

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

DESIGN FILL = 11.00 FT.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

FOR POURING SEQUENCE OF CONCRETE IN CULVERT, SEE "STAGING DETAILS", SHEET C1-2.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF THE EXTERIOR WALLS AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR

THE REINFORCED CONCRETE BOX CULVERT SHALL BE PLACED ON THE STANDARD 1.0 FT. BLANKET OF FOUNDATION CONDITIONING MATERIAL. SEE SECTION 414 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.

FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

TRAFFIC ON NC 45 SHALL BE MAINTAINED. IN ORDER TO MAINTAIN TRAFFIC THE CULVERT SHALL BE CONSTRUCTED IN SECTIONS AS SHOWN ON THESE PLANS AS DIRECTED BY THE ENGINEER

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF 4 @ 72" RCP WITH HEAD-WALL AND WINGWALLS; 106' ALONG CENTERLINE OF CULVERT WITH 22' CLEAR ROADWAY AND LOCATED AT THE SITE SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE STRUCTURE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

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FINAL UNLESS ALL

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FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

SEE SECTION 414 OF THE STANDARD SPECIFICATIONS FOR CULVERT EXCAVATION AND BACKFILLING.

EXCAVATE 1 FOOT BELOW CULVERT AND FOOTING, AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH ARTICLE 414-4 OF THE STANDARD SPECIFICATIONS.

CULVERT BARREL SHOULD BE BACK FILLED WITH NATIVE MATERIAL TO BURY DEPTH (1.0'). NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED AT THE PROJECT SITE DURING CULVERT CONSTRUCTION. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

ALL REINFORCING STEEL AND BAR SUPPORTS SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOLLOWING CONSTRUCTION OF PHASE I, AND PRIOR TO BACKFILLING, THE CONTRACTOR SHALL TAKE MEASURES TO PROTECT ALL REINFORCING STEEL EXTENDING FROM PHASE I INTO PHASE IV WHEN BACKFILLING AND RE-EXCAVATING. ANY REINFORCING STEEL DAMAGED DURING THESE OPERATIONS SHALL BE REPAIRED OR REPLACED AT THE DISCRETION OF THE ENGINEER. ALL REINFORCING STEEL REMEDIATION IS SUBJECT TO REVIEW AND APPROVAL OF THE ENGINEER.

HYDRAULIC DATA

DESIGN DISCHARGE ----- 965 CFS FREQUENCY OF DESIGN FLOOD ----- 50 YR. DESIGN HIGH WATER ELEVATION ----- 11.8 DRAINAGE AREA ------ 6.16 SO. MI. BASE DISCHARGE (Q100) ----- 1170 CFS BASE HIGH WATER ELEVATION ----- 12.6

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE ----- 6400 CFS FREQUENCY OF OVERTOPPING FLOOD --- >500 YR. OVERTOPPING FLOOD ELEVATION ----- 22.38* * OVERTOPPING WILL OCCUR AT STA. 398+87.50 -L-

TOTAL STRUCTURE QUANTITIES CLASS A CONCRETE PHASE 1 ______ 100.6 C.Y. PHASE 2 ______ 168.0 C.Y. PHASE 3 ______ 231.4 C.Y. PHASE 4 ______ 135.7 C.Y. TOTAL _____ 635.7 C.Y. **EPOXY COATED REINFORCING STEEL** PHASE 1 ______ 18,134 LBS. PHASE 2 ______ 31,093 LBS. PHASE 3 ______ 33,395 LBS. PHASE 4 ______ 19,116 LBS. TOTAL _ 101,738 LBS. CULVERT EXCAVATION LUMP SUM FOUNDATION CONDITIONING MATERIAL PHASE 1 ______ 44.0 TONS PHASE 2 ______ 81.0 TONS PHASE 3 ______ 128.0 TONS PHASE 4 ______ 70.0 TONS TOTAL ______ 323.0 TONS

ROADWAY DATA

GRADE POINT EL. @ STA. 398+84.00 -L- = 22.38' INVERT ELEVATION @ STA. 398+84.00 -L- = -0.75' ROADWAY SLOPES 3:1

→ BEGIN CULVERT **►** END CULVERT **←**— -L-48'-3" 55'-9" 67'-3" (PHASE II AND III) 36'-9" (PHASE I AND IV) 19'-0" - STREAM BED **ELEVATION ⊢**1'-3" ►EL. -0.84' - EL. -0.41' EL. -1.00' - TOP OF FLOOR SLAB - EL. -0.75' @ CULVERT PROFILE ALONG & CULVERT

10/14/2024

Raleigh, NC 27601-1772

R-5809A PROJECT NO. __ **BERTIE** COUNTY 398+84.00 -L-STATION: _ SHEET 1 OF 13 BRIDGE NO. 070165

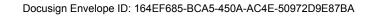
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

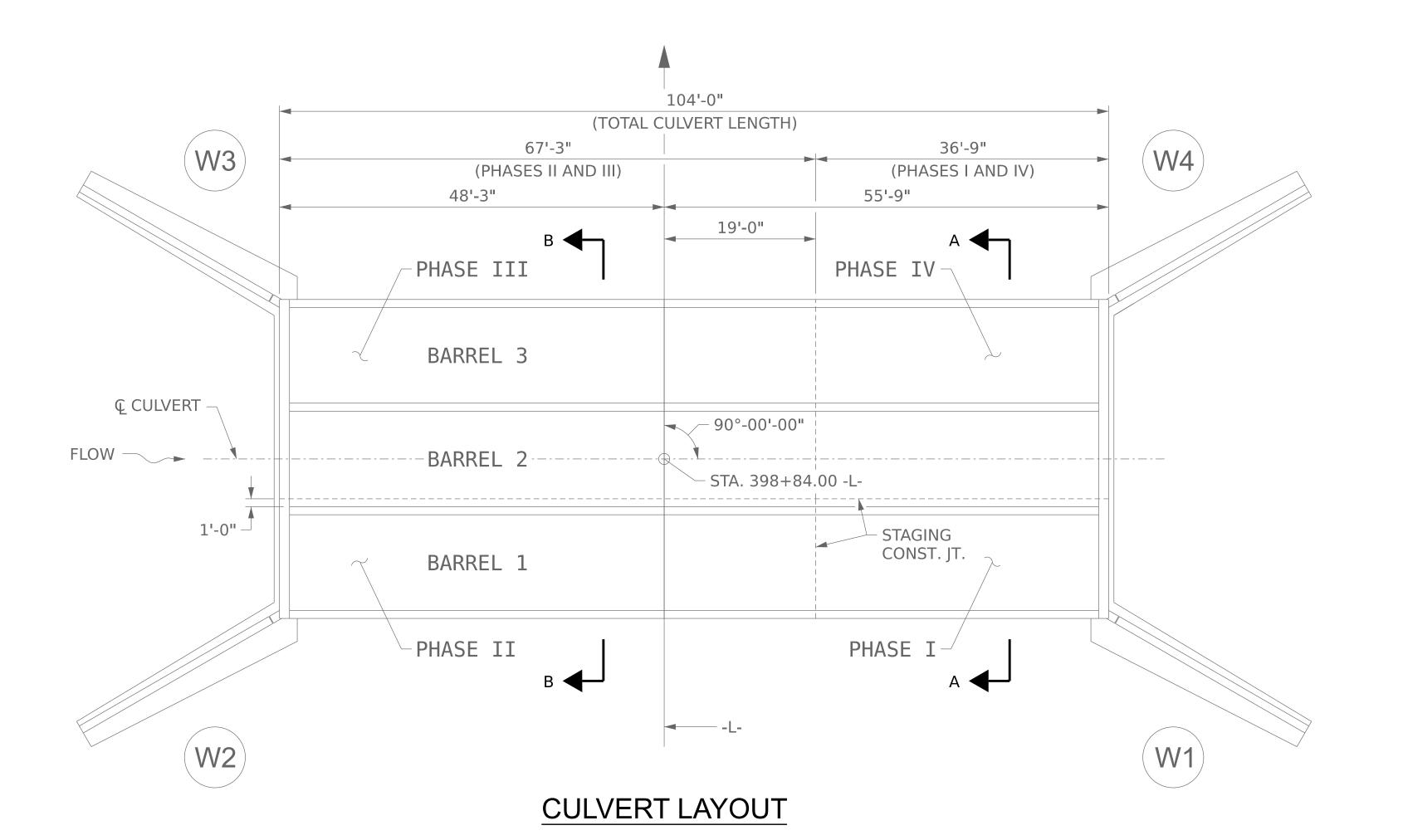
TRIPLE 12 FT. X 11 FT. **CONCRETE BOX CULVERT** 90° SKEW

SHEET NO. REVISIONS C1-1 NO. BY: DATE: DATE: TOTAL SHEETS 13

_ DATE : <u>03/2024</u> D. D. LOWERY DRAWN BY : ___ _ DATE : 03/2024 C.T. POOLE CHECKED BY : _____ DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 03/2024

SYSTIME
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NOTES

CONCRETE IN CULVERT TO BE POURED IN THE FOLLOWING ORDER:

PHASE I

OUTLET WING W1; TOP AND BOTTOM SLAB INCLUDING 1'-0" EXTENSION INTO BARREL 2 AND BOTH EXTERIOR AND INTERIOR WALLS OF BARREL 1. 36'-9" LENGTH OF BARREL 1.

