CONTENTS

-0063

Ż

REFERENCE

<u>SHEET NO.</u>	DESCRIPTION
I	TITLE SHEET
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4	PROFILE
5 - 6	CROSS SECTIONS
7 - 12	BORE LOGS, CORE REPORTS, & CORE PHOTOGRAPHS
13	SOIL TEST RESULTS
14	SITE PHOTOGRAPHS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY ANSON

PROJECT DESCRIPTION REPLACEMENT OF BRIDGE 030087 OVER RICHARDSON CREEK ON NC 742

SITE DESCRIPTION BRIDGE NO. 87 -L- (NC 742) OVER RICHARDSON CREEK AT -L- STA. 20+15

STATE PROJECT REFERENCE NO. STATE SHEETS NO. N.C 1 14 BR-0063

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLT TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-8050. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

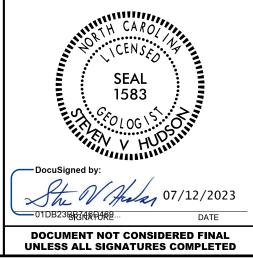
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNICS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-FLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE ONSERVED WATER LEVELS OR SOL MOISTURE CONDITIONS MOLATED IN THE SUBSURFACE RELIVESTIGATIONS AND REAS RECORDED AT THE TIME OF THE INVESTIGATION. THES WATER LEVELS OR SOL MOISTURE CONDITIONS MAY LARY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS NICLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIODER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBJURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPHION OF THE DEPARTMENT AS TO THE TYPE OF WATERALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR SLAUTONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACULAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES: I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL
CAMERON STRATTON
THOMAS PARK
INVESTIGATED BY <u>CATLIN</u>
DRAWN BY <u>S. V. HUDSON, LG</u>
CHECKED BY J. LEE STONE, LG
SUBMITTED BY <u>S. V. HUDSON, LG</u>
DATE





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

			SOIL	DESC	RIPTIO	N				<u> </u>		GR	ADATION						F	ROCK DES	CRIPTION
BE PENET ACCORDI IS B	SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE ASHTID SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO L CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH					R FOOT ATION NG:	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.						HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TEST ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EOUAL TO OR LESS THAN Ø. BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK REPRESENTED BY A ZONE OF WEATHERED ROCK.								
AS	5 MINERALO	GICAL COMPO	ITION, ANGUL	ARITY, S	RUCTURE,	PLASTICIT	Y,ETC. FOR	EXAMPLE,	S SUCH	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:						ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:				:	
v		RAY, SILTY CLAY	MOIST WITH II					STIC, A-7-6		ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.					WEATHERED ROCK (WR)					I MATERIAL THAT WOULD YIELD SP DT IF TESTED.	
GENERAL		GRANULAR MATE		SILT-CLAY MATERIALS DEFAULT MATERIALS MINERALOGICAL COMPOSITION										CRYSTALLIN		2.2	FINE T	O COARSE GP	AIN IGNEOUS AND METAMORPHIC R		
CLASS. GROUP	A-1	≤ 35% PASSING A-3	*200) A-2		A-5	NG \$200) 4-6 A-7	A-1, A-2	A-4, A-5					N THEY ARE CONSID			ROCK (CR)		12	CNEISS	GABBRO, SCH	
	A-1-a A-1-b		-2-5 A-2-6 A	_		A-7-5. A-7-6	A-3	A-6, A-7				COMPF	RESSIBILITY			NON-CRYSTA ROCK (NCR)	LLINE		SEDIME	ENTARY ROCK	RAIN METAMORPHIC AND NON-COAST THAT WOULD YEILD SPT REFUSAL
SYMBOL	000000000000000000000000000000000000000			S								MPRESSIBLE COMPRESSIBL	E	LL < 31 LL = 31	50	COASTAL PL			COAST(YPE INCLUDE	S PHYLLITE, SLATE, SANDSTONE, ET DIMENTS CEMENTED INTO ROCK, BUT
% PASSING	00000000		2203 (11, 11)					SILT-			LY COMPR	RESSIBLE		LL > 50		SEDIMENTAR (CP)	Y ROCK			EFUSAL. ROCK BEDS, ETC.	TYPE INCLUDES LIMESTONE, SAND
	50 MX 30 MX 50 MX	51 MN					GRANULAR SOILS	CLAY	MUCK, PEAT		<u>P</u>		GE OF MATER							WEATH	ERING
	5 MX 25 MX	10 MX 35 MX 3	5 MX 35 MX 35	MX 36 M	N 36 MN 31	5 MN 36 MN		SUILS		ORGANIC MATERIAL TRACE OF ORGANIC MA		GRANULAR <u>SOILS</u> 2 - 3%	SILT - CLAY <u>SOILS</u> 3 - 5%	<u>OTHEI</u> TRACE	<u>R MATERIAL</u> 1 - 10%	FRESH		FRESH, CRYS		T, FEW JOINTS	5 MAY SHOW SLIGHT STAINING. ROCK
MATERIAL PASSING 40							SOILS	MITU		LITTLE ORGANIC MATT	TER	3 - 5%	5 - 12%	LITTLE	10 - 20%	VERY SLIGHT				ITS STAINED. S	OME JOINTS MAY SHOW THIN CLAY (
LL PI	_ 6 мх		1 MN 40 MX 41 3 MX 11 MN 11				LITTLE	e or	HIGHLY	MODERATELY ORGANIC HIGHLY ORGANIC		5 - 10% > 10%	12 - 20% > 20%	SOME HIGHL Y	20 - 35% 35% AND ABOVE	(V SLI.)	CRYST		BROKEN SPEC		HINE BRIGHTLY. ROCK RINGS UNDER H
GROUP INDEX	0	0 0	4 MX	_		MX NO MX	Modef Amoun1		ORGANIC SOILS			GROL	JND WATER			SLIGHT				ITS STAINED A	ND DISCOLORATION EXTENDS INTO R
	TONE FRAGS.	FINE SIL	TY OR CLAYEY	s	ILTY	CLAYEY	ORGA MAT1		50125	∇	WATEF	R LEVEL IN E	BORE HOLE IMMEDIA	ATELY AFTER	DRILLING	(SLI.)					N GRANITOID ROCKS SOME OCCASION STALLINE ROCKS RING UNDER HAMME
OF MAJOR MATERIALS	GRAVEL, AND SAND	SAND GR	vel and sand	s	OILS	SOILS				▼	STATI	IC WATER LEV	VEL AFTER 24	HOURS		MODERATE	SIGNIF	ICANT PORT	TIONS OF RO	CK SHOW DISC	COLORATION AND WEATHERING EFFECT
GEN. RATING		EXCELLENT TO	500D		FAIR TO	POOR	FAIR TO	POOR	UNSUITABLE	<u> </u>	PERCH	HED WATER, SA	ATURATED ZONE, OF	R WATER BEA	RING STRATA	(MOD.)					ULL AND DISCOLORED, SOME SHOW CL IOWS SIGNIFICANT LOSS OF STRENGT
