



## LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAY, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE CO	ORDINATES				
LATITUDE	LONGITUDE				
36°-03′-06.70′′	79°-44′-06.42′′				

## GENERAL NOTES

SEE TRANSPORTATION MANAGEMENT PLANS FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURÉS FOR STAGING OF OVERLAY SURFACE PREPARATION AND POLYMER CONCRETE (PC) PLACEMENT.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT DUE TO THE NATURE OF PRESERVATION PROJECTS, THE EXTENT OF WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. REPAIR LOCATIONS AND ESTIMATES OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIR.

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE, THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN WHAT IS SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, EXCEPT WHERE THE CONTRACTOR'S PLAN USED PLATFORMS, NETS, SCREEN OR OTHER PROTECTIVE DEVICES TÓ CATCH THE MATERIAL. THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS. ANY DAMAGE TO EXISTING REINFORCING STEEL DURING CONTRACTOR'S OPERATIONS SHALL BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST TO THE DEPARTMENT.

FOR CONTROL OF TRAFFIC AND LIMITS OF PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLANS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE ELEVATIONS AND CLEARANCES SHOWN ON THE PLANS AT THE POINTS OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION. VERIFY THE ELEVATIONS ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER, ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH BRIDGES, SEE SPECIAL PROVISIONS.

ALL PAVEMENT MARKING WILL BE IN ACCORDANCE WITH THE PAVEMENT MARKING NOTES.

EXISTING JOINTS AND DECK DRAINS SHALL BE SEALED PRIOR TO BEGINNING SURFACE PREPARATIONS OF THE BRIDGE DECK. THE CONTRACTOR SHALL TAKE CARE THAT ANY CONSTRUCTION DEBRIS THAT COLLECTS IN THE DRAINS IS CONTAINED. DRAINS IN SHOULDERS OF ADJACENT TRAVEL LANES SHALL BE KEPT FREE AND CLEAR OF DEBRIS.

LONGITUDINAL CONSTRUCTION JOINTS OF OVERLAYS SHALL BE LOCATED ALONG THE CENTERLINE OR EDGE OF TRAVEL LANES.

FOR SHOTBLASTING BRIDGE DECK AND CLASS II SURFACE PREPARATION, SEE OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE SPECIAL PROVISION.

FOR FINE MILLING, SEE SPECIAL PROVISIONS.

FOR CONCRETE DECK REPAIR FOR PC OVERLAY, PC MATERIALS, AND PLACING AND FINISHING PC OVERLAY, SEE POLYMER CONCRETE BRIDGE DECK OVERLAY SPECIAL PROVISION.

FOR EXPANSION JOINT SEAL REPAIR, SEE SPECIAL PROVISIONS.

FOR FOAM JOINT SEALS FOR PRESERVATION, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING AND DEBRIS REMOVAL. SEE SPECIAL PROVISIONS.

FOR PAINTING EXISTING WEATHERING STEEL STRUCTURE, SEE SPECIAL PROVISIONS.

FOR PAINTING CONTAINMENT AND POLLUTION CONTROL, SEE PAINTING EXISTING WEATHERING STEEL STRUCTURE SPECIAL PROVISION.

FOR REPOSITIONING BEARINGS, SEE SPECIAL PROVISIONS.

PROJECT NO. I-5955A GUILFORD \_ COUNTY 400367 BRIDGE NO. \_\_\_

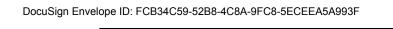
SHEET 2 OF 2 STATE OF NORTH CAROLINA

> GENERAL DRAWING FOR BRIDGE ON I-40 WBL OVER SR 4240 (EAST GATE CITY BOULEVARD)

DEPARTMENT OF TRANSPORTATION RALEIGH

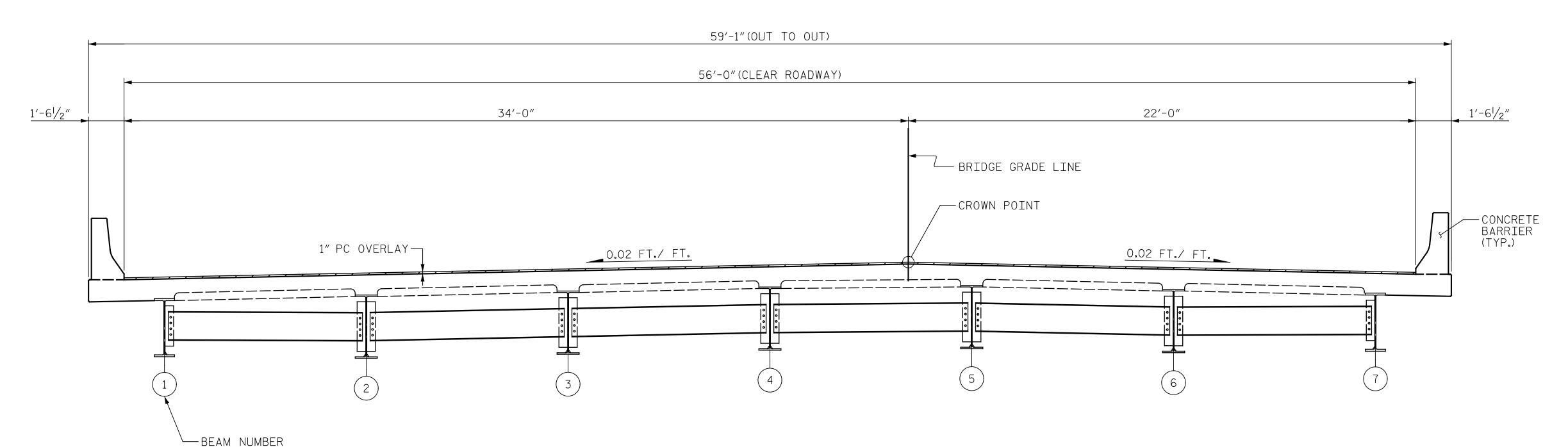
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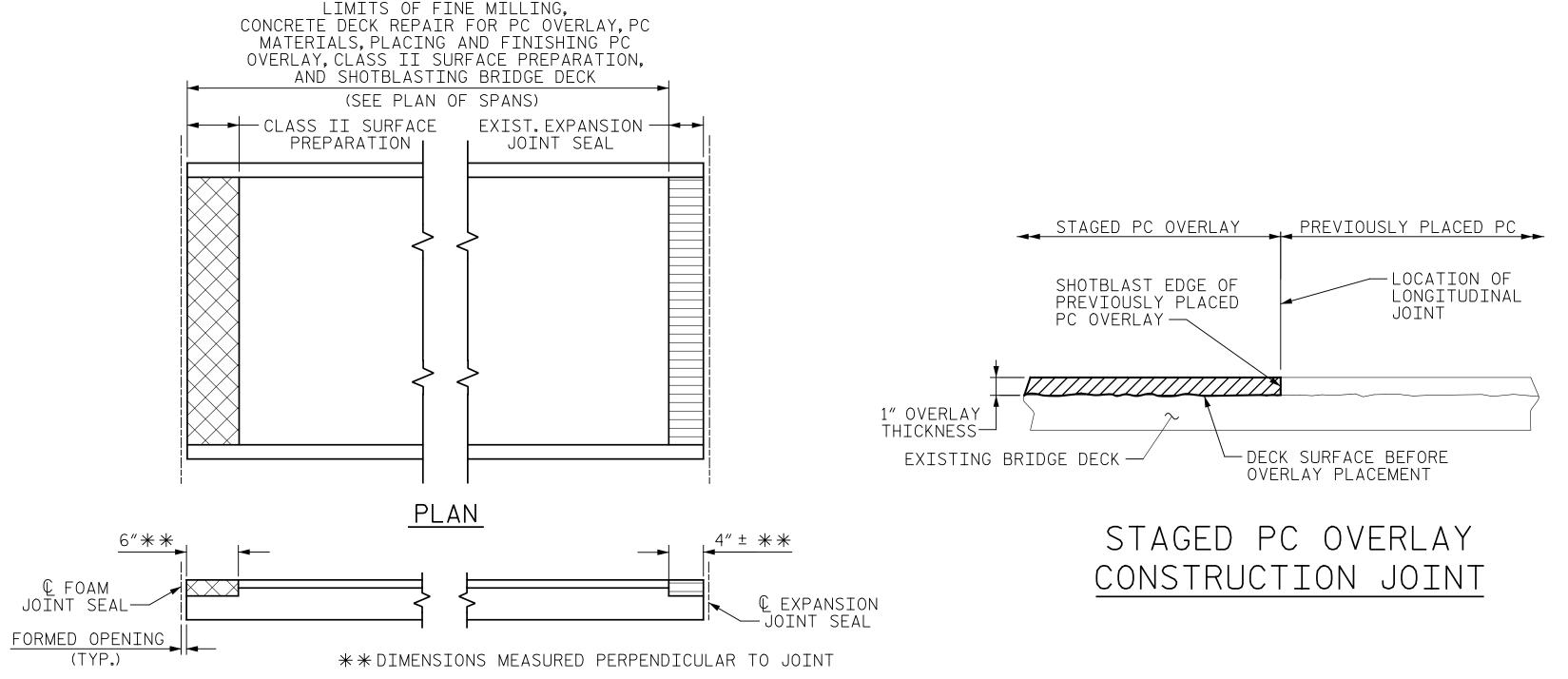


## NOTES:

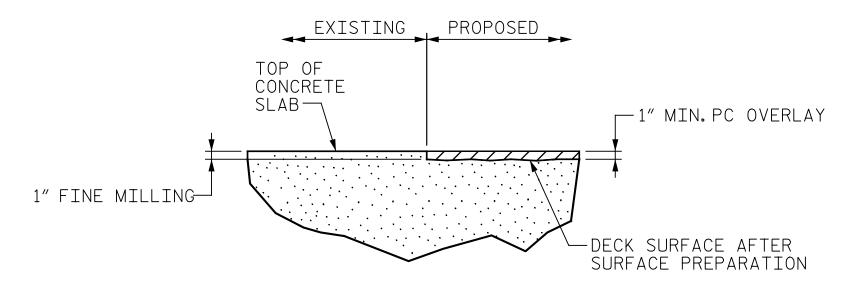
SEE TRANSPORTATION MANAGEMENT PLANS FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND PC PLACEMENT.



## TYPICAL SECTION (PROPOSED)



EXPANSION JOINT SEAL



DETAIL OF POLYMER CONCRETE OVERLAY

PROJECT NO. I-5955A GUILFORD \_ COUNTY 400367 BRIDGE NO. \_\_\_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TYPICAL SECTION AND SURFACE PREPARATION DETAILS

GANNETT

Suite 900

Raleigh, NC 27603

919-420-7660

NC Lic. No. F-0270

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PAY LIMITS FOR OVERLAY BID ITEMS

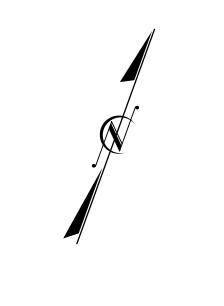
ELEVATION

J. HARRIS \_ DATE : <u>10/2022</u> DRAWN BY : J. YANNACCONE \_ DATE : <u>10/2022</u> CHECKED BY : \_\_

© FOAM

CLASS II SURFACE PREPARATION

(TYP.)



 $48'-6^{11}/_{16}''$ 49'-511/16''73′-1″ 73′-1″ 17'-0" 17'-0" (APPROACH SLAB) (SPAN B) (SPAN C) (SPAN D) (APPROACH SLAB) (SPAN A) REMOVE BARRIER RAIL REMOVE BARRIER RAIL Ų JOINT COVER PLATE 0.8 CF ₽ JOINT COVER PLATE 0.8 CF © JOINT -@ BENT 3 CONCRETE REPAIR — @ BENT 1 CONCRETE REPAIR -@BENT 2 (SEE NOTES)— (SEE NOTES)— └─ GUTTER LINE **⟨**/**™** FILL FACE @ END BENŢ 1-BRIDGE-GRADE LINE TO GREENSBORO -FILL FACE @ END BENT 2 END OF-END OF · **K** APPROACH APPROACH ROADWAY SLAB ROADWAY SLAB — LINE -REMOVE BARRIER RAIL COVER - REMOVE BARRIER RAIL COVER PLATE 0.8 CF CONCRETE REPAIR PLATE 0.8 CF CONCRETE REPAIR (SEE NOTES) (SEE NOTES) APPROACH SLAB APPROACH SLAB @ END BENT 2 SPAN A SPAN D SPAN B SPAN C @ END BENT 1

#### REPAIR QUANTITY TABLE TOP OF DECK REPAIR ESTIMATE ACTUAL APPR. SLAB @ EB1 101.5 SY SPAN A 301.0 SY 450.9 SY SPAN B FINE MILLING 447.9 SY SPAN C 303.7 SY SPAN D APPR. SLAB @ EB2 101**.**5 SY APPR. SLAB @ EB1 4.2 SY SPAN A 6.8 SY 2.6 SY SPAN B CLASS II SURFACE PREPARATION SPAN C 0.0 SY 4.2 SY SPAN D APPR. SLAB @ EB2 4.2 SY 0 SF APPR. SLAB @ EB1 SPAN A 30 SF 107 SF SPAN B CONCRETE WORK FOR JOINT REPLACEMENT SPAN C 77 SF SPAN D 0 SF APPR.SLAB @ EB2 0 SF

APPR. SLAB @ EB1

APPR. SLAB @ EB2

APPR. SLAB @ EB1

APPR. SLAB @ EB2

APPR. SLAB @ EB1

APPR. SLAB @ EB2

SPAN A

SPAN B

SPAN C

SPAN D

3.5 CY

10.5 CY

15.7 CY

15.6 CY

10.5 CY

3.5 CY

101.5 SY

301.0 SY

450.9 SY

447.9 SY

303.7 SY

101**.**5 SY

805 SF

2467 SF

3780 SF

3791 SF

2527 SF

805 SF

0.8 CF

1.6 CF

0.8 CF

0.0 CF

REPAIR QUANTITY TABLE

UNDERSIDE OF DECK REPAIR

SPAN B

SPAN C

SPAN D

SPAN A

SPAN B

SPAN C

SPAN D

AREA

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

SHOTCRETE REPAIRS

UNDERSIDE OF DECK

OVERHANG DIAPHRAGMS

UNDERSIDE OF OVERHANG

INTERIOR DIAPHRAGMS

UNDERSIDE EPOXY RESIN

INJECTION

ESTIMATE ACTUAL

CF

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

0.0

ACTUAL

ESTIMATE

0.0 LF

0.0 LF

0.0 LF

0.0 LF

VOLUME AREA VOLUME

### NOTES:

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

PLAN

PAYMENT FOR CLASS II SURFACE PREPARATION IS BASED UPON SQUARE YARDS OF ADDITIONAL DEMOLITION REQUIRED FOLLOWING FINE MILLING OF BRIDGE DECK, SEE "OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE" SPECIAL PROVISION.

CONCRETE COVER FOR TOP BARS IN THE DECK SLAB IS  $2\frac{1}{2}$ "PER THE EXISTING BRIDGE PLANS.

FOR SECTION A-A, B-B AND D-D, SEE "FOAM JOINT SEALS" SHEET.

FOR SECTION E-E, SEE "EXPANSION JOINT SEAL DETAILS" SHEET.

AFTER REMOVAL OF THE BARRIER RAIL COVER PLATE, FILL THE RECESS IN THE BARRIER WITH A POLYMER MODIFIED CEMENT MORTAR CONFORMING TO THE CONCRETE REPAIR SPECIAL PROVISION. SHAPE THE SURFACE OF THE REPAIR TO CONFORM TO THE GEOMETRY OF THE BARRIER RAIL.

FOR CONCRETE WORK FOR JOINT REPLACEMENT. SEE SPECIAL PROVISIONS.

FINE MILLING AND SHOTBLASTING OF BRIDGE DECK

CLASS II SURFACE PREPARATION



UNDERSIDE OF DECK REPAIR

CONCRETE WORK FOR JOINT REPLACEMENT

PROJECT NO. I-5955A GUILFORD \_ COUNTY

400367 BRIDGE NO. \_



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

PLAN OF SPANS

SHEET NO.

