

REFERENCE: B-5982

PROJECT: 47814

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY HAYWOOD
PROJECT DESCRIPTION REPLACE BRIDGE 430095 ON
US 74 OVER SOUTHERN RAILROAD

SITE DESCRIPTION STA. 20+37.51 -L-

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5982	1	22

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

H. HANCOCK, EI

TRIGON EXPLORATION

E. ESTEP

T. PRESTON

INVESTIGATED BY H. HANCOCK, EI

^{DS}
DCE

DRAWN BY D. BROWN, PE

CHECKED BY H. HANCOCK, EI

SUBMITTED BY D. BROWN, PE

DATE FEBRUARY 2023



STEWART



DocuSigned by:

Donald W. Brown Jr.

03/14/2023

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SIGNATURE

DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 208, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">GENERAL CLASS.</th> <th colspan="7">GRANULAR MATERIALS (≤ 35% PASSING #200)</th> <th colspan="3">SILT-CLAY MATERIALS (> 35% PASSING #200)</th> <th colspan="3">ORGANIC MATERIALS</th> </tr> <tr> <th>A-1</th> <th>A-3</th> <th>A-2</th> <th>A-2-4</th> <th>A-2-5</th> <th>A-2-6</th> <th>A-2-7</th> <th>A-4</th> <th>A-5</th> <th>A-6</th> <th>A-7</th> <th>A-1, A-2</th> <th>A-3</th> <th>A-4, A-5</th> <th>A-6, A-7</th> </tr> </thead> <tbody> <tr> <td>GROUP CLASS.</td> <td>A-1-a</td> <td>A-1-b</td> <td>A-2-4</td> <td>A-2-5</td> <td>A-2-6</td> <td>A-2-7</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>A-1, A-2</td> <td>A-3</td> <td>A-4, A-5</td> <td>A-6, A-7</td> </tr> <tr> <td>SYMBOL</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>% PASSING #10 #40 #200</td> <td>50 MX 30 MX 15 MX</td> <td>50 MX 25 MX</td> <td>51 MN 35 MX</td> <td>35 MX 35 MX</td> <td>35 MX 35 MX</td> <td>35 MX 35 MX</td> <td>36 MN 36 MN</td> <td>36 MN 36 MN</td> <td>36 MN 36 MN</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>MATERIAL PASSING #40 LL PI</td> <td>-</td> <td>-</td> <td>40 MX 10 MX</td> <td>41 MN 10 MX</td> <td>40 MX 11 MN</td> <td>41 MN 11 MN</td> <td>40 MX 10 MX</td> <td>41 MN 11 MN</td> <td>40 MX 11 MN</td> <td>41 MN 11 MN</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>GROUP INDEX</td> <td>0</td> <td>0</td> <td>0</td> <td>4 MX</td> <td>8 MX</td> <td>12 MX</td> <td>16 MX</td> <td>NO MX</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>USUAL TYPES OF MAJOR MATERIALS</td> <td>STONE FRAGS. GRAVEL, AND SAND</td> <td>FINE SAND</td> <td>SILTY OR CLAYEY GRAVEL AND SAND</td> <td>SILTY SOILS</td> <td>CLAYEY SOILS</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>GEN. RATING AS SUBGRADE</td> <td colspan="3">EXCELLENT TO GOOD</td> <td colspan="3">FAIR TO POOR</td> <td>FAIR TO POOR</td> <td>POOR</td> <td>UNSATURABLE</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td colspan="4">PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30</td> </tr> <tr> <td colspan="4" style="text-align: center;">CONSISTENCY OR DENSENESS</td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>PRIMARY SOIL TYPE</th> <th>COMPACTNESS OR CONSISTENCY</th> <th>RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)</th> <th>RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT²)</th> </tr> </thead> <tbody> <tr> <td>GENERALLY GRANULAR MATERIAL (NON-COHESIVE)</td> <td>VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE</td> <td>< 4 4 TO 10 10 TO 30 30 TO 50 > 50</td> <td>N/A</td> </tr> <tr> <td>GENERALLY SILT-CLAY MATERIAL (COHESIVE)</td> <td>VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD</td> <td>< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30</td> <td>< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4</td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">TEXTURE OR GRAIN SIZE</td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>U.S. STD. SIEVE SIZE OPENING (MM)</th> <th>4</th> <th>10</th> <th>40</th> <th>60</th> <th>200</th> <th>270</th> </tr> </thead> <tbody> <tr> <td></td> <td>4.75</td> <td>2.00</td> <td>0.42</td> <td>0.25</td> <td>0.075</td> <td>0.053</td> </tr> <tr> <td>Boulder (BLDR.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Cobble (COB.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Gravel (GR.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Coarse Sand (CSE, SD.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Fine Sand (F SD.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Silt (SL.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Clay (CL.)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">SOIL MOISTURE - CORRELATION OF TERMS</td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>SOIL MOISTURE SCALE (ATTERBERG LIMITS)</th> <th>FIELD MOISTURE DESCRIPTION</th> <th>GUIDE FOR FIELD MOISTURE DESCRIPTION</th> </tr> </thead> <tbody> <tr> <td>LL - LIQUID LIMIT</td> <td>- SATURATED - (SAT.)</td> <td>USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE</td> </tr> <tr> <td>PL - PLASTIC LIMIT</td> <td>- WET - (W)</td> <td>SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE</td> </tr> <tr> <td>OM - OPTIMUM MOISTURE SHRINKAGE LIMIT</td> <td>- MOIST - (M)</td> <td>SOLID; AT OR NEAR OPTIMUM MOISTURE</td> </tr> <tr> <td>SL - SHRINKAGE LIMIT</td> <td>- DRY - (D)</td> <td>REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE</td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">PLASTICITY</td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>NON PLASTIC</th> <th>PLASTICITY INDEX (PI)</th> <th>DRY STRENGTH</th> </tr> </thead> <tbody> <tr> <td></td> <td>0-5</td> <td>VERY LOW</td> </tr> <tr> <td></td> <td>6-15</td> <td>SLIGHT</td> </tr> <tr> <td></td> <td>16-25</td> <td>MEDIUM</td> </tr> <tr> <td></td> <td>26 OR MORE</td> <td>HIGH</td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">COLOR</td> </tr> <tr> <td colspan="4"> <p>DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-BROWN). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">GRADATION</td> </tr> <tr> <td colspan="4"> <p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">ANGULARITY OF GRAINS</td> </tr> <tr> <td colspan="4"> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">MINERALOGICAL COMPOSITION</td> </tr> <tr> <td colspan="4"> <p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">COMPRESSIBILITY</td> </tr> <tr> <td colspan="4"> <p>SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">PERCENTAGE OF MATERIAL</td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> </thead> <tbody> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE 1 - 10%</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE 10 - 20%</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME 20 - 35%</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>> 10%</td> <td>> 20%</td> <td>HIGHLY 35% AND ABOVE</td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">GROUND WATER</td> </tr> <tr> <td colspan="4"> <p> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">MISCELLANEOUS SYMBOLS</td> </tr> <tr> <td colspan="4"> <p> ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY</p> <p> DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION</p> <p> SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD TEST BORING WITH CORE SPT N-VALUE</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">RECOMMENDATION SYMBOLS</td> </tr> <tr> <td colspan="4"> <p> UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">ABBREVIATIONS</td> </tr> <tr> <td colspan="4"> <p>AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED W - UNIT WEIGHT W_g - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">EQUIPMENT USED ON SUBJECT PROJECT</td> </tr> <tr> <td colspan="4"> <p>DRILL UNITS: <input type="checkbox"/> CME-45C <input type="checkbox"/> CME-55 <input checked="" type="checkbox"/> CME-550 <input type="checkbox"/> VANE SHEAR TEST <input type="checkbox"/> PORTABLE HOIST</p> <p>ADVANCING TOOLS: <input type="checkbox"/> CLAY BITS <input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input checked="" type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> TUNG-CARBIDE INSERTS <input checked="" type="checkbox"/> CASING <input checked="" type="checkbox"/> W/ ADVANCER <input type="checkbox"/> TRICONE 2 1/16" STEEL TEETH <input checked="" type="checkbox"/> TRICONE " TUNG-CARB. <input type="checkbox"/> CORE BIT</p> <p>HAMMER TYPE: <input type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL CORE SIZE: <input type="checkbox"/> -B <input type="checkbox"/> -H <input checked="" type="checkbox"/> -N HAND TOOLS: <input type="checkbox"/> POST HOLE DIGGER <input type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">ROCK DESCRIPTION</td> </tr> <tr> <td colspan="4"> <p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p> </td> </tr> <tr> <td colspan="4"> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>WEATHERED ROCK (WR)</th> <th>NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.</th> </tr> </thead> <tbody> <tr> <td></td> <td>FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</td> </tr> <tr> <td></td> <td>FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</td> </tr> <tr> <td></td> <td>COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</td> </tr> </tbody> </table> </td> </tr> <tr> <td colspan="4" style="text-align: center;">WEATHERING</td> </tr> <tr> <td colspan="4"> <p>FRESH - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (IV SLI.) - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF</i> VERY SEVERE (IV SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i> COMPLETE - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</p> </td> </tr> <tr> <td colspan="4" style="text-align: center;">ROCK HARDNESS</td> </tr> <tr> <td colspan="4"> <p>VERY HARD - CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD - CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD - CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. 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FRIABLE - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.</p> </td> </tr> <tr> <td colspan="4" style="text-align: right;">ELEVATION: 2583.10 FEET</td> </tr> <tr> <td colspan="4" style="text-align: center;">NOTES:</td> </tr> <tr> <td colspan="4"> <p>FIAD = FILLED IMMEDIATELY AFTER DRILLING</p> </td> </tr> <tr> <td colspan="4" style="text-align: right;">DATE: 8-15-14</td> </tr> </tbody> </table>				GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)			ORGANIC MATERIALS			A-1	A-3	A-2	A-2-4	A-2-5	A-2-6	A-2-7	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7	GROUP CLASS.	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7						A-1, A-2	A-3	A-4, A-5	A-6, A-7	SYMBOL																% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 35 MX	35 MX 35 MX	35 MX 35 MX	35 MX 35 MX	36 MN 36 MN	36 MN 36 MN	36 MN 36 MN							MATERIAL PASSING #40 LL PI	-	-	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 11 MN	40 MX 11 MN	41 MN 11 MN						GROUP INDEX	0	0	0	4 MX	8 MX	12 MX	16 MX	NO MX								USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND	SILTY SOILS	CLAYEY SOILS											GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD			FAIR TO POOR			FAIR TO POOR	POOR	UNSATURABLE							PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30				CONSISTENCY OR DENSENESS				<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>PRIMARY SOIL TYPE</th> <th>COMPACTNESS OR CONSISTENCY</th> <th>RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)</th> <th>RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT²)</th> </tr> </thead> <tbody> <tr> <td>GENERALLY GRANULAR MATERIAL (NON-COHESIVE)</td> <td>VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE</td> <td>< 4 4 TO 10 10 TO 30 30 TO 50 > 50</td> <td>N/A</td> </tr> <tr> <td>GENERALLY SILT-CLAY MATERIAL (COHESIVE)</td> <td>VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD</td> <td>< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30</td> <td>< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4</td> </tr> </tbody> </table>				PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)	GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A	GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4	TEXTURE OR GRAIN SIZE				<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>U.S. STD. 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SOIL MOISTURE - CORRELATION OF TERMS				<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>SOIL MOISTURE SCALE (ATTERBERG LIMITS)</th> <th>FIELD MOISTURE DESCRIPTION</th> <th>GUIDE FOR FIELD MOISTURE DESCRIPTION</th> </tr> </thead> <tbody> <tr> <td>LL - LIQUID LIMIT</td> <td>- SATURATED - (SAT.)</td> <td>USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE</td> </tr> <tr> <td>PL - PLASTIC LIMIT</td> <td>- WET - (W)</td> <td>SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE</td> </tr> <tr> <td>OM - OPTIMUM MOISTURE SHRINKAGE LIMIT</td> <td>- MOIST - (M)</td> <td>SOLID; AT OR NEAR OPTIMUM MOISTURE</td> </tr> <tr> <td>SL - SHRINKAGE LIMIT</td> <td>- DRY - (D)</td> <td>REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE</td> </tr> </tbody> </table>				SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION	LL - LIQUID LIMIT	- SATURATED - (SAT.)	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MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>				GRADATION				<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>				ANGULARITY OF GRAINS				<p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>				MINERALOGICAL COMPOSITION				<p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. 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SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>				<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>WEATHERED ROCK (WR)</th> <th>NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.</th> </tr> </thead> <tbody> <tr> <td></td> <td>FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</td> </tr> <tr> <td></td> <td>FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</td> </tr> <tr> <td></td> <td>COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. 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ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. 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CAN BE SCRATCHED READILY BY FINGER NAIL.</p>				FRACTURE SPACING		BEDDING		<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>TERM</th> <th>SPACING</th> </tr> </thead> <tbody> <tr> <td>VERY WIDE</td> <td>MORE THAN 10 FEET</td> </tr> <tr> <td>WIDE</td> <td>3 TO 10 FEET</td> </tr> <tr> <td>MODERATELY CLOSE</td> <td>1 TO 3 FEET</td> </tr> <tr> <td>CLOSE</td> <td>0.16 TO 1 FOOT</td> </tr> <tr> <td>VERY CLOSE</td> <td>LESS THAN 0.16 FEET</td> </tr> </tbody> </table>		TERM	SPACING	VERY WIDE	MORE THAN 10 FEET	WIDE	3 TO 10 FEET	MODERATELY CLOSE	1 TO 3 FEET	CLOSE	0.16 TO 1 FOOT	VERY CLOSE	LESS THAN 0.16 FEET	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>TERM</th> <th>THICKNESS</th> </tr> </thead> <tbody> <tr> <td>VERY THICKLY BEDDED</td> <td>4 FEET</td> </tr> <tr> <td>THICKLY BEDDED</td> <td>1.5 - 4 FEET</td> </tr> <tr> <td>THINLY BEDDED</td> <td>0.16 - 1.5 FEET</td> </tr> <tr> <td>VERY THINLY BEDDED</td> <td>0.03 - 0.16 FEET</td> </tr> <tr> <td>THICKLY LAMINATED</td> <td>0.008 - 0.03 FEET</td> </tr> <tr> <td>THINLY LAMINATED</td> <td>< 0.008 FEET</td> </tr> </tbody> </table>		TERM	THICKNESS	VERY THICKLY BEDDED	4 FEET	THICKLY BEDDED	1.5 - 4 FEET	THINLY BEDDED	0.16 - 1.5 FEET	VERY THINLY BEDDED	0.03 - 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET	THINLY LAMINATED	< 0.008 FEET	INDURATION				<p>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. 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GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)			ORGANIC MATERIALS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
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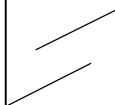
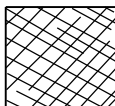


