	SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATIONCONTENTSLINESTATION 10+00 to 21+35PLAN 4 - 5PROFILE 6	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT TOTAL CONTRACTOR OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT
REFERENCE: B-5766	CROSS SECTIONSLineSTATION-L-20+20 and 20+507GEOTECHNICAL BORING REPORTSSHEETS8 - 9	SUBSURFACE INVESTIGATION SUBSURFACE INVESTIGATION COUNTY STOKES PROJECT DESCRIPTION BRIDGE NUMBER & OVER DAN RIVER ON SR 1674 (SHEPPARD MILL ROAD) DANBURY, NORTH CAROLINA
PROJECT: 45722		

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5766	1	9

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (99) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAIL

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UNI-FLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOL MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

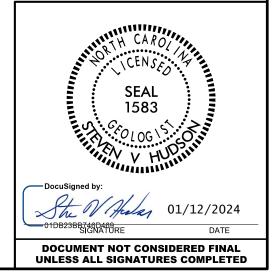
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE VIBSURFACE PLANS. THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS TO SE ENVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACTUAL CONDENSATION.

- NOTES: I, THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR CUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REDUCETED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

T. PARK
C. STRATTON
T.J. WHITE, CWC
S. PUGH, CWC
INVESTIGATED BY
DRAWN BY S.V. HUDSON, PG
CHECKED BY J. LEE STONE, PG
SUBMITTED BYS.V. HUDSON, PG
DATE JANUARY 2024





