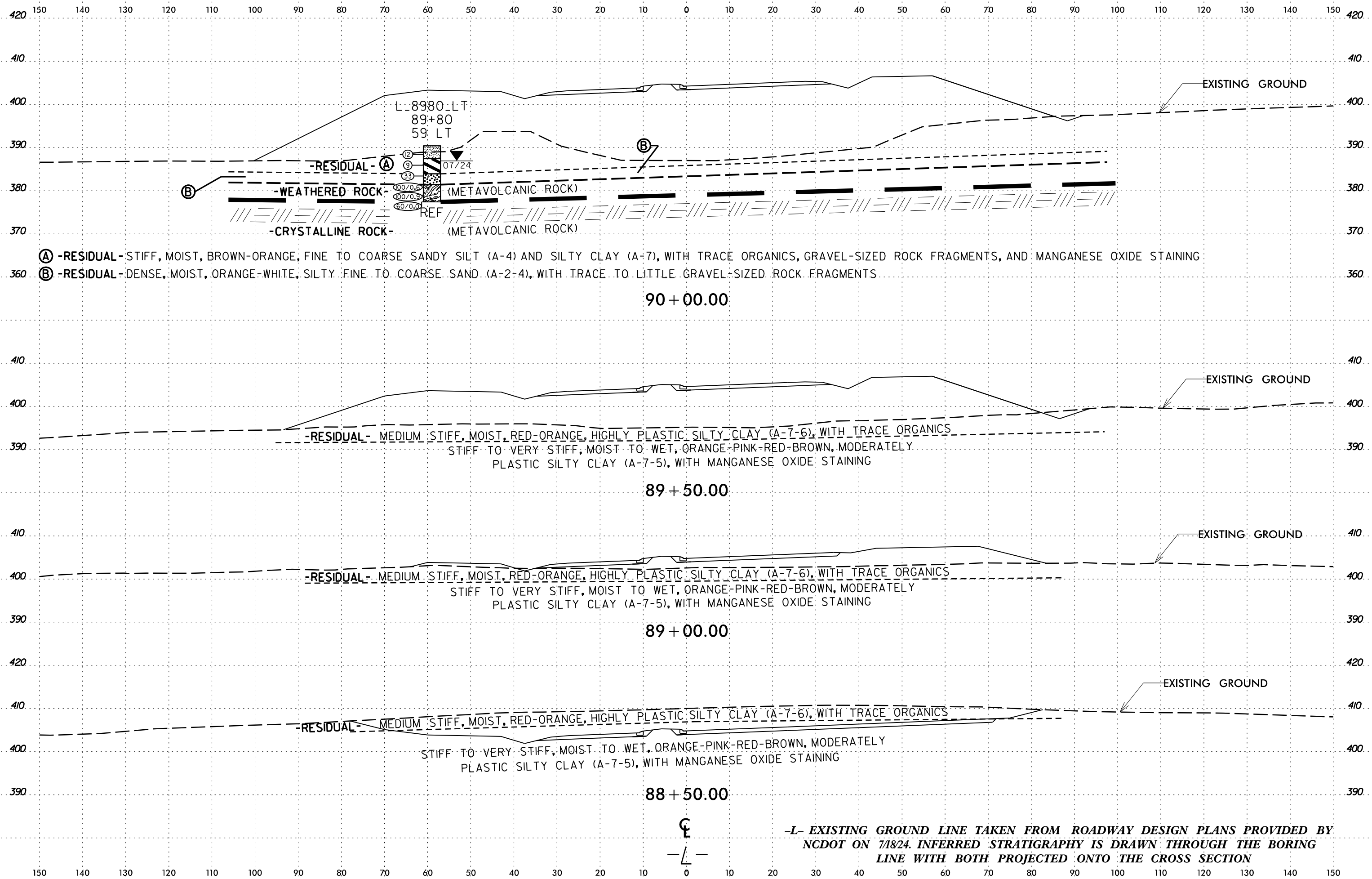


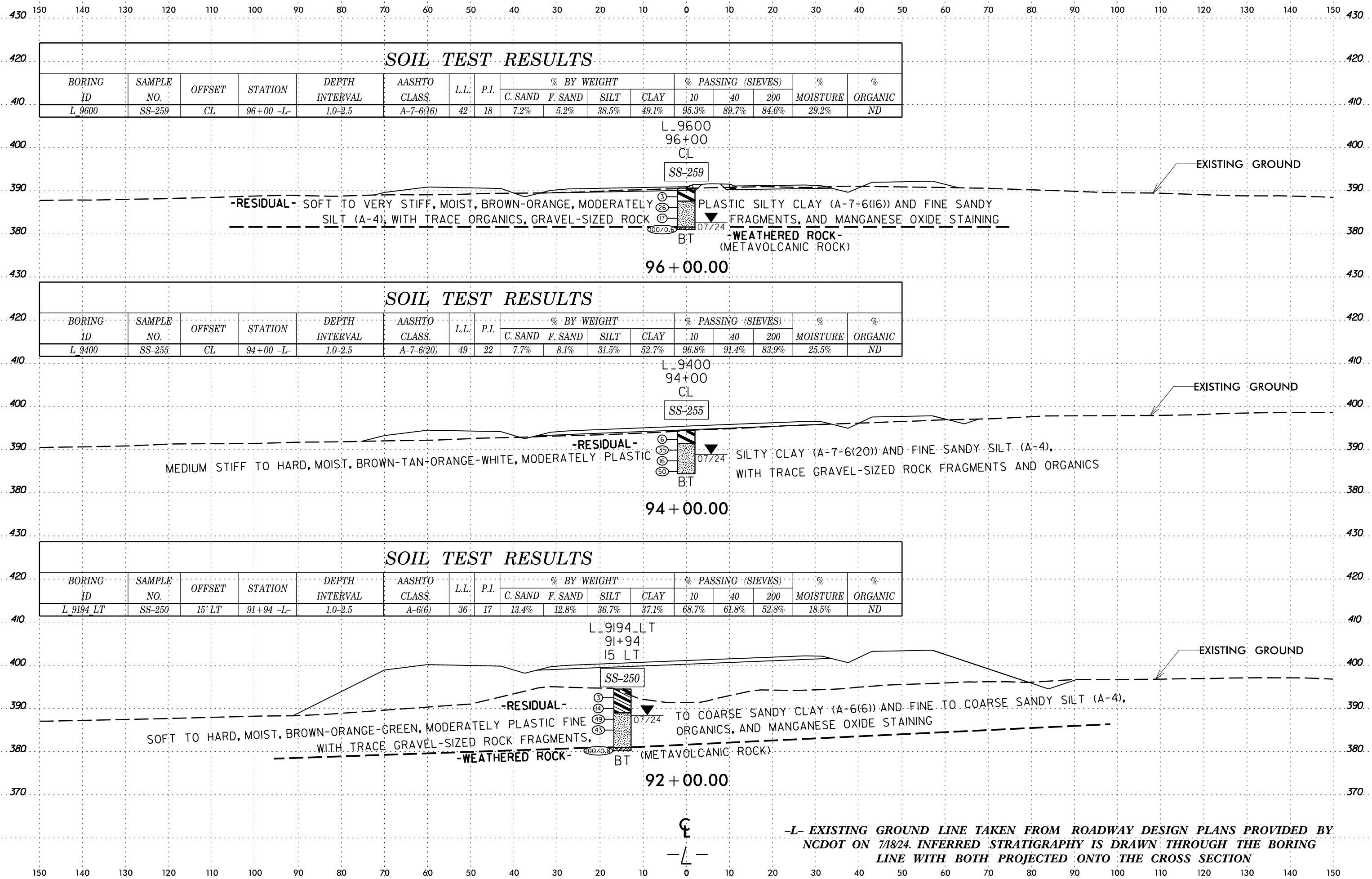
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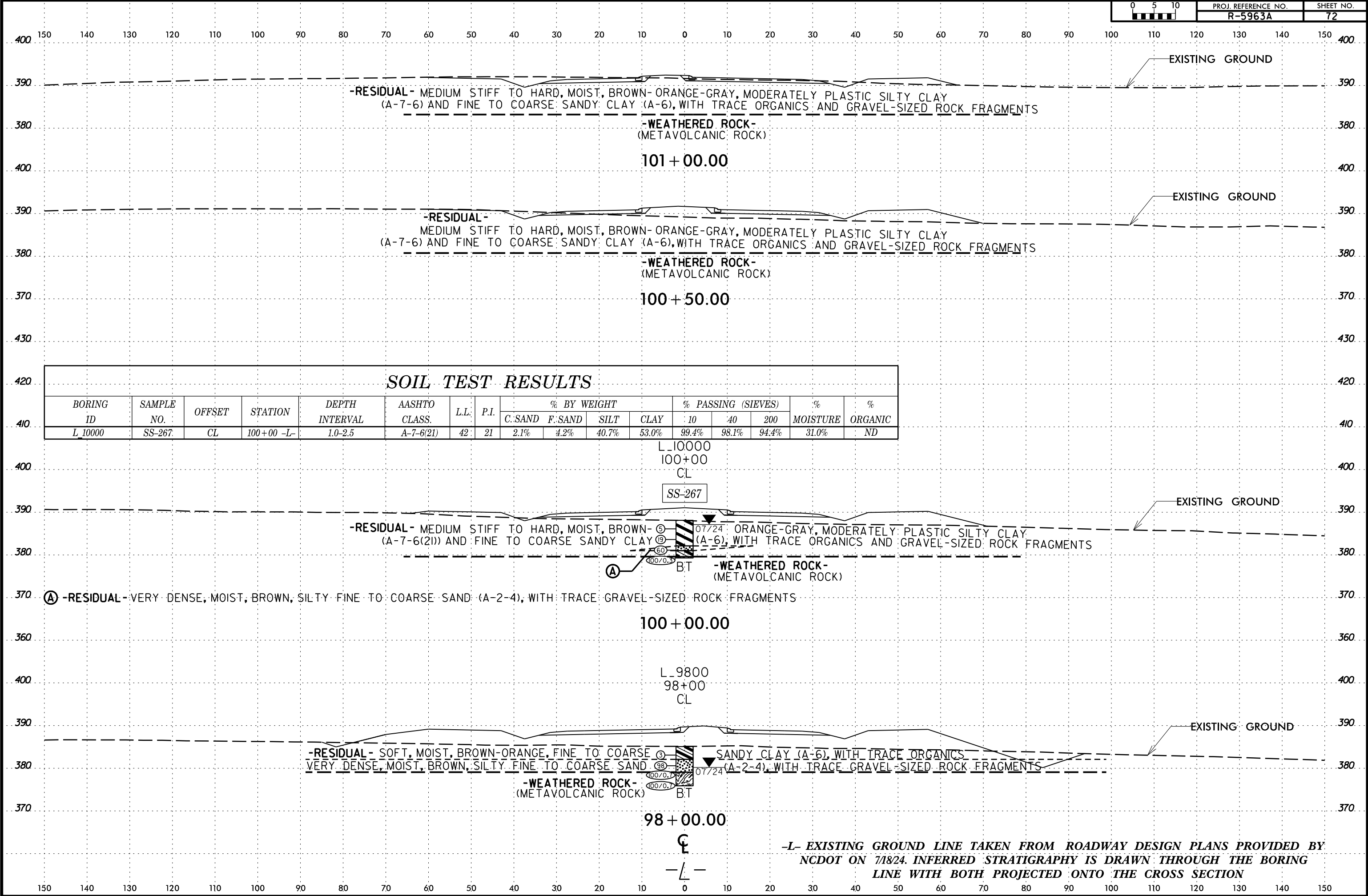






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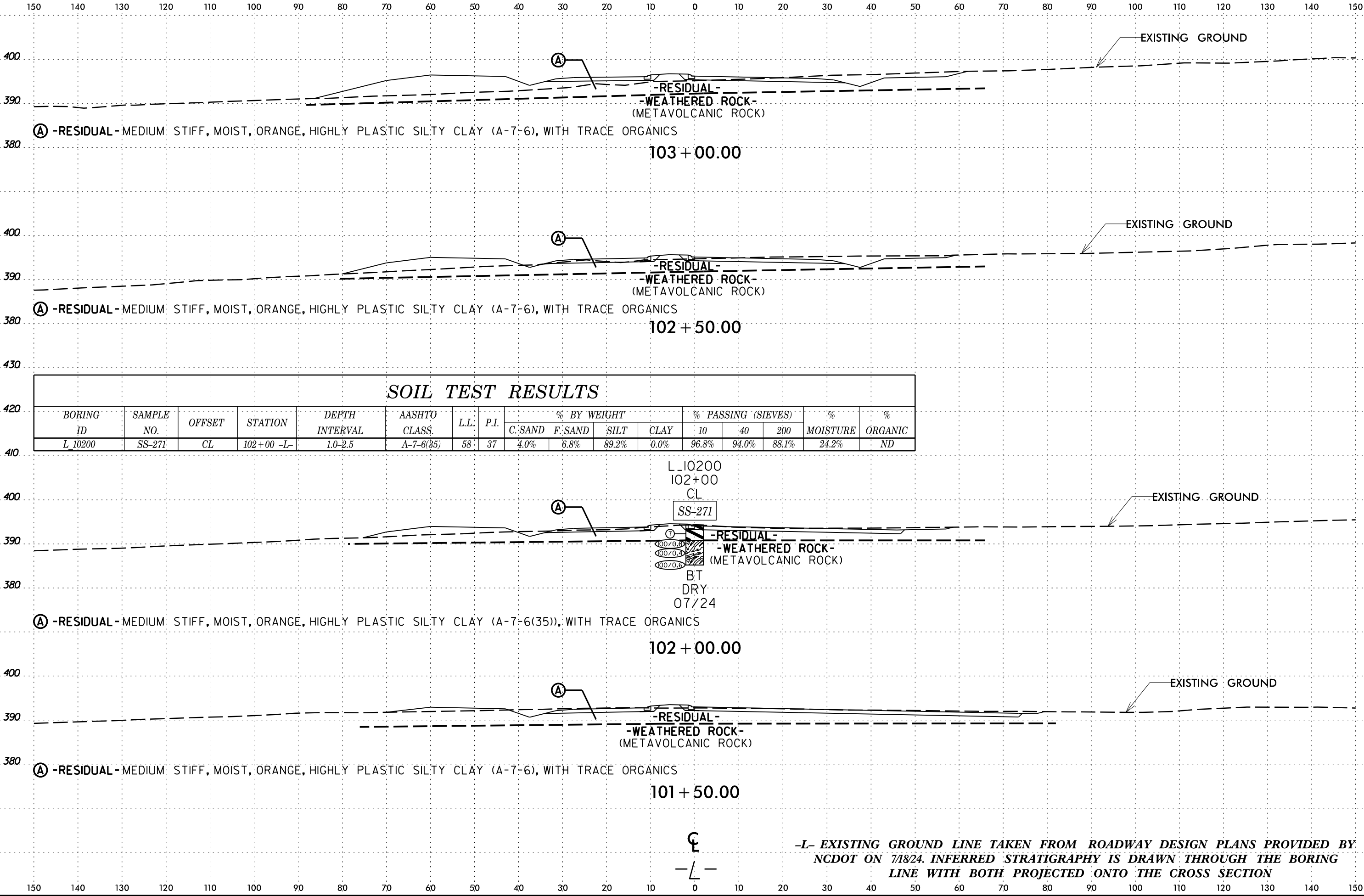


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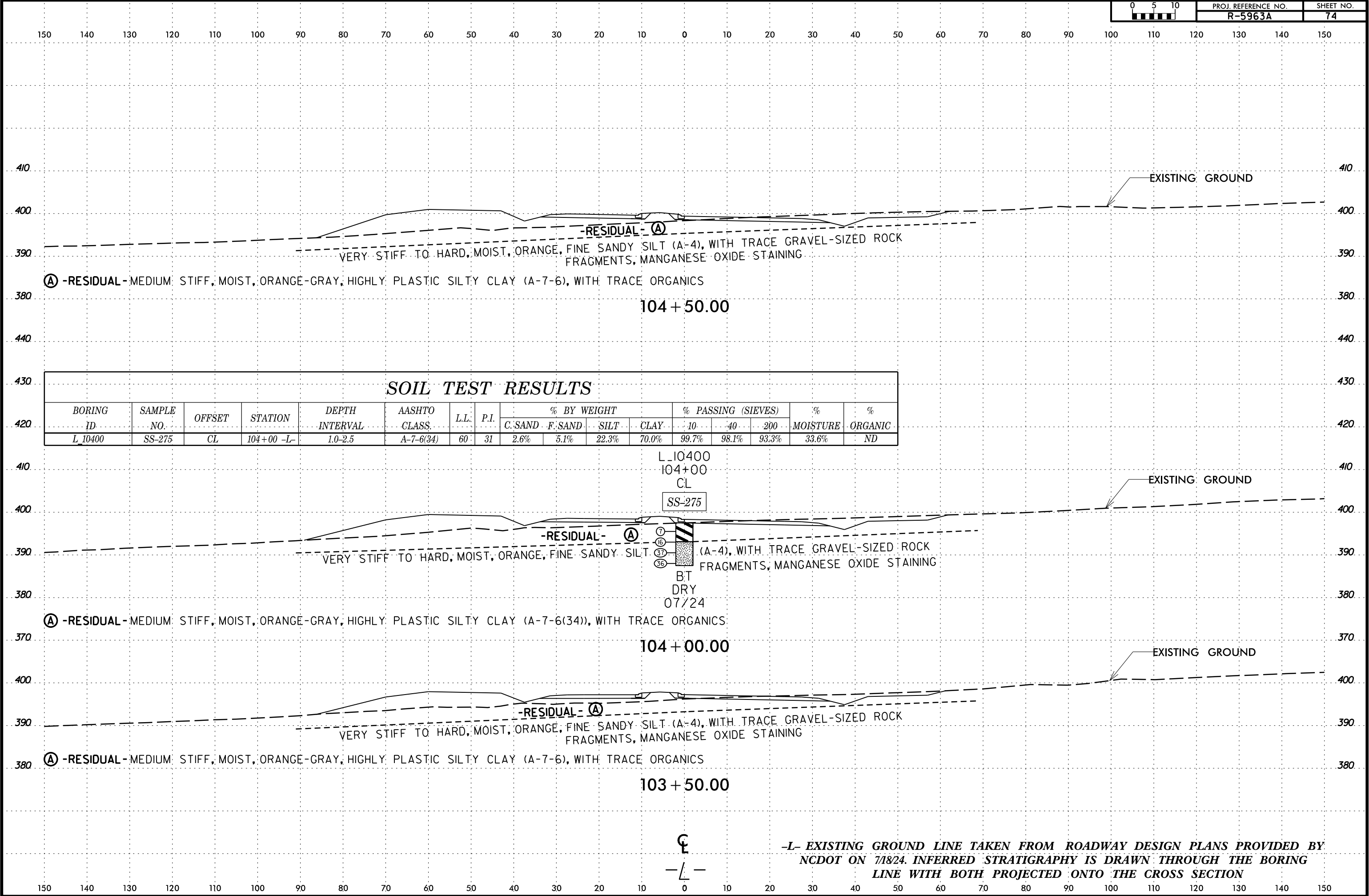


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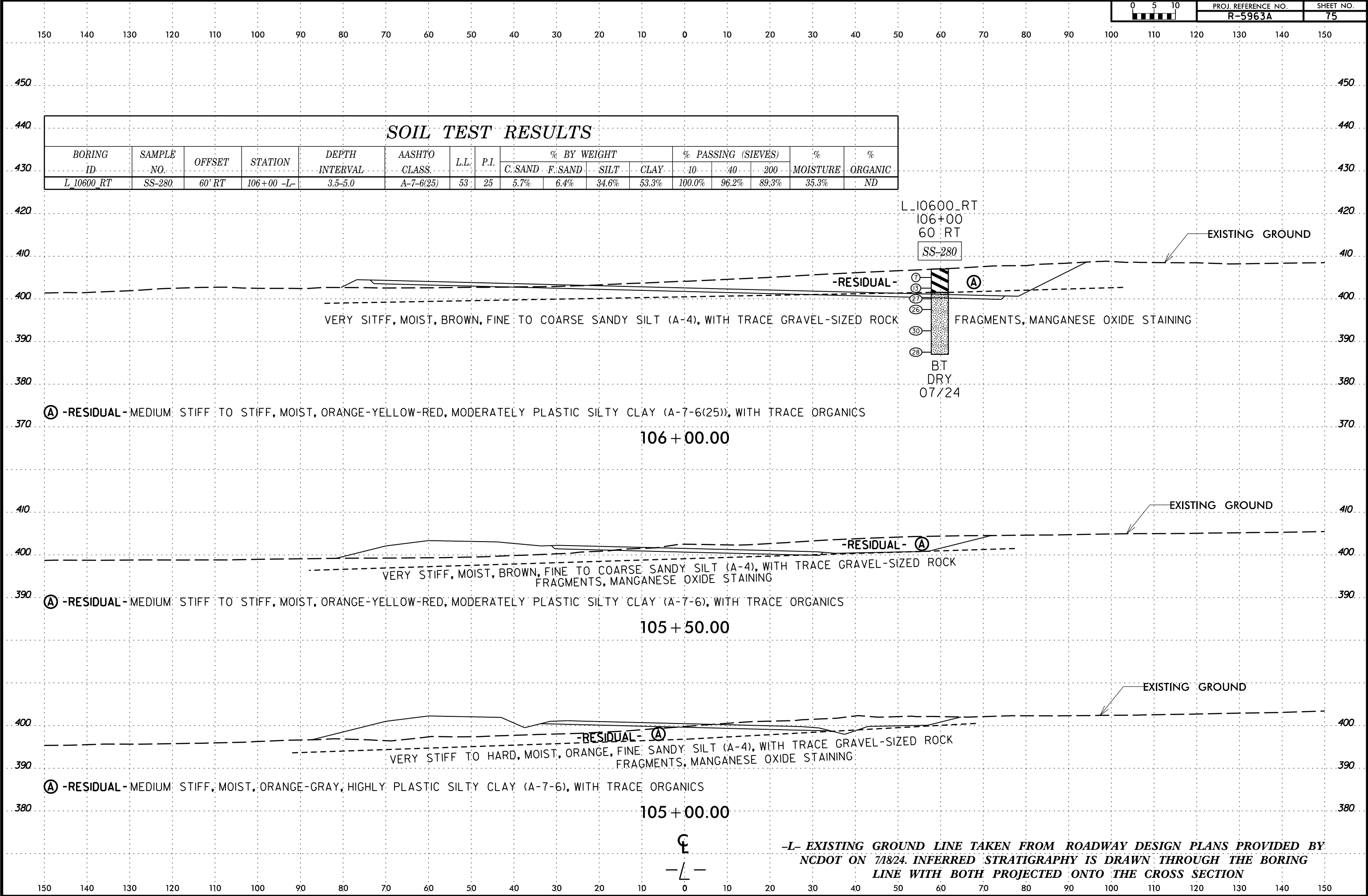
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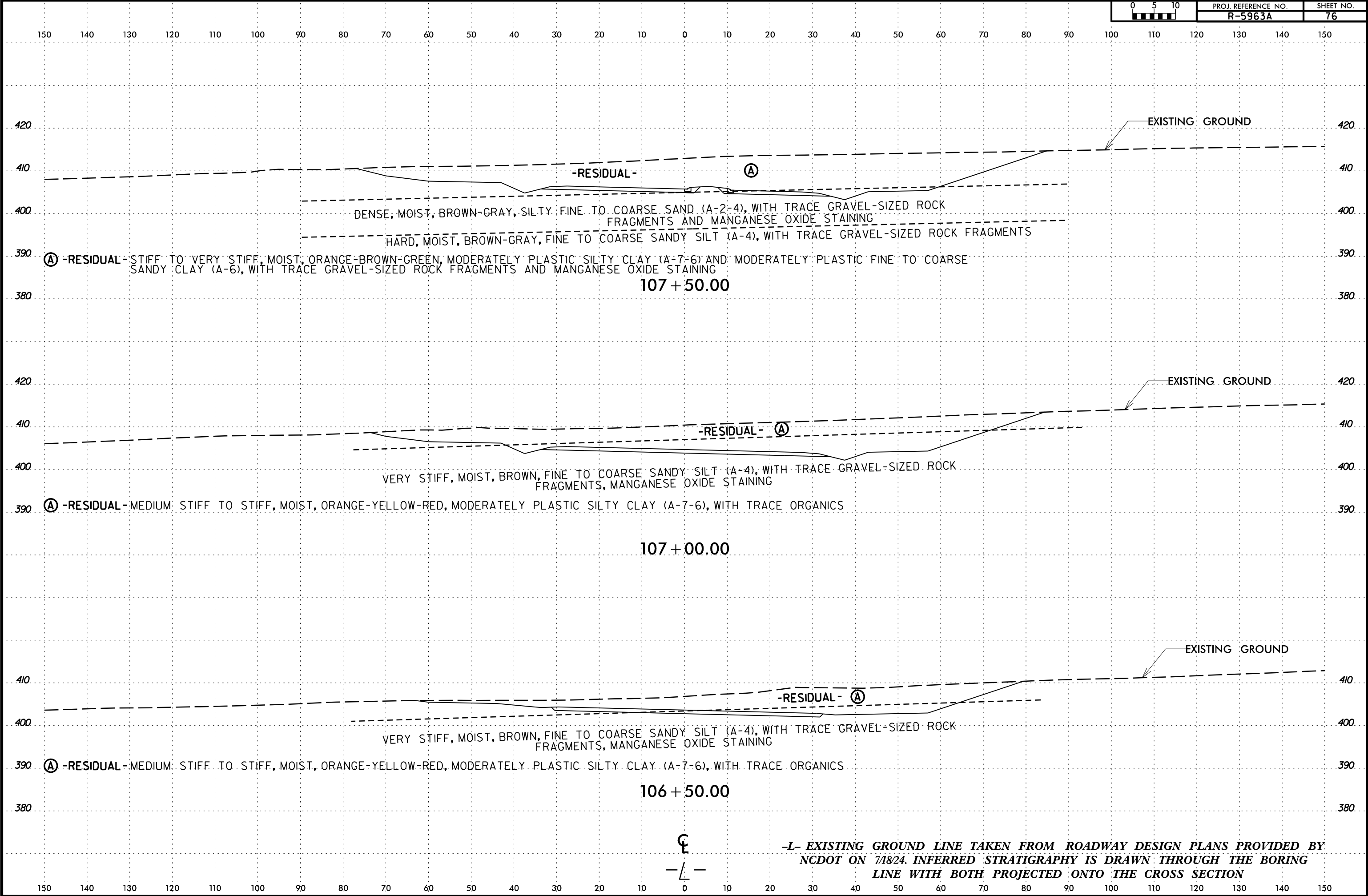
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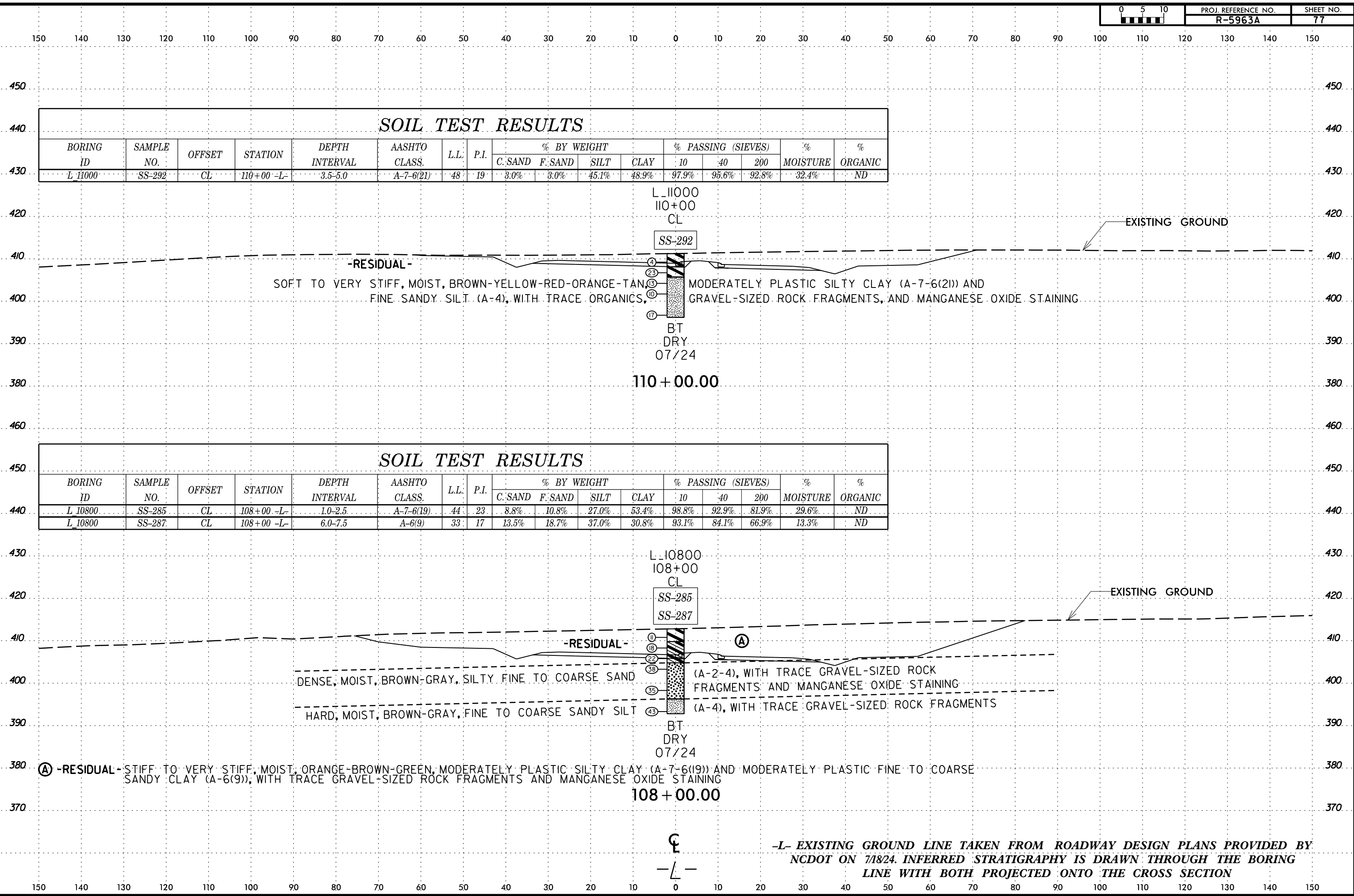