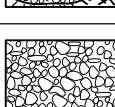
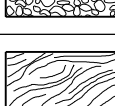
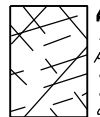


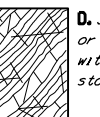
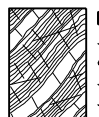


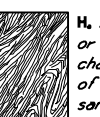
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

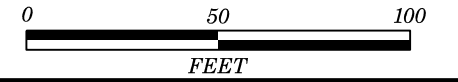
**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
 FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

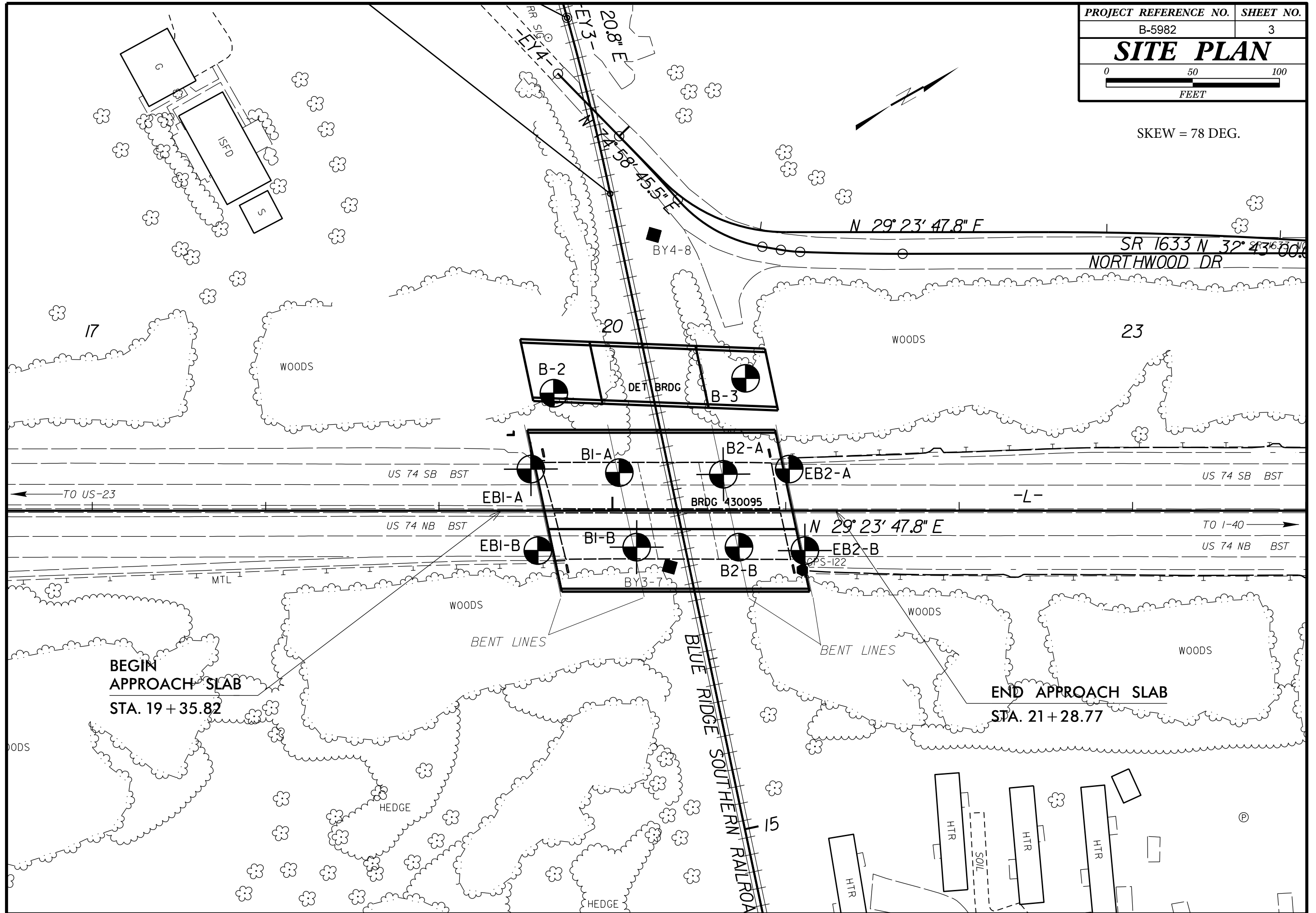
AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

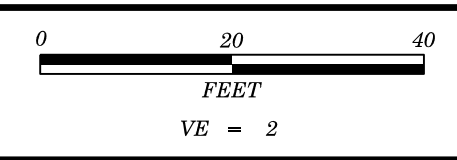
<p>GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)</p> <p>From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.</p> <p>STRUCTURE</p>	<p>SURFACE CONDITIONS</p> <p>VERY GOOD Very rough, fresh unweathered surfaces</p> <p>GOOD Rough, slightly weathered, iron stained surfaces</p> <p>FAIR Smooth, moderately weathered and altered surfaces</p> <p>POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments</p> <p>VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings</p> <p>DECREASING SURFACE QUALITY →</p>					<p>GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)</p> <p>From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.</p> <p>COMPOSITION AND STRUCTURE</p>	<p>SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)</p> <p>VERY GOOD - Very Rough, fresh unweathered surfaces</p> <p>GOOD - Rough, slightly weathered surfaces</p> <p>FAIR - Smooth, moderately weathered and altered surfaces</p> <p>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</p> <p>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</p>				
<p>DECREASING INTERLOCKING OF ROCK PIECES</p> <p>↓</p> <p> INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities</p> <p> BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets</p> <p> VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets</p> <p> BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity</p> <p> DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces</p> <p> LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes</p>	<p>90</p> <p>80</p> <p>70</p> <p>60</p> <p>50</p> <p>40</p> <p>30</p> <p>20</p> <p>10</p>	<p>N/A</p> <p>N/A</p>	<p>N/A</p> <p>N/A</p>	<p>N/A</p> <p>N/A</p>	<p>N/A</p> <p>N/A</p>	<p> A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.</p> <p> B. Sandstone with thin inter-layers of siltstone</p> <p> C. Sandstone and siltstone in similar amounts</p> <p> D. Siltstone or silty shale with sandstone layers</p> <p> E. Weak siltstone or clayey shale with sandstone layers</p> <p>C, D, E, and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.</p> <p> F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure</p> <p> G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers</p> <p> H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.</p> <p>→ Means deformation after tectonic disturbance</p>	<p>70</p> <p>60</p> <p>50</p> <p>40</p> <p>30</p> <p>20</p> <p>10</p>	<p>A</p> <p>B</p> <p>C</p> <p>D</p> <p>E</p>	<p>F</p> <p>G</p> <p>H</p>	<p>10</p>	

