

REFERENCE: B-5766

PROJECT: 45722

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5766	1	15

**STRUCTURE**  
**SUBSURFACE INVESTIGATION**

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SHEET NO.	DESCRIPTION
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COUNTY STOKES  
PROJECT DESCRIPTION BRIDGE NUMBER 82 OVER  
DAN RIVER ON SR 1674 (SHEPPARD MILL ROAD)  
DANBURY, NORTH CAROLINA  
SITE DESCRIPTION \_\_\_\_\_  
-L- STATION 13+11.16 (BEGIN BRIDGE)

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

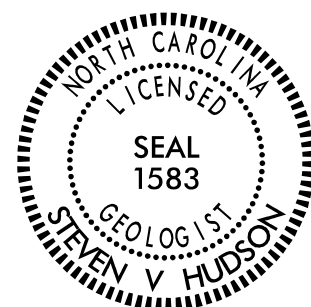
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  - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

T. PARK  
C. STRATTON  
T. J. WHITE, CWC  
S. PUGH, CWC

INVESTIGATED BY CATLIN  
DRAWN BY S.V. HUDSON, PG  
CHECKED BY J. LEE STONE, PG  
SUBMITTED BY S. V. HUDSON, PG  
DATE DECEMBER 2023



DocuSigned by:  
Steve V Hudson 12/14/2023  
01DB23BB8740D660  
SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL  
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# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION												
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>												
SOIL LEGEND AND AASHTO CLASSIFICATION												
GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)						SILT-CLAY MATERIALS (> 35% PASSING #200)			ORGANIC MATERIALS		
GROUP CLASS.	A-1	A-3	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7	
SYMBOL												
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX 10 MX	35 MX 10 MX	35 MX 10 MX	35 MX 10 MX	36 MN 10 MX	36 MN 10 MX	36 MN 10 MX	36 MN 10 MX	
MATERIAL PASSING #40 LL PI							40 MX 10 MX	41 MN 10 MX	40 MX 10 MX	41 MN 10 MX	40 MX 10 MX	41 MN 10 MX
GROUP INDEX							0	4 MX	8 MX	12 MX	16 MX	NO MX
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND			SILTY SOILS	CLAYEY SOILS		HIGHLY ORGANIC SOILS		
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD						FAIR TO POOR			FAIR TO POOR	POOR	UNSUITABLE
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30												
CONSISTENCY OR DENSENESS												
PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )									
GENERALLY GRANULAR MATERIAL (NON-COHESSIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A									
GENERALLY SILT-CLAY MATERIAL (COHESSIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4									
TEXTURE OR GRAIN SIZE												
U.S. STD. SIEVE SIZE	4	10	40	60	200	270						
OPENING (MM)	4.75	2.00	0.42	0.25	0.075	0.053						
BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)						
GRAIN SIZE	MM 305 IN. 12	75 3	2.0	0.25	0.05	0.005						
SOIL MOISTURE - CORRELATION OF TERMS												
SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION										
LL - LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE										
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE										
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE										
SL - SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE										
PLASTICITY												
			PLASTICITY INDEX (PI)		DRY STRENGTH							
NON PLASTIC	0-5		VERY LOW									
SLIGHTLY PLASTIC	6-15		SLIGHT									
MODERATELY PLASTIC	16-25		MEDIUM									
HIGHLY PLASTIC	26 OR MORE		HIGH									
COLOR												
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.												

GRADATION			
WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.			
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.			
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.			
ANGULARITY OF GRAINS			
THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.			
MINERALOGICAL COMPOSITION			
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.			
COMPRESSIBILITY			
SLIGHTLY COMPRESSIBLE	LL < 31		
MODERATELY COMPRESSIBLE	LL = 31 - 50		
HIGHLY COMPRESSIBLE	LL > 50		
PERCENTAGE OF MATERIAL			
ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE
GROUND WATER			
	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING		
	STATIC WATER LEVEL AFTER 24 HOURS		
	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA		
	SPRING OR SEEP		
MISCELLANEOUS SYMBOLS			
	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION		
	SOIL SYMBOL		
	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT		
	INFERRED SOIL BOUNDARY		
	INFERRED ROCK LINE		
	ALLUVIAL SOIL BOUNDARY		
	DIP & DIP DIRECTION OF ROCK STRUCTURES		
	SPT TEST BORING		
	AUGER BORING		
	CORE BORING		
	MONITORING WELL		
	PIEZOMETER INSTALLATION		
	SLOPE INDICATOR INSTALLATION		
	CONE PENETROMETER TEST		
	SOUNDING ROD		
	TEST BORING WITH CORE		
	SPT N-VALUE		
RECOMMENDATION SYMBOLS			
	UNDERCUT		
	SHALLOW UNDERCUT		
	UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE		
	UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK		
	UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL		
ABBREVIATIONS			
AR - AUGER REFUSAL	MED. - MEDIUM	VST - VANE SHEAR TEST	
BT - BORING TERMINATED	MICA - MICACEOUS	WEA. - WEATHERED	
CL. - CLAY	MOD. - MODERATELY	UW - UNIT WEIGHT	
CPT - CONE PENETRATION TEST	NP - NON PLASTIC	UW - DRY UNIT WEIGHT	
CSE. - COARSE	ORG. - ORGANIC	SAMPLE ABBREVIATIONS	
DMT - DILATOMETER TEST	PMT - PRESSUREMETER TEST	S - BULK	
DPT - DYNAMIC PENETRATION TEST	SAP. - SAPROLITIC	SS - SPLIT SPOON	
e - VOID RATIO	SD. - SAND, SANDY	ST - SHELBY TUBE	
F - FINE	SL. - SILTY, SILTY	RS - ROCK	
FOSS. - FOSSILIFEROUS	SLI. - SLIGHTLY	RT - RECOMPACTED TRIAXIAL	
FRAC. - FRACTURED, FRACTURES	TCR - TRICONE REFUSAL	CBR - CALIFORNIA BEARING RATIO	
FRAGS. - FRAGMENTS	w - MOISTURE CONTENT		
HI. - HIGHLY	V - VERY		
EQUIPMENT USED ON SUBJECT PROJECT			
DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE:	
<input type="checkbox"/> CME-45C	<input type="checkbox"/> CLAY BITS	<input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL	
<input type="checkbox"/> CME-55	<input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER		
<input type="checkbox"/> CME-550	<input checked="" type="checkbox"/> 8" HOLLOW AUGERS	CORE SIZE:	
<input type="checkbox"/> VANE SHEAR TEST	<input type="checkbox"/> HARD FACED FINGER BITS	<input type="checkbox"/> -B <input type="checkbox"/> -H	
<input checked="" type="checkbox"/> MOBILE B-57	<input type="checkbox"/> TUNG-CARBIDE INSERTS	<input checked="" type="checkbox"/> -N Q	
<input type="checkbox"/>	<input checked="" type="checkbox"/> CASING <input checked="" type="checkbox"/> W/ ADVANCER	HAND TOOLS:	
<input type="checkbox"/>	<input type="checkbox"/> TRICONE <input type="checkbox"/> STEEL TEETH	<input type="checkbox"/> POST HOLE DIGGER	
<input type="checkbox"/>	<input checked="" type="checkbox"/> TRICONE 2 7/8" TUNG-CARB.	<input type="checkbox"/> HAND AUGER	
<input type="checkbox"/>	<input checked="" type="checkbox"/> CORE BIT	<input type="checkbox"/> SOUNDING ROD	
<input type="checkbox"/>		<input type="checkbox"/> VANE SHEAR TEST	

