

NOTES

A WAITING PERIOD IS NOT REQUIRED FOR THE CONSTRUCTION OF END BENT 1 AND END BENT 2.

ALL PILES ARE HP 12 x 53 STEEL PILES.

PILE DIMENSIONS ARE SHOWN TO THE CENTERLINE OF THE PILES AT THE BOTTOM OF THE END BENT CAPS.

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DO NOT DEWATER DRILLED PIER EXCAVATIONS AT BENT 1 AND BENT 2. CLEAN THE BOTTOM OF EXCAVATIONS WITH A SUBMERSIBLE PUMP OR AN AIRLIFT. WET PLACEMENT OF CONCRETE IS REQUIRED.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFCATIONS.

> BR-0063 PROJECT NO. **ANSON** COUNTY

20 + 15.00 - L -STATION:

SHEET 2 OF 3

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

SEAL

MCINEER.

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046289

10/2/2024 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

BRIDGE OVER RICHARDSON CREEK ON NC 742 BETWEEN SR 1459 AND SR 1454

REVISIONS SHEET No. S-2 DATE: DATE: TOTAL SHEETS

ARCADIS 175 REGENCY WOOD PLACE, SUITE 400 CARY, NORTH CAROLINA 27518 NC LICENSE No. 0486277 OR NORTH CAROLINA DEPARTMENT OF TRANSPORTAT

K. E. LOFTON DATE : 6–22 CHECKED BY : G. P. HOOVER DATE : 9-23 ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

SUIMMARY OF PILLE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

Fred Board		Pile Cut-Off (Top of Pile) Elevation FT		Scour Critical Elevation FT		Driven Piles			Predrilling for Piles*		Ī	Orilled-In Piles	
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS		Estimated Pile Lenth per Pile FT		Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile Exc In Soil per Pile Lin FT
End Bent 1, Piles 1-7	126	296.46	30			210							
End Bent 2, Piles 1-7	126	298.88	25			210]						
]						

^{*}Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile TONS	Factored Downdrag Load per Pile TONS	Factored Dead Load* per Pile TONS	Dynamic Resistance Factor	Nominal Downdrag Resistance per Pile TONS	Nominal Scour Resistance per Pile TONS	Scour Resistance Factor (Default = 1.00)
End Bent 1, Piles 1-7	126			0.60			
End Bent 2, Piles 1-7	126			0.60			

^{*}Factored Dead Load is factored weight of pile above the ground line.

SUMMARY OF DRILLED PIER INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

Bent 1, Piers 1-3 650 Bent 2, Piers 1-3 650										
Bent 2, Piers 1-3 650	246.0	15	265	6.0		16.7	10.1	Yes	268.1	4.7
	246.0	50	262	6.0		16.3	10.5	Yes	265.4	7.4
	<u> </u>									
TOTAL QTY:						99	62			37

^{*}Permanent Steel Casing Length equals the difference between the ground line or top of drilled pier elevation, whichever is higher, and the permanent casing tip elevation.

SUIMMARY OF PIDA/PILLE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

F	Pile Driving Analyz		Pile Order Lengths			
End Bent/ Bent No	PDA Testing Required? YES or MAYBE	PDA Test Pile Length FT	Total PDA Testing Quantity EACH	End Bent/ Bent No(s)	Pile Order Length Basis* EST or PDA	
End Bent 1	Yes					
End Bent 2	Yes		1			
			2			

^{*}EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

SUMMARY OF PILE ACCESSORIES

(Blank entries indicate item is not applicable to structure)

Find Donati	Dina Dila	s	Steel Pile Points							
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates Required? YES or MAYBE	Pipe Pile Cutting Shoes Required? YES	Pipe Pile Conical Points Required? YES	H-Pile Points Required? YES	Steel Pile Tips Required? YES					
End Bent 1, Piles 1-7				Yes						
End Bent 2, Piles 1-7				Yes						
			<u> </u>							
TOTAL OTV				4.4						
TOTAL QTY:				14						

SUIMMARY OF IDRILLED PIER TESTING

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pier(s) #-# (e.g., "Bent 1, Piers 1-3")	Standard Penetration Test (SPT) Required? YES or MAYBE	Crosshole Sonic Logging (CSL) Required?* YES or MAYBE	Total CSL Tube Length (For All Tubes) per Pier Lin FT	Shaft Inspection Device (SID) Required? YES or MAYBE	Pile Integrity Test (PIT) Required MAYBE
Bent 1, Piers 1-3		Maybe	114	Maybe	
Bent 2, Piers 1-3		Maybe	114	Maybe	
TOTAL QTY:		1	684	1	

*CSL Tubes are required if CSL Testing is or may be required. The number of CSL Tubes per drilled pier is equal to one tube per foot of design pier diameter with at least 4 tubes per pier. The length of each CSL Tube is equal to the drilled pier length plus 1.5 ft.

PROJECT NO.	<u>67063.1.</u>	1 (BR-0063)
	Anson	COUNTY
STATION:	STA. 20+	15.00 -L-

NOTES:

- 1. The Pile and Drilled Pier Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Yinhui Liu, PE # 034020) on 8-17-2023.
- 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
- 3. The Engineer will determine the need for PDA Testing, CSL Testing, and SID Inspections when these items may be required.

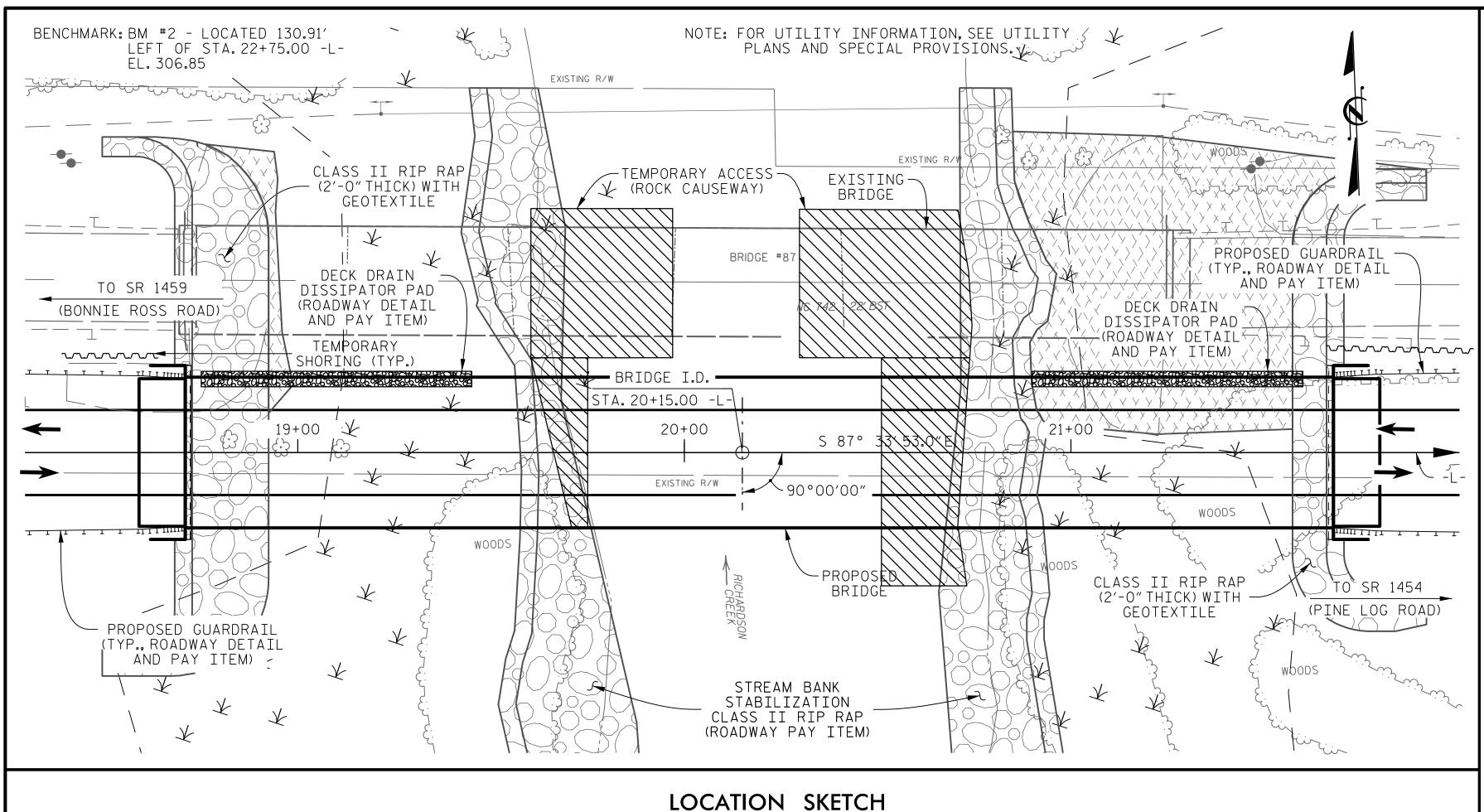
SEAL 046289 VGINE Signed by: Hua P Horoce PE D899D787261D49A	10/2/2024		PILE	E AI
SIGNATURE	DATE			RI
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STATE OF NORTH CAROLINA RTMENT OF TRANSPORTATION

ND DRILLED PIER FOUNDATION **TABLES**

SIGNATURE DATE			SHEET NO. S-2A				
DCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL
FINAL UNLESS ALL	1			3			SHEETS
SIGNATURES COMPLETED	2			1			10

Factored Resistance + Factored Downdrag Load + Factored Dead Load + Nominal Downdrag Resistance + Nominal Scour Resistance Factor Nominal Scour Resistance



NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE, AND REMOVAL OF TEMPORARY ACCESS AT STATION 20+15.00 -L-.

FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS, FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE ROADWAY PLANS.

FOR SURVEY CONTROL SHEET, SEE ROADWAY PLANS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 35'-0" EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN THE ROADWAY QUANTITY ON ROADWAY PLANS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF 6 SPANS @ 42'-6"EACH WITH A CLEAR ROADWAY WIDTH OF 24'-O"ON RC DECK GIRDERS; END BENT 1 CONSISTING OF 2 SLOPED COLUMN ON SPREAD FOOTING, END BENT 2 CONSISTING OF RC CAP ON PCC PILES, INTERIOR BENTS CONSISTING OF 2 COLUMN RC POST & BEAM BENT ON SPREAD FOOTINGS WITH CONCRETE STRUTS AT BENTS 2, 3, AND 4; AND LOCATED AT THE SITE OF PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

						- TOT	'AL BILI	OF MAT	ERIAL -						
	REMOVAL O EXISTING STRUCTURE STA. 20+15.00	MAIN AT & RE -L- TEMP	TRUCTION, TENANCE, MOVAL OF ACCESS AT)+15.00 -L-	ASBEST ASSESSM		4'-0"Ø DRILLED PIERS IN SOIL	4'-0"Ø DRILLED PIERS NOT IN SOIL		SID INSPECTION	CSL TESTING	ST EXCA	LASSIFIED TRUCTURE AVATION AT 20+15.00 -L-	CLASS A CONCRETE	SL	APPROAC ABS AT 0+15.00 -L
	LUMP SUM	LU	MP SUM	LUMP S	UM	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	LI	UMP SUM	CU. YDS.	LUI	MP SUM
SUPERSTRUCTURE															
END BENT 1													31.8		
BENT 1						30.3	50.1	14.1					49.5		
BENT 2						31.5	48.9	22.2					50.3		
END BENT 2													31.8		
TOTAL	LUMP SUM	LU	MP SUM	LUMP S	UM	61.8	99.0	36.3	1	1	LI	UMP SUM	163.4	LUI	MP SUM
	REINFORCING STEEL	SPIRAL COLUMN REINFORCIN STEEL	PILE DE EQUIPMEN FOR HP STEEL	IT SETUP 12 x 53		12 x 53 EL PILES	STEEL PILE POINTS	DYNAMIC PILE TESTING	VERTICAL CONCRETE BARRIER RAI	RIP CLAS IL (2'-0"	SII	GEOTEXTILE FOR DRAINAGE	ELASTOME BEARIN	IGS PF	-0"x 3'-3 RESTRESSE CONCRETE OX BEAMS
	LBS.	LBS.	EAG	CH	No.	LIN.FT.	EACH	EACH	LIN.FT.	TO	N	SQ. YD.	LUMP S	SUM No	LIN.FT
SUPERSTRUCTURE									600.0					3'	3,900.0
END BENT 1	4,545		7		7	210.0	7			32	25	361			
BENT 1	14,420	3,838													
BENT 2	14,620	3,947													
END BENT 2	4,545		7		7	175.0	7			17	2	191			
TOTAL	70 170	7 705	1	1	14	70F ^	1 /	2	(00.0	49	7	552	LUMP	71111	3,900.0
TOTAL	38,130	7 , 785	14	1	14	385.0	14	2	600.0	45	<i>J</i> 1	222	LUMP S		J J,300.(