PHASE II

INLET WING W2; TOP AND BOTTOM SLAB INCLUDING 1'-0" EXTENSION INTO BARREL 2 AND BOTH EXTERIOR AND INTERIOR WALLS OF BARREL 1. 67'-3" LENGTH OF BARREL 1.

PHASE III

INLET WING W3; TOP AND BOTTOM SLAB AND BOTH EXTERIOR AND INTERIOR WALLS OF BARRELS 2 AND 3. 67'-3" LENGTH OF BARRELS 2 AND 3.

PHASE IV

INLET WING W4; TOP AND BOTTOM SLAB AND BOTH EXTERIOR AND INTERIOR WALLS OF BARRELS 2 AND 3. 36'-9" LENGTH OF BARRELS 2 AND 3.



SECTION B-B

PHASE III

PHASE II



SECTION A-A

PHASE IV

PHASE I

R-5809A PROJECT NO.__ BERTIE COUNTY 398+84.00 -L-STATION:_



SHEET 2 OF 13

421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
Phone (919) 677-2000

NC LICENSE #
F-0102

TRIPLE 12 FT. X 11 FT. **CONCRETE BOX CULVERT** 90° SKEW

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SHEET NO. REVISIONS NO. BY: DATE: DATE: TOTAL SHEETS 13

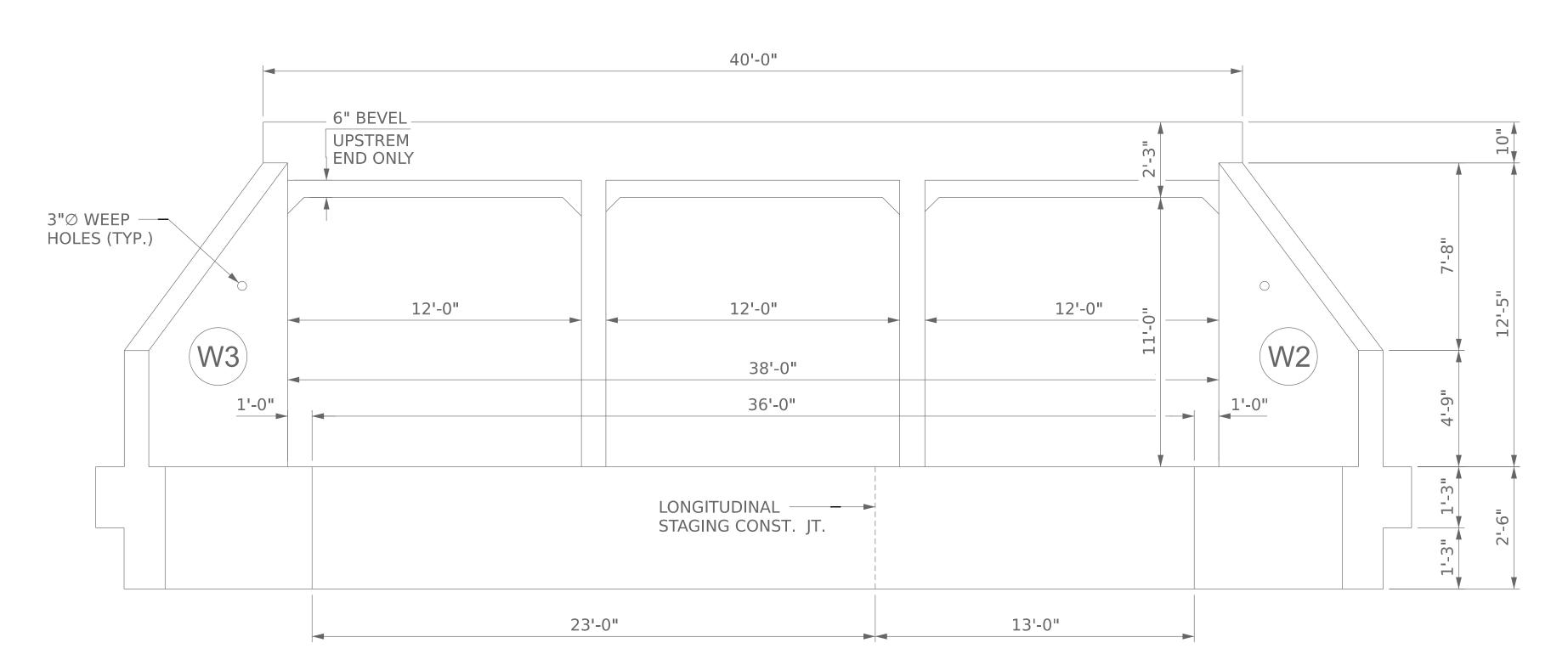
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DATE : 02/2024 DATE : 02/2024

D. D. LOWERY

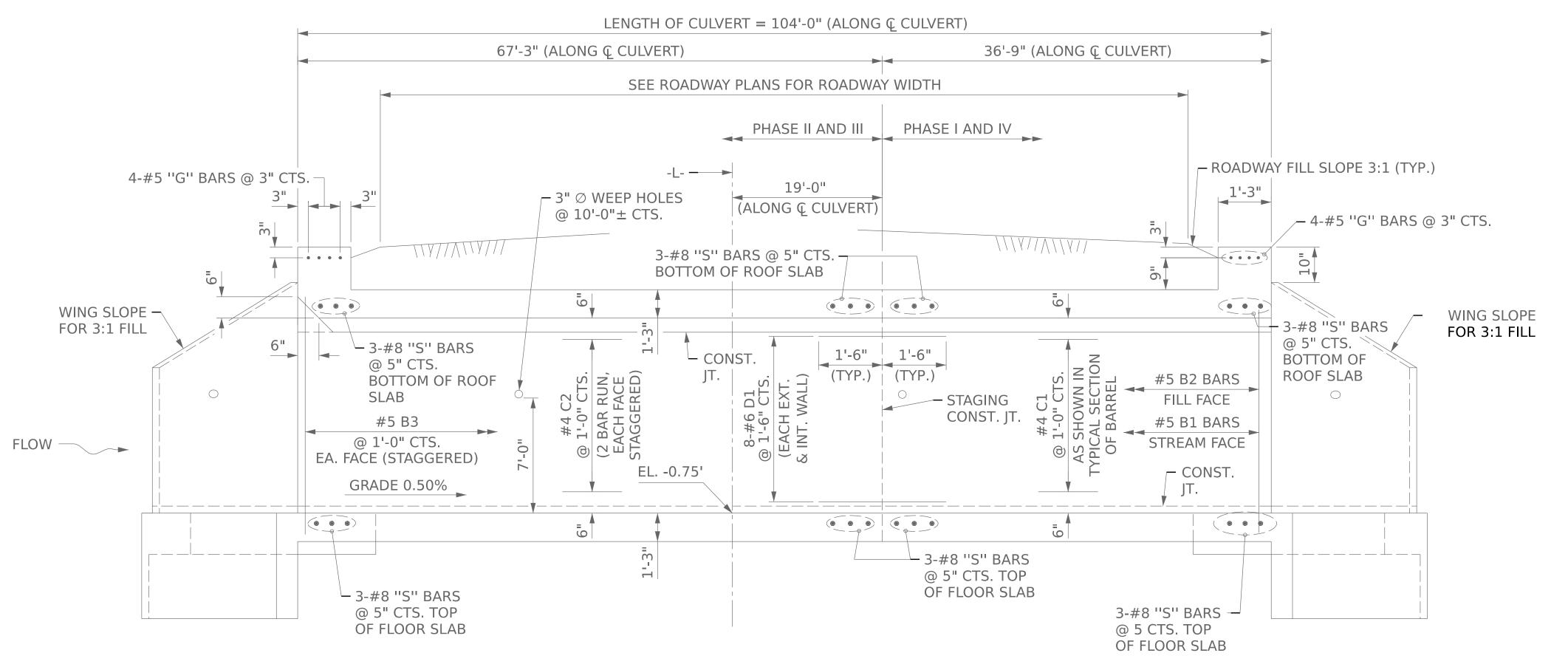
C.T. POOLE DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024