ROCK DESCRIPTION	
HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	
	WEATHERED ROCK (WR)
	CRYSTALLINE ROCK (CR)
	NON-CRYSTALLINE ROCK (NCR)
	COASTAL PLAIN SEDIMENTARY ROCK (CP)
NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	
FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	
FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	
WEATHERING	
FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SL.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SL.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH, OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL.
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF.
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF.
COMPLETE	ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.
ROCK HARDNESS	
VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.
FRACTURE SPACING	
TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET
BEDDING	
TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET
INDURATION	
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS	
<b>ALLUVIUM (ALLUV.)</b> - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.	
<b>AQUIFER</b> - A WATER BEARING FORMATION OR STRATA.	
<b>ARENACEOUS</b> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.	
<b>ARGILLACEOUS</b> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.	
<b>ARTESIAN</b> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.	
<b>CALCAREOUS (CALC.)</b> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.	
<b>COLLUVIUM</b> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.	
<b>CORE RECOVERY (REC.)</b> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
<b>DIKE</b> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.	
<b>DIP</b> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.	
<b>DIP DIRECTION (DIP AZIMUTH)</b> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.	
<b>FAULT</b> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.	
<b>FISSILE</b> - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.	
<b>FLOAT</b> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.	
<b>FLOOD PLAIN (FP)</b> - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.	
<b>FORMATION (FM)</b> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.	
<b>JOINT</b> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.	
<b>LEDGE</b> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.	
<b>LENS</b> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.	
<b>MOTTLED (MOT.)</b> - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.	
<b>PERCHED WATER</b> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.	
<b>RESIDUAL (RES.) SOIL</b> - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.	
<b>ROCK QUALITY DESIGNATION (RQD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
<b>SAPROLITE (SAP.)</b> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.	
<b>SILL</b> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.	
<b>SLICKENSIDE</b> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.	
<b>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)</b> - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.	
<b>STRATA CORE RECOVERY (SCREC.)</b> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.	
<b>STRATA ROCK QUALITY DESIGNATION (SROD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.	
<b>TOPSOIL (TS)</b> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
<b>BENCH MARK:</b> BORING LOCATIONS DETERMINED WITH RTK GPS. ELEVATIONS OF STRUCTURE BORINGS OBTAINED WITH RTK GPS; ROADWAY BORINGS OBTAINED FROM 22105.Ls.tin.tin. ELEVATION: NGVD 88 US FT.	
<b>NOTES:</b>	
INVESTIGATION COMPLETED ON PRELIMINARY DESIGN FILES PROVIDED BY NCDOT ON MAY 26, 2023	
FIAD = FILLED IMMEDIATELY AFTER DRILLING	

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
**GEOTECHNICAL ENGINEERING UNIT**

**SUBSURFACE INVESTIGATION**

**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES  
 FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

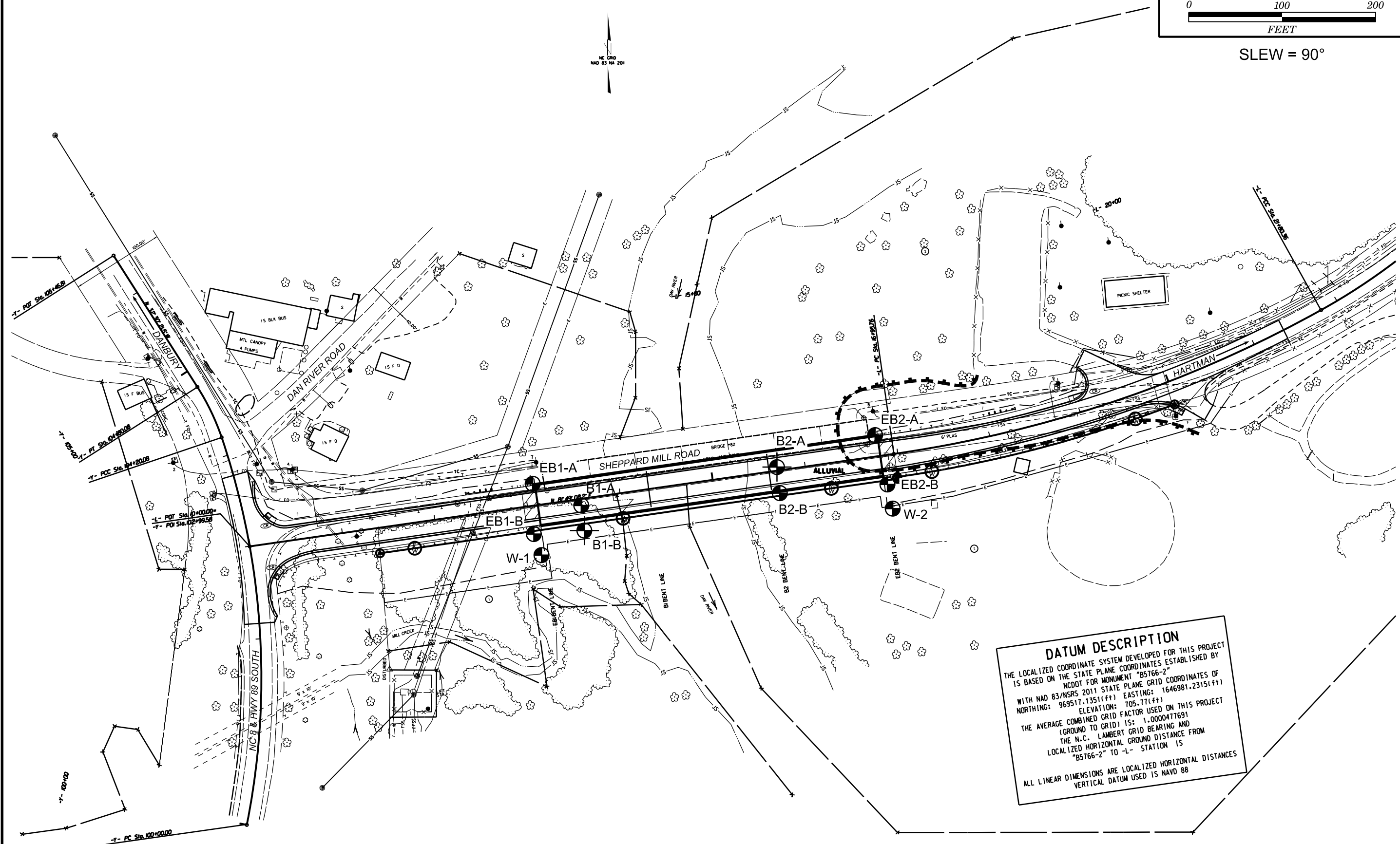
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

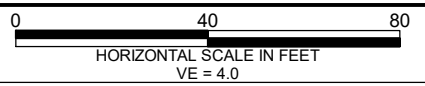
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		SURFACE CONDITIONS					GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)		SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)				
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.		VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings
STRUCTURE		DECREASING SURFACE QUALITY →					COMPOSITION AND STRUCTURE						
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities		90			N/A	N/A	<b>A. Thick bedded, very blocky sandstone</b> The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70					
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets		80					<b>B. Sandstone with thin inter-layers of siltstone</b>	60					
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets			70				<b>C. Sandstone and siltstone in similar amounts</b>		50				
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity			60				<b>D. Siltstone or silty shale with sandstone layers</b>			40			
DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces				50			<b>E. Weak siltstone or clayey shale with sandstone layers</b>				30		
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes					40		<b>F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure</b>					20	
					30		<b>G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers</b>						10
					20		<b>H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.</b>						
					10								
		N/A	N/A										

