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CROSS SECTIONS

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# 48409

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

### **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_STANLY

PROJECT DESCRIPTION REPLACE BRIDGE NO. 102 ON SR 1917 (BETHLEHEM CHURCH ROAD) OVER LONG CREEK

STATE PROJECT REFERENCE NO. 19 B-6046

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

CG2 EXPLORATION

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DATE APRIL 2022



WENNET WITH 05/02/2022 957A789AED704CB

CAROLINAS GEOTECHNICAL

GROUP

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REPERENCE NO. SHEET NO. 2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNICONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST CARSHTO T 2006, ASTM DISBOS, SOIL CLASSIFICATION IS BASED ON THE AGSHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLIOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AGSHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANDLU ARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SUTV CLAY, MOST WITH MITERBEDOE PINE SAND LAWERS, HIGHLY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SUTV CLAY, MOST WITH MITERBEDOE PINE SAND LAWERS, HIGHLY, ETC. FOR EXAMPLE, CLASS.  GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7 A-1, A-2 A	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.  MINERAL OGICAL COMPOSITION  MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.  COMPRESSIBLE LL ≤ 31  MODERATELY COMPRESSIBLE LL ≤ 31  MODERATELY COMPRESSIBLE LL > 50  PERCENTAGE OF MATERIAL  ORGANIC MATERIAL  ORGANIC MATERIAL  SOILS  TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  MODERATELY ORGANIC > 10% 12 - 20%  MODERATELY ORGANIC > 10% 22%  HIGHLY ORGANIC > 10% 22%  HIGHLY ORGANIC > 10% 22%  MODERATELY ORGANIC STATIC BORE HOLE IMMEDIATELY AFTER DRILLING  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING  PRICH OF WATER LEVEL AFTER 24 HOURS  PRICH ORGANIC STRATA	HARD ROCK IS NON-COASTAL PLAIN MATERIAL. THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.  ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED  ONO-COASTAL PLAIN MATERIAL. THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  ONO-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, ROCK (KCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.  WEATHERING  FRESH  ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.  WEATHERING  FRESH  ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NOTURE.  SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS. HOUDS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	TERMS AND DEFINITIONS  ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  AQUIFER - A WATER BEARING FORMATION OR STRATA.  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  ARGILACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.  CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.  COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.  CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  DIVE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.  DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.  SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  FISSILE - A PROPERTY OF SPLITTING ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARAENT MATERIAL.  FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30   CONSISTENCY OR DENSENESS	SPRING OR SEEP  MISCELLANEOUS SYMBOLS  ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION  SOIL SYMBOL  ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  AUGER BORING  INFERRED SOIL BOUNDARY  INFERRED ROCK LINE  MONITORING WELL  TEST BORING  SUNDING ROD  TEST BORING WITH CORE WITH CORE  PIEZOMETER INSTALLATION  SOUNDING ROD  TEST BORING WITH CORE  PIEZOMETER INSTALLATION  SPT N-VALUE	WITH FRESH ROCK.  MODERATELY SEVERE (MOD. SEV.)  AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK' SOUND WHEN STRUCK.  IF TESTED, WOULD YIELD SPT REFUSAL  SEVERE (SEV.)  ALL ROCK EXCEPT QUARITZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY  ALL ROCK EXCEPT QUARITZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.)  REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINDR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF  COMPLETE  ROCK REDUCED TO SOIL. ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF  COMPLETE  ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE. ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARITZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.  JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.  LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.  LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTILED (MOI) - IRREGULABLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTITLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.  RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.  ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  DPENING (MM)  4 76 2.00 0.42 0.25 0.075 0.053  BOULDER (COB) GRAVEL (CORSE (BLDR.)) (FSD.)  GRAIN MM 305 75 2.0 0.25 0.05 0.005  SIZE IN. 12 3  SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE (ATTERBERG LIMITS)  CATTERBERG LIMITS  OM OPTIMUM MOISTURE SCRIPTION  OPTIMUM MOISTURE SCRIPTION  OPTIMUM MOISTURE - WET - (W)  PLASTIC LIMIT  OM OPTIMUM MOISTURE - WET - (W)  SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE  PLASTICITY INDEX (P)  PLASTICIT	UNDERCUT UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED WEA WEATHERED - VEATHERED - VAN UNIT WEIGHT - VANE SHEAR TEST UNG ORGANIC UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST  WEA WEATHERED - VAN UNIT WEIGHT - VAN UNIT WE	VERY HARD  CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK, COUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT, HARD  CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES I INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  TERM  VERY WIDE  MORE THAN 10 FEET  WIDE  THICKLY BEDDED  TERM  SPACING  YERY WIDE  MORE THAN 10 FEET  WIDE  THICKLY BEDDED  THICKNESS  VERY WIDE  MORE THAN 10 FEET  THICKLY BEDDED  THICKNESS  VERY WIDE  MORE THAN 10 FEET  THICKLY BEDDED  THICKNESS  VERY CLOSE  LESS THAN 0.16 FEET  THICKLY BEDDED  THICKNESS  **VERY** CLOSE***  **INDURATION***  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  **RUBBING WITH FINGER FREES NUMEROUS GRAINS;**  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  **MODERATELY INDURATED***  MODERATELY INDURATED  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;*  BREAKS EASILY WHEN HIT WITH HAMMER.  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;*  BREAKS EASILY WHEN HIT WITH HAMMER.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  JOPSOIL (15.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK; BL-IO7, N-538,629,6280, E-I,623,668.9390, ELEV 306.19 FT BL-IO6, N-538,910.1580, E-I,624,234.7820, ELEV 291.22 FT ELEVATION: FEET  NOTES:  ROADWAY DESIGN FILES PROVIDED BY NCDOT ON 03/10/2022.  FIAD = FILLED IMMEDIATELY AFTER DRILLING
COLOR  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X MOBILE B-29 TRICONE TUNGCARB. SOUNDING ROD  CORE BIT VANE SHEAR TEST	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X DIEDRICH D-50	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