BR-0063 PROJECT NO. **ANSON** COUNTY

20 + 15.00 - L -STATION:

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

UNLESS ALL SIGNATURES COMPLETED **BRIDGE OVER RICHARDSON** CREEK ON NC 742 BETWEEN SR 1459 AND SR 1454

NC LICENSE No. 0486277

SEAL 04628 NCINE	7
Hiva P Hoovoz PE D899D787261D49A	10/23/2024

DOCUMENT NOT CONSIDERED FINAL

			REV	'ISIONS	ı		SHEET No
	No.	BY:	DATE:	No.	BY:	DATE:	S –3
	1			3			TOTAL SHEETS
4	2			1			10

K. E. LOFTON DATE : 9–23 G. P. HOOVER DATE : 9-23 ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

							STRENGTH I LIMIT STATE						SERVICE III LIMIT STATE											
					MOMENT SHEAR				MOMENT															
LOAD TYPE		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft,)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft,)	LIVE-LOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft.)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.035		1.75	0.272	1.26	100′	EL	49.25	0.489	1.34	100′	EL	4.925	0.80	0.272	1.04	100′	EL	49.25	
DESIGN		HL-93 (OPERATING)	N/A		1.633		1.35	0.272	1.63	100′	EL	49.25	0.489	1.73	100′	EL	4.925	N/A						
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.44	51.84	1.75	0.272	1.75	100′	EL	49.25	0.489	1.81	100′	EL	4.925	0.80	0.272	1.44	100′	EL	49.25	
IVATINO	_	HS-20 (OPERATING)	36.000		2.271	81.756	1.35	0.272	2.27	100′	EL	49.25	0.489	2.35	100′	EL	4.925	N/A						
	SINGLE VEHICLE (SV)	SNSH	13.500		3.413	46.079	1.4	0.272	5.19	100′	EL	49.25	0.489	5.59	100′	EL	4.925	0.80	0.272	3.41	100′	EL	49.25	
		SNGARBS2	20.000		2.473	49.452	1.4	0.272	3.76	100′	EL	49.25	0.489	3.91	100′	EL	4.925	0.80	0.272	2.47	100′	EL	49.25	
		SNAGRIS2	22.000		2.313	50.885	1.4	0.272	3 . 52	100′	EL	49.25	0.489	3.6	100′	EL	4.925	0.80	0.272	2.31	100′	EL	49.25	
		SNCOTTS3	27.250		1.696	46.228	1.4	0.272	2.58	100′	EL	49.25	0.489	2.78	100′	EL	4.925	0.80	0.272	1.70	100′	EL	49.25	
		SNAGGRS4	34.925		1.39	48.556	1.4	0.272	2.11	100′	EL	49.25	0.489	2.26	100′	EL	4.925	0.80	0.272	1.39	100′	EL	49.25	
		SNS5A	35.550		1.361	48.398	1.4	0.272	2.07	100′	EL	49.25	0.489	2.27	100′	EL	4.925	0.80	0.272	1.36	100′	EL	49.25	
		SNS6A	39.950		1.238	49.456	1.4	0.272	1.88	100′	EL	49.25	0.489	2.05	100′	EL	4.925	0.80	0.272	1.24	100′	EL	49.25	
LEGAL		SNS7B	42.000		1.178	49.496	1.4	0.272	1.79	100′	EL	49.25	0.489	2	100′	EL	4.925	0.80	0.272	1.18	100′	EL	49.25	
LOAD	ER	TNAGRIT3	33.000		1.506	49.709	1.4	0.272	2.29	100′	EL	49.25	0.489	2.46	100′	EL	4.925	0.80	0.272	1.51	100′	EL	49.25	
RATING	RAIL	TNT4A	33.075		1.510	49.942	1.4	0.272	2.3	100′	EL	49.25	0.489	2.41	100′	EL	4.925	0.80	0.272	1.51	100′	EL	49.25	
	L-IW	TNT6A	41.600		1.224	50.926	1.4	0.272	1.86	100′	EL	49.25	0.489	2.09	100′	EL	4.925	0.80	0.272	1.22	100′	EL	49.25	
	SE ST)	TNT7A	42.000		1.225	51.442	1.4	0.272	1.86	100′	EL	49.25	0.489	2.05	100′	EL	4.925	0.80	0.272	1.22	100′	EL	49.25	
	TOR (TT)	TNT7B	42.000		1.254	52.657	1.4	0.272	1.91	100′	EL	49.25	0.489	1.96	100′	EL	4.925	0.80	0.272	1.25	100′	EL	49.25	
	TRAC	TNAGRIT4	43.000		1.203	51.711	1.4	0.272	1.83	100′	EL	49.25	0.489	1.91	100′	EL	4.925	0.80	0.272	1.20	100′	EL	49.25	
		TNAGT5A	45.000		1.139	51.236	1.4	0.272	1.73	100′	EL	49.25	0.489	1.87	100′	EL	4.925	0.80	0.272	1.14	100′	EL	49.25	
	TRUC	TNAGT5B	45.000	3	1.129	50.805	1.4	0.272	1.72	100′	EL	49.25	0.489	1.82	100′	EL	4.925	0.80	0.272	1.13	100′	EL	49.25	
EMERGENC	Υ	EV2	28.750		2.129	61.213	1.3	0.272	2.87	100′	EL	49.25	0.489	3.06	100′	EL	4.925	0.80	0.272	2.13	100′	EL	49.25	
VEHICLE		EV3	43.000	4	1.403	60.325	1.3	0.272	1.89	100′	EL	49.25	0.489	2.06	100′	EL	4.925	0.80	0.272	1.40	100′	EL	49.25	

98'-6" SPAN A 98'-6" SPAN B 98'-6" SPAN C (BRG. TO BRG.) (BRG. TO BRG.) (BRG. TO BRG.) END BENT 1 END BENT 2 BENT 1 BENT 2 LRFR SUMMARY

LOAD FACTORS

LIMIT STATE YDC 1.25 1.50 STRENGTH I RATING FACTORS SERVICE III | 1.00 | 1.00

NOTES

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

4 EMERGENCY VEHICLE LOAD RATING ** ** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SEAL 046289

10/2/2024 2

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

BR-0063 PROJECT NO. **ANSON** COUNTY

STATION: ____20+15.00__L_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

LRFR SUMMARY FOR 100' BOX BEAM UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

SHEET No.	revisions									
S-4	DATE:	BY:	No.	DATE:	BY:					
TOTAL SHEETS			3							
10			1							

K. E. LOFTON DATE : <u>5–22</u> CHECKED BY : G. P. HOOVER DATE : 9-23 ENGINEER OF RECORD: G. P. HOOVER DATE : 9-23

PLANS PREPARED BY :
ARCADIS NC LICENSE No. 0486277

STD. No. 39LRFR1_90S_100L

ASSEMBLED BY: K.E.LOFTON DATE: 5-22 CHECKED BY: G.P.HOOVER DATE: 9-23 DRAWN BY: TMG 11/11 REV. 06/23 APK/AAI CHECKED BY: AAC 11/11

