

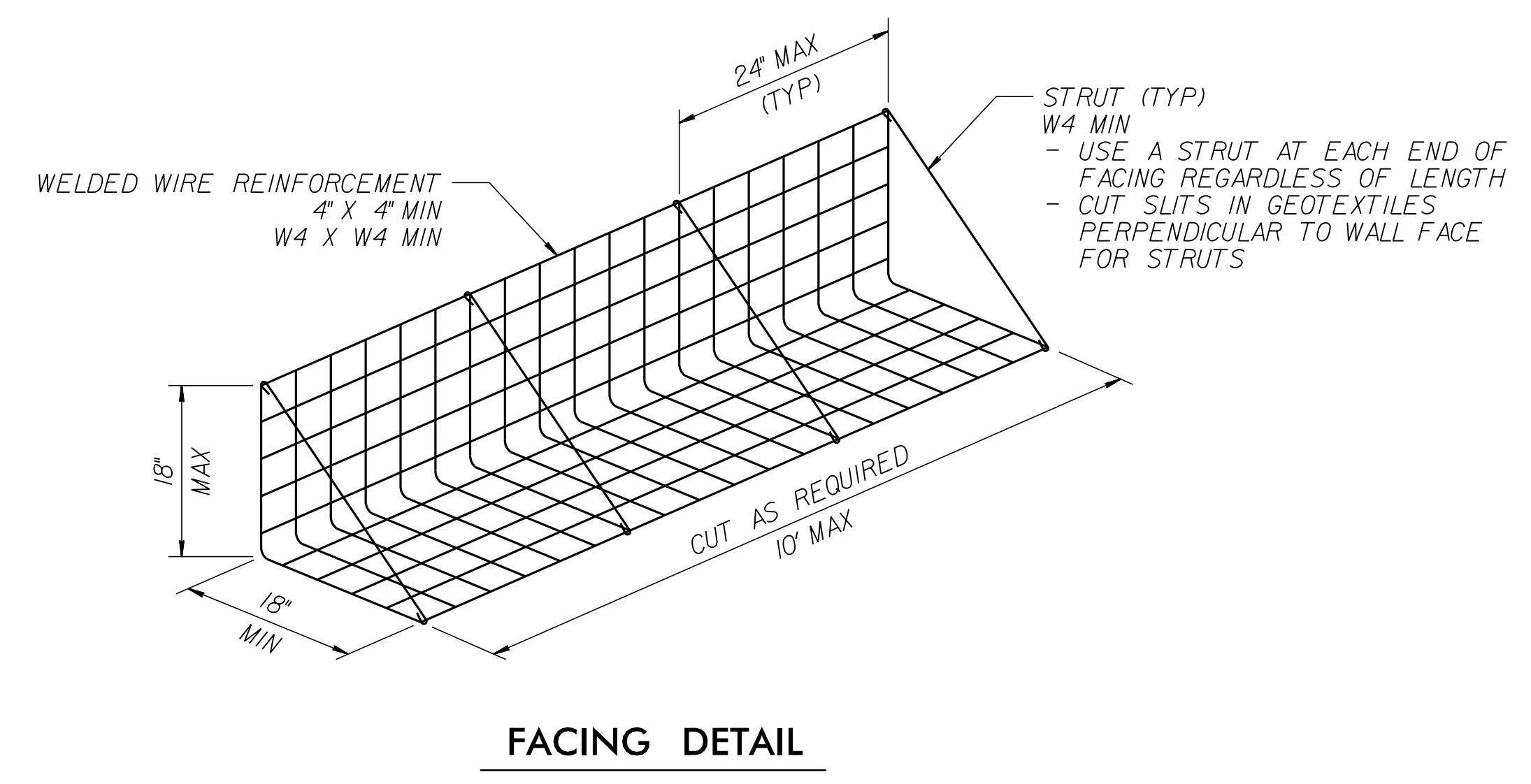
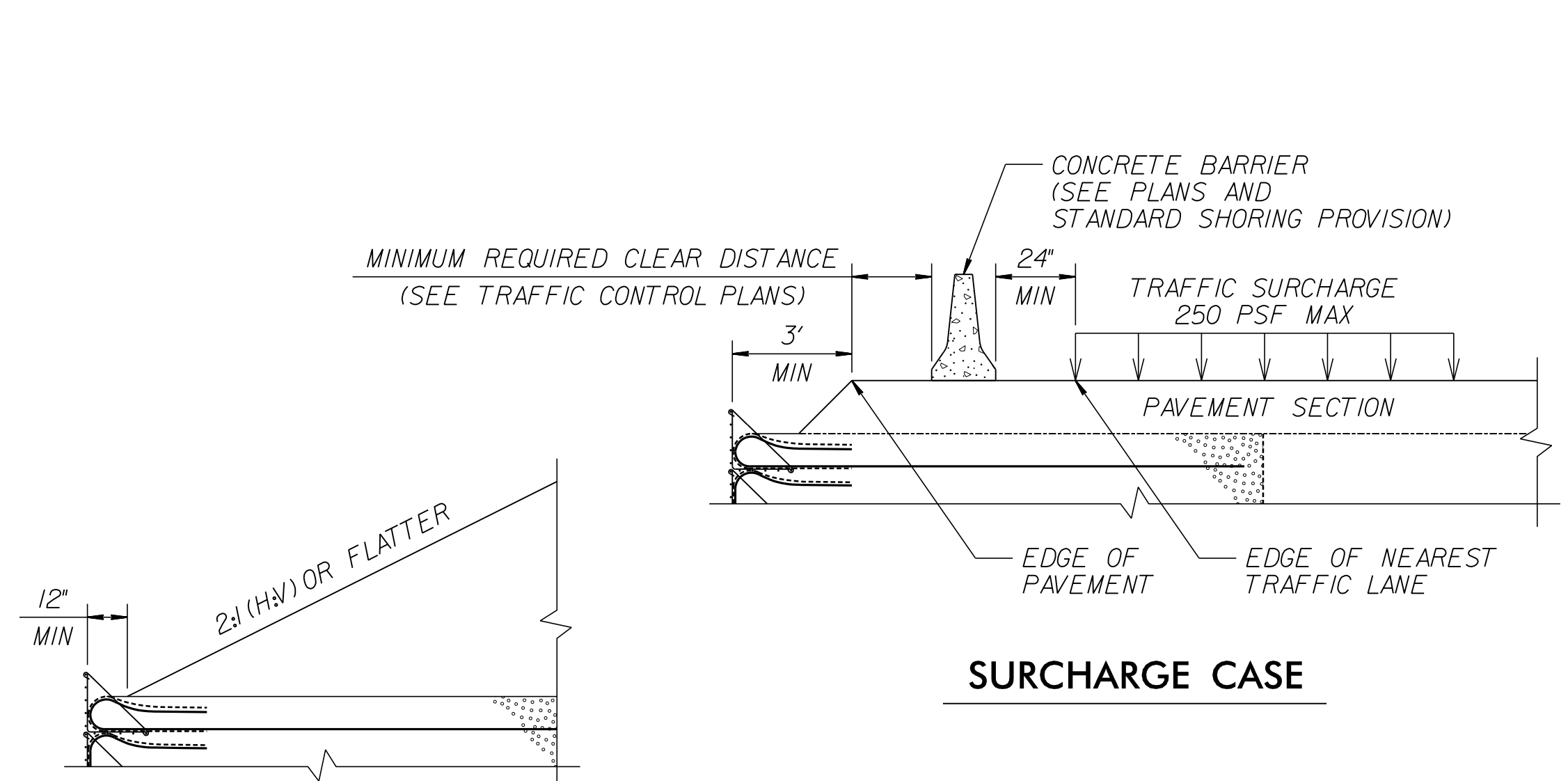
GEOTECHNICAL ENGINEER

ENGINEER



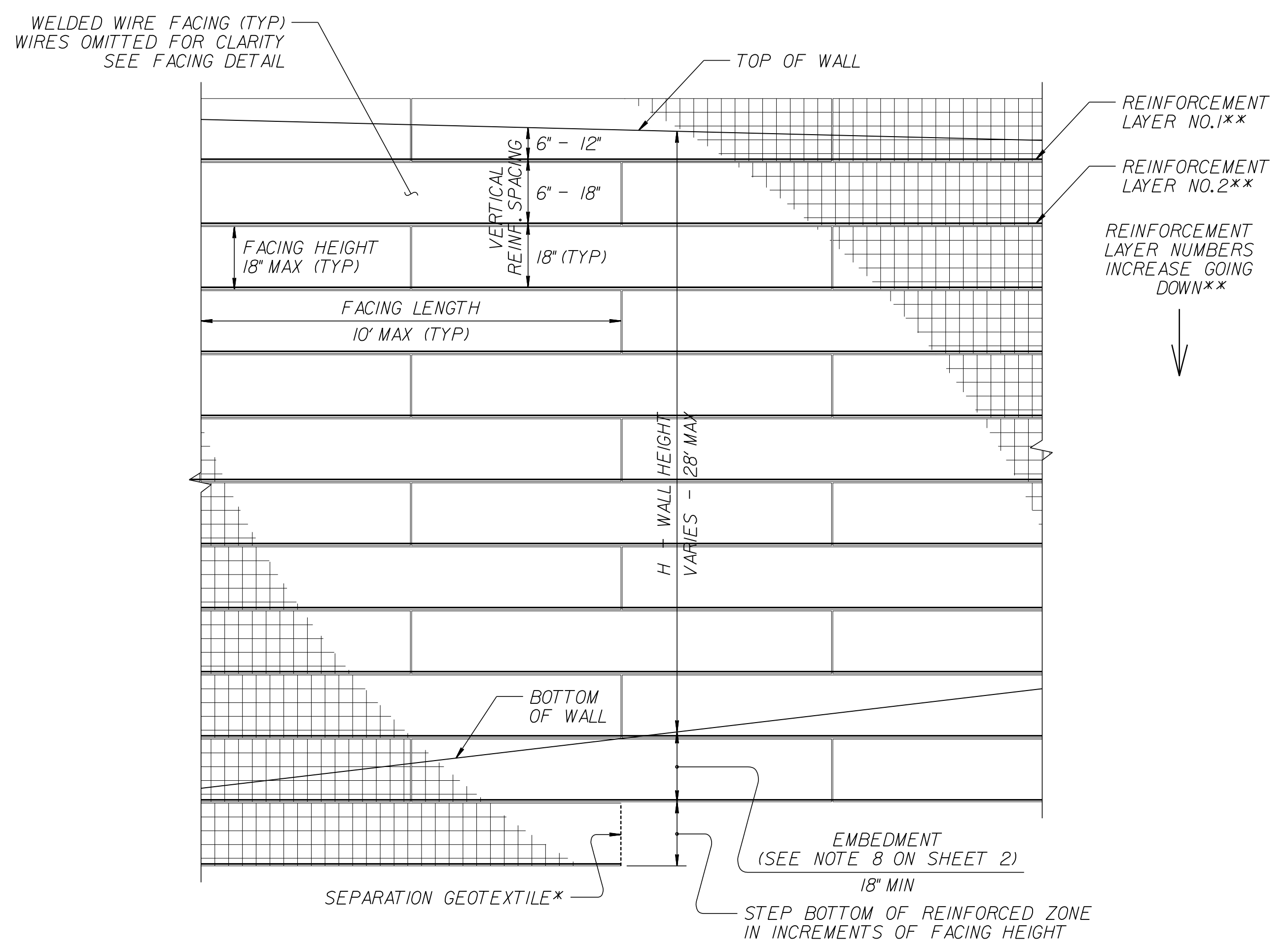
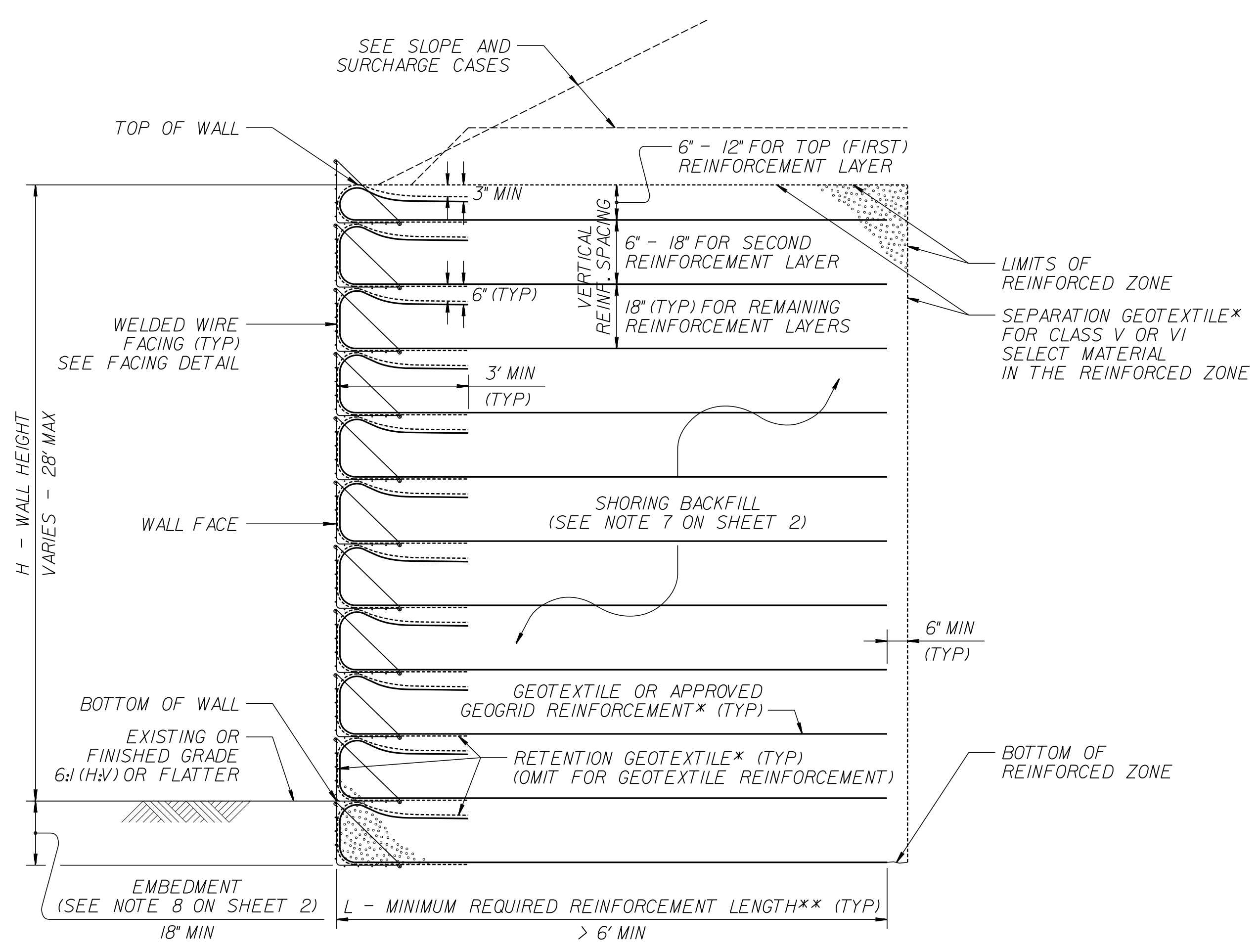
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SLOPE CASE

FACING DETAIL

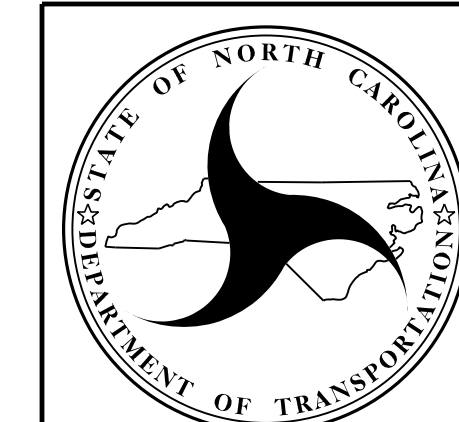


STANDARD TEMPORARY WALL

(FOR STANDARD TEMPORARY WALLS ON STRUCTURES, SEE TEMPORARY WALL ON STRUCTURE DETAIL ON SHEET 2.)  
 \*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.  
 \*\*SEE REINFORCEMENT TABLES ON SHEET 3.