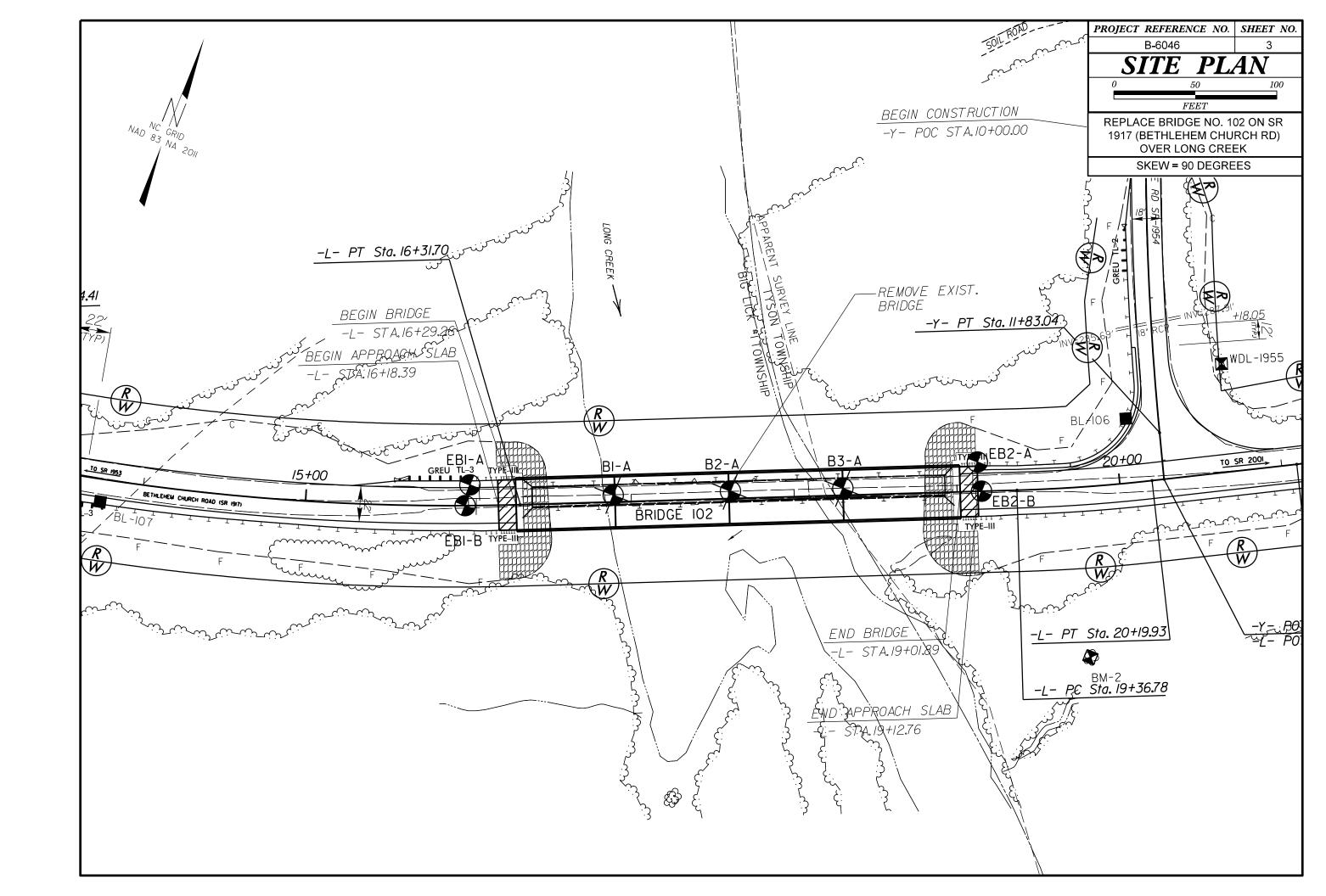
ROJECT REFERENCE NO.	SHEET NO.
B-6046	2A

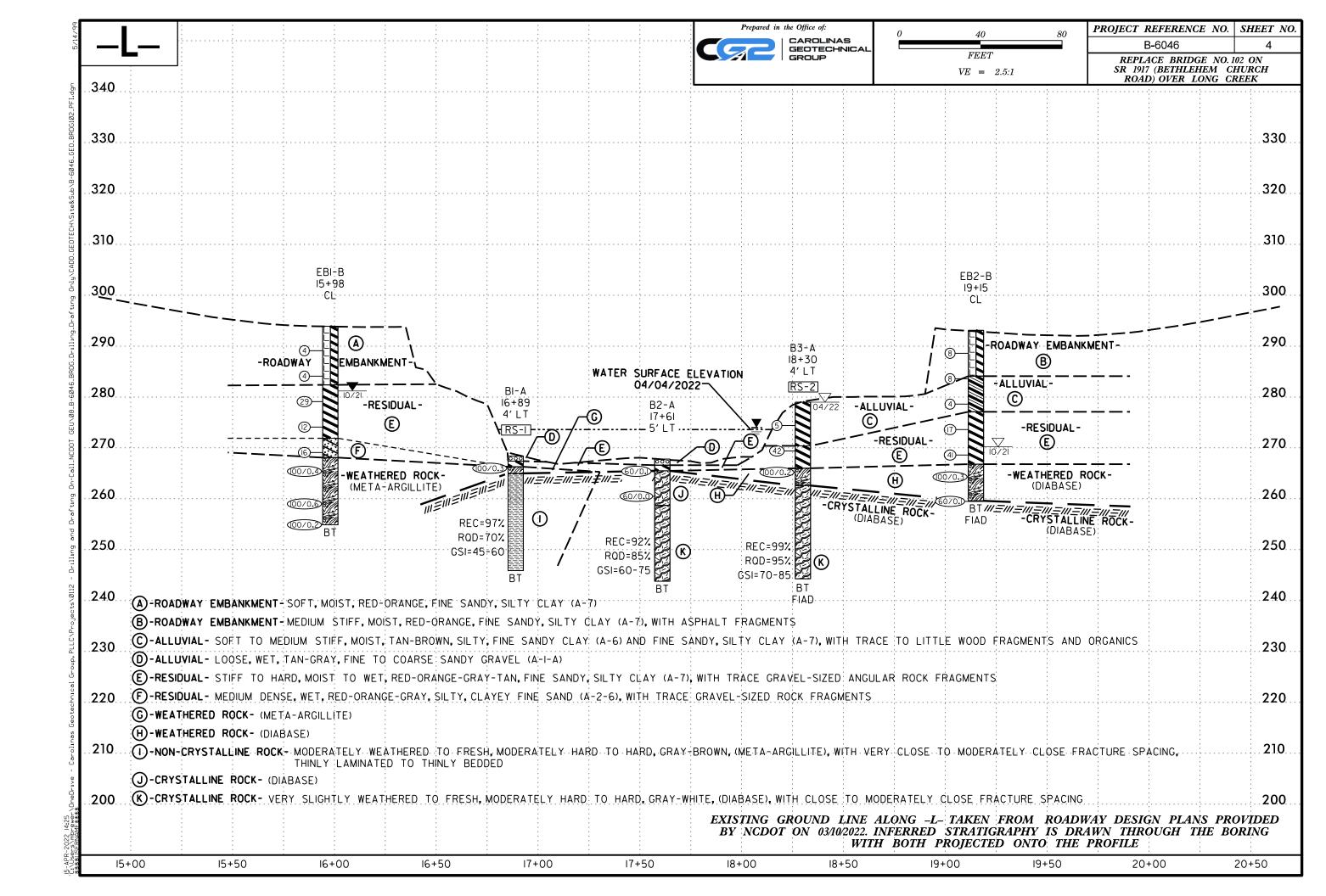
#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

