REFERENCE

**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

ROADWAY TITLE SHEET

TITLE SHEET

SITE PLAN

BORE LOGS SOIL TEST RESULTS

SHEET NO.

4-I9B

20-23

1006 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY **CUMBERLAND** 

PROJECT DESCRIPTION CAMDEN ROAD WIDENING FROM FUTURE FAYETTEVILLE OUTER LOOP TO NC 59

SITE DESCRIPTION 8 BORING LOCATIONS FOR TEMPORARY SHORING

STATE PROJECT REFERENCE NO. 28 U-3422

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABDRATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GLARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

	ROSE
	ATLIN INC.
	I HOLLAND
INVESTIGATED BY _	
DRAWN BY <i>J. HC</i>	DLLAND
CHECKED BY J. C	CRENSHAW

LDOCE



SUBMITTED BY \_SCHNABEL ENG.

DATE MARCH 2023



**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

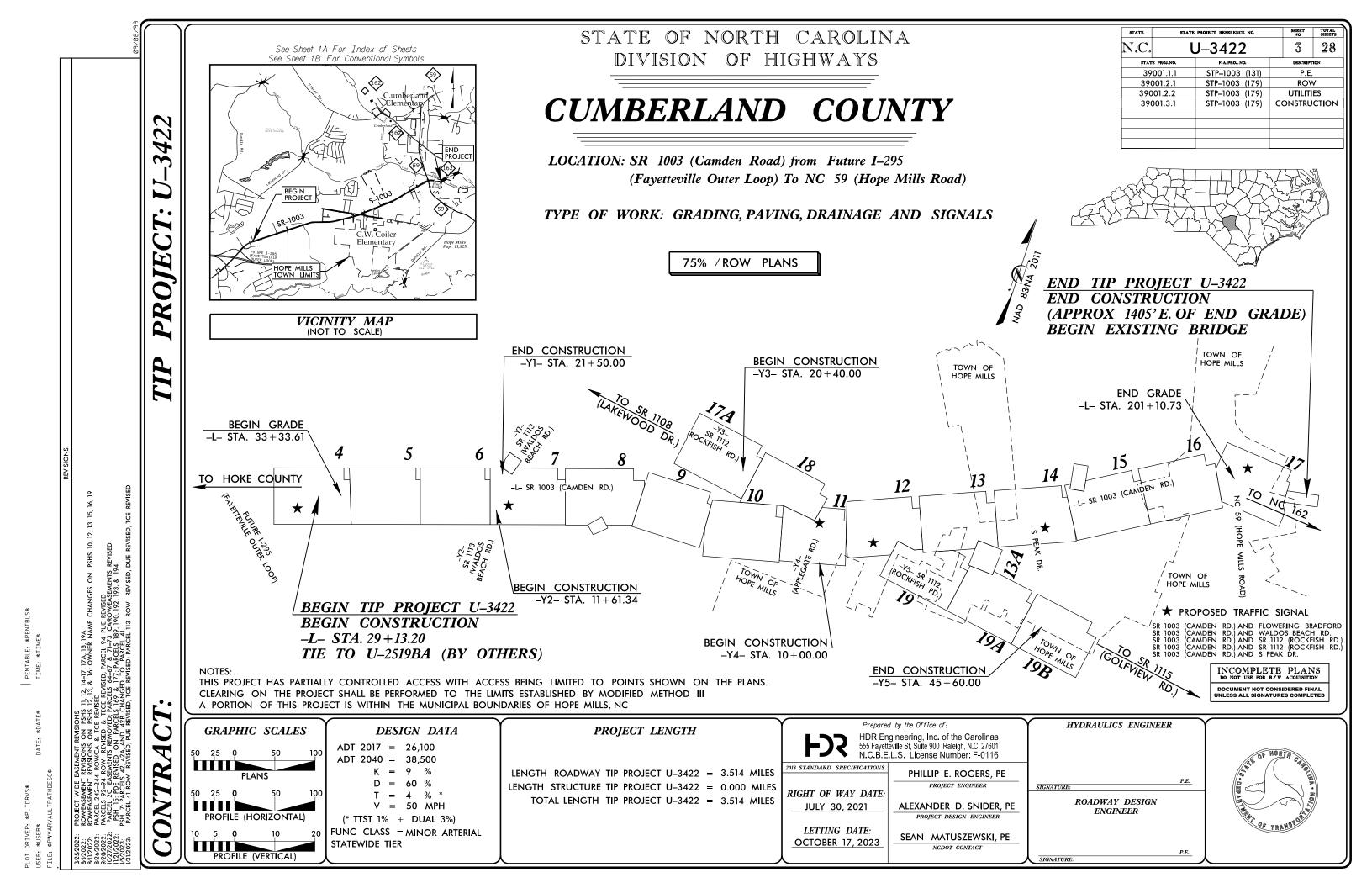
U-3422 2 SHEET NO.