ASSEMBLED BY: K.E.LOFTON DATE: 6-22

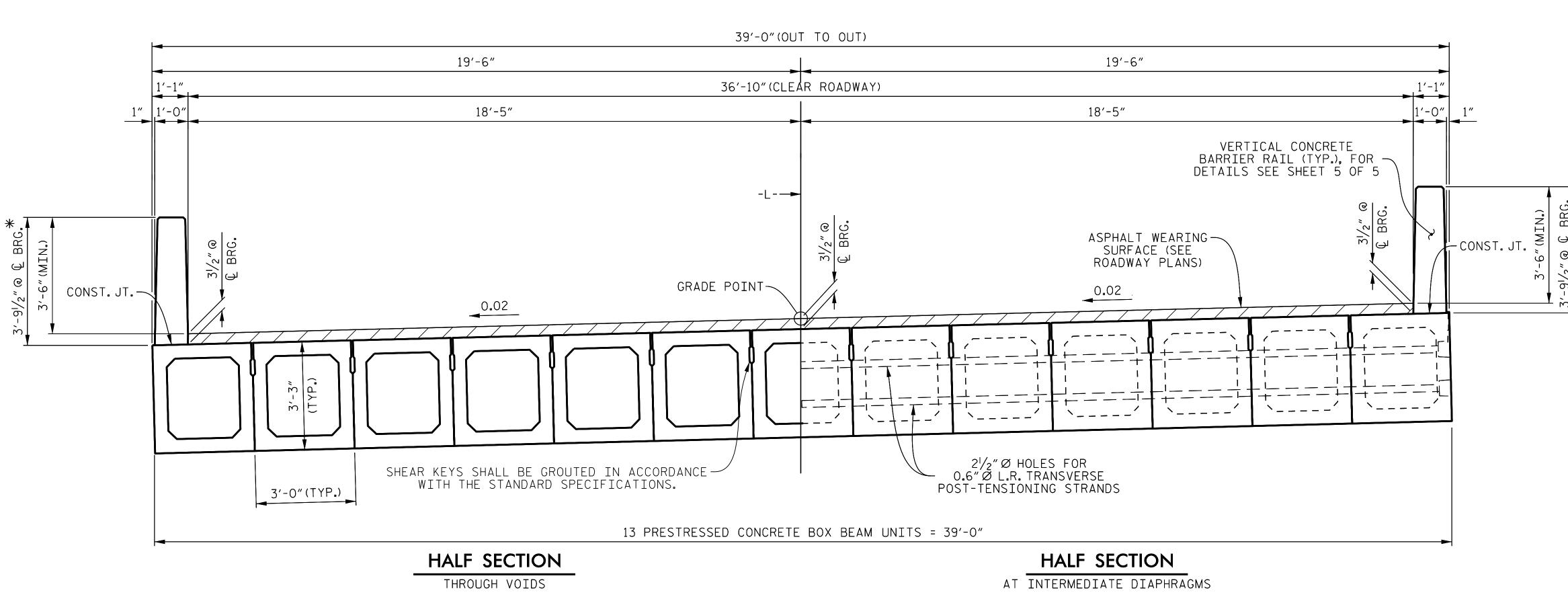
DATE: 9-23

REV. 10/15 MAA/TMG

CHECKED BY : G. P. HOOVER

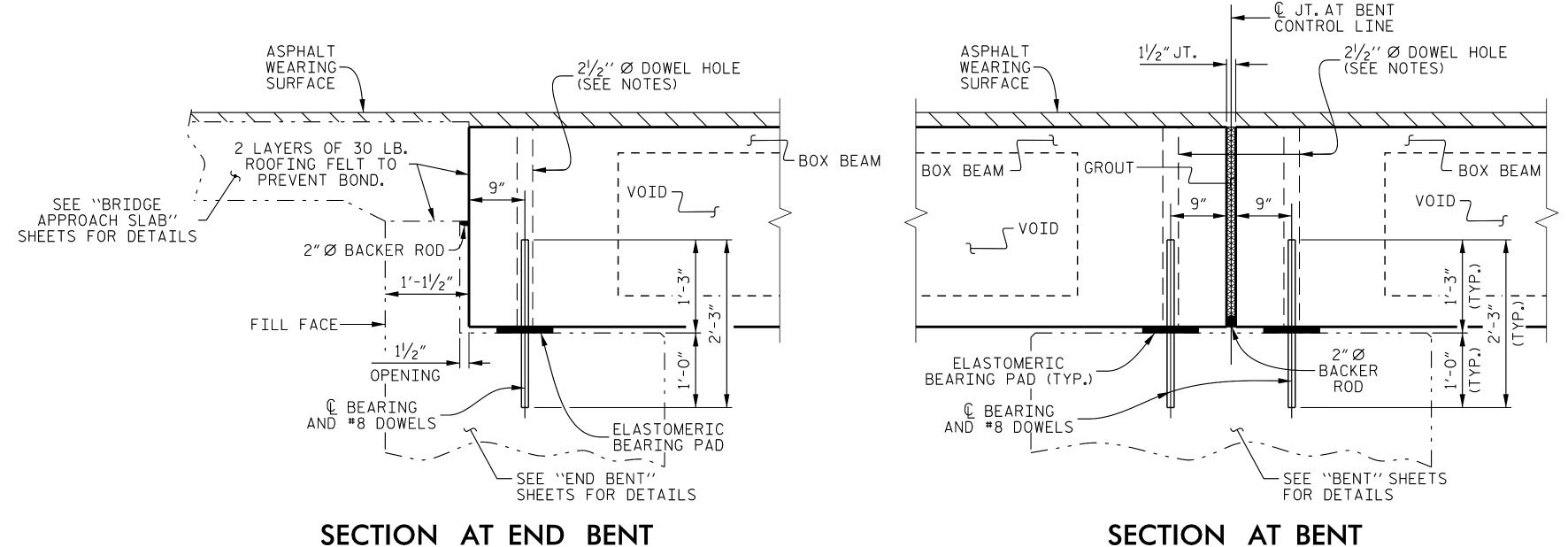
DRAWN BY: DGE 8/11

CHECKED BY : TMG 11/11



TYPICAL SECTION

*- THE MAXIMUM BARRIER RAIL HEIGHT AND CONCRETE OVERLAY THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND CONCRETE OVERLAY THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND CONCRETE OVERLAY THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL DETAILS", SHEET 5 OF 5.



PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF CORED SLAB UNITS 1 AND 11, RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.

THREADED INSERT DETAIL

DRAWN BY: K. E. LOFTON DATE: 6-22
CHECKED BY: G. P. HOOVER DATE: 9-23
ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

FOR NORTH CAR

PLANS PREPARED BY:

ARCADIS

175 REGENCY WOOD PLACE, SUITE 400
CARY, NORTH CAROLINA 27518
NC LICENSE No. 0486277
FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6500 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE $5'' \times 6''$. THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

PROJECT NO. BR-0063

ANSON COUNTY

STATION: 20+15.00 -L-

SHEET 1 OF 5

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10/2/2024

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0" x 3'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT
90° SKEW

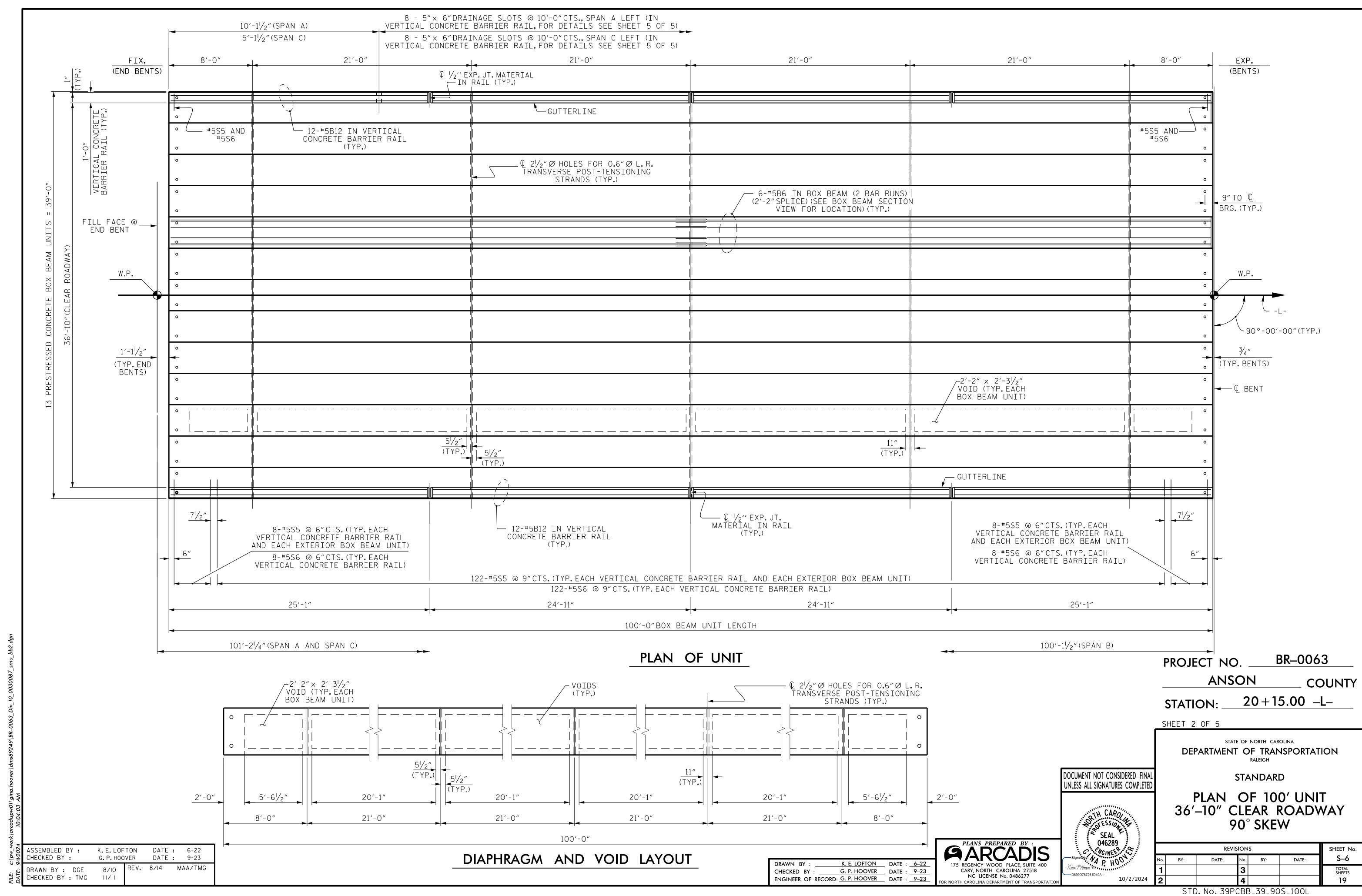
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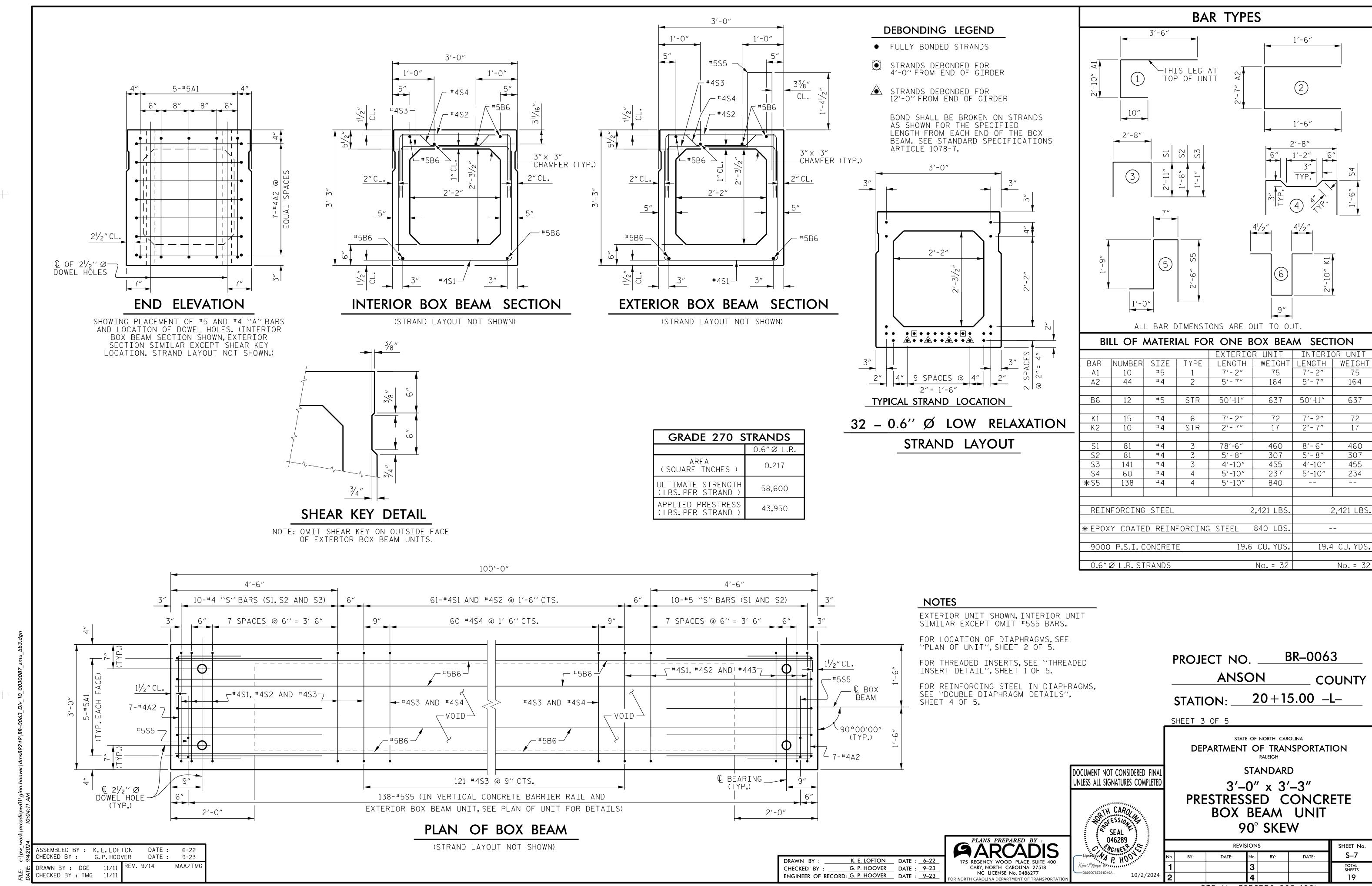
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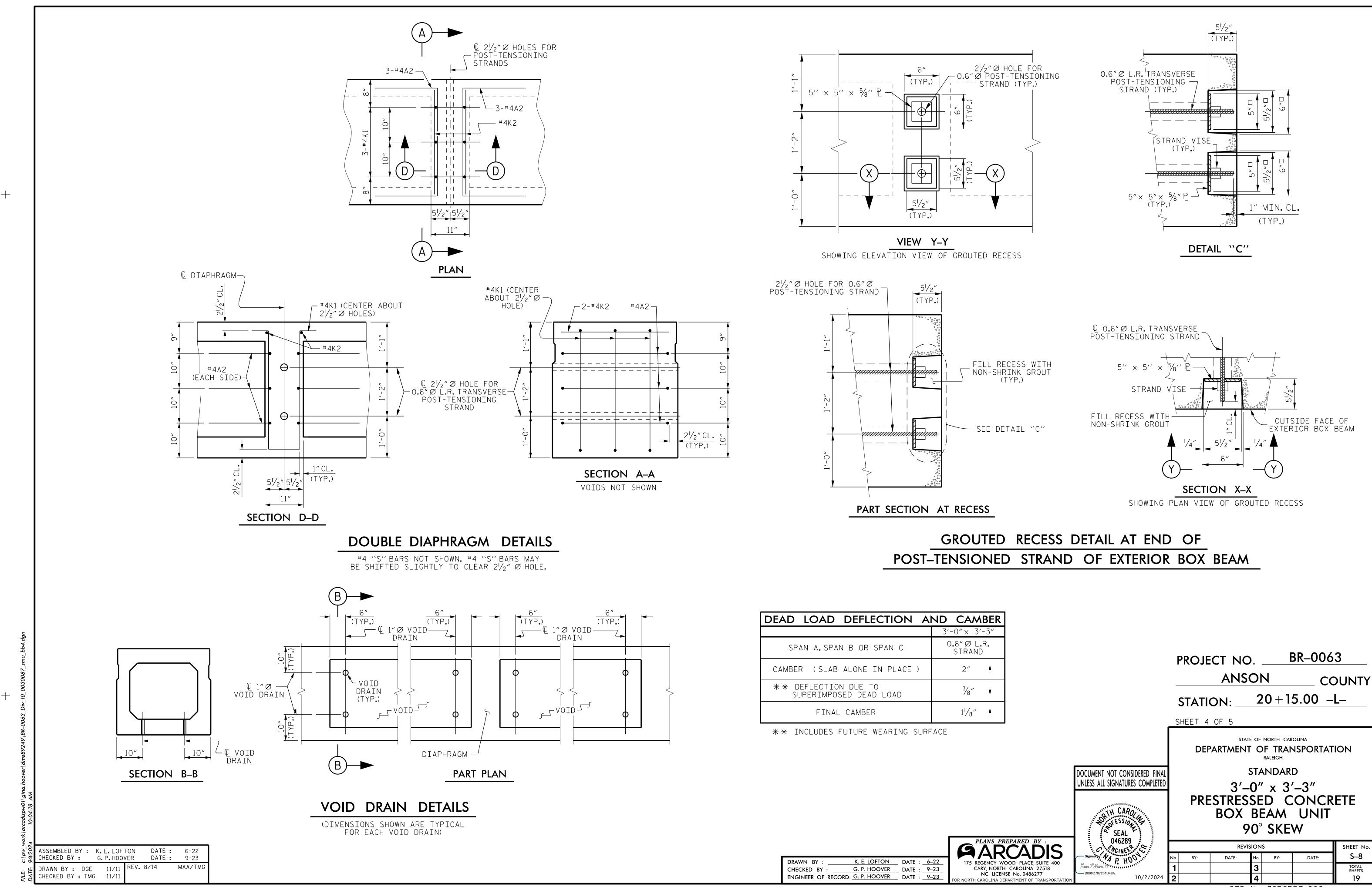
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STD. No. 39PCBB1_39

