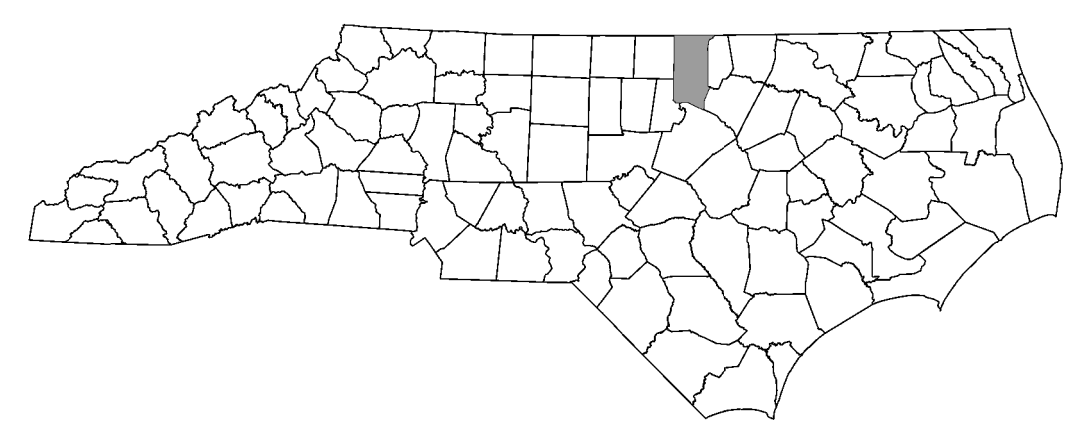


9/18/2024 1:35:46 PM L:\Structures\2024\U6020 Updates - NCDOT Bridge Preservation\DCN\400\_001\_U-6020\_SMU\_TSH.dgn  
printer.pitcfig Total.tbl

**TIP PROJECT: U-6020**  
**CONTRACT: C204944**



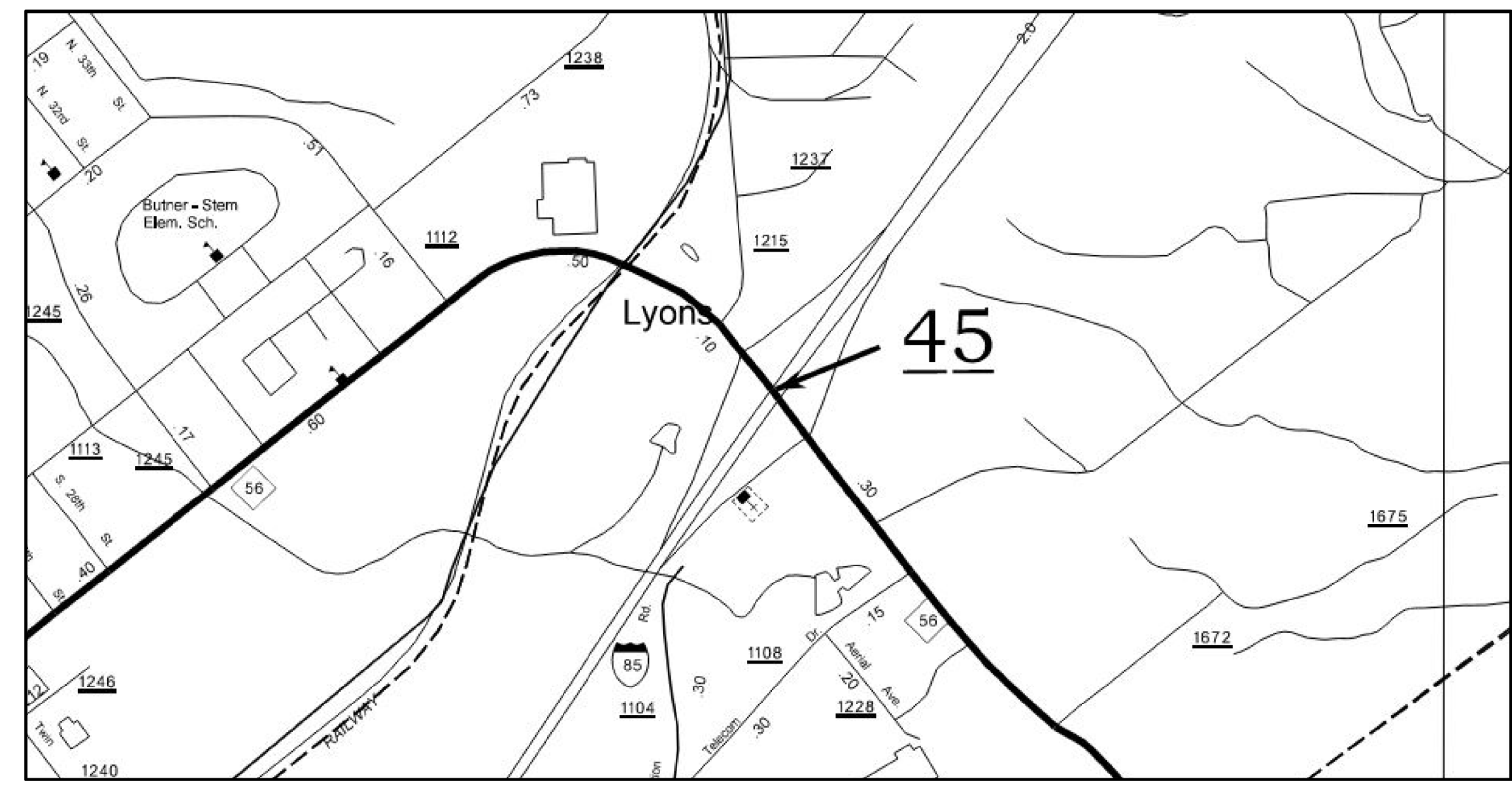
STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**GRANVILLE COUNTY**

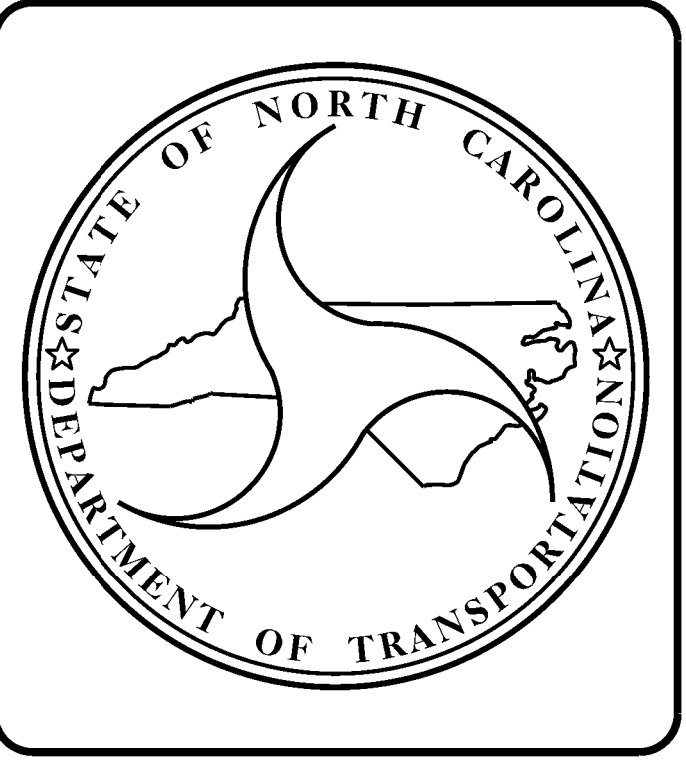
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-6020	1	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
47165.1.1	-	P.E.	
47165.3.1	-	CONSTRUCTION	

**LOCATION - GRANVILLE COUNTY :**  
**BRIDGE #45 ON NC 56 OVER I-85**

**TYPE OF WORK: BRIDGE PRESERVATION - SUBSTRUCTURE REPAIRS, BEAM REPAIRS, AND CLEANING AND PAINTING OF EXISTING STEEL BRIDGE STRUCTURE.**



**VICINITY MAP - GRANVILLE COUNTY**

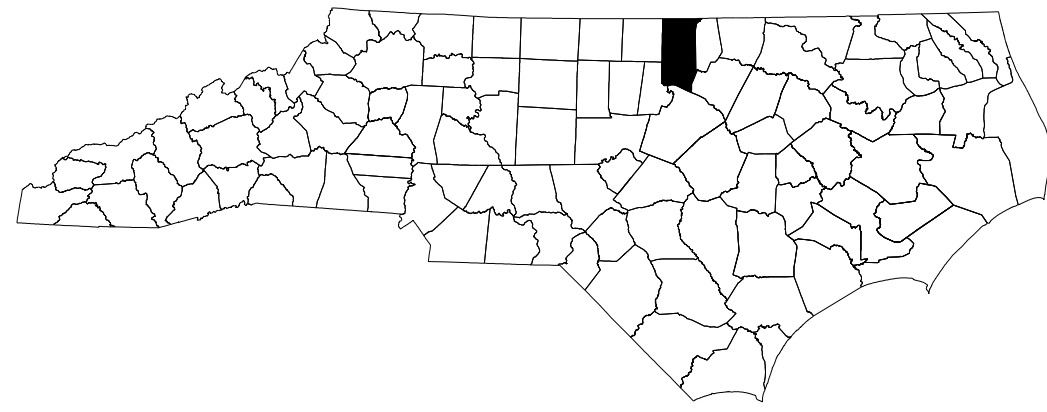


**DESIGN DATA**  
GRANVILLE COUNTY  
#45 ADT 2013 = 15,000

**PROJECT LENGTH**  
GRANVILLE COUNTY  
- #45 = 0.1 MILES

**wsp**  
WSP USA Inc.  
128 TALBERT RD.  
SUITE A  
MOORESVILLE, NC 28117  
TEL: 1.704.662.0100  
LICENSE NO. F-0165  
**MICHAEL W. CRAIG**  
PROJECT ENGINEER  
2024 STANDARD SPECIFICATIONS  
**LETTING DATE: NOVEMBER 19, 2024**

**PROFESSIONAL SEAL**  
NORTH CAROLINA  
ENGINEER  
MICHAEL W. CRAIG  
027320  
9/18/2024  
**MICHAEL W. CRAIG**

**TIP PROJECT: U-6020****CONTRACT: C204944**

STATE OF NORTH CAROLINA

DIVISION OF HIGHWAYS

# GRANVILLE COUNTY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-6020	1A	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
47165.1.1	-	P.E.	
47165.3.1	-	CONSTRUCTION	

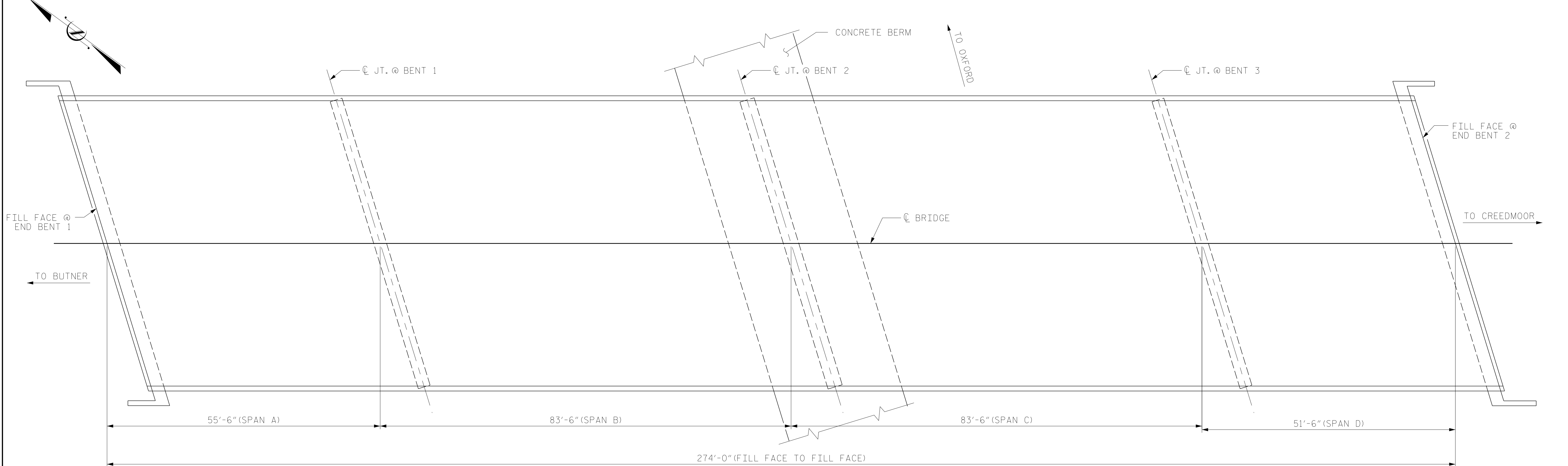
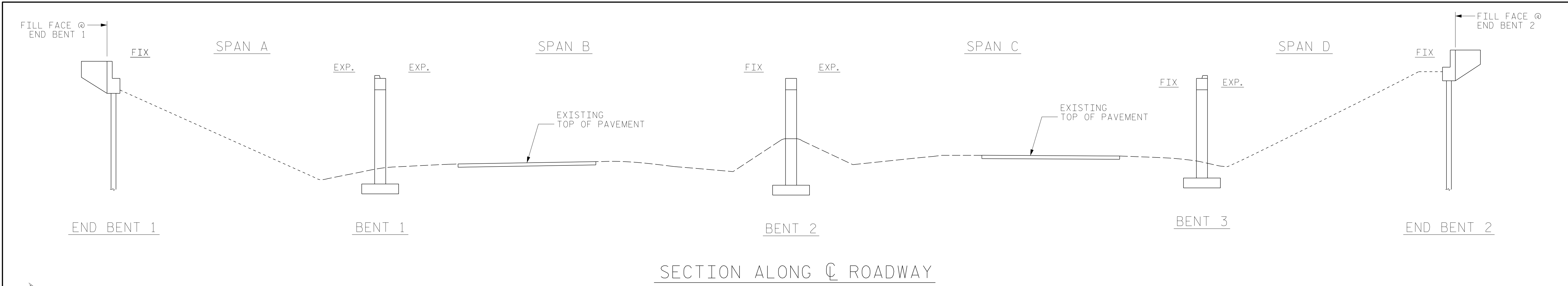
**LOCATION - GRANVILLE COUNTY :**

**BRIDGE #45 ON NC 56 OVER I-85**

**TYPE OF WORK: BRIDGE PRESERVATION - SUBSTRUCTURE REPAIRS, BEAM REPAIRS, AND CLEANING AND PAINTING OF EXISTING STEEL BRIDGE STRUCTURE.**

## INDEX OF SHEETS

<b>1</b>	<b>TITLE SHEET</b>
<b>1A</b>	<b>INDEX OF SHEETS</b>
<b>S-1 THRU S-11</b>	<b>STRUCTURAL PLANS - GRANVILLE COUNTY, BRIDGE NO. 45</b>
<b>S-12</b>	<b>BEAM REPAIR DETAILS</b>
<b>S-13</b>	<b>BEAM PLATING REPAIR DETAILS</b>
<b>S-14</b>	<b>BEARING DETAILS</b>
<b>S-15</b>	<b>TYPICAL CAP AND COLUMN REPAIR DETAILS</b>
<b>S-16</b>	<b>JACKING DETAILS</b>
<b>SN</b>	<b>STANDARD NOTES</b>



**PLAN**

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

**NOTES:**

PROFILE INFORMATION IS TAKEN FROM THE ORIGINAL PLANS AND THE ROUTINE INSPECTION REPORT DATED 7/27/2018.

BRIDGE ORIENTATION CONFORMS TO EXISTING BRIDGE PLANS.

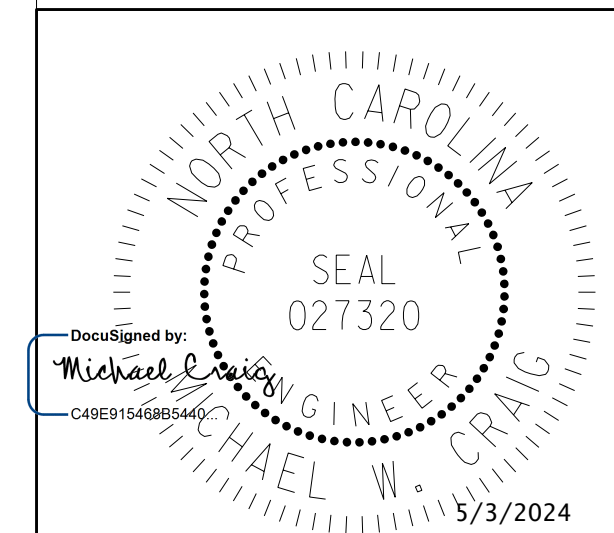
**SCOPE OF WORK**

- REPAIR STEEL BEAMS AND BEARINGS.
- CLEANING AND PAINTING OF STEEL BEAMS.
- REMOVE DEBRIS FROM TOP OF END BENT AND BENT CAPS.
- EPOXY INJECTION OF CONCRETE CRACKS.
- CLEAN AND REPAIR REBAR IN CONCRETE REPAIR AREAS.
- PERFORM SHOTCRETE AND CONCRETE REPAIRS IN PREPARED AREAS.
- APPLY EPOXY COATING TO TOP OF BENT CAPS.

I hereby certify that this structure was rehabilitated according to these plans or as noted herein.

\_\_\_\_\_  
 Resident Engineer Date

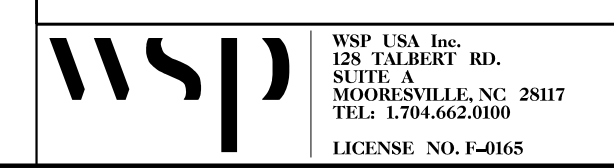
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



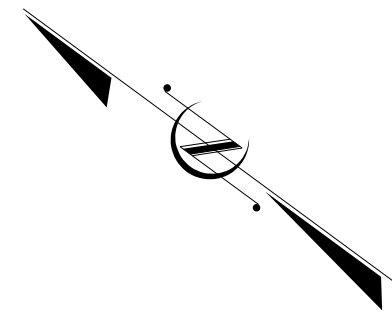
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
 FOR BRIDGE ON NC 56  
 BETWEEN BUTNER AND  
 CREDMOOR OVER I-85

DRAWN BY :	J.MYA	DATE :	3/2019
CHECKED BY :	J.YANNACCONE	DATE :	3/2019
DESIGN ENGINEER OF RECORD :	MICHAEL W. CRAIG	DATE :	4/2024



REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			TOTAL SHEETS
2			4			16



LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECT THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

TOTAL BILL OF MATERIAL

BRIDGE NO.	CONCRETE REPAIRS	SHOTCRETE REPAIRS	EPOXY RESIN INJECTION	CLEANING AND REPAINTING OF BRIDGE #45	PAINTING CONTAINMENT FOR BRIDGE #45	POLLUTION CONTROL	BEAM REPAIR	BEAM PLATING REPAIR	EPOXY COATING	STEEL BEARING REPLACEMENT	TYPE I BRIDGE JACKING BRIDGE #45
	CU. FT.	CU. FT.	LIN. FT.	LUMP SUM	LUMP SUM	LUMP SUM	LBS.	LBS.	SQ. FT.	EA.	EA.
380045	8.3	90.0	68.5	LUMP SUM	LUMP SUM	LUMP SUM	2,370	330	459	10	19

NOTES:

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLAN SHEETS.

FOR BEAM REPAIR, SEE SPECIAL PROVISIONS.

FOR BEAM PLATING REPAIR, SEE SPECIAL PROVISIONS.

FOR STEEL BEARING REPLACEMENT, SEE SPECIAL PROVISIONS.

FOR CLEANING AND REPAINTING OF BRIDGE, SEE PAINTING EXISTING STRUCTURE SPECIAL PROVISION.

FOR PAINTING CONTAINMENT, SEE PAINTING EXISTING STRUCTURE SPECIAL PROVISION.

FOR POLLUTION CONTROL, SEE PAINTING EXISTING STRUCTURE SPECIAL PRVISION.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR TYPE I AND TYPE II BRIDGE JACKING, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSE WORK AND FORM WORK, SEE SPECIAL PROVISIONS.

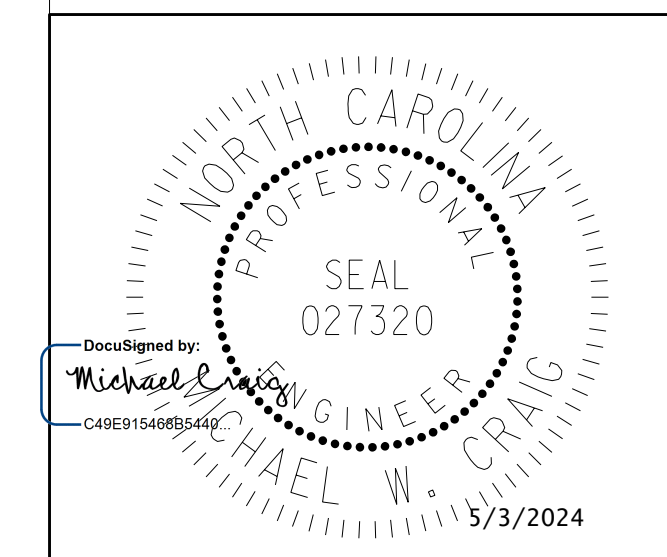
FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

BRIDGE COORDINATES	
LATITUDE	LONGITUDE
36°08'50.96"	78°43'29.32"

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

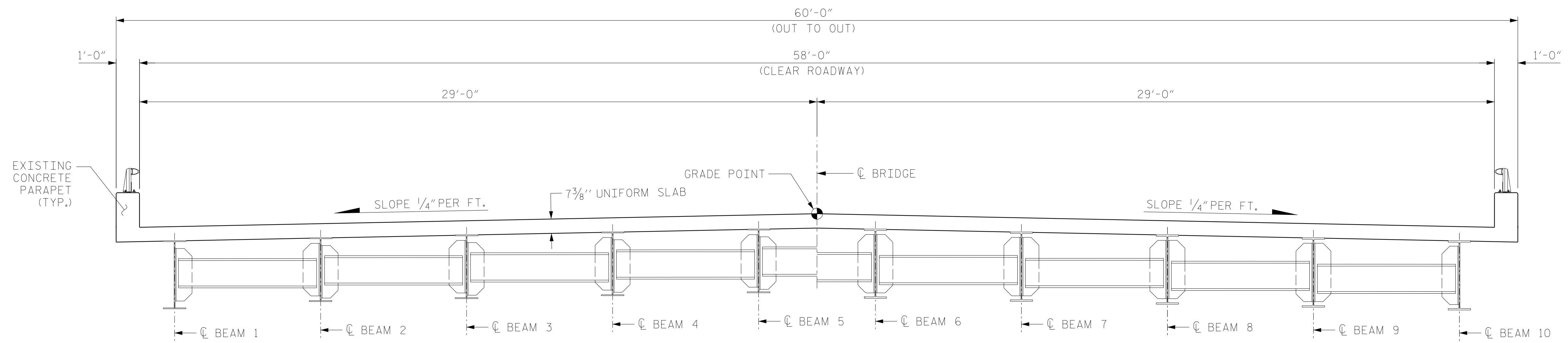
GENERAL DRAWING  
 FOR BRIDGE ON NC 56  
 BETWEEN BUTNER AND  
 CREEDMOOR OVER I-85

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S-2
2			4			TOTAL SHEETS 16

DRAWN BY : J. MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

**wsp**  
 WSP USA, Inc.  
 128 TOLBERT RD.  
 SUITE A  
 MOOREVILLE, NC 28117  
 TEL: 1.704.662.0100  
 LICENSE NO. F-0165



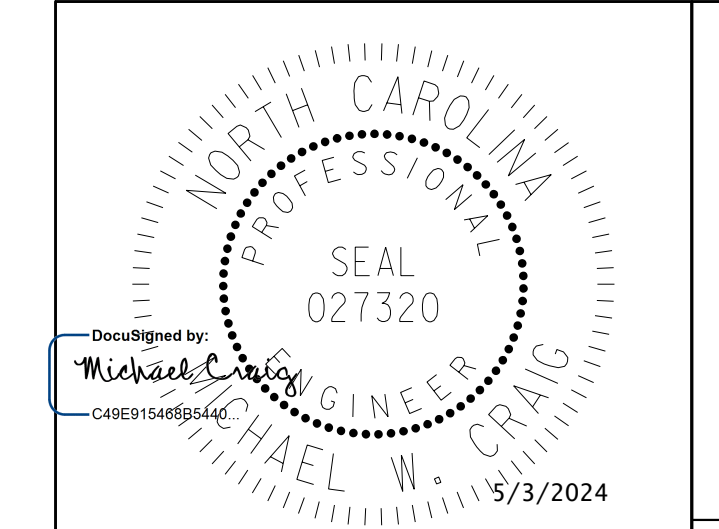
SPAN A AND D

SPAN B AND C

TYPICAL SECTION  
(EXISTING)

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

TYPICAL SECTION

DRAWN BY : J. MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

	WSP USA, Inc. 128 TOLBERT RD. SUITE A MOORESVILLE, NC 28117 TEL: 1.704.662.0100 LICENSE NO. F-0165				NO. BY: DATE: NO. BY: DATE:		SHEET NO. S-3
	1 2	3 4	5 6	7 8	9 10	11 12	TOTAL SHEETS 16

**NOTES**

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

ALL BEAM ENDS AT BENTS 1, 2 AND 3 SHALL BE CLEANED AND PAINTED. FOR ZONE PAINTING, SEE PAINTING EXISTING STRUCTURE SPECIAL PROVISION.