→ Means deformation after tectonic disturbance

SLEW = 90°

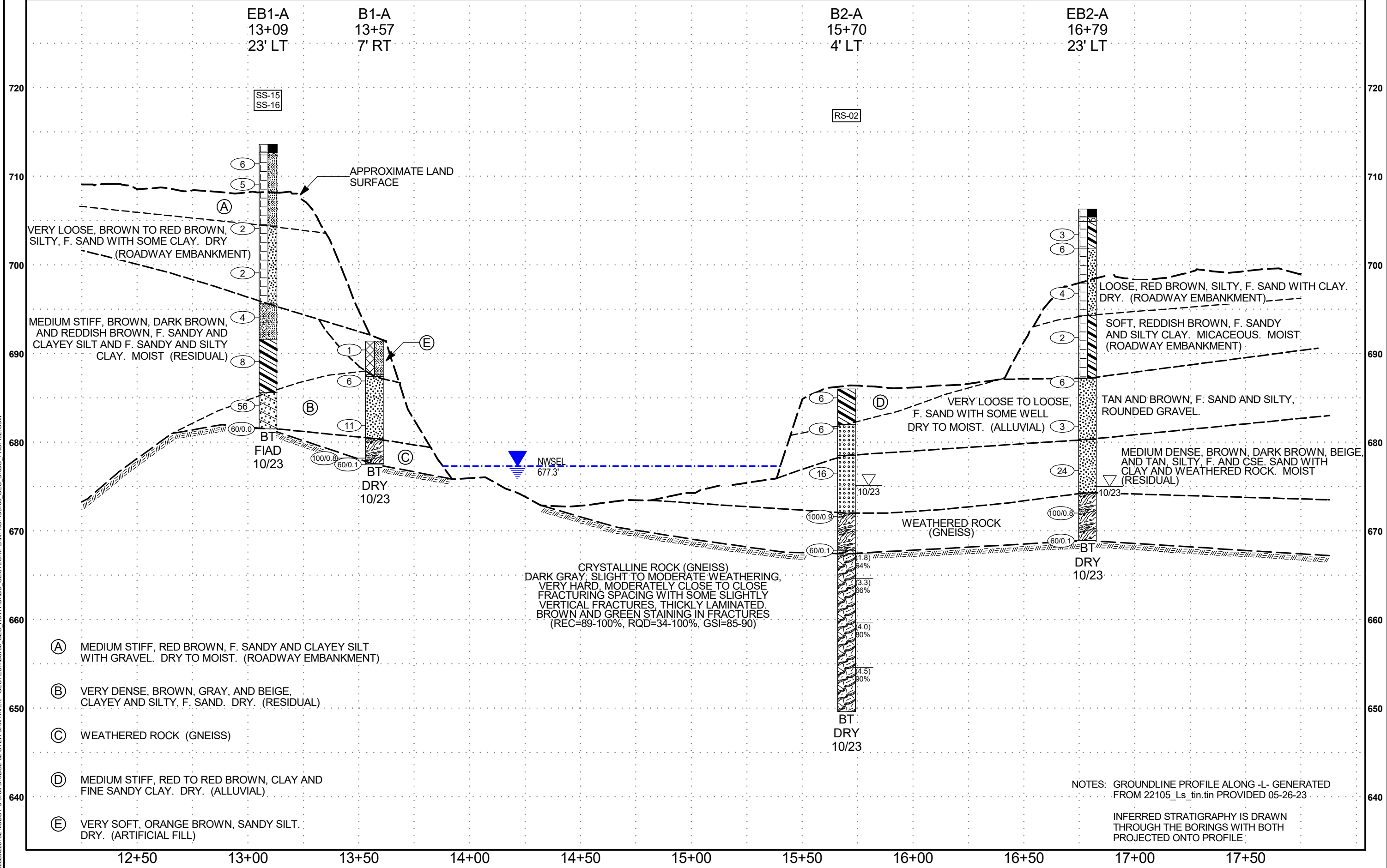


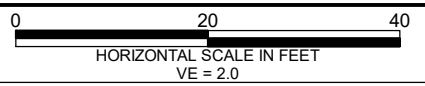
**DATUM DESCRIPTION**  
 THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "B5766-2" WITH NAD 83/NSRS 2011 STATE PLANE GRID COORDINATES OF NORTHING: 969517.1351(ft) EASTING: 1646981.2315(ft) ELEVATION: 705.77(ft)  
 THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 1.000047691  
 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "B5766-2" TO "L- STATION" IS  
 ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 88



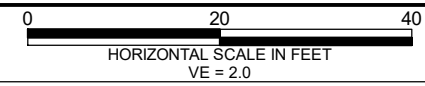
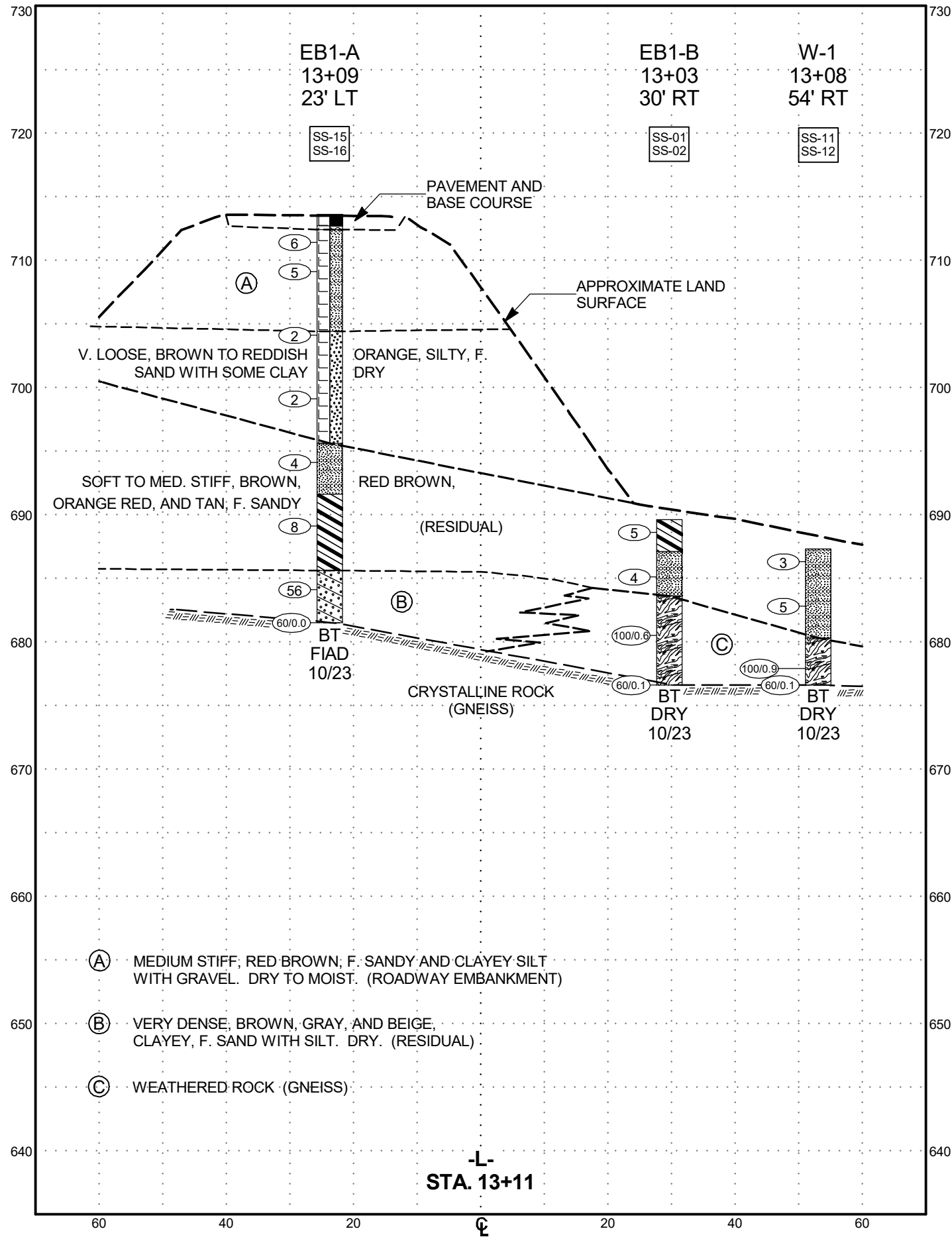
# PROFILE THROUGH BORINGS PROJECTED ALONG -L-

