

09/08/19

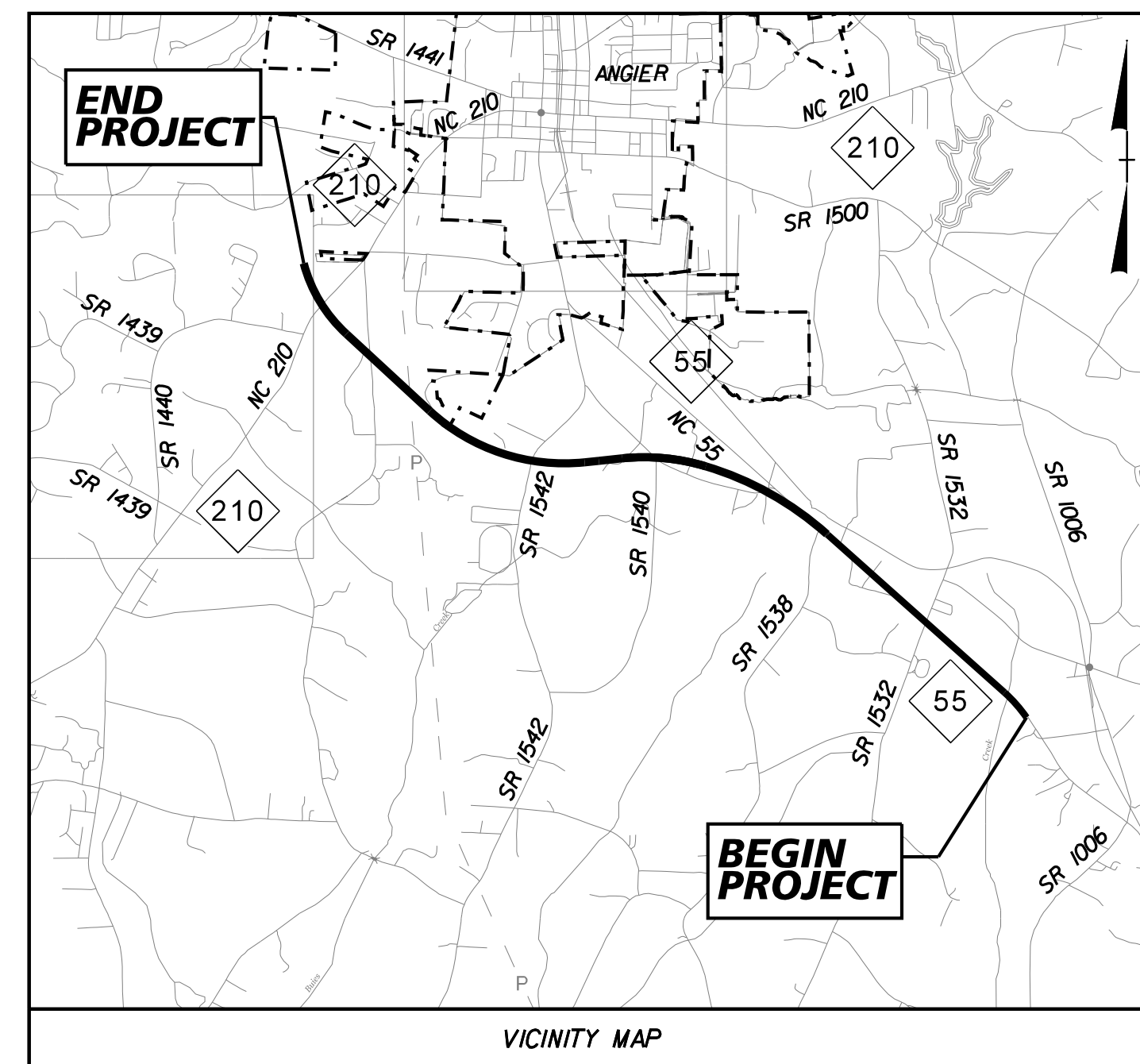
K:\RAL_Roadway\01036479 - R-5705A - NC 55_Roadway\Pro\R5705A_rdy_tshdgn

8/8/2023

TIP PROJECT: R-5705A

CONTRACT: C204785

See Sheet 1A For Index of Sheets
See Sheet 1B For Conventional Plan Sheet Symbols

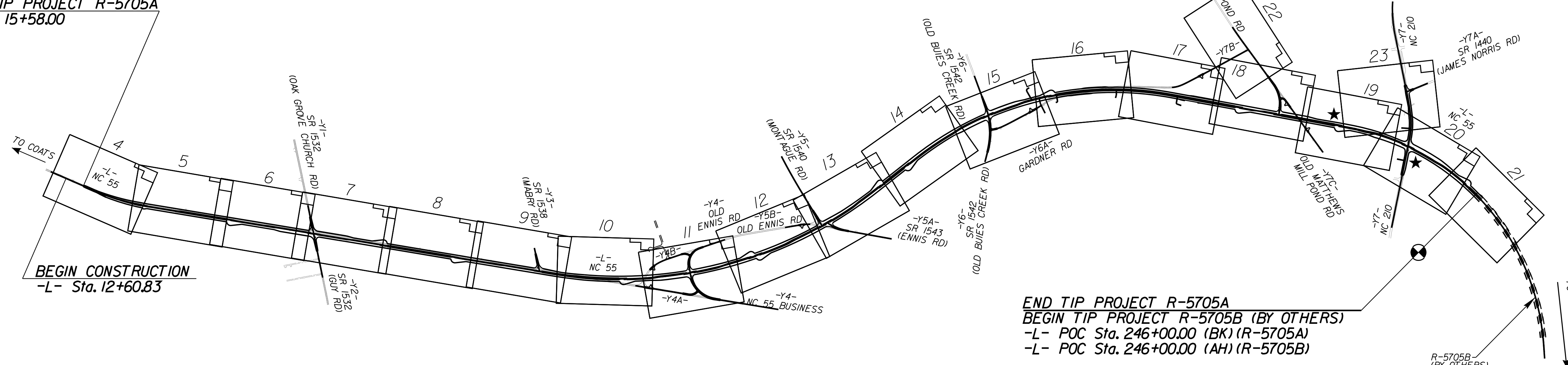


STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS
HARNETT COUNTY

LOCATION: NC 55 FROM JUST SOUTH OF SR 1532 (OAK GROVE CHURCH ROAD) TO NC 210
TYPE OF WORK: DRAINAGE, GRADING, PAVING, SIGNING, SIGNALS, AND CULVERTS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-5705A	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
46377.1.2		P.E.	
46377.2.1		RIGHT-OF-WAY	
46377.2.5		UTILITIES	
46377.3.1		CONSTRUCTION	

BEGIN TIP PROJECT R-5705A
-L- Sta. 15+58.00



END TIP PROJECT R-5705A
BEGIN TIP PROJECT R-5705B (BY OTHERS)
-L- POC Sta. 246+00.00 (BK) (R-5705A)
-L- POC Sta. 246+00.00 (AH) (R-5705B)

** DESIGN SPEED = 50 MPH FROM -L- STA. 15+58.00 TO STA. 112+18.00 (EXISTING NC 55)
60 MPH FROM -L- STA. 112+18.00 TO STA. 246+00.00 (NEW LOCATION)

★ TRAFFIC SIGNAL
THIS IS A PARTIAL CONTROLLED-ACCESS PROJECT WITH ACCESS BEING LIMITED TO POINTS SHOWN ON THE PLANS

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

<p>GRAPHIC SCALES</p> <p>50 25 0 50 100 PLANS</p> <p>50 25 0 50 100 PROFILE (HORIZONTAL)</p> <p>5 2.5 0 5 10 PROFILE (VERTICAL)</p>	<p>DESIGN DATA</p> <p>AADT 2023 = 16,400 AADT 2045 = 27,100 K = 9% D = 60% T = 4%* ** V = 50/60 MPH * (TTST 1% + DUAL 3%) FUNCTIONAL CLASSIFICATION: MINOR ARTERIAL REGIONAL TIER</p>	<p>PROJECT LENGTH</p> <p>LENGTH ROADWAY TIP PROJECT R-5705A = 4.364 MILES TOTAL LENGTH ROADWAY TIP PROJECT R-5705A = 4.364 MILES</p>	<p>PLANS PREPARED FOR THE NCDOT BY:</p> <p>Kimley Horn</p> <p>2018 STANDARD SPECIFICATIONS</p> <p>RIGHT OF WAY DATE: MARCH 9, 2021</p> <p>LETTING DATE: SEPTEMBER 19, 2023</p> <p>JEFFREY W. MOORE, P.E. PROJECT ENGINEER EVERETT J. LOVING, P.E. PROJECT DESIGN ENGINEER DAVID WEBB, P.E. SENIOR PROJECT MANAGER NCDOT PROJECT MANAGEMENT UNIT</p>	<p>HYDRAULIC ENGINEER</p> <p><i>Jeffrey W. Moore</i> 052128 SEP 2023 JEFFREY W. MOORE P.E.</p> <p>SIGNATURE: ROADWAY DESIGN ENGINEER</p> <p><i>David Webb</i> 074436 9/19/2023 DAVID WEBB P.E.</p> <p>SIGNATURE:</p>	
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GENERAL NOTES

2018 SPECIFICATIONS

EFFECTIVE: 01-16-18

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 OR STD. NO. 225.05 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01 AND STD. NO. 560.02.

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

BERM DITCHES:

BERM DITCHES SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 240.01 AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

SUBSURFACE DRAINS:

SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT LOCATIONS DIRECTED BY THE ENGINEER.

STREET TURNOUT:

STREET RETURNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 848.04 USING THE RADI NOTED ON PLANS.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC NOT SHOWN ON THE PLANS WILL BE PAID FOR AT THE CONTRACT PRICE FOR 'TEMPORARY SHORING'.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE :

- DUKE ENERGY
PIEDMONT NATURAL GAS
CHARTER
CENTURYLINK
CONTERRA
SEGRA
HARNETT REGIONAL WATER

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON THE PLANS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.

2018 ROADWAY ENGLISH STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN 'ROADWAY STANDARD DRAWINGS' HIGHWAY DESIGN BRANCH - N. C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N. C., DATED JANUARY, 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

Table with 2 columns: STD.NO. and TITLE. Lists various standards for earthwork, pipe culverts, subgrade, asphalt bases, pavement repairs, incidentals, and drainage structures.

EFF. 01-16-2018

R-5705A
HARNETT COUNTY

INDEX OF SHEETS

Table with 2 columns: SHEET NUMBER and SHEET. Lists sheet numbers (I, 1A, 1B, 2A-1 THRU 2A-7, etc.) and their corresponding sheet titles.

REVISIONS

8/8/2023

STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS CONVENTIONAL PLAN SHEET SYMBOLS

BOUNDARIES AND PROPERTY:

State Line	-----
County Line	-----
Township Line	-----
City Line	-----
Reservation Line	-----
Property Line	-----
Existing Iron Pin	○ EIP
Computed Property Corner	----->
Property Monument	□ ECM
Parcel/Sequence Number	(23)
Existing Fence Line	-x-x-x-
Proposed Woven Wire Fence	○
Proposed Chain Link Fence	□
Proposed Barbed Wire Fence	◇
Existing Wetland Boundary	-----MLB
Proposed Wetland Boundary	-----MLB
Existing Endangered Animal Boundary	-----EAB
Existing Endangered Plant Boundary	-----EPB
Existing Historic Property Boundary	-----HPB
Known Contamination Area: Soil	☠-S-☠
Potential Contamination Area: Soil	??-S-??
Known Contamination Area: Water	☠-W-☠
Potential Contamination Area: Water	??-W-??
Contaminated Site: Known or Potential	☠??

BUILDINGS AND OTHER CULTURE:

Gas Pump Vent or U/G Tank Cap	○
Sign	○ S
Well	○ W
Small Mine	✕
Foundation	□
Area Outline	□
Cemetery	⊕
Building	□
School	□
Church	⊕
Dam	▬

HYDROLOGY:

Stream or Body of Water	-----
Hydro, Pool or Reservoir	□
Jurisdictional Stream	-----JS
Buffer Zone 1	-----BZ 1
Buffer Zone 2	-----BZ 2
Flow Arrow	←
Disappearing Stream	-----
Spring	○
Wetland	-----
Proposed Lateral, Tail, Head Ditch	-----
False Sump	▽

RAILROADS:

Standard Gauge	-----
RR Signal Milepost	○ MILEPOST 35
Switch	□ SWITCH
RR Abandoned	-----
RR Dismantled	-----

Note: Not to Scale

*S.U.E. = Subsurface Utility Engineering

RIGHT OF WAY & PROJECT CONTROL:

Secondary Horiz and Vert Control Point	◆
Primary Horiz Control Point	○
Primary Horiz and Vert Control Point	●
Exist Permanent Easment Pin and Cap	◇
New Permanent Easment Pin and Cap	◆
Vertical Benchmark	⊕
Existing Right of Way Marker	△
Existing Right of Way Line	-----
New Right of Way Line	-----
New Right of Way Line with Pin and Cap	-----
New Right of Way Line with Concrete or Granite RW Marker	-----
New Control of Access Line with Concrete C/A Marker	-----
Existing Control of Access	-----
New Control of Access	-----
Existing Easement Line	-----E
New Temporary Construction Easement	-----E
New Temporary Drainage Easement	-----TDE
New Permanent Drainage Easement	-----PDE
New Permanent Drainage / Utility Easement	-----DUE
New Permanent Utility Easement	-----PUE
New Temporary Utility Easement	-----TUE
New Aerial Utility Easement	-----AUE

ROADS AND RELATED FEATURES:

Existing Edge of Pavement	-----
Existing Curb	-----
Proposed Slope Stakes Cut	-----C
Proposed Slope Stakes Fill	-----F
Proposed Curb Ramp	-----
Existing Metal Guardrail	-----
Proposed Guardrail	-----
Existing Cable Guiderail	-----
Proposed Cable Guiderail	-----
Equality Symbol	⊕
Pavement Removal	-----

VEGETATION:

Single Tree	☼
Single Shrub	☼

Hedge	-----
Woods Line	-----
Orchard	☼☼☼☼
Vineyard	-----Vineyard

EXISTING STRUCTURES:

MAJOR:	
Bridge, Tunnel or Box Culvert	-----CONC
Bridge Wing Wall, Head Wall and End Wall	-----CONC WW
MINOR:	
Head and End Wall	-----CONC HW
Pipe Culvert	-----
Footbridge	-----
Drainage Box: Catch Basin, DI or JB	□ CB
Paved Ditch Gutter	-----
Storm Sewer Manhole	○ S
Storm Sewer	-----S

UTILITIES:

POWER:	
Existing Power Pole	●
Proposed Power Pole	○
Existing Joint Use Pole	●
Proposed Joint Use Pole	○
Power Manhole	⊕
Power Line Tower	⊕
Power Transformer	⊕
U/G Power Cable Hand Hole	-----
H-Frame Pole	●●
U/G Power Line LOS B (S.U.E.*)	-----P
U/G Power Line LOS C (S.U.E.*)	-----P
U/G Power Line LOS D (S.U.E.*)	-----P

TELEPHONE:

Existing Telephone Pole	●
Proposed Telephone Pole	○
Telephone Manhole	⊕
Telephone Pedestal	⊕
Telephone Cell Tower	⊕
U/G Telephone Cable Hand Hole	-----
U/G Telephone Cable LOS B (S.U.E.*)	-----T
U/G Telephone Cable LOS C (S.U.E.*)	-----T
U/G Telephone Cable LOS D (S.U.E.*)	-----T
U/G Telephone Conduit LOS B (S.U.E.*)	-----TC
U/G Telephone Conduit LOS C (S.U.E.*)	-----TC
U/G Telephone Conduit LOS D (S.U.E.*)	-----TC
U/G Fiber Optics Cable LOS B (S.U.E.*)	-----T FO
U/G Fiber Optics Cable LOS C (S.U.E.*)	-----T FO
U/G Fiber Optics Cable LOS D (S.U.E.*)	-----T FO

WATER:

Water Manhole	⊕
Water Meter	○
Water Valve	⊕
Water Hydrant	⊕
U/G Water Line LOS B (S.U.E.*)	-----
U/G Water Line LOS C (S.U.E.*)	-----
U/G Water Line LOS D (S.U.E.*)	-----
Above Ground Water Line	-----A/G Water

TV:

TV Pedestal	⊕
TV Tower	⊕
U/G TV Cable Hand Hole	-----
U/G TV Cable LOS B (S.U.E.*)	-----TV
U/G TV Cable LOS C (S.U.E.*)	-----TV
U/G TV Cable LOS D (S.U.E.*)	-----TV
U/G Fiber Optic Cable LOS B (S.U.E.*)	-----TV FO
U/G Fiber Optic Cable LOS C (S.U.E.*)	-----TV FO
U/G Fiber Optic Cable LOS D (S.U.E.*)	-----TV FO

GAS:

Gas Valve	◇
Gas Meter	◇
U/G Gas Line LOS B (S.U.E.*)	-----G
U/G Gas Line LOS C (S.U.E.*)	-----G
U/G Gas Line LOS D (S.U.E.*)	-----G
Above Ground Gas Line	-----A/G Gas

SANITARY SEWER:

Sanitary Sewer Manhole	⊕
Sanitary Sewer Cleanout	⊕
U/G Sanitary Sewer Line	-----SS
Above Ground Sanitary Sewer	-----A/G Sanitary Sewer
SS Forced Main Line LOS B (S.U.E.*)	-----FSS
SS Forced Main Line LOS C (S.U.E.*)	-----FSS
SS Forced Main Line LOS D (S.U.E.*)	-----FSS

MISCELLANEOUS:

Utility Pole	●
Utility Pole with Base	□
Utility Located Object	○
Utility Traffic Signal Box	⊕
Utility Unknown U/G Line LOS B (S.U.E.*)	-----TUTL
U/G Tank; Water, Gas, Oil	□
Underground Storage Tank, Approx. Loc.	⊕
A/G Tank; Water, Gas, Oil	□
Geoenvironmental Boring	⊕
U/G Test Hole LOS A (S.U.E.*)	⊕
Abandoned According to Utility Records	AATUR
End of Information	E.O.I.

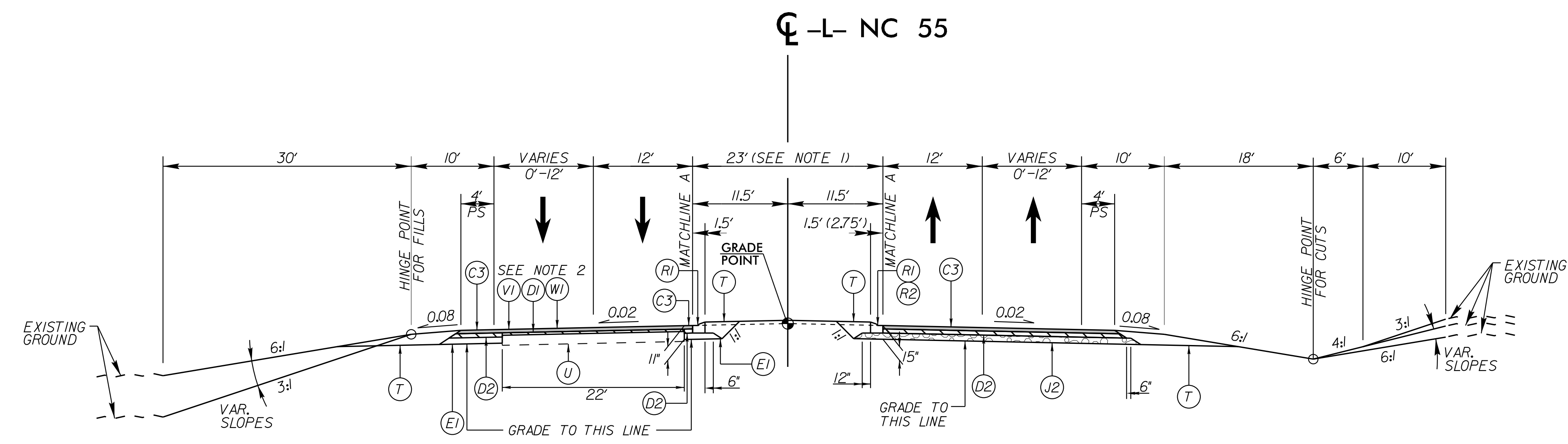
5/14/99

- NOTES:
 1. SEE PLANS FOR MEDIAN WIDTHS
 2. MILL EXIST. PAVEMENT TO A DEPTH OF 2.5" (V1), REPLACE WITH 2.5" 11.9.0C (D1), AND OVERLAY WITH 3" S9.5C (C3)
 3. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED

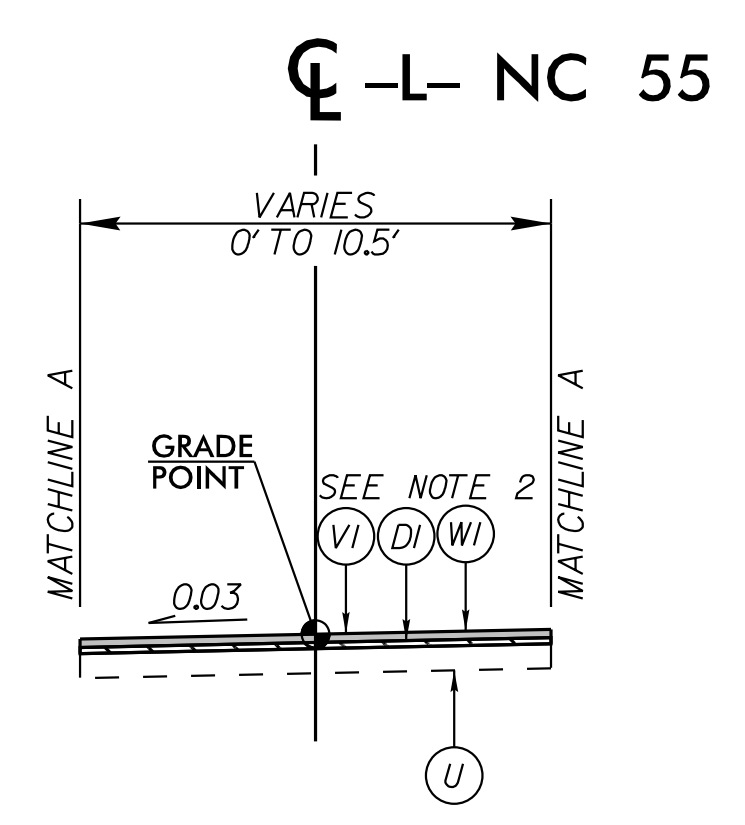
Kimley Horn
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2A-1
ROADWAY DESIGN ENGINEER	PAVEMENT DESIGN ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

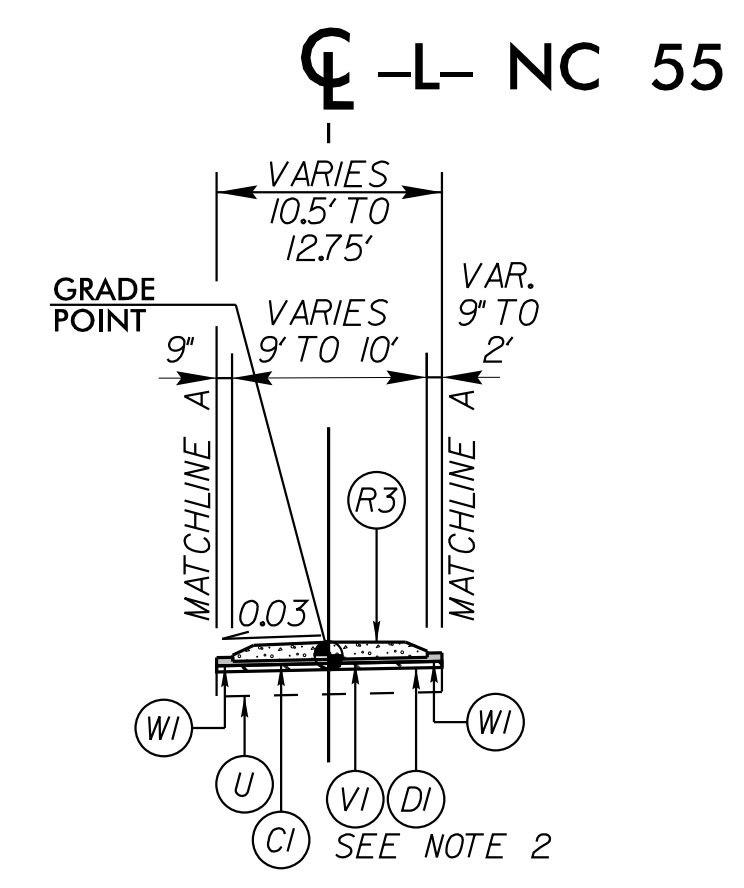
REVISIONS



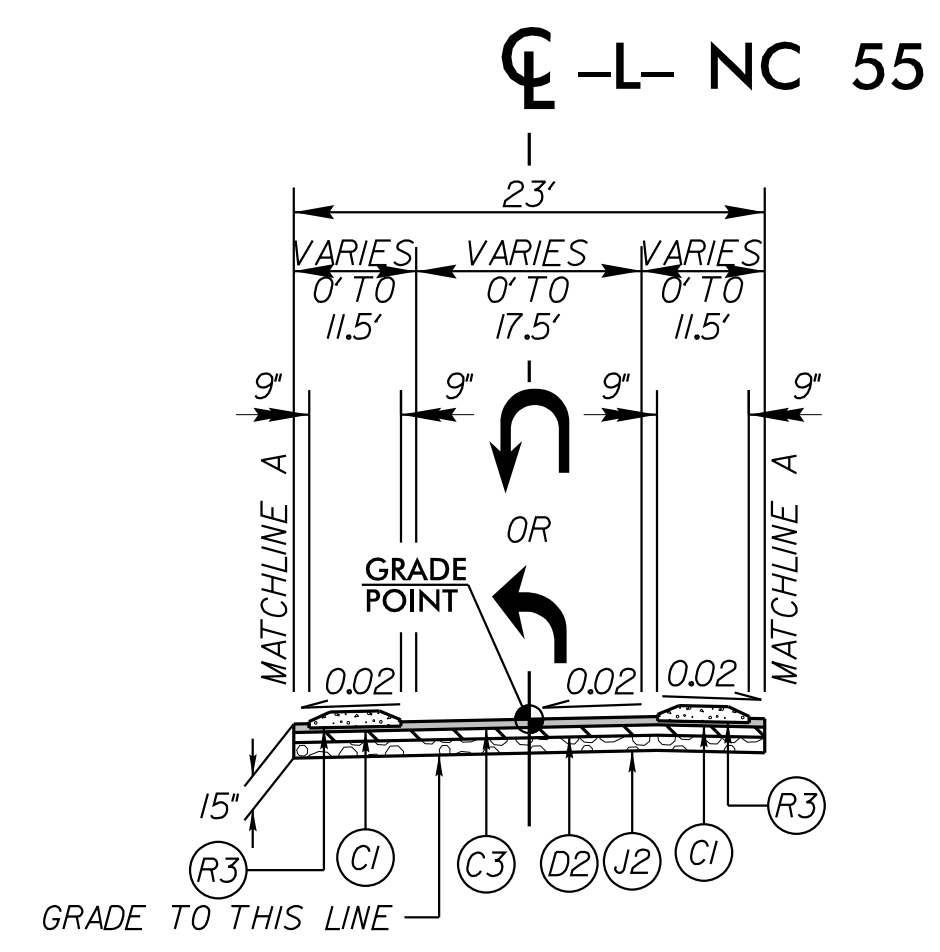
TYPICAL SECTION NO. 1
 -L- STA 15+58.00 TO 92+74.00



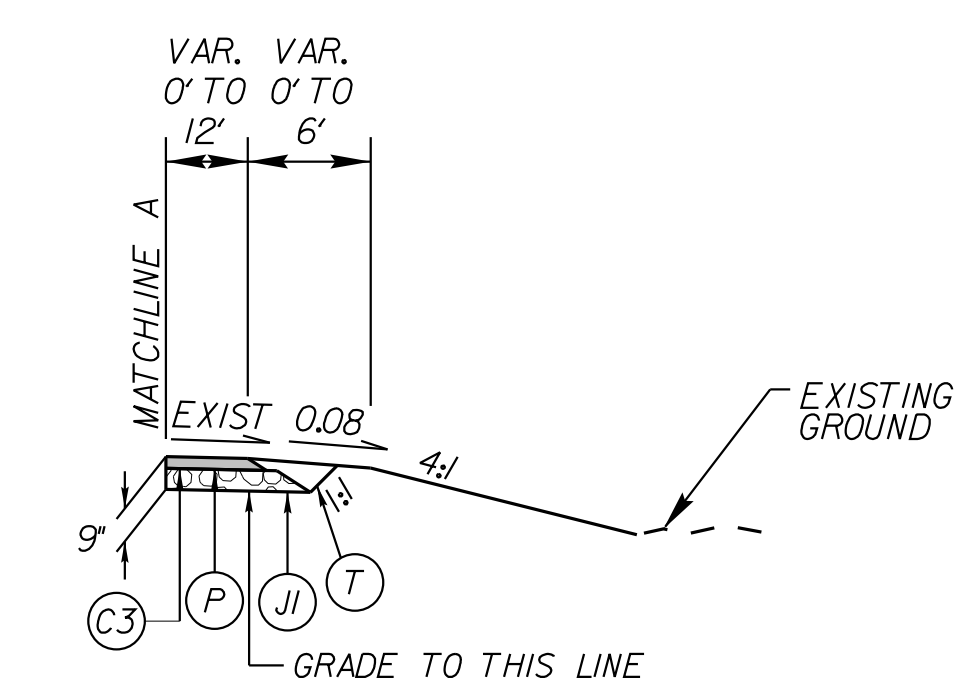
TYPICAL SECTION NO. 1A
 -L- STA 15+58.00 TO 21+12.50



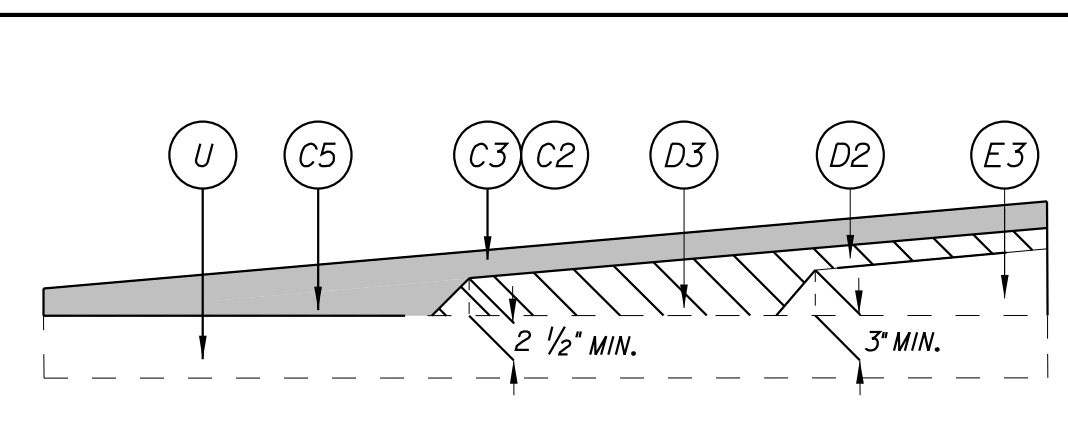
TYPICAL SECTION NO. 1B
 -L- STA 21+12.50 TO 21+80.00



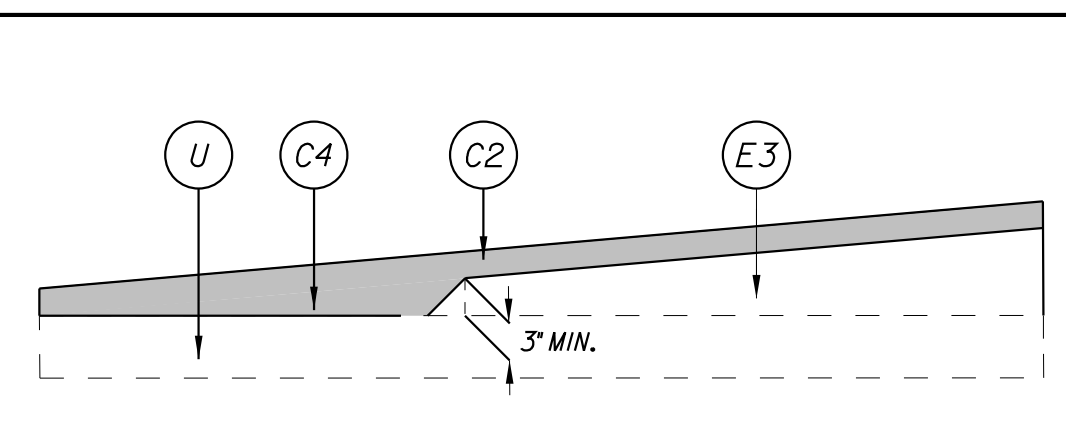
TYPICAL SECTION NO. 1C
 -L- STA 25+21.83 TO 30+54.33
 -L- STA 38+69.48 TO 64+26.00
 -L- STA 74+32.36 TO 88+79.10



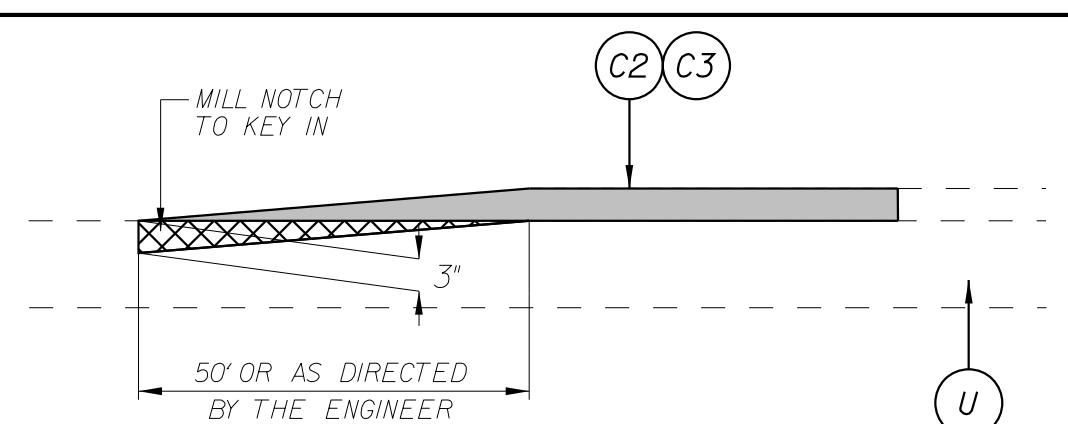
TYPICAL SECTION NO. 1D
 TEMPORARY WIDENING
 -L- STA 21+21.92 TO 26+55.00 (LT)



WEDGING DETAIL W1 FOR RESURFACING



WEDGING DETAIL W2 FOR RESURFACING



INCIDENTAL MILLING DETAIL

8/10/2023

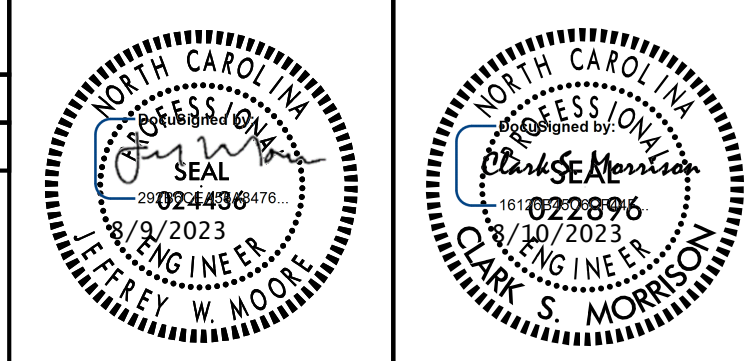
5/14/99

NOTES:
 1. SEE PLANS FOR MEDIAN WIDTHS
 2. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED

Kimley Horn
 ROADWAY DESIGN ENGINEER
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO. R-5705A
 SHEET NO. 2A-2

