	SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION CONTENTS LINE STATION -L- 10+00 to 21+35	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT					
REFERENCE: B-5766	CROSS SECTIONSLineSTATION-L-20+20 and 20+507GEOTECHNICAL BORING REPORTSSHEETS8 - 9	SUBSURFACE INVESTIGATION SUBSURFACE INVESTIGATION COUNTY STOKES PROJECT DESCRIPTION BRIDGE NUMBER & OVER DAN RIVER ON SR 1674 (SHEPPARD MILL ROAD) DANBURY, NORTH CAROLINA					
PROJECT: 45722							

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5766	1	9

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (99) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAIL

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UNI-FLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOL MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE VIBSURFACE PLANS. THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS TO SE ENVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS TO REVENTION OF FOR AN EXITENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL COMPENSATION. OF FOR AN THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES: I, THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR CUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REDUCETED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

T. PARK
C. STRATTON
T.J. WHITE, CWC
S. PUGH, CWC
INVESTIGATED BYCATLIN
DRAWN BY S.V. HUDSON, PG
CHECKED BY J. LEE STONE, PG
SUBMITTED BY S.V. HUDSON, PG
DATE JANUARY 2024





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

			SOIL	DESCF	RIPTIO	<u>۱</u>						GR	ADATION							ROC	K DES	RIPTION
SOIL IS C BE PENETE ACCORDIN IS BA	CONSIDERED RATED WITH NG TO THE ASED ON TH	UNCONSOLIO A CONTINU STANDARD F	ATED, SEMI-CO DUS FLIGHT P ENETRATION T YSTEM, BASIC	INSOLIDA DWER AU EST (AAS DESCRIP	TED, OR WI GER AND ' GHTO T 20 TIONS OF	ATHERED	EARTH MAT THAN 100 586). SOIL	ERIALS TH BLOWS PE CLASSIFIC E FOLLOWIN	AT CAN R FOOT CATION NG:	WELL GRADED - INDICAT UNIFORMLY GRADED - IN GAP-GRADED - INDICATE	ES A GOO IDICATES S A MIXT	OD REPRESEN THAT SOIL F TURE OF UNIF	TATION OF PARTIC PARTICLES ARE AL ORM PARTICLE SI	LE SIZES FF L APPROXIMA ZES OF TWO	ROM FINE TO COARSE. TELY THE SAME SIZE. OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTE ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN Ø.1 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK						
CONSISTEN	NCY, COLOR,	TEXTURE, MI	ISTURE, AASH	O CLASS	IFICATION	, AND OTHE	R PERTINE	NT FACTOR	S SUCH		4	ANGULARI	TY OF GRAI	٧S		REPRESENTED	BY A	ZONE OF	JF WEAT	HERED ROC	K. FOLLOWS:	
V	ERY STIFF.G	RAY, SILTY CLA	MOIST WITH IN	ITERBEDD	ED FINE S	AND LAYERS	HIGHLY PLA	STIC.A-7-6		THE ANGULARITY	Y OR ROU	UNDNESS OF :	SOIL GRAINS IS DI	ESIGNATED B	Y THE TERMS:	WEATHERED		V/S	1 775	NON-COAST	TAL PLAIN	MATERIAL THAT WOULD YIELD SF