DRAWN BY :



INLET END ELVATION NORMAL TO SKEW

(OUTLET SIMILAR)



INTERIOR WALL

EXTERIOR WALL

CULVERT SECTION NORMAL TO ROADWAY

#6 D1 DOWEL BARS IN THE SLABS NOT SHOWN FOR CLARITY. SEE SHEETS C1-6 & C1-8 FOR SLAB DOWEL DETAILS.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

10/14/2024

421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TRIPLE 12 FT. X 11 FT. CONCRETE BOX CULVERT 90° SKEW DEVICTORS SHEET NO

PROJECT NO._

STATION:_

SHEET 3 OF 13

BERTIE

R-5809A

398+84.00 -L-

COUNTY

		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	C1-3
1			3			TOTAL SHEETS
2			4			13

DATE : 02/2024 DATE : 02/2024

D. D. LOWERY

C.T. POOLE

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024

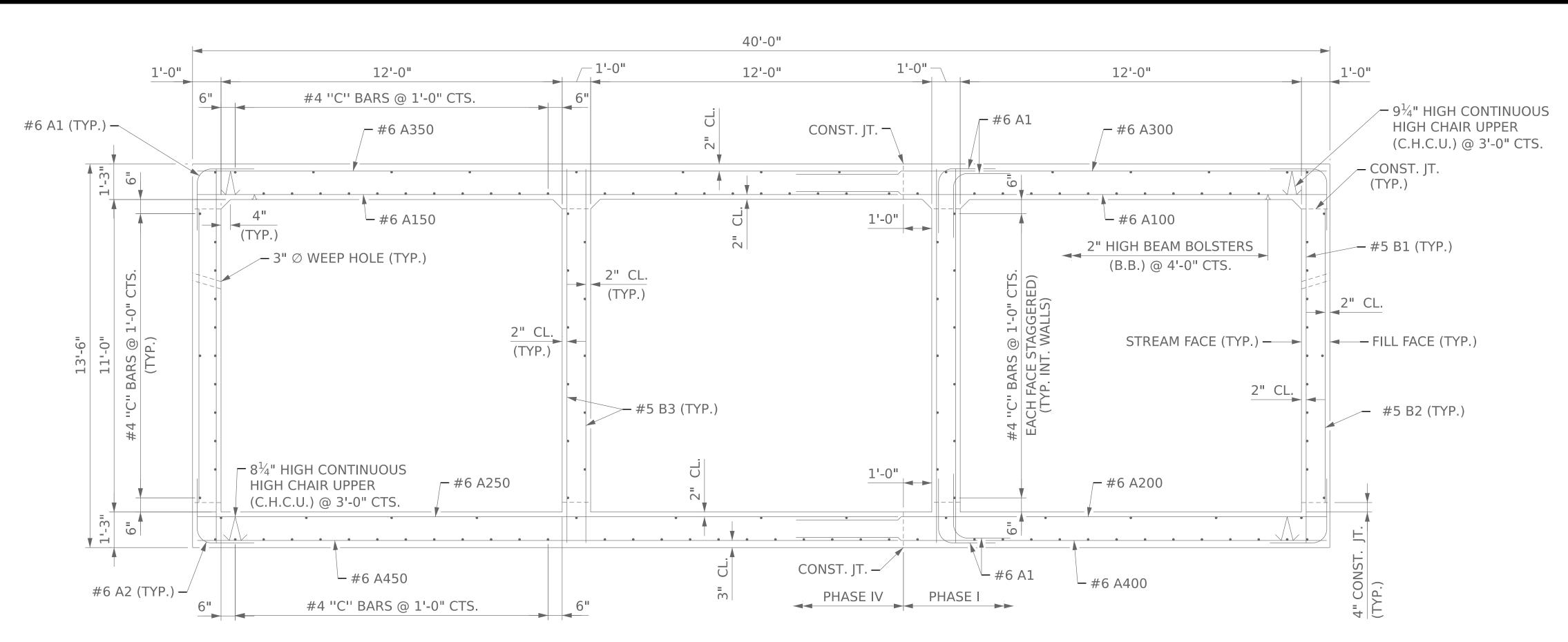
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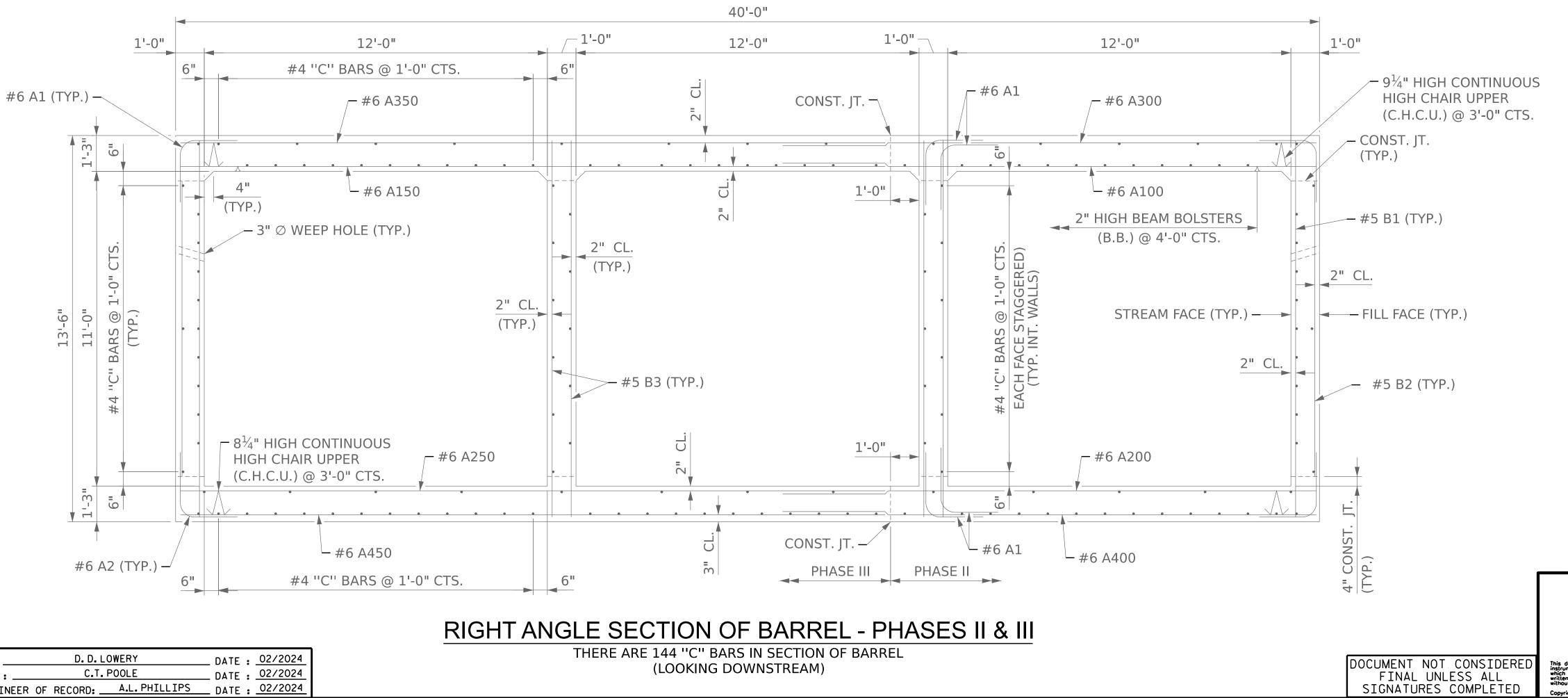
C.T. POOLE

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024



RIGHT ANGLE SECTION OF BARREL - PHASES I & IV

THERE ARE 144 "C" BARS IN SECTION OF BARREL (LOOKING DOWNSTREAM)



R-5809A PROJECT NO._ BERTIE COUNTY 398+84.00 -L-STATION:_

SHEET 4 OF 13

10/14/2024

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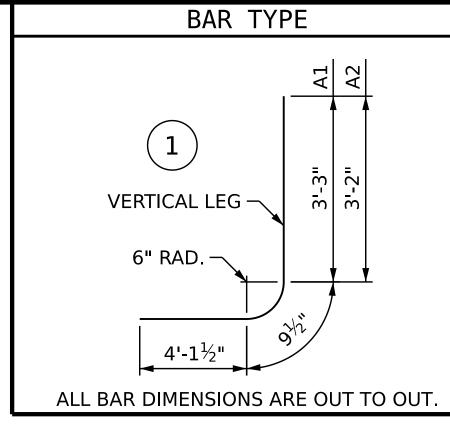
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