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

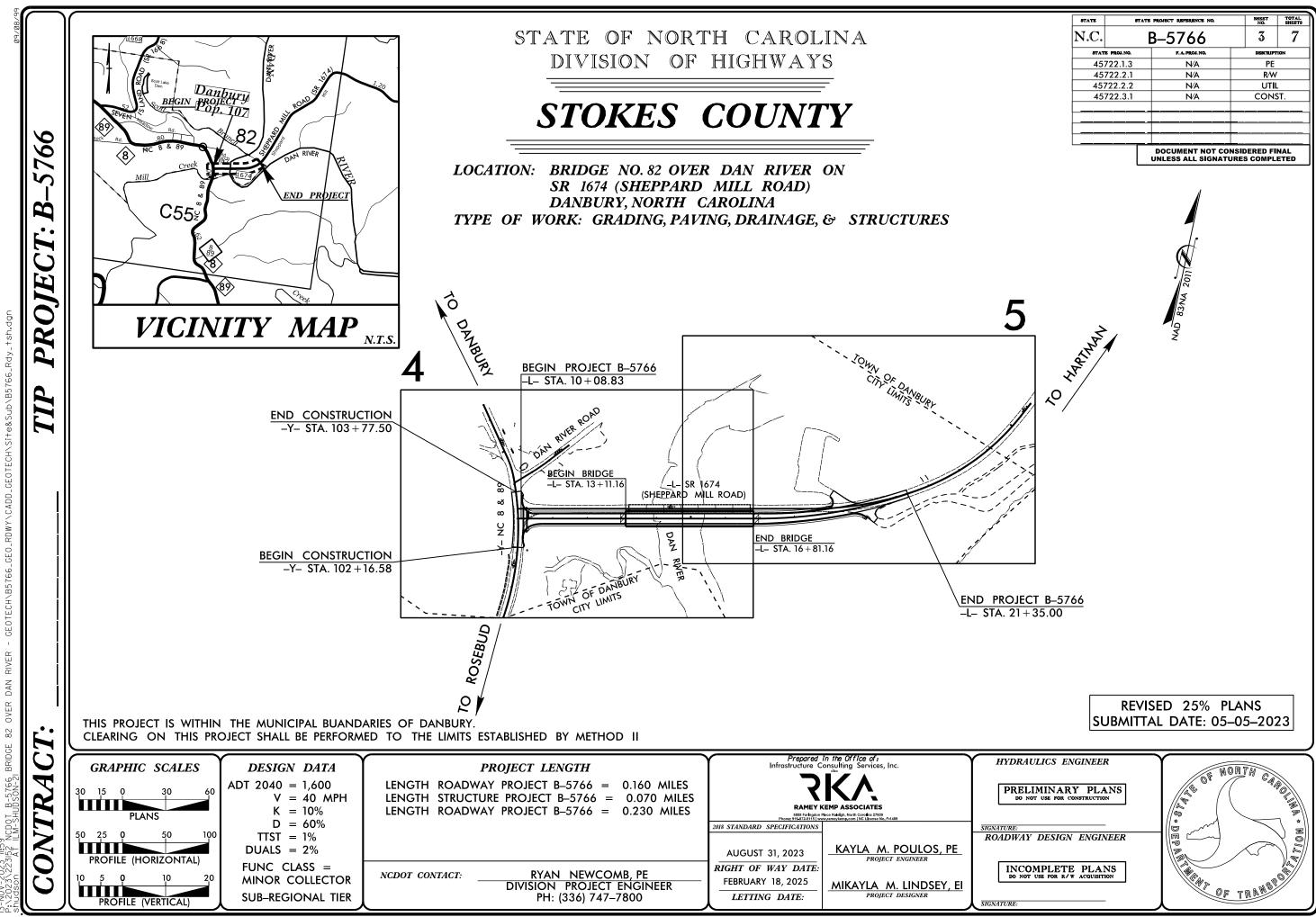
			SOIL	DESCF	RIPTION	<u></u>				1		GF	RADATION			1	—			R	OCK DES	SCRIPTION	
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLICHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:						R FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.				HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TEST ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EOUAL TO OR LESS THAN Ø, BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK												
CONSISTER	NCY, COLOR,	TEXTURE, MO	STURE, AASHT	O CLASSI	IFICATION.	AND OTHE	R PERTINE	NT FACTOR		ANGULARITY OF GRAINS			REPRESENTED BY A ZONE OF WEATHERED ROCK.										
AS V	ERY STIFF.G	RAY, SILTY CLAY	MOIST WITH IN	TERBEDDE	ED FINE S	AND LAYERS	, ETC. FUR HIGHLY PLA	STIC.A-7-6					SOIL GRAINS IS D	ESIGNATED E	Y THE TERMS:	WEATHERED	11.5 HN		STA .			N MATERIAL THAT WOUL	
	S	OIL LEG	END AND	AASH	TO CL	ASSIFI	CATION			ANGULAR, SUBAN			ICAL COMPOS	ITION		ROCK (WR)						OT IF TESTED.	.D TILLD ST
GENERAL CLASS.	(GRANULAR MATE ≤ 35% PASSING			T-CLAY MAT 35% Passin		OR	GANIC MATERI	ALS	MINERAL NAM			Z, FELDSPAR, MICA,		ETC.	CRYSTALLINE						RAIN IGNEOUS AND MET REFUSAL IF TESTED. RO	
GROUP	A-1	A-3	A-2		A-5 A	-6 A-7	A-1, A-2	A-4, A-5					N THEY ARE CONSI			ROCK (CR)		22	<u>Z</u>	GNEISS,	GABBRO, SC		
	4-1-а А-1-ь	A-2-4	-2-5 A-2-6 A-2	2-7		A-7-5 A-7-6	A-3	A-6, A-7					RESSIBILITY			NON-CRYSTALL ROCK (NCR)	.INE			SEDIMEN	NTARY ROCK	THAT WOULD YEILD SP	PT REFUSAL
SYMBOL				S	1.7.1					MODEF	RATELY C	MPRESSIBLE COMPRESSIBL	LE	LL < 31 LL = 31	50	COASTAL PLAT		÷		COASTAL	L PLAIN SE	ES PHYLLITE, SLATE, SA DIMENTS CEMENTED INT	TO ROCK, BUT
% PASSING								SILT-		HIGHL	LY COMPR			LL > 50		SEDIMENTARY (CP)	ROCK				FUSAL. ROCI BEDS, ETC.	K TYPE INCLUDES LIMES	STONE, SANDS
*40 3i	0 MX 0 MX 50 MX	51 MN					granular Soils	CLAY SOILS	MUCK, PEAT			GRANULAR	GE OF MATER								WEATH	ERING	
	5 MX 25 MX	10 MX 35 MX 3	5 MX 35 MX 35	MX 36 MN	36 MN 36	MN 36 MN		50125		ORGANIC MATERIAL TRACE OF ORGANIC MA	<u>.</u>	<u>SOILS</u> 2 - 3%	SILT - CLAY <u>SOILS</u> 3 - 5%	<u>OTHE</u> TRACE	<u>R MATERIAL</u> 1 - 10%				CRYSTAL RYSTALL		.FEW JOINT	S MAY SHOW SLIGHT STA	AINING. ROCK
MATERIAL PASSING #40							SOILS	WITH		LITTLE ORGANIC MATT	TER	3 - 5%	5 - 12%	LITTLE	10 - 20%						S STAINED.	SOME JOINTS MAY SHOW	THIN CLAY C
LL PI	- 6 MX		1 MN 40 MX 41 0 MX 11 MN 11				LITTI	E OR	HIGHLY	MODERATELY ORGANIC HIGHLY ORGANIC		5 - 10% > 10%	12 - 20% > 20%	SOME HIGHL Y	20 - 35% 35% AND ABOVE				A BROK		MEN FACE S	HINE BRIGHTLY. ROCK RI	INGS UNDER H
GROUP INDEX	0	0 0	4 MX		12 MX 16		Mode Amoun	RATE ITS OF	ORGANIC			GROU	UND WATER								S STAINED	AND DISCOLORATION EXTE	ENDS INTO RO
	TONE FRAGS.	FINE SIL	TY OR CLAYEY	SI	ILTY	CLAYEY	org Mat		SOILS	∇	WATER	R LEVEL IN I	BORE HOLE IMMEDI	ATELY AFTER	DRILLING							IN GRANITOID ROCKS SOM YSTALLINE ROCKS RING U	
OF MAJOR C MATERIALS	SRAVEL, AND SAND		VEL AND SAND		DILS	SOILS				▼	STATIC	C WATER LE	VEL AFTER 24	HOURS								COLORATION AND WEATHE	
GEN. RATING		EXCELLENT TO	2000	-	FAIR TO PO	000	FAIR TO	POOR	UNSUITABLE		PERCH	ED WATER, S	ATURATED ZONE, OF	R WATER BEA	RING STRATA							ULL AND DISCOLORED, SOM HOWS SIGNIFICANT LOSS	
AS SUBGRADE							POOR	FUUR	UNSULTHOLE		SPRINC	G OR SEEP						FRESH F		HHHER DE	LOWS HIND SI	IOWS STONE TCHINE LOSS	OF STRENOTH
			GROUP IS ≤ LL				> LL - 30				M		NEOUS SYMB									STAINED. IN GRANITOID	
				1	NGE OF ST		RANO	E OF UNC	ONFINED					ULJ		(MOD. SEV.)	AND C	CAN BE E	EXCAVAT	TED WITH	A GEOLOGIS	T'S PICK. ROCK GIVES "CI	
PRIMARY SO	OIL TYPE		TNESS OR STENCY	PENET	ration re (N-VALU)		COMP	RESSIVE S (TONS/FT	TRENGTH	L ROADWAY EMBA			DIP & DIP DIP DIP & DIP DIP DIP & DIP DIP DIP & DIP DIP	RECTION						IELD SPT I		STAINED. ROCK FABRIC	
GENERAL		VERY	LOOSE		< 4					SOIL SYMBOL			OPT DAT TEST BO		SLOPE INDICATOR	(SEV.)	REDUCE	CED IN S	STRENGT	TH TO STR	RONG SOIL. I	N GRANITOID ROCKS ALL	FELDSPARS A
GRANULA	R		OSE 1 DENSE		4 TO 1 10 TO 3			N/A		l 🛋		-	VST PMT		INSTALLATION						MENIS UF SI <u>N VALUES ></u>	TRONG ROCK USUALLY REM <u>100 BPF</u>	MAIN.