HORIZONTAL LIMITS OF ZONE PAINTING SHALL EXTEND 6" BEYOND THE MAXIMUM HORIZONTAL EXTENT OF ANY BEAM REPAIRS OR 3 FEET FROM THE BEAM END, WHICHEVER IS GREATER.

VERTICAL LIMITS OF ZONE PAINTING SHALL EXTEND FOR THE FULL DEPTH OF THE BEAMS.

FOR BEAM REPAIRS, SEE "BEAM REPAIR DETAILS" SHEET.

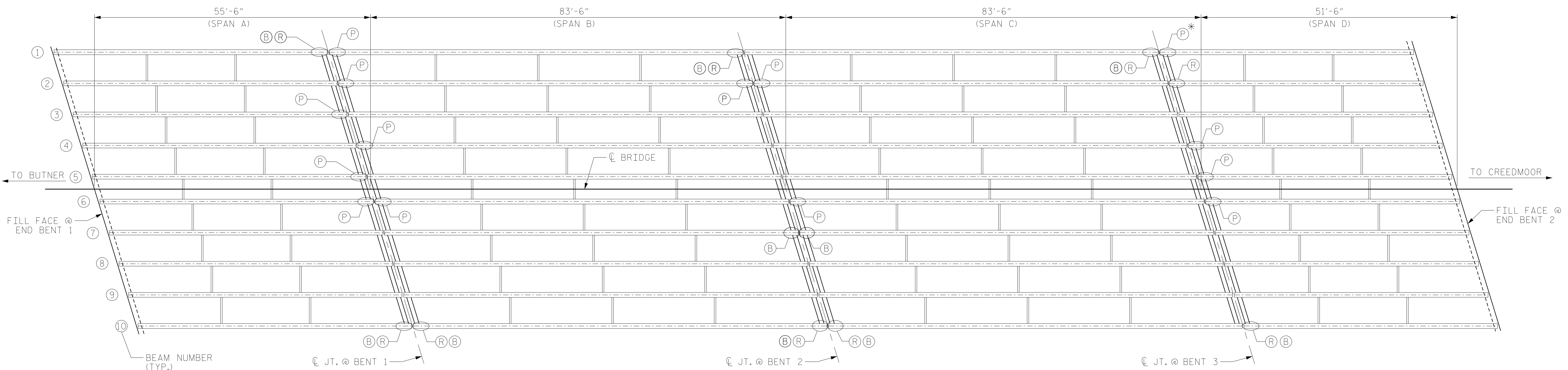
FOR BEAM PLATING REPAIRS, SEE "BEAM PLATING REPAIRS DETAILS SHEET".

FOR STEEL BEARING REPLACEMENT, SEE "BEARING DETAILS" SHEET.

- Ⓡ BEAM REPAIR
- Ⓟ BEAM PLATING REPAIR
- Ⓟ STEEL BEARING REPLACEMENT

**REPAIR QUANTITY TABLE**

SUPERSTRUCTURE REPAIR		
	ESTIMATE	ACTUAL
BEAM REPAIR	2,370 LBS.	
BEAM PLATING REPAIR	330 LBS.	
STEEL BEARING REPLACEMENT	10 EA.	



**SUPERSTRUCTURE REPAIR LOCATIONS**

(OTHER LOCATIONS MAY EXIST, SEE NOTES)

\* BEAM PLATING REQUIRED ON INTERIOR FACE OF WEB ONLY. MATCH PLATE THICKNESS WITH EXISTING PLATE ON EXTERIOR FACE OF WEB.

**ANTICIPATED REPAIR LOCATIONS**

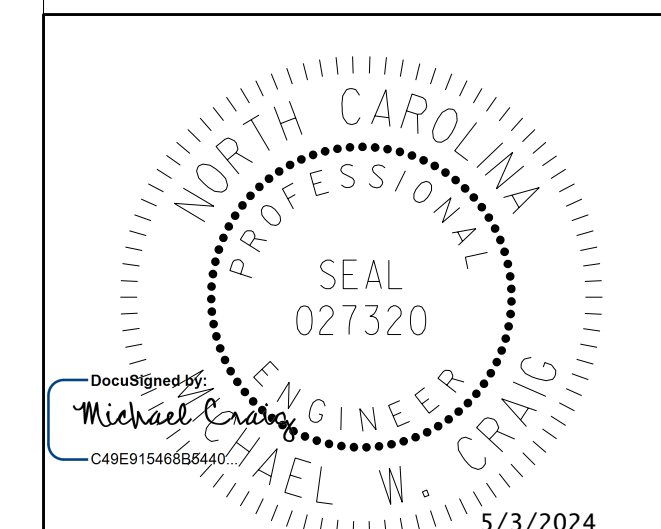
SPAN	BEAM	LOCATION	DIM "A"	DIM "B"	DIM "C"	DIM "D"	DIM "E"	DIM "F"	
A	1	BENT 1	28"	16"	6"	44"	---	---	
A	1	BENT 1	STEEL BEARING REPLACEMENT (EXPANSION)						
A	3	BENT 1	8"	10"	---	---	0"	---	
A	5	BENT 1	6"	10"	---	---	---	---	
A	6	BENT 1	4"	10"	---	---	---	---	
A	10	BENT 1	28"	16"	9"	49"	---	---	
A	10	BENT 1	STEEL BEARING REPLACEMENT (EXPANSION)						
B	1	BENT 1	16"	10"	---	---	12"	---	
B	2	BENT 1	8"	10"	---	---	0"	---	
B	4	BENT 1	8"	10"	---	---	0"	---	
B	6	BENT 1	6"	10"	---	---	0"	---	
B	10	BENT 1	28"	14"	12"	34"	6"	43"	
B	10	BENT 1	STEEL BEARING REPLACEMENT (EXPANSION)						
B	1	BENT 2	4"	30"	---	---	---	---	
B	1	BENT 2	STEEL BEARING REPLACEMENT (FIXED)						
B	2	BENT 2	14"	10"	---	---	0"	---	
B	7	BENT 2	STEEL BEARING REPLACEMENT (FIXED)						
B	10	BENT 2	28"	15"	6"	15"	---	---	
B	10	BENT 2	STEEL BEARING REPLACEMENT (FIXED)						

**ANTICIPATED REPAIR LOCATIONS**

SPAN	BEAM	LOCATION	DIM "A"	DIM "B"	DIM "C"	DIM "D"	DIM "E"	DIM "F"	
C	2	BENT 2	16"	10"	---	---	12"	---	
C	6	BENT 2	14"	10"	---	---	0"	---	
C	7	BENT 2	STEEL BEARING REPLACEMENT (EXPANSION)						
C	10	BENT 2	28"	14"	6"	31"	---	---	
C	10	BENT 2	STEEL BEARING REPLACEMENT (EXPANSION)						
C	1	BENT 3	5"	27"	---	---	---	---	
D	1	BENT 3	17"	10"	---	---	12"	---	
D	1	BENT 3	STEEL BEARING REPLACEMENT (FIXED)						
D	2	BENT 3	6"	24"	---	---	---	---	
D	4	BENT 3	22"	10"	---	---	0"	---	
D	5	BENT 3	22"	10"	---	---	0"	---	
D	6	BENT 3	2"	10"	---	---	26"	---	
D	10	BENT 3	4"	50"	---	---	---	---	
D	10	BENT 3	STEEL BEARING REPLACEMENT (EXPANSION)						

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUPERSTRUCTURE REPAIR LOCATIONS**

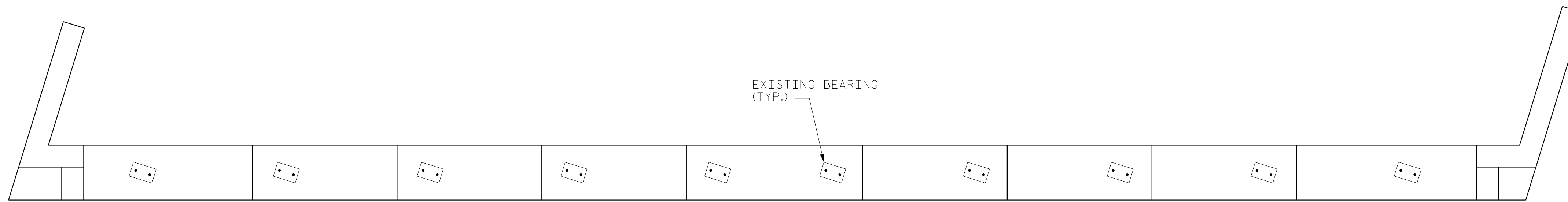
REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

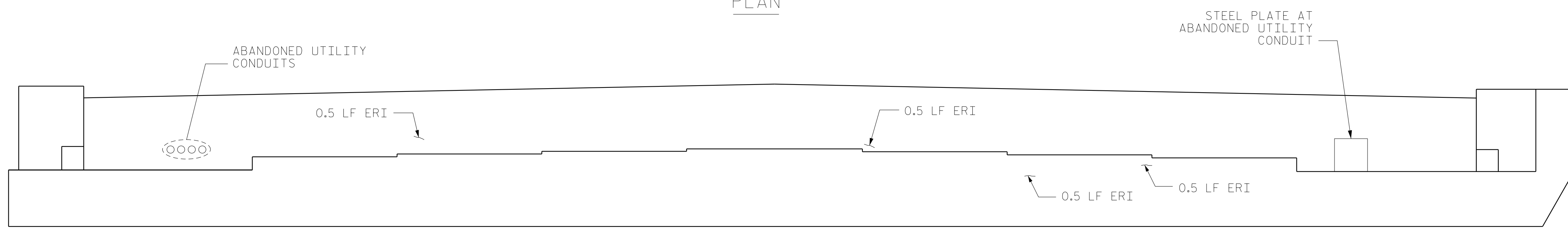
SHEET NO. S-4  
 TOTAL SHEETS 16

DRAWN BY : J. MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

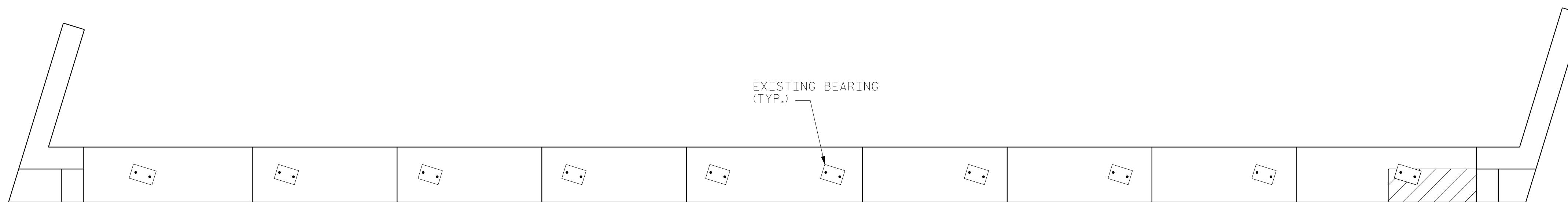
**wsp**  
 WSP USA, Inc.  
 128 TOLBERT RD.  
 MOORESVILLE, NC 28117  
 TEL: 1.704.662.0100  
 LICENSE NO. F-0165



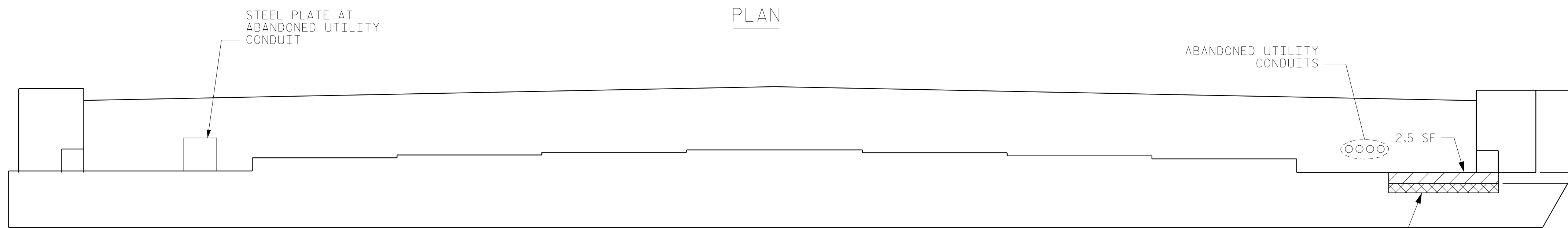
PLAN



ELEVATION  
END BENT 1



PLAN



ELEVATION  
END BENT 2

- SHOTCRETE REPAIR
- CONCRETE REPAIR
- EPOXY RESIN INJECTION (ERI)

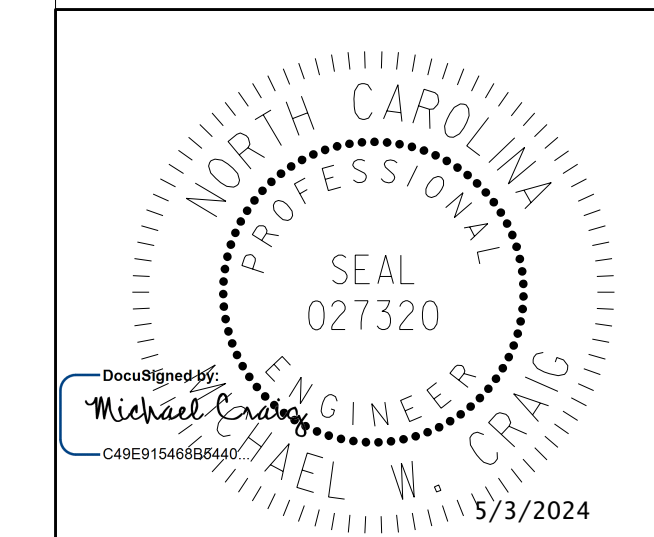
REPAIR QUANTITY TABLE				
END BENT 1	QUANTITIES			
	ESTIMATE		ACTUAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	0.0	0.0		
CURTAIN WALL	0.0	0.0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	0.0	0.0		
CURTAIN WALL	0.0	0.0		
EPOXY RESIN INJECTION	LN. FT		LN. FT	
CAP	1.0			
CURTAIN WALL	1.0			
EPOXY COATING	AREA SF	AREA SF		
TOP OF END BENT CAP	0.0			
END BENT 2	QUANTITIES			
SHOTCRETE REPAIRS	ESTIMATE		ACTUAL	
	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	2.1	1.0		
CURTAIN WALL	0.0	0.0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	7.9	3.2		
CURTAIN WALL	0.0	0.0		
EPOXY RESIN INJECTION	LN. FT		LN. FT	
CAP	0.0			
CURTAIN WALL	0.0			
EPOXY COATING	AREA SF	AREA SF		
TOP OF END BENT CAP	0.0			

VALUES IN CHARTS REPRESENT ESTIMATED REPAIRS TOTALS AFTER REMOVAL OF SOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAW CUT. SEE REPAIR DETAILS.

**NOTES**  
 REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.  
 FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.  
 FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.  
 FOR REPAIRS, SEE "TYPICAL CAP AND COLUMN REPAIRS" SHEET.

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

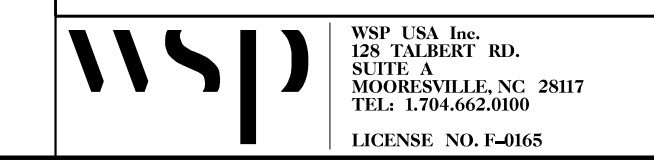


STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

END BENT 1 & 2

DRAWN BY : J.MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3			TOTAL SHEETS
2			4			16



REPAIR QUANTITY TABLE

REPAIRS BENT 1	QUANTITIES			
	ESTIMATE		ACTUAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	18.5	7.3		
COLUMN	116.7	47.4		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	0.0	0.0		
COLUMN	0.0	0.0		
EPOXY RESIN INJECTION	LN. FT		LN. FT	
CAP	1.0			
COLUMN	18.5			
EPOXY COATING	AREA SF		AREA SF	
TOP OF BENT CAP	153 SF			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CL TO SAWCUT. SEE REPAIR DETAILS.

NOTES

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

FOR REPAIRS, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.


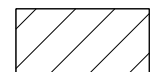

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP FACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

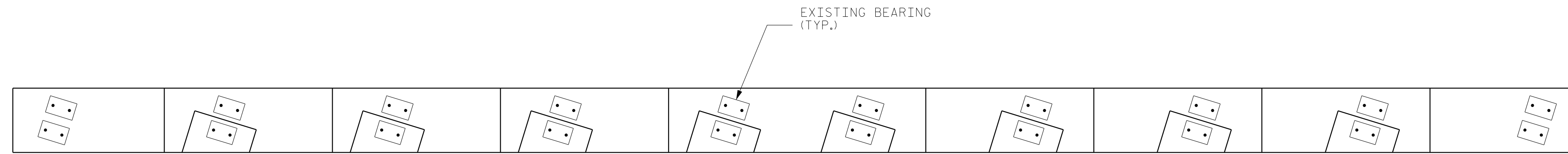
FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

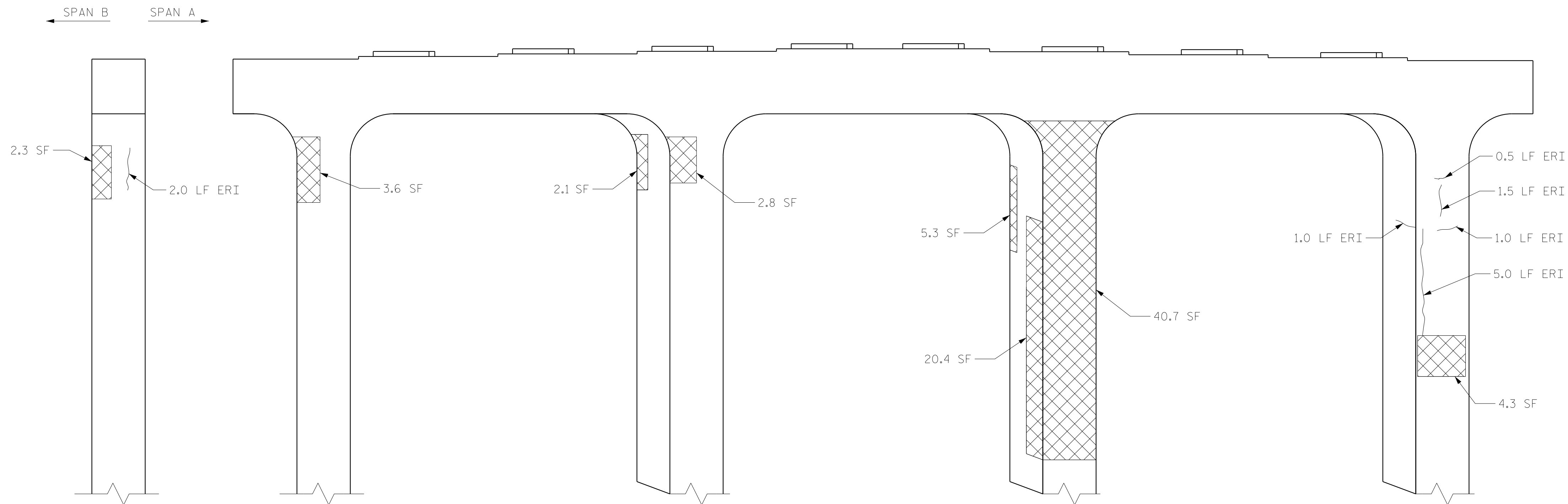
FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAY BE SUBSTITUTED IN PLACE OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

-  SHOTCRETE REPAIR
-  CONCRETE REPAIR
-  EPOXY RESIN INJECTION (ERI)



TOP OF CAP

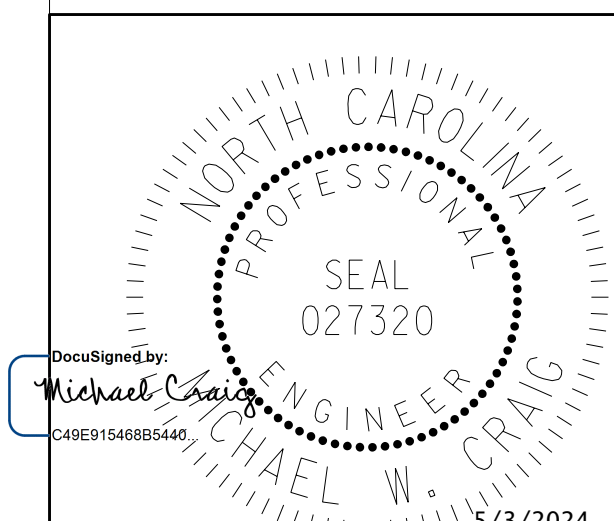


END VIEW

ELEVATION

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUBSTRUCTURE  
 BENT 1  
 SPAN A FACE

REVISIONS

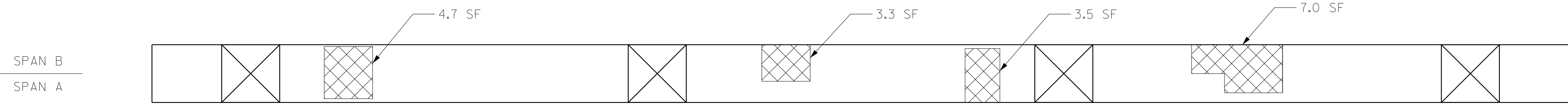
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S-6
2			4			TOTAL SHEETS 16

DRAWN BY : J.MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

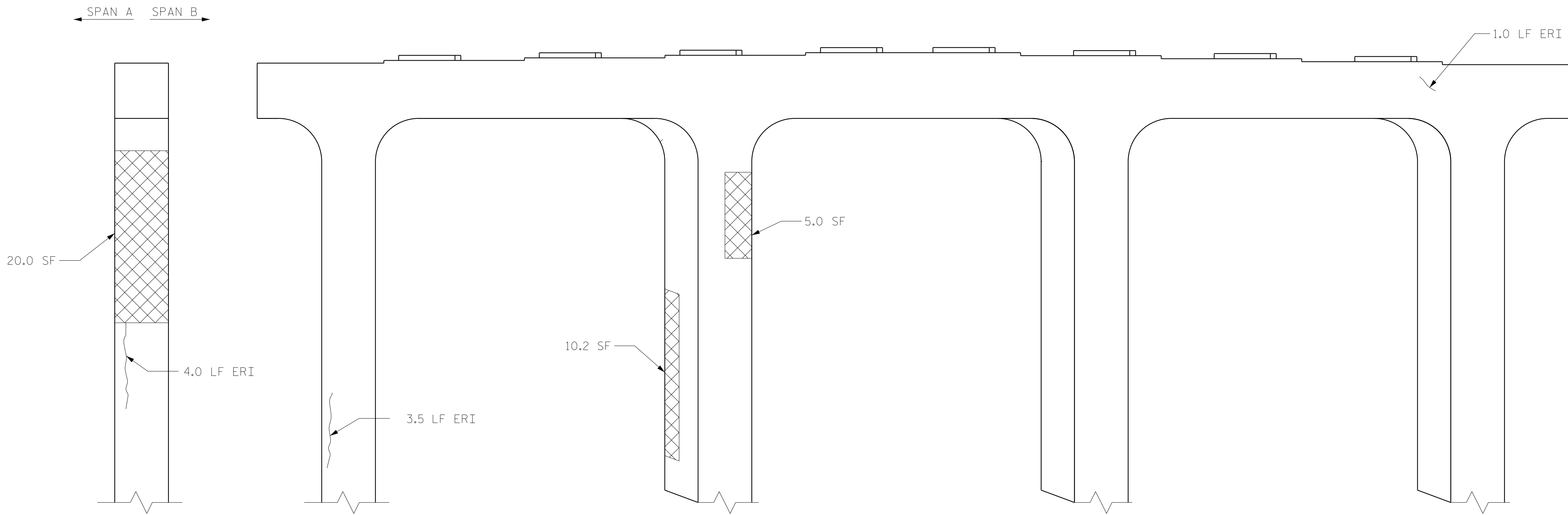


WSP USA Inc.  
 128 TOLBERT RD.  
 SUITE A  
 MOORESVILLE, NC 28117  
 TEL: 1.704.662.0100  
 LICENSE NO. F-0165





BOTTOM OF CAP



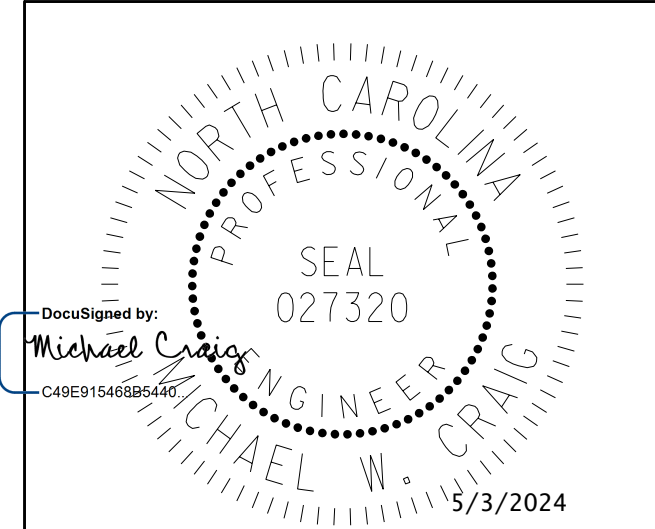
END VIEW

ELEVATION

- SHOTCRETE REPAIR
- CONCRETE REPAIR
- EPOXY RESIN INJECTION (ERI)

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

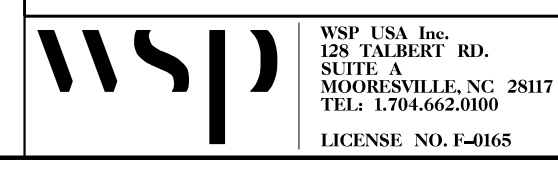
DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
  
 SUBSTRUCTURE  
 BENT 1  
 SPAN B FACE

DRAWN BY :	J.MYA	DATE :	3/2019
CHECKED BY :	J.YANNACCONE	DATE :	3/2019
DESIGN ENGINEER OF RECORD :	MICHAEL W. CRAIG	DATE :	4/2024

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-7
1			3			TOTAL SHEETS
2			4			16



REPAIR QUANTITY TABLE

REPAIRS BENT 1	QUANTITIES			
	ESTIMATE		ACTUAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	45.3	17.9		
COLUMN	17.9	7.3		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	4.8	1.9		
COLUMN	0.0	0.0		
EPOXY RESIN INJECTION	LN. FT	LN. FT	LN. FT	LN. FT
CAP	37.0			
COLUMN	7.5			
EPOXY COATING	AREA SF	AREA SF	AREA SF	AREA SF
TOP OF BENT CAP	153 SF			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CL TO SAWCUT. SEE REPAIR DETAILS.