AS SUBGRADE		PIOF A-7-5 SU	GROUP IS ≤ L	- 30 : Pi	OF A-7-6	SUBGROUP IS	> LL - 30				SPRIN	NG OR SEEP						FRESH ROCK			
			NSISTEN								- M	ISCELLA	NEOUS SYMBO	OLS		MODERATELY SEVERE	AND D	ISCOLORED	AND A MAJO	DRITY SHOW KA	STAINED. IN GRANITOID ROCKS,ALL AOLINIZATION. ROCK SHOWS SEVERE I
PRIMARY S			NESS OR		NGE OF S	TANDARD ESISTENCE		E OF UNCO RESSIVE S				T (RF) 25/02	25 DIP & DIP DIF	RECTION		(MOD. SEV.)			AVATED WITH <u>7 <i>YIELD SPT</i></u>		'S PICK. ROCK GIVES "CLUNK" SOUND
			STENCY		(N-VAL	JE)		(TONS/FT		WITH SOIL DE			OF ROCK STRU OF ROCK STRU			SEVERE					STAINED. ROCK FABRIC CLEAR AND
GENERAL			LOOSE OSE		< 4 4 TO					SOIL SYMBOL		6	OPTOMT TEST BOI	RING	SLOPE INDICATOR INSTALLATION	(SEV.)	TO SO	ME EXTENT.	. SOME FRAG	GMENTS OF STR	N GRANITOID ROCKS ALL FELDSPARS RONG ROCK USUALLY REMAIN.
GRANULA MATERIA	L		1 DENSE NSE		10 TO 30 TO			N/A		ARTIFICIAL FI			AUGER BORING		CONE PENETROMETER TEST	VERY				N VALUES > .	<u>100 BPF</u> STAINED. ROCK FABRIC ELEMENTS A
(NON-COF	HESIVE)	VERY	DENSE		> 50	I							`	\bigcirc		SEVERE	BUT M	ASS IS EFF	ECTIVELY R	EDUCED TO SO	DIL STATUS, WITH ONLY FRAGMENTS (
GENERAL	LY.		SOFT DFT		< 2 2 TO			< 0.25 0.25 TO 0	0.5	INFERRED SOI	L BOUND	, · · · · · · · · · · · · · · · · · · ·)- CORE BORING	•	SOUNDING ROD	(V SEV.)					ROCK WEATHERED TO A DEGREE THA N. <u>IF TESTED, WOULD YIELD SPT N</u>
SILT-CL MATERIA			1 STIFF IFF		4 TO 8 TO			0.5 TO 1. 1 TO 2		INFERRED ROC	K LINE	MW ()) MONITORING W	ELL 🕂 🕂	TEST BORING WITH CORE	COMPLETE					DISCERNIBLE, OR DISCERNIBLE ONLY BE PRESENT AS DIKES OR STRINGER
(COHESI)		VERY	STIFF		15 TO > 30	30		2 TO 4		ALLUVIAL SOI	L BOUND		PIEZOMETER INSTALLATION	Ò	- SPT N-VALUE			AN EXAMPLE		donini 2 mini	SE TRESERT HS BIRES ON STRINGER
				ORC							R	ECOMMEN	DATION SYME	BOLS						ROCK HA	
U.S. STD. SIE	VE SIZE		4 10	4	0 6	0 200	270					LASSIFIED E			SIFIED EXCAVATION -	VERY HARD				NIFE OR SHARP E GEOLOGIST'S	P PICK. BREAKING OF HAND SPECIMEN
OPENING (MM			4.76 2.0				5 0.053					SUITABLE WAS		USED I	ABLE,BUT NOT TO BE N THE TOP 3 FEET OF	HARD				OR PICK ONL	Y WITH DIFFICULTY. HARD HAMMER E
BOULDEF (BLDR.)		BBLE	GRAVEL	COA SA	ND	F INE SANE	' ' ' '	SILT SL.)	CLAY (CL.)			CEPTABLE DEG	GRADABLE ROCK	EMBANK	MENT OR BACKFILL	MODERATELY		TACH HAND SE SCRATCHE		OR PICK. GOI	JGES OR GROOVES TO 0.25 INCHES D
				(CSE.		(F SE	.)						REVIATIONS			HARD	EXCAV		ARD BLOW OF		T'S PICK. HAND SPECIMENS CAN BE (
GRAIN MM SIZE IN.		75 3	2.0	0	0.3	25	0.05	0.005		AR - AUGER REFUSAL BT - BORING TERMINATED	D		MICACEOUS	WEA.	- VANE SHEAR TEST - WEATHERED	MEDIUM	CAN B	E GROOVED	OR GOUGED		DEEP BY FIRM PRESSURE OF KNIFE
	S	OIL MOI	STURE -	CORF	ELATI	ON OF	TERMS			CL CLAY CPT - CONE PENETRATION	N TEST		MODERATELY ON PLASTIC		UNIT WEIGHT DRY UNIT WEIGHT	HARD			ED IN SMALL .OGIST'S PICK		ICES 1 INCH MAXIMUM SIZE BY HARD
	MOISTURE ERBERG LI			MOISTUR RIPTION	E GL	JIDE FOR	FIELD MOIS	STURE DES	CRIPTION	CSE COARSE DMT - DILATOMETER TES		ORG	ORGANIC PRESSUREMETER T	- 0	MPLE ABBREVIATIONS	SOF T	CAN B	E GROVED C	OR GOUGED R	READILY BY KN	NIFE OR PICK. CAN BE EXCAVATED IN
		11.07	1							DPT - DYNAMIC PENETRA		ST SAP	SAPROLITIC	S - E	BULK					INGER PRESSU	BY MODERATE BLOWS OF A PICK POI RE.
			- SATU (SA)				DUID;VERY / THE GROU			e - VOID RATIO F - FINE			AND, SANDY		SPLIT SPOON SHELBY TUBE	VERY SOF T					VATED READILY WITH POINT OF PICK FINGER PRESSURE. CAN BE SCRATC
PLASTIC		LIMIT					REQUIRES D			FOSS FOSSILIFEROUS FRAC FRACTURED, FRAC	TURES		SLIGHTLY TRICONE REFUSAL	RS - RT -	ROCK RECOMPACTED TRIAXIAL	3011	FINGE			JE BROKEN DI	THOER THESSORE, CHA BE SCHARE
RANGE <			- WET	- (W)			IMUM MOIST			FRAGS FRAGMENTS HI HIGHLY		ω - ΜC V - VE	DISTURE CONTENT		- CALIFORNIA BEARING RATIO			TURE SI			BEDDING
PLL.	PLASTI	LIMII									UIPME		ON SUBJEC	T PROJE		VERY WI			SPACING	0 FEET	TERM VERY THICKLY BEDDED
OM . SL		M MOISTURE AGE LIMIT	- MOIS	f - (M)	S	DLID;AT O	R NEAR OP	TIMUM MOI	ISTURE	DRILL UNITS:	1	NCING TOOLS:		HAMMER		WIDE MODERAT	ELY CL		3 TO 10 FE 1 TO 3 FE		THICKLY BEDDED THINLY BEDDED Ø
5L .		HOE LIMIT			R	QUIRES A	DDITIONAL	WATER TO		Х СМЕ-45В	🔲 '	CLAY BITS		X AU	TOMATIC MANUAL	CLOSE VERY CL			0.16 TO 1 F SS THAN 0.1		VERY THINLY BEDDED 0.0 THICKLY LAMINATED 0.0
L			- DRY	- (U)			IMUM MOIST			CME-55			S FLIGHT AUGER	CORE SIZ	_						THINLY LAMINATED
				.ASTI(8" HOLLOW AU		П-в —	Ц-н						
	PLASTIC		PLAS	<u>TICITY 1</u> Ø-5	NDEX (PI	<u>)</u>		Y STRENG		CME-550		HARD FACED F		<u>Х</u> -N <u>Q</u>				RULKS, INDU			NG OF MATERIAL BY CEMENTING, HI INGER FREES NUMEROUS GRAINS;
SL1G	GHTLY PLAS			6-15	i			SLIGHT		VANE SHEAR TEST			W/ ADVANCER	HAND TO		FRIA	3LE		GEN	NTLE BLOW B	Y HAMMER DISINTEGRATES SAMPLE
	ERATELY PI ILY PLASTI			16-2 26 OR 1				MEDIUM HIGH		PORTABLE HOIST					T HOLE DIGGER	MODE	RATELY	INDURATED			SEPARATED FROM SAMPLE WITH S WHEN HIT WITH HAMMER.
				COLO	R								7/8 TUNGCARB.		ND AUGER JNDING ROD	INDU	RATED		GRA	AINS ARE DIF	FICULT TO SEPARATE WITH STEEL
DESCRIPT	IONS MAY	INCLUDE COL	OR OR COLO	R COMBI	NATIONS	(TAN, RED,	YELLOW-BR	ROWN, BLUE	-GRAY).	X MOBILE B-57		CORE BIT			E SHEAR TEST		HILU				REAK WITH HAMMER.
		ICH AS LIGH														EXTR	EMELY J	INDURATED			BLOWS REQUIRED TO BREAK SAMPL ACROSS GRAINS.