	S	DIL LEC	END AND	AASH	TO CL	ASSIFI	CATION				MIN	FRAL OGI	CAL COMPOSI			ROCK (WR)				100 BLOWS	PER FOO	IF TESTED.
GENERAL CLASS.	(Granular mat ≤ 35% passini	=RIALS = 200)	SII (>	_T-CLAY MA 35% Passin	ERIALS IG =200)	ORC	GANIC MATERI	ALS	MINERAL NAM	MES SUCH	AS QUARTZ,	FELDSPAR, MICA, T	ALC, KAOLIN,	ETC.	CRYSTALLINE	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPH			IN IGNEOUS AND METAMORPHIC R EFUSAL IF TESTED, ROCK TYPE I		
GROUP	A-1	A-3	A-2	A-4	A-5 A	-6 A-7	A-1, A-2	A-4, A-5		ARE USED IN	DESCRIP	PTIONS WHEN	THEY ARE CONSID	ERED OF SIC	NIFICANCE.	ROCK (CR)			<u>Lin</u>	GNEISS, GA	BBRO, SCHI	ST.ETC. AIN METAMORPHIC AND NON-COAS!
CLASS. A	1-1-a A-1-b	A-2-4	A-2-5 A-2-6 A-	2-7		A-7-5 A-7-6	A-3	A-6, A-7					ESSIBILITY	11 2 31		ROCK (NCR)	INE	==		SEDIMENTA	RY ROCK	HAT WOULD YEILD SPT REFUSAL
SYMBOL 80				2	17.1					MODE	RATELY C		E	LL = 31 -	50	COASTAL PLAI	N	170		COASTAL F	LAIN SEDI	MENTS CEMENTED INTO ROCK, BUT
% PASSING	awy							SILT-	MUCK	HIGHL						CP)	RULK			SHELL BEC	JS, ETC.	TYPE INCLUDES LIMESTONE, SAND
*40 30	0 MX 50 MX	51 MN					SOILS	CLAY SOILS	PEAT			GRANULAR	SILT - CLAY			1					WEATHE	RING
*200 IS	5 MX 25 MX	10 MX 35 MX	35 MX 35 MX 35	MX 36 MP	1 36 MN 36	MN 36 MN				- DRGANIC MATERIAL TRACE OF ORGANIC M	ATTER	<u>501LS</u> 2 - 3%	<u>SOILS</u> 3 - 5%	TRACE	1 - 10%	FRESH F	ROCK I	FRESH, CF	RYSTALS	S BRIGHT, FF	EW JOINTS	MAY SHOW SLIGHT STAINING. ROCK
PASSING 40							SOILS	WITH		LITTLE ORGANIC MATT	/ER	3 - 5%	5 - 12%	LITTLE	10 - 20%	VERY SLIGHT	ROCK	GENERAL	LY FRE	SH. JOINTS	STAINED, S	ME JOINTS MAY SHOW THIN CLAY
LL PI	- 6 MX	- 40 MX NP 10 MX	41 MN 40 MX 41 10 MX 11 MN 11	MN 40 MX MN 10 MX	(41 MN 40 (10 MX 11	MX 41 MN MN 11 MN	LITTL	E OR	HIGHLY	HIGHLY ORGANIC		> 10%	> 20%	HIGHLY	35% AND ABOVE	(V SLI.)	CRYST	ALS ON A	A BROKE	EN SPECIME	N FACE SH	NE BRIGHTLY. ROCK RINGS UNDER
GROUP INDEX	0	0 0	4 MX	8 MX	12 MX 16	MX NO MX	AMOUN	ITS OF	ORGANIC			GROU	ND WATER			SLIGHT	ROCK	GENERAL	LY FRE	SH. JOINTS	STAINED AF	ID DISCOLORATION EXTENDS INTO R
USUAL TYPES ST	TONE FRAGS.	FINE SI	TY OR CLAYEY	S	LTY	CLAYEY	ORG4 MAT	ANIC TER	50125	∇	WATER	LEVEL IN B	ORE HOLE IMMEDIA	TELY AFTER	DRILLING	(SLI.)	1 INCH. CRYST	. OPEN J	JOINTS I	MAY CONTAI	N CLAY. IN ORED. CRYS	GRANITOID ROCKS SOME OCCASION
of Major G Materials	SAVEL, AND	SAND GF	avel and sand	SI	DILS	SOILS				▼	STATIC	WATER LEV	EL AFTER 24	HOURS		MODERATE	SIGNIF	ICANT P	ORTIONS	S OF ROCK	SHOW DISC	LORATION AND WEATHERING EFFEC
GEN. RATING		EXCELLENT TO	6000			OUB	FAIR TO	POOR		√Р₩	PERCHE	ED WATER, SA	TURATED ZONE, OR	WATER BEAK	RING STRATA	(MOD.)	GRANIT	TOID ROC	INDER H	T FELDSPAF	RS ARE DUL	L AND DISCOLORED, SOME SHOW CL
AS SUBGRADE							POOR	10011	ONSOLUBEL		SPRING	G OR SEEP					WITH F	FRESH R	OCK.			