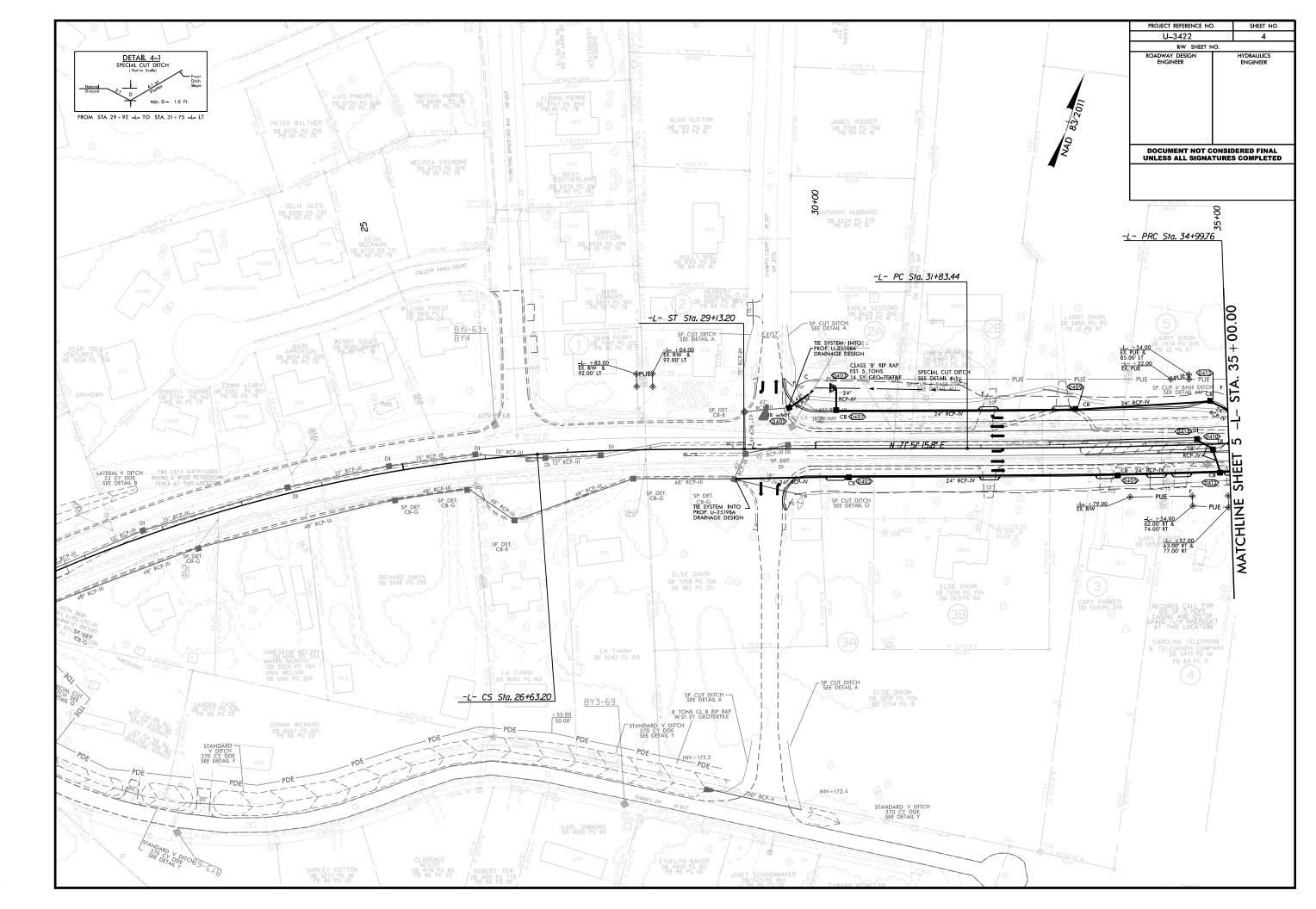
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

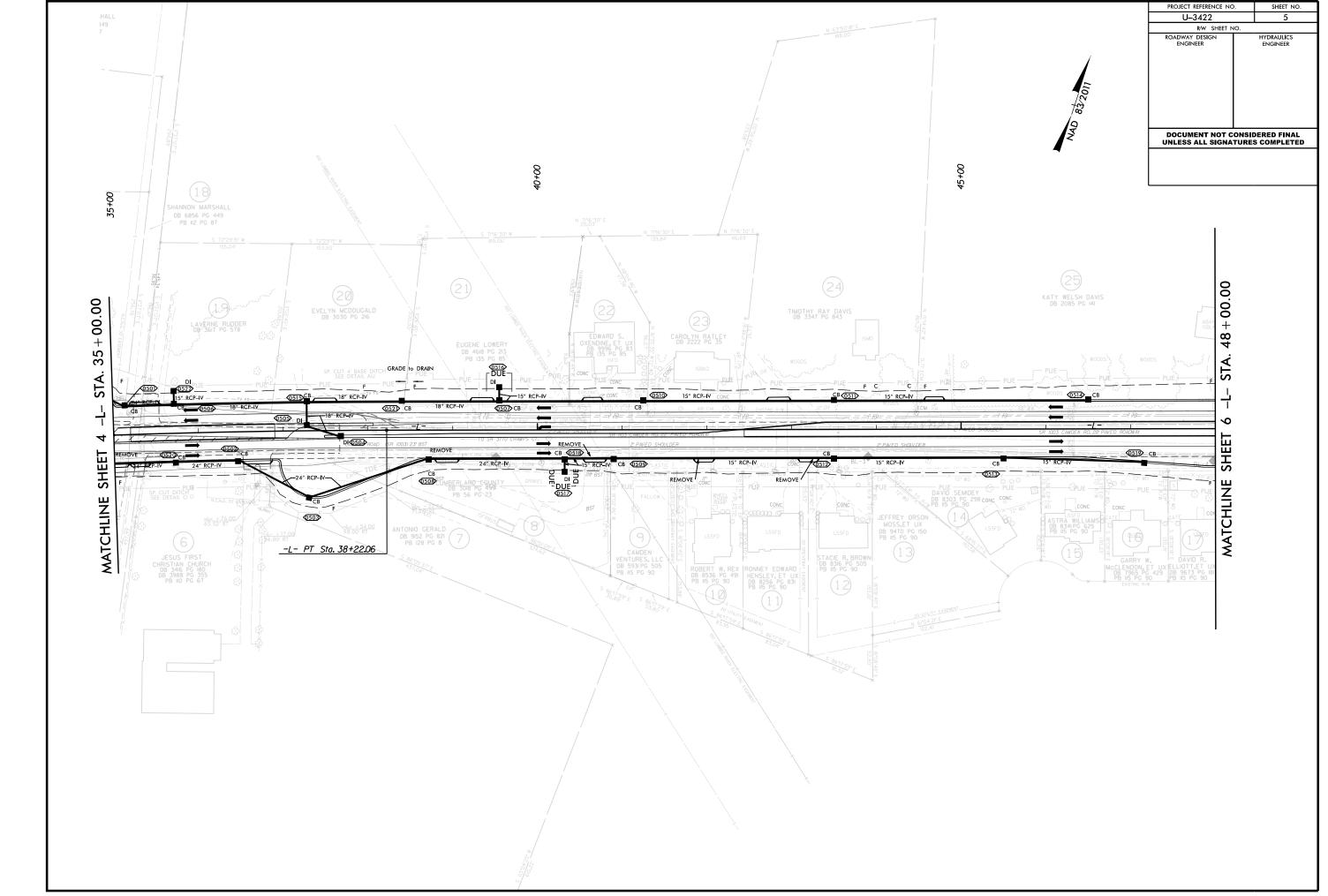
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