PROJECT REFERENCE NO.

BR-0063

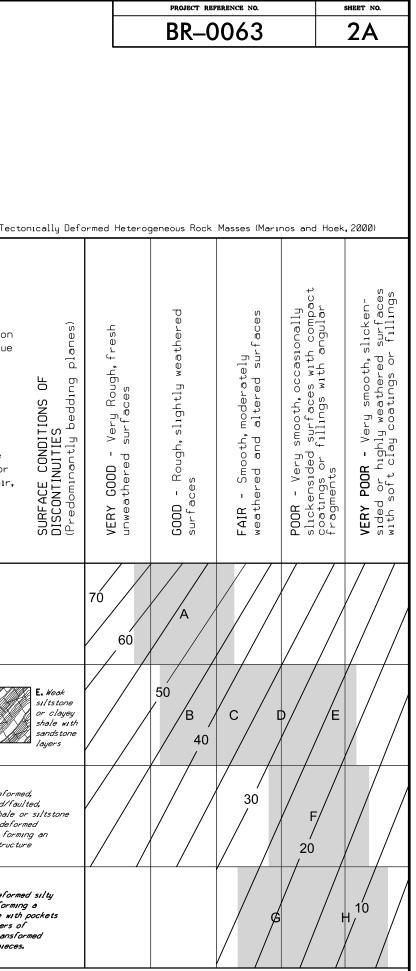
2

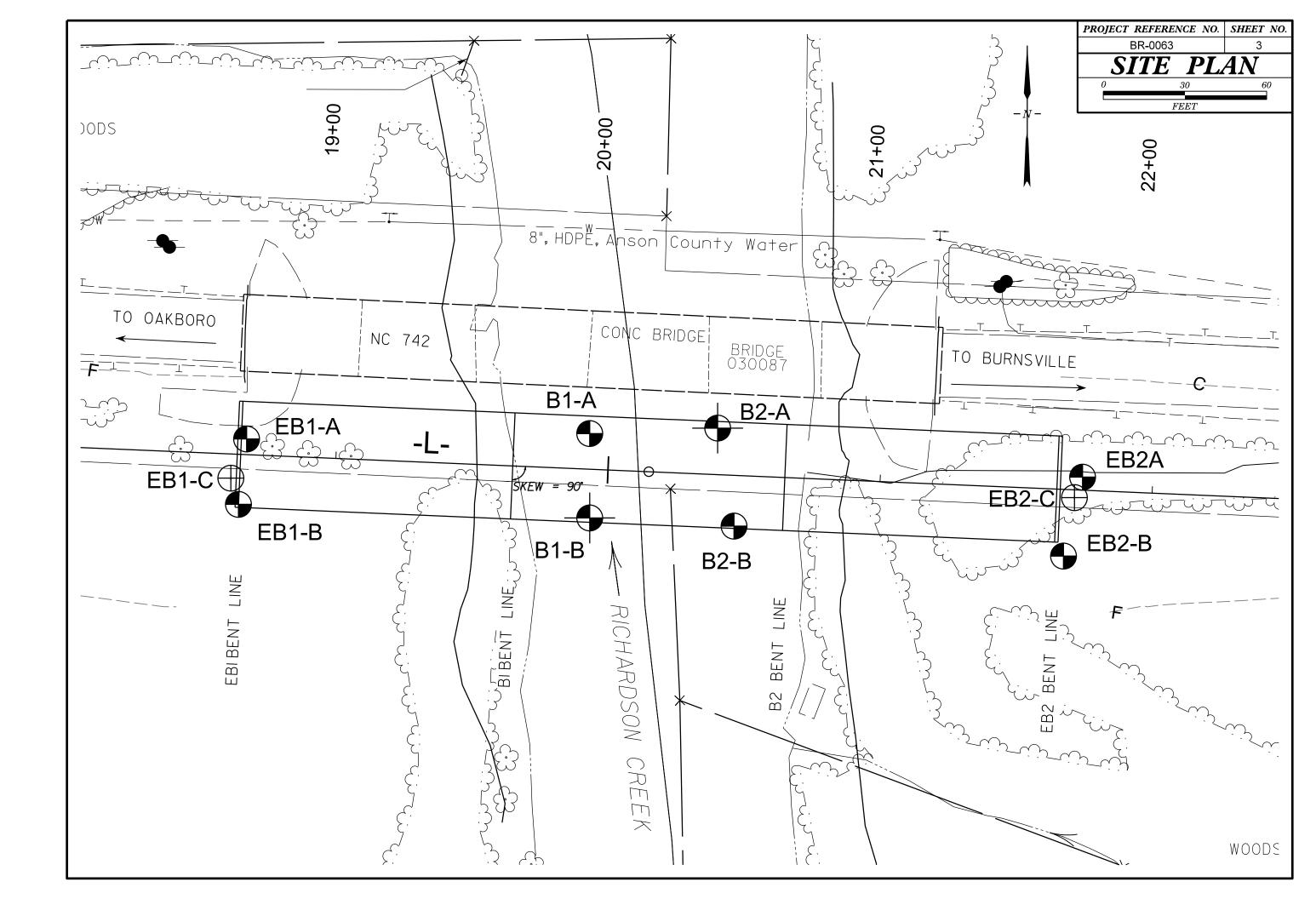
	TERMS AND DEFINITIONS
ED. AN INFERRED) SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS OFTEN	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
T N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
IN VALUES >	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
ОСК ТНАТ	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLUDES GRANITE,	SURFACE.
	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
AL PLAIN IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
с.	OF SLOPE.
MAY NOT YIELD STONE.CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
AMMER BLOWS IF	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
DCK UP TO AL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
R BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
S. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AY. ROCK HAS	PARENT MATERIAL.
H AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
FELDSPARS DULL OSS OF STRENGTH	FIELD.
WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
EVIDENT BUT	ITS LATERAL EXTENT.
ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
RE DISCERNIBLE NF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
T ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
S. SAPROLITE IS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
	RUN AND EXPRESSED AS A PERCENTAGE.
	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
IS REQUIRES	ROCK.
	<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BLOWS REQUIRED	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
550 CAN DE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
EEP CAN BE DETACHED	OR SLIP PLANE.
	STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF)OF
OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
I FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
NI. SMALL, TAIN	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
HED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	BENCH MARK: LOCATIONS OBTAINED WITH A FEAL TIME KINEMATIC (RTK)
THICKNESS	GLOBAL POSITIONING SYSTEM (GPS) UTILIZING THE NORTH CAROLINA
4 FEET	STATE PLANE NORTH AMERICAN DATUM 1983 ELEVATION: NAVD88 FEET
I.5 - 4 FEET 16 - 1.5 FEET	
03 - 0.16 FEET	NOTES:
08 - 0.03 FEET	FIAD = FILLED IMMEDIATELY AFTER DRILLING
0.008 FEET	NAVD88 = NORTH AMERICAN VERTICAL DATUM 1988
EAT, PRESSURE, ETC.	
.	
TEEL PROBE:	
PROBE:	
E;	
	DATE: 8-15-14

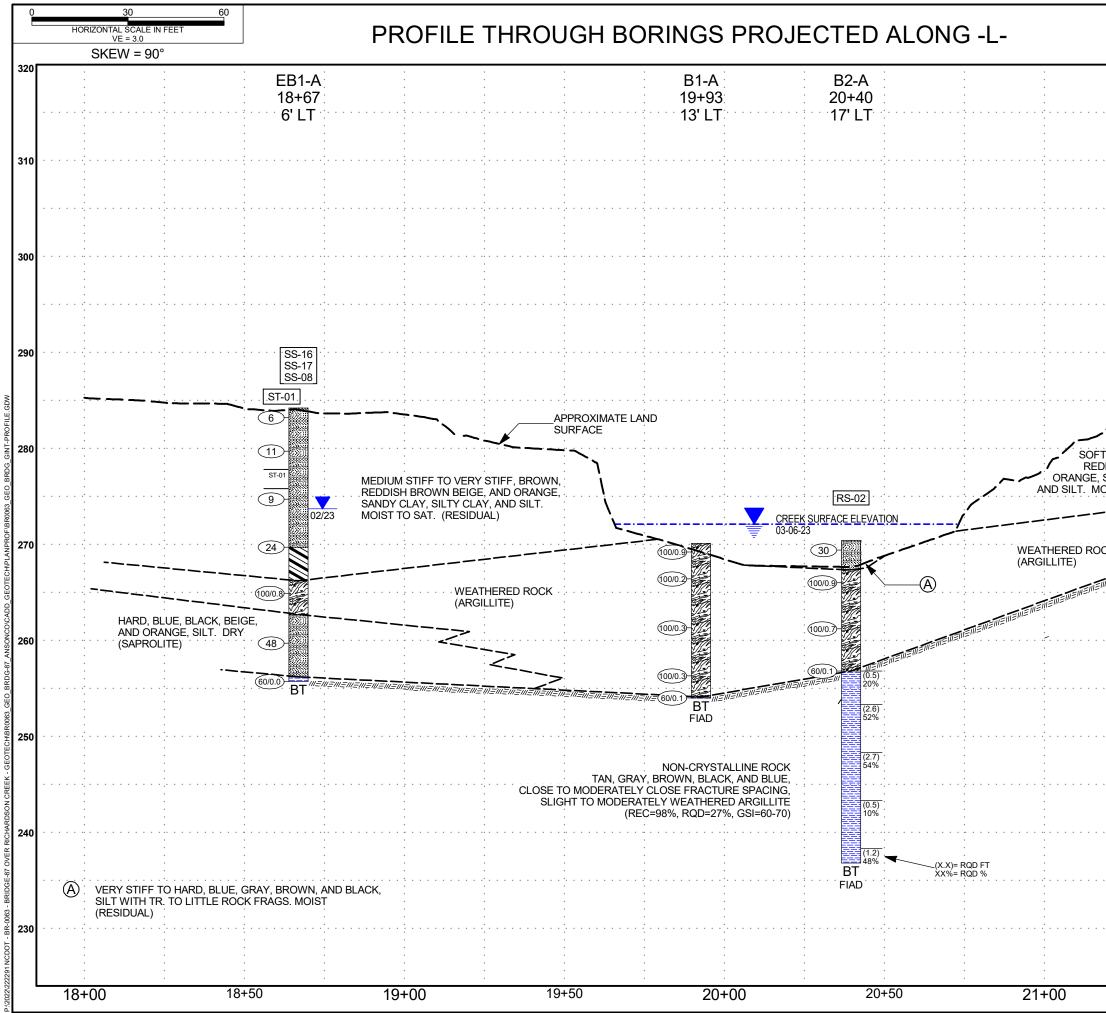
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