STANDARD TEMPORARY WALL - PARTIAL ELEVATION

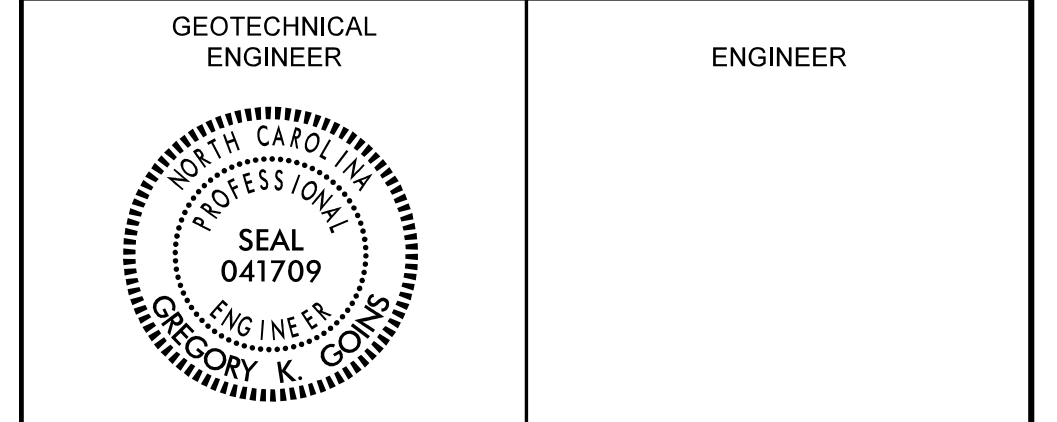
\*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.  
 \*\*SEE REINFORCEMENT TABLES ON SHEET 3.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS  
 GEOTECHNICAL ENGINEERING UNIT

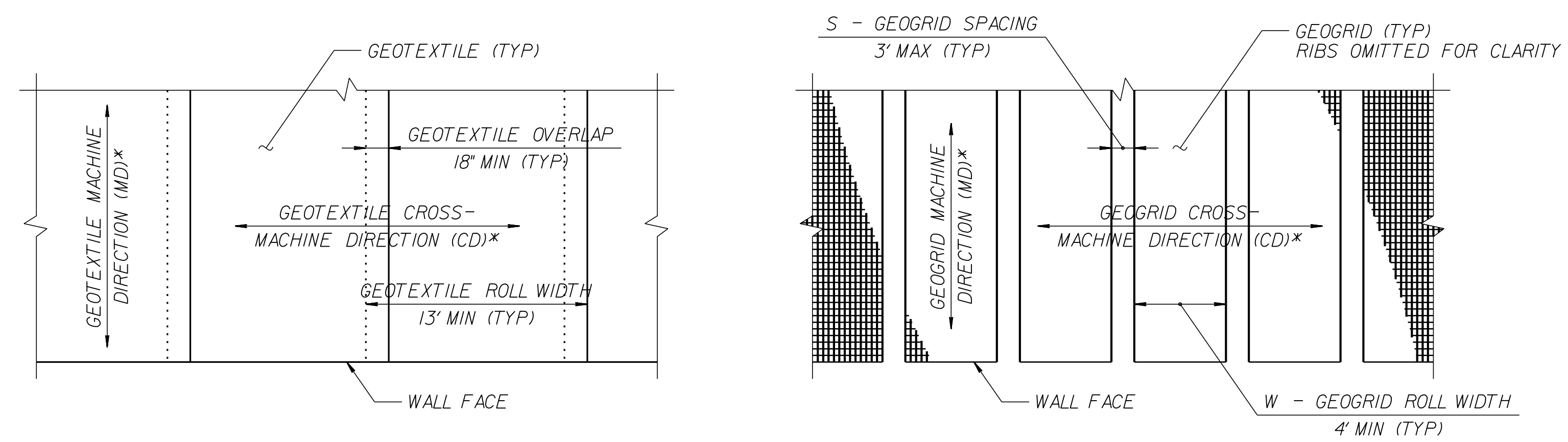
STANDARD DETAIL NO. 1801.02

STANDARD TEMPORARY WALL SHEET 1 OF 3



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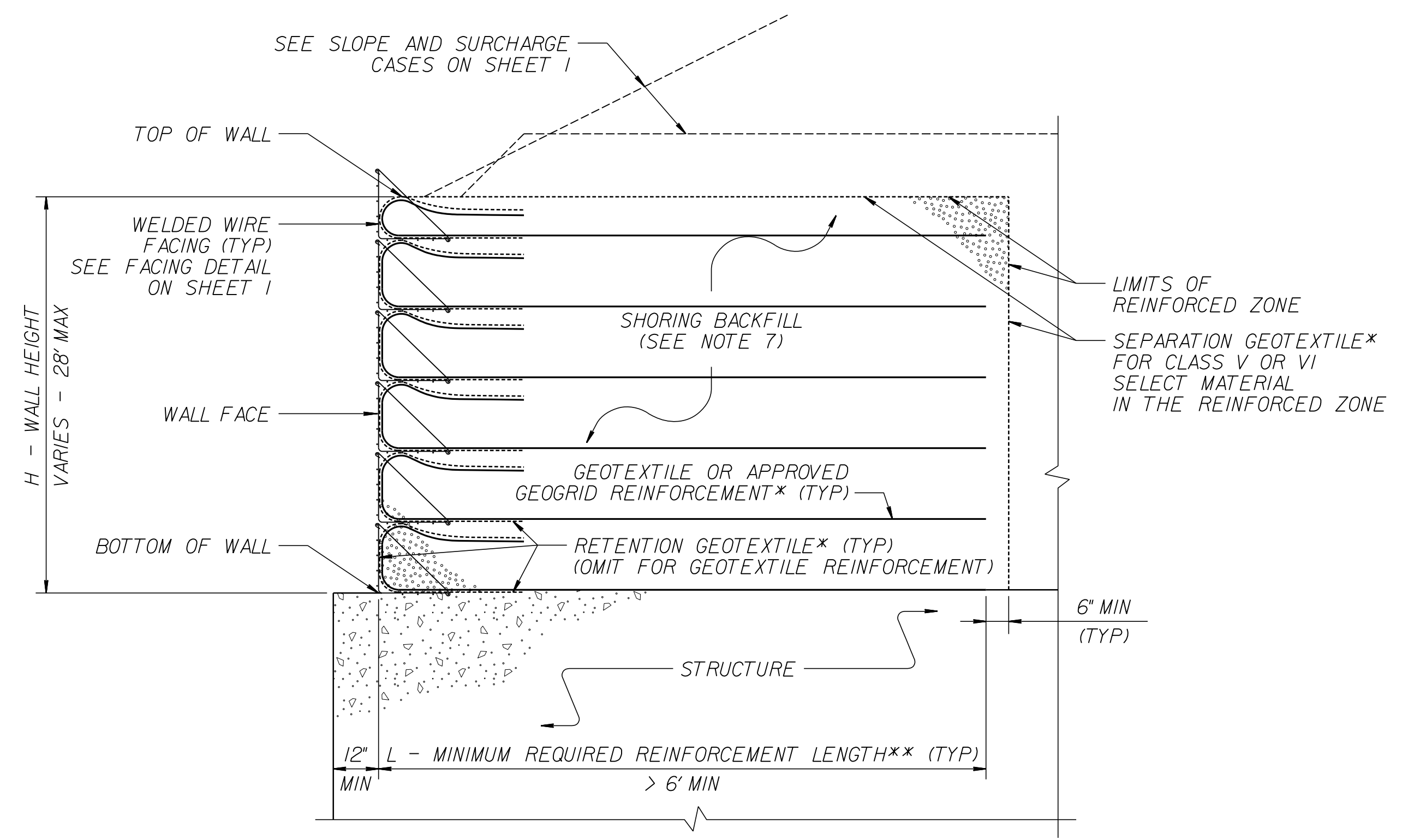


**GEOTEXTILE PLACEMENT**  
 (100% COVERAGE MIN FOR GEOTEXTILE REINFORCEMENT)

**GEOGRID PLACEMENT**  
 (80% COVERAGE MIN FOR GEOGRID REINFORCEMENT –  
 $\frac{W}{W+S} \times 100 \geq 80\%$ ,  
 SEE NOTE 11)

**GEOSYNTHETIC PLACEMENT DETAILS**

(PLAN VIEW)  
 \*SEE NOTE 12.



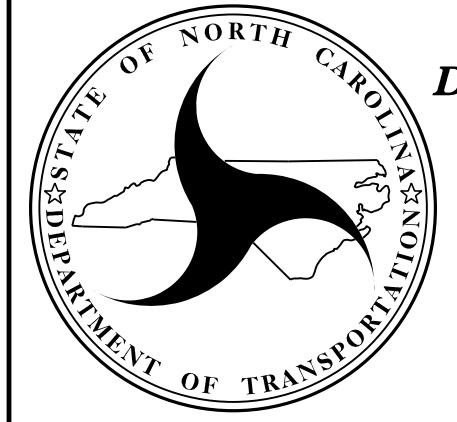
**TEMPORARY WALL ON STRUCTURE DETAIL**  
 \*SEE GEOSYNTHETIC PLACEMENT DETAILS.  
 \*\*SEE REINFORCEMENT TABLES ON SHEET 3.

**NOTES:**

- AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY WALLS AS NOTED IN THE PLANS.
- FOR STANDARD TEMPORARY WALLS, SEE STANDARD SHORING PROVISION.
- STANDARD TEMPORARY WALLS ARE BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS:  
 UNIT WEIGHT,  $\gamma = 120$  PCF  
 FRICTION ANGLE,  $\phi = 30$  DEGREES  
 COHESION,  $c = 0$  PSF
- DO NOT USE STANDARD TEMPORARY WALLS IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
- DO NOT USE STANDARD TEMPORARY WALLS WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS BELOW TEMPORARY WALLS.
- USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, ASSUME GROUNDWATER DEPTH IS LESS THAN 7' BELOW BOTTOM OF REINFORCED ZONE. DO NOT USE STANDARD TEMPORARY WALLS IF GROUNDWATER IS ABOVE BOTTOM OF REINFORCED ZONE.
- DO NOT USE A-2-4 SOIL FOR STANDARD TEMPORARY WALLS AROUND CULVERTS OR IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS FOR SLOPE CASES. DO NOT USE CLASS VI SELECT MATERIAL IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS WITH GEOTEXTILE REINFORCEMENT.
- EMBEDMENT IS NOT REQUIRED FOR STANDARD TEMPORARY WALLS ON STRUCTURES OR ROCK AS DETERMINED BY THE ENGINEER.
- DO NOT USE MORE THAN 4 DIFFERENT REINFORCEMENT STRENGTHS FOR EACH STANDARD TEMPORARY WALL.
- GEOGRIDS ARE TYPICALLY APPROVED FOR ULTIMATE TENSILE STRENGTHS IN THE MACHINE DIRECTION (MD) AND CROSS-MACHINE DIRECTION (CD) OR SHORT-TERM DESIGN STRENGTHS FOR A 3-YEAR DESIGN LIFE IN THE MD BASED ON MATERIAL TYPE. THE LIST OF APPROVED GEOGRIDS WITH DESIGN STRENGTHS IS AVAILABLE FROM:  
[connect.ncdot.gov/resources/Materials/Pages/Materials-Manual-by-Manual.aspx](http://connect.ncdot.gov/resources/Materials/Pages/Materials-Manual-by-Manual.aspx)  
 DEFINE MATERIAL TYPE FROM THE WEBSITE ABOVE FOR SHORING BACKFILL AS FOLLOWS:

MATERIAL TYPE	SHORING BACKFILL
BORROW	A-2-4 SOIL
FINE AGGREGATE	CLASS II, TYPE I OR CLASS III SELECT MATERIAL
COARSE AGGREGATE	CLASS V OR VI SELECT MATERIAL

- IF THE WEBSITE DOES NOT LIST A SHORT-TERM DESIGN STRENGTH FOR AN APPROVED GEOGRID, USE A SHORT-TERM DESIGN STRENGTH EQUAL TO THE ULTIMATE TENSILE STRENGTH DIVIDED BY 3.5 FOR THE GEOGRID REINFORCEMENT.
- FOR GEOGRID REINFORCEMENT WITH LESS THAN 100% COVERAGE, STAGGER REINFORCEMENT SO GEOGRIDS ARE CENTERED OVER GAPS IN THE REINFORCEMENT LAYER BELOW.
  - AT THE CONTRACTOR'S OPTION, REINFORCEMENT MAY BE INSTALLED WITH THE MD PARALLEL TO THE WALL FACE IF BOTH OF THE FOLLOWING CONDITIONS OCCUR:  
 -  $W$  (REINFORCEMENT ROLL WIDTH)  $\geq$  (MINIMUM REQUIRED REINFORCEMENT LENGTH) + 4.5' AND  
 - REINFORCEMENT STRENGTH IN CD  $\geq$  MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD.
  - SUBMIT A "STANDARD TEMPORARY WALL SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING TEMPORARY WALL CONSTRUCTION. STANDARD SHORING SELECTION FORMS ARE AVAILABLE FROM:  
[connect.ncdot.gov/resources/Geological/Pages/Geotech\\_Forms\\_Details.aspx](http://connect.ncdot.gov/resources/Geological/Pages/Geotech_Forms_Details.aspx)
  - DO NOT PLACE SHORING BACKFILL OR REINFORCEMENT UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.
  - FOR STANDARD TEMPORARY WALLS WITH PILE FOUNDATIONS IN THE REINFORCED ZONE, DRIVE PILES THROUGH REINFORCEMENT AFTER CONSTRUCTING TEMPORARY WALLS.
  - DO NOT SPLICE OR OVERLAP REINFORCEMENT SO SEAMS ARE PARALLEL TO THE WALL FACE.
  - CONTACT THE ENGINEER WHEN EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT.
  - FOR STANDARD TEMPORARY WALLS WITH INTERIOR ANGLES LESS THAN 90 DEGREES, WRAP GEOSYNTHETICS AT ACUTE CORNERS AS DIRECTED BY THE ENGINEER.
  - FOR STANDARD TEMPORARY WALLS WITH TOP OF WALL WITHIN 5' OF FINISHED GRADE, REMOVE TOP FACING AND INCORPORATE TOP REINFORCEMENT LAYER INTO FILL WHEN PLACING FILL IN FRONT OF WALL.



**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS**

**GEOTECHNICAL ENGINEERING UNIT**

**STANDARD DETAIL NO. 1801.02**

**STANDARD TEMPORARY WALL SHEET 2 OF 3**

DATE: 11-19-13

GEOTECHNICAL ENGINEER ENGINEER



DocuSigned by: Gregory K. Gowis 10/30/2023 DATE SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SLOPE OR SURCHARGE CASE	GROUNDWATER DEPTH BELOW BOTTOM OF REINFORCED ZONE (SEE NOTE 6 ON SHEET 2) (FT)	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)	H - WALL HEIGHT (FT)																									
			< 4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	
SLOPE CASE	> 0	CLASS II, TYPE I, CLASS III, CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	8	9	11	12	13	13	14	15	16	17	18	19	20	21	22	23	24	24	25	26	27	27	
SURCHARGE CASE	> 0 TO 7 FOR H < 20' > 0 TO 10 FOR H ≥ 20'	ALL SHORING BACKFILL TYPES	6	7	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	17	17	18	19	19	20	21	22	
		A-2-4 SOIL	6	6	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	16	17	18	18	19	20	20	21	
		CLASS II, TYPE I OR CLASS III SELECT MATERIAL	6	6	7	7	8	8	9	10	10	11	11	12	12	13	14	15	15	16	16	17	17	18	18	19	20	
	> 7 FOR H < 20' > 10 FOR H ≥ 20'	CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	7	7	8	8	9	9	10	10	11	12	13	13	14	14	15	15	16	17	17	18	19	19	

**L - MINIMUM REQUIRED REINFORCEMENT LENGTH (FT)**  
(FOR ALL REINFORCEMENT TYPES)

WALL HEIGHT (H) + EMBEDMENT (FT)	NUMBER OF REINFORCEMENT LAYERS*
2.5 - 4	3
4 - 5.5	4
5.5 - 7	5
7 - 8.5	6
8.5 - 10	7
10 - 11.5	8
11.5 - 13	9
13 - 14.5	10
14.5 - 16	11
16 - 17.5	12
17.5 - 19	13
19 - 20.5	14
20.5 - 22	15
22 - 23.5	16
23.5 - 25	17
25 - 26.5	18
26.5 - 28	19
28 - 29.5	20

\*BASED ON VERTICAL REINFORCEMENT SPACING SHOWN ON SHEET 1.

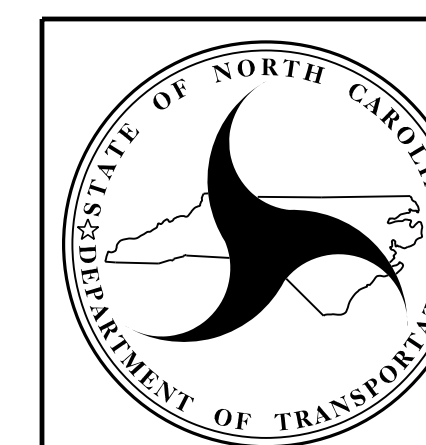
REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL
1	2400	2400	2400	2400	2400
2	2400	2400	2400	2400	2400
3	2400	2400	2400	2400	2400
4	2400	2400	2500	2400	2400
5	2500	2400	3000	2400	2400
6	3000	2400	3500	2800	2400
7	3500	2700	4000	3200	2600
8	4000	3100	4500	3600	2900
9	4500	3500	5000	4000	3200
10	5000	3900	5500	4400	3500
11	5500	4300	6000	4800	3800
12	6000	4700	6500	5200	4100
13	6500	5100	7000	5600	4400
14	7000	5400	7500	6000	4700
15	7500	5800	8000	6400	5000
16	8000	6200	8500	6800	5300
17	8500	6600	9000	7200	5600
18	9000	7000	9500	7600	5900
19	9500	7400	10000	8000	6200
20	10000	7800	10500	8400	6500

**GEOTEXTILE REINFORCEMENT**  
**ULTIMATE TENSILE STRENGTH (LB/FT)**

REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL
1	240	200	340	290	240
2	380	310	520	430	350
3	530	420	700	570	460
4	690	550	870	720	570
5	860	690	1050	860	680
6	1030	830	1220	1000	790
7	1200	970	1400	1150	900
8	1370	1110	1580	1290	1010
9	1550	1240	1750	1430	1120
10	1720	1380	1930	1580	1230
11	1890	1520	2100	1720	1340
12	2060	1660	2280	1860	1450
13	2240	1800	2450	2010	1560
14	2410	1940	2630	2150	1670
15	2580	2080	2800	2290	1780
16	2750	2220	2980	2440	1890
17	2930	2360	3160	2580	2000
18	3100	2500	3330	2720	2110
19	3270	2640	3510	2860	2220
20	3440	2780	3690	3000	2330

**GEOGRID REINFORCEMENT**  
**SHORT-TERM DESIGN STRENGTH (LB/FT)**  
(SEE NOTE 10 ON SHEET 2.)

**MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD**  
(SEE NOTE 9 ON SHEET 2.)  
\*SEE PARTIAL ELEVATION ON SHEET 1 FOR REINFORCEMENT LAYER NUMBERING.



NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
**GEOTECHNICAL ENGINEERING UNIT**

STANDARD DETAIL NO. 1801.02

STANDARD TEMPORARY WALL  
SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS  
**SUMMARY OF EARTHWORK**  
IN CUBIC YARDS

CHAIN	BEGINNING STATION	ENDING STATION	UNCL. EXCA. C.Y.	UNDERCUT C.Y.	EMBANK. +% C.Y.	BORROW C.Y.	WASTE C.Y.	CHAIN	BEGINNING STATION	ENDING STATION	UNCL. EXCA. C.Y.	UNDERCUT C.Y.	EMBANK. +% C.Y.	BORROW C.Y.	WASTE C.Y.
<b>SUMMARY 1</b>								<b>SUMMARY 7</b>							
-L- (L_DET1)	12+09.43	24+50.00	186		25,763	25,577		-L-	63+21.00	78+00.00	1,314		6,487	5,173	
-L- (L_DET1)	26+00.00	42+38.95	22,516		18,242		4,274	-L- RT	78+00.00	104+68.00	31,623		22,299		9,324
-L-	54+37.00	63+21.00	409		2,392	1,983		-L-	106+16.00	107+00.00	2,640		35		2,605
-L- (L_DET2)	79+85.99	104+00.00	79,655		3,308		76,348	-L-	107+00.00	108+29.00	2,606		8		2,598
-L-	101+50.00	107+00.00	17,485		1,416		16,069	-L- LT	108+29.00	133+82.00	44,045	626	28,436		16,235
-L- (L_DET3)	121+83.12	134+61.40	8,475	1,298	22,645	14,170	1,298	-L- RT	142+67.00	157+00.00	815		20,047	19,232	
-L- (L_DET4)	140+70.08	173+66.15	46,284		20,585		25,699	-L-	174+00.00	183+25.00	124,370		65,206		59,164
-Y12-	18+67.00	25+31.00	107		73		34	-L- (L_DET6)	183+25.00	199+00.00	84,904		626		84,278
-Y12-	29+81.00	35+00.00	115		2		113	-L- RT	199+50.00	209+49.00	41		596	555	
-DR7-	10+09.43	13+75.00	20,117				20,117	-Y5-	10+20.07	11+40.00			486	486	
-DR20-	10+55.00	10+98.32	31		25		6	-Y6-	11+30.00	17+06.36	10,279		386		9,893
<b>SUBTOTAL</b>			<b>195,380</b>	<b>1,298</b>	<b>94,451</b>	<b>41,730</b>	<b>143,957</b>	-Y8-	10+50.00	12+64.64	1		827	826	
<b>SUMMARY 2</b>								-DR8-	12+00.00	13+68.26	79		138	59	
-L-	49+50.00	54+37.00	780		3,376	2,596		-DR11-	10+13.88	11+75.00	6		3,342	3,336	
-Y3-	10+31.53	13+50.00	2,838		3,676	838		-DR12-	10+18.00	11+55.00			1,468	1,468	
-Y3A-	11+50.00	12+40.00	40		88	48		-DR13-	10+11.55	11+40.00	11		26	15	
-DR6-	12+22.00	12+81.40	37		13		24	-DR18-	10+18.00	11+45.00			1,336	1,336	
<b>SUBTOTAL</b>			<b>3,695</b>		<b>7,152</b>	<b>3,481</b>	<b>24</b>	-DET_DR18-	10+09.97	11+40.00	354		26		328
<b>SUMMARY 3</b>								-DR18B-	10+54.96	11+85.00	13		569	556	
-L- (L_DET1)	24+50.00	26+00.00			6,524	6,524		-DET_DR18B-	10+21.08	11+40.00	684				684
-DET_Y1-	10+40.00	13+13.59	333		2,063	1,730		-DR19-	10+13.46	11+75.00	1		719	718	
<b>SUBTOTAL</b>			<b>333</b>		<b>8,587</b>	<b>8,254</b>		-DET_DR19-	10+17.17	11+40.00	34		125	91	
<b>SUMMARY 4</b>								-DET_DR22-	10+43.66	14+12.68	9,914		91		9,823
-L- RT	10+25.00	40+00.00	81,895		8,668		73,227	-DR29-	10+15.28	12+01.12	5,076		59		5,017
-L- RT	40+00.00	49+50.00	145,771		123		145,649	<b>SUBTOTAL</b>			<b>318,810</b>	<b>626</b>	<b>153,339</b>	<b>33,851</b>	<b>199,947</b>
-Y2-	10+18.00	14+70.00	14,188		130		14,058	<b>SUMMARY 8</b>							
<b>SUBTOTAL</b>			<b>241,854</b>		<b>8,920</b>		<b>232,934</b>	-L- RT	139+60.00	142+01.00	75		473	398	
<b>SUMMARY 5</b>								<b>SUBTOTAL</b>			<b>75</b>		<b>473</b>	<b>398</b>	
-L- LT	25+00.00	30+00.00	8,865		71		8,794	<b>SUMMARY 9</b>							
-L- (REMOVE L_DET1)	25+00.00	30+00.00	4,803		733		4,070	-L-	142+01.00	145+00.00	7		2,118	2,111	
-Y1-	10+30.00	16+59.03	4,587		905		3,682	<b>SUBTOTAL</b>			<b>7</b>		<b>2,118</b>	<b>2,111</b>	
-Y1- (REMOVE DET_Y1)	13+00.00	15+50.00			158	158		<b>SUMMARY 10</b>							
<b>SUBTOTAL</b>			<b>18,255</b>		<b>1,867</b>	<b>158</b>	<b>16,546</b>	-L- LT	142+01.00	151+78.00	2		4,885	4,883	
<b>SUMMARY 6</b>								-L- (REMOVE L_DET4)	142+01.00	151+50.00	9,361		7,433		1,928
-L- LT	137+29.00	143+22.00	3,358		13		3,345	-Y10A-	11+00.00	13+88.20	262		12,840	12,578	
<b>SUBTOTAL</b>			<b>3,358</b>		<b>13</b>		<b>3,345</b>	-DR16-	10+45.00	11+79.53	16		743	727	
<b>SUMMARY 7</b>								-DR17-	10+96.61	11+48.88	49			49	
<b>SUMMARY 8</b>								-DR26-	10+55.00	11+30.00	6		138	132	
<b>SUMMARY 9</b>								<b>SUBTOTAL</b>			<b>9,696</b>		<b>26,039</b>	<b>18,320</b>	<b>1,977</b>
<b>SUMMARY 10</b>								<b>SUMMARY 11</b>							
<b>SUMMARY 11</b>								-L- RT	107+00.00	114+00.00	8,321		79		8,242
<b>SUMMARY 12</b>								-Y7-	10+17.15	13+10.00	961		85		876
<b>SUMMARY 13</b>								-DR14-	10+18.18	12+38.00	7,060				7,060
<b>SUMMARY 14</b>								-DR28-	10+18.00	11+77.00	2,593				2,593
<b>SUMMARY 15</b>								<b>SUBTOTAL</b>			<b>18,935</b>		<b>164</b>		<b>18,771</b>
<b>SUMMARY 16</b>								<b>SUMMARY 16</b>							
<b>SUMMARY 17</b>								-L- RT	157+00.00	174+00.00	187,807		1,206		186,601
<b>SUMMARY 18</b>								-Y11-	10+18.07	14+43.61	1,595		8,275	6,680	
<b>SUMMARY 19</b>								-Y11A-	10+09.69	12+35.00	392		967	575	
<b>SUMMARY 20</b>								-Y11A-	13+34.26	14+36.90	7		274	267	
<b>SUMMARY 21</b>								<b>SUBTOTAL</b>			<b>189,801</b>		<b>10,722</b>	<b>7,522</b>	<b>186,601</b>

Note: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

8/17/19  
10/15/2023  
Responsible  
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STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS  
**SUMMARY OF EARTHWORK**  
IN CUBIC YARDS

CHAIN	BEGINNING STATION	ENDING STATION	UNCL. EXCA. C.Y.	UNDERCUT C.Y.	EMBANK. +% C.Y.	BORROW C.Y.	WASTE C.Y.
<b>SUMMARY 13</b>							
-L- LT	10+25.00	25+00.00	4,432		1,685		2,747
-L- (REMOVE L_DET1)	14+50.00	25+00.00	20,063		53		20,010
-L- LT	30+00.00	44+00.00	3,218		558		2,660
-L- (REMOVE L_DET1)	30+00.00	42+00.00	12,338		2,495		9,843
-L- LT	48+00.00	51+77.00	144		559	415	
-L- LT	78+00.00	101+50.00	1,141		5,159	4,018	
-L- (REMOVE L_DET2)	84+00.00	103+00.00	1,311		23,216	21,905	
-L-	104+68.00	106+16.00	11		115	104	
-L- RT	114+00.00	139+60.00	21,231		713		20,518
-L- (REMOVE L_DET3)	122+00.00	133+00.00	15,215		8,741		6,474
-L- LT	133+82.00	141+50.00	2,389		815		1,574
-L- LT	151+78.00	172+50.00	38,502		362		38,140
-L- (REMOVE L_DET4)	151+78.00	173+50.00	2,847		3,973	1,126	
-L-	183+25.00	197+00.00	31,760	546	6,688		25,618
-L- LT	199+30.00	209+00.00	17		942	925	
-Y4-	12+72.00	15+28.75	564		6,876	6,312	
-Y9-	10+12.02	13+50.00	420	60	392		88
-Y10B-	14+00.00	19+08.01	27,610		124		27,486
-Y12- RT	18+66.98	24+00.00	104				104
-DR1-	9+80.00	11+11.20	11		508	497	
-DR2-	10+20.00	12+20.00	1,135		864		271
-DR5-	11+20.00	12+45.23	6		84	78	
-DR9-	10+10.00	11+72.31			374	374	
-DR10-	11+10.00	12+15.84			503	503	
-DR15-	10+12.00	13+30.00	2,635		330		2,305
-DR21-	10+00.00	10+94.73	437				437
-DR25-	10+05.00	12+29.68	846		40		806
-DR27-	9+82.00	11+07.42	21		156	135	
<b>SUBTOTAL</b>			<b>188,408</b>	<b>606</b>	<b>66,324</b>	<b>36,393</b>	<b>159,083</b>
<b>SUMMARY 14</b>							
-Y10B-	11+86.00	14+00.00	1,034		155		879
-Y13-	10+12.47	12+75.00	3,829		1		3,828
<b>SUBTOTAL</b>			<b>4,863</b>		<b>156</b>		<b>4,707</b>
<b>SUMMARY 15</b>							
-L- (REMOVE L_DET6)	179+50.00	197+00.00	1,051		77,196	76,145	
-L-	197+00.00	199+50.00	519		300		219
-DR22-	10+12.00	11+00.00	683				683
<b>SUBTOTAL</b>			<b>2,253</b>		<b>77,496</b>	<b>76,145</b>	<b>902</b>
<b>SHEET TOTALS</b>			<b>1,195,723</b>	<b>2,530</b>	<b>457,821</b>	<b>228,362</b>	<b>968,794</b>
<b>ADDITIONAL UNDERCUT</b>				<b>9,000</b>	<b>10,800</b>	<b>10,800</b>	<b>9,000</b>
<b>ROCK WASTE USED TO REPLACE BORROW</b>						<b>-41,084</b>	<b>-41,084</b>
<b>ADJUST FOR ROCK WASTE</b>					<b>-8,217</b>	<b>-8,217</b>	
<b>ELIMINATE SHRINKAGE FACTOR ON ROCK</b>					<b>-9,860</b>	<b>-9,860</b>	
<b>UNSUITABLE MATERIAL</b>			<b>500</b>				<b>500</b>
<b>EARTH WASTE IN LIEU OF BORROW</b>						<b>-180,001</b>	<b>-180,001</b>
<b>GRAND TOTAL</b>			<b>1,196,223</b>	<b>11,530</b>	<b>450,544</b>		<b>757,209</b>
<b>SAY</b>			<b>1,197,000</b>				

-L- PAVEMENT STRUCTURE VOLUME = 26,237 C.Y.

SHOULDER BORROW = 1,400 C.Y.

DRAINAGE DITCH EXCAVATION = 15,980 C.Y.

SHALLOW UNDERCUT = 500 C.Y.

SHALLOW UNDERCUT (CONTINGENCY) = 500 C.Y.

TOTAL UNCLASSIFIED EXCAVATION - ACCEPTABLE = 18,500 C.Y.

BUT NOT TO BE USED IN TOP 3' OF EMBANKMENT OR BACKFILL

-L- STA. 21+75 TO STA. 26+75

-L- STA. 137+25 TO STA. 139+25

-L- STA. 170+75 TO STA. 172+75

NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT. = C.Y.





STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**TEMPORARY GUARDRAIL SUMMARY**

SURVEY LINE	BEG. STA.	END STA.	LOCATION	LENGTH			WARRANT POINT		"N" DIST. FROM E.O.L.	TOTAL SHOULDER WIDTH	FLARE LENGTH		W		ANCHORS											CRASH CUSHION	PORTABLE CONCRETE BARRIER	REMOVE & RESET EXISTING GUARDRAIL	REMOVE & STOCKPILE EXISTING GUARDRAIL	REMARKS								
				STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END			APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	XI	GREU TL-3	MEDIA N TL-3	XIII	CAT-1	B-77	AT-1	TES	TYPE III														
L_DET1	10+00.66	19+41.12	LT	937.50'			--	10+41	4'	6'	50	50	1	1																								
L_DET1	19+71.15	24+64.90	LT	500.00'			24+93	--	4'	6'	50	50	1	1																								
L_DET1	16+78.71	34+97.64	RT				19+74	28+42																														
L_DET1	29+73.26	36+01.57	LT	625.00'			35+22	30+85	4'	6'	50	50	1	1																								
L_DET1	36+21.07	39+98.00	LT	375.00'			38+21	39+72	4'	6'		50		1																								
L_DET2	14+58.18	16+17.91	LT	160.38'			21+70	16+18	4'	6'		50		1																								
L_DET2	21+70.64	23+31.01	LT	160.38'			21+70	16+18	4'	6'		50		1																								
L_DET2	16+17.91	21+69.85	LT									50																										554
L_DET3	10+00.00	18+02.66	LT									50		1																								
L_DET3	11+18.04	18+00.24	RT	687.50'			12+72	17+34	4'	6'	50	50	1	1																								
L_DET4	12+62.98	18+93.37	LT	637.50'			17+93	13+34	4'	6'	50	50	1	1																								
L_DET4	29+00.00	39+50.00	RT	1050.00'			30+50	39+00	4'	6'	50	50	1	1																								
L_DET4	39+07.46	42+98.89	LT	387.50'			41+80	40+28	4'	6'		50		1																								
TEMPORARY GUARDRAIL (TRAFFIC CONTROL) : PHASE 1																																						
-L-	18+89.19	21+14.73	LT	225.00'			19+39	20+64	2'	4'	50	50	1	1																								
-L-	21+93.69	26+94.01	LT	500.00'			22+44	26+44	2'	4'	50	50	1	1																								
-L-	28+03.20	31+28.32	LT	325.00'			30+78	28+53	2'	4'	50	50	1	1																								
-L-	68+26.27	73+91.00	RT	537.50'	43.75'		68+77	73+91	2'	4'	50		1	1							1																	
-L-	71+55.00	71+75.00	LT		37.50'		71+75		2'	4'																												
-L-	169+73.63	173+62.18	LT	387.50'			39+58	42+99	2'	4'	50		1																									
-L-	153+09.00	160+84.00	RT																																			
TEMPORARY GUARDRAIL (TRAFFIC CONTROL) : PHASE 2																																						
-L-	78+00.00	78+99.00	LT																																			
			SHEET TOTALS	7495.75	81.25																0	0	24	0	0	1	2	1	0	0								99
LESS ANCHOR DEDUCTIONS																																						
			QUANTITY	LF PER EA	TOTAL LF																																	
	GREU TL-3		24	50	1200																																	
	CAT-1		1	6.25	6.25																																	
	TYPE III		0	18.75	0																																	
	TYPE B-77		2	22.875	45.75																																	
	AT-1		1	6.25	6.25																																	
	R-5861 TOTAL (LF)			6,250.00	68.75																																	
	R-5861 SAY (LF)			6,262.50	75.00																																	
															0	0	24	0	0	1	2		0	0	4	3102	875											
SEE TMP PLANS FOR MORE INFORMATION																																						

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS REGIONAL TIER

Note: Invert Elevations Indicated are for Information Purposes only and should be verified by the contractor for project construction stakeout.

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

Main data table with columns for LINE & STATION, OFFSET, STRUCTURE NO., TOP ELEVATION, INVERT ELEVATION, DRAINAGE PIPE (RCP, CSP, CAAP, HDPE, or PVC), R.C. PIPE CLASS III, R.C. PIPE CLASS IV, PIPE AS NOTED, ENDWALLS, QUANTITIES FOR DRAINAGE STRUCTURES, FRAMES, GRATES, AND HOOD STANDARD, CONCRETE TRANSITIONAL SECTION, and ABBREVIATIONS. Includes sections for SHEET 4 and SHEET 5.

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PKK logo and contact information: P: (919) 378-9500, 8601 Six Forks Road, Forum 1, Suite 700, Raleigh, North Carolina 27615-3960, NC License No. F-0112. Also includes website www.pkk.com and tagline 'Responsive People | Creative Solutions'.























NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
REGIONAL TIER

Note: Invert Elevations indicated are for Information Purposes only and should be verified by the contractor for project construction stakeout.

LIST OF TEMPORARY PIPES, ENDWALLS, ETC. (FOR PIPES 54" & OVER)

Main table with columns for Station, Size, Thickness, Offsets, Structure No., Top/Invert Elevations, R.C. Pipe, C.S. Pipe, Temporary Pipe, Trenchless Installation, Reinforced Endwalls, Frames, Grates, and Hoods. Includes a summary row at the bottom for 'TEMP & DETOUR TOTALS' and 'PROJECT TOTALS'.