**NOTES**  
 REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

FOR REPAIRS, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.


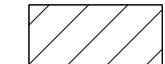

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP FACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAY BE SUBSTITUTED IN PLACE OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

-  SHOTCRETE REPAIR
-  CONCRETE REPAIR
-  EPOXY RESIN INJECTION (ERI)

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

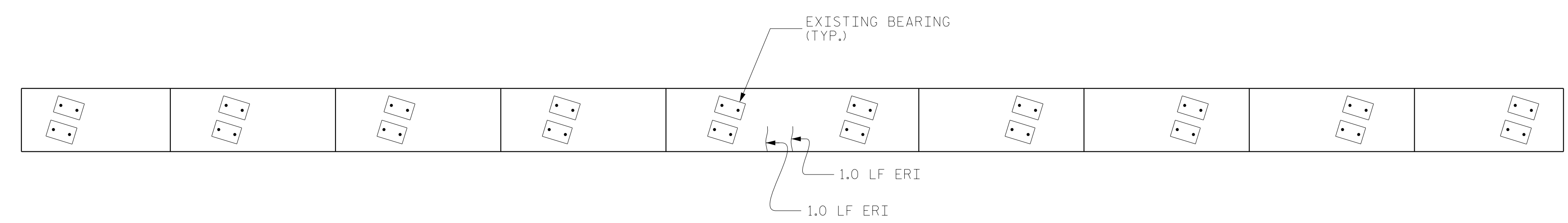
DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



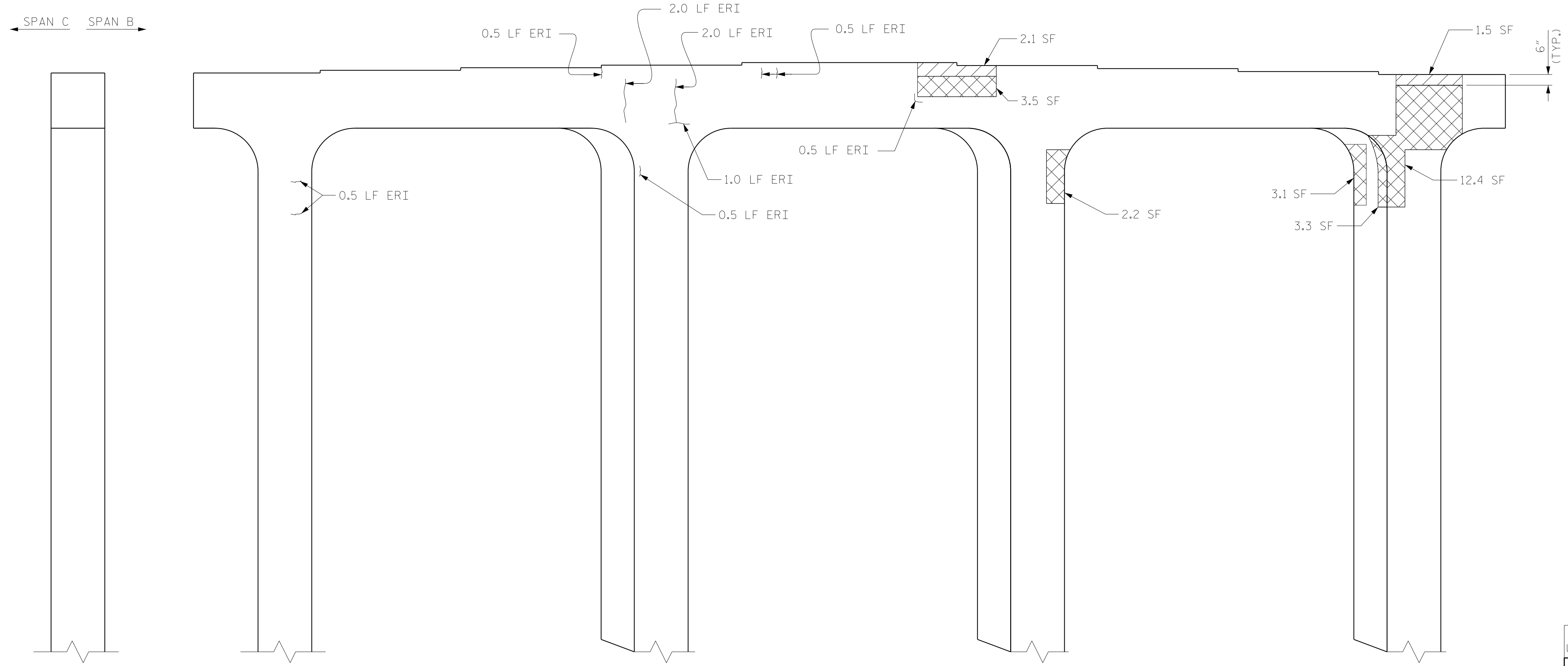
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUBSTRUCTURE  
 BENT 2  
 SPAN B FACE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			TOTAL SHEETS
2			4			16



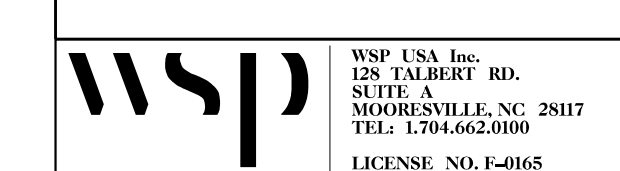
TOP OF CAP

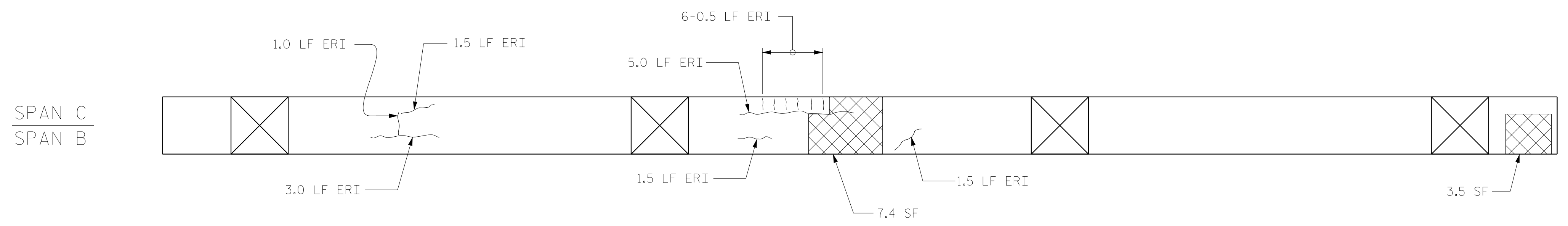


END VIEW

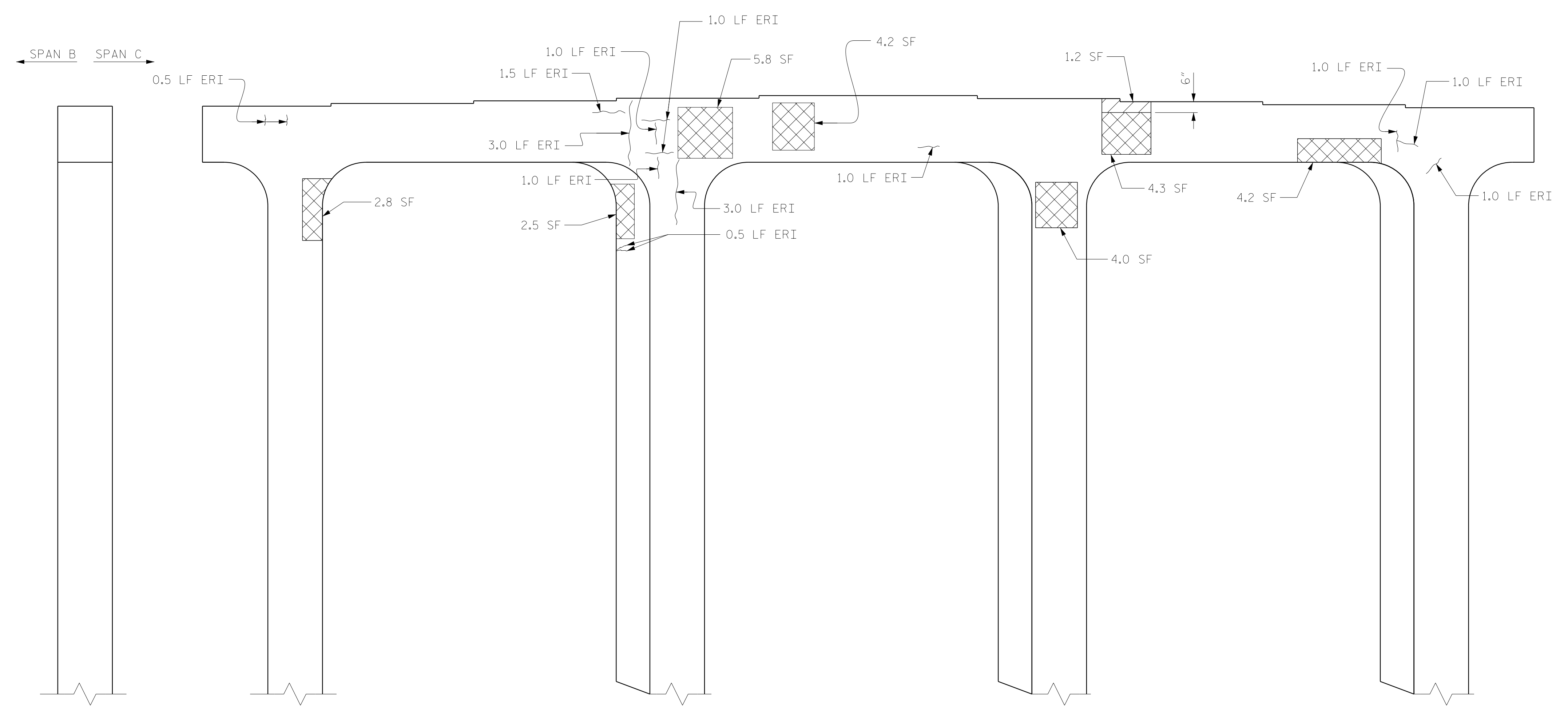
ELEVATION

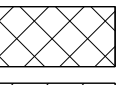


DRAWN BY : J.MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024





BOTTOM OF CAP



 SHOTCRETE REPAIR  
 CONCRETE REPAIR  
 EPOXY RESIN INJECTION (ERI)

END VIEW

ELEVATION

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED



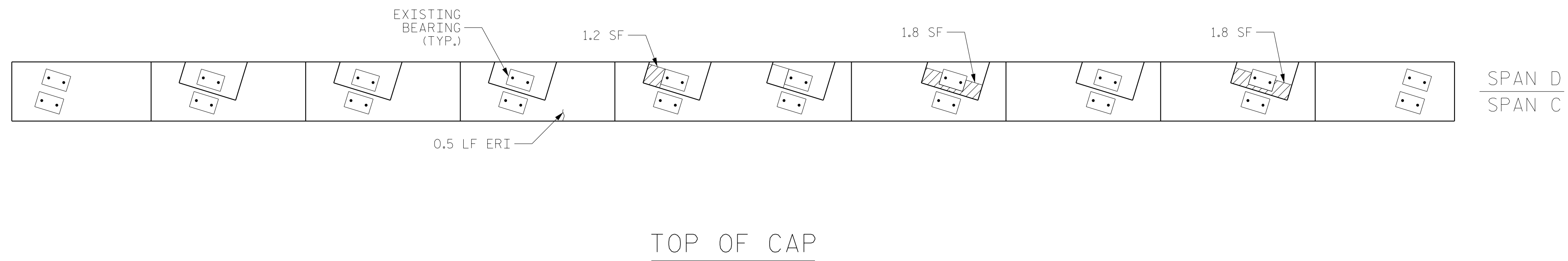
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
  
 SUBSTRUCTURE  
 BENT 2  
 SPAN C FACE

DRAWN BY : J.MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

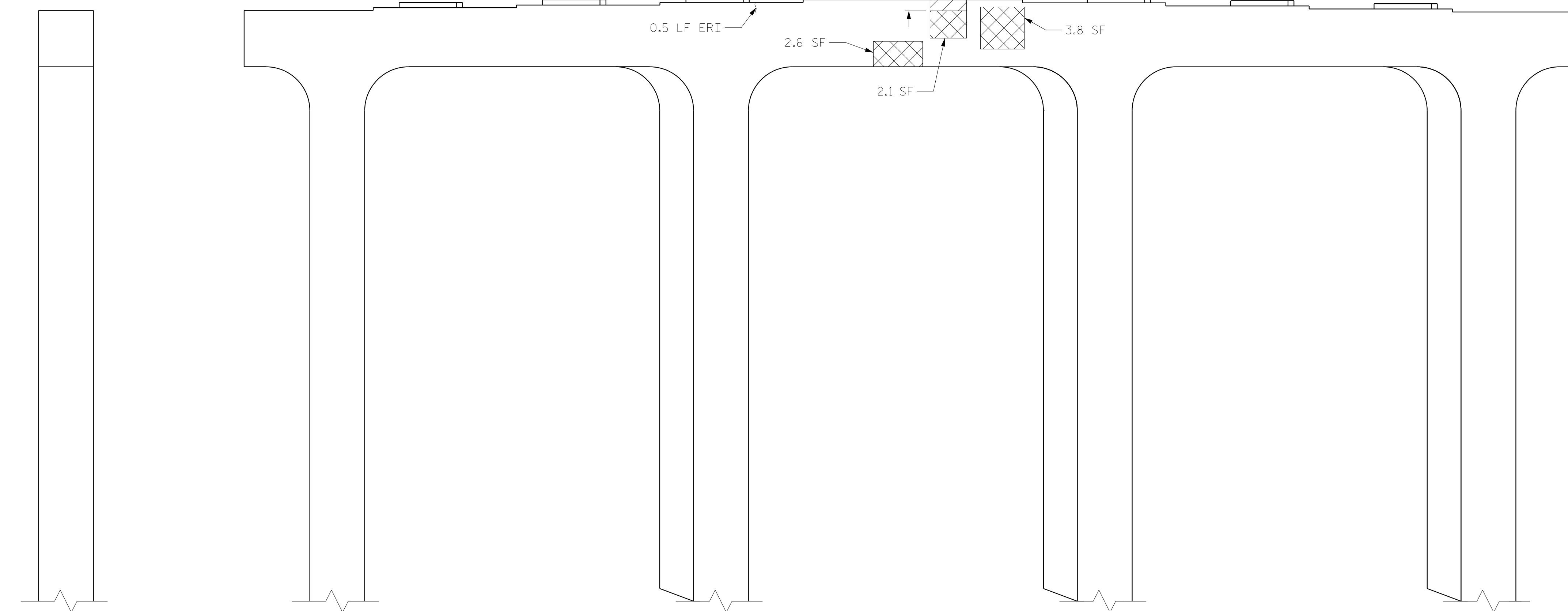
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
1			3			TOTAL SHEETS
2			4			16



WSP USA Inc.  
 128 TOLBERT RD.  
 SUITE A  
 MOORESVILLE, NC 28117  
 TEL: 1.704.662.0100  
 LICENSE NO. F-0165



← SPAN D SPAN C →



END VIEW

ELEVATION

REPAIR QUANTITY TABLE

REPAIRS BENT 3	QUANTITIES			
	ESTIMATE		ACTUAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	15.4	6.1		
COLUMN	7.2	3.0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	6.8	3.2		
COLUMN	0.0	0.0		
EPOXY RESIN INJECTION		LN. FT		LN. FT
CAP		2.5		
COLUMN		0.0		
EPOXY COATING	AREA SF		AREA SF	
TOP OF BENT CAP	153 SF			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CL TO SAWCUT. SEE REPAIR DETAILS.




NOTES  
REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

FOR REPAIRS, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP FACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

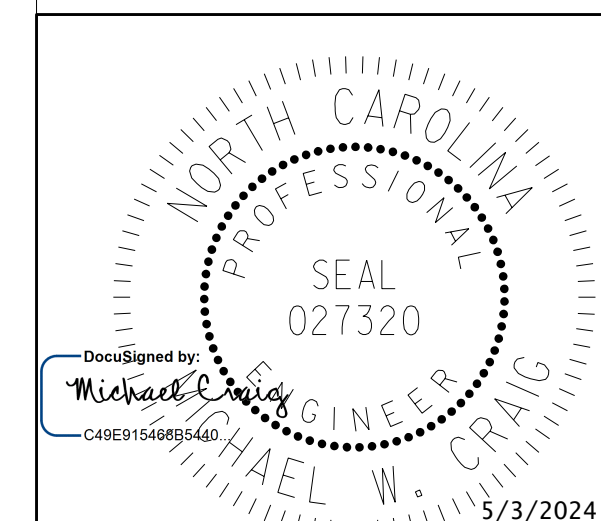
FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.  
FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.  
FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAY BE SUBSTITUTED IN PLACE OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

-  SHOTCRETE REPAIR
-  CONCRETE REPAIR
-  EPOXY RESIN INJECTION (ERI)

PROJECT NO. U-6020  
GRANVILLE COUNTY  
BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



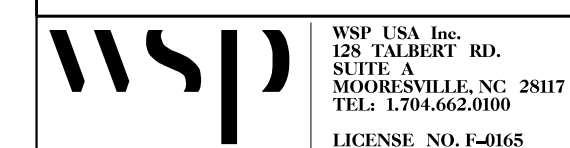
STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

SUBSTRUCTURE  
BENT 3  
SPAN C FACE

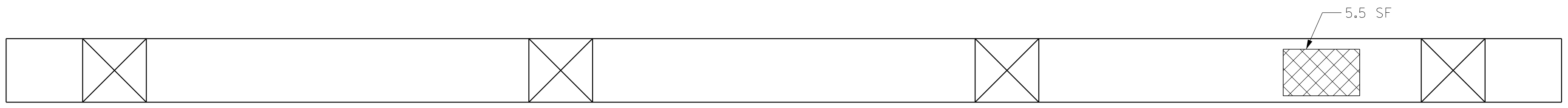
REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S-10
2			4			TOTAL SHEETS 16

DRAWN BY : J.MYA DATE : 3/2019  
CHECKED BY : J.YANNAACONE DATE : 3/2019  
DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

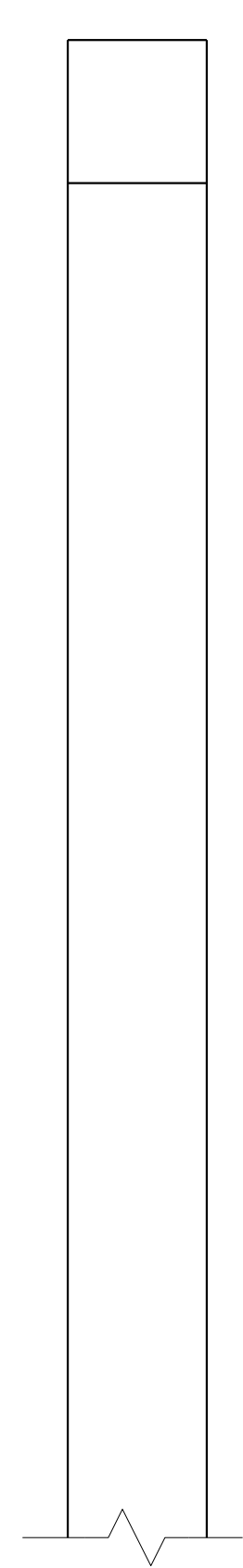


SPAN C  
SPAN D

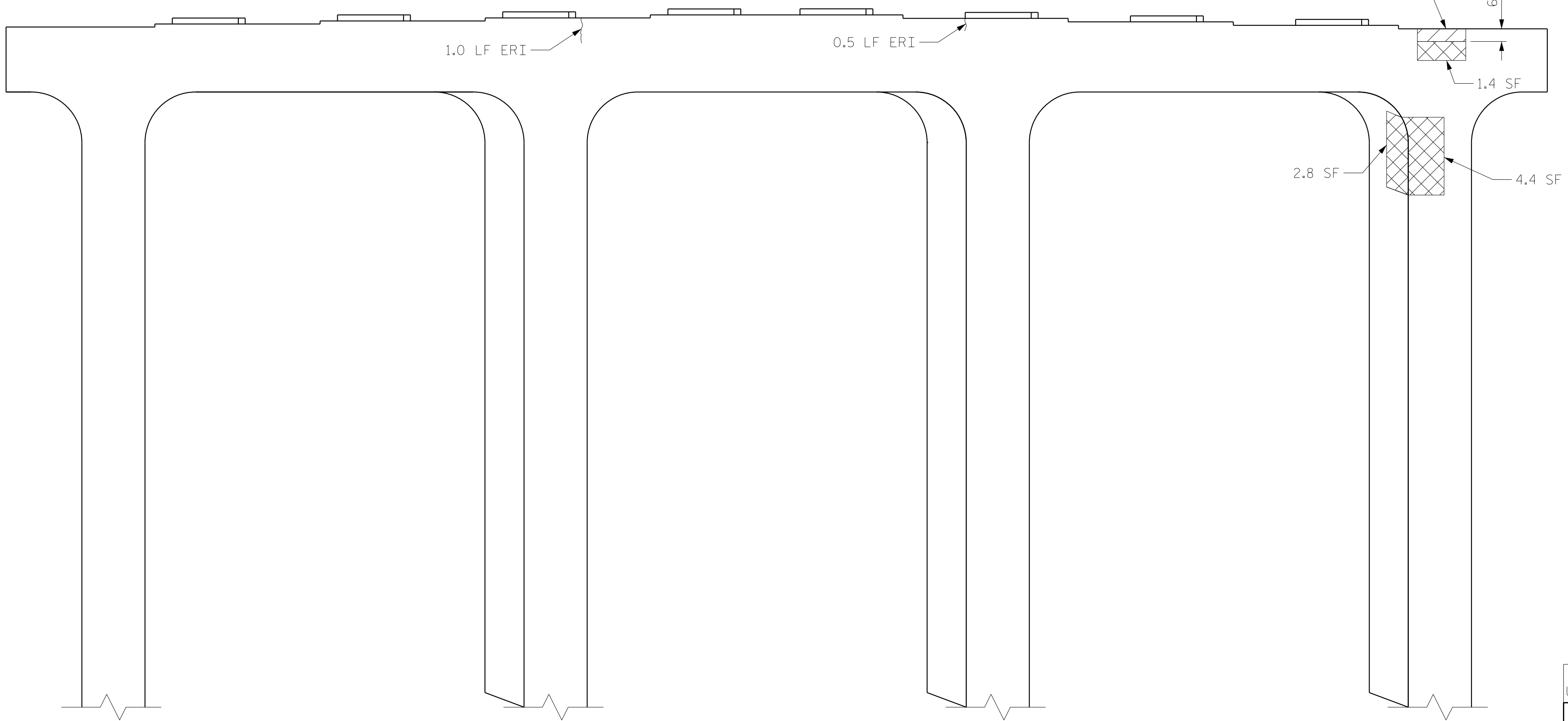


BOTTOM OF CAP

← SPAN C SPAN D →



END VIEW

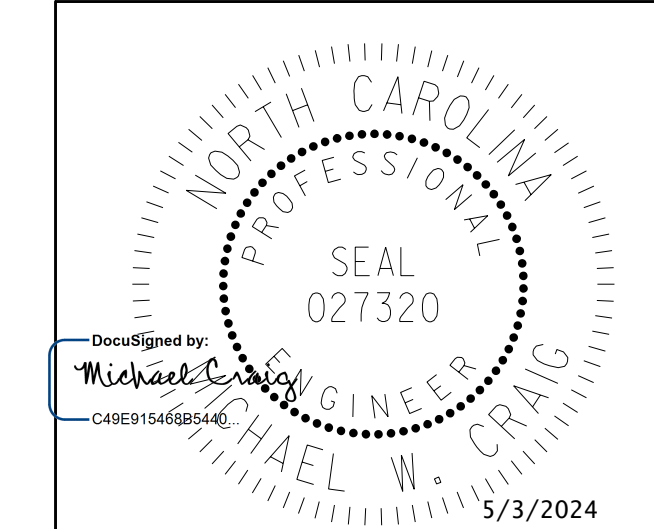


ELEVATION

- SHOTCRETE REPAIR
- CONCRETE REPAIR
- EPOXY RESIN INJECTION (ERI)