AASHTO LRFD Figure 10.4.6.4–1 — Determination of GSI for Jointed F	Rock Mass (Marı	nos and Hoek,2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for T
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000) From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the	Gook Mass (Mari GOOD rough, fresh unweathered surfaces	ghtly weathered, ıron staıned	moderately weathered and surfaces	ided, highly weathered surfaces pact coatings or fillings ar fragments	JR ided, highly weathered surfaces t clay coatings or fillings	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000) From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average valu of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for
fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis. STRUCTURE	VERY Very	CREASING SI	FAIR Smooth, altered	POOR Slickenside with compact	VERY POOR Slickensided, F with soft clay	by a slight shift to the right in the columns for fai poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.
				ΑLITY		COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90			N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
BLOCKY - well interlocked un- disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets		70 60				B. Sand- stone with thin inter-
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets		5	0			layers of siltstone
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity			40	30		C, D, E, and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H .
discontinuity sets. Persistence of bedding planes or schistosity DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces				20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	N/A	N/A			10	Means deformation after tectonic disturbance





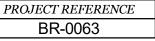


222291
222291



EB2-A

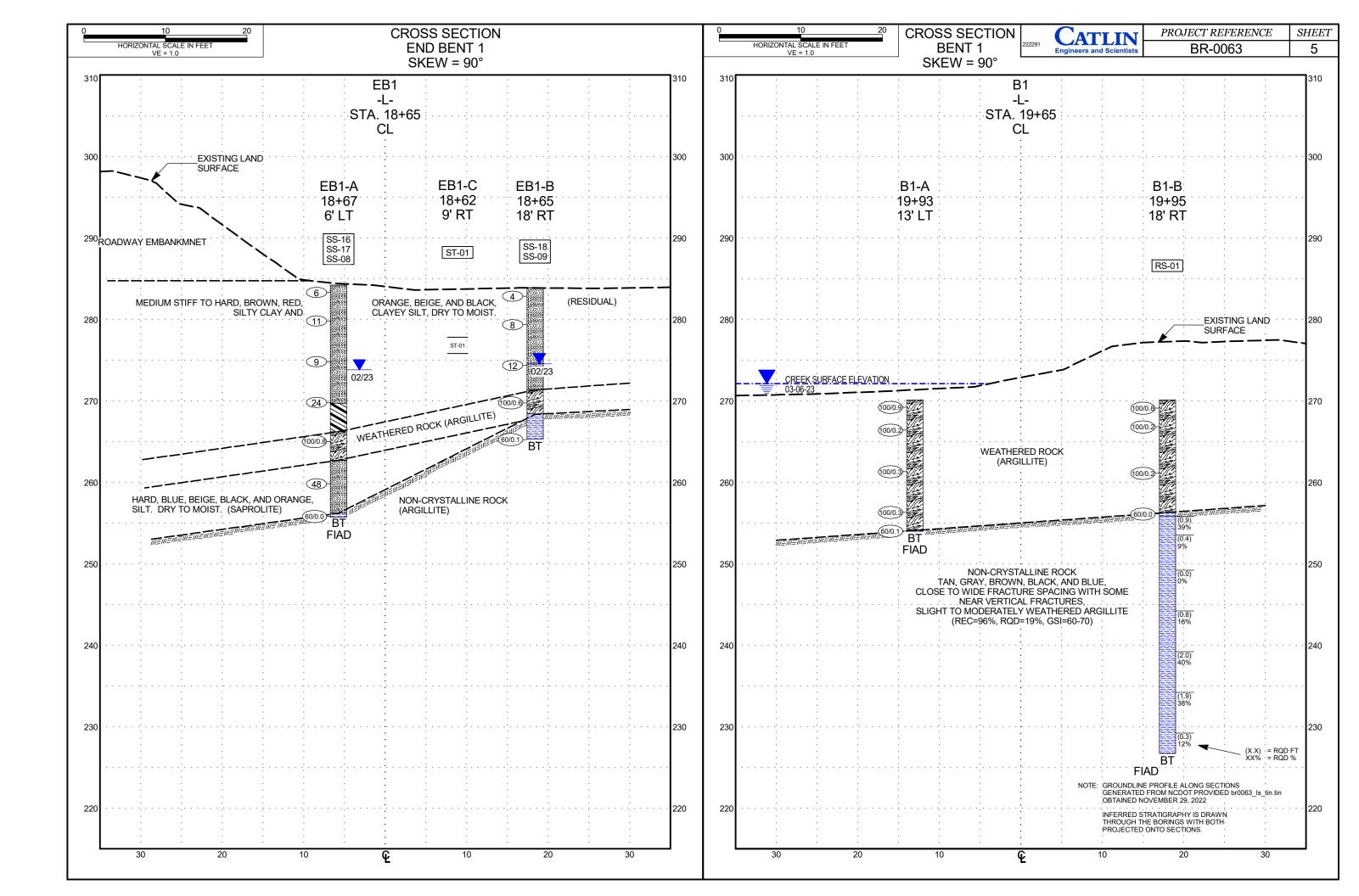
21+74 5' LT

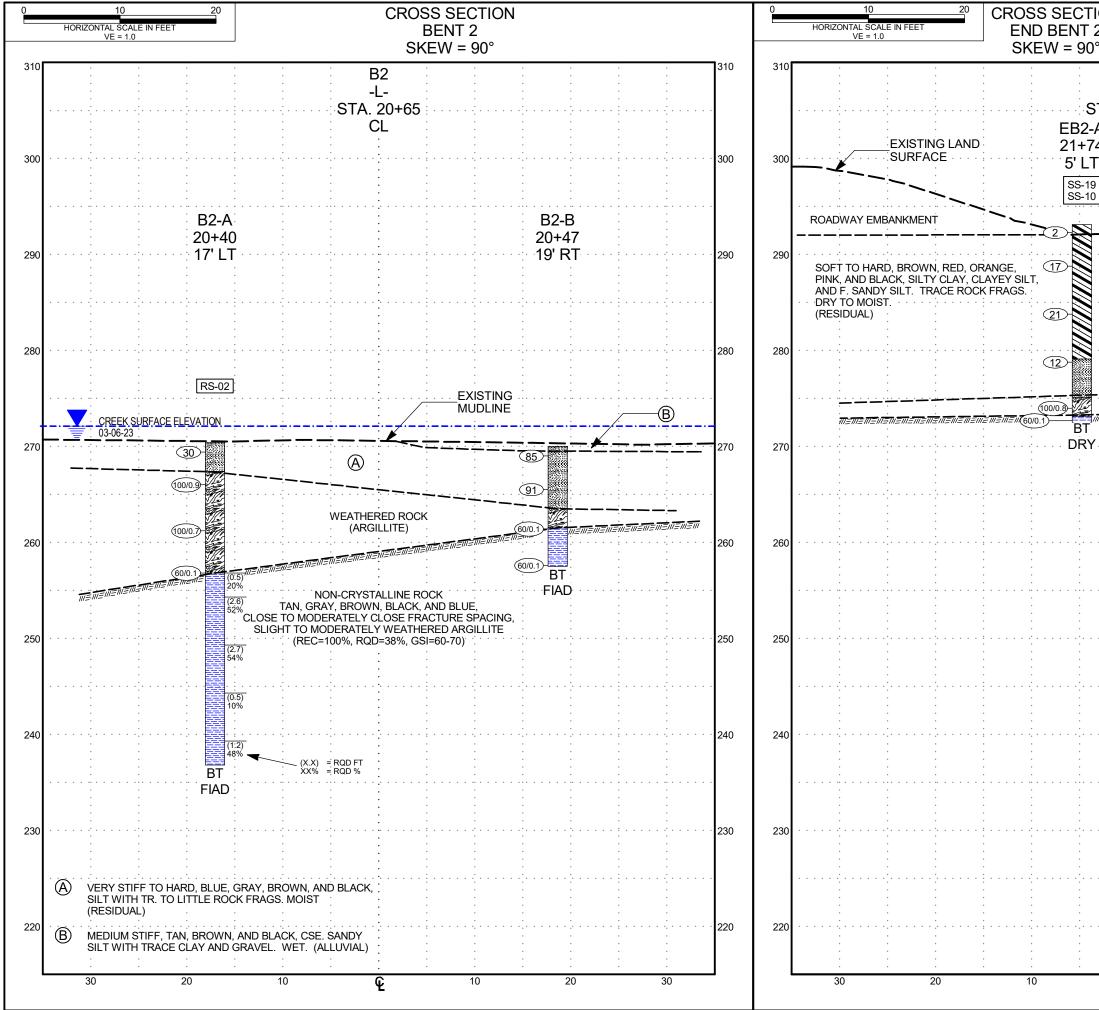


R-0063 4

SHEET

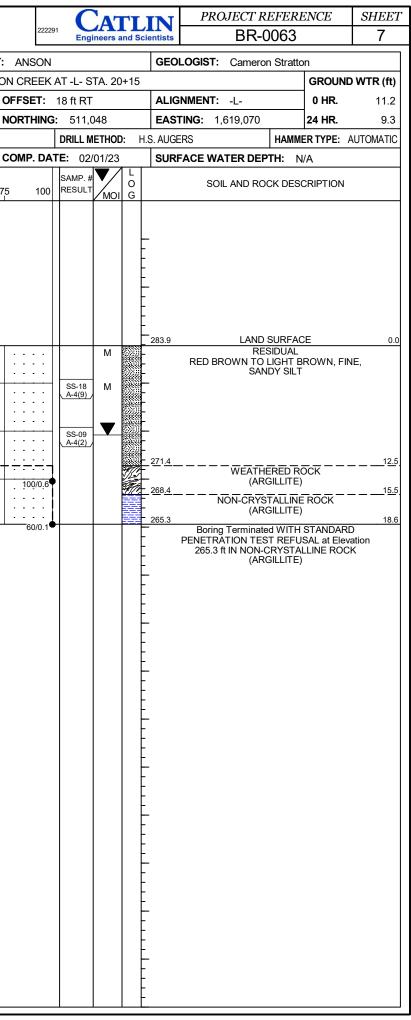
		310
· · · · · · · · · · · · · · · · · · ·	SS-19	300
	SS-10 ST-02	
		290
	21	280
TO VERY STIFF, BR DISH BROWN BEIGE SANDY CLAY, SILTY DIST TO WET. (RESIL	, AND 12 CLAY,	200
·		
	nĒnē ⁿ ≡"" DRY (01/23)	270
<u>المجارية</u>		
· · · · · · · · · · · · · · · · · · ·		260
		250
· · · · · · · · · · · · · · · · · · ·		240
		240
NOTES	GROUNDLINE PROFILE ALONG -L- GENERATED	
· · · · · · · · · · · · · · · · · · ·	FROM br0063_Is_tin.tin PROVIDED NOV. 2022 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO PROFILE	230



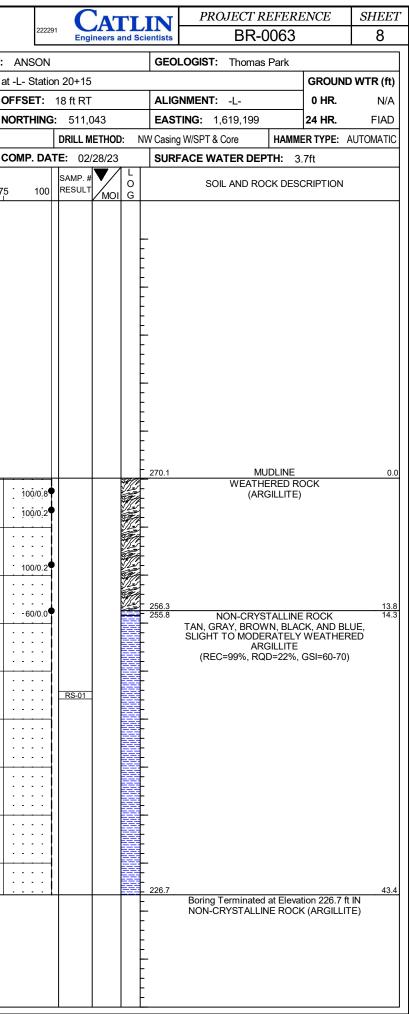


ION	(INI	PROJ	SHEET		
2	222291	ngineers and	Scientists		BR-00	63	6
							310
	B2	• •	•	•	•	-	010
	∟- 21+65	· · · · · · ·				· · · · · · · · ·	
A C		· · ·			EB2		
'4 r · · ·	EB2-C	•			21+6 24' F		300
Г і і Э]	21+72	2					
	3' RT	· · · · · ·			SS-2 SS-1	0	
	ST-02	· · ·			•	— .	
	· · · · · · · · ·			· · · · · · · · ·	5		 290
	•	· · · · ·	• • • • •	• • • •	20-	* * * *	290
•	ST-02	· · ·	· ·	•		•	
	•	· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	
	•	• •		•	36		
	•••••	 	· · · · · · · · · · · · · · · · · · ·			02/23	• • • 280
			<u>:</u>	(13 00/0.2 5477		
· · WE	ATHERE	DROCK (A	RGILLITE)				
1 <u>11=111</u> =111=	•				BT	///=///=//	