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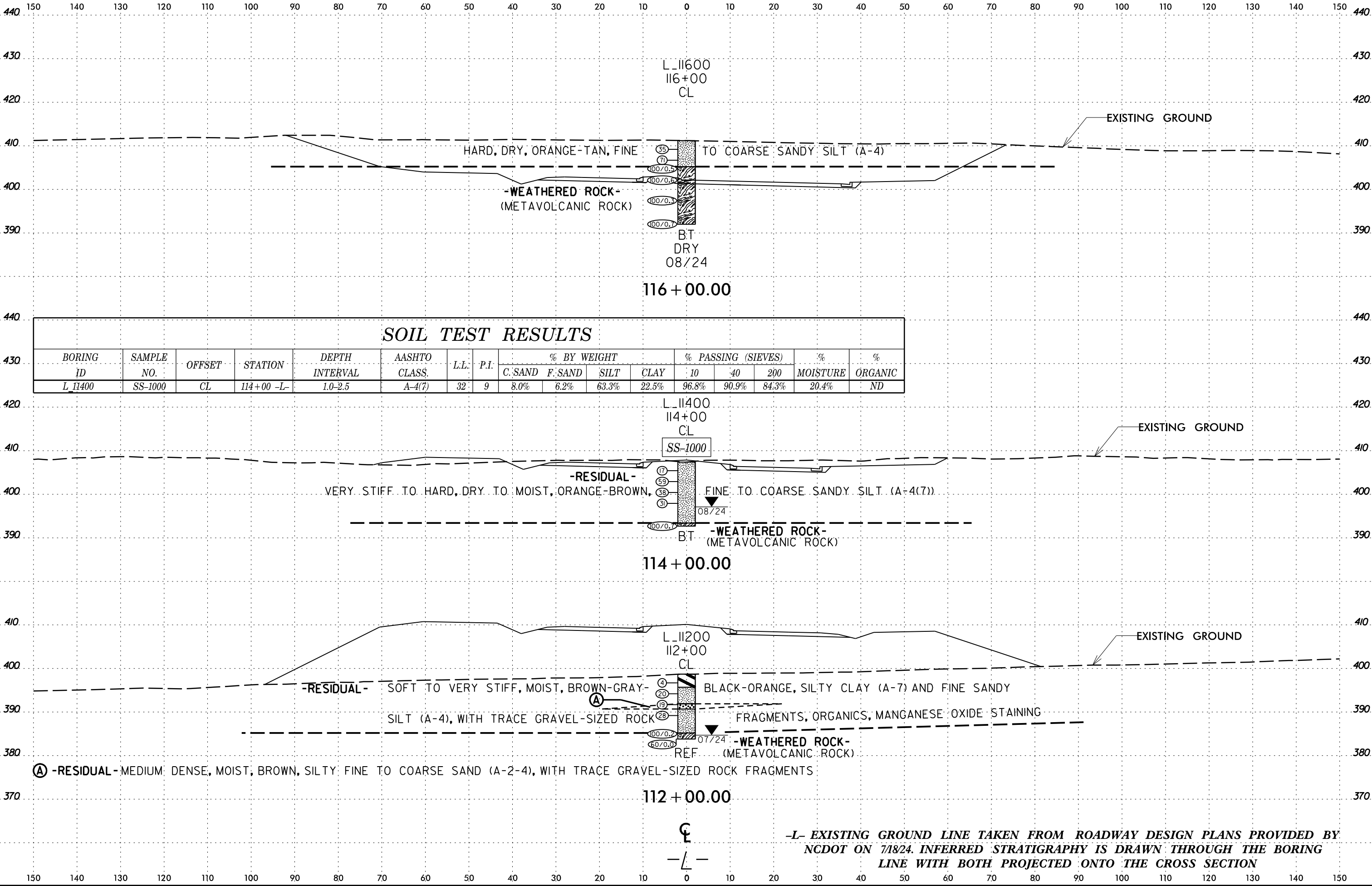






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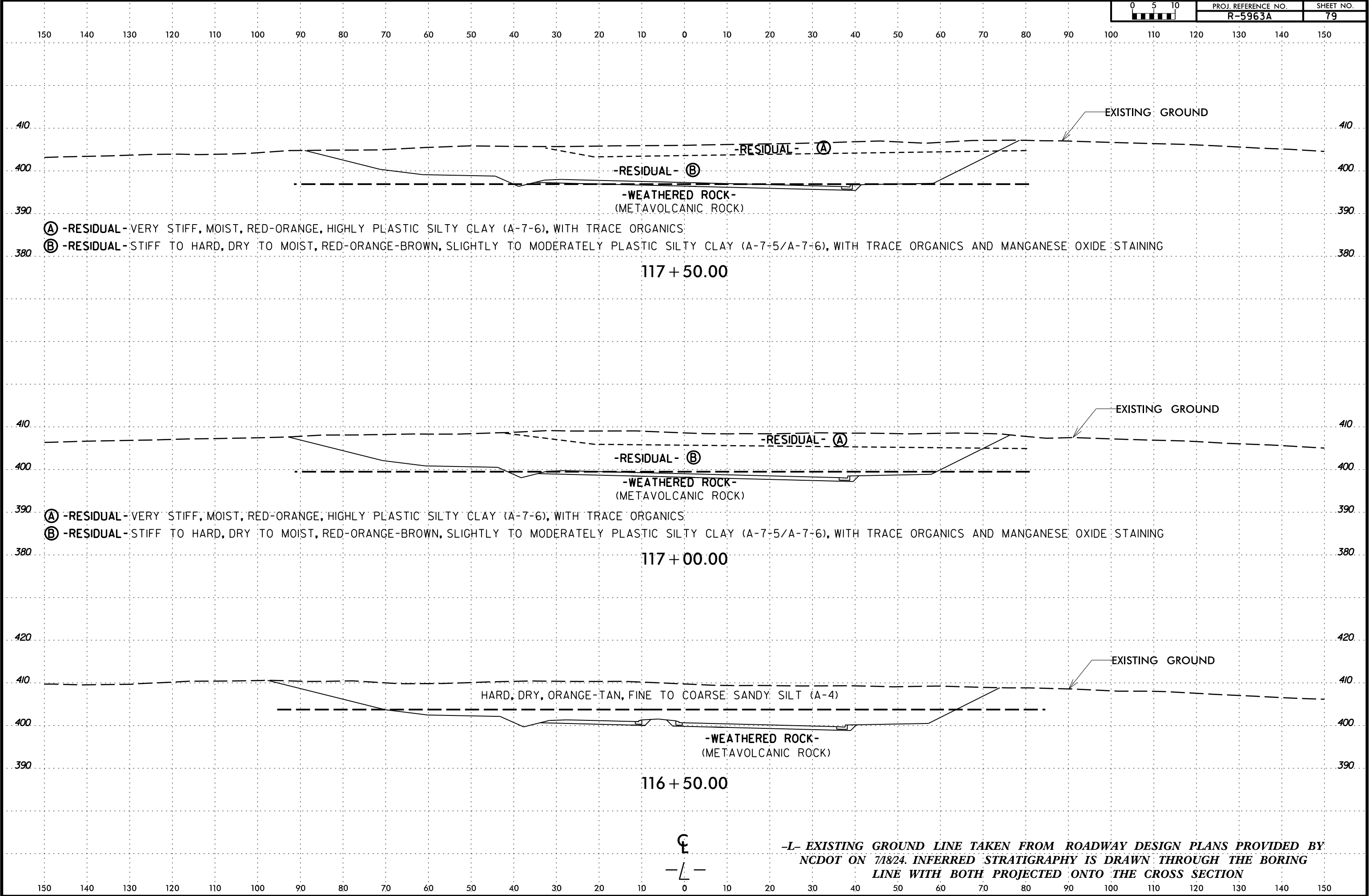
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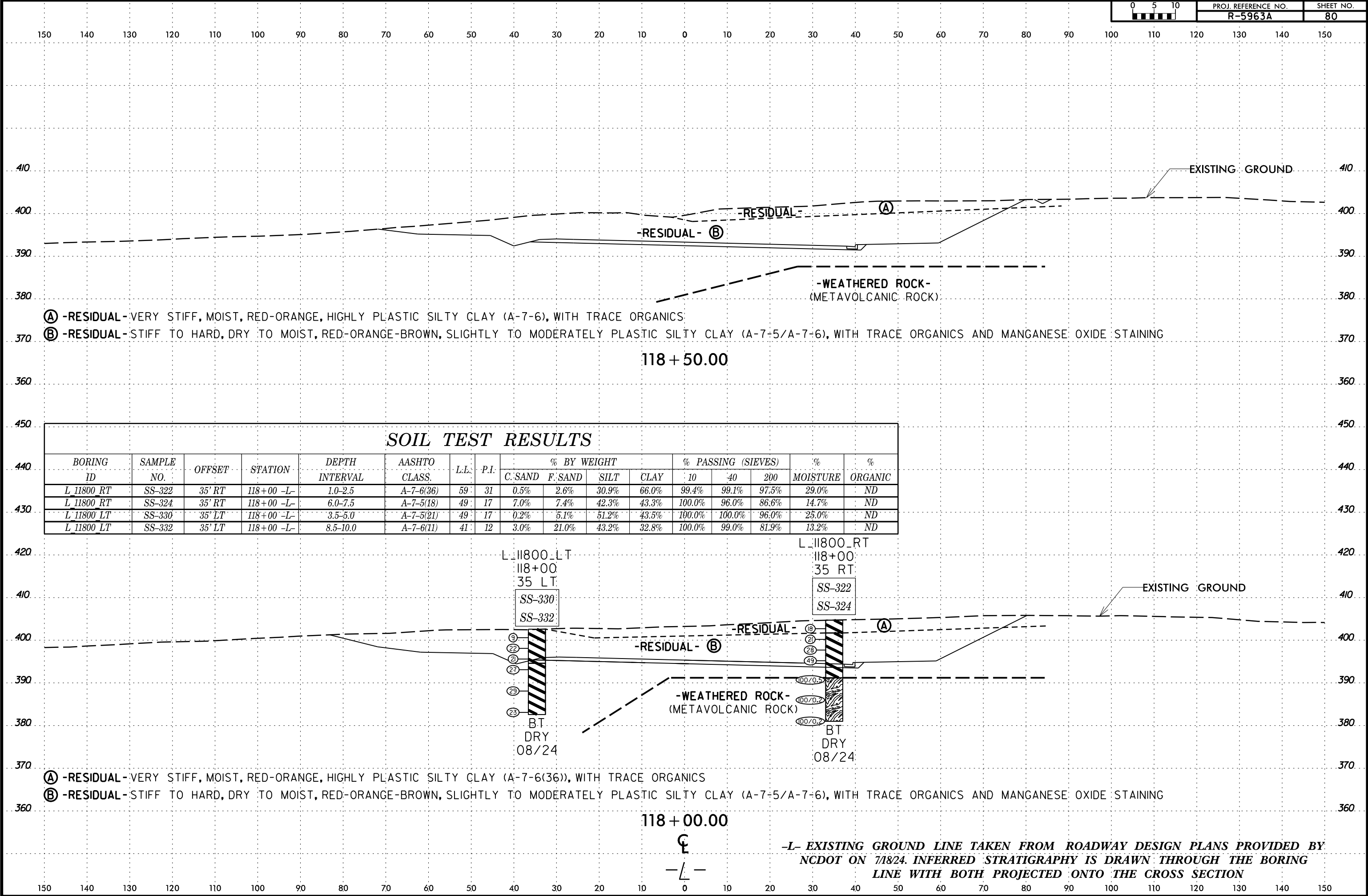


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R-5963A

SHEET NO.
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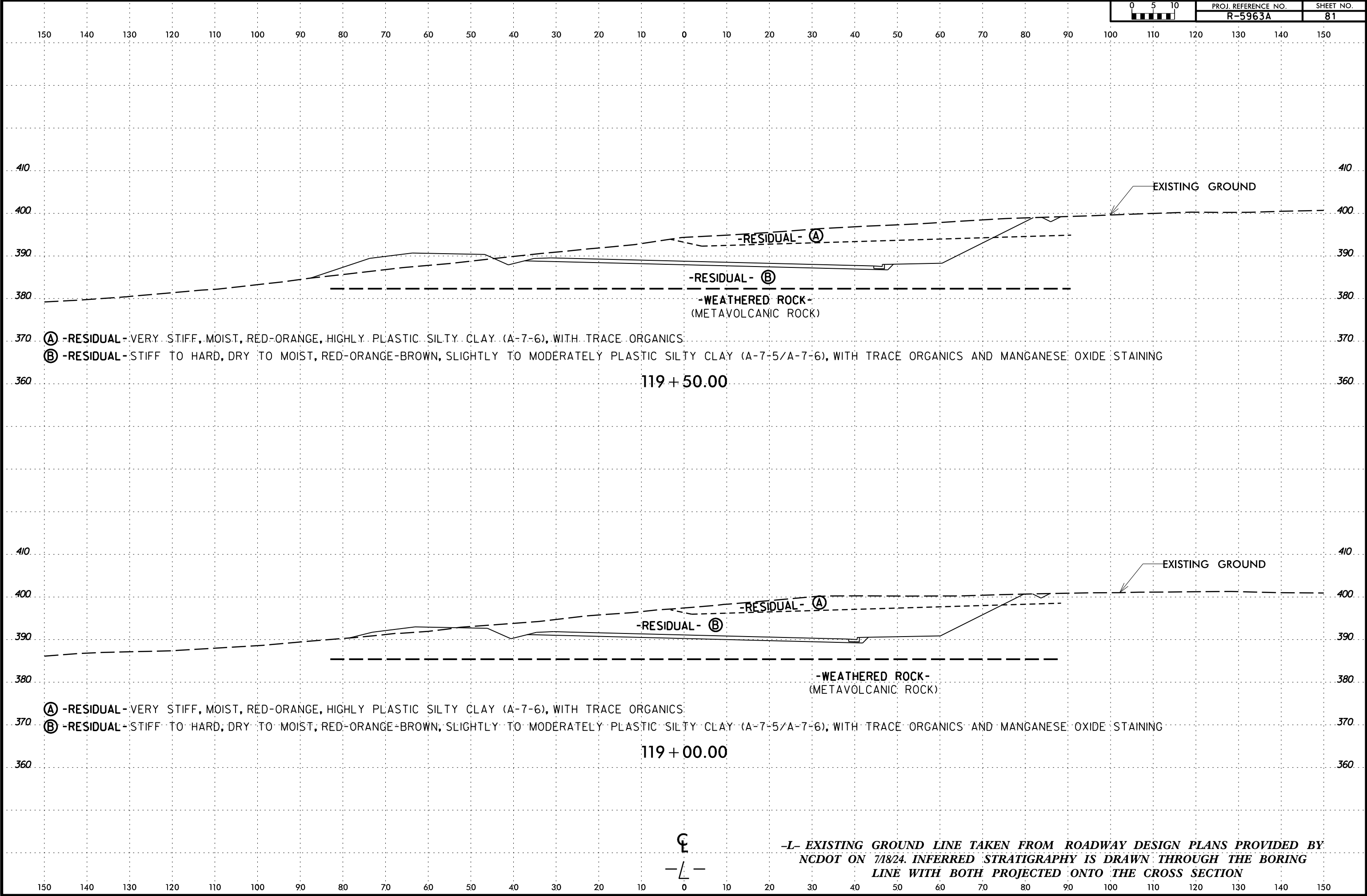


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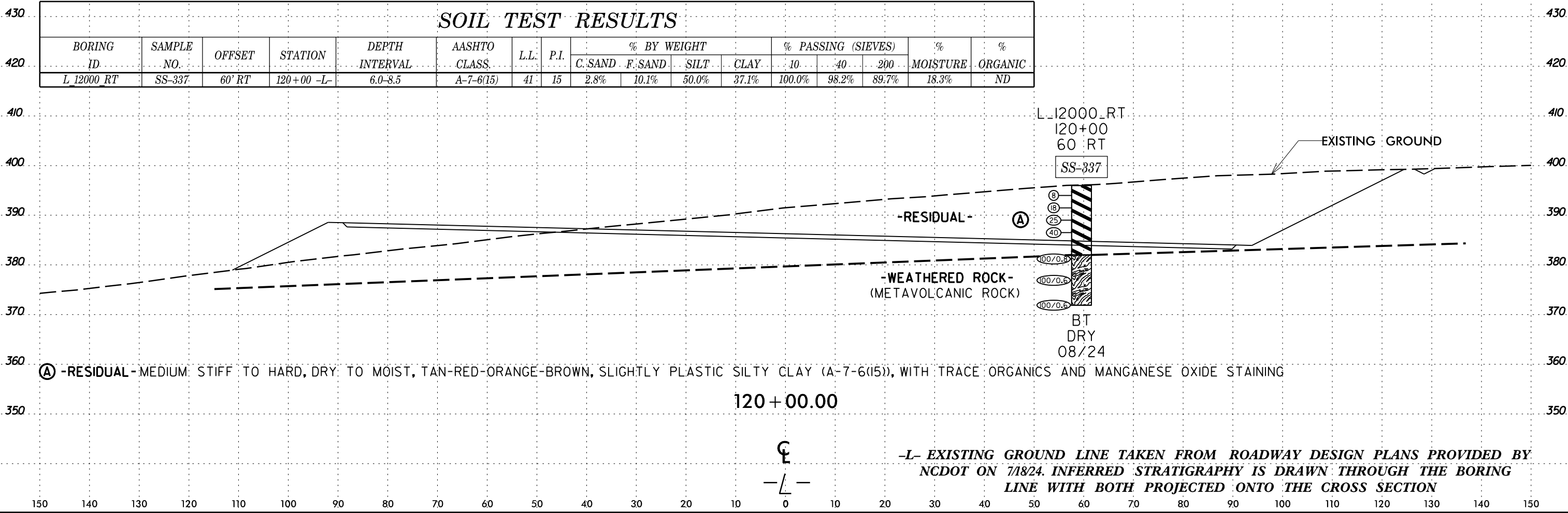
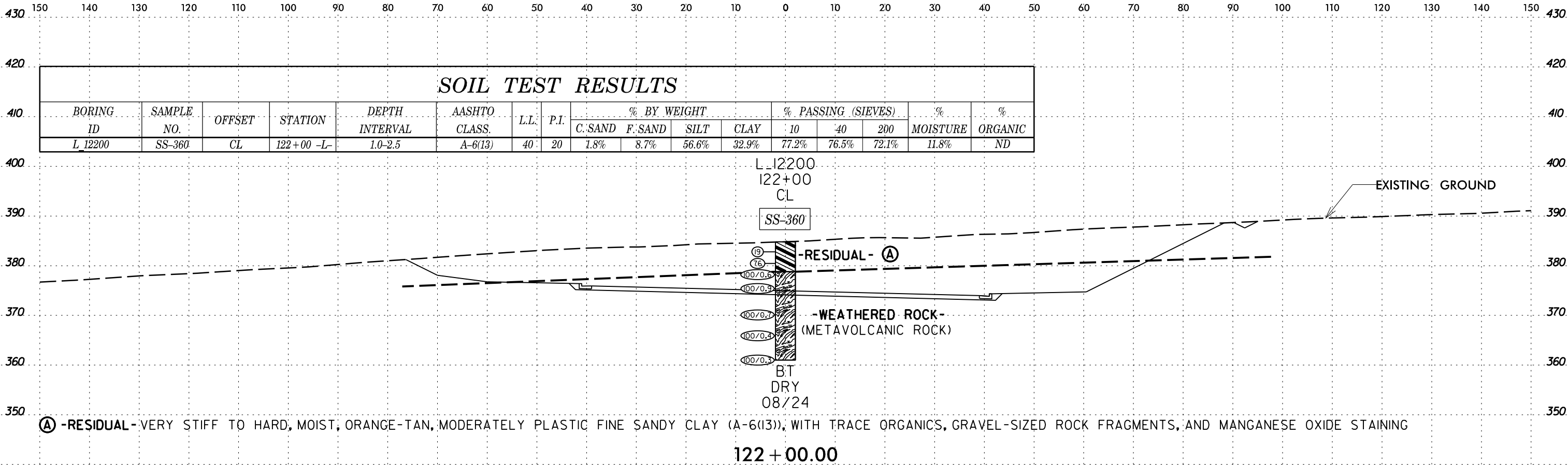


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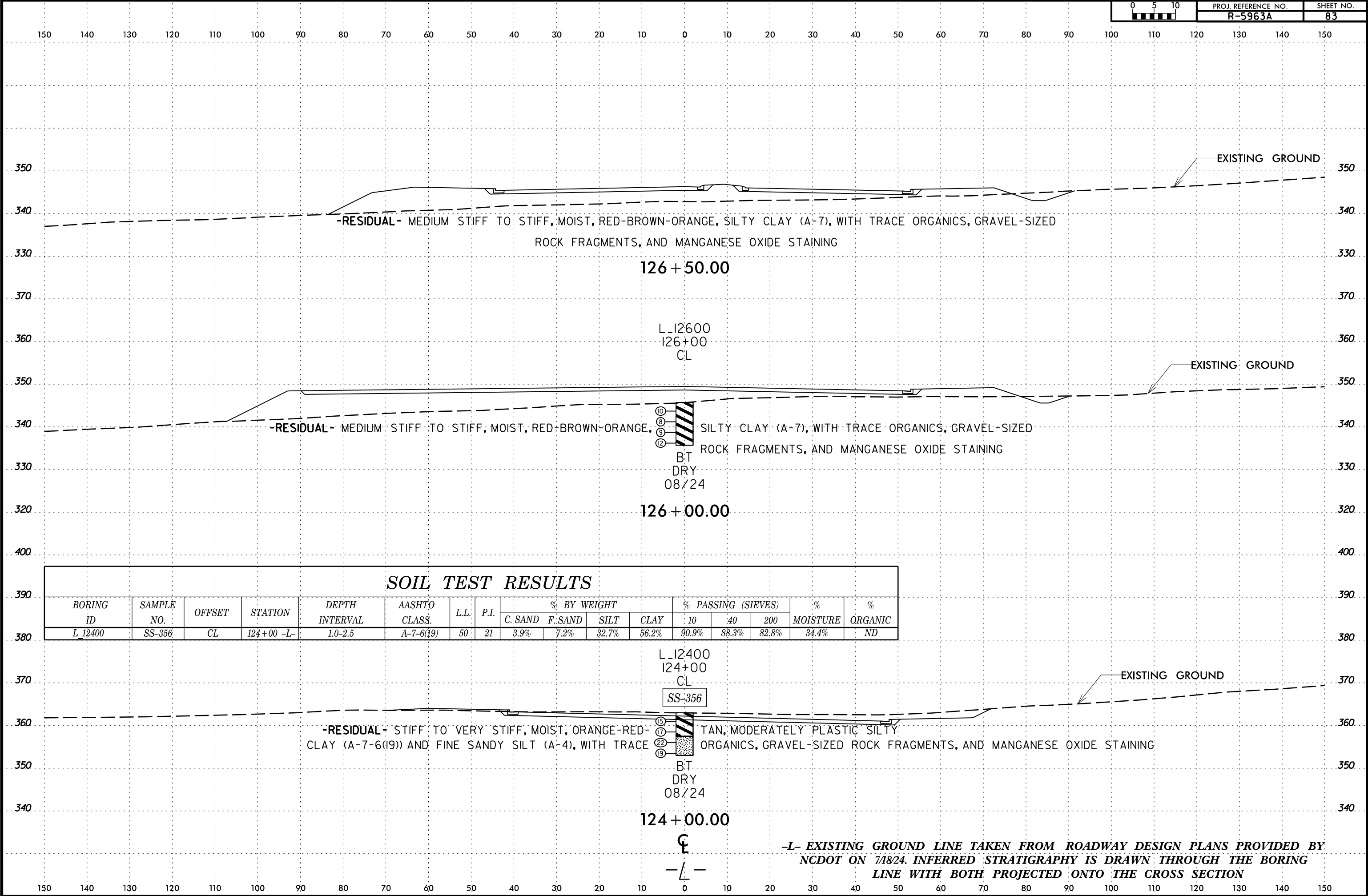
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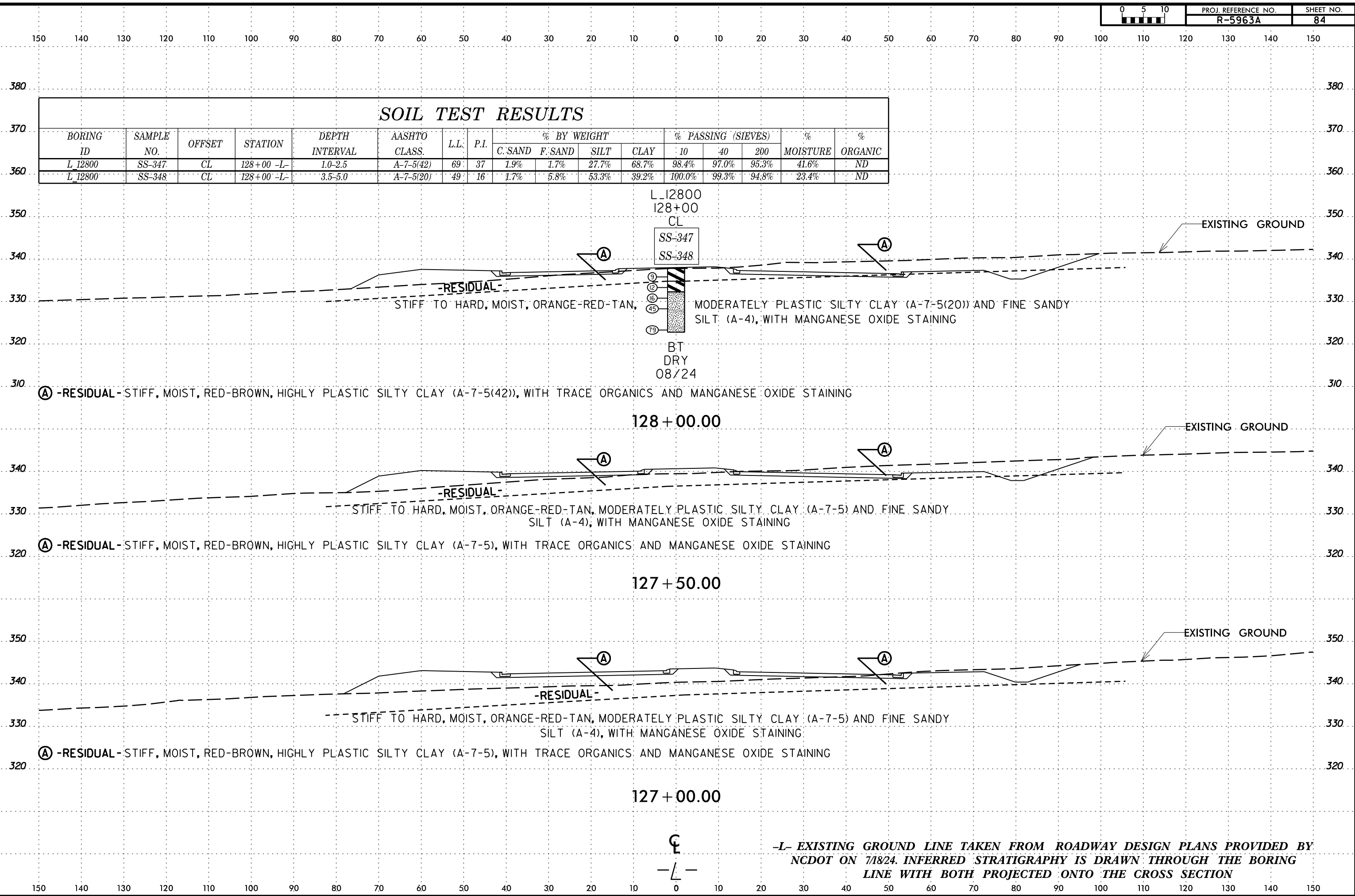