SITE PLAN

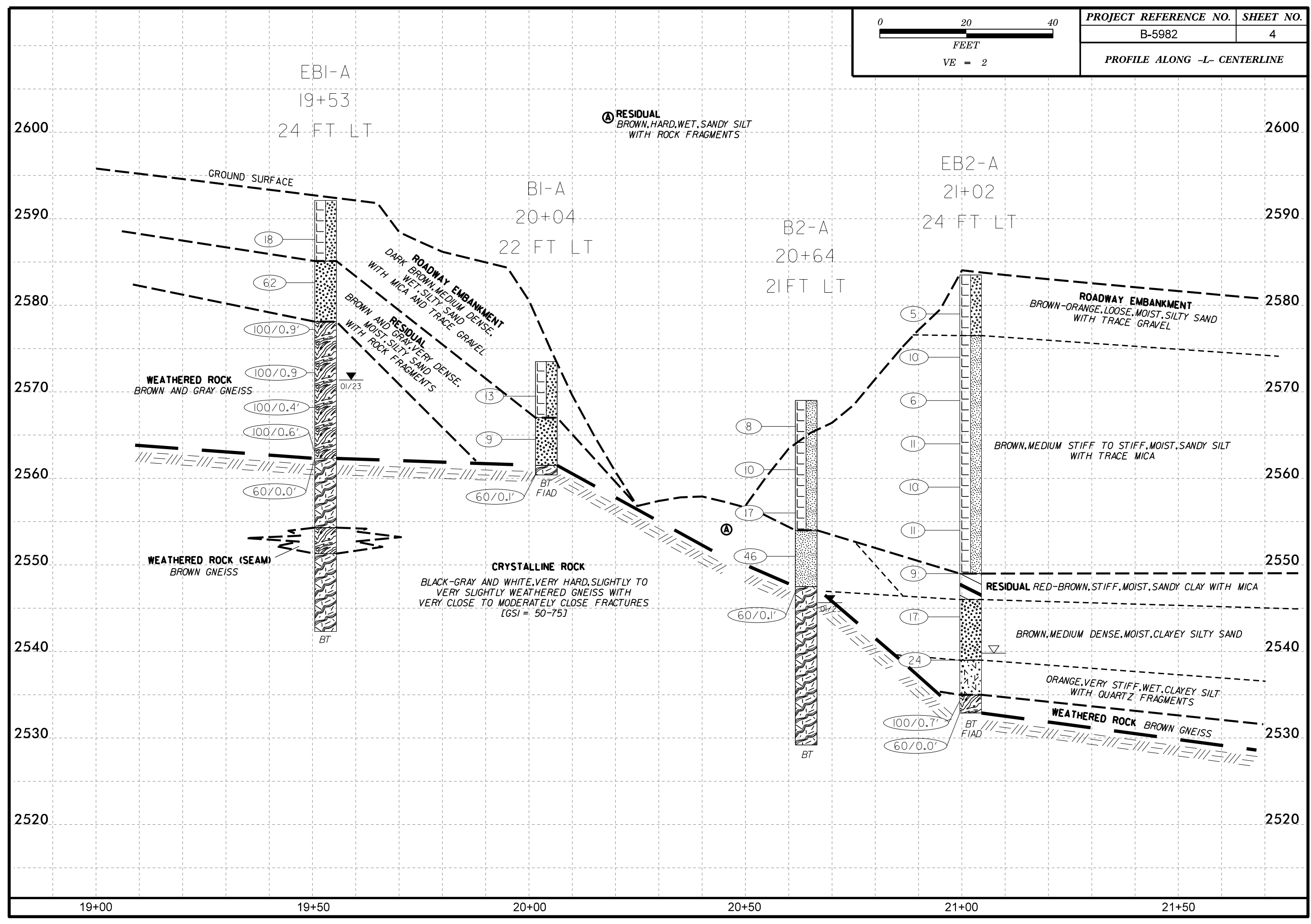


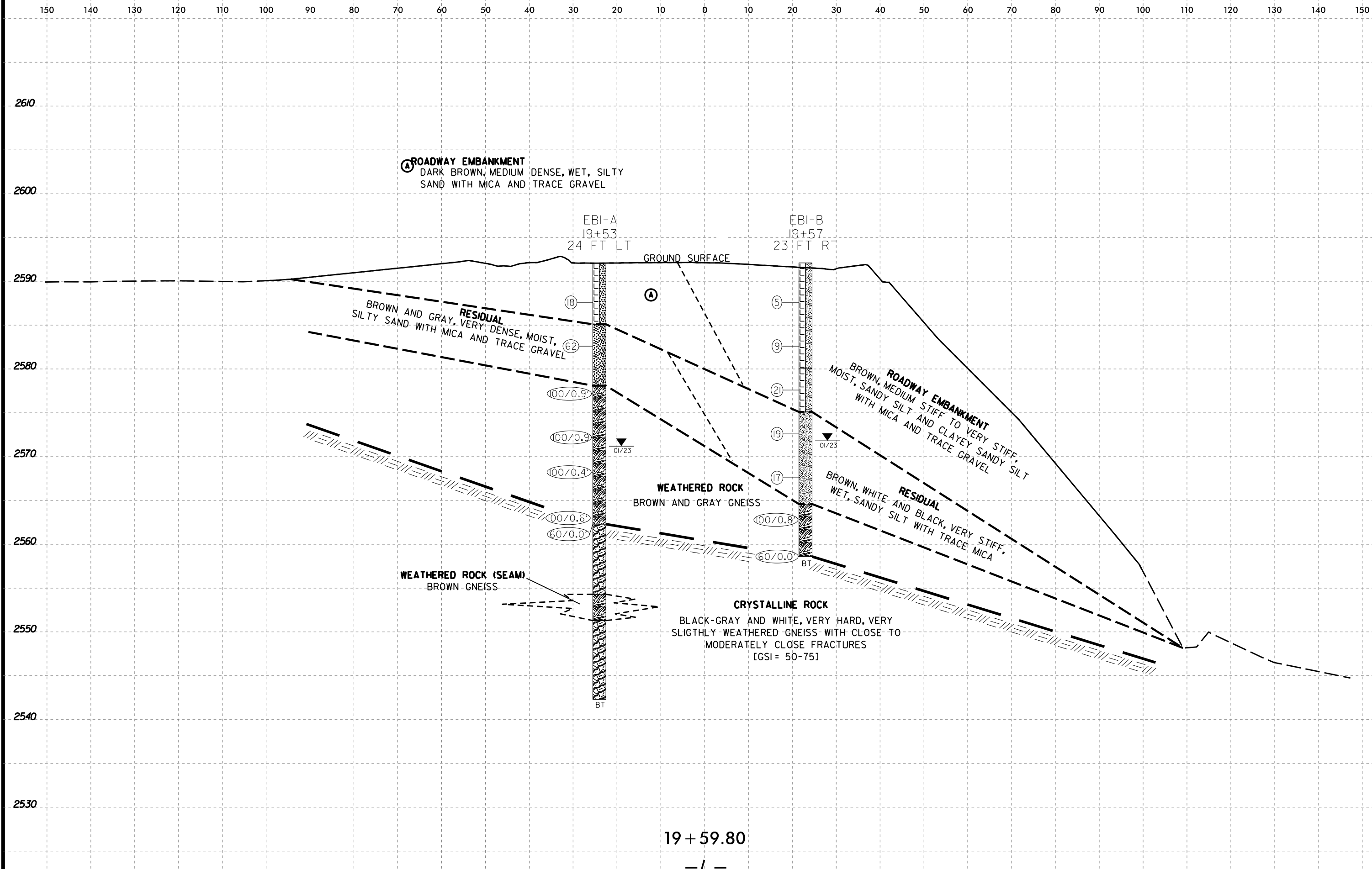
SKEW = 78 DEG.





PROJECT REFERENCE NO.	SHEET NO.
B-5982	4
PROFILE ALONG -L- CENTERLINE	





A ROADWAY EMBANKMENT
DARK BROWN, MEDIUM DENSE, WET, SILTY SAND WITH MICA AND TRACE GRAVEL

EBI-A
19+53
24 FT LT

EBI-B
19+57
23 FT RT

GROUND SURFACE

RESIDUAL
BROWN AND GRAY, VERY DENSE, MOIST, SILTY SAND WITH MICA AND TRACE GRAVEL

ROADWAY EMBANKMENT
BROWN, MEDIUM STIFF TO VERY STIFF, MOIST, SANDY SILT AND CLAYEY SANDY SILT WITH MICA AND TRACE GRAVEL

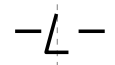
WEATHERED ROCK
BROWN AND GRAY GNEISS

RESIDUAL
BROWN, WHITE AND BLACK, VERY STIFF, WET, SANDY SILT WITH TRACE MICA

WEATHERED ROCK (SEAM)
BROWN GNEISS

CRYSTALLINE ROCK
BLACK-GRAY AND WHITE, VERY HARD, VERY SLIGHTLY WEATHERED GNEISS WITH CLOSE-TO-MODERATELY CLOSE FRACTURES
[GSI = 50-75]