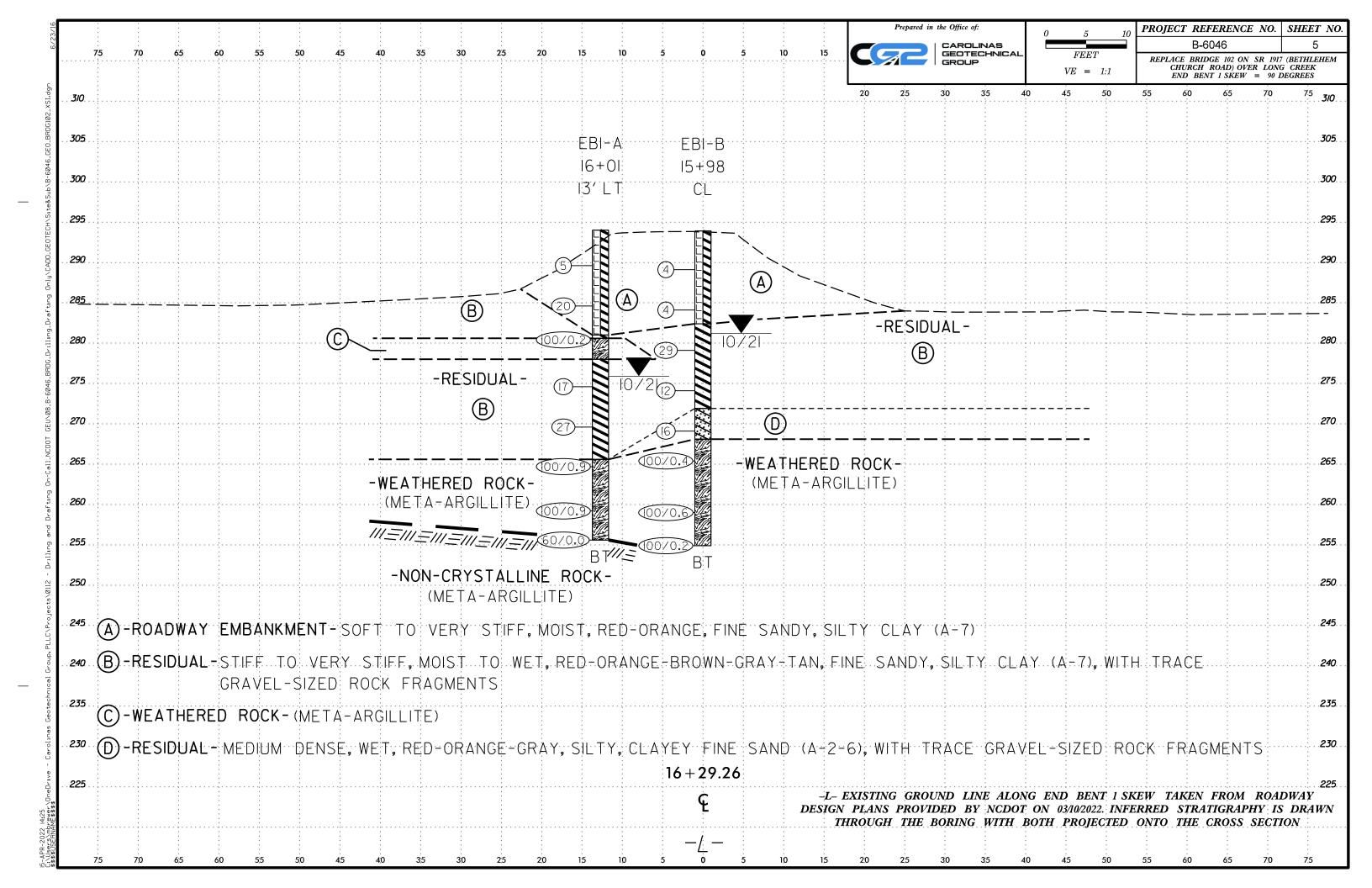
# SUBSURFACE INVESTIGATION

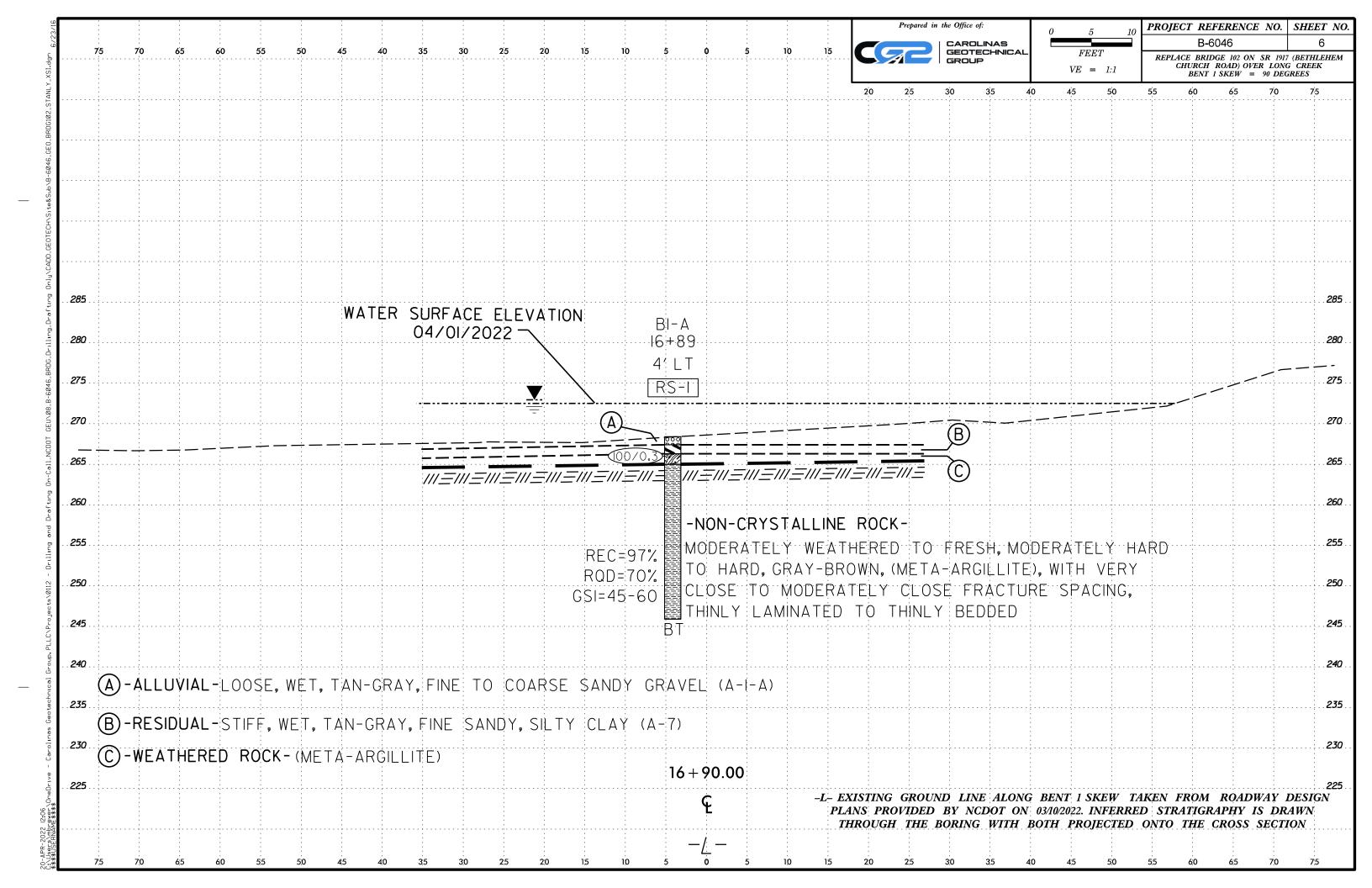
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