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STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

### SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
L	17+75	18+75	LT/RT	SD	200
L	23+50	30+25	LT/RT	SD	1,350
L	43+50	45+50	LT/RT	SD	400
L	73+50	75+50	LT/RT	SD	400
L	88+25	88+75	LT/RT	SD	100
L	105+00	111+00	LT/RT	SD	1,200
L	149+25	151+00	LT/RT	SD	350
L_DET2	18+40	18+90	LT/RT	SD	100
L_DET2	18+70	20+80	LT/RT	SD	420
Y1	15+00	16+77	LT/RT	SD	354
Y2	10+00	11+50	LT/RT	SD	300
Y6	15+00	17+23	LT/RT	SD	446
Y9	13+20	13+50	LT/RT	SD	60
Y10B	16+30	19+50	LT/RT	SD	640
CONTINGENCY					500
TOTAL LF:					6,820

\*UD = Underdrain  
\*BD = Blind Drain  
\*SD = Subsurface Drain

### SUMMARY OF AGGREGATE SUBGRADE /STABILIZATION

LINE	Station	Station	Aggregate Type* ASU(1/2)/ AST	Aggregate Thickness INCHES [8" for ASU(2)]	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Subgrade Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
L	158+75	160+75	ASU	12	350	700	1050		
DR17	10+20	11+40	ASU	12	100	200	300		
DR21	10+00	10+95	ASU	12	50	100	150		
CONTINGENCY					500	1000	1500		
TOTAL CY/TONS/SY:					1000	2000**	3000**	0	0

\*ASU(1/2) = Aggregate Subgrade (Type 1 or 2)  
\*AST = Aggregate Stabilization

\*\*Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Subgrade Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

### SUMMARY OF REINFORCED SOIL SLOPES AND SLOPE EROSION CONTROL

LINE	Beginning Slope/ RSS (H:V)	Approx. Station	Ending Slope/ RSS (H:V)	Approx. Station	Location LT/RT	Reinforced Soil Slope (RSS) SY	Geocells SY	Coir Fiber Mat SY	Matting for Erosion Control SY
L	1:1	93+25	1:5:1	97+25	RT	900	900		
	1.5:1	119+75	1:5:1	121+25	LT	600	600		
TOTAL SY:						1500	1500	0*	0**

\*Total square yards of "Coir Fiber Mat" is only the estimated quantity for slopes steeper than 2:1 (H:V) and may only represent a portion of the coir fiber mat quantity shown in the Item Sheets of the Proposal.

\*\*Total square yards of "Matting for Erosion Control" is only the estimated quantity for RSS and may only represent a portion of the matting quantity shown in the Item Sheets of the Proposal.

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STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

# PARCEL INDEX SHEET

PARCEL No.	SHEET No.	PROPERTY OWNER NAME	PARCEL No.	SHEET No.	PROPERTY OWNER NAME
1	4	JAMES S. SCROGGS	60	12	ROBERT IRWIN, ET AL
2	4	SARA INC	61	12	BRUCE BROCKETT
3	4,5	TRACY MORROW KEPHART, ET AL	62	12,13	MGHI INTRESTS LLC
4	4	WILMA EVA HUGHES ESTATE	63	12,13	MANUEL HERNANDEZ
5	4	AMY M. STILES	64	13	BRUCE C. CHYNOWETH
6	4,5	TRACEY M. KEPHART	65	13	TITAN SELF STORAGE LLC
7	4,5	JAMES TANNER	66	13	RICHARD DIEHL
8	5	JOHNNY R. NATION	67	13	RICHARD M. STOVER
9	5	JASON AND CHRISTINA HOBBY LIVING TRUST	68	13,14	FRONTIER COMMUNICATIONS OF THE CAROLINAS, LLC
10	5	DAVID HOBBY	69	13	GENE P. FARMER
11	5	JAMES S. SCROGGS	70	13	BARBARA A. ROBERSON
12	5	RICHARD S. HOHMAN	71	13	NO CLAIM
13	5,6	JANET B. STILES	72	13	WILLIAM CONNELL, HEIRS
14	5	CHRISTOPHER W. WOOD	73	13,14	RENEGADE SELF-STORAGE, LLC
15	6	MURPHY HIGHWAY LLC	73A	14	RENEGADE SELF-STORAGE, LLC
16	6	ROY C. BEBEE	74	14	WILLIAM J. KOLB
17	6	ROY C. BEBEE	75	14	KELLY R. HOPKINS AND WIFE, ANGELIA HOPKINS; SHELIA HOEPNER AND HUSBAND KENNETH HOEPPNER
18	6,7	CHEROKEE WELL DRILLING & PUMP CO INC	76	14	CARLY HOPKINS
19		NO CLAIM	77	14	KELLY HOPKINS AND SHEILA H. HOEPPNER
20	7	NAOMI MARTIN	78		DELETED
21	7	BELLVIEW COMMUNITY FIRE DEPT	79	14	GEORGETTE HOWERTON
22	7,8	RAYMONDE T. BAUMEISTER	80	14	BRENT HEENAN
23	6,7	SUELLEN CHAVET	81	14	JUSTIN M. HEDDEN
24	7	DAVID FRANKLIN MILLER	82	14,15	FREDERICK DEAN DALRYMPLE, LIFE ESTATE
25		NO CLAIM	83	14,15	CHRISTINA L. GUTHRIE
26	8	RAYMONDE T. BAUMEISTER	84	14,15	ALFRED F. KINCAID ESTATE
27	8	RAYMONDE T. BAUMEISTER	85	15	BARBARA GUTHRIE
28	8	KATHERINE L. MERCER	86	15	BARBARA GUTHRIE
29	8	DANIEL J. WEEKS	87	15,21	ERICA LEANN PORTAL
30	8	TIMOTHY J. BARNEY	88	15,16,21	JAMMIE ADAMS SAVUGOT
31	8	DENNIS R. ABSHIER & WIFE BARBARA ABSHIER	89		DELETED
32	8	D&G RESIDENTIAL SERVICES, LLC	90	15	PHOEBE M. HEDDEN
33	8,9	JAMES TARASZEWSKI	91	15	CHARLES GREEN HEIRS, ET AL
34	9	MARGARET ELWOOD	92	15	PAUL W. BEASTER
35	9	DEWEY SENEAL	93	15,16	JARRAD S. HAMPTON
36	9	DON SMITH AND WIFE, EDNA SMITH	94		DELETED
37	9,10	FRANCES H. HARTZELL	95	15,16	REBEKA FRANKLIN AND HUSBAND, CORY FRANKLIN
38	8,9	TRICIA WISEMAN BURKE, AND HUSBAND WILLIAM BURKE	96	16,21	RONNIE D. WHITENER AND BETTY SUE WHITENER
39	9	HOMER A. SHAFFER	97	16,21	RITA COLLEEN KELLY AND CRYSTAL KELLY ROGERS
40	9	WILLIAM J. TOVAR, ET UX	98	16,17	PAUL Q. RAPER HEIRS AND WILLIAM DALE CLONTS
41	9,10	CANDACE LOVE, HEIRS	98A		DELETED
41A	10	MARGUERITE C PROSPERO O'BRYON	99	17	OLIVER MINOR
42	10	HOMER A. SHAFFER JR.	100	17,18	PATRICIA FOSTER AND TRACY FOSTER
43	10	HOWARD J. BAUMEISTER	101	17,18	EVA B. DECKER
44	10,11	WILLIAM E. CAMPBELL HEIRS, ET AL	102	18	DAVID S. DUFFEK AND WIFE, SHANNON F. DUFFEK
45	10	KATIE A. ERWIN	103	18	JOHN T. MCGUIRE
46		DELETED	104	18	DELETED
47	10,11	RAYMONDE T. BAUMEISTER	105	18	CHAD DECKER AND WIFE, SONYA DECKER
48	11	BRITTANY RHODES	106	21	STEPHANIE TANNER
49	11	KENNETH B. LANCE	107	21	BETTY BEAVERS ESTATE
50	11,12	THOMAS STADTHAGEN AND WIFE, MICHELE STADTHAGEN	108	10	ROBERT TRUETT
51	11	PATRICIA E. PRATT			
52	11	GENE SNEED			
53	11	KENNETH P. LANCE			
54	11	CLINTON L. ROBERTS JR.			
55	11	DIANE J. ANDERSON			
56	11	HAROLD M. ROBERSON			
57	11	NO CLAIM			
58	11,12	SMOKY MOUNTAIN HOLDING			
59	12,13	SARAH SKOMP			

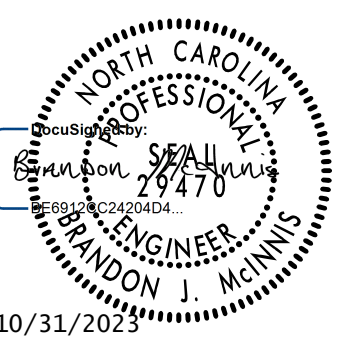
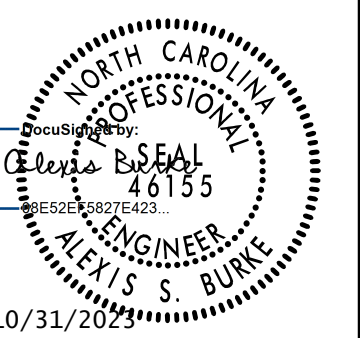
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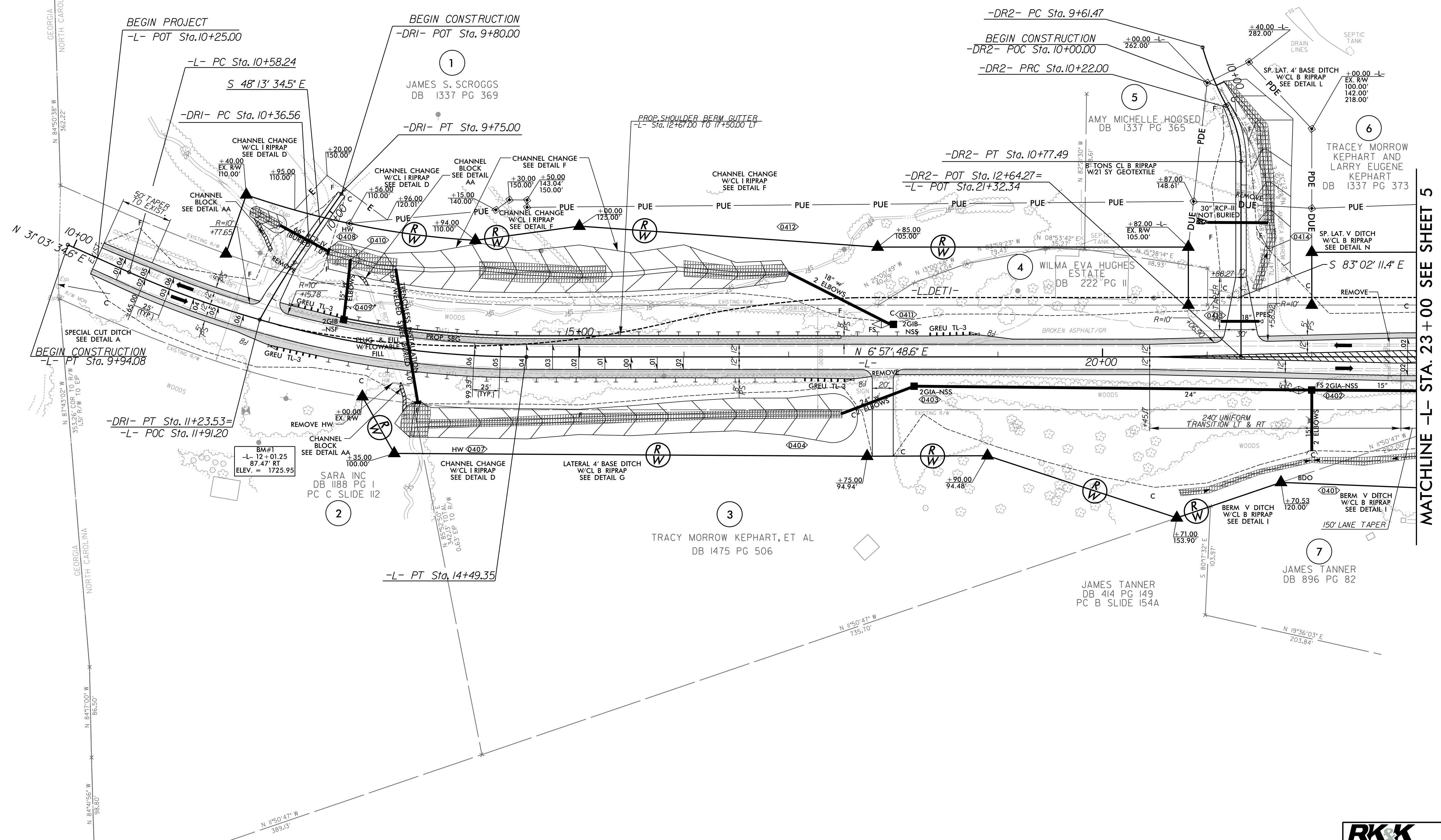


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-L-	-DRI-	-DR2-	
PI Sta 12+56.73	PI Sta 10+80.09	PI Sta 9+91.87	PI Sta 10+50.35
$\Delta = 24^{\circ}05'46.1''$ (LT)	$\Delta = 6^{\circ}06'49.5''$ (LT)	$\Delta = 13^{\circ}05'15.7''$ (LT)	$\Delta = 28^{\circ}53'59.0''$ (RT)
D = 6'09'39.0"	D = 7'01'48.6"	D = 21'37'15.8"	D = 52'05'13.5"
L = 391.12'	L = 86.96'	L = 60.53'	L = 55.48'
T = 198.49'	T = 43.52'	T = 30.40'	T = 28.35'
R = 930.00'	R = 815.00'	R = 265.00'	R = 110.00'
SE = 0.06	SE = 0.02	SE = 0.02	SE = 0.02
RO = 150'	DS = 15 MPH	DS = 15 MPH	DS = 15 MPH
DS = 50 MPH (MATCH EXIST)			

NAD 83/2011

PROJECT REFERENCE NO. R-5861	SHEET NO. 4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
 BRANDON J. MEEKS 10/31/2023	 ALEX S. BURNE 10/31/2023
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>	

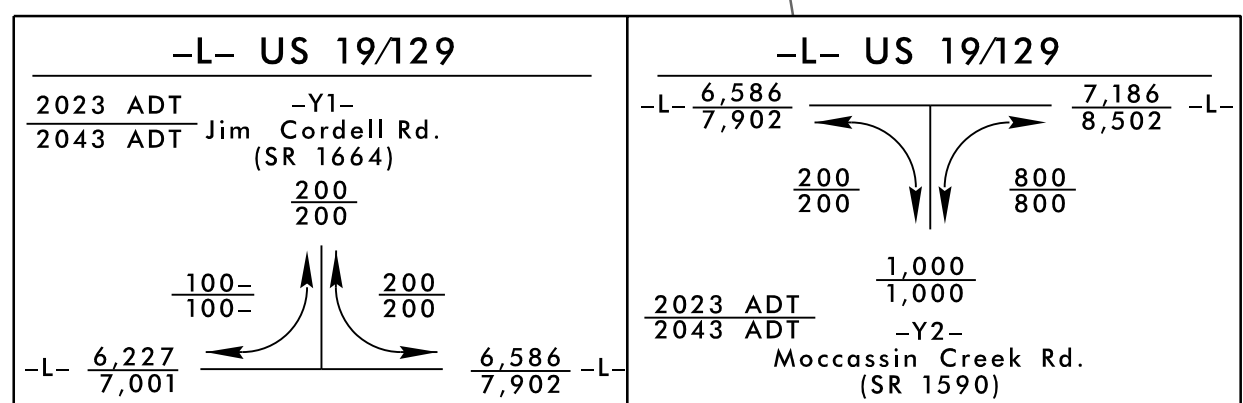
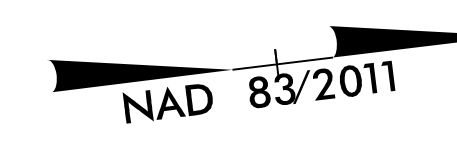


MATCHLINE -L- STA. 23+00 SEE SHEET 5

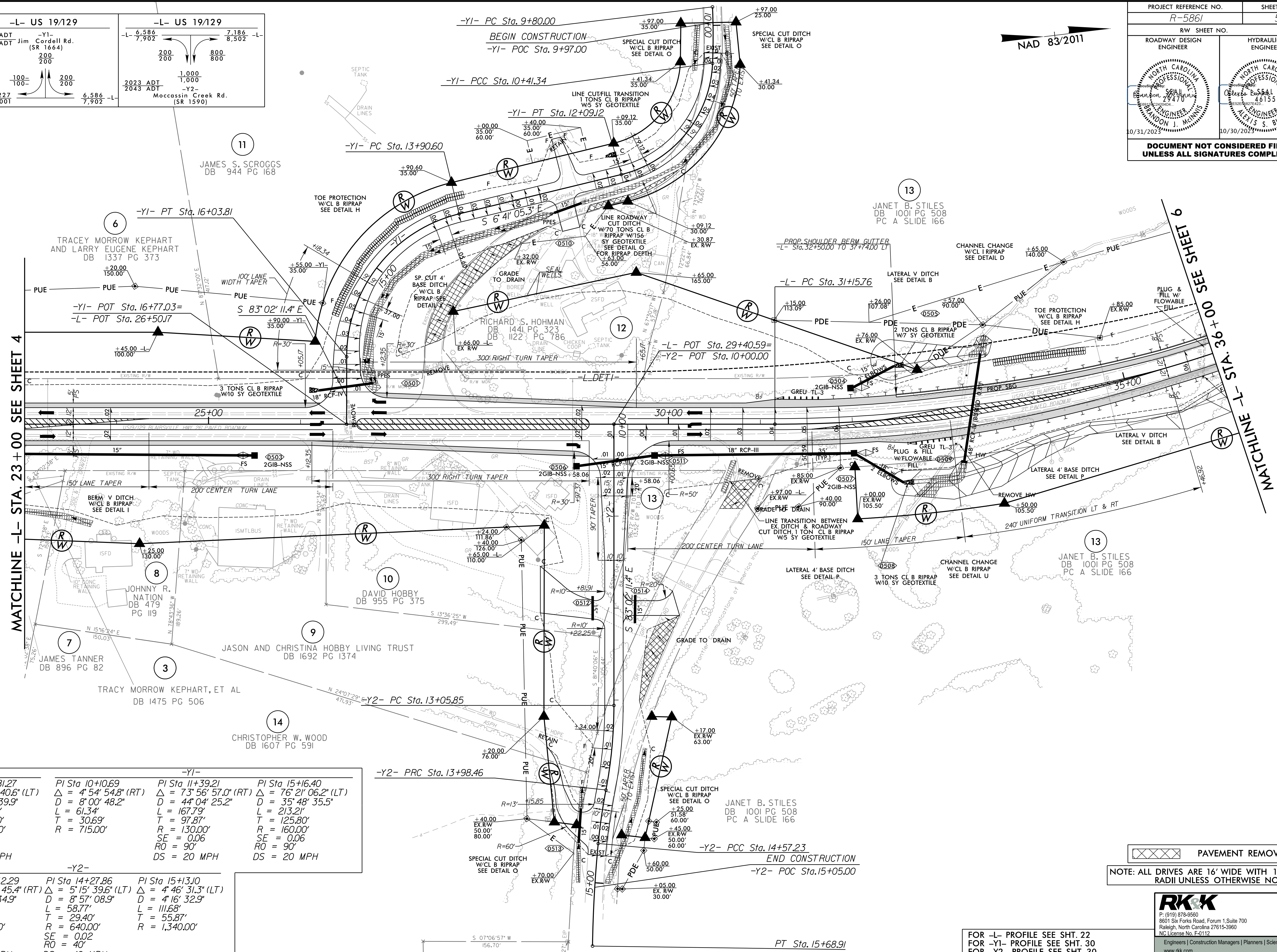
NOTES: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2  
 FOR -L- PROFILE SEE SHT. 22  
 FOR -DRI- PROFILE SEE SHT. 34  
 FOR -DR2- PROFILE SEE SHT. 34