		21 OF A-7-5 SI	BGROUP IS ≤ LI	30 ; PI		FNESS	> LL - 30				N			א ה		MODERATELY (ALL R		EPT QU	ARTZ DISCO	LORED OR S	STAINED, IN GRANITOID ROCKS, ALL
		COMBA		RAI	NGE OF ST	ANDARD	RANG	E OF UNC	ONFINED				-			(MOD. SEV.)	AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND					
PRIMARY SC	DIL TYPE	COMPA	ISTENCY	PENE	TRATION R (N-VALU	SISTENCE	COMPR	RESSIVE S (TONS/FT	TRENGTH	L ROADWAY EMBA	ANKMENT	(RE) 25/62	DIP & DIP DIR OF ROCK STRU	ECTION CTURES		SEVERE	IF IES	STED, WOL	<u>ULU YIE</u>	ARTZ DISCO	USAL	STAINED BOCK EARDIC CLEAR AND
CENERALI		VER	LOOSE		< 4										SLOPE INDICATOR	(SEV.)	REDUC	ED IN ST	TRENGTH	H TO STRON	G SOIL. IN	GRANITOID ROCKS ALL FELDSPARS
GRANULAR	R		DOSE M DENSE		4 TO 10 TO	0 30		N/A					VST PMT		INSTALLATION		TO SOI <u>IF TE</u>	ME EXTER	INT. SOM DULD YIE	1E FRAGMEN <u>ELD SPT N</u>	TS OF STR	JNG ROCK USUALLY REMAIN. 00 BPF
MATERIAL (NON-COH	L IESIVE)		ENSE		30 TO	50				THAN ROADWAY	LL (AF)U Y EMBANK		AUGER BORING	۵	TEST	VERY	ALL R	OCK EXC	EPT QU	ARTZ DISCO	LORED OR (STAINED. ROCK FABRIC ELEMENTS A
		VER			> 50		-	(0 25			L BOUNDA		CORE BORING	•	SOUNDING ROD	SEVERE (V SEV.)	BUT M REMAII	NING, SA	EFFECTI PROLITE	IVELY REDUC	JED TO SOI AMPLE OF F	L STATUS, WITH ONLY FRAGMENTS (ROCK WEATHERED TO A DEGREE THA
GENERALI	LY		OFT		2 TO	4		Ø.25 TO 0	0.5			MWO			TEST BORING	,	VESTIC	JES OF C	ORIGINAL	L ROCK FAB	RIC REMAIN	I. IF TESTED, WOULD YIELD SPT N
MATERIAL	ar L	MEDI	M SIIFF TIFF		4 TU 8 TO	3 15		0.5 TO 1 1 TO 2	.0	INFERRED RUC	K LINE	0	MUNITURING WE	··· 🌱	WITH CORE	COMPLETE	ROCK I	REDUCED	I TO SOI	IL. ROCK FA	ABRIC NOT I ARTZ MAY F	JISCERNIBLE, OR DISCERNIBLE ONLY BE PRESENT AS DIKES OR STRINGEF
(COHESIV	E)	VER	STIFF		15 TO > 30	30		2 TO 4		ALLUVIAL SOI	L BOUNDA	ary 🛆	INSTALLATION	\bigcirc	- SPT N-VALUE	<u>′</u>	ALSO (AN EXAM	IPLE.			
		1	TEXTURE	OR G	RAIN	SIZE					RE	COMMENC	ATION SYMB	OLS		1					JCK HAP	<u>IDNESS</u>
U.S. STD. SIEV	VE SIZE		4 10	4(9 60	200	270					ASSIFIED EX	CAVATION -	*** UNCLAS	SIFIED EXCAVATION -	VERY HARD	SEVER	T BE SCH AL HARD	.RATCHED) BLOWS	D BY KNIFE	OR SHARP	PICK. BREAKING OF HAND SPECIMEN PICK.
OPENING (MM))		4.76 2.0	0 0.4	2 0.2	5 0.075	0.053					ASSIFIED FX	E L	ACCEPT	ABLE,BUT NOT TO BE N THE TOP 3 FEET OF	HARD	CAN B	E SCRAT	CHED B	Y KNIFE OR	PICK ONLY	WITH DIFFICULTY. HARD HAMMER
BOULDER	co	BLE	GRAVEL	COAF SAI	RSE	F INE SAND	9	SILT	CLAY			PTABLE DEGR	RADABLE ROCK	EMBANK	MENT OR BACKFILL		TO DE	TACH HAT	IND SPEU	UMEN.		GES OR GROOVES TO 0 25 INCHES (
(BLUR.)		08.)	(GR.)	(CSE.	SD.)	(F SD.		SL./	(CL.)			ABBR	EVIATIONS			HARD	EXCAV	ATED BY	HARD F	BLOW OF A	GEOLOGIST	'S PICK. HAND SPECIMENS CAN BE
GRAIN MM SIZE IN.	305 12	75 3	2.0)	0.2	5	0.05	0.005		AR - AUGER REFUSAL BT - BORING TERMINATED	J	MED N MICA	MEDIUM MICACEOUS	VST - WEA. ·	VANE SHEAR TEST WEATHERED	MEDIUM	CAN R	UERATE I	BLUWS.	GOUGED 0.0"	5 INCHES C	FEP BY FIRM PRESSURE OF KNIFF