S13-4

TOTAL SHEETS

127

DATE:

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J. MYA DATE : 10/2022 DRAWN BY : \_ DATE : <u>10/2022</u> J. YANNACCONE CHECKED BY : \_\_\_\_\_

PC MATERIALS

PLACING AND

FINSIHING

PC OVERLAY

GROOVING BRIDGE

FLOORS

CONCRETE REPAIR

PROVIDE WATERTIGHT SEAL

DRAWN BY

CHECKED BY : \_\_\_

AT END OF FOAM JOINT SEAL AS RECOMMENDED BY

(@ END BENTS)

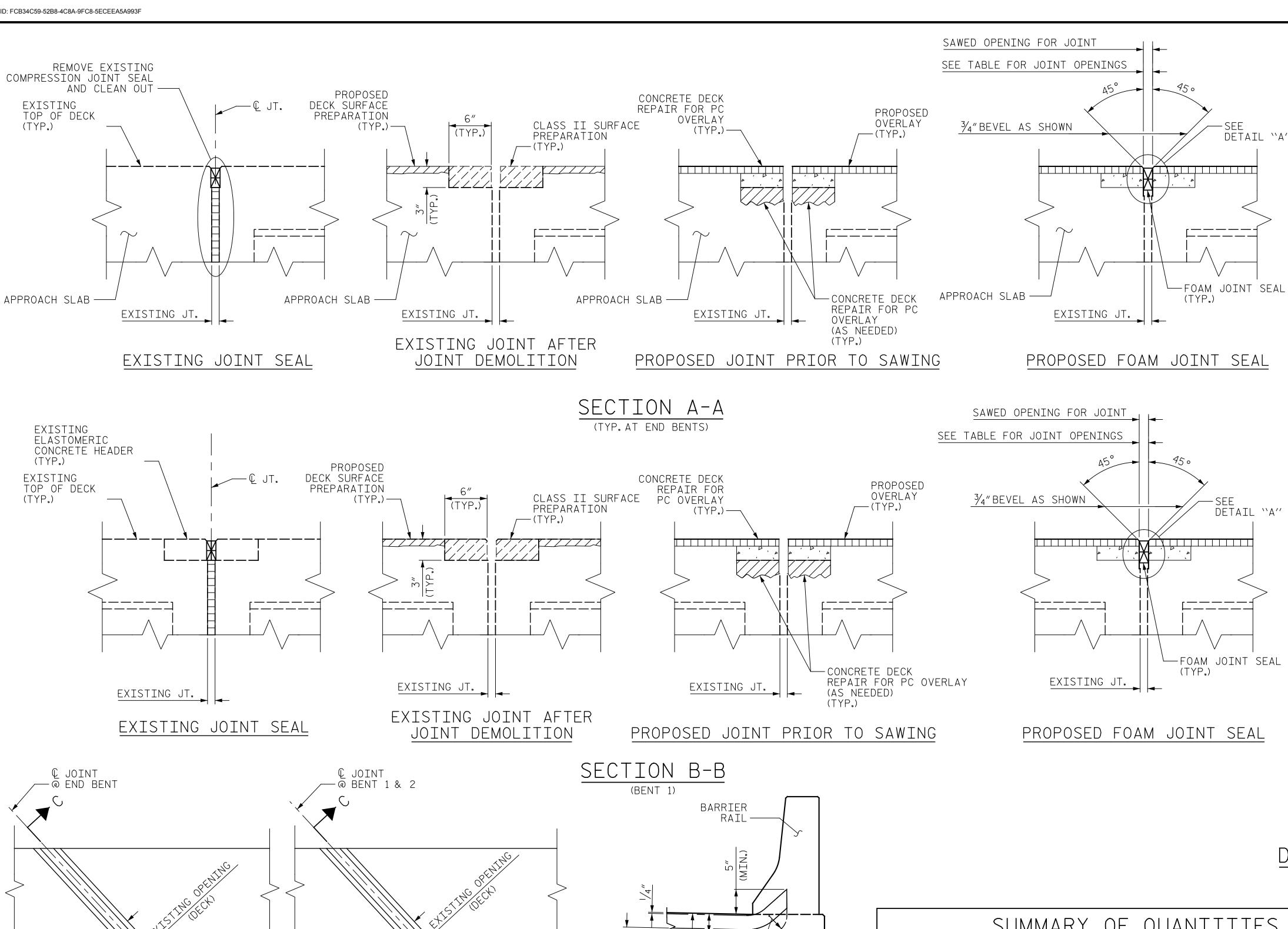
J. MYA

J. YANNACCONE

MANUFACTURER

DATE : 10/2022

DATE : 10/2022



RADIUS OF SAW BLADE

-BOTTOM OF SEAL

SECTION C-C

FOAM JOINT SEAL SHALL BE FACTORY FORMED OR CUT, HEAT WELDED AND TURNED

UP PARALLEL TO SLOPED FACE OF CURB.

PROVIDE WATERTIGHT SEAL—

AT END OF FOAM JOINT

SEAL AS RECOMMENDED BY

MANUFACTURER

PLAN

(@ BENT 1 & 2)

NOTES:

FINAL JOINT SEALS SHALL NOT BE INSTALLED UNTIL THE OVERLAY OR SEALANT WORK IS COMPLETE.

THE CONTRACTOR SHALL FIELD VERIFY THE EXISTING JOINT OPENING PRIOR TO ORDERING JOINT SEAL MATERIAL. IF THE ACTUAL JOINT OPENING VARIES FROM THE OPENING INDICATED IN THE DETAILS BY MORE THAN  $\frac{1}{4}$ , NOTIFY THE ENGINEER.

DETAIL "A" THE MANUFACTURER IS TO PROVIDE THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL FOR THE SIZE OF THE OPENING ON THE PLANS AND ACCOMMODATE THE MINIMUM EXPANSION SHOWN ON THE PLANS.

FOAM JOINTS SHALL BE INSTALLED AS PER THE MANUFACTURER'S RECOMMENDATIONS.

THE CONTRACTOR SHALL TAKE CARE DURING JOINT REHAB OPERATIONS NOT TO DROP ANY MATERIAL BELOW THE BRIDGE WITHOUT PROTECTIVE DEVICES BELOW TO CATCH THE MATERIAL ANY MATERIAL THAT FALLS BELOW THE BRIDGE SHALL BE CONTAINED, REMOVED AND DISPOSED OF BY THE CONTRACTOR AT NO EXTRA COST TO THE DEPARTMENT. IF THE ENGINEER DETERMINES THAT THE PROTECTIVE DEVICES ARE NOT ADEQUATE OR NOT BEING EMPLOYED, THE WORK SHALL BE SUSPENDED UNTIL ADEQUATE PROTECTION IS PROVIDED.

THE CONTRACTOR WILL NOT BE PERMITTED TO FORM THE JOINT IN LIEU OF SAWING THE JOINT.

THE INSTALLED FOAM JOINTS SHALL BE WATERTIGHT.

THE CONTRACTOR SHALL SAW CUT TO A NOMINAL DEPTH OF  $\frac{1}{2}$ " BUT REINFORCING STEEL SHALL NOT BE DAMAGED. CONTRACTOR SHALL REMOVE ŚURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.

QUANTITIES SHOWN IN THE ELASTOMERIC CONCRETE FOR PRESERVATION TABLE ARE BASED ON THE MINIMUM JOINT DEMOLITION SHOWN.

FOR EXCAVATION BELOW THE BOTTOM OF PLANNED JOINT DEMOLITION, CONCRETE FOR DECK REPAIR SHALL BE PLACED IN THE EXCAVATED AREA TO THE ELEVATION AT THE BOTTOM OF THE PROPOSED ELASTOMERIC CONCRETE FOR PRESERVATION HEADERS

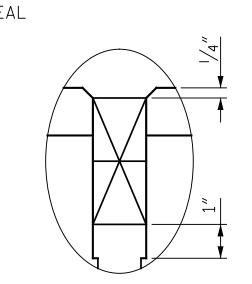
FOR CLASS II SURFACE PREPARATION, SEE OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE SPECIAL PROVISION.

FOR CONCRETE DECK REPAIR FOR PC OVERLAY, SEE SPECIAL PROVISIONS.

NO SEPARATE PAYMENT WILL BE MADE FOR REMOVING THE EXISTING MEDIAN AND BARRIER RAIL COVER PLATES. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR "FOAM JOINT SEALS FOR PRESERVATION".

FOR SECTION D-D, SEE SHEET 2 OF 2.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE BENT CAPS AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAPS. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAPS BENEATH THE ELASTOMERIC BEARINGS. FOR EPOXY COATING. SEE SPECIAL PROVISIONS.



#### SAWED JOINT OPENING TABLE SAWED JT. OPENING TABLE (PERPENDICULAR TO JT.) LOCATION AT 45° AT 60° AT 90° 1<sup>15</sup>/<sub>16</sub>" 1<sup>15</sup>/<sub>16</sub>" 1<sup>15</sup>/<sub>16</sub>" END BENT 1 2<sup>13</sup>/<sub>16</sub>" 2<sup>11</sup>/<sub>16</sub>" 21/2" BENT 1 1%/6″ 1%i6″ 1%/6″ END BENT 2

## DETAIL "A"

SUMMARY OF QUANTITIES FOAM JOINT SEALS FOR PRESERVATION CONCRETE DECK REPAIR EPOXY COATING FOR PC OVERLAY LOCATION ESTIMATED ESTIMATED | ACTUAL ESTIMATED ACTUAL ACTUAL (SQ.FT.) (LIN. FT.) (LIN. FT.) (SQ. YDS.) (SQ. YDS.) (SQ.FT.) END BENT 1 78.5 8.5 184 46.5 5.2 144 BENT 1 END BENT 2 78.5 8.5 184

PROJECT NO. <u>I-</u>5955A GUILFORD \_ COUNTY

400367 BRIDGE NO. \_\_\_

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

FOAM JOINT SEALS

S13-5

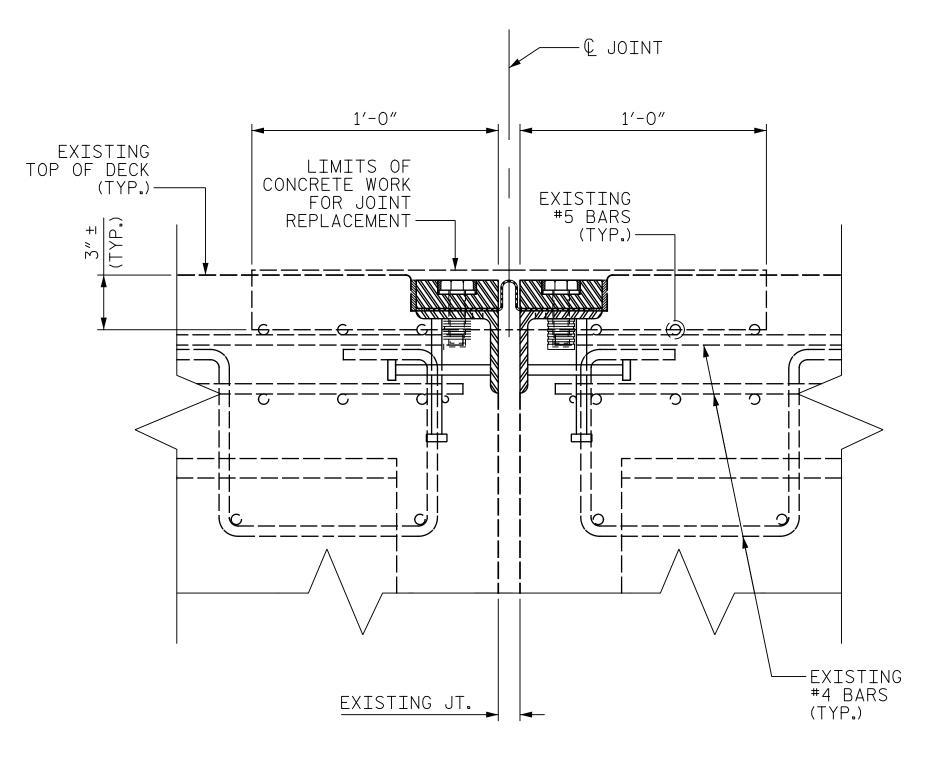
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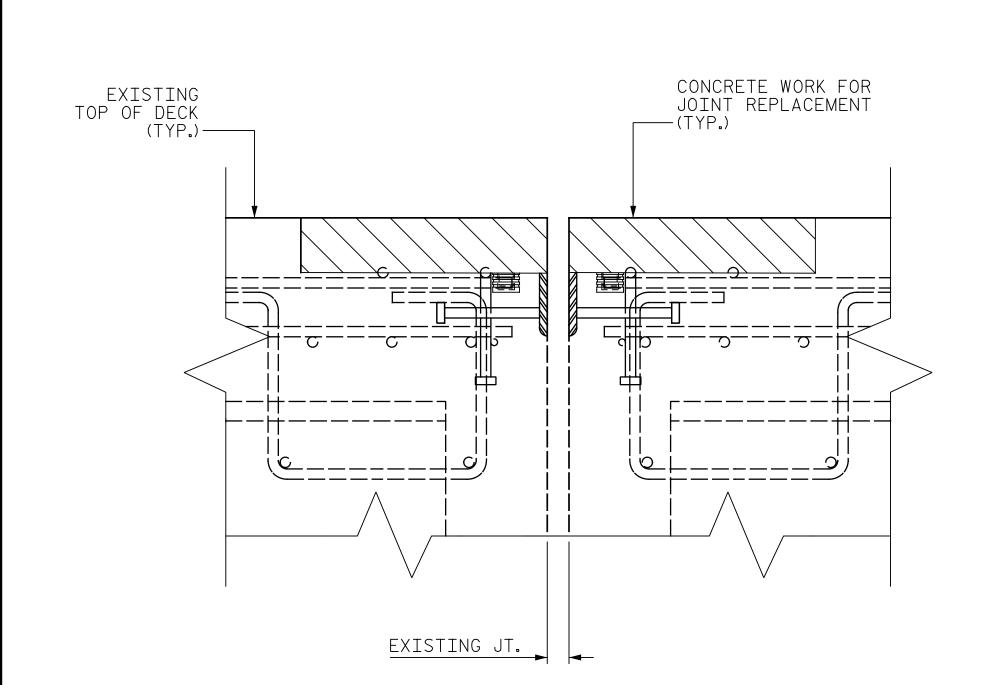
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EXISTING EXPANSION JOINT SEAL



PROPOSED JOINT PRIOR TO SAWING

\_ DATE : <u>10/2022</u>

\_ DATE : <u>10/2022</u>

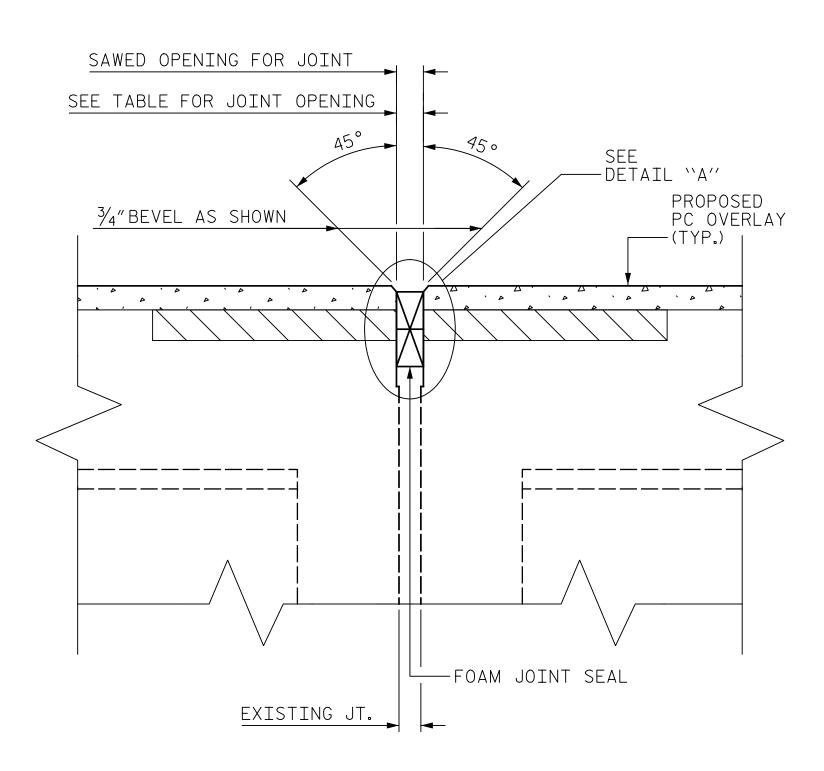
J. YANNACCONE

CHECKED BY : \_\_\_

SECTION D-D (BENT 1 & 2)

1'-0" EXISTING (TYP.) TOP OF DECK (TYP.)— ==*-----*+ (6-----NOTE: EXISTING REINFORCING EXISTING JT. STEEL SHALL REMAIN IN PLACE.

EXISTING JOINT AFTER JOINT DEMOLITION



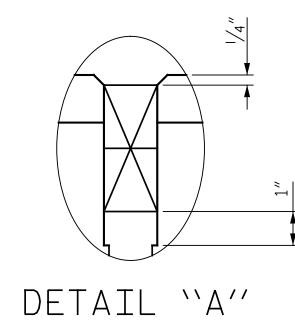
PROPOSED FOAM JOINT SEAL

NOTES

ALL HORIZONTAL DIMENSIONS ARE MEASURED PERPENDICULAR TO THE JOINT UNLESS NOTED OTHERWISE.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE BENT CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAPS BENEATH THE ELASTOMERIC BEARINGS AND MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR ADDITIONAL NOTES, SEE SHEET 1 OF 2.



SUMMARY OF QUANTITIES								
	FOAM JOINT PRESER	EPOXY COATING						
LOCATION	ESTIMATED (LIN. FT)	ACTUAL (LIN.FT)	ESTIMATED (SQ.FT)	ACTUAL (SQ.FT)				
BENT 1	30.0		93					
BENT 2	78.5		237					

SAWED JOINT	OPEN:	ING T	ABLE
LOCATION		JT.OPENIN NDICULAR	
	AT 45°	AT 60°	AT 90°
BENT 1	2 <sup>13</sup> / <sub>16</sub> "	2 <sup>11</sup> / <sub>16</sub> "	21/2"
BENT 2	23/4"	2 <sup>11</sup> / <sub>16</sub> "	2%6"

PROJECT NO. I-5955A GUILFORD \_\_ COUNTY BRIDGE NO. 400367

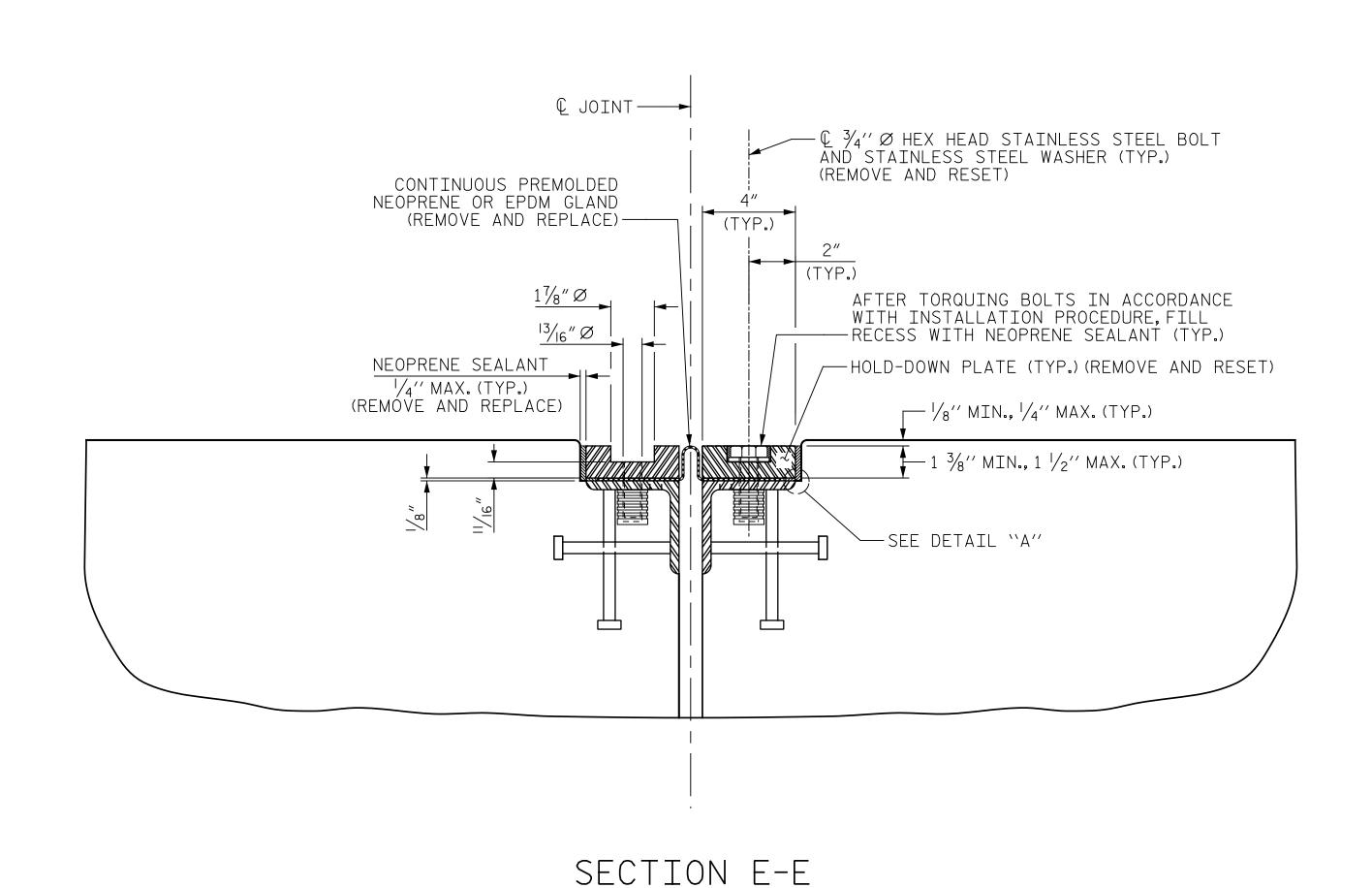
SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

FOAM JOINT SEALS

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SECTION NORMAL TO JOINT -- STEEL SUPERSTRUCTURE

NEOPRENE SEALANT -

& BOLT HOLE

CROSS SECTION

INSTALLATION SKETCH

BOLT HOLE-

CROSS SECTION

DETAIL A

<u>√2"(MAX.)</u> (TYP.)