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



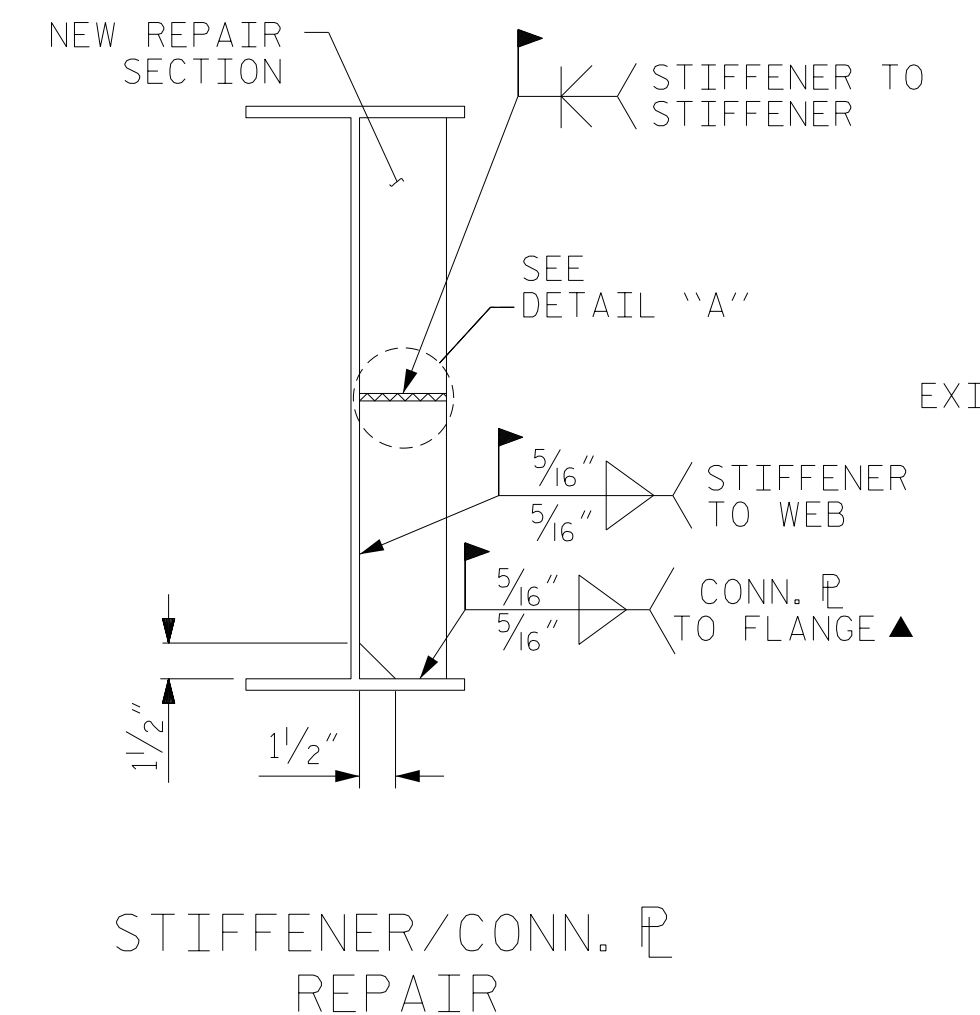
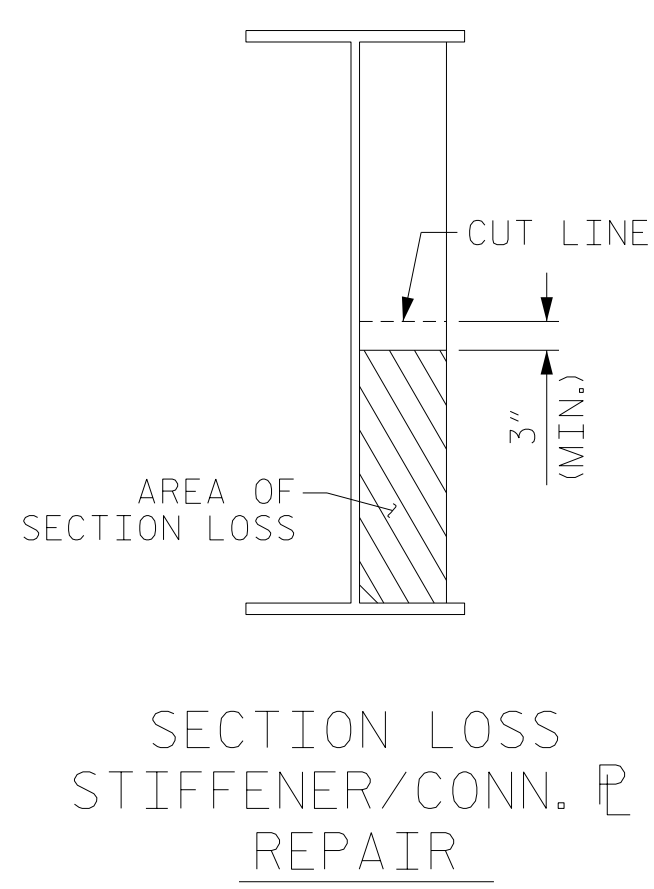
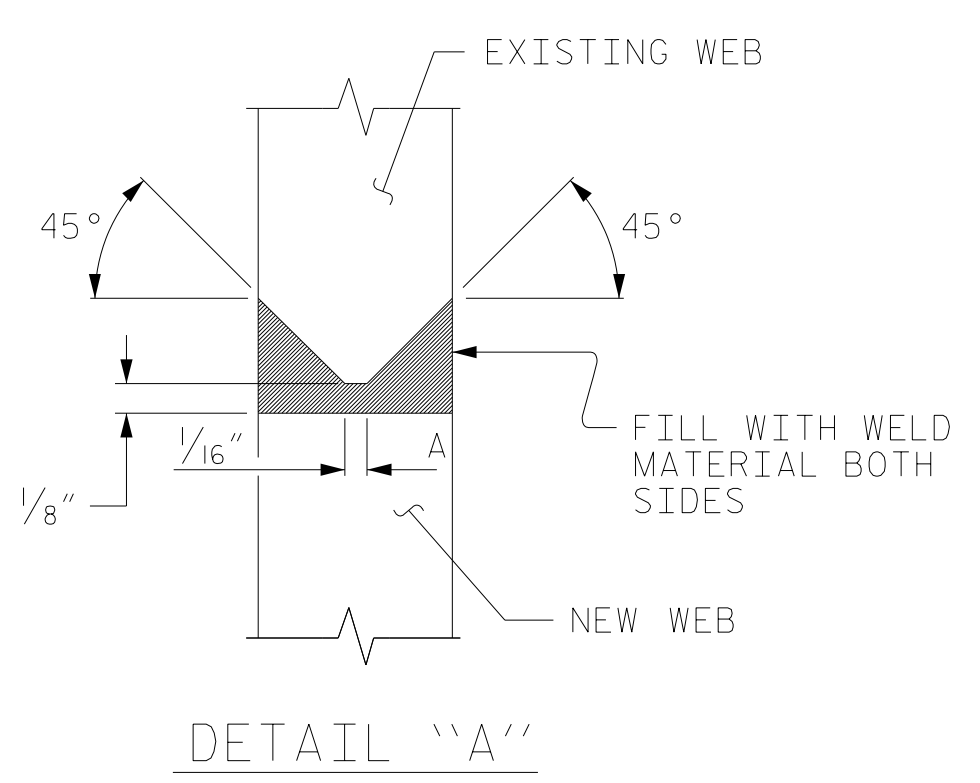
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**SUBSTRUCTURE**  
**BENT 3**  
**SPAN D FACE**

DRAWN BY :	J.MYA	DATE :	3/2019
CHECKED BY :	J.YANNACCONE	DATE :	3/2019
DESIGN ENGINEER OF RECORD :	MICHAEL W. CRAIG	DATE :	4/2024

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			TOTAL SHEETS
2			4			16

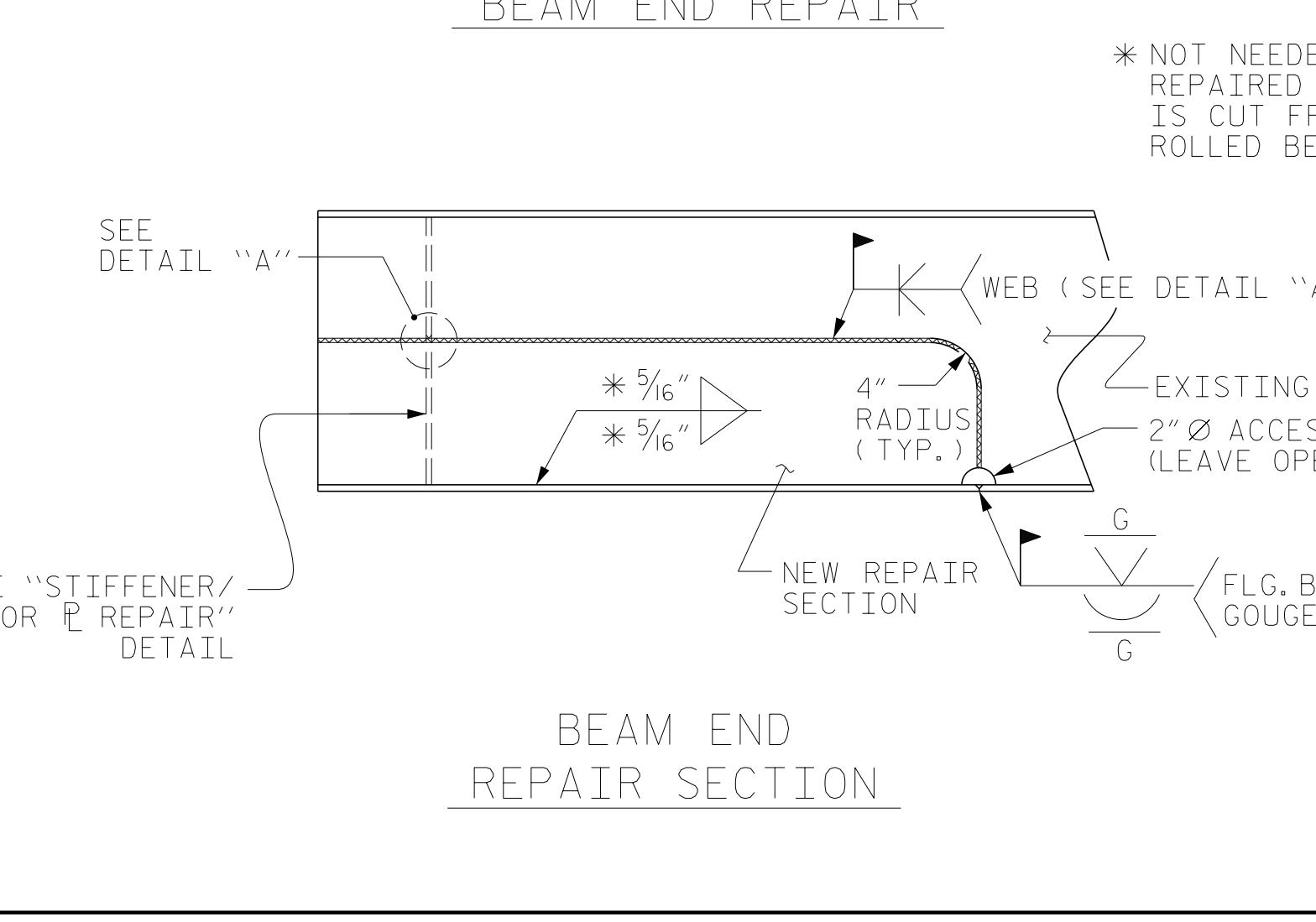
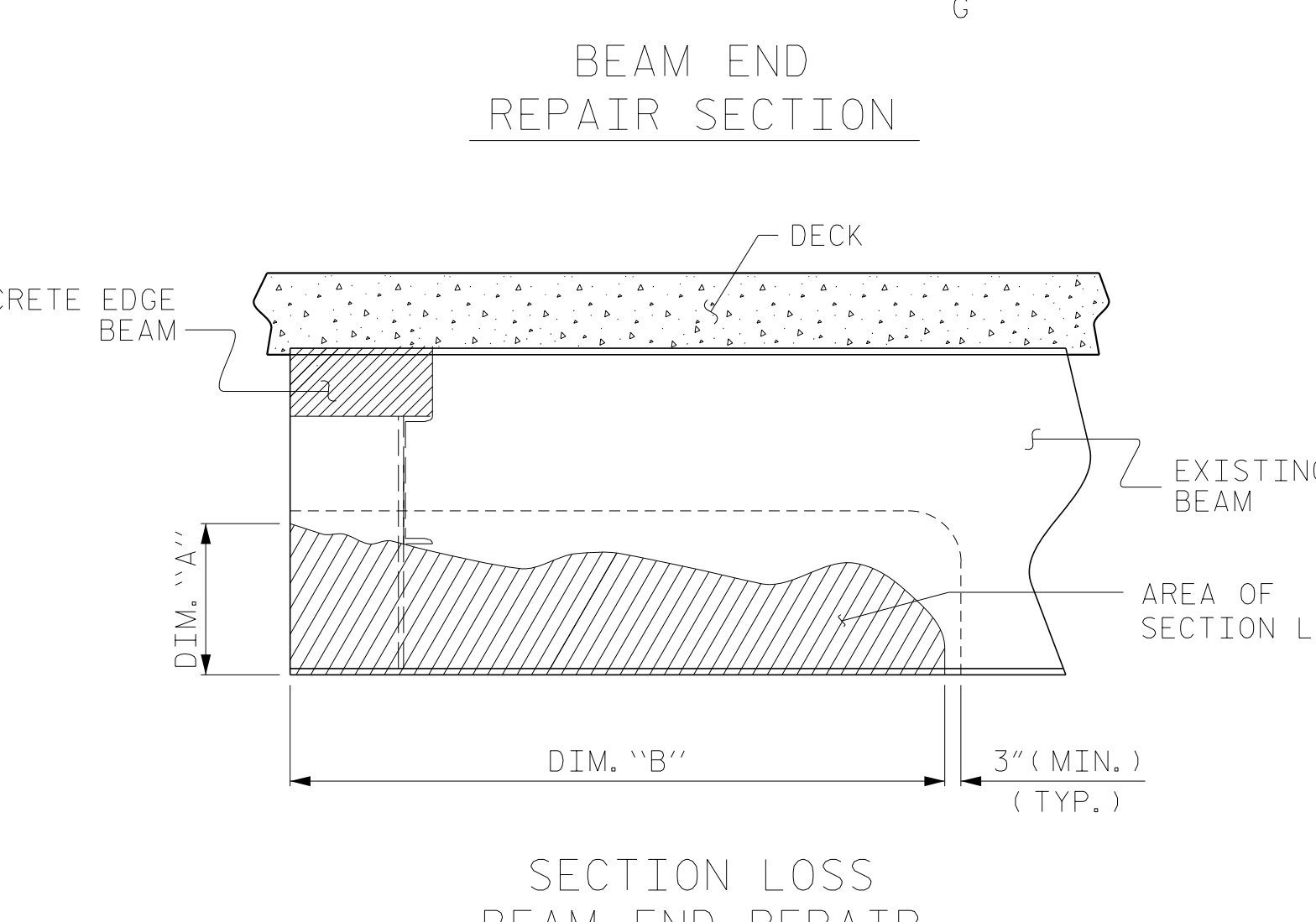
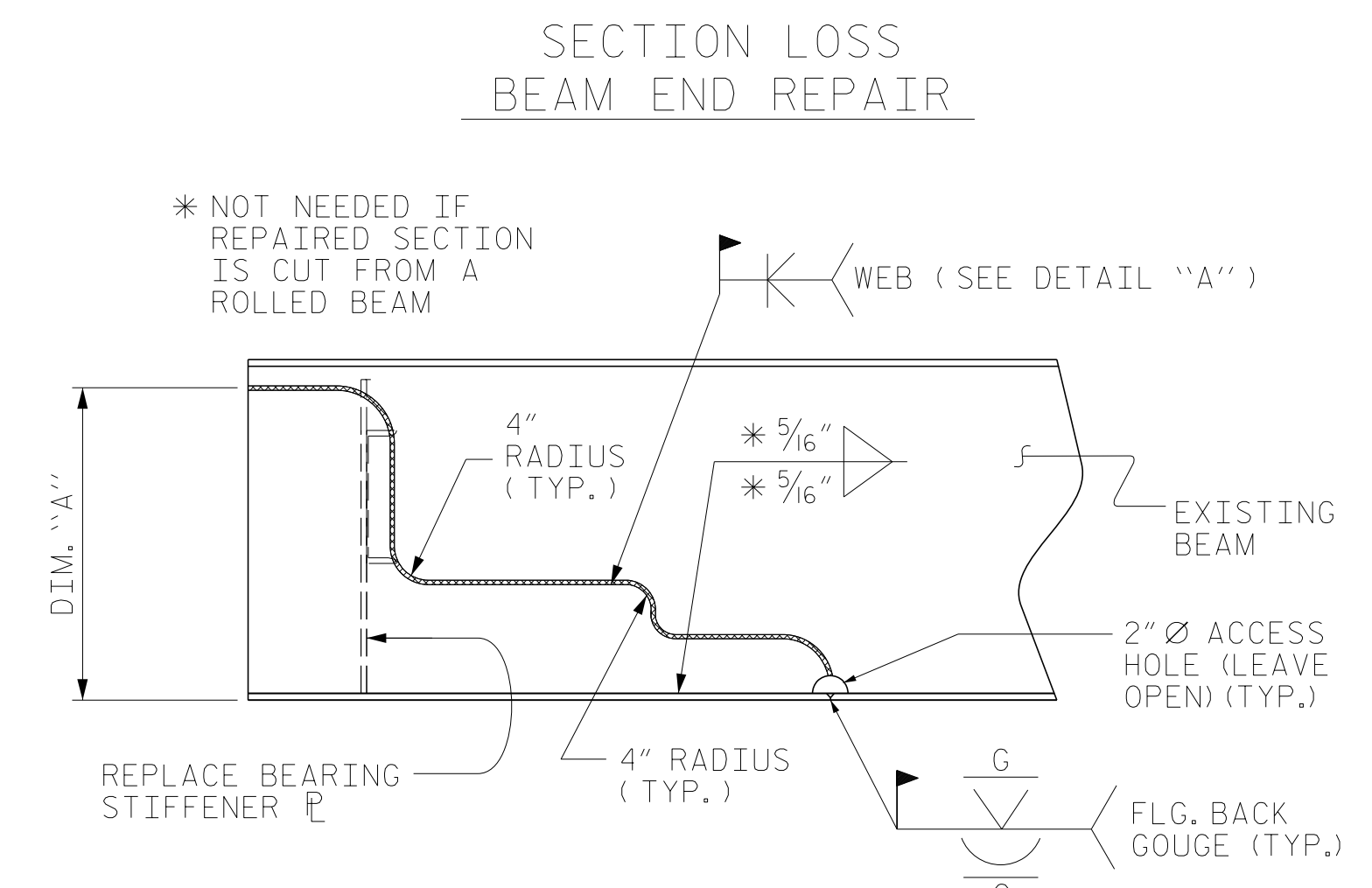
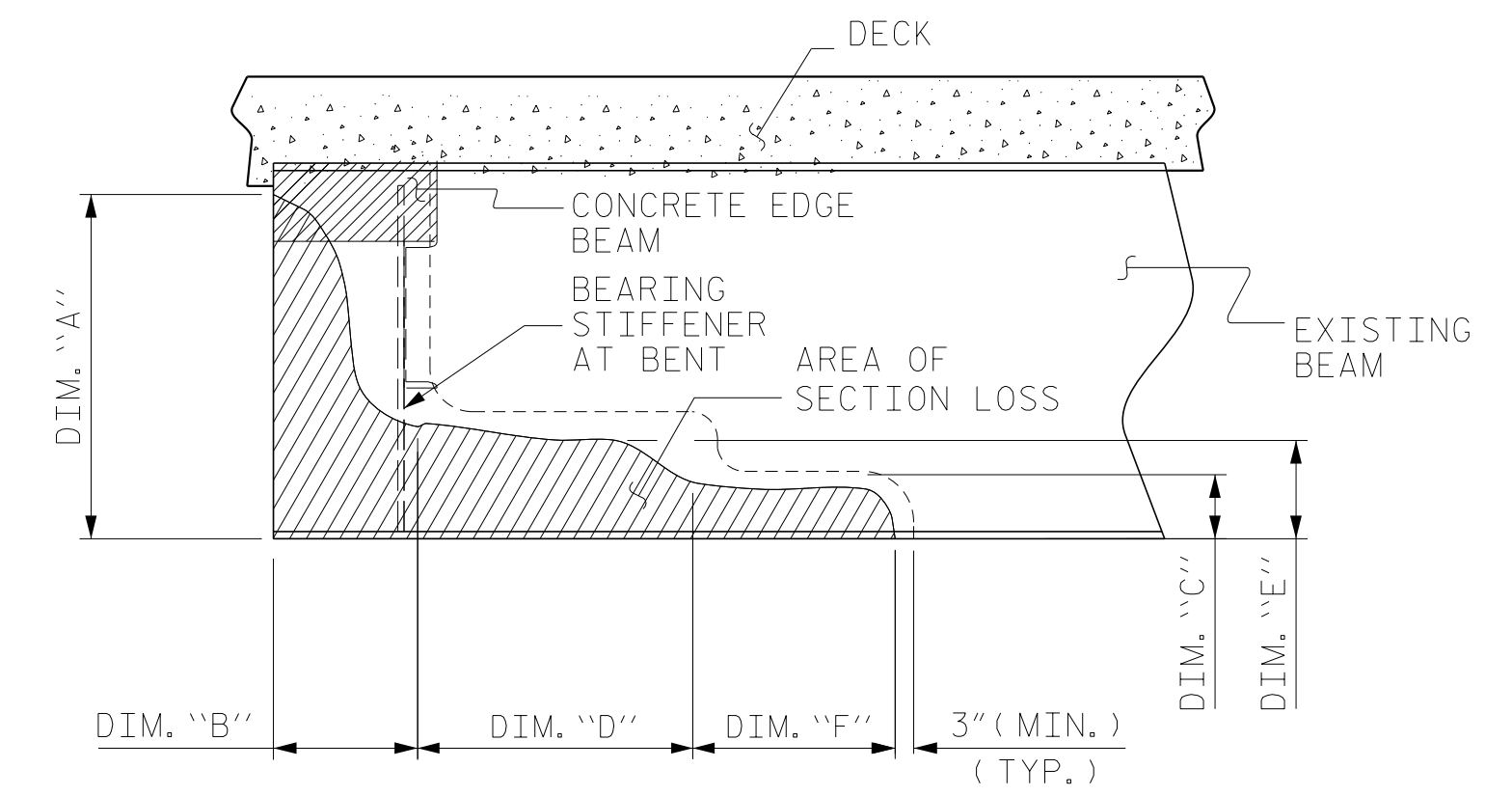
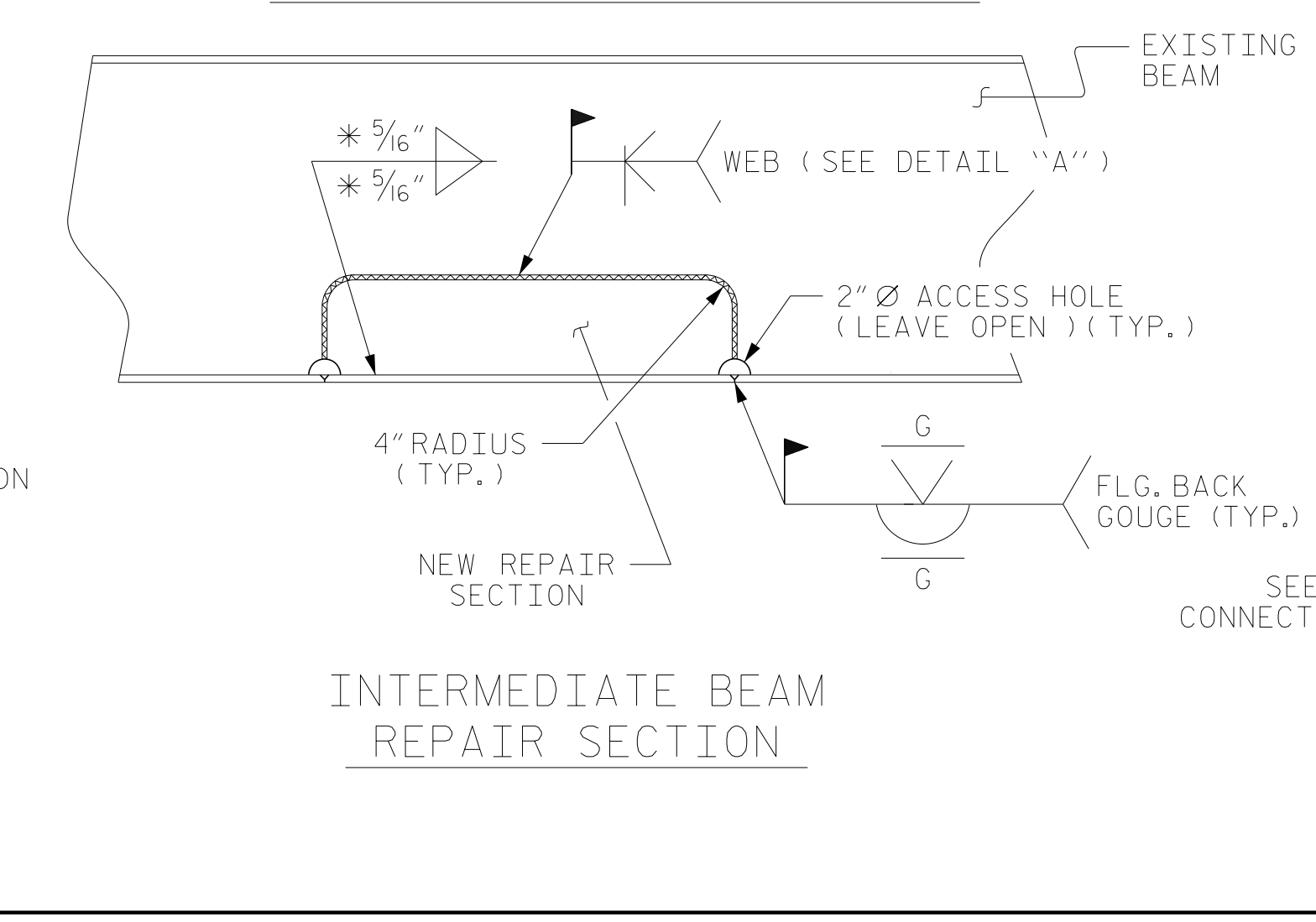
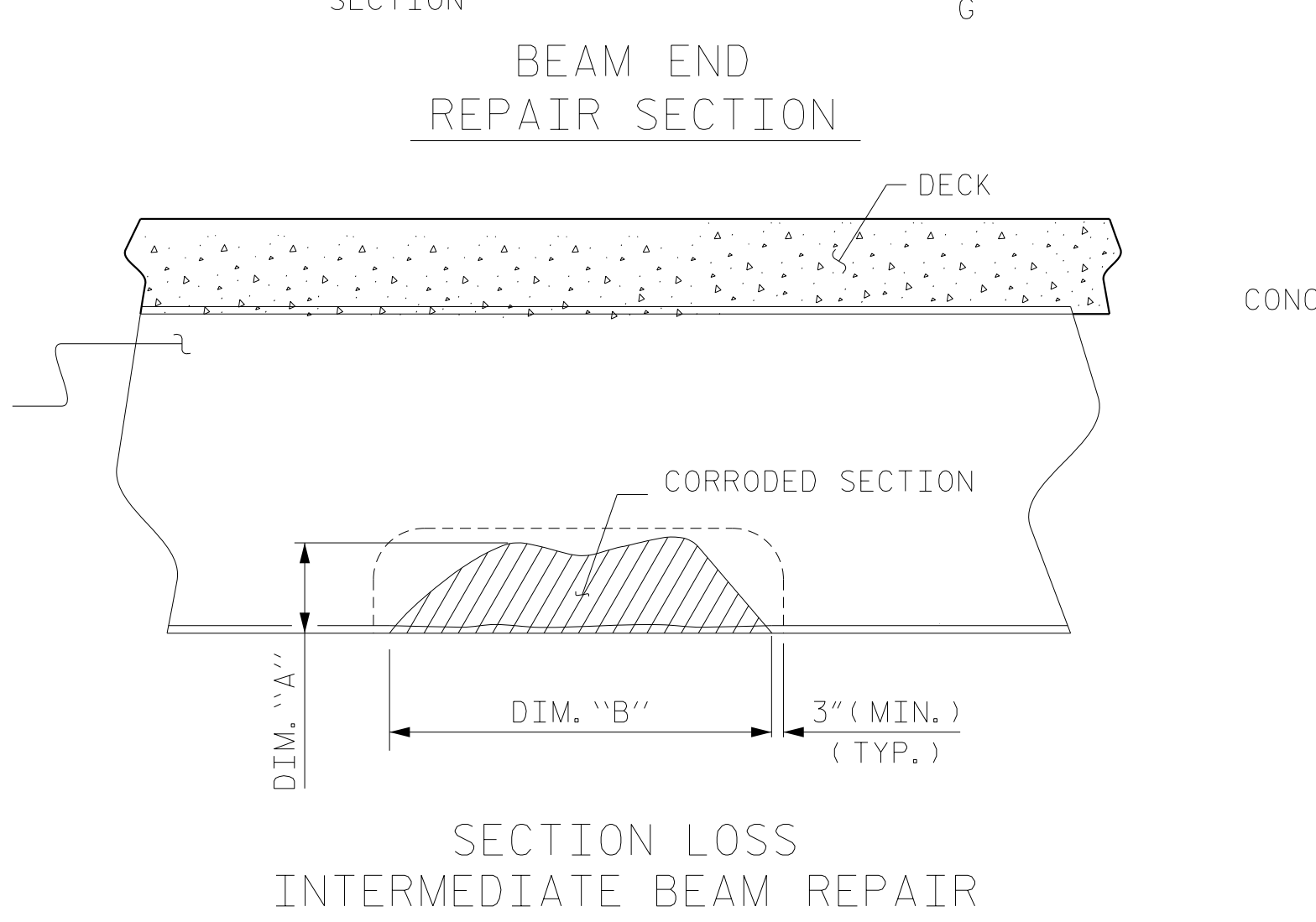
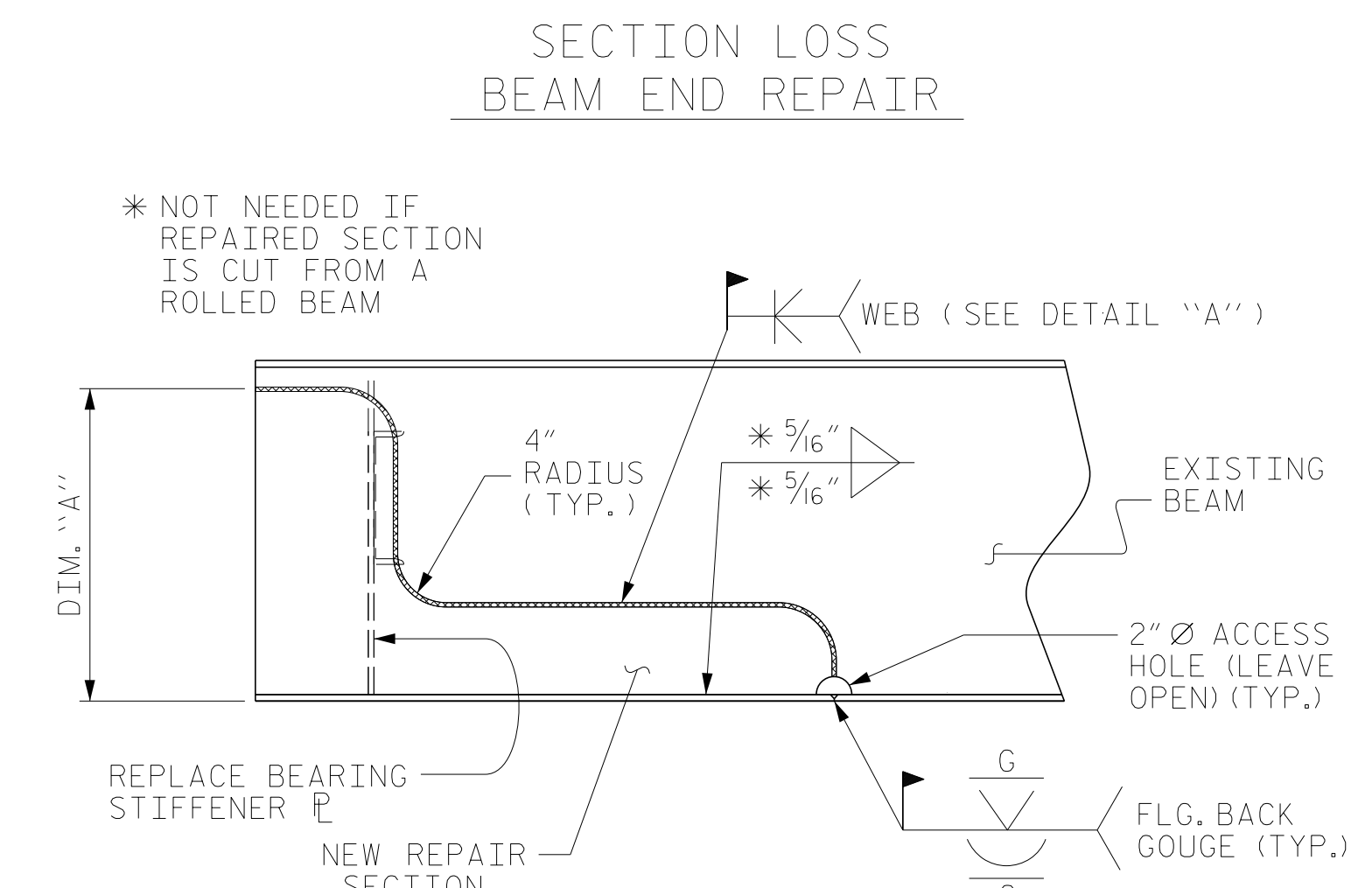
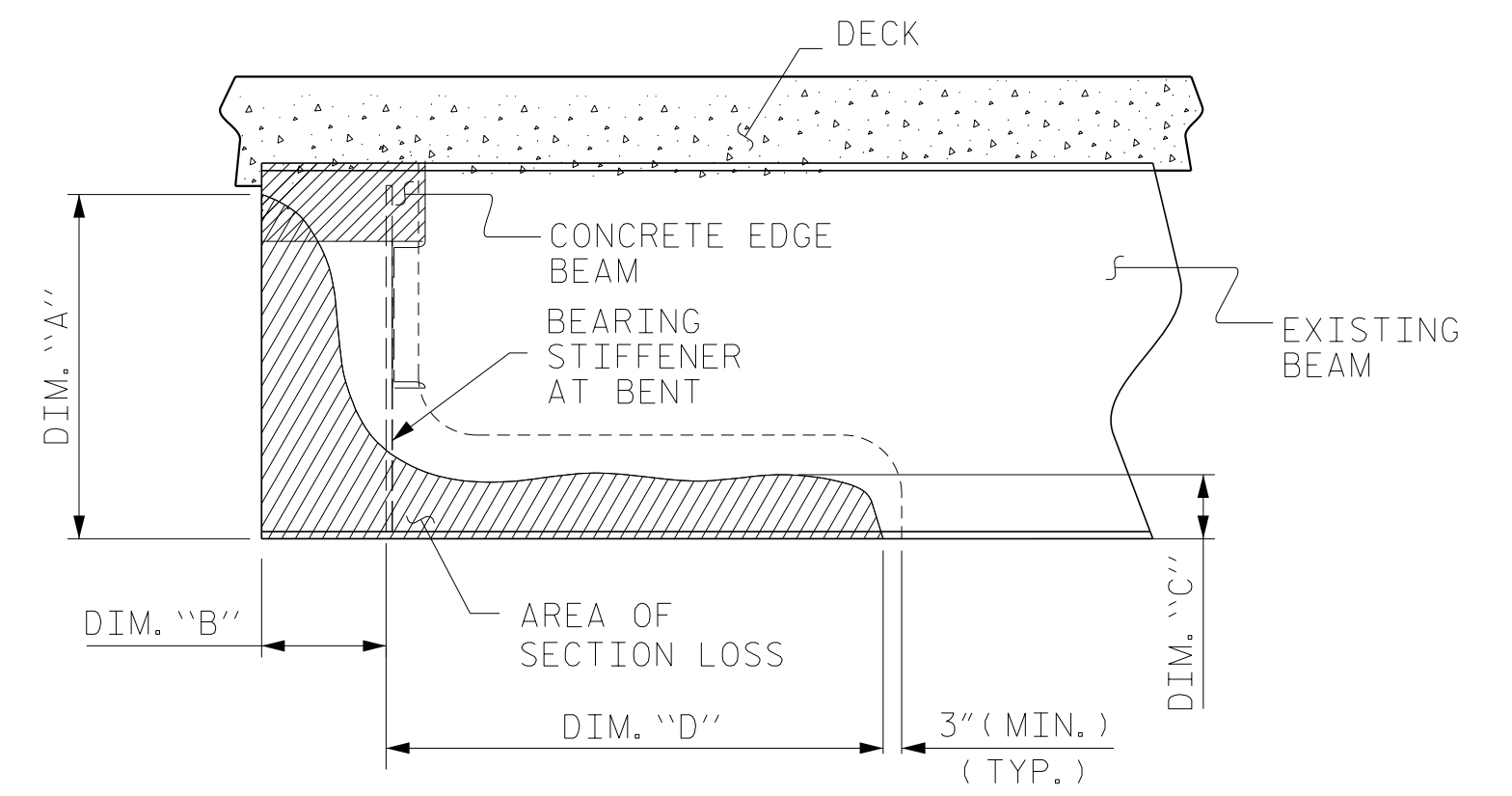