	(ARGI	CRYSTALLI LLITE)	NE ROUK		:	••••••	• • • 270
	•	• •	•	•	•	•	
	• • • • • • • •	· · · · · · · · ·				• • • • • • • •	
•	•	· · ·			•	•	
	• • • • • • • •	· · · · · · · ·	· · · · · · · ·	· • • • • • • •	• • • • • •	•	· · · 260
	•	• •		•	•	•	
• • • •	• • • • • • • • •	· · · · · ·	•		•	•	
	•	· ·	•	•	•	•	
	•	,	•		:	• •	250
	•	, . 	•	•	•	•	
•	•	,	•	• • • • • • • • •	:	•	
	•	, . 	•	•	•	• •	
	•	· · · · ·	•	•	•	•	240
••••	•	· · · · ·	• • • • • • •	• • • • • • •	• • • • • •	• • • • • • •	• • • 240
•	•	· · ·	•	•	•	•	
• • • • .	• • • • • • •	· · · · · · · · · · · · · · · · · · ·			· · · · · · · · · · · · · · · · · · ·	• • • • • • • • •	
	•	· · · · · · · · · · · · · · · · · · ·		•	•	• •	
	• • • • •	: 	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · ·	:	· · · · · · · · · · · · · · · · · · ·	• • • 230
	•	· · ·	•	•	•	• •	
	•••••	: · · · · · · · · · · · · · · · · · · ·	•••••		•••••	· · · · · · · · · · · · · · · · · · ·	
•	•	G	GENERATED		T PROVIDED	DNS br0063_ls_tin.tin	
	••••••			OVEMBER 29		• • • • • • • •	• • • 220
	•	· т	HROUGH TH	E BORINGS	WITH BOTH	•	
(<u>.</u>	10		20)	30	
٩ ٩	Ł	10	,	20	,	00	

		CAL BORING RE BORE LOG	PORT		
WBS: 67063.1.1		TY: ANSON	GEOLOGIST: Cameron Stratton	WBS: 67063.1.1	TIP: BR-0063 COUNTY:
SITE DESCRIPTION: BRIDG	E NO. 87 ON NC 742 OVER RICHAR	DSON CREEK AT -L- STA. 20+15	GROUND WTR (ft)	SITE DESCRIPTION: BRIDGE NO). 87 ON NC 742 OVER RICHARDSO
BORING NO .: EB1-A	STATION: 18+67	OFFSET: 6 ft LT	ALIGNMENT: -L- 0 HR. 10.5	BORING NO.: EB1-B	STATION: 18+65
COLLAR ELEV.: 284.2 ft	TOTAL DEPTH: 28.5 ft	NORTHING: 511,072	EASTING: 1,619,073 24 HR. 10.5	COLLAR ELEV.: 283.9 ft	TOTAL DEPTH: 18.6 ft
DRILL RIG/HAMMER EFF./DATE:	CAT2002 MOBILE B-57 92.3% 12/23/22	DRILL METHOD:	H.S. AUGERS HAMMER TYPE: AUTOMATIC	DRILL RIG/HAMMER EFF./DATE: CAT	2002 MOBILE B-57 92.3% 12/23/22
DRILLER: T. Jason White	START DATE: 02/01/23	COMP. DATE: 02/01/23	SURFACE WATER DEPTH: N/A	DRILLER: T. Jason White	START DATE: 02/01/23
ELEV DRIVE DEPTH BLOW (ft) (ft) 0.5ft 0.5			SOIL AND ROCK DESCRIPTION	ELEV DRIVE DEPTH BLOW COUN (ft) (ft) 0.5ft 0.5ft 0	
(ft) (ft) (ft) 0.5ft 0.5	ft 0.5ft 0 25 50	75 100 RESULT MOI G	ELEV. (ft) DEPTH (f	(ft) (ft) (ft) (ft) $0.5ft$ $0.5ft$ 0	.5ft 0 25 50 75
295				295	
290			- - 	290	
285 284.2 0.0 WOH 2	4	SS-16 M	- 	283.9 - 0.0	2
280.7 + 3.5 280 - 5 5		A-4(8)	REDDISH BROWN, LIGHT BROWN, LIGHT BLUE, GRAY, AND BLACK, SANDY SILT	280 280.4 7 3.5	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
		· · · · · · <u>A-4(9)</u>			
275 275.7 - 8.5				275 275.4 8.5	
			-		4
270.7 + 13.5	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		-	270 270.4 13.5	
	11		269.7 14.3 ORANGE, BLUE, BIEGE, BROWN, SILTY		
			CLAY, TR. ROCK FRAGS.		
<u>265</u> <u>265.7</u> <u>18.5</u> <u>15</u> <u>85/0</u>		D	WEATHERED ROCK (ARGILLITE)	265.4 18.5	
	:::: :::: .:::		262.7		
<u>260</u> <u>260.7 - 23.5</u> <u>46</u> 23			BLUE, BIEGE, BLACK, ORANGE, SILT		
		· · · · · · <u>A-4(7)</u>			
255.7 + 28.5			256.2		
			(ARGILLITE) Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation		
			255.7 ft IN NON-CRYSTALLINE ROCK (ARGILLITE)		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		
			-		