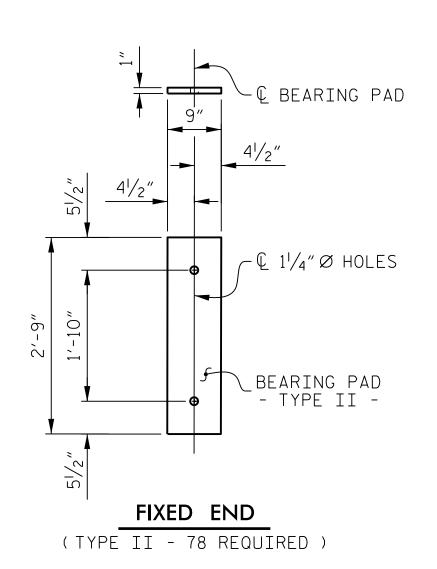




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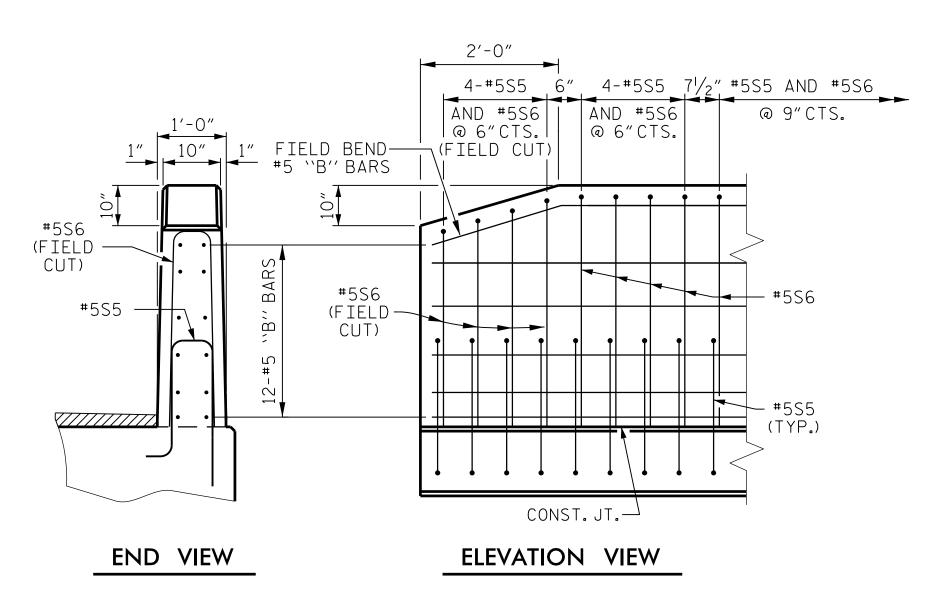


STD. No. 39PCBB7_90S



ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.



€ 1/2" EXP. JT. MATERIAL HELD IN PLACE WITH GALVANIZED NAILS.

(NOTE: OMIT EXP.JT. WHEN SLIP FORM IS USED.)

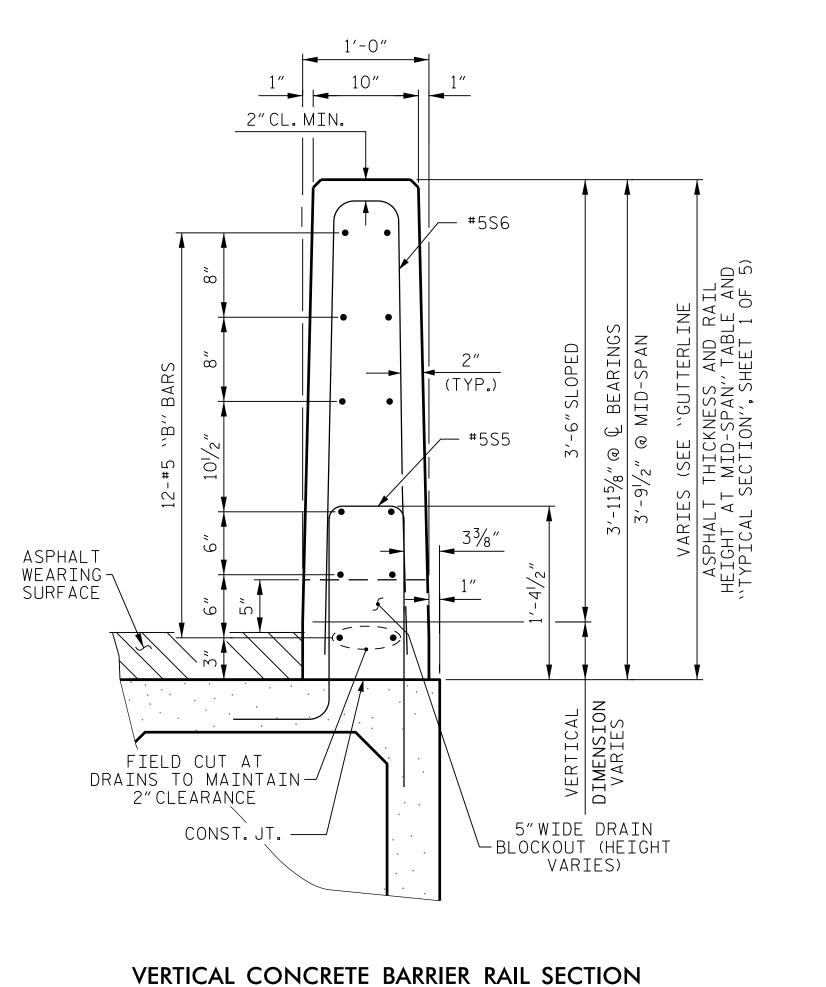
CHAMFER |

CHAMFER

CONST.JT.

ELEVATION AT EXPANSION JOINTS

END OF RAIL DETAILS

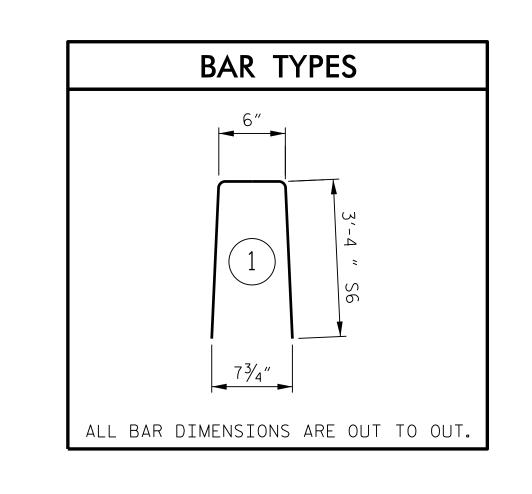


WHEN SLIP FORM IS USE

4				
9/4/202	ASSEMBLED BY: K, CHECKED BY: G	.E.LOFTON .P.HOOVER	DATE : DATE :	6-22 9-23
ATE:	DRAWN BY : DGE CHECKED BY : TMG	10/11 11/11 REV.	5/18	MAA/THC

VERTICAL CONCRETE BARRIER RAIL DETAILS

SECTION S-S



BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL								
BAR	BARS PER ONE EXTERIOR UNITS	TOTAL No.	SIZE	TYPE	LENGTH	WEIGHT		
	100'-0" UNIT							
∗ B12	48	96	#5	STR	24'- 7"	2,461		
* S6	138	276	#5	1	7'- 2"	2,063		
*EPOXY COATED REINFORCING STEEL 4,524 LBS.								
CLASS AA CONCRETE 25.9 CU. YDS.								
VERT	TICAL CONCRETE BARRIER RAIL				200 . 0 l	_IN.FT.		

BOX BEAMS REQUIRED									
SPAN A, SPAN B OR SPAN C	NUMBER	LENGTH	TOTAL LENGTH						
EXTERIOR BOX BEAM	2	100'-0"	200'-0"						
INTERIOR BOX BEAM	11	100'-0"	1,100'-0"						
TOTAL	13		1,300′-0″						

GUTTERLINE ASPHALT THICKNESS AND RAIL HEIGHT AT MID-SPAN							
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN					
LEFT	23/8"	3′-8 ³ ⁄ ₈ ″					
RIGHT	2 ³ / ₈ "	3'-83/8"					

PROJECT NO. BR-0063

ANSON COUNTY

STATION: 20+15.00 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALFIGH

STANDARD

3'-0" x 3'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT
90° SKEW

 REVISIONS
 SHEET No.