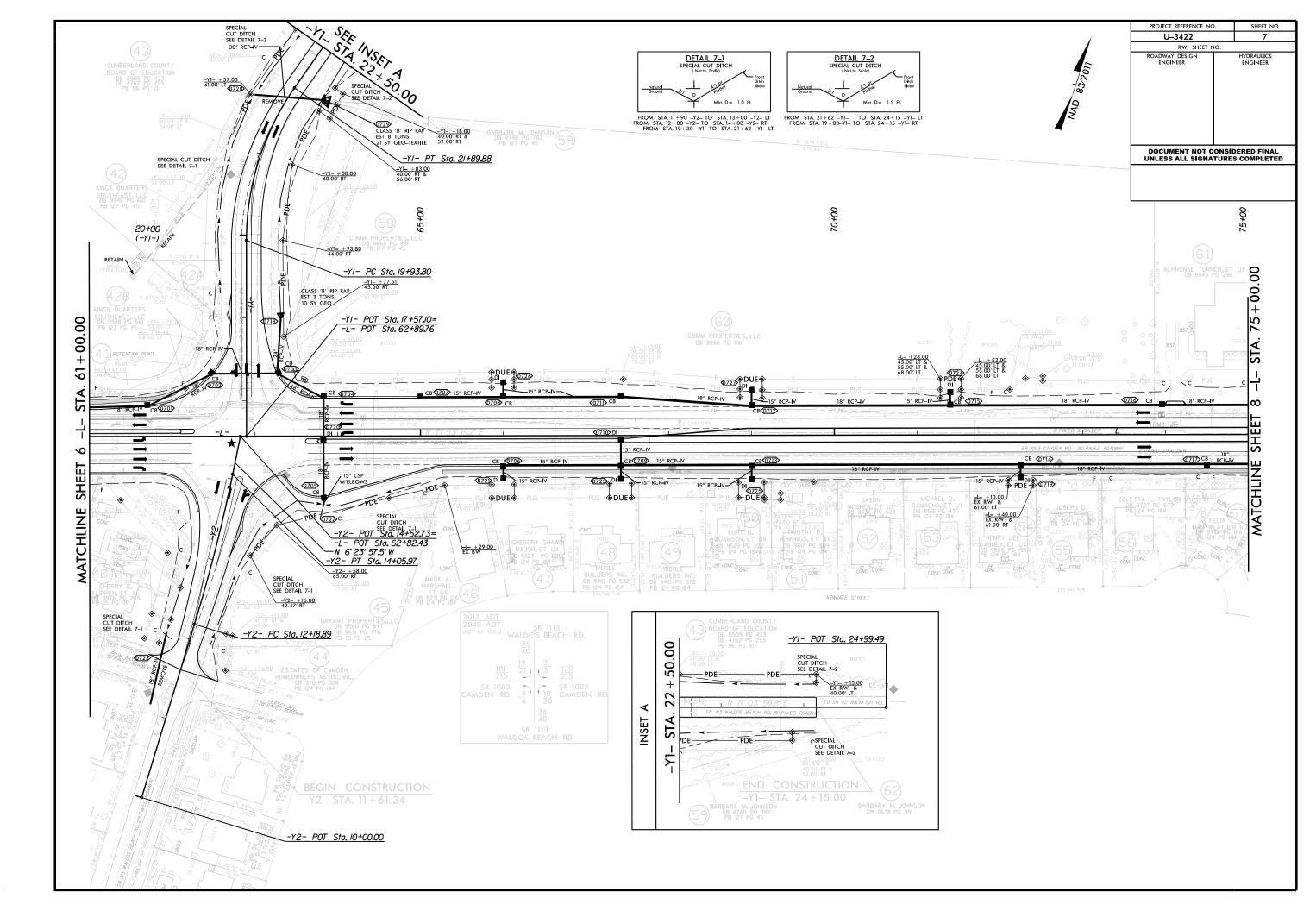
			T
SOIL DESCRIPTION	GRADATION CONTROL TO C	ROCK DESCRIPTION  HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED WILL NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200)  GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR)  WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-0 A-2-4 A-2-5 A-2-6 A-2-7 A-4-7-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7. PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN  SEDIMENTARY ROCK  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD  SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR SIL1-	PERCENTAGE OF MATERIAL	(CP) ————————————————————————————————————	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
#40 30 MX 50 MX 51 MN PEAT SOILS SOILS SOILS SOILS SOILS	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40  LL 48 MX 41 MN 50ILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
P1 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOUR	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL, AND MATERIALS SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
CEN BATING FAIR TO		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS  COMPAGENESS OF RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	<u>IF TESTED, WOULD YIELD SPT REFUSAL</u>	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-VALUE) (TUNS/FT-)	WITH SOIL DESCRIPTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE	SOIL SYMBOL  OPT DMT TEST BORING  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR MEDIUM DENSE 10 TO 30 N/A MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT TEST	VERY  ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE  SEVERE  BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING ■ SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</i>	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MN MONITORING WELL TEST BORING	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK,
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	TTTTT ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY SPT N-VALUE	ALSO AN EXAMPLE.  ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER	UNDERCUT ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
	CL CLAY MOD MODERATELY $\gamma$ - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE FIELD MOISTURE COURSE FOR FIELD MOISTURE OF SCANDISCHOOL	CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 <sub>d</sub> - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS)  (ATTERBERG LIMITS)  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRACT - FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS $w$ - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BM-10, NORTHING: 4456801 EASTING: 1998989
" " PL L PLASTIC LIMIT	HI HIGHLY V - VERY RATIO  EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	FI FWATTON 170 00 FFFT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: I72.02 FEET
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE         1 TO 3 FEET         THINLY BEDDED         0.16 - 1.5 FEET           CLOSE         0.16 TO 1 FOOT         VERY THINLY BEDDED         0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6 CONTINUOUS FLIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	BORING AND COLLAR ELEVATIONS OBTAINED WITH CONVENTIONAL SURVEY EQUIPMENT - SURVEY DATED 02/21/23
PLASTICITY	X CME-55	INDURATION	UCP - UNDIVIDED COASTAL PLAIN
PLASTICITY INDEX (PI) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	FIAD - FILLED IMMEDIATELY AFTER DRILLING
NON PLASTIC 0-5 VERY LOW	CME-550 HARD FACED FINGER BITS -N	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM	VANE SHEAR TEST CASING W/ ADVANCER HAND TOOLS:	GENILE BLOW BY HAMMER DISINIEGRATES SAMPLE.	- PAVEMENT / ABC STONE
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE:	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING ROD VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	DATE: 8-15-14
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		SAMPLE BREAKS ACROSS GRAINS.	