			STURF -	COBB			TERMS			- CL CLAY		MOD N	MODERATELY	2-	JNIT WEIGHT	HARD	CAN B	E EXCAV	ATED IN	N SMALL CH	IPS TO PEI	CES 1 INCH MAXIMUM SIZE BY HARE
SOIL N	MOISTURE	SCALE	FIELD	IOISTURE					CRIPTION	CSE COARSE	I IESI	ORG C	DRGANIC	/d- 1	JRY UNIT WEIGHT	SOFT	CAN B	UF A GE	D OR G	OUGED READ	ILY BY KN	FF OR PICK, CAN BE EXCAVATED 1
(ATTE	ERBERG LI	AITS)	DESC	RIPTION	00		TELD MOIS	STURE DES	CRIFTION	DMT - DILATOMETER TES	T TION TES	PMT - F T SAP 9	PRESSUREMETER TE	EST <u>SA</u>	MPLE ABBREVIATIONS		FROM	CHIPS TO	0 SEVEF	RAL INCHES	IN SIZE B	Y MODERATE BLOWS OF A PICK POI
			- SATU	RATED -	US	UALLY LIC	UID: VERY	WET, USUA	ALLY	e - VOID RATIO		SD S4	AND, SANDY	ss -	SPLIT SPOON	VERY	CAN B	E CARVE	E BRUKE	KNIFE, CAN	A PRESSUR	L. ATED READILY WITH POINT OF PICK
ᄔᆮ		LIMIT		.,		JM BELUW	THE GRO	UND WHILE	RIHBLE	- FOSS FOSSILIFEROUS		SL SI SLI S	LI, SILIY LIGHTLY	ST - RS -	SHELBY TUBE ROCK	SOFT	OR MO	RE IN TH	HICKNES	S CAN BE E	3ROKEN BY	FINGER PRESSURE. CAN BE SCRATC
PLASTIC RANGE <			- WET	- (W)	SE	MISOLID: F	EQUIRES D	DRYING TO		FRAC FRACTURED, FRAC	TURES	TCR - 1	RICONE REFUSAL	RT -	RECOMPACTED TRIAXIAL		PINOEP	TURE	SPAC		<u> </u>	BEDDING
(PI) PL	PLASTI	C LIMIT			AI		MUM MUIS	TURE		HI HIGHLY		V - VEF	RY	CDN -	RATIO	TERM			<u></u>	SPACING		
			- MOIST	- (M)	SC	LID: AT OF	NEAR OP	тімим мо	ISTURE	EOI	<u>UIPMEN</u>	NT USED	ON SUBJECT	PROJEC	T	VERY WIDE			MORE 1	THAN 10 FE	ET.	VERY THICKLY BEDDED
OM _ SL _	L OPTIMU	M MOISTURE AGE LIMIT								DRILL UNITS:	ADVANC	CING TOOLS:		HAMMER	IYPE:	MODERATEL	Y CLC	JSE	1 T	0 3 FEET		THINLY BEDDED 0
				- (D)	RE	OUIRES A	DITIONAL	WATER TO)	CME-45C		LAY BITS			OMATIC MANUAL	VERY CLOS	Æ	I	0.16 LESS TI	TO 1 FOOT HAN 0.16 F	EET	THICKLY LAMINATED 0.
					AT	TAIN OPTI	MUM MOIS	TURE		CME-55			FLIGHT AUGER	CORE SIZ	E:							THINLY LAMINATED
			ΡL	ASTIC	ITY						₿."	ARD EACED T	INCER BITS	│ └── ⁻ ^в ─	∐""	FOR SEDIMENT						
	PLASTICITY INDEX (PI) DRY STRENGTH					TH				INCEDIC	X -N Q		FOR SEDIMENT	ent H	JULKS, IN	JOUNHII	RUBBIN	IG WITH F!	NGER FREES NUMEROUS GRAINS:			
SLIG	HTLY PLAS	TIC		6-15	-			SLIGHT		VANE SHEAR TEST		ASING V	W/ ADVANCER	HAND TOO	DLS:	FRIABLE	-			GENTLE	E BLOW BY	HAMMER DISINTEGRATES SAMPLE
MODE HIGHL	LY PLASTI	_ASTIC C		16-25 26 OR M	o IORE			HIGH					STEEL TEETH		T HOLE DIGGER	MODERA	TELY	INDURAT	TED	GRAINS	CAN BE	SEPARATED FROM SAMPLE WITH S
				COLO	R							RICONE 2	%" TUNGCARB		D AUGER					GRAINS	ARE DIFF	ICULT TO SEPARATE WITH STFFI
DESCRIPTI						TAN RED			-GRAY)			ORE BIT	<u>/u</u>		NUING KUU F SHEAR TEST	INDURAT	ED			DIFFIC	ULT TO BF	EAK WITH HAMMER.
мор	MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.								10.					EXTREM	ELY I	.NDURATE	ED	SHARP SAMPLI	HAMMER B E BREAKS	LOWS REQUIRED TO BREAK SAMPL ACROSS GRAINS.		
-																						