19+59.80

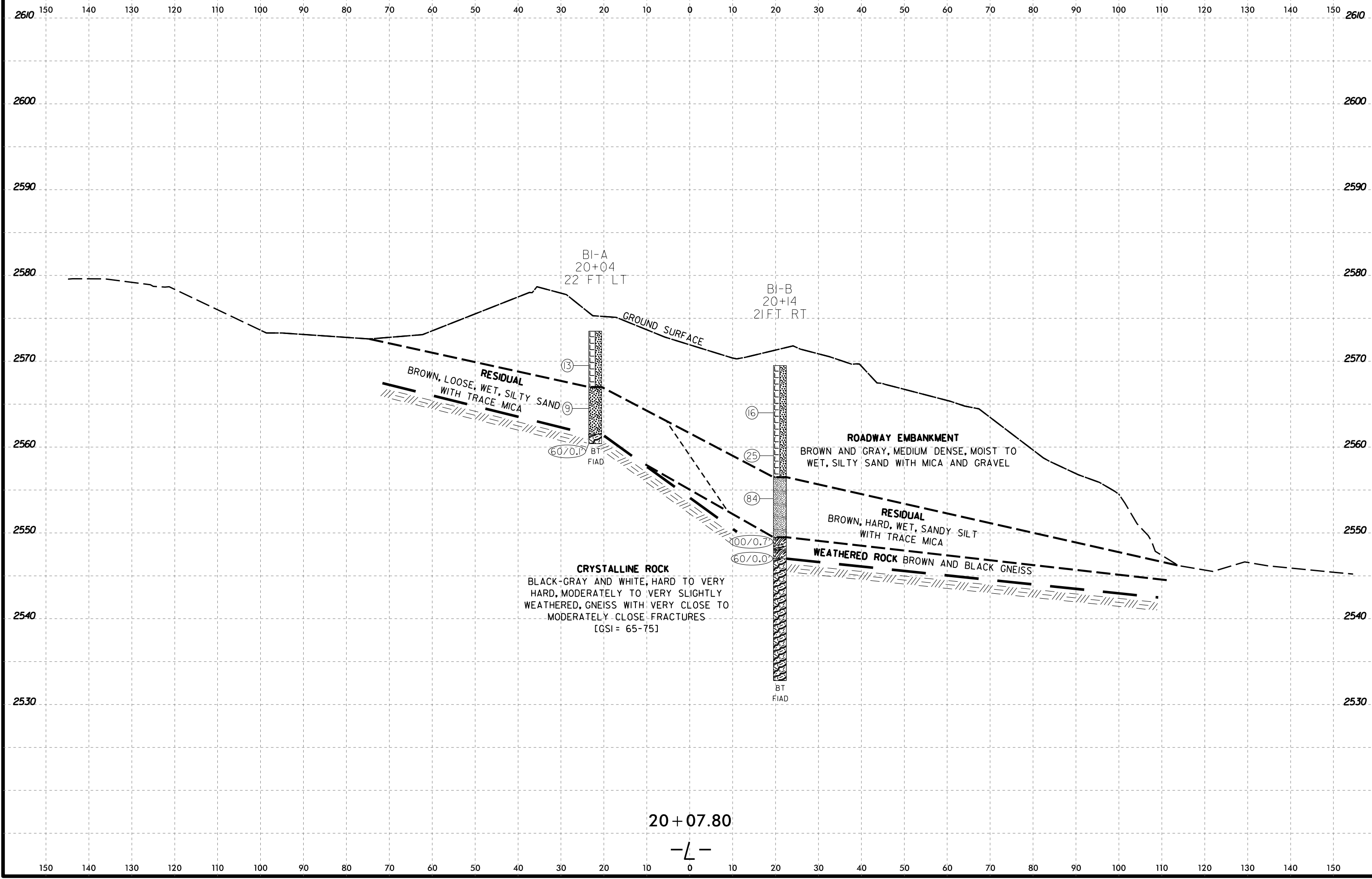


VE = 2



PROJ. REFERENCE NO. B-5982

SHEET NO. 6



BI-A
20+04
22 FT. LT

BI-B
20+14
21 FT. RT

GROUND SURFACE

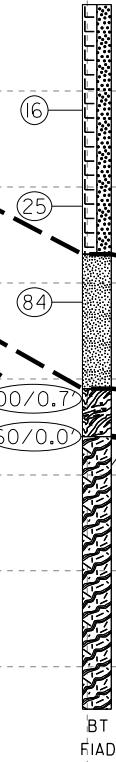
RESIDUAL
BROWN, LOOSE, WET, SILTY SAND
WITH TRACE MICA

ROADWAY EMBANKMENT
BROWN AND GRAY, MEDIUM DENSE, MOIST TO
WET, SILTY SAND WITH MICA AND GRAVEL

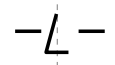
RESIDUAL
BROWN, HARD, WET, SANDY SILT
WITH TRACE MICA

CRYSTALLINE ROCK
BLACK-GRAY AND WHITE, HARD TO VERY
HARD, MODERATELY TO VERY SLIGHTLY
WEATHERED, GNEISS WITH VERY CLOSE TO
MODERATELY CLOSE FRACTURES
[GSI = 65-75]

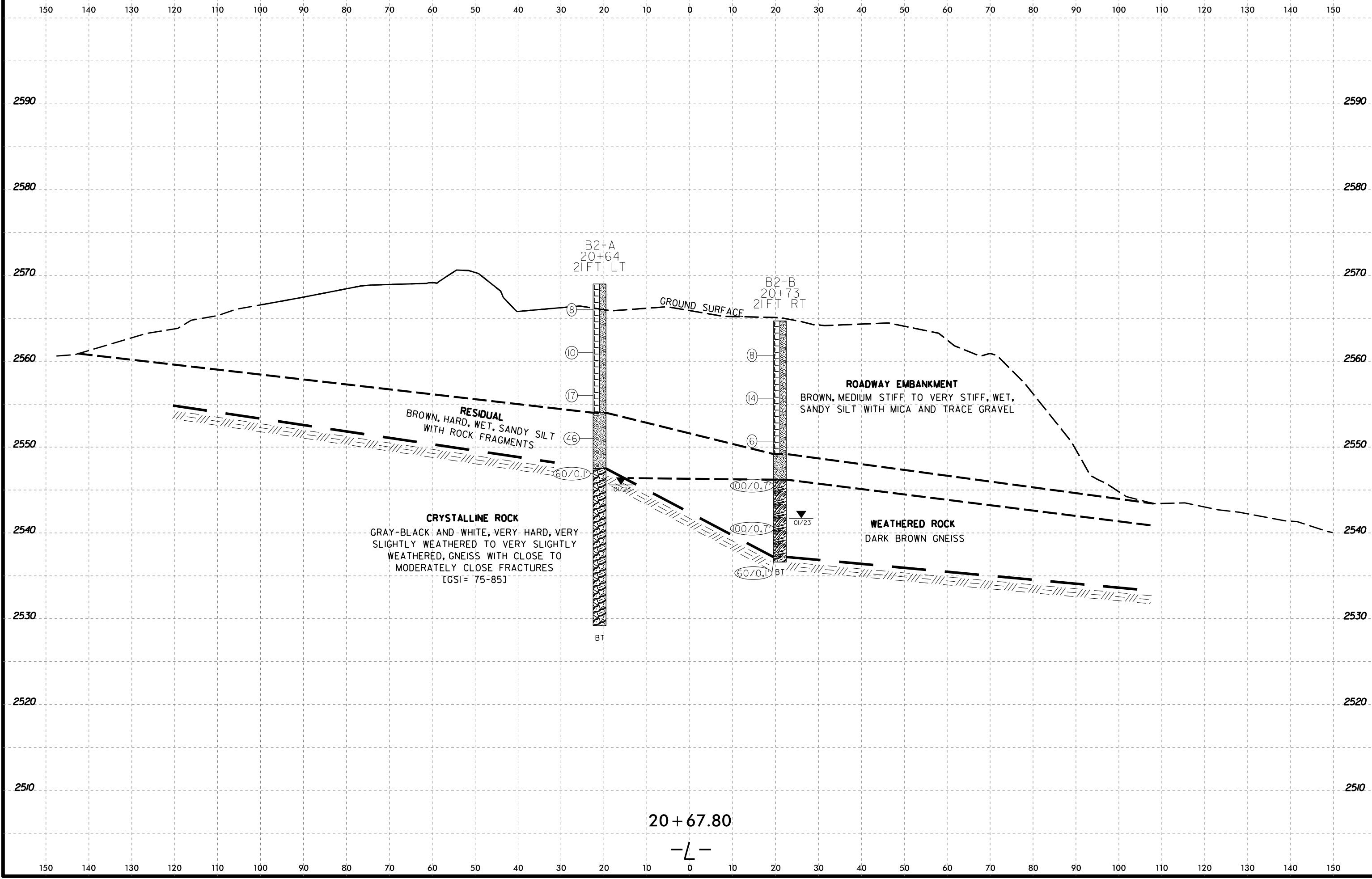
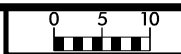
WEATHERED ROCK BROWN AND BLACK GNEISS



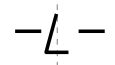
20 + 07.80

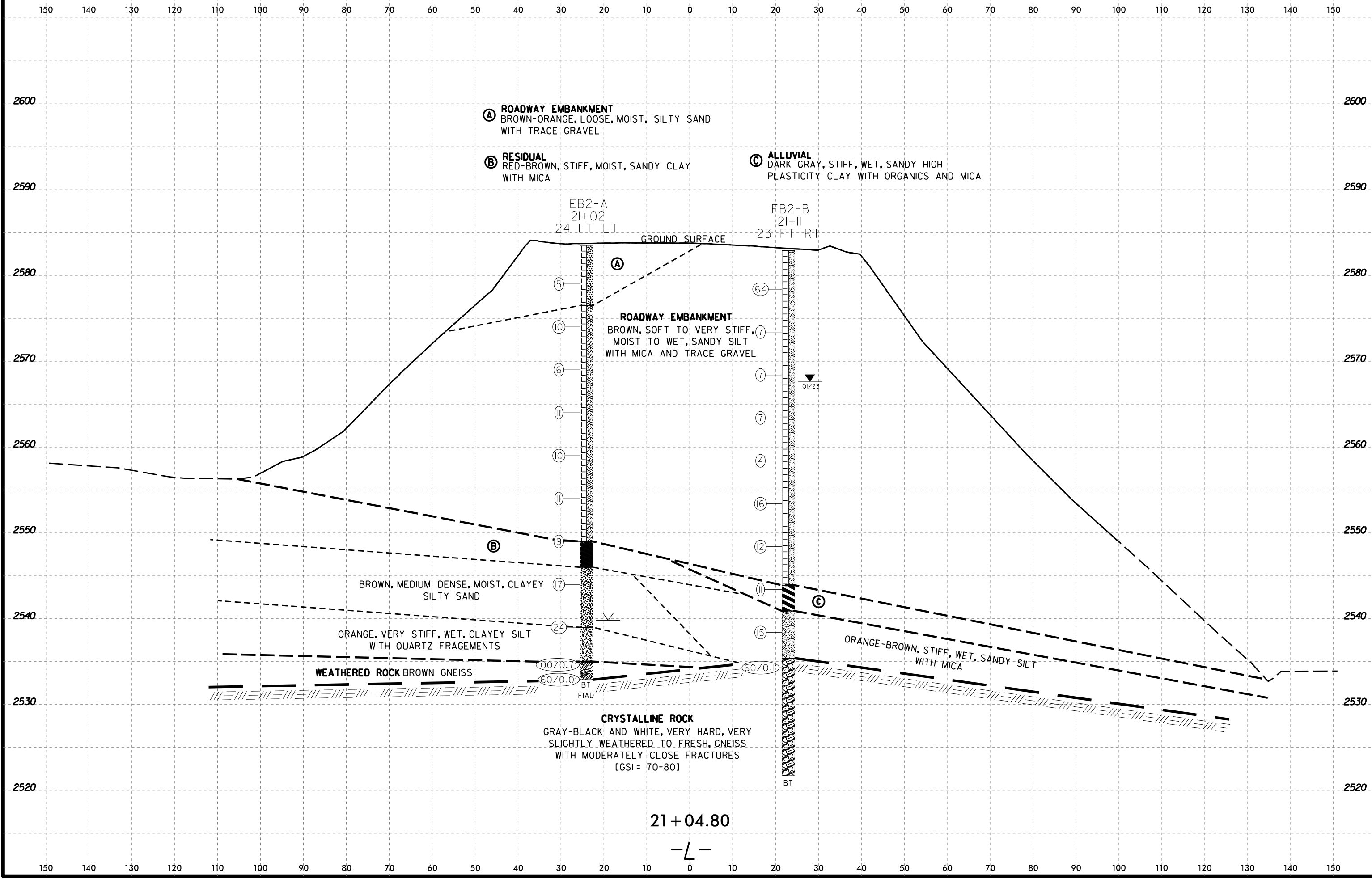


VE = 2



20 + 67.80





(A) ROADWAY EMBANKMENT
BROWN-ORANGE, LOOSE, MOIST, SILTY SAND WITH TRACE GRAVEL

(B) RESIDUAL
RED-BROWN, STIFF, MOIST, SANDY CLAY WITH MICA

(C) ALLUVIAL
DARK GRAY, STIFF, WET, SANDY HIGH PLASTICITY CLAY WITH ORGANICS AND MICA

EB2-A
21+02
24 FT. LT.