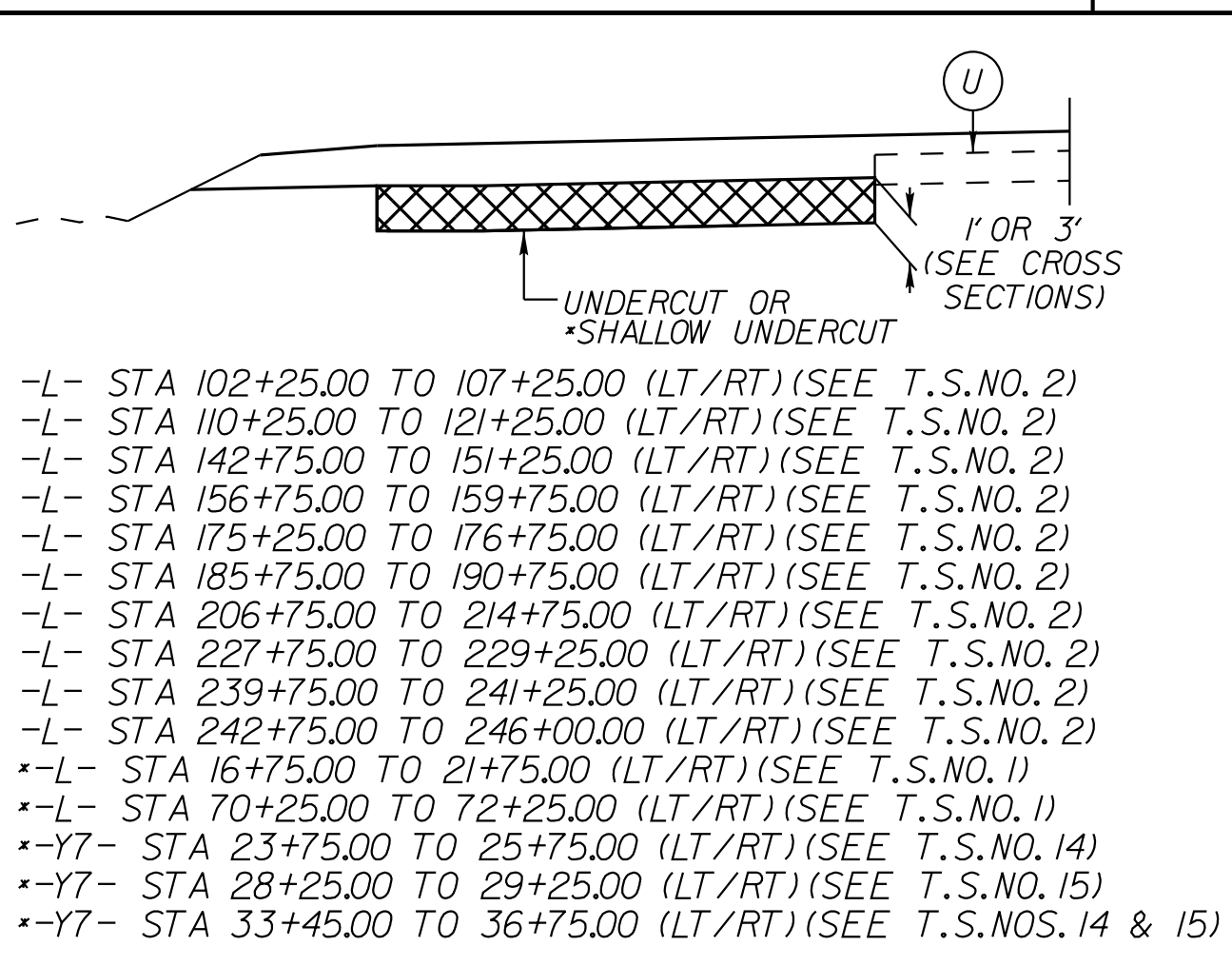
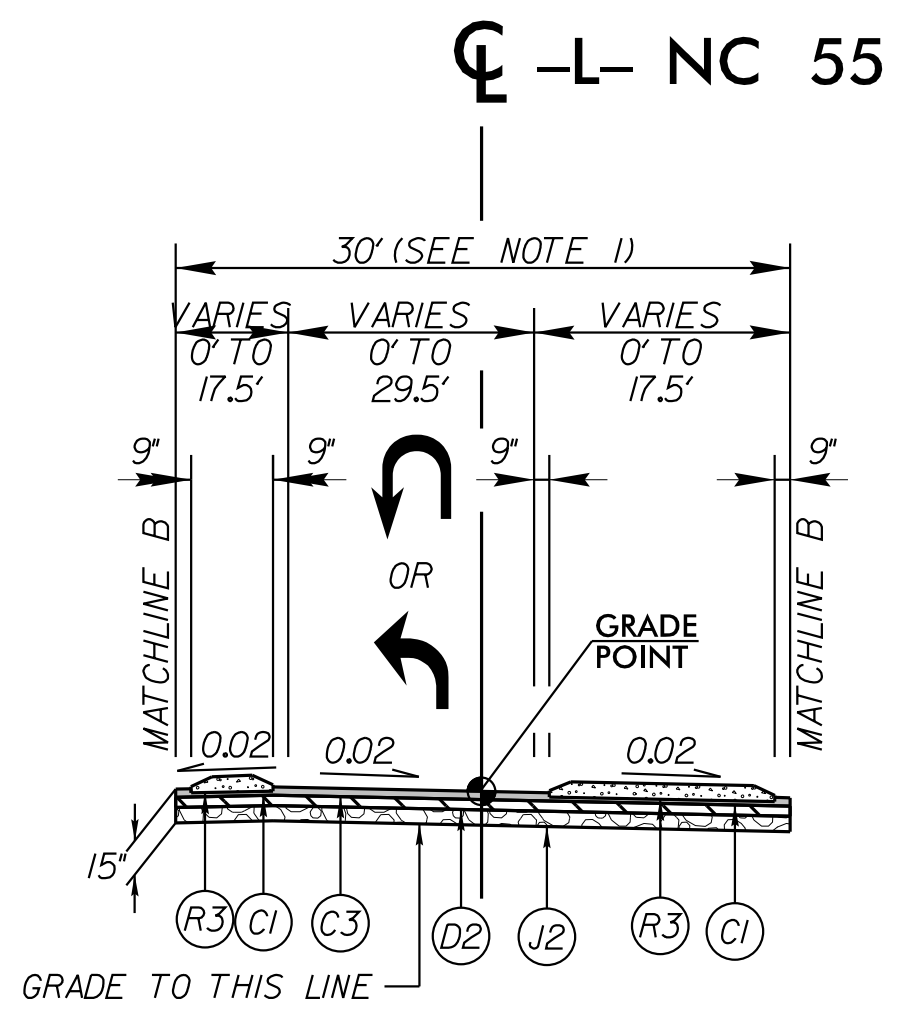
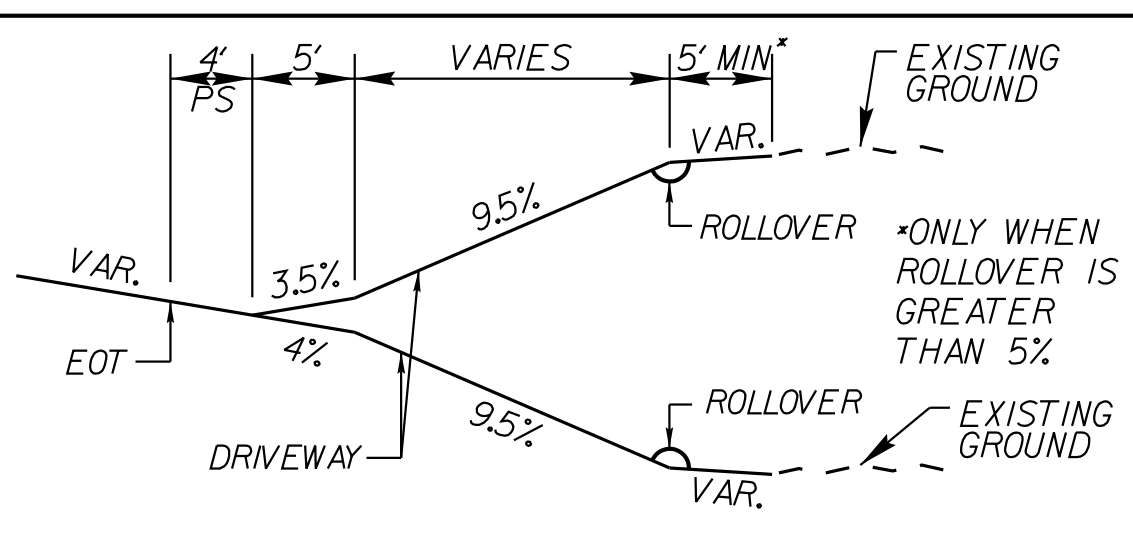
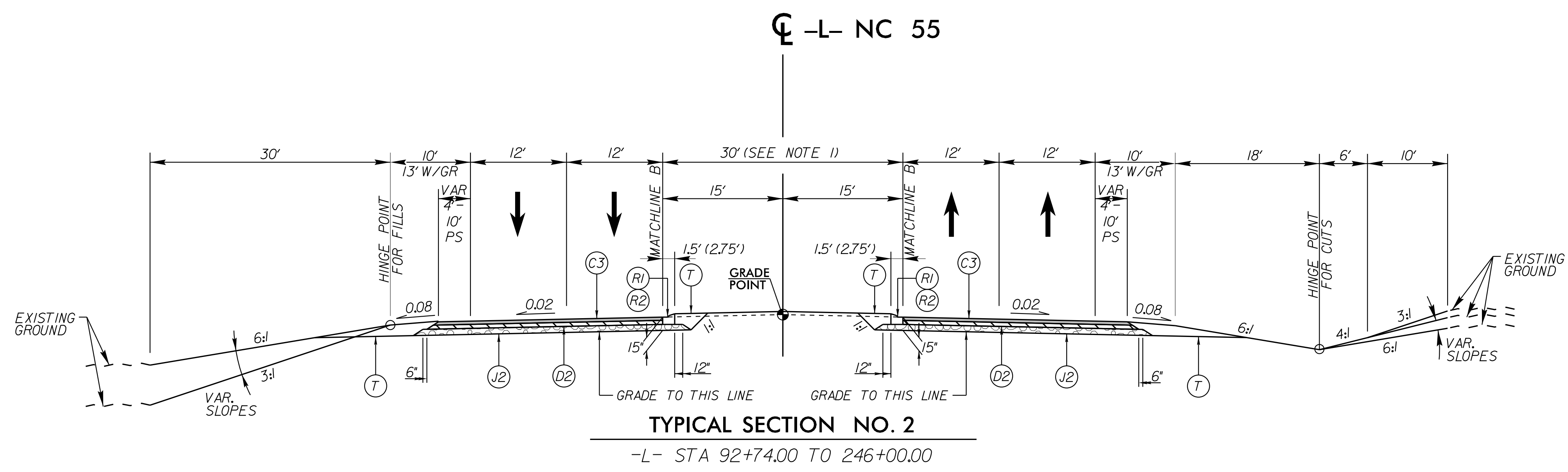
RIGHT-OF-WAY REV.
 CONST. REV.



**DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED**

PAVEMENT SCHEDULE

C1	PROPOSED APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C4	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
C5	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.
D1	PROPOSED APPROX. 2.5" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D2	PROPOSED APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D3	PROPOSED VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 2.5" OR GREATER THAN 4" IN DEPTH.
E1	PROPOSED APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROPOSED APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E3	PROPOSED VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.
J1	PROPOSED 6" AGGREGATE BASE COURSE
J2	PROPOSED 8" AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GAL/PER SQYD
R1	PROPOSED 1'-6" CONCRETE CURB & GUTTER
R2	PROPOSED 2'-9" CONCRETE CURB & GUTTER
R3	PROPOSED 5" MONOLITHIC CONCRETE ISLAND (KEYED-IN)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V1	MILLING ASPHALT PAVEMENT, 2.5" DEPTH
V2	MILLING ASPHALT PAVEMENT, 3" DEPTH
W1	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W1 FOR RESURFACING)
W2	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W2 FOR RESURFACING)



- L- STA 102+25.00 TO 107+25.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 110+25.00 TO 121+25.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 142+75.00 TO 151+25.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 156+75.00 TO 159+75.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 175+25.00 TO 176+75.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 185+75.00 TO 190+75.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 206+75.00 TO 214+75.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 227+75.00 TO 229+25.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 239+75.00 TO 241+25.00 (LT/RT) (SEE T.S. NO. 2)
- L- STA 242+75.00 TO 246+00.00 (LT/RT) (SEE T.S. NO. 2)
- *-L- STA 16+75.00 TO 21+75.00 (LT/RT) (SEE T.S. NO. 1)
- *-L- STA 70+25.00 TO 72+25.00 (LT/RT) (SEE T.S. NO. 1)
- *-Y7- STA 23+75.00 TO 25+75.00 (LT/RT) (SEE T.S. NO. 14)
- *-Y7- STA 28+25.00 TO 29+25.00 (LT/RT) (SEE T.S. NO. 15)
- *-Y7- STA 33+45.00 TO 36+75.00 (LT/RT) (SEE T.S. NOS. 14 & 15)

UNDERCUT EXCAVATION/SHALLOW UNDERCUT DETAIL

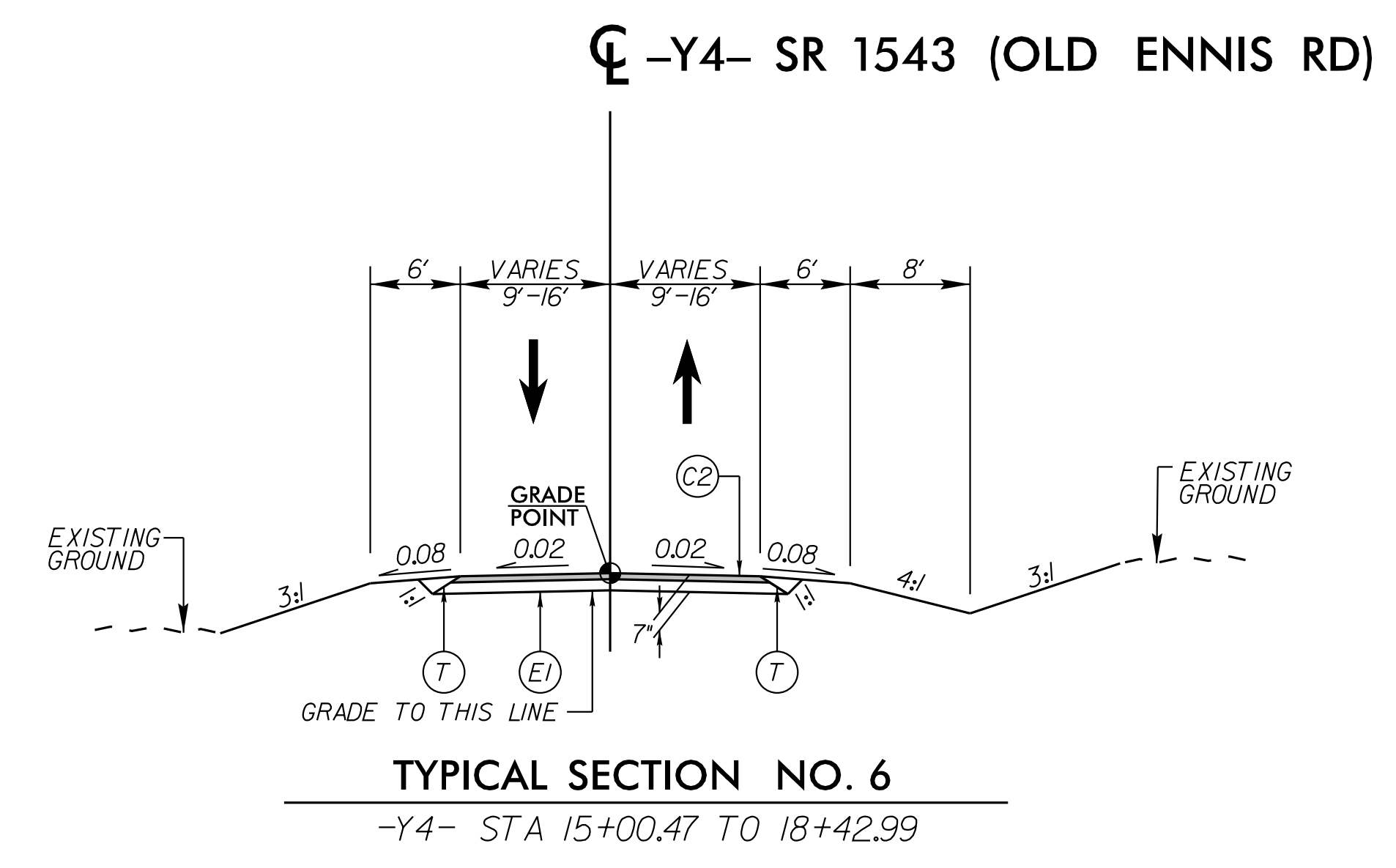
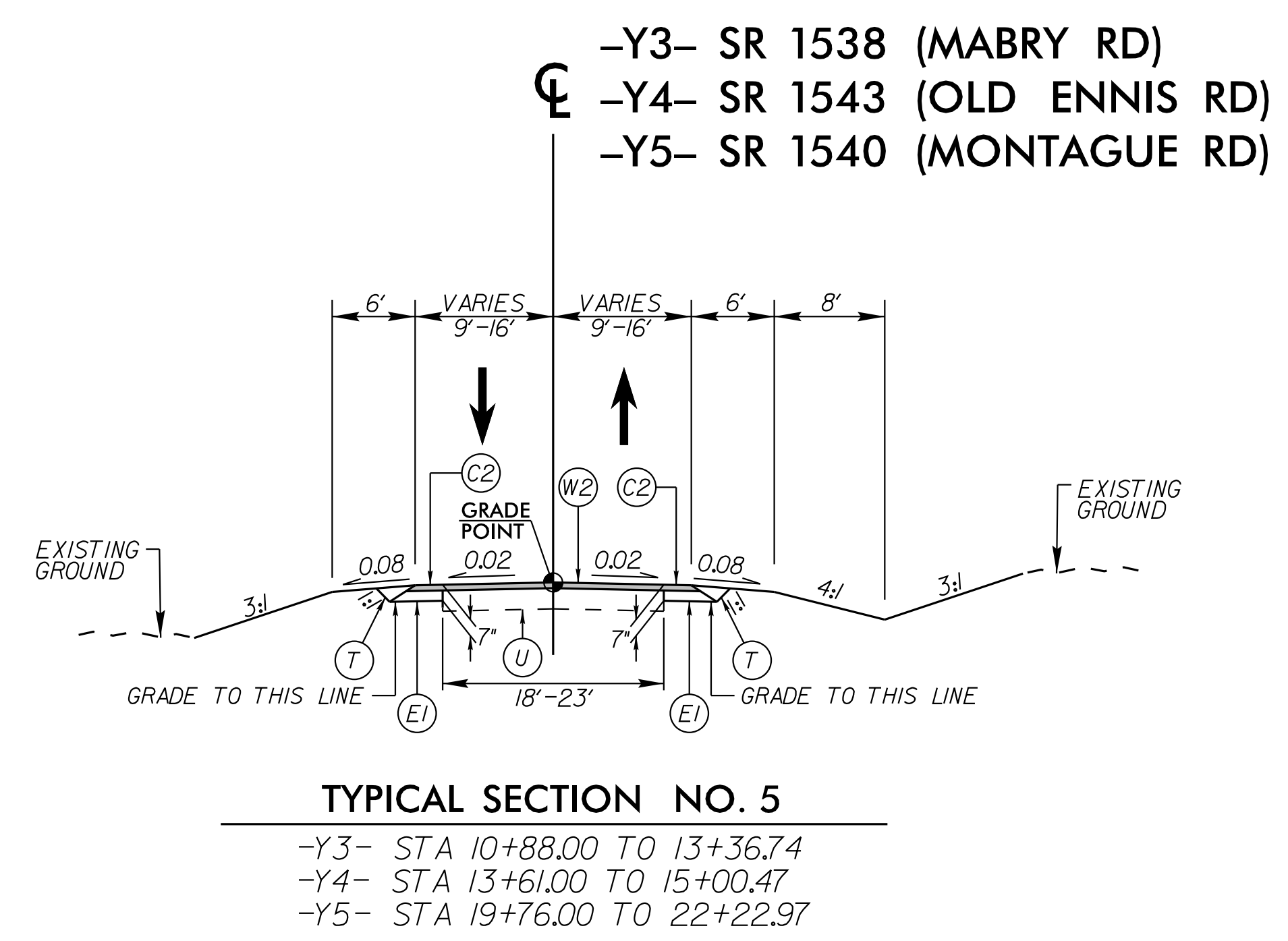
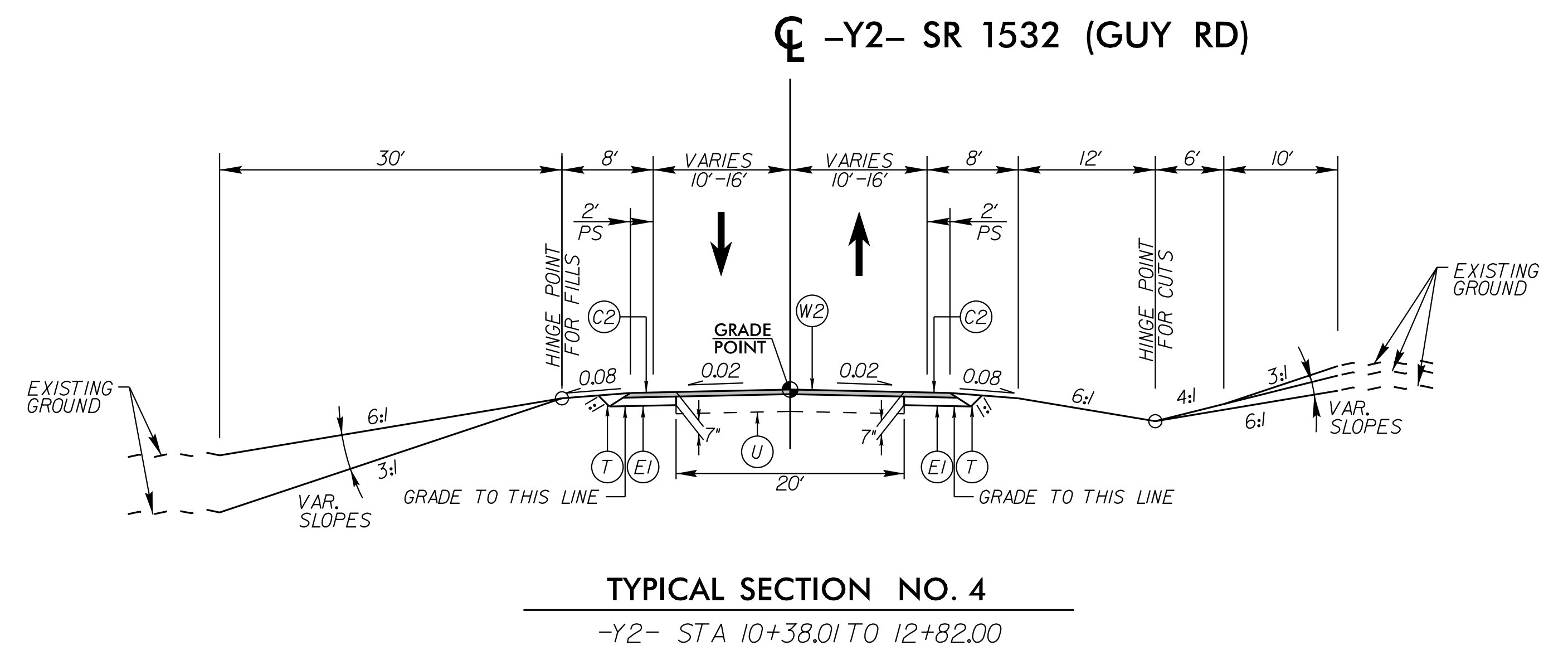
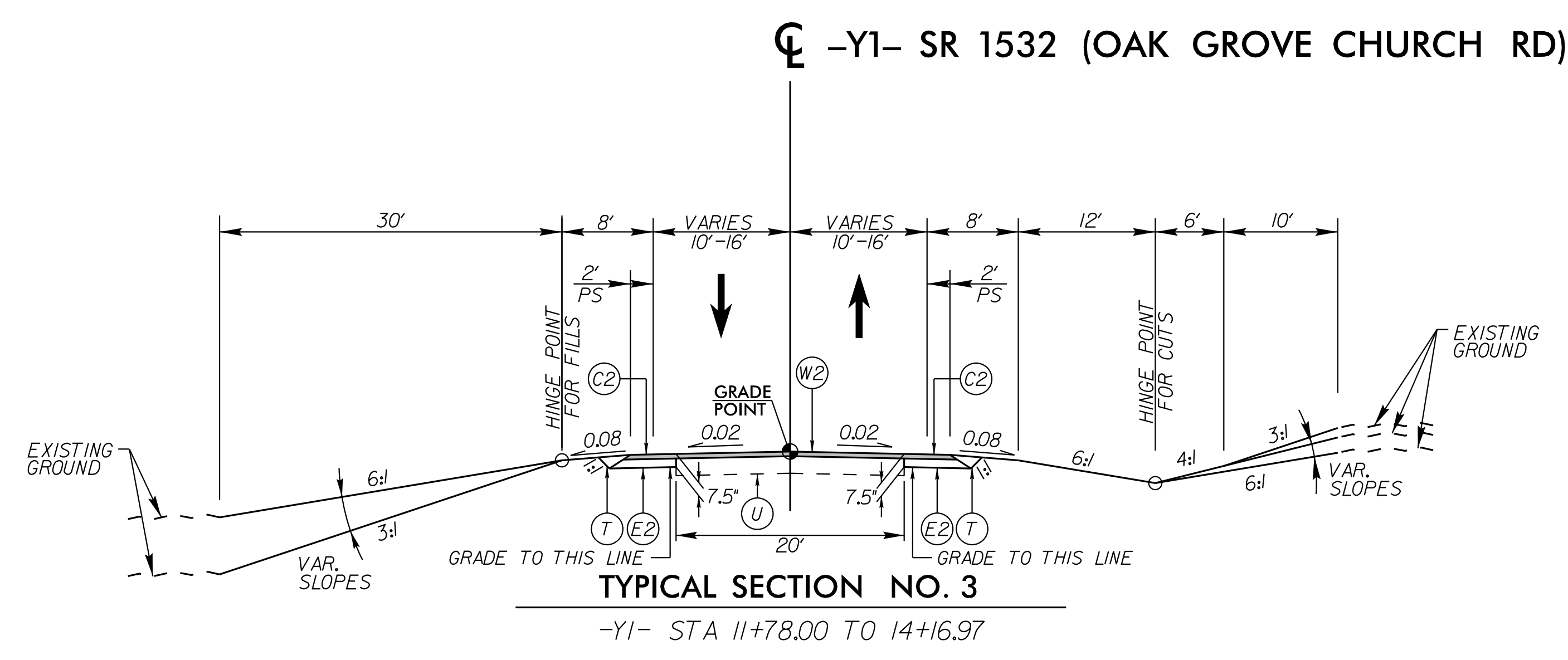
REVISIONS

8/8/2023

5/14/1999

NOTES:
1. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED

<p>P.O. BOX 33068 • RALEIGH, N.C. 27636-3068</p> <p>RIGHT-OF-WAY REV.</p> <p>CONST. REV.</p>	PROJECT REFERENCE NO. R-5705A	SHEET NO. 2A-3
	ROADWAY DESIGN ENGINEER	PAVEMENT DESIGN ENGINEER
<p>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</p>		



PAVEMENT SCHEDULE	
C1	PROPOSED APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C4	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
C5	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.
D1	PROPOSED APPROX. 2.5" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D2	PROPOSED APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D3	PROPOSED VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 2.5" OR GREATER THAN 4" IN DEPTH.
E1	PROPOSED APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROPOSED APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B25C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E3	PROPOSED VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.
J1	PROPOSED 6" AGGREGATE BASE COURSE
J2	PROPOSED 8" AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GAL/PER SQYD
R1	PROPOSED 1'-6" CONCRETE CURB & GUTTER
R2	PROPOSED 2'-9" CONCRETE CURB & GUTTER
R3	PROPOSED 5' MONOLITHIC CONCRETE ISLAND (KEYED-IN)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V1	MILLING ASPHALT PAVEMENT, 2.5" DEPTH
V2	MILLING ASPHALT PAVEMENT, 3" DEPTH
W1	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W1 FOR RESURFACING)
W2	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W2 FOR RESURFACING)

REVISIONS

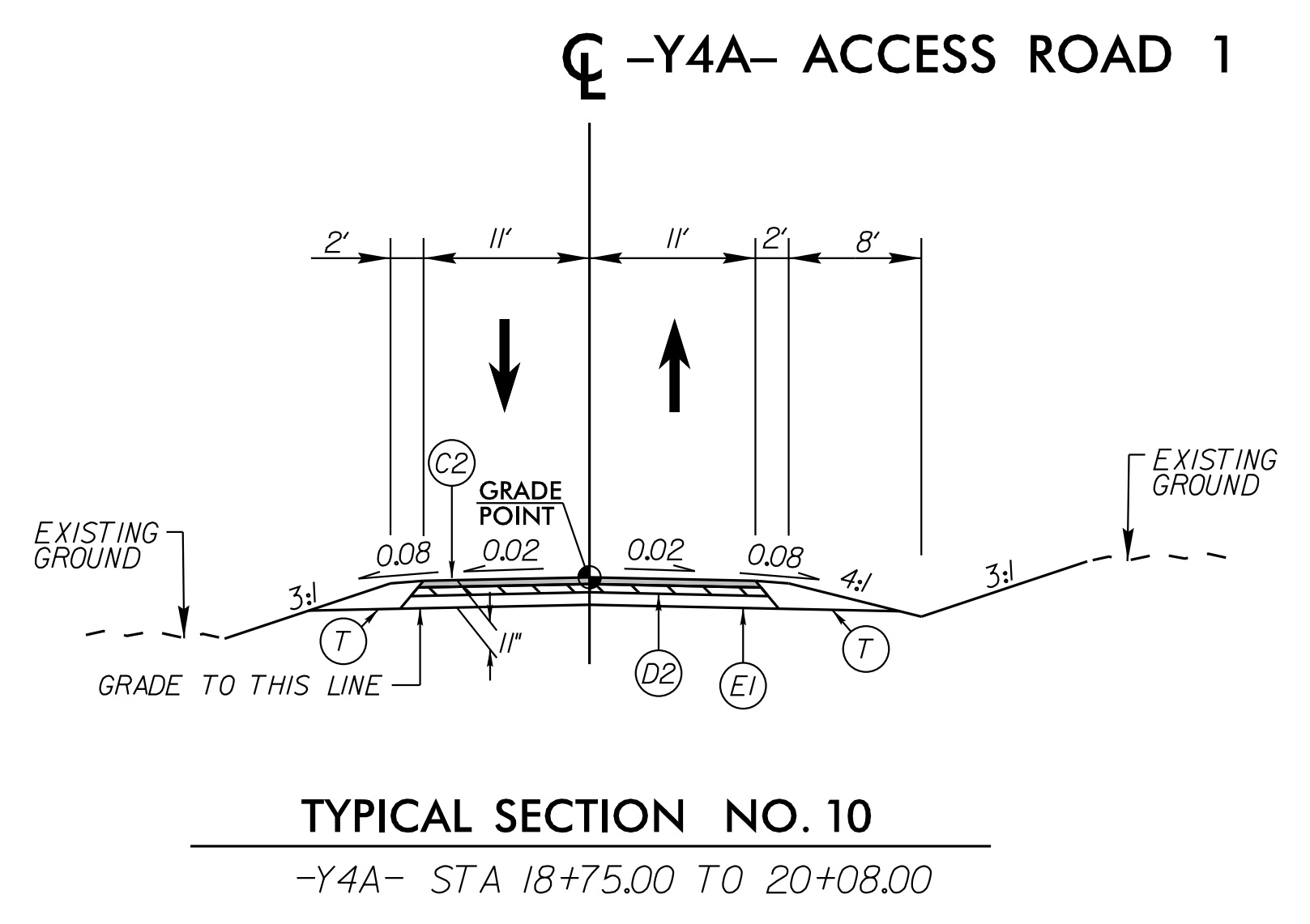
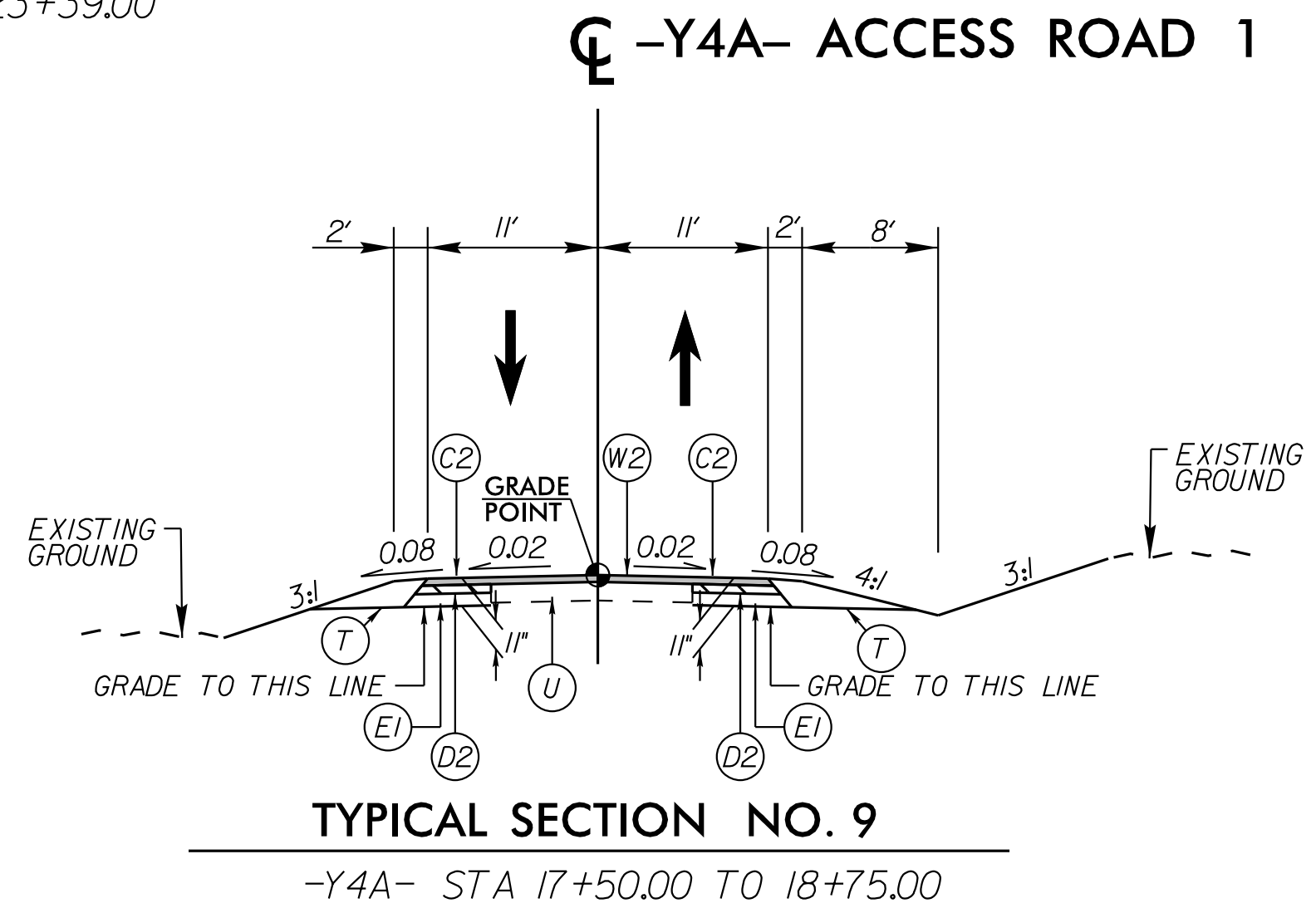
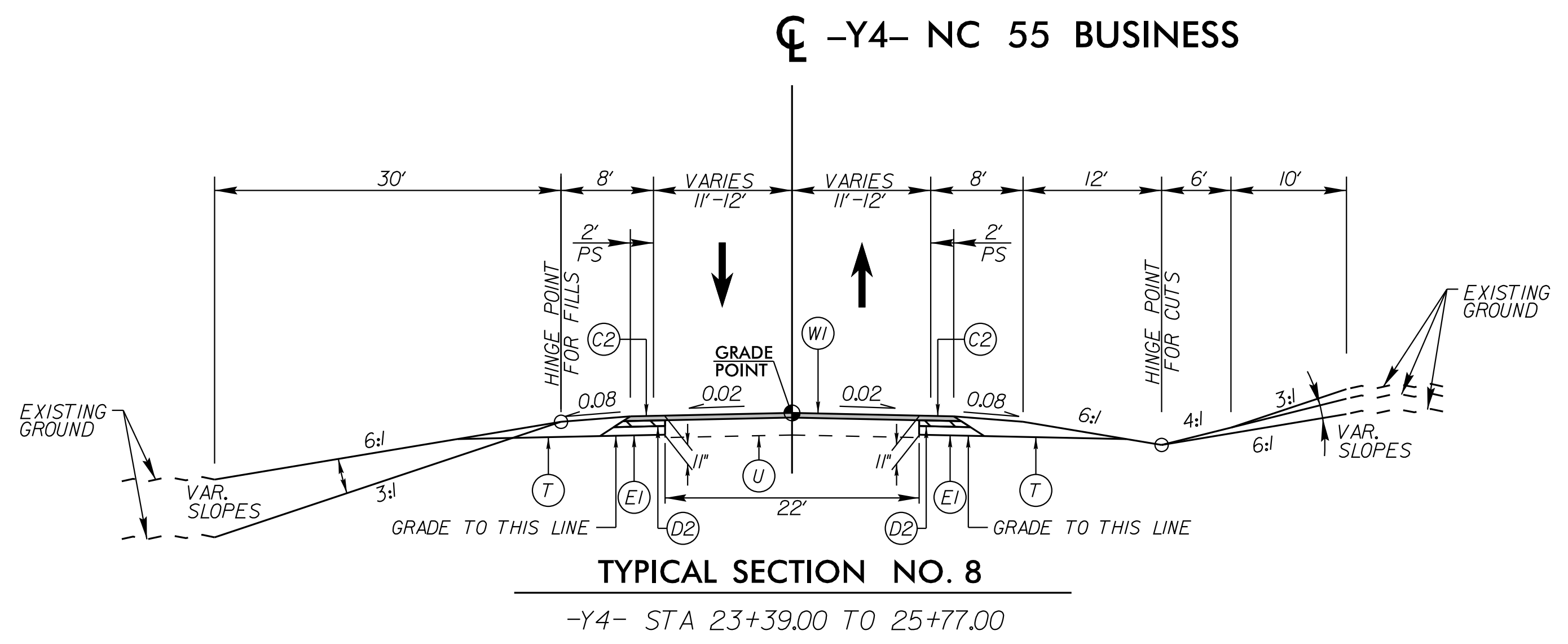
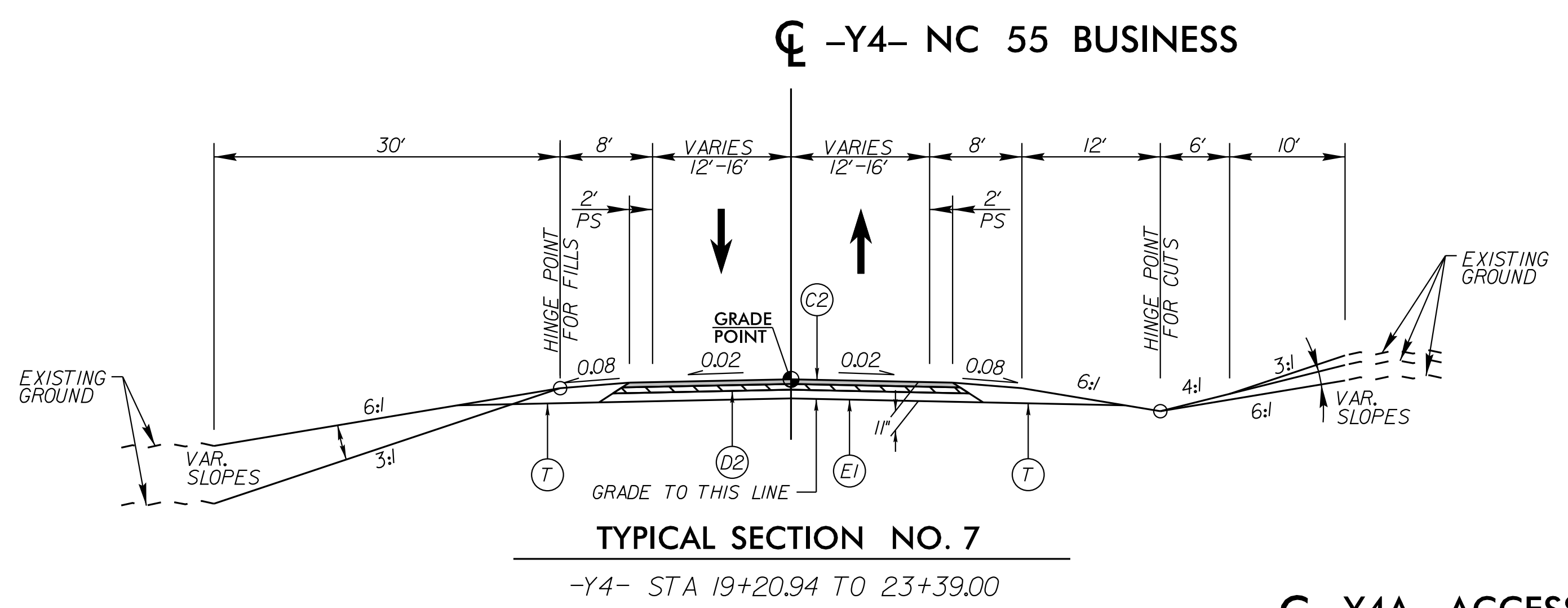
8/8/2023

5/14/2019

NOTES:
1. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED

Kimley Horn
P.O. BOX 33068 • RALEIGH, N.C. 27636-3068
RIGHT-OF-WAY REV.
CONST. REV.