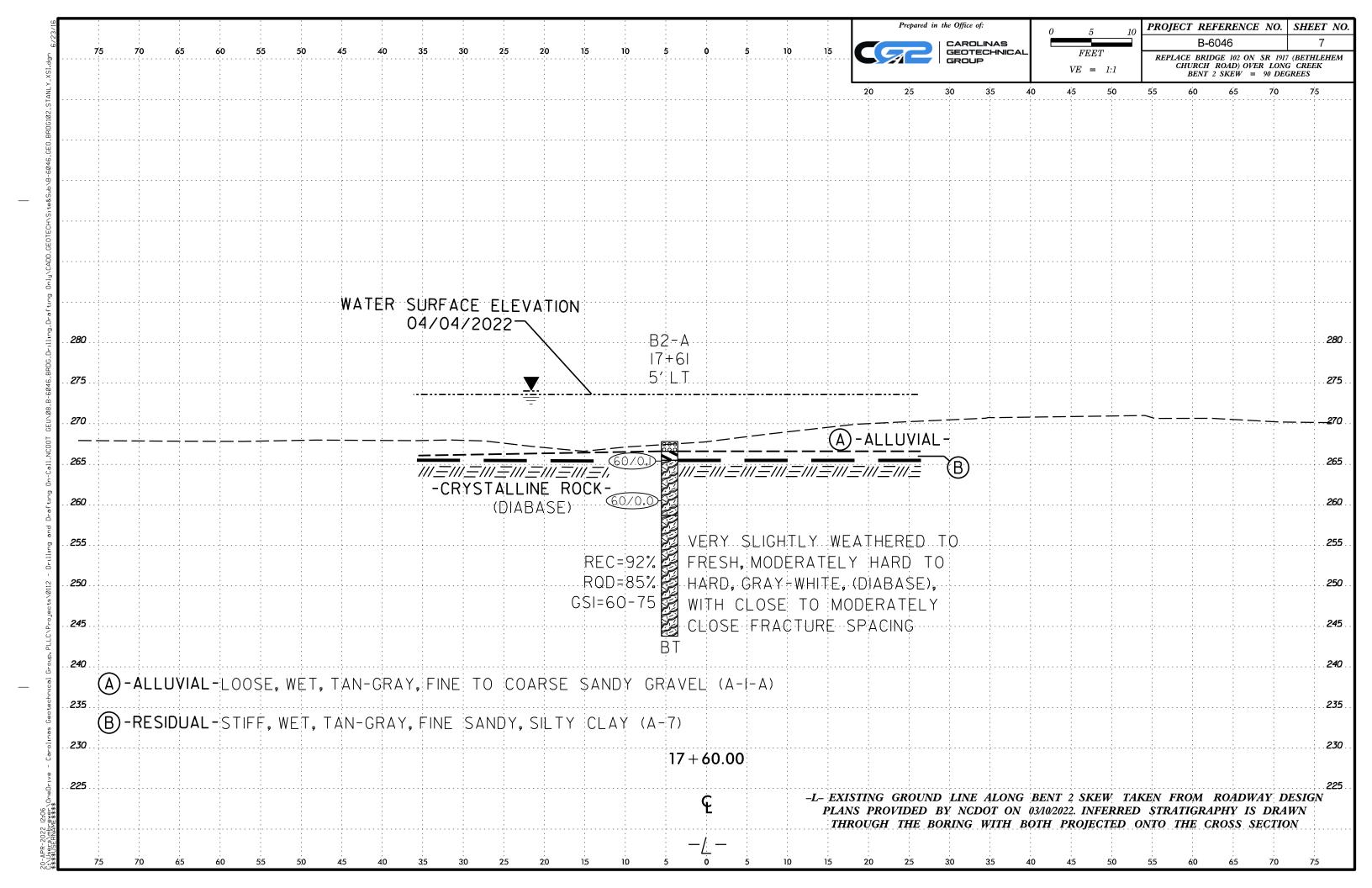
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	nted Ro	ock Mass (Marinos and Hoek, 2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		s p		ν 0 0	8 9 9	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS	VERY GOOD Very rough, fresh unweathered surfaces GOOD Rough, slightly weathered, iron stained surfaces	<b>FAIR</b> Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfa- with compact coatings or fillings or angular fragments	<b>VERY POOR</b> Slickensided, highly weathered surf with soft clay coatings or fillings	Exercise of the first ocontrolled failures. Where authors of the presence of groundwater and this controlled surfaces of toolands of the strength of some thereof our highly weather the sales and salight shift to the right in the columns to the allowed to highly weather soft colladed or highly weather soft collades of to clade of the soft collades of to clade of the soft collades of to clade of the soft collades of toolades
STRUCTURE		DECREASING SU	URFACE QU	ALITY =	>	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities  BLOCKY - well interlocked un-	PIECES 	90 80		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets  VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks	OCKING OF ROCK	70 60	50			B. Sand- stone with thin inter- layers of siltstone amounts  D. Siltstone or silty shale with sand- stone layers stone with siltstone or clayey shale with sandstone layers  40
formed by 4 or more joint sets  BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	   ASING INTERLOC 		40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.  F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed as sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces	 			20		G. Undisturbed silty or clayey shale with or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone layers
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	٧	N/A N/A		$\langle \ / \ \rangle$	10	Into small rock pieces.   → Means deformation after tectonic disturbance  DATE: 8-19-