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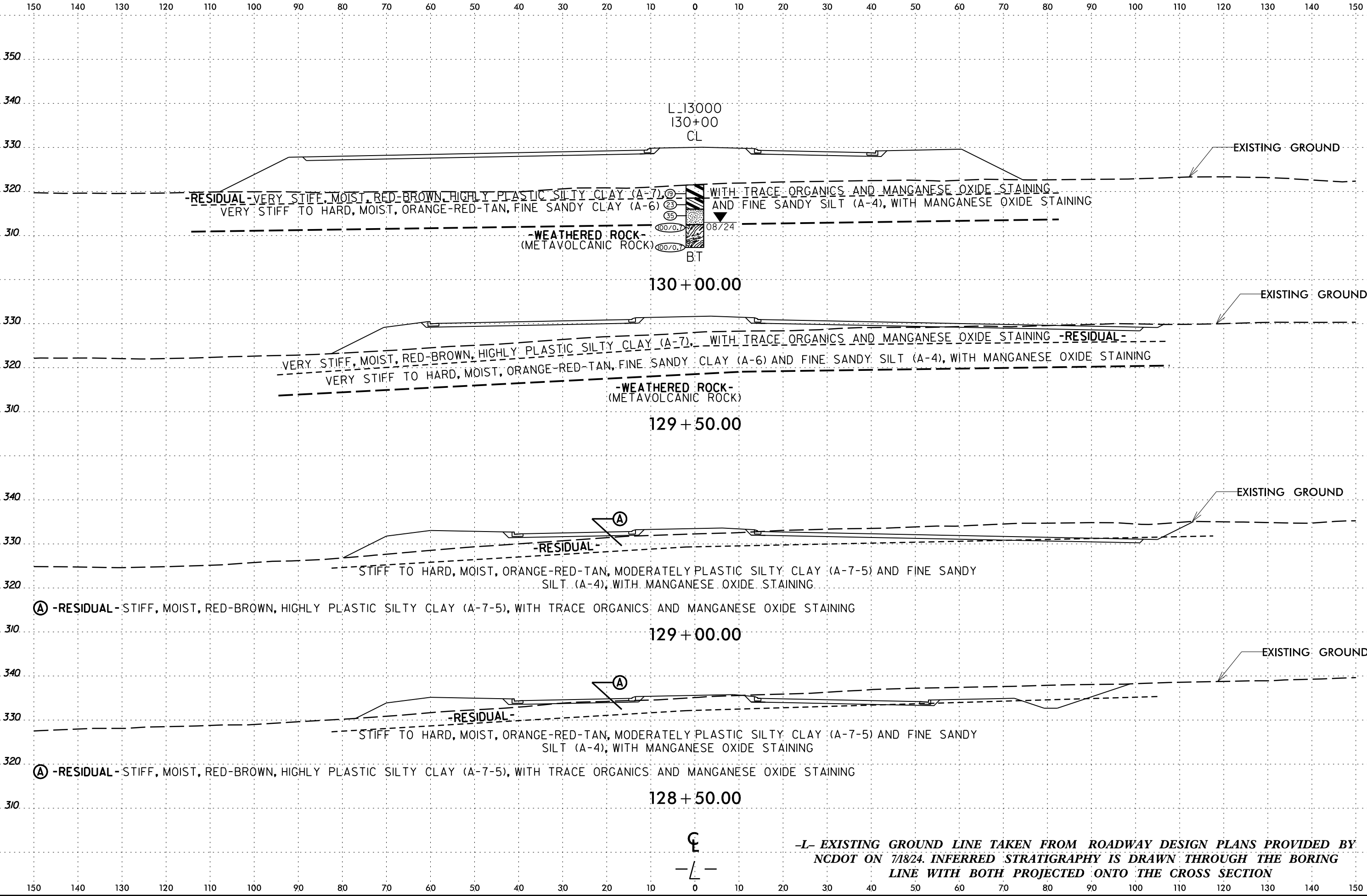
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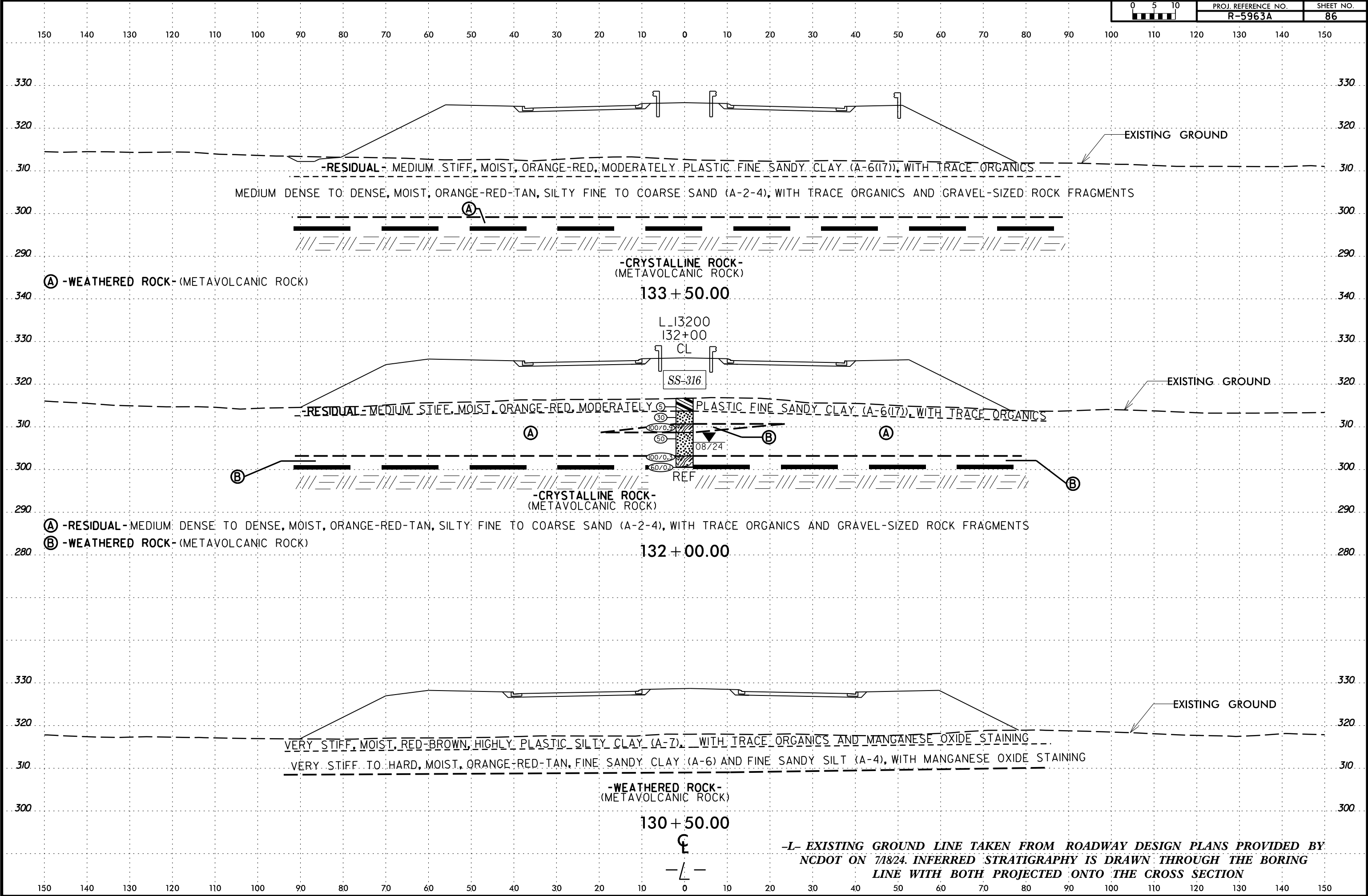
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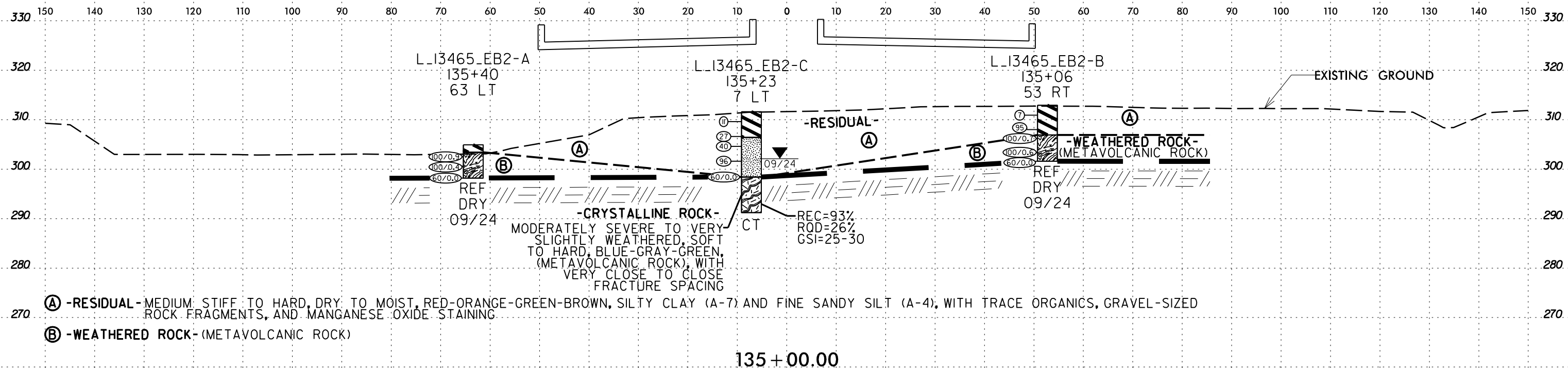
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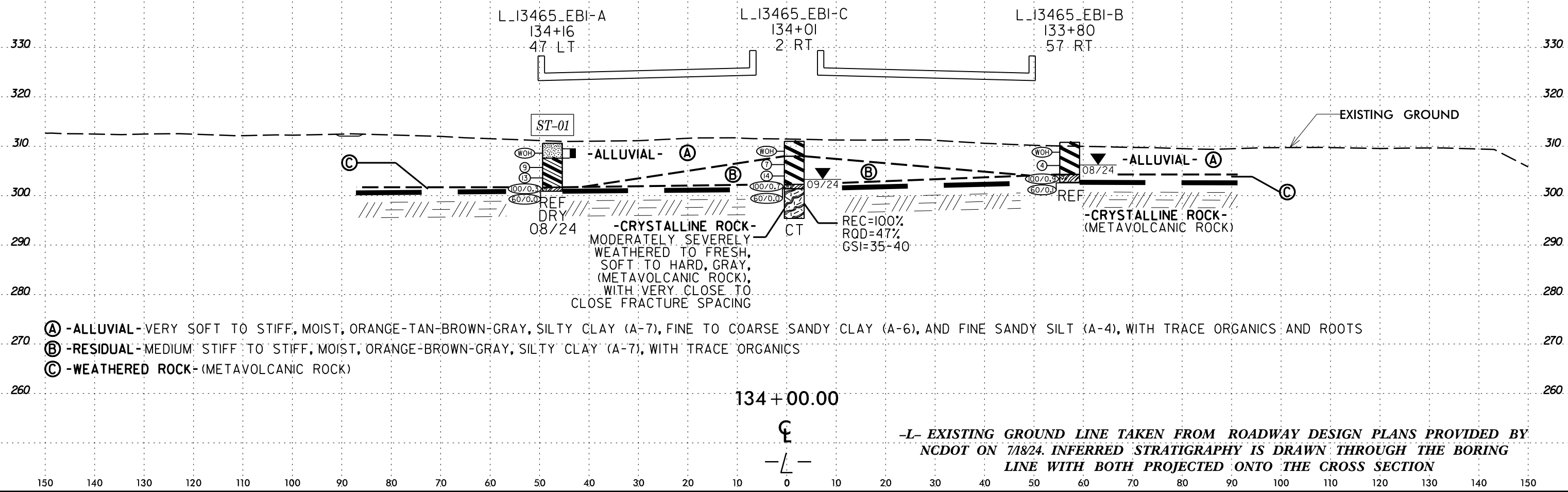
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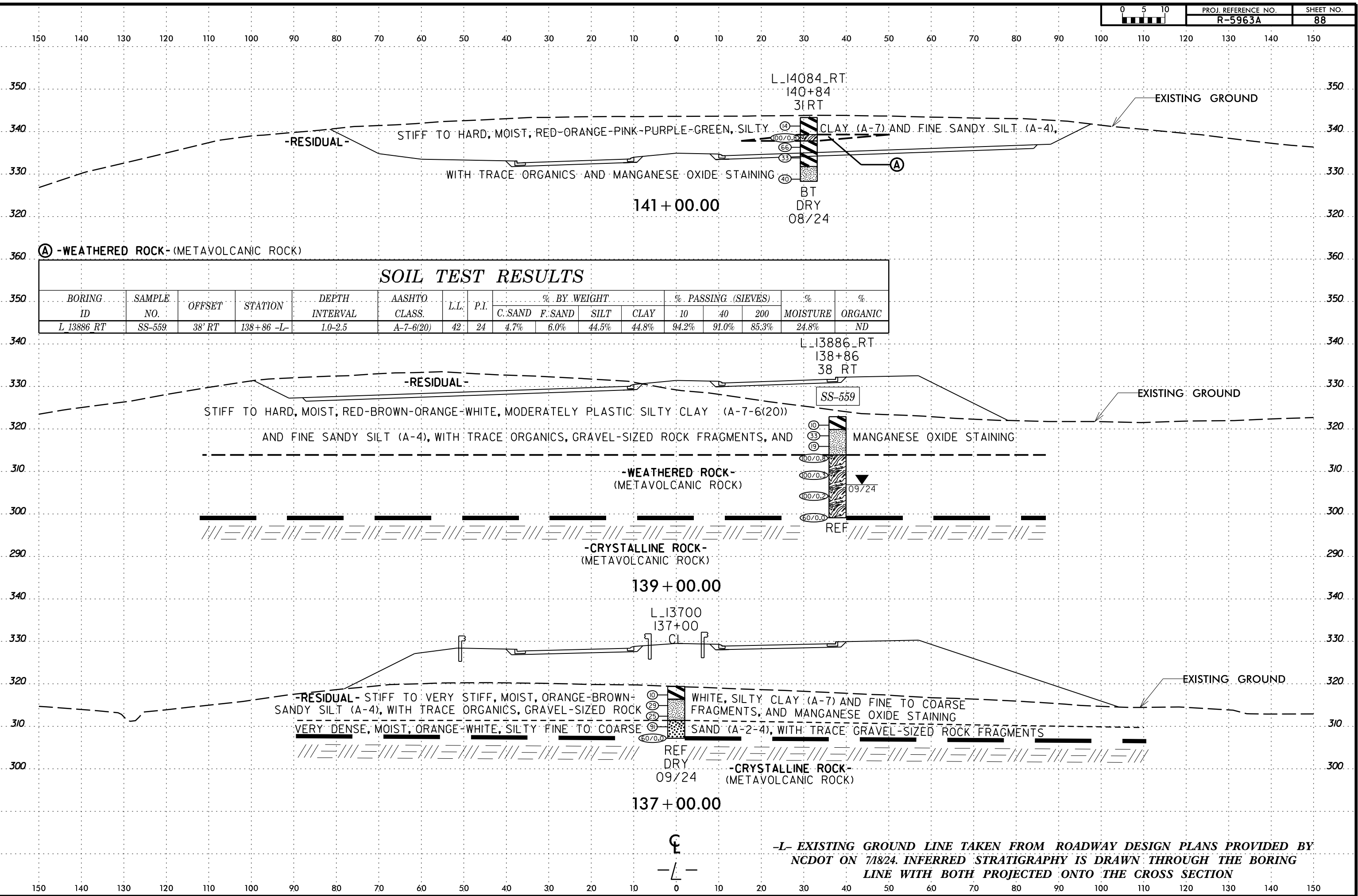
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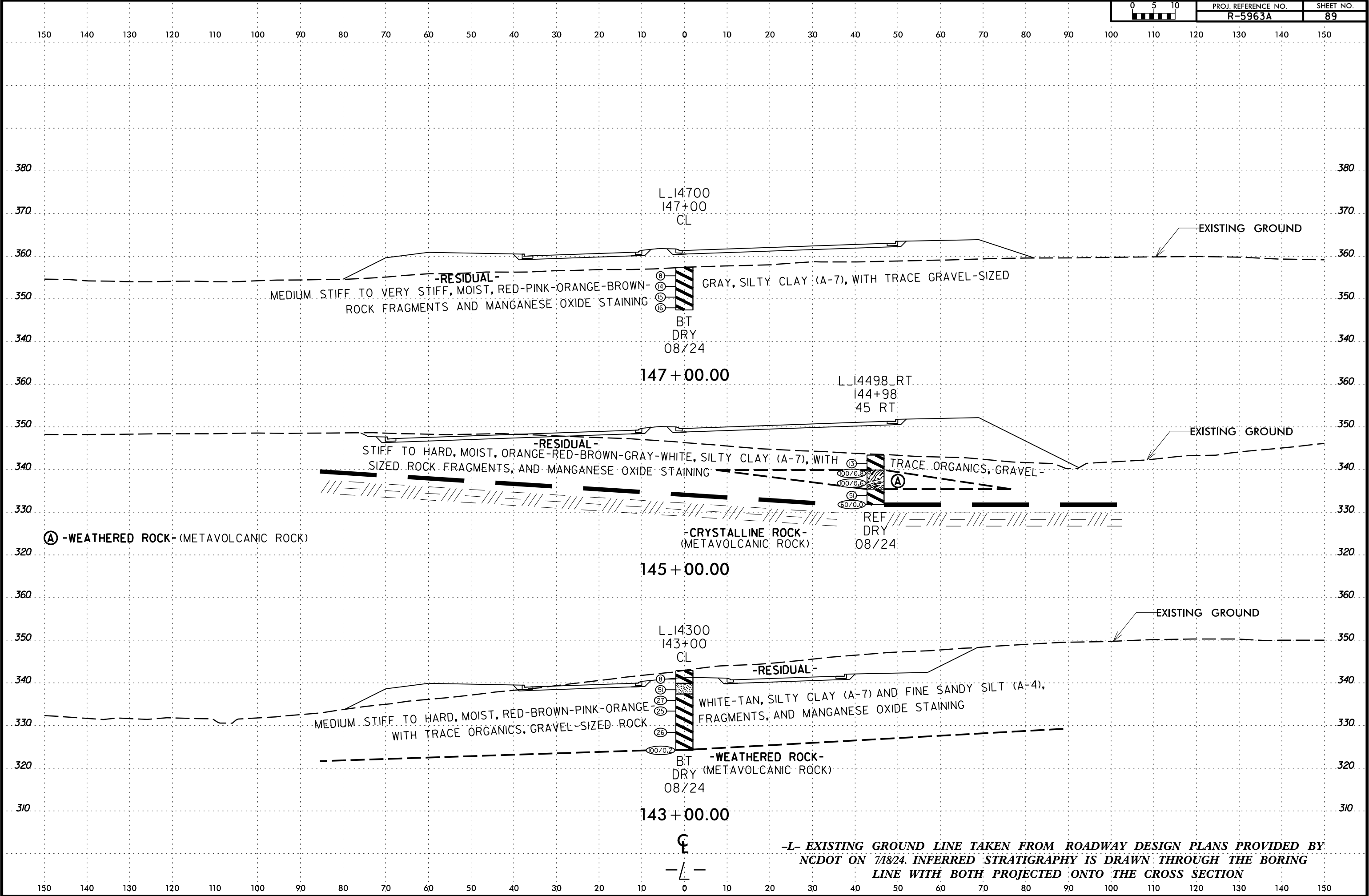
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BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C.SAND	F.SAND	SILT	CLAY	10	40	200		
L 13456 EB1-A	ST-01	47' LT	134+16 -L-	1.0-3.0	A-4(4)	26	8	7.8%	22.9%	35.0%	34.3%	100.0%	98.1%	73.8%	22.0%	2.4%