EB2-B
21+11
23 FT. RT.

GROUND SURFACE

ROADWAY EMBANKMENT
BROWN, SOFT TO VERY STIFF, MOIST TO WET, SANDY SILT WITH MICA AND TRACE GRAVEL

BROWN, MEDIUM DENSE, MOIST, CLAYEY SILTY SAND

ORANGE, VERY STIFF, WET, CLAYEY SILT WITH QUARTZ FRAGEMENTS

WEATHERED ROCK BROWN GNEISS

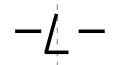
100/0.7
60/0.0
BT
FIAD

CRYSTALLINE ROCK
GRAY-BLACK AND WHITE, VERY HARD, VERY SLIGHTLY WEATHERED TO FRESH, GNEISS WITH MODERATELY CLOSE FRACTURES [GSI = 70-80]

60/0.1
BT

ORANGE-BROWN, STIFF, WET, SANDY SILT WITH MICA

21 + 04.80



GEOTECHNICAL BORING REPORT

BORE LOG

GEOTECHNICAL BORING REPORT

CORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI									
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)								
BORING NO. EB1-A		STATION 19+53		OFFSET 24 ft LT		ALIGNMENT L									
COLLAR ELEV. 2,592.1 ft		TOTAL DEPTH 49.8 ft		NORTHING 671,848		EASTING 833,421									
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic									
DRILLER Estep, J. E.		START DATE 01/17/23		COMP. DATE 01/18/23		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2595															
2590	2,588.6	3.5	10	9	9										
2585	2,583.6	8.5	17	27	35										
2580	2,578.6	13.5	20	40	60/0.4										
2575	2,573.6	18.5	26	46	60/0.4										
2570	2,568.6	23.5	100/0.4												
2565	2,563.6	28.5	71	30/0.1											
2560	2,562.3	29.8	60/0.0												
2555															
2550															
2545															

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI	
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)
BORING NO. EB1-A		STATION 19+53		OFFSET 24 ft LT		ALIGNMENT L	
COLLAR ELEV. 2,592.1 ft		TOTAL DEPTH 49.8 ft		NORTHING 671,848		EASTING 833,421	
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic	
DRILLER Estep, J. E.		START DATE 01/17/23		COMP. DATE 01/18/23		SURFACE WATER DEPTH N/A	
CORE SIZE NQ		TOTAL RUN 20.0 ft		DESCRIPTION AND REMARKS			
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RQD (ft) %	SAMP. NO.
2562.3	2,562.3	29.8	5.0	3:00/1.0 3:40/1.0 3:43/1.0 4:26/1.0 4:43/1.0	(5.0) 100%	(3.0) 59%	
2555	2,557.3	34.8	5.0	3:05/1.0 3:35/1.0 8:25/1.0 1:55/1.0 1:25/1.0	(1.4) 28%	(0.0) 0%	
2550	2,552.3	39.8	5.0	2:14/1.0 6:09/1.0 8:40/1.0 8:03/1.0 6:01/1.0	(4.3) 86%	(3.0) 60%	
2545	2,547.3	44.8	5.0	6:21/1.0 5:10/1.0 5:20/1.0 8:25/1.0 8:55/1.0	(5.0) 100%	(4.3) 86%	RS-1
	2,542.3	49.8					

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ_NC_DOT.GDT 02/08/23

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI									
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)								
BORING NO. EB1-B		STATION 19+57		OFFSET 23 ft RT		ALIGNMENT L									
COLLAR ELEV. 2,592.1 ft		TOTAL DEPTH 33.5 ft		NORTHING 671,828		EASTING 833,464									
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER Estep, J. E.		START DATE 01/30/23		COMP. DATE 01/30/23		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2595															
														2,592.1	0.0
2590	2,588.6	3.5	2	2	3								M	ROADWAY EMBANKMENT BROWN SANDY SILT WITH MICA AND TRACE GRAVEL	
2585	2,583.6	8.5	3	4	5								M		
2580	2,578.6	13.5	9	13	8								M	BROWN AND GRAY, CLAYEY SANDY SILT WITH TRACE MICA AND TRACE GRAVEL	12.0
2575	2,573.6	18.5	6	8	11								W	RESIDUAL BROWN, WHITE, AND BLACK SANDY SILT WITH TRACE MICA	17.0
2570	2,568.6	23.5	6	8	9								W		
2565	2,563.6	28.5	30	70/0.3'										WEATHERED ROCK BROWN AND GRAY GNEISS	27.5
2560	2,558.6	33.5	60/0.0'											Boring Terminated with Standard Penetration Test Refusal at Elevation 2,558.6 ft on Crystalline Rock	33.5

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI									
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)								
BORING NO. B1-A		STATION 20+04		OFFSET 22 ft LT		ALIGNMENT L									
COLLAR ELEV. 2,573.5 ft		TOTAL DEPTH 13.1 ft		NORTHING 671,892		EASTING 833,448									
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD Wash Boring		HAMMER TYPE Automatic									
DRILLER Estep, J. E.		START DATE 01/16/23		COMP. DATE 01/16/23		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2575															
														2,573.5	0.0
2570	2,570.5	3.0	7	6	7								M	ROADWAY EMBANKMENT BROWN, SILTY SAND WITH TRACE MICA	
2565	2,565.5	8.0	4	4	5								W	RESIDUAL BROWN, SILTY SAND WITH TRACE MICA	6.5
	2,560.5	13.0	60/0.1'											CRYSTALLINE ROCK GNEISS	12.0
														Boring Terminated with Standard Penetration Test Refusal at Elevation 2,560.4 ft in Crystalline Rock	13.1

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ NC_DOT.GDT 02/08/23

GEOTECHNICAL BORING REPORT BORE LOG

GEOTECHNICAL BORING REPORT CORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI										
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)									
BORING NO. B1-B		STATION 20+14		OFFSET 21 ft RT		ALIGNMENT L										
COLLAR ELEV. 2,569.5 ft		TOTAL DEPTH 36.7 ft		NORTHING 671,879		EASTING 833,491										
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022			DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic											
DRILLER Estep, J. E.		START DATE 01/31/23		COMP. DATE 01/31/23		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
2570														2,569.5	0.0	GROUND SURFACE
																ROADWAY EMBANKMENT DARK BROWN AND GRAY SILTY SAND WITH GRAVEL AND TRACE MICA
2565	2,565.0	4.5	7	8	8											
2560	2,560.0	9.5	9	14	11											
2555	2,555.0	14.5	14	14	70											RESIDUAL BROWN SANDY SILT WITH TRACE MICA
2550	2,550.0	19.5	30	19	81/0.2'											WEATHERED ROCK BROWN AND BLACK GNEISS
2545	2,547.0	22.5	60/0'													CRYSTALLINE ROCK BLACK-GRAY, AND WHITE, HARD TO VERY HARD, MODERATELY TO VERY SLIGHTLY WEATHERED GNEISS WITH VERY CLOSE TO MODERATELY CLOSE FRACTURES [GSI = 65-75]
2540																
2535																Boring Terminated at Elevation 2,532.8 ft in Crystalline Rock

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI					
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)				
BORING NO. B1-B		STATION 20+14		OFFSET 21 ft RT		ALIGNMENT L					
COLLAR ELEV. 2,569.5 ft		TOTAL DEPTH 36.7 ft		NORTHING 671,879		EASTING 833,491					
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022			DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic						
DRILLER Estep, J. E.		START DATE 01/31/23		COMP. DATE 01/31/23		SURFACE WATER DEPTH N/A					
CORE SIZE N/A		TOTAL RUN 14.2 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	RQD (ft) %	LOG	DESCRIPTION AND REMARKS
2547	2,547.0	22.5	4.2	1:11/0.2 5:10/1.0 2:55/1.0 4:17/1.0 6:02/1.0	(3.7) 88%	(2.1) 50%		(13.5) 95%	(8.8) 62%		Begin Coring @ 22.5 ft CRYSTALLINE ROCK
2545	2,542.8	26.7									2,547.0 BLACK-GRAY, AND WHITE, HARD TO VERY HARD, MODERATELY TO VERY SLIGHTLY WEATHERED GNEISS WITH VERY CLOSE TO MODERATELY CLOSE FRACTURES [GSI = 65-75]
2540			5.0	5:20/1.0 4:27/1.0 6:50/1.0 6:49/1.0 9:40/1.0	(5.0) 100%	(2.9) 58%					
2535	2,537.8	31.7					RS-3				
			5.0	6:14/1.0 6:47/1.0 7:38/1.0 7:12/1.0 6:43/1.0	(4.8) 96%	(3.8) 76%					
	2,532.8	36.7									Boring Terminated at Elevation 2,532.8 ft in Crystalline Rock

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ_NC_DOT.GDT 02/08/23

GEOTECHNICAL BORING REPORT

BORE LOG

GEOTECHNICAL BORING REPORT

CORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI											
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)										
BORING NO. B2-A		STATION 20+64		OFFSET 21 ft LT		ALIGNMENT L											
COLLAR ELEV. 2,569.0 ft		TOTAL DEPTH 39.8 ft		NORTHING 671,944		EASTING 833,478											
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022		DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic													
DRILLER Estep, J. E.		START DATE 01/18/23		COMP. DATE 01/19/23		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
2570														2,569.0	GROUND SURFACE	0.0	
2565	2,567.0	2.0	3	3	5								W		ROADWAY EMBANKMENT BROWN, SANDY SILT WITH MICA AND TRACE GRAVEL		
2560	2,562.0	7.0	4	4	6								W				
2555	2,557.0	12.0	5	9	8								W				
2550	2,552.0	17.0	7	17	29								W		RESIDUAL BROWN, SANDY SILT WITH ROCK FRAGMENTS AT ~18 FEET	15.0	
2545	2,547.0	22.0	60/0.1										W		CRYSTALLINE ROCK GRAY-BLACK AND WHITE, VERY HARD, SLIGHTLY TO VERY SLIGHTLY WEATHERED GNEISS WITH CLOSE TO MODERATELY CLOSE FRACTURES [GSI = 75-85]	21.5	
2540																	
2535																	
2530																	
Boring Terminated at Elevation 2,529.2 ft in Crystalline Rock																	

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI			
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)		
BORING NO. B2-A		STATION 20+64		OFFSET 21 ft LT		ALIGNMENT L			
COLLAR ELEV. 2,569.0 ft		TOTAL DEPTH 39.8 ft		NORTHING 671,944		EASTING 833,478			
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022		DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic					
DRILLER Estep, J. E.		START DATE 01/18/23		COMP. DATE 01/19/23		SURFACE WATER DEPTH N/A			
CORE SIZE NQ			TOTAL RUN 17.7 ft					LOG	DESCRIPTION AND REMARKS
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (%)	RQD (%)	SAMP. NO.		
2546.9	2,546.9	22.1	2.7	5:14/0.7 5:30/1.0 6:04/1.0	(2.7) 100%	(2.7) 100%			
2545	2,544.2	24.8	5.0	6:17/1.0 4:40/1.0 3:55/1.0 4:30/1.0 6:02/1.0	(5.0) 100%	(2.7) 54%			
2540	2,539.2	29.8	5.0	4:39/1.0 4:53/1.0 5:48/1.0	(5.0) 100%	(5.0) 100%			
2535	2,534.2	34.8	5.0	5:23/1.0 5:35/1.0 5:25/1.0 5:39/1.0 5:45/1.0	(5.0) 100%	(5.0) 100%	RS-2		
2530	2,529.2	39.8							
Boring Terminated at Elevation 2,529.2 ft in Crystalline Rock									