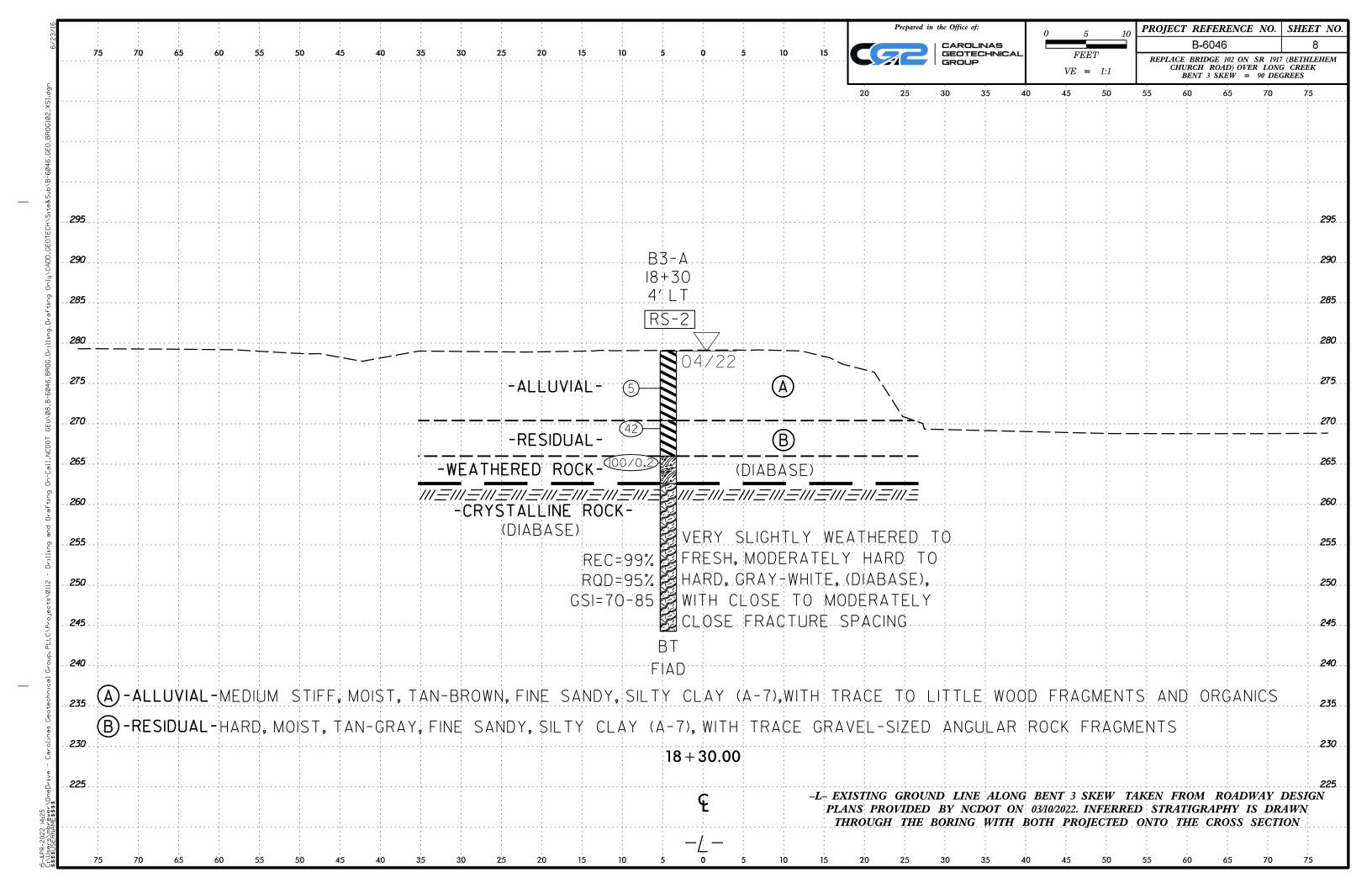


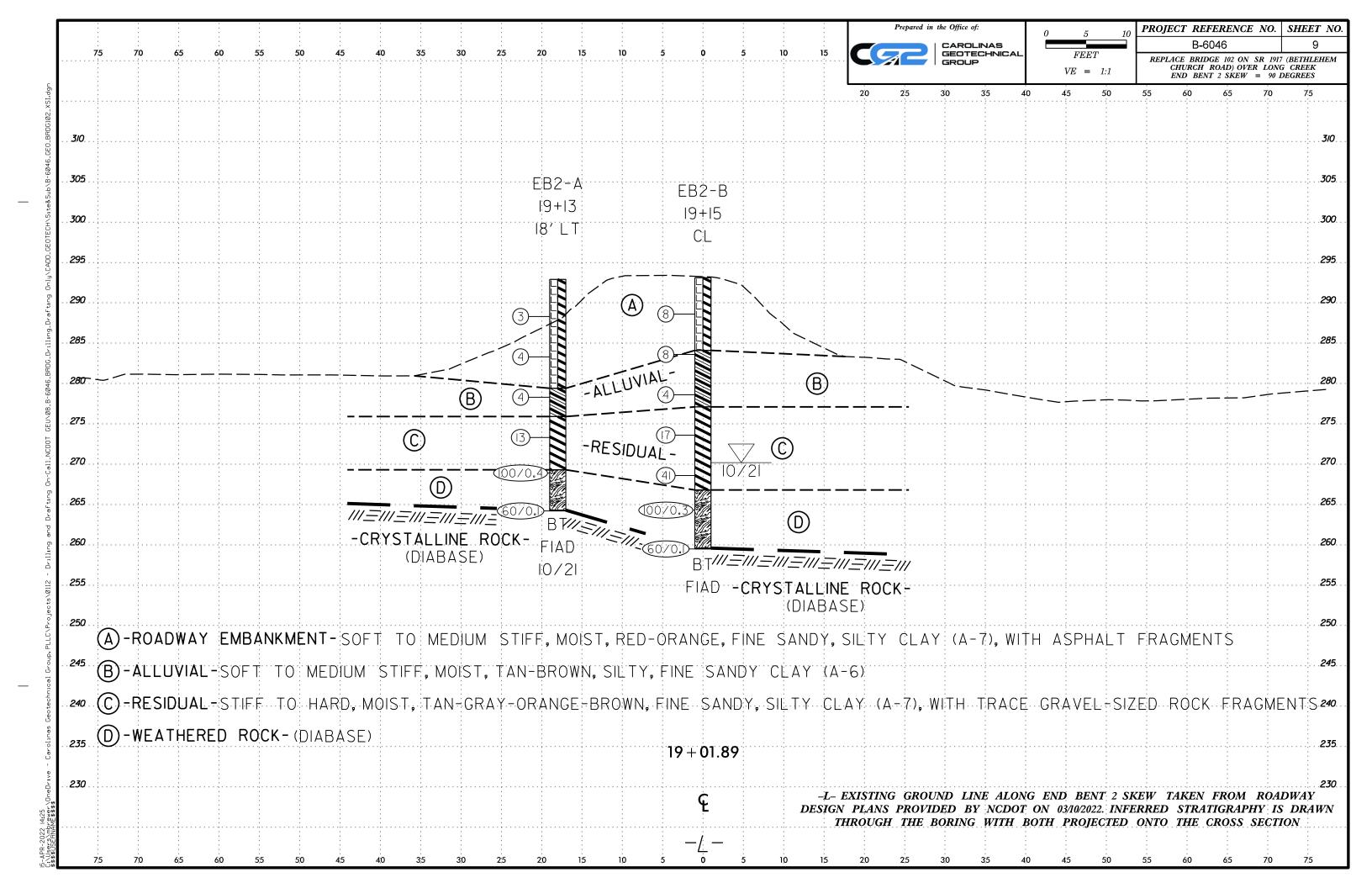


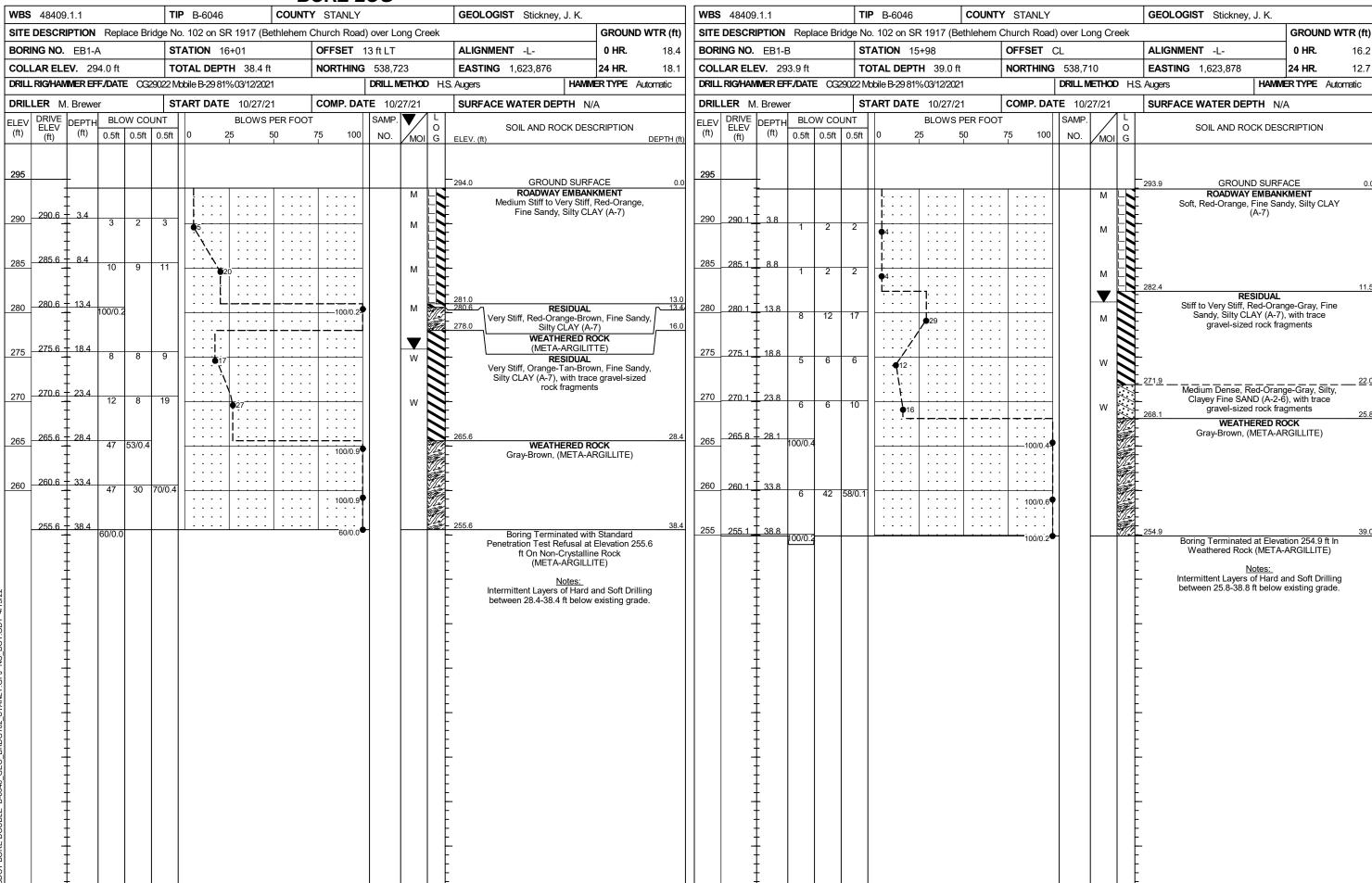












#### GEOTECHNICAL BORING REPORT CORE LOG

		BORE LOG								COL	RE LOG			
<b>WBS</b> 48409.1.1	TIP B-6046 COUN	NTY STANLY	GEOLOGIST Stickney, J. K.		<b>WBS</b> 48409.	1.1		TIP B-604	46	COUNTY	STANLY	GEOLOGIST Stickney, J. K.		
SITE DESCRIPTION Replace	Bridge No. 102 on SR 1917 (Bethlehei	m Church Road) over Long Creek	GF	ROUND WTR (ft)	SITE DESCRI	PTION	Replace Brid	lge No. 102 or	n SR 1917 (B	ethlehem Chu	ırch Road) over Long Creek		GROUND WTR	(ft)
BORING NO. B1-A	<b>STATION</b> 16+89	OFFSET 4 ft LT	ALIGNMENT -L- 0	HR. N/A	BORING NO.			STATION			FFSET 4 ft LT	ALIGNMENT -L-	0 HR. 1	N/A
COLLAR ELEV. 268.4 ft	TOTAL DEPTH 22.5 ft	NORTHING 538,750	<b>EASTING</b> 1,623,960 <b>24</b>	HR. N/A	COLLAR ELEV. 268.4 ft		TOTAL DE	<b>EPTH</b> 22.5 f	t NC	<b>DRTHING</b> 538,750	<b>EASTING</b> 1,623,960	24 HR.	N/A	
	 DG20446 Diedrich D50 76%06/14/2021	DRILL METHOD		TYPE Automatic	DRILL RIG/HAM						DRILL METHOD SF		MERTYPE Automati	
DRILLER C. Odom	<b>START DATE</b> 04/01/22	COMP. DATE 04/01/22	SURFACE WATER DEPTH 4.1ft		DRILLER C.				ATE 04/01/2		 <b>DMP. DATE</b> 04/01/22	SURFACE WATER DEPTH 4		$\dashv$
		OOT SAMP V	. [		CORE SIZE			TOTAL RU		2   00	JIII : DATE 04/01/22	SON ACE WATER DEFINE	<del>+. 11t</del>	-
ELEV CRIVE CRIPTH BLOW (ft) (ft) 0.5ft 0.		'/   0	SOIL AND ROCK DESCRIP  ELEV. (ft)	PTION DEPTH (ft)			DRILL			STRATA I				$\dashv$
			WATER SURFACE (04/01		ELEV RUN ELEV (ft)	(ft)	RUN RATE (ft) (Min/ft	RUN REC. RQD (ft) (ft) ) % %	SAMP. R	STRATA L EC. RQD O ft) (ft) G		DESCRIPTION AND REMARKS		