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-L-		-Y1-	
PI Sta 33+81.27	PI Sta 10+10.69	PI Sta 11+39.21	PI Sta 15+16.40
$\Delta = 19^\circ 56' 40.6''$ (LT)	$\Delta = 4^\circ 54' 54.8''$ (RT)	$\Delta = 73^\circ 56' 57.0''$ (RT)	$\Delta = 76^\circ 21' 06.2''$ (LT)
D = 3' 47' 39.9"	D = 8' 00' 48.2"	D = 44' 04' 25.2"	D = 35' 48' 35.5"
L = 525.63'	L = 61.34'	L = 167.79'	L = 213.21'
T = 265.50'	T = 30.69'	T = 97.87'	T = 125.80'
R = 1,510.00'	R = 715.00'	R = 130.00'	R = 160.00'
SE = 0.06	SE = 0.06	SE = 0.06	SE = 0.06
RO = 210'	RO = 90'	RO = 90'	RO = 90'
DS = 60 MPH	DS = 20 MPH	DS = 20 MPH	DS = 20 MPH

-Y2-		
PI Sta 13+52.29	PI Sta 14+27.86	PI Sta 15+13.10
$\Delta = 10^\circ 49' 45.4''$ (RT)	$\Delta = 5^\circ 15' 39.6''$ (LT)	$\Delta = 4^\circ 46' 31.3''$ (LT)
D = 11' 41' 34.9"	D = 8' 57' 08.9"	D = 4' 16' 32.9"
L = 92.61'	L = 58.77'	L = 111.68'
T = 46.44'	T = 29.40'	T = 55.87'
R = 490.00'	R = 640.00'	R = 1,340.00'
SE = NC	SE = 0.02	SE = 0.02
RO = 40'	RO = 40'	RO = 40'
DS = 40 MPH	DS = 40 MPH	DS = 40 MPH

PAVEMENT REMOVAL  
NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

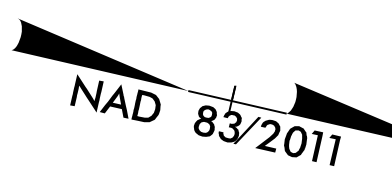
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FOR -L- PROFILE SEE SHT. 22  
FOR -Y1- PROFILE SEE SHT. 30  
FOR -Y2- PROFILE SEE SHT. 30  
FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2

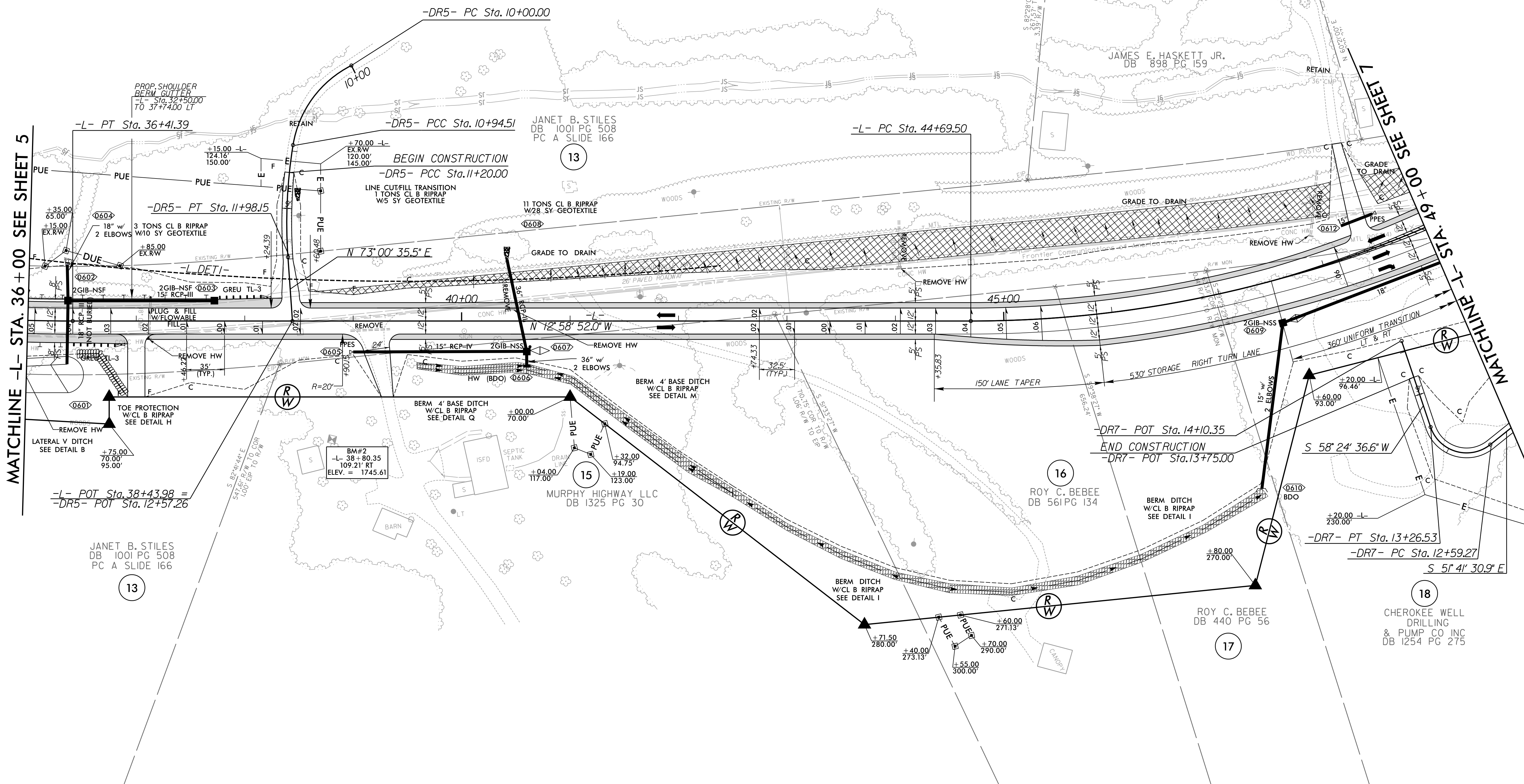
8/17/99

10/6/2023  
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-L-		-DR5-		-DR7-	
PI Sta 33+81.27	PI Sta 50+66.87	PI Sta 10+51.11	PI Sta 11+46.56	PI Sta 13+09.35	
$\Delta = 19^{\circ} 56' 40.6" (LT)$	$\Delta = 58^{\circ} 48' 26.3" (LT)$	$\Delta = 54^{\circ} 08' 51.5" (LT)$	$\Delta = 13^{\circ} 11' 45.2" (LT)$	$\Delta = 110^{\circ} 06' 07.5" (RT)$	
$D = 3^{\circ} 47' 39.9"$	$D = 5^{\circ} 24' 18.9"$	$D = 57^{\circ} 17' 44.8"$	$D = 12^{\circ} 43' 56.6"$	$D = 163^{\circ} 42' 08.0"$	
$L = 525.63'$	$L = 1,087.96'$	$L = 94.51'$	$L = 103.64'$	$L = 67.26'$	
$T = 265.50'$	$T = 597.37'$	$T = 51.11'$	$T = 52.05'$	$T = 50.08'$	
$R = 1,510.00'$	$R = 1,060.00'$	$R = 100.00'$	$R = 450.00'$	$R = 35.00'$	
$SE = 0.06$	$SE = 0.06$	$SE = 0.02$	$SE = 0.02$	$SE = 0.02$	
$RO = 210'$	$RO = 195'$	$DS = 15 MPH$	$DS = 15 MPH$	$DS = 15 MPH$	



PROJECT REFERENCE NO. R-5861		SHEET NO. 6	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
10/31/2023		10/30/2023	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR -L- PROFILE SEE SHT. 23  
FOR -DR5- PROFILE SEE SHT. 34  
FOR -DR7- PROFILE SEE SHT. 35  
FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2

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10/6/2023  
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-L-		-Y3A-		-DR6-		-DR7-	
PI Sta 50+66.87	PI Sta 59+39.36	PI Sta 11+44.57	PI Sta 11+21.86	PI Sta 11+78.95	PI Sta 12+00.39		
$\Delta = 58^\circ 48' 26.3" (LT)$	$\Delta = 1^\circ 27' 31.2" (RT)$	$\Delta = 63^\circ 45' 38.0" (RT)$	$\Delta = 53^\circ 56' 30.0" (LT)$	$\Delta = 52^\circ 28' 26.9" (LT)$	$\Delta = 19^\circ 53' 23.0" (LT)$		
D = 5' 24' 18.9"	D = 0' 11' 27.5"	D = 77' 2' 51.7"	D = 76' 23' 39.7"	D = 114' 35' 29.6"	D = 47' 44' 47.3"		
L = 1,087.96'	L = 763.75'	L = 82.42'	L = 70.61'	L = 45.79'	L = 41.66'		
T = 597.37'	T = 381.90'	T = 49.76'	T = 38.17'	T = 24.64'	T = 21.04'		
R = 1,060.00'	R = 30,000.00'	R = 80.00'	R = 75.00'	R = 50.00'	R = 120.00'		
SE = 0.06	SE = 0.02	SE = 0.06	SE = 0.02	SE = 0.02	SE = 0.02		
RO = 195'	RO = 70'	RO = 90'	DS = 15 MPH	DS = 15 MPH	DS = 15 MPH		
DS = 55 MPH	DS = 60 MPH	DS = 15 MPH					

PROJECT REFERENCE NO. R-5861 SHEET NO. 7

RW SHEET NO.

ROADWAY DESIGN ENGINEER

HYDRAULICS ENGINEER

**SEAL**

NORTH CAROLINA PROFESSIONAL ENGINEER

ANDREW J. MCINTOSH

10/31/2023

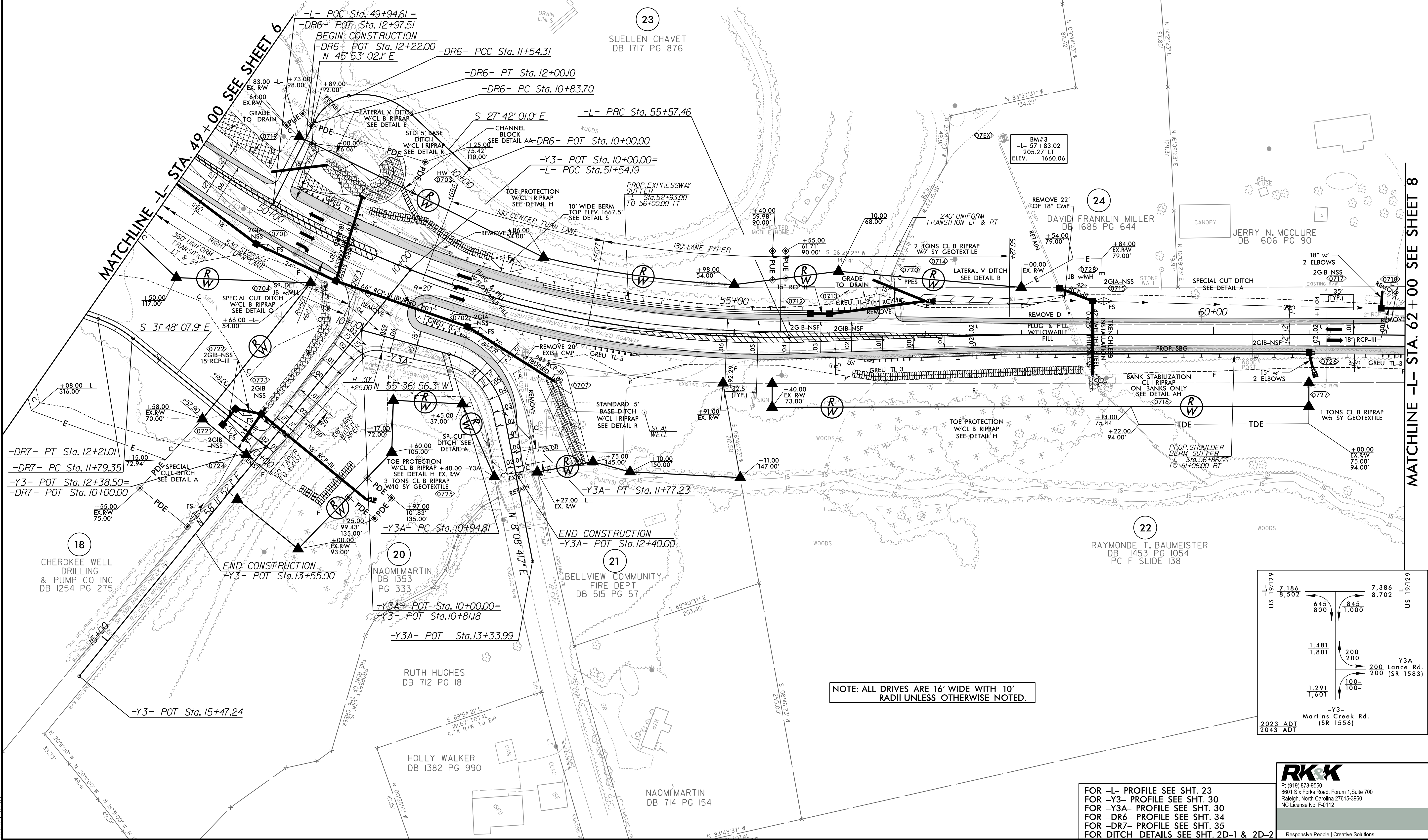
**SEAL**

NORTH CAROLINA PROFESSIONAL ENGINEER

ALEXIS S. BURKE

10/30/2023

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**



**MATCHLINE -L- STA 49+00 SEE SHEET 6**

**MATCHLINE -L- STA. 62+00 SEE SHEET 8**

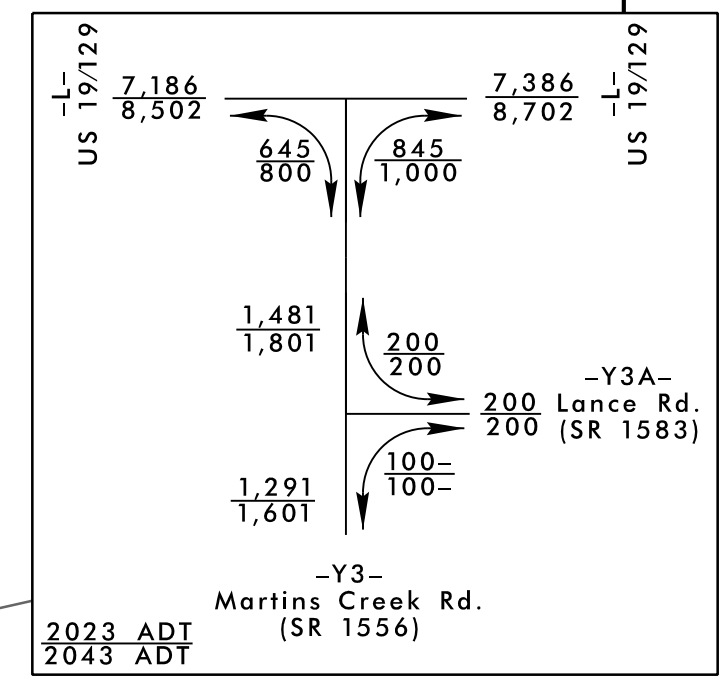
-DR7- PT Sta. 12+21.01  
-DR7- PC Sta. 11+79.35  
-Y3- POT Sta. 12+38.50  
-DR7- POT Sta. 10+00.00

END CONSTRUCTION  
-Y3- POT Sta. 13+55.00

-Y3A- PC Sta. 10+94.81  
-Y3A- POT Sta. 10+00.00=  
-Y3- POT Sta. 10+81.18  
-Y3A- POT Sta. 13+33.99

END CONSTRUCTION  
-Y3A- POT Sta. 12+40.00

NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.