TRIPLE 12 FT. X 11 FT. CONCRETE BOX CULVERT 90° SKEW

SHEET NO. **REVISIONS** NO. BY: DATE: DATE: TOTAL SHEETS 13

THERE ARE 144 "C" BARS IN SECTION OF BARREL (LOOKING DOWNSTREAM)

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david.lowery



SPLICE CHART							
BAR	SIZE	SPLICE LENGTH					
A100, A200, A300, A400	#6	4'-8"					
C1, C2, C3, C4	#4	2'-10"					
G1	#5	3'-7"					
S1	#8	6'-2"					

	BILL OF MATERIAL BILL OF MATERIAL (PHASE I) (PHASE II)					BILL OF MATERIAL BILL OF MATERIAL (PHASE III) (PHASE IV)								-										
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	A1	147	6	1	8'-2"	1,803	A1	270	6	1	8'-2"	3,312	A1	135	6	1	8'-2"	1,656	A1	73	6	1	8'-2"	895
	A2	147	6	1	8'-1"	1,785	A2	270	6	1	8'-1"	3,278	A2	135	6	1	8'-1"	1,639	A2	73	6	1	8'-1"	886
	4100	73	6	STR	19'-8"	2,156	A100	134	6	STR	19'-8"	3,958	A150	134	6	STR	24'-8"	4,965	A150	73	6	STR	24'-8"	2,705
ľ	1100			3111	13 0	2,130	7 (100	13.			13 0	3,330	71130	10.		3110	210	1,303	71130			3110	210	2,703
4	4200	73	6	STR	19'-8"	2,156	A200	134	6	STR	19'-8"	3,958	A250	134	6	STR	24'-8"	4,965	A250	73	6	STR	24'-8"	2,705
	4300	73	6	STR	19'-8"	2,156	A300	134	6	STR	19'-8"	3,958	A350	134	6	STR	24'-8"	4,965	A350	73	6	STR	24'-8"	2,705
										<u> </u>		3,000				<u> </u>		1,000	7 10 0					
4	4400	73	6	STR	19'-8"	2,156	A400	134	6	STR	19'-8"	3,958	A450	134	6	STR	24'-8"	4,965	A450	73	6	STR	24'-8"	2,705
ŀ	B1	37	5	STR	13'-1"	505	B1	68	5	STR	13'-1"	928	B1	68	5	STR	13'-1"	928	B1	37	5	STR	13'-1"	505
	B2	37	5	STR	10'-4"	399	B2	68	5	STR	10'-4"	733	B2	68	5	STR	10'-4"	733	B2	37	5	STR	10'-4"	399
4	В3	74	5	STR	13'-1"	1,010	В3	136	5	STR	13'-1"	1,856	В3	136	5	STR	13'-1"	1,856	В3	74	5	STR	13'-1"	1,010
L																								
F	C1	59	4	STR	36'-5"	1,435	C2	118	4	STR	34'-11"	2,752	C3	170	4	STR	34'-11"	3,965	C4	85	4	STR	36'-5"	2,068
L	D1	38	6	STR	3'-0"	171	G1	4	5	STR	18'-7"	78	D1	50	6	STR	3'-0"	225	G2	4	5	STR	24'-8"	103
F	G1	4	5	STR	18'-7"	78	S1	12	8	STR	21'-4"	684	G2	4	5	STR	24'-8"	103	S2	12	8	STR	24'-8"	790
L	S1	12	8	STR	21'-4"	684							S2	12	8	STR	24'-8"	790						
ŀ																								
ŀ				<u> </u>		ı				<u> </u>	l	l				<u>I</u>	I	ı						J
	EPOX REINF		ATED NG ST	EEL	LBS.	16,494		Y COA	ATED NG ST	EEL	LBS.	29,453		Y COA		EEL	LBS.	31,755	EPOX REIN		ATED ING ST	EEL	LBS.	17,476

PHASE I QU	JANTITIES	S	PHASE II C	UANTITIE	S	PHASE III C	UANTITIE	S	PHASE IV C		ES
CLASS A CONCRETE BARREL @ 2.210 WINGS, ETC.	C.Y./FT. 81.2 19.4		<u> </u>	C.Y./FT. 148.6 19.4				C.Y. C.Y.		C.Y./FT. 115.2 20.5	
TOTAL	19.4		,	168.0		,	231.4			135.7	
EPOXY COATED REINFO	ORCING STEEL		EPOXY COATED REINI	FORCING STEEL		EPOXY COATED REINF	ORCING STEEL		EPOXY COATED REINF	FORCING STEEL	
BARREL	16,494	LBS.	BARREL	29,453	LBS.	BARREL	31,755	LBS.	BARREL	17,476	LBS.
WINGS, ETC.	1,640	LBS.	WINGS, ETC.	1,640	LBS.	WINGS, ETC.	1,640	LBS.	WINGS, ETC.	1,640	LBS.
TOTAL	18,134	LBS.	TOTAL	31,093	LBS.	TOTAL	33,395	LBS.	TOTAL	19,116	LBS.
CULVERT EXCAVATION	LUMF	SUM	CULVERT EXCAVATION	N LUMP	SUM	CULVERT EXCAVATION	I LUMF	SUM	CULVERT EXCAVATION	\ LUM	P SUM
FOUNDATION COND. N	MATERIAL 44	TONS	FOUNDATION COND.	MATERIAL 81	TONS	FOUNDATION COND.	MATERIAL 128	TONS	FOUNDATION COND.	MATERIAL 70	TONS

PROUSE STANDARD SEAL OF THE PHILITING TO SEAL OF THE PHILIPING TO SEAL

Kimley >>> Horn
421 Favetteville Street Suite 600

421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
Phone (919) 677-2000

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PROJECT NO. R-5809A

BERTIE COUNTY

STATION: 398+84.00 -L-

SHEET 5 OF 13

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

TRIPLE 12 FT. X 11 FT.
CONCRETE BOX CULVERT
90° SKEW

REVISIONS

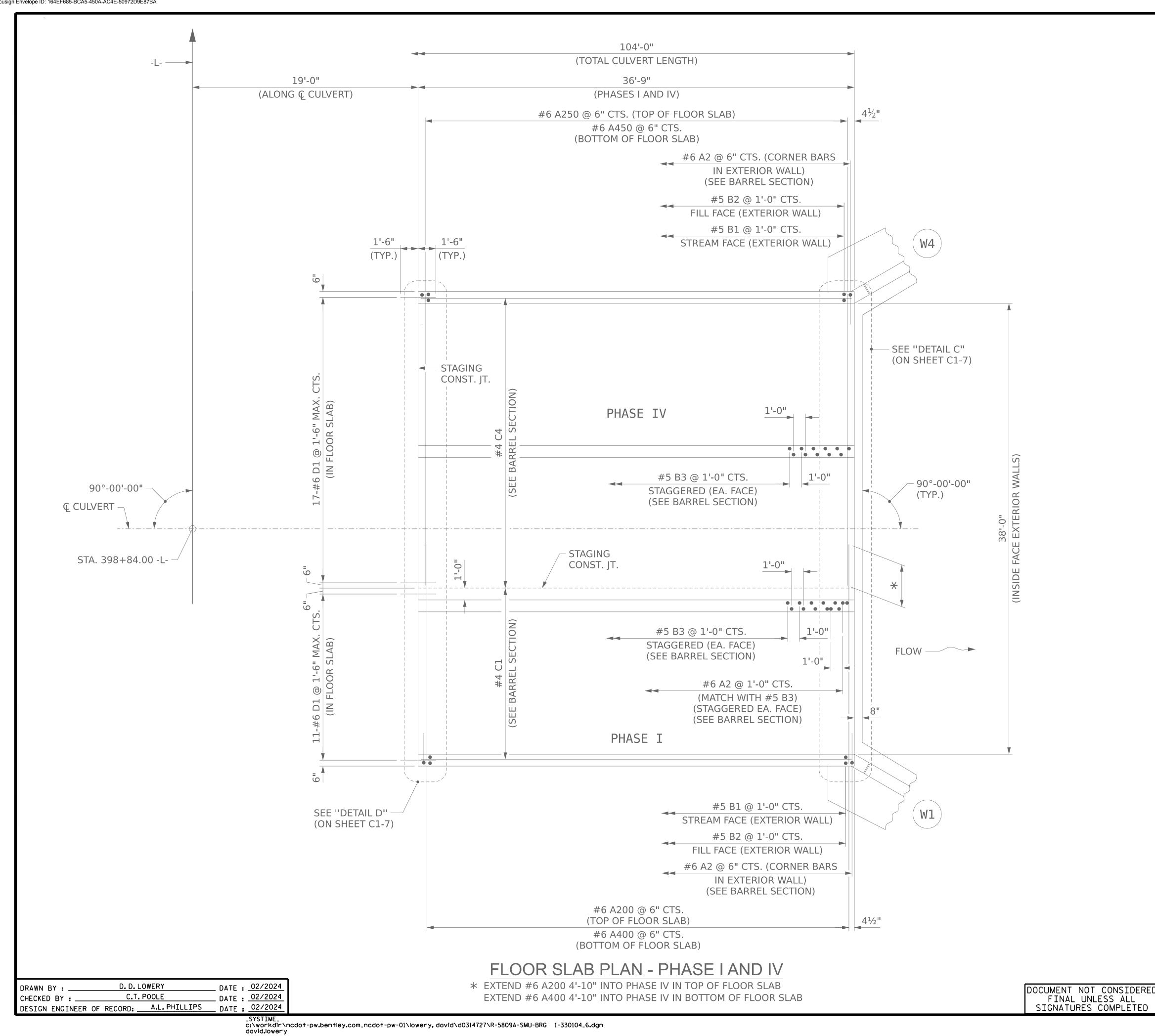
SHEET NO.