 BY:
 DATE:
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 BY:
 DATE:
 S-9

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 TOTAL SHEETS
 SHEET No.
 10

DRAWN BY: K. E. LOFTON DATE: 6–22
CHECKED BY: G. P. HOOVER DATE: 9–23
ENGINEER OF RECORD: G. P. HOOVER DATE: 9–23

PLANS PREPARED BY:

ARCADIS

175 REGENCY WOOD PLACE, SUITE 400
CARY, NORTH CAROLINA 27518
NC LICENSE No. 0486277

Aua P Hoovas PE 11 3

PORTATION 10/2/2024 2 4

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SEAL 046289

STD. No. 39PCBB8_90S

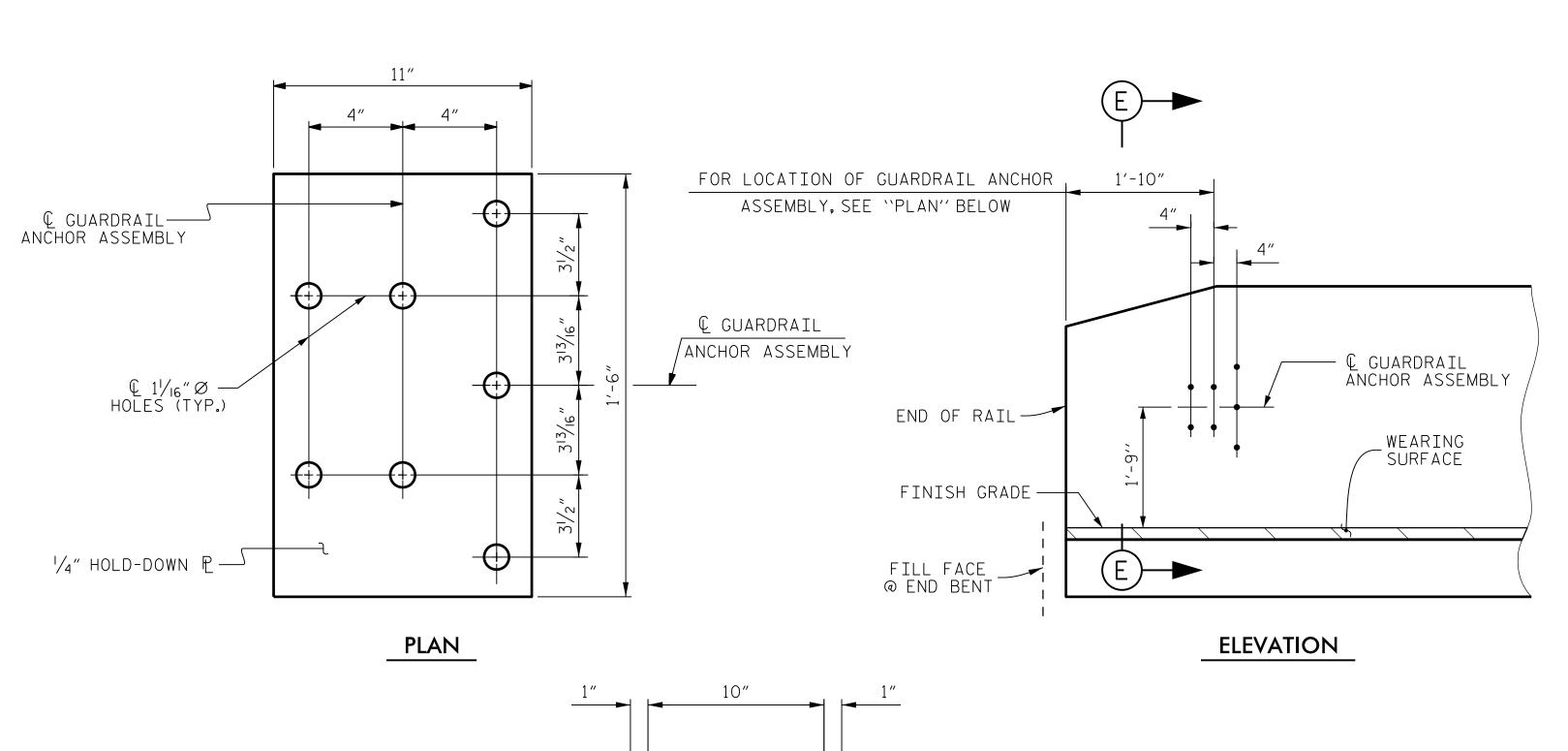
ASSEMBLED BY: K.E.LOFTON DATE: 6-22

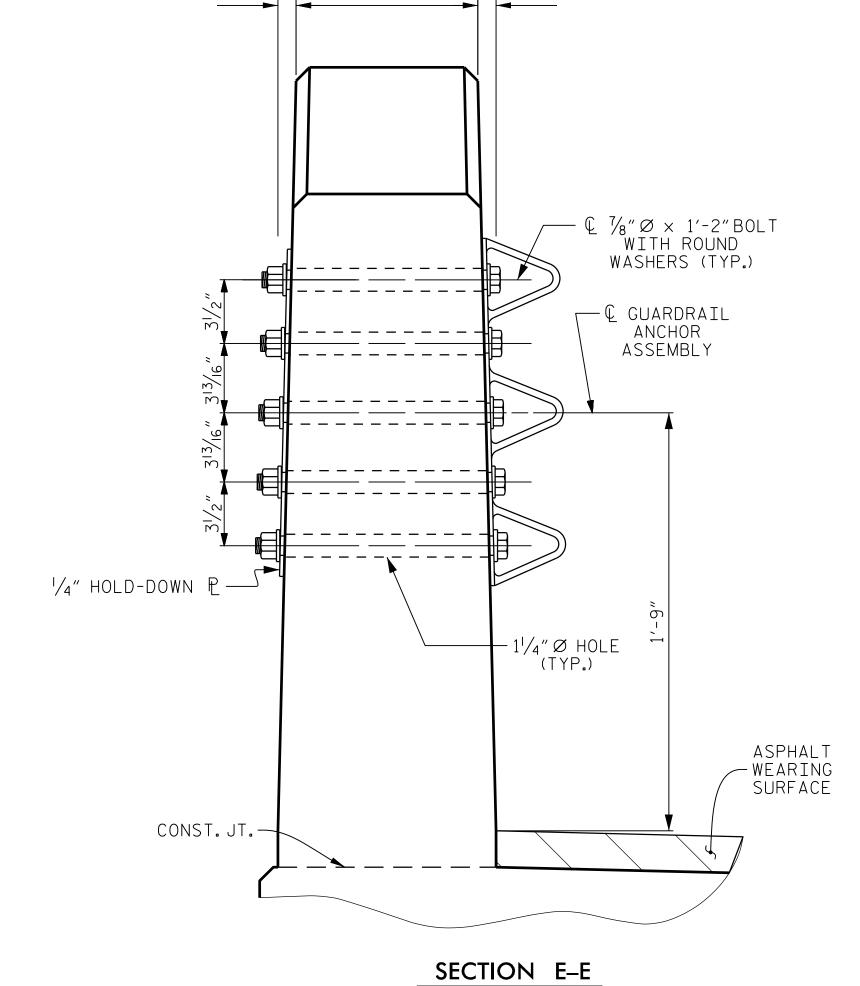
G.P.HOOVER DATE: 9-23

MAA/TMG MAA/THC

CHECKED BY :

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10





GUARDRAIL ANCHOR ASSEMBLY DETAILS

END OF RAIL 4" 4" 4" ANCHOR ASSEMBLY FILL FACE © END BENT 1'-10" Q GUARDRAIL ANCHOR ASSEMBLY 4" 4" 4" 4" 4" 4" 4" 4" ANCHOR ASSEMBLY

LOCATION OF ANCHORS FOR GUARDRAIL

PLAN

END BENT 1 SHOWN, END BENT 2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " \varnothing BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

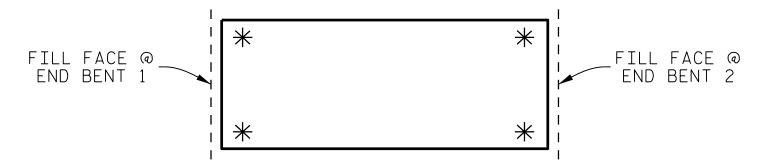
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. BR-0063

ANSON COUNTY

STATION: 20+15.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALFIGH

STANDARD

GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

 REVISIONS
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 BY:
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 TOTAL SHEETS 19

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9-23
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DRAWN BY: K. E. LOFTON DATE: 6–22