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2A-4
ROADWAY DESIGN ENGINEER J. W. MOORE	PAVEMENT DESIGN ENGINEER CLARK S. MORRISON
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



-Y4A- ACCESS ROAD 1 TURNAROUND
 -Y4B- ACCESS ROAD 2 TURNAROUND
 -Y5B- OLD ENNIS RD TURNAROUND
 -Y6A- GARDNER RD TURNAROUND
 -Y7B- ACCESS ROAD 3 TURNAROUND
 -Y7C- OLD MATTHEWS MILL POND RD TURNAROUND

CL

TYPICAL SECTION NO. 11
-Y4A- TURNAROUND STA 11+40.00 TO 12+90.74
-Y4B- TURNAROUND STA 10+02.00 TO 11+10.00
-Y5B- TURNAROUND STA 12+15.00 TO 13+19.00
-Y6A- TURNAROUND STA 16+66.00 TO 18+19.00
-Y7B- TURNAROUND STA 18+00.00 TO 19+00.00
-Y7C- TURNAROUND STA 19+84.00 TO 20+84.00

	A	B
-Y4A- TURNAROUND	10.5' TO 52.5'	11.5'
-Y4B- TURNAROUND	10'	10' TO 54'
-Y5B- TURNAROUND	9'	11' TO 55'
-Y6A- TURNAROUND	10.5' TO 54.5'	9.5'
-Y7B- TURNAROUND	10' TO 54'	10'
-Y7C- TURNAROUND	9' TO 55'	9' TO 55'

REVISIONS

8/10/2023

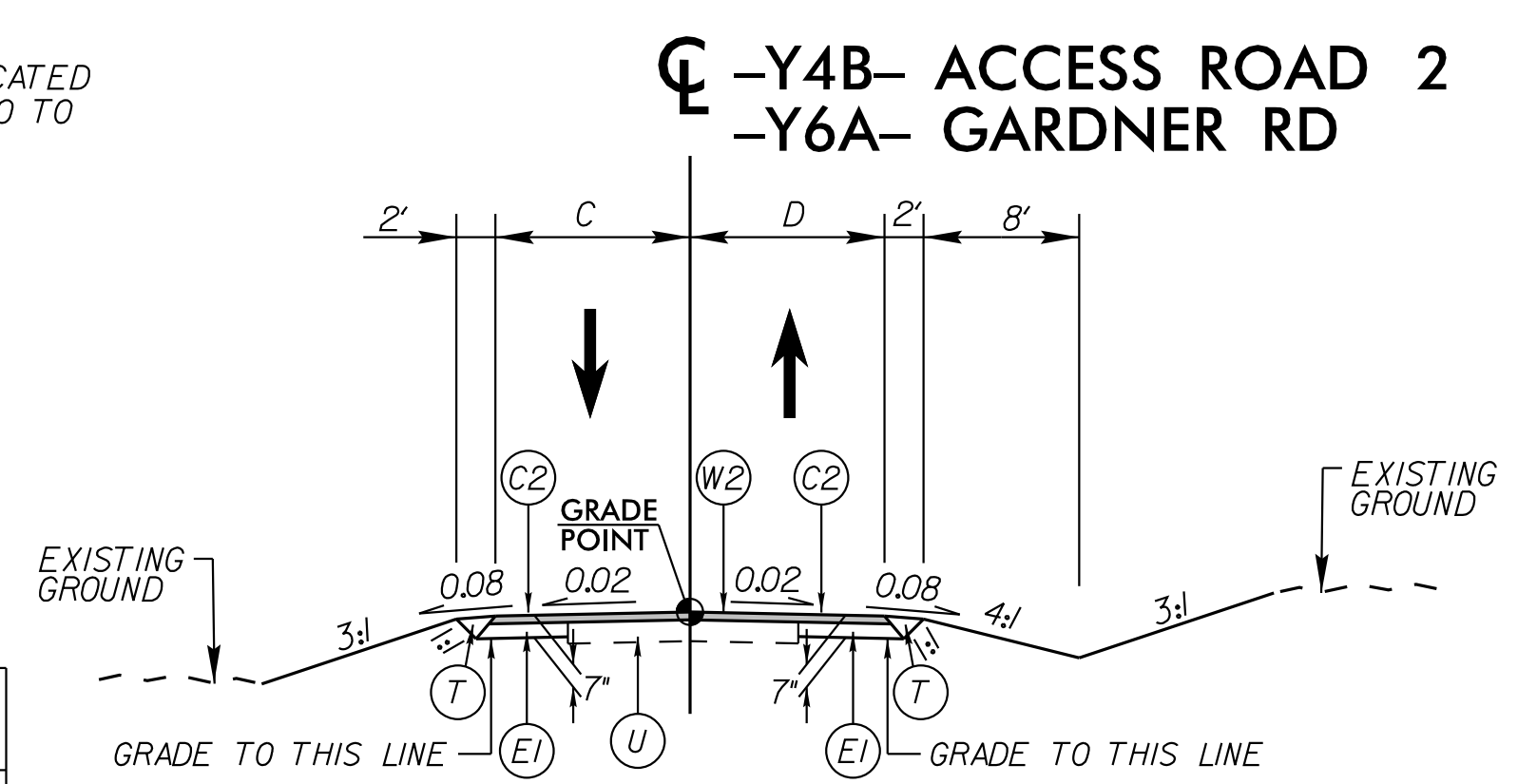
5/14/99

NOTES:
 1. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED
 2. CONSTRUCT 3:1 BACK SLOPE FROM -Y7- STA 23+30.00 TO -Y7A- INTERSECTION

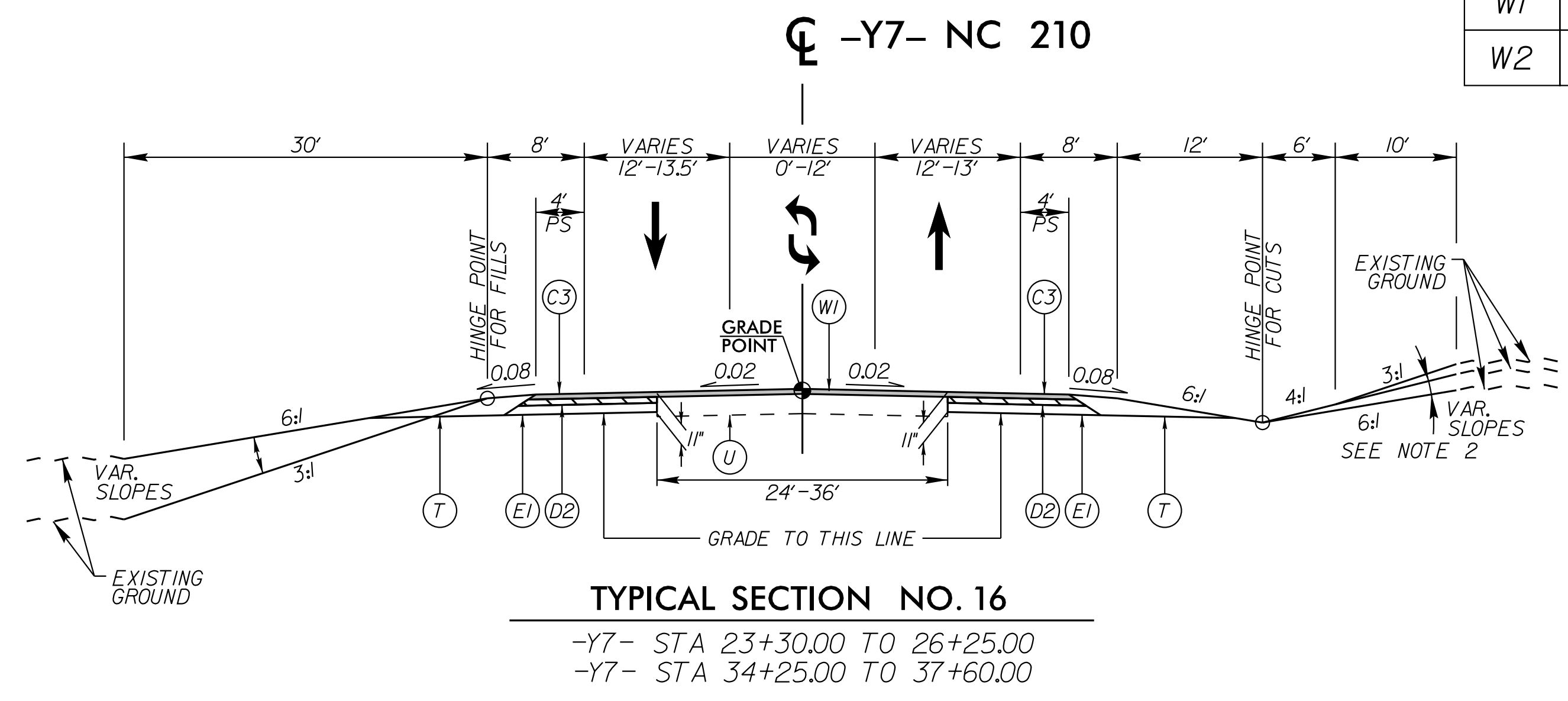
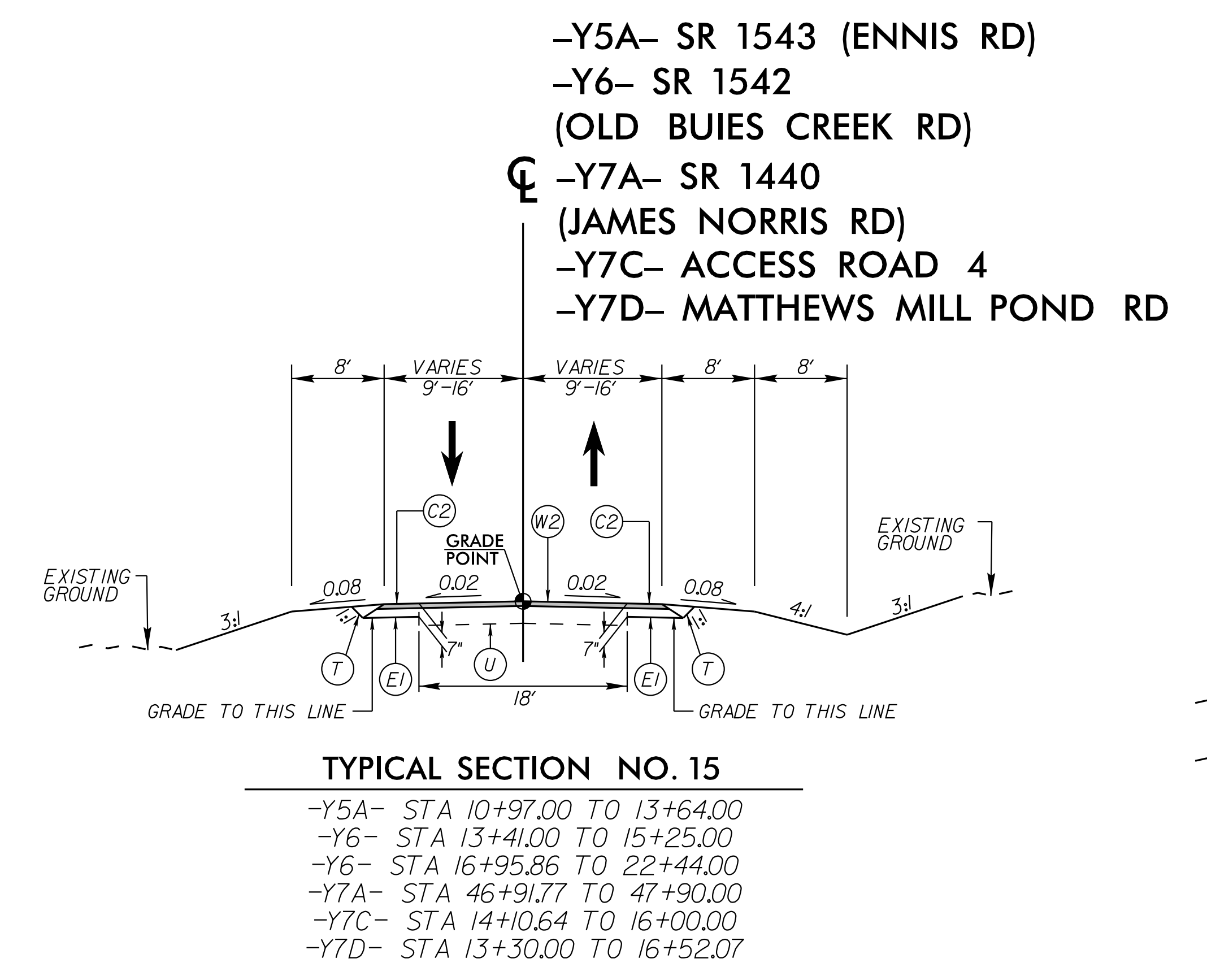
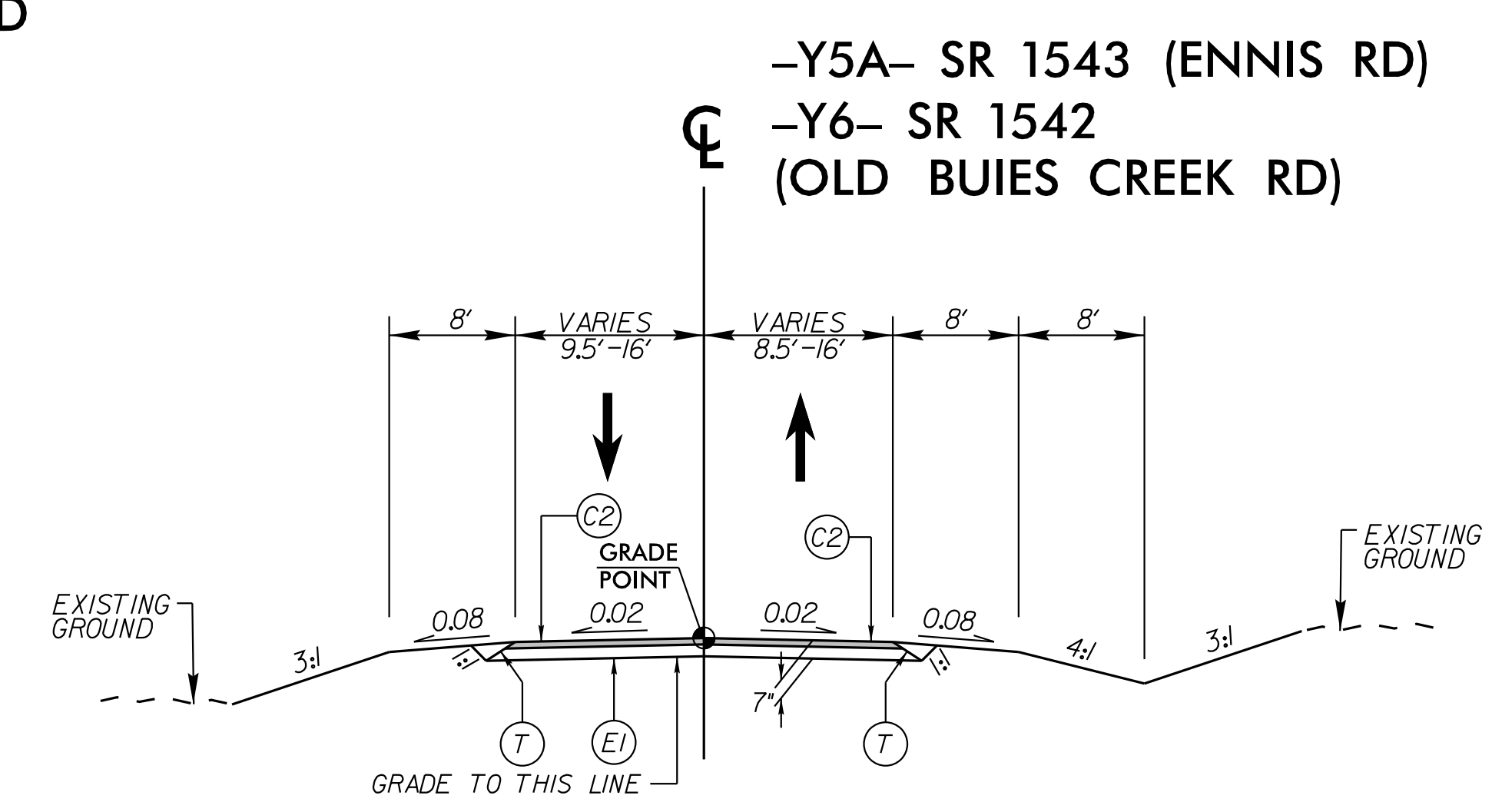
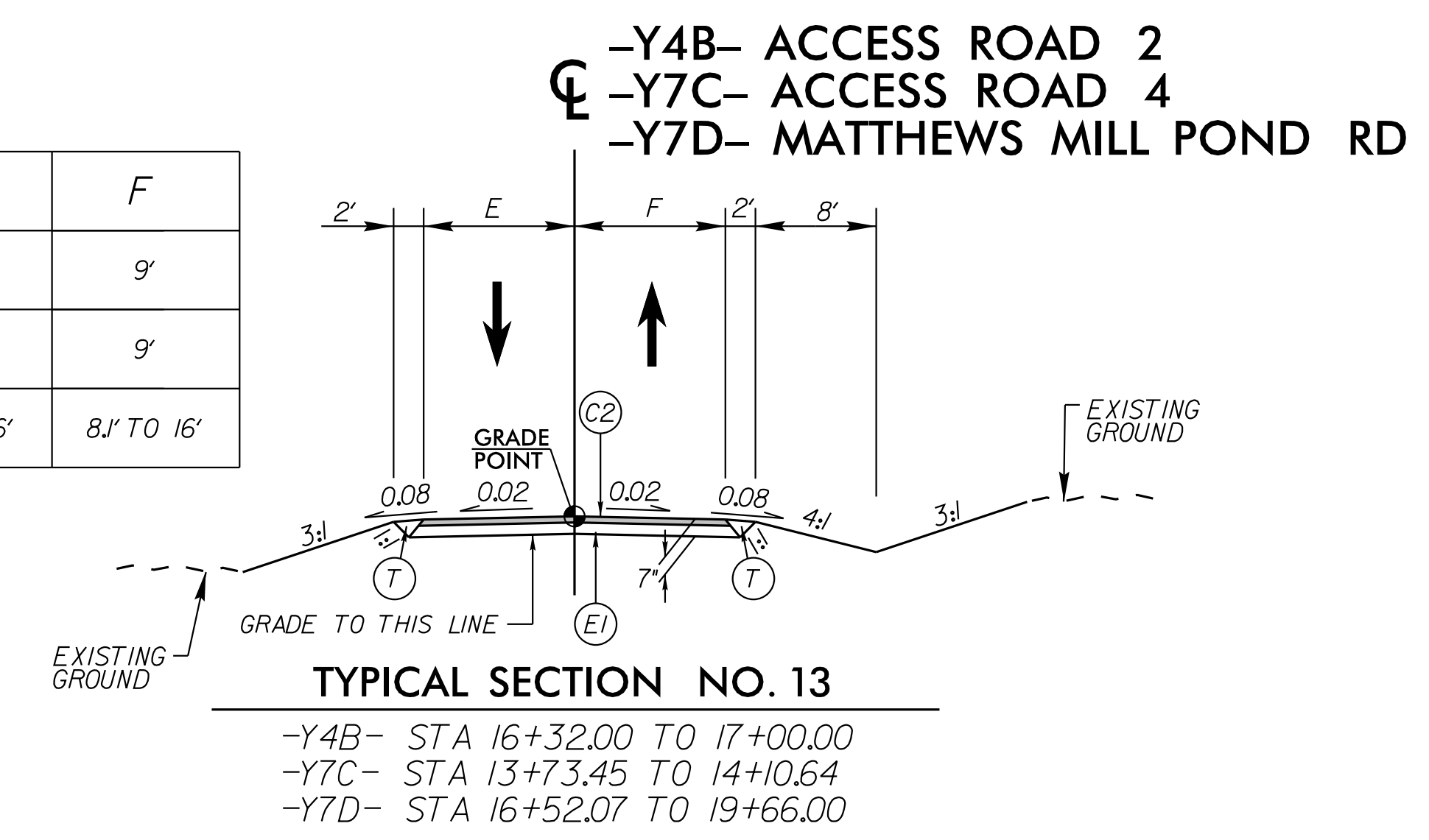
Kimley Horn
 ROADWAY DESIGN ENGINEER
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068
 RIGHT-OF-WAY REV.
 CONST. REV.

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2A-5
ROADWAY DESIGN ENGINEER	PAVEMENT DESIGN ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

	C	D
-Y4B-	9'	9'
-Y6A-	10'	10'



	E	F
-Y4B-	9'	9'
-Y7C-	9'	9'
-Y7D-	8.3' TO 16'	8.1' TO 16'



PAVEMENT SCHEDULE	
C1	PROPOSED APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C4	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
C5	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.
D1	PROPOSED APPROX. 2.5" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I90C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D2	PROPOSED APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I90C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D3	PROPOSED VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I90C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 2.5" OR GREATER THAN 4" IN DEPTH.
E1	PROPOSED APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E2	PROPOSED APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E3	PROPOSED VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.
J1	PROPOSED 6" AGGREGATE BASE COURSE
J2	PROPOSED 8" AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GAL PER SQ. YD.
R1	PROPOSED 1'-6" CONCRETE CURB & GUTTER
R2	PROPOSED 2'-9" CONCRETE CURB & GUTTER
R3	PROPOSED 5" MONOLITHIC CONCRETE ISLAND (KEYED-IN)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V1	MILLING ASPHALT PAVEMENT, 2.5" DEPTH
V2	MILLING ASPHALT PAVEMENT, 3" DEPTH
W1	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W1 FOR RESURFACING)
W2	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W2 FOR RESURFACING)

REVISIONS

8/8/2023

5/14/99

NOTES:
 1. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED
 2. TEMPORARY SHOULDERS AND SLOPES MATCH THE FINAL CONDITIONS.

Kimley Horn
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068
 ROADWAY DESIGN ENGINEER
 PAVEMENT DESIGN ENGINEER

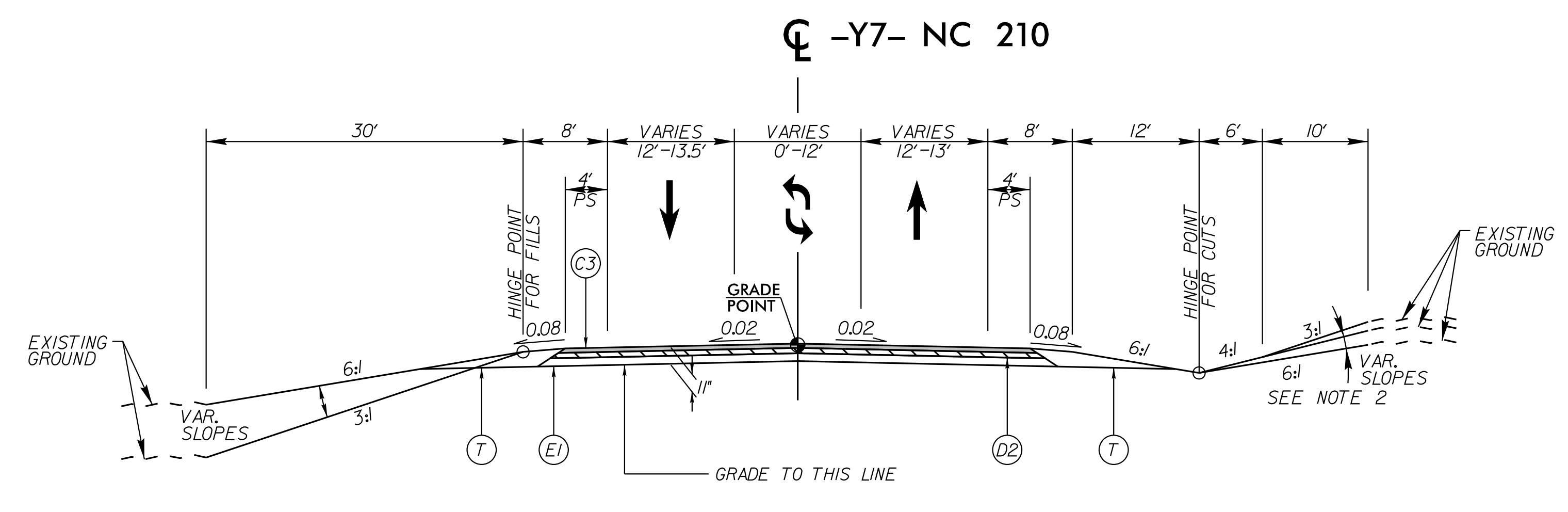
PROJECT REFERENCE NO. R-5705A SHEET NO. 2A-6

RIGHT-OF-WAY REV. CONST. REV.

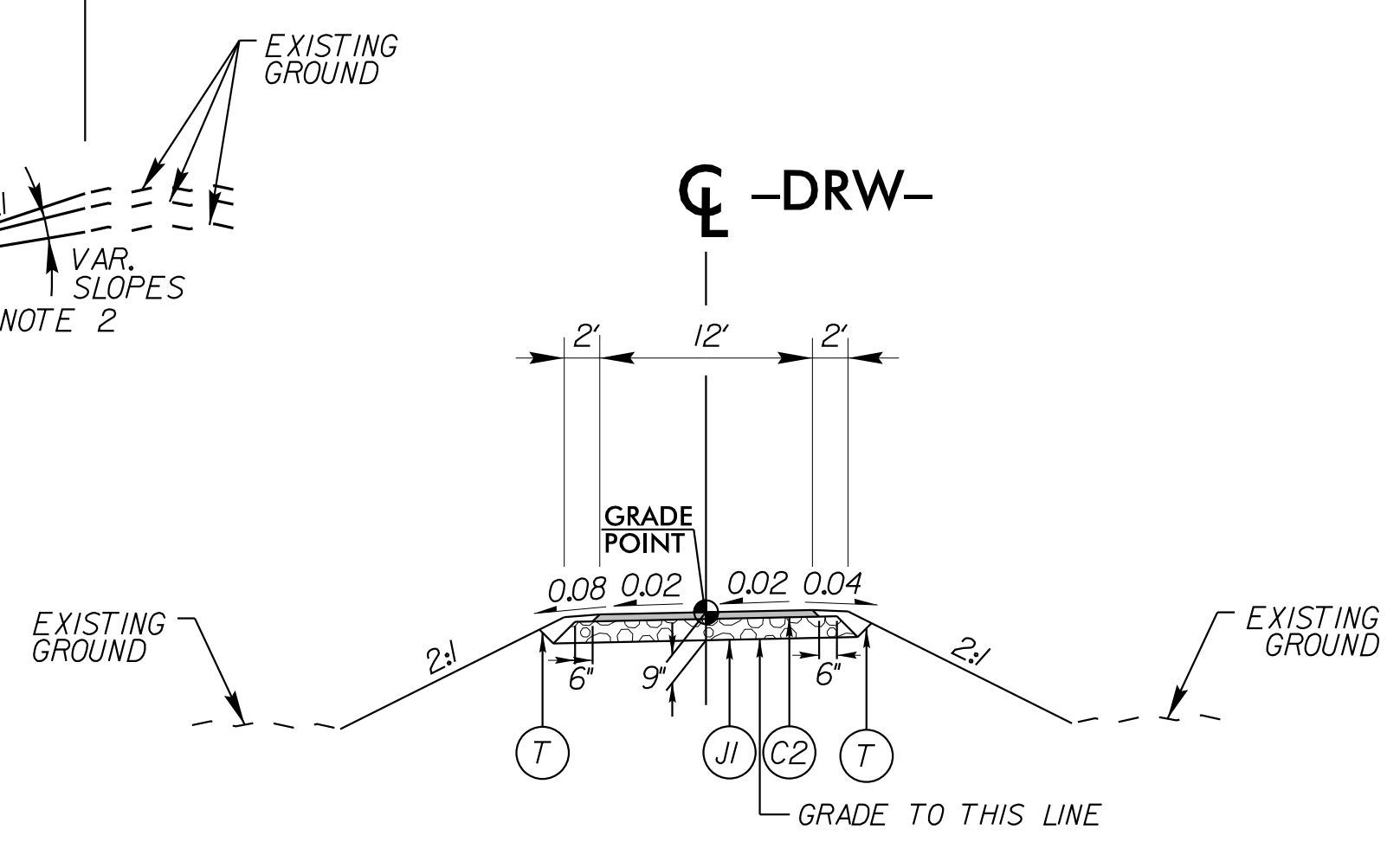
PROFESSIONAL SEAL
 NORTH CAROLINA
 PROFESSIONAL ENGINEER
 024436
 8/8/2023
 CLAY E. HORN