MATERIAL (NON-COH		DE	NSE DENSE		30 TO 9 > 50	50				ARTIFICIAL FI	Y EMBAN		-) AUGER BORING		CONE PENETROMETER TEST							STAINED. ROCK FABRIC	
			SOFT	<u> </u>	< 2			< 0.25		INFERRED SOIL	L BOUND	ARY -		•	SOUNDING ROD							OIL STATUS, WITH ONLY F ROCK WEATHERED TO A	
GENERAL		s	DF T		2 TO 4			Ø.25 TO 0				MW			TEST BORING							IN. IF TESTED, WOULD YI	
SILT-CLA MATERIAL	L	S	4 STIFF TFF		8 TO 1	15		0.5 TO 1 1 TO 2		INFERRED ROC	.K LINE) MONITORING W	Ψ	WITH CORE	COMPLETE						DISCERNIBLE, OR DISCER BE PRESENT AS DIKES (
(COHESIV	'E)		STIFF ARD		15 TO 3 > 30			2 TO 4 > 4		ALLUVIAL SOIL	L BOUNDA	ARY 🛆		\circ	- SPT N-VALUE		ALSO /	AN EXAN	MPLE.				
			TEXTURE	OR G	RAIN S	SIZE					RE	COMMEN	DATION SYME	BOLS								ARDNESS	
U.S. STD. SIE			4 10		60		270					LASSIFIED E			SIFIED EXCAVATION - ABLE, BUT NOT TO BE						GEOLOGIST"	P PICK. BREAKING OF HA S PICK.	IND SPECIMEN
OPENING (MM	0		4.76 2.00	0 0.4			0.053					UITABLE WAS LASSIFIED E	XCAVATION -	USED I	N THE TOP 3 FEET OF				ATCHED B		OR PICK ON	LY WITH DIFFICULTY. HAP	rd hammer bi
BOULDER (BLDR.)		BBLE	GRAVEL	SAN	ND DI	F INE SAND		SILT SL.)	CLAY (CL.)				GRADABLE ROCK	EMDHIN	MENT OR BACKFILL						OR PICK. GO	UGES OR GROOVES TO Ø.	.25 INCHES DE
				(CSE.		(F SD.	.)			AR - AUGER REFUSAL			REVIATIONS MEDIUM	WCT	- VANE SHEAR TEST				BY HARD E BLOWS.		A GEOLOGIS	ST'S PICK. HAND SPECIME	INS CAN BE D
GRAIN MM SIZE IN.	305 12	75 3	2.0		0.2	5	0.05	0.005		BT - BORING TERMINATED	C	MICA	- MICACEOUS	WEA.	- WEATHERED						0.05 INCHES	DEEP BY FIRM PRESSURE	E OF KNIFE O
	9	OIL MOI	STURE -	CORRI	ELATI	N OF	TERMS			CL CLAY CPT - CONE PENETRATION	N TEST		MODERATELY NON PLASTIC	γ_{-}	UNIT WEIGHT DRY UNIT WEIGHT					IN SMALL IST'S PICK.		EICES 1 INCH MAXIMUM S	JIZE BY HARD
	MOISTURE		FIELD M		GU	IDE FOR 1	IELD MOI	STURE DES	CRIPTION	CSE COARSE		ORG	ORGANIC	-	MPLE ABBREVIATIONS	SOFT	CAN BE	BE GROVE	ED OR G	GOUGED RE	EADILY BY K	NIFE OR PICK. CAN BE E	
	ERBERG LI	MITS)	1	RIPTION						DMT - DILATOMETER TEST DPT - DYNAMIC PENETRAT			PRESSUREMETER T SAPROLITIC	ESI <u>S</u> - E							ES IN SIZE	BY MODERATE BLOWS OF URE.	A PICK POIN
			- SATUR (SAT					WET, USU		e – VOID RATIO F – FINE			SAND, SANDY SILT, SILTY		SPLIT SPOON SHELBY TUBE							AVATED READILY WITH PO	
		LIMIT								FOSS FOSSILIFEROUS		SL1	SLIGHTLY	RS -	ROCK		OR MO		THICKNES	SS CAN BI	E BROKEN B	Y FINGER PRESSURE. CAN	N BE SCRATCH
RANGE <			- WET -	(W)		MISOLID; F TAIN OPTI		DRYING TO		FRAC FRACTURED, FRAC FRAGS FRAGMENTS	TURES		TRICONE REFUSAL DISTURE CONTENT		RECOMPACTED TRIAXIAL	F	RAC.	TURE	SPA	CING		E	BEDDING
(PI) PL	PLASTI	C LIMIT								HI HIGHLY		V - VE			RATIO	TERM				SPACING			0050
ОМ		M MOISTURE	- MOIST	- (M)	SO	LID; AT OF	R NEAR OF	тімим мо	ISTURE	DRILL UNITS:		NI USED	ON SUBJEC	HAMMER		VERY WIDE WIDE			З Т	THAN 10 TO 10 FE	ET	VERY THICKLY BED THICKLY BEDDED	1.
		AGE LIMIT								CME-45C		CLAY BITS			TOMATIC MANUAL	MODERATEL CLOSE	Y CLO	JSE		TO 3 FEE 6 TO 1 FO		THINLY BEDDED VERY THINLY BEDD	0.1 DED 0.0
			- DRY -	(D)		OUIRES AD		WATER TO)				S FLIGHT AUGER			VERY CLOS	E			THAN 0.16		THICKLY LAMINATE THINLY LAMINATED	ED 0.0 0
			DI	ASTIC						CME-55		8 HOLLOW AU		CORE SI							INDUF		
					NDEX (PI)			Y STRENG	тн	CME-550		HARD FACED	FINGER BITS		_	FOR SEDIMENT	ARY F	ROCKS, I	INDURAT	TION IS T	HE HARDEN	ING OF MATERIAL BY CE	EMENTING, HE
	PLASTIC		12451	0-5				VERY LOW			י 🗖	TUNGCARBID	E INSERTS			FRIABL	E					FINGER FREES NUMEROU BY HAMMER DISINTEGRAT	
	HTLY PLAS RATELY P			6-15 16-25				SLIGHT MEDIUM		VANE SHEAR TEST	X	CASING X	W/ ADVANCER		DLS: ST HOLE DIGGER							SEPARATED FROM SAME	
	LY PLASTI			26 OR M	IORE			HIGH		X MOBILE B-57	י 🗍		STEEL TEETH		ID AUGER	MODERA	TELY	INDURA	ŧΤΕD			WHEN HIT WITH HAMME	
I				COLOF	2					4 🗖 🤺		TRICONE 2	7/8" TUNGCARB.		INDING ROD	INDURA	TED					FFICULT TO SEPARATE N BREAK WITH HAMMER.	WITH STEEL
			OR OR COLOF								XC	CORE BIT			E SHEAR TEST							BREAK WITH HAMMER. BLOWS REQUIRED TO BE	REAK CAMPIE
MOE	MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.							🗌 .					EXTREM	IELY I	INDURAT	/ED			ACROSS GRAINS.	CERK SHEPPLE			