FOR -L- PROFILE SEE SHT. 23  
FOR -Y3- PROFILE SEE SHT. 30  
FOR -Y3A- PROFILE SEE SHT. 30  
FOR -DR6- PROFILE SEE SHT. 34  
FOR -DR7- PROFILE SEE SHT. 35  
FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2

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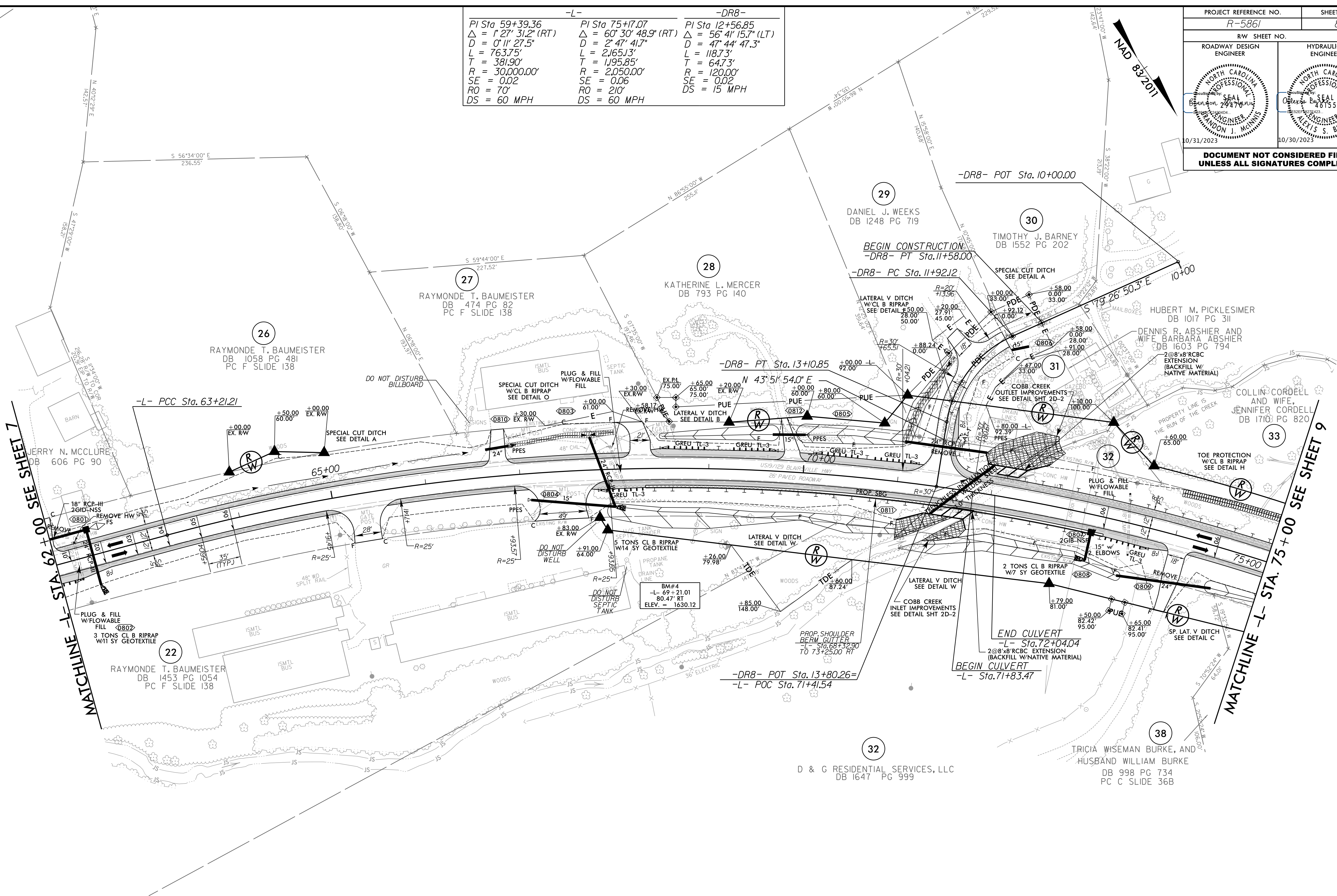
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-L-		-DR8-	
PI Sta 59+39.36	PI Sta 75+17.07	PI Sta 12+56.85	
$\Delta = 1' 27' 31.2''$ (RT)	$\Delta = 60' 30' 48.9''$ (RT)	$\Delta = 56' 41' 15.7''$ (LT)	
D = 0' 11' 27.5"	D = 2' 47' 41.7"	D = 47' 44' 47.3"	
L = 763.75'	L = 2,165.13'	L = 118.73'	
T = 381.90'	T = 1,955.85'	T = 64.73'	
R = 30,000.00'	R = 2,050.00'	R = 120.00'	
SE = 0.02	SE = 0.06	SE = 0.02	
RO = 70'	RO = 210'	DS = 15 MPH	
DS = 60 MPH	DS = 60 MPH		

PROJECT REFERENCE NO. R-5861	SHEET NO. 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>	



NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

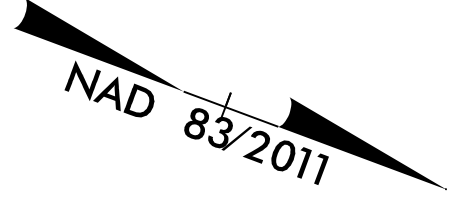
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FOR -L- PROFILE SEE SHTS. 23-24  
FOR -DR8- PROFILE SEE SHT. 35  
FOR DITCH DETAILS SEE SHTS. 2D-1 & 2D-2

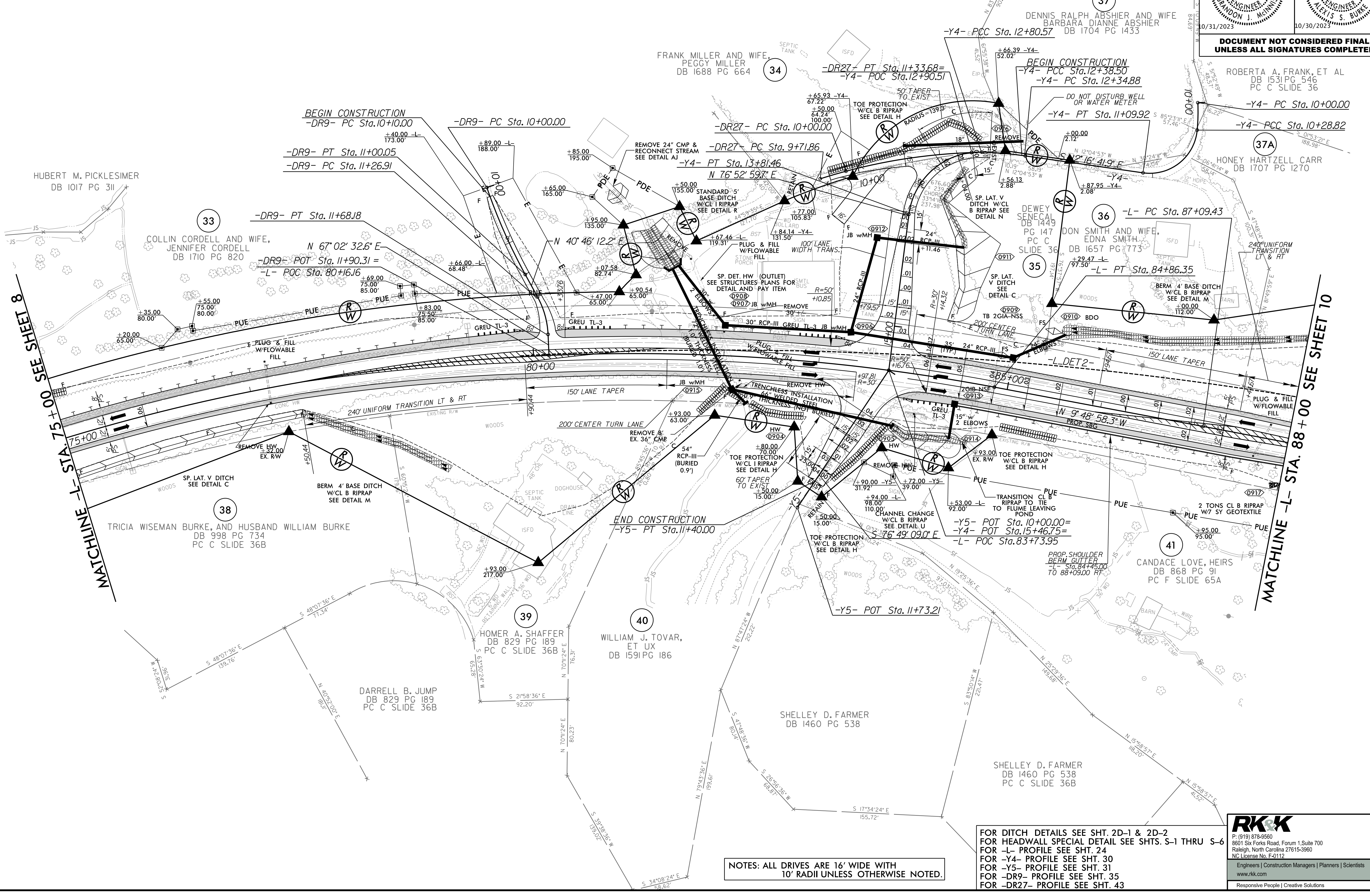
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8/17/199

-DR9-		-L-		-Y4-		-DR27-	
PI Sta 10+50.15	PI Sta 11+47.91	PI Sta 75+17.07	PI Sta 90+43.98	PI Sta 10+14.44	PI Sta 10+81.45	PI Sta 12+57.81	PI Sta 13+48.53
$\Delta = 9^{\circ}53'01.0"$ (LT)	$\Delta = 26^{\circ}16'20.4"$ (RT)	$\Delta = 60^{\circ}30'48.9"$ (RT)	$\Delta = 2^{\circ}47'40.83"$ (RT)	$\Delta = 7^{\circ}51'52.2"$ (RT)	$\Delta = 92^{\circ}55'38.0"$ (RT)	$\Delta = 6^{\circ}28'50.8"$ (LT)	$\Delta = 84^{\circ}21'27.6"$ (LT)
D = 9'52'42.9"	D = 63'39'43.1"	D = 2'47'41.7"	D = 0'25'03.9"	D = 27'17'01.3"	D = 114'35'29.6"	D = 14'10'59.9"	D = 83'37'14.3"
L = 100.05'	L = 41.27'	L = 2,165.13'	L = 668.97'	L = 28.82'	L = 81.09'	L = 45.69'	L = 100.88'
T = 50.15'	T = 21.00'	T = 1,195.85'	T = 334.55'	T = 14.44'	T = 52.62'	T = 22.93'	T = 67.96'
R = 580.00'	R = 90.00'	R = 2,050.00'	R = 13,715.00'	R = 210.00'	R = 50.00'	R = 405.00'	R = 75.00'
SE = 0.02	SE = 0.02	SE = 0.06	SE = 0.02	SE = 0.02	SE = 0.02	SE = 0.02	SE = 0.02
DS = 15 MPH	DS = 15 MPH	RO = 210'	RO = 70'	DS = 15 MPH	DS = 15 MPH	RO = 30'	RO = 30'
		DS = 60 MPH	DS = 60 MPH			DS = 15 MPH	DS = 15 MPH



PROJECT REFERENCE NO. R-5861	SHEET NO. 9
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>DOCUMENT NOT CONSIDERED FINAL</b> <b>UNLESS ALL SIGNATURES COMPLETED</b>	
DENNIS RALPH ABSHIER AND WIFE BARBARA DIANNE ABSHIER DB 1704 PG 1433	
ROBERTA A. FRANK, ET AL DB 1531 PG 546 PC C SLIDE 36	
HONEY HARTZELL CARR DB 1707 PG 1270	



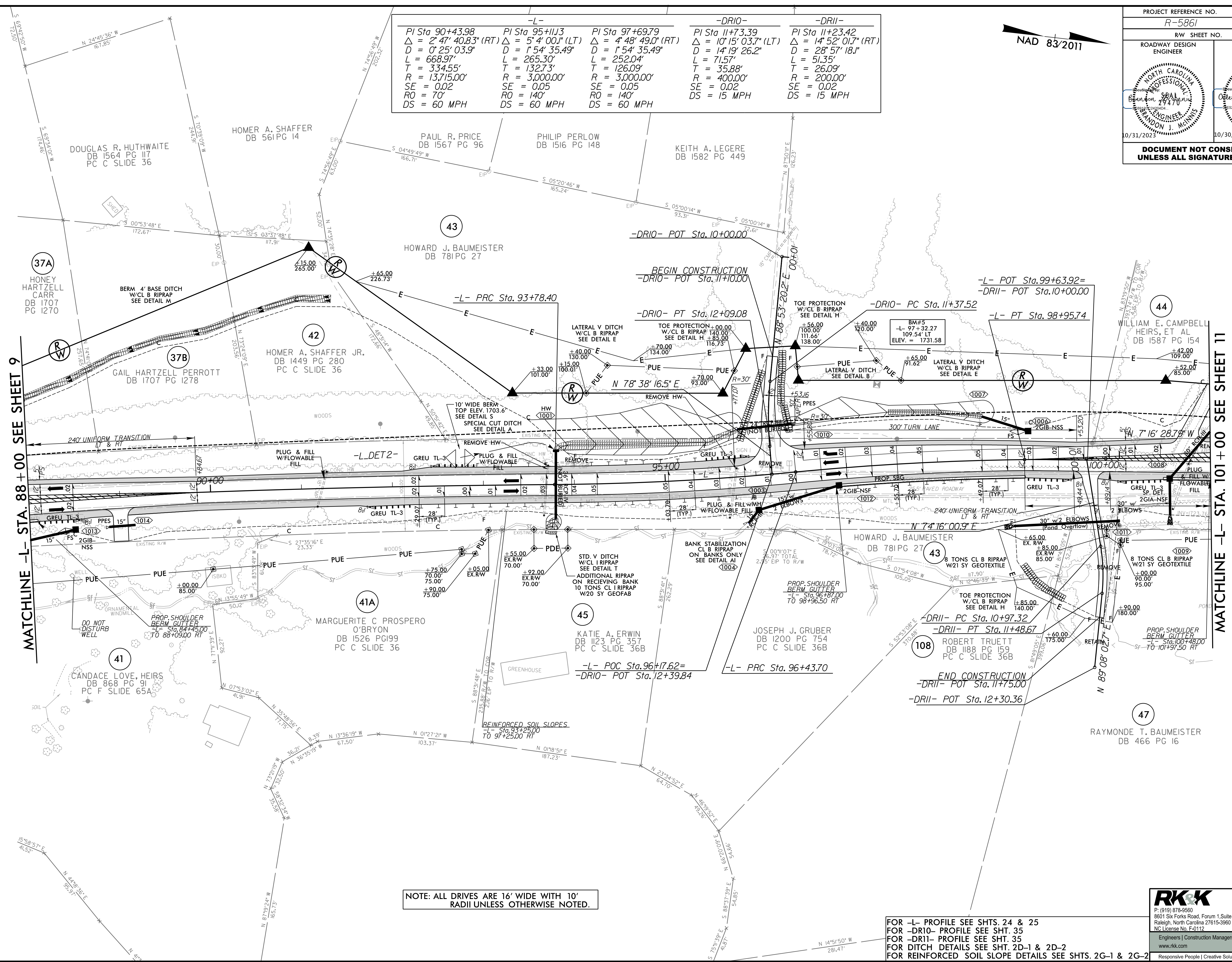
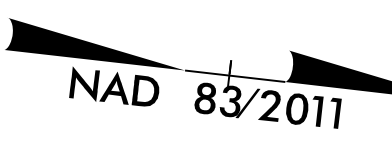
NOTES: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2  
 FOR HEADWALL SPECIAL DETAIL SEE SHTS. S-1 THRU S-6  
 FOR -L- PROFILE SEE SHT. 24  
 FOR -Y4- PROFILE SEE SHT. 30  
 FOR -Y5- PROFILE SEE SHT. 31  
 FOR -DR9- PROFILE SEE SHT. 35  
 FOR -DR27- PROFILE SEE SHT. 43

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 psh09

-L-			-DRIO-	-DRII-
PI Sta 90+43.98	PI Sta 95+111.3	PI Sta 97+69.79	PI Sta 11+73.39	PI Sta 11+23.42
$\Delta = 2' 47" 40.83" (RT)$	$\Delta = 5' 4" 00.1" (LT)$	$\Delta = 4' 48" 49.0" (RT)$	$\Delta = 10' 15" 03.7" (LT)$	$\Delta = 14' 52" 01.7" (RT)$
D = 0' 25' 03.9"	D = 1' 54' 35.49"	D = 1' 54' 35.49"	D = 14' 19' 26.2"	D = 28' 57' 18.1"
L = 668.97'	L = 265.30'	L = 252.04'	L = 71.57'	L = 51.35'
T = 334.55'	T = 132.73'	T = 126.09'	T = 35.88'	T = 26.09'
R = 13,715.00'	R = 3,000.00'	R = 3,000.00'	R = 400.00'	R = 200.00'
SE = 0.02	SE = 0.05	SE = 0.05	SE = 0.02	SE = 0.02
RO = 70'	RO = 140'	RO = 140'	DS = 15 MPH	DS = 15 MPH
DS = 60 MPH	DS = 60 MPH	DS = 60 MPH		



MATCHLINE -L- STA. 101+00 SEE SHEET 11

NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR -L- PROFILE SEE SHTS. 24 & 25  
 FOR -DRIO- PROFILE SEE SHT. 35  
 FOR -DRII- PROFILE SEE SHT. 35  
 FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2  
 FOR REINFORCED SOIL SLOPE DETAILS SEE SHTS. 2G-1 & 2G-2