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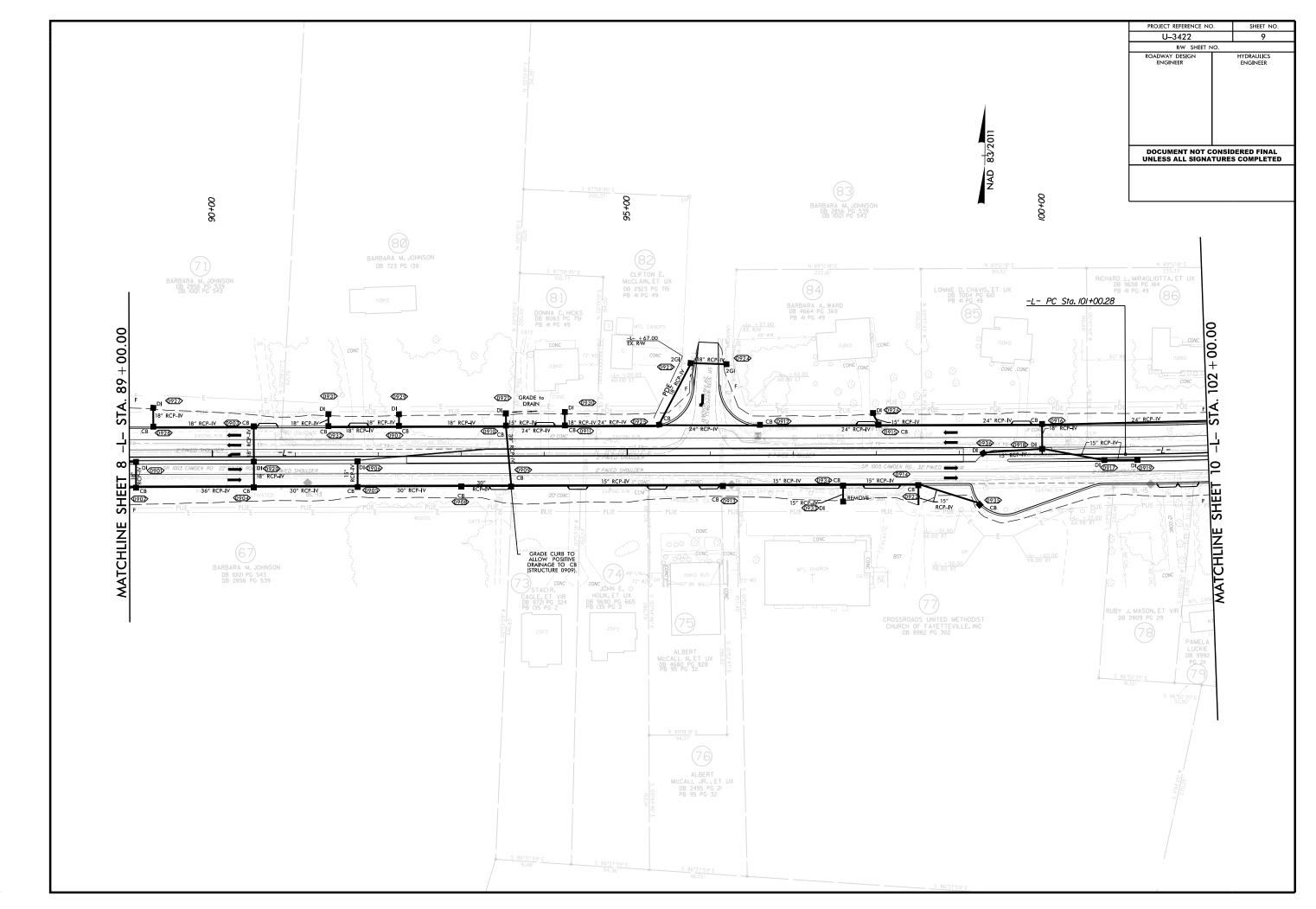
DETAIL 8-2 ROCK FILL IN POND (Not to Scale) DETAIL 8-4 STANDARD 6' BASE DITCH (Not to Scale) RW SHEET NO. HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER Min. D = 2 Ft. Max. d = 2 Ft. B = Varies from 6' to 12' Ft. 83,2011 Type of Liner= Class B Rip-Rap Type of Liner = Class | Rip-Rap |
FROM STA. 80+37 TO STA. 80+80 -L- RT FROM STA. 80+80 TO STA. 84+57 -L- RT FROM STA. 76+93 TO STA. 78+36 -L- RT FROM STA. 76+75 TO STA. 79+50 -L- LT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 80+00 75+00 85+00 BARBARA M. JOHNSON DB 3762 PG 93 +00.00<u>-L- +25.00</u> 74.00′ LT <u>-L-</u> +06.00 71.00' LT <u>-L- +90.00</u> 90.00' LT 75 -L- +93.00 56.00' LT STA. 89+00.00 ST – PUE — ® ® CB 0815 15" /RCP-IV **0810 0**818 **082** SHEET €804 PUE CB (081) **€**0808 CB MATCHLINE <u>-L- +81.00\</u> 65.00' RT SHEET ROCK FILL POND
IN POND
EST. 220 TONS
80 SY GEOTEXTILE C
SEE DETAIL 8-2 L- +37.00 45.00' RT \\_L\_ + 95.00 100.00' RT -L- PC Sta. 84+01.81 MATCHLINE -L- +52.00 60.00' RT & 66.00' RT -L- PT Sta. 88+58.22 STANDARD 6' BASE DITCH SEE DETAIL 8-3 SLOPE=0.51% LENGTH = 670' Q DDE=7,000 CY BARBARA M. JOHNSON DB 10121 PG 543 DB 2856 PG 539 <u>-L-</u> +95.00 553.00′ RT **INSET** -L- + 59.00 407.00' RT В PDE

STANDARD BASE DITCH
WCLASS I RIP RAP
TRANSITION BASE FROM
6' TO 12'
SEE DETAIL 8-4
SLOPE=0.51%
LENGTH=50'
RIP RAP=120 TONS
GEOTEXTILE=120 SY
DDE=100 CY BARBARA M. JOHNSON

PROJECT REFERENCE NO.

U-3422

SHEET NO.



PROJECT REFERENCE NO.

RW SHEET NO.

ROADWAY DESIGN
ENGINEER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

116 + 00.00

MATCHLINE SHEET

EDNA
CGRIFFIN
LOYD
DB 2134

JB 2134

STUART HITE.
ET UX
DB 2590 PG 325

U-3422

SHEET NO.

HYDRAULICS ENGINEER

10

PROJECT REFERENCE NO.

SHEET NO.