CHECKED BY: G. P. HOOVER DATE: 9–23

DESIGN ENGINEER: S. K. CHRISTOPH DATE: 9–23

FOR NOF

NC LICENSE No. 0486277

ORTH CAROLINA DEPARTMENT OF TRANSPORTATION

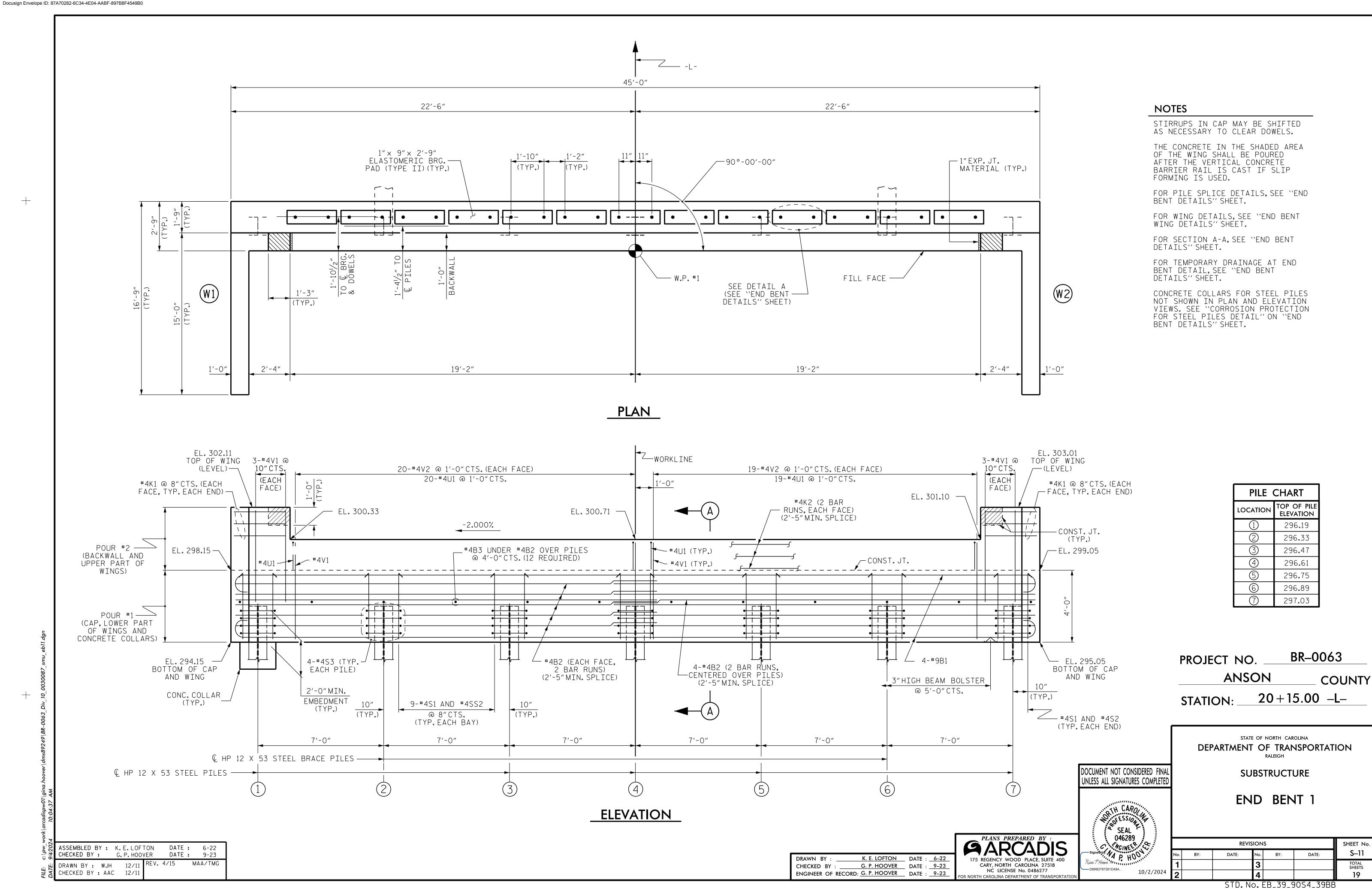
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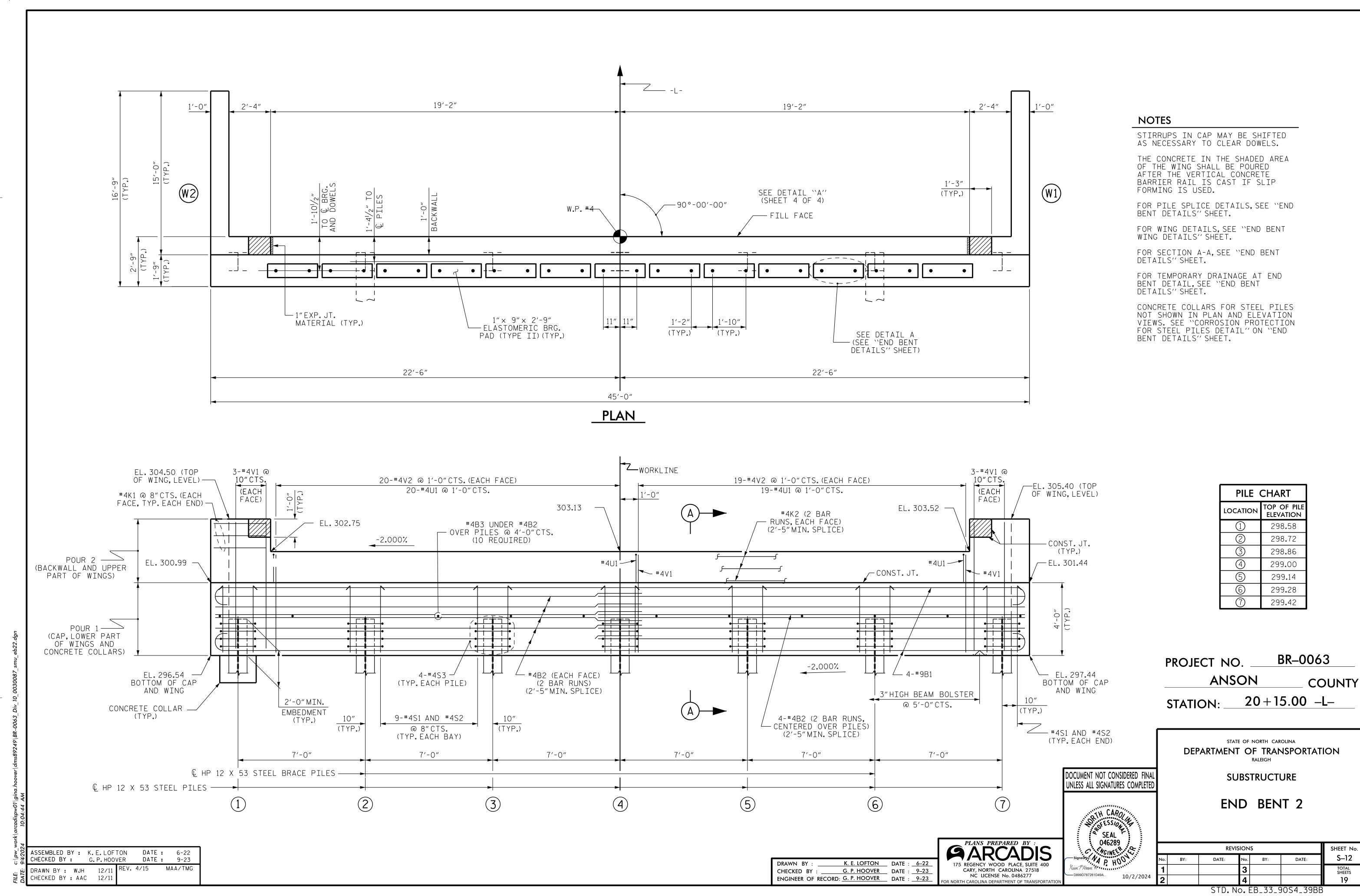
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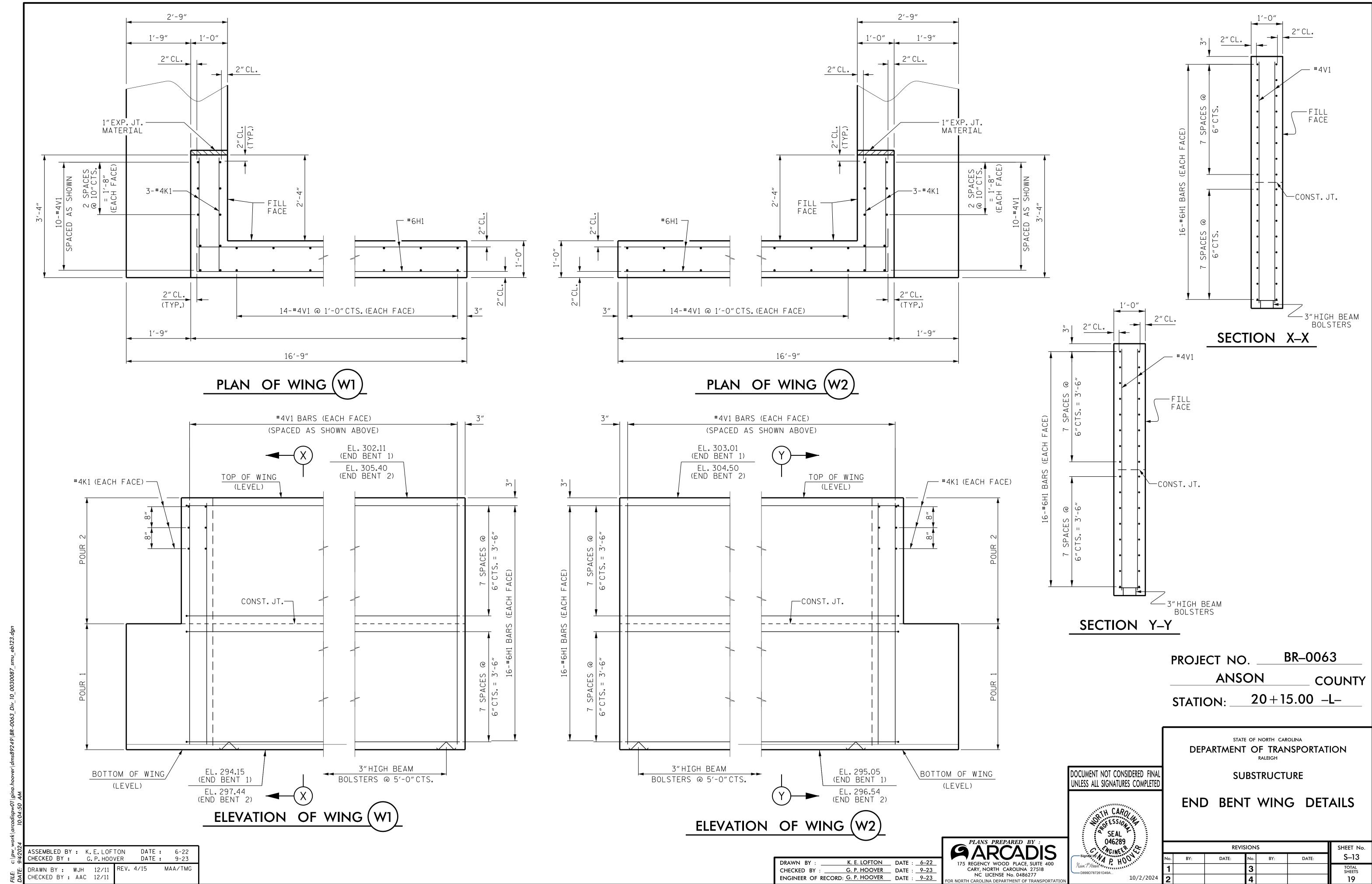
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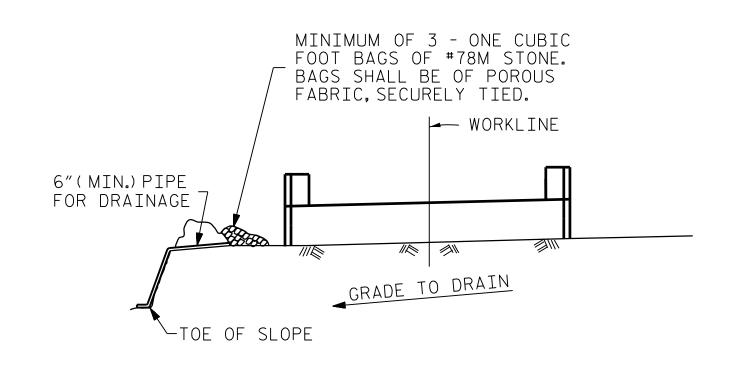
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STD. No. EB_39_90S4-39BB

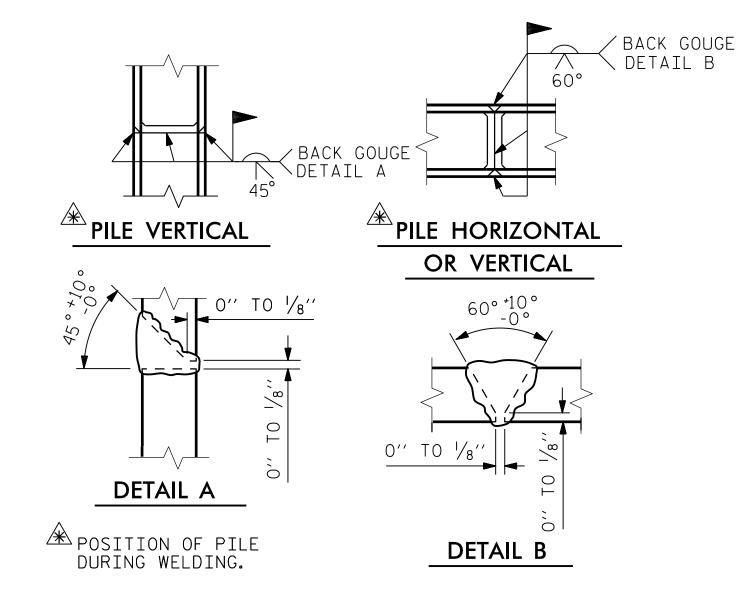


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



PILE SPLICE DETAILS

BAR TYPES 2'-5" S2 HK. HK. 44'-6" B1 1'-3" HK. 14'-8" H1 1'-8" Ø S3 8" U1 2'-5" S1 POUR 1 CAP, LOWER PART ALL BAR DIMENSIONS ARE OUT TO OUT.

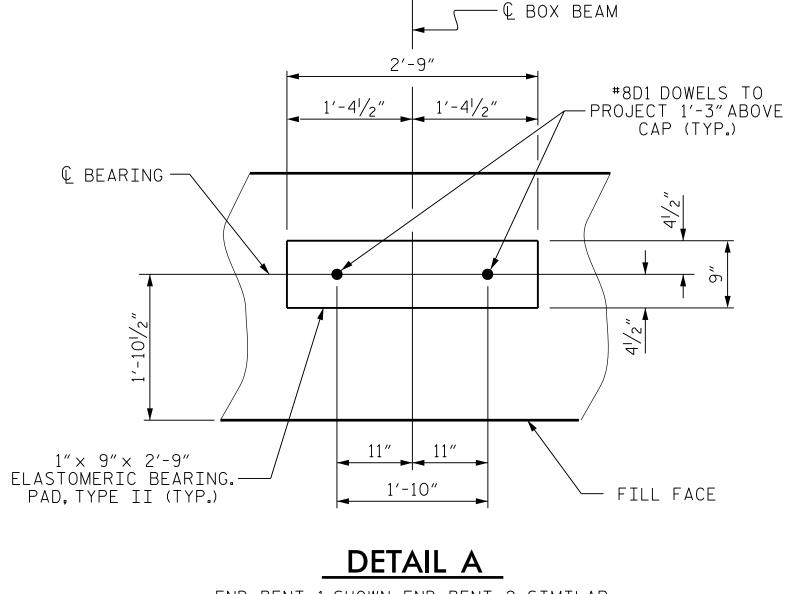
FOR ONE END BENT BAR | No. | SIZE | TYPE | LENGTH | WEIGHT #9 | 1 | 47'-0" В2 28 #4 | STR | 23'-7" 441 В3 #4 | STR | 2'-5" 19 D1 | 26 | #8 | STR | 2'-3" 156 1,024 64 | #5 | 2 | 15′-4″ #4 | STR | 2'-11" 23 12 K2 12 #4 | STR | 23'-7" 189 S1 | 56 | #4 | 3 | 10'-5" 390 S2 56 #4 4 3′-2″ 118 S3 | 28 | #4 5 6′-6″ 122 U1 | 39 | #4 | 6 | 96 3′-8″ V1 | 76 | #4 | STR | 7'-8" 389 V2 | 78 | #4 | STR | 5'-9" 300 REINFORCING STEEL 4,545 LBS. (FOR ONE END BENT) CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT)