PROJECT REFERENCE NO.



2

ED. AN INFERRED	TERMS AND DEFINITIONS
D SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
1 FOOT PER 60 IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
T N VALUES >	ABGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ADTECIAN, CODUND WATER THAT IS INDER SUPERIEITENT DESCRIPT TO RESCARDANT THE LEVEL AT
OCK THAT NCLUDES GRANITE.	A <u>RTESIAN</u> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
AL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
IF TESTED.	$\underline{\text{COLLUVIUM}}$ - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
MAY NOT YIELD STONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
RINGS UNDER	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
COATINGS IF OPEN,	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
HAMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
OCK UP TO AL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
R BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
TS. IN AY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
H AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FELDSPARS DULL LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
RE DISCERNIBLE DF STRONG ROCK T ONLY MINOR	USUBLET INDICATES FOUR AERATION AND LACK OF GUUD DRAINAGE. <u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
IN SMALL AND IS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
	RUN AND EXPRESSED AS A PERCENTAGE. <u>SAPPOLITE (SAP.)</u> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
NS REQUIRES BLOWS REQUIRED	SOLT - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
DETACHED	UR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF) OF
OR PICK POINT. BLOWS OF THE	A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
N FRAGMENTS NT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
. PIECES 1 INCH HED READILY BY	STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK DUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SECMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	BENCH MARK: BORING LOCATIONS DETERMINED WITH RTK GPS. ELEVATIONS
<u>THICKNESS</u> 4 FEET 1.5 - 4 FEET	OF STRUCTURE BORINGS OBTAINED WITH RTK GPS; ROADWAY BORINGS OBTAINED FROM 22105_Ls_tin.tin. ELEVATION: NGVD 88 US FT
.16 - 1.5 FEET	NOTES:
03 - 0.16 FEET 08 - 0.03 FEET < 0.008 FEET	NUTES: INVESTIGATION COMPLETED ON PRELIMINARY DESIGN FILES PROVIDED BY NCDOT ON MAY 26,2023
	FIAD = FILLED IMMEDIATELY AFTER DRILLING
EAT, PRESSURE, ETC.	NEM = NOT ENOUGH MATERIAL FOR FULL ANALYSIS
	ALM ANT ENGLINE TENAL FOR FOLL ANALISIS
• TEEL PROBE;	
PROBE:	
E;	
	DATE: 8-15-14



STATE	8	TATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS						
N.C.		B-5766	3	7						
STAT	E PROJ.NO.	F. A. PROJ. NO.		DESCRIPT	ION					
45	722.1.3	N⁄A		PE						
45	722.2.1	N⁄A		R/W						
45	722.2.2	N⁄A		-						
45	722.3.1	N⁄A		CONS	CONST.					
	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED									



December 2023

WBS Number: TIP Number: Project ID: County: Description:	45722.1.1 B-5766 42290 STOKES Bridge Number 82 over Dan River on SR 1674 (Sheppard Mill Road) Danbury, North Carolina
CATLIN Number:	223152
SUBJECT:	Roadway Subsurface Inventory Report

Project Description

This project is located on Sheppard Mill Road immediately east of Danbury in Stokes County, North Carolina. Approximately one-half of the project is located within Moratock Park which was added to the National Register of Historic Places in 1974. The proposed project consists of improvement of SR 1674 (Sheppard Mill Road) along with the replacement of existing bridge Number 82 over the Dan River. This geotechnical investigation was conducted by Richard Catlin and Associates (DBE Catlin Engineers and Scientists – (CATLIN)) and was confined to the areas of proposed construction.