WSP USA Inc.  
 128 TOLBERT RD.  
 SUITE A  
 MOORESVILLE, NC 28117  
 TEL: 1.704.662.0100  
 LICENSE NO. F-0165



STIFFENER/CONN. REPAIR  
▲ FOR STIFFENERS, MILL TO BEAR AND DO NOT WELD

\* NOT NEEDED IF REPAIRED SECTION IS CUT FROM A ROLLED BEAM



**BEAM REPAIR NOTES:**

ALL CONDITIONS AND DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION OR INSTALLATION OF ANY COMPONENTS.

REPAIR PLATES SHALL BE MINIMUM 36 KSI STEEL. USE NEW OR SALVAGED "LIKE NEW" STEEL ONLY.

ALL WELDING SHALL BE IN ACCORDANCE WITH CURRENT APPLICABLE AWS AND NCDOT STANDARD SPECIFICATIONS.

PROVIDE RUN-OFF WELD TABS, WHERE APPLICABLE, TO PROVIDE PROPER WELD START AND TERMINATION. SEE NCDOT M&T FIELD WELD MANUAL AND AWS D1.5 SECTION 3.12.

ALL WELDS SHALL BE INSPECTED AND TESTED BY THE NCDOT MATERIALS AND TESTS UNIT IN ACCORDANCE WITH THE CURRENT AWS BRIDGE WELDING CODE AND STANDARD SPECIFICATIONS.

BEAM REPAIR SEQUENCE:

COORDINATE SCHEDULE WITH MATERIALS AND TESTS UNIT WELD INSPECTOR AT LEAST FOUR DAYS PRIOR TO ANTICIPATED WORK.

REMOVE TRAFFIC LOAD FROM REPAIR AREA BY EITHER CLOSING BRIDGE TO TRAFFIC OR SHIFTING TRAFFIC AWAY FROM REPAIR AREA.

JACK BEAM AND SUPPORT WITH BLOCKING TO FREE BEAM END FROM BEARING. LIMIT DIFFERENTIAL JACKING BETWEEN ADJACENT BEAMS TO 1/8".

STEEL DIAPHRAGM CHANNELS AND/OR STIFFENERS MAY BE TEMPORARILY REMOVED, IF NECESSARY, AND REPLACED AFTER BEAM REPAIR.

CUT OUT BY APPROPRIATE MEANS THE DAMAGED BEAM AREA AND/OR BEARING STIFFENER. IF BEAM DETERIORATION EXTENDS INTO THE CONCRETE DIAPHRAGM, CHIP AWAY CONCRETE AND REMOVE DAMAGED AREA.

MECHANICALLY CLEAN RUST, SCALE AND EXISTING PAINT TO AT LEAST 3" BEYOND REPAIR AREA.

INSTALL THE CUT-TO-FIT SECTION, FULLY WELD ALONG TOP AND SIDES OF PLATE.

ONCE THE REPAIR IS COMPLETE, GRIND ALL WELDS FLUSH. ANY GOUGES OR INDENTATIONS FROM IMPACT ON BEAMS SHALL BE GROUND SMOOTH. CLEAN AREA TO REMOVE DEBRIS AND OILS FROM REPAIR PROCESS PRIOR TO PAINTING.

LOWER SPAN TO BEAR; CHECK FOR DISTRESS.

REMOVE JACKING EQUIPMENT AND TEMPORARY SUPPORTS.

CLEAN AND PAINT STRUCTURAL STEEL. CLEANING AND PAINTING OF REPAIRED STRUCTURAL STEEL SHALL BE PERFORMED AS PART OF THE OVERALL CLEANING AND PAINTING CONTRACT.

FOR CLEANING AND PAINTING, SEE PAINTING EXISTING STRUCTURE SPECIAL PROVISIONS.

AFTER BEAMS AND/OR STIFFENERS/CONNECTOR PLATES ARE REPAIRED AND PAINTED, ANY CONCRETE REMOVED FROM THE BENT DIAPHRAGMS, EDGE BEAM OR DECK SHALL BE CAST BACK. ANY REINFORCING STEEL CUT DURING THE REMOVAL PROCESS SHALL BE SPLICED WITH A SIMILAR SIZE BAR WITH AT LEAST A ONE FOOT SPLICE TO THE EXISTING STEEL. NO SEPARATE PAYMENT SHALL BE MADE FOR CONCRETE AND REINFORCING STEEL AS THIS IS CONSIDERED INCIDENTAL TO THE PAY ITEM "BEAM REPAIR". FOR BEAM REPAIR, SEE SPECIAL PROVISIONS.

RETURN TRAFFIC TO NORMAL PATTERN.

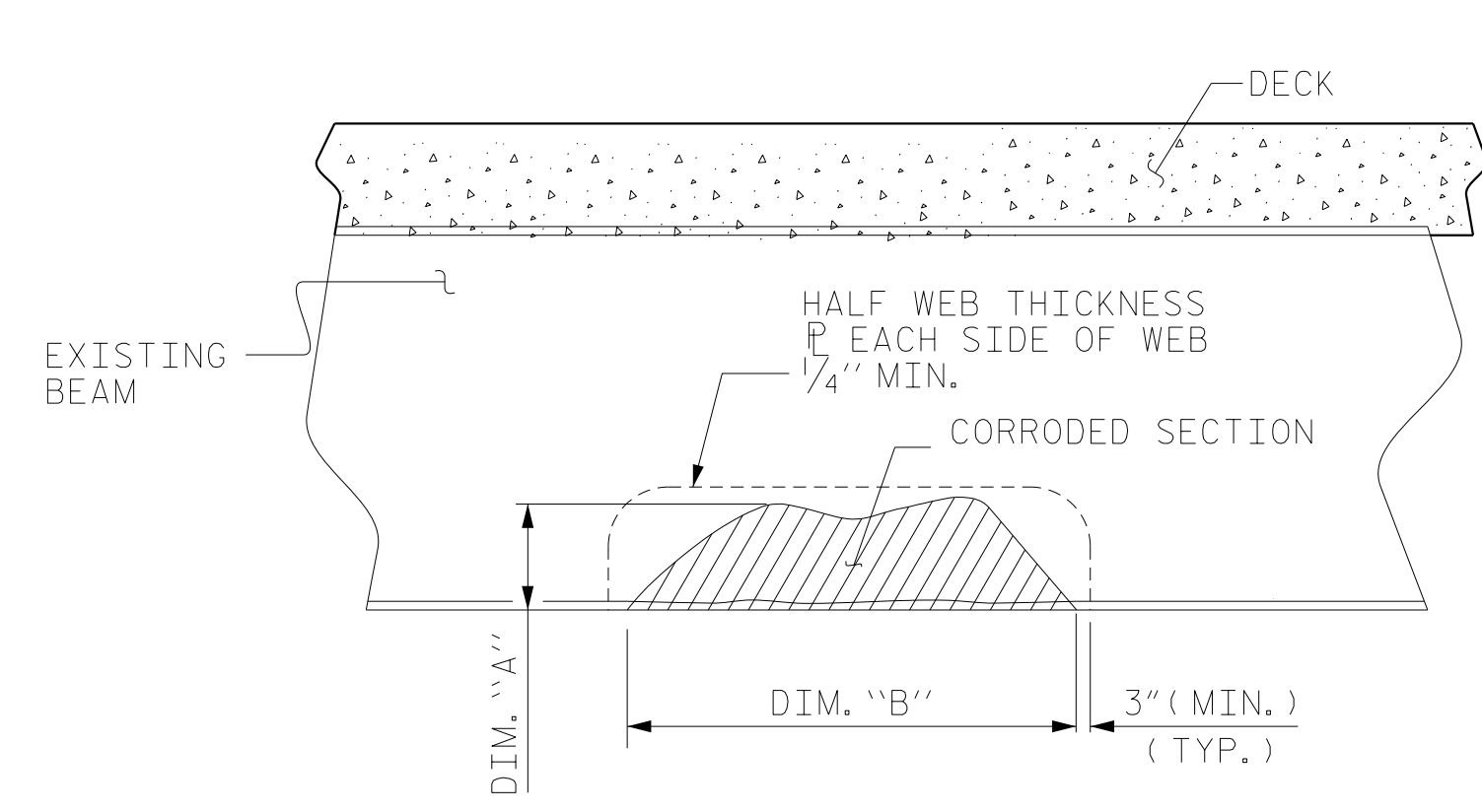
PROJECT NO. U-6020  
GRANVILLE COUNTY  
BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

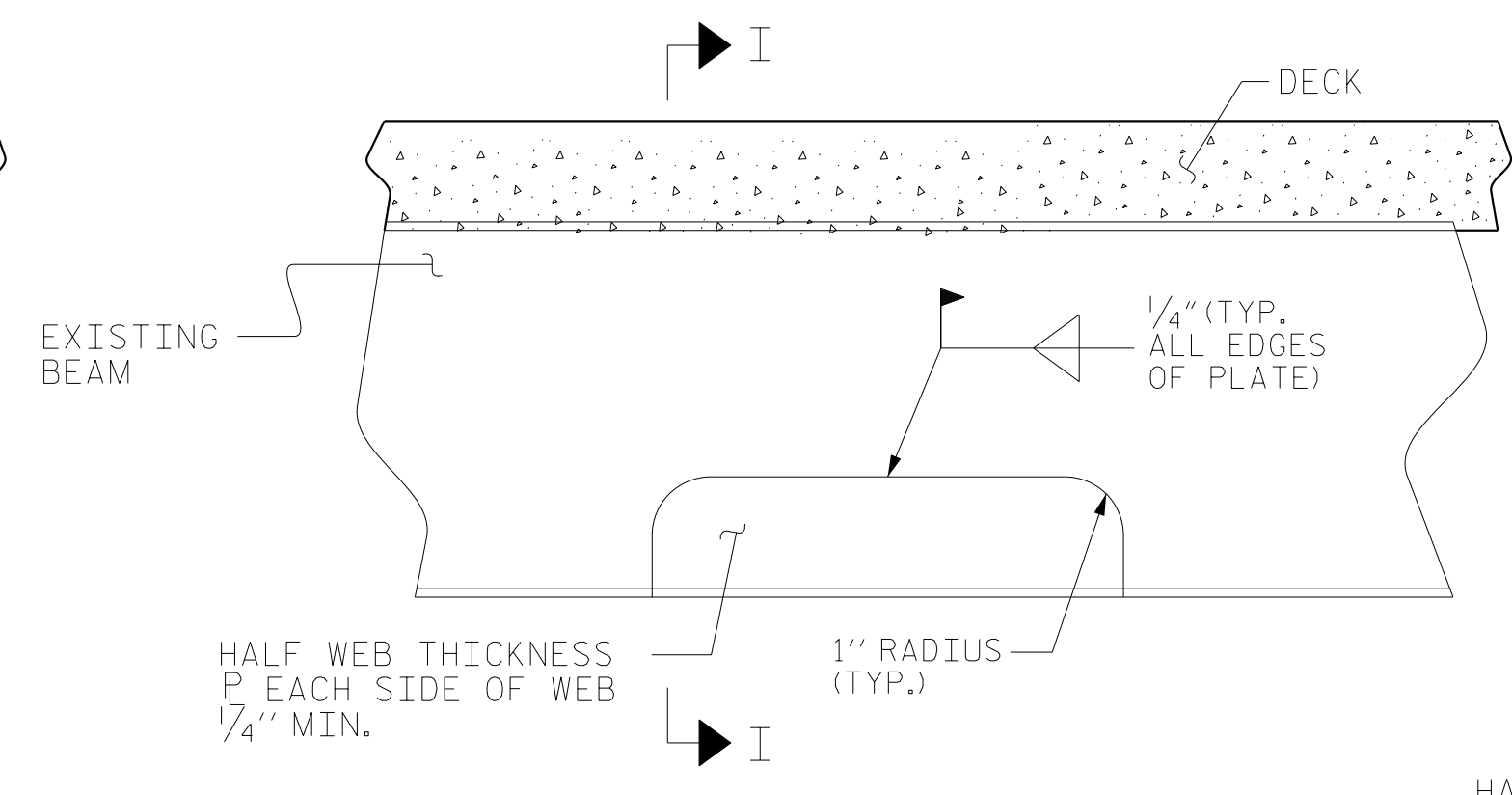
DocuSigned by:  
Michael W. Craig  
498954685460  
5/3/2024

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
<b>BEAM REPAIR DETAILS</b>					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-12
					TOTAL SHEETS 16

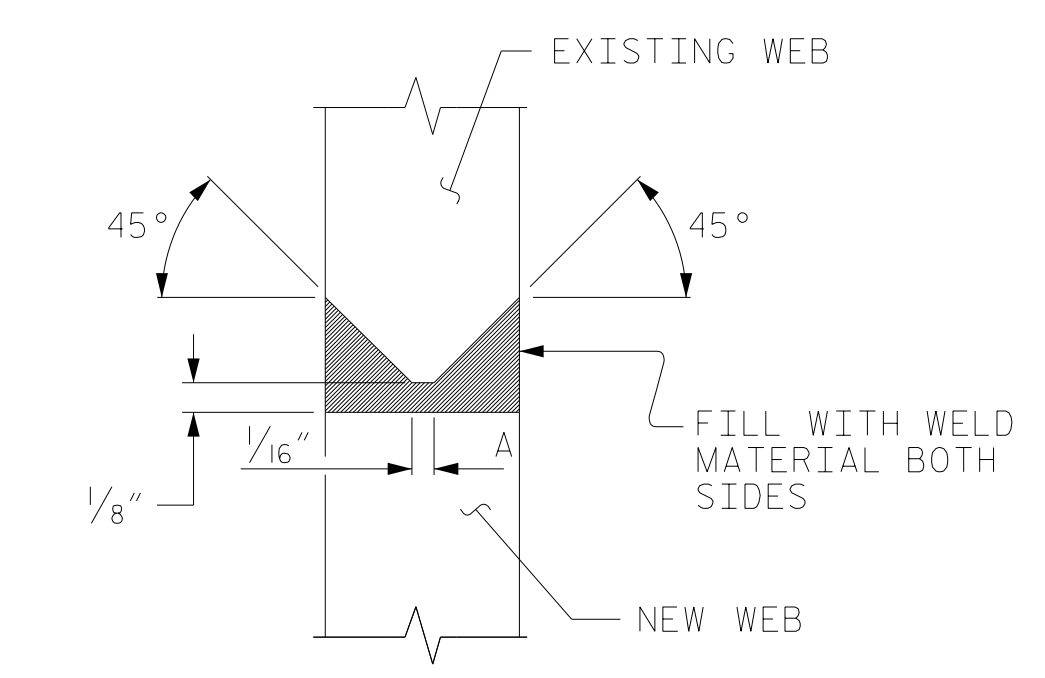
DRAWN BY : J.MYA DATE : 3/2019  
CHECKED BY : J.YANNACCONE DATE : 3/2019  
DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024