C1-5

STOTAL
SHEETS
13

DRAWN BY: D.D.LOWERY DATE: 02/2024
CHECKED BY: C.T. POOLE DATE: 02/2024
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024

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398+84.00 -L-STATION:_

SHEET 6 OF 13

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

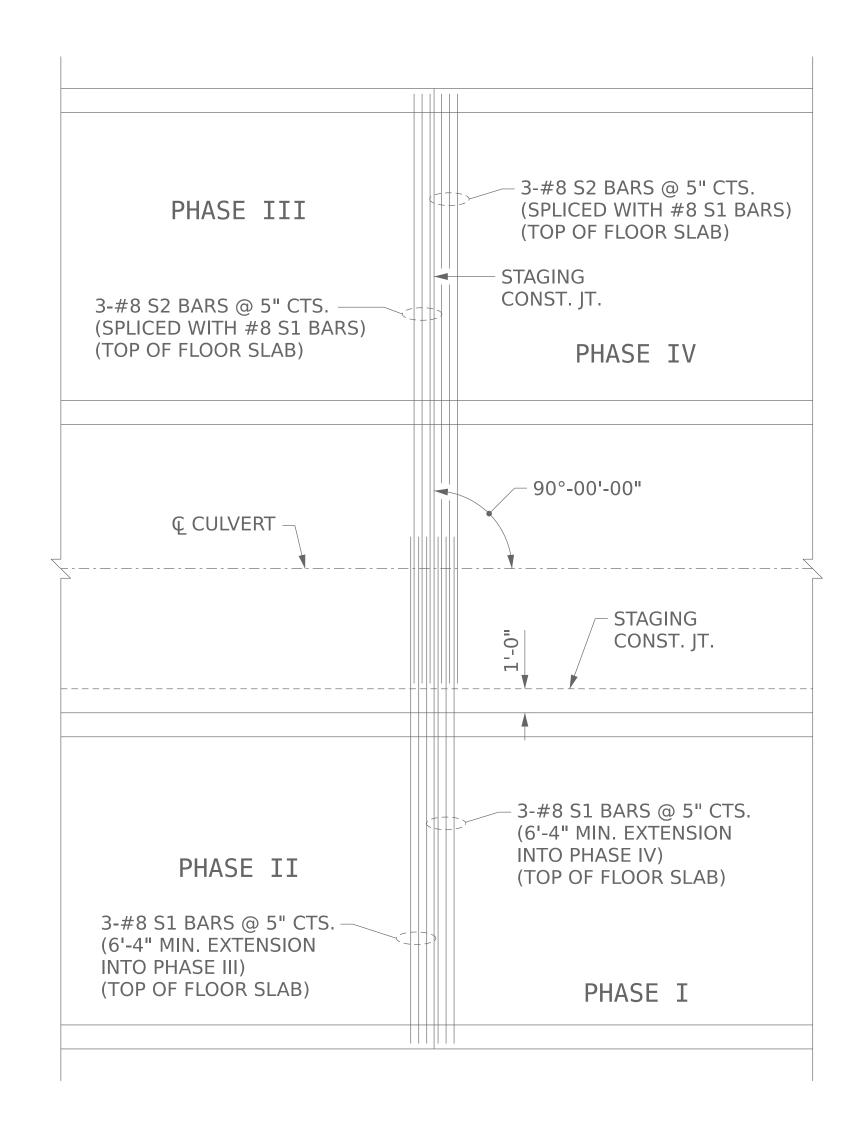
TRIPLE 12 FT. X 11 FT. **CONCRETE BOX CULVERT** 90° SKEW (PHASES I AND IV)

SHEET NO.

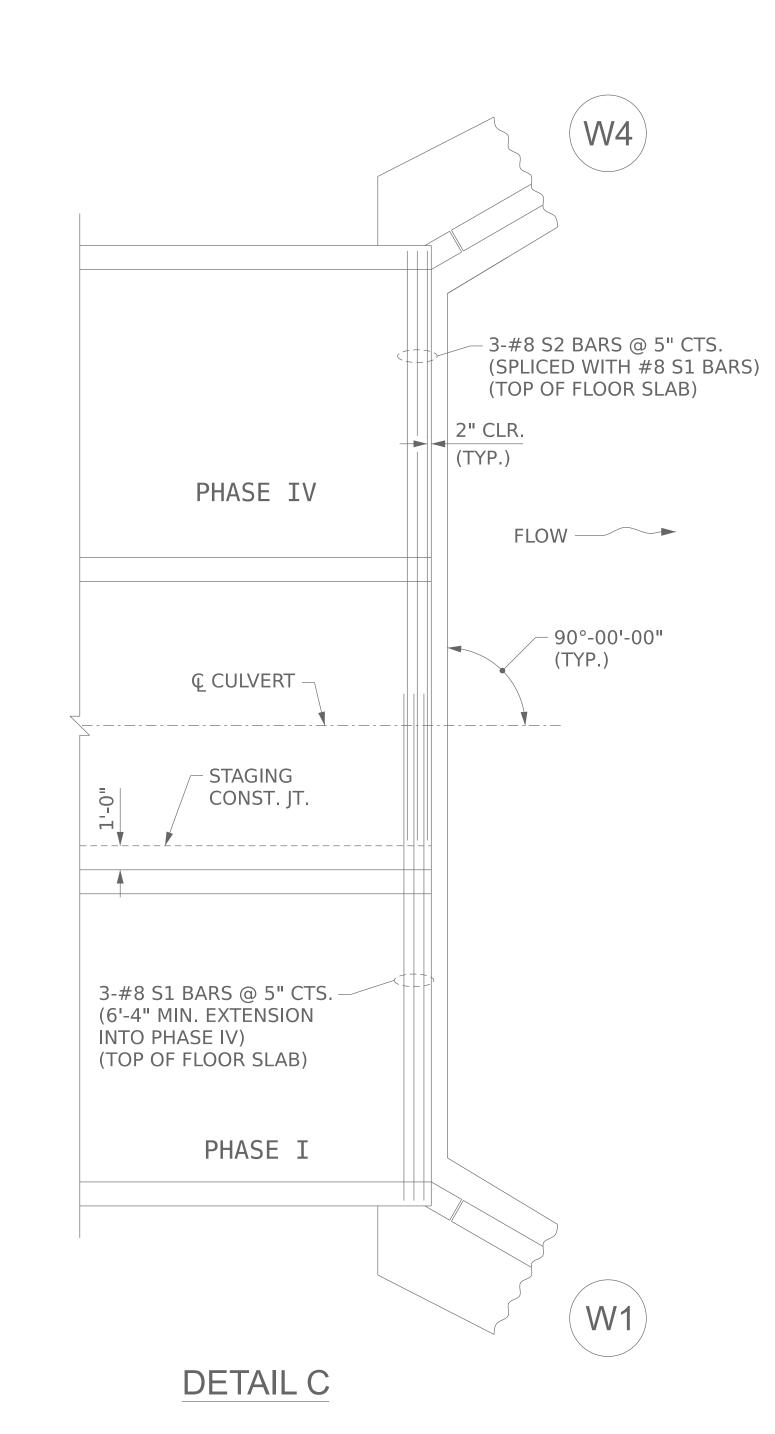
TOTAL SHEETS

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DETAIL D #6-D1 NOT SHOWN FOR CLARITY



PROJECT NO. R-5809A

BERTIE COUNTY

STATION: 398+84.00 -L-

SHEET 7 OF 13



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NC LICENSE #
F-0102

TRIPLE 12 FT. X 11 FT.
CONCRETE BOX CULVERT
90° SKEW
(PHASES I - IV)