PROJECT REFERENCE NO.



2

IERMS AND DEFINITIONS
ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ADUIFER - A WATER BEARING FORMATION OR STRATA.
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
ARGILLACEDUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION. SUCH AS SHALE. SLATE. ETC.
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
DURENCE.
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT ROTTOM
OF SLOPE.
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
$\underline{\text{DIKE}}$ - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
$\underline{\text{DIP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
FIELD.
JUINI - FRACIURE IN RUCK ALUNG WHICH NU APPRECIABLE MOVEMENT HAS OCCURRED.
ITS LATERAL EXTENT.
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
OF AN INTERVENING IMPERVIOUS STRATUM.
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
HOUK UUALITY DESIGNATION (HUD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
$\underline{\text{SLICKENSIDE}}$ - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF)OF A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOLL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEQMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EVERGESCH OG A DEPENDACE
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
OF STRUCTURE BORINGS OBTAINED WITH RTK GPS; ROADWAY BORINGS
OBTAINED FROM 22105_Ls_tin.tin. ELEVATION: NGVD 88 US FT
NOTES:
INVESTIGATION COMPLETED ON PRELIMINARY DESIGN FILES PROVIDED BY
NUDUT UN MAY 26, 2023
FIAU = FILLED IMMEDIATELY AFTER URILLING
NEM = NOT ENOUGH MATERIAL FOR FULL ANALYSIS



STATE	8	TATE PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS			
N.C.		3	7					
8TAT	E PROJ.NO.	F. A. PROJ. NO.		DESCRIPTION				
45	722.1.3	N⁄A						
45	722.2.1	N⁄A		R/W				
45	722.2.2	N⁄A		UTIL				
45	722.3.1	N⁄A		CONST.				
	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED							