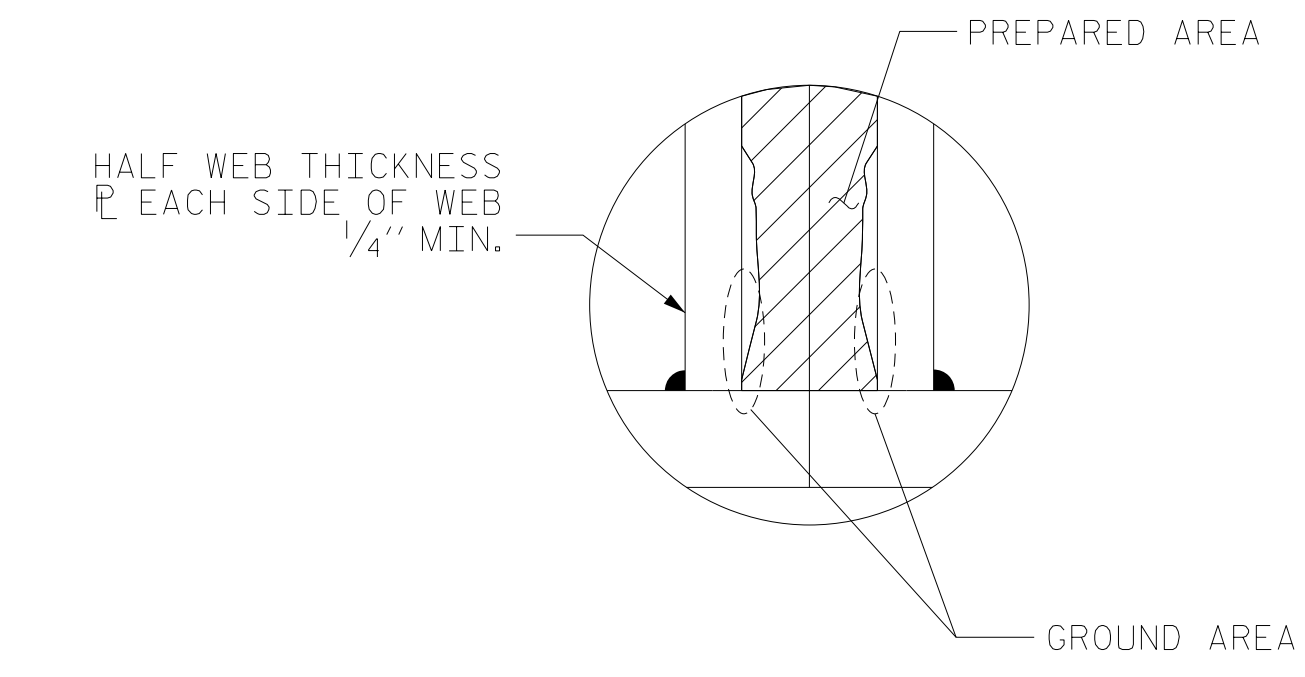
INTERMEDIATE SECTION LOSS  
BEAM PLATING REPAIR



INTERMEDIATE BEAM  
PLATING REPAIR



DETAIL "A"



DETAIL "B"

INTERMEDIATE BEAM  
PLATING REPAIR

BEAM PLATING REPAIR NOTES:

ALL CONDITIONS AND DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION OR INSTALLATION OF ANY COMPONENTS.

REPAIR PLATES SHALL BE MINIMUM 36 KSI STEEL. USE NEW OR SALVAGED "LIKE NEW" STEEL ONLY.

ALL WELDING SHALL BE IN ACCORDANCE WITH CURRENT APPLICABLE AWS AND NCDOT STANDARD SPECIFICATIONS.

ALL WELDS SHALL BE INSPECTED AND TESTED BY THE NCDOT MATERIALS AND TESTS UNIT IN ACCORDANCE WITH THE CURRENT AWS BRIDGE WELDING CODE AND STANDARD SPECIFICATIONS.

BEAM PLATING REPAIR SEQUENCE:

COORDINATE SCHEDULE WITH MATERIALS AND TESTS UNIT WELD INSPECTOR AT LEAST FOUR DAYS PRIOR TO ANTICIPATED WORK.

REMOVE TRAFFIC LOAD FROM REPAIR AREA BY EITHER CLOSING BRIDGE TO TRAFFIC OR SHIFTING TRAFFIC AWAY FROM REPAIR AREA.

MECHANICALLY CLEAN RUST, SCALE AND EXISTING PAINT TO AT LEAST 3" BEYOND REPAIR AREA.

IF NECESSARY, REMOVE EXISTING STIFFENER TO INSTALL WELDED PLATE REPAIR. REPLACE WITH A NEW STIFFENER PLATE OF SIMILAR SIZE. FOR STIFFENER/CONNECTOR PLATE REPAIR DETAILS, SEE "BEAM REPAIR DETAILS" SHEET.

IF BEAM DETERIORATION EXTENDS INTO THE CONCRETE DIAPHRAGM THEN CHIP AWAY CONCRETE TO DETERMINE THE EXTENT OF THE DAMAGE.

PRIME ENTIRE REPAIR AREA AND REPAIR PLATES WITH AN ORGANIC ZINC PRIMER PRIOR TO WELDING NEW PLATES. REMOVE PRIMER IN WELD AREA.

ONE PLATE SHALL BE PLACED, AS INDICATED, ON EACH SIDE OF THE BEAM WEB.

EACH PLATE SHALL BE APPROXIMATELY ONE-HALF THE ORIGINAL THICKNESS OF THE BEAM WEB.

FULLY WELD ALONG TOP AND SIDES OF PLATE.

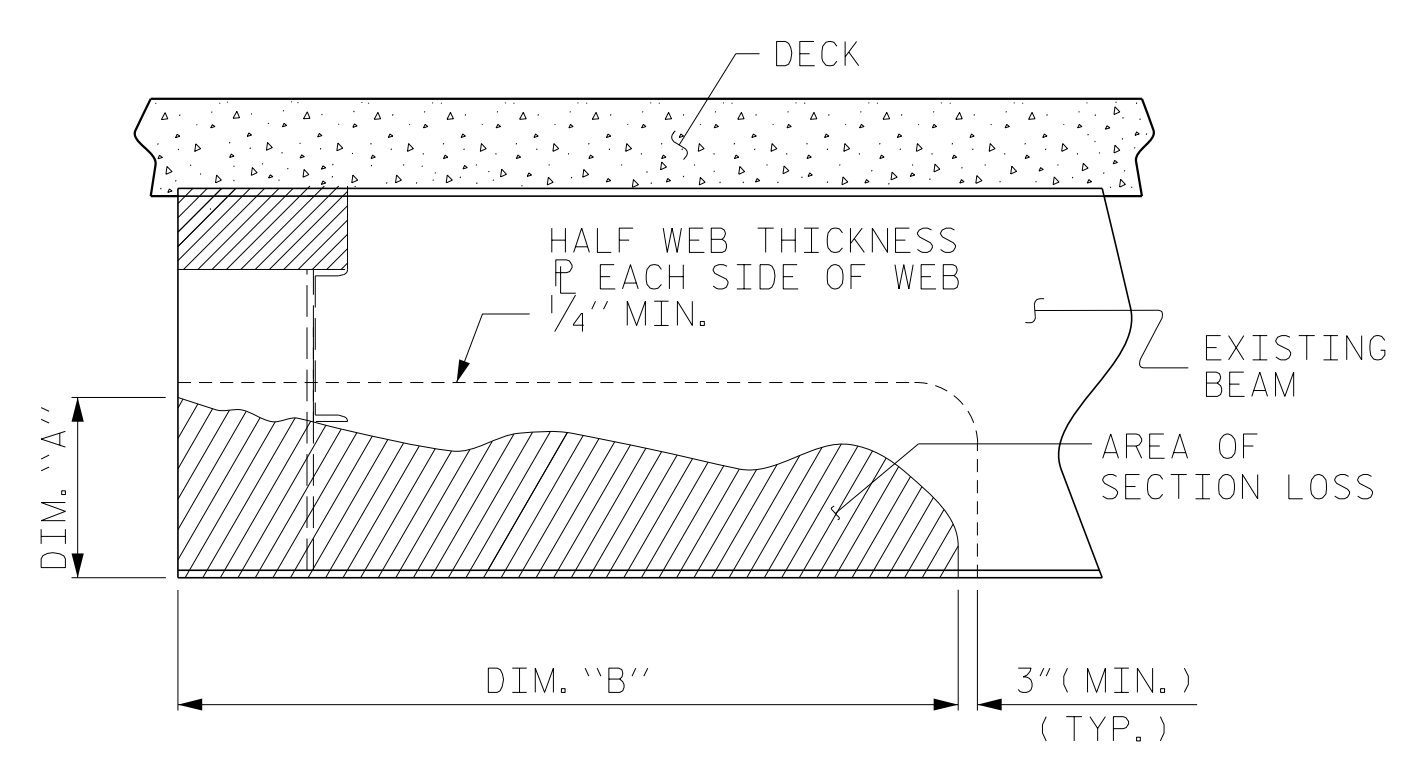
ONCE THE REPAIR IS COMPLETE, GRIND ALL WELDS FLUSH. ANY GOUGES OR INDENTATIONS FROM IMPACT ON BEAMS SHALL BE GROUND SMOOTH. CLEAN AREA TO REMOVE DEBRIS AND OILS FROM REPAIR PROCESS PRIOR PAINTING.

CLEAN AND PAINT STRUCTURAL STEEL. CLEANING AND PAINTING OF REPAIRED STRUCTURAL STEEL SHALL BE PERFORMED AS PART OF THE OVERALL CLEANING AND PAINTING CONTRACT.

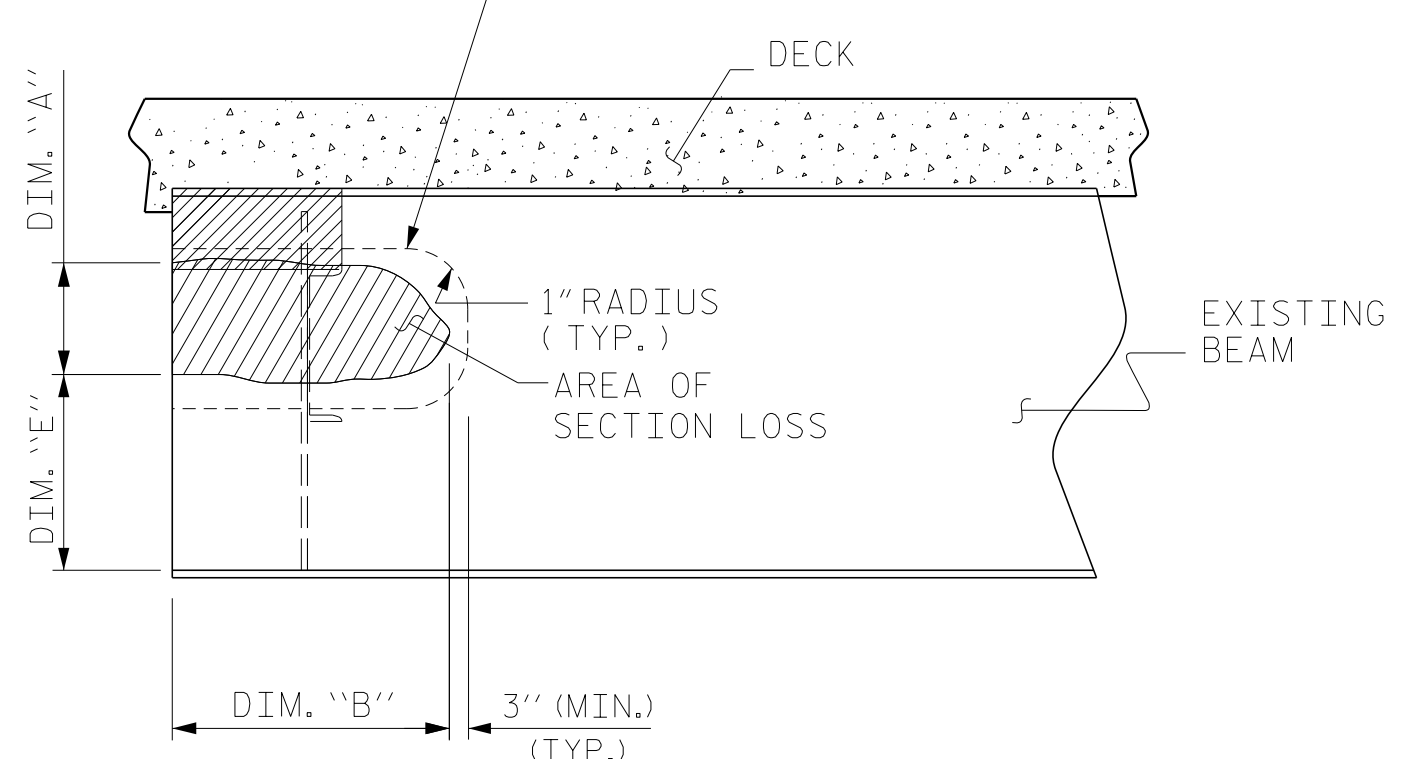
FOR CLEANING AND PAINTING, SEE PAINTING EXISTING STRUCTURE SPECIAL PROVISIONS.

AFTER BEAMS AND/OR STIFFENERS/CONNECTOR PLATES ARE REPAIRED AND PAINTED, ANY CONCRETE REMOVED FROM THE BENT DIAPHRAGMS SHALL BE CAST BACK. ANY REINFORCING STEEL CUT DURING THE REMOVAL PROCESS SHALL BE SPLICED WITH A SIMILAR SIZE BAR WITH AT LEAST A ONE FOOT SPLICE TO THE EXISTING STEEL. NO SEPARATE PAYMENT SHALL BE MADE FOR CONCRETE AND REINFORCING STEEL AS THIS IS CONSIDERED INCIDENTAL TO THE PAY ITEM "BEAM PLATING REPAIR". FOR BEAM PLATING REPAIR, SEE SPECIAL PROVISIONS.

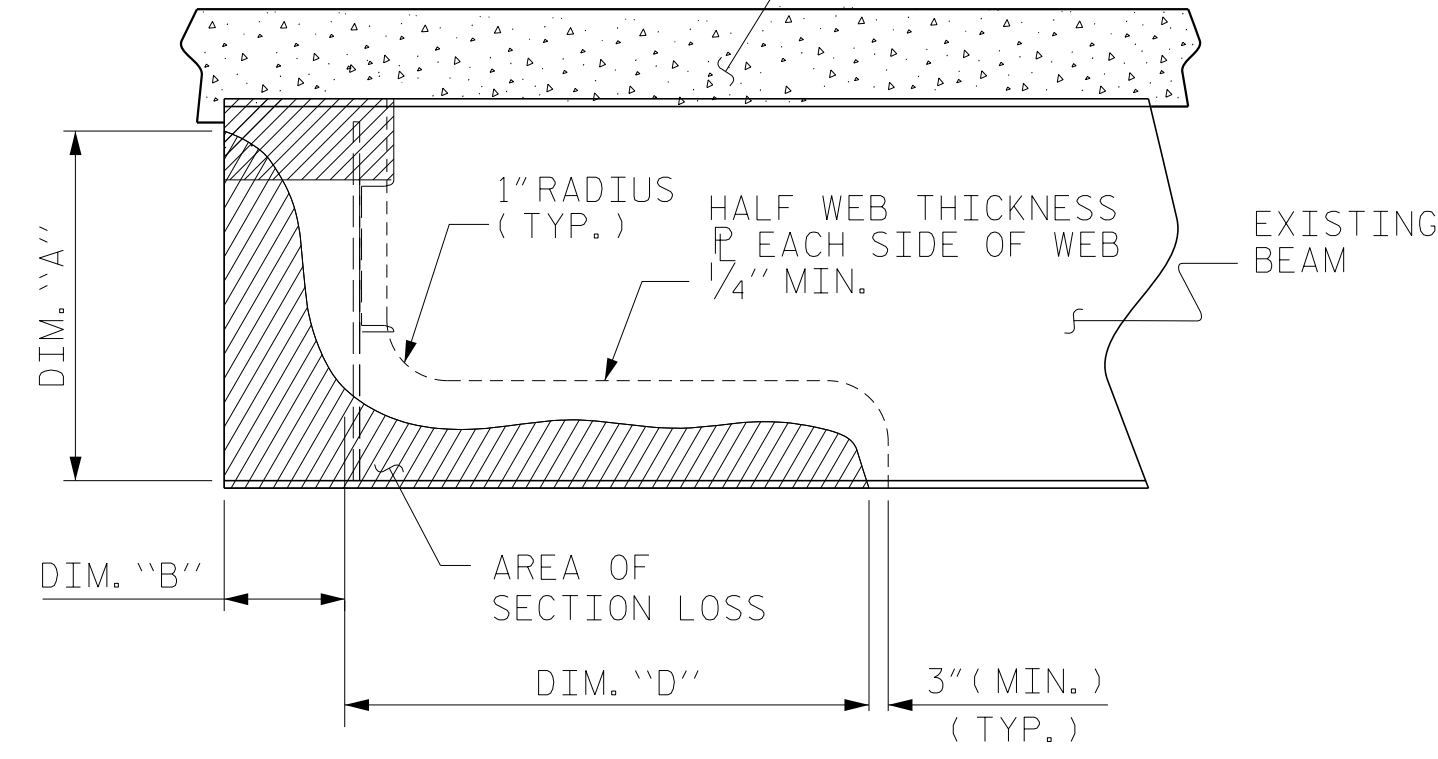
RETURN TRAFFIC TO NORMAL PATTERN.