NEW CONTINUOUS— PREMOLDED NEOPRENE OR EPDM GLAND

## REPAIR INSTALLATION PROCEDURE

LOOSEN THE EXISTING BOLTS AND HOLD DOWN PLATES TO REMOVE AND REPLACE THE EXISTING GLAND. REMOVE THE EXISTING NEOPRENE SEALANT AND CLEAN THE EXISTING BASE ANGLE OR OIL. GREASE AND OTHER LATENTS.

LAY THE NEW GLAND ON THE BASE ANGLE AND FIELD MARK THE NEW GLAND FOR THE BOLT HOLES, HOLES IN THE NEW GLAND SHALL BE PUNCHED 1/8" IN DIAMETER WITH A HAND PINCH.

IN ORDER TO CHECK THE PROPER ALIGNMENT, PLACE THE NEW GLAND HOLD-DOWN PLATES ON THE BASE ANGLE. DO NOT APPLY NEW NEOPRENE SEALANT. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE, BUT DO NOT TIGHTEN. THE ENGINEER WILL INSPECT THE JOINT SEAL DEVISE FOR PROPER ALIGNMENT.

AFTER INSPECTION, REMOVE THE HOLD DOWN PLATES AND NEW GLAND. APPLY NEW NEOPRENE SEALANT TO THE BASE ANGLE IN ACCORDANCE WITH THE "INSTALLATION SKETCH". PLACE NEW GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE ASSEMBLY AND TORQUE THE BOLTS TO 88 FT-LBS WITH A TORQUE WRENCH. CHECK THE TORQUE AFTER THREE (3) HOURS AND, IF NECESSARY, RETIGHTEN TO 88 FT-LBS. A FINAL CHECK SHALL BE MADE AT SEVEN (7) DAYS. TORQUE SHALL NOT BE LESS THAN 80 FT-LBS AFTER SEVEN (7) DAYS.

AFTER PROPER TORQUING, CLEAN THE BOLT HOLE RECESSES AND THE RECESS BETWEEN THE JOINT SEAL DEVICE AND CONCRETE. COMPLETELY FILL THESE RECESSES WITH NEW NEOPRENE SEALANT.

-NEOPRENE SEALANT

## GENERAL NOTES

ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°. FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.

THE FINISHED EXPANSION SEAL DEVICE SHALL BE A MINIMUM 1/8" AND A MAXIMUM OF 1/4" BELOW THE TOP OF SLAB.

FOR EXPANSION JOINT SEAL REPAIR, SEE SPECIAL PROVISIONS.

NO SEPARATE PAYMENT WILL BE MADE FOR REMOVING AND REINSTALLING MEDIAN AND BARRIER RAIL COVER PLATES. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR "EXPANSION JOINT SEAL REPAIR".

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE BENT CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAPS. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAPS BENEATH THE ELASTOMERIC BEARINGS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

MOVEMENT AND SETTING AT JOINT									
LOCATION   SKEW   MOV		TOTAL MOVEMENT (ALONG © RDWY)	PERPENDICULAR JOINT OPENING AT 45° F	PERPENDICULAR JOINT OPENING AT 60° F	PERPENDICULAR JOINT OPENING AT 90° F				
BENT 3	47°-16′-54″	3/8″	3/8" 15/8"		11/2"				

SUMMARY OF QUANTITIES								
LOCATION (EAST BOUND	EXPANSION REPA		EPOXY COATING					
LANES)	ESTIMATED (LIN. FT)	ACTUAL (LIN. FT)	ESTIMATED (SQ.FT)	ACTUAL (SQ. FT)				
BENT 3	78.5		237					

PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400367



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

EXPANSION JOINT SEAL DETAILS

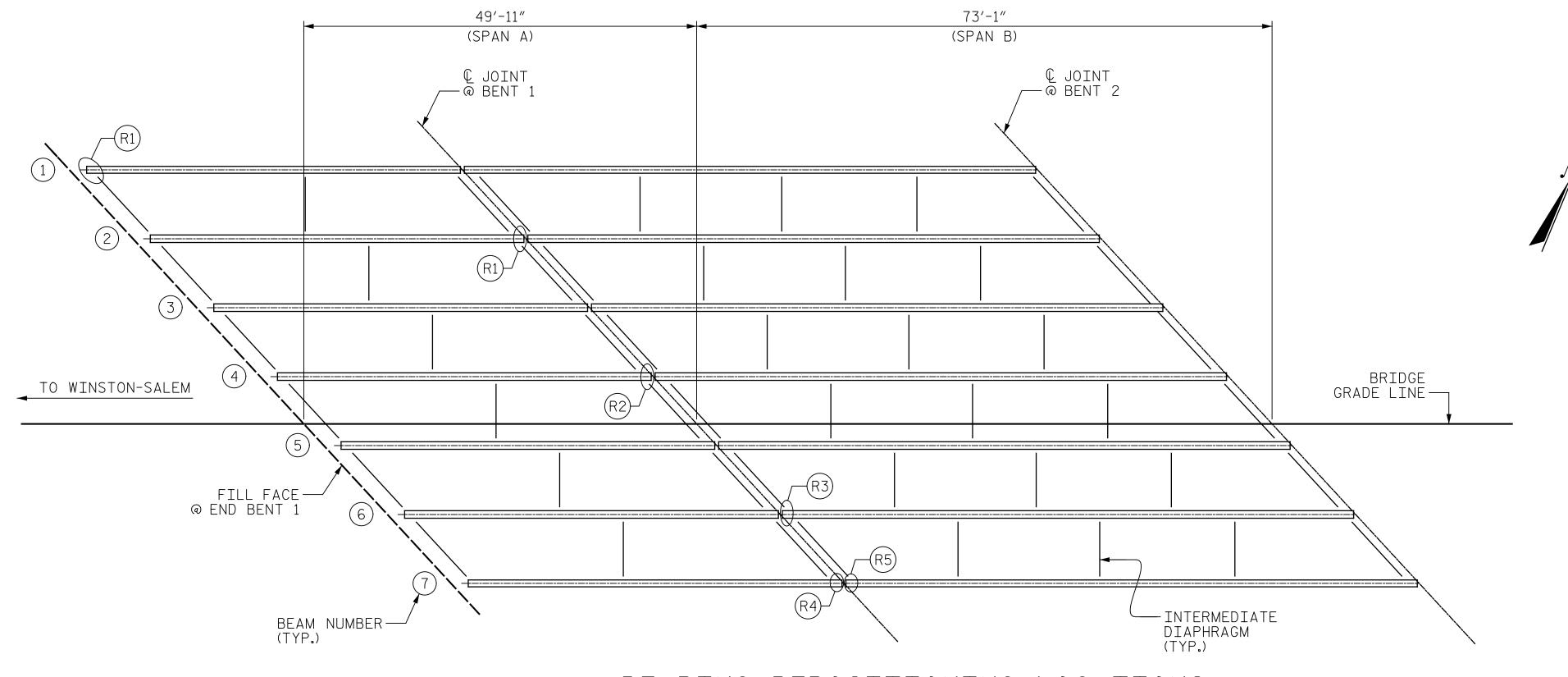
GANNETT One Glenwood Avenue Suite 900
Ralelgh, NC 27603
919-420-7660
NC Lic. No. F-0270

SEAL DETAILS

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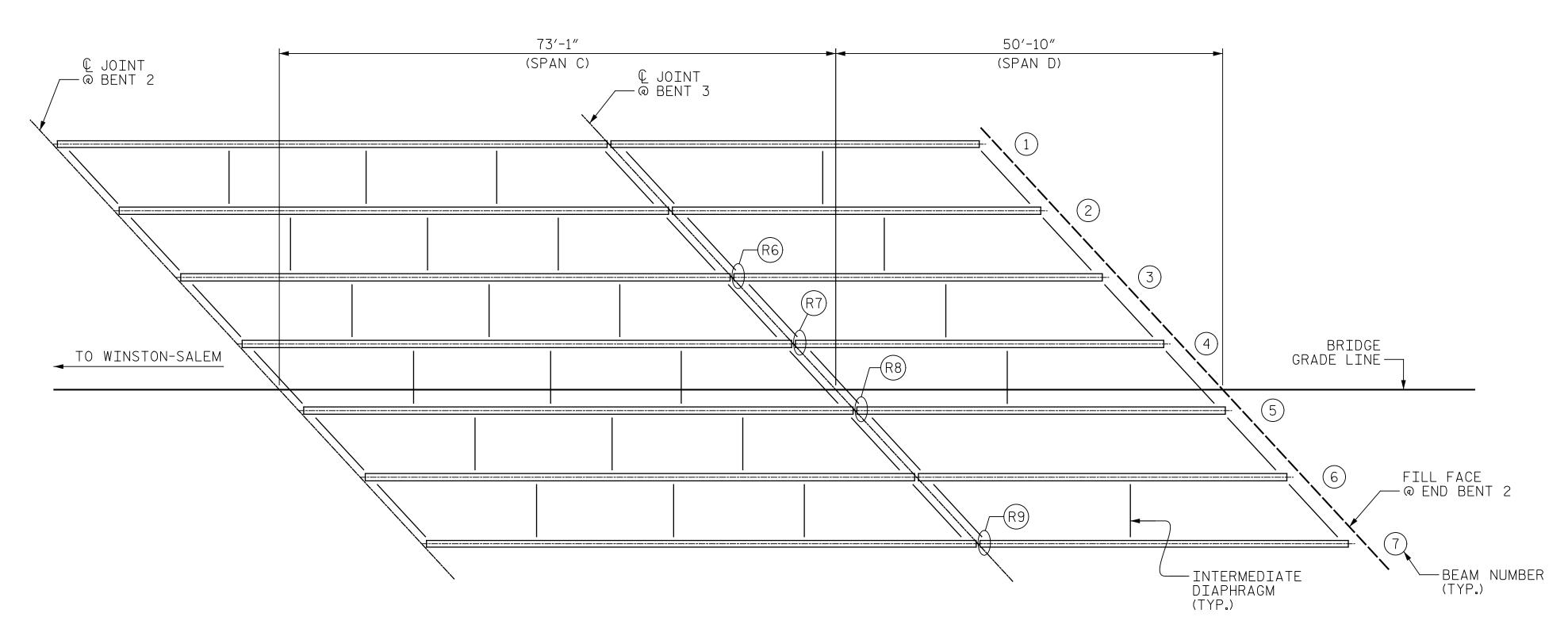
DRAWN BY: J. MYA DATE: 10/2022
CHECKED BY: J. YANNACCONE DATE: 10/2022

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## BEARING REPOSITIONING LOCATIONS

(OTHER LOCATIONS MAY EXIST, SEE NOTES)



## BEARING REPOSITIONING LOCATIONS

(OTHER LOCATIONS MAY EXIST, SEE NOTES)



REPAIR QUAN	TITY T	ABLE
REPOSITIONIN	NG BEARIN	IGS
	ESTIMATE	ACTUAL
REPOSITIONING SOLE PLATES	5 EA	
REPOSITIONING BEARING PLATES	4 EA	

(R#) REPOSITIONING SOLE PLATE / BEARING PAD

### NOTES

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

- FOR REPOSITIONING BEARINGS, SEE SPECIAL PROVISIONS AND "BEARING REPOSITIONING DETAILS" SHEET.
- FOR BRIDGE JACKING, SEE SPECIAL PROVISIONS.
- FOR BRIDGE JACKING DETAILS, SEE "JACKING DETAILS" SHEET.

THE REPAIR QUANTITIES REPORTED IN THE TABLE ARE ONLY AN ESTIMATE. THE CONTRACTOR AND ENGINEER SHALL INSPECT THE BEARINGS TO DETERMINE THE BEST REPAIR OPTION AND CORRESPONDING PAY ITEM FOR EACH BEARING THAT REQUIRES REPOSITIONING.

SOLE PLATES AND BEARING PADS SHALL BE REPOSITIONED BEFORE ANY JOINT REPAIRS OR GIRDER END PAINTING BEGINS.

> PROJECT NO. I-5955A GUILFORD \_\_\_ COUNTY BRIDGE NO. 400367



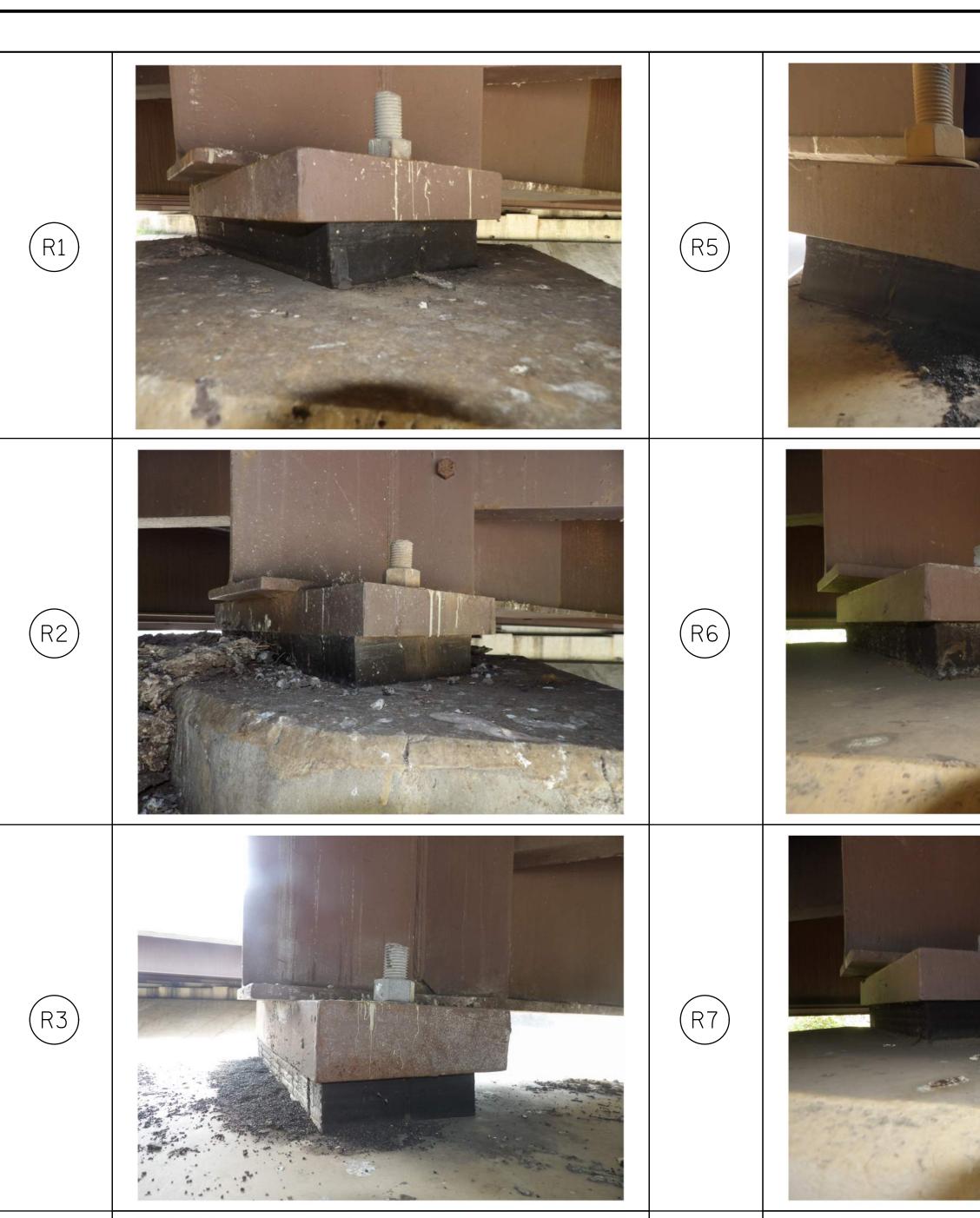
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