NCDOT BORE DOUBLE_B5982_GEO_BRDG0095_BH.GPJ_NC_DOT.GDT_02/08/23

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI										
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)									
BORING NO. B2-B		STATION 20+73		OFFSET 21 ft RT		ALIGNMENT L										
COLLAR ELEV. 2,564.7 ft		TOTAL DEPTH 28.1 ft		NORTHING 671,931		EASTING 833,519										
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD Mud Rotary		HAMMER TYPE Automatic										
DRILLER Estep, J. E.		START DATE 01/20/23		COMP. DATE 01/20/23		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
2565														2,564.7	GROUND SURFACE	0.0
	2,561.7	3.0	3	4	4								W		ROADWAY EMBANKMENT BROWN, SANDY SILT WITH MICA	
2560													W			
	2,556.7	8.0	5	6	8								W			
2555																
	2,551.7	13.0	3	3	3								W			
2550																
	2,546.7	18.0	30	64	36/0.2'									2,549.2	RESIDUAL DARK BROWN, SANDY SILT	15.5
2545														2,546.2	WEATHERED ROCK DARK BROWN, GNEISS	18.5
	2,541.7	23.0	36	60	40/0.2'											
2540																
	2,536.7	28.0	60/0.1'											2,537.2	CRYSTALLINE ROCK GNEISS	27.5
														2,536.6	Boring Terminated with Standard Penetration Test Refusal at Elevation 2,536.6 ft in Crystalline Rock	

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI										
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)									
BORING NO. EB2-A		STATION 21+02		OFFSET 24 ft LT		ALIGNMENT L										
COLLAR ELEV. 2,583.5 ft		TOTAL DEPTH 50.6 ft		NORTHING 671,978		EASTING 833,495										
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic										
DRILLER Estep, J. E.		START DATE 01/16/23		COMP. DATE 01/16/23		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
2585														2,583.5	GROUND SURFACE	0.0
	2,580.0	3.5	3	3	2								M		ROADWAY EMBANKMENT BROWN-ORANGE, SILTY SAND WITH TRACE GRAVEL	
2580																
	2,575.0	8.5	5	5	5								M		BROWN, SANDY SILT WITH TRACE GRAVEL AND TRACE MICA	7.0
2575																
	2,570.0	13.5	3	3	3								M			
2570																
	2,565.0	18.5	3	5	6								M			
2565																
	2,560.0	23.5	3	5	5								M			
2560																
	2,555.0	28.5	3	4	7								M			
2555																
	2,550.0	33.5	3	3	6								M			
2550																
	2,545.0	38.5	5	7	10								M		RESIDUAL RED-BROWN, SANDY CLAY WITH MICA	34.5
2545														2,549.0		
	2,540.0	43.5	7	7	17								M		BROWN, CLAYEY SILTY SAND	37.5
2540																
	2,539.0	44.5											W		ORANGE, CLAYEY SILT WITH QUARTZ FRAGMENTS	44.5
2535																
	2,535.0	48.5	21	79/0.2'												
	2,532.9	50.6	60/0.0'											2,532.9	WEATHERED ROCK BROWN GNEISS	50.6
															Boring Terminated with Standard Penetration Test Refusal at Elevation 2,532.9 ft on Crystalline Rock	

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ NC_DOT.GDT 02/13/23

GEOTECHNICAL BORING REPORT

BORE LOG

GEOTECHNICAL BORING REPORT

CORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI										
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)									
BORING NO. EB2-B		STATION 21+11		OFFSET 23 ft RT		ALIGNMENT L										
COLLAR ELEV. 2,582.9 ft		TOTAL DEPTH 61.2 ft		NORTHING 671,963		EASTING 833,539										
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022			DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic											
DRILLER Estep, J. E.		START DATE 01/20/23		COMP. DATE 01/30/23		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
2585														2,582.9	GROUND SURFACE	0.0
2580	2,579.4	3.5	2	54	10								W		ROADWAY EMBANKMENT BROWN SANDY SILT WITH MICA	
2575	2,574.4	8.5	3	3	4								W			
2570	2,569.4	13.5	2	4	3								W			
2565	2,564.4	18.5	2	3	4								W			
2560	2,559.4	23.5	2	2	2								W			
2555	2,554.4	28.5	5	7	9								W			
2550	2,549.4	33.5	4	5	7								W			
2545	2,544.4	38.5	4	4	7								W			
2540	2,539.4	43.5	4	6	9								W			
2535	2,534.4	48.5	60/0.1										W			
2530																
2525													RS-4			
														2,543.9	ALLUVIAL DARK GRAY SANDY HIGH PLASTICITY CLAY WITH ORGANICS AND MICA	39.0
														2,540.9	RESIDUAL ORANGE-BROWN SANDY SILT WITH TRACE MICA	42.0
														2,535.4	CRYSTALLINE ROCK GRAY-BLACK, AND WHITE, VERY HARD, VERY SLIGHTLY WEATHERED TO FRESH GNEISS WITH MODERATELY CLOSE FRACTURES [GSI = 70-80]	47.5
														2,521.7		61.2
Boring Terminated at Elevation 2,521.7 ft in Crystalline Rock																
Caved at 15.5 ft at 24 hr. Water depth not true GW (likely drill mud).																

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST H. HANCOCK, EI					
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US 74 OVER SOUTHERN RAILROAD (STA. 20+37.51 -L-)							GROUND WTR (ft)				
BORING NO. EB2-B		STATION 21+11		OFFSET 23 ft RT		ALIGNMENT L					
COLLAR ELEV. 2,582.9 ft		TOTAL DEPTH 61.2 ft		NORTHING 671,963		EASTING 833,539					
DRILL RIG/HAMMER EFF./DATE TRI9435 CME-55 87% 05/09/2022			DRILL METHOD NW Casing w/ Advancer		HAMMER TYPE Automatic						
DRILLER Estep, J. E.		START DATE 01/20/23		COMP. DATE 01/30/23		SURFACE WATER DEPTH N/A					
CORE SIZE N/A		TOTAL RUN 12.6 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	RQD (ft) %	LOG	DESCRIPTION AND REMARKS
2534.3	2,534.3	48.6	2.6	3:00/0.6 9:50/1.0 11:00/1.0	(2.6) 100%	(2.6) 100%					Begin Coring @ 48.6 ft
2530	2,531.7	51.2	5.0	8:31/1.0 6:53/1.0 6:55/1.0 9:26/1.0 11:35/1.0	(4.6) 92%	(4.5) 90%					GRAY-BLACK, AND WHITE, VERY HARD, VERY SLIGHTLY WEATHERED TO FRESH GNEISS WITH MODERATELY CLOSE FRACTURES [GSI = 70-80]
2525	2,526.7	56.2	5.0	15:45/1.0 9:15/1.0 8:10/1.0 8:15/1.0 7:31/1.0	(5.0) 100%	(5.0) 100%	RS-4				
	2,521.7	61.2									Boring Terminated at Elevation 2,521.7 ft in Crystalline Rock Caved at 15.5 ft at 24 hr. Water depth not true GW (likely drill mud).

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ NC_DOT_GDT 02/08/23

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST Johnson, C. D.											
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US-74 OVER SOUTHER RAILROAD							GROUND WTR (ft)										
BORING NO. B-2		STATION 17+70		OFFSET 12 ft RT		ALIGNMENT -L DET-											
COLLAR ELEV. 2,587.0 ft		TOTAL DEPTH 23.6 ft		NORTHING 671,881		EASTING 833,389											
DRILL RIG/HAMMER EFF./DATE AFO6744 CME - 45C 96% 04/08/2019			DRILL METHOD H.S. Augers		HAMMER TYPE Automatic												
DRILLER Coffey, Jr., C.		START DATE 05/02/22		COMP. DATE 05/02/55		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	LOG G	SOIL AND ROCK DESCRIPTION	ELEV. (ft)	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
2590																	
															2,587.0	0.0	GROUND SURFACE
2585	2,584.1	2.9	2	7	8								M				SAPROLITE BROWN, SL MICACEOUS, SANDY-SILT w/ CLAY; TR MnO SEAMS and WR FRAGS T/O
2580	2,579.1	7.9	2	4	5								M				
2575	2,574.1	12.9	1	4	6								M				
2570	2,569.1	17.9	1	20	16								M				
2565	2,565.4	21.6	100/0.4												2,565.8	21.2	WEATHERED ROCK
	2,564.1	22.9													2,563.4	23.6	HIGHLY WEATHERED BIOTITE GNEISS w/QUARTZ STRINGERS
	2,563.4	23.6	16	84/0.2													CRYSTALLINE ROCK BIOTITE GNEISS Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,563.4 ft ON CRYSTALLINE ROCK

WBS 47814.1.1		TIP B-5982		COUNTY HAYWOOD		GEOLOGIST Johnson, C. D.											
SITE DESCRIPTION REPLACE BRIDGE 430095 ON US-74 OVER SOUTHER RAILROAD							GROUND WTR (ft)										
BORING NO. B-3		STATION 18+81		OFFSET CL		ALIGNMENT -L DET-											
COLLAR ELEV. 2,561.8 ft		TOTAL DEPTH 20.9 ft		NORTHING 671,982		EASTING 833,436											
DRILL RIG/HAMMER EFF./DATE AFO6744 CME - 45C 96% 04/08/2019			DRILL METHOD H.S. Augers		HAMMER TYPE Automatic												
DRILLER Coffey, Jr., C.		START DATE 04/28/22		COMP. DATE 04/28/22		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	LOG G	SOIL AND ROCK DESCRIPTION	ELEV. (ft)	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
2565																	
															2,561.8	0.0	GROUND SURFACE
2560	2,558.2	3.6	6	8	9								M				SAPROLITE BROWN-BLACK, SL MICACEOUS, COARSE SANDY-SILT w/CLAY; WR FRAGS T/O
2555	2,553.2	8.6	36	43	57/0.3								M				
2550	2,548.2	13.6	6	6	3								M		2,551.6	10.2	WEATHERED ROCK WEA BIOTITE GNEISS w/QUARTZ STRINGER
2545	2,543.2	18.6	WOH	43	57/0.1										2,542.1	19.7	WEATHERED ROCK WEA BIOTITE GNEISS
	2,540.9	20.9	60/0.0												2,540.9	20.9	CRYSTALLINE ROCK BIOTITE GNEISS Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,540.9 ft ON CRYSTALLINE ROCK