PROJECT REFERENCE NO. MATCHLINE BB SHEET 13A

SHEET 13 MATCHLINE AA SHEET 19A \_Y5\_\_ +20.00 /430.00′ LT PDE -48" RCP-IV MATCHLINE BB LLC B 9309 PG 149

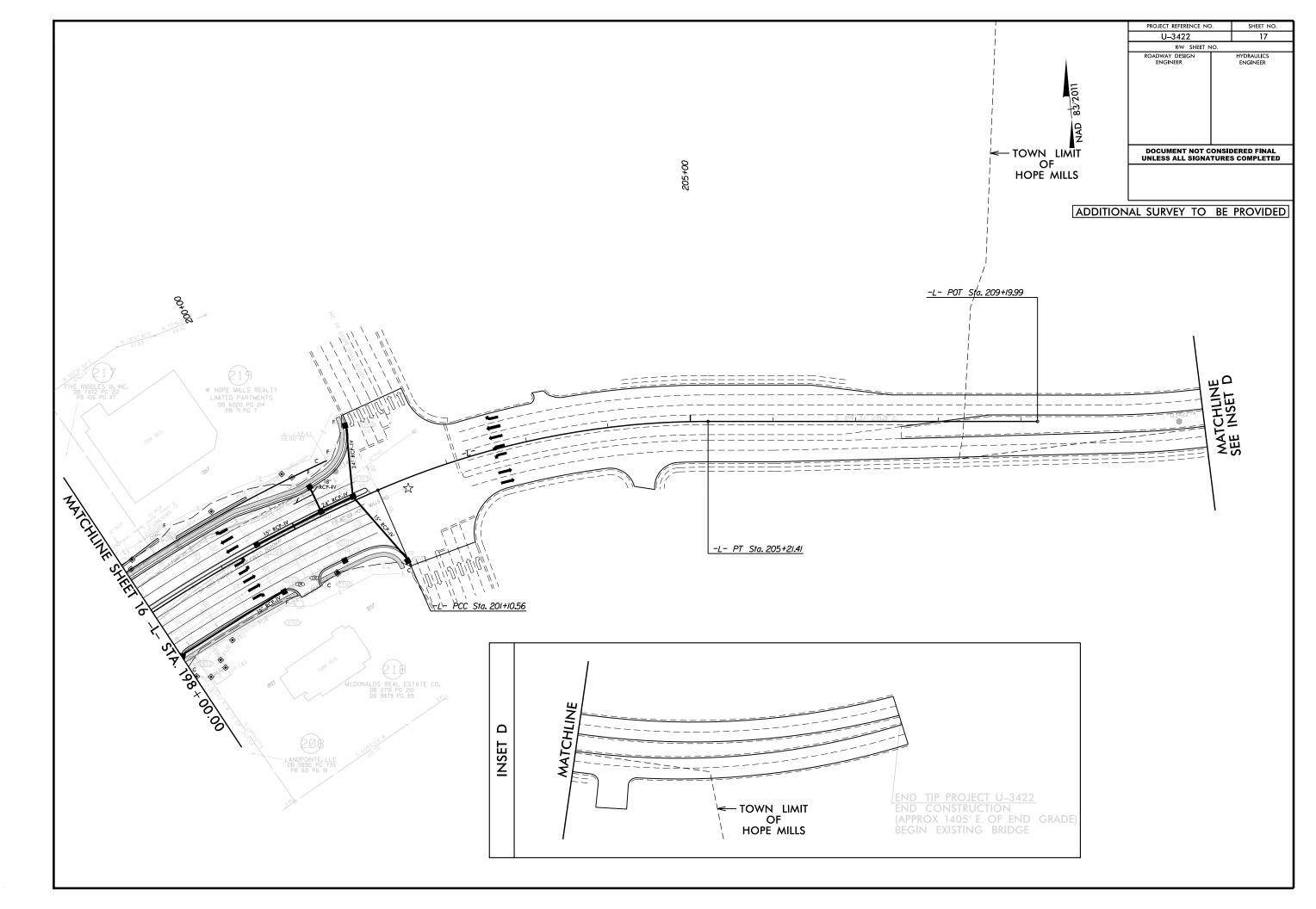
SHEET NO. PROJECT REFERENCE NO. U-3422 RW SHEET NO.

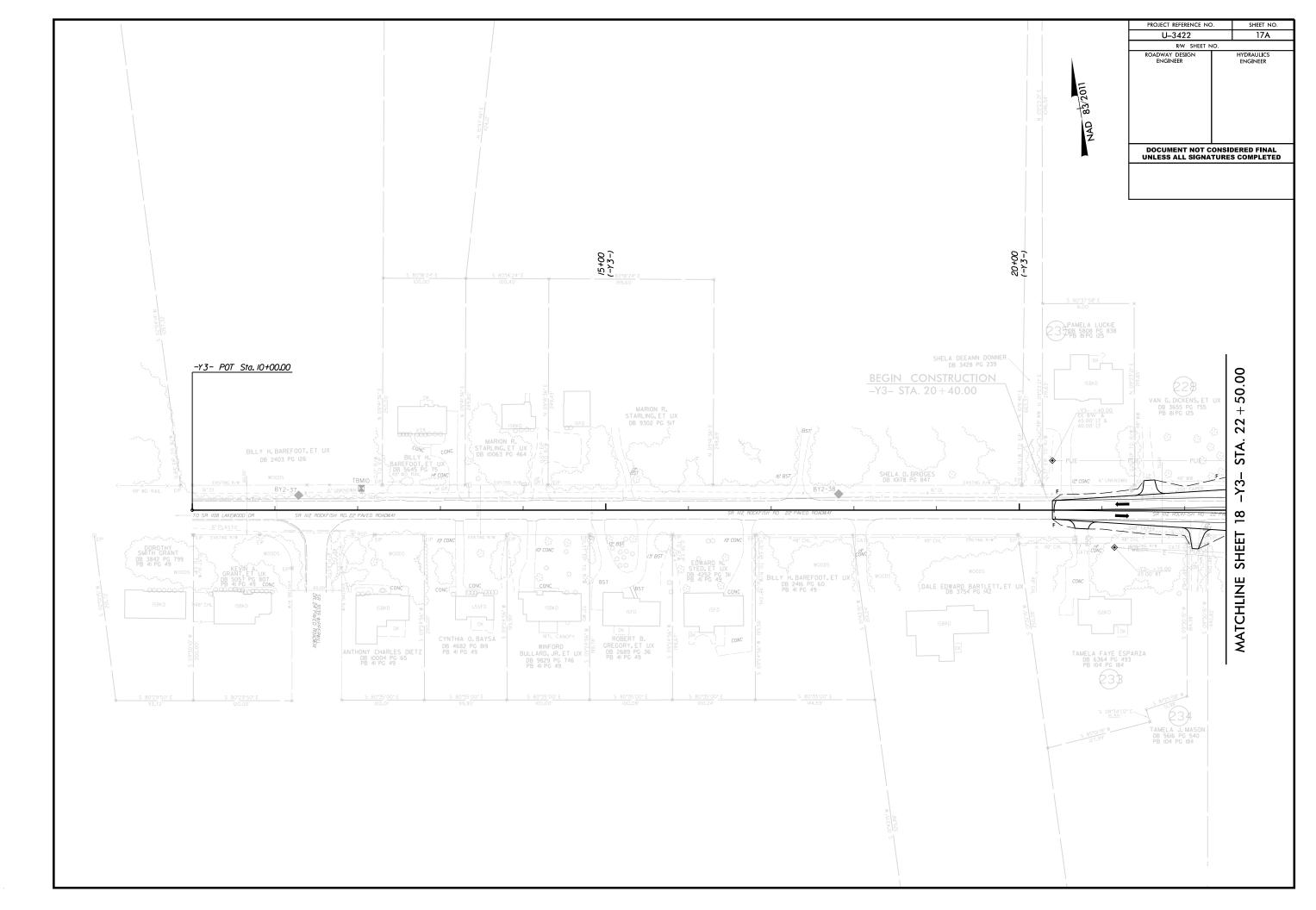
ROADWAY DESIGN
ENGINEER NAD 83/2011 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO.