SKEW = 90°





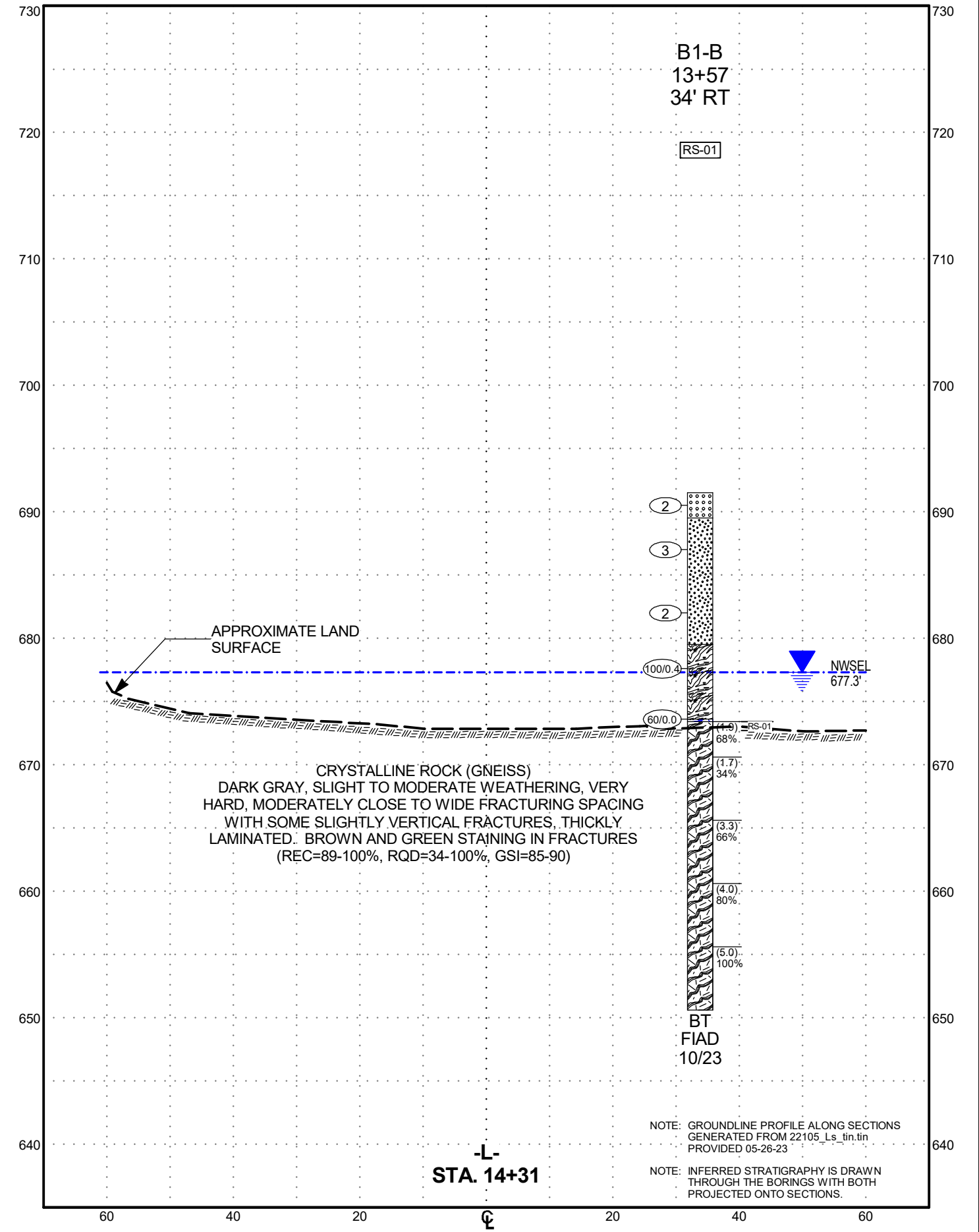
**CROSS SECTION  
END BENT 1  
SKEW = 90°**

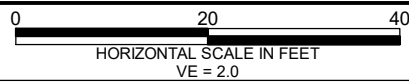


**CROSS SECTION  
BENT 1  
SKEW = 90°**

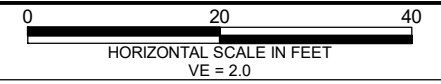
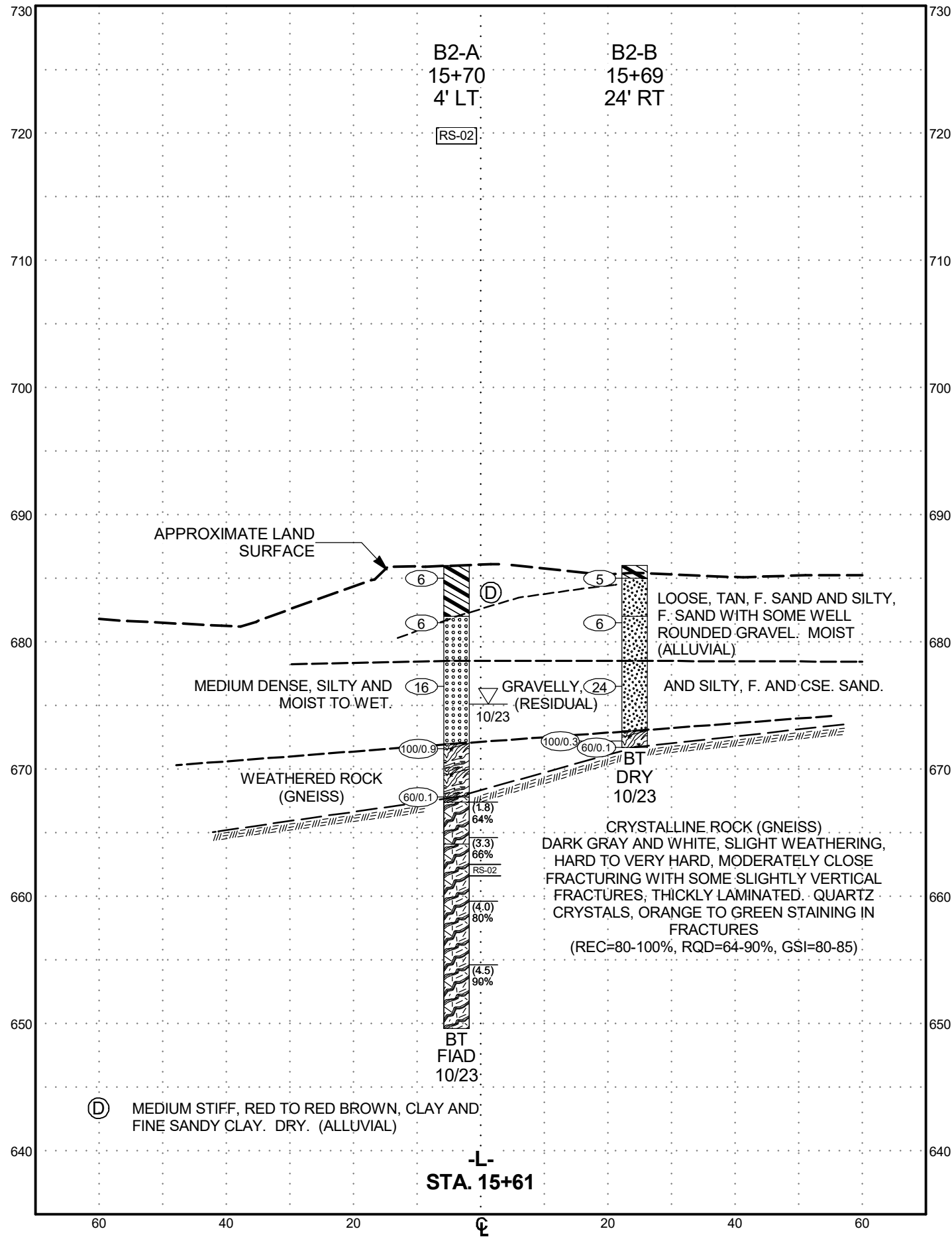