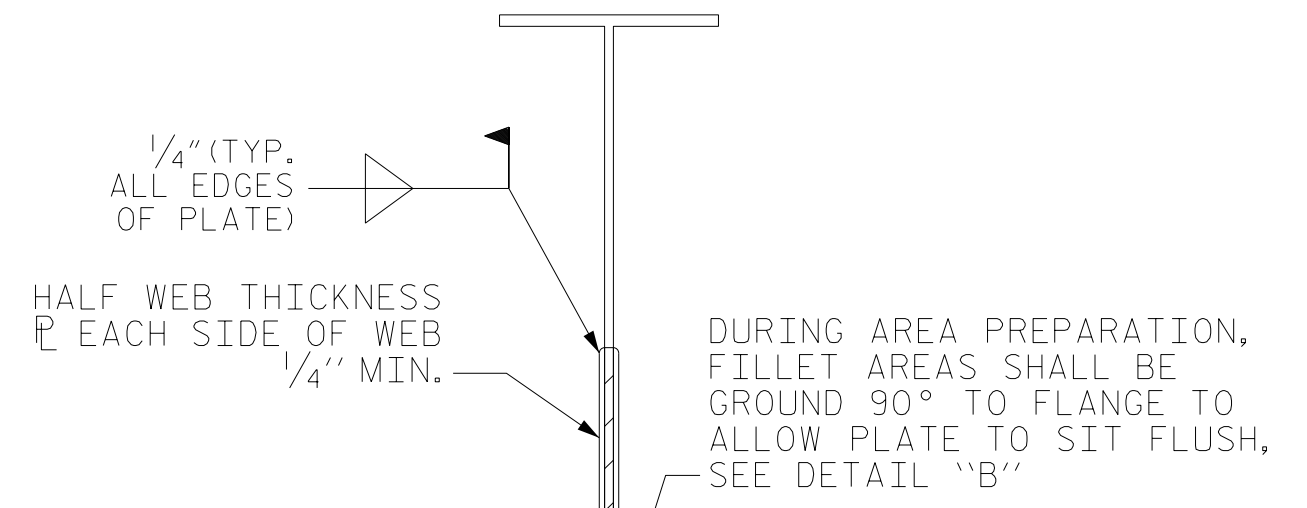
BEAM END SECTION LOSS



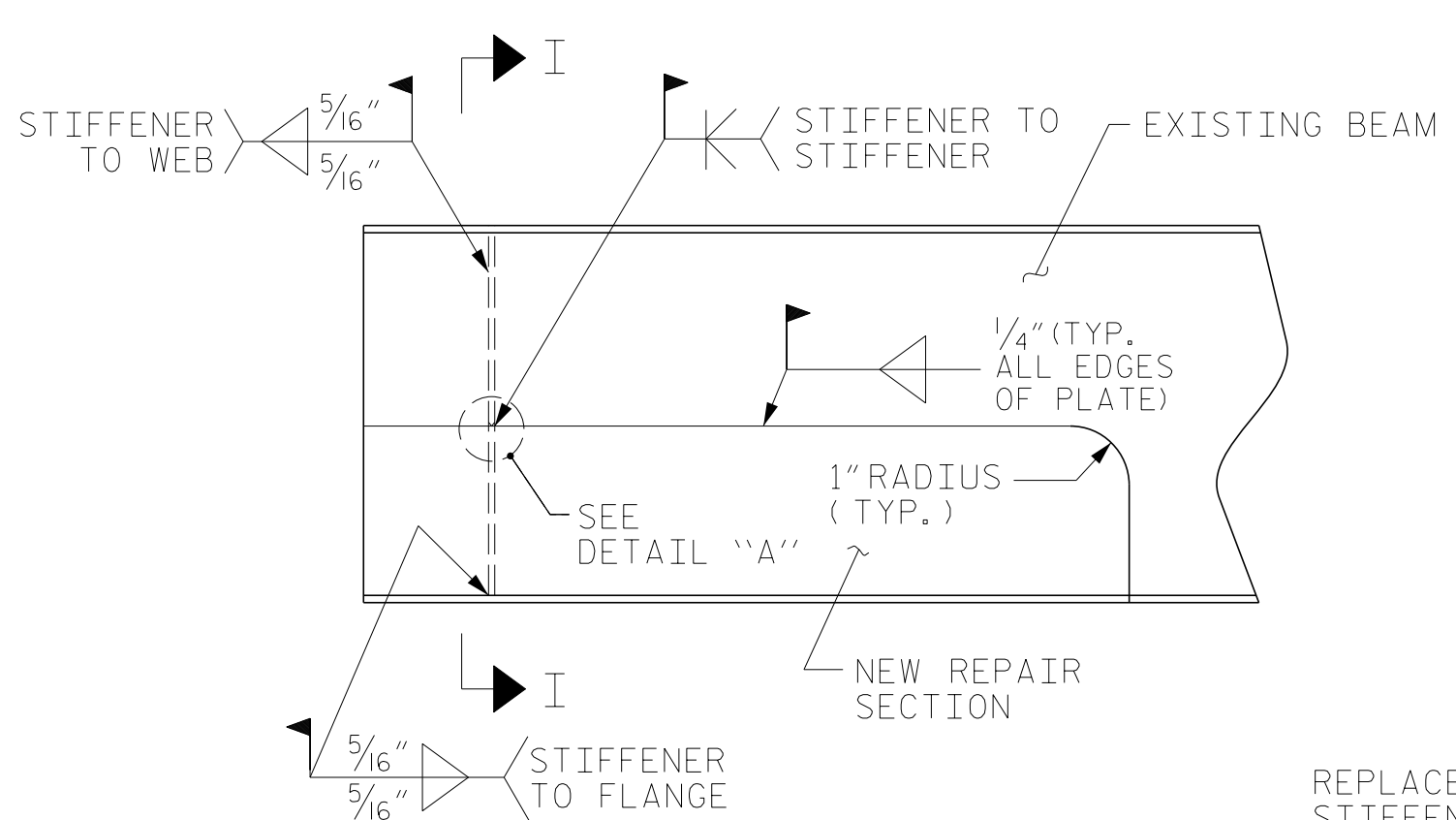
BEAM END SECTION LOSS



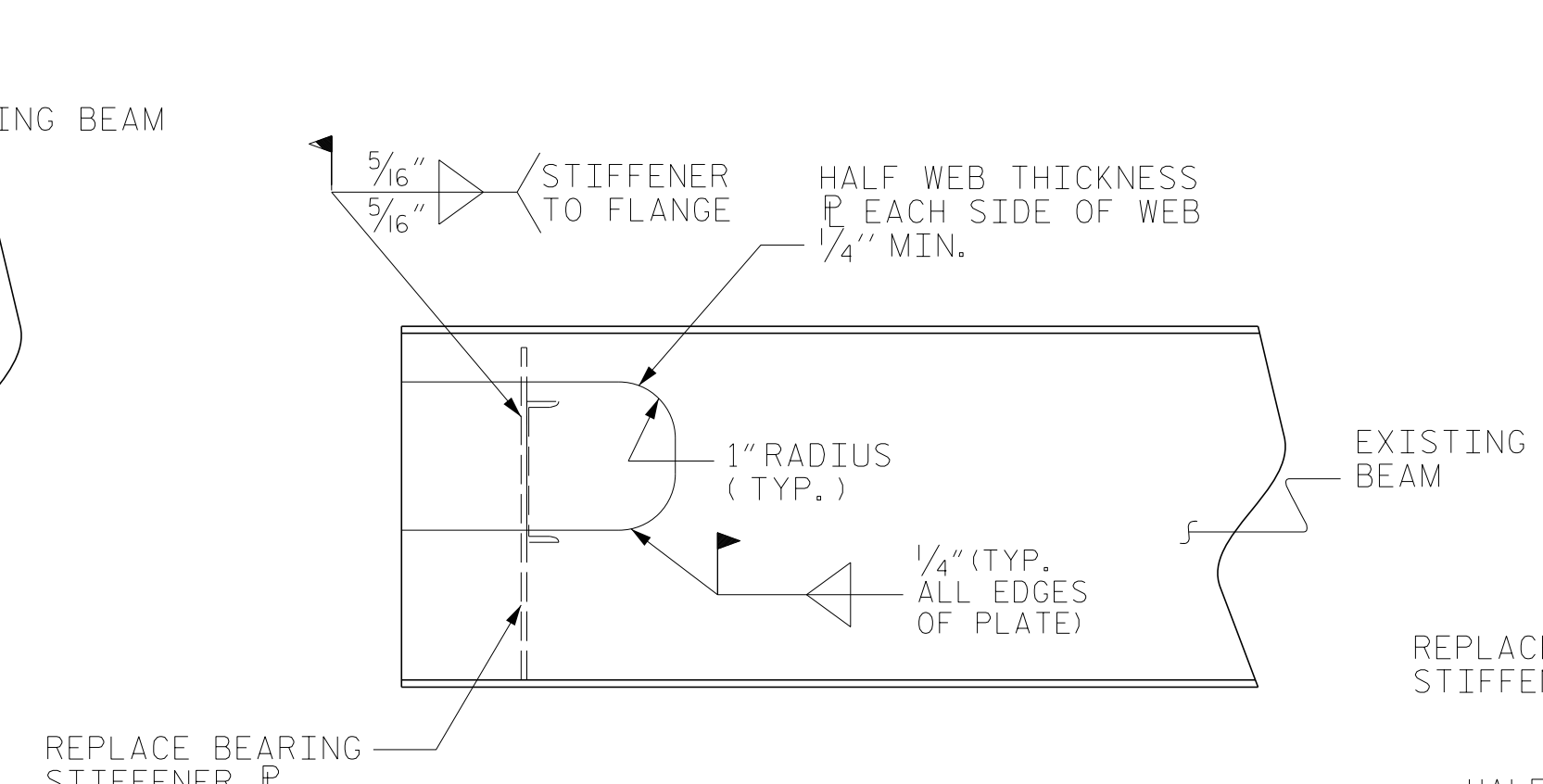
BEAM END SECTION LOSS



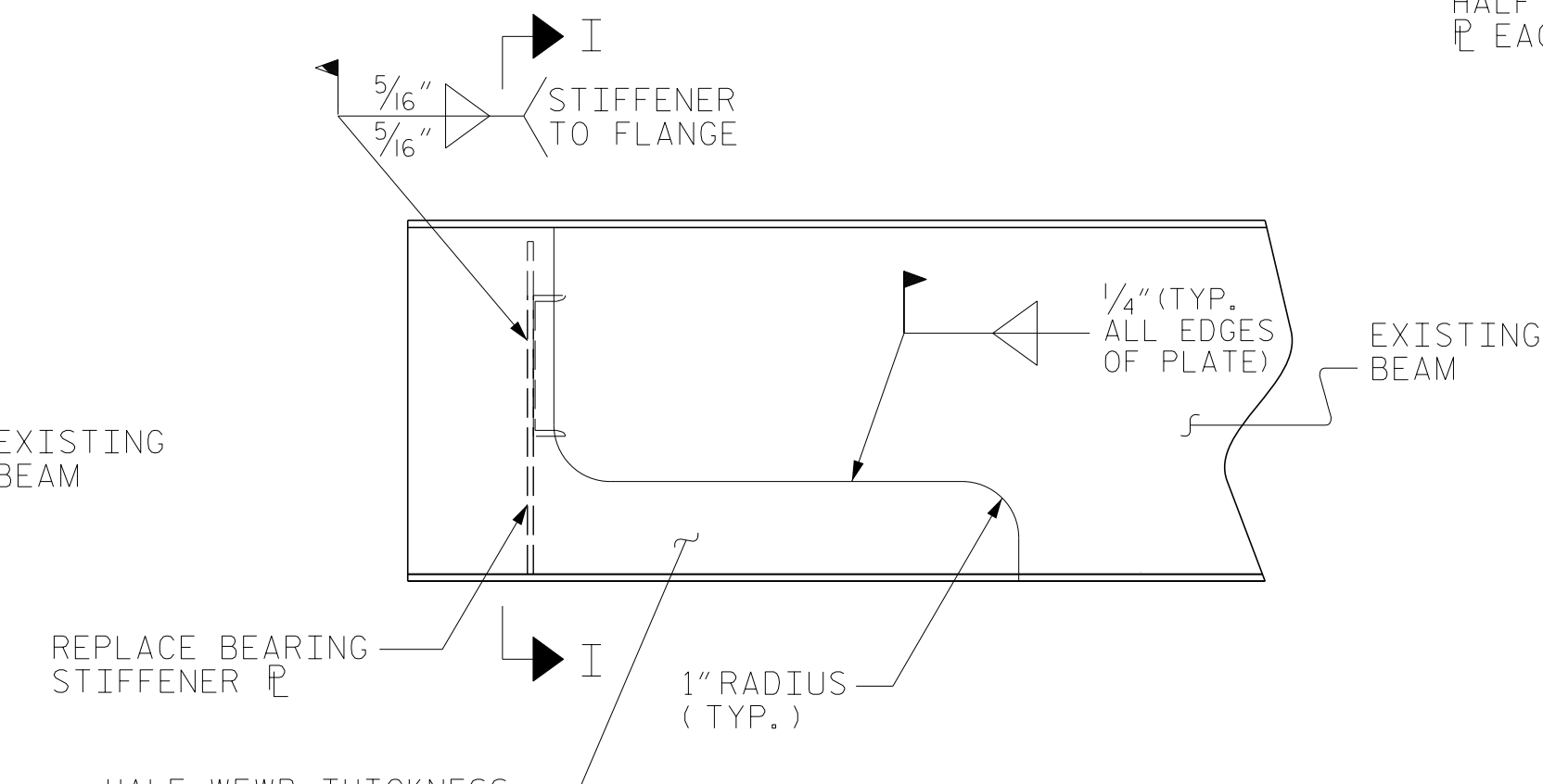
SECTION I-I



BEAM END  
PLATING REPAIR



BEAM END  
PLATING REPAIR



BEAM END  
PLATING REPAIR

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED



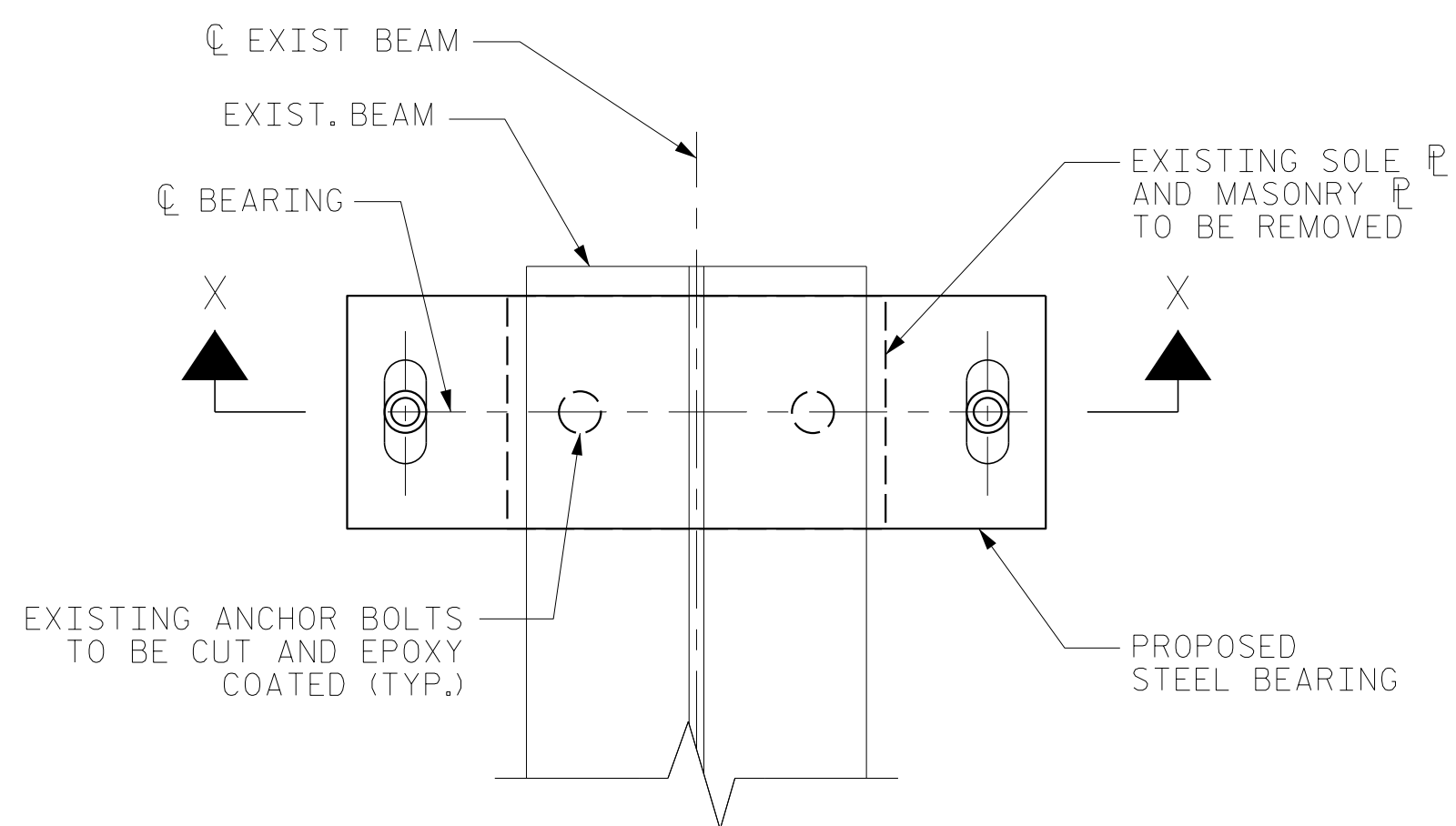
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

BEAM PLATING  
 REPAIR DETAILS

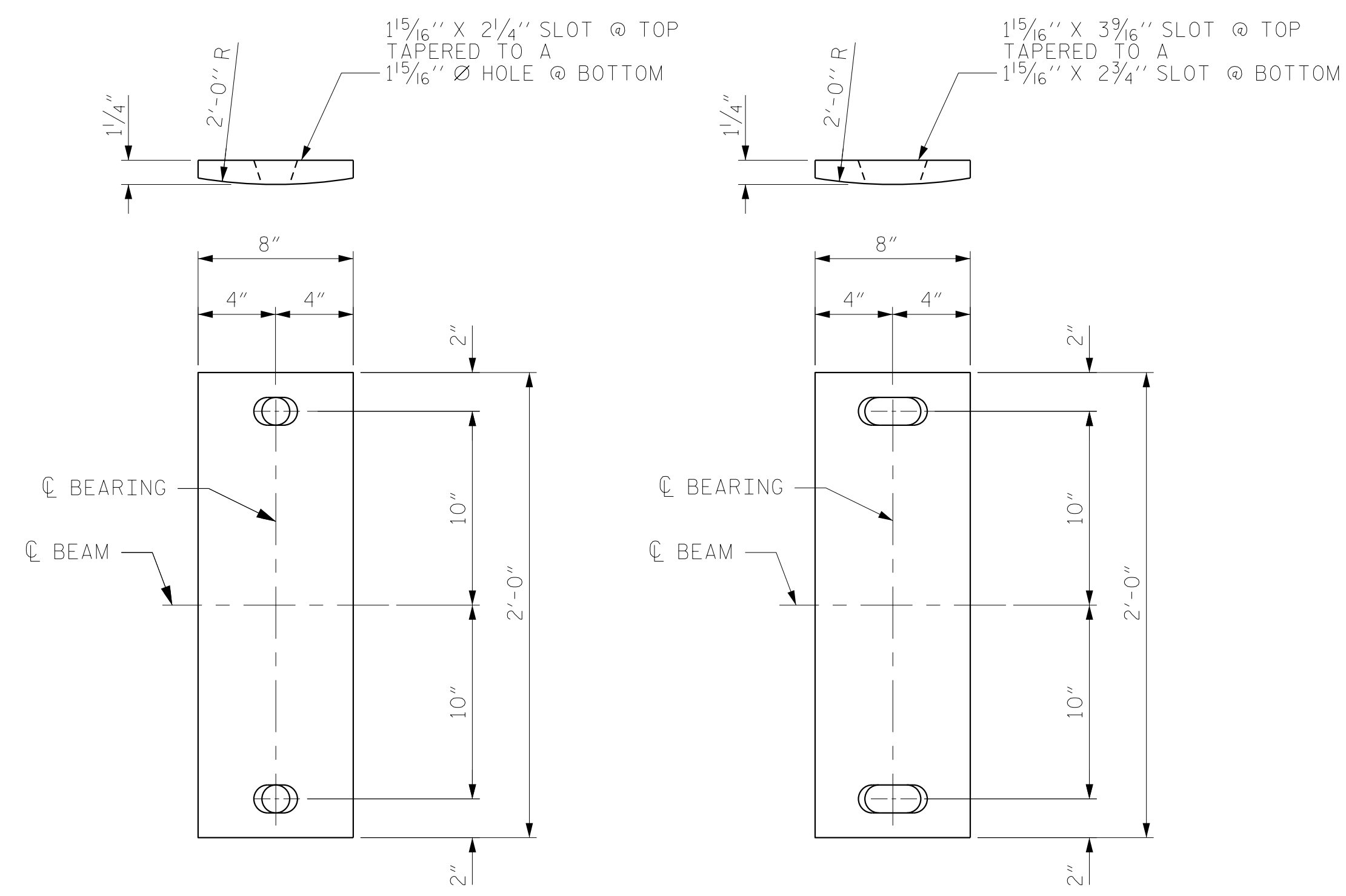
DRAWN BY : J.MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

BEAM END PLATING REPAIR

wsp		WSP USA Inc. 128 TIM BERT RD. MOOREVILLE, NC 28117 TEL: 1.704.662.0100 LICENSE No. F-0165		REVISIONS		SHEET NO. S-13	
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS 16	
1			3				
2			4				

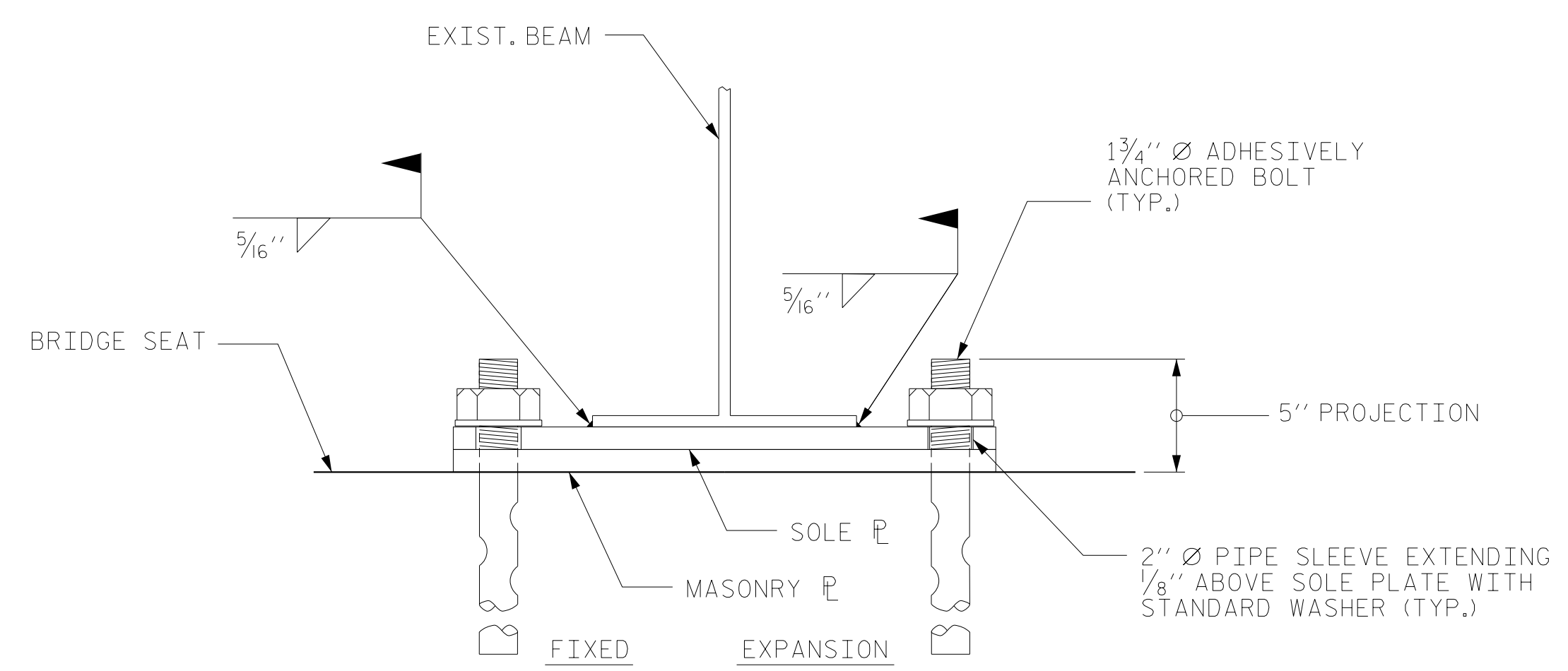


SOLE PLATE PLAN

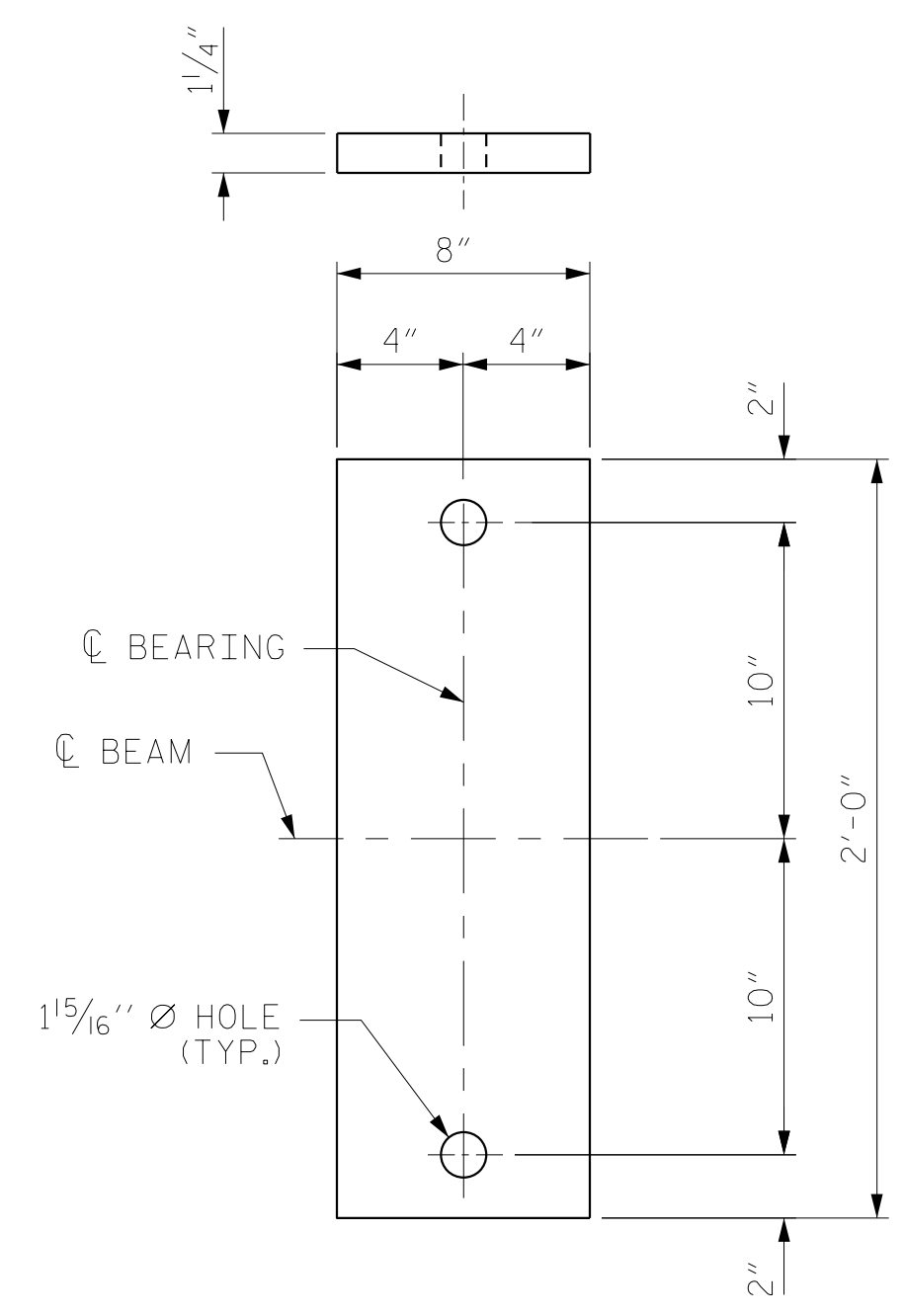


SOLE PLATE (FIXED)

SOLE PLATE (EXPANSION)



SECTION X-X



MASONRY PLATE

PLATE DETAILS

NOTES:

- CONTRACTOR SHALL FIELD VERIFY PLATE THICKNESS REQUIRED.
- CUT EXISTING ANCHOR BOLTS FLUSH TO TOP OF THE CONCRETE. BOLT ENDS SHALL BE COATED WITH AN APPROVED EPOXY PAINT.
- THE CONTRACTOR SHALL CORE INTO EXISTING BENT CAP TO INSTALL 1 3/4" Ø ANCHOR BOLTS. BOLTS SHALL BE ADHESIVELY ANCHORED. FOR ADHESIVELY ANCHORED ANCHOR BOLTS, SEE STANDARD SPECIFICATIONS.
- CONTRACTOR SHALL SUBMIT PROPOSED ADHESIVE FOR APPROVAL. ADHESIVE FOR NEW ANCHOR BOLTS SHALL BE ON THE NCDOT APPROVED PRODUCT LIST FOR THE PROPOSED USED.
- EMBEDMENT DEPTH OF ANCHOR BOLT SHALL BE 9", OR THE DEPTH RECOMMENDED BY THE ADHESIVE MANUFACTURER TO ATTAIN PULL-OUT STRENGTH OF THE TEST LOAD GIVEN BELOW, WHICHEVER DEPTH IS GREATER.
- NEW ADHESIVELY ANCHORED BOLTS SHALL BE SUBJECT TO LEVEL 1 FIELD TESTING, IN ACCORDANCE WITH STANDARD SPECIFICATION ARTICLE 420-13 (C), EXCEPT THAT THE TEST LOAD SHALL BE 18,000 LBS. TENSION.
- AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THE BACKED OFF 1/2" TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.
- THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENT OF ASTM D1785.
- THE PAYMENT FOR THE PIPE SLEEVES SHALL BE INCLUDED IN THE PAY ITEM, "STEEL BEARING REPLACEMENT".
- SOLE PLATES, BOLT, NUT, AND WASHERS SHALL BE GALVANIZED ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATION.
- REMOVE GALVANIZING OR ANY OTHER COATING AT THE LOCATION OF FIELD WELDS AND PREPARE THE WELD AREAS AS PER ARTICLE 440-7 OF THE STANDARD SPECIFICATION.
- FIELD PAINTING IS CONDUCTED AFTER ERECTION, OR WHEN DAMAGE TO A SHOP APPLIED COATING SYSTEM IS REPAIRED OR WHEN STEEL IS OTHERWISE PAINTED OUTSIDE AN ENCLOSED SHOP ENVIRONMENT. THE STRUCTURAL STEEL SHOP COATINGS PROGRAM SHALL BE CONSIDERED IN CONJUNCTION WITH THE PROJECT SPECIAL PROVISIONS FOR FIELD APPLICATIONS.
- THE CONTRACTOR SHALL VERIFY THE BOLT SPACING PRIOR TO FABRICATION.
- ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

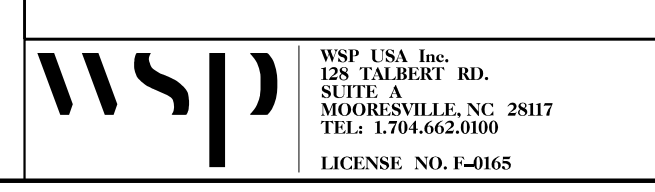
DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