STATE OF NORTH CAROLINA

	REVI:	SIO	NS		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	C1-7
		3			TOTAL SHEETS
		4			13

DRAWN BY: D.D.LOWERY

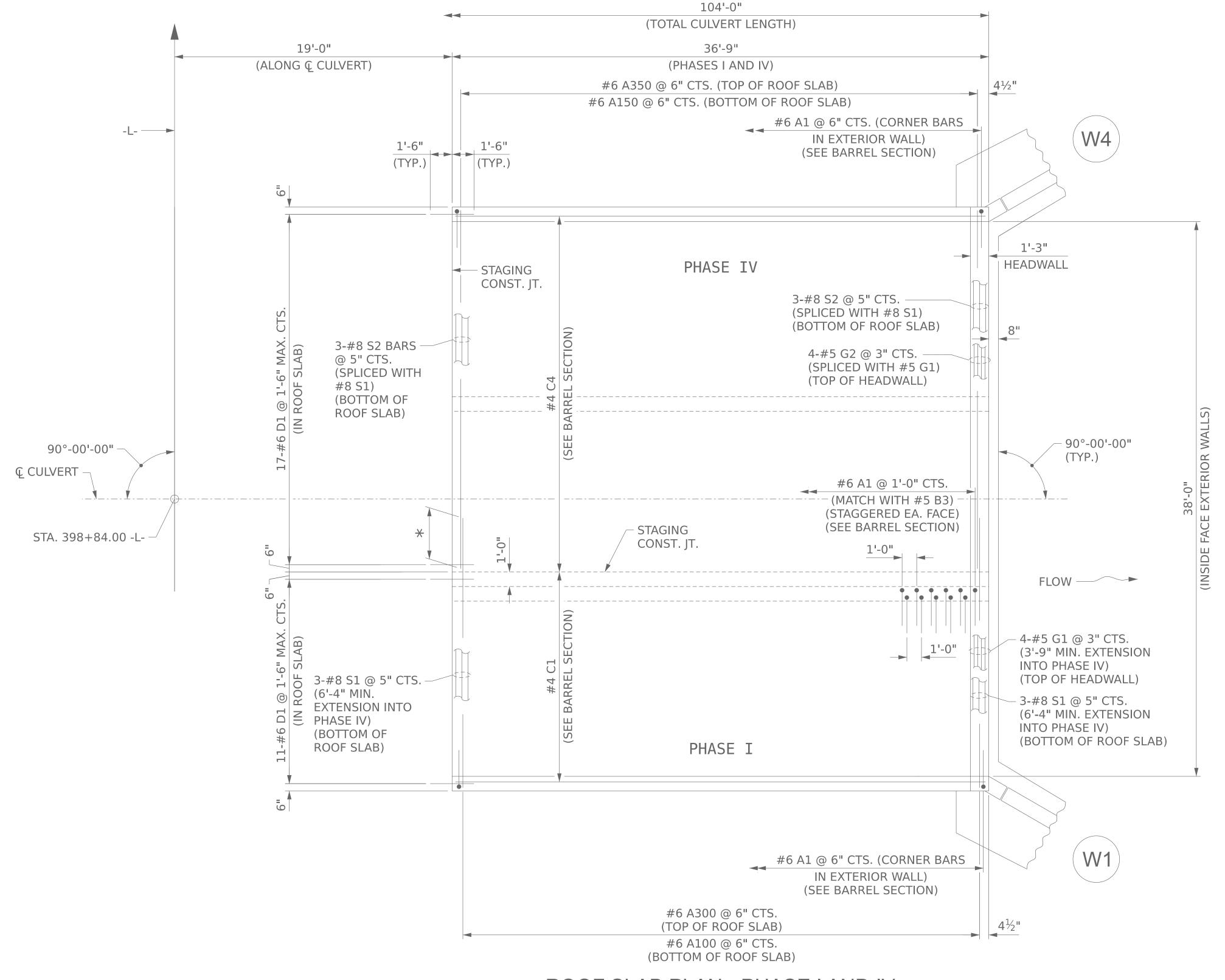
CHECKED BY: C.T. POOLE

DATE: 02/2024

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS

DATE: 02/2024

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ROOF SLAB PLAN - PHASE I AND IV

* EXTEND #6 A300 4'-10" INTO PHASE IV IN TOP OF FLOOR SLAB EXTEND #6 A100 4'-10" INTO PHASE IV IN BOTTOM OF FLOOR SLAB

D. D. LOWERY _ DATE : <u>02/2024</u> DRAWN BY : _ DATE : 02/2024 C.T. POOLE CHECKED BY : ___ DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024

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421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102

398+84.00 -L-STATION:_ SHEET 8 OF 13 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TRIPLE 12 FT. X 11 FT. **CONCRETE BOX CULVERT** 90° SKEW (PHASES I AND IV)