PROJECT REFERENCE	SHEET
B-5766	5

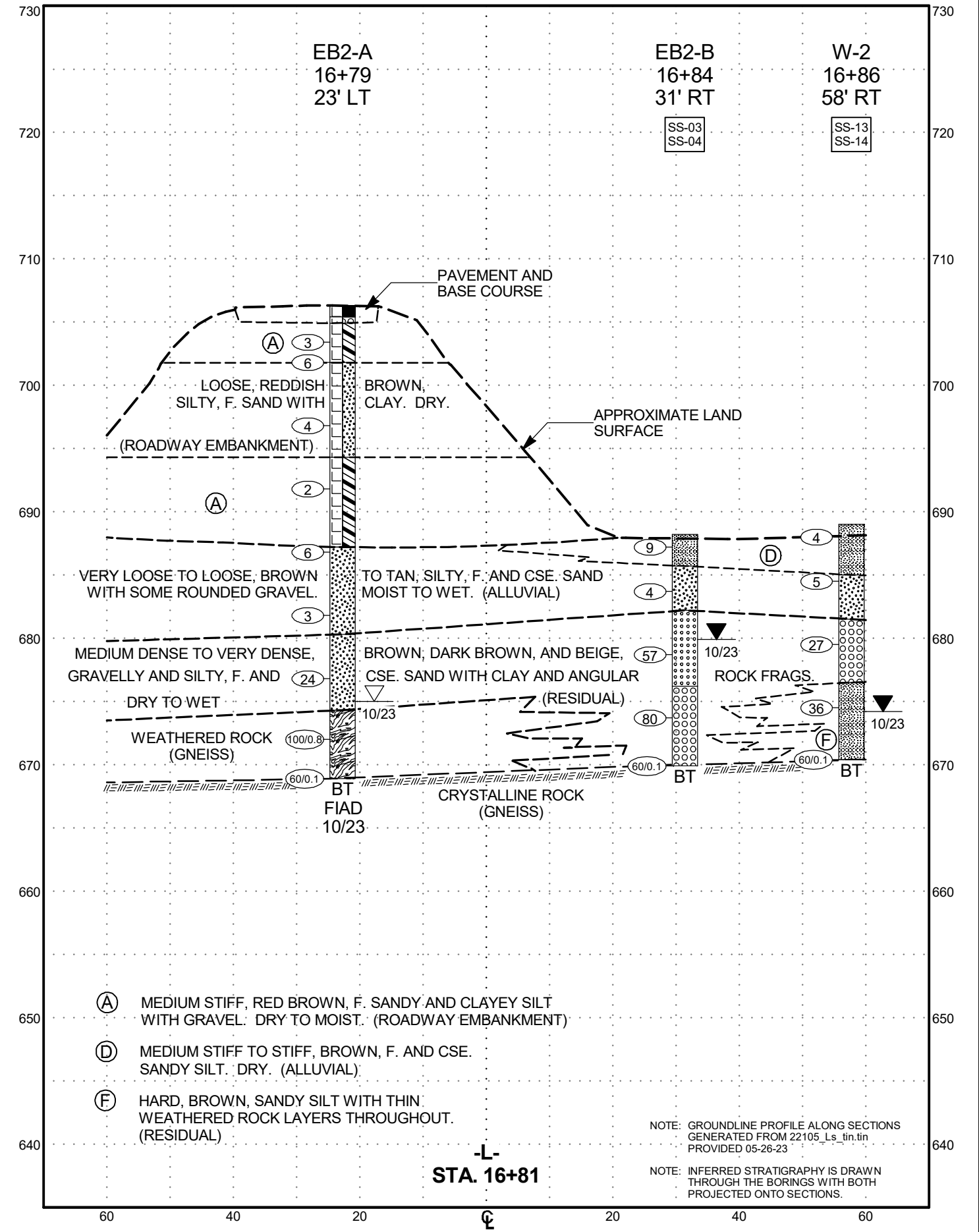




**CROSS SECTION  
BENT 2  
SKEW = 90°**



**CROSS SECTION  
END BENT 2  
SKEW = 90°**



# GEOTECHNICAL BORING REPORT BORE LOG



WBS: 45722.1.1	TIP: B-5766	COUNTY: STOKES	GEOLOGIST: C. Stratton
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River			GROUND WTR (ft)
BORING NO.: EB1-A	STATION: 13+09	OFFSET: 23 ft LT	ALIGNMENT: -L-
COLLAR ELEV.: 713.6 ft	TOTAL DEPTH: 31.7 ft	NORTHING: 969,473	EASTING: 1,646,607
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22		DRILL METHOD: H.S. AUGERS	HAMMER TYPE: AUTOMATIC
DRILLER: Shawn Pugh	START DATE: 10/10/23	COMP. DATE: 10/10/23	SURFACE WATER DEPTH: N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
715																
	712.4	1.2														
710	710.1	3.5	4	3	3											
			2	2	3											
705	705.1	8.5	WOH	1	1											
700	700.1	13.5	WOH	1	1											
695	695.1	18.5	2	2	2						SS-15 A-4(4)	25% M				
690	690.1	23.5	3	4	4						SS-16 A-6(11)	24% M				
685	685.1	28.5	11	15	41											
	681.9	31.7	60/0.0													

WBS: 45722.1.1	TIP: B-5766	COUNTY: STOKES	GEOLOGIST: T. Park
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River			GROUND WTR (ft)
BORING NO.: EB1-B	STATION: 13+03	OFFSET: 30 ft RT	ALIGNMENT: -L-
COLLAR ELEV.: 689.6 ft	TOTAL DEPTH: 13.0 ft	NORTHING: 969,419	EASTING: 1,646,608
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22		DRILL METHOD: H.S. AUGERS	HAMMER TYPE: AUTOMATIC
DRILLER: Shawn Pugh	START DATE: 10/05/23	COMP. DATE: 10/05/23	SURFACE WATER DEPTH: N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
715																
710																
705																
700																
695																
690	689.6	0.0	WOH	2	3						SS-01 A-6(11)	25%				
685	686.1	3.5	3	2	2						SS-02 A-4(0)	12%				
680	681.1	8.5	75	25/0.1												
	676.7	12.9	60/0.1													

NCDOT BORE DOUBLE B5766\_GEO\_BRDG&RDWY\_DRILLED.GPJ NCDOT\_CATLIN.GDT 10/24/23





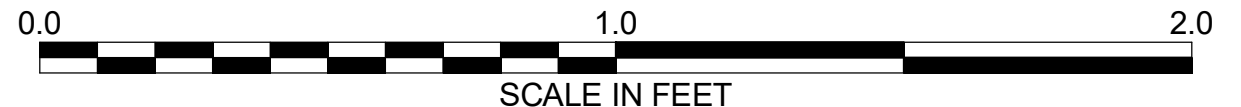
# GEOTECHNICAL BORING REPORT CORE LOG

**B1-B**  
DEPTH: 18.1 to 40.9 ft

WBS: 45722.1.1	TIP: B-5766	COUNTY: STOKES	GEOLOGIST: T. Park
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River			GROUND WTR (ft)
BORING NO.: B1-B	STATION: 13+57	OFFSET: 34 ft RT	ALIGNMENT: -L-
COLLAR ELEV.: 691.5 ft	TOTAL DEPTH: 40.9 ft	NORTHING: 969,423	EASTING: 1,646,662
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22		DRILL METHOD: NW Casing W/SPT & Core	HAMMER TYPE: AUTOMATIC
DRILLER: Shawn Pugh	START DATE: 10/05/23	COMP. DATE: 10/06/23	SURFACE WATER DEPTH: N/A
CORE SIZE: NQ	TOTAL RUN: 22.8 ft		