NCDOT BORE DOUBLE B5982_GEO_BRDG0095_BH.GPJ NC_DOT_GDT 02/09/23

ROCK TEST RESULTS



Rock Core Compressive Strength (ASTM D7012)

Proj Number: F22038.00 Proj Name: B-5982 BRDG 430095 (Haywood) Report Date: 01/25/2023

Sample No.: RS-1 Location: Boring EB1-A Depth (ft): 46.3

Test Specimen Weight (lb): 1.21 Calc. Unit Weight (lb/CF): 172.7

Core Diameter, D (in)			Core Length, L (in)			
#1	#2	Average	#1	#2	#3	Average
1.982	1.982	1.982	3.920	3.920	3.930	3.923

Compressive Strength			
L/D Ratio	Cross-Sectional Area (in ²)	Applied Load (lbf)	Compressive Strength (psi)
1.979	3.09	5,807	1,880

Test Method: C

Comments:

PHOTOGRAPHS



Tested by: J. Evans



Rock Core Compressive Strength (ASTM D7012)

Proj Number: F22038.00 Proj Name: B-5982 BRDG 430095 (Haywood) Report Date: 02/07/2023

Sample No.: RS-3 Location: Boring B1-B Depth (ft): 32.0

Test Specimen Weight (lb): 1.21 Calc. Unit Weight (lb/CF): 174.1

Core Diameter, D (in)			Core Length, L (in)			
#1	#2	Average	#1	#2	#3	Average
1.971	1.972	1.972	3.934	3.927	3.938	3.933

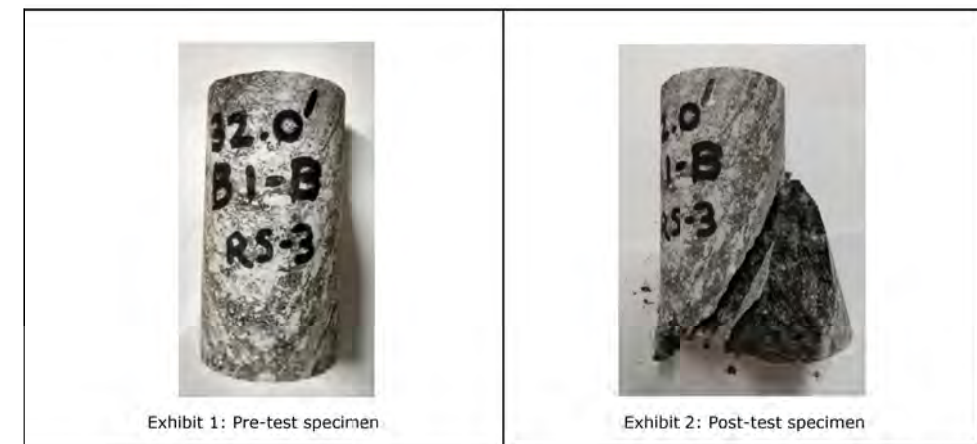
Compressive Strength			
L/D Ratio	Cross-Sectional Area (in ²)	Applied Load (lbf)	Compressive Strength (psi)
1.995	3.05	8,815	2,890

Test Method: C

Comments:

Specimen broke along seam

PHOTOGRAPHS



Tested by: J. Evans

ROCK TEST RESULTS



Rock Core Compressive Strength (ASTM D7012)

Proj Number: F22038.00 Proj Name: B-5982 BRDG 430095 (Haywood) Report Date: 01/25/2023

Sample No.: RS-2 Location: Boring B2-A Depth (ft): 34.3

Test Specimen Weight (lb): 1.21 Calc. Unit Weight (lb/CF): 172.1

Core Diameter, D (in)		
#1	#2	Average
1.982	1.984	1.983

Core Length, L (in)			
#1	#2	#3	Average
3.930	3.930	3.940	3.933

Compressive Strength			
L/D Ratio	Cross-Sectional Area (in ²)	Applied Load (lbf)	Compressive Strength (psi)
1.984	3.09	20,278	6,560

Test Method: C

Comments:

PHOTOGRAPHS



Tested by: J. Evans



Rock Core Compressive Strength (ASTM D7012)

Proj Number: F22038.00 Proj Name: B-5982 BRDG 430095 (Haywood) Report Date: 02/07/2023

Sample No.: RS-4 Location: Boring EB2-B Depth (ft): 57.3

Test Specimen Weight (lb): 1.19 Calc. Unit Weight (lb/CF): 171.2

Core Diameter, D (in)		
#1	#2	Average
1.956	1.957	1.957

Core Length, L (in)			
#1	#2	#3	Average
3.994	3.994	4.000	3.996

Compressive Strength			
L/D Ratio	Cross-Sectional Area (in ²)	Applied Load (lbf)	Compressive Strength (psi)
2.042	3.01	24,567	8,160

Test Method: C

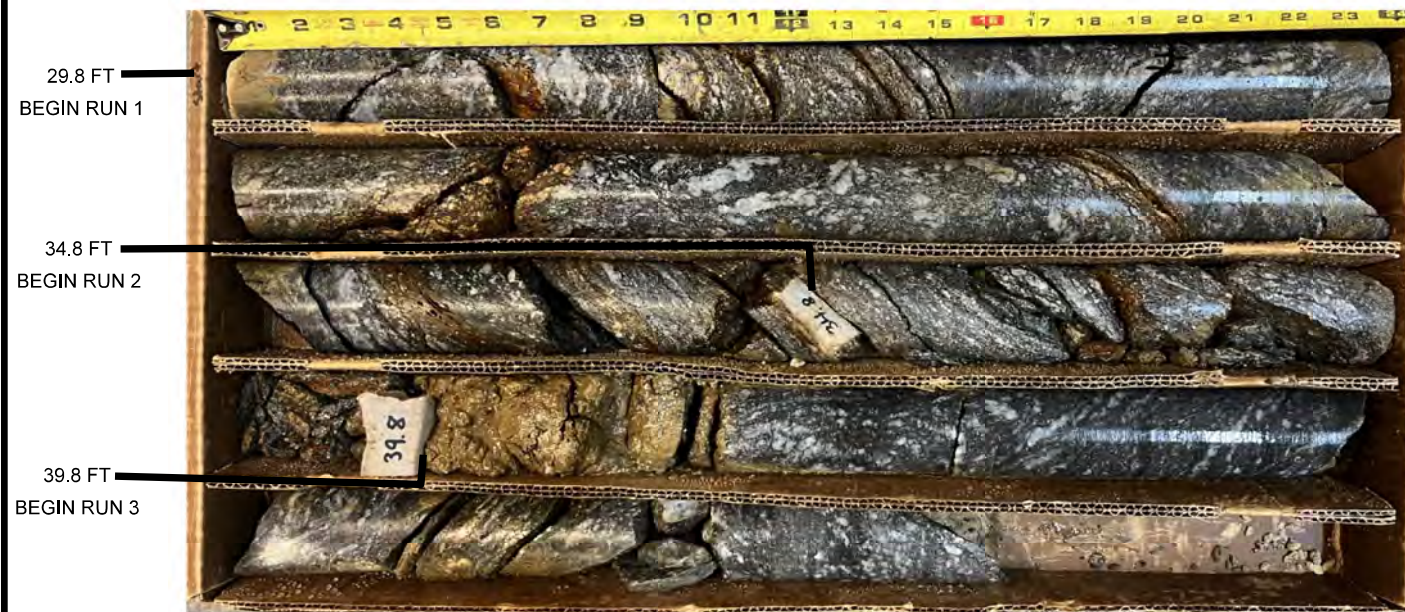
Comments:

PHOTOGRAPHS



Tested by: J. Evans

CORE PHOTOGRAPHS



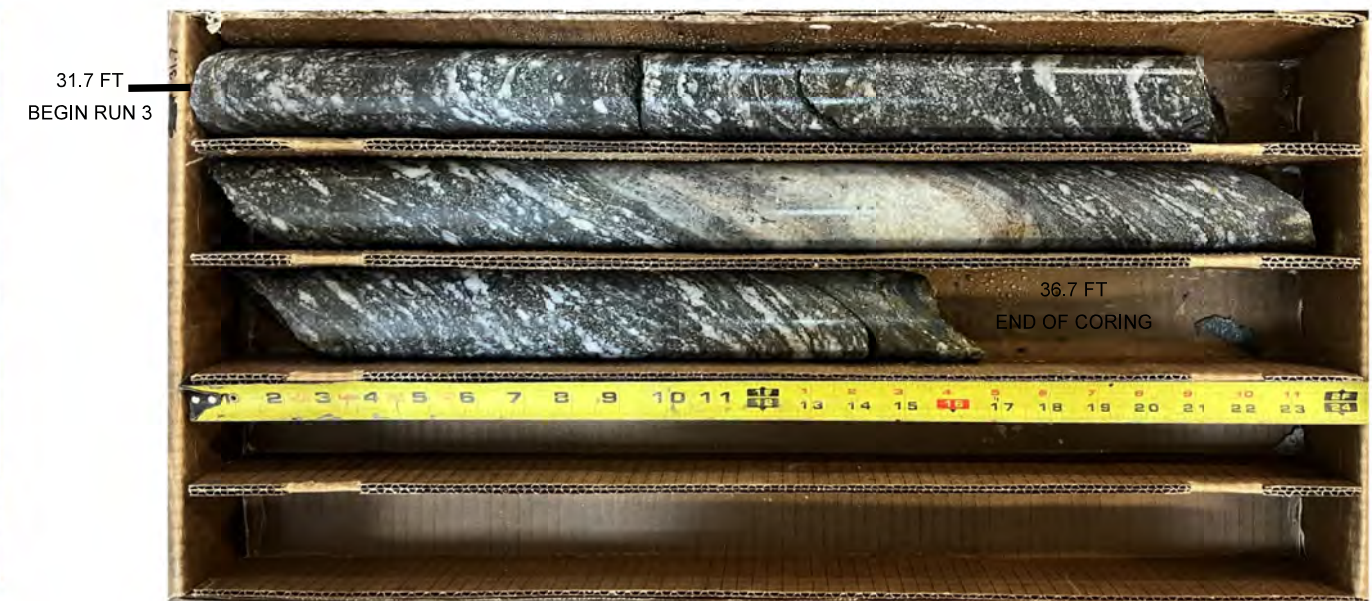
BORING EBI-A - RUNS 1, 2, AND 3



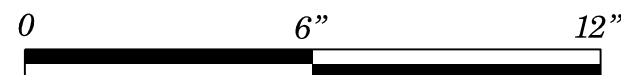
BORING EBI-A - RUNS 3 AND 4



BORING BI-B - RUNS 1 AND 2



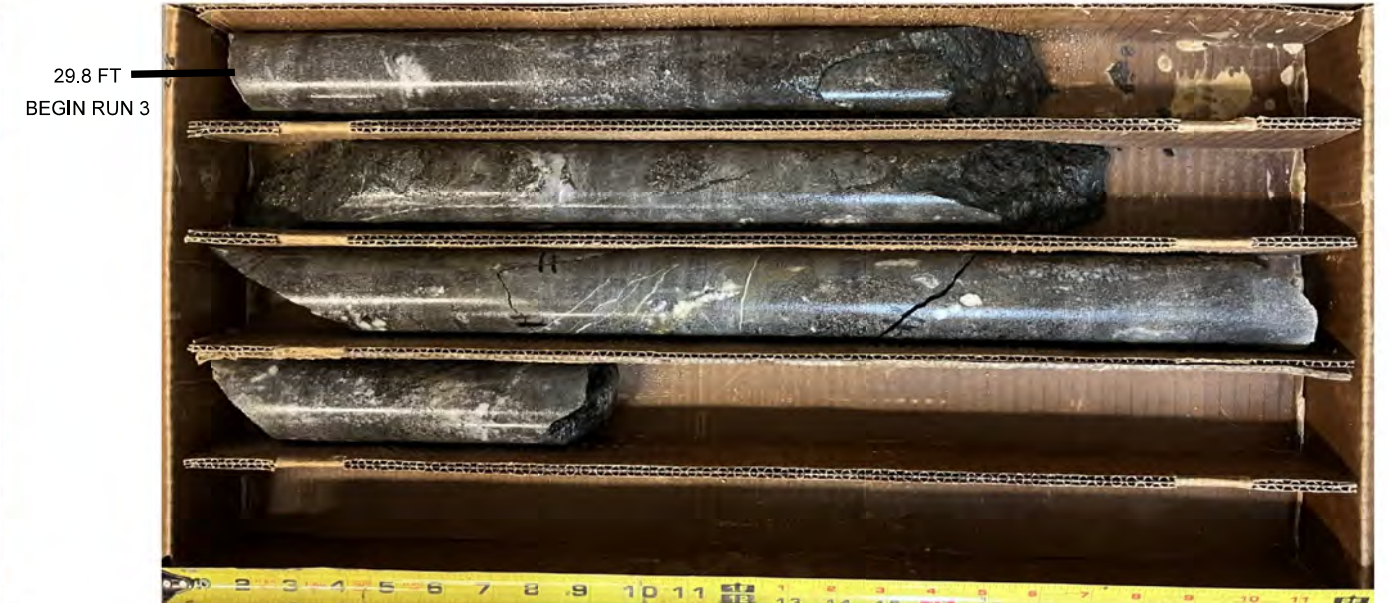
BORING BI-B - RUN 3



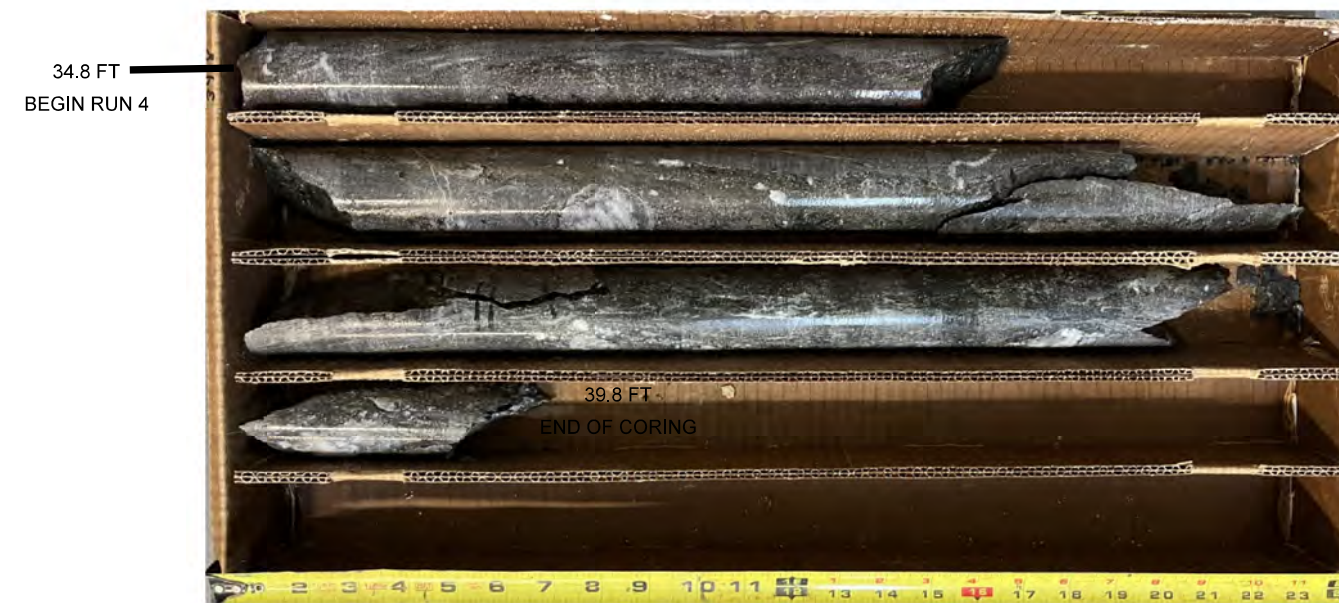
CORE PHOTOGRAPHS



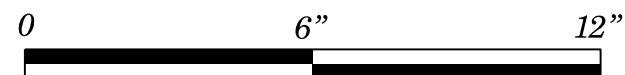
BORING B2-A - RUNS 1 AND 2



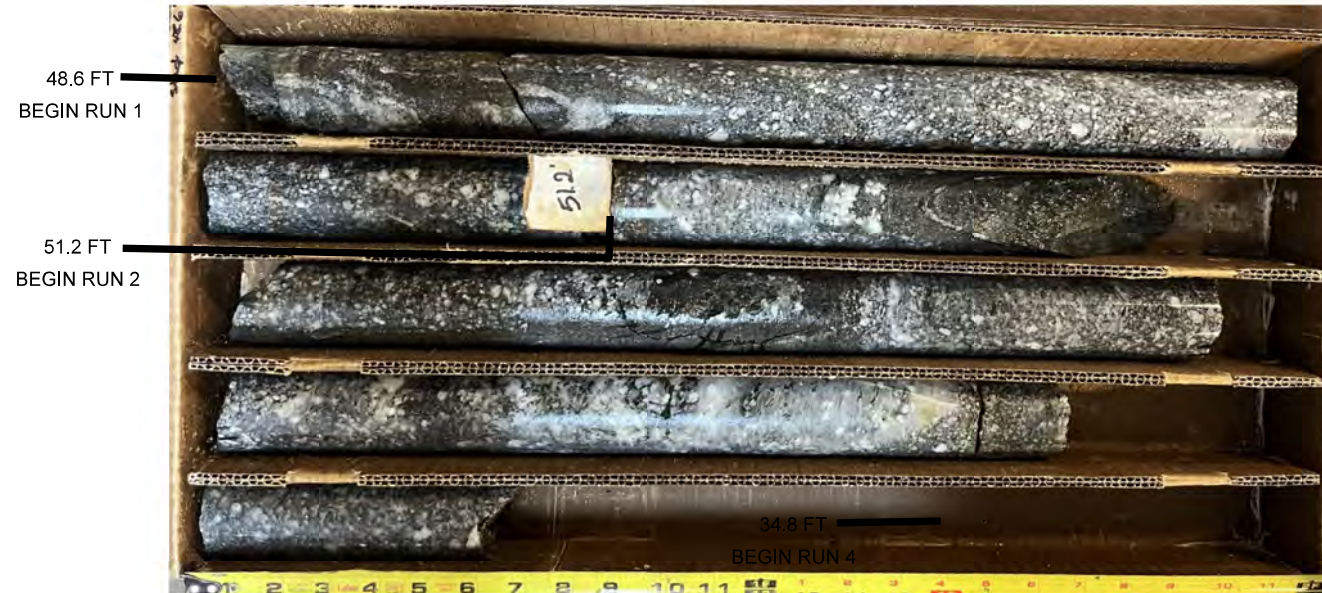
BORING B2-A - RUN 3



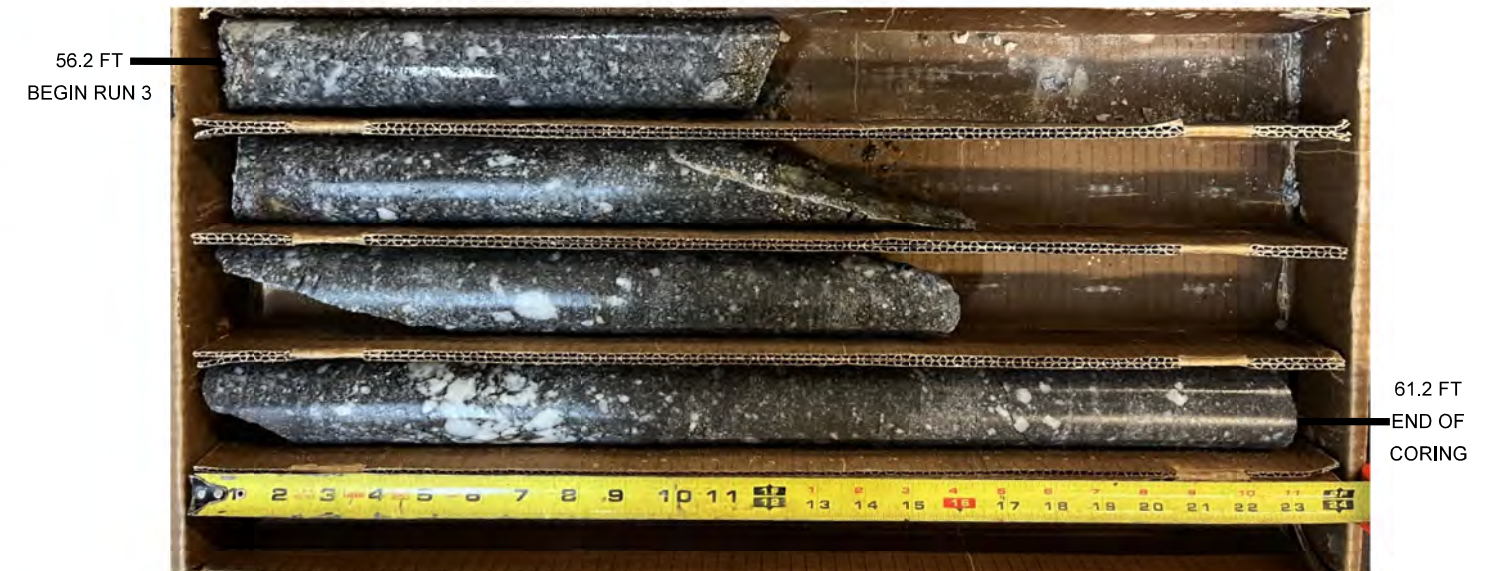
BORING B2-A - RUN 4



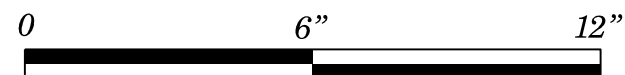
CORE PHOTOGRAPHS



BORING EB2-B - RUNS 1 AND 2



BORING EB2-B - RUN 3



SITE PHOTOGRAPH