PROFESSIONAL SEAL
 NORTH CAROLINA
 PROFESSIONAL ENGINEER
 022896
 10/10/2023
 CLAY S. MORRISON

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

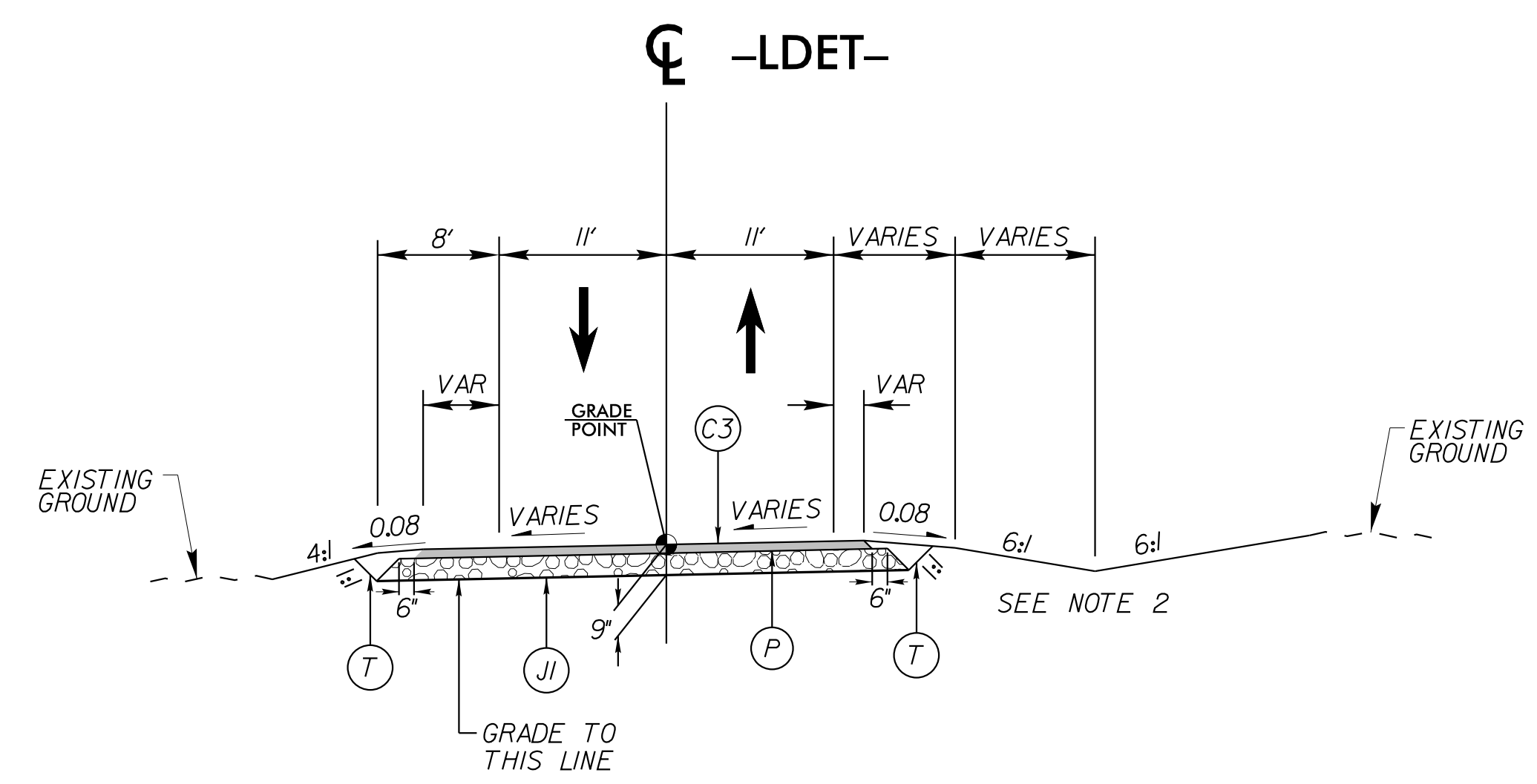


TYPICAL SECTION NO. 17
 -Y7- STA 26+25.00 TO 32+60.28
 -Y7- STA 33+40.63 TO 34+25.00

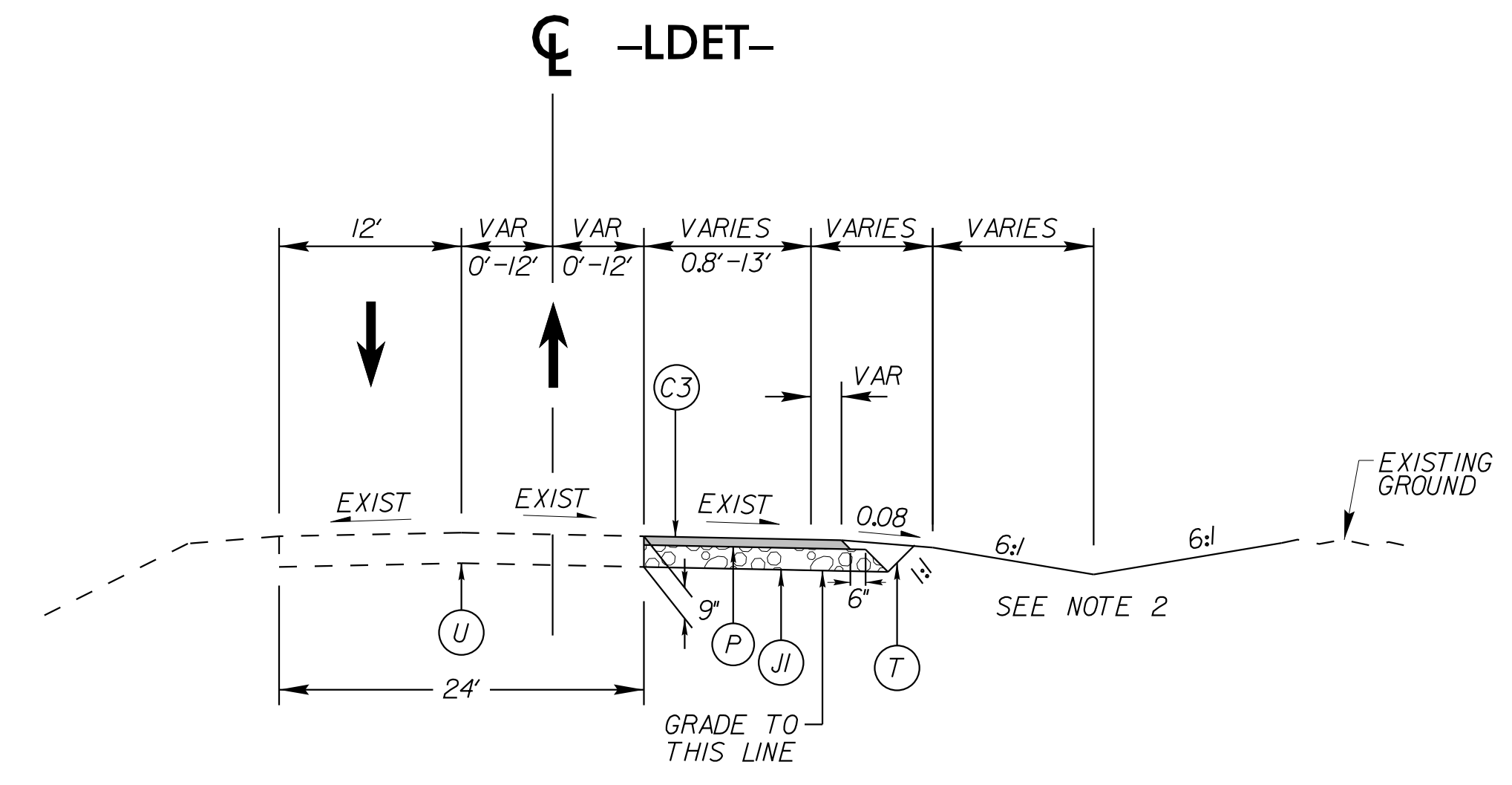


TYPICAL SECTION NO. 18

- DRW2- STA 10+43.00 TO 12+00.00
- DRW3- STA 10+43.00 TO 12+38.00
- DRW4- STA 10+43.00 TO 11+26.90
- DRW5- STA 10+43.00 TO 11+07.00
- DRW7- STA 10+43.00 TO 11+45.00
- DRW8- STA 10+28.00 TO 10+75.00
- DRW10- STA 10+22.00 TO 10+85.00



TYPICAL SECTION NO. 19
 TEMPORARY WIDENING
 -LDET- STA 94+50.58 TO 97+73.77



TYPICAL SECTION NO. 20
 TEMPORARY WIDENING
 -LDET- STA 97+73.77 TO 100+14.37

PAVEMENT SCHEDULE

C1	PROPOSED APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C4	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
C5	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S95C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.
D1	PROPOSED APPROX. 2.5" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I190C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D2	PROPOSED APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I190C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D3	PROPOSED VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I190C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 2.5" OR GREATER THAN 4" IN DEPTH.
E1	PROPOSED APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROPOSED APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E3	PROPOSED VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B250C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.
J1	PROPOSED 6" AGGREGATE BASE COURSE
J2	PROPOSED 8" AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GAL/PER SQ. YD.
R1	PROPOSED 1'-6" CONCRETE CURB & GUTTER
R2	PROPOSED 2'-9" CONCRETE CURB & GUTTER
R3	PROPOSED 5" MONOLITHIC CONCRETE ISLAND (KEYED-IN)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V1	MILLING ASPHALT PAVEMENT, 2.5" DEPTH
V2	MILLING ASPHALT PAVEMENT, 3" DEPTH
W1	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W1 FOR RESURFACING)
W2	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W2 FOR RESURFACING)

REVISIONS

8/8/2023

5/14/99

NOTES:
 1. PAVEMENT EDGE SLOPES 1:1 UNLESS OTHERWISE INDICATED
 2. TEMPORARY SHOULDERS AND SLOPES MATCH THE FINAL CONDITIONS.
 3. MATCH GRADE AND SLOPE OF PROPOSED -Y5-/L-/Y5A- CONSTRUCTION

Kimley Horn
 ROADWAY DESIGN ENGINEER
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

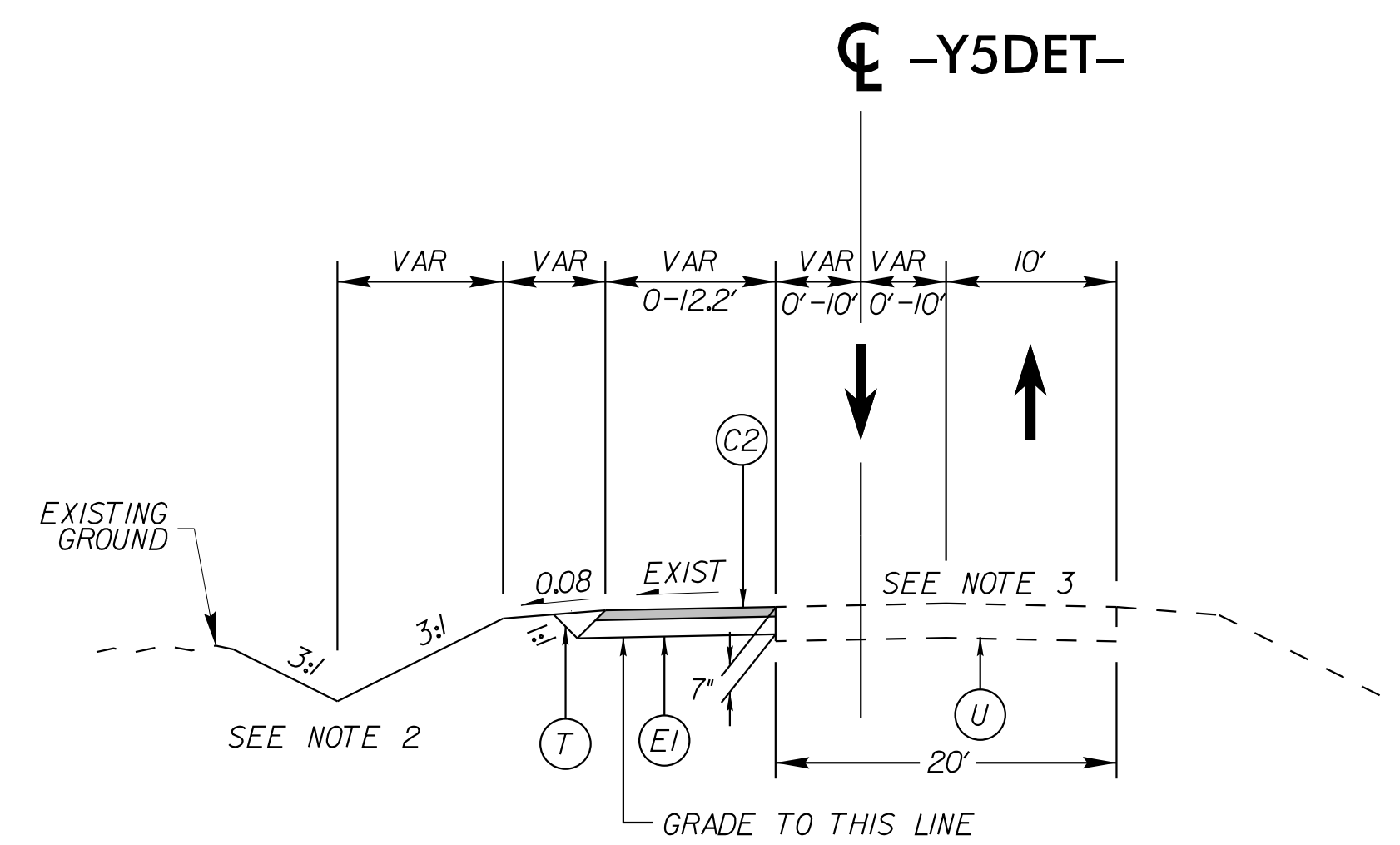
PROJECT REFERENCE NO. R-5705A
 SHEET NO. 2A-7

RIGHT-OF-WAY REV.
 CONST. REV.

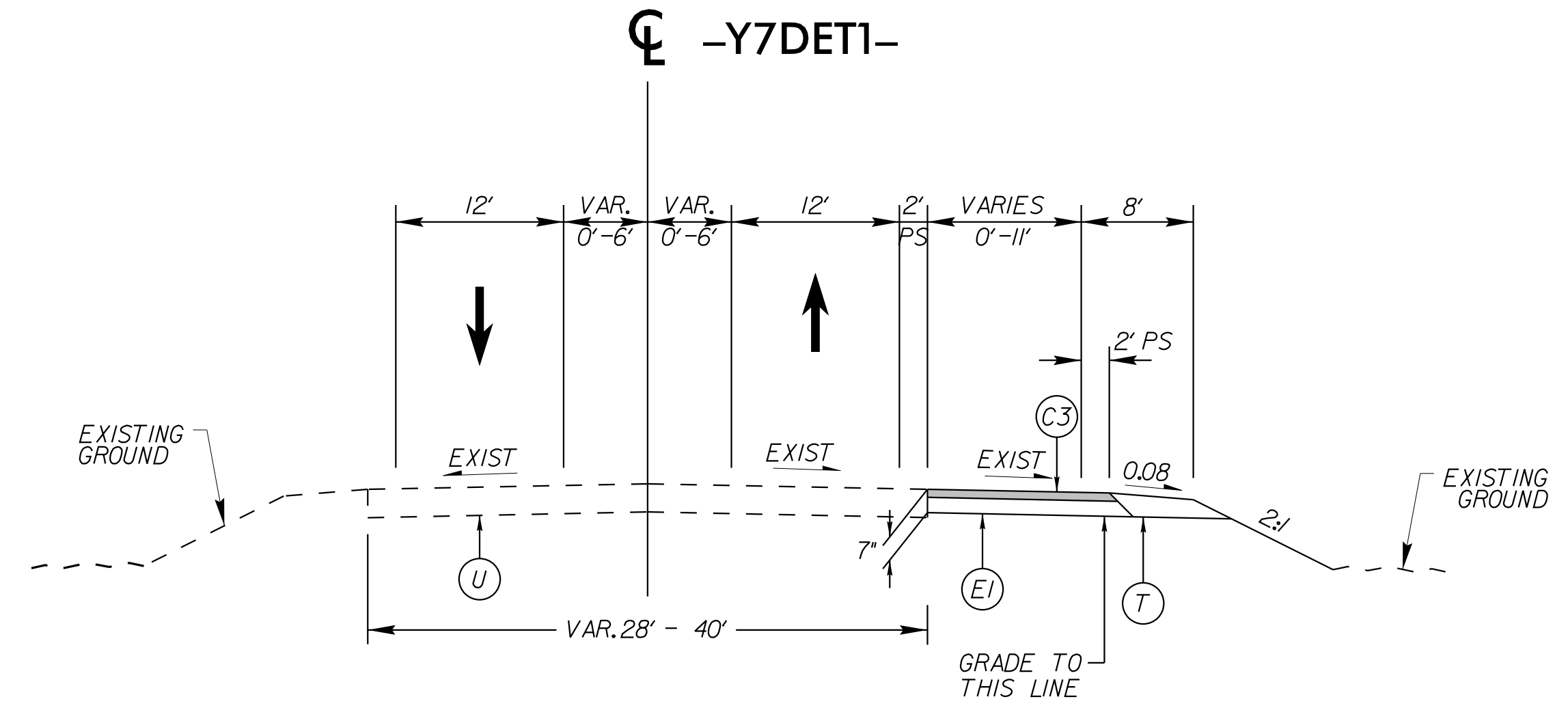
PROFESSIONAL SEAL
 J. F. W. MOORE
 024406
 8/8/2023
 ENGINEER

PROFESSIONAL SEAL
 CLARK S. MORRISON
 022896
 8/10/2023
 ENGINEER

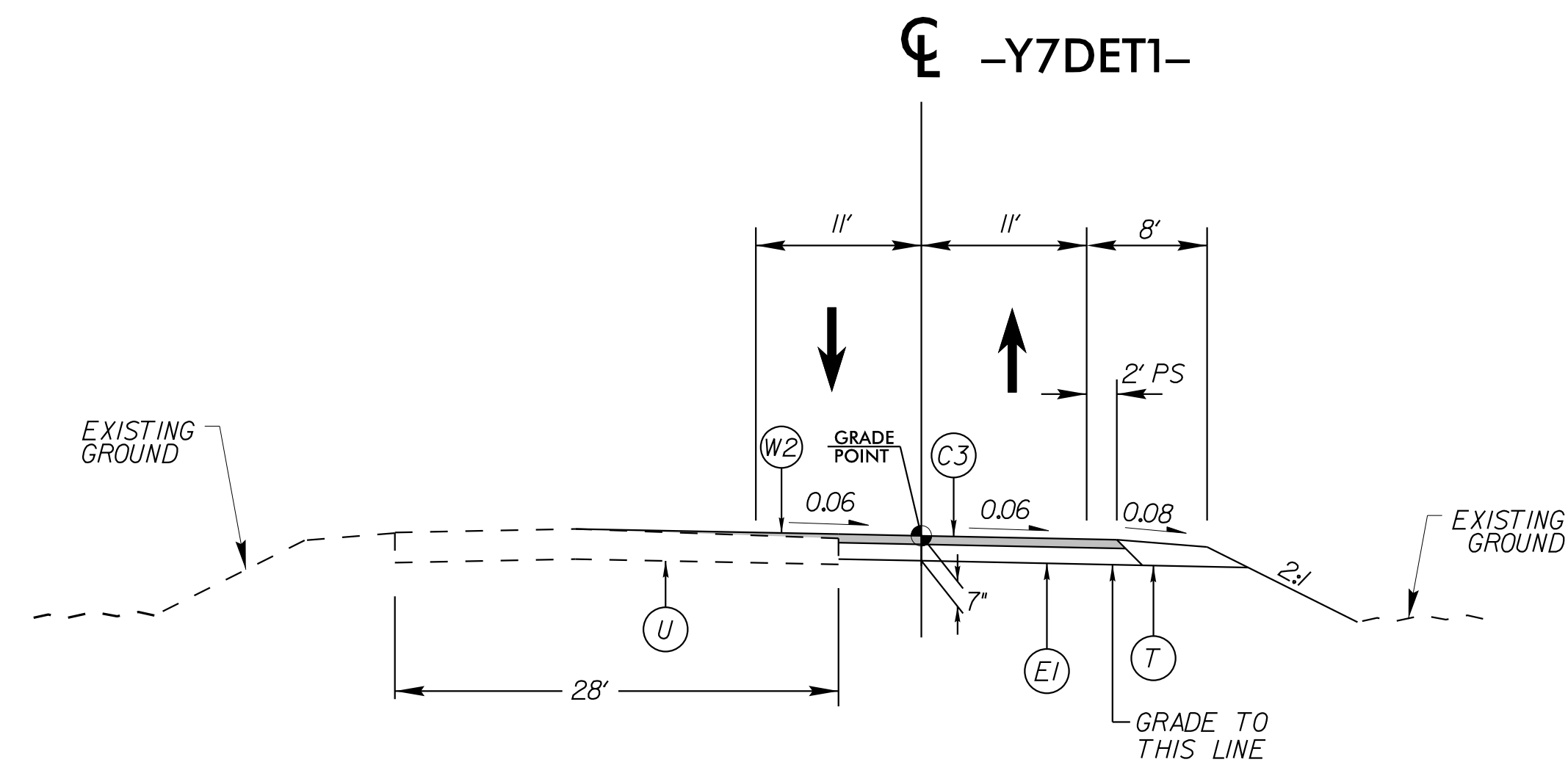
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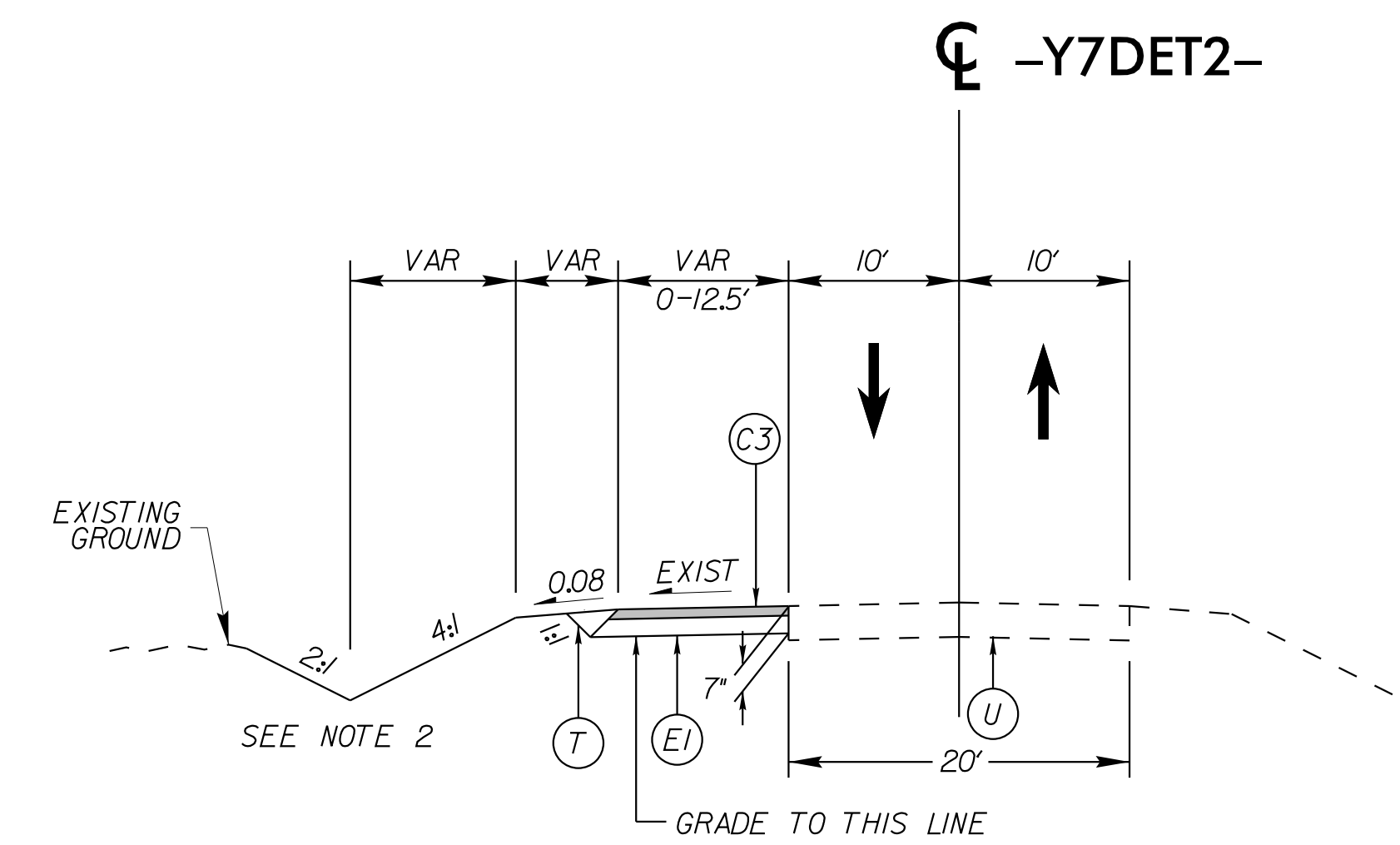
TYPICAL SECTION NO. 21
 TEMPORARY WIDENING
 -Y5DET- STA 11+69.18 TO 12+94.56 (LT)
 -Y5DET- STA 14+10.49 TO 15+88.94 (RT)



TYPICAL SECTION NO. 22
 TEMPORARY WIDENING
 -Y7DET1- STA 12+46.51 TO 15+61.19
 -Y7DET1- STA 23+37.98 TO 27+86.59



TYPICAL SECTION NO. 23
 TEMPORARY WIDENING
 -Y7DET1- STA 15+61.19 TO 23+37.98



TYPICAL SECTION NO. 24
 TEMPORARY WIDENING
 -Y7DET2- STA 22+52.23 TO 24+15.92 (LT)

PAVEMENT SCHEDULE

C1	PROPOSED APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROPOSED APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C4	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
C5	PROPOSED VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.
D1	PROPOSED APPROX. 2.5" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D2	PROPOSED APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D3	PROPOSED VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I9.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 2.5" OR GREATER THAN 4" IN DEPTH.
E1	PROPOSED APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROPOSED APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YD.
E3	PROPOSED VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.
J1	PROPOSED 6" AGGREGATE BASE COURSE
J2	PROPOSED 8" AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GAL/PER SQ.YD.
R1	PROPOSED 1'-6" CONCRETE CURB & GUTTER
R2	PROPOSED 2'-9" CONCRETE CURB & GUTTER
R3	PROPOSED 5" MONOLITHIC CONCRETE ISLAND (KEYED-IN)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
V1	MILLING ASPHALT PAVEMENT, 2.5" DEPTH
V2	MILLING ASPHALT PAVEMENT, 3" DEPTH
W1	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W1 FOR RESURFACING)
W2	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL W2 FOR RESURFACING)

REVISIONS

8/8/2023

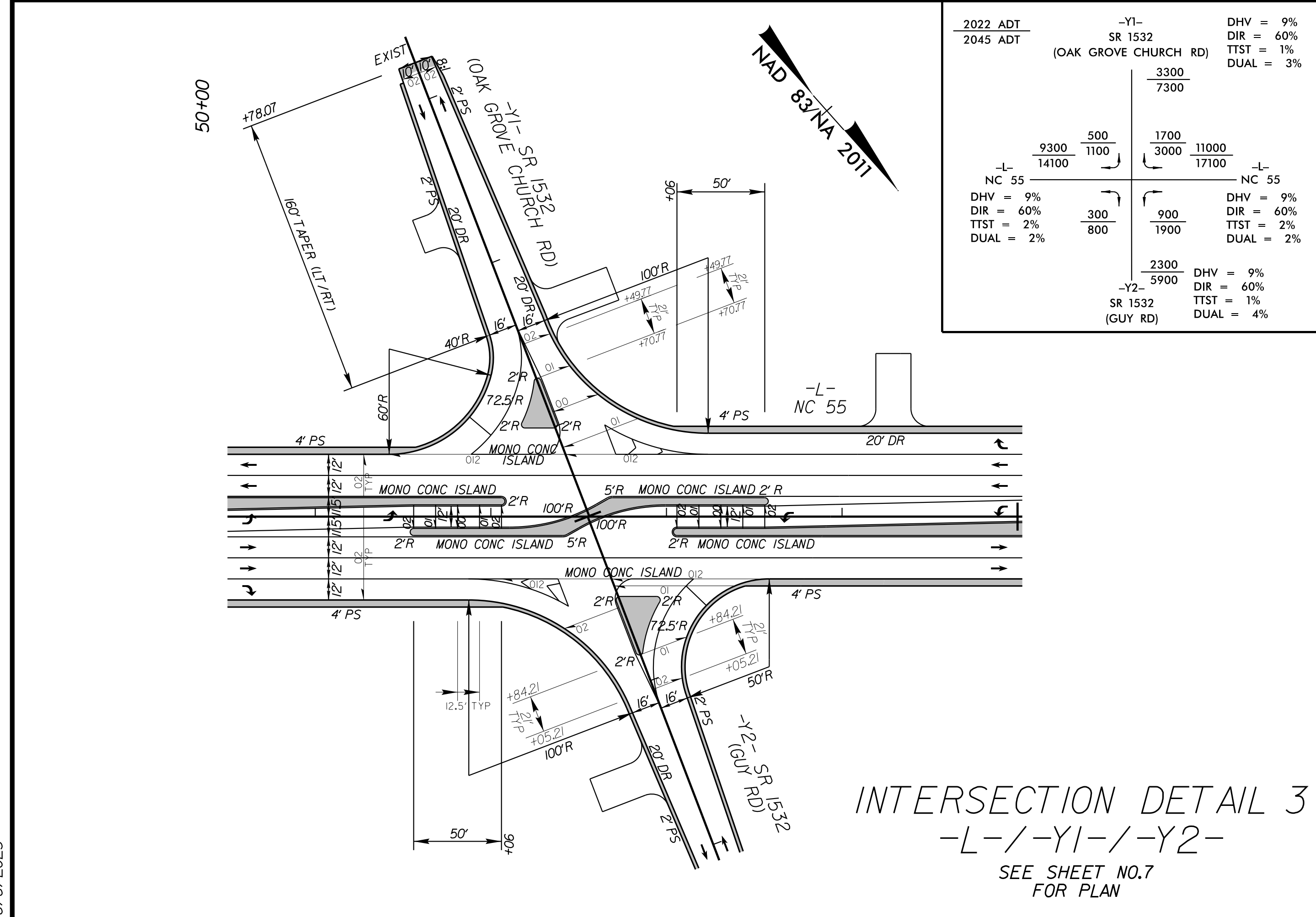
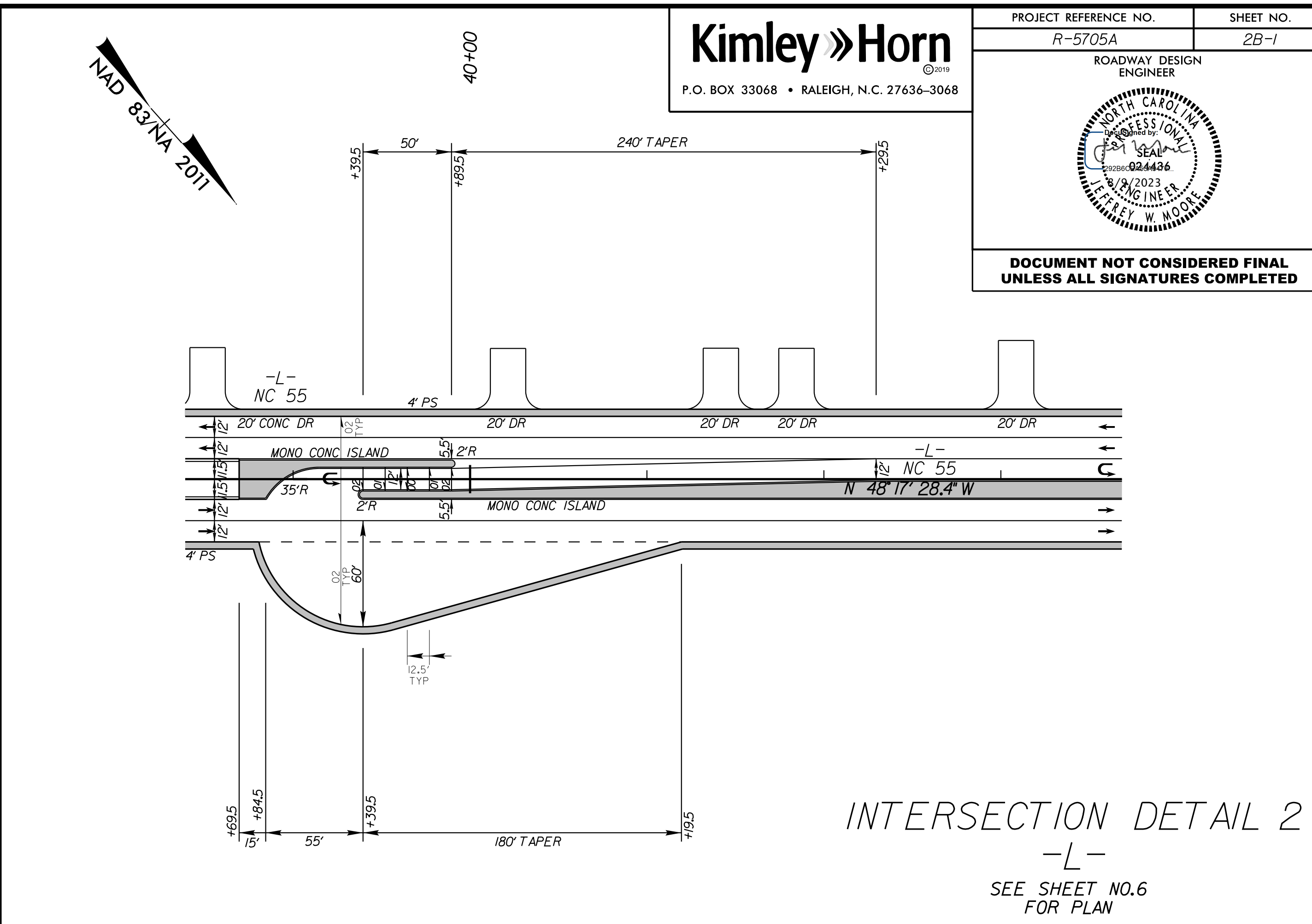
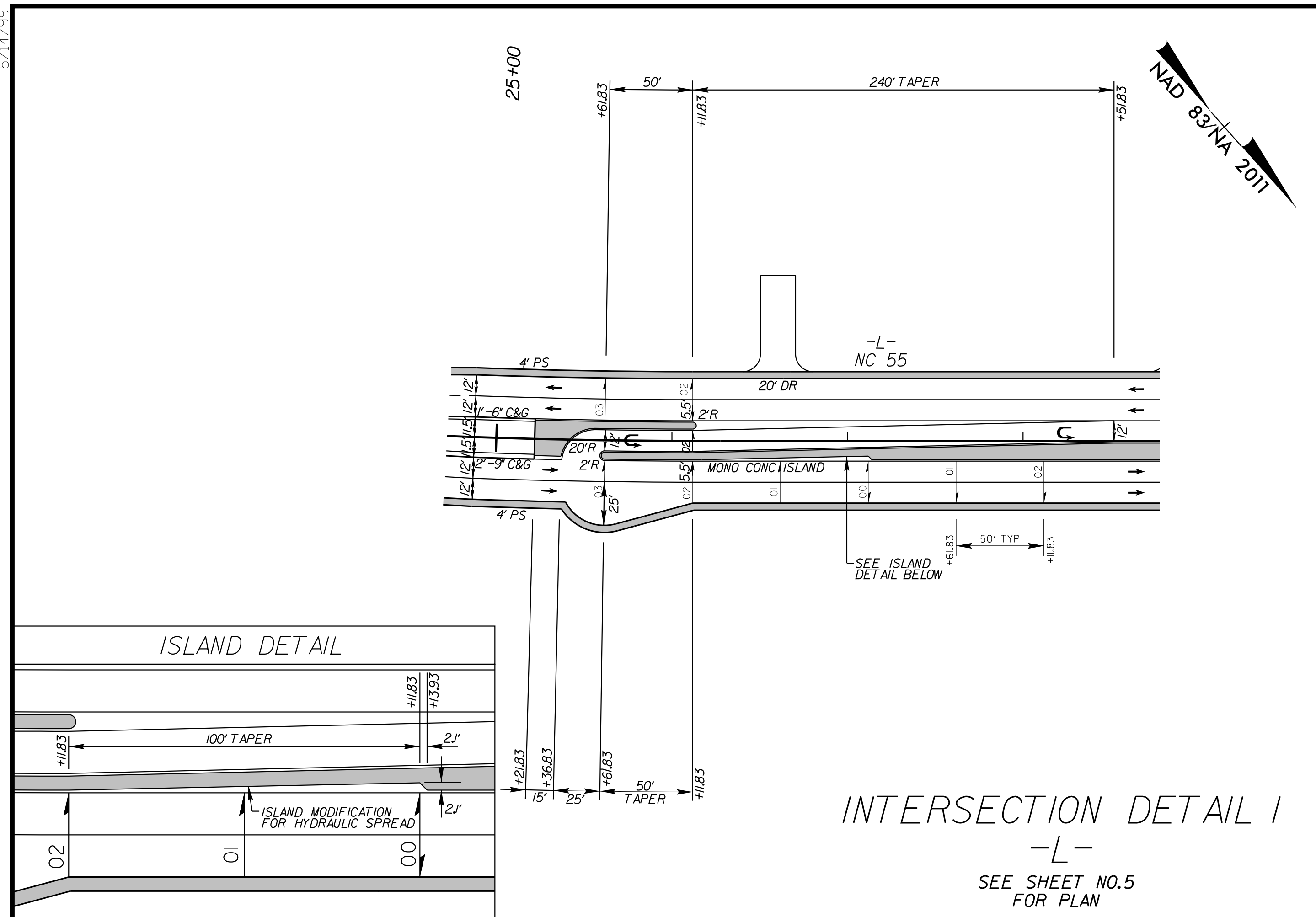
5/14/1999

8/8/2023

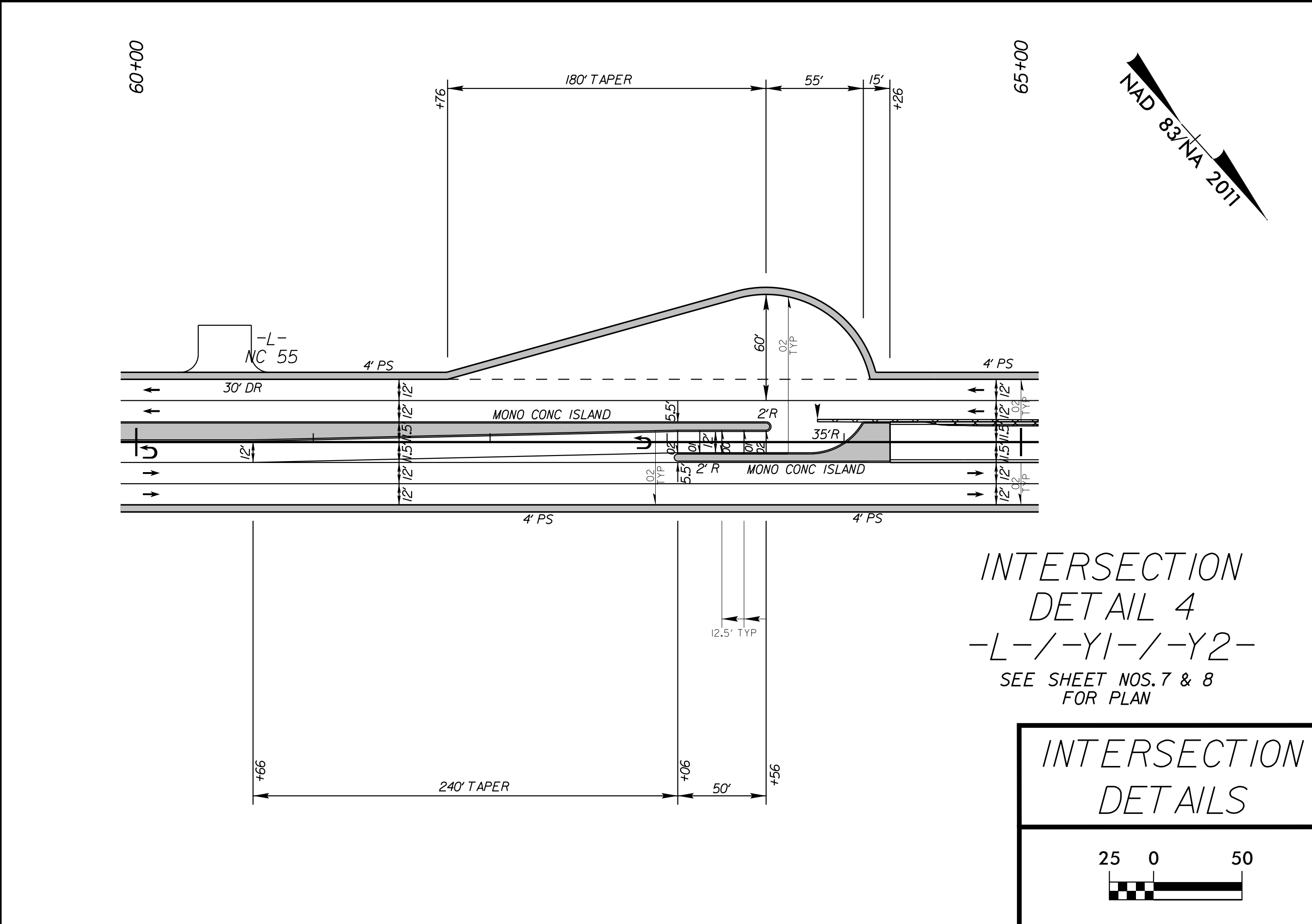
Kimley Horn
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO. R-5705A SHEET NO. 2B-1
 ROADWAY DESIGN ENGINEER

DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

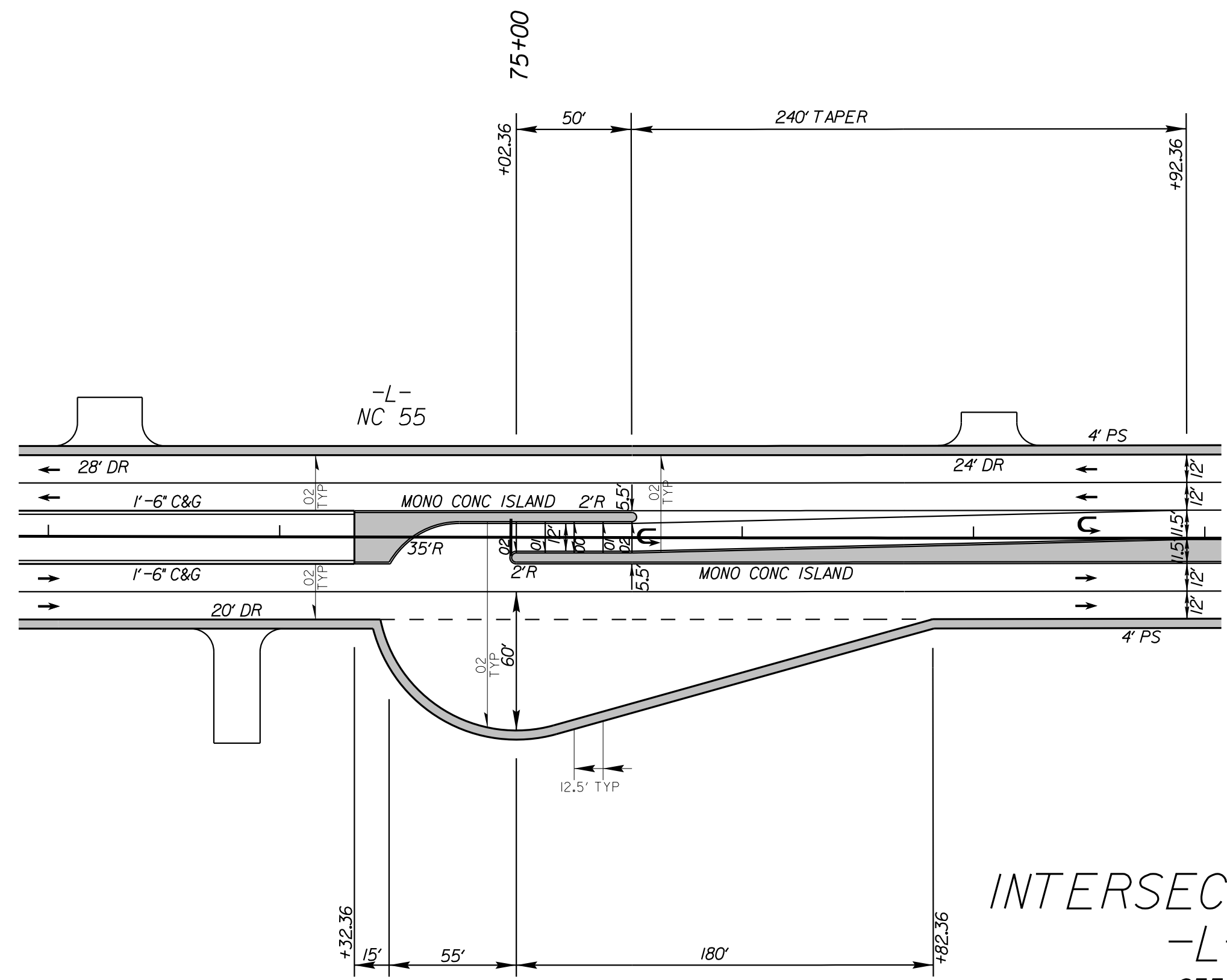


2022 ADT	-Y1-	DHV = 9%
2045 ADT	SR 1532	DIR = 60%
	(OAK GROVE CHURCH RD)	TTST = 1%
		DUAL = 3%
	3300	
	7300	
	9300	
	1100	
	14100	
	1700	
	3000	
	11000	
	17100	
	2300	
	5900	
	-Y2-	DHV = 9%
	SR 1532	DIR = 60%
	(GUY RD)	TTST = 1%
		DUAL = 4%

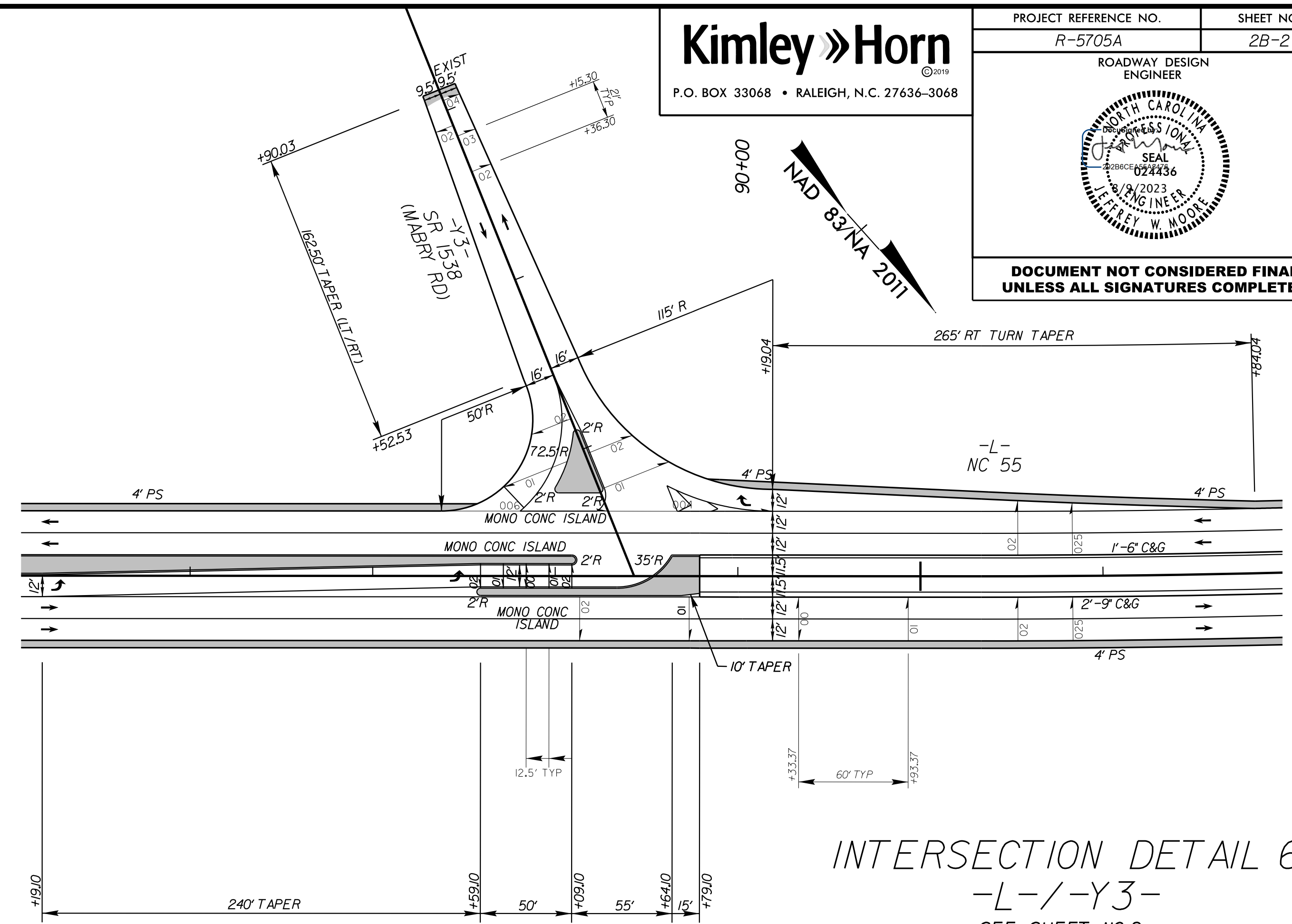


5/14/19

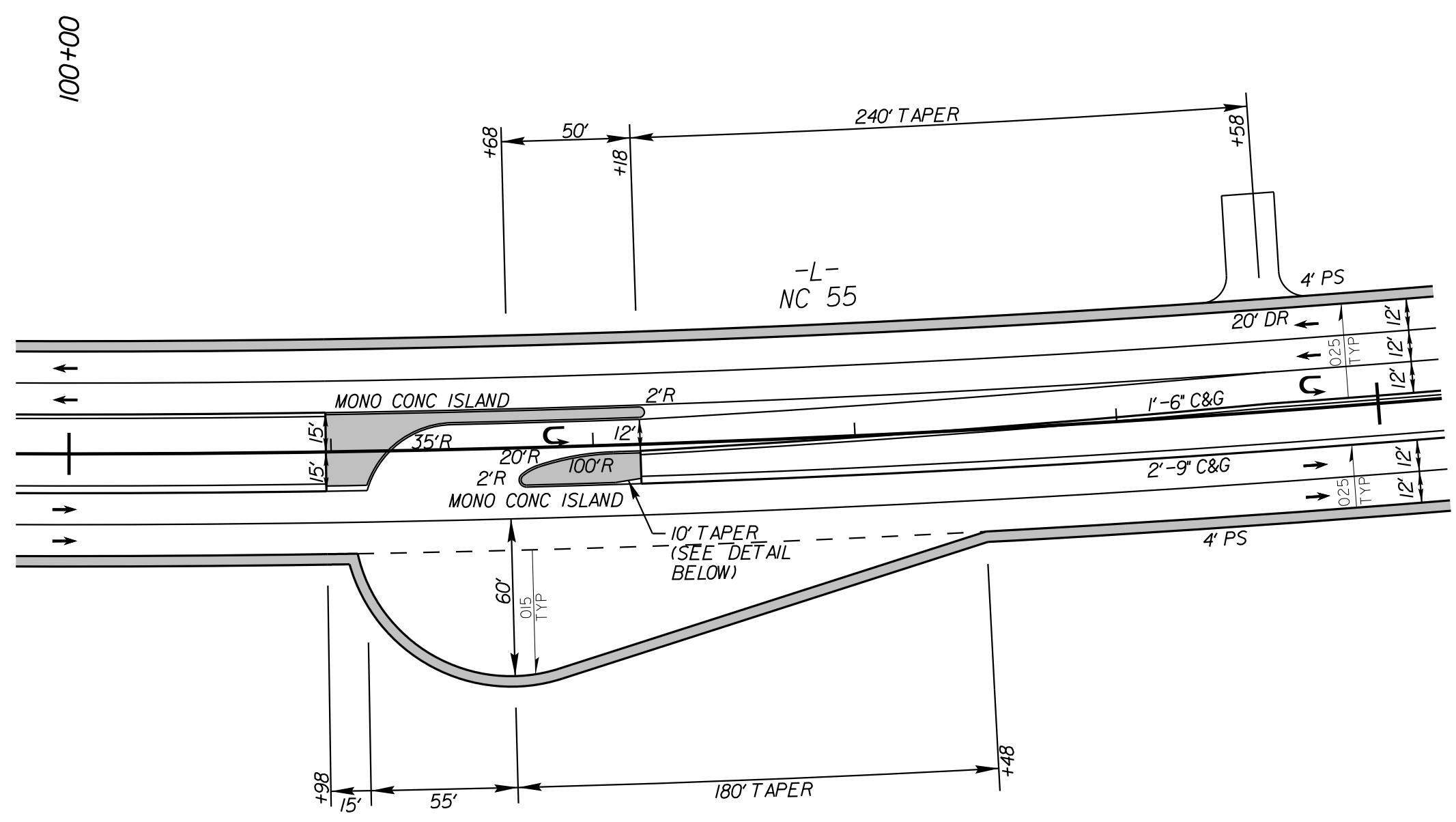
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-2
ROADWAY DESIGN ENGINEER	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



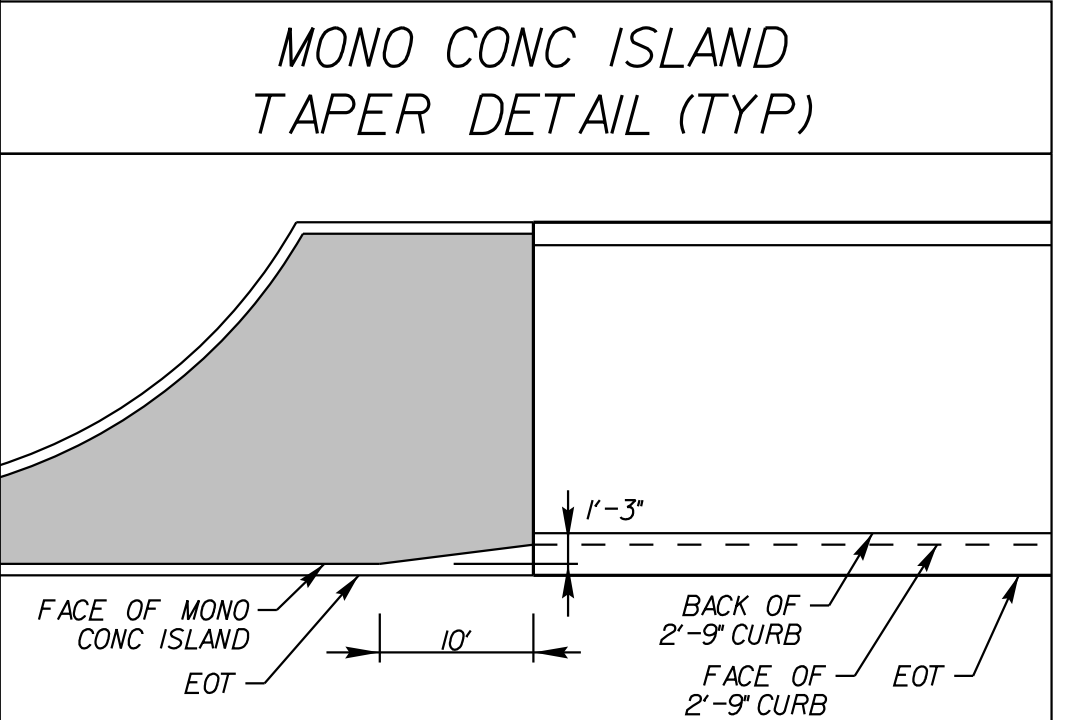
INTERSECTION DETAIL 5
 -L-/-Y3-
 SEE SHEET NO.8 FOR PLAN



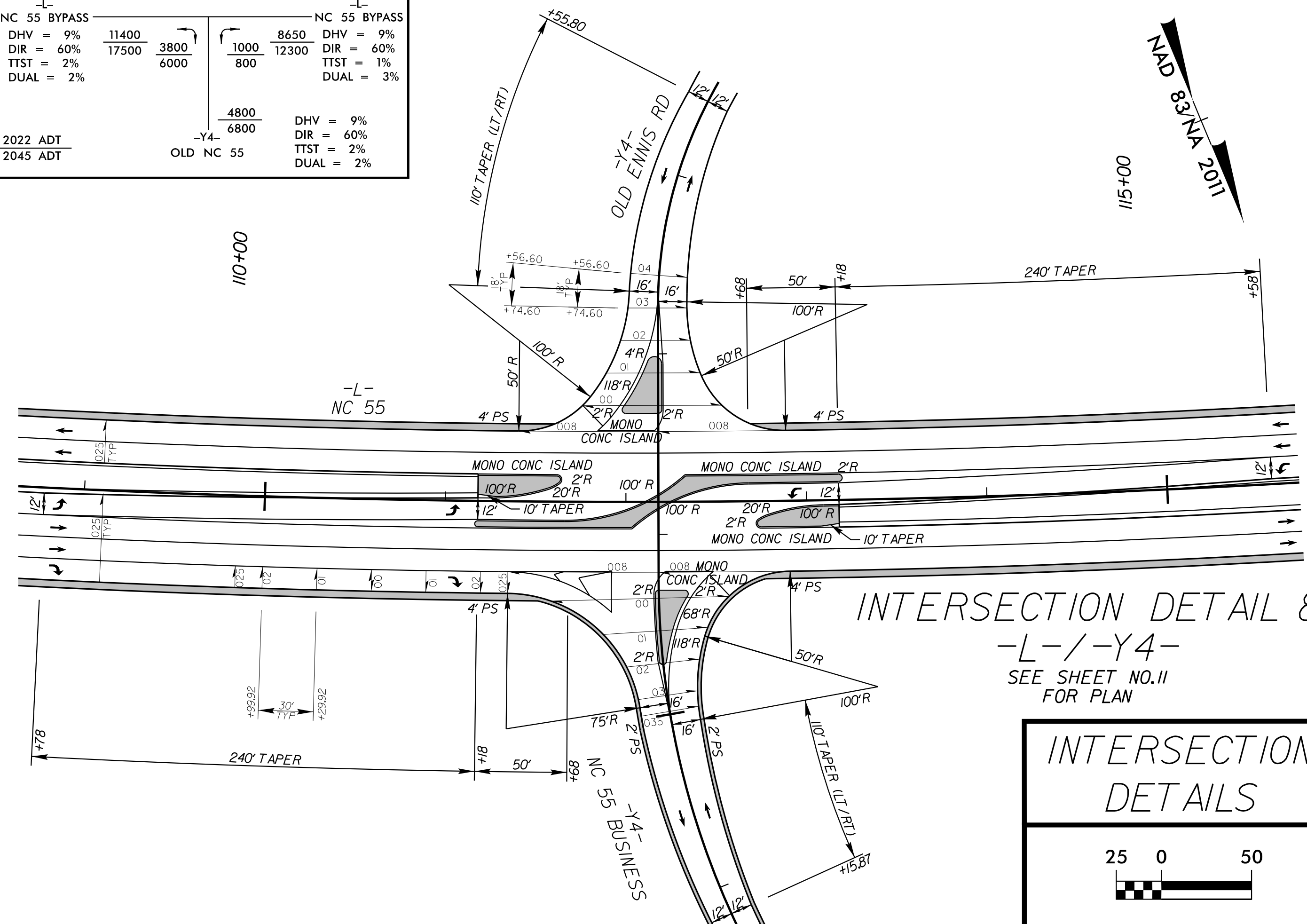
INTERSECTION DETAIL 6
 -L-/-Y3-
 SEE SHEET NO.9 FOR PLAN



INTERSECTION DETAIL 7
 -L-/-Y4-
 SEE SHEET NO.10 FOR PLAN



NC 55 BYPASS	NC 55 BYPASS	NC 55 BYPASS	NC 55 BYPASS
DHV = 9%	11400	1000	8650
DIR = 60%	17500	800	12300
TTST = 2%	3800		
DUAL = 2%	6000		
		4800	
		6800	
			DHV = 9%
			DIR = 60%
			TTST = 1%
			DUAL = 3%
2022 ADT			
2045 ADT			



INTERSECTION DETAIL 8
 -L-/-Y4-
 SEE SHEET NO.11 FOR PLAN

INTERSECTION DETAILS

8/8/2023

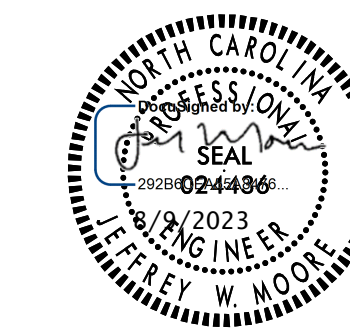
5/14/1999

Kimley Horn

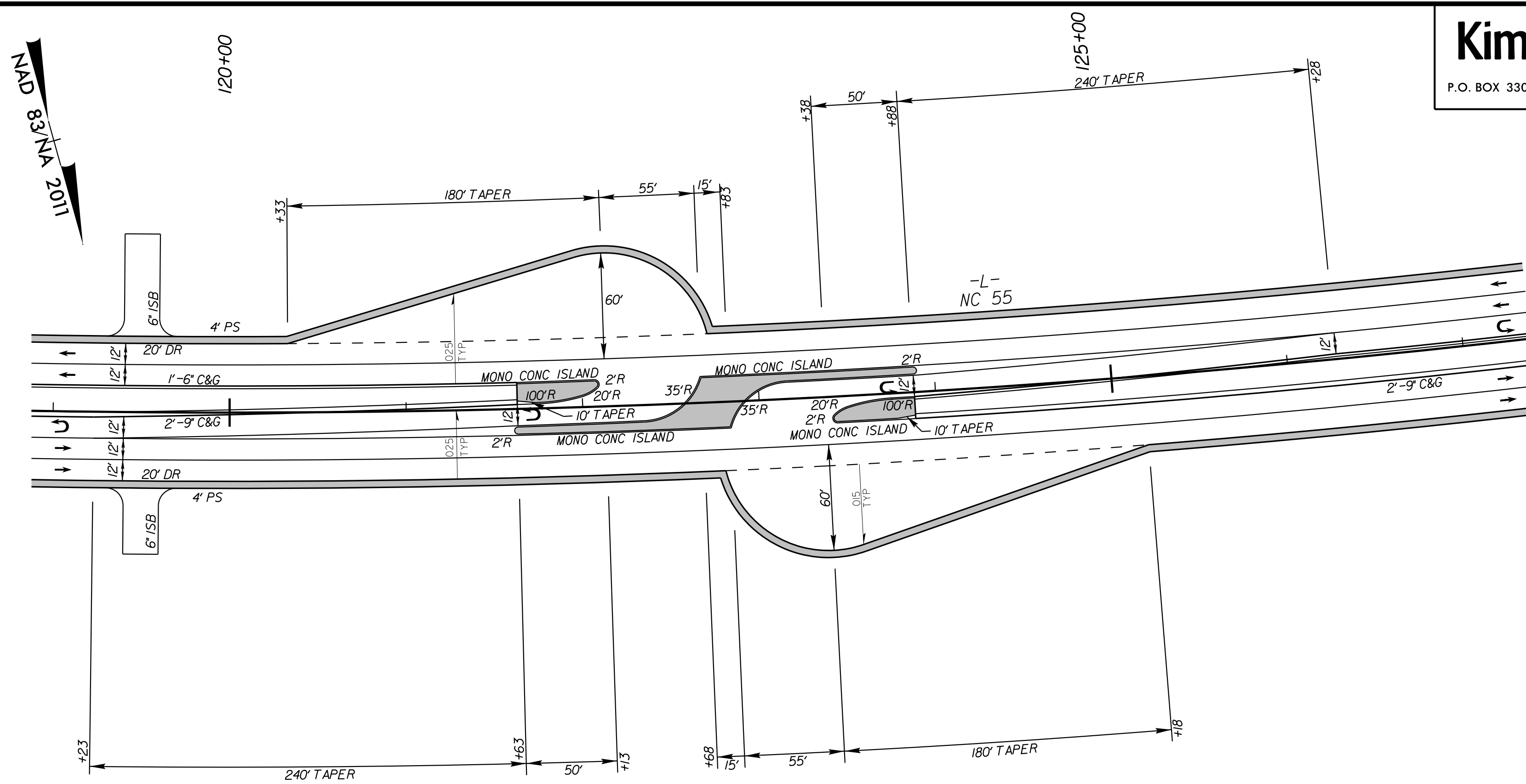
P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO. R-5705A SHEET NO. 2B-3

ROADWAY DESIGN ENGINEER

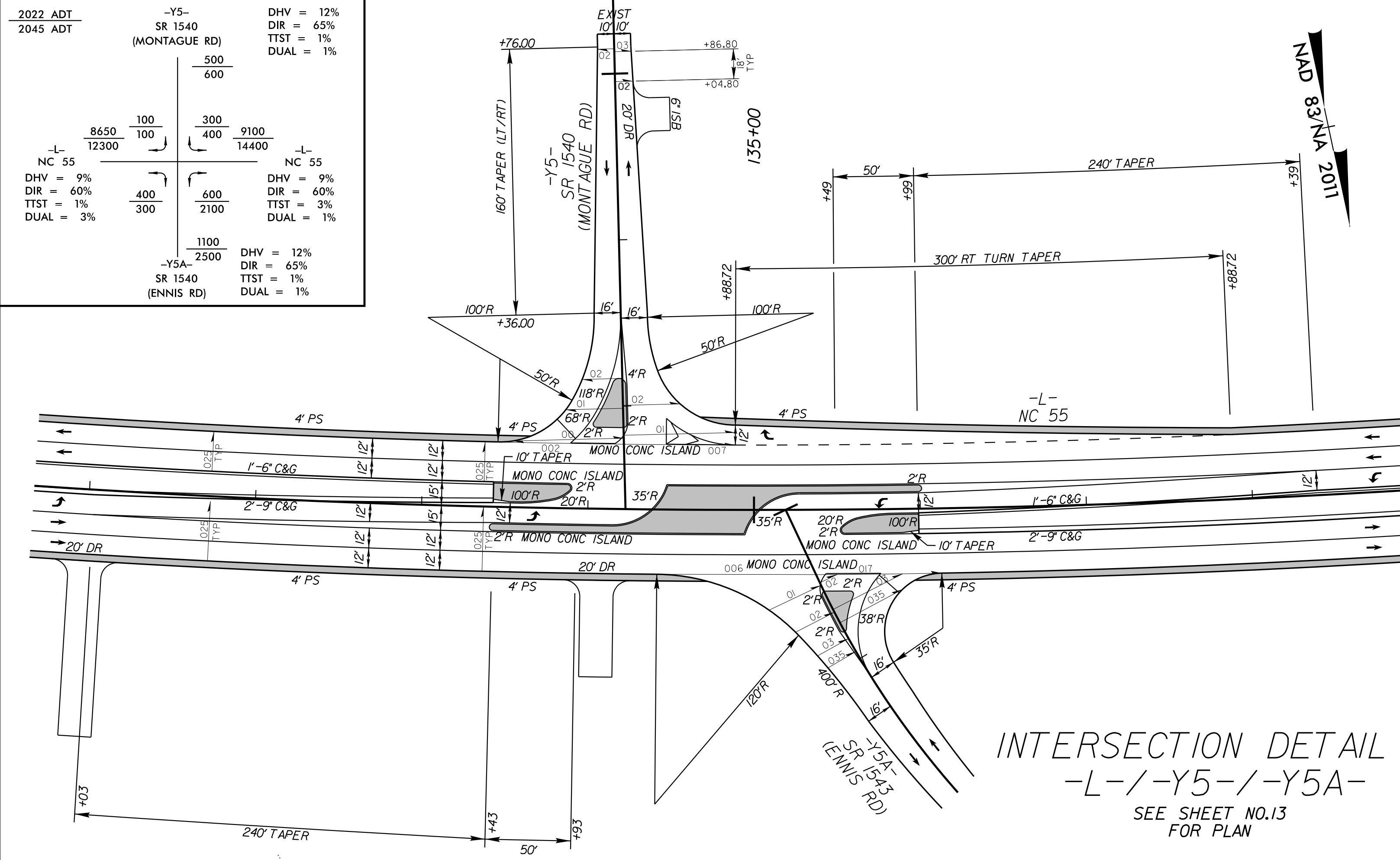


DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

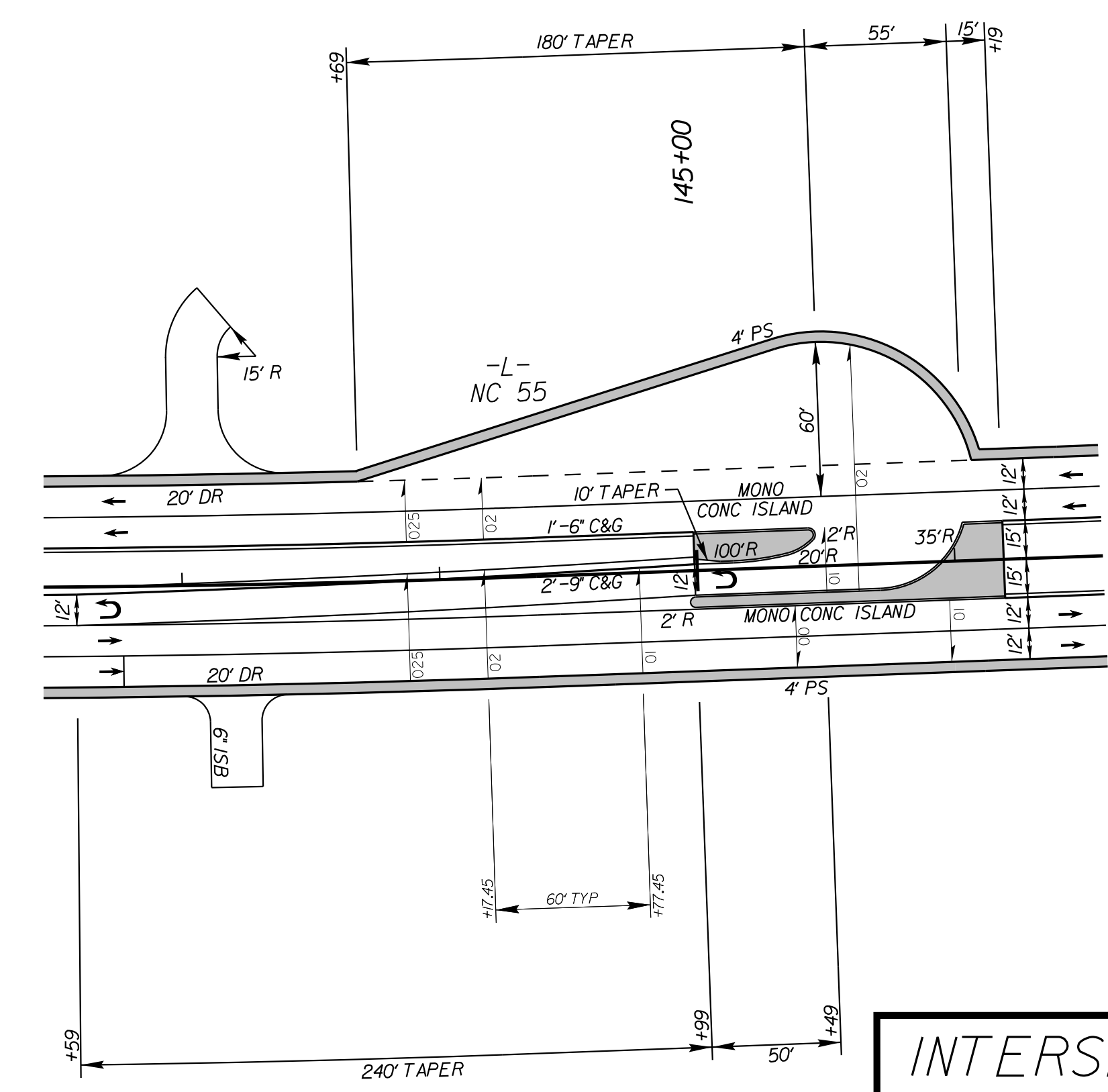


INTERSECTION DETAIL 9
-L-/-Y4-/-Y5-
SEE SHEET NO.12 FOR PLAN

2022 ADT	-Y5-	DHV = 12%
2045 ADT	SR 1540	DIR = 65%
	(MONTAGUE RD)	TTST = 1%
		DUAL = 1%
	500	
	600	
	100	300
	100	400
	8650	9100
	12300	14400
	400	2100
	300	
	1100	
	2500	
	(ENNIS RD)	DHV = 12%
		DIR = 65%
		TTST = 1%
		DUAL = 1%