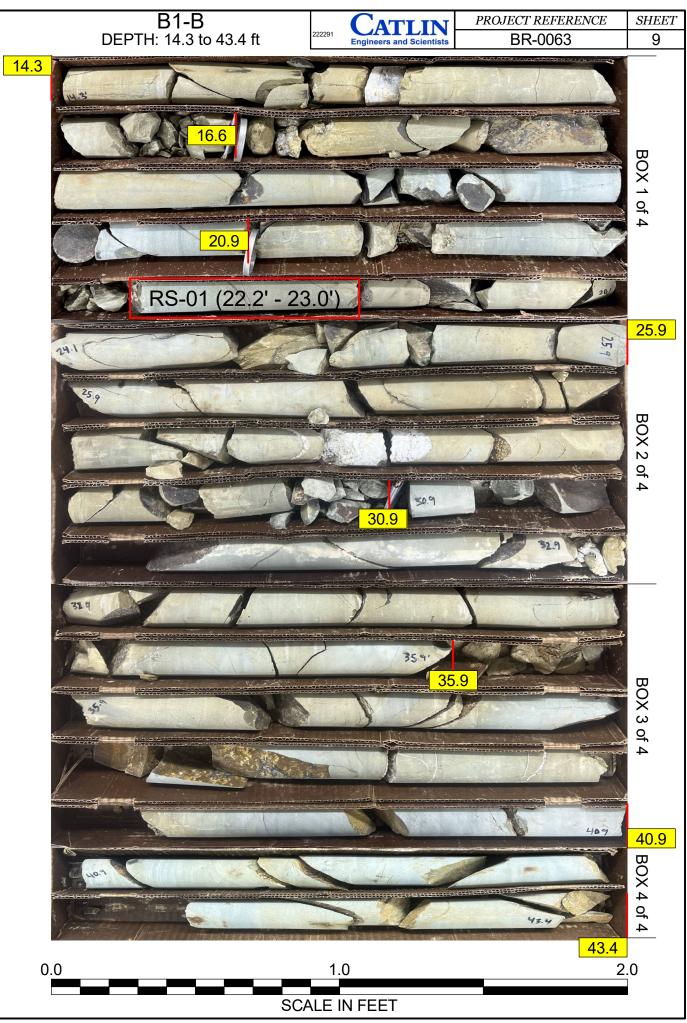


		CAL BORING RE ORE LOG	PORT		
WBS: 67063.1.1		Y: ANSON	GEOLOGIST: Cameron Stratton	WBS: 67063.1.1	TIP: BR-0063 COUNTY:
SITE DESCRIPTION: Bridge No. 8	37 on NC 742 over Richardson Cree	k at -L- Station 20+15	GROUND WTR (ft)	SITE DESCRIPTION: Bridge No. 8	7 on NC 742 over Richardson Creek a
BORING NO.: B1-A	STATION: 19+93	OFFSET: 13 ft LT	ALIGNMENT: -L- 0 HR. 0.0	BORING NO.: B1-B	STATION: 19+95
COLLAR ELEV.: 270.1 ft	TOTAL DEPTH: 16.1 ft	NORTHING: 511,074	EASTING: 1,619,199 24 HR. FIAD	COLLAR ELEV.: 270.1 ft	TOTAL DEPTH: 43.4 ft
DRILL RIG/HAMMER EFF./DATE: CAT	1314 CME-45B 85.8% 02/15/2022	DRILL METHOD:	W Casing w/ SPT HAMMER TYPE: AUTOMATIC	DRILL RIG/HAMMER EFF./DATE: CAT	1314 CME-45B 85.8% 02/15/2022
DRILLER: Austin Fowler	START DATE: 03/02/23	COMP. DATE: 03/02/23	SURFACE WATER DEPTH: 3.8ft	DRILLER: Jordan Edmondson	START DATE: 02/27/23
ELEV DRIVE DEPTH BLOW COUN (ft) (ft) (ft) 0.5ft 0.5ft 0			SOIL AND ROCK DESCRIPTION	ELEV DRIVE ELEV (ft) 0.5ft 0.5ft 0	
(II) (ft) (II) 0.5ft 0.5ft 0		75 100 RESULT MOI G	ELEV. (ft) DEPTH (ft) (ft) (ft) 0.5ft 0.5ft 0	
295				295	
-			-	-	
			-		
290 +			-		
			-		
285			-	285	
			-		
			-		
			-		
			-		
275			-	275	
			-		
270 270.1 0.0					
		100/0.9	WEATHERED ROCK (ARGILLITE)		/0.3
266.6 + 3.5			-	267.0 3.1	
			-		
261.6 + 8.5			-	261.3 8.8	
260 + 100/0.3		100/0.3	-	260 100/0.2	
			-		
<u>256.6 + 13.5</u> 255 <u>+</u> 100/0.3				256.3 13.8 60/0.0	
254.1 16.0		60/0.1	► 254.0/ NON-CRYSTALLINE ROCK		
			(ARGILLITE) Boring Terminated WITH STANDARD		
			 PENETRĂTION TEST at Elevation 254.0 ft IN NON-CRYSTALLINE ROCK (ARGILLITE) 		
			-	245	
			-		
			-	240	
			-		
			-		
			-		
			-		
			-	230	
			-		
			-		
			-		
			-		
			-		



GEOTECHNICAL BORING REPORT

WBS	: 6706	3.1.1			TIP:	BR-0	063	С			RE L			GEOLOGIST: Thomas	s Park								
	DESCR		: Brid	dge No. 8								n 20+15				GROUN	D WTR (ft						
BORI	NG NO.:	: B1-E		<u> </u>	1							7 on NC 742 over Richardson Creek at -L- Station 20+15 STATION: 19+95 OFFSET: 18 ft RT ALIGN								ALIGNMENT: -L-		0 HR.	N/A
COLI	AR ELE	EV .: 2	70.1 ft		тот	AL DE	PTH : 43	8.4 ft		NO	RTHING	: 511,043		EASTING: 1,619,199		24 HR.	FIAD						
DRILL	RIG/HAN	IMER EF	F./DATI	E: CAT	1314 CN	1E-45B	85.8% 02/1	5/2022				DRILL METH	IOD: NW	IW Casing W/SPT & Core HAMMER TYPE: AUTOMATIC									
DRIL	LER: J	lordan E	Edmon	dson	STA	RT DA	TE: 02/2	27/23		co	MP. DA	TE: 02/28/2	23	SURFACE WATER DE	JRFACE WATER DEPTH: 3.7ft								
COR	E SIZE:	NQ			тоти	AL RU	N: 29.1	ft															
ELEV	RUN ELEV	DEPTH	RUN	DRILL RATE	REC.	JN RQD	SAMP.	STR REC.	RATA RQD	L O					VS								
(ft)	elev (ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	G	ELEV. (ft)	Di	ESCRIPTION AND REMAR	NO		DEPTH						
255.8	0EE 0	14.3				(2.2)		(Begin Coring @ 14.3 ft									
255	255.8_ 253.5 -		2.3	2:09/1.0	100%	(0.9) 39%		(28.7) 99%			- 255.8			NON-CRYSTALLINE ROON, BLACK, AND BLUE, SLI	GHT TO N								
	-	÷	4.3	0:29/0.3	(4.1) 95%	(0.4) 9%				薹		WEATHE		DSE TO WIDE FRACTURE CAL FRACTURES. HARD /			R						
250	- 249.2	20.9		0:51/1.0 2:46/1.0 1:02/1.0 1:32/1.0 2:01/0.3		-				÷													
	-	-	5.0	2:01/0.3 3:02/1.0	(4.8) 96%	(0.0) 0%				<u></u>													
245	-	ŧ		3:02/1.0 3:14/1.0 3:22/1.0 3:01/1.0	3070	070	RS-01	1		霻													
145	244.2	25.9	5.0	3:51/1.0 3:24/1.0		(0.8)				_	-												
	-	ł	0.0	3:37/1.0	100%	16%				薹													
240	- 239.2	- 30.9		2:04/1.0																			
	-		5.0	1:19/1.0 2:03/1.0	(5.0)	(2.0) 40%																	
35	-	ŧ		2:05/1.0 2:13/1.0	100 %	40 %																	
.55	234.2	35.9	5.0	1:51/1.0		(1.9)	-			Ē													
	-	ł	0.0	1:39/1.0	100%	38%				薹													
30	- 229.2	40.9		2:32/1.0																			
	-	ţ	2.5	1:49/1.0	(2.5)	(0.3) 12%				Ē	•												
	226.7 -	<u>43.4</u>		0:51/0.5		12 /0					226.7	Boring Ter	rminated a	t Elevation 226.7 ft IN NON	-CRYSTA	LLINE RO	43 CK						
	-	ŧ									-			(ARGILLITE)									
	-	ł																					
	-	ŧ																					
	-	+																					
	-	+																					
	-	÷									-												
	-	ŧ																					
	-	ŧ									—												
	-	ł				1	R	00	K 7	F	ST I	RESU											
	-	Į		SAI	MPLE		DEPTH		RO			NIT WT.		XIAL COMPRESSIVE									
	-	ł			MBER	Γ	NTERVA	L	TY	PE		(lb/ft3)		STRENGTH (psi)									
	-	+		R	S-01	2	2.2' - 23.	.0' /	ARGI		Ξ	162.9		14,720									
	-	Ē																					
	-	L .																					
	-	ŧ																					
	-	ŧ																					
	-	F									-												
	-	ŧ																					
	-	Ł								[
	-	É									•												
	-	£																					
		Ē									-												
	-	F									•												
	-	T	1	1	1	1	1	1	1	1 H	•												