					(11)		(IVIIII/IL	) % %		%   %   G	ELEV. (ft)		DEPT	H (ft)
270			- 268.4 GROUND SURFACE		264.97 265.0	3.4	4.1	(4.0) (3.1)	) (1	8.5) (13.3)	265.0	Begin Coring @ 3.4 ft NON-CRYSTALLINE ROCK		3.4
		W		1.0	<del> </del>	.		(4.0) (3.1) 98% 76%	RS-1 9	8.5) (13.3) 7% 70%	Moderately Weath	ered to Fresh, Moderately Hard to Ha TE), with Very Close to Moderately C	ard, Gray-Brown	
265 266.3 7 2.1 100/0.3			267.4 ALLUVIAL 266.3 Loose, Tan-Gray, Fine to Coars 265.0 GRAVEL (A-1-a)	rse Sandy 2.1	260.9	7.5	5.0	(4.9) (2.0)	1 1		Space	ing, Thinly Laminated to Thinly Bedd	led	
+			E RESIDUAL			-	5.0	(4.8) (2.9) 96% 58%			<del>-</del>			
		RS-1	Stiff, Tan-Gray, Fine Sandy, Si (A-7)								-	RS-1: 4.8-5.4' Unit Weight: 161.8 pcf		
260 🛨			WEATHERED ROCK Gray-Brown, (META-ARGIL		255.9		5.0	(5.0) (3.0)	1		Unconfined	d Compressive Strength: 7,530 psi (1	,084 ksf)	
		1 1 1 1 1	NON-CRYSTALLINE RO	OCK	1   ±	.		100% 60%			- -			
			Gray-Brown (META-ARGIL	-L:!E)	250.9						1			
255		<del>  </del>	REC=97% RQD=70%		250		5.0	(4.7) (4.3) 94% 86%	7					
‡		· ·   · · · ·	GSI=45-60		‡	:		3-70   00%						
250		1 1 1 1 1			245.9	22.5			<b>↓</b>		245.9	ated at Elevation 245.9 ft In Non-Crys	stalling Pook	22.5
†						-					boring rermin	ated at Elevation 245.9 ft in Non-Crys (META-ARGILLITE)	stanne ROCK	
			== == ==-		‡	.					<u> </u>			
			245.9 Boring Terminated at Elevation	22.5 245.9 ft In		-								
			Non-Črystalline Rock (META-AF	RGILLITE)		.					_			
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# Bridge No. 102 on SR 1917 (Bethlehem Chuch Road) over Long Creek Rock Core Photographs Boring: B1-A