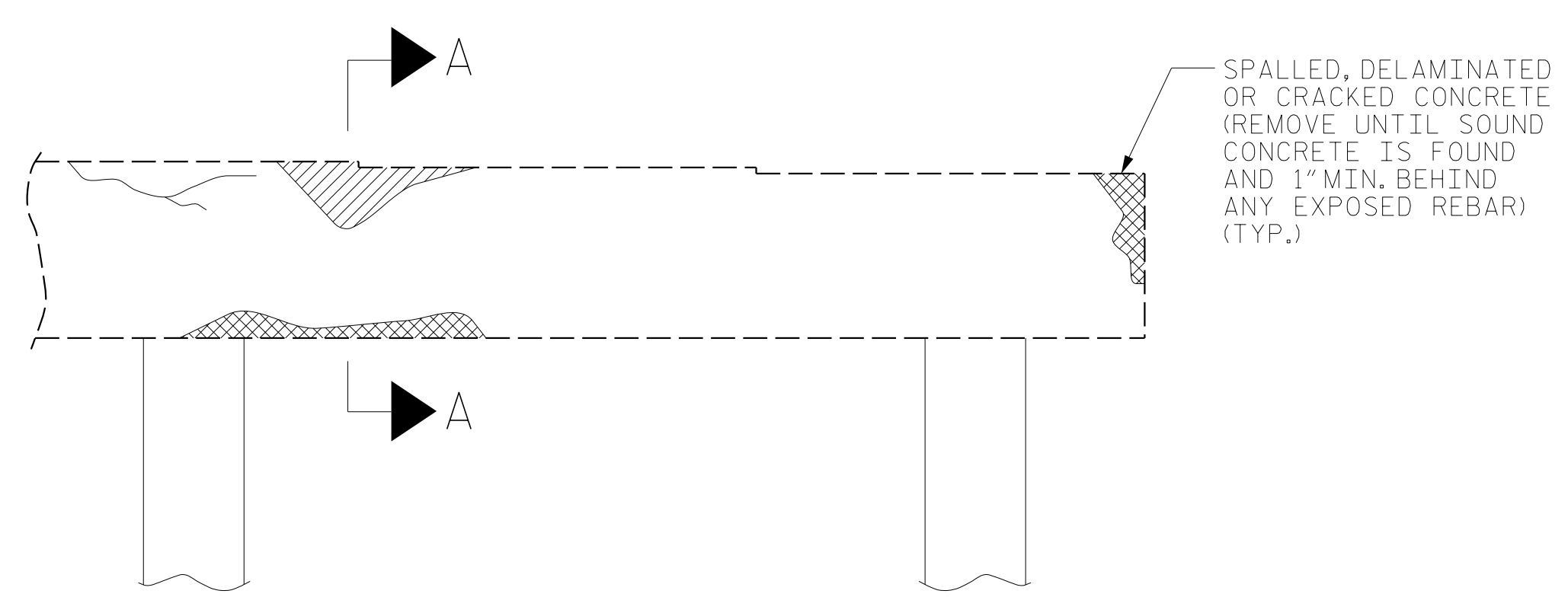
BEARING DETAILS

DRAWN BY :	J.MYA	DATE :	3/2019
CHECKED BY :	J.YANNACCONE	DATE :	3/2019
DESIGN ENGINEER OF RECORD :	MICHAEL W. CRAIG	DATE :	4/2024

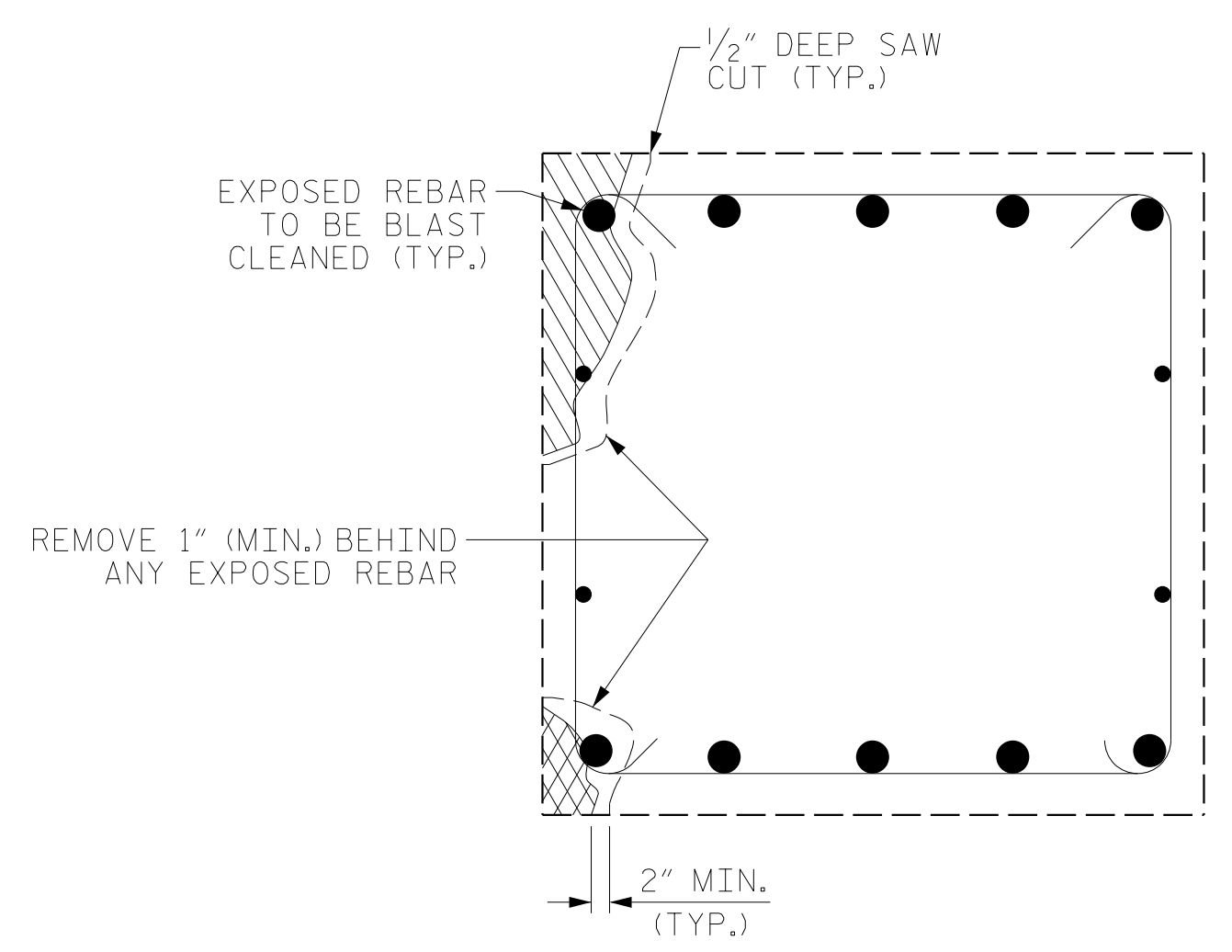


REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			TOTAL SHEETS
2			4			16

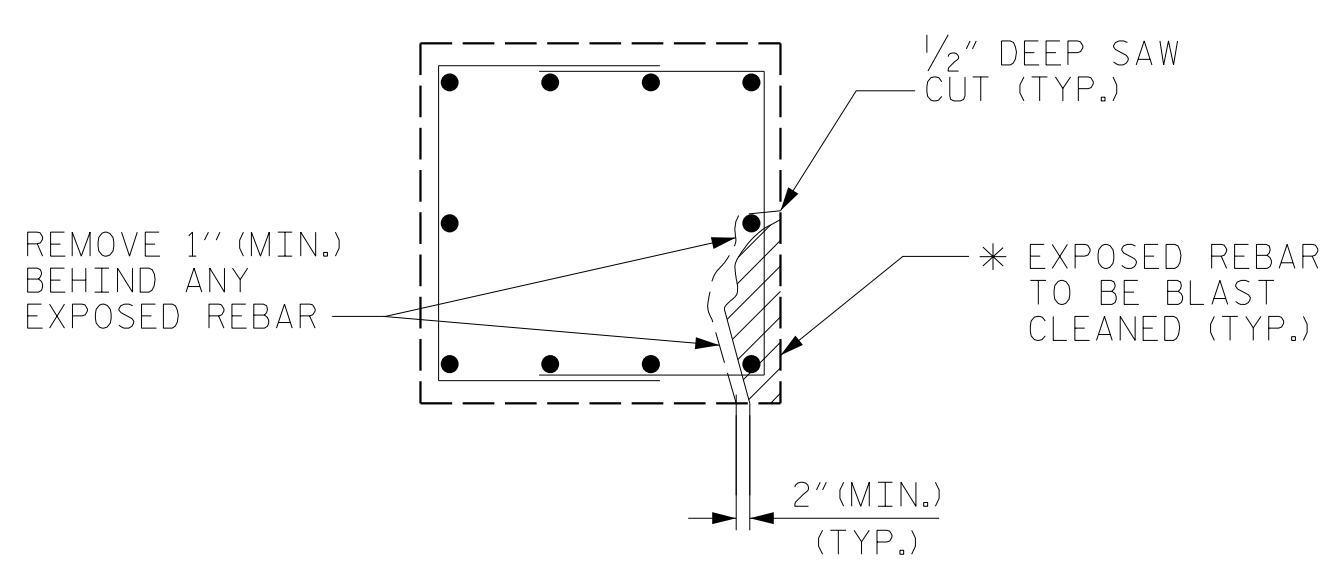




BENT CAP REPAIR



SECTION A-A  
CAP REPAIR

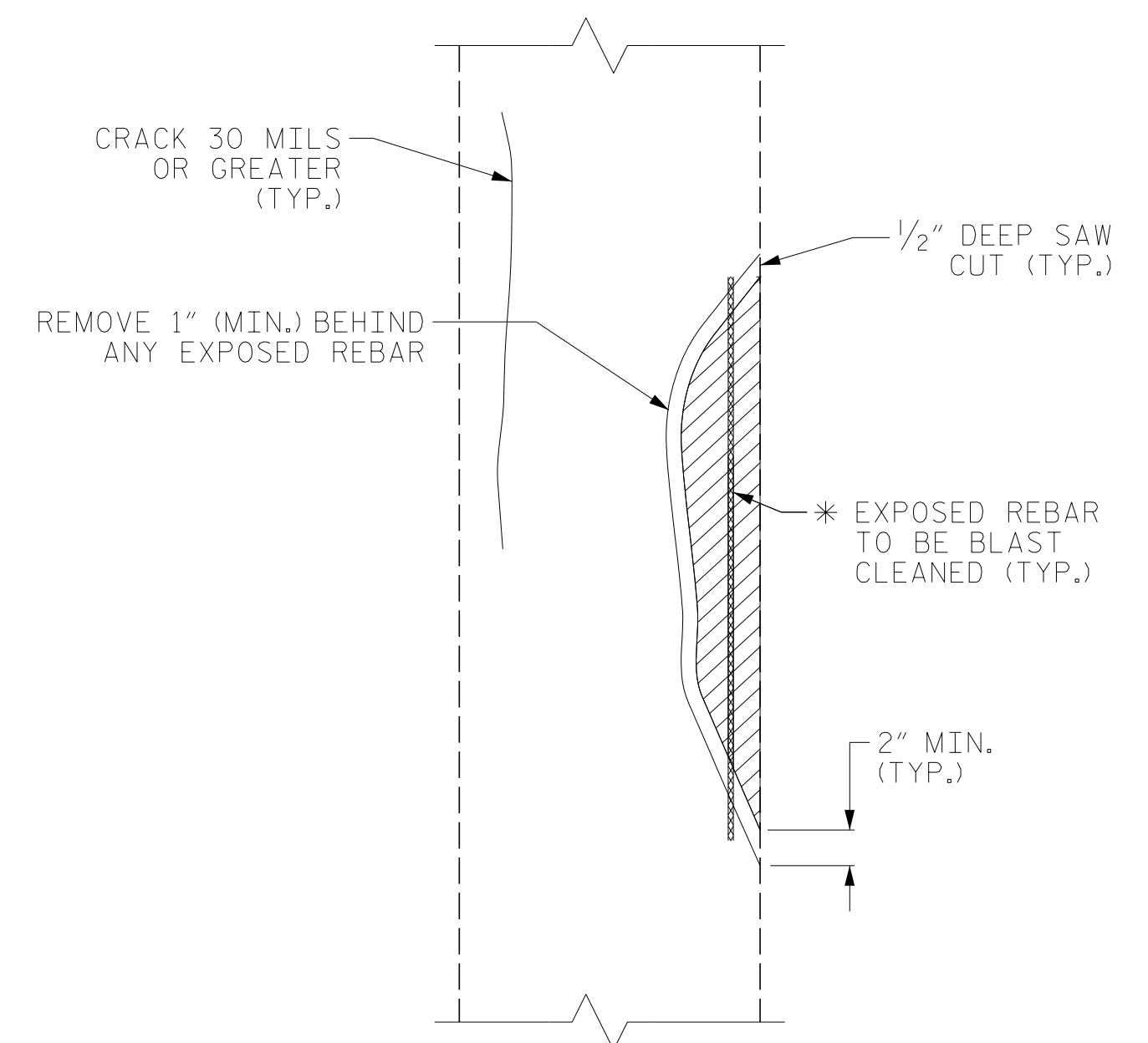


PLAN OF COLUMN

REPAIR KEY

- CONCRETE REPAIR AREA (FORM AND POUR)
- SHOTCRETE REPAIR AREA
- EPOXY RESIN INJECTION

SPLICE LENGTH TABLE	
BAR SIZE	MINIMUM SPLICE LENGTH
#4	2'-4"
#5	2'-9"
#6	4'-0"
#7	5'-3"
#8	6'-9"
#9	8'-6"
#10	10'-11"
#11	13'-4"



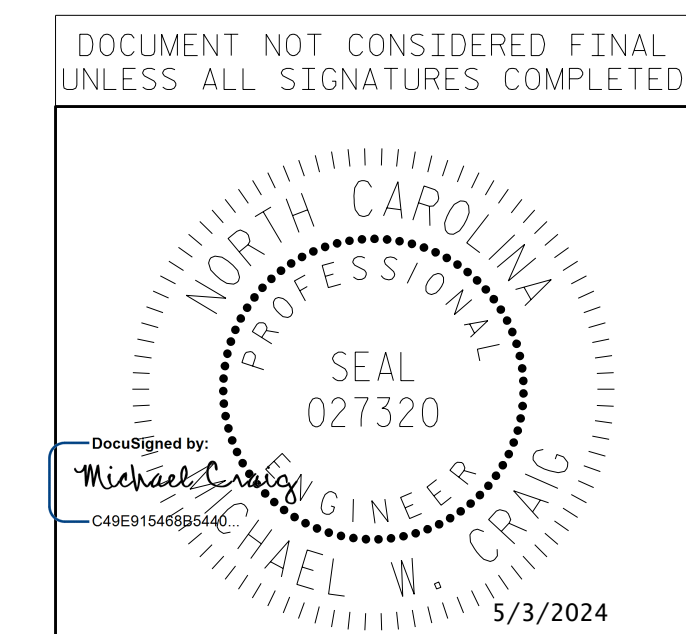
ELEVATION OF COLUMN  
COLUMN REPAIR

\* THE MAXIMUM LENGTH OF ANY ONE REPAIR SEGMENT AT ONE TIME SHALL NOT EXCEED 10 FEET. THE REMAINING REPAIR SEGMENTS SHALL BE ADDRESSED ONLY AFTER REPAIR OF THE FIRST SEGMENT IS COMPLETE.

NOTES:

- TYPICAL BENT CAP REPAIRS ARE SHOWN. REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.
- SOUND CONCRETE TO DETERMINE EXTENT OF REPAIR LOCATIONS. THE METHOD USED TO DELINEATE THE AREAS OF THE UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.
- THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND STANDARD SPECIFICATIONS.
- REMOVE ALL LOOSE OR WEAKENED MATERIAL THEN CLEAN THE AREA OF DIRT, GREASE, OIL AND FOREIGN MATTER.
- REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL. SAW CUT AROUND REPAIR AREA TO A NOMINAL DEPTH OF 1/2".
- REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM 1" BEHIND THE REBAR AND MINIMUM 2" CLEARANCE TO SAW CUT.
- NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CROSS SECTIONAL AREA SHALL BE REMOVED AT ONE TIME. SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.
- SIMULTANEOUSLY REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP OR COLUMN. IF AREAS OF REMOVAL ARE NOT ADJACENT TO OR DIRECTLY OPPOSITE ONE ANOTHER. IF REMOVAL EXTENDS 1/2" BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.
- CLEAN ALL EXPOSED REINFORCING BARS. FOR BARS WITH MORE THAN 10% SECTION LOSS, SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED.
- REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.
- FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.
- COAT ALL SURFACE AREAS ON THE TOP OF THE BENT CAPS. INCLUDING CHAMFER, WITH EPOXY PROTECTIVE COATING.
- FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.
- FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.
- FOR EPOXY COATING, SEE SPECIAL PROVISIONS.
- FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

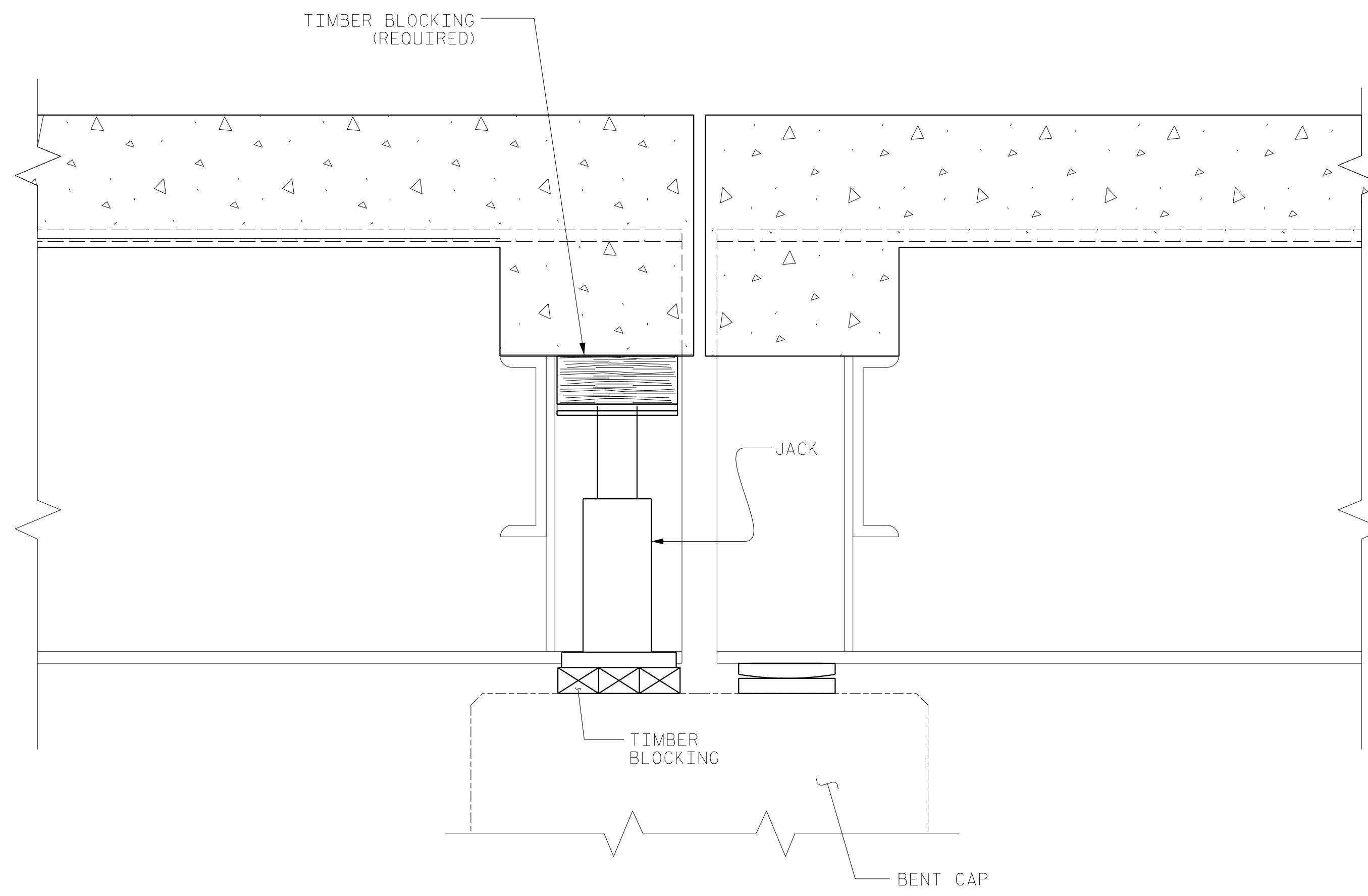


STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

TYPICAL CAP  
 AND COLUMN  
 REPAIR DETAILS

DRAWN BY :	J.MYA	DATE :	3/2019
CHECKED BY :	J.YANNACCONE	DATE :	3/2019
DESIGN ENGINEER OF RECORD :	MICHAEL W. CRAIG	DATE :	4/2024

wsp	WSP USA, Inc. 128 TOLBERT RD. MOORESVILLE, NC 28117 TEL: 1.704.662.0100 LICENSE NO. F-0165	REVISIONS				SHEET NO. S-15 TOTAL SHEETS 16
		NO.	BY:	DATE:	NO.	
1			3			
2			4			



SECTION THRU BENT

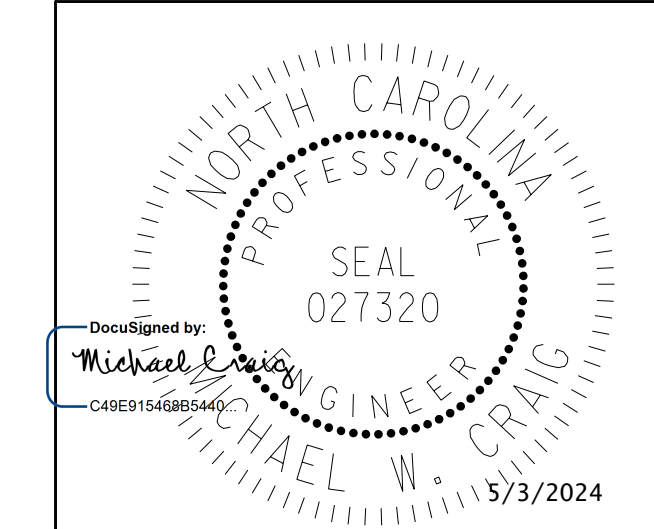
NOTE:  
 SKETCH IS PROVIDED AS AN ILLUSTRATIVE EXAMPLE ONLY.  
 CONTRACTOR SHALL VERIFY EXISTING CONDITIONS, GEOMETRIES,  
 DIMENSIONS, ETC., AND SHALL DEVELOP JACKING PLAN FOR  
 INDIVIDUAL BRIDGES AND BENTS, AS NECESSARY, BASED ON  
 EXISTING CONDITIONS AND REQUIRED AND ANTICIPATED LOADS.  
 CONTRACTOR SHALL SUBMIT JACKING PLAN FOR REVIEW AND  
 SHALL RECEIVE APPROVAL PRIOR TO ORDERING OR FABRICATING  
 JACKING MATERIAL.

NOTES:

- THE CONTRACTOR SHALL SUBMIT PLANS AND CALCULATIONS FOR REVIEW AND APPROVAL PRIOR TO MATERIAL PURCHASE OR FABRICATION OF THE JACKING SYSTEM.
- THE BEAM SHALL BE LIFTED ENOUGH THAT THE BEAM CLEARS THE BEARINGS AND ALL LOAD IS SUPPORTED BY THE JACKS. AFTER JACKING IS COMPLETE THE CONTRACTOR SHALL PROVIDE A METHOD TO SUPPORT THE BEAM FOR DEAD AND LIVE LOADS AND REMOVE THE JACK DURING THE REPAIR OPERATIONS. IF THE JACKS REMAIN IN PLACE DURING THE ENTIRE JACKING AND REPAIR OPERATION, THEY SHALL HAVE MECHANICAL LOCK-OFF CAPABILITIES.
- IF DURING THE JACKING PROCESS OR WHILE THE BEAM IS BEING SUPPORTED THE BEAM SHIFTS FROM ITS ORIGINAL POSITION, ALL WORK SHALL CEASE AND THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.
- PRIOR TO JACKING, THE CONTRACTOR SHALL ENSURE THERE ARE NO OBSTACLES PREVENTING THE BEAM FROM BEING LIFTED.
- ALL ADJACENT BEARINGS OF BEAMS NOT BEING JACKED MAY BE LOOSENED TO DECREASE THE RESISTANCE OF THE DECK SLAB DURING JACKING. ALL BEARINGS LOOSENED SHALL BE TIGHTENED BACK AFTER REPAIR OPERATIONS ARE COMPLETED AND THE JACKS AND BLOCKING HAVE BEEN REMOVED.
- THE MAXIMUM DIFFERENTIAL BETWEEN ADJACENT BEAMS THAT ARE BEING JACKED IS 1/8".
- FOR BRIDGE JACKING DETAILS SEE SPECIAL PROVISIONS.

PROJECT NO. U-6020  
GRANVILLE COUNTY  
 BRIDGE NO. 380045

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

JACKING DETAILS

DRAWN BY : J. MYA DATE : 3/2019  
 CHECKED BY : J.YANNACCONE DATE : 3/2019  
 DESIGN ENGINEER OF RECORD : MICHAEL W. CRAIG DATE : 4/2024

	WSP USA Inc. 128 TOLBERT RD. SUITE A MOORESVILLE, NC 28117 TEL: 1.704.662.0100 LICENSE NO. F-0165		REVISIONS			SHEET NO. S-16
	NO. 1 2	BY:	DATE:	NO. 3 4	BY:	DATE:

## STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS -----	AASHTO (CURRENT)
LIVE LOAD -----	SEE PLANS
IMPACT ALLOWANCE -----	SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 ----	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W ----	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50 ----	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60 -----	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION -----	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR -----	SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS ----	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER -----	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH -----	30 LBS. PER CU. FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1 1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 3/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16" OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINISHES AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.