GEOTECHNICAL BORING REI BORE LOG	PORT		222291 CATLIN PROJECT REFERENCE SHEET BR-0063 10
WBS: 67063.1.1 TIP: BR-0063 COUNTY: ANSON	GEOLOGIST: Cameron Stratton	WBS: 67063.1.1 TIP: BR-0063 COUNTY:	ANSON GEOLOGIST: Cameron Stratton
SITE DESCRIPTION: Replace Bridge Number 87 on NC 742 over Richardson Creek	GROUND WTR (ft)	SITE DESCRIPTION: Replace Bridge Number 87 on NC 742 over Richard	rdson Creek GROUND WTR (ft)
BORING NO.: B2-A STATION: 20+40 OFFSET: 17 ft LT	ALIGNMENT: -L- 0 HR. 0.0		OFFSET: 19 ft RT ALIGNMENT: -L- 0 HR. 0.0
COLLAR ELEV.: 270.4 ft TOTAL DEPTH: 33.6 ft NORTHING: 511,076	EASTING: 1,619,246 24 HR. FIAD		IORTHING: 511,040 EASTING: 1,619,252 24 HR. FIAD
	W Casing W/SPT & Core HAMMER TYPE: AUTOMATIC	DRILL RIG/HAMMER EFF./DATE: CAT1314 CME-45B 85.8% 02/15/2022	DRILL METHOD: NW Casing w/ SPT HAMMER TYPE: AUTOMATIC
DRILLER: Austin Fowler START DATE: 03/03/23 COMP. DATE: 03/03/23	SURFACE WATER DEPTH: 3.7ft		COMP. DATE: 03/02/23 SURFACE WATER DEPTH: 3.7ft
ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUNT BLOWS PER FOOT SAMP. # L O (ft) 0.5ft 0.5ft 0.5ft 0 25 50 75 100 RESULT MOI G	SOIL AND ROCK DESCRIPTION	ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUNT BLOWS PER FOOT 0 25 50 75	5 100 RESULT MOI G
	ELEV. (ft) DEPTH (ft) 	295	
		290	
270 WOH 4 26 • 30 M	BLUE, GRAY, AND BROWN, SILT WITH TR. ROCK FRAGS.	270 270.0 0.0 7 24 61	
266.9 3.5 49 51/0.4 <th< td=""><td></td><td>265 - 27 23 68 · · · · · · · · · · · · · · ·</td><td> \ SILT W/TR. CLAY AND GRAVEL</td></th<>		265 - 27 23 68 · · · · · · · · · · · · · · ·	\ SILT W/TR. CLAY AND GRAVEL
	(ARGILLITE)		$\begin{array}{c c} \hline & & \\ \hline \\ \hline$
			WEATHERED ROCK
260 48 52/0.2	-		
	- - 256 9 12 6		(ARGILLITE)
	256.8 13.6 NON-CRYSTALLINE ROCK TAN, GRAY, BROWN, BLACK, AND BLUE,	60/0.1	60/0.1 Boring Terminated WITH STANDARD
RS-02	SLIGHT TO MODERATELY WEATHERED ARGILLITE		REFUSAL at Elevation 257.5 ft IN NONCRYSTALLINE ROCK (ARGILLITE)
	(REC=100%, RQD=38%, GSI=60-70)		
8	-		
	-		
	-		
g ‡ ···· ··· ···· ····	236.8 33.6		
	Boring Terminated at Elevation 236.8 ft IN NONCRYSTALLINE ROCK (ARGILLITE)		
	-		
	-		
	-		

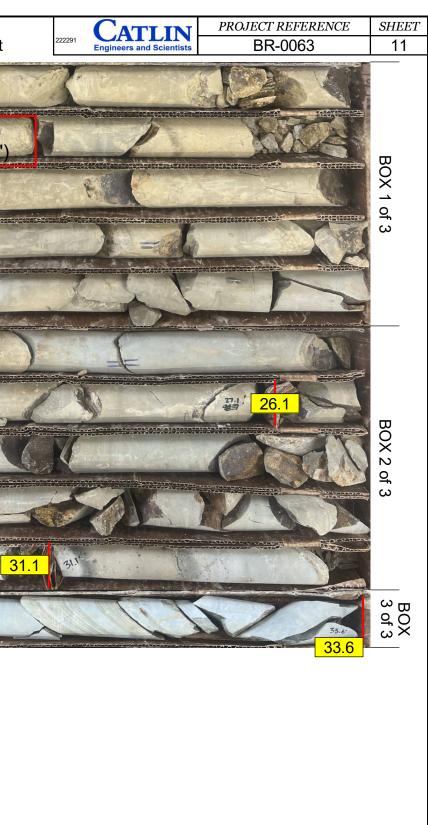
GEOTECHNICAL BORING REPORT CORE LOG

WBS: 67063.1.1					CORE LOG TIP: BR-0063 COUNTY: ANSON GEOLOGIST: Camera								T: Camero	n Stratto	n			
SITE	DESCR	IPTION:	Brie	dge No. 8	7 on N	C 742	over Rich	ardsor	n Cree	k at -L	- Statio	n 20+15					GROUND	WTR (fi
BORI	NG NO.:	: B2-A	4		STATION: 20+40						SET:	17 ft LT		ALIGNMEN		0 HR.	0.0	
COLL	AR ELE	EV.: 2	70.4 ft		TOTAL DEPTH: 33.6 ft						NORTHING: 511,076			EASTING: 1,619,246			24 HR.	FIAD
DRILL	RIG/HAN	IMER EF	F./DAT	E: CAT	1314 CME-45B 85.8% 02/15/2022							DRILL METH	IOD: NV	V Casing W/SPT	& Core	HAMM	ER TYPE: A	UTOMATI
DRILI	LER: A	Austin F	owler		START DATE: 03/03/23						IP. DA	TE: 03/03/	23	SURFACE \	NATER DEP	тн: з.	.7ft	
CORE	E SIZE:	NQ			тоти	AL RUI	N: 20.0	ft										
ELEV RUN ELEV (ft) DEPTH RUN (ft) DRILL RATE (Min/ft)					RUN SAMP. STRATA REC. RQD NO. (ft) (f													DEPTH (
56.8	050 0													Begin Corin				
255	256.8 _ 254.3 -	13.6 16.1	2.5	1:52/1.0 2:34/1.0	(2.5) 100%	(0.5) 20%		(20.0) 100%		<u></u>	256.8			NON-CRYST IGHT BLUE, G	RAY, AND BL	ACK, MC		
	-	- 10.1	5.0	2:05/0.5	(5.0) 100%	(2.6) 52%	RS-02			_				ERED WITH S OSE FRACTU				
	-	ŧ		2:25/1.0 2:54/1.0 3:04/1.0 3:40/1.0	100%	52.70				<u></u>								
250	249.3	21.1	50	3:44/1.0	(5.0)	(0.7)												
	-	Ł	5.0	2:45/1.0 3:30/1.0	(5.0) 100%	(2.7) 54%				<u>_</u>								
245	-			3:23/1.0 2:43/1.0						-	_							
	244.3	26.1	5.0	3:01/1.0 2:34/1.0	(5.0)	(0.5)				<u>-</u>	-							
	-	F		2:23/1.0 2:55/1.0	100%	10%												
240	239.3-	31.1		2:23/1.0 2:27/1.0						臺-	-							
	236.8	33.6	2.5	2:33/1.0 3:47/1.0	(2.5)	(1.2) 48%				-	236.8							0.
	230.0 -	- 33.0		1:07/0.5		-				-	230.0	Boring Te	rminated a	at Elevation 236	6.8 ft IN NON- ILLITE)	CRYSTA	LLINE ROC	3: K
	-																	
	-	+									-							
	-	ł																
	-	Ł								ΙĿ	-							
	-	ŧ																
	-	ŧ.																
	-	F																
	-	Ł																
	-	Ł								ΙĿ	_							
	-	E								ΙE								
	-	F																
	-						R	OC	'K 7	TES	ST I	RESU	LTS					
			MPLE DEPTH MBER INTERVAL			RO TY	ROCK UNIT WT. TYPE (lb/ft3)				UNIAXIAL COMPRESSIVE STRENGTH (psi)							
RS-02 16.1' - 16				6.1' - 16	.6' /	ARGI	LITE		161.0		14,720							
	-	F									-							
	-	+																
	-	ŧ.																
	-	ł																
	-	ŧ																
	-	F									-							
	-	Ł								ΙĿ								
	-	Ł								E	_							
	-	F								F	-							
	-	F																
	-	F								-	-							
	-	t	I	1	1			1	1	1 F								