3.4 to 22.5 Feet



FEET

#### GEOTECHNICAL BORING REPORT CORE LOG

	<b>L</b>	BORE LOG									COF	RE LOG			
<b>WBS</b> 48409.1.1		NTY STANLY	GEOLOGIST Stickney, J. K.		WBS	48409.1.1		<b>TIP</b> B-604	46	C	OUNTY S	TANLY	GEOLOGIST Stickney	/, J. K.	
SITE DESCRIPTION Replace Bridge	ge No. 102 on SR 1917 (Bethlehen	m Church Road) over Long Creek	GRO	OUND WTR (ft)	SITE	DESCRIPTION	Replace Brido	ge No. 102 or	n SR 1917	' (Bethle	ehem Chur	ch Road) over Long Creek		GROUND V	WTR (ft)
BORING NO. B2-A	STATION 17+61	OFFSET 5 ft LT	ALIGNMENT -L- 0 Hi	R. N/A	BOR	ING NO. B2-A		STATION	17+61		OF	FSET 5 ft LT	ALIGNMENT -L-	0 HR.	N/A
COLLAR ELEV. 267.8 ft	TOTAL DEPTH 24.0 ft	<b>NORTHING</b> 538,779	<b>EASTING</b> 1,624,026 <b>24 Hi</b>	R. N/A	COL	LAR ELEV. 26	7.8 ft	TOTAL DE	<b>EPTH</b> 24	.0 ft	NO	<b>RTHING</b> 538,779	<b>EASTING</b> 1,624,026	24 HR.	N/A
DRILL RIG/HAMMER EFF/DATE 03204	46 Diedrich D50 76% 06/14/2021	DRILL METHOD S	SPT Core Boring HAMMER TYP	PE Automatic	DRIL	L RIG/HAMMER EF	F./DATE CG204	446 Diedrich D5	0 76%06/14	4/2021		<b>DRILL METHOD</b> SP	T Core Boring	HAMMER TYPE Aut	tomatic
DRILLER C. Odom	<b>START DATE</b> 04/04/22	COMP. DATE 04/04/22	SURFACE WATER DEPTH 5.8ft		DRIL	LER C. Odom		START DA	ATE 04/0	)4/22	co	<b>MP. DATE</b> 04/04/22	SURFACE WATER DE	<b>PTH</b> 5.8ft	
ELEV DRIVE DEPTH BLOW COUN			SOIL AND ROCK DESCRIPTI	ION	COR	RE SIZE NQ		TOTAL RU	<b>JN</b> 14.9 f		ΛTΛ .				
(ft) (ft) (ft) 0.5ft 0.5ft (	.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft)	DEPTH (ft)	ELEV (ft)	CLCV   /ft\	RUN DRILL RATE	RUN REC. RQD (ft) (ft) %	SAMP. NO.	REC.	ATA L RQD O (ft) G		DESCRIPTION AND REMAR		
					<u> </u>		(π) (Min/ft)	% %	1	%′	%′ G	ELEV. (ft)			DEPTH (ft)
275					258.67	7 258.7 <u>9.1</u>	4.9	(3.7) (2.8) 76% 57%		(13.7)	(12.6) 85%	- 258.7 Very Slightly Weath	Begin Coring @ 9.1 ft ered to Fresh, Moderately H ith Close to Moderately Close	ard to Hard, Gray-White	e 9.1
			E	/	255			76%   57%		92%	85%	- (DIABASE), w	ith Close to Moderately Close	e Fracture Spacing	
270			Ł		255	253.8 14.0	5.0	(5.0) (4.0)							
			_ 267.8 GROUND SURFACE	0.0		‡	5.0	(5.0) (4.8) 100% 96%				- -			
265 265.5 + 2.3		· ·   · · · ·	265.5 Loose, Tan-Gray, Fine to Coarse	1.2 Sandy 2.3	250	248.8 19.0						<del>-</del>			
265 265.5 7 2.3 60/0.1		60/0.1	GRÁVEL (A-1-a)  RESIDUAL			- 19.0	5.0	(5.0) (5.0) 100% 100%				- -			
			Stiff, Tan-Gray, Fine Sandy, Silty (A-7)	/ CLAY	245	‡		100% 100%				- -			
260 260.5 + 7.3 60/0.0		60/0.19	CRYSTALLINE ROCK			243.8 24.0			4			243.8 Boring Terminated	at Elevation 243.8 ft In Crys	talline Rock (DIABASE)	24.0
			258.7 Gray, (DIABASE) Gray-White (DIABASE)	9.1		ļ <u></u>						- Bonng Tenninated	at Lievation 240.0 it in Oryo	tailine Nock (DIABAGE)	
255			REC=92% RQD=85%			<del> </del>						- - -			
+			RQD=85% GSI=60-75									- - -			
						1 1						- -			
250						1 1						- -			
						<u> </u>						- -			
245			+			+						<del>_</del> -			
			243.8 Boring Terminated at Elevation 24	24.0								<u>-</u> -			
			- Crystalline Rock (DIABASE	=3.6 11 111		‡						- -			
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#### Bridge No. 102 on SR 1917 (Bethlehem Chuch Road) over Long Creek Rock Core Photographs Boring: B2-A