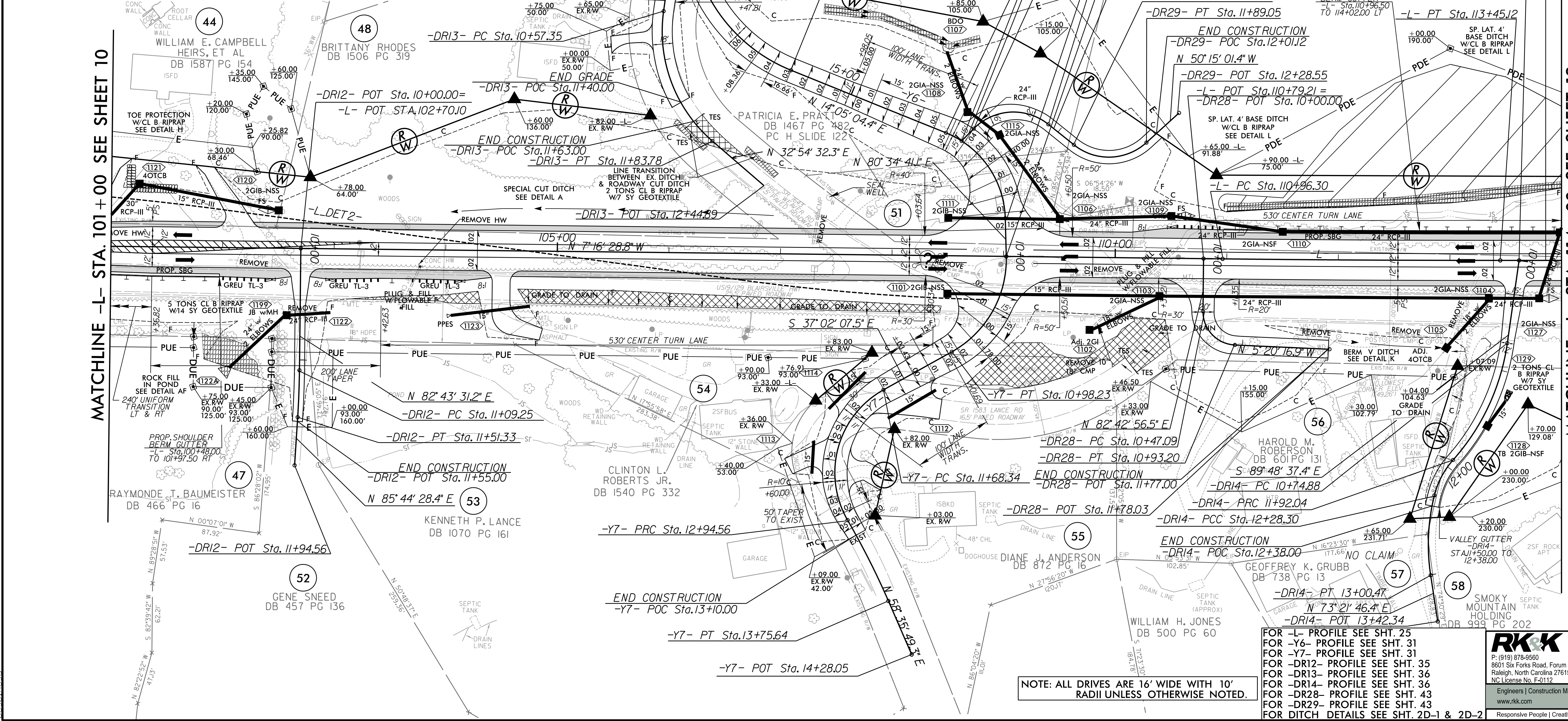
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8/17/19

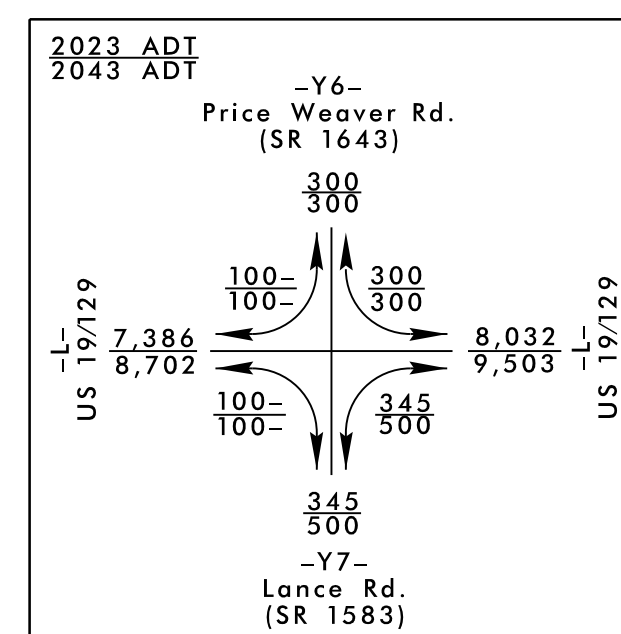
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-L-		-Y6-		-DR12-	
PI Sta 112+20.71 Δ = 0° 38' 52.86" (LT) D = 0' 15' 37.57" L = 248.82' T = 124.41' R = 22,000.00' SE = NC RO = 70' DS = 60 MPH	PI Sta 13+60.37 Δ = 39° 30' 51.6" D = 204.99' T = 123.85' R = 145.00' SE = 0.06 RO = 100' DS = 25 MPH	PI Sta 16+47.26 Δ = 66° 29' 36.7" (RT) D = 67' 24' 24.5" L = 98.65' T = 55.72' R = 85.00' SE = 0.05 RO = 75' DS = 20 MPH	PI Sta 11+30.31 Δ = 3° 00' 57.2" (RT) D = 7' 09' 59.9" L = 42.08' T = 21.06' R = 800.00' SE = 0.02 DS = 15 MPH		
-DR28-		-DR29-			
PI Sta 10+57.52 Δ = 62° 38' 11.4" (RT) D = 63' 30' 49.4" L = 98.23' T = 57.52' R = 95.00' SE = 0.02 RO = 30' DS = 20 MPH	PI Sta 12+54.78 Δ = 90° 57' 44.5" (LT) D = 72' 03' 49.5" L = 126.22' T = 86.44' R = 85.00' SE = 0.02 RO = 30' DS = 20 MPH	PI Sta 13+35.18 Δ = 6° 35' 41.2" (RT) D = 8' 08' 02.0" L = 81.08' T = 40.62' R = 705.00' SE = 0.02 RO = 15 MPH	PI Sta 10+76.09 Δ = 88° 03' 13.4" (LT) D = 190' 59' 09.4" L = 46.10' T = 29.00' R = 30.00' SE = 0.02 DS = 15 MPH		
-DR13-		-DR29-			
PI Sta 10+28.00 Δ = 49° 09' 14.4" (LT) D = 229' 10' 59.2" L = 21.45' T = 11.43' R = 25.00' SE = 0.02 DS = 15 MPH	PI Sta 11+28.22 Δ = 57° 11' 36.3" (LT) D = 45' 14' 23.0" L = 126.42' T = 70.87' R = 130.00' SE = 0.02 DS = 15 MPH	PI Sta 10+60.63 Δ = 84° 11' 20.2" (RT) D = 114' 35' 29.6" L = 73.47' T = 42.79' R = 50.00' SE = 0.02 DS = 15 MPH	PI Sta 11+61.05 Δ = 81° 06' 48.1" (LT) D = 114' 35' 29.6" L = 70.78' T = 42.79' R = 50.00' SE = 0.02 DS = 15 MPH		
-DR14-		-DR29-			
PI Sta 11+35.19 Δ = 33° 33' 54.5" (RT) D = 28' 38' 52.4" L = 117.16' T = 60.32' R = 200.00' SE = 0.02 DS = 15 MPH	PI Sta 12+10.53 Δ = 27° 41' 53.8" (LT) D = 76' 23' 39.7" L = 36.26' T = 18.49' R = 75.00' SE = 0.02 DS = 15 MPH	PI Sta 12+64.86 Δ = 22° 41' 36.9" (LT) D = 31' 26' 41.0" L = 72.17' T = 36.56' R = 182.21' SE = 0.02 DS = 15 MPH			



MATCHLINE -L- STA. 101 + 00 SEE SHEET 10

MATCHLINE -L- STA. 114 + 00 SEE SHEET 12



PROJECT REFERENCE NO. R-5861	SHEET NO. 11
ROADWAY DESIGN ENGINEER NORTH CAROLINA PROFESSIONAL ENGINEER SEAL Brandon J. McInnis 10/31/2023	HYDRAULICS ENGINEER NORTH CAROLINA PROFESSIONAL ENGINEER SEAL Alexis S. Burke 10/30/2023
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>	

FOR -L- PROFILE SEE SHT. 25  
 FOR -Y6- PROFILE SEE SHT. 31  
 FOR -Y7- PROFILE SEE SHT. 31  
 FOR -DR12- PROFILE SEE SHT. 35  
 FOR -DR13- PROFILE SEE SHT. 36  
 FOR -DR14- PROFILE SEE SHT. 36  
 FOR -DR28- PROFILE SEE SHT. 43  
 FOR -DR29- PROFILE SEE SHT. 43  
 FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2

NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

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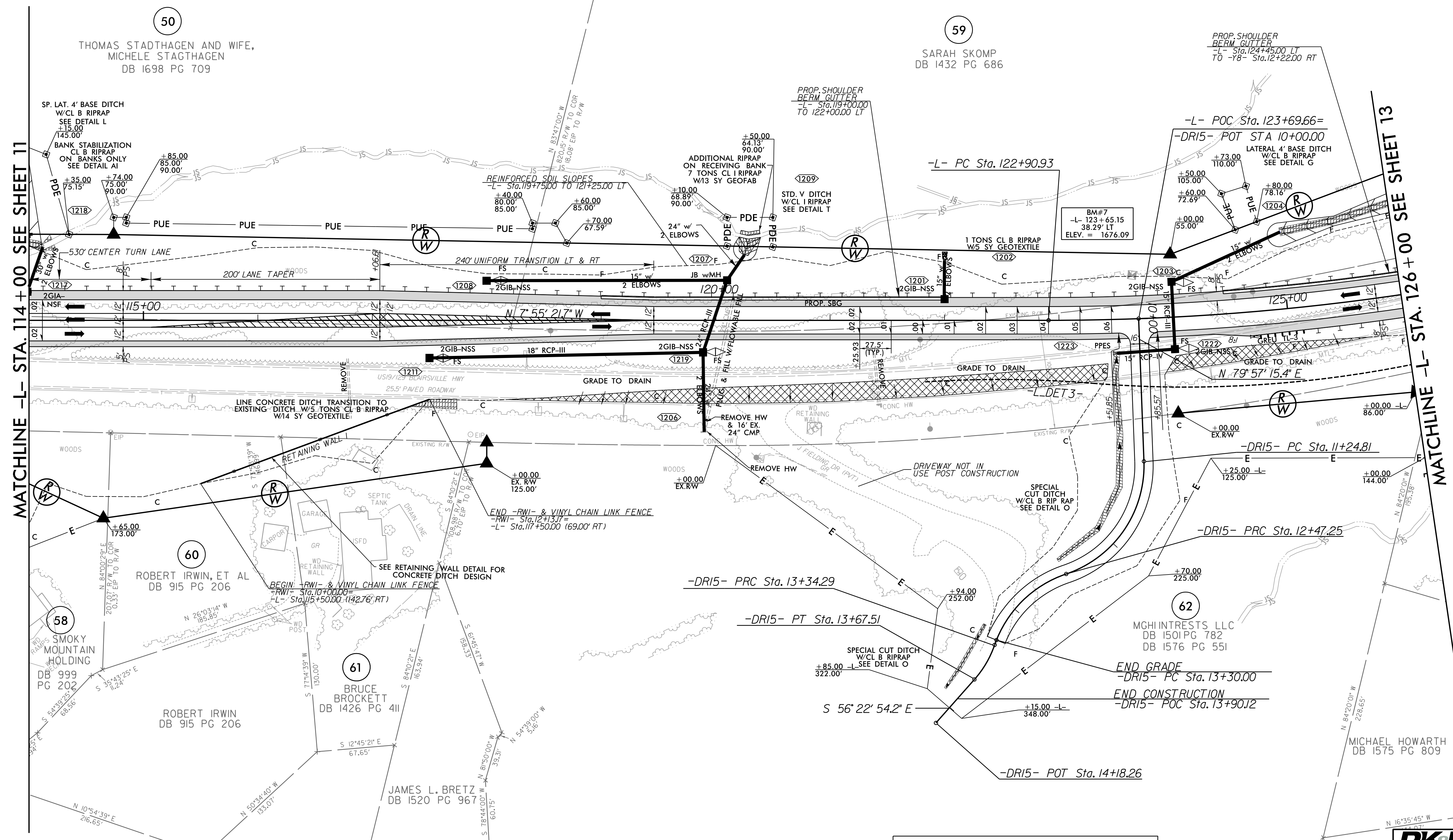


8/17/199

-L-	-DR15-	-DR15-	-DR15-
PI Sta 127+30.02	PI Sta 11+99.43	PI Sta 12+96.26	PI Sta 13+52.13
$\Delta = 23^{\circ} 20' 58.3" (LT)$	$\Delta = 73^{\circ} 27' 47.3" (RT)$	$\Delta = 52^{\circ} 13' 28.3" (LT)$	$\Delta = 22^{\circ} 25' 31.3" (RT)$
$D = 2^{\circ} 41' 46.6"$	$D = 60^{\circ} 00' 00.0"$	$D = 60^{\circ} 00' 00.0"$	$D = 67^{\circ} 29' 52.7"$
$L = 865.99'$	$L = 122.44'$	$L = 87.04'$	$L = 33.22'$
$T = 439.09'$	$T = 74.62'$	$T = 49.02'$	$T = 17.84'$
$R = 2,125.00'$	$R = 100.00'$	$R = 100.00'$	$R = 90.00'$
$SE = 0.06$	$SE = 0.02$	$SE = 0.02$	$SE = 0.02$
$RO = 165'$	$DS = 15 MPH$	$DS = 15 MPH$	$DS = 15 MPH$

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PROJECT REFERENCE NO. R-5861		SHEET NO. 12
RW SHEET NO.		HYDRAULICS
ROADWAY DESIGN ENGINEER	ENGINEER	
10/31/2023	10/30/2023	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		



NOTES: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR -L- PROFILE SEE SHTS. 25 & 26  
 FOR -DR15- PROFILE SEE SHT. 36  
 FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2  
 FOR RETAINING WALL ENVELOPE SEE SHT. W-1 THRU W-3  
 FOR REINFORCED SOIL SLOPE DETAILS SEE SHTS. 2G-1 & 2G-2

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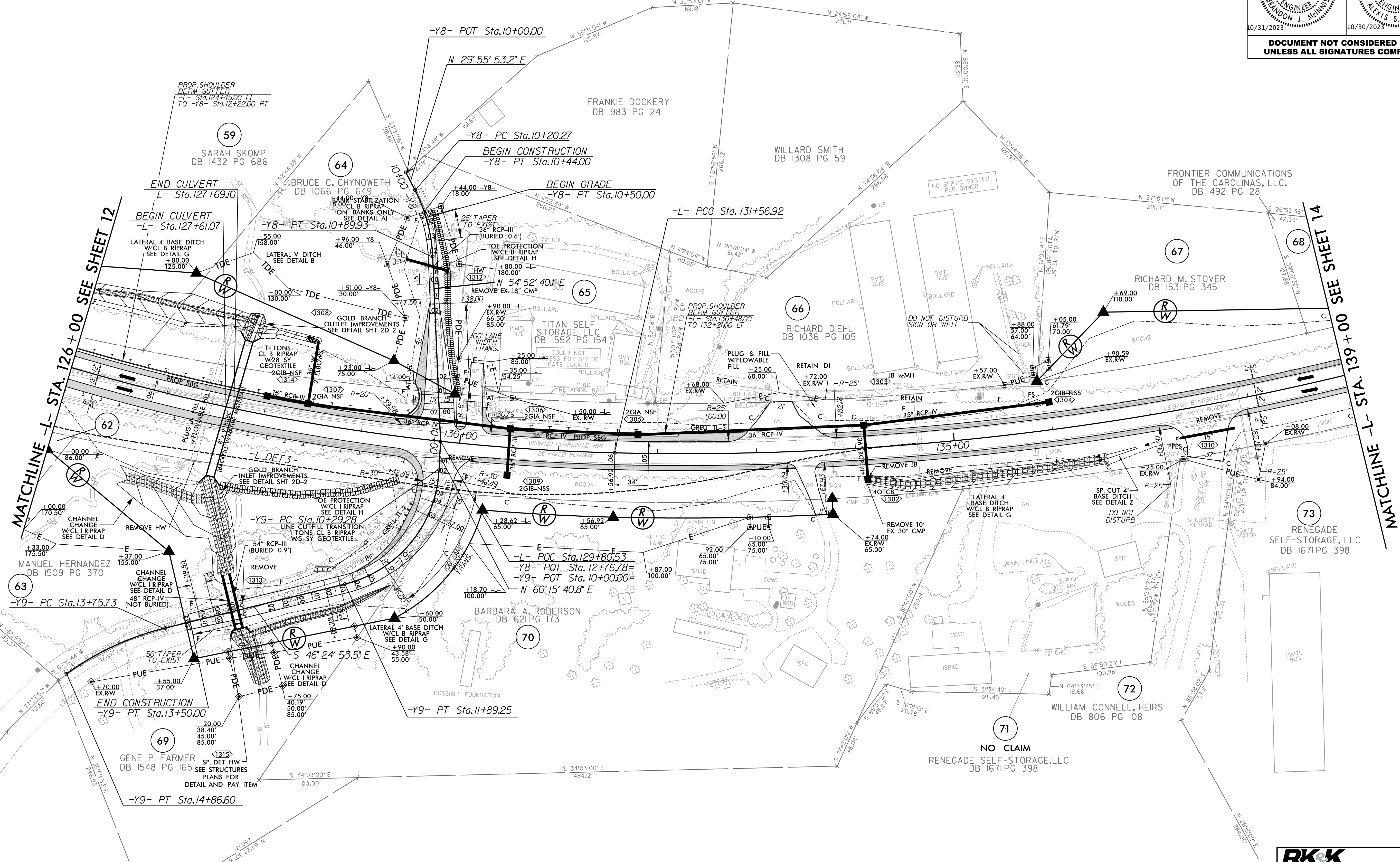
8/17/99