13.6 **RS-02** 16.1 (16.1' - 16.6') 21.1 223

B2-A DEPTH: 13.6 to 33.6 ft







		AL BORING REF DRE LOG	PORT			
WBS: 67063.1.1	TIP: BR-0063 COUNTY:		GEOLOGIST: Cameron Stratt	on	WBS: 67063.1.1	TIP: BR-0063 COUNTY
SITE DESCRIPTION: BRIDGE NO	D. 87 ON NC 742 OVER RICHARDSON	N CREEK AT -L- STA. 20+15		GROUND WTR (ft)	SITE DESCRIPTION: BRIDGE NO	D. 87 ON NC 742 OVER RICHARDS
BORING NO.: EB2-A	STATION: 21+74 O	DFFSET: 5 ft LT	ALIGNMENT: -L-	0 HR. Dry	BORING NO.: EB2-B	STATION: 21+68
COLLAR ELEV.: 293.1 ft	TOTAL DEPTH: 20.6 ft N	IORTHING: 511,058	EASTING: 1,619,380	24 HR. Dry	COLLAR ELEV.: 292.3 ft	TOTAL DEPTH: 18.5 ft
DRILL RIG/HAMMER EFF./DATE: CAT	2002 MOBILE B-57 92.3% 12/23/22	DRILL METHOD: H.S	S. AUGERS HAMN	IER TYPE: AUTOMATIC	DRILL RIG/HAMMER EFF./DATE: CAT	T2002 MOBILE B-57 92.3% 12/23/22
DRILLER: T. Jason White	START DATE: 01/31/23 C	COMP. DATE: 01/31/23	SURFACE WATER DEPTH:	N/A	DRILLER: T. Jason White	START DATE: 01/31/23
		SAMP. #	SOIL AND ROCK DES	SCRIPTION		
(ft) (ft) (ft) 0.5ft 0.5ft 0).5ft 0 25 50 75	5 100 RESULT MOI G	ELEV. (ft)	DEPTH (ft)	(ft) (ft) (ft) 0.5ft 0.5ft 0	D.5ft 0 25 50 7
295				05	295	
		w 🗙	293.1 LAND SURFA RESIDUAL		292.3 1 0.0	3
290 289.6 3.5	· · · · · · · · · · · · · · · · · · ·		TAN, RED, BLACK, BROW	N, SILTY CLAY	290	° ♠5
		SS-19 A-6(11)			288.8 + 3.5	$10 \begin{vmatrix} \cdot \cdot \cdot \cdot \\ \cdot \cdot \cdot \\ 20 \end{vmatrix} \cdot \cdot \cdot \cdot \begin{vmatrix} \cdot \cdot \cdot \cdot \\ \cdot \cdot \cdot \\ \cdot \cdot \cdot \end{vmatrix}$
284.6 + 8.5	$12 \qquad \cdot \cdot \cdot \cdot 1 \qquad \cdot \cdot \cdot \cdot 1 \qquad \cdot \cdot \cdot \cdot \cdot 1 \qquad \cdot \cdot \cdot \cdot$	···· ss-10 M			283 8 8 5	21
		<u>A-6(11)</u>				• • • • • • • • • • • • • • • • • • • •
280 279.6 13.5			279.1	14.0	280	
		D	BLUE/GRAY SILT, TRACE			7
275 274 5 40 5			275.1	18.0	276.3 16.0	
274.6 + 18.5 272.6 + 20.5 272.6 + 20.5		100/0.8• D		ROCK	273.8 18.5	
- 60/0.1		60/0.1	NON-CRYSTALLIN (ARGILLITE)	EROCK []		
			Boring Terminated BY AUG Elevation 272.5 ft IN NON-	ER REFUSAL at		
			ROCK (ARGILL			
		E				
NCDOT BORE DOUBLE BR0063 GEO_BRDG87_CATLIN.GPJ_NCDOT_CATLIN.GDT_04/05/23						

						D		ים די	EFERF	INCE	SHEET
222291 CATLI Engineers and Scie						<u>г</u> .		-	063	INCE	12
Y: ANS	SON				GEO	LOGIS			Stratto	n	·]
ON CRE	EEK A	AT -L- S	TA. 20	+15						GROUNE	OWTR (ft)
OFFSE	:T: ;	24 ft RT			ALIG	NMEN	T: -L-			0 HR.	15.8
NORTH	IING:	511,0)29		EAS	ring:	1,619,	373		24 HR.	12.9
		DRILL M): H	I.S. AUGE	-				ER TYPE: /	AUTOMATIC
COMP.		E: 01/		LI	SUR		WATER	R DEP	TH: N	/A	
75	100	SAMP. # RESULT	моі	O G		5	SOIL AN	D ROC	K DESC	CRIPTION	
	<u>10.2</u>	SS-20 A-6(12)/ SS-11 A-4(5)			292.3	O RED, CLA BOI PENET	LIGHT XIDIZEC BLUE, C YEY SIL WI	RES BLUE,) SILTY ORANC T WIT AND EATHE (ARC minates NON-C	(TO SA GE, PINK H TR. R SAND RED RC SILLITE) J WITH T REFUS	V, AND GR NDY CLAY (, AND BLA OCK FRAG	1 <u>1.5</u> . CK, SS. <u>16.0</u> 18.5 D ation

LABORATORY **SUMMARY SHEET**

AASHTO Standard Specifications

(As modified by NCDOT, Material and Tests Unit, 2000.)

						т	EST RES	ULTS					
Proj. Sample Number	ST-01	ST-02	SS-16	SS-17	SS-08	SS-18	SS-09	SS-19	SS-10	SS-20	SS-11		
Lab Sample Number	ST-01	ST-02	SS-16	SS-17	SS-08	SS-18	SS-09	SS-19	SS-10	SS-20	SS-11		
Retained #4 Sieve %	0	1.3	0.1	0.4	0	0.1	1.5	0	5.4	18.2	10.4		
Passing #10 Sieve %	99.8	97.4	99.4	98.4	100	99.9	97.9	97.5	90.1	62.3	85.7		
Passing #40 Sieve %	91	96	98	96	100	99	85	95	85	55	80		
Passing #200 Sieve %	58	88	90	89	99	93	61	92	78	52	68		
				11		MINUS	NUMBER 1	FRACTION		1			
SOIL MORTAR - 100%													
Coarse Sand Ret#60 %	23.7	2.9	3.0	3.9	0.5	2.3	26.4	2.9	8.0	12.9	11.5		
Fine Sand Ret#270 %	21.1	8.3	9.9	9.1	2.6	8.4	14.0	4.9	7.6	5.1	11.4		
Silt 0.05 - 0.005mm %	29.0	53.0	45.7	50.5	65.9	50.3	35.2	54.1	49.1	55.0	47.1		
Clay <0.005mm %	26.3	35.8	41.4	36.5	31.0	39.0	24.4	38.0	35.3	27.0	30.0		
	1												
Liquid Limit (LL	25	36	32	36	32	33	25	33	36	38	29		
Plasticity Index (PI) 7	17	9	9	6	9	7	11	12	14	7		
AASHTO Classification /Group Index	A-4(2)	A-6(15)	A-4(8)	A-4(9)	A-4(7)	A-4(9)	A-4(2)	A-6(11)	<mark>A-6(11</mark>)	A-6(12)	A-4(5)		
Organic Content %	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A		
Station	18+62	21+72	18+67	18+67	18+67	18+65	18+65	21+74	21+74	21+68	21+68		
Offset	9ft RT	3ft RT	6ft LT	6ft LT	6ft LT	18ft RT	18ft RT	5ft LT	5ft LT	24ft RT	24ft RT		
Alignment	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-		
Boring Identification	EB1-C	EB2-C	EB1-A	EB1-A	EB1-A	EB1-B	EB1-B	EB2-A	EB2-A	EB2-B	EB2-B		
Depth (FT)	6.0	5.0	0.0	3.5	23.5	3.5	8.5	3.5	8.5	0.0	13.5		
to	8.0	7.0	1.5	5.0	25.0	5.0	10.0	5.0	10.0	1.5	15.0		
Field Moist. Content %													
Tested By	Geotechnics	Geotechnics	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON	MDMASON		
Submitted By	C. Stratton	C. Stratton	S.V. HUDSON	S.V. HUDSON	CDFUTRAL	S.V. HUDSON	CDFUTRAL	S.V. HUDSON	CDFUTRAL	S.V. HUDSON	CDFUTRAL		
Date Submitted	04/04/23	04/04/23	03/23/23	03/23/23	02/13/23	03/23/23	02/13/23	03/23/23	02/13/23	03/23/23	02/13/23		

NP = Non-Plastic

NEM = Not Enough Material for Analysis

N/A = Not Applicable / Not Analyzed

Laboratory Manager



Report Date: 6/14/2023

Laboratory Report Page 1 of 1

SITE PHOTOGRAPHS



RIG ON EB1-C FACING UP STATION



EB2 IN FOREGROUND FACING DOWN STATION





PROJECT REFERENCE	SHEET
BR-0063	14

NEAR EB2 FACING DOWN STATION