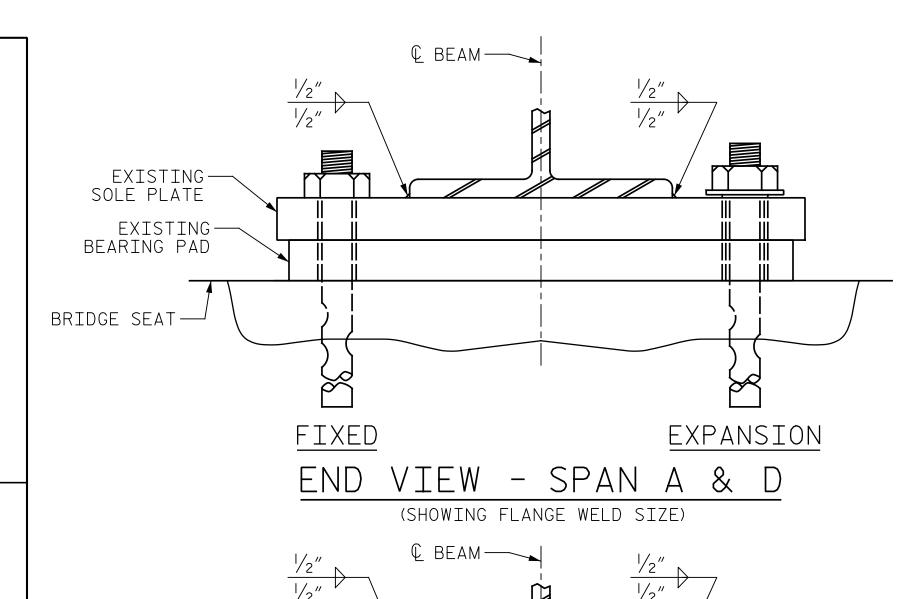
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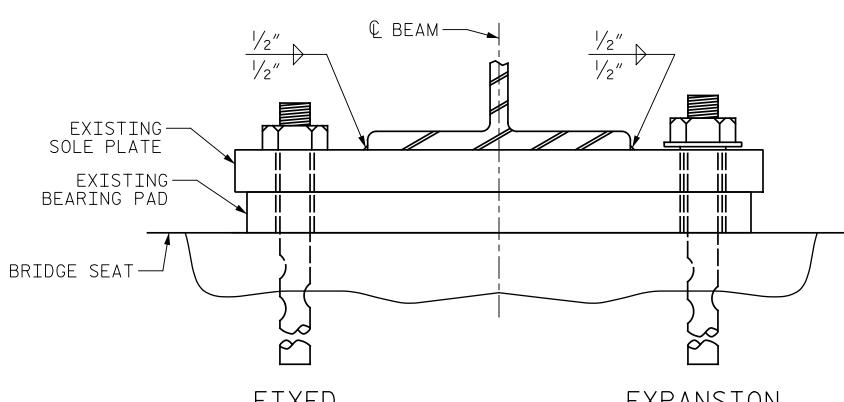
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J. YANNACCONE \_ DATE : <u>10/2022</u> CHECKED BY : \_\_\_\_

\_ DATE : <u>10/2022</u>







EXPANSION FIXED END VIEW - SPAN B & C (SHOWING FLANGE WELD SIZE)

## NOTES:

THE GIRDERS SHALL BE JACKED AND THE BEARING PAD SHALL BE REPOSITIONED IN ORDER TO MEET THE MINIMUM CLEARANCES SHOWN IN THE BEARING ORIENTATION DETAIL. DUE TO THE DEFORMED ANCHOR BOLTS, ONE THE FOLLOWING MEANS MAY BE REQUIRED TO ACCOMPLISH THIS TASK:

1. DETACH THE EXISTING SOLE PLATE FROM THE GIRDER AND REPOSITION THE SOLE PLATE.

### 2. REPOSITIONING THE BEARING PAD.

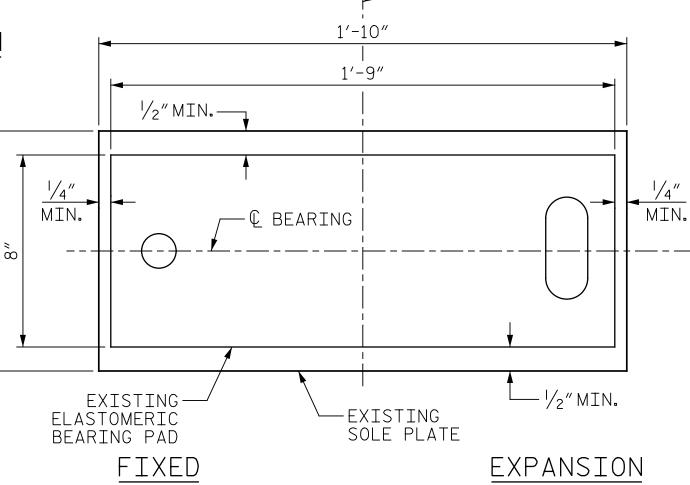
THE CONTRACTOR MAY PROPOSE ALTERNATE METHODS, PROVIDED DETAILS ARE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.

WHEN FIELD WELDING THE SOLE PLATE TO THE GIRDER FLANGE, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

— Ų BEAM

FOR REPOSITIONING BEARINGS, SEE SPECIAL PROVISIONS.

SOLE PLATES AND BEARING PADS SHALL BE REPOSITIONED BEFORE ANY JOINT REPAIRS OR GIRDER END PAINTING BEGINS.



## BEARING ORIENTATION DETAIL

(SHOWING MINIMUM REQUIRED CLEARANCES)

PROJECT NO. I-5955A GUILFORD \_\_\_ COUNTY 400367 BRIDGE NO. \_\_\_\_



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

> BEARING REPOSITIONING DETAILS

REVISIONS DATE: DATE:



(R9)

BEARING PHOTOGRAPHS

(FOR BEARING LOCATIONS, SEE "BEARING REPOSITIONING LOCATIONS" SHEET)

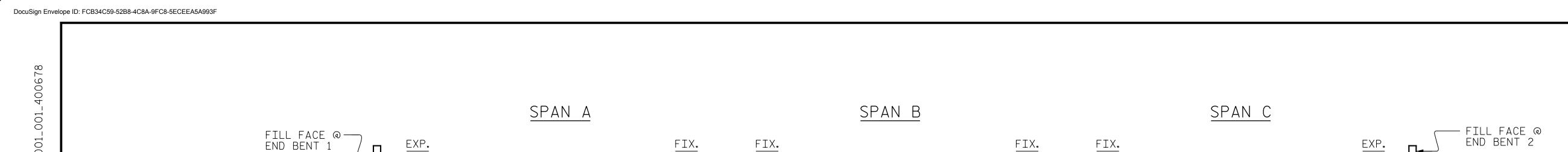
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\_ DATE : <u>10/2022</u> \_ DATE : <u>10/2022</u> J. YANNACCONE

(R4)



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BENT 3

NOTE:

GENERAL DRAWING INFORMATION IS TAKEN FROM THE ORIGINAL PLANS AND THE ROUTINE INSPECTION REPORT DATED 07/12/2022.

BRIDGE ORIENTATION CONFORMS TO THE EXISTING BRIDGE PLANS AND ROUTINE INSPECTION REPORT.

### SCOPE OF WORK

- PARTIALLY REMOVE TOP OF BRIDGE DECK CONCRETE BY FINE MILLING AND SHOTBLASTING METHODS.
- OVERLAY PREPARED TOP OF BRIDGE DECK WITH POLYMER CONCRETE (PC).
- REPLACE EXISTING JOINT GLAND OF EXPANSION JOINT SEALS.
- GROOVE PC BRIDGE DECK.
- REMOVE DEBRIS FROM TOP OF EXISTING END BENT CAPS AND APPLY EPOXY COATING.

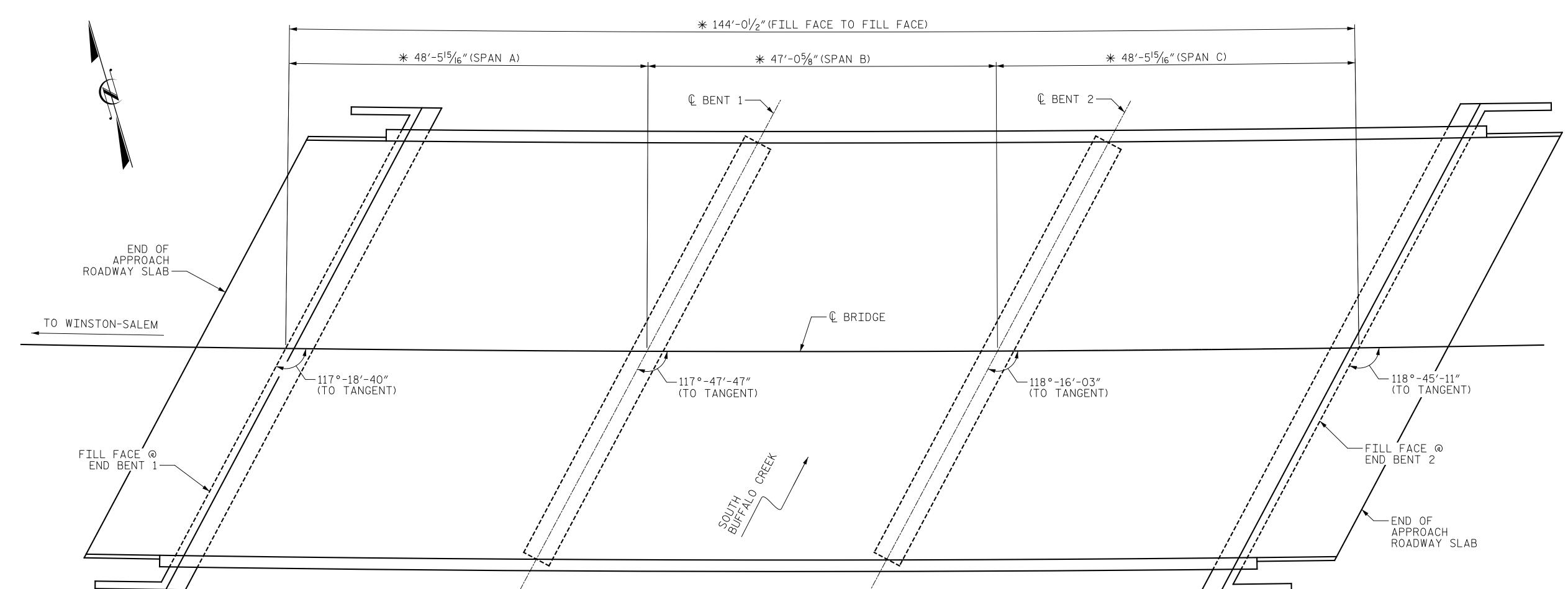
- REMOVE VEGETATION FROM STRUCTURE.

## SECTION ALONG & BRIDGE (SECTION AT BENTS AND END BENTS ARE AT RIGHT ANGLES)

BENT 2

APPROX. WATER —

SURFACE ELEV.



PROJECT NO. I-5955A GUILFORD \_\_\_ COUNTY

BRIDGE NO. \_\_\_\_400678

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING FOR BRIDGE ON I-40 WBL OVER SOUTH BUFFALO CREEK

GANNETT One Glenwood Avenue Suite 900
Ralelgh, NC 27603
919-420-7660
NC Lic. No. F-0270

END BENT 2

SHEET NO REVISIONS DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED S14-1 BY: DATE: DATE:

### PLAN (PILES NOT SHOWN FOR CLARITY)

I HEREBY CERTIFY THAT THIS STRUCTURE WAS REHABILITATED ACCORDING TO THESE PLANS OR AS NOTED HEREIN.

APPROXIMATE EXISTING — GROUND LINE

END BENT 1

DATE RESIDENT ENGINEER

\_ DATE : <u>10/2022</u> J. MYA J. YANNACCONE \_ DATE : <u>1/2024</u> CHECKED BY : \_\_\_\_\_

\*DIMENSIONS MEASURED ALONG ARC



## LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAY, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE CO	ORDINATES		
LATITUDE	LONGITUDE		
36°-01′-57.88′′	79°-48′-48.77′′		

## GENERAL NOTES

SEE TRANSPORTATION MANAGEMENT PLANS FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND POLYMER CONCRETE PLACEMENT.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT DUE TO THE NATURE OF PRESERVATION PROJECTS, THE EXTENT OF WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. REPAIR LOCATIONS AND ESTIMATES OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIR.

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN WHAT IS SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, EXCEPT WHERE THE CONTRACTOR'S PLAN USED PLATFORMS, NETS, SCREEN OR OTHER PROTECTIVE DEVICES TO CATCH THE MATERIAL. THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS. ANY DAMAGE TO EXISTING REINFORCING STEEL DURING CONTRACTOR'S OPERATIONS SHALL BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST TO THE DEPARTMENT.

FOR CONTROL OF TRAFFIC AND LIMITS OF PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLANS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ALL PAVEMENT MARKING WILL BE IN ACCORDANCE WITH THE PAVEMENT MARKING NOTES.

EXISTING JOINTS AND DECK DRAINS SHALL BE SEALED PRIOR TO BEGINNING SURFACE PREPARATIONS OF THE BRIDGE DECK. THE CONTRACTOR SHALL TAKE CARE THAT ANY CONSTRUCTION DEBRIS THAT COLLECTS IN THE DRAINS IS CONTAINED. DRAINS IN SHOULDERS OF ADJACENT TRAVEL LANES SHALL BE KEPT FREE AND CLEAR OF DEBRIS.

LONGITUDINAL CONSTRUCTION JOINTS OF OVERLAYS SHALL BE LOCATED ALONG THE CENTERLINE OR EDGE OF TRAVEL LANES.

FOR SHOTBLASTING BRIDGE DECK AND CLASS II SURFACE PREPARATION, SEE OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE SPECIAL PROVISION.

FOR FINE MILLING, SEE SPECIAL PROVISIONS

FOR CONCRETE DECK REPAIR FOR PC OVERLAYS, PC MATERIALS, AND PLACING AND FINISHING PC OVERLAY, SEE POLYMER CONCRETE BRIDGE DECK OVERLAY SPECIAL PROVISION.

FOR EXPANSION JOINT SEAL REPAIR, SEE SPECIAL PROVISIONS.

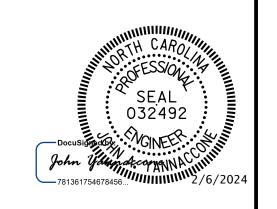
FOR EPOXY COATING AND DEBRIS REMOVAL, SEE SPECIAL PROVISIONS.

PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400678

SHEET 2 OF 2



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

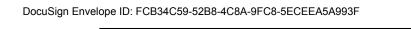
RALEIGH

GENERAL DRAWING FOR BRIDGE ON I-40 WBL OVER SOUTH BUFFALO CREEK



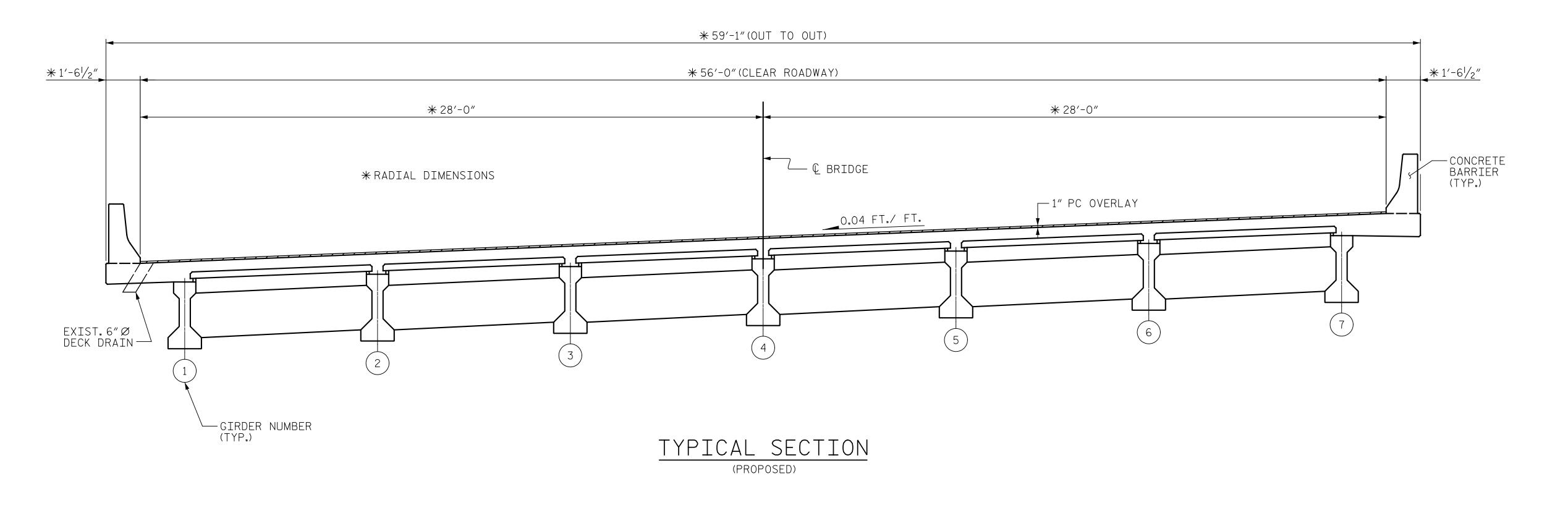
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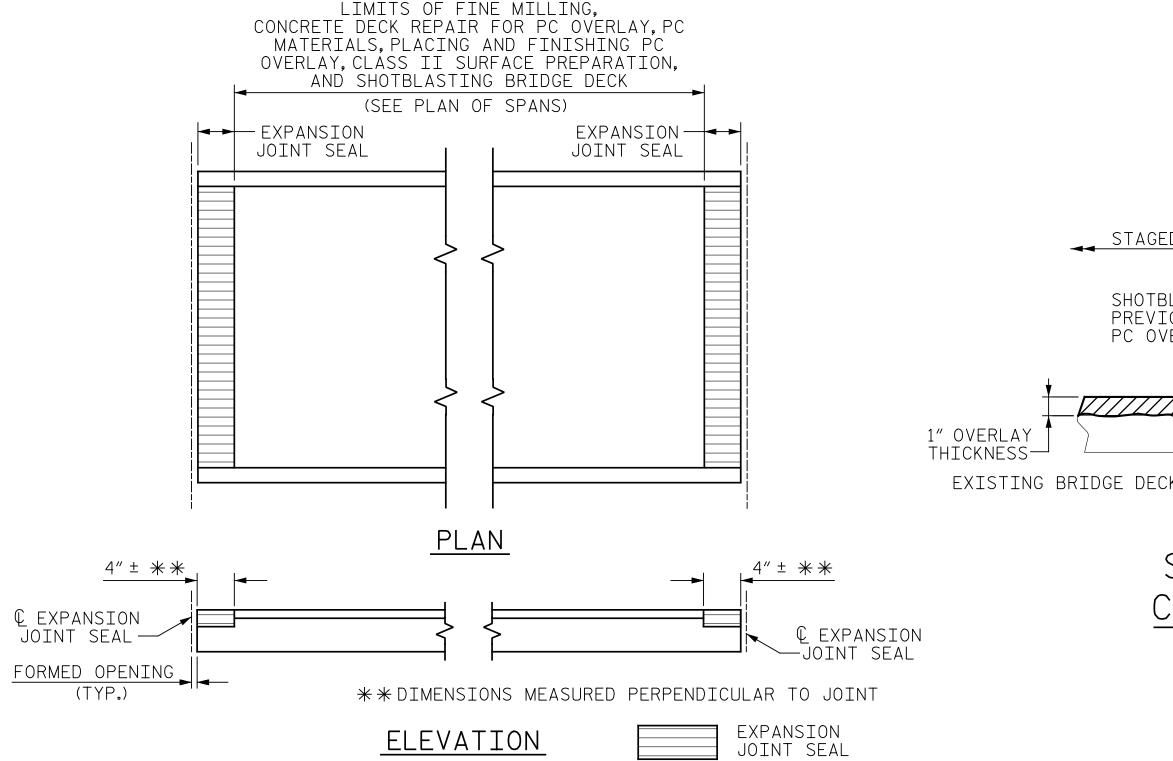
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### NOTES:

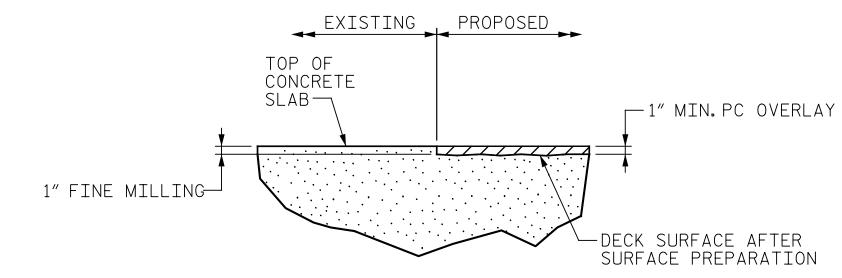
SEE TRANSPORTATION MANAGEMENT PLANS FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND PC PLACEMENT.





STAGED PC OVERLAY PREVIOUSLY PLACED PC - LOCATION OF LONGITUDINAL JOINT SHOTBLAST EDGE OF PREVIOUSLY PLACED PC OVERLAY — DECK SURFACE BEFORE OVERLAY PLACEMENT EXISTING BRIDGE DECK —

STAGED PC OVERLAY



DETAIL OF POLYMER CONCRETE OVERLAY

PROJECT NO. I-5955A GUILFORD \_ COUNTY 400678 BRIDGE NO. \_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TYPICAL SECTION AND SURFACE PREPARATION DETAILS



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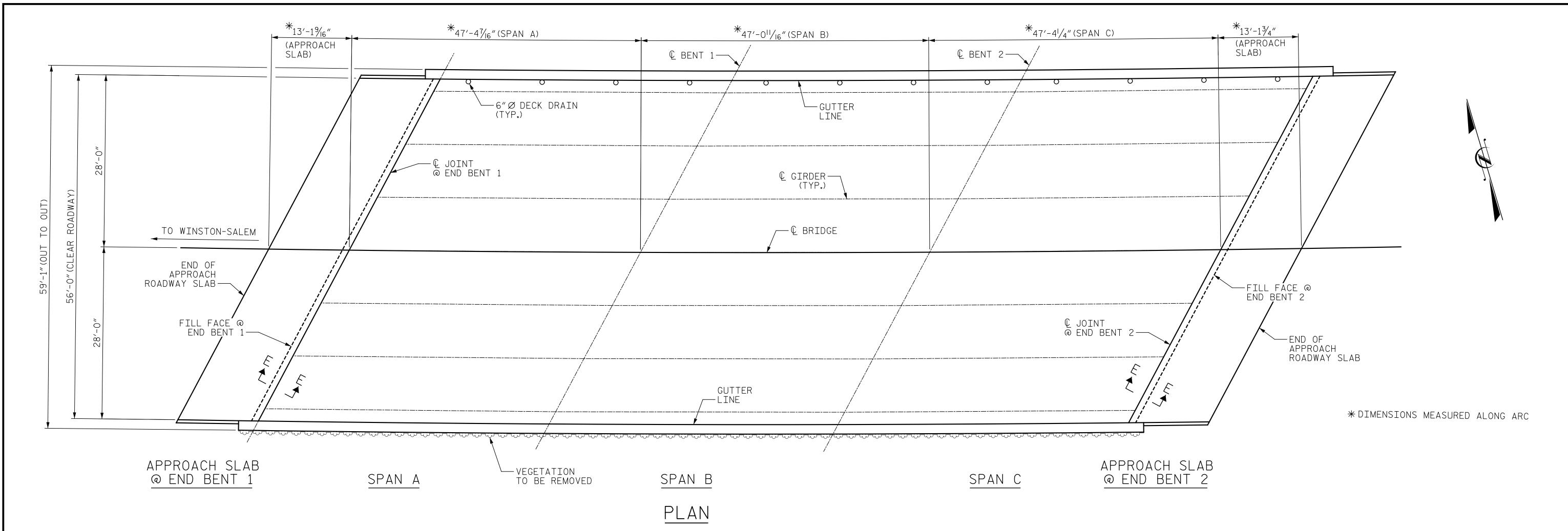
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PAY LIMITS FOR OVERLAY BID ITEMS

J. HARRIS \_ DATE : <u>10/2022</u> DRAWN BY : J. YANNACCONE \_ DATE : <u>10/2022</u> CHECKED BY : \_\_

CONSTRUCTION JOINT

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		REPAI	R QUAN	NTITY TABLE					
T	OP OF DECK RE	EPAIR		UNDERSI	DE OF	DECK	REPAI	R	
		ESTIMATE	ACTUAL				IMATE		TUAL
	APPR.SLAB @ EB1	75.7 SY		SHOTCRETE REP <i>A</i>	AIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
	SPAN A	291.5 SY			SPAN A	0.0	0.0		
FINE MILLING	SPAN B	292.8 SY		UNDERSIDE OF DECK	SPAN B	0.0	0.0		
	SPAN C	291.4 SY			SPAN C	0.0	0.0		
	APPR.SLAB @ EB2	75.7 SY			SPAN A	0.0	0.0		
	APPR.SLAB @ EB1	0.0 SY		OVERHANG DIAPHRAGMS	SPAN B	0.0	0.0		
	SPAN A	0.0 SY			SPAN C	0.0	0.0		
CLASS II SURFACE PREPARATION	SPAN B	0.0 SY			SPAN A	0.0	0.0		
	SPAN C	0.0 SY		UNDERSIDE OF OVERHANG	SPAN B	0.0	0.0		
	APPR.SLAB @ EB2	0.0 SY			SPAN C	0.0	0.0		
	APPR.SLAB @ EB1	2.6 CY			SPAN A	0.0	0.0		
	SPAN A	10.1 CY		INTERIOR DIAPHRAGMS	SPAN B	0.0	0.0		
PC MATERIALS	SPAN B	10.2 CY			SPAN C	0.0	0.0		
	SPAN C	10.1 CY				EST	ESTIMATE		TUAL
	APPR.SLAB @ EB2	2.6 CY			SPAN A	0.	) LF		
	APPR.SLAB @ EB1	75.7 SY		UNDERSIDE EPOXY RESIN INJECTION	SPAN B	0.	) LF		
DI ACTNO AND	SPAN A	291.5 SY			SPAN C	0.0 LF			
PLACING AND FINSIHING	SPAN B	292.8 SY							
PC OVERLAY	SPAN C	291.4 SY							
	APPR. SLAB @ EB2	75.7 SY							
				•					

APPR. SLAB @ EB1

APPR. SLAB @ EB2

SPAN A

SPAN B

SPAN C

GROOVING BRIDGE

FLOORS

625 SF

2473 SF

2494 SF

2472 SF

625 SF

### NOTES:

REPAIR LOCATIONS AND ESTIMATED QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER WILL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATIONS AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

PAYMENT FOR CLASS II SURFACE PREPARATION IS BASED UPON SQUARE YARDS OF ADDITIONAL DEMOLITION REQUIRED FOLLOWING FINE MILLING OF BRIDGE DECK, SEE "OVERLAY SURFACE PREPARATION FOR PC OVERLAY" SPECIAL PROVISION.

CONCRETE COVER FOR TOP BARS IN THE DECK SLAB IS  $1\frac{1}{2}$  PER THE EXISTING BRIDGE PLANS.

FOR SECTION E-E, SEE "EXPANSION JOINT SEAL DETAILS" SHEET.

FOR REMOVAL OF VEGETATION, SEE EPOXY COATING AND DEBRIS REMOVAL SPECIAL PROVISION.

FINE MILLING AND SHOTBLASTING OF BRIDGE DECK

CLASS II SURFACE PREPARATION

GUILFORD \_ COUNTY

UNDERSIDE OF DECK REPAIR

400678 BRIDGE NO. \_\_\_\_

PROJECT NO. I-5955A



DEPARTMENT OF TRANSPORTATION RALEIGH

STATE OF NORTH CAROLINA

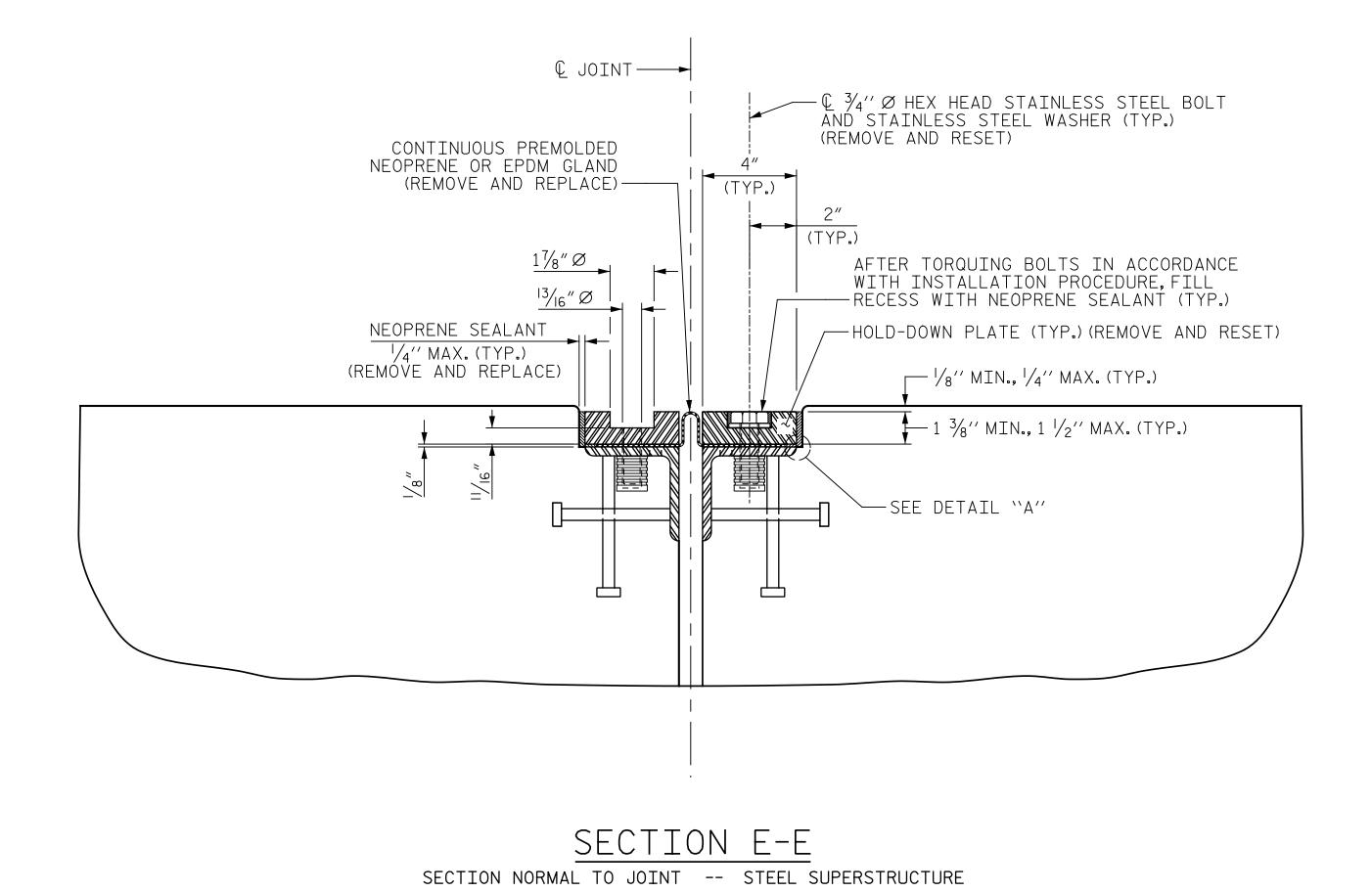
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\_ DATE : <u>10/2022</u> \_ DATE : <u>10/2022</u>



## REPAIR INSTALLATION PROCEDURE

LOOSEN THE EXISTING BOLTS AND HOLD-DOWN PLATES TO REMOVE AND REPLACE THE EXISTING GLAND. REMOVE THE EXISTING NEOPRENE SEALANT AND CLEAN THE EXISTING BASE ANGLE OF OIL, GREASE AND OTHER LATENTS.

LAY THE NEW GLAND ON THE BASE ANGLE AND FIELD MARK THE NEW GLAND FOR THE BOLT HOLES. HOLES IN THE NEW GLAND SHALL BE PUNCHED 1/8" IN DIAMETER WITH A HAND PUNCH.

IN ORDER TO CHECK FOR PROPER ALIGNMENT, PLACE THE NEW GLAND HOLD-DOWN PLATES ON THE BASE ANGLE, DO NOT APPLY NEW NEOPRENE SEALANT, BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE, BUT DO NOT TIGHTEN, THE ENGINEER WILL INSPECT THE JOINT SEAL DEVICE FOR PROPER ALIGNMENT.

AFTER INSPECTION, REMOVE THE HOLD-DOWN PLATES AND NEW GLAND. APPLY NEW NEOPRENE SEALANT TO THE BASE ANGLE IN ACCORDANCE WITH THE "INSTALLATION SKETCH". PLACE NEW GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE ASSEMBLY AND TORQUE THE BOLTS TO 88 FT-LBS WITH A TORQUE WRENCH. CHECK THE TORQUE AFTER THREE (3) HOURS AND, IF NECESSARY, RETIGHTEN TO 88 FT-LBS. A FINAL CHECK SHALL BE MADE AT SEVEN (7) DAYS. TORQUE SHALL NOT BE LESS THAN 80 FT-LBS AFTER SEVEN (7) DAYS.

AFTER PROPER TORQUING, CLEAN THE BOLT HOLE RECESSES AND THE RECESS BETWEEN THE JOINT SEAL DEVICE AND CONCRETE. COMPLETELY FILL THESE RECESSES WITH NEW NEOPRENE SEALANT.

## GENERAL NOTES

ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°. FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.

THE FINISHED EXPANSION SEAL DEVICE SHALL BE A MINIMUM 1/8" AND A MAXIMUM OF 1/4" BELOW THE TOP OF SLAB.

FOR EXPANSION JOINT SEAL REPAIR, SEE SPECIAL PROVISIONS.

NO SEPARATE PAYMENT WILL BE MADE FOR REMOVING AND REINSTALLING MEDIAN AND BARRIER RAIL COVER PLATES. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR "EXPANSION JOINT SEAL REPAIR".