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (%)	RQD (%)		REC. (%)	RQD (%)			
673.4	673.4	18.1	2.8	3:38/1.0 3:30/1.0 1:43/0.8	(2.5)	(1.9)	RS-01	(22.5)	(15.9)	673.4	Begin Coring @ 18.1 ft CRYSTALLINE ROCK	18.1
670	670.6	20.9	5.0	2:19/1.0 2:19/1.0 2:21/1.0 2:55/1.0 2:09/1.0	(5.0)	(1.7)				670	DARK GRAY, GNEISS, SLIGHT TO MODERATE WEATHERING, VERY HARD, MODERATELY CLOSE TO CLOSE FRACTURING SPACING WITH SOME SLIGHTLY VERTICAL FRACTURES, THICKLY LAMINATED. BROWN AND GREEN STAINING IN FRACTURES. GNEISS (GSI=85-90)	
665	665.6	25.9	5.0	2:29/1.0 1:48/1.0 2:28/1.0 1:59/1.0 2:30/1.0	(5.0)	(3.3)				665		
660	660.6	30.9	5.0	2:29/1.0 2:09/1.0 1:49/1.0 2:14/1.0 2:00/1.0	(5.0)	(4.0)				660		
655	655.6	35.9	5.0	1:55/1.0 1:29/1.0 1:34/1.0 2:30/1.0 2:04/1.0	(5.0)	(5.0)				655		
	650.6	40.9								650.6	Boring Terminated at Elevation 650.6 ft IN CRYSTALLINE ROCK (GNEISS)	40.9

ROCK TEST RESULTS				
SAMPLE NUMBER	DEPTH INTERVAL	ROCK TYPE	UNIT WT. (lb/ft <sup>3</sup> )	UNIAXIAL COMPRESSIVE STRENGTH (psi)
RS-01	18.1' - 18.8'	GNEISS	162.4	13,950



NCDOT CORE W-PHOTO B5766\_GEO\_BRD&RDWY\_DRILLED.GPJ CATLIN.GDI 10/24/23

# GEOTECHNICAL BORING REPORT BORE LOG

WBS: 45722.1.1	TIP: B-5766	COUNTY: STOKES	GEOLOGIST: T. Park
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River			GROUND WTR (ft)
BORING NO.: B2-A	STATION: 15+70	OFFSET: 4 ft LT	ALIGNMENT: -L-
COLLAR ELEV.: 686.0 ft	TOTAL DEPTH: 36.4 ft	NORTHING: 969,491	EASTING: 1,646,868
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22		DRILL METHOD: NW Casing W/SPT & Core	
DRILLER: Shawn Pugh		HAMMER TYPE: AUTOMATIC	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
690															
685	686.0	0.0	2	3	3							M	LAND SURFACE ALLUVIAL RED BROWN, CLAY	0.0	
680	682.5	3.5	2	2	4							M	TAN, F. SAND	4.0	
675	677.5	8.5	7	7	9							W	RESIDUAL TAN, GRAVELLY, F. AND CSE. SAND.	7.5	
670	672.5	13.5	43	57/0.4								M	WEATHERED ROCK (GNEISS)	14.0	
665	667.9	18.1											CRYSTALLINE ROCK DARK GRAY AND WHITE, SLIGHT WEATHERING, HARD TO VERY HARD, MODERATELY CLOSE FRACTURING WITH SOME SLIGHTLY VERTICAL FRACTURES, THICKLY LAMINATED. QUARTZ CRYSTALS, ORANGE TO GREEN STAINING IN FRACTURES. GNEISS (REC=93%, RQD=76%, GSI=80-85)	18.2	
660															
655															
650															

WBS: 45722.1.1	TIP: B-5766	COUNTY: STOKES	GEOLOGIST: T. Park
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River			GROUND WTR (ft)
BORING NO.: B2-B	STATION: 15+69	OFFSET: 24 ft RT	ALIGNMENT: -L-
COLLAR ELEV.: 686.0 ft	TOTAL DEPTH: 14.3 ft	NORTHING: 969,463	EASTING: 1,646,871
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22		DRILL METHOD: H.S. AUGERS	
DRILLER: Shawn Pugh		HAMMER TYPE: AUTOMATIC	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
690															
685	686.0	0.0	2	2	3							M	LAND SURFACE ALLUVIAL RED, SANDY CLAY	1.0	
680	682.5	3.5	2	2	4							M	TAN, SILTY, F. AND CSE. SAND W/WELL ROUNDED GRAVEL TAN, SILTY, F. SAND	4.0	
675	677.5	8.5	10	11	13							M	RESIDUAL TAN, SILTY F. AND CSE. SAND W/GRAVEL	7.5	
670	672.5	13.5											WEATHERED ROCK (GNEISS)	13.0	
665	671.8	14.2											WEATHERED ROCK (GNEISS)	14.3	

NGDOT BORE DOUBLE B5766 GEO BRDG&RDWY DRILLED.GPJ NCDOT CATLIN.GDT 11/21/23

Boring Terminated at Elevation 649.6 ft IN CRYSTALLINE ROCK (GNEISS)

Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 671.7 ft ON CRYSTALLINE ROCK (GNEISS)

# GEOTECHNICAL BORING REPORT CORE LOG

**B2-A**  
DEPTH: 18.6 to 36.4 ft



PROJECT REFERENCE

B-5766

SHEET

11

WBS: 45722.1.1		TIP: B-5766		COUNTY: STOKES		GEOLOGIST: T. Park	
SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River							GROUND WTR (ft)
BORING NO.: B2-A		STATION: 15+70		OFFSET: 4 ft LT		ALIGNMENT: -L-	
COLLAR ELEV.: 686.0 ft		TOTAL DEPTH: 36.4 ft		NORTHING: 969,491		EASTING: 1,646,868	
DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22			DRILL METHOD: NW Casing W/SPT & Core			HAMMER TYPE: AUTOMATIC	
DRILLER: Shawn Pugh		START DATE: 10/03/23		COMP. DATE: 10/03/23		SURFACE WATER DEPTH: N/A	
CORE SIZE: NQ		TOTAL RUN: 17.8 ft					

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (ft)	RQD (%)		REC. (ft)	RQD (%)			
667.4	667.4	18.6	2.8	1:51/1.0	(2.6)	(1.8)		(16.5)	(13.6)		Begin Coring @ 18.6 ft	18.6
665	664.6	21.4		1:33/1.0 1:19/0.8	93%	64%		93%	76%		CRYSTALLINE ROCK	
			5.0	1:23/1.0 2:30/1.0	80%	66%	RS-02				DARK GRAY AND WHITE, SLIGHT WEATHERING, HARD TO VERY HARD, MODERATELY CLOSE FRACTURING WITH SOME SLIGHTLY VERTICAL FRACTURES, THICKLY LAMINATED. QUARTZ CRYSTALS, ORANGE TO GREEN STAINING IN FRACTURES, GNEISS (GSI=80-85)	
660	659.6	26.4		1:49/1.0 1:32/1.0 1:24/1.0	(4.0)	(3.3)						
			5.0	1:43/1.0 1:21/1.0 1:23/1.0 1:31/1.0 1:28/1.0	(4.9)	(4.0)						
655	654.6	31.4		1:49/1.0 1:53/1.0 1:41/1.0 1:37/1.0 1:58/1.0	100%	90%						
650	649.6	36.4									Boring Terminated at Elevation 649.6 ft IN CRYSTALLINE ROCK (GNEISS)	36.4

## ROCK TEST RESULTS

SAMPLE NUMBER	DEPTH INTERVAL	ROCK TYPE	UNIT WT. (lb/ft <sup>3</sup> )	UNIAXIAL COMPRESSIVE STRENGTH (psi)
RS-02	23.5' - 24.4'	GNEISS	163.6	7,210

18.6



21.4

RS-02 (23.5' - 24.4')