Fieldwork was conducted by CATLIN personnel in October 2023. Standard Penetration Test (SPT) borings were completed along the project corridor with an average distance of approximately 150 linear feet between borings. Additional SPT borings advanced during the associated Structure Inventory were utilized to augment the roadway data. Representative soil samples were collected for visual classification in the field and for laboratory analysis.

The following alignment was investigated. Plan sheets and subsurface profiles are included in this report and were generated from files received from NCDOT on May 26, 2023.

<u>Line</u>	<u>Station (±)</u>
-L-	10+00 to 21+35

Areas of Special Geotechnical Interest

1) Seasonal high groundwater was encountered at the following location:

<u>Line</u>	<u>Station (±)</u>
-L-	12+00

2) Cohesive soils that may have the potential to cause embankment/subgrade and or slope stability problems during construction were identified at the following locations:

<u>Line</u>	<u>Station (±)</u>
-L-	11+00 to 13+50
-L-	15+40 to 17+25

Physiography and Geology

This project area is located within the North Carolina Inner Piedmont physiographic province. The North Carolina Piedmont is typically characterized by gently rolling, well-rounded hills with a few hundred feet of relief between the hills and valleys. According to the 1985 North Carolina Geologic Map, the area of investigation lies within the Chauga Belt with the predominant rock type being metagraywacke (biotite gneiss) with muscovite-biotite schist interlayered and gradational throughout. Land use along the project area consists primarily of recreational with some residential housing and small business.

Groundwater

Groundwater data was collected in October 2023. Ground water was encountered from within 2.1 feet to greater than 36 feet from the ground surface along the area of investigation. The project spans the Dan River which as reported in the Bridge Survey & Hydraulic Design Report (BSR) has a normal water surface elevation (NWSEL) of 677.3 feet. Mill Creek crosses highway 89 terminating in the Dan River approximately 100 feet south of the project site.

Soils

Soils encountered along the project site include Roadway Embankment, Artificial Fill, Alluvial, Residual, Weathered Rock, and Crystalline Roack.

- expected to be comprised of materials similar to those used for Roadway Embankment.
- encountered along the eastern bank of the Dan River.
- east of the Dan River.
- River. No rock core was collected during the Roadway Investigation.

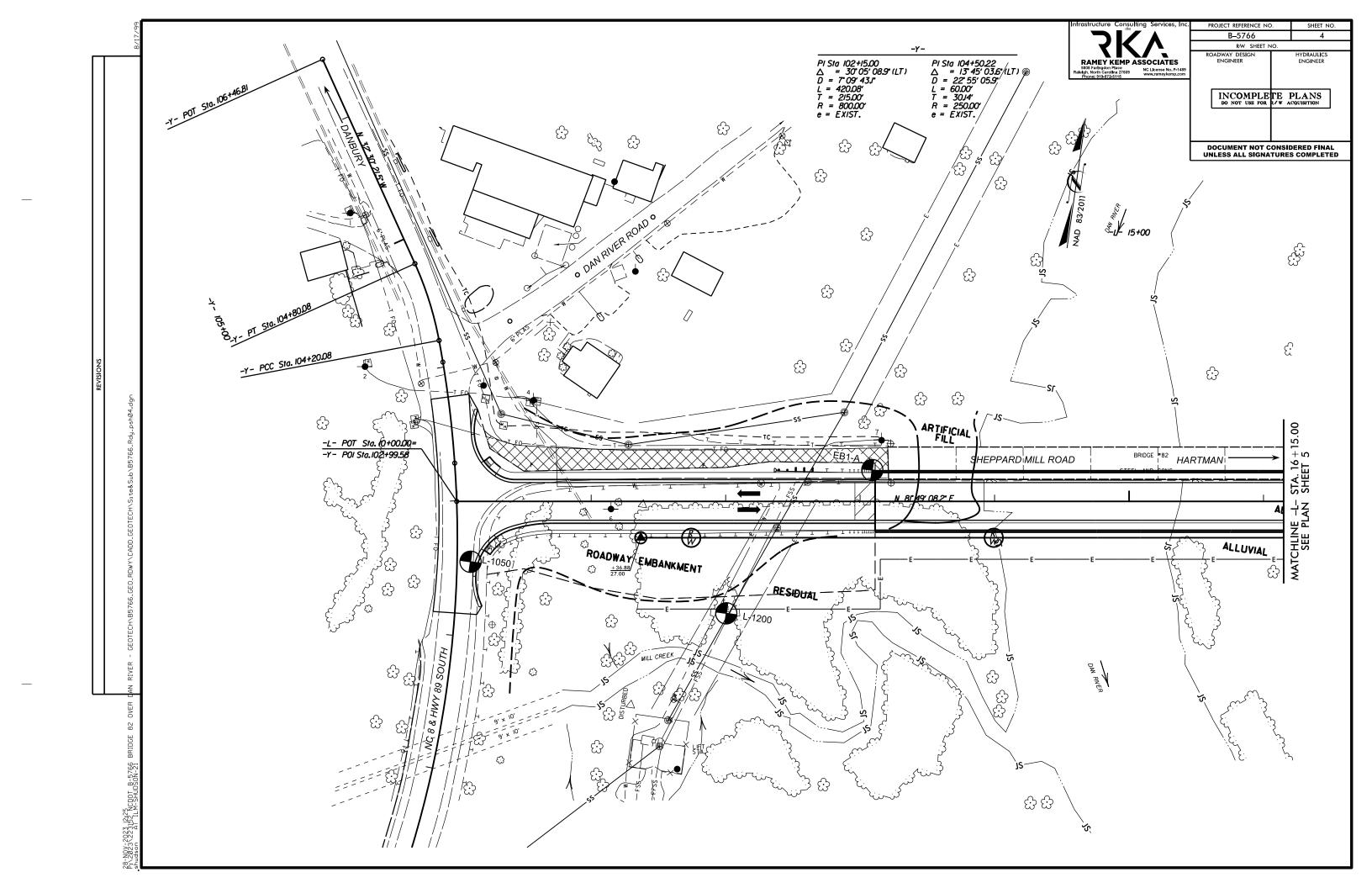
 Roadway Embankment soils exist beneath and adjacent to existing -L- (Sheppard Mill Road) and consist of approximately three (3) to 20 feet of material. The embankment soils are comprised of soft to stiff, fine grained sandy clay and sandy silt (A-4, A-6) and very loose to loose, fine grained silty sands (A-2-4). • Artificial Fill was identified and inferred along the project corridor adjacent to the Dan River and under the Moratock Park access roads. Although not sampled, the materials under the access roads are