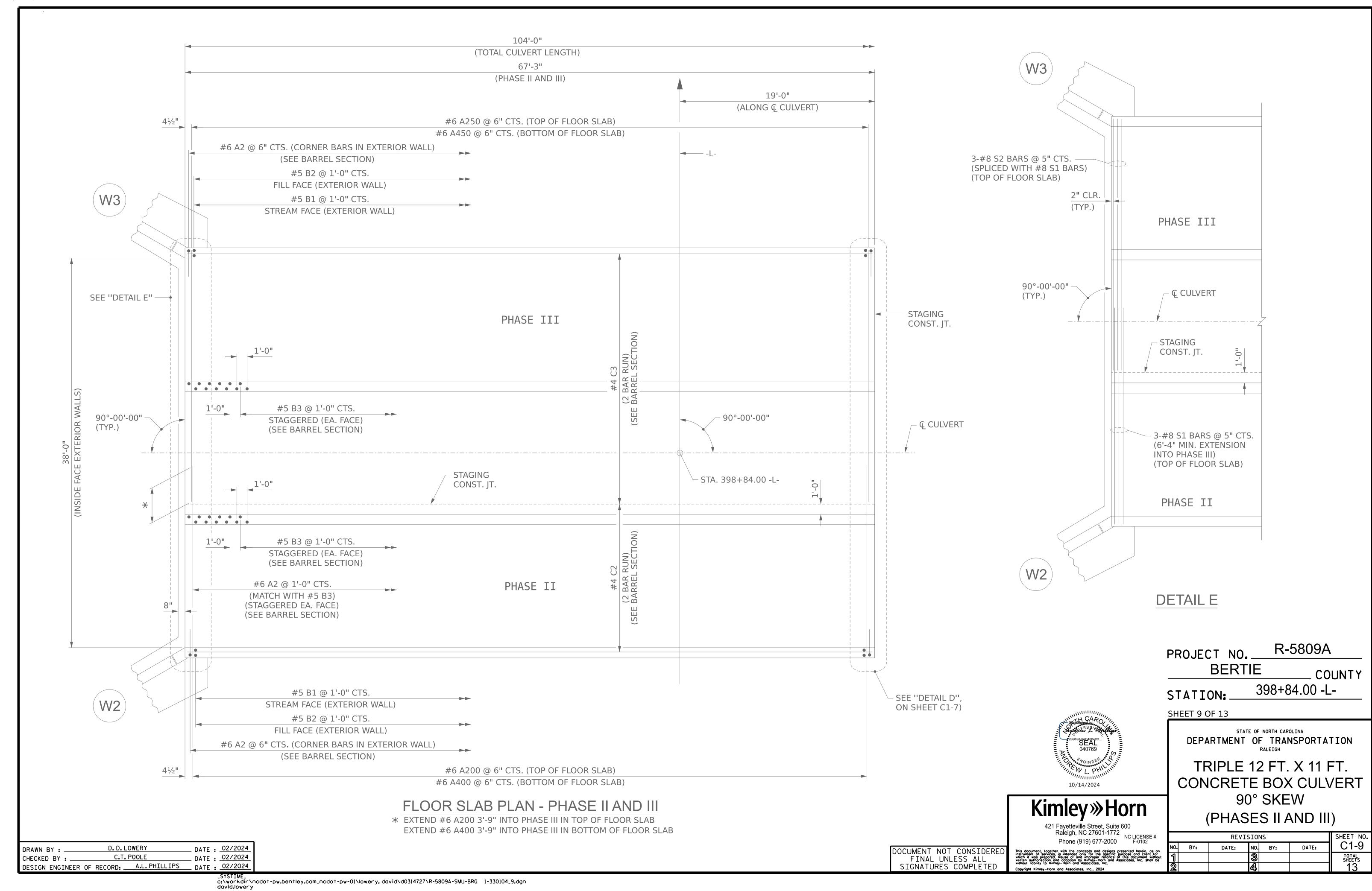
PROJECT NO._

BERTIE

R-5809A

COUNTY

SHEET NO. C1-8 REVISIONS NO. BY: DATE: DATE: TOTAL SHEETS 13



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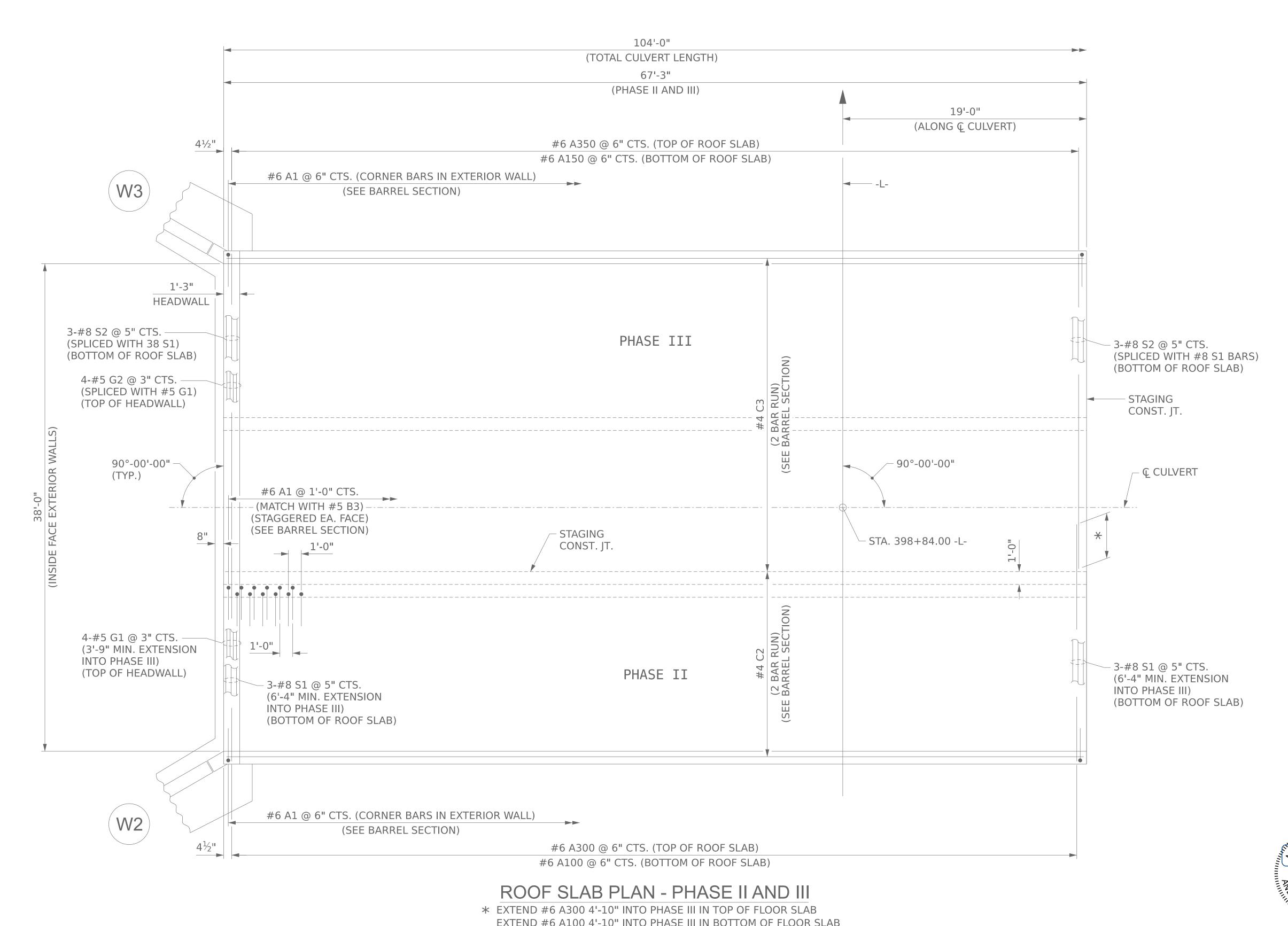
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 02/2024

DRAWN BY :

CHECKED BY : ___

_ DATE : 02/2024

_ DATE : 02/2024



BERTIE 398+84.00 -L-STATION:_

PROJECT NO.__

SHEET 10 OF 13

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

R-5809A

COUNTY

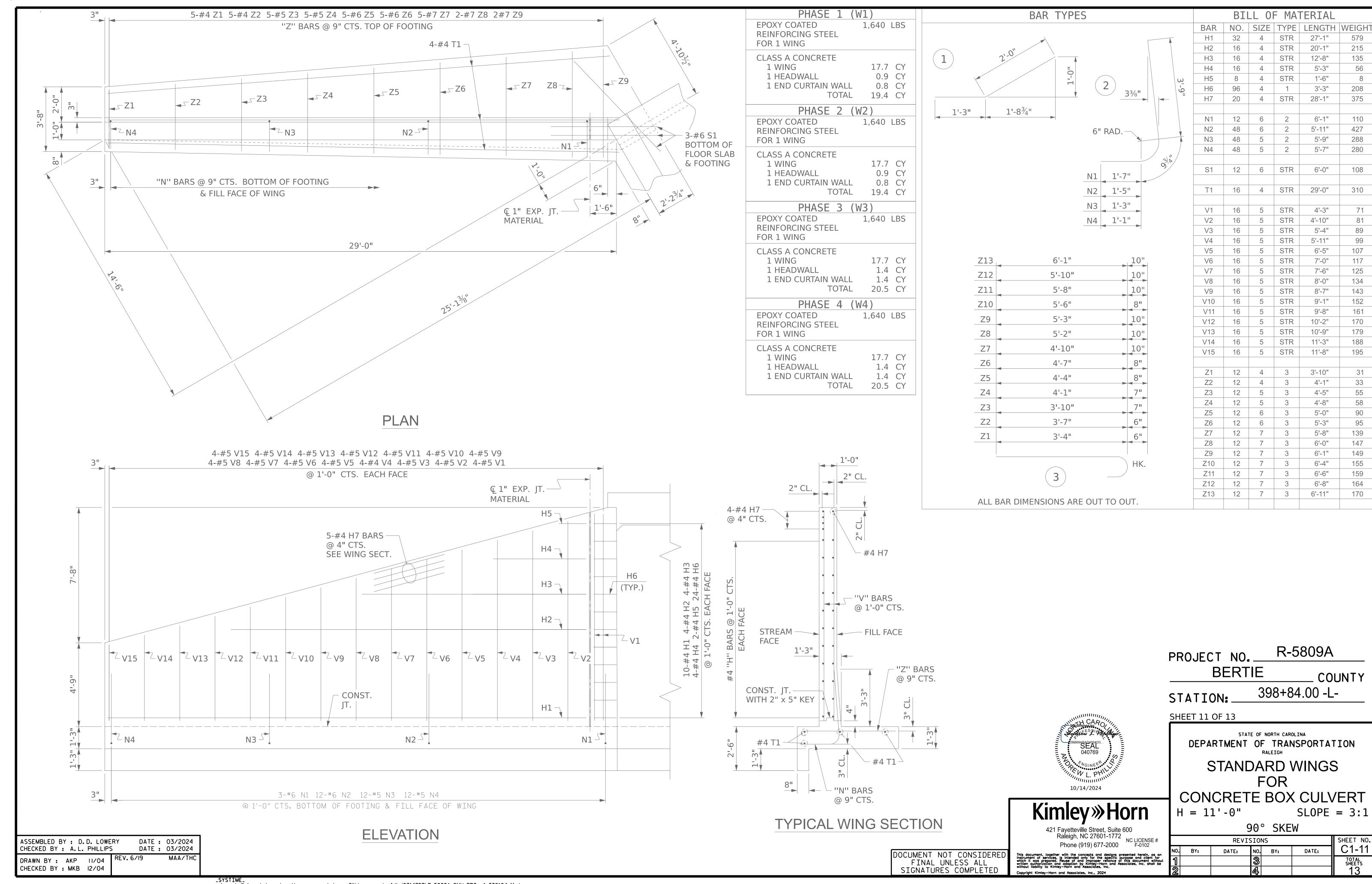
TRIPLE 12 FT. X 11 FT. CONCRETE BOX CULVERT 90° SKEW (PHASES II AND III)

421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102 SHEET NO. C1-10 REVISIONS DATE: NO. BY: DATE: TOTAL SHEETS 13