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE END BENT CAPS AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAPS. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAPS BENEATH THE ELASTOMERIC BEARINGS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

MOVEMENT AND SETTING AT JOINT							
LOCATION	SKEW ANGLE	TOTAL MOVEMENT (ALONG (LEDWY)	PERPENDICULAR JOINT OPENING AT 45° F	PERPENDICULAR JOINT OPENING AT 60° F	PERPENDICULAR JOINT OPENING AT 90° F		
END BENT 1	117°-18′-40″	7/16″	13/16″	3/4"	9/16″		
END BENT 2	118°-45′-11″	7/16″	23/16"	21/8"	1 <sup>15</sup> / <sub>16</sub> "		

SUMMARY OF QUANTITIES							
LOCATION		JOINT SEAL AIRS	EPOXY COATING				
LOCATION	ESTIMATED (LIN. FT)	ACTUAL (LIN.FT)	ESTIMATED (SQ.FT)	ACTUAL (SQ.FT)			
END BENT 1	65.0		164				
END BENT 2	65.0		164				

PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400678



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

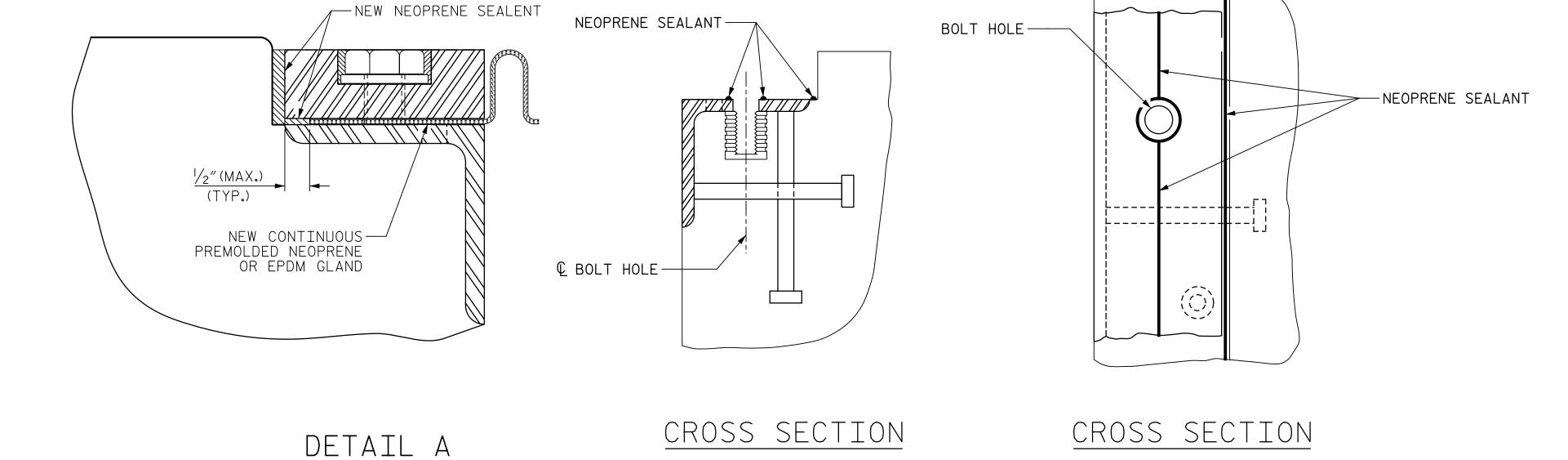
RALEIGH

EXPANSION JOINT SEAL DETAILS

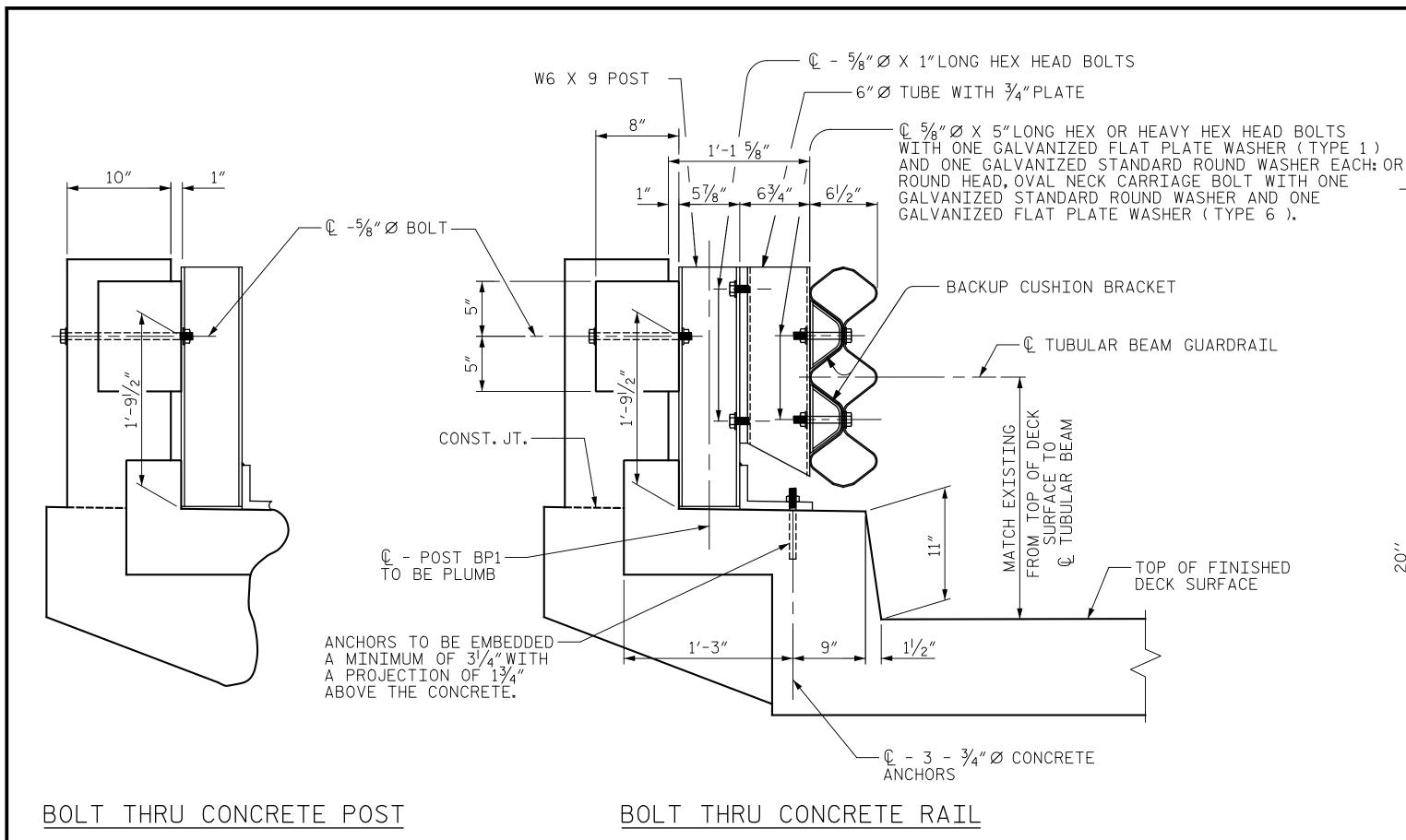
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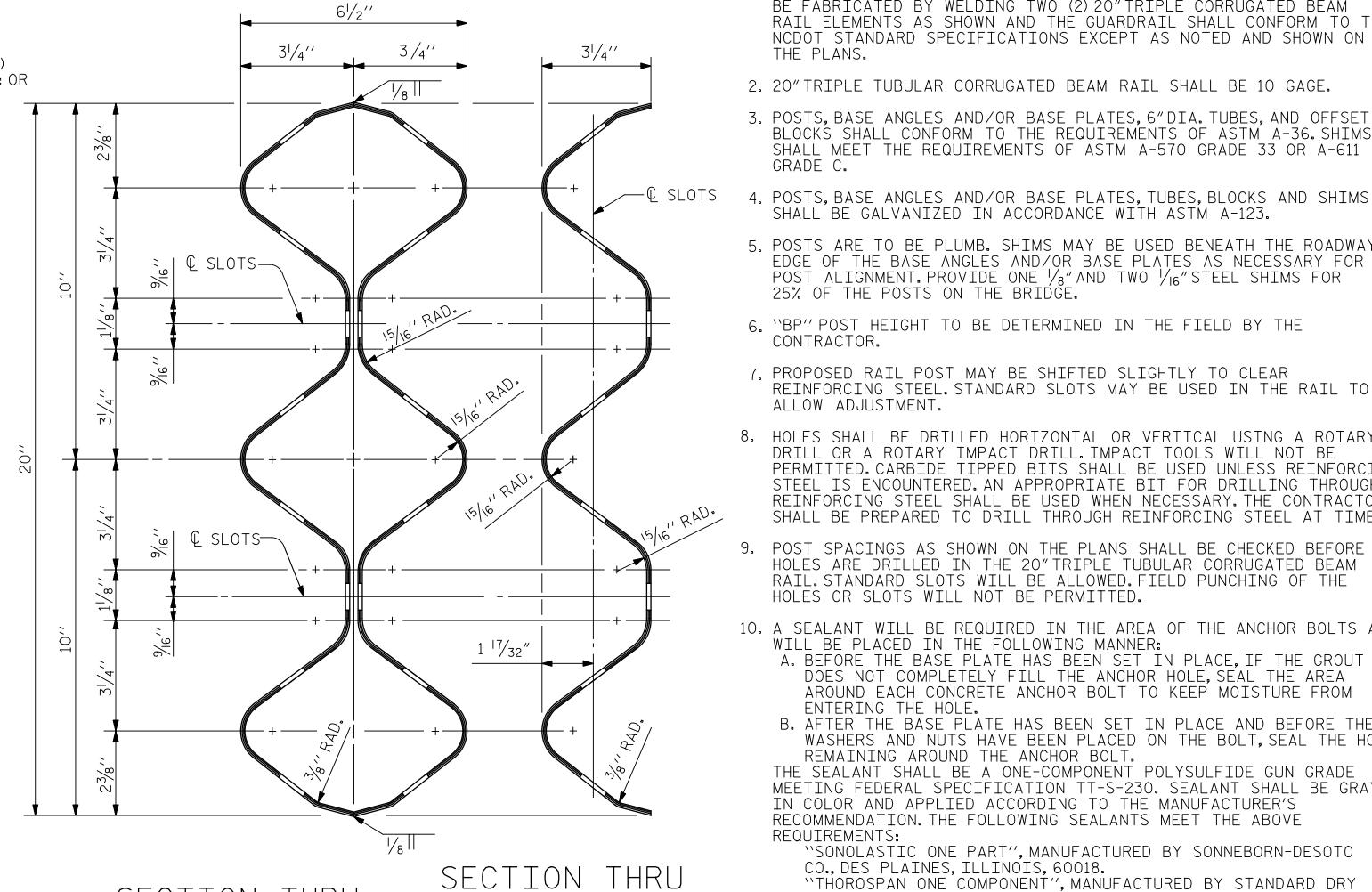
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INSTALLATION SKETCH



RETROFIT EXISTING RAIL WITH TUBULAR BEAM GUARDRAIL WITHOUT WEARING SURFACE )



SECTION THRU TUBULAR BEAM 20" TRIPLE

### CONCRETE ANCHOR NOTES:

- 1. FOR ADHESIVELY ANCHORED BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. A) THE  $\frac{3}{4}$ " DIAMETER ANCHOR BOLTS SHALL BÉ TESTED USING LEVEL 2 FIELD TESTING AS SHOWN IN THE STANDARD SPECIFICATIONS.
- THE YIELD LOAD OF THE 3/4"DIAMETER ANCHOR IS 10 KIPS. B) THE SUCCESSFULLY TESTED ANCHOR MAY BE USED IN THE FINAL RAIL ASSEMBLY, IF APPROPRIATELY LOCATED. IF NOT SO LOCATED. OR IF THE ANCHOR FAILS THE TEST, THE TEST AREA SHALL BE REPAIRED AS DAMAGED CONCRETE, SEE "GENERAL NOTES".
- 2. EMBEDMENT SHOWN ON THE PLANS IS A MINIMUM. BUT THE MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.
- 3. THE  $\frac{3}{4}$ " DIAMETER CONCRETE ANCHOR SHALL CONSIST OF A STUD, THREADED ON ONE END. WITH NUT AND WASHERS, THE ANCHOR SHALL BE GALVANIZED TO CONFORM TO THE REQUIREMENTS OF ASTM A-153.
- 4. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL ANCHORS MAY BE USED AS AN ALTERNATE FOR THE GALVANIZED CONCRETE ANCHORS. THEY SHALL MEET OR EXCEED THE MECHANICAL REQUIREMENTS FOR THE GALVANIZED ANCHORS. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- 5. EXPANSION ANCHORS WILL NOT BE PERMITTED.
- 6. FOR ANCHOR BOLTS, SEE STANDARD SPECIFICATIONS.

### NOTES:

- TUBULAR BEAM POSTS ARE TO BE MOUNTED AGAINST THE EXISTING CONCRETE RAIL.
- HOLES FOR THE 5/8" DIAMETER BOLTS, THRU THE EXISTING CONCRETE RAIL OR POST, SHALL BE  $\frac{3}{4}$ " DIAMETER.
- $\frac{3}{4}$ " and  $\frac{5}{8}$ " diameter bolts shall conform to the requirements of astm a-307 AND SHALL BE GALVANIZED TO CONFORM TO THE REQUIREMENTS OF ASTM A-153

# One Glenwood Avenue

Raleigh, NC 27603 919-420-7660 NC L1c.No. F-0270

# Suite 900

### REINFORCING STEEL. STANDARD SLOTS MAY BE USED IN THE RAIL TO ALLOW ADJUSTMENT. HOLES SHALL BE DRILLED HORIZONTAL OR VERTICAL USING A ROTARY DRILL OR A ROTARY IMPACT DRILL. IMPACT TOOLS WILL NOT BE PERMITTED. CARBIDE TIPPED BITS SHALL BE USED UNLESS REINFORCING

REINFORCING STEEL SHALL BE USED WHEN NECESSARY. THE CONTRACTOR SHALL BE PREPARED TO DRILL THROUGH REINFORCING STEEL AT TIMES. POST SPACINGS AS SHOWN ON THE PLANS SHALL BE CHECKED BEFORE HOLES ARE DRILLED IN THE 20"TRIPLE TUBULAR CORRUGATED BEAM RAIL. STANDARD SLOTS WILL BE ALLOWED. FIELD PUNCHING OF THE

STEEL IS ENCOUNTERED. AN APPROPRIATE BIT FOR DRILLING THROUGH

1. THE 20"TRIPLE TUBULAR CORRUGATED BEAM RAIL SECTION SHALL

BE FABRICATED BY WELDING TWO (2) 20" TRIPLE CORRUGATED BEAM

3. POSTS, BASE ANGLES AND/OR BASE PLATES, 6"DIA. TUBES, AND OFFSET

SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A-123.

6. "BP" POST HEIGHT TO BE DETERMINED IN THE FIELD BY THE

7. PROPOSED RAIL POST MAY BE SHIFTED SLIGHTLY TO CLEAR

25% OF THE POSTS ON THE BRIDGE.

HOLES OR SLOTS WILL NOT BE PERMITTED.

BLOCKS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-36. SHIMS

SHALL MEET THE REQUIREMENTS OF ASTM A-570 GRADE 33 OR A-611

5. POSTS ARE TO BE PLUMB, SHIMS MAY BE USED BENEATH THE ROADWAY

EDGE OF THE BASE ANGLES AND/OR BASE PLATES AS NECESSARY FOR POST ALIGNMENT. PROVIDE ONE  $\frac{1}{8}$  AND TWO  $\frac{1}{16}$  STEEL SHIMS FOR

RAIL ELEMENTS AS SHOWN AND THE GUARDRAIL SHALL CONFORM TO THE NCDOT STANDARD SPECIFICATIONS EXCEPT AS NOTED AND SHOWN ON

GENERAL NOTES:

GRADE C.

CONTRACTOR.

- 10. A SEALANT WILL BE REQUIRED IN THE AREA OF THE ANCHOR BOLTS AND WILL BE PLACED IN THE FOLLOWING MANNER: A. BEFORE THE BASE PLATE HAS BEEN SET IN PLACE, IF THE GROUT DOES NOT COMPLETELY FILL THE ANCHOR HOLE, SEAL THE AREA AROUND EACH CONCRETE ANCHOR BOLT TO KEEP MOISTURE FROM ENTERING THE HOLE.
  - B. AFTER THE BASE PLATE HAS BEEN SET IN PLACE AND BEFORE THE WASHERS AND NUTS HAVE BEEN PLACED ON THE BOLT, SEAL THE HOLE REMAINING AROUND THE ANCHOR BOLT. THE SEALANT SHALL BE A ONE-COMPONENT POLYSULFIDE GUN GRADE

MEETING FEDERAL SPECIFICATION TT-S-230, SEALANT SHALL BE GRAY IN COLOR AND APPLIED ACCORDING TO THE MANUFACTURER'S RECOMMENDATION. THE FOLLOWING SEALANTS MEET THE ABOVE REQUIREMENTS:

"SONOLASTIC ONE PART", MANUFACTURED BY SONNEBORN-DESOTO CO., DES PLAINES, ILLINOIS, 60018. "THOROSPAN ONE COMPONENT", MANUFACTURED BY STANDARD DRY WALL PRODUCTS, INC., MIAMI, FLORIDA, 33166. "HORNFLEX ONE COMPONENT". MANUFACTURED BY W.R. GRACE AND

CO., CAMBRIDGE, MASSACHUSETTS, 02140. CORRUGATED BEAM 11. ALL CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE

- SATISFACTION OF THE ENGINEER.
- 12. VERTICAL SLOTS IN THE 6" TUBE ALLOW FOR SOME VERTICAL ADJUSTMENT OF RAIL HEIGHT IN ORDER TO OBTAIN THE CENTERLINE OF RAIL HEIGHT OF 2'-1" ABOVE RIDING SURFACE.
- 13. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES". ELECTROSLAG WELDING WILL NOT BE PERMITTED.
- 14. LAP BEAM RAIL JOINTS IN DIRECTION OF TRAFFIC.

OFESSION, SEAL

032492

NGINEER.

ACCOMPINA!