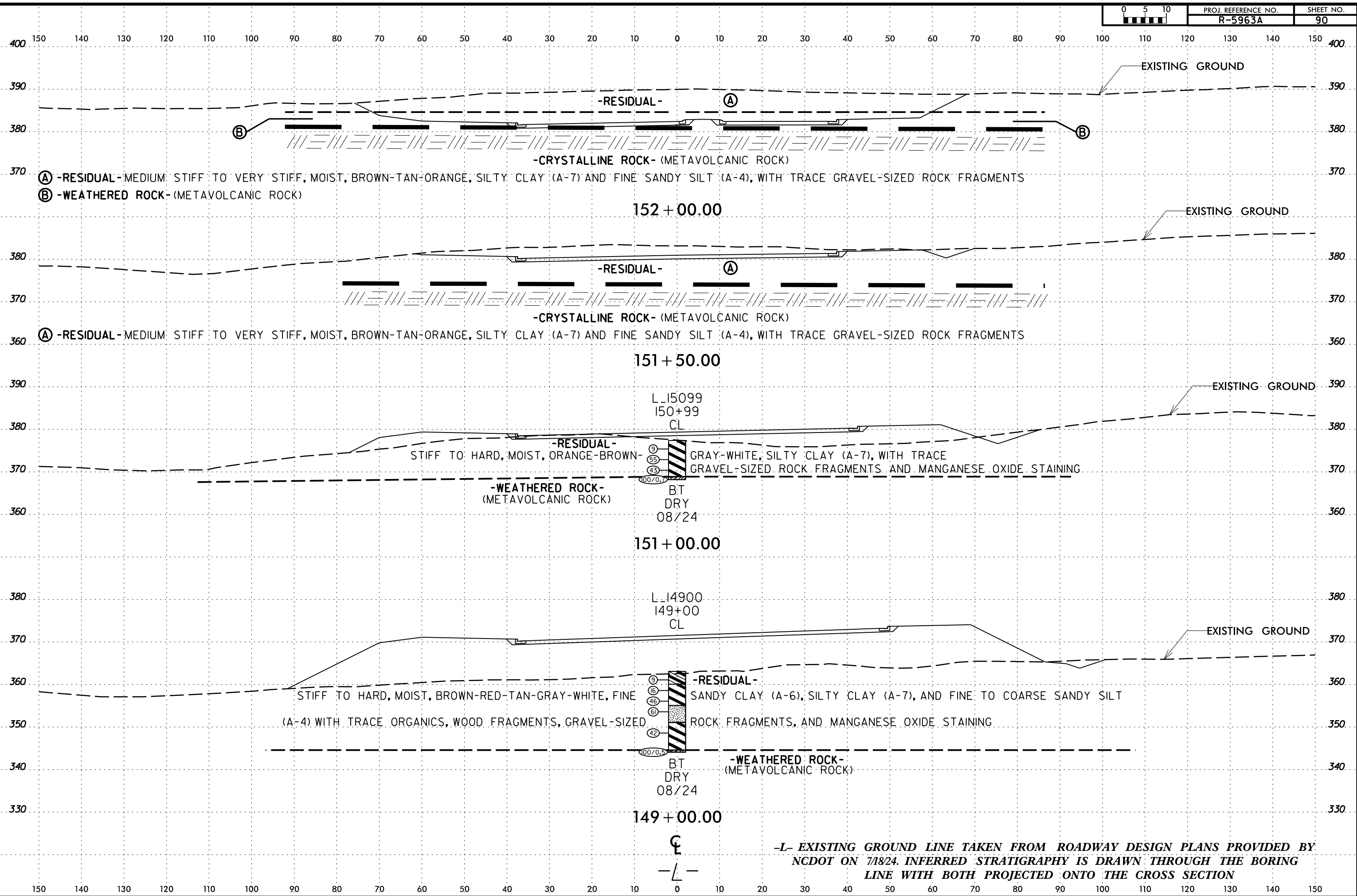




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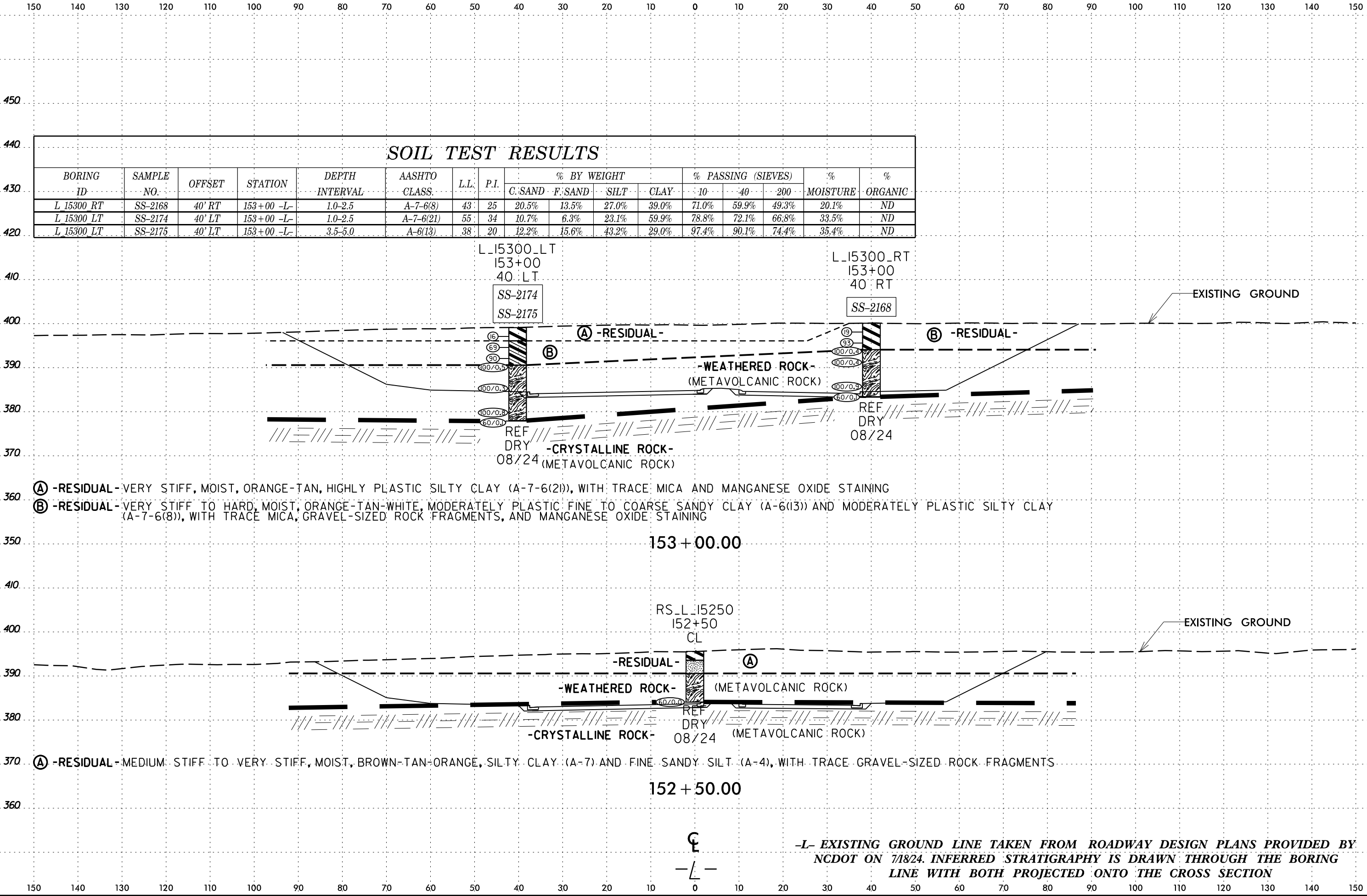
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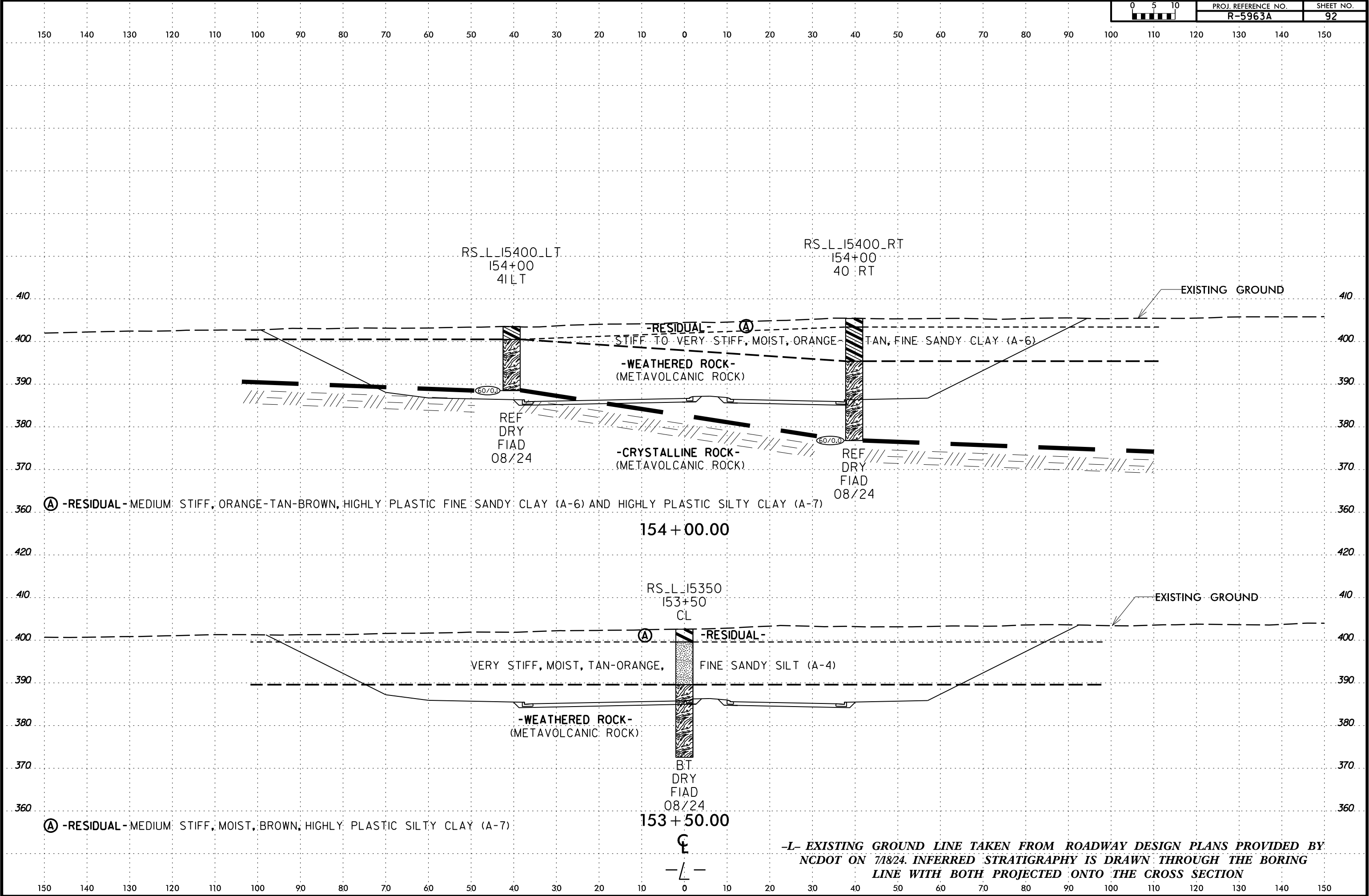
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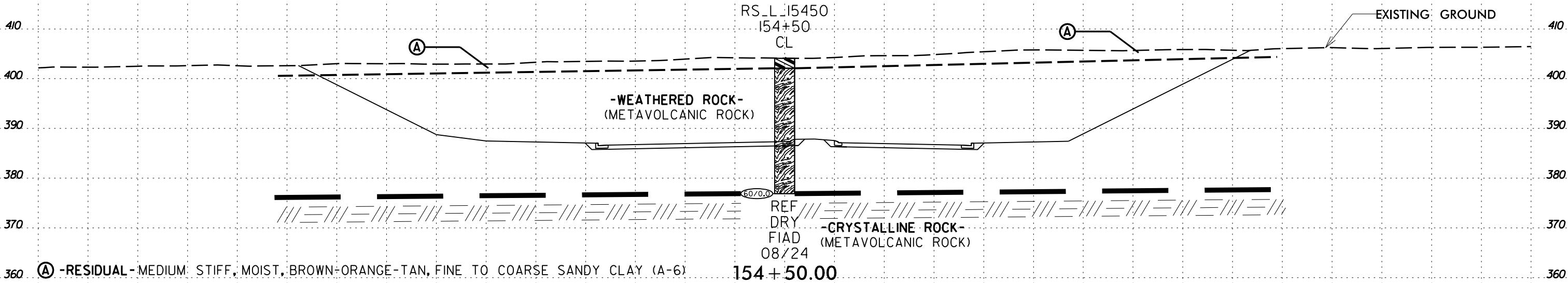
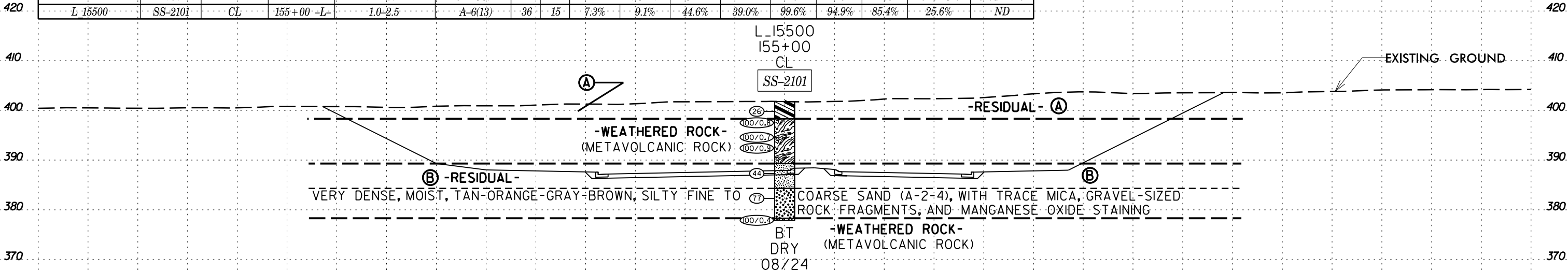
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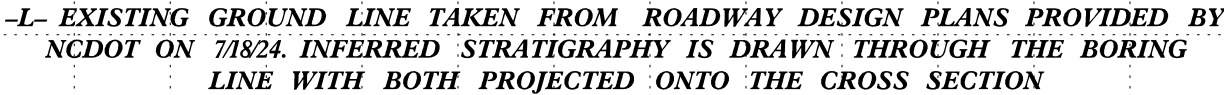


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SOIL TEST RESULTS																
BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
L 15500	SS-2101	CL	155+00 -L-	1.0-2.5	A-6(13)	36	15	7.3%	9.1%	44.6%	39.0%	99.6%	94.9%	85.4%	25.6%	ND

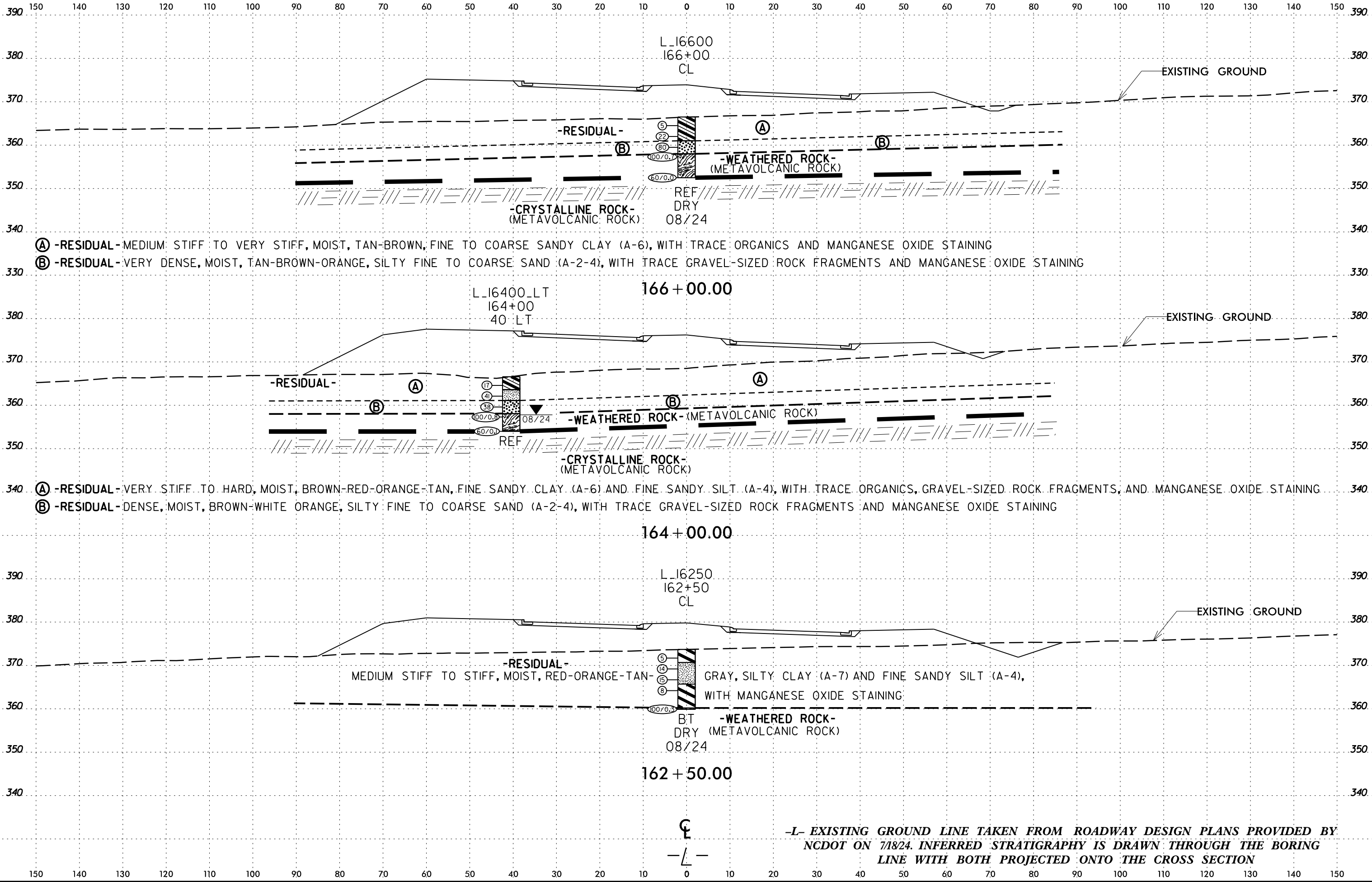


-L- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION



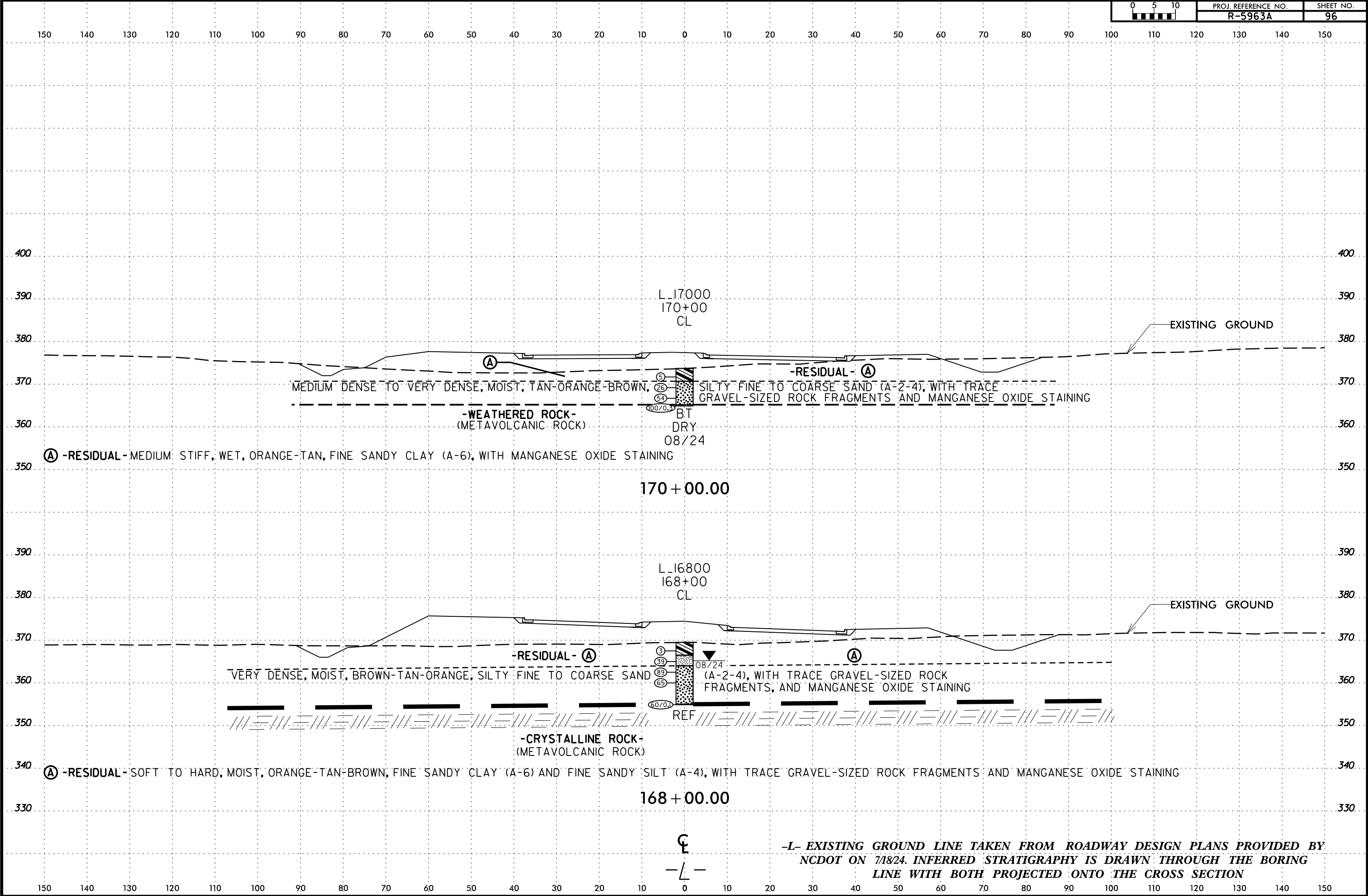
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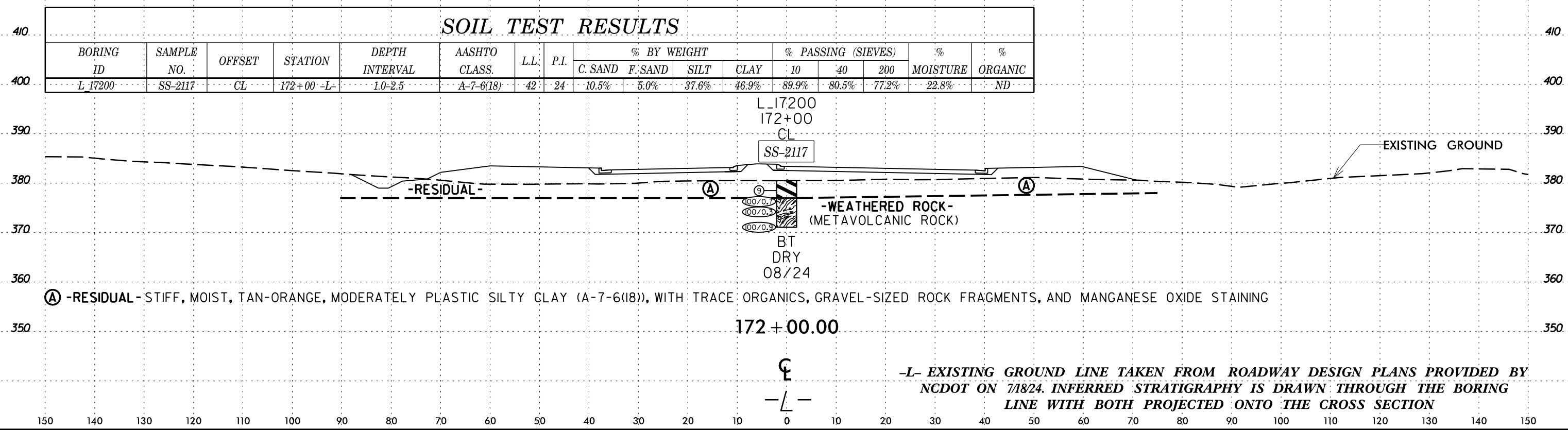
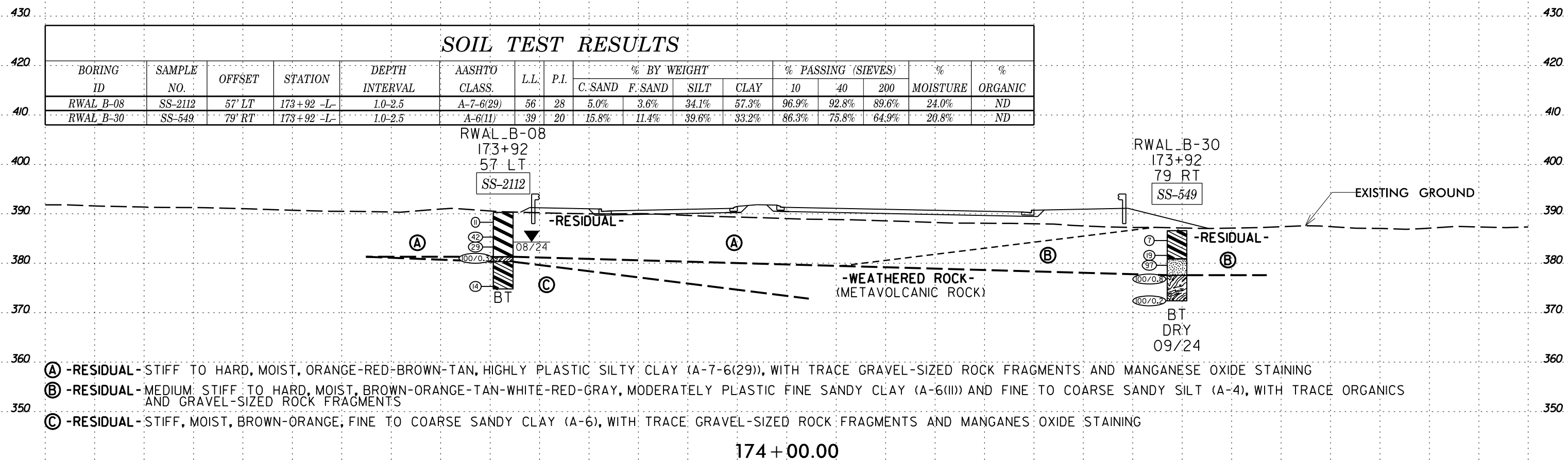
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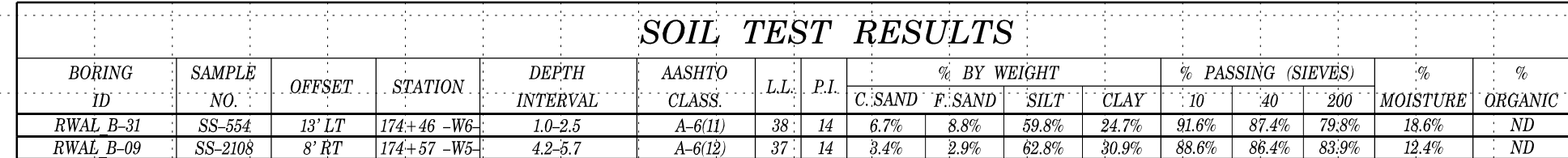
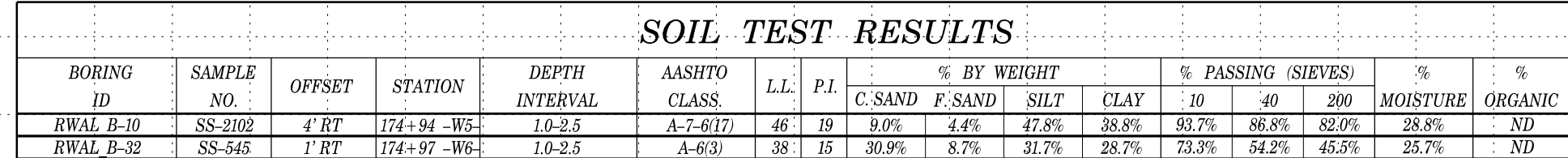


6/23/16

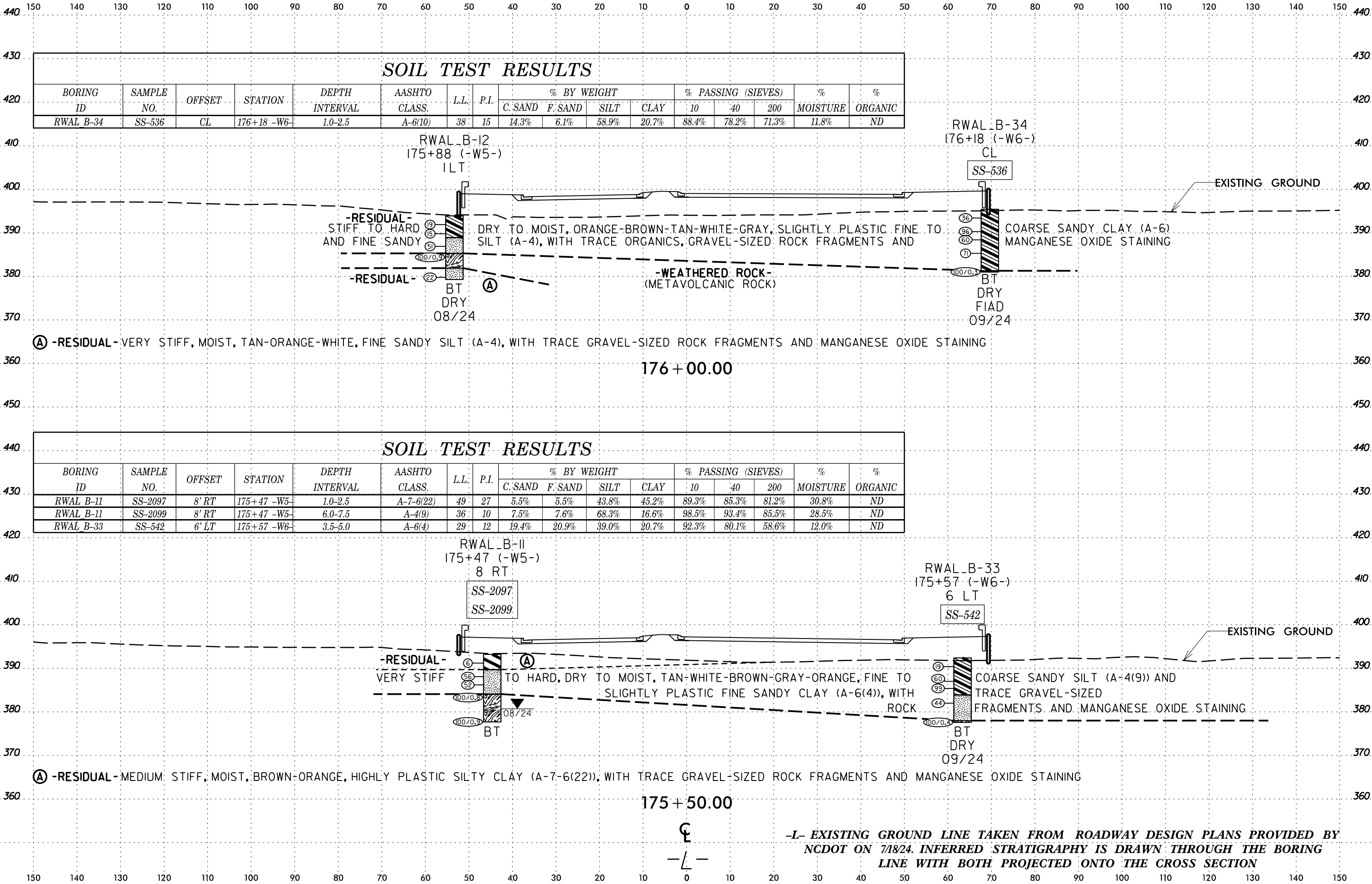
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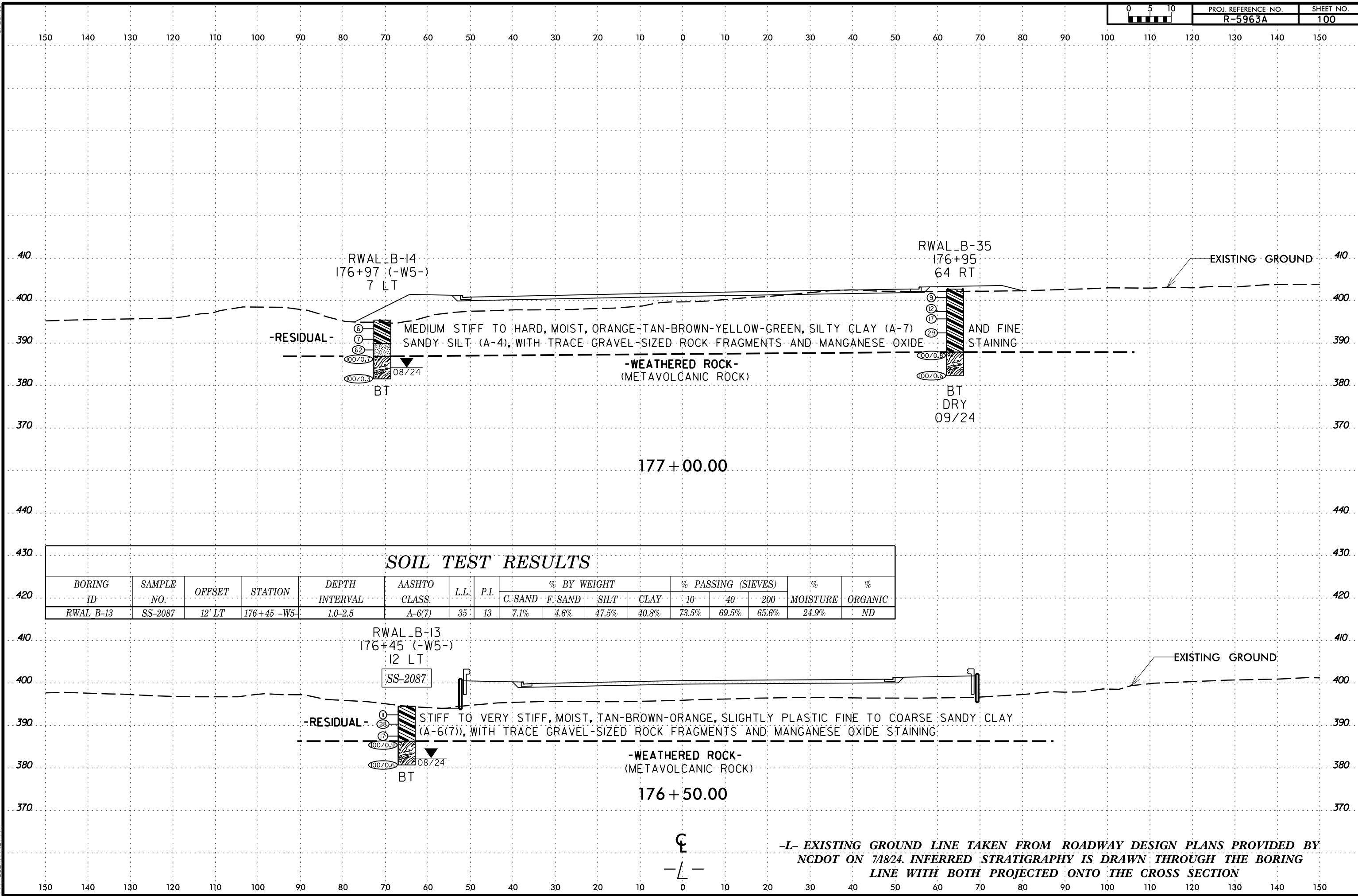