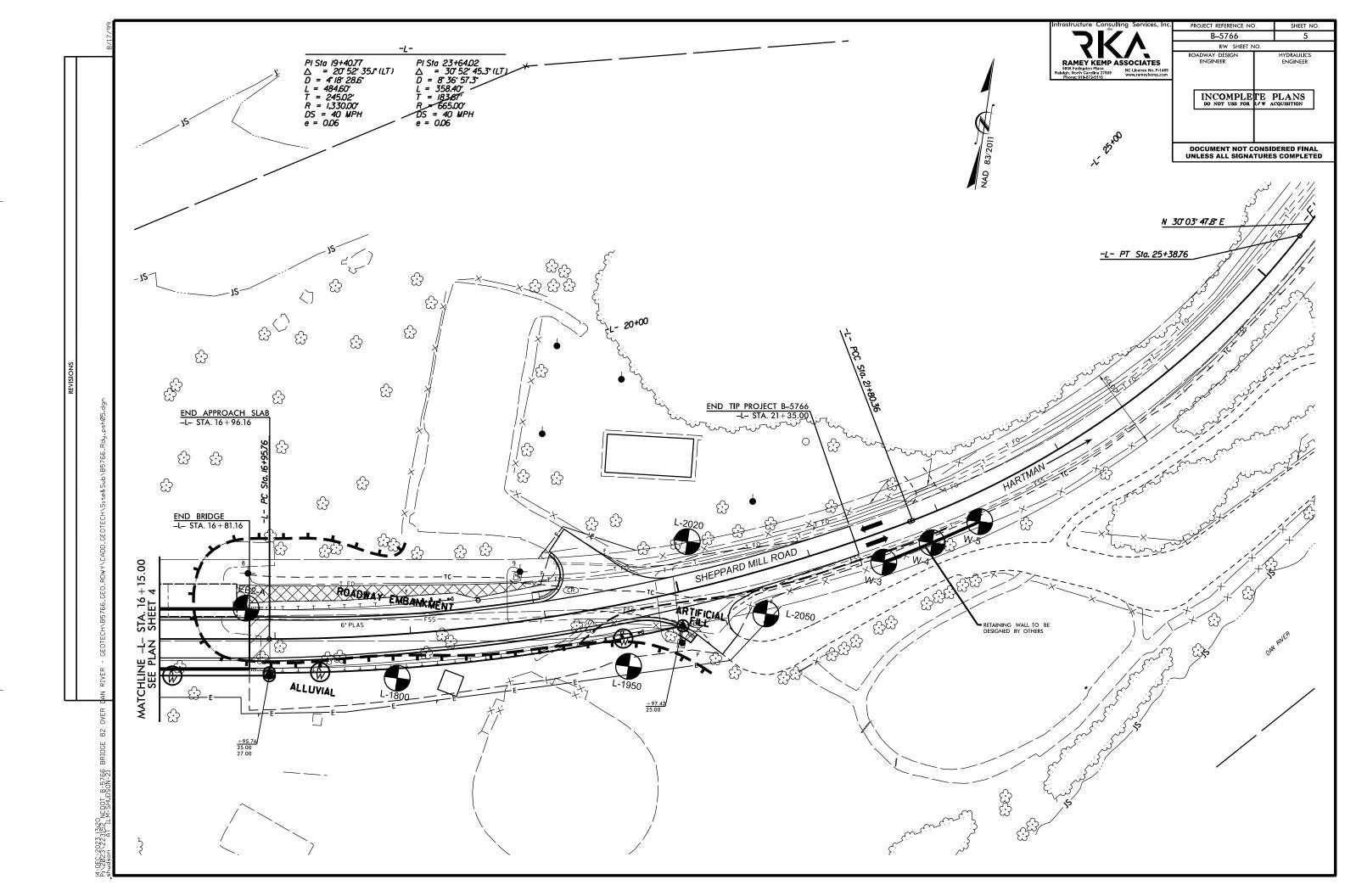
• Alluvial soils were identified across the flood plain to the east of the Dan River and south of Sheppard Mill Road at thickness ranging from six (6) to 12 feet. The soils were predominantly described as very loose to loose, fine grained sands and silty, fine grained sands (A-3, A-2-4) with some well-rounded gravels at the base of the stratum. Medium stiff, clay and fine grained sandy clay (A-6) was