JANICE C TERRY MATCHLINE SEE INSET C STANDARD 6' BASE DITCH SEE DETAIL 15–2 SLOPE=0.52% LENGTH=320' DDE=2120 CY DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED DWIGHT DAVIS LOURDES BELTRE STANDARD 6' BASE DITCH
W/CLASS I RIP RAP
SEE DETAIL 15-1
SLOPE=1-6.79%
LENGTH=180'
RIP RAP=360 TONS
GEOTEXTILE=360 SY
DDE=1600 CY STA. 184+00.00 170 + 00.00STA. SHEIT 18" RCP-IV SHEET MATCHLINE MATCHLINE EL TIO INVESTMENTS, LLC DB 8689 PG 571 PB 129 PG 161 JANICE C TERRY JANICE C TERRY MARTA C. BULLARD OWN LIMIT OF JANICE C TERRY HOPE MILLS <u>-L- +14.00</u> 510.00' LT <u>-L- +45.00</u> 828.00′ LT  $\circ$ **INSET** STANDARD 6' BASE DITCH SEE DETAIL 15–2 SLOPE= 0.51% LENGTH=293' DDE=1000 CY <u>-L-</u> +97.00 451.00′ LT B&T LAND DEVELOPMENT LLC

PROJECT REFERENCE NO. SHEET NO. U-3422 15 R/W SHEET NO. ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER 190



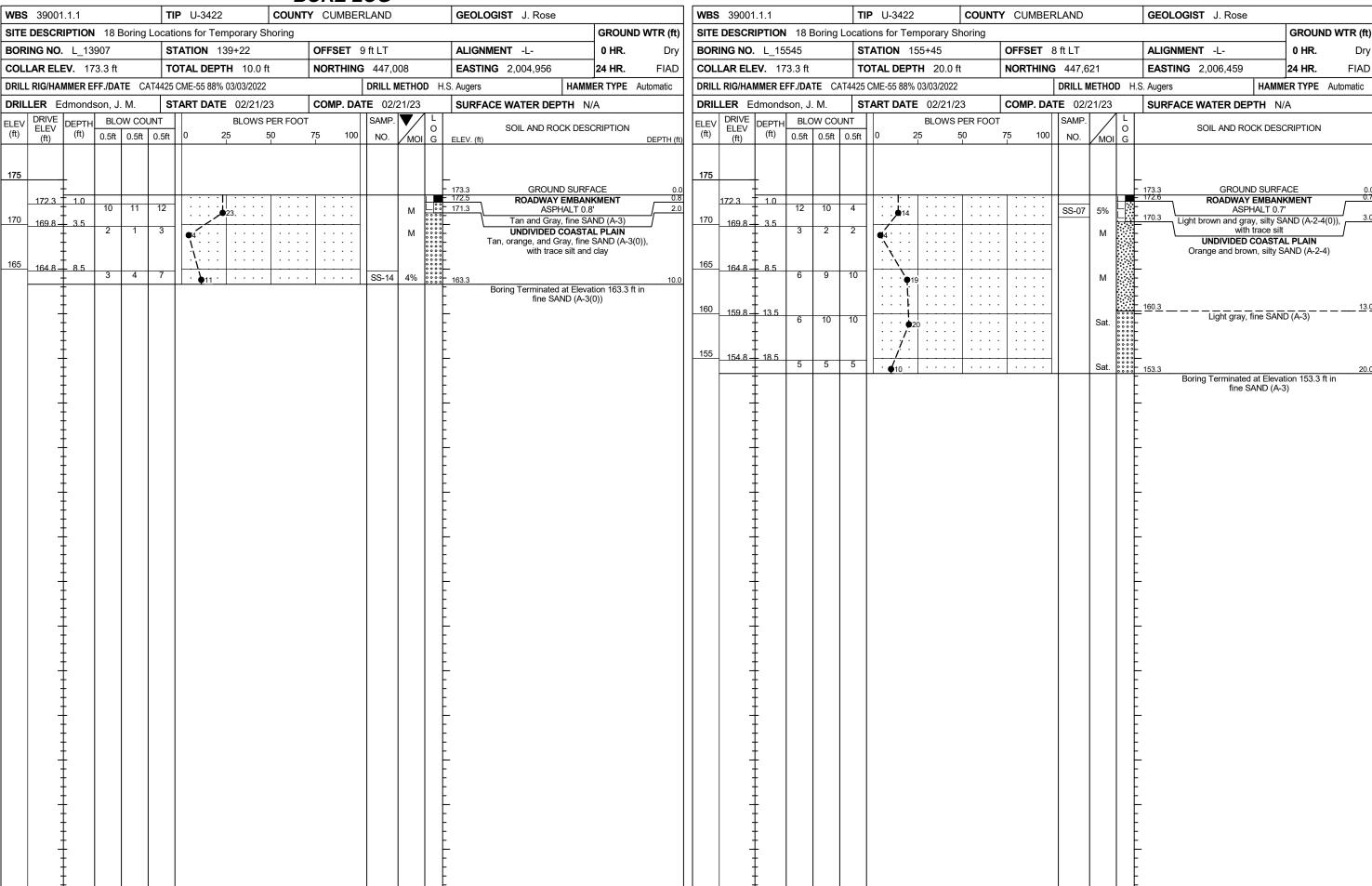


PROJECT REFERENCE NO. SHEET NO. U-3422 RW SHEET NO. ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER NAD 83/2011 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 25+00 (-Y3-) 30+00 (-Y3-) 35+00 (-Y3-) JAMES R. HARRIS, ET UX DB 2941PG 345 PB 39 PG 72 STA. 22 + 50.00STA. 36+50.00 -\3-MATCHLINE SHEET 11 -Y3-17A MATCHLINE SHEET HAMBLIN, ET UX DB 3724 PG 318 <u>-Y3- PT Sta. 36+34.9</u>1 HAMBLIN, ET UX DB 3854 PC 859 INNICUTT CORPORATION DB 9518 PG 348 DB 4916 PG 171 RANDY L. NOBLES, ET UX DB 9590 PG I8