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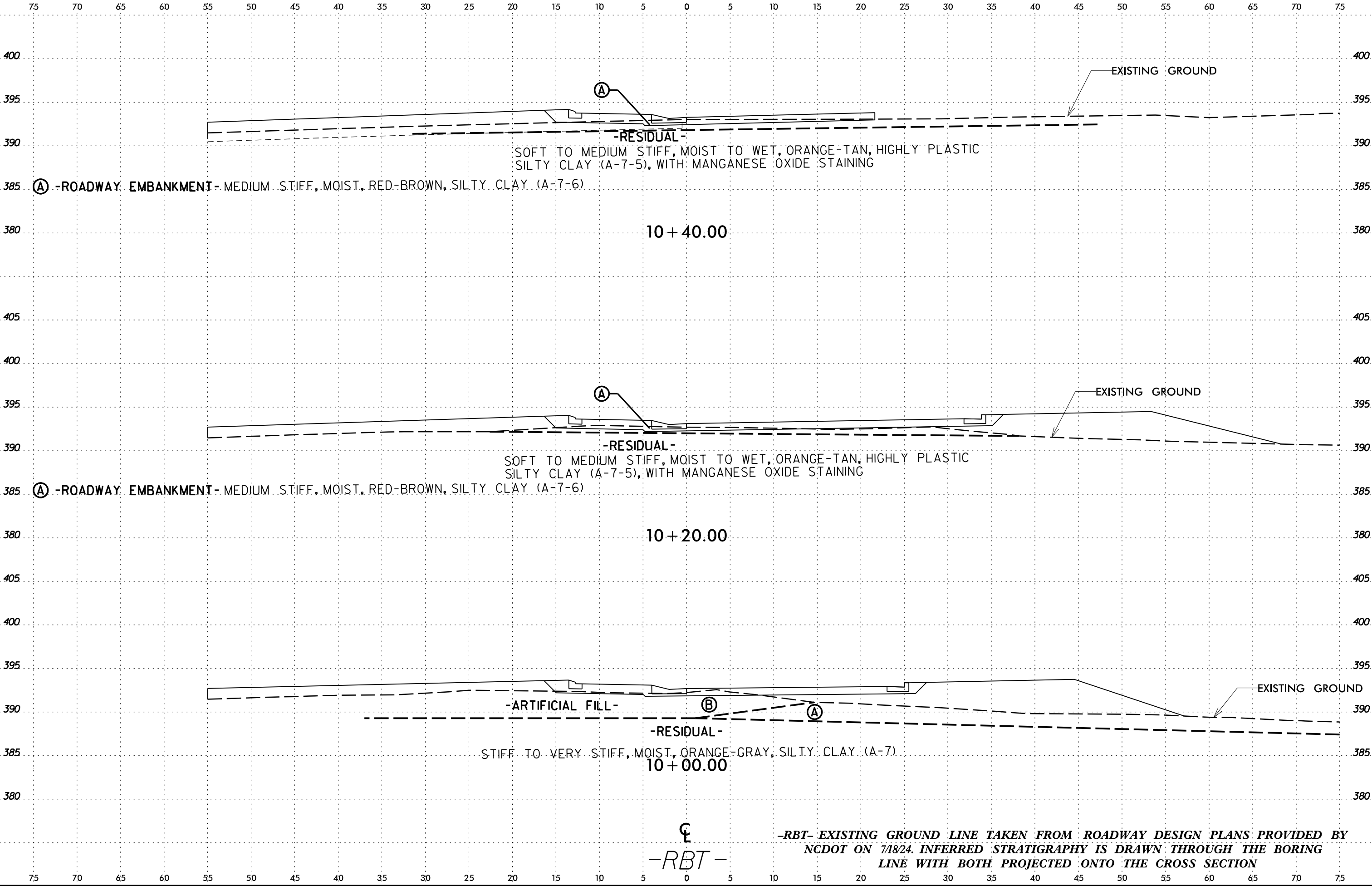
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PROJ. REFERENCE NO.
R-5963A

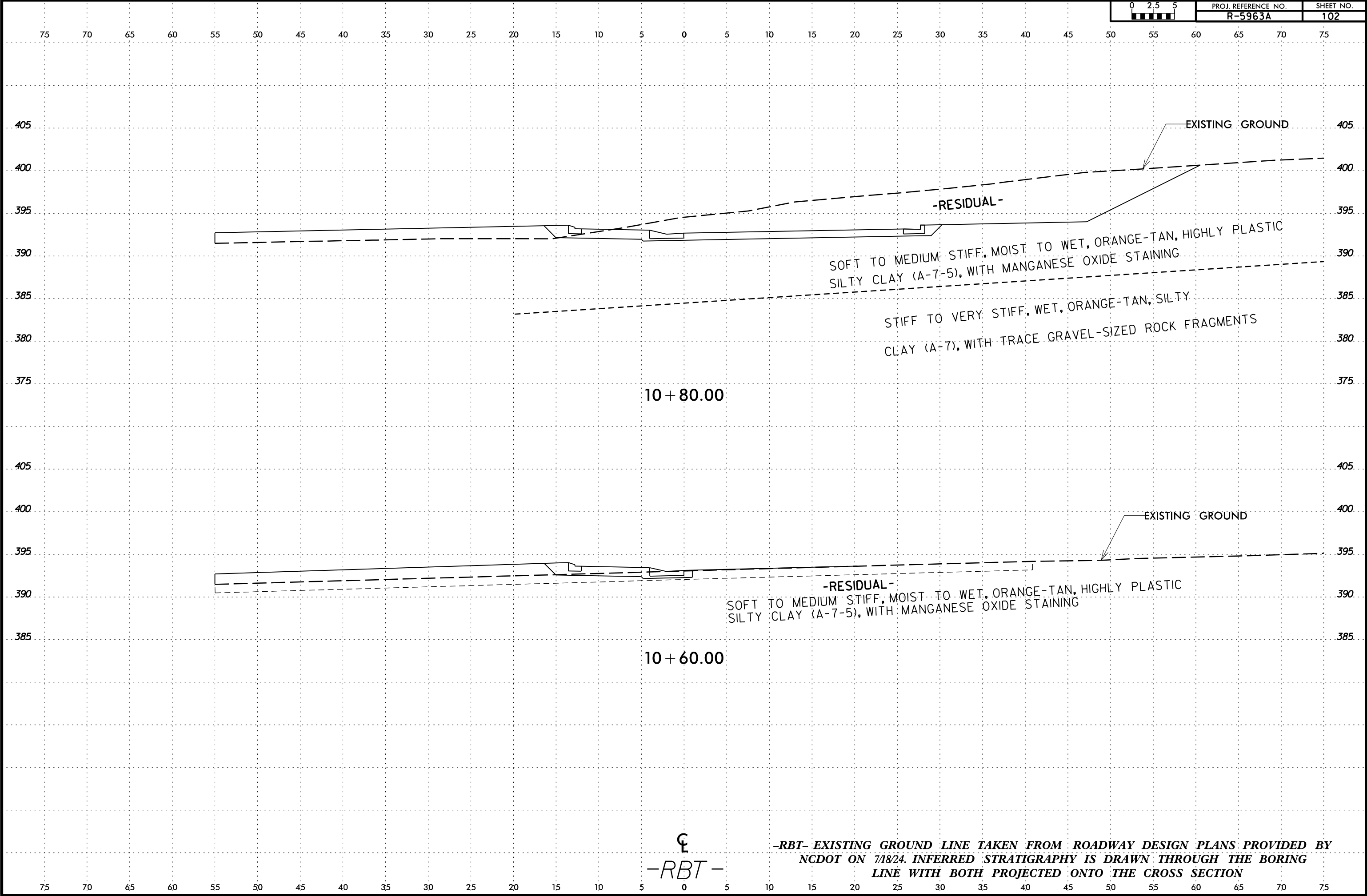
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101

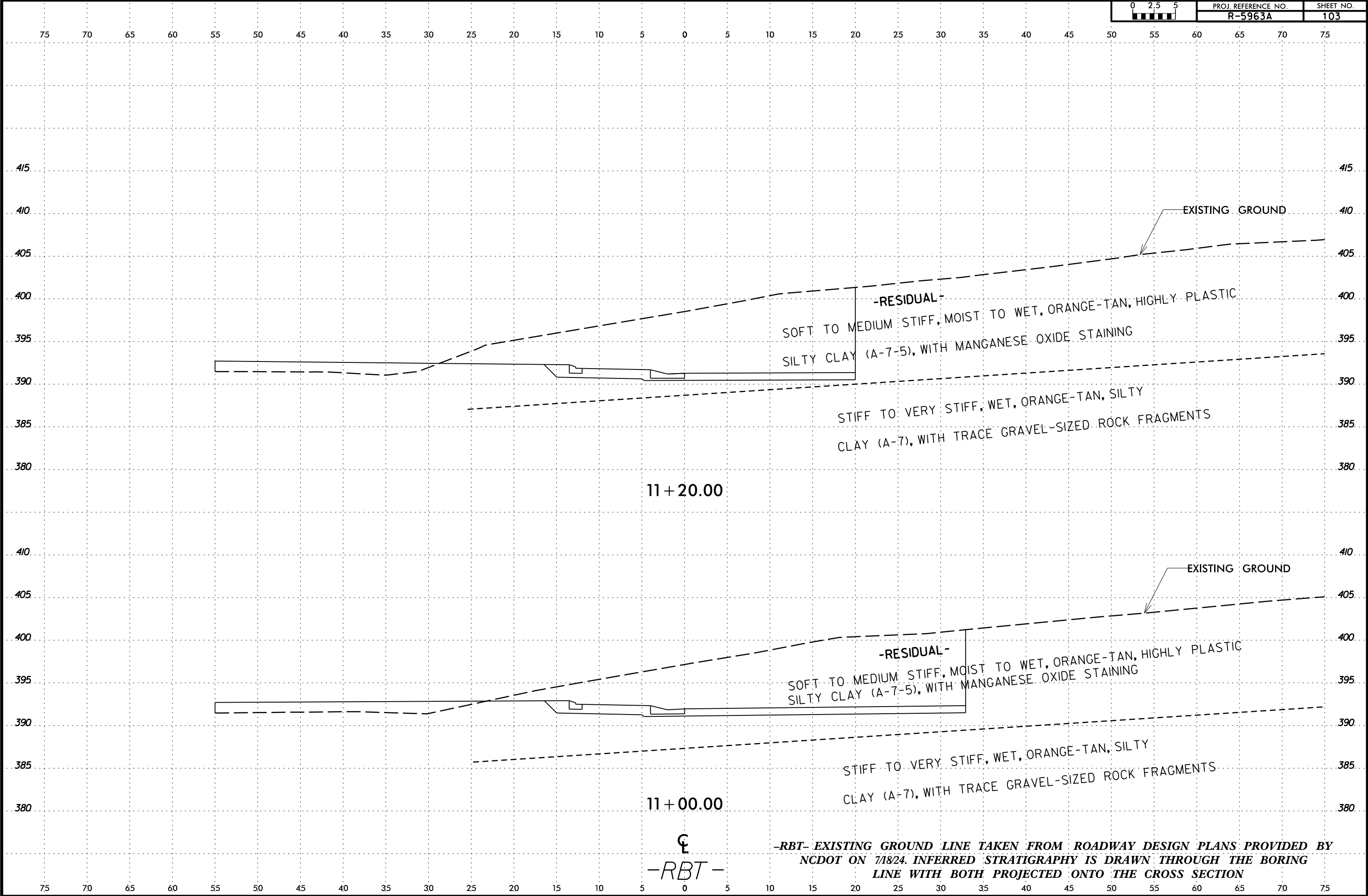


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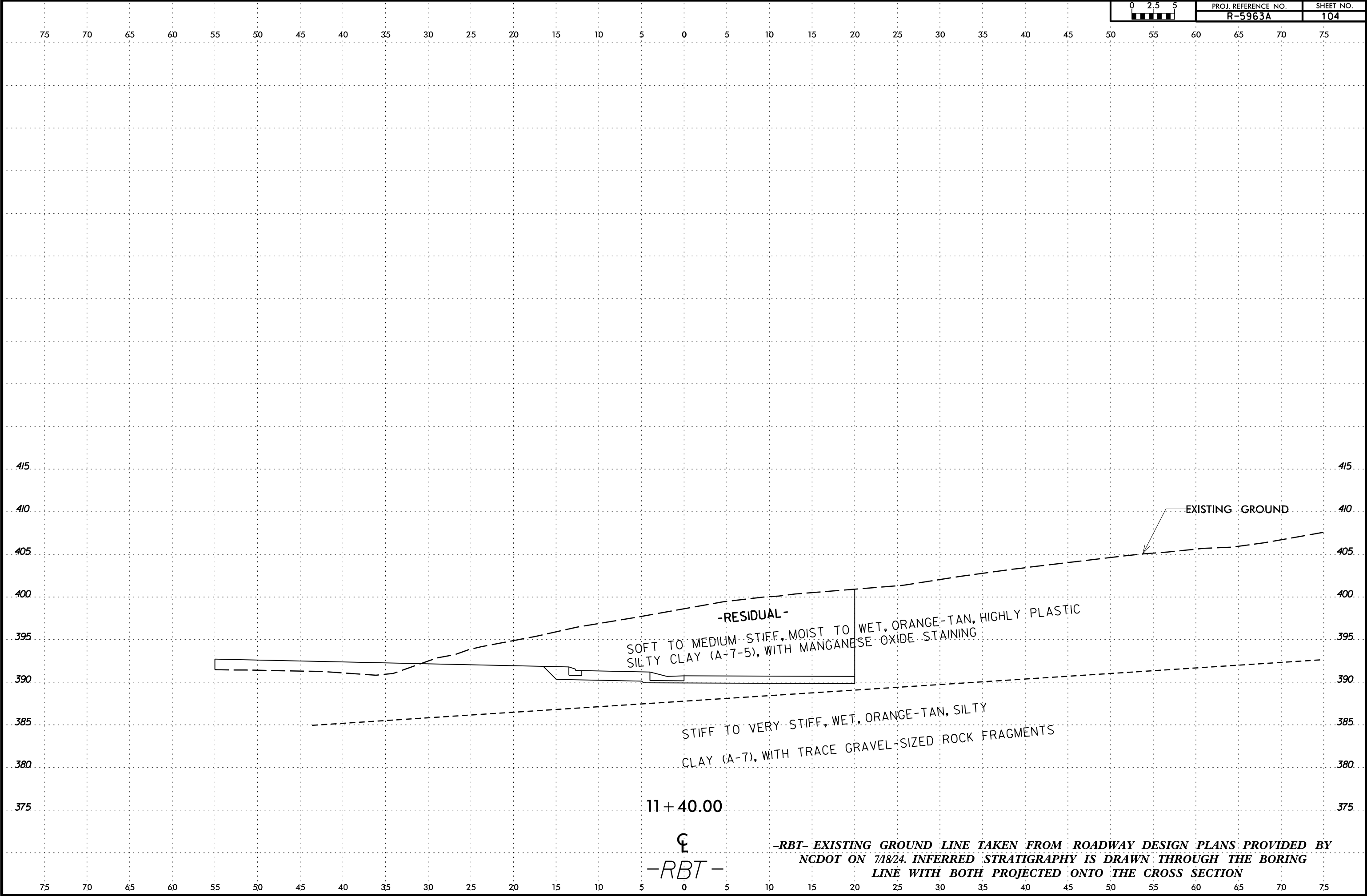
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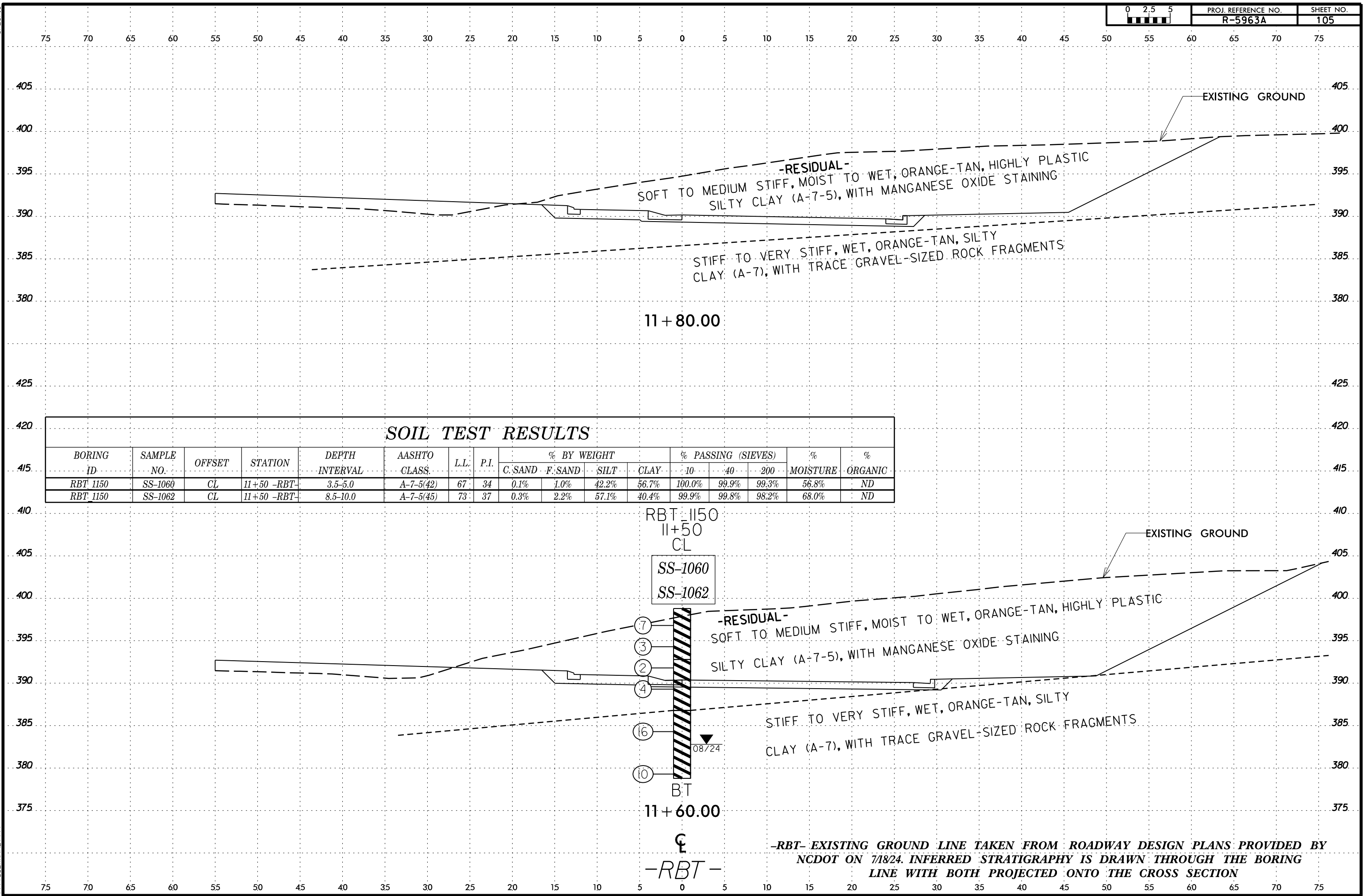




-RBT- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

-RBT-

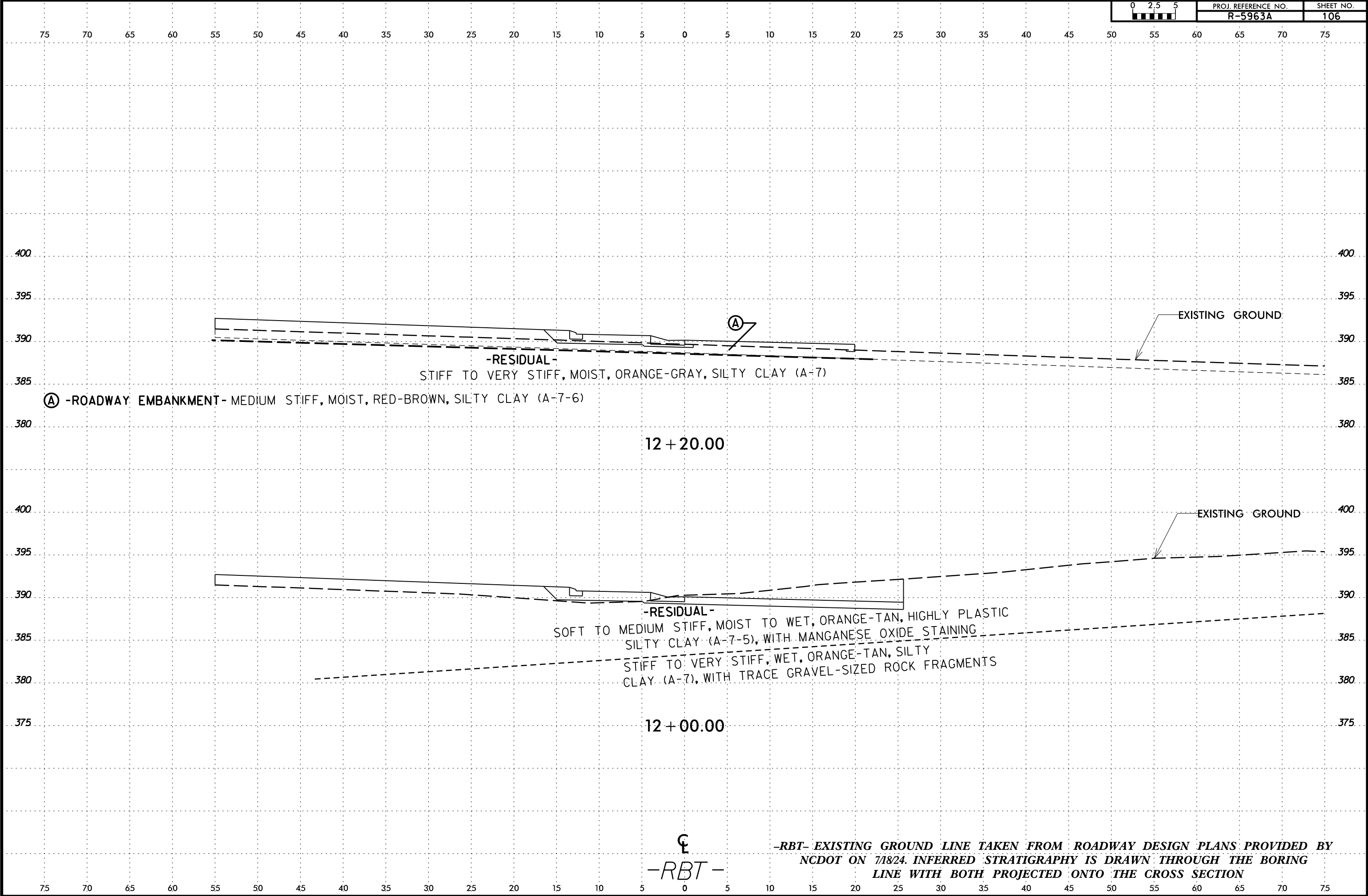




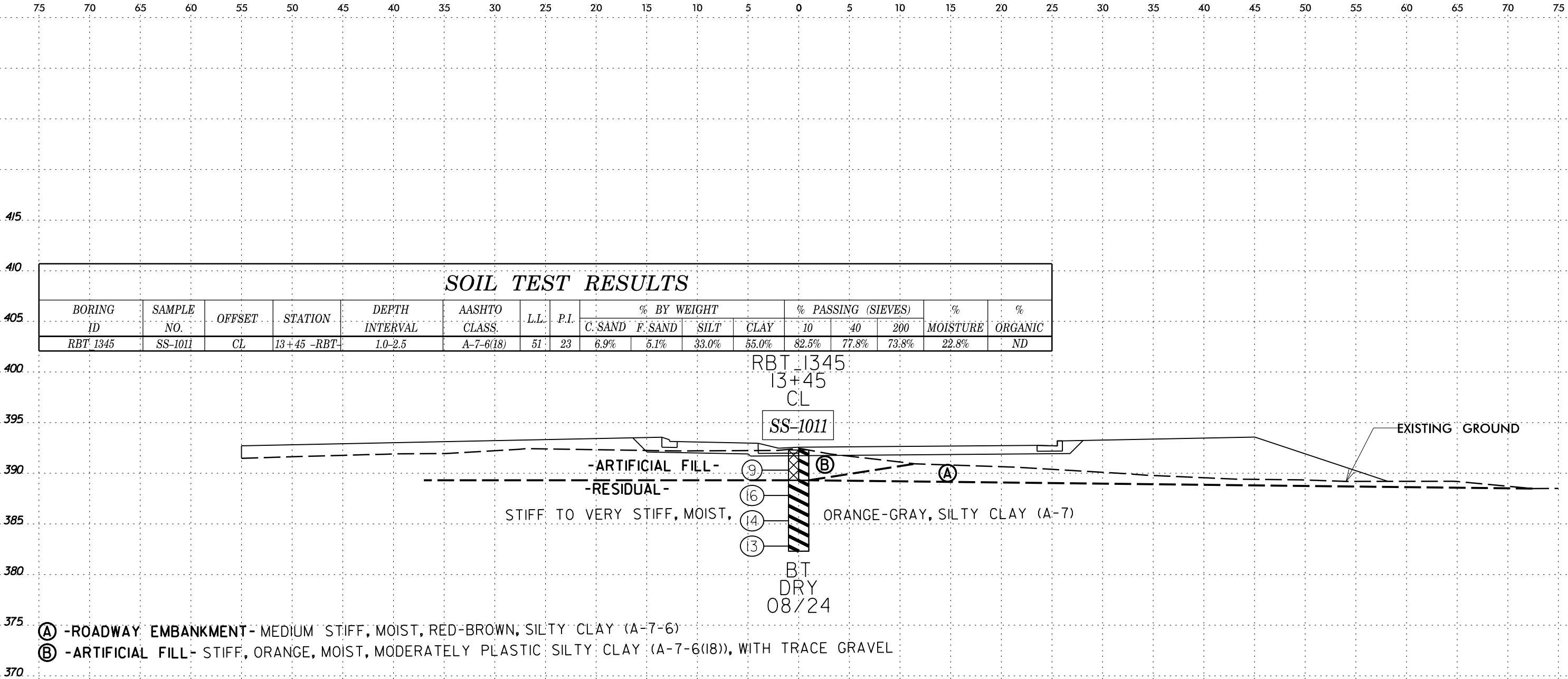
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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	R-5963A	106



6/23/16
14-NOV-2024 18:53
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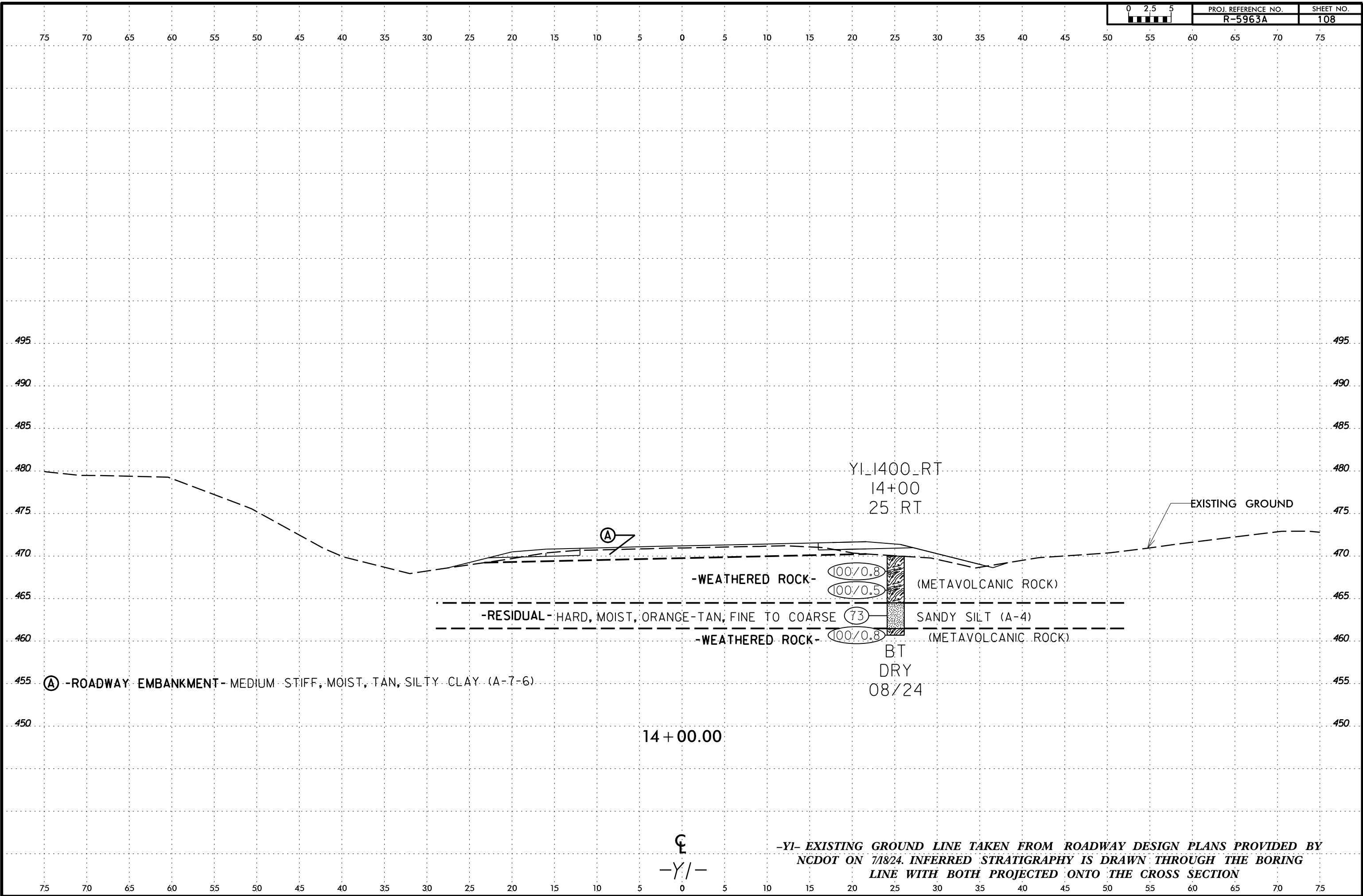