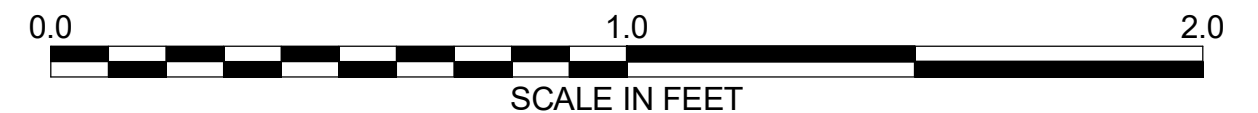
26.4

31.4

36.4

BOX 1 of 2

BOX 2 of 2



NCDOT CORE W-PHOTO B5766\_GEO\_BRD&RDWY\_DRILLED.GPJ CATLIN.GDI 10/24/23

# GEOTECHNICAL BORING REPORT BORE LOG



WBS: 45722.1.1    TIP: B-5766    COUNTY: STOKES    GEOLOGIST: C. Stratton

SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River    GROUND WTR (ft) 0 HR. 31.3

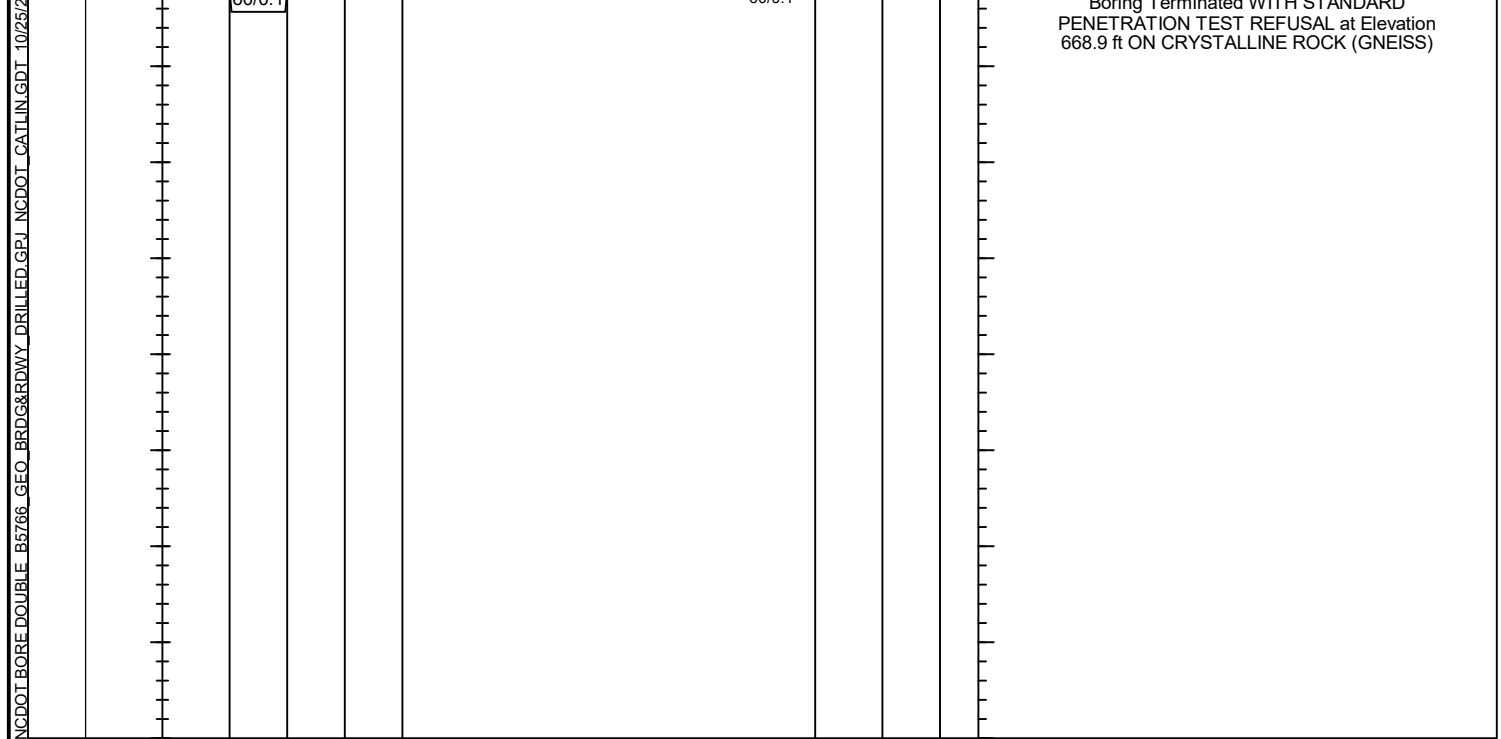
BORING NO.: EB2-A    STATION: 16+79    OFFSET: 23 ft LT    ALIGNMENT: -L-    24 HR. FIAD

COLLAR ELEV.: 706.3 ft    TOTAL DEPTH: 37.4 ft    NORTHING: 969,525    EASTING: 1,646,973

DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22    DRILL METHOD: H.S. AUGERS    HAMMER TYPE: AUTOMATIC

DRILLER: Shawn Pugh    START DATE: 10/10/23    COMP. DATE: 10/10/23    SURFACE WATER DEPTH: N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
710														
705	704.4	1.9											LAND SURFACE	0.0
	702.8	3.5	2	2	1								ROADWAY EMBANKMENT PAVEMENT	0.9
	702.8	3.5	2	3	3								ABC BASE STONE	1.4
700													REDDISH BROWN, F. SANDY AND SILTY CLAY	4.5
	697.8	8.5	2	2	2								REDDISH BROWN, SILTY, F. SAND WITH CLAY	
695														
	692.8	13.5	1	1	1								REDDISH BROWN, F. SANDY AND SILTY CLAY. MICACEOUS	12.0
690														
	687.8	18.5	1	3	3								ALLUVIAL BROWN, BEIGE, AND TAN, SILTY, F. SAND. MICACEOUS	19.1
685														
	682.8	23.5	1	2	1								RESIDUAL BROWN TO DARK BROWN, BEIGE, AND TAN, SILTY, F. AND CSE. SAND WITH CLAY AND WEATHERED ROCK FRAGS.	26.0
680														
	677.8	28.5	9	13	11								WEATHERED ROCK (GNEISS)	32.0
675														
	672.8	33.5	35	65/0.3										
670														
	669.0	37.3	60/0.1										Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 668.9 ft ON CRYSTALLINE ROCK (GNEISS)	37.4



WBS: 45722.1.1    TIP: B-5766    COUNTY: STOKES    GEOLOGIST: T. Park

SITE DESCRIPTION: Replace Bridge No. 82 on SR 1674 over Dan River    GROUND WTR (ft) 0 HR. Dry