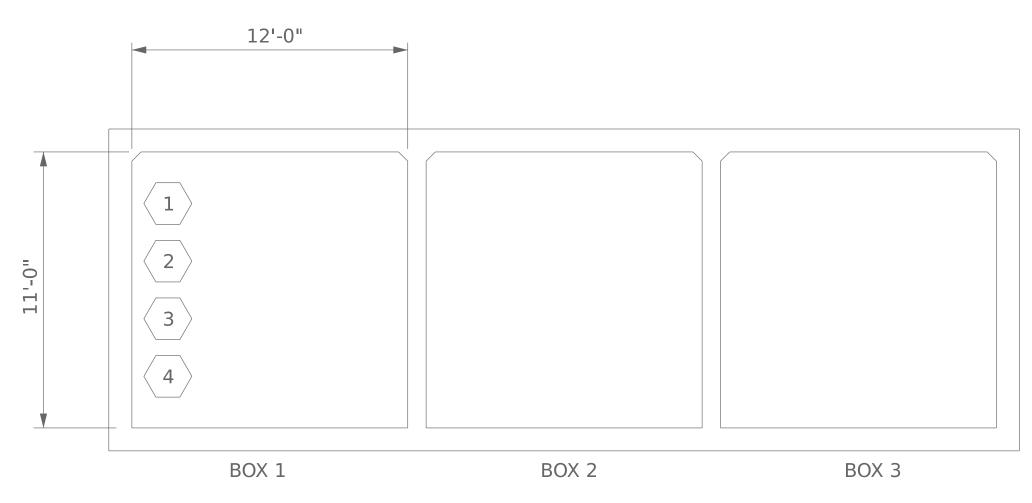
EXTEND #6 A100 4'-10" INTO PHASE III IN BOTTOM OF FLOOR SLAB

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FINAL UNLESS ALL

SIGNATURES COMPLETED



LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
							STRENGTH I LIMIT STATE									
				(#)						MOMENT				SHEAR		E C
LOAD TYPE		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	$TONS = W \times RF$	LIVE-LOAD FACTORS (7 LL)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBE
		HL-93 (INVENTORY)	N/A	<u>(1)</u>	2.52		1.75	2.52	1	EXTERIOR WALL	1.00	3.62	1	BOTTOM SLAB	0.00	
DESI		HL-93 (OPERATING)	N/A		3.27		1.35	3.27	1	EXTERIOR WALL	1.00	4.69	1	BOTTOM SLAB	0.00	
LOA	VD	HS-20 (INVENTORY)	36.000	2	2.53	91.08	1.75	2.53	1	EXTERIOR WALL	1.00	3.62	1	BOTTOM SLAB	0.00	
		HS-20 (OPERATING)	36.000		3.28	118.08	1.35	3.28	1	EXTERIOR WALL	1.00	4.69	1	BOTTOM SLAB	0.00	
		SNSH	13.500		3.18	42.93	1.40	3.18	1	EXTERIOR WALL	1.00	11.00	1	EXTERIOR WALL	0.50	
	E VEHICLE (SV)	SNGARBS2	20.000		3.18	63.60	1.40	3.18	1	EXTERIOR WALL	1.00	9.57	1	BOTTOM SLAB	0.00	
		SNAGRIS2	22.000		3.18	69.96	1.40	3.18	1	EXTERIOR WALL	1.00	8.72	1	BOTTOM SLAB	0.00	
		SNCOTTS3	27.250	3	3.15	85.84	1.40	3.15	1	EXTERIOR WALL	1.00	5.53	1	TOP SLAB	0.00	
		SNAGGRS4	34.925		3.15	110.01	1.40	3.15	1	EXTERIOR WALL	1.00	5.37	1	TOP SLAB	40.00	
	SING	SNS5A	35.550		3.15	111.98	1.40	3.15	1	EXTERIOR WALL	1.00	5.18	3	TOP SLAB	40.00	
	S	SNS6A	39.950		3.15	125.84	1.40	3.15	1	EXTERIOR WALL	1.00	4.84	1	BOTTOM SLAB	0.00	
LEGAL		SNS7B	42.000		3.15	132.30	1.40	3.15	1	EXTERIOR WALL	1.00	4.70	3	BOTTOM SLAB	40.00	
LOAD		TNAGRIT3	33.000		3.16	104.28	1.40	3.16	1	EXTERIOR WALL	1.00	5.86	1	BOTTOM SLAB	0.00	
	OR R.	TNT4A	33.075		3.16	104.52	1.40	3.16	1	EXTERIOR WALL	1.00	5.82	1	BOTTOM SLAB	0.00	
	ACT (TNT6A	41.600		3.15	131.04	1.40	3.15	1	EXTERIOR WALL	1.00	4.83	3	BOTTOM SLAB	40.00	
	TR/ TR/ TSI	TNT7A	42.000		3.15	132.30	1.40	3.15	1	EXTERIOR WALL	1.00	4.77	3	BOTTOM SLAB	40.00	
	N N N N N N N N N N N N N N N N N N N	TNT7B	42.000		3.15	132.30	1.40	3.15	1	EXTERIOR WALL	1.00	4.76	1	BOTTOM SLAB	40.00	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT4	43.000		3.16	135.88	1.40	3.16	1	EXTERIOR WALL	1.00	4.50	1	BOTTOM SLAB	0.00	
		TNAGT5A	45.000		3.16	142.20	1.40	3.16	1	EXTERIOR WALL	1.00	4.32	3	BOTTOM SLAB	40.00	
		TNAGT5B	45.000		3.16	142.20	1.40	3.16	1	EXTERIOR WALL	1.00	4.29	3	BOTTOM SLAB	40.00	
EMERG	GENCY	EV2	28.750		3.41	98.04	1.30	3.41	1	EXTERIOR WALL	1.00	7.17	1	EXTERIOR WALL	0.00	
VEHICL	E (EV)	EV3	43.000	4	3.38	145.34	1.30	3.38	1	EXTERIOR WALL	1.00	4.74	3	TOP SLAB	0.00	



LRFR SUMMARY
(LOOKING DOWNSTREAM)

ASSEMBLED BY: D. D. LOWERY
CHECKED BY: A. L. PHILLIPS

DATE: 03/2024

DRAWN BY: WMC
7/II
CHECKED BY: GM

7/II
REV. 10/1/II
REV. 12/17
REV. 04/23

MAA/GM
MAA/THC
BNB/AAI

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

NOTES:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATES.

COMMENTS:

- Τ.
- 2.
- 3.
- 4.
- # CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING * *
- 4 EMERGENCY VEHICLE LOAD RATING * *
- * * SEE CHART FOR VEHICLE TYPE

PROJECT NO. R-5809A

BERTIE COUNTY

STATION: 398+84.00 -L-

SHEET 12 OF 13



Kimley» Horn

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Raleigh, NC 27601-1772
Phone (919) 677-2000

NC LICENSE #
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NC LICENSE #

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR

REINFORCED CONCRETE
BOX CULVERTS
(NON-INTERSTATE TRAFFIC)

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	 AASHTO (CURRENT)
LIVE LOAD	 SEE PLANS
IMPACT ALLOWANCE	 SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	 20,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50W _	 27,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50	 27,000 LBS. PER SQ. IN
REINFORCING STEEL IN TENSION - GRADE 60	 24,000 LBS. PER SQ. IN
CONCRETE IN COMPRESSION	 1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	 SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	 1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	 375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	 30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE %" \varnothing SHEAR STUDS FOR THE 34" \varnothing STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 78" \varnothing STUDS FOR 4 - 34" \varnothing STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 78" \varnothing STUDS ALONG THE BEAM AS SHOWN FOR 34" \varnothing STUDS BASED ON THE RATIO OF 3 - 78" \varnothing STUDS FOR 4 - 34" \varnothing STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $^1\!\!/_16$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS _____ AASHTO (CURRENT) LIVE LOAD SEE PLANS STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 _ _ _ 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W _ _ 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 _ _ _ 27,000 LBS, PER SO, IN. REINFORCING STEEL IN TENSION - GRADE 60 _____ 24,000 LBS, PER SO, IN. CONCRETE IN SHEAR SEE AASHTO STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS ____ 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER _____ 375 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

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CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

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STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION. HE MAY SUBSTITUTE 1/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " \varnothing STUDS FOR 4 - $\frac{3}{4}$ " \varnothing STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \emptyset STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \emptyset STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " \emptyset STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

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REV. 10-1-11 MAA (✔) GM REV. 10-23 BNB (✔) NAP REV. 5-7-03 RWW (✔) JTE REV. 5-1-06 TLA (✔) GM

REV. 12-17 MAA (✔) THC

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