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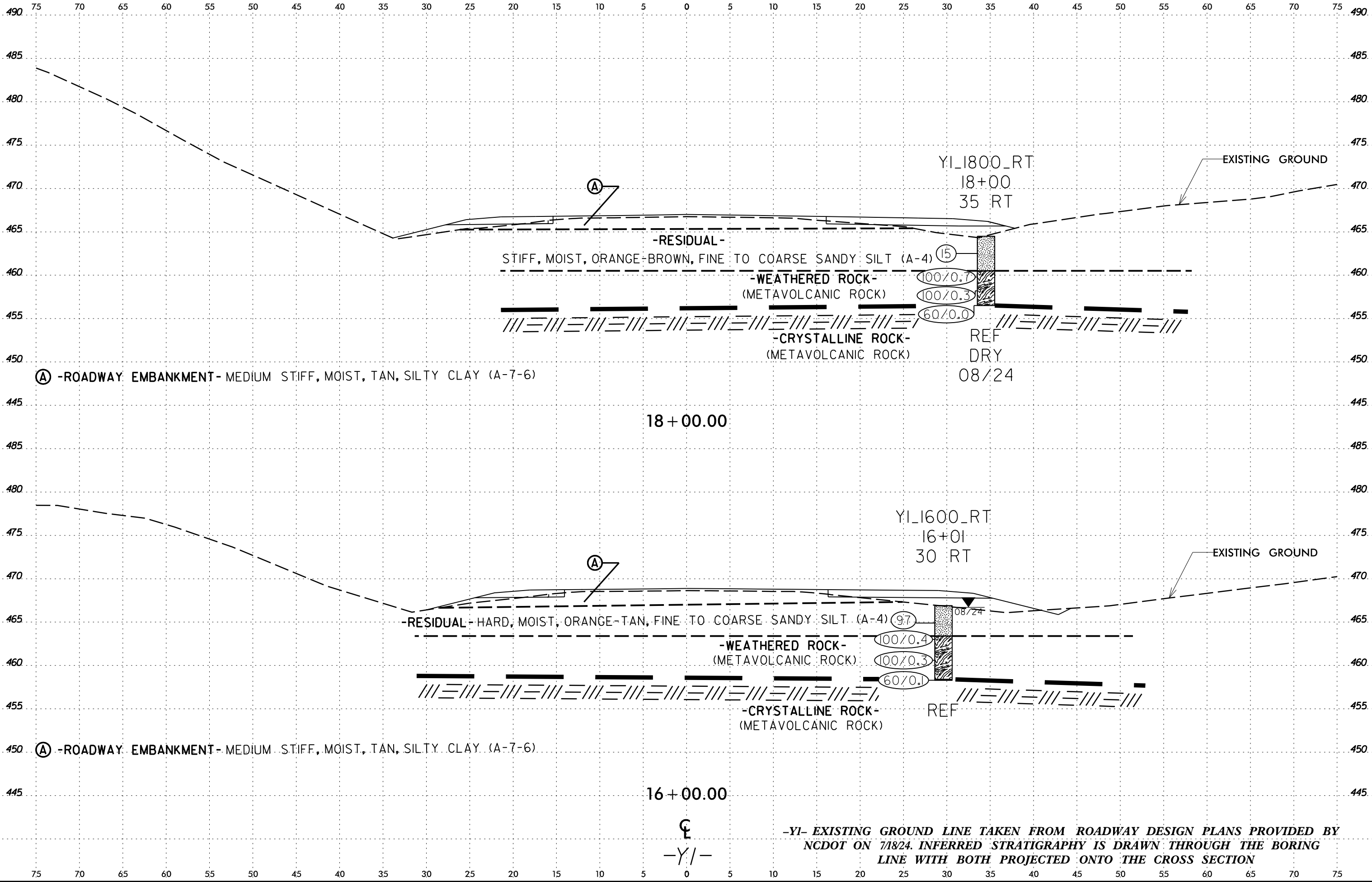
-RBT-

-RBT- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

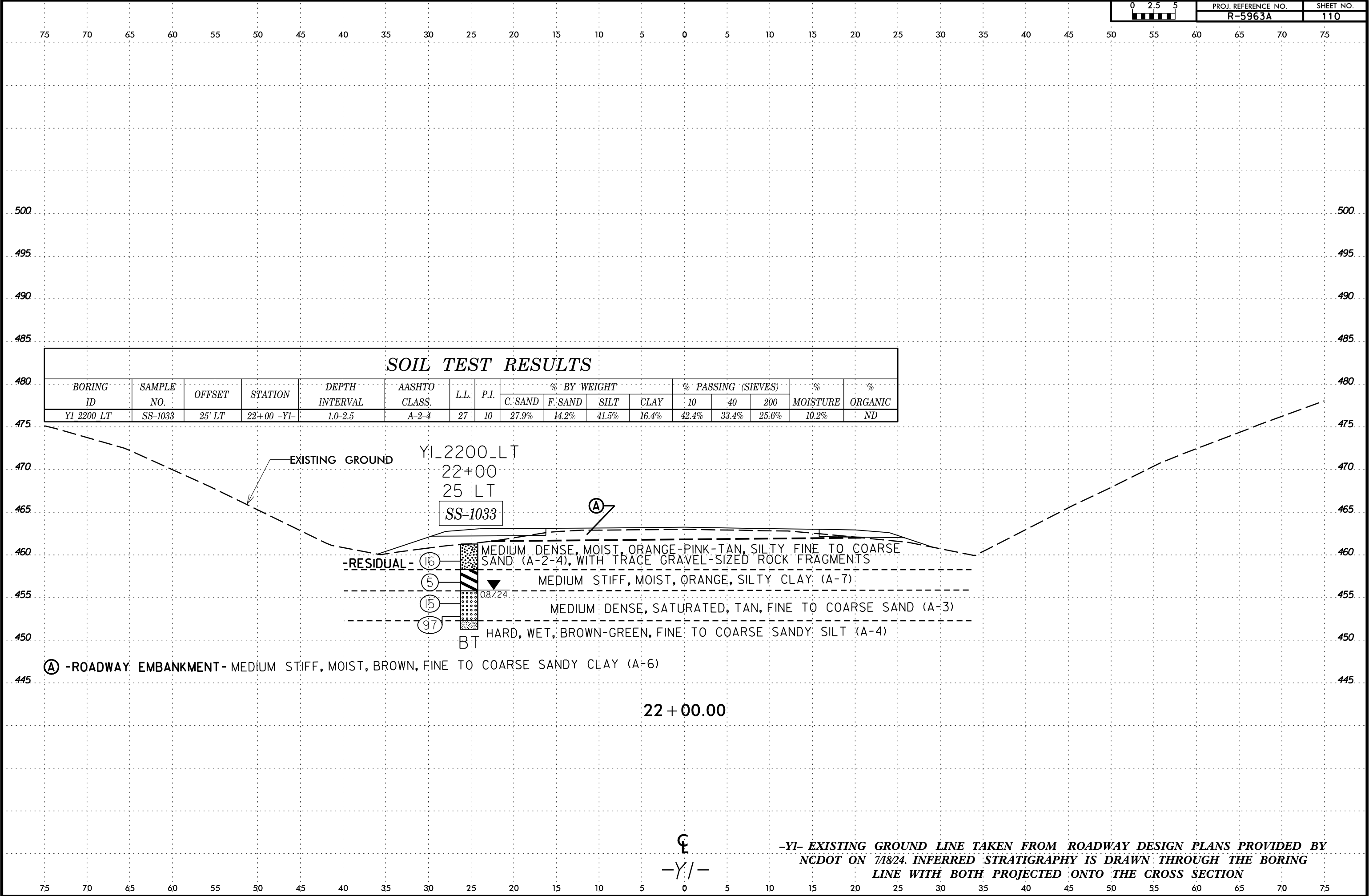


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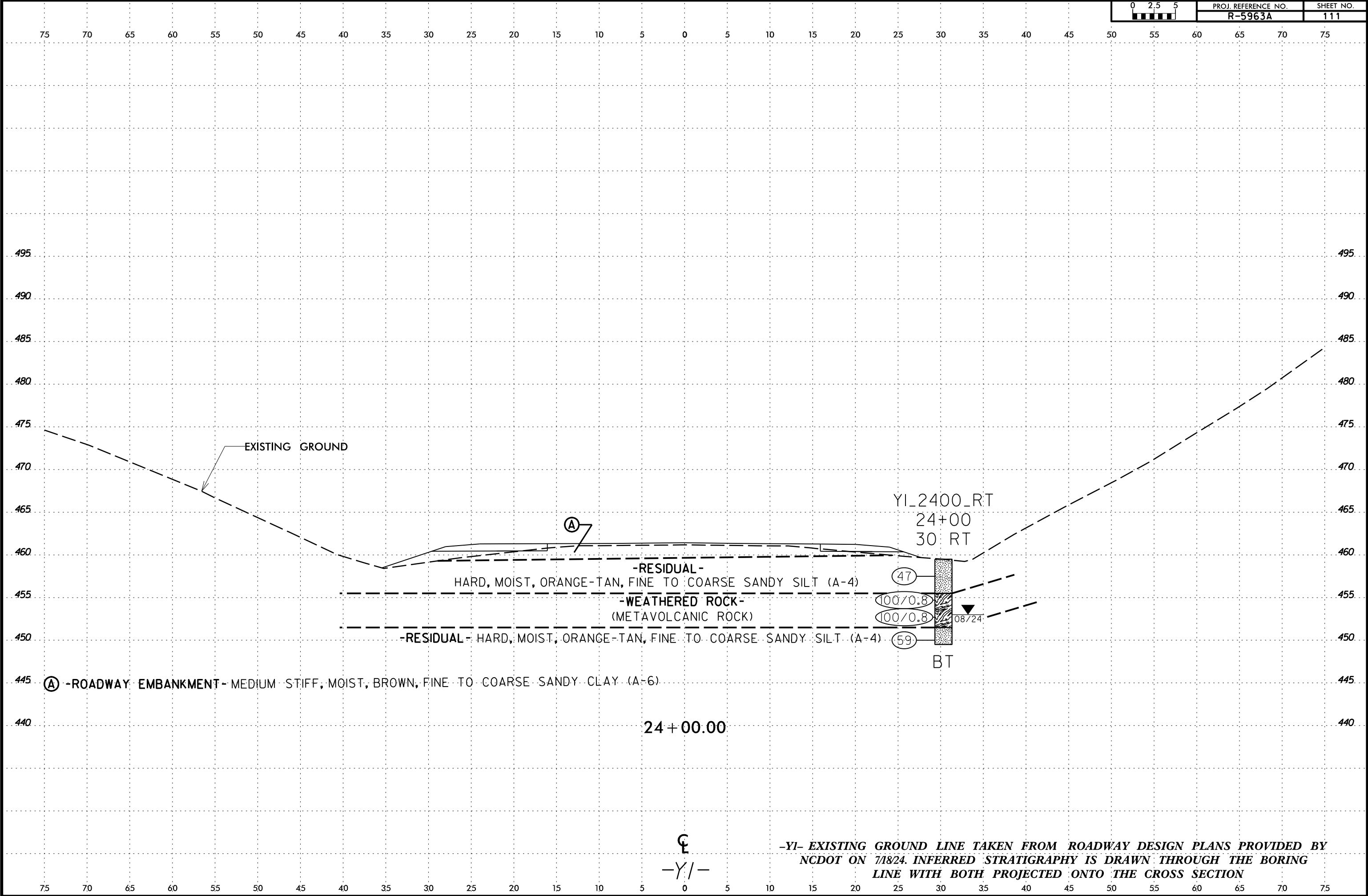
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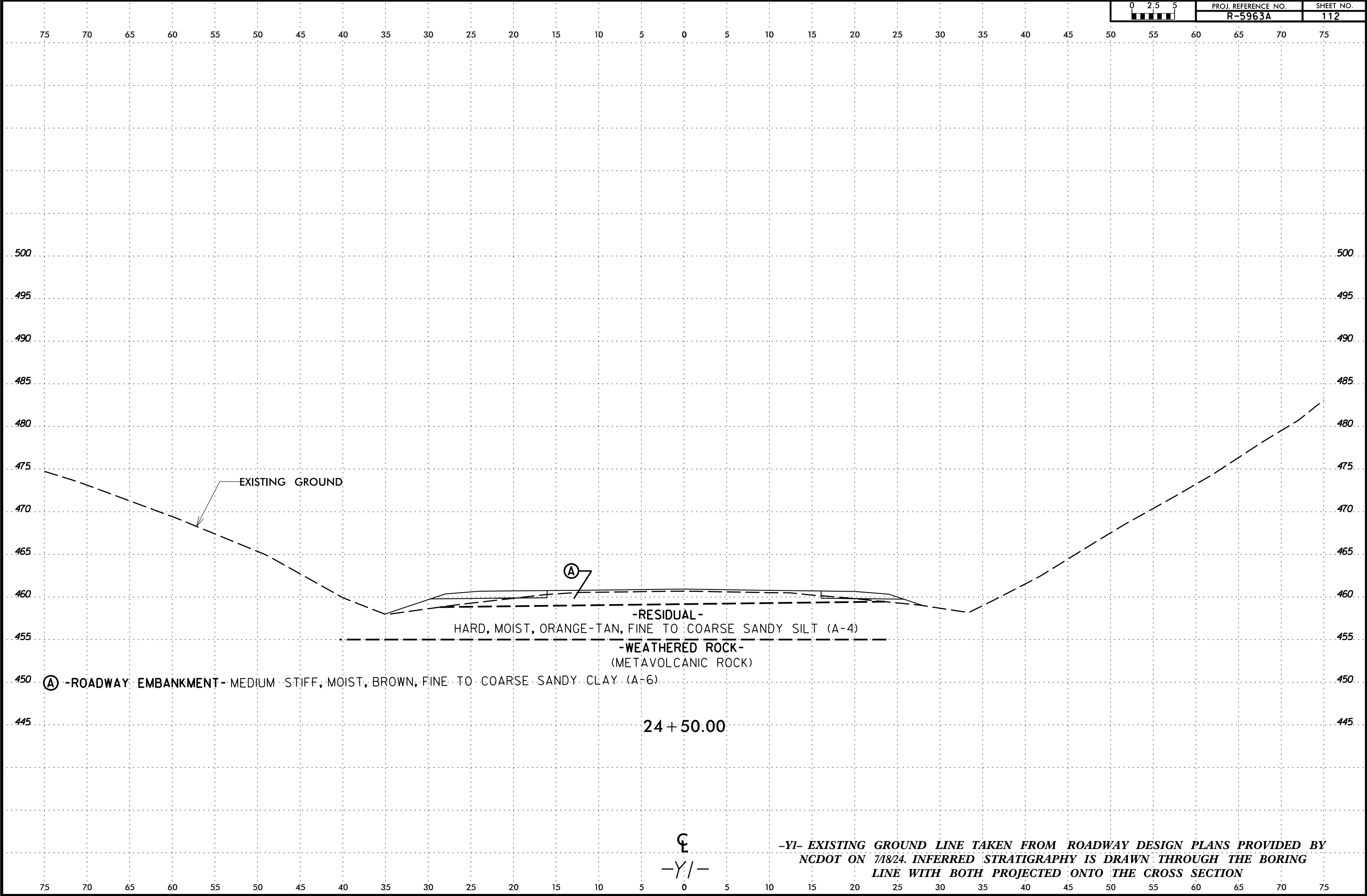
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	R-5963A	111



6/23/16

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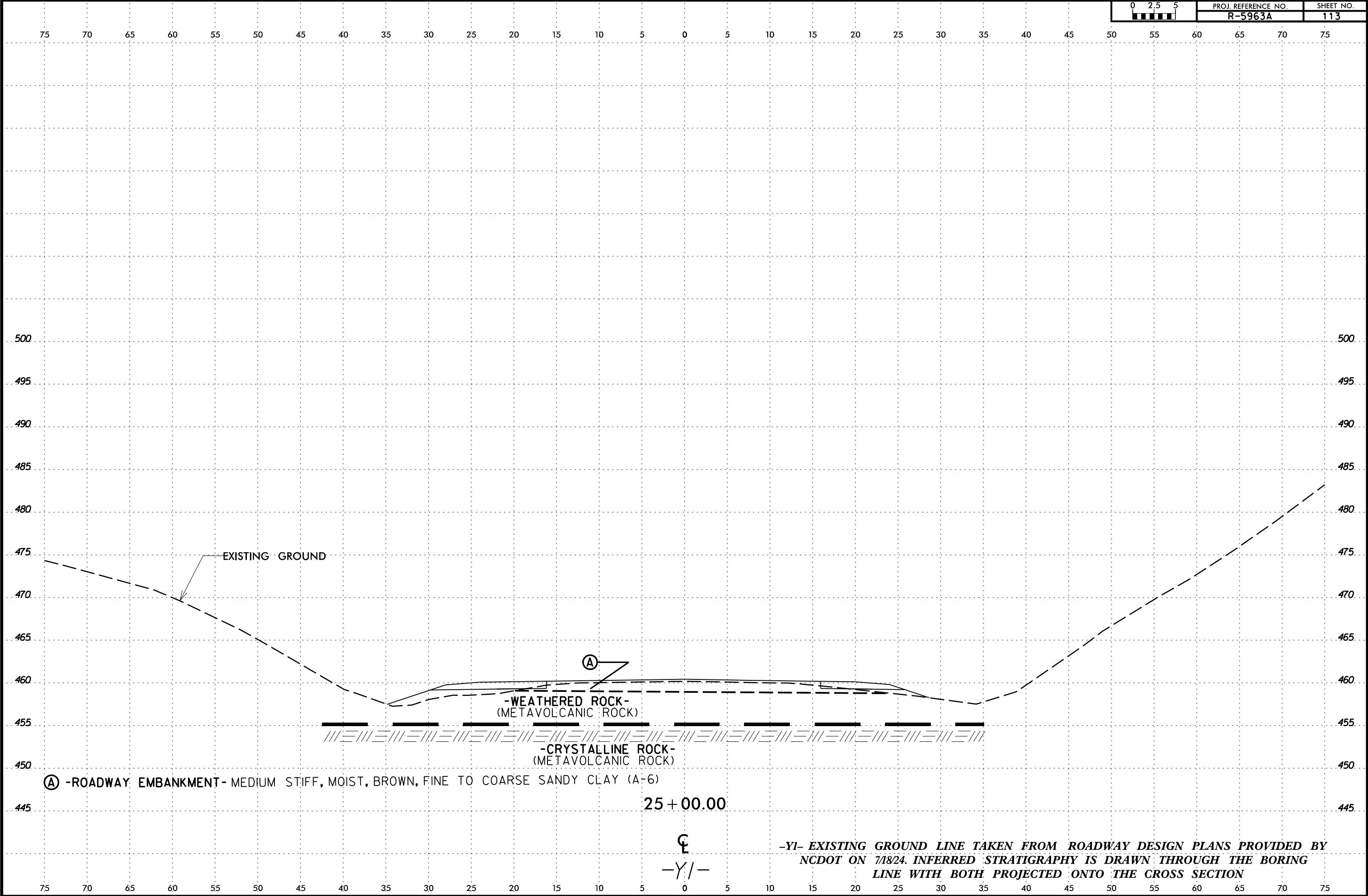
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6/23/16

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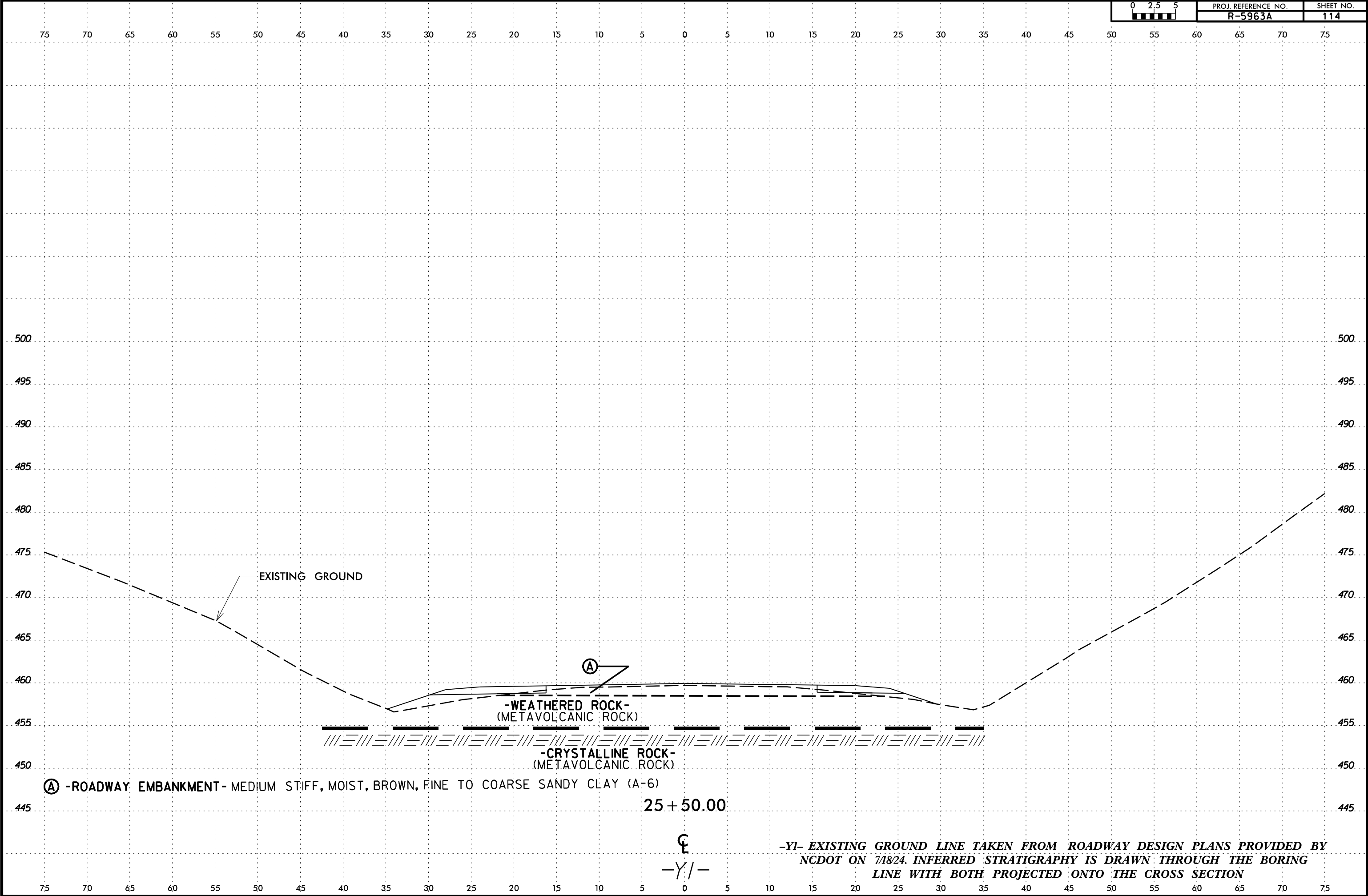
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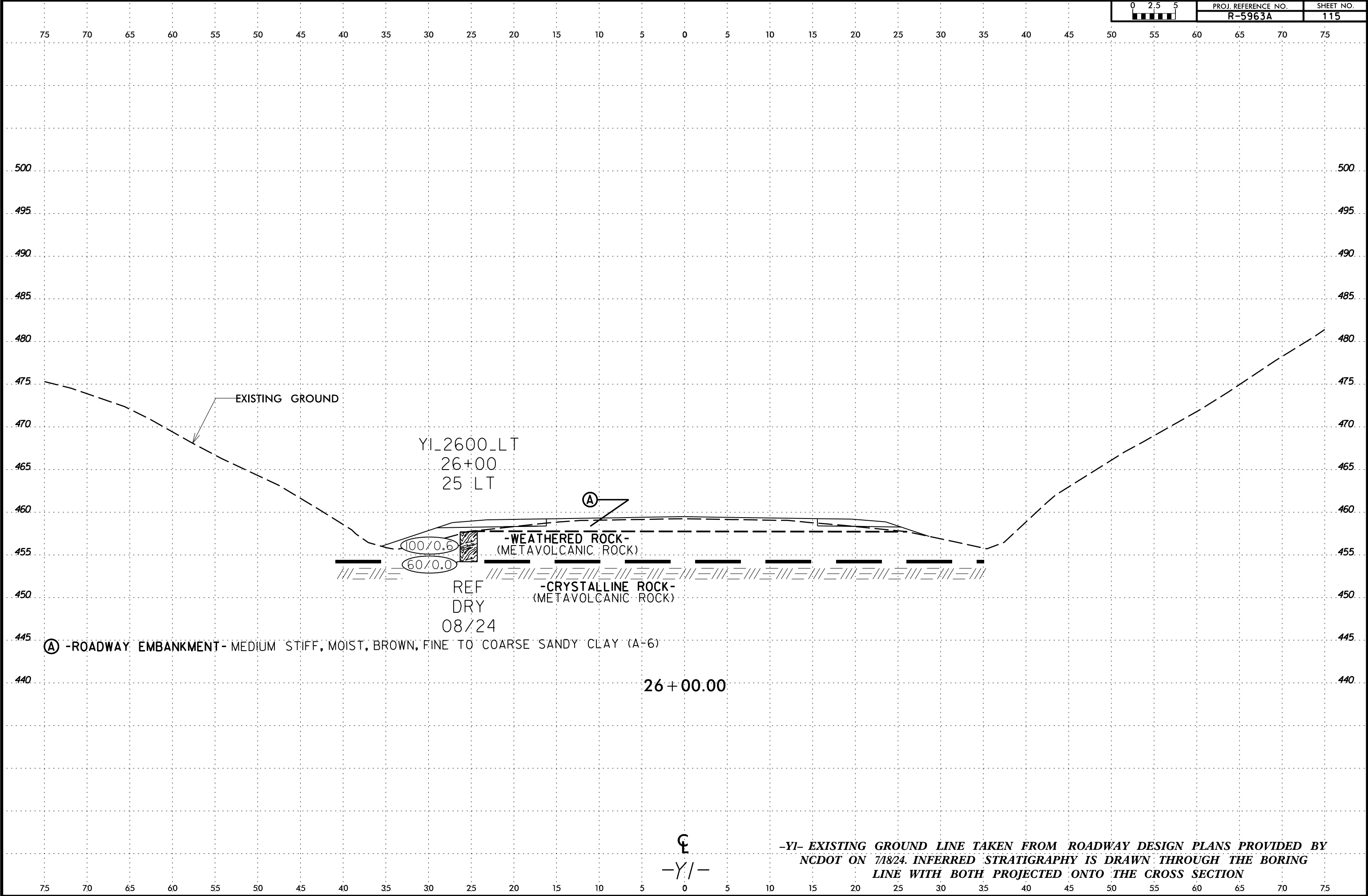
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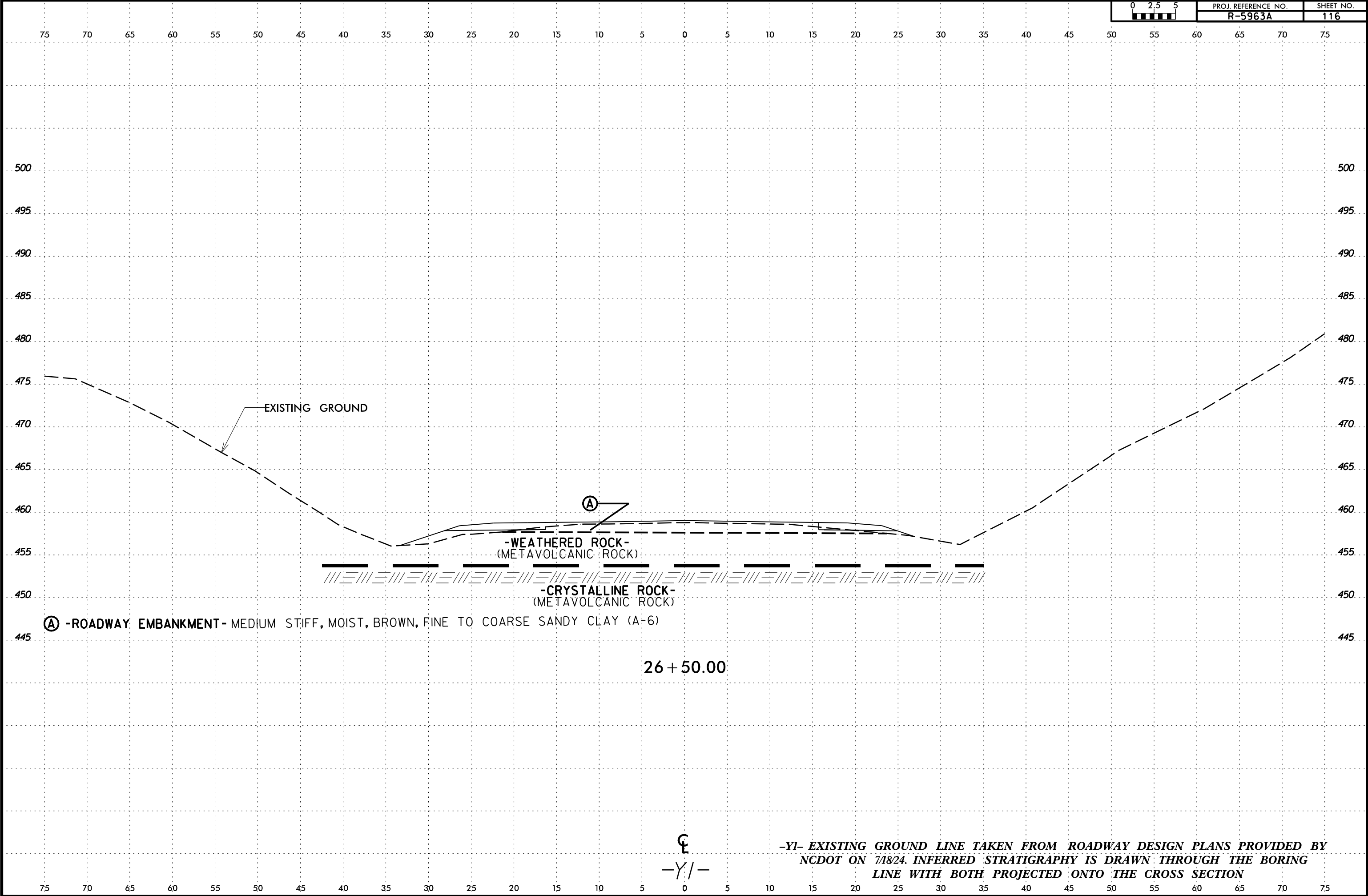
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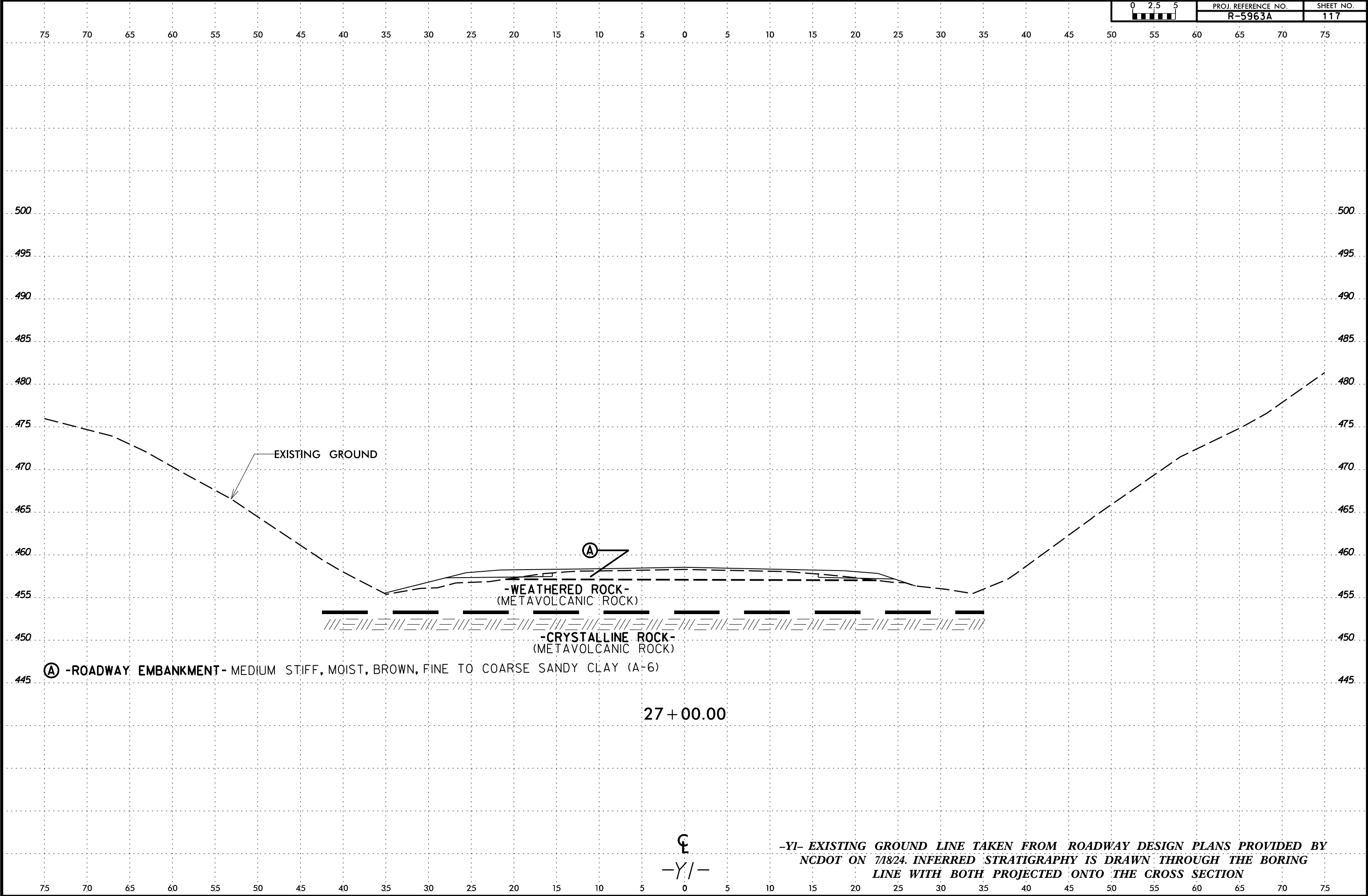
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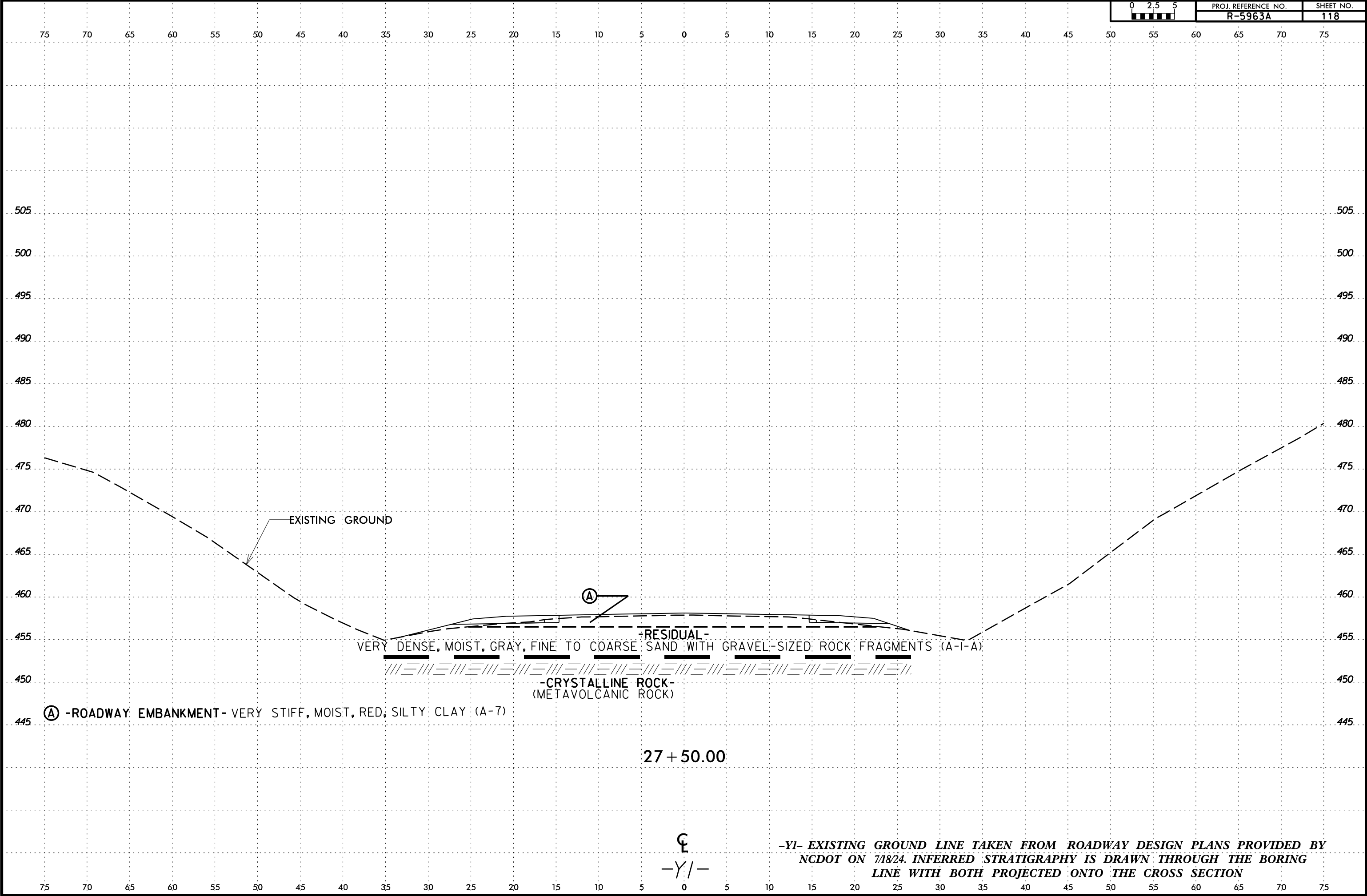
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-YI- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16