INTERSECTION DETAIL 10
-L-/-Y5-/-Y5A-
SEE SHEET NO.13 FOR PLAN

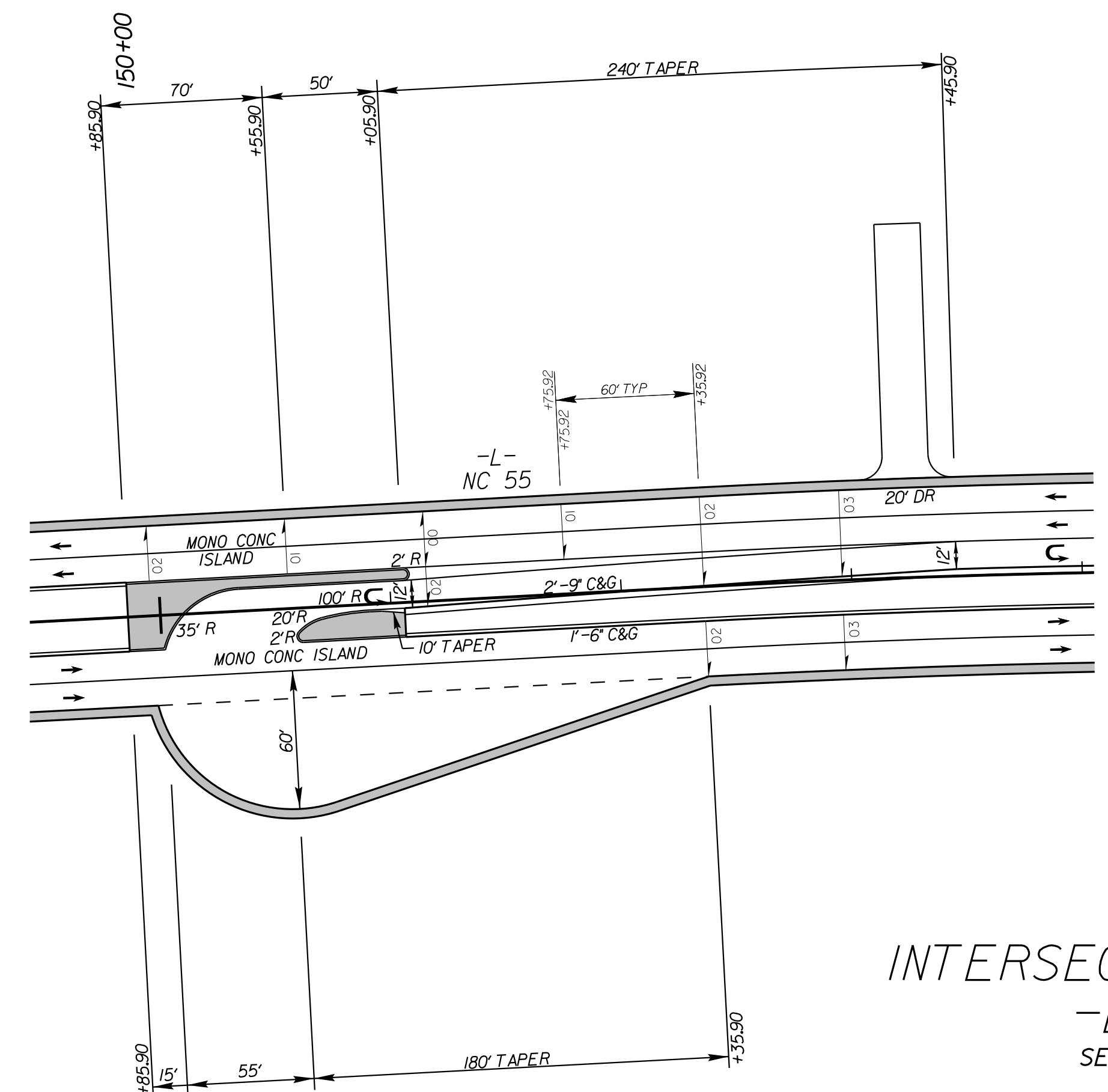


INTERSECTION DETAIL 11
-L-/-Y5-
SEE SHEET NO.13 FOR PLAN

INTERSECTION DETAILS

8/8/2023

5/14/99

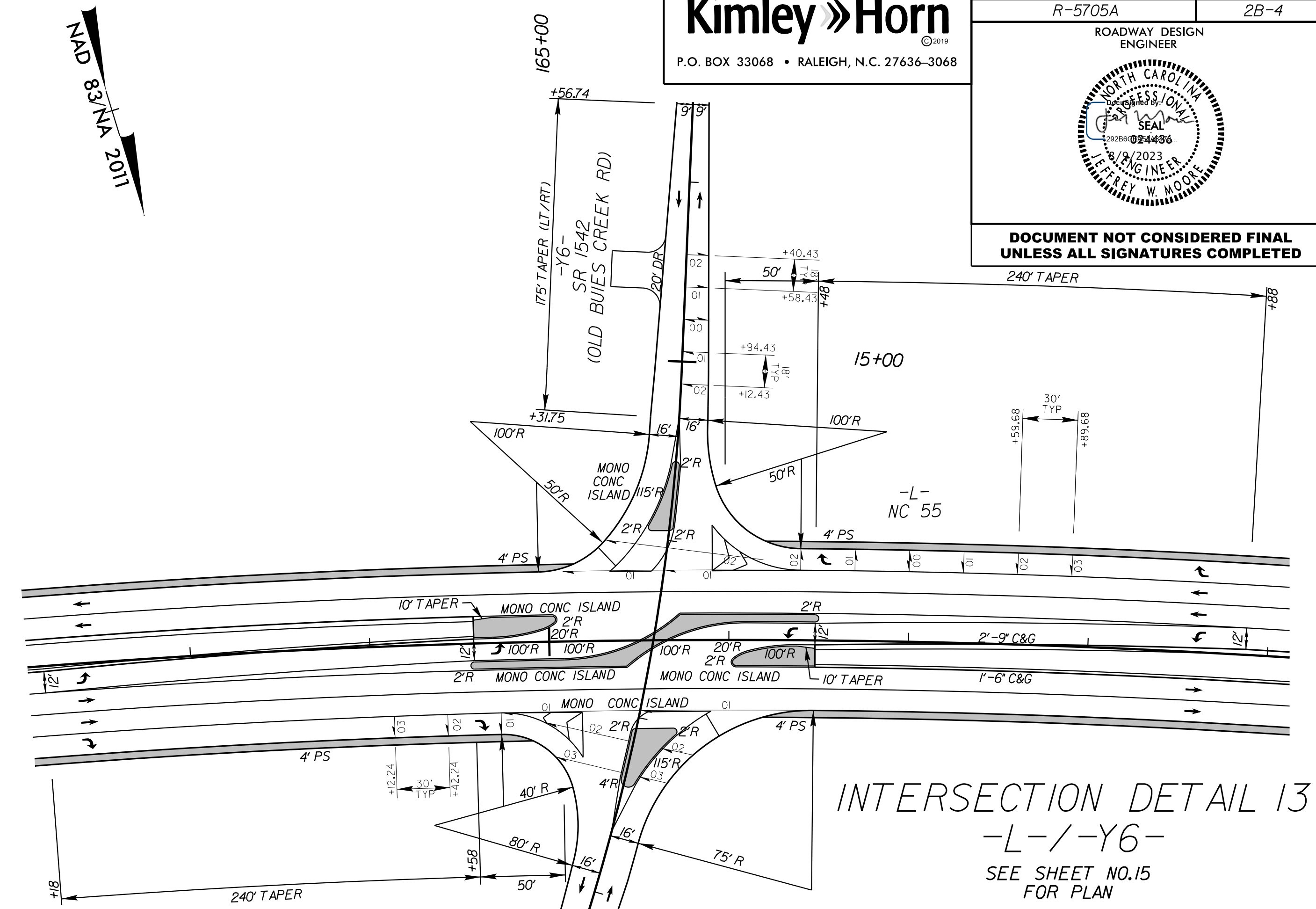


INTERSECTION DETAIL 12
-L-/-Y6-
SEE SHEET NO.14
FOR PLAN

NAD 83/NA 2011

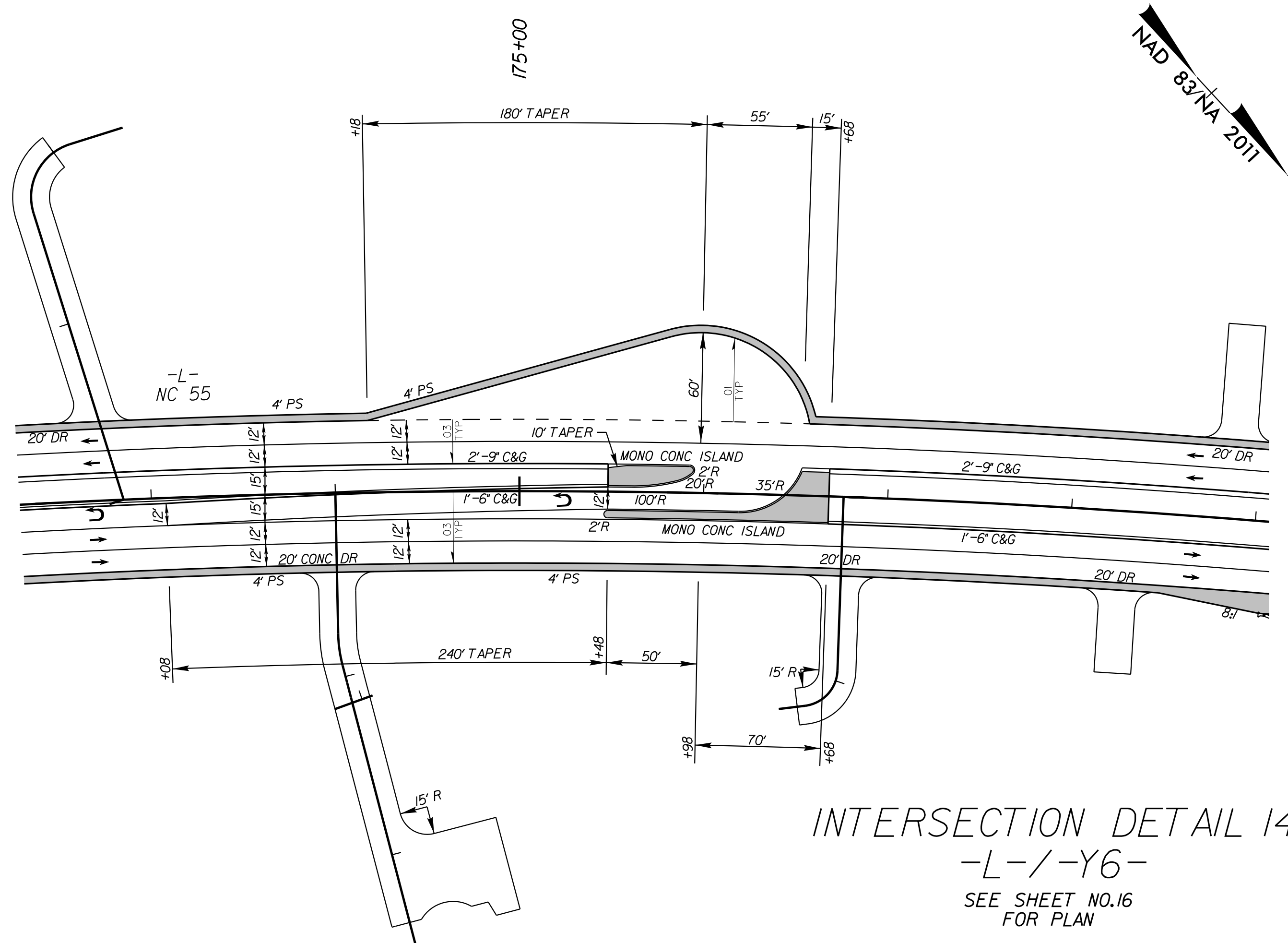
Kimley Horn
ROADWAY DESIGN ENGINEER
P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-4
NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 20286-024436 3/2023 KIMLEY HORN ENGINEERS W. MOORE	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



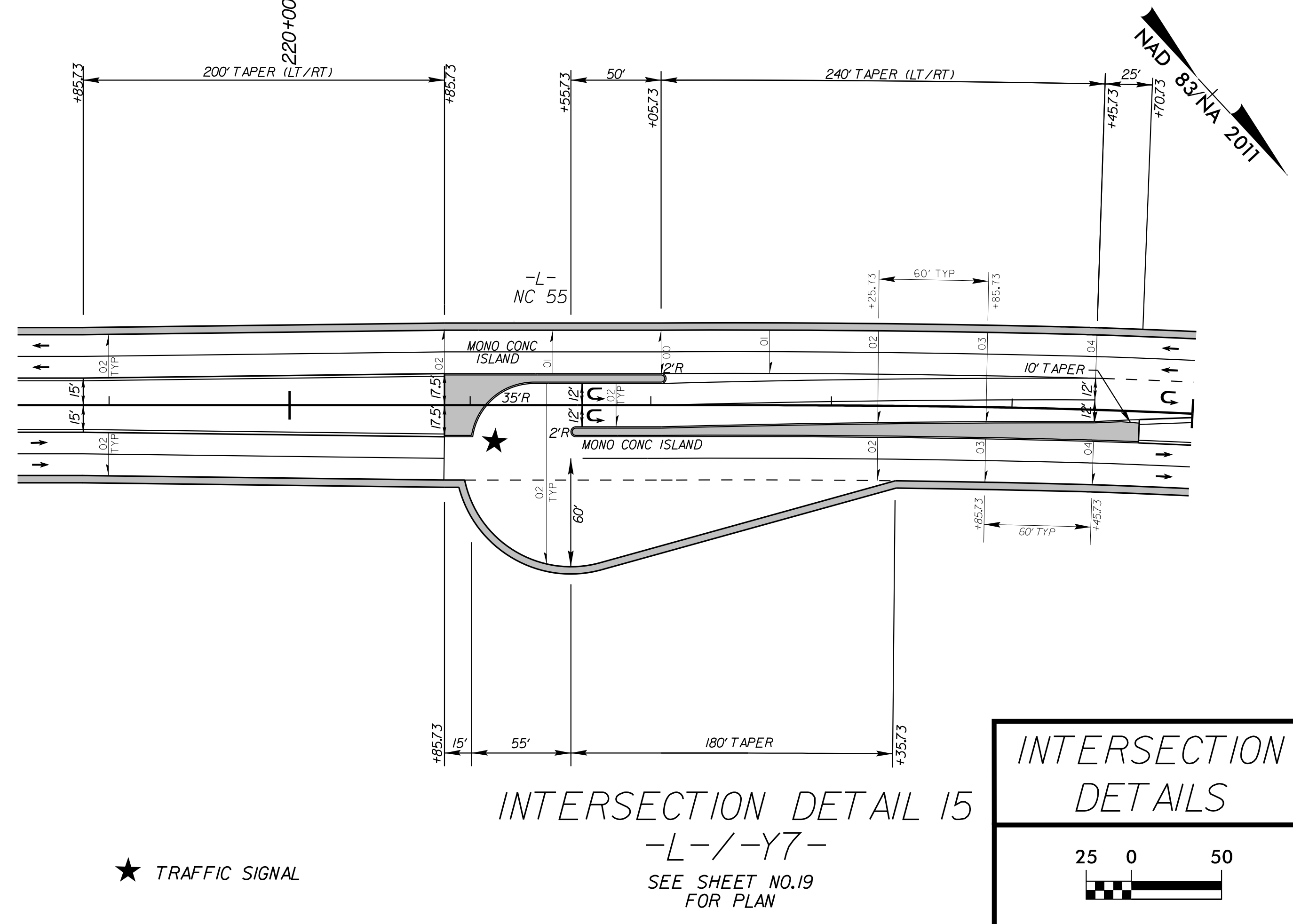
INTERSECTION DETAIL 13
-L-/-Y6-
SEE SHEET NO.15
FOR PLAN

NAD 83/NA 2011



INTERSECTION DETAIL 14
-L-/-Y6-
SEE SHEET NO.16
FOR PLAN

NAD 83/NA 2011



INTERSECTION DETAIL 15
-L-/-Y7-
SEE SHEET NO.19
FOR PLAN

NAD 83/NA 2011

★ TRAFFIC SIGNAL

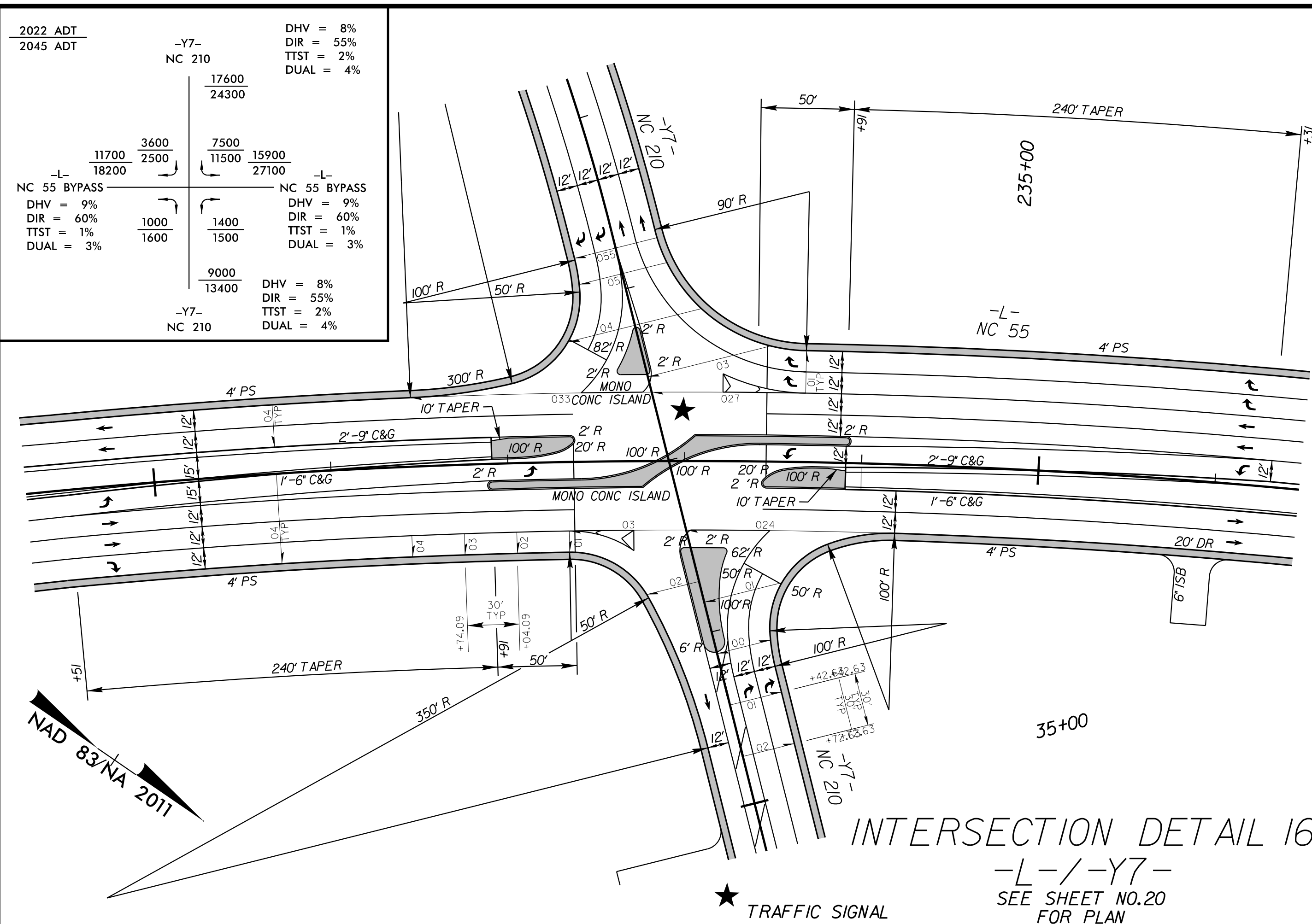
INTERSECTION DETAILS

25 0 50

8/8/2023

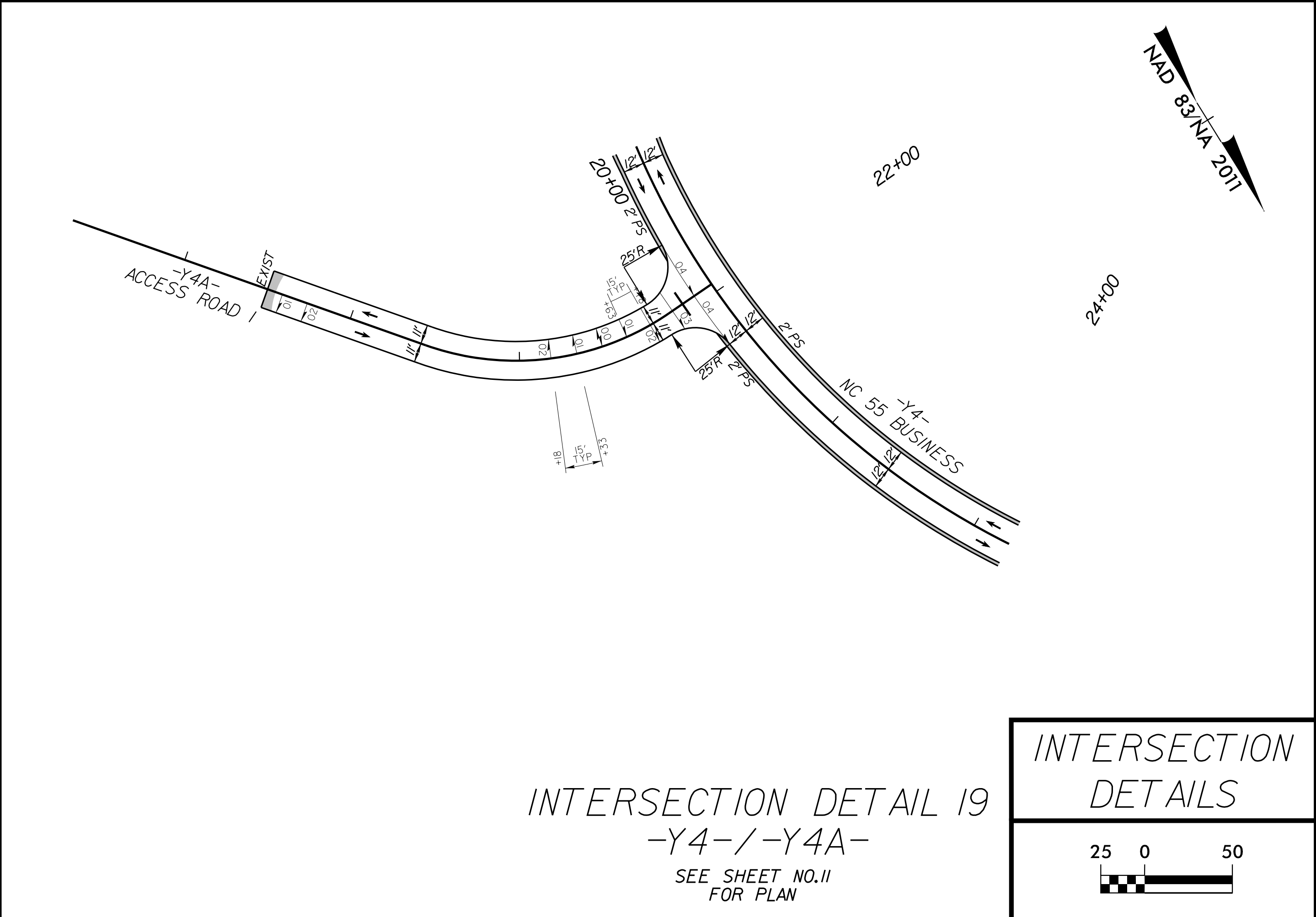
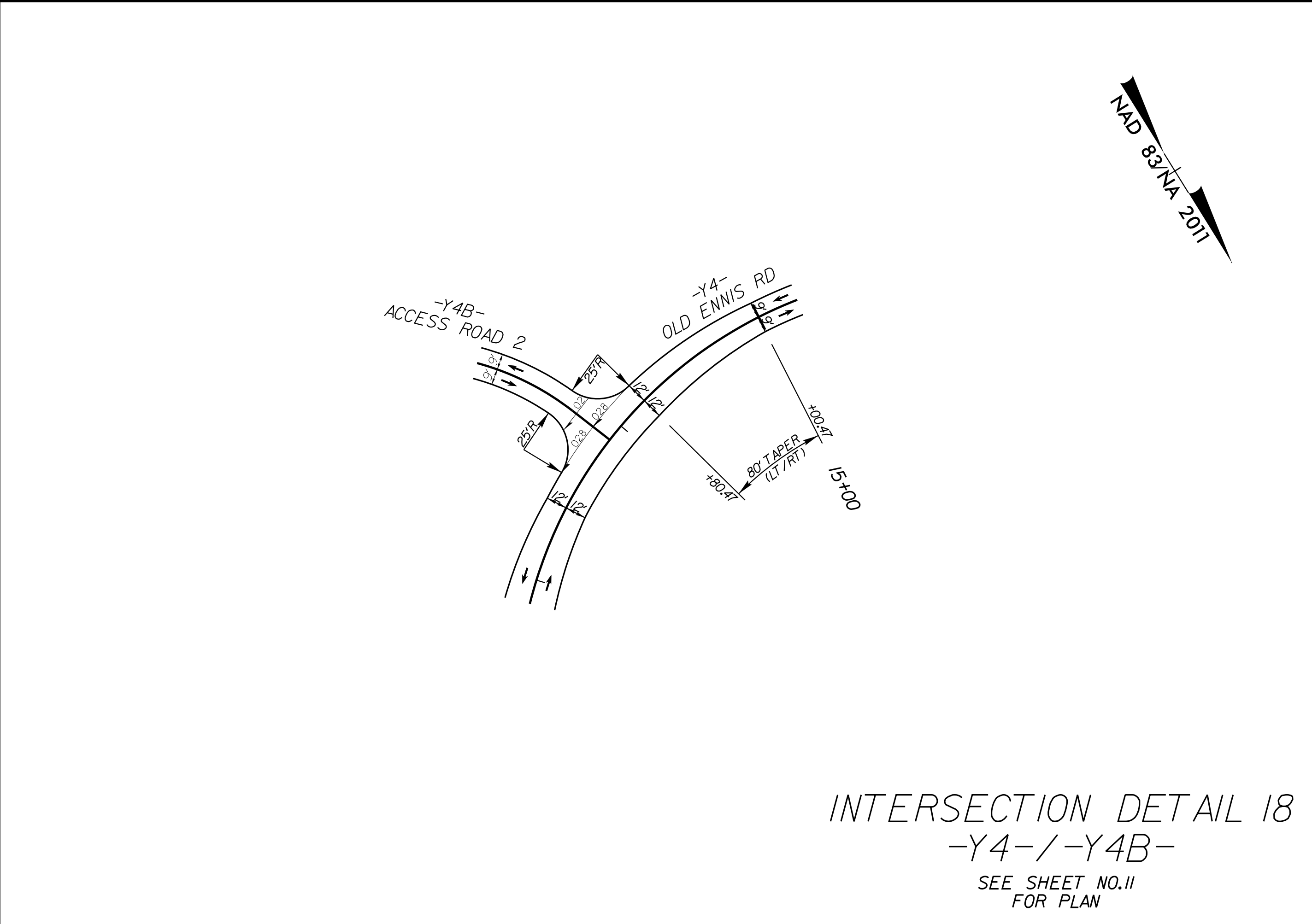
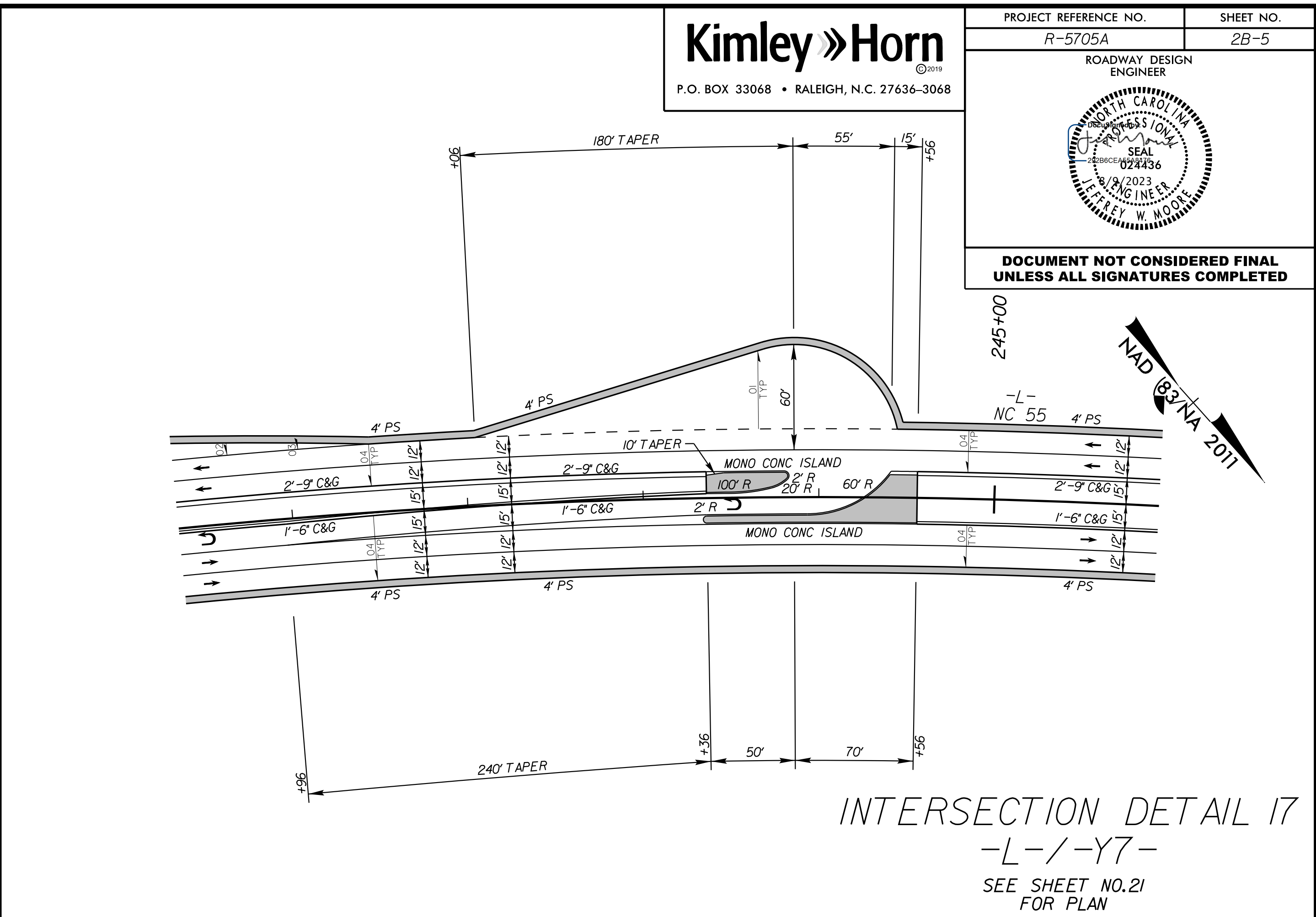
5/14/2017

2022 ADT	-Y7-	DHV = 8%
2045 ADT	NC 210	DIR = 55%
		TTST = 2%
		DUAL = 4%
17600		
24300		
11700	3600	7500
18200	2500	11500
		15900
		27100
NC 55 BYPASS		NC 55 BYPASS
DHV = 9%		DHV = 9%
DIR = 60%		DIR = 60%
TTST = 1%		TTST = 1%
DUAL = 3%		DUAL = 3%
	1000	1400
	1600	1500
		9000
		13400
	-Y7-	DHV = 8%
	NC 210	DIR = 55%
		TTST = 2%
		DUAL = 4%



Kimley Horn
P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

PROJECT REFERENCE NO.	SHEET NO.
R-5705A	2B-5
ROADWAY DESIGN ENGINEER	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



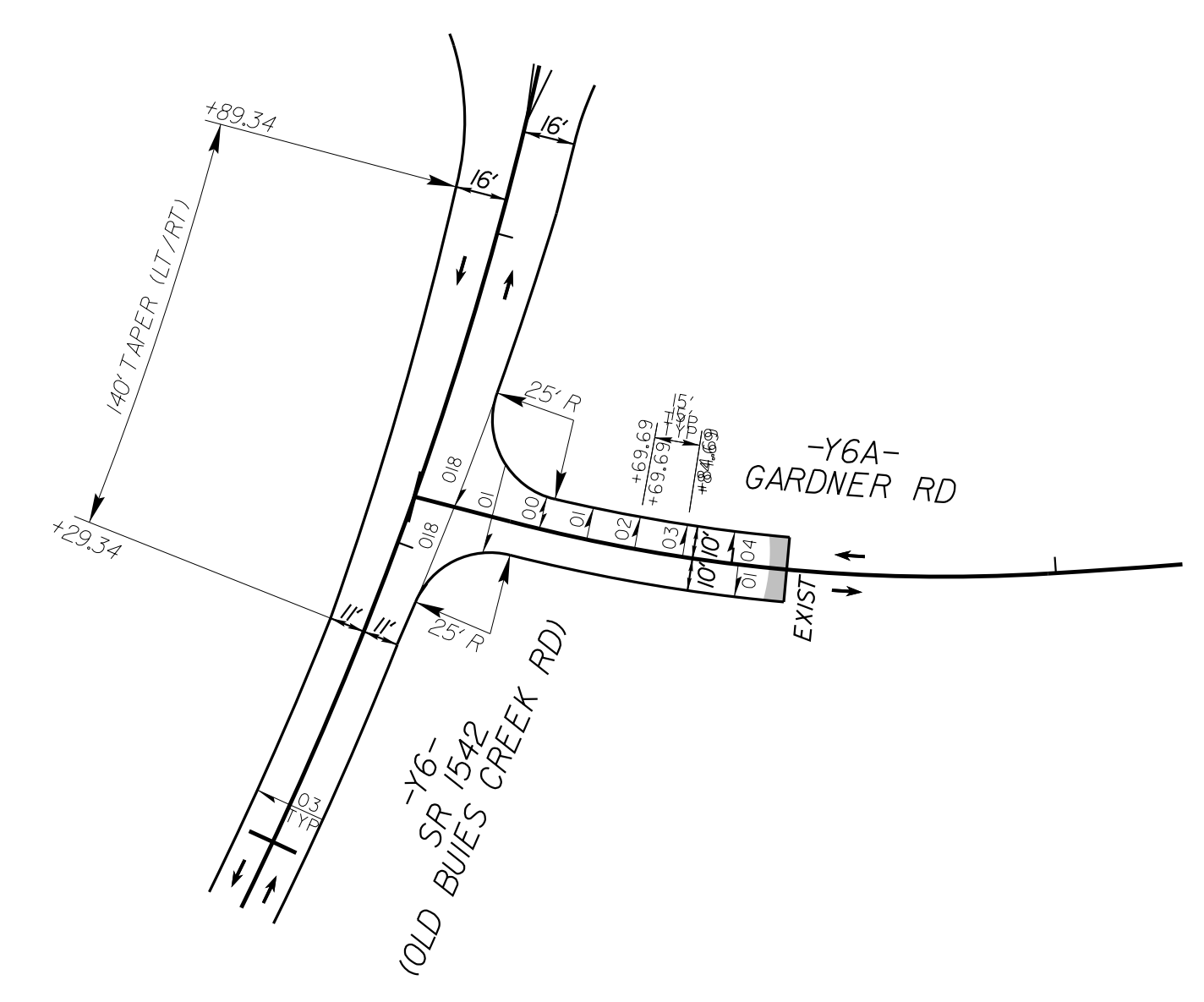
INTERSECTION DETAILS

8/8/2023

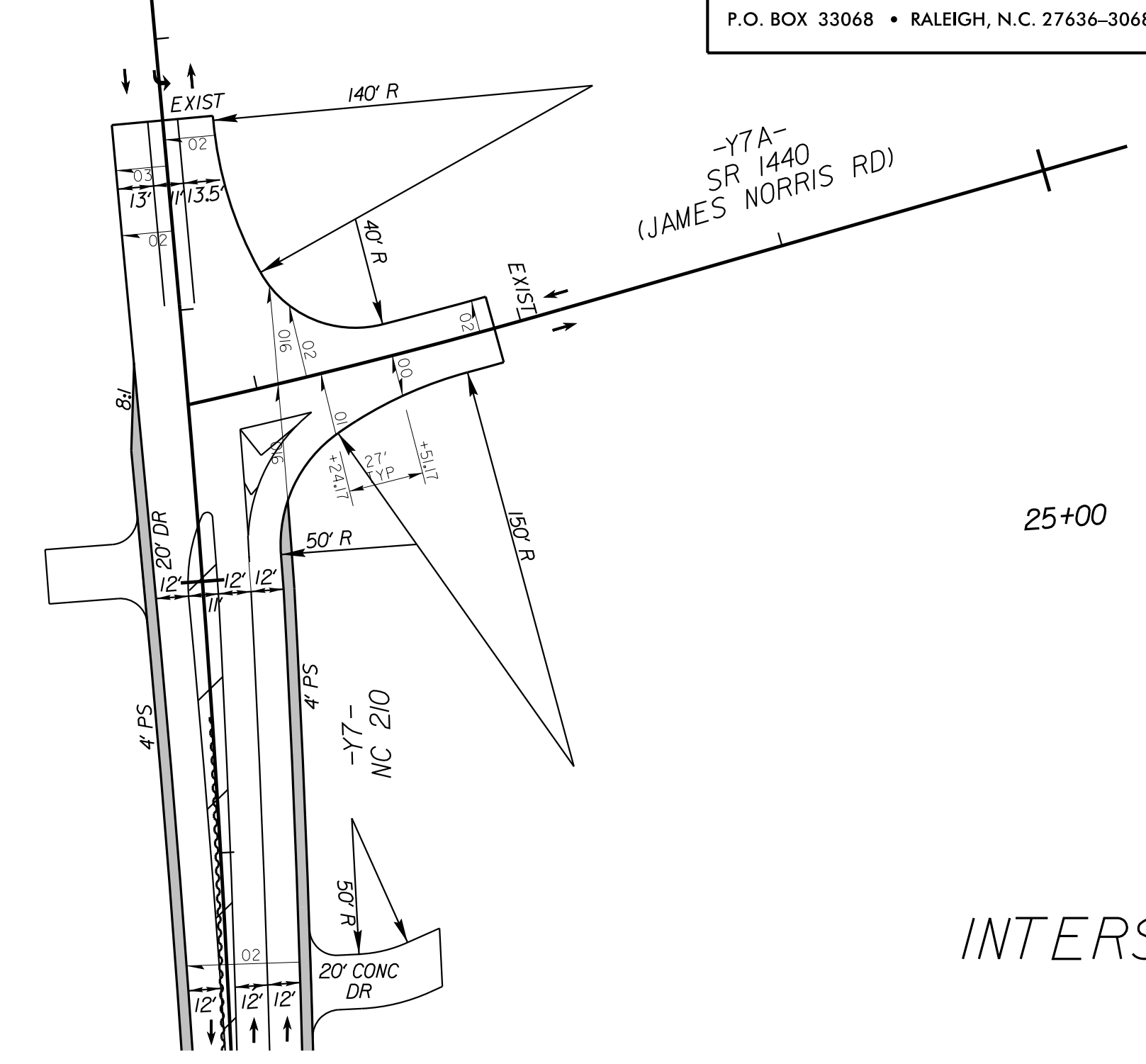
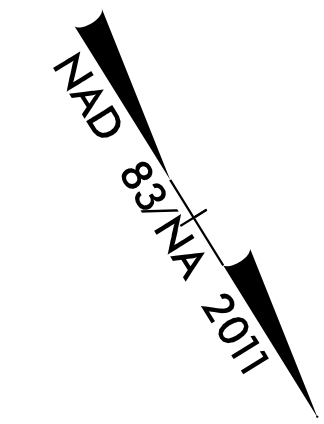
5/14/1999

Kimley Horn
 ROADWAY DESIGN ENGINEER
 P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

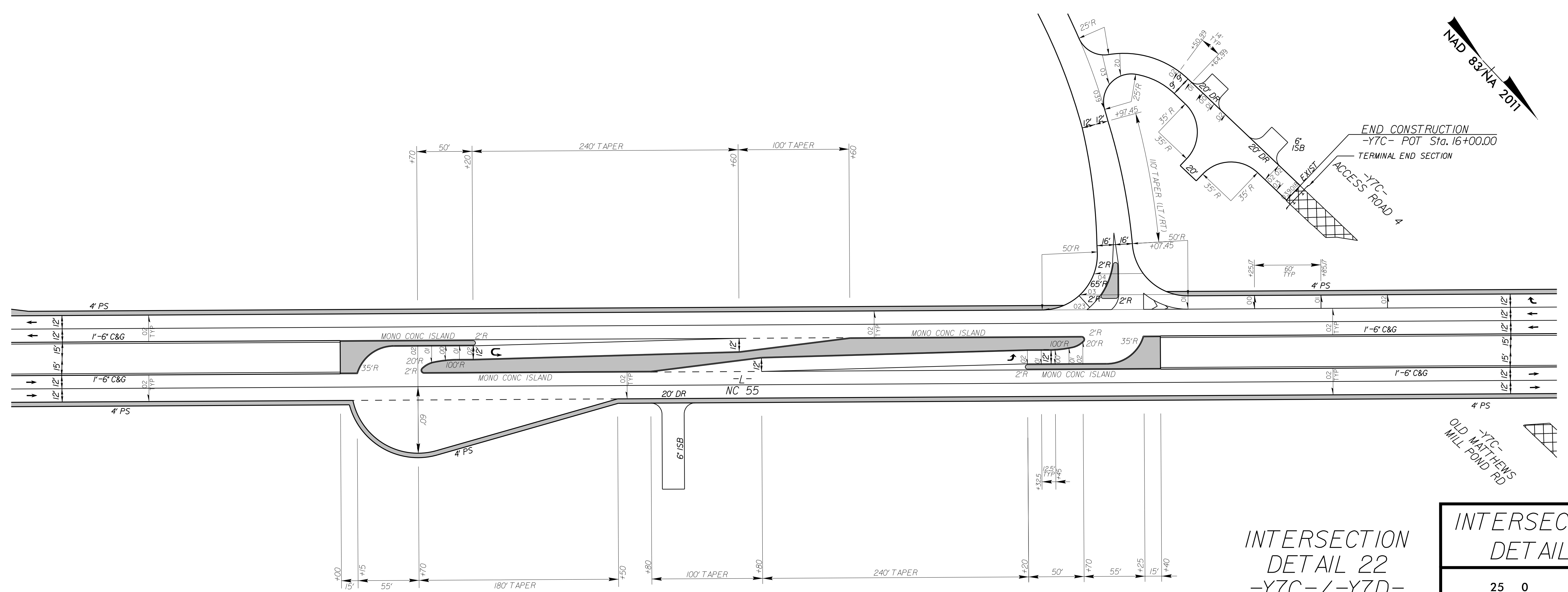
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-6
ROADWAY DESIGN ENGINEER	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