15. THE EXISTING DIMENSIONS AND BRIDGE CONDITIONS ARE FROM THE BEST INFORMATION AVAILABLE, PRIOR TO FABRICATION OF THE RAIL SYSTEM, THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

> I-5955A PROJECT NO.\_

> > GUILFORD COUNTY

BRIDGE NO. 400299, 400339 & 400340

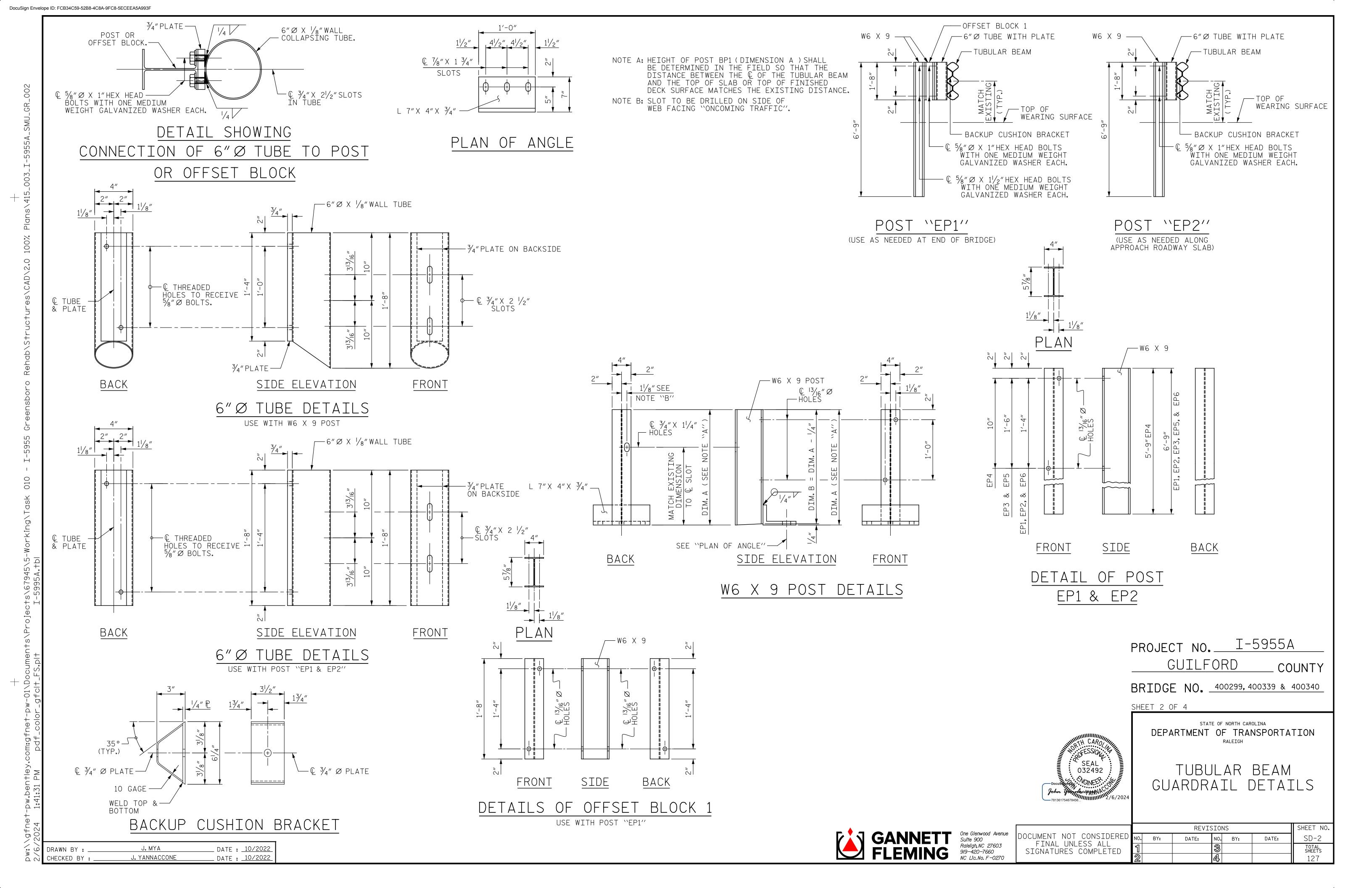
SHEET 1 OF 4

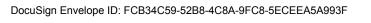
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

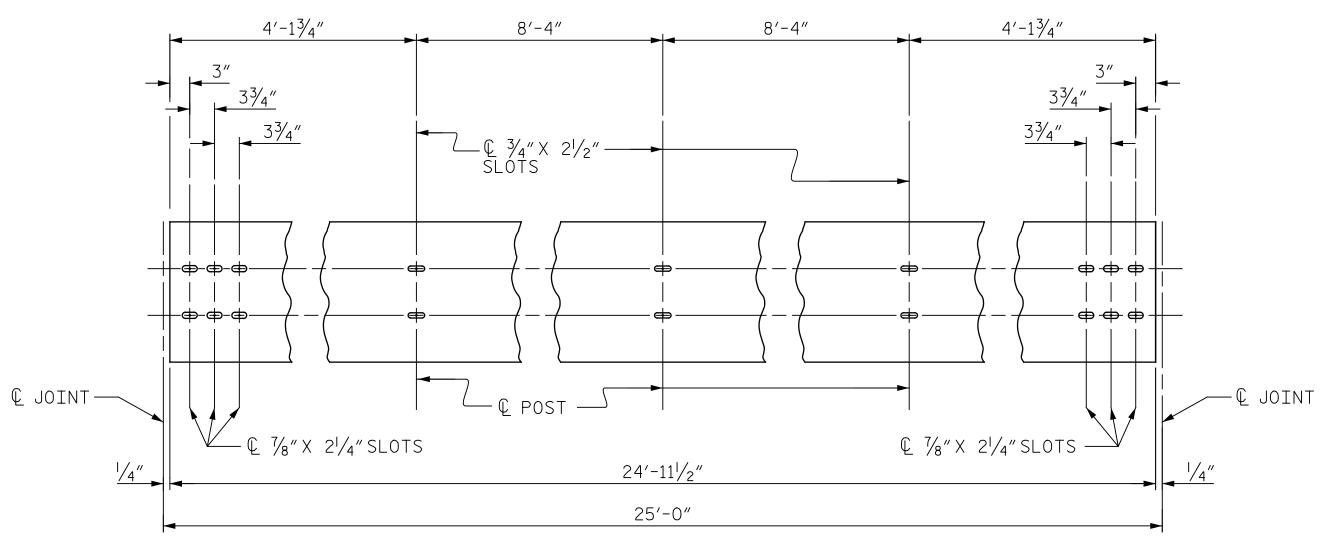
TUBULAR BEAM GUARDRAIL DETAILS

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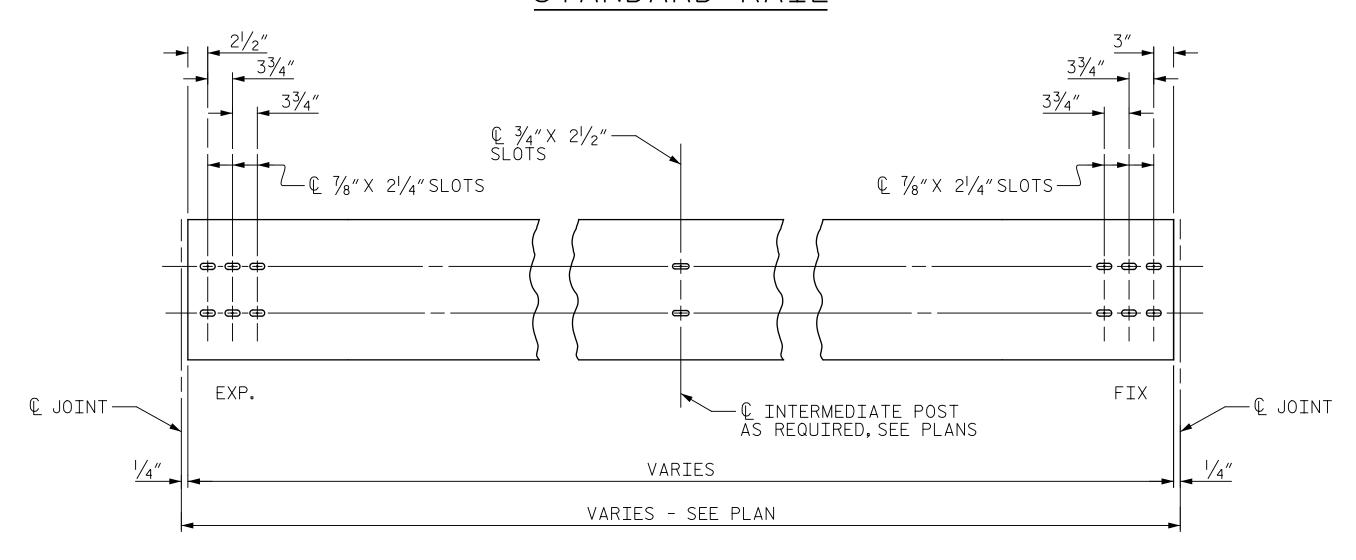
J. MYA DATE : 10/2022 DRAWN BY : DATE : 10/2022 J. YANNACCONE CHECKED BY : .







## ELEVATION TUBULAR BEAM STANDARD RAIL



ELEVATION TUBULAR BEAM EXPANSION RAIL FOR TYPE 1 SPLICE

> PROJECT NO. I-5955A GUILFORD \_ COUNTY

BRIDGE NO. 400299, 400339 & 400340

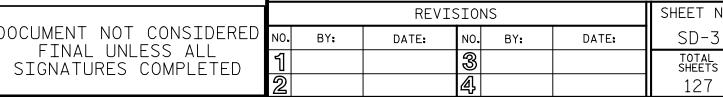
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

TUBULAR BEAM GUARDRAIL DETAILS



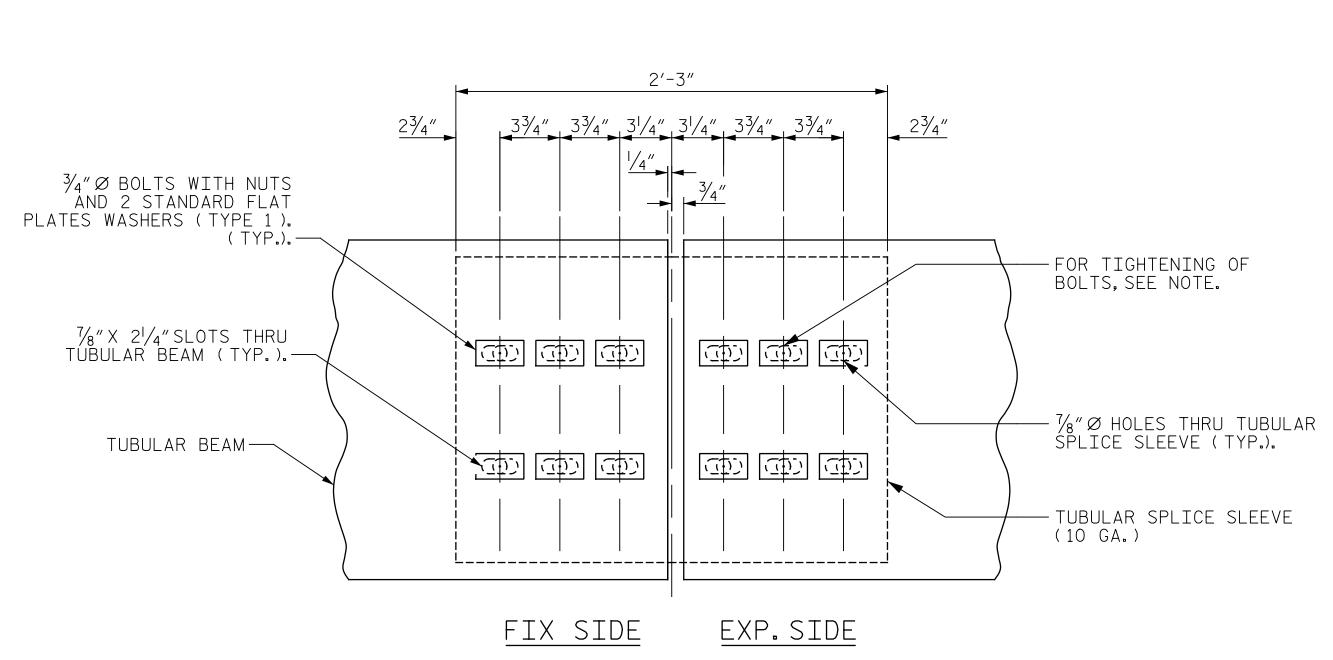




J. MYA \_ DATE : <u>10/2022</u> J. YANNACCONE \_ DATE : <u>10/2022</u> CHECKED BY : \_

## FIXED SPLICE BETWEEN POST (TYPE 1)

TUBULAR BEAM SPLICE

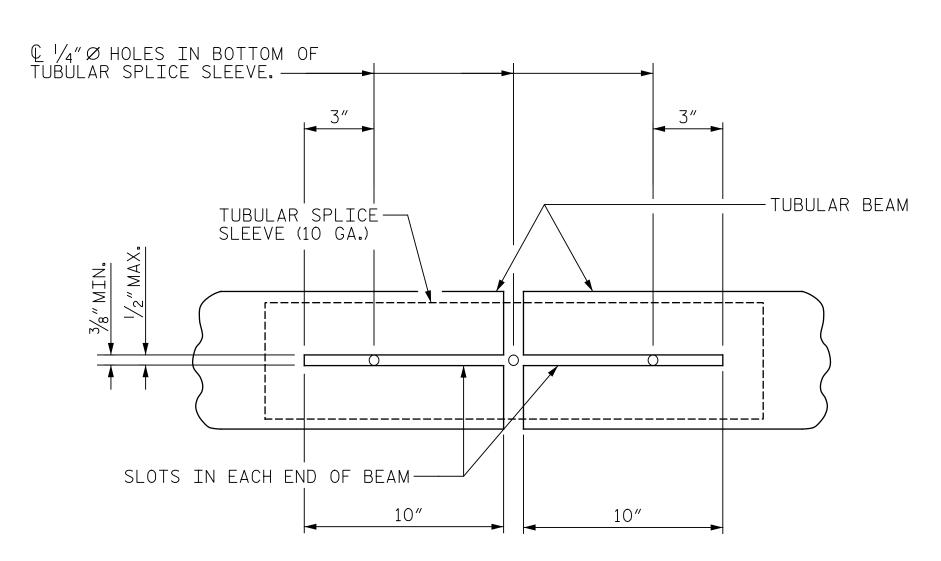


## EXPANSION SPLICE BETWEEN POST (TYPE 1)

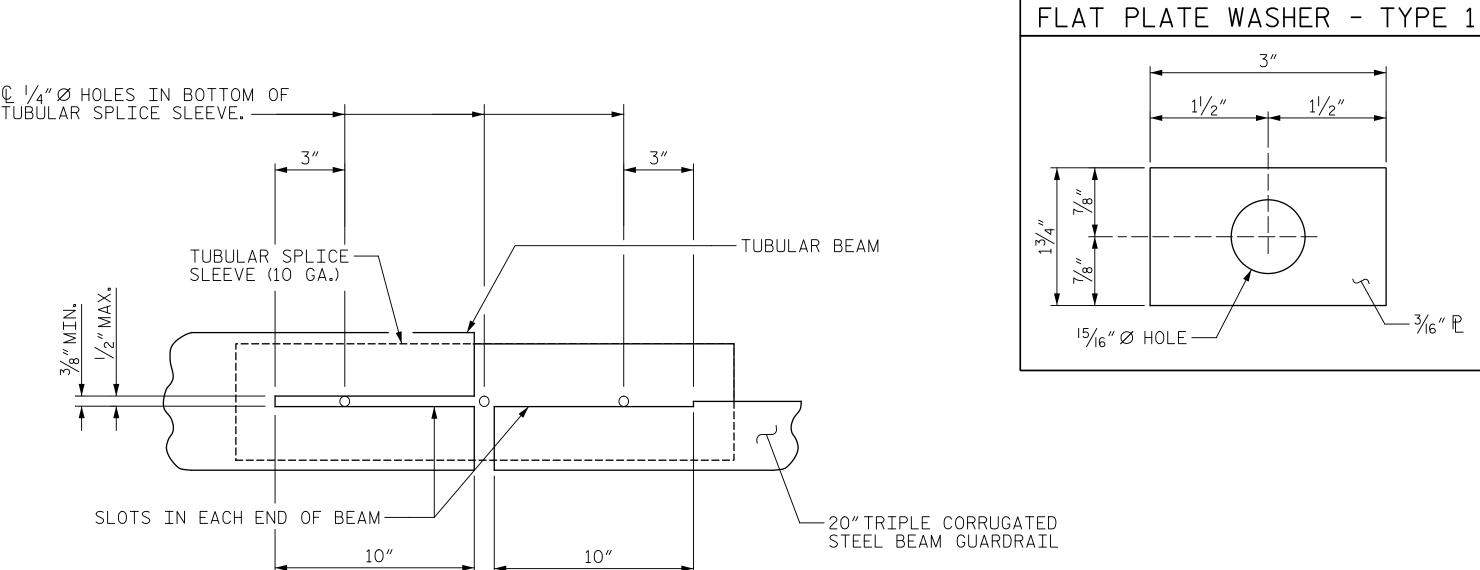
TUBULAR BEAM SPLICE

## NOTE:

BOLTS ON EXPANSION SIDE OF TUBULAR BEAM SPLICE SHALL BE TIGHTENED FINGER TIGHT. DOUBLE NUTS SHALL BE USED AND TIGHTENED AGAINST EACH OTHER TO PREVENT THE NUTS FROM BECOMING LOOSE ON THE BOLT.



BOTTOM VIEW OF TUBULAR BEAM SPLICE



## BOTTOM VIEW OF TUBULAR AND 20"TRIPLE CORRUGATED STEEL BEAM SPLICE

PROJECT NO. I-5955A GUILFORD COUNTY

BRIDGE NO. 400299, 400339 & 400340

SEAL 032492

<sup>15</sup>/<sub>16</sub>"∅ HOLE —

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TUBULAR BEAM GUARDRAIL DETAILS



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SHEET 4 OF 4

FLAT PLATE WASHER - TYPE 6

J. MYA \_ DATE : <u>10/2022</u> DRAWN BY : J. YANNACCONE \_ DATE : <u>10/2022</u> CHECKED BY : \_

# 1'' MIN.CL.BEHIND REBAR AND UNTIL SOUND CONCRETE 1/2" DEEP SAW\_CUT -IS FOUND (TYP.)— € GIRDER

TYPICAL SECTION

\* REMOVE CONCRETE UNTIL SOUND CONCRETE IS FOUND (1" MIN. DEPTH)

SECTION G=G

OVERHANG DETAILS

|/2" DEEP SAW CUT (TYP.)



\_ DATE : <u>10/2022</u>

\_ DATE : <u>10/2022</u>

J. MYA

J. YANNACCONE

DRAWN BY :

CHECKED BY : \_\_

SEAL 032492

# PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400285, 400299, 400329, 400339 & 400340

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> OVERHANG AND UNDERSIDE REPAIR DETAILS



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IC   Itc. No. F -0270	

NOTES

4 - 1/4" Ø TAPCON CONCRETE \
ANCHORS OR EQUIVALENT

-EXISTING CONCRETE

DIAPHRAGM

ELEVATION

#4 BARS¬

SECTION H-H

INTERMEDIATE DIAPHRAGM REPAIR

(BRIDGE #400299)

EXISTING —/ CONCRETE DIAPHRAGM

(TYP.)