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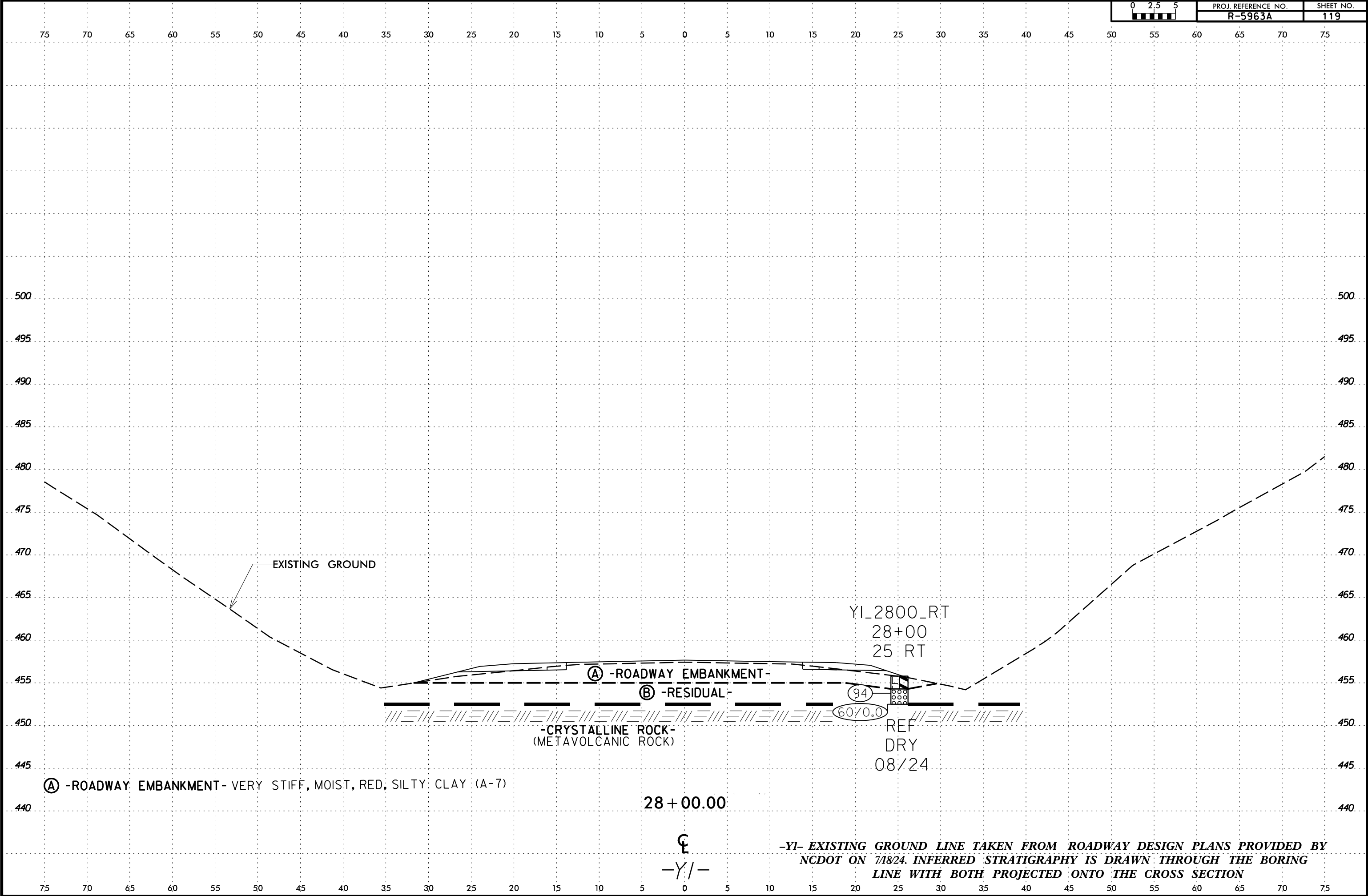
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	R-5963A	118



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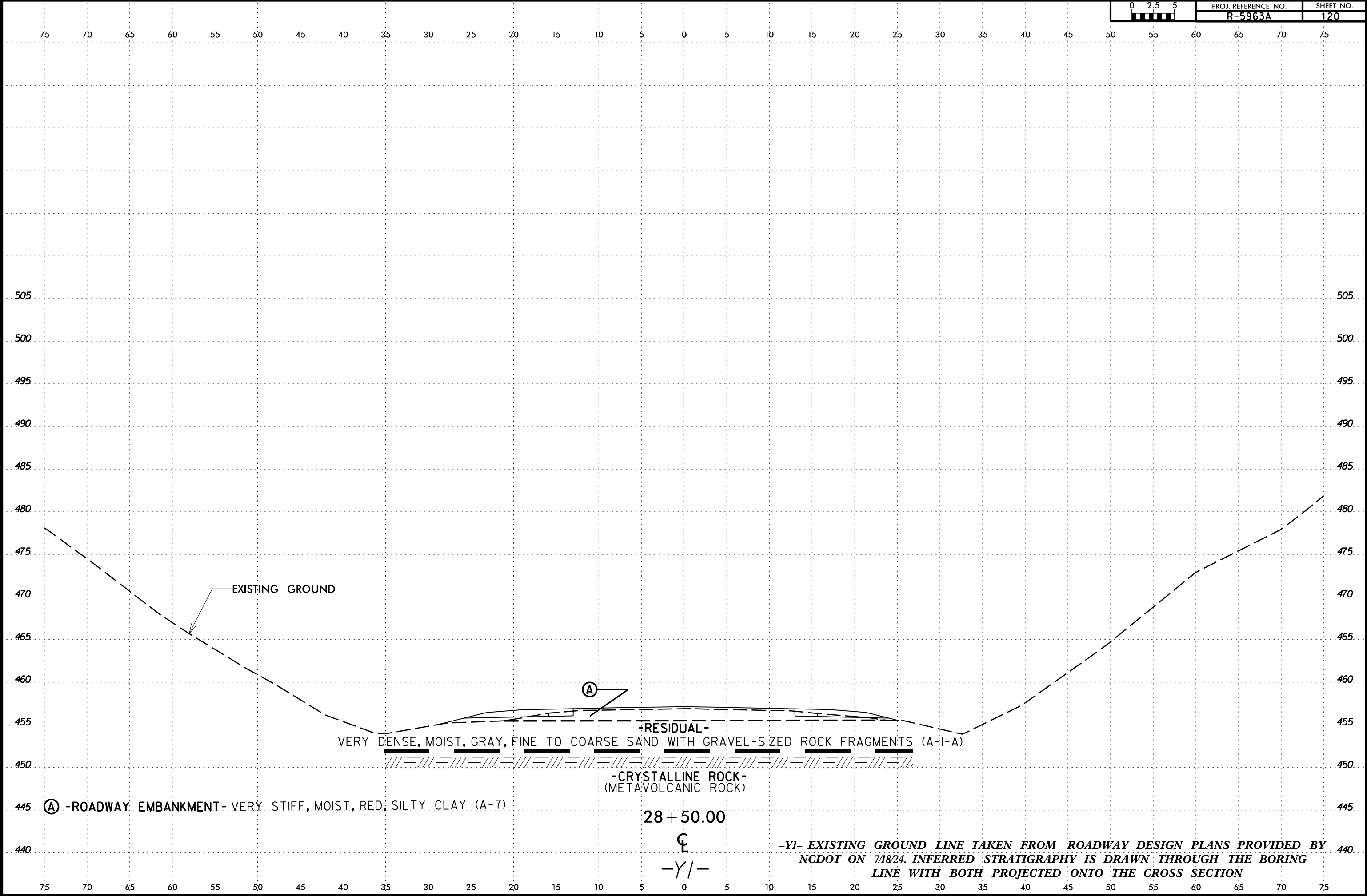
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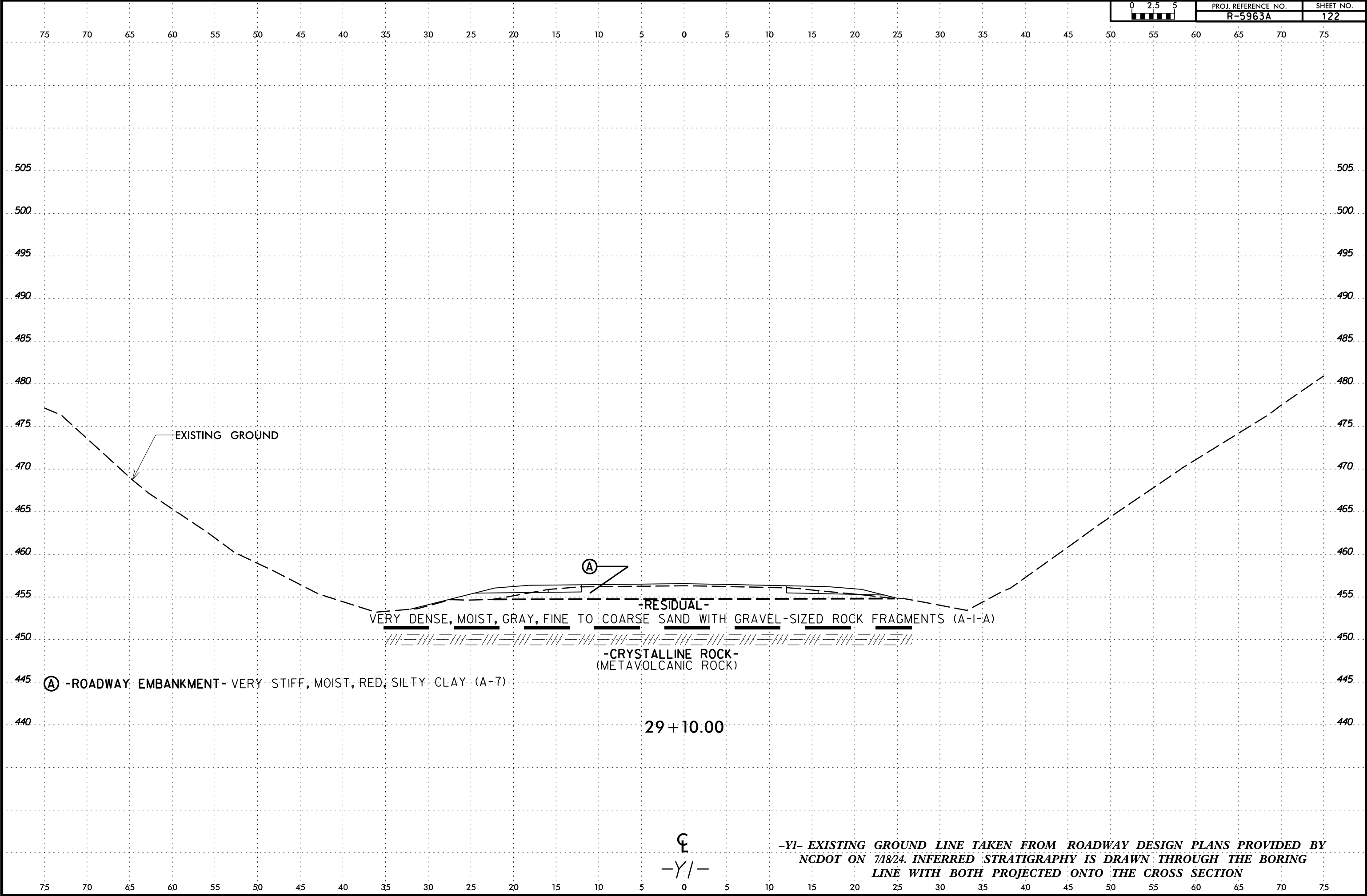
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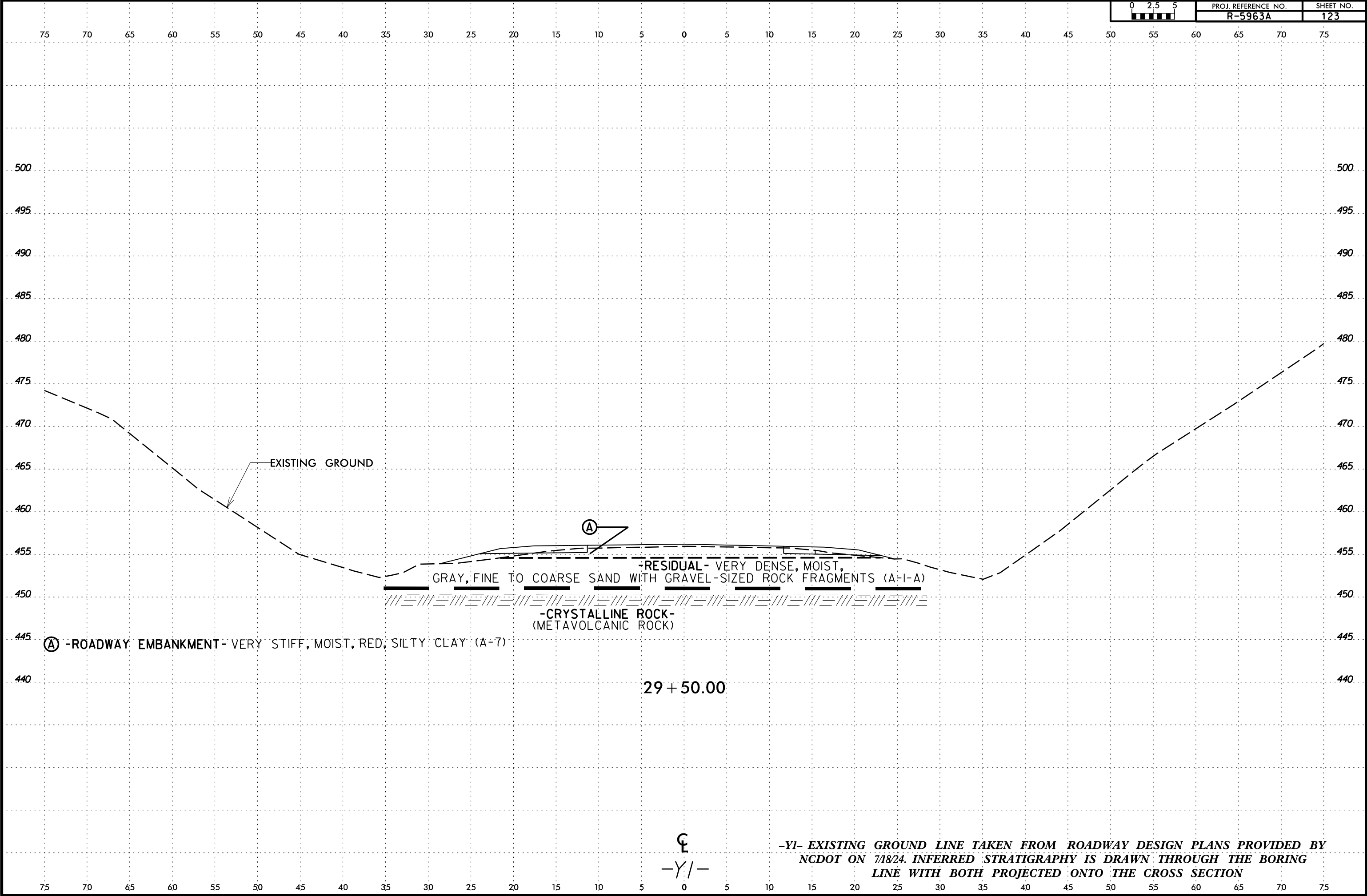




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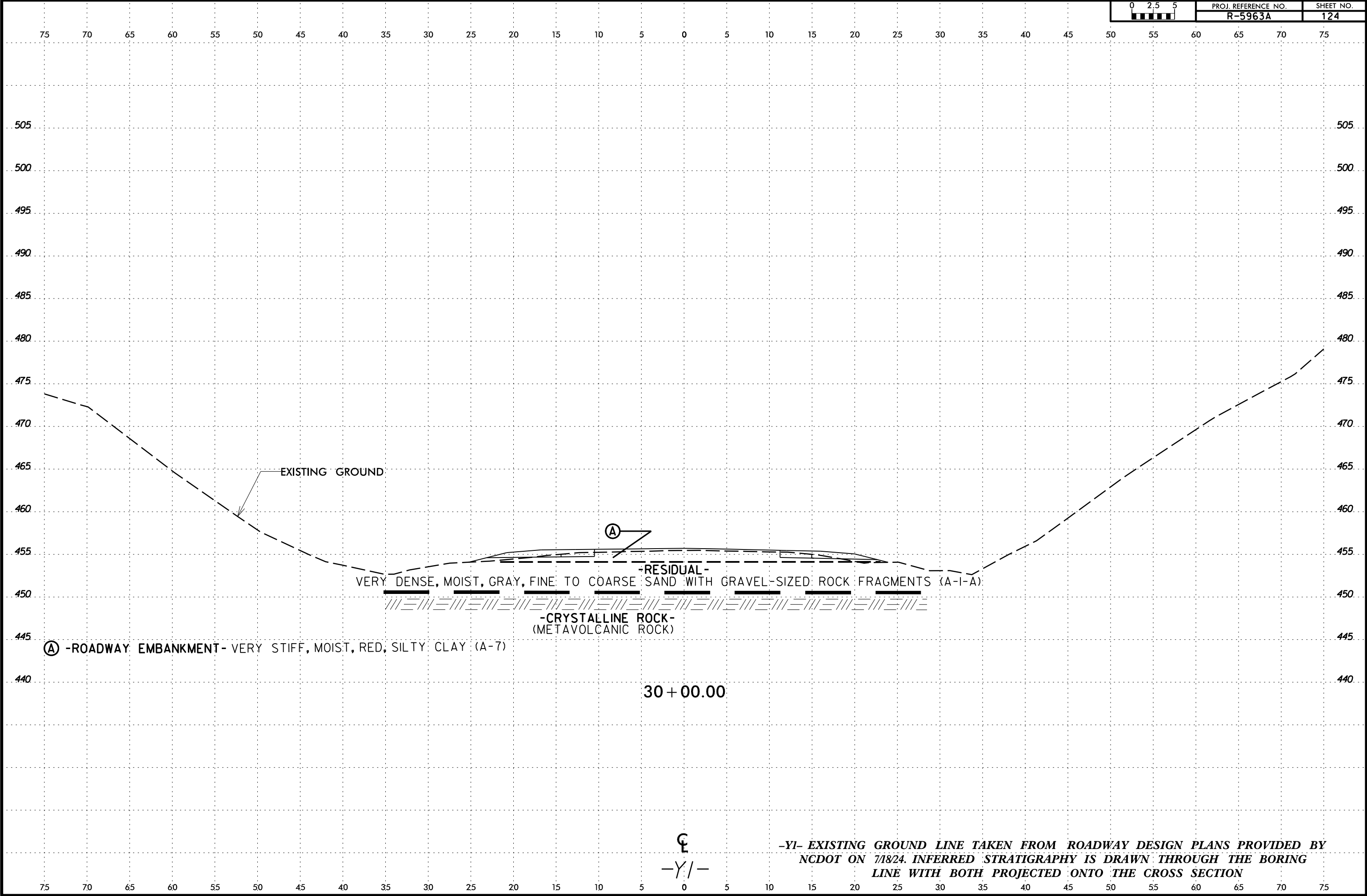
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-Y/-