December 2023

WBS Number: TIP Number: Project ID: County: Description:	45722.1.1 B-5766 42290 STOKES Bridge Number 82 over Dan River on SR 1674 (Sheppard Mill Road) Danbury, North Carolina
CATLIN Number:	223152
SUBJECT:	Roadway Subsurface Inventory Report

Project Description

This project is located on Sheppard Mill Road immediately east of Danbury in Stokes County, North Carolina. Approximately one-half of the project is located within Moratock Park which was added to the National Register of Historic Places in 1974. The proposed project consists of improvement of SR 1674 (Sheppard Mill Road) along with the replacement of existing bridge Number 82 over the Dan River. This geotechnical investigation was conducted by Richard Catlin and Associates (DBE Catlin Engineers and Scientists – (CATLIN)) and was confined to the areas of proposed construction.

Fieldwork was conducted by CATLIN personnel in October 2023. Standard Penetration Test (SPT) borings were completed along the project corridor with an average distance of approximately 150 linear feet between borings. Additional SPT borings advanced during the associated Structure Inventory were utilized to augment the roadway data. Representative soil samples were collected for visual classification in the field and for laboratory analysis.

The following alignment was investigated. Plan sheets and subsurface profiles are included in this report and were generated from files received from NCDOT on May 26, 2023.

<u>Line</u>	<u>Station (±)</u>
-L-	10+00 to 21+35

Areas of Special Geotechnical Interest

1) Seasonal high groundwater was encountered at the following location:

Line	<u>Station (±)</u>
-L-	12+00

2) Cohesive soils that may have the potential to cause embankment/subgrade and or slope stability problems during construction were identified at the following locations:

Line	<u>Station (±)</u>
-L-	11+00 to 13+50
-L-	15+40 to 17+25

Physiography and Geology

This project area is located within the North Carolina Inner Piedmont physiographic province. The North Carolina Piedmont is typically characterized by gently rolling, well-rounded hills with a few hundred feet of relief between the hills and valleys. According to the 1985 North Carolina Geologic Map, the area of investigation lies within the Chauga Belt with the predominant rock type being metagraywacke (biotite gneiss) with muscovite-biotite schist interlayered and gradational throughout. Land use along the project area consists primarily of recreational with some residential housing and small business.

Groundwater

Groundwater data was collected in October 2023. Ground water was encountered from within 2.1 feet to greater than 36 feet from the ground surface along the area of investigation. The project spans the Dan River which as reported in the Bridge Survey & Hydraulic Design Report (BSR) has a normal water surface elevation (NWSEL) of 677.3 feet. Mill Creek crosses highway 89 terminating in the Dan River approximately 100 feet south of the project site.

Soils

Soils encountered along the project site include Roadway Embankment, Artificial Fill, Alluvial, Residual, Weathered Rock, and Crystalline Roack.

- expected to be comprised of materials similar to those used for Roadway Embankment.
- encountered along the eastern bank of the Dan River.
- east of the Dan River.
- River. No rock core was collected during the Roadway Investigation.

 Roadway Embankment soils exist beneath and adjacent to existing -L- (Sheppard Mill Road) and consist of approximately three (3) to 20 feet of material. The embankment soils are comprised of soft to stiff, fine grained sandy clay and sandy silt (A-4, A-6) and very loose to loose, fine grained silty sands (A-2-4). • Artificial Fill was identified and inferred along the project corridor adjacent to the Dan River and under the Moratock Park access roads. Although not sampled, the materials under the access roads are

• Alluvial soils were identified across the flood plain to the east of the Dan River and south of Sheppard Mill Road at thickness ranging from six (6) to 12 feet. The soils were predominantly described as very loose to loose, fine grained sands and silty, fine grained sands (A-3, A-2-4) with some well-rounded gravels at the base of the stratum. Medium stiff, clay and fine grained sandy clay (A-6) was

 Residual materials consisting of very soft to hard, fine sandy and clavey silt and fine sandy and silty clay (A-6, A-7, A-4) were encountered beneath the Roadway Embankment on the west side of the Dan River at thicknesses of up to approximately 23 feet. A small amount of loose to very dense sand and clayey sand (A-2-4, A-2-6) was identified beneath the fine material adjacent to the Dan River. Residual materials on the east side of the Dan River were comprised primarily of loose to dense, silty, fine and coarse sand (A-2-4, A-3) with some clay and weathered rock fragments at thickness up to 15 feet.