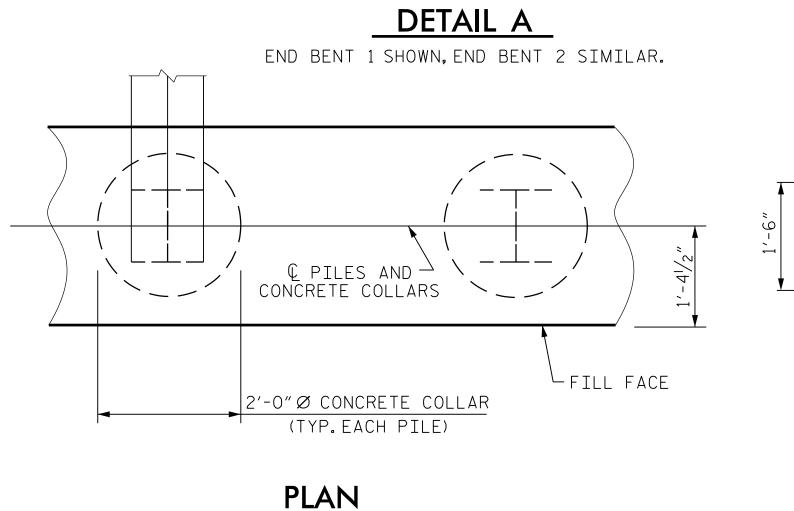
BILL OF MATERIAL

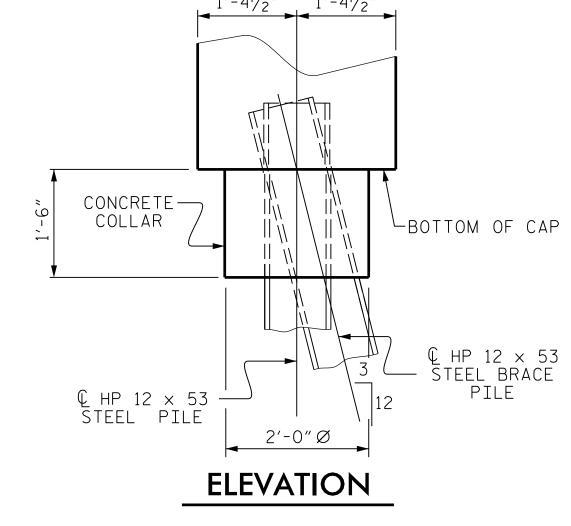
CONCRETE COLLARS 23.7 C.Y. POUR 2 BACKWALL AND UPPER 8.7 C.Y. PART OF WINGS

OF WINGS AND

TOTAL CLASS A CONCRETE 31.8 C.Y.







 $1'-4\frac{1}{2}''$ $1'-4\frac{1}{2}''$

BEARING AND 2"CL. #8D1 DOWEL #4U1— 1-#4K2 — (EACH FACE) CONST. JT.-4-#9B1 — 4-#4B2 @ 4″CTS. (CENTERED OVER 1-#4B2 — FILL FACE— (EACH FACE) PILES) #4B3-— #4S3 2-#9 B1 2"CL.(TYP.)— 2-**#**9B1 © HP 12 X 53 - 3"HIGH BEAM BOLSTERS STEEL PILE— © HP 12 X 53 - STEEL BRACE PILE 1'-41/2" 1'-41/2" 2'-9"

11" 8" 1'-2"

1'-0"

1'-101/2"

SECTION A-A

(CONCRETE COLLAR NOT SHOWN, SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

BR-0063 PROJECT NO. **ANSON** COUNTY 20+15.00 -L-STATION:

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > **SUBSTRUCTURE**

END RENT DETAILS

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

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REVISIONS SHEET No. S-14 TOTAL SHEETS

CORROSION PROTECTION FOR STEEL PILES DETAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.

K. E. LOFTON DATE : 6–22 G. P. HOOVER DATE : 9-23 CHECKED BY ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

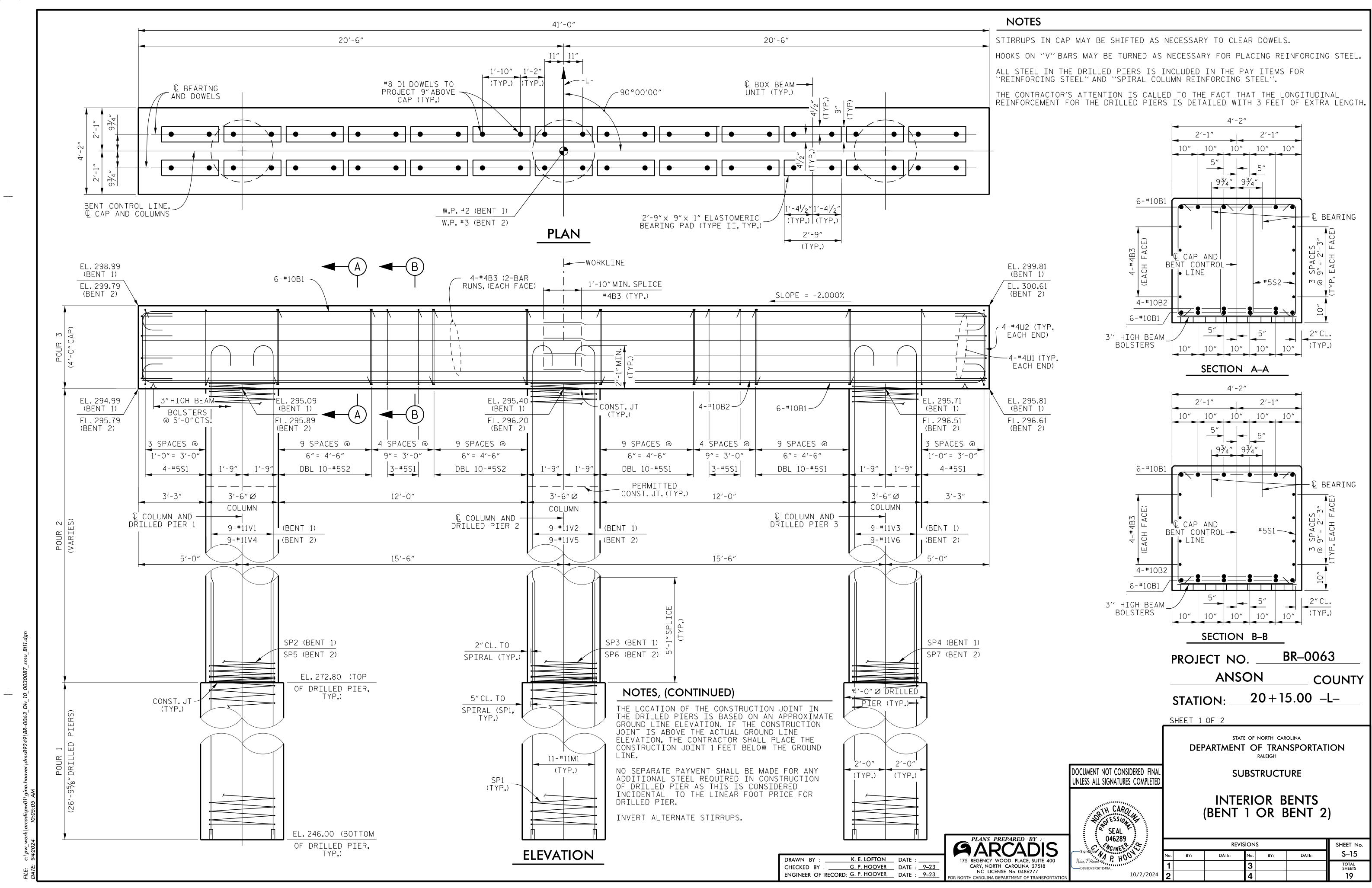
PLANS PREPARED BY :
ARCADIS 175 REGENCY WOOD PLACE, SUITE 400 CARY, NORTH CAROLINA 27518 NC LICENSE No. 0486277