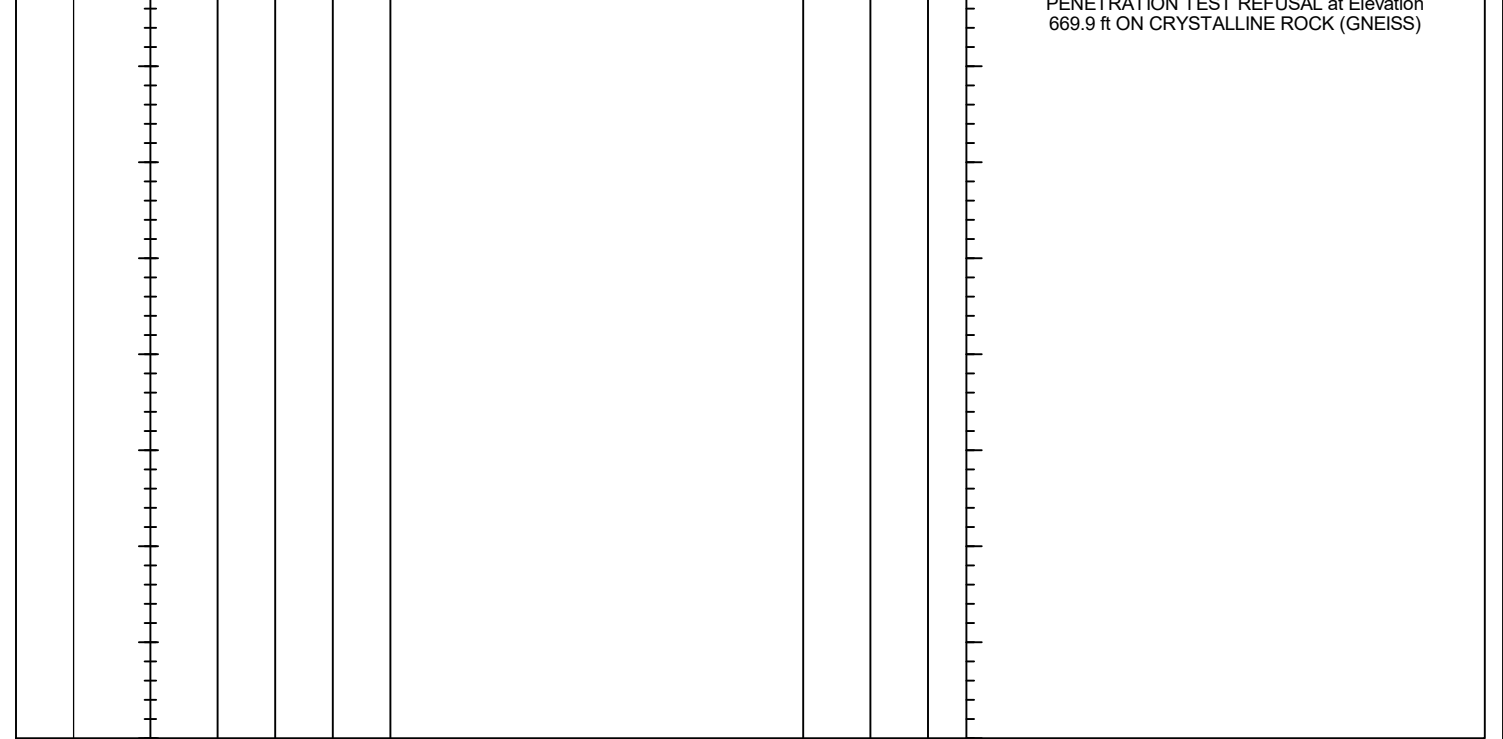
BORING NO.: EB2-B    STATION: 16+84    OFFSET: 31 ft RT    ALIGNMENT: -L-    24 HR. 8.3

COLLAR ELEV.: 688.2 ft    TOTAL DEPTH: 18.3 ft    NORTHING: 969,472    EASTING: 1,646,986

DRILL RIG/HAMMER EFF./DATE: CAT2002 MOBILE B-57 92.3% 12/23/22    DRILL METHOD: H.S. AUGERS    HAMMER TYPE: AUTOMATIC

DRILLER: Shawn Pugh    START DATE: 10/03/23    COMP. DATE: 10/03/23    SURFACE WATER DEPTH: N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. # RESULT	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
710														
705														
700														
695														
690														
685	688.2	0.0	3	4	5						SS-03 A-4(0)	10% D	ALLUVIAL BROWN, F. AND CSE. SANDY SILT	2.5
	684.7	3.5	2	2	2						SS-04 A-2-4(0)	4% D	BROWN, SILTY, F. AND CSE. SAND	6.0
680													RESIDUAL BROWN, GRAVELLY, F. SAND. ANGULAR GRAVEL	6.0
	679.7	8.5	16	42	15								TAN, GRAVELLY F. SAND	12.0
675														
	674.7	13.5	15	20	60									
670														
	670.0	18.2	60/0.1										Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 669.9 ft ON CRYSTALLINE ROCK (GNEISS)	18.3



NCDOT BORE DOUBLE: B5766\_GEO\_BRDG&RDWY\_DRILLED.GPJ NCDOT\_CATLIN.GDT 10/25/23



# LABORATORY SUMMARY SHEET

## AASHTO Standard Specifications

(As modified by NCDOT, Material and Tests Unit, 2000.)

### TEST RESULTS

Proj. Sample Number	SS-15	SS-16	SS-01	SS-02	SS-03	SS-04	SS-11	SS-12	SS-13	SS-14					
Lab Sample Number	SS-15	SS-16	SS-01	SS-02	SS-03	SS-04	SS-11	SS-12	SS-13	SS-14					
Retained #4 Sieve %	3.4	0	0.3	1.7	5.2	0	0.6	0	0	3.0					
Passing #10 Sieve %	96.3	99.9	99.6	98.2	94	99.9	99.3	100	99.9	91.7					
Passing #40 Sieve %	95	99	98	95	88	95	98	99	97	82					
Passing #200 Sieve %	62	71	74	44	41	12	67	58	44	18					
<b>MINUS NUMBER 10 FRACTION</b>															
<b>SOIL MORTAR - 100%</b>															
Coarse Sand Ret. #60 %	3.5	3.9	4.5	8.6	19.8	29.8	4.3	4.2	12.2	33.0					
Fine Sand Ret. #270 %	32.9	32.3	28.4	59.2	45.3	61.2	36.8	38.0	52.2	50.9					
Silt 0.05 - 0.005mm %	40.1	25.2	36.1	19.5	22.8	6.2	38.2	36.6	22.4	11.0					
Clay <0.005mm %	23.5	38.6	31.0	12.7	12.1	2.8	20.7	21.2	13.1	5.1					
Liquid Limit (LL)	32	39	40	NP	26	NP	40	25	31	NP					
Plasticity Index (PI)	9	16	15	NP	3	NP	7	4	4	NP					
AASHTO Classification /Group Index	<b>A-4(4)</b>	<b>A-6(11)</b>	<b>A-6(11)</b>	<b>A-4(0)</b>	<b>A-4(0)</b>	<b>A-2-4(0)</b>	<b>A-4(5)</b>	<b>A-4(0)</b>	<b>A-4(0)</b>	<b>A-2-4(0)</b>					
Organic Content %	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A					
Station	13+09	13+09	13+03	13+03	16+84	16+84	13+08	13+08	16+86	16+86					
Offset	23ft LT	23ft LT	30ft RT	30ft RT	31ft RT	31ft RT	54ft RT	54ft RT	58ft RT	58ft RT					
Alignment	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-					
Boring Identification	<b>EB1-A</b>	<b>EB1-A</b>	<b>EB1-B</b>	<b>EB1-B</b>	<b>EB2-B</b>	<b>EB2-B</b>	<b>W-1</b>	<b>W-1</b>	<b>W-2</b>	<b>W-2</b>					
Depth (FT)	18.5	23.5	0.0	3.5	0.0	3.5	0.0	3.5	0.0	4.0					
to	20.0	25.0	1.5	5.0	1.5	5.0	1.5	5.0	1.5	5.0					
Field Moist. Content %	25	24	25	12	10	4	28	13	22	6					
Tested By	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON					
Submitted By	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON	SVHUDSON					
Date Submitted	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23	10/09/23					

NP = Non-Plastic  
NEM = Not Enough Material for Analysis  
N/A = Not Applicable / Not Analyzed

*Michael D. Mason*  
Laboratory Manager

Report Date: 10/24/2023  
Laboratory Report Page 1 of 1

# SITE PHOTOGRAPHS



NORTH OF BRIDGE - EAST OF RIVER  
FACING DOWN STATION LEFT OF -L-



SOUTH OF BRIDGE - WEST OF RIVER  
FACING UP STATION



SOUTH OF BRIDGE - WEST OF RIVER  
FACING UP STATION



SOUTH OF BRIDGE - EAST OF RIVER  
FACING DOWN STATION