2-#4 BARS —

(TYP.)

/-1/4"Ø TAPCON CONCRETE ANCHOR OR EQUIVALENT

CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT

CONTRACTOR SHALL SAW CUT TO A NOMINAL DEPTH OF  $\frac{1}{2}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED.

CONTRACTOR SHALL SAW CUT THE REPAIR AREAS SO THAT THE CORNERS ARE SQUARE AS INDICATED ON THE DETAILS.

DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS...

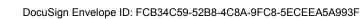
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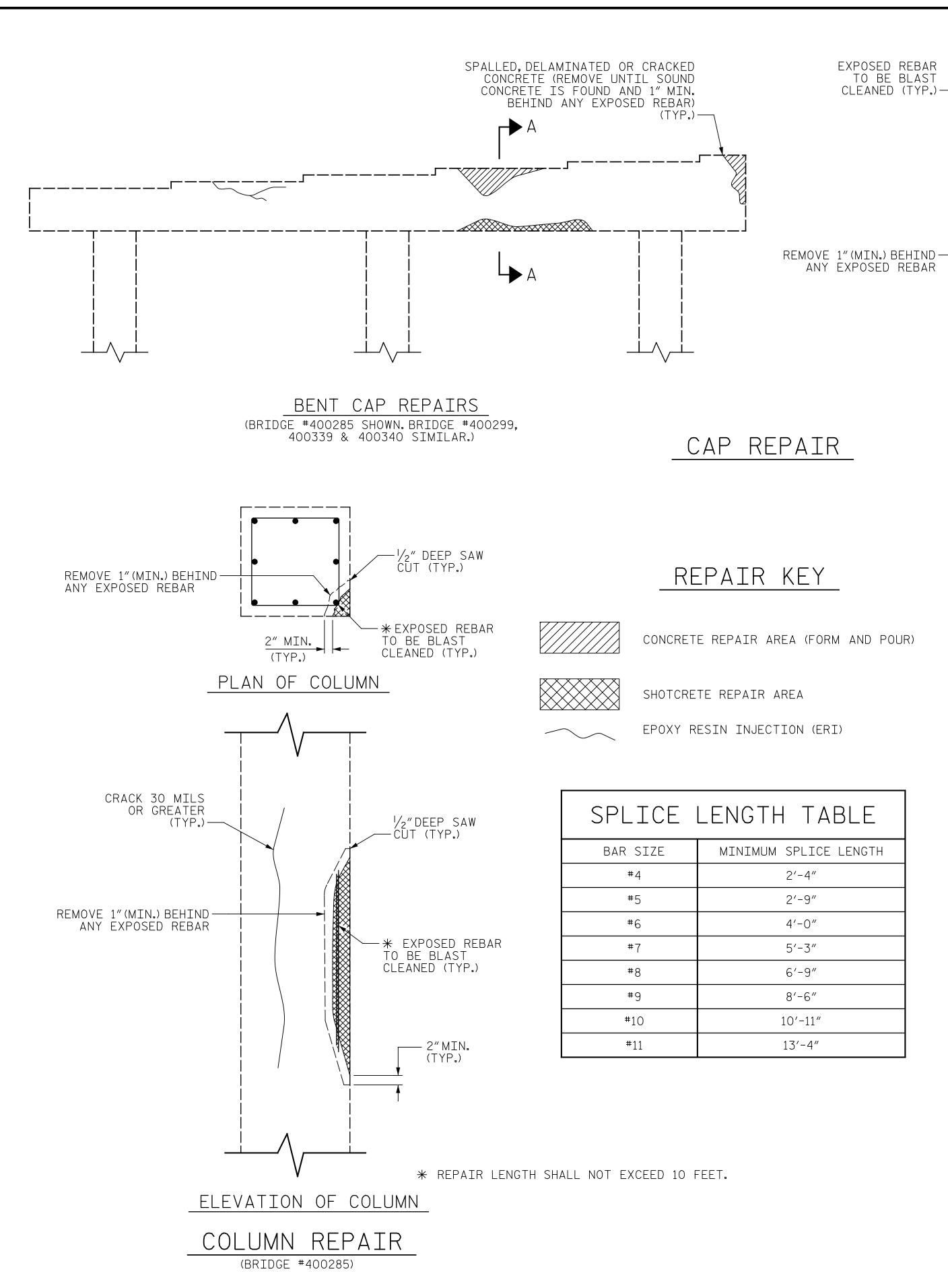
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DAMAGED AREA

EXISTING REBAR TO REMAIN IN PLACE. CLEAN AND REPAIR AS NECESSARY.





\_ DATE : <u>10/2022</u>

\_ DATE : <u>10/2022</u>

J. MYA

J. YANNACCONE

DRAWN BY

CHECKED BY : \_\_\_

NOTES:

1/2" DEEP SAW

CŪT (TYP.)

2"MIN.

(TYP.)

CRACK 30 MILS OR GREATER

(TYP.)—

SECTION A-A

——————

2" MIN.

PLAN OF COLUMN

ELEVATION OF COLUMN

COLUMN REPAIR

(BRIDGE #400299, 400339 & 400340)

-EXISTING HP14X73 AND WIRE MESH

1/2" DEEP SAW

1/2"DEEP SAW -CUT (TYP.)

> - 2″MIN. (TYP.)

(TYP.)

TYPICAL BENT CAP REPAIRS ARE SHOWN. REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1"BEHIND REBAR AND MINIMUM OF 2"CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CROSS SECTIONAL AREA SHALL BE REMOVED AT ONE TIME. SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR COLUMN, IF THE AREAS OF REMOVAL ARE NOT ADJACENT TO OR DIRECTLY OPPOSITE ONE ANOTHER. IF REMOVAL EXTENDS MORE THAN 1/2"BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

COAT ALL REPAIR SURFACE AREAS ON THE TOP OF CAPS, INCLUDING CHAMFERS, WITH EPOXY PROTECTIVE COATING, OVERLAPPING THE REPAIR AREA BY A MINIMUM OF 3"ON ALL POSSIBLE SIDES.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY PROTECTIVE COATING, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION (ERI), SEE SPECIAL PROVISIONS.

FOR THE REPAIRS ON THE PILE COLUMNS, ANCHOR PATCH MATERIAL USING  $\frac{1}{4}$ " GALVANIZED BOLTS. EPOXY ANCHORED WITH 2" EMBEDMENT. PLACE IN A 6" GRID.

PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400285, 400299, 400339 & 400340



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

TYPICAL CAP AND COLUMN REPAIR DETAILS

REVISIONS

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FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS

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GANNETT Sur FLEMING

One Glenwood Avenue
Suite 900
Raleigh, NC 27603
919-420-7660
NC Lic. No. F-0270

BRIDGE JACKING TABLE PRELIMINARY GIRDER REACTIONS (MAXIMUM) DEAD LOAD BRIDGE SPAN GIRDER BRIDGE NO. LOCATION JACKING (DC+DW)TYPE (KIPS) 22 400299 BENT 1 ALL 400299 В BENT 1 ALL 22 400299 В BENT 2 22 ALL 400299 С BENT 2 ALL 22 400329 53 В BENT 2 ALL 400329 BENT 2 43 ALL 400339 END BENT ALL 23 22 400340 END BENT ALL 400364 ALL 45 BENT 1 400364 В BENT 1 ALL 59 400364 BENT 3 ALL 59 400364 BENT 3 39 D ALL 400367 BENT 1 ALL 39 400367 BENT 1 ALL 59 В

BENT 3

BENT 3

ALL

ALL

59

40

NOTE: LOADS ARE UNFACTORED

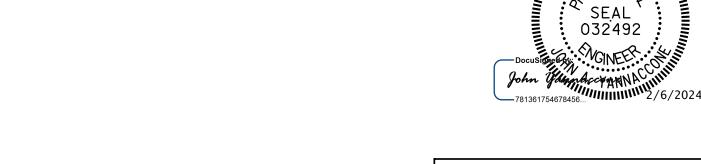
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SECTION THRU DIAPHRAGM

BRIDGE JACKING DETAILS

J. MYA DATE : 10/2022 DRAWN BY J. YANNACCONE DATE : 10/2022 CHECKED BY : \_



**FLEMING** 

PROJECT NO. I-5955A

GUILFORD COUNTY

BRIDGE NO. 400299, 400329, 400339 400340, 400364 & 400367

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

JACKING DETAILS

One Glenwood Avenue Suite 900 Raleigh, NC 27603 919-420-7660 OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

PROVISION.

DOCUMENTATION.

CAPABILITIES.

IMMEDIATELY.

OPERATIONS.

PROVISIONS.

SHALL REMAIN IN PLACE.

BEING JACKED IS 1/8".

THIS DETAIL IS A GENERIC EXAMPLE OF A JACKING SCHEME

PRIOR TO BRIDGE JACKING OPERATIONS, THE ENGINEER AND

MEMBERS. ALL NOTABLE DEFECTS SHALL BE DOCUMENTED AND REPORTED TO THE AREA BRIDGE MAINTENANCE ENGINEER PRIOR TO COMMENCEMENT OF ANY BRIDGE JACKING. THE CONTRACTOR

NOTABLE DEFECTS TO THE PRIMARY AND SECONDARY STRUCTURAL

STRUCTURAL MEMBERS FOR THE ENGINEER TO ESTABLISH PROPER

PRIOR TO JACKING, THE CONTRACTOR SHALL ENSURE THERE ARE

JACKING IS COMPLETE, THE CONTRACTOR SHALL PROVIDE FOR A

DEAD AND LIVE LOAD DURING THE REPAIR OPERATIONS. IF THE

IF. DURING THE JACKING PROCESS. OR WHILE THE BEAM IS BEING

SUPPORTED, THE BEAM SHIFTS FROM ITS ORIGINAL POSITION, ALL

METHOD TO REMOVE THE JACKS AND SUPPORT THE BEAM FOR

JACKS REMAIN IN PLACE DURING THE ENTIRE JACKING AND

REPAIR OPERATION, THEY SHALL HAVE MECHANICAL LOCK OFF

WORK SHALL CEASE AND THE ENGINEER SHALL BE NOTIFIED

BEARINGS ADJACENT TO THE BEAM BEING JACKED MAY BE

AND BLOCKING HAVE BEEN REMOVED.

DURING THE BRIDGE JACKING OPERATIONS.

LOOSENED TO DECREASE THE RESISTANCE OF THE DECK SLAB

DURING JACKING, ALL BEARINGS LOOSENED SHALL BE TIGHTENED BACK AFTER REPAIR OPERATIONS ARE COMPLETED AND THE JACKS

THE MAXIMUM DIFFERENTIAL BETWEEN ADJACENT BEAMS THAT ARE

LOADS PROVIDED IN THE "BRIDGE JACKING TABLE" ARE SHOWN

ENGINEER SHALL DETERMINE THE EXPECTED LOADS TO BE LIFTED

PROFESSIONAL ENGINEER IN THE STATE OF NORTH CAROLINA TO

FOR WORKING DRAWING SUBMITTALS. SEE SPECIAL PROVISIONS.

TYPE II BRIDGE JACKING SHALL BE DONE WITH A HYDRUALIC

THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY

ANY STEEL THAT HAS BEEN WELDED TO THE EXISTING STRUCTURE

JACKING SYSTEM THAT LIFTS EACH BEAM ALONG ENTIRE SPAN END WITH EQUAL FORCE AND AT AN EQUAL RATE.

DAMAGE CAUSED TO THE EXISTING STRUCTURE BY BRIDGE JACKING OPERATIONS AT NO ADDITIONAL COST TO THE DEPARTMENT.

FOR INFORMATIONAL PURPOSES ONLY, THE CONTRACTOR'S

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS AND CALCULATIONS OF THE JACKING PROCEDURE(S) SEALED BY A

THE ENGINEER FOR APPROVAL PRIOR TO BRIDGE JACKING

FOR TYPE I OR TYPE II BRIDGE JACKING, SEE SPECIAL

THE BEAM SHALL BE LIFTED ENOUGH THAT THE BEAM CLEARS THE BEARINGS AND ALL LOAD IS SUPPORTED BY THE JACKS, AFTER

NO OBSTACLES PREVENTING THE BEAM FROM BEING LIFTED.

CONTRACTOR SHALL INSPECT THE STRUCTURE FOR ANY

SHALL PROVIDE SAFE AND SUFFICIENT ACCESS TO ALL

AND DOES NOT NECESSARILY REPRESENT SPECIFIC

CONDITIONS AT A PARTICULAR BRIDGE. ACTUAL BRIDGE GEOMETRIES, DIMENSIONS, AND CONDITIONS MAY DIFFER FROM THIS DETAIL. PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL INVESTIGATE THE BRIDGES ON THE PROJECT AND DEVELOP A JACKING PLAN TO BE SUBMITTED FOR REVIEW AND APPROVAL. SEE BRIDGE JACKING SPECIAL

SHEET NO REVISIONS SD-7 DATE: BY: DATE: TOTAL SHEETS

### STANDARD NOTES

### **DESIGN DATA:**

SPECIFICATIONS		AASHTO (CURRENT)
LIVE LOAD		SEE PLANS
IMPACT ALLOWANC	E	SEE AASHTO
STRESS IN EXTREM STRUCTURAL STE	E FIBER OF EL - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN
	- AASHTO M270 GRADE 50W	27,000 LBS. PER SQ. IN
	- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN
REINFORCING STEE	EL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN
CONCRETE IN COM	PRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEA	AR	SEE AASHTO
STRUCTURAL TIMBI	ER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PER	PENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID	PRESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### **CONCRETE:**

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### **CONCRETE CHAMFERS:**

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO  $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A  $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A  $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ "  $\emptyset$  SHEAR STUDS FOR THE  $\frac{3}{4}$ "  $\emptyset$  STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF  $3 - \frac{7}{8}$ "  $\emptyset$  STUDS FOR  $4 - \frac{3}{4}$ "  $\emptyset$  STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ "  $\emptyset$  STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ "  $\emptyset$  STUDS BASED ON THE RATIO OF  $3 - \frac{7}{8}$ "  $\emptyset$  STUDS FOR  $4 - \frac{3}{4}$ "  $\emptyset$  STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST  $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY  $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

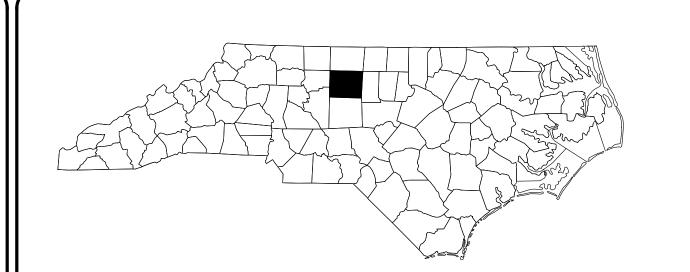
METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

REV. 5-7-03 RWW (\*) JTE REV. 10-1-11 MAA (\*) GM REV. 10-23 BNB (\*) NAP REV. 5-1-06 TLA (\*) GM REV. 12-17 MAA (\*) THC



## STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# GUILFORD COUNTY

STATE PROJECT REFERENCE NO PM NOTES I-5955A F. A. PROJ. NO. STATE PROJ. NO. DESCRIPTION 45908.1.2 4590801 P.E. 45908.3.2 4590801 CONST.

LOCATION:	<b>GUILFORD</b>	COUNTY

BRIDGE #400285 ON US29 SBL OVER SR 3762 (MARTIN LUTHER KING JR. DRIVE) BRIDGE #400340 ON I-40 WBL OVER SR 3762 (MARTIN LUTHER KING JR. DRIVE) BRIDGE #400291 ON I-40 WBL RAMP OVER US29N/US70E/US220N (SOUTH O. HENRY BLVD.) BRIDGE #400299 ON I-40 OVER SOUTH BUFFALO CREEK

BRIDGE #400327 ON I-40 EBL OVER SOUTH BUFFALO CREEK BRIDGE #400329 ON PATTON AVENUE OVER I-40

BRIDGE #400336 ON I-40 EBL OVER US 29 SBL/US 70 WBL

BRIDGE #400339 ON I-40 EBL OVER SR 3762 (MARTIN LUTHER KING JR. DRIVE)

BRIDGE #400348 ON I-40 EBL OVER US29N/US70E/US220N (SOUTH O. HENRY BLVD.)

BRIDGE #400349 ON I-40 WBL OVER US29N/US70E/US220N (SOUTH O. HENRY BLVD.)

BRIDGE #400357 ON I-40 OVER SOUTH BUFFALO CREEK

BRIDGE #400364 ON I-40 EBL OVER SR 4240 (EAST GATE CITY BOULEVARD)

BRIDGE #400367 ON I-40 WBL OVER SR 4240 (EAST GATE CITY BOULEVARD)

BRIDGE #400678 ON I-40 WBL OVER SOUTH BUFFALO CREEK

## PAVEMENT MARKING NOTES:

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE DIRECTED BY THE ENGINEER.

FINAL PAVEMENT MARKINGS AND MARKERS SHALL MATCH THE EXISTING AND SHALL ADHERE TO THE APPROPRIATE STANDARDS OF "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N. C., DATED JANUARY 2024. SUCH STANDARDS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:

STD. NO. PAVEMENT MARKINGS - LINE TYPES & OFFSETS 1205.01 PAVEMENT MARKINGS - 2 LANE & MULTILANE ROADWAYS PAVEMENT MARKINGS - EXIT & ENTRANCE RAMPS 1205.03 PAVEMENT MARKINGS - LANE DROPS 1205.06 1205.08 PAVEMENT MARKINGS - SYMBOLS & WORD MESSAGES PAVEMENT MARKINGS - BRIDGES 1205.12 RAISED PAVEMENT MARKERS - INSTALLATION SPACING 1250.01 1251.01 RAISED PAVEMENT MARKERS - PERMANENT & TEMPORARY

TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS.

PAVEMENT MARKINGS BILL OF MATERIAL		
ITEM	UNITS	QUANTITY
COLD APPLIED PLASTIC PAVEMENT MARKING CHARACTER, TYPE II	EA	1
POLYUREA PAVEMENT MARKING LINES (6", 20 MILS)	LF	8983
PERMANENT RAISED PAVEMENT MARKERS	EA	100