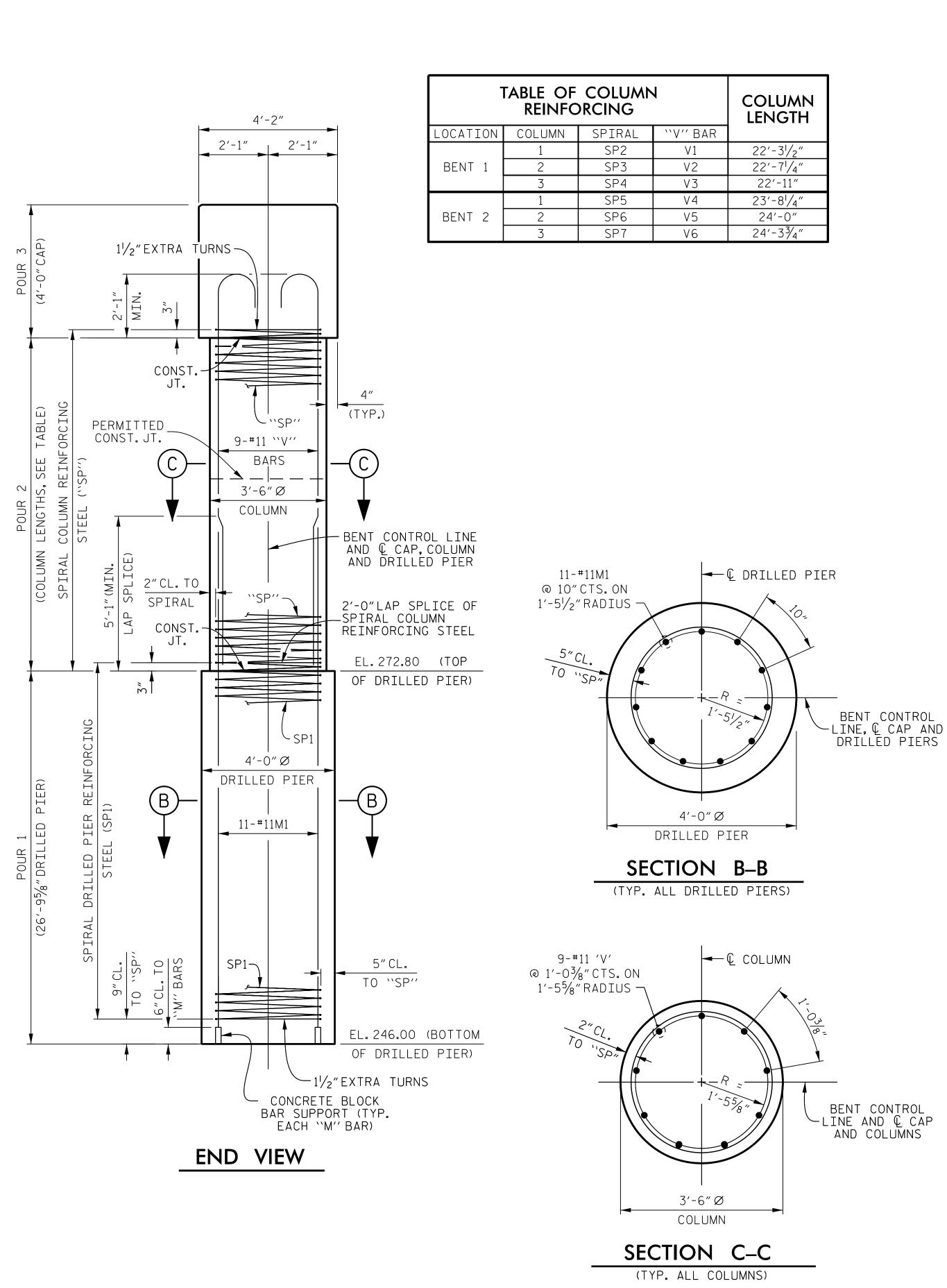
STD. No. EB_33_90S4_39BB

DRAWN BY: WJH 12/11 REV. 4/17 MAA/THC CHECKED BY : AAC 12/11

ASSEMBLED BY: K.E.LOFTON DATE: 6-22 CHECKED BY: G.P.HOOVER DATE: 9-23

10/2/2024





BILL OF MATERIAL BENT 2 BENT ' BAR No. SIZE TYPE LENGTH WEIGHT B1 12 #10 3 43'-6" 2,246 BAR No. SIZE TYPE LENGTH WEIGHT 2,246 700 227 12 3 43'- 6" 700 227 STR 40'- 8" 4 #10 STR 40'- 8" #10 В2 16 #4 STR 21'- 3" #4 STR 21'- 3" В3 #8 STR 2'- 3" D1 52 #8 STR 2'- 3" 312 #11 STR 34'- 4" 33 #11 STR 34'- 4" 10'- 7" 80 883 45 35 1,259 26'- 4" #11 #11 ٧5 4 27'- 9" 1,275 9 #11 4 26′- 8″ ٧6 28′- 0″ REINFORCING STEEL 14,420 LBS. REINFORCING STEEL 14,620 LBS. 918'- 3" 928'- 1" 613 620 628 SP5 * * * 969′-10″ 648 SP3 982'- 1" ***SP4 5 940'- 4" SP7 5 996′-10″ 1 *** SPIRAL COLUMN SPIRAL COLUMN REINFORCING STEEL 3,838 LBS. REINFORCING STEEL 3,947 LBS. CLASS "A" CONCRETE CLASS "A" CONCRETE POUR 3 CAP POUR 3 CAP 25.3 CU. YDS. 25.3 CU. YDS. POUR 2 COLUMNS 24.2 CU. YDS. 25.0 CU. YDS. POUR 2 COLUMNS 49.5 CU. YDS. 50.3 CU. YDS. TOTAL TOTAL DRILLED PIER CONCRETE DRILLED PIER CONCRETE POUR 1 37.4 CU. YDS. 37.4 CU. YDS. POUR 1

*** THE SP2 THRU SP7 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

** THE SP1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL".

AND "SPIRAL COLUMN REINFORCING STEEL".

26'-5" V6 HOOK 26'-2" V5 25'-10" V4 3′-10″ S1 25′-1″ V3 2'-5" S2 24'-9" V2 24'-5" V1 4 24'-7" SP7 3′-8″ U1 24'-3" SP6 3′-6″ U2 23'-11" SP5 `NOT REQUIRED 23'-2" SP4 AT CONST. JT. BETWEEN COLUMN 22'-10" SP3 AND DRILLED PIER. 22'-7" SP2 4 SPACERS 26'-3" SP1 $1\frac{1}{2}$ EXTRA TURNS *-5"PITCH 3"PITCH SP2 THRU SP7 ALL BAR DIMENSIONS ARE OUT TO OUT.

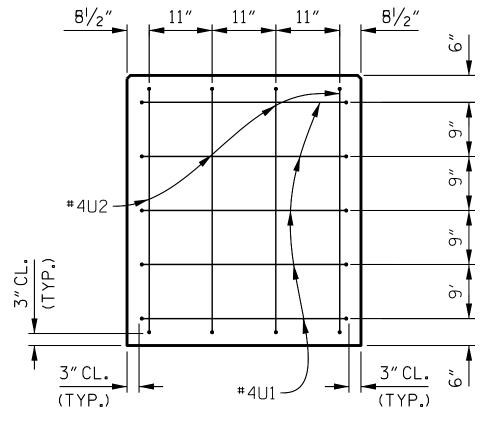
BAR TYPES

HOOK

40'-8" B1

1′-5″

HOOK



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PROJECT NO. BR-0063

ANSON COUNTY

STATION: 20+15.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

SHEET No.

TOTAL SHEETS

INTERIOR BENT DETAILS AND BILL OF MATERIALS

SEAL 046289

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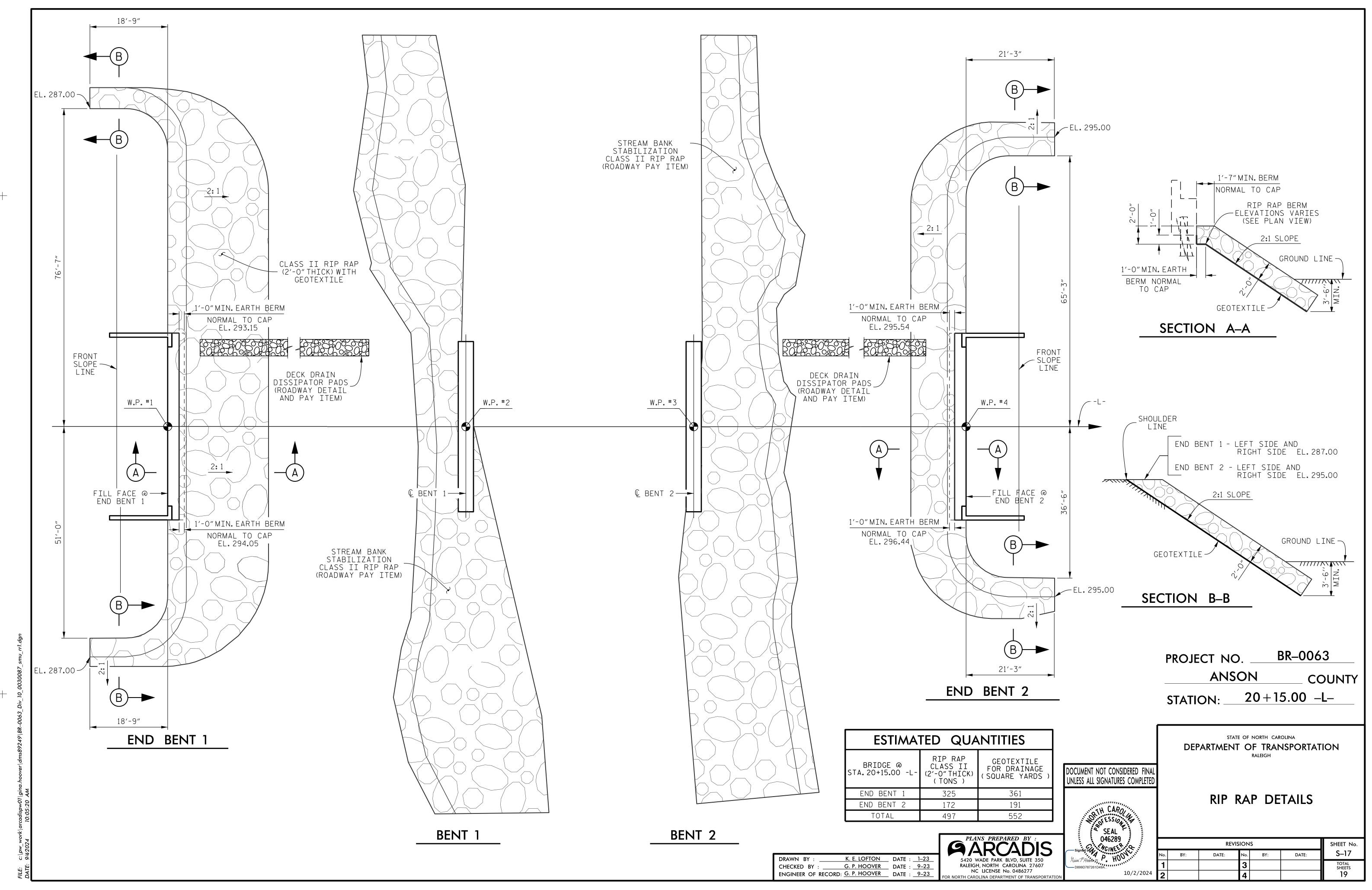
DRAWN BY: K. E. LOFTON DATE: 7-22
CHECKED BY: G. P. HOOVER DATE: 9-23
ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

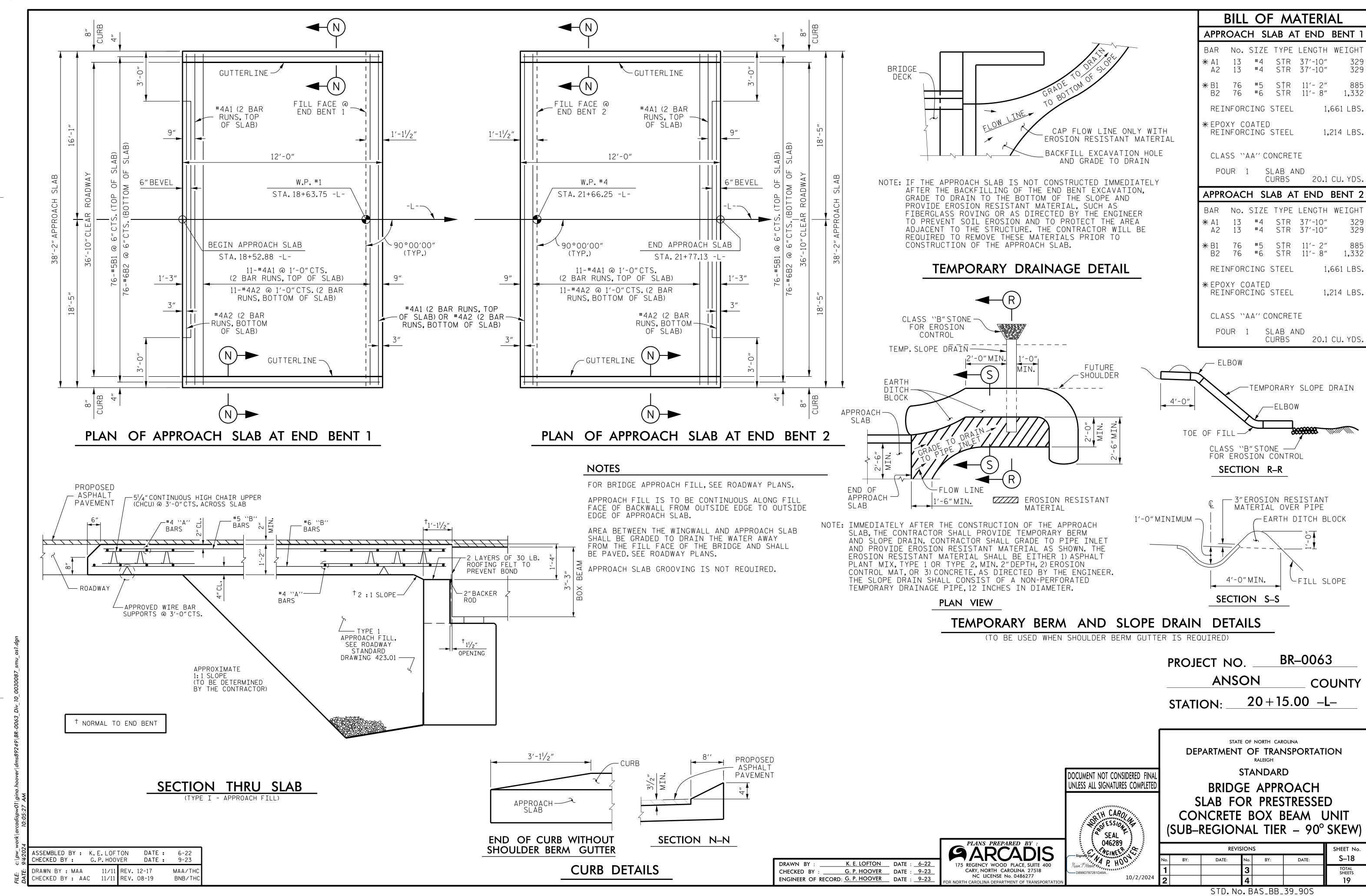
CHECKED BY: G. P. HOOVER DATE: 9-23

ENGINEER OF RECORD: G. P. HOOVER DATE: 9-23

DATE: 9-23

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STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS.PER SQ.IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ----375 LBS. PER SQ. IN. 30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 1'-O"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16" OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

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