20+00 INTERSECTION DETAIL 20
 -Y6-/-Y6A-
 SEE SHEET NO.15 FOR PLAN

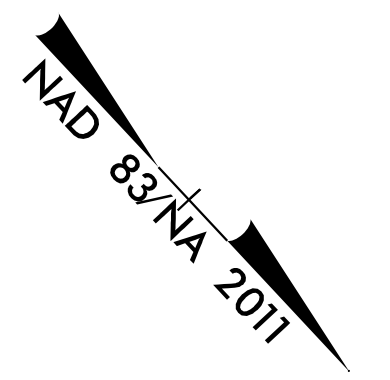


INTERSECTION DETAIL 21
 -Y7-/-Y7A-
 SEE SHEET NO.23 FOR PLAN



INTERSECTION DETAIL 22
 -Y7C-/-Y7D-
 SEE SHEET NO.18 FOR PLAN

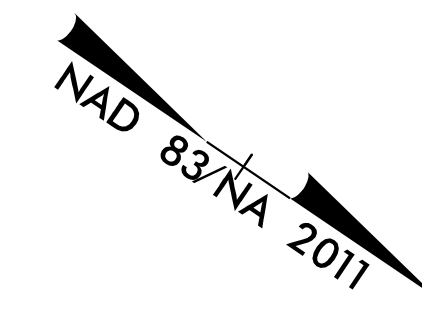
INTERSECTION DETAILS



Y7C-
 OLD MATTHEWS
 MILL POND RD

8/8/2023

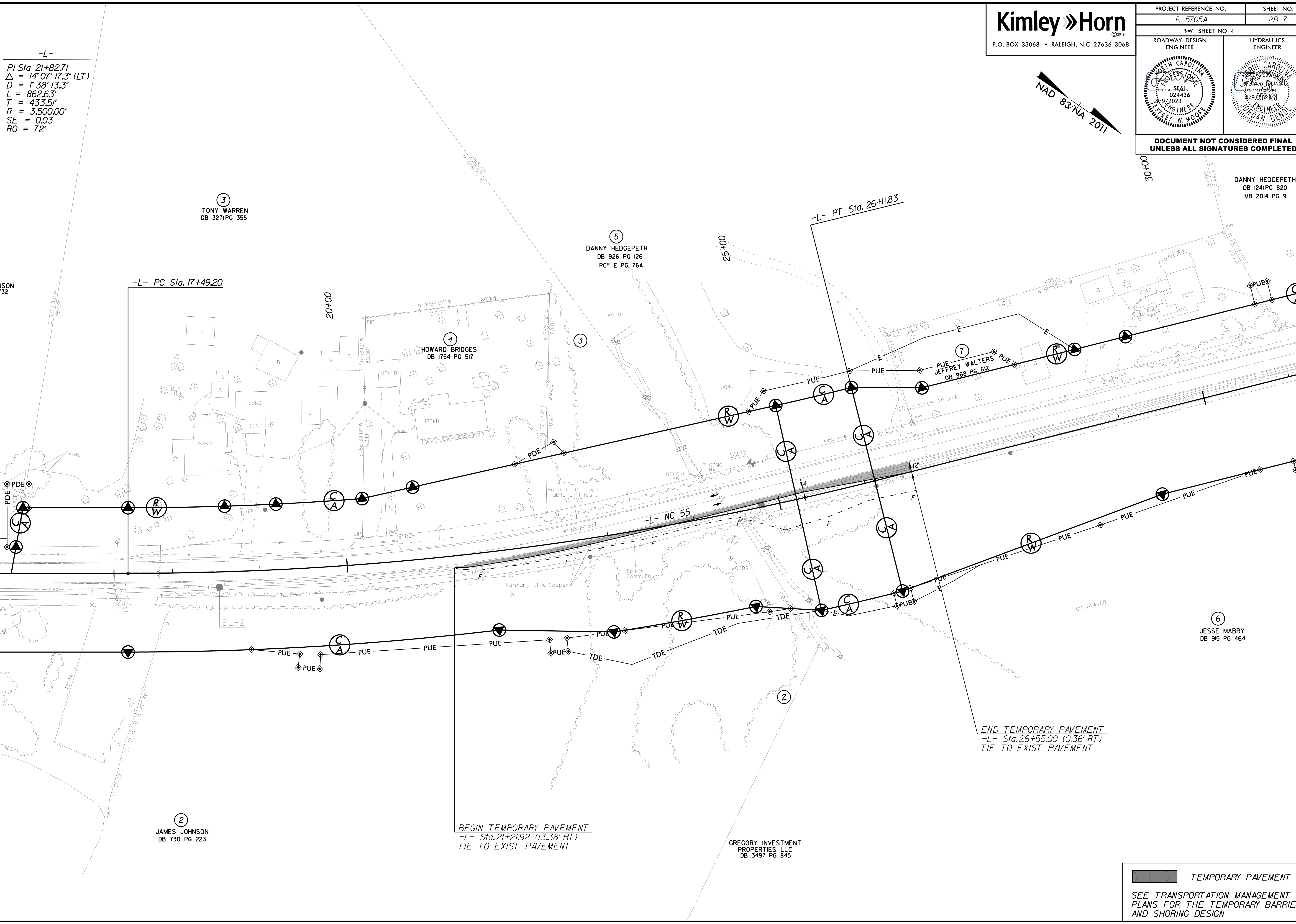
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-7
RW SHEET NO. 4	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



5/14/99

-L-
 PI Sta 21+82.71
 $\Delta = 14^{\circ} 07' 17.3''$ (LT)
 $D = 138' 13.3''$
 $L = 862.63'$
 $T = 433.51'$
 $R = 3,500.00'$
 $SE = 0.03$
 $RO = 72'$

REVISIONS



-L- PC Sta. 17+49.20

-L- PT Sta. 26+11.83

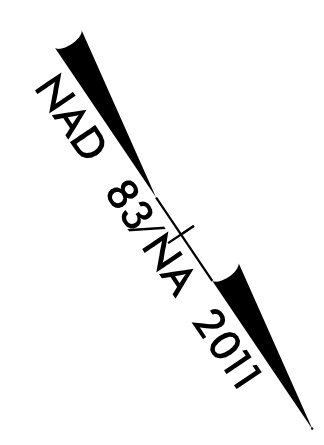
BEGIN TEMPORARY PAVEMENT
 -L- Sta. 21+21.92 (13.38' RT)
 TIE TO EXIST PAVEMENT

END TEMPORARY PAVEMENT
 -L- Sta. 26+55.00 (0.36' RT)
 TIE TO EXIST PAVEMENT

TEMPORARY PAVEMENT
 SEE TRANSPORTATION MANAGEMENT
 PLANS FOR THE TEMPORARY BARRIER
 AND SHORING DESIGN

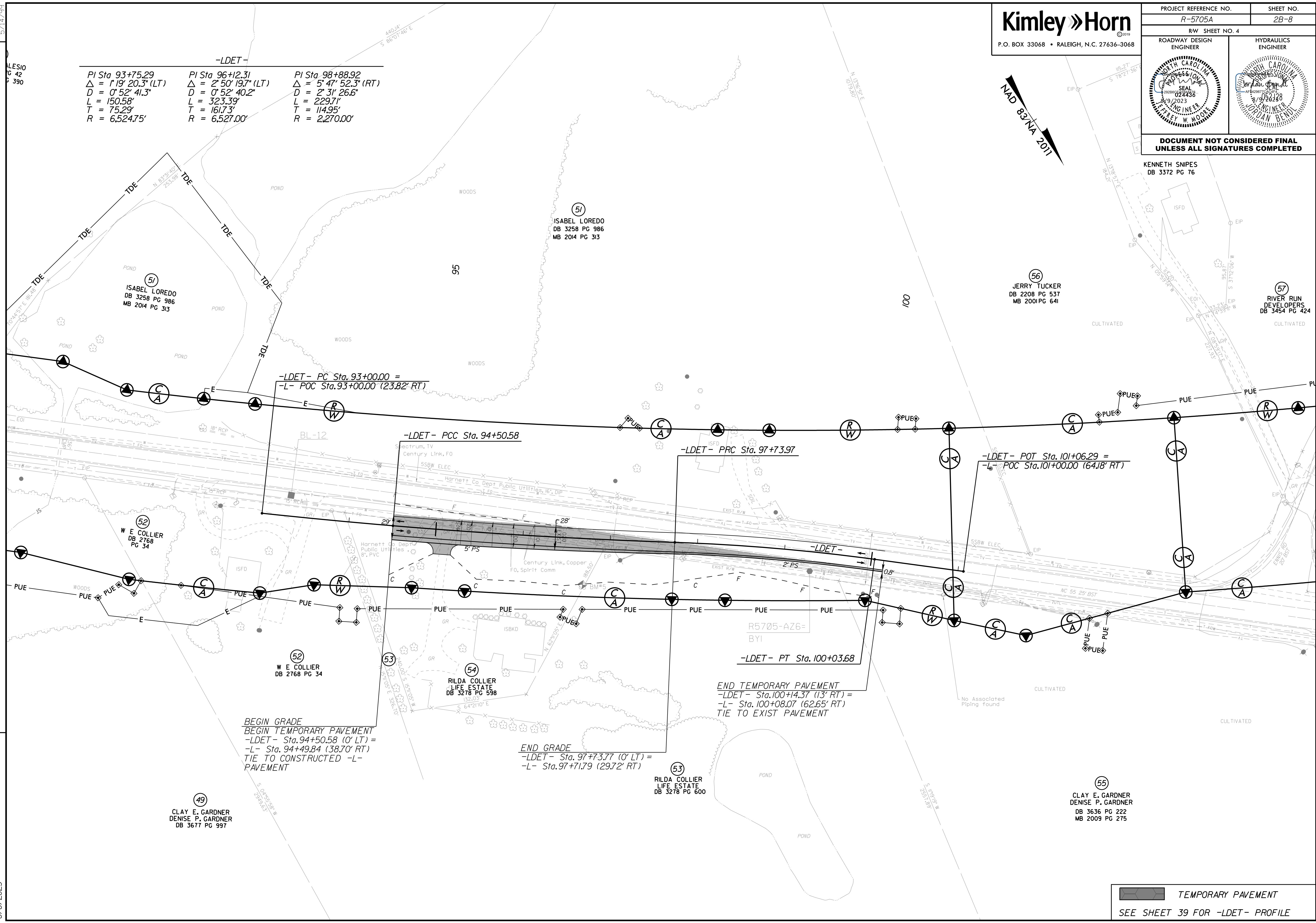
8/8/2023

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-8
RW SHEET NO. 4	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



-LDET-		
PI Sta 93+75.29 Δ = 1° 19' 20.3" (LT) D = 0° 52' 41.3" L = 150.58' T = 75.29' R = 6,524.75'	PI Sta 96+12.31 Δ = 2° 50' 19.7" (LT) D = 0° 52' 40.2" L = 323.39' T = 161.73' R = 6,527.00'	PI Sta 98+88.92 Δ = 5° 47' 52.3" (RT) D = 2° 31' 26.6" L = 229.71' T = 114.95' R = 2,270.00'

REVISIONS



BEGIN GRADE
BEGIN TEMPORARY PAVEMENT
-LDET- Sta. 94+50.58 (0' LT) =
-L- Sta. 94+49.84 (38.70' RT)
TIE TO CONSTRUCTED -L-
PAVEMENT

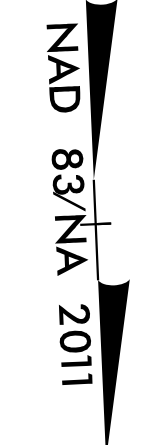
END GRADE
-LDET- Sta. 97+73.77 (0' LT) =
-L- Sta. 97+71.79 (29.72' RT)

END TEMPORARY PAVEMENT
-LDET- Sta. 100+14.37 (13' RT) =
-L- Sta. 100+08.07 (62.65' RT)
TIE TO EXIST PAVEMENT

TEMPORARY PAVEMENT
SEE SHEET 39 FOR -LDET- PROFILE

8/8/2023

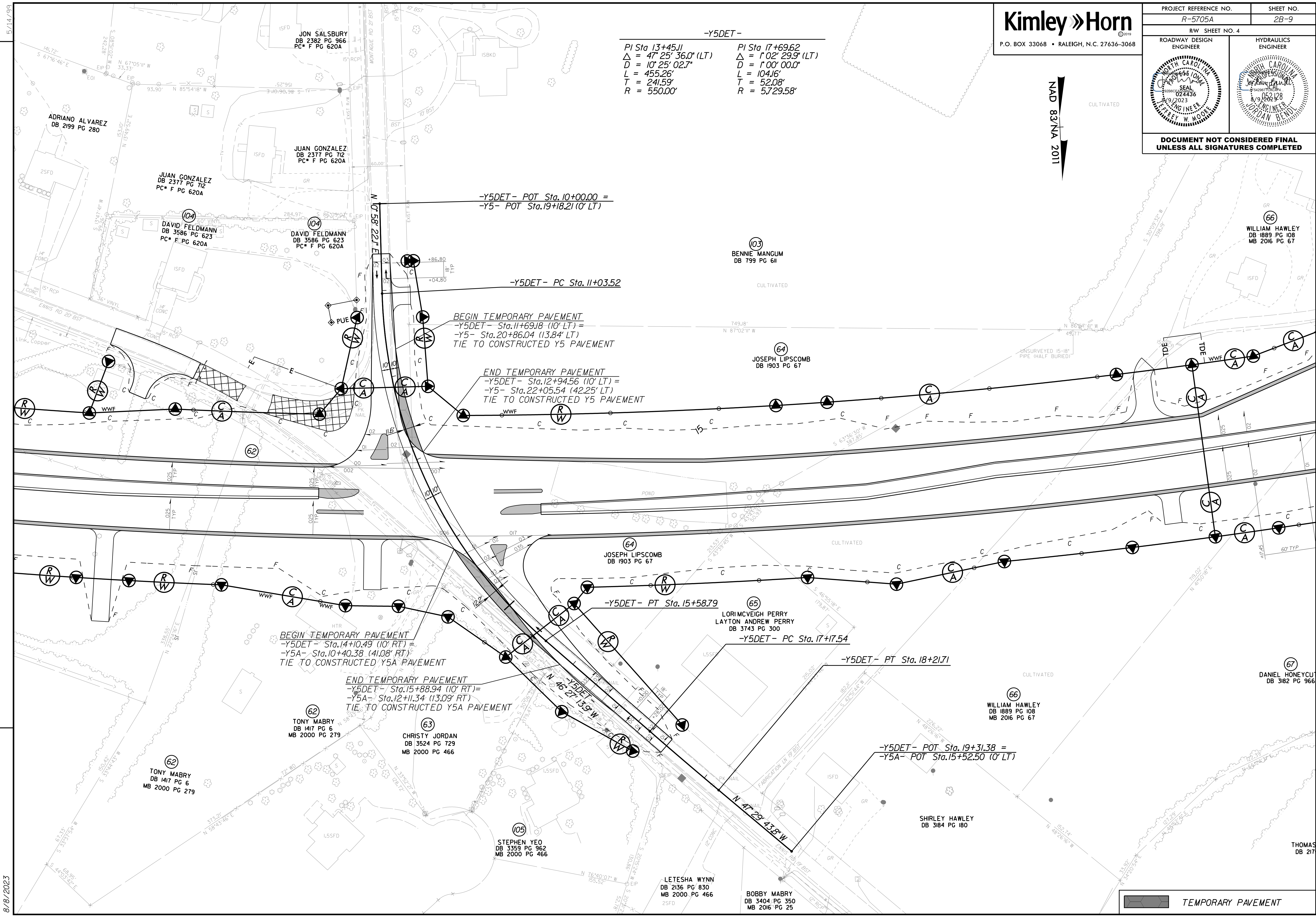
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-9
RW SHEET NO. 4	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



-Y5DET-

PI Sta 13+45.11 Δ = 47° 25' 36.0" (LT) D = 10' 25' 02.7" L = 455.26' T = 241.59' R = 550.00'	PI Sta 17+69.62 Δ = 1° 02' 29.9" (LT) D = 1' 00' 00.0" L = 104.16' T = 52.08' R = 5729.58'
---	---

REVISIONS



TEMPORARY PAVEMENT

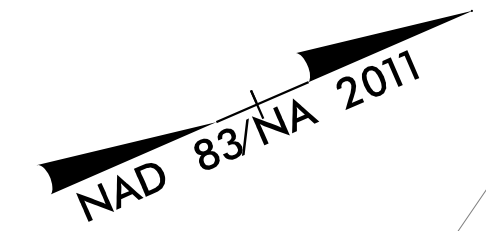
8/8/2023

5/14/1999

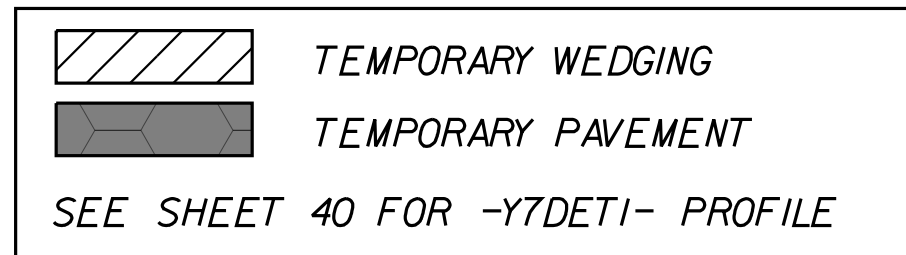
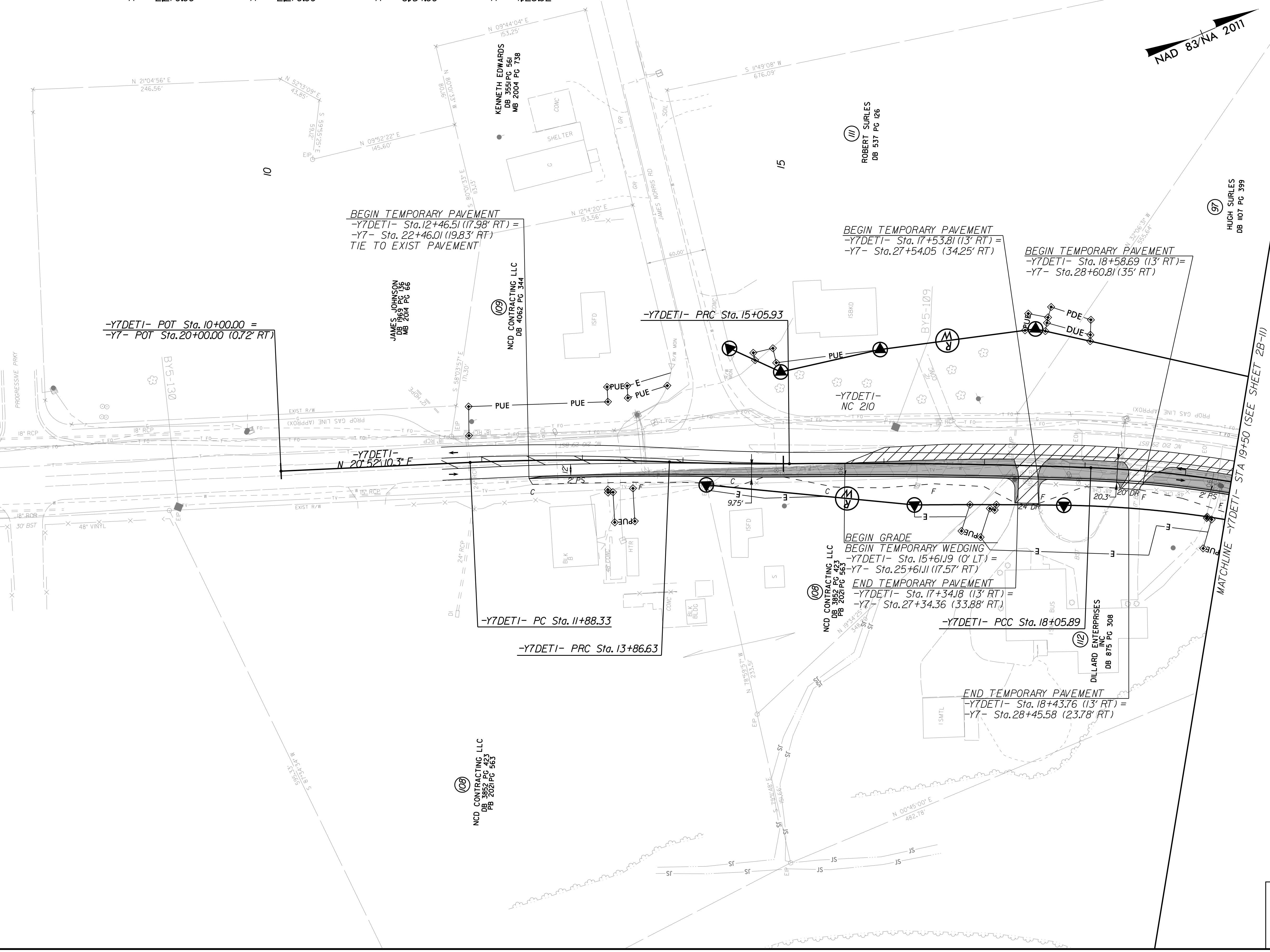
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-10
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-Y7DETI-

PI Sta 12+87.54 Δ = 5°00'18.5" (RT) D = 2°31'26.6" L = 198.30' T = 99.21' R = 2270.00'	PI Sta 14+46.29 Δ = 3°00'40.9" (LT) D = 2°31'26.6" L = 119.31' T = 59.67' R = 2270.00'	PI Sta 16+55.94 Δ = 2°47'33.8" (RT) D = 0°55'51.7" L = 299.96' T = 150.01' R = 6154.00'	PI Sta 19+71.12 Δ = 16°43'35.1" (RT) D = 5°05'52.3" L = 328.11' T = 165.23' R = 1123.92'
---	---	--	---

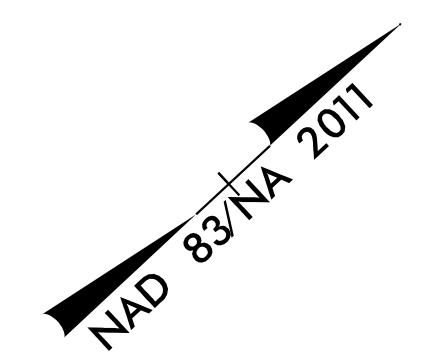


REVISIONS



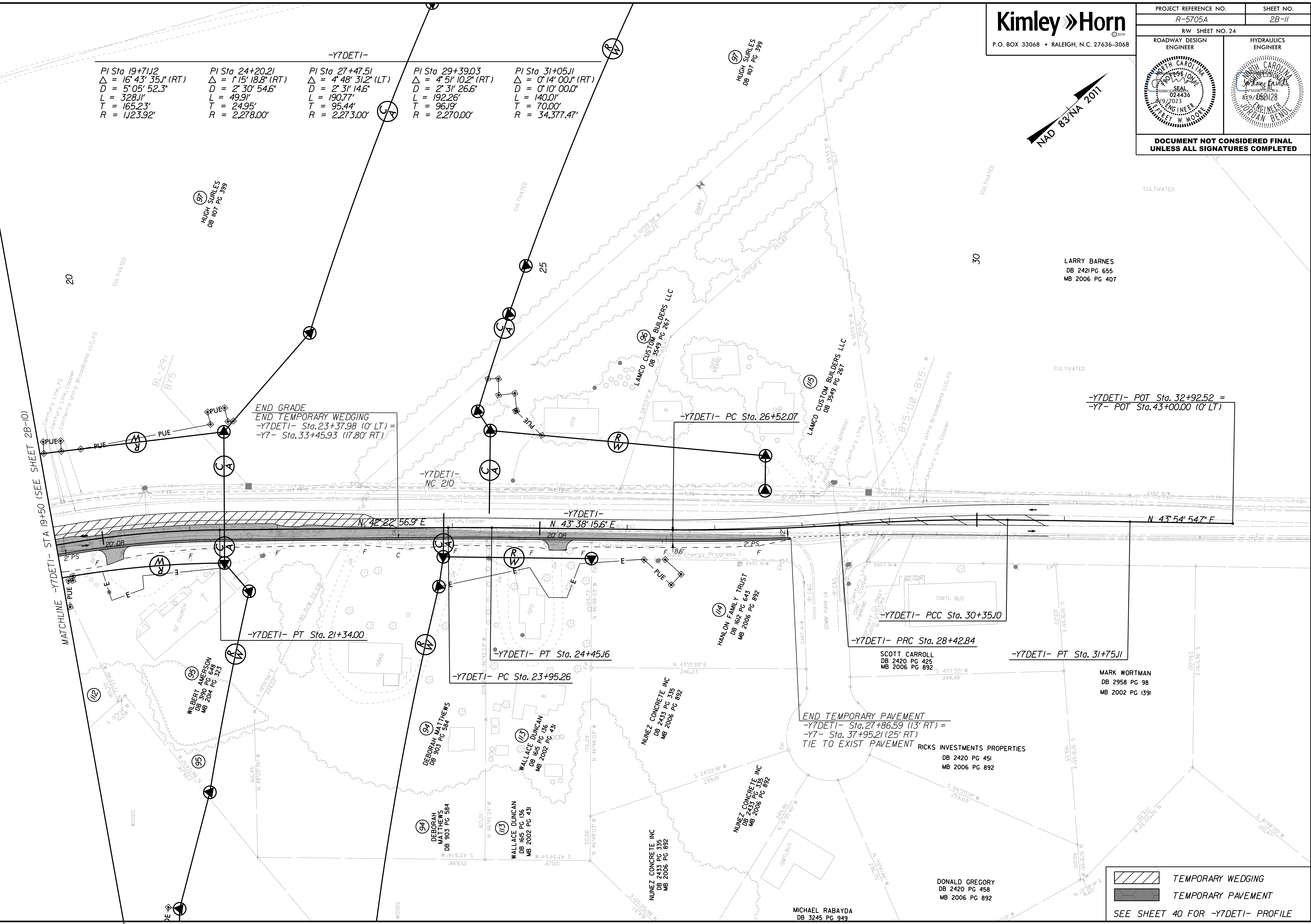
8/8/2023

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-II
RW SHEET NO. 24	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



PI Sta	Delta	D	L	T	R
19+71.2	16' 43" 35" (RT)	5' 05" 52.3"	328.11'	165.23'	1123.92'
24+20.21	1' 15" 18.8" (RT)	2' 30" 54.6"	49.91'	24.95'	2278.00'
27+47.51	4' 48" 31.2" (LT)	2' 31" 14.6"	190.77'	95.44'	2273.00'
29+39.03	4' 51" 10.2" (RT)	2' 31" 26.6"	192.26'	96.19'	2270.00'
31+05.11	0' 14" 00.0" (RT)	0' 10" 00.0"	140.01'	70.00'	34377.47'

REVISIONS



MATCHLINE -Y7DETI- STA 19+50 (SEE SHEET 2B-10)

END GRADE
END TEMPORARY WEDGING
-Y7DETI- Sta. 23+37.98 (0' LT) =
-Y7- Sta. 33+45.93 (17.80' RT)

-Y7DETI- POT Sta. 32+92.52 =
-Y7- POT Sta. 43+00.00 (0' LT)

END TEMPORARY PAVEMENT
-Y7DETI- Sta. 27+86.59 (13' RT) =
-Y7- Sta. 37+95.21 (25' RT)
TIE TO EXIST PAVEMENT

TEMPORARY WEDGING
 TEMPORARY PAVEMENT
SEE SHEET 40 FOR -Y7DETI- PROFILE

8/8/2023

5/14/1999

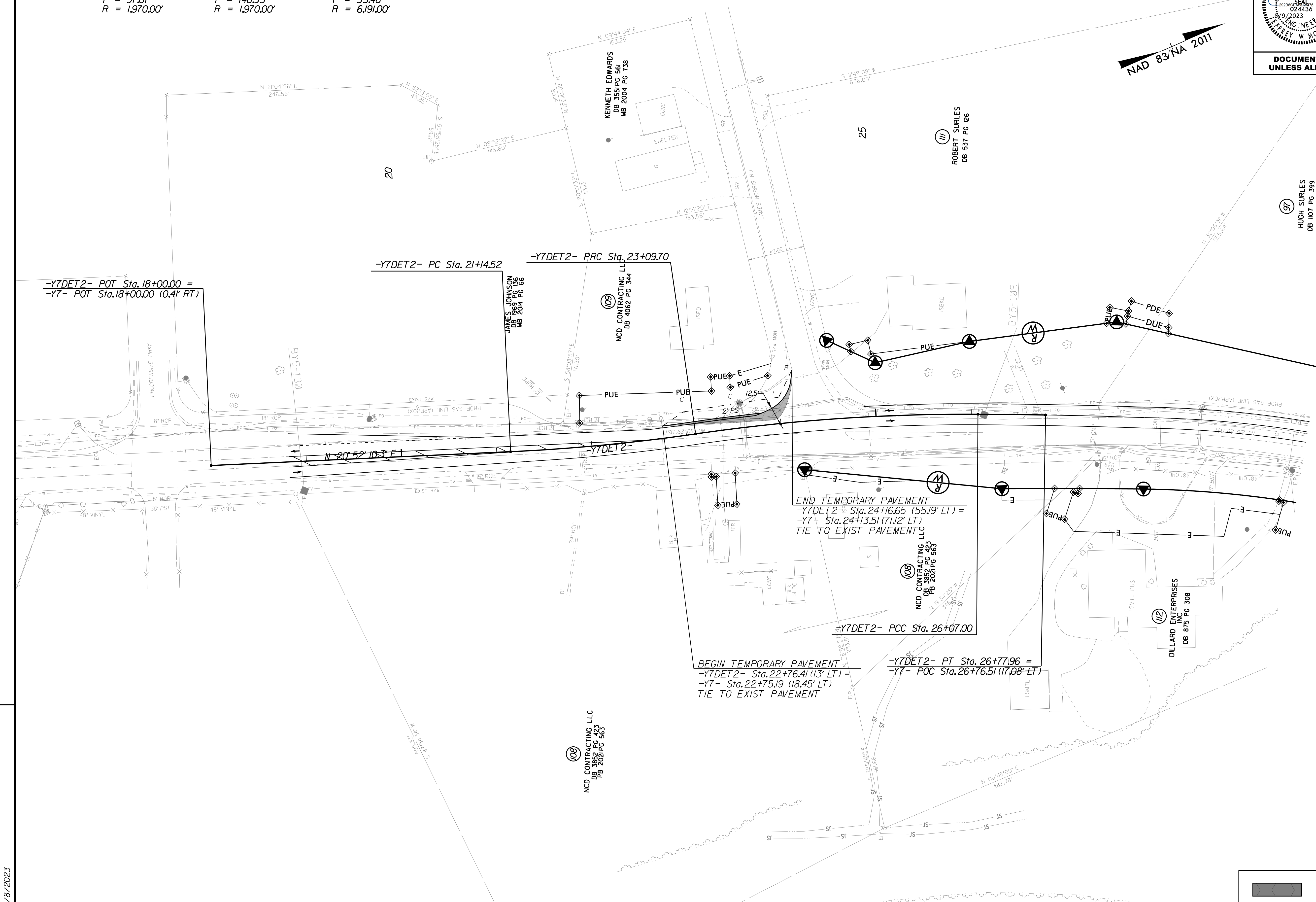
5/14/1999

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2B-12
RW SHEET NO. 24	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-Y7DET2-

PI Sta 22+12.19 Δ = 5° 40' 36.3" (LT) D = 2° 54' 30.3" L = 195.18' T = 97.67' R = 1,970.00'	PI Sta 24+58.63 Δ = 8° 38' 47.7" (RT) D = 2° 54' 30.3" L = 297.30' T = 148.93' R = 1,970.00'	PI Sta 26+42.48 Δ = 0° 39' 24.4" (RT) D = 0° 55' 31.7" L = 70.97' T = 35.48' R = 6,191.00'
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REVISIONS



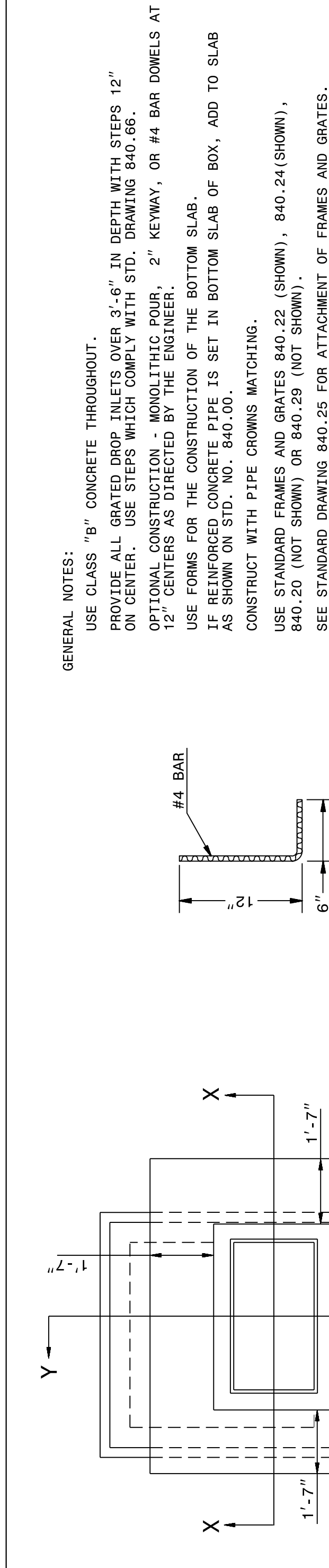
TEMPORARY PAVEMENT

8/8/2023

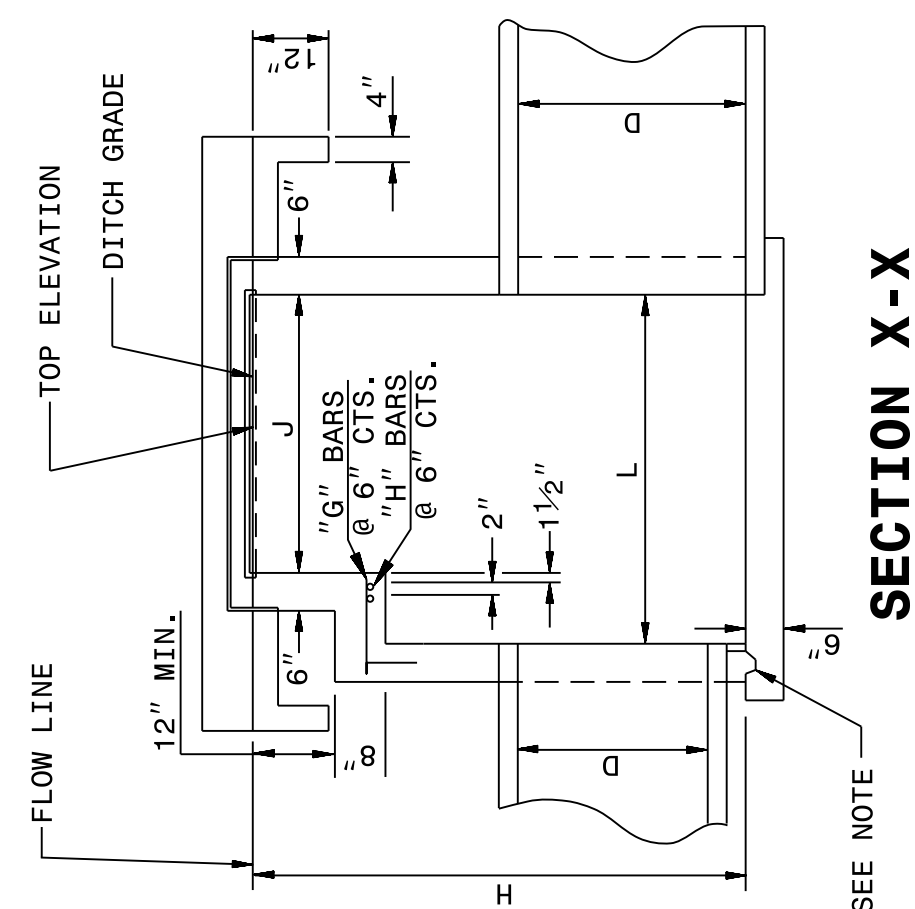
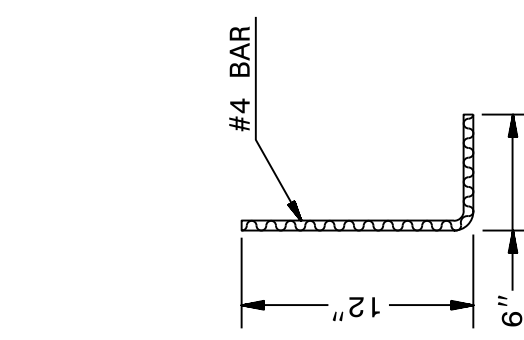
STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
CONCRETE GRATED DROP INLET TYPE 'A'
MINIMUM DEPTH
12" THRU 72" PIPE