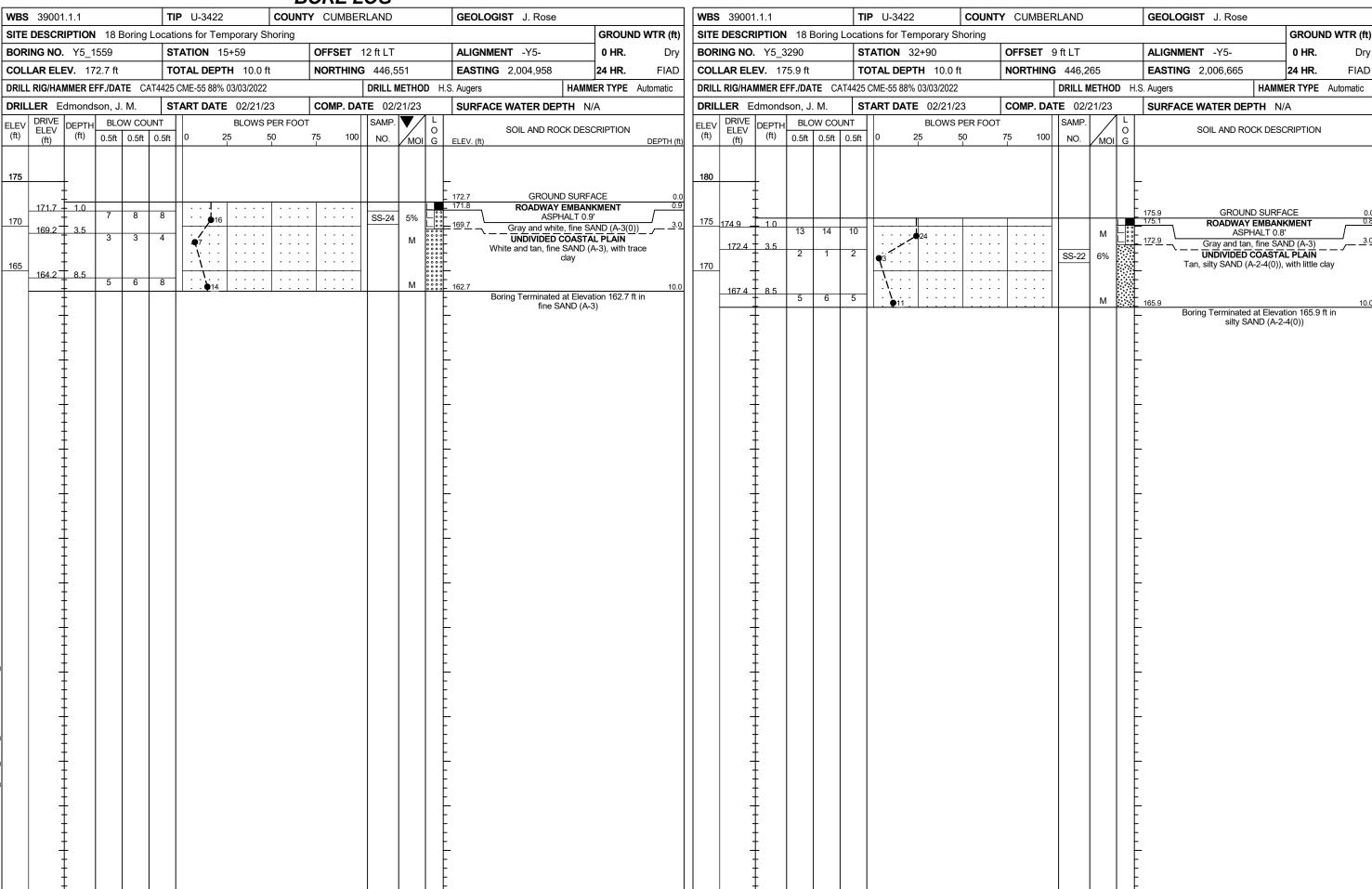
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USER: jholland

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	CUMBERLAND GEOLOGIST J. Rose				CUMBERLAND	GEOLOGIST J. Rose
SITE DESCRIPTION 18 Boring Locations for Temporary Shoring		GROUND WTR (ft)	SITE DESCRIPTION 18 Boring Locati	· · · · · · · · · · · · · · · · · · ·		GROUND WTR (ft)
<b>BORING NO.</b> L_7745 <b>STATION</b> 77+45 <b>OI</b>	DFFSET 5 ft LT ALIGNMENT -L-	<b>0 HR.</b> 5.8	<b>BORING NO</b> . L_12359 <b>S1</b>	STATION 123+59 OF	FSET 6 ft LT	ALIGNMENT -L- 0 HR. Dry
COLLAR ELEV. 173.5 ft TOTAL DEPTH 10.0 ft NO	NORTHING 445,716 EASTING 1,998,969	24 HR. FIAD	COLLAR ELEV. 184.7 ft	TOTAL DEPTH 10.0 ft NO	<b>DRTHING</b> 446,483	<b>EASTING</b> 2,003,487 <b>24 HR.</b> FIAD
<b>DRILL RIG/HAMMER EFF./DATE</b> CAT4425 CME-55 88% 03/03/2022	DRILL METHOD H.S. Augers	HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE CAT4425	5 CME-55 88% 03/03/2022	DRILL METHOD H.S	S. Augers HAMMER TYPE Automatic
	COMP. DATE 02/21/23 SURFACE WATER DE	PTH N/A			OMP. DATE 02/21/23	SURFACE WATER DEPTH N/A
ELEV   ORIVE   ELEV (ft)   O.5ft   O	SAMP. V L O SOIL AND R	DCK DESCRIPTION DEPTH (ft)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	BLOWS PER FOOT  5 0 25 50 75	100 NO. MOI G	SOIL AND ROCK DESCRIPTION
175		ND SURFACE 0.0 Y EMBANKMENT 0.7	185 183.7 - 1.0 8 4 3			184.7
170 170.0 3.5 2 2 3 0 5	M SS-19 18% AS Gray, orange, and 167.0 Brown and tar	PHALT 0.7'  tan, fine SAND (A-3), with ,3.0 ome silt  LUVIAL silty SAND (A-2-4(0)) ,6.5		T (0		Tan, fine SAND (A-3), with trace silt, with trace clay  Tan, clayey SAND (A-2-6(0))
165 165.0 8.5	Dark Brown, silty organics, cor	SAND (A-2-4), with little ains wood fragments  d at Elevation 163.5 ft in AND (A-2-4)	175 5 7 9	<b>」</b> │	SS-17 15%	Boring Terminated at Elevation 174.7 ft in clayey SAND (A-2-6(0))  - 174.7