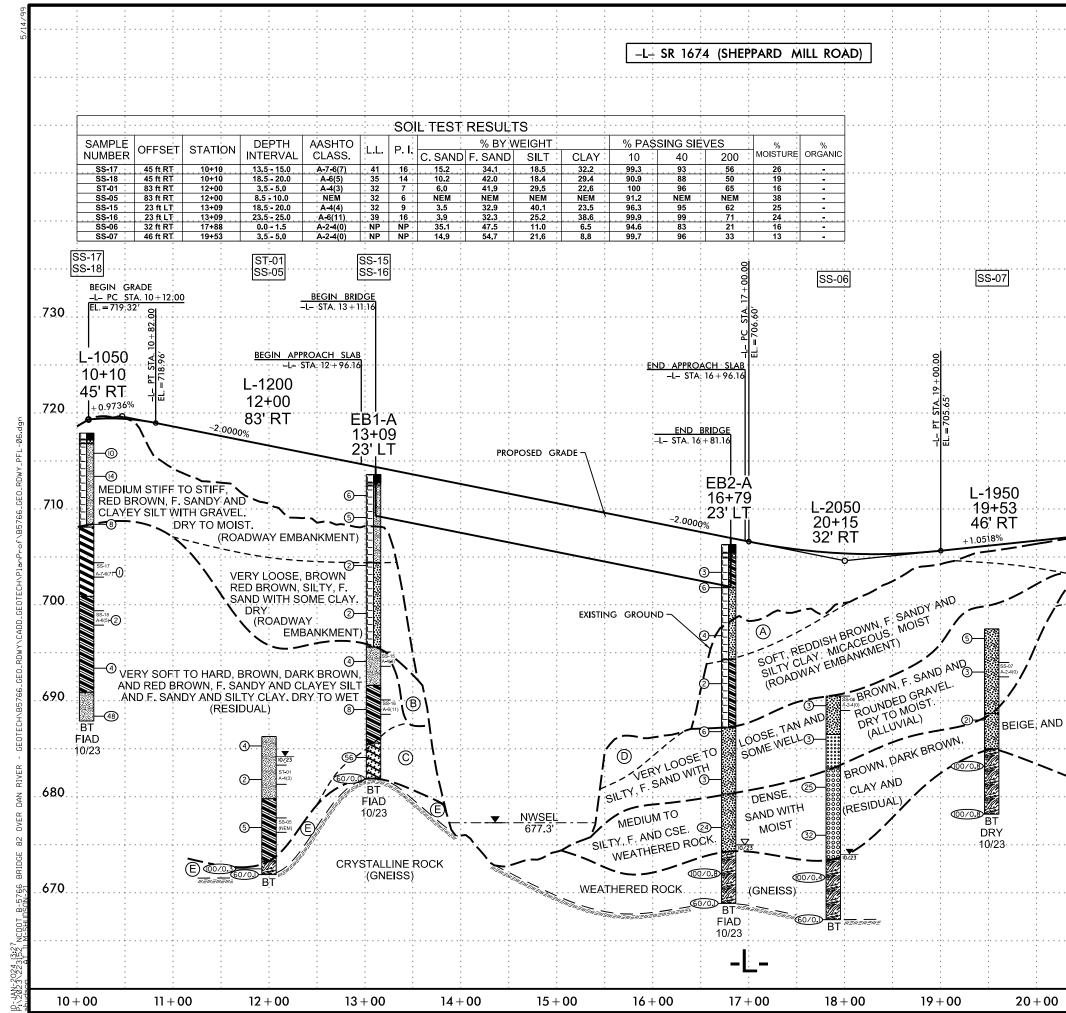
 Residual materials consisting of very soft to hard, fine sandy and clavey silt and fine sandy and silty clay (A-6, A-7, A-4) were encountered beneath the Roadway Embankment on the west side of the Dan River at thicknesses of up to approximately 23 feet. A small amount of loose to very dense sand and clayey sand (A-2-4, A-2-6) was identified beneath the fine material adjacent to the Dan River. Residual materials on the east side of the Dan River were comprised primarily of loose to dense, silty, fine and coarse sand (A-2-4, A-3) with some clay and weathered rock fragments at thickness up to 15 feet.

• Weathered Rock (Gneiss) was identified beneath the Residual soils at elevations ranging from 682 feet 673 feet west of the Dan River and from elevations ranging from approximately 665 feet to 685 feet