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-L-	-Y8-	-Y9-
PI Sta 127+30.02	PI Sta 135+82.23	PI Sta 10+55.66
$\Delta = 23^\circ 20' 58.3" (LT)$	$\Delta = 16^\circ 58' 31.5" (LT)$	$\Delta = 24^\circ 56' 46.9" (RT)$
$D = 2' 4" 46.6"$	$D = 2' 00' 37.4"$	$D = 35' 48' 35.5"$
$L = 865.99'$	$L = 844.39'$	$L = 696.6'$
$T = 439.09'$	$T = 425.31'$	$T = 35.39'$
$R = 2,125.00'$	$R = 2,850.00'$	$R = 160.00'$
$SE = 0.06$	$SE = 0.05$	$SE = 0.03$
$DS = 60 \text{ MPH}$	$DS = 60 \text{ MPH}$	$DS = 15 \text{ MPH}$
		$RO = 45'$
		$DS = 15 \text{ MPH}$
		$DS = 15 \text{ MPH}$



PROJECT REFERENCE NO. R-5861	SHEET NO. 13
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>	

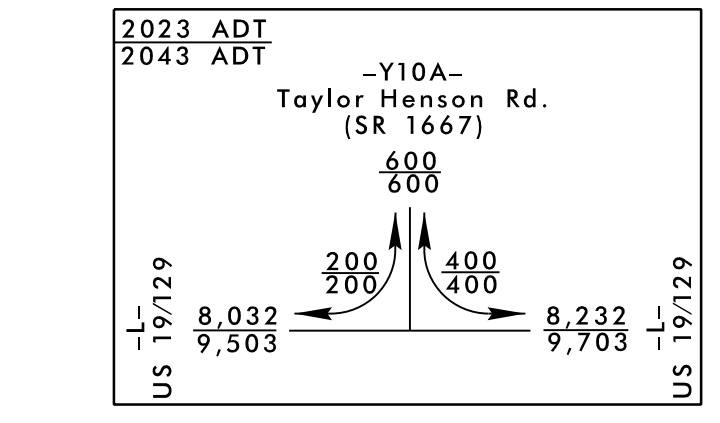


NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

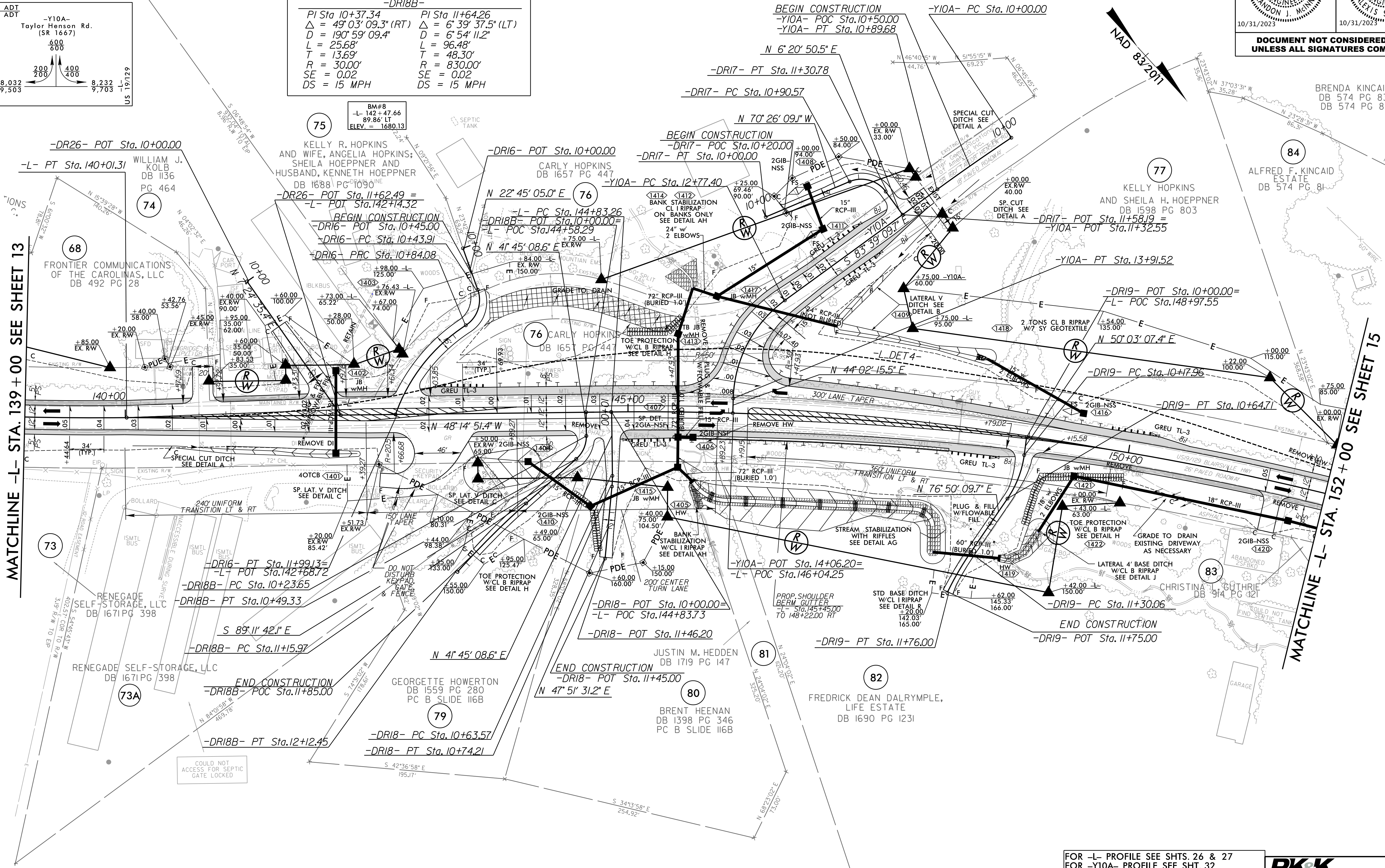
FOR -L- PROFILE SEE SHT. 26  
 FOR -Y8- PROFILE SEE SHT. 31  
 FOR -Y9- PROFILE SEE SHT. 32  
 FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2  
 FOR HEADWALL SPECIAL DETAIL SEE SHTS. S-1 THRU S-6

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<b>-L-</b> PI Sta 135+82.23 $\Delta = 16' 58" 31.5" (LT)$ $D = 2' 00" 37.4"$ $L = 844.39'$ $T = 425.31'$ $R = 2,850.00'$ $SE = 0.05$ $DS = 60 MPH$	<b>-L-</b> PI Sta 151+02.43 $\Delta = 24' 25" 52.3" (RT)$ $D = 2' 00" 12.1"$ $L = 1,219.52'$ $T = 619.17'$ $R = 2,860.00'$ $SE = 0.05$ $RO = 170'$ $DS = 60 MPH$	<b>-Y10A-</b> PI Sta 10+44.85 $\Delta = 2' 20" 08.2" (LT)$ $D = 2' 36" 15.7"$ $L = 89.68'$ $T = 44.85'$ $R = 2,200.00'$	<b>-Y10A-</b> PI Sta 13+38.78 $\Delta = 52' 18" 34.8" (LT)$ $D = 45' 50" 11.8"$ $L = 114.12'$ $T = 61.39'$ $R = 125.00'$ $SE = 0.03$ $RO = 45'$ $DS = 20 MPH$	<b>-DR16-</b> PI Sta 10+67.65 $\Delta = 76' 43" 07.2" (RT)$ $D = 190' 59" 09.4"$ $L = 40.17'$ $T = 23.74'$ $R = 30.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR17-</b> PI Sta 11+43.77 $\Delta = 37' 40" 05.5" (LT)$ $D = 32' 44" 25.6"$ $L = 115.05'$ $T = 59.69'$ $R = 175.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR17-</b> PI Sta 11+14.34 $\Delta = 76' 46" 59.6" (RT)$ $D = 190' 59" 09.4"$ $L = 40.20'$ $T = 23.77'$ $R = 30.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR18-</b> PI Sta 10+68.89 $\Delta = 6' 05" 49.0" (RT)$ $D = 57' 17" 44.8"$ $L = 10.64'$ $T = 5.33'$ $R = 100.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR19-</b> PI Sta 10+41.77 $\Delta = 26' 47" 02.3" (RT)$ $D = 57' 17" 44.8"$ $L = 46.75'$ $T = 23.81'$ $R = 100.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR19-</b> PI Sta 11+53.07 $\Delta = 8' 46" 24.5" (LT)$ $D = 19' 05" 54.9"$ $L = 45.94'$ $T = 23.01'$ $R = 300.00'$ $SE = 0.02$ $DS = 15 MPH$
--	---	---	--	---	--	---	---	---	--



<b>-DR18B-</b> PI Sta 10+37.34 $\Delta = 49' 03" 09.3" (RT)$ $D = 190' 59" 09.4"$ $L = 25.68'$ $T = 13.69'$ $R = 30.00'$ $SE = 0.02$ $DS = 15 MPH$	<b>-DR18B-</b> PI Sta 11+64.26 $\Delta = 6' 39" 37.5" (LT)$ $D = 6' 54" 11.2"$ $L = 96.48'$ $T = 48.30'$ $R = 830.00'$ $SE = 0.02$ $DS = 15 MPH$
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MATCHLINE -L- STA. 139+00 SEE SHEET 13

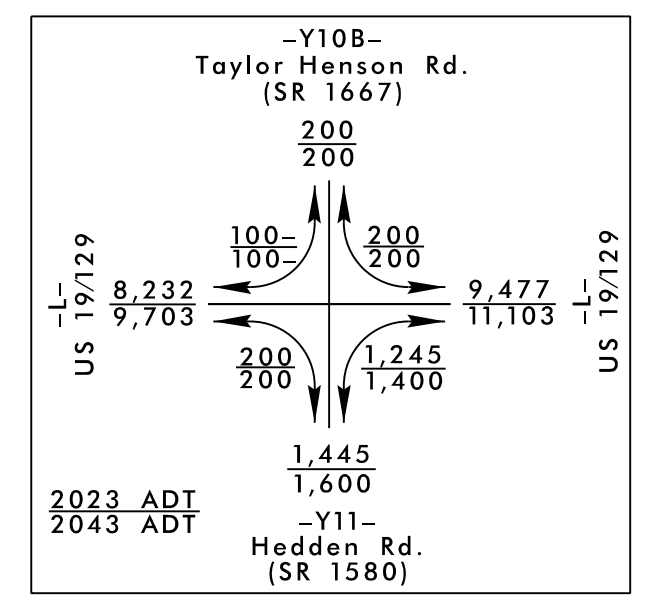
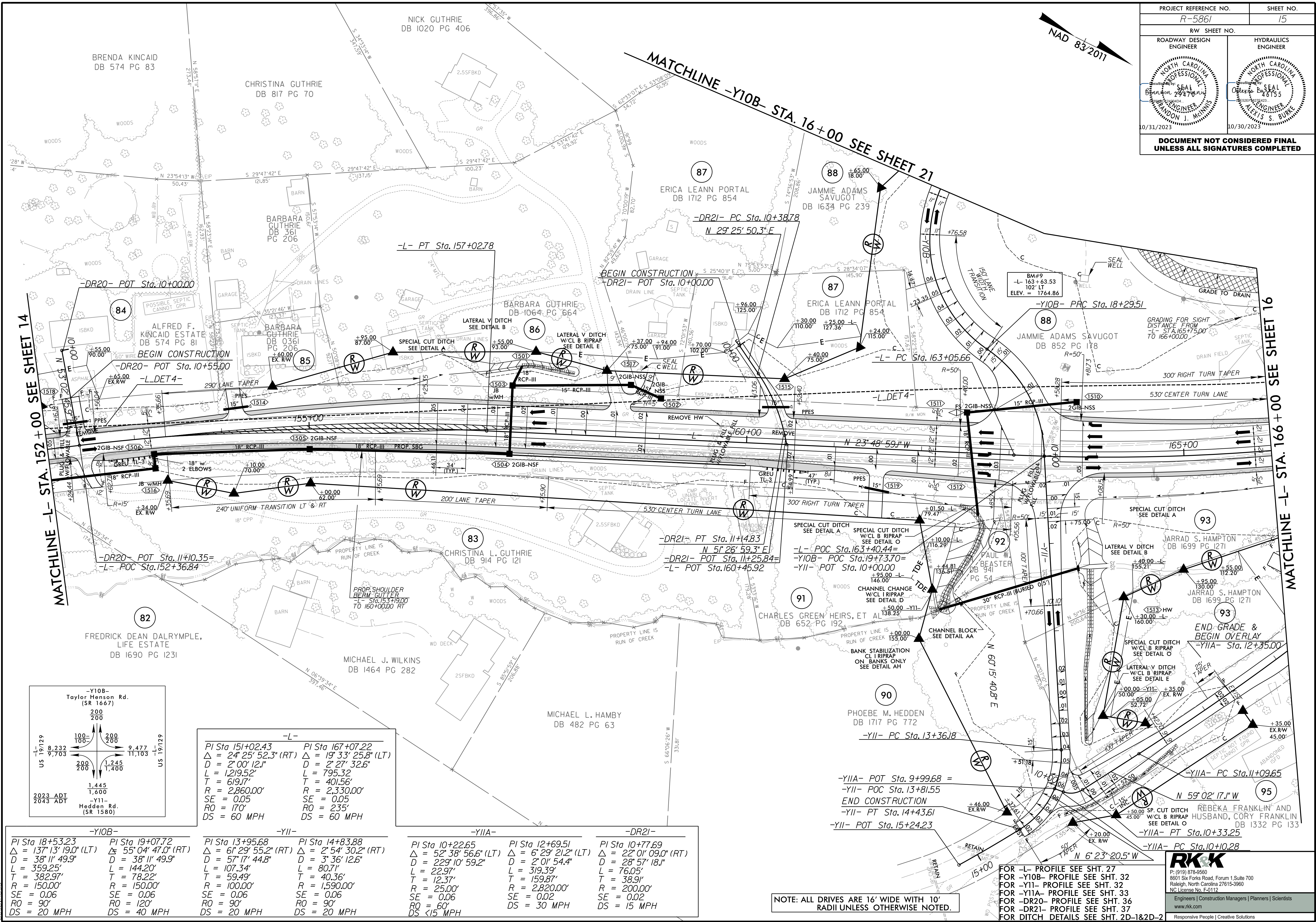
MATCHLINE -L- STA. 152+00 SEE SHEET 15

NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR -L- PROFILE SEE SHTS. 26 & 27  
 FOR -Y10A- PROFILE SEE SHT. 32  
 FOR -DR16- PROFILE SEE SHT. 36  
 FOR -DR17- PROFILE SEE SHT. 36  
 FOR -DR18- PROFILE SEE SHT. 36  
 FOR -DR18B- PROFILE SEE SHT. 37  
 FOR -DR19- PROFILE SEE SHT. 36  
 FOR -DR26- PROFILE SEE SHT. 43  
 FOR DITCH DETAILS SEE SHT. 2D-1 & 2D-2

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-L-	
PI Sta 151+02.43	PI Sta 167+07.22
$\Delta = 24^\circ 25' 52.3''$ (RT)	$\Delta = 19^\circ 33' 25.8''$ (LT)
D = 2' 00" 12.1"	D = 2' 27" 32.6"
L = 1,219.52'	L = 795.32'
T = 619.17'	T = 401.56'
R = 2,860.00'	R = 2,330.00'
SE = 0.05	SE = 0.05
RO = 170'	RO = 235'
DS = 60 MPH	DS = 60 MPH

-Y10B-	
PI Sta 18+53.23	PI Sta 19+07.72
$\Delta = 137^\circ 13' 19.0''$ (LT)	$\Delta = 55^\circ 04' 47.0''$ (RT)
D = 38' 11" 49.9"	D = 38' 11" 49.9"
L = 359.25'	L = 144.20'
T = 382.29'	T = 78.22'
R = 150.00'	R = 150.00'
SE = 0.06	SE = 0.06
RO = 90'	RO = 120'
DS = 20 MPH	DS = 40 MPH

-Y11-	
PI Sta 13+95.68	PI Sta 14+83.88
$\Delta = 61^\circ 29' 55.2''$ (RT)	$\Delta = 2^\circ 54' 30.2''$ (RT)
D = 57' 17" 44.8"	D = 3' 36" 12.6"
L = 107.34'	L = 80.71'
T = 59.49'	T = 40.36'
R = 100.00'	R = 1,590.00'
SE = 0.06	SE = 0.06
RO = 90'	RO = 90'
DS = 20 MPH	DS = 20 MPH

-Y11A-	
PI Sta 10+22.65	PI Sta 12+69.51
$\Delta = 52^\circ 38' 56.6''$ (LT)	$\Delta = 6^\circ 29' 21.2''$ (LT)
D = 229' 10" 59.2"	D = 2' 01" 54.4"
L = 22.97'	L = 319.39'
T = 12.37'	T = 159.87'
R = 25.00'	R = 2,820.00'
SE = 0.06	SE = 0.02
RO = 60'	DS = 30 MPH

-DR21-	
PI Sta 10+77.69	PI Sta 10+77.69
$\Delta = 22^\circ 01' 09.0''$ (RT)	$\Delta = 22^\circ 01' 09.0''$ (RT)
D = 28' 57" 18.1"	D = 28' 57" 18.1"
L = 76.05'	L = 76.05'
T = 38.91'	T = 38.91'
R = 200.00'	R = 200.00'
SE = 0.02	SE = 0.02
DS = 15 MPH	DS = 15 MPH

NOTE: ALL DRIVES ARE 16' WIDE WITH 10' RADII UNLESS OTHERWISE NOTED.

FOR -L- PROFILE SEE SHT. 27  
 FOR -Y10B- PROFILE SEE SHT. 32  
 FOR -Y11- PROFILE SEE SHT. 32  
 FOR -Y11A- PROFILE SEE SHT. 33  
 FOR -DR20- PROFILE SEE SHT. 36  
 FOR -DR21- PROFILE SEE SHT. 37  
 FOR DITCH DETAILS SEE SHT. 2D-1&2D-2

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