_											В	UH	RE L	<u>.UC</u>	7																																				
W	<b>3S</b> 3	9001.	1.1			T	<b>P</b> U-3	3422		С	OUNT	<b>Y</b> C	JMBE	RLAN	)		G	EOLO	GIST	J. Ros	e							3900						I <b>P</b> U-3					CUM	BERI	LAND				GEOLO	OGIST	J. R	ose			
$\vdash$					Boring		ions fo				ring														NTR (ft	`   ├─						oring		ions for			Shori	<del>-</del>													D WTR (f
В	RING	NO.	L_170	079			ΓΑΤΙΟΙ					_	SET				A	LIGNN	ENT	-L-			0 HR.		Dr			NG NO						TATION					OFFSE	<b>T</b> 6	ft LT			$\rightarrow$	ALIGNI				0	HR.	Dr
_			<b>/</b> . 167				DTAL [					NOF	RTHING					ASTIN	<b>G</b> 2,0	07,840			24 HR.		FIAD	$\dashv \vdash$		AR EL						OTAL D					NORTI						EASTIN		2,008,8			HR.	FIA
_							CME-55										H.S. A							E Au	tomatic	$\dashv$								CME-55										_	Augers					TYPE	Automatic
			monds T				TART I	DATE					/IP. DA					URFA	E WA	TER DI	EPTH	N/A	١					LER						TART D					COMP				3 // /	$\perp$	SURFA	ICE W	ATER	DEPTH	H N/A		
EL (1	EV E	EV [ft]	EPTH_ (ft)	0.5ft	0.5ft	0.5ft	0	25		VS PEF 50	R F001	75 	100	I I	P. N	/   C	ELI	≣V. (ft)	SOII	AND F	ROCK I	DESCF	RIPTIO		DEPTH (		EV ft)	DRIVE ELEV (ft)	DEF (fi	PTH 0	.5ft (	0.5ft	O.5ft	0	25		50 	FOOT	75 	100	SAMP.	1 /	O OI G			S	OIL AND	D ROCK	DESCRI	PTION	
1	0	_															-									1 1	70 1	168.8	<u> </u>	0	15	14	14								00.04	<u> </u>		1	169.8		ROADV		SURFACE		(
10	5	36.0		5	6	5						-			N.	i i	167	5.3		GROU OADWA AS	AY EMI	<b>BANKI</b> T 0.7'	MENT			0 7 1	65	166.3	<sup>∓</sup> 3.	5		2	3			28	: :				SS-01	M M	000	1	166.8		n, silty SA	AND (A-2	2-4(0)), w ASTAL P A-3), with	LAIN	<u>silt</u> 3
11		3.5	3.5	2	2	1	√3 √3 √3.				· · · · · · · · ·	:   :			N	1		~~_	UN	DIVIDE Brown,	D COA	STAL	PLAIN	l	<u>3</u>		60	161.3	‡ 8.	5	9	8	8	: \			.   .					   M	000000000000000000000000000000000000000			2.0.	,	<i></i>	,		
10		58.5	8.5	3	5	6	, i	 D11	: : :					SS-	06 22	%	157		)i T			-1	457	0.6:	10		-		<del> </del> -			0			<b>⊉</b> 16 <u>↓</u>							IVI	000	-	159.8	Borinç	g Termin fi	nated at ine SAN	Elevation D (A-3)	159.8 ff	in 10
		‡																1	soring i	erminat silty S	SAND (A	A-2-4(0	)))	O IL III					‡ ‡															F							
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## CAMDEN ROAD WIDENING FROM FUTURE FAYETTEVILLE OUTER LOOP TO NC 59 (U-3422)

	-L- SOIL TEST RESULTS														
SAMPLE	STATION	OEESET	DEPTH INTERVAL	P.I.		% BY	WEIGHT		% PAS	SING (SI	EVES)	%	%		
NO.	STATION	OI I SLI	DEFINITERVAL	CLASS.	L.L	F.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	<b>ORGANIC</b>
SS-19	77+45	5' LT	3.5-5.0	A-2-4(0)	NP	NP	54.8	28.7	8.9	7.6	95.2	57	18	18	-
SS-20	77+45	5' LT	8.5-10.0	-	-	-	-	-	-	-	-		-	-	3.8
SS-17	123+59	6' LT	8.5-10.0	A-2-6(0)	33	14	65.6	14.8	2.9	16.7	97.9	59	20	15	-
SS-14	139+22	9' LT	8.5-10.0	A-3(0)	NP	NP	71.1	21.2	1.8	5.8	97.3	50	8	4	-
SS-07	155+45	8' LT	1.0-2.5	A-2-4(0)	NP	NP	46.2	41.2	6.6	6.0	99.9	74	14	5	-
SS-06	170+79	6' LT	8.5-10.0	A-2-4(0)	NP	NP	4.9	80.5	1.6	13.0	100	98	15	22	-
SS-01	182+13	6' LT	1.0-2.5	A-2-4(0)	NP	NP	39.1	51.4	4.5	5.0	99.7	81	11	5	-

						-Y5- SOIL	. TEST I	RESUL	TS						
SAMPLE	STATION	OFFSET	DEPTH INTERVAL	AASHTO	1.1	P.I.		% BY	WEIGHT		% PAS	SING (SI	IEVES)	%	%
NO.	STATION	OFFSET	DEPTHINTERVAL	CLASS.	L.L	F.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	<b>ORGANIC</b>
SS-24	15+59	12' LT	1.0-2.5	A-3(0)	NP	NP	27.2	65.0	2.8	5.0	100	88	9	5	-
SS-22	32+90	9' LT	3.5-5.0	A-2-4(0)	NP	NP	46.3	42.5	2.2	9.0	99.8	76	12	6	-