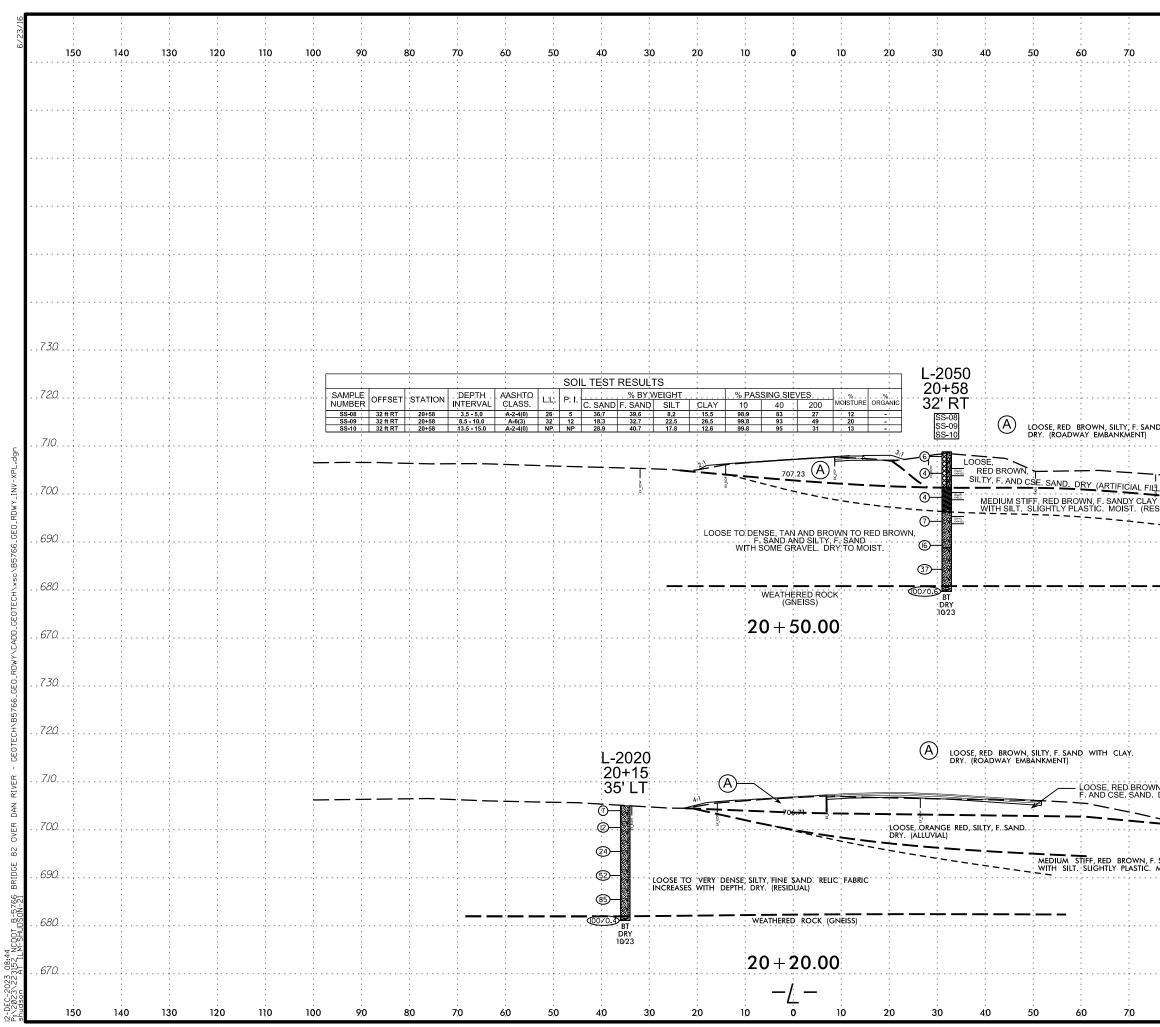
 Crystalline Rock (Gneiss) was identified beneath the Weathered Gneiss at elevations ranging from a high of 682 feet near the west side of the Dan River to a low of 668 feet on the east side of the Dan







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GEOTECHNICAL BORING REI HAND AUGER AND DCP			223152 CATLIN PROJECT REFERENCE SHEET B-5766 8
WBS: 45722.1.1 TIP: B-5766 COUNTY: STOKES	GEOLOGIST: C. Stratton	WBS: 45722.1.1 TIP: B-5766 COUNTY: STO	KES GEOLOGIST: C. Stratton
SITE DESCRIPTION: Bridge Number 82 over Dan River on SR 1674 (Sheppard Mill Road), Danbury, No.	orth Carolna GROUND WTR (ft)	SITE DESCRIPTION: Bridge Number 82 over Dan River on SR 1674 (Sheppard	Mill Road), Danbury, North Carolna GROUND WTR (ft)
BORING NO.: W-3 STATION: 21+50 OFFSET: 21 ft RT	ALIGNMENT: -L- 0 HR. Dry	BORING NO.: W-4 STATION: 21+92 OFFSE	T: 19 ft RT ALIGNMENT: -L- 0 HR. Dry
COLLAR ELEV.: 707.4 ft TOTAL DEPTH: 6.5 ft NORTHING: 969,626	EASTING: 1,647,433 24 HR. FIAD	COLLAR ELEV.: 707.4 ft TOTAL DEPTH: 4.0 ft NORTH	ING: 969,648 EASTING: 1,647,470 24 HR. FIAD
DRILL RIG/HAMMER EFF./DATE: HAND AUGER DRILL METHOD: H.	AND AUGER W/DCP HAMMER TYPE: MANUAL	DRILL RIG/HAMMER EFF./DATE: HAND AUGER	DRILL METHOD: HAND AUGER W/DCP HAMMER TYPE: MANUAL
DRILLER: N/A START DATE: 12/07/23 COMP. DATE: 12/07/23	SURFACE WATER DEPTH: N/A		DATE: 12/07/23 SURFACE WATER DEPTH: N/A
ELEV DRIVE ELEV DEPTH BLOW COUNT AVERAGE DCP VALUES SAMP. # L O (ft) (ft) 1.75in 1.75in 0 25 50 75 100 RESULT MOU G	SOIL AND ROCK DESCRIPTION	ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUNT AVERAGE DCP VALUES 0 25 50 75	100 RESULT THOUSE SOIL AND ROCK DESCRIPTION
(ii) (ii) 1.75in 1.75in 1.75in 0 25 50 75 100 RESULT MOL G	ELEV. (ft) DEPTH (ft)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	100 KESULI MOI G
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	Boring Terminated WITH HAND AUGER REFUSAL at Elevation 700.9 ft ON		
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GEOTECHNICAL BORING REPORT HAND AUGER AND DCP

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