-YI- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16

I4-NOV-2024 18:53
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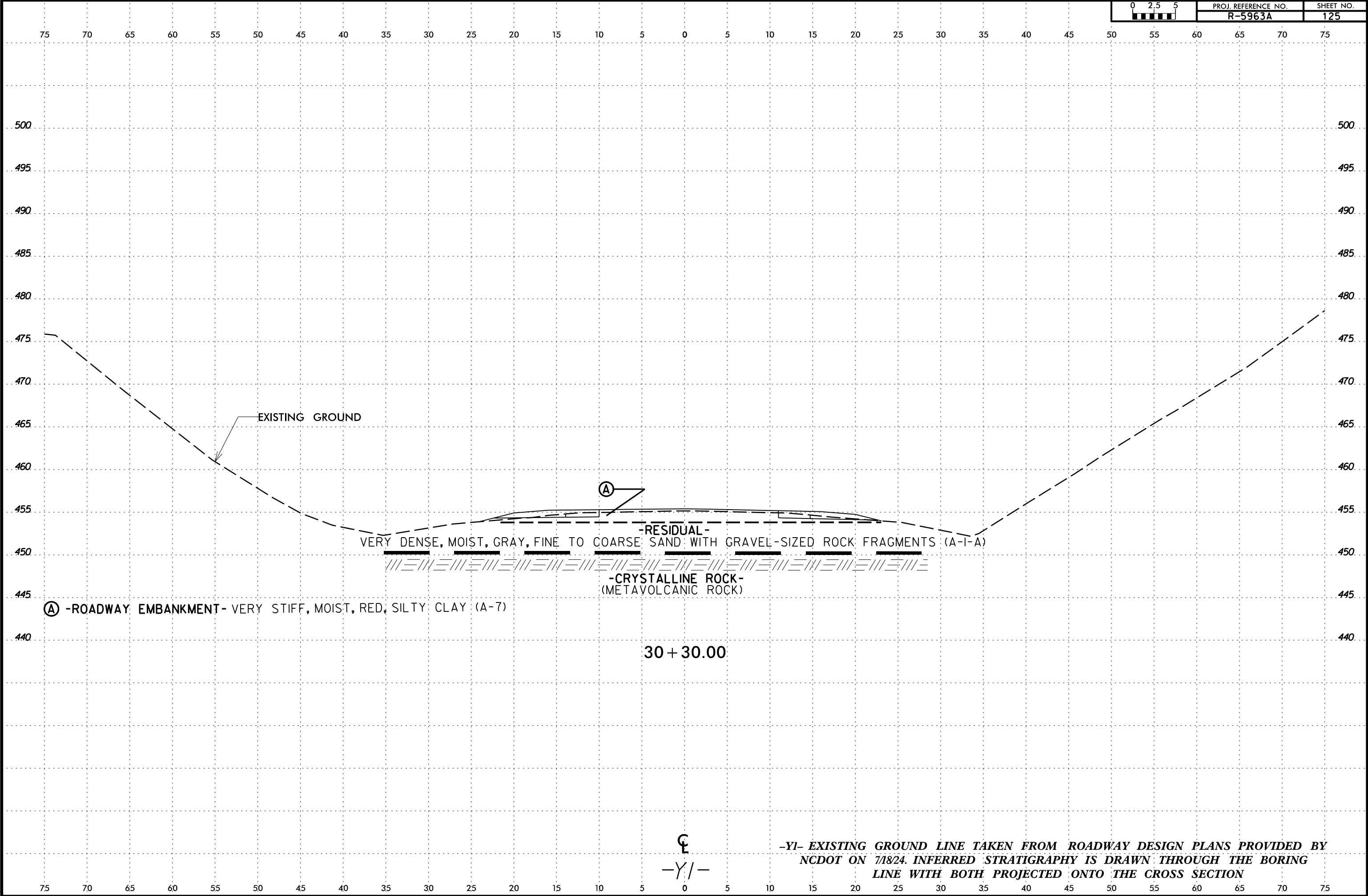
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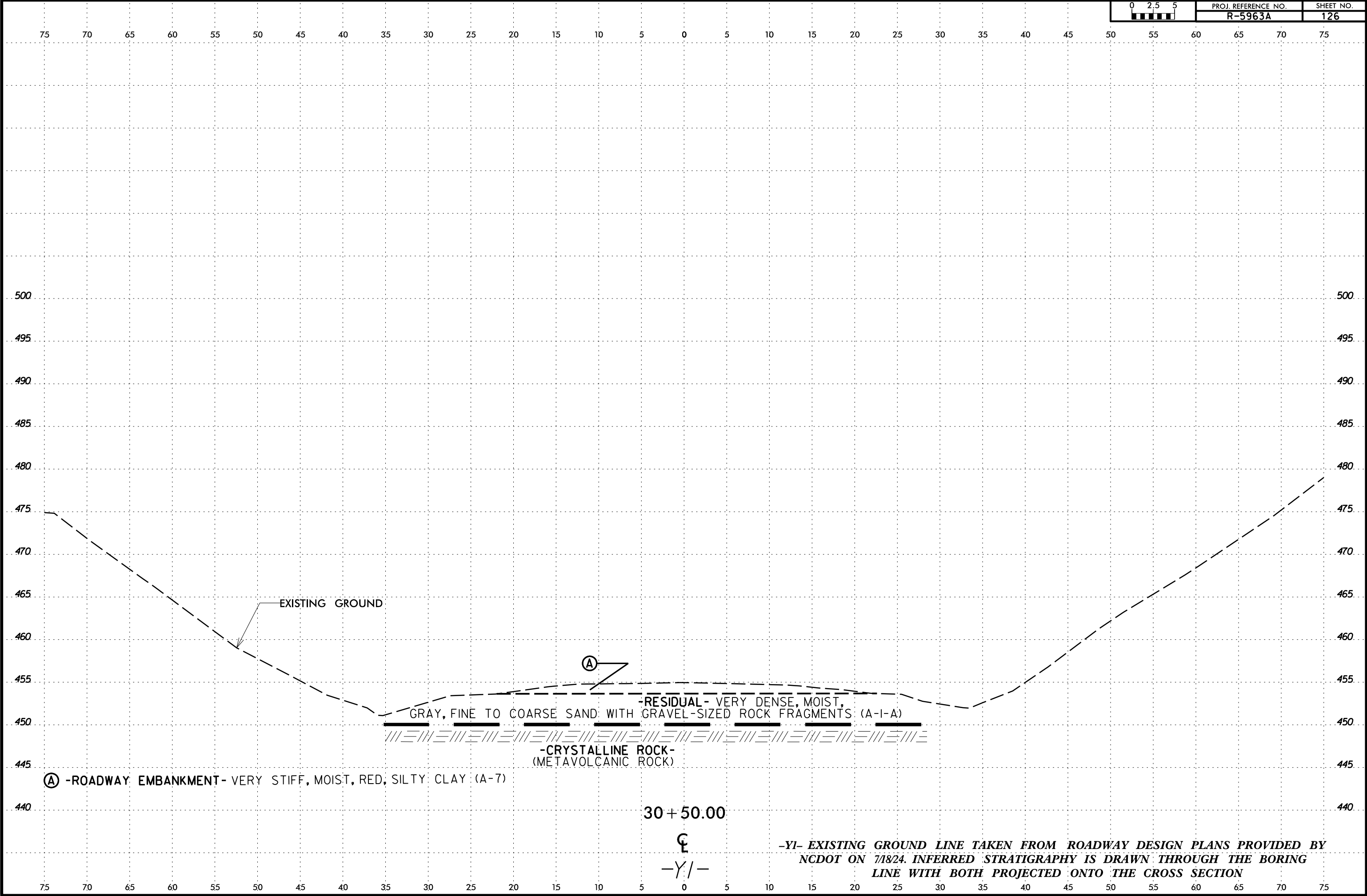
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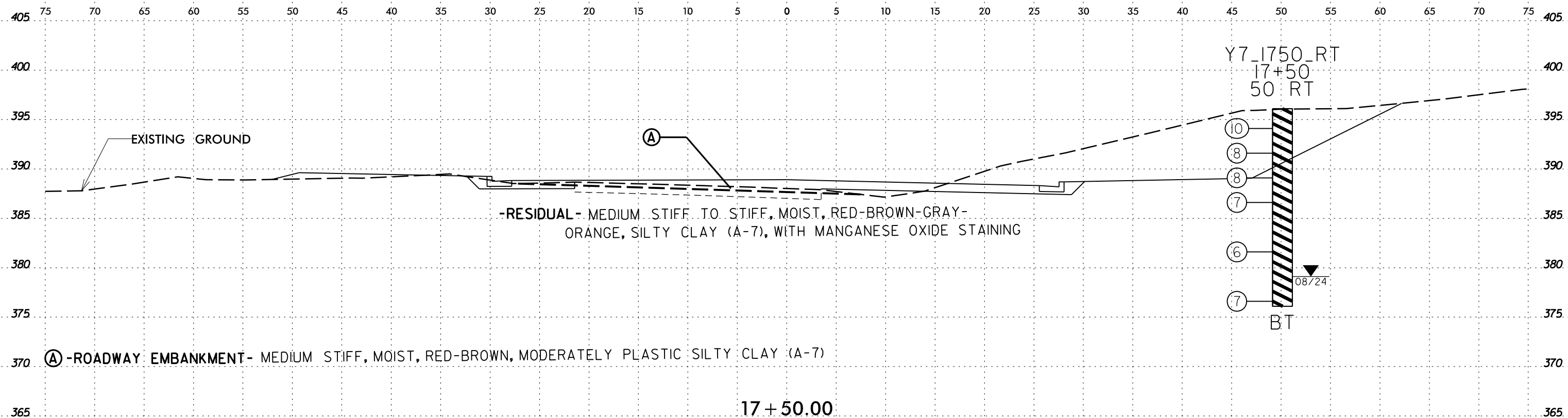


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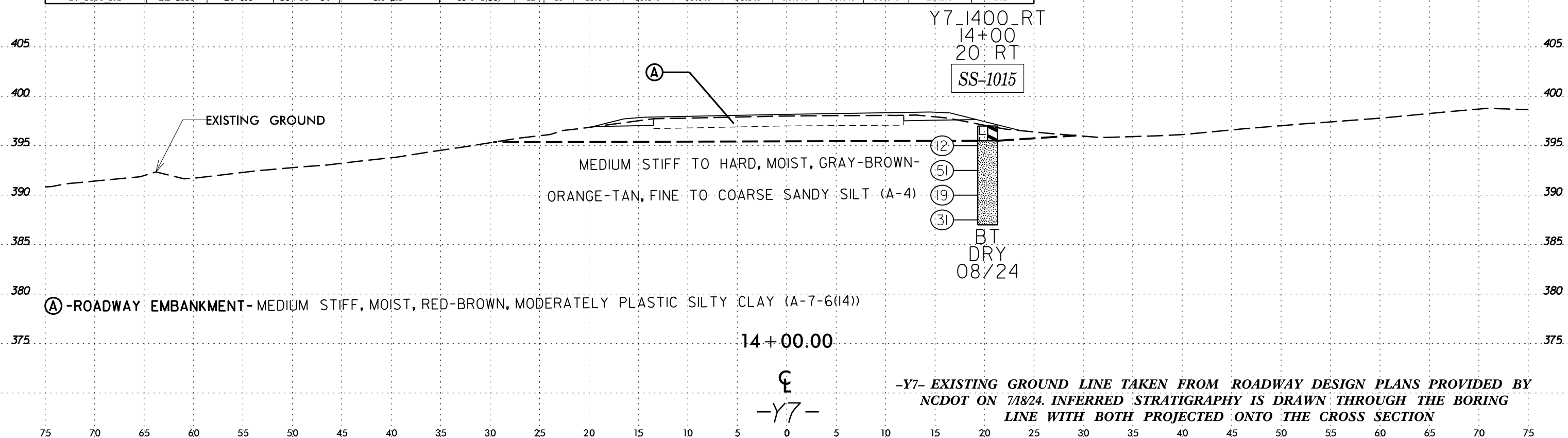
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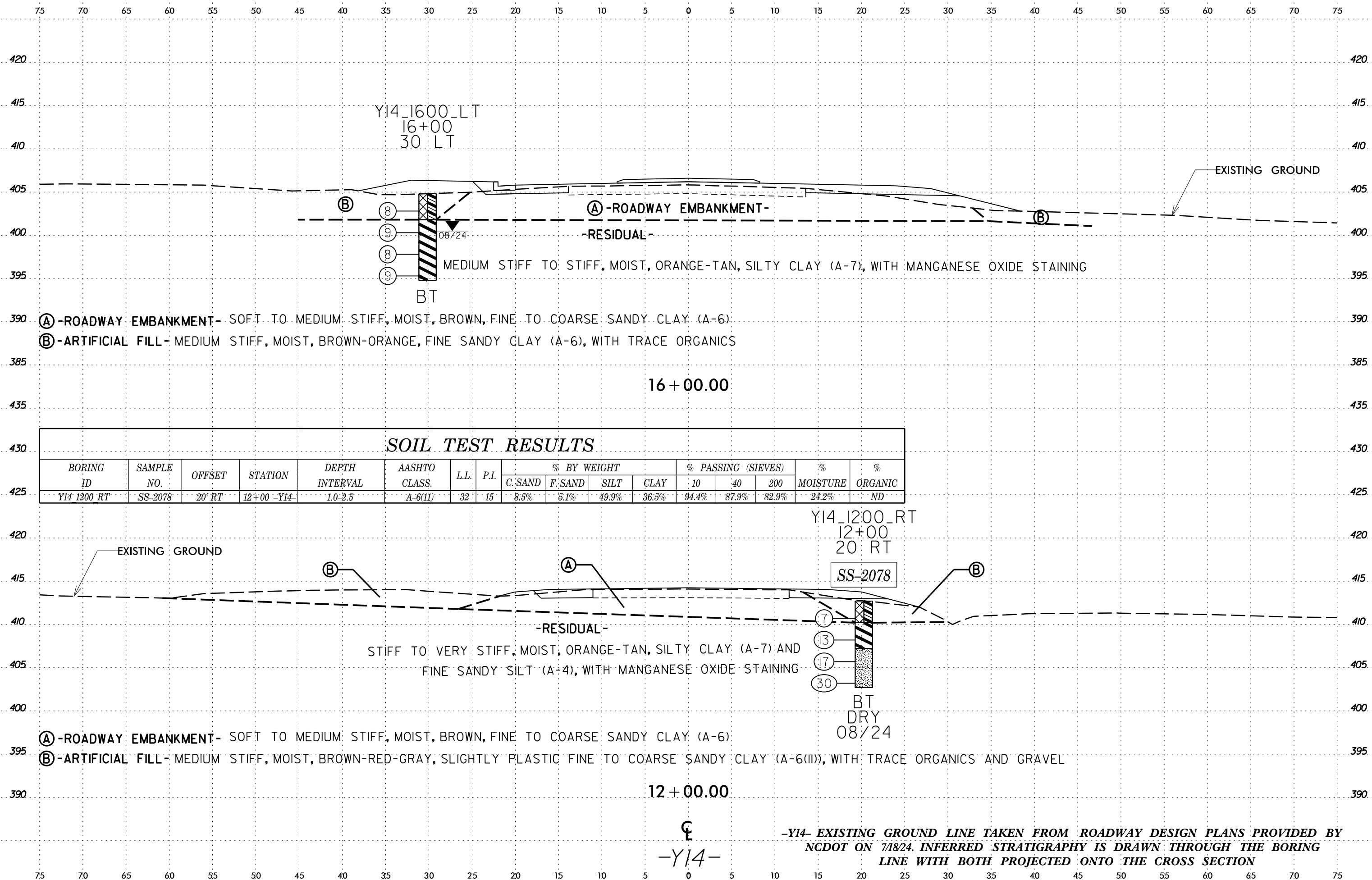
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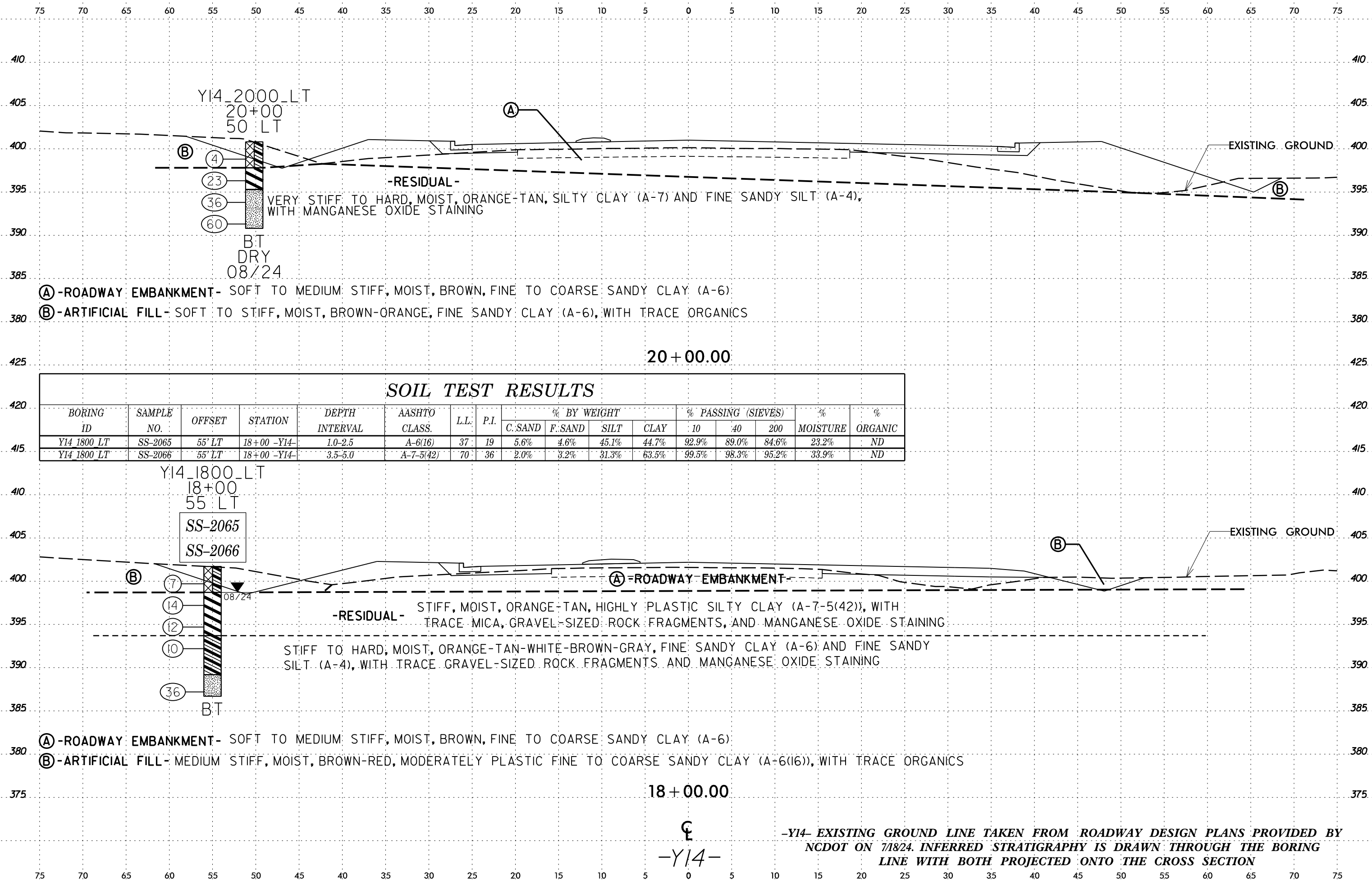




SOIL TEST RESULTS																
BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
								C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
Y7-1400-RT	SS-1015	20'-RT	14+00 -Y7-	1.0-2.5	A-7-6(14)	42	19	13.6%	10.5%	39.0%	36.9%	96.9%	86.6%	76.6%	29.1%	ND







6/23/16

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430

425

420

415

410

405

400

395

390

385

380

SOIL TEST RESULTS

BORING ID	SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
Y14 2300 LT	SS-2046	55' LT	23+00 -Y14-	1.0-2.5	A-7-6(23)	48	24	4.8%	5.7%	38.2%	51.3%	94.9%	91.5%	86.5%	12.6%	ND

Y14_2300_LT

23+00

55' LT

SS-2046

Ⓑ

Ⓘ3

100/0.9

Ⓘ6

100/0.7

-RESIDUAL- HARD, MOIST, ORANGE-TAN-GRAY, FINE SANDY SILT (A-4), WITH TRACE MICA, GRAVEL-SIZED ROCK FRAGMENTS, AND MANGANESE OXIDE STAINING

-WEATHERED ROCK-
(METAVOLCANIC ROCK)

-RESIDUAL- Ⓘ83 HARD, MOIST, ORANGE-TAN-WHITE, FINE SANDY SILT (A-4), WITH TRACE MICA, GRAVEL-SIZED ROCK FRAGMENTS, AND MANGANESE OXIDE STAINING

BT

DRY

08/24

Ⓐ -ROADWAY EMBANKMENT- SOFT TO MEDIUM STIFF, MOIST, BROWN, FINE TO COARSE SANDY CLAY (A-6)

Ⓑ -RESIDUAL- STIFF, MOIST, ORANGE-TAN, MODERATELY PLASTIC SILTY CLAY (A-7-6(23)), WITH TRACE MICA AND GRAVEL-SIZED ROCK FRAGMENTS

Ⓒ -WEATHERED ROCK- (METAVOLCANIC ROCK)

23 + 00.00

Ⓔ

-Y14-

-Y14- EXISTING GROUND LINE TAKEN FROM ROADWAY DESIGN PLANS PROVIDED BY
NCDOT ON 7/18/24. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORING
LINE WITH BOTH PROJECTED ONTO THE CROSS SECTION

EXISTING GROUND

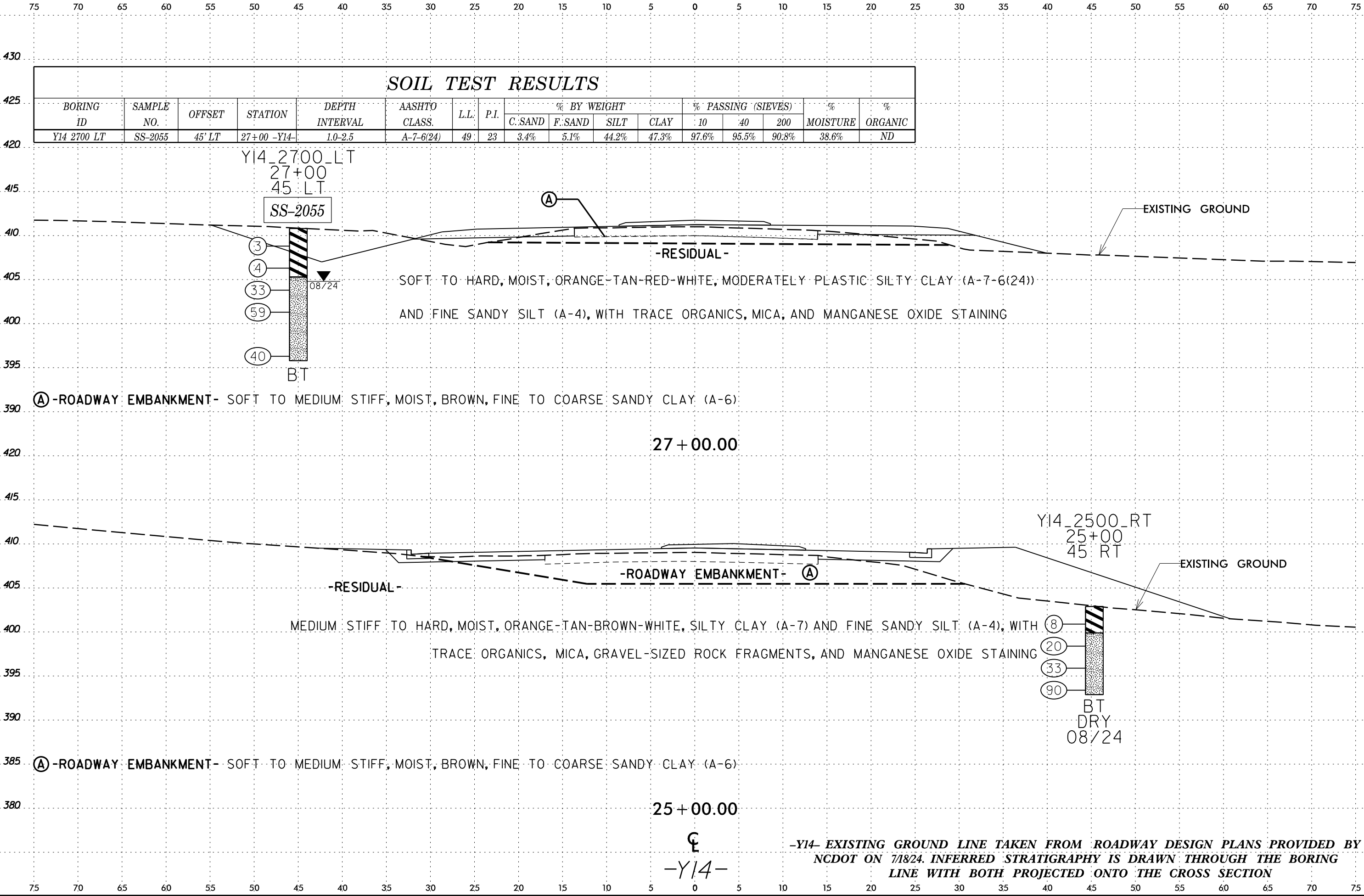
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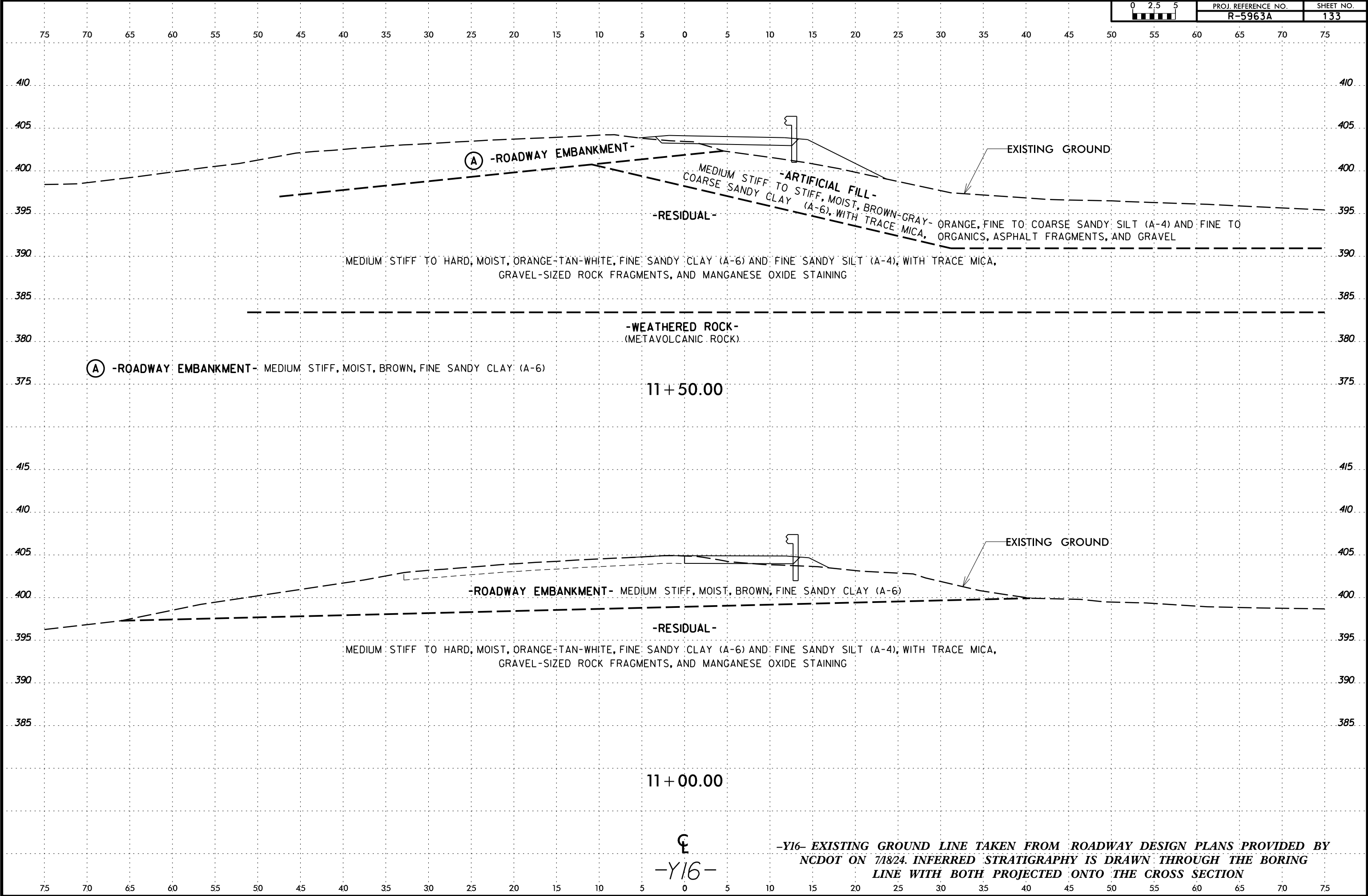
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