9.1 to 24.0 Feet



FEET

#### GEOTECHNICAL BORING REPORT CORE LOG

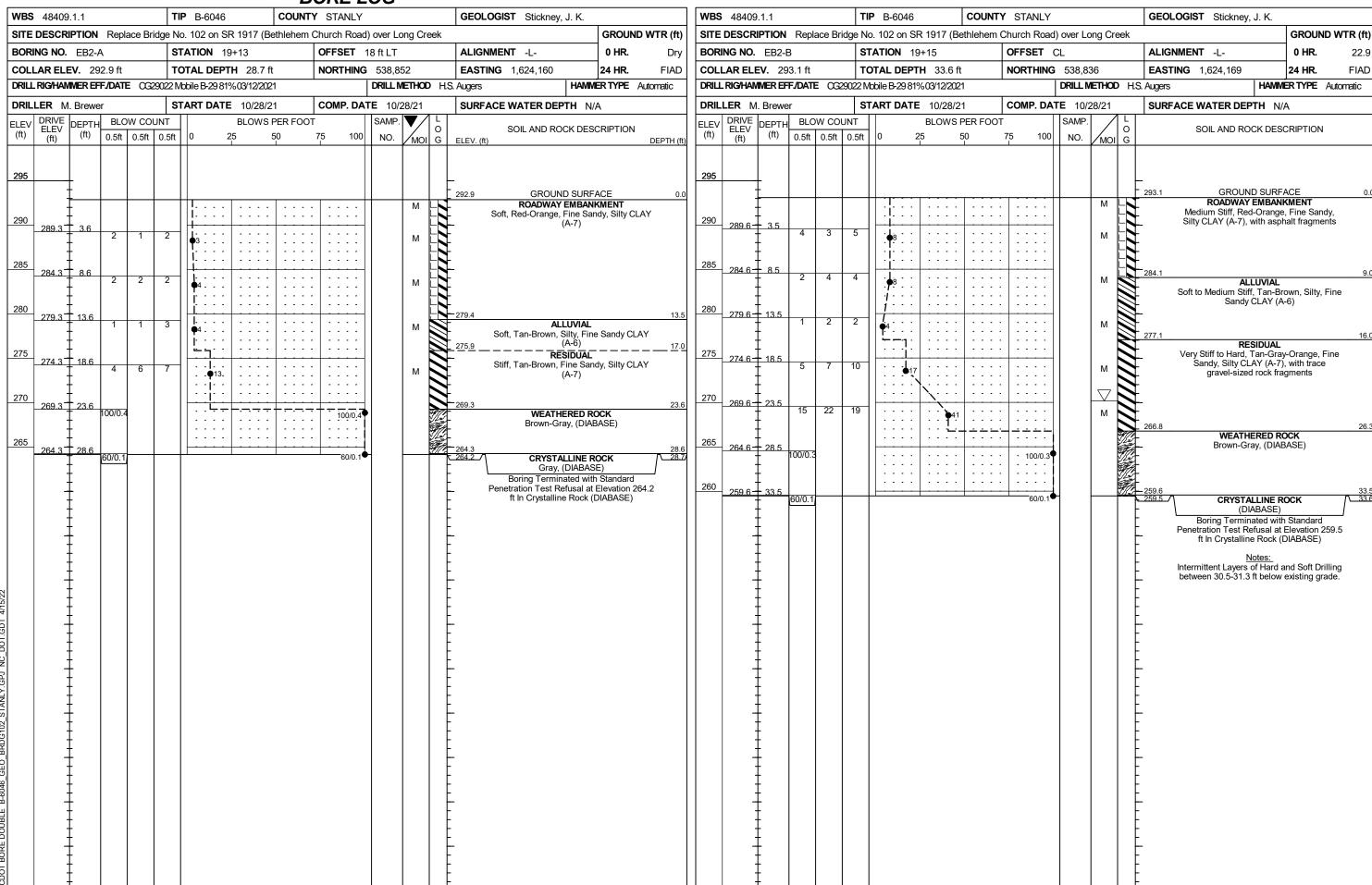
							В	ORE I	LOG														CORE LOG									
WB	48409	).1.1		TI	<b>P</b> B-6046		COUNT	Y STANLY	,		G	EOLOGIST Stickney,	, J. K.			<b>WBS</b> 4840	9.1.1			TIP	B-6046	COUN	TY S	TANLY		GEOL	OGIST Stickney,	J. K.				
SITI	DESCR	IPTION	Replace B	Bridge N	o. 102 on S	R 1917 (B	ethlehem	Church Roa	d) over Lo	ng Creel	k		G	ROUND	WTR (ft)	SITE DESC	RIPTION	Repla	ace Bridg	ge No. 1	102 on SR 1917	' (Bethlehem	n Chui	ch Road	over Long Cre	ek		GI	ROUND WI	R (ft)		
BOF	ING NO.	В3-А		S	TATION 18	8+30		OFFSET	4 ft LT		Α	LIGNMENT -L-		0 HR.	0.0	BORING NO	<b>).</b> B3-A			STAT	<b>FION</b> 18+30		OF	FSET 4	ft LT	ALIGN	MENT -L-	0	HR.	0.0		
COI	LAR EL	<b>EV</b> . 279.	0 ft	T	OTAL DEPT	<b>TH</b> 34.7 f	ft	NORTHIN	<b>G</b> 538,80	)6	E	<b>ASTING</b> 1,624,089	24	4 HR.	FIAD	COLLAR E					AL DEPTH 34		NC		538,806		<b>NG</b> 1,624,089			FIAD		
DRIL	L RIG/HAI	MER EFF.	DATE CO	320446 D	iedrich D50 76	6%06/14/20	)21		DRILL M	ETHOD	SPTCo	ore Boring	HAMMER	TYPE AL	utomatic				E CG204		ich D50 76%06/1				DRILL METHOD	SPT Core Bori	ing	HAMMER T	YPE Auton	natic		
DRI	LER C				TART DATE			COMP. DA			s	URFACE WATER DEF	PTH N/A			DRILLER				_	RT DATE 04/0		CC	MP. DA	E 04/04/22	SURFA	ACE WATER DEP	TH N/A				
ELE\ (ft)			BLOW CO				PER FOOT		SAMP.	'/   (		SOIL AND RO	CK DESCRI	IPTION		CORE SIZE	_		DDILL	TOTA	AL RUN 18.31		4.									
(10)	(ft)	(11)	J.5π   U.5π	υ.5π	1 0 4	25 	50	75 100	NO.	/MOI G	3 ELE	EV. (ft)			DEPTH (ft)	ELEV RUN (ft) ELEV	, DEPTH (ft)	RUN (ft)	DRILL RATE	REC.	JN SAMP. (ft) NO.	STRATA REC. RQD (ft) (ft) %				DESCRIPT	ON AND REMARK	<b>KS</b>				
																(11)	+ ` ′		(Min/ft)	%	%	% %	G	ELEV. (1	t)	Danie (	One in a 10 10 1 ft		DE	EPTH (fi		
280	_				Ц.			ı		$\nabla$	279		ID SURFACE	E	0.0	1 1	16.4	3.3		(3.1)	(2.7) 82%	(18.1) (17.3 99% 95%	3)	- 262.6		CRY	Coring @ 16.4 ft STALLINE ROCK		14/1 1/1	16.		
		‡			: : :						*	Medium Stiff, Tan-l	LUVIAL Brown, Fine	Sandy, Sil	ilty	260 259.3	19.7					99%   95%		_	Very Slightly V (DIABAS	Veathered to Fr E), with Close to	esh, Moderately Ha o Moderately Close	rd to Hard, G Fracture Spa	ray-White cing			
275	275.4	3.6	1 1	4			1 : : :		<u> </u>	м	*	CLAY (A-7), with fragments	th trace to litt s and organio	tle wood ics			Ŧ	5.0		(5.0) 100%	(5.0)  100%						S-2: 22.9-23.6'					
		‡					l l			· ·	*					255	Ĭ.,,				RS-2				Uncon		Weight: 181.9 pcf sive Strength: 9,570	) psi (1,378 k	sf)			
270	270.4	8.6			<u>                                   </u>	· · · · ·					270				8.6	254.3	24.7	5.0		(5.0) 100%	(4.8)											
2.0	i -	†	6 18	24		•4:	2		1	М	3	Hard, Tan-Gray, F	<b>SIDUAL</b> Fine Sandy, §	Silty CLAY	Υ	250	‡			100%	96%			<u>-</u>								
		<u> </u>									266	(A 7) with trace or	ravel-sized a igments	angular roc	ck 13.0	250 249.3	29.7	5.0		(5.0)	(4.8)			- - - - - - - - - - - - - - - - - - -								
265	265.4	13.6	00/0.2					100/0.2	•			WEATH Gray,	IERED ROCK (DIABASE)	K			‡	0.0		(5.0) 100%	96%			<b>-</b> -								
											262	2.6 CRYSTA	ALLINE ROCI	K	16.4	245 244.3	34.7							- <sub>244.3</sub>						34.		
260									<u> </u>			Gray-Whi	ite, (DIABAS				+							-	Boring Termi	nated at Elevation	on 244.3 ft In Crysta	alline Rock (D	IABASE)			
		‡										RE RC	EC=99% QD=95%				Ŧ							-								
255		‡							RS-2			GS	SI=70-85				Ŧ															
233	-	‡					l l		1 10-2								Ī															
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250	-	‡							<b>↓</b>								‡							_								
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245		Ŧ															‡							_								
				+					1		244				34.7 In		‡							-								
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# Bridge No. 102 on SR 1917 (Bethlehem Chuch Road) over Long Creek Rock Core Photographs Boring: B3-A



FEET



PROJECT REFERENCE NO.	SHEET NO.
B-6046	18
LAB RESU	<b>ILTS</b>

				ROCE	X TEST RESULTS		
SAMPLE NO.	BORING	STATION	OFFSET	DEPTH INTERVAL	ROCK TYPE	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH
RS-1	B1–A	16+89 -L-	4' LT	4.8 - 5.4'	META-ARGILLITE	161.8	7,530 psi/1,084 ksf
RS–2	B3-A	18+30 -L-	4' LT	22.9 - 23.5'	DIABASE	181.9	9,570 psi/1,378 ksf

LAB TESTING PERFORMED BY NCDOT LAB CERT NO. 117-1104

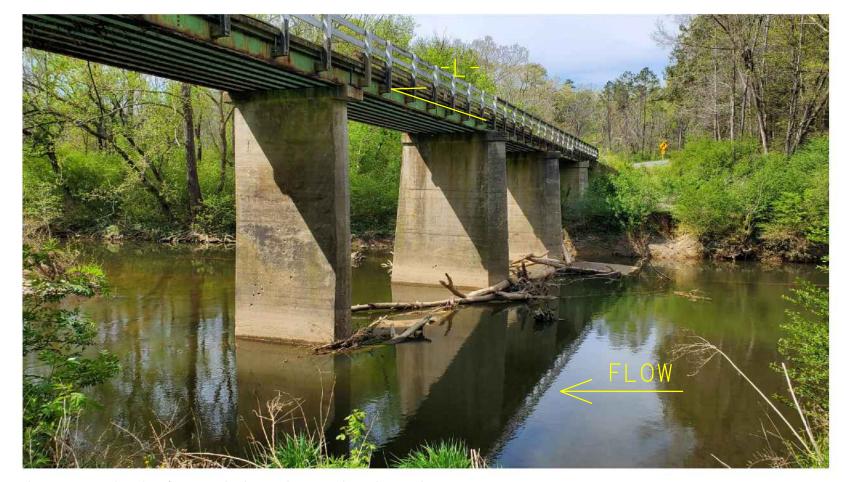


Photo #1: North side of existing bridge looking south/southwest (downstation)



Photo #2: North side of existing bridge looking south/southwest (downstation)



Photo #3: End Bent 1 looking north/northeast (upstation)