• Weathered Rock (Gneiss) was identified beneath the Residual soils at elevations ranging from 682 feet 673 feet west of the Dan River and from elevations ranging from approximately 665 feet to 685 feet

 Crystalline Rock (Gneiss) was identified beneath the Weathered Gneiss at elevations ranging from a high of 682 feet near the west side of the Dan River to a low of 668 feet on the east side of the Dan







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GEOTECHNICAL BORING REI HAND AUGER AND DCP	PORT		223152 CATLIN PROJECT REFERENCE SHEET B-5766 8
WBS: 45722.1.1 TIP: B-5766 COUNTY: STOKES	GEOLOGIST: C. Stratton	WBS: 45722.1.1 TIP: B-5766 COUNTY: STO	KES GEOLOGIST: C. Stratton
SITE DESCRIPTION: Bridge Number 82 over Dan River on SR 1674 (Sheppard Mill Road), Danbury, No.	orth Carolna GROUND WTR (ft)	SITE DESCRIPTION: Bridge Number 82 over Dan River on SR 1674 (Sheppard	Mill Road), Danbury, North Carolna GROUND WTR (ft)
BORING NO.: W-3 STATION: 21+50 OFFSET: 21 ft RT	ALIGNMENT: -L- 0 HR. Dry	BORING NO.: W-4 STATION: 21+92 OFFSE	T: 19 ft RT ALIGNMENT: -L- 0 HR. Dry
COLLAR ELEV.: 707.4 ft TOTAL DEPTH: 6.5 ft NORTHING: 969,626	EASTING: 1,647,433 24 HR. FIAD	COLLAR ELEV.: 707.4 ft TOTAL DEPTH: 4.0 ft NORTH	ING: 969,648 EASTING: 1,647,470 24 HR. FIAD
DRILL RIG/HAMMER EFF./DATE: HAND AUGER DRILL METHOD: H.	AND AUGER W/DCP HAMMER TYPE: MANUAL	DRILL RIG/HAMMER EFF./DATE: HAND AUGER	DRILL METHOD: HAND AUGER W/DCP HAMMER TYPE: MANUAL
DRILLER: N/A START DATE: 12/07/23 COMP. DATE: 12/07/23	SURFACE WATER DEPTH: N/A	DRILLER: N/A START DATE: 12/07/23 COMP.	DATE: 12/07/23 SURFACE WATER DEPTH: N/A
ELEV DRIVE DEPTH BLOW COUNT AVERAGE DCP VALUES SAMP. # (ft) 1.75in 1	SOIL AND ROCK DESCRIPTION	ELEV DRIVE DEPTH BLOW COUNT AVERAGE DCP VALUES	
	ELEV. (ft) DEPTH (ft)		
710		710	
	-		
	707.4 LAND SURFACE 0.0 ROADWAY EMBANKMENT		ROADWAY EMBANKMENT
	. RED BROWN, SILTY, F. SAND W/TRACE GRAVEL AT BASE OF STRATUM		. L.O. RED BROWN, SILTY, F. SAND W/SOME L. CLAY. ASPHALT FRAGMENTS AT
	DCP DRIVES:		BOTTOM OF STRATUM.
	DEPTH (ft) 1.75" 1.75" 1.75" AVG. 1.0 2 1 1 1.0		DCP DRIVES:
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	Boring Terminated WITH HAND AUGER REFUSAL at Elevation 700.9 ft ON		
	WEATHERED ROCK (GNEISS)		
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GEOTECHNICAL BORING REPORT HAND AUGER AND DCP

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S	ITE	DESCR	RIPTION	l: Bri	dge Nu	umber 8	32 over Da	n River on S	R 1674 (Sheppard N	lill Road),	Danbury, N	Iorth Caroln	а		GROUN	D WTR (ft)
В	ORI	NG NO.	: W-5	5		S	TATION:	22+27		OFFSET:	20 ft RT		ALIGNM	ENT: -L-		0 HR.	Dry
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