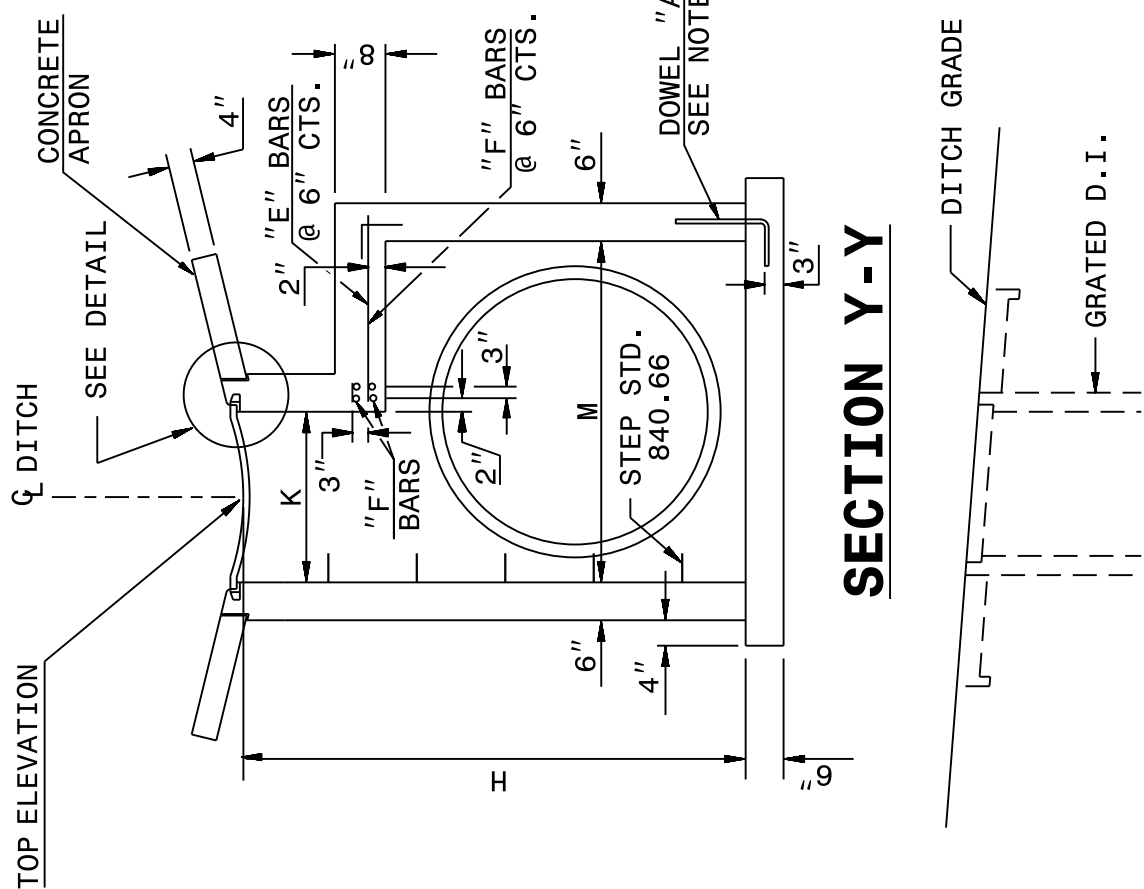
SHEET 1 OF 2
840d17



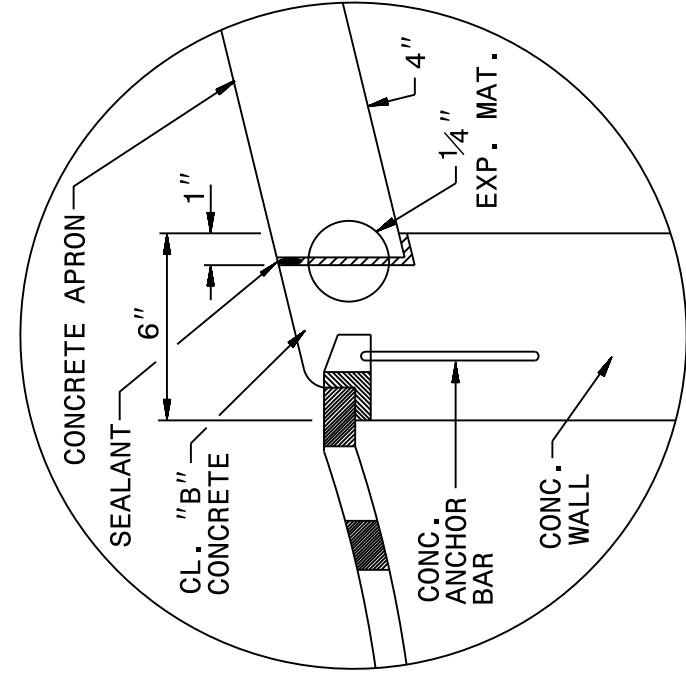
DOWEL - A



SECTION X-X



SECTION Y-Y



DETAIL
(APRON SUPPORT NOTCH)

STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
CONCRETE GRATED DROP INLET TYPE 'A'
MINIMUM DEPTH
12" THRU 72" PIPE

SHEET 1 OF 2
840d17

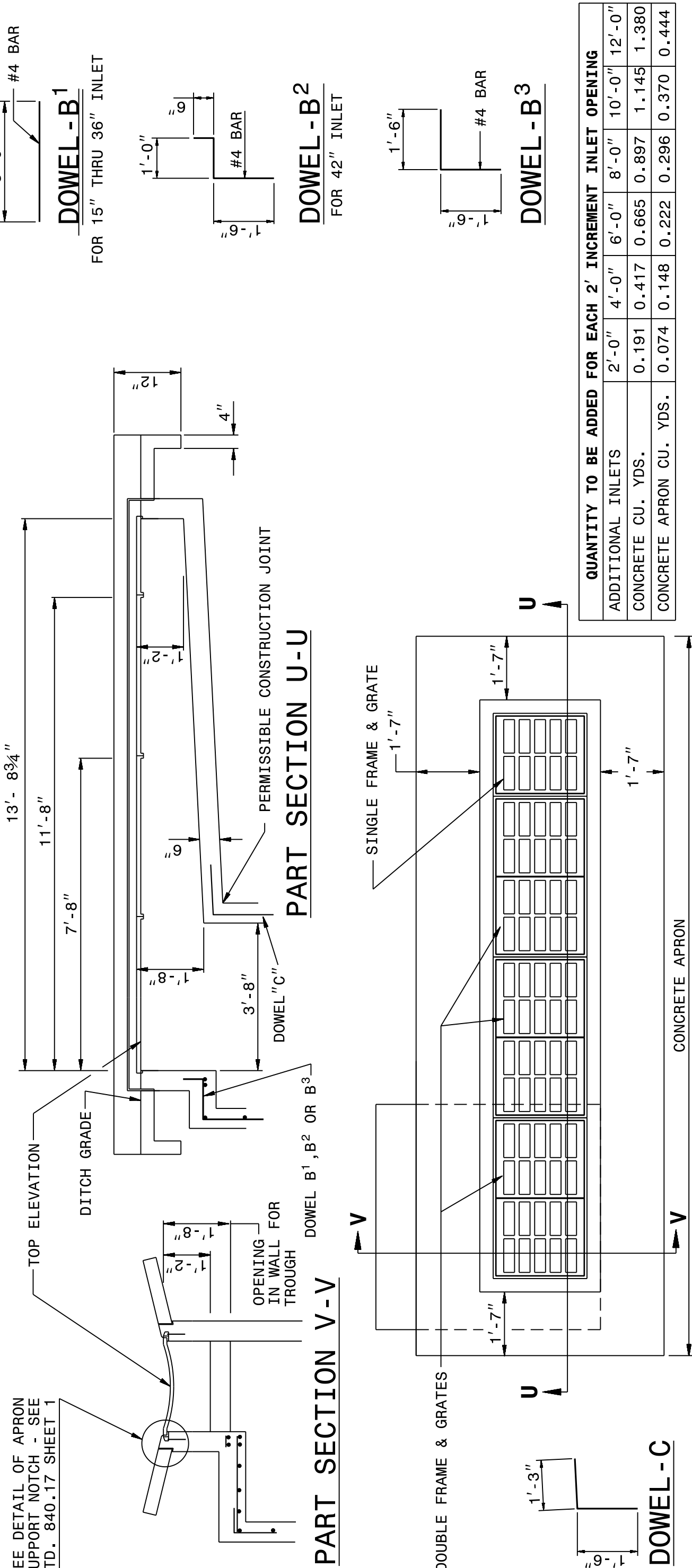
GENERAL NOTES:
USE CLASS "B" CONCRETE THROUGHOUT.
PROVIDE ALL GRATED DROP INLETS OVER 3'-6" IN DEPTH WITH STEPS 12" ON CENTER. USE STEPS WHICH COMPLY WITH STD. DRAWING 840.66.
OPTIONAL CONSTRUCTION - MONOLITHIC POUR, 2" KEYWAY, OR #4 BAR DOWELS AT 12" CENTERS AS DIRECTED BY THE ENGINEER.
USE FORMS FOR THE CONSTRUCTION OF THE BOTTOM SLAB.
IF REINFORCED CONCRETE PIPE IS SET IN BOTTOM SLAB OF BOX, ADD TO SLAB AS SHOWN ON STD. NO. 840.00.
CONSTRUCT WITH PIPE CROWNS MATCHING.
USE STANDARD FRAMES AND GRATES 840.22 (SHOWN), 840.24 (SHOWN), 840.20 (NOT SHOWN) OR 840.29 (NOT SHOWN).
SEE STANDARD DRAWING 840.25 FOR ATTACHMENT OF FRAMES AND GRATES.
CHAMFER ALL EXPOSED CORNERS 1".
DRAWING NOT TO SCALE.
MAX. DEPTH OF THIS STRUCTURE FROM TOP OF BOTTOM SLAB TO TOP ELEVATION IS 12 FEET.

I:\SEP-2017\1455\portassets\Special Details\Howerton\840d17 Minimum Depth Type A.dgn
Howerton At CSD 2/25/15

STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
CONCRETE GRATED DROP INLET TYPE 'A'
MINIMUM DEPTH
12" THRU 72" PIPE

SHEET 2 OF 2
840d17



PART SECTION V-V

PART SECTION U-U

DOWEL - C

DOWEL - B3

DOWEL - B2
FOR 42" INLET

DOWEL - B1
FOR 15" THRU 36" INLET

QUANTITY TO BE ADDED FOR EACH 2' INCREMENT INLET OPENING

ADDITIONAL INLETS	2'-0"	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"
CONCRETE CU. YDS.	0.191	0.417	0.665	0.897	1.145	1.380
CONCRETE APRON CU. YDS.	0.074	0.148	0.222	0.296	0.370	0.444

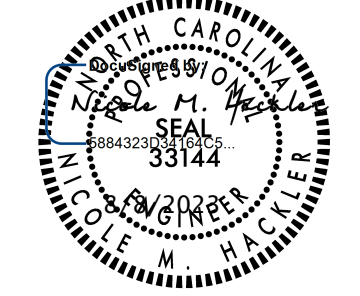
PIPE	DIMENSIONS OF BOX AND PIPE			REINFORCING STEEL - NO. 4 BARS				MIN. DIMENSIONS AND QUANTITIES FOR CONCRETE GRATED DROP INLET (BASED ON MIN. HEIGHT, H)		CU YDS CONC. IN BOX		DEDUCTIONS FOR ONE PIPE			
	SPAN	WIDTH	HEIGHT	NO.	LENGTH	NO.	LENGTH	BARS H	TOTAL	APRON	TOTAL	C.S.	R.C.		
12"	3'-8"	2'-0"	2'-3"	—	—	—	—	—	0.362	0.926	0.247	0.395	1.683	0.015	0.024
15"	3'-8"	2'-0"	2'-5"	—	—	—	—	—	0.362	0.988	0.247	0.395	1.745	0.023	0.036
18"	—	—	2'-0"	2'-8"	—	—	—	—	0.362	1.050	0.247	—	1.807	0.033	0.049
24"	—	—	2'-10"	3'-3"	8	1'-5"	6	4'-9"	27	0.444	1.362	0.278	2.201	0.059	0.085
30"	—	—	3'-6"	3'-10"	8	2'-0"	7	4'-9"	33	0.502	1.644	0.288	2.541	0.082	0.127
36"	4'-0"	4'-0"	4'-4"	4'-8"	8	2'-5"	8	4'-11"	47	0.560	1.931	0.321	2.920	0.132	0.178
42"	4'-0"	4'-10"	5'-0"	5'-4"	10	3'-1"	9	5'-7"	67	0.704	2.500	0.370	3.677	0.180	0.243
48"	5'-4"	5'-4"	5'-6"	5'-10"	11	3'-7"	10	6'-1"	87	0.823	3.013	0.407	4.315	0.235	0.317
54"	6'-0"	6'-0"	6'-2"	6'-6"	12	4'-1"	11	6'-7"	107	0.951	3.589	0.444	5.072	0.287	0.401
60"	6'-6"	6'-6"	6'-7"	7'-1"	13	4'-9"	12	7'-3"	135	1.311	4.539	0.494	6.170	0.367	0.495
66"	7'-2"	7'-2"	7'-1"	7'-10"	14	5'-4"	14	7'-10"	168	1.136	5.061	0.537	6.901	0.444	0.599
72"	3'-8"	2'-0"	7'-8"	7'-8"	15	5'-11"	15	8'-5"	199	1.500	5.860	0.580	0.395	7.868	0.528

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**CONTRACT STANDARDS
AND DEVELOPMENT UNIT**
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SEE TITLE BLOCK

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MODIFIED BY: DATE:
CHECKED BY: DATE:
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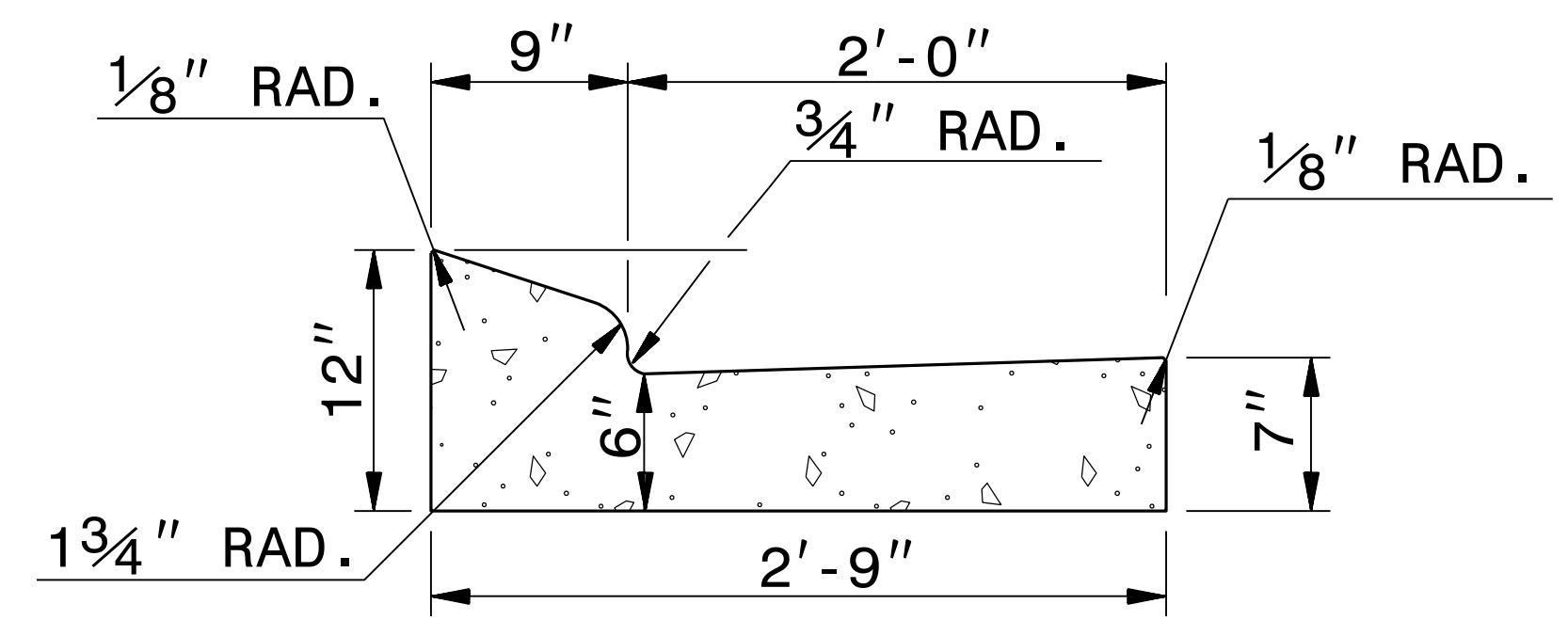


STATE OF
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DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
2'-9" CONCRETE CURB & GUTTER

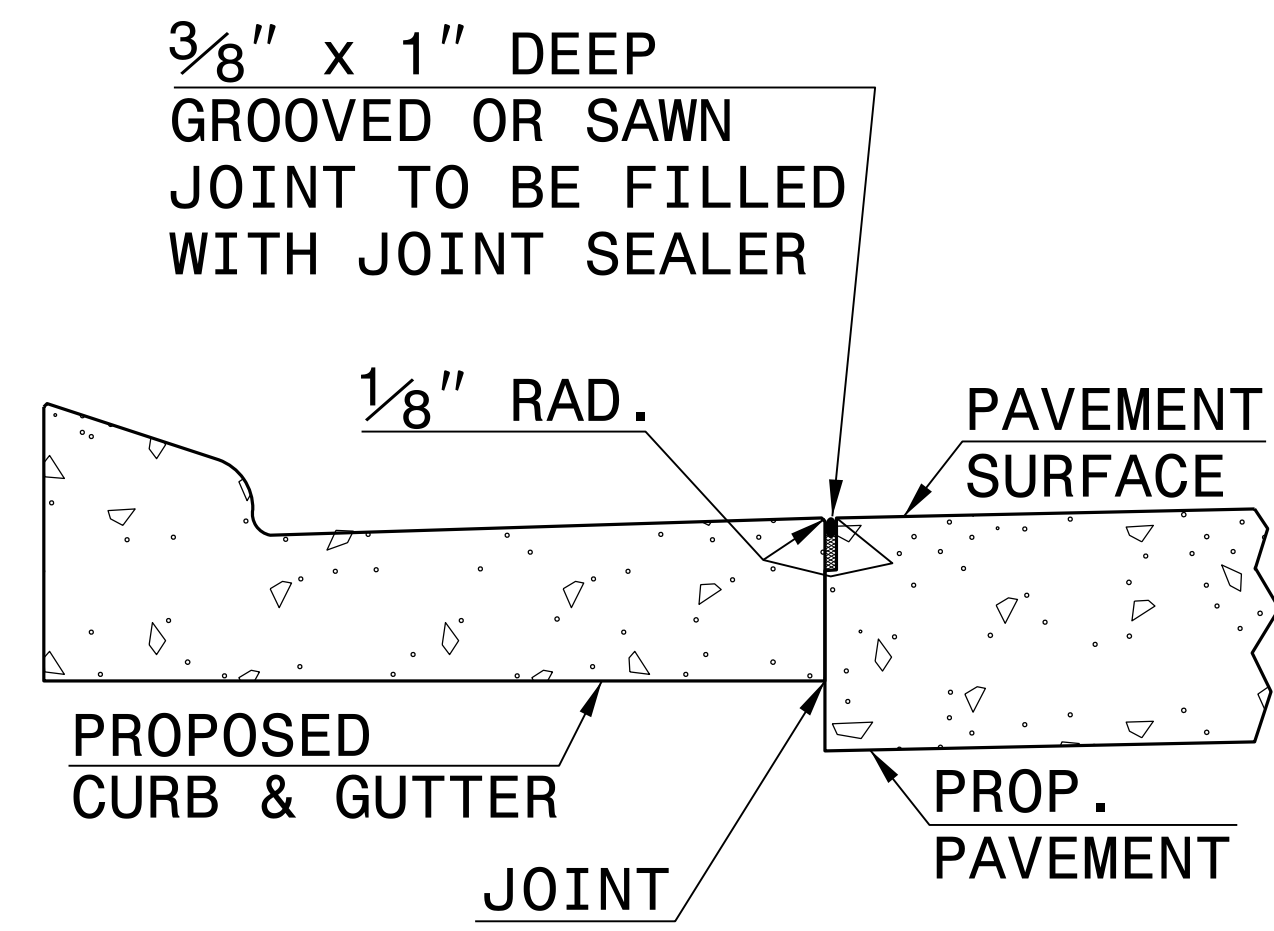
SHEET 1 OF 1
846D01

- GENERAL NOTES:
- PLACE CONTRACTION JOINTS AT 10' INTERVALS, EXCEPT THAT A 15' SPACING MAY BE USED WHEN A MACHINE IS USED OR WHEN SATISFACTORY SUPPORT FOR THE FACE FORM CAN BE OBTAINED WITHOUT THE USE OF TEMPLATES AT 10' INTERVALS.
 - JOINT SPACING MAY BE ALTERED IF REQUIRED BY THE ENGINEER.
 - CONTRACTION JOINTS MAY BE INSTALLED WITH THE USE OF TEMPLATES OR FORMED BY OTHER APPROVED METHODS. MAKE NON-TEMPLATE FORMED JOINTS A MIN. OF 1½" DEEP.
 - FILL ALL CONSTRUCTION JOINTS WITH JOINT FILLER AND SEALER.
 - SPACE EXPANSION JOINTS AT 90' INTERVALS AND ADJACENT TO ALL RIGID OBJECTS.
 - SEE RDWY. STD. DWG. NO. 846.01, SHEET 2 OF 3 FOR PLACEMENT IN SUPERELEVATIONS. (USE 2'-6" CURB AND GUTTER RATES)

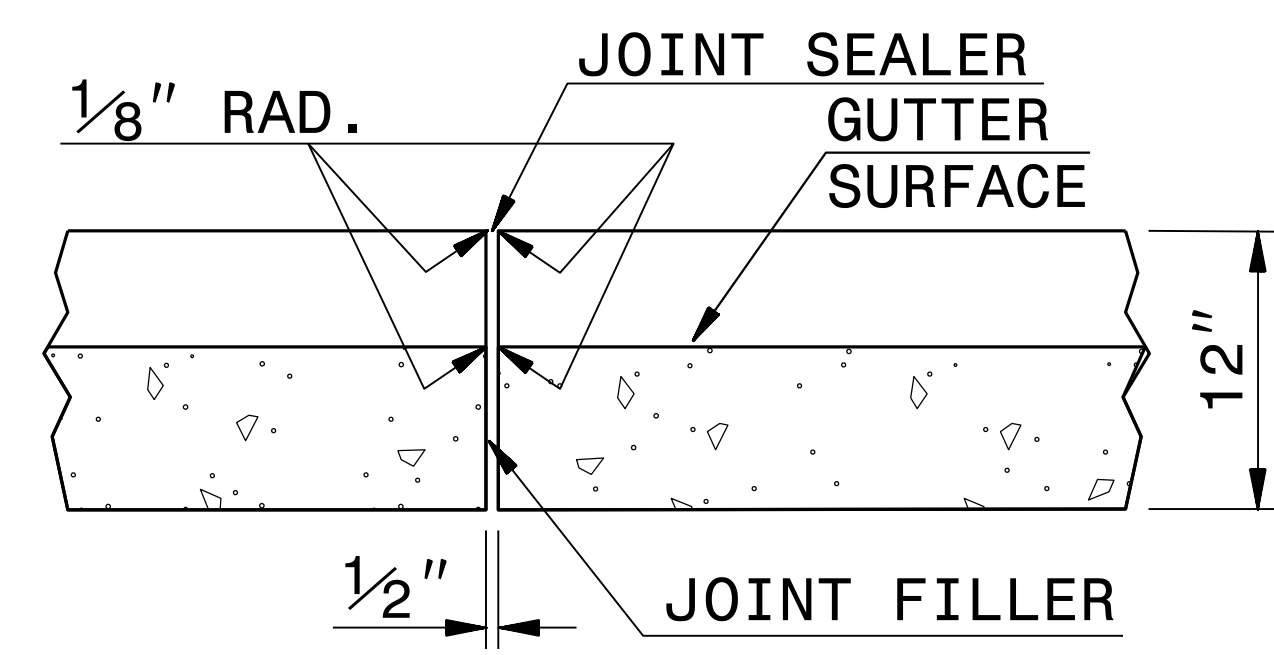


2'-9" CURB AND GUTTER

SECTION VIEW OF CURB AND GUTTER



LONGITUDINAL JOINT



**TRANSVERSE EXPANSION JOINT
IN CURB AND GUTTER**

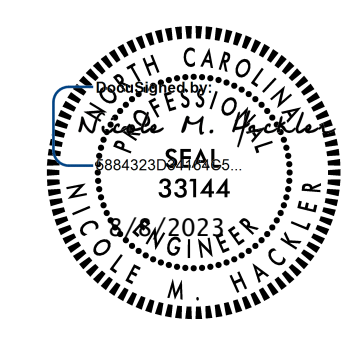
SECTION VIEW OF JOINTS

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RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
2'-9" CONCRETE CURB & GUTTER

SHEET 1 OF 1
846D01

J:\AUG-2017\1146\Projects\Stand\stand\c&g2'-9.dgn

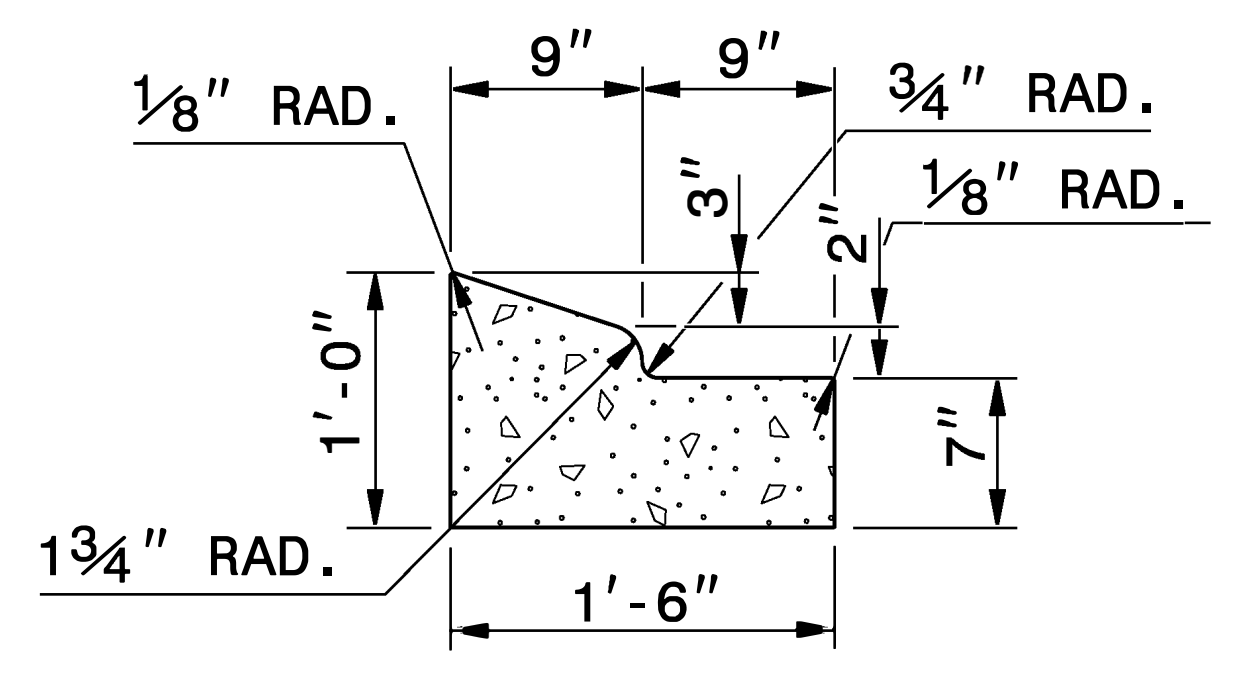


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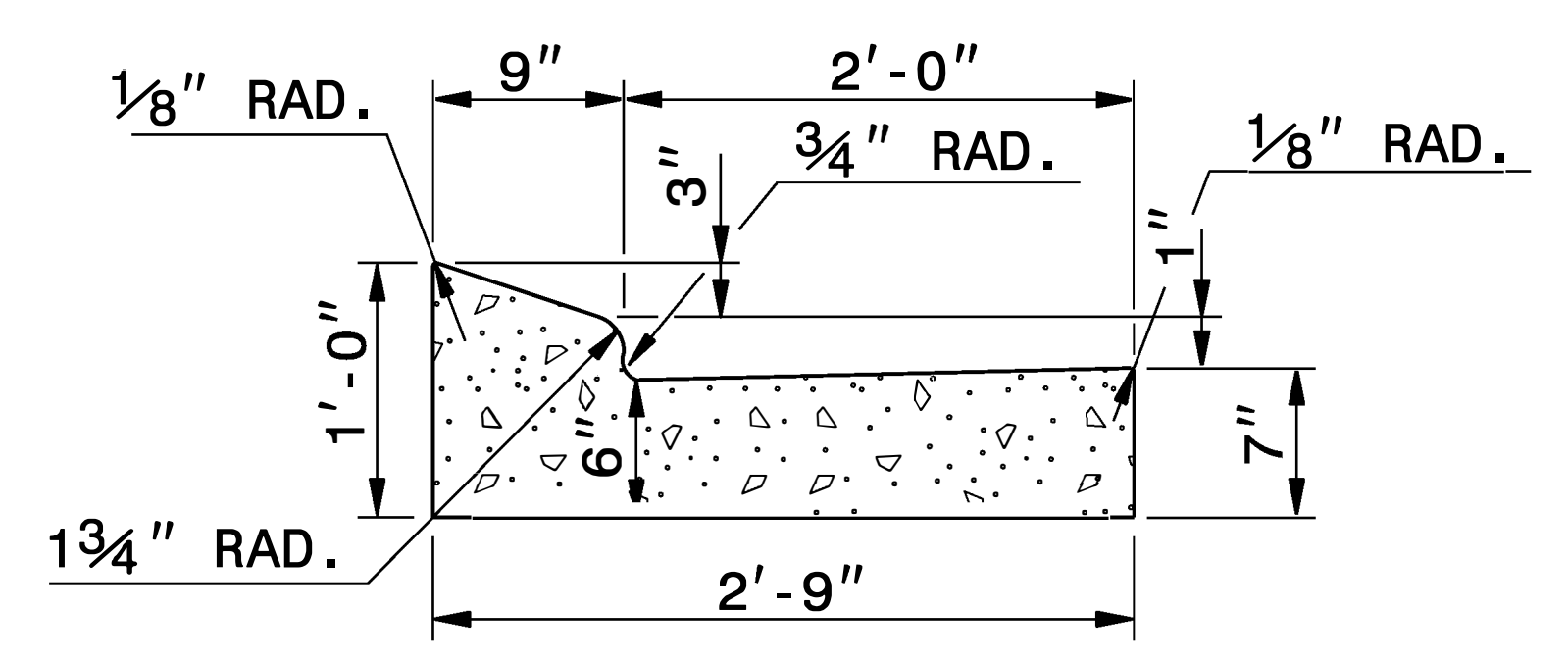
SEE PLATE FOR TITLE

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 MODIFIED BY: E.E. WARD DATE: 8-15-00
 CHECKED BY: _____ DATE: _____
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DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



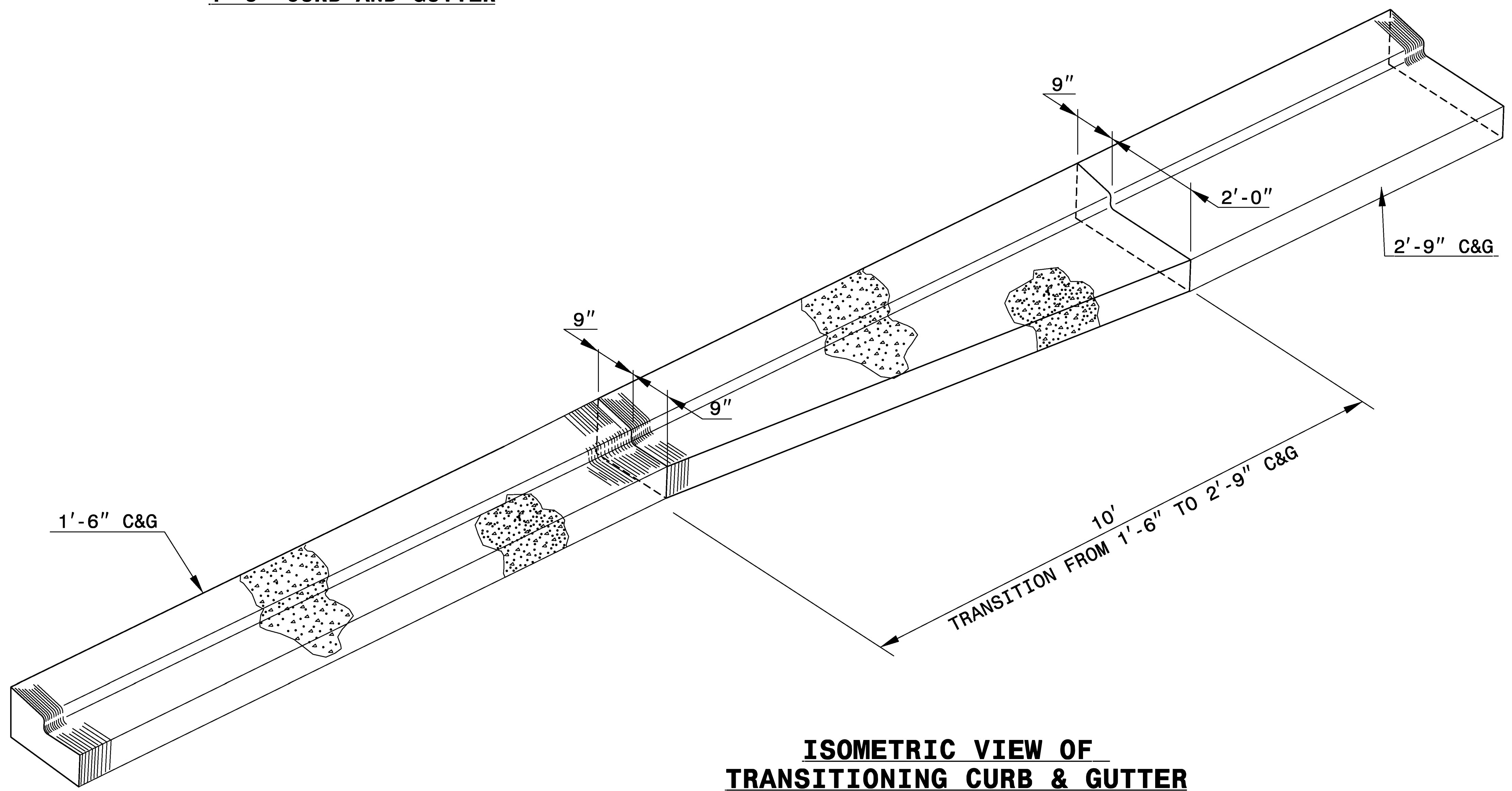
1'-6" CURB AND GUTTER



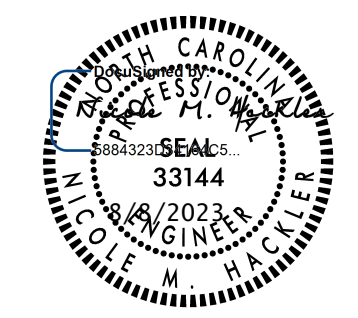
2'-9" CURB AND GUTTER

NOTE: SEE STD. DWG. 846.01 FOR ADDITIONAL CURB AND GUTTER INFORMATION.

SEE ROADWAY PLANS FOR LOCATION OF CURB TRANSITION.



**ISOMETRIC VIEW OF
TRANSITIONING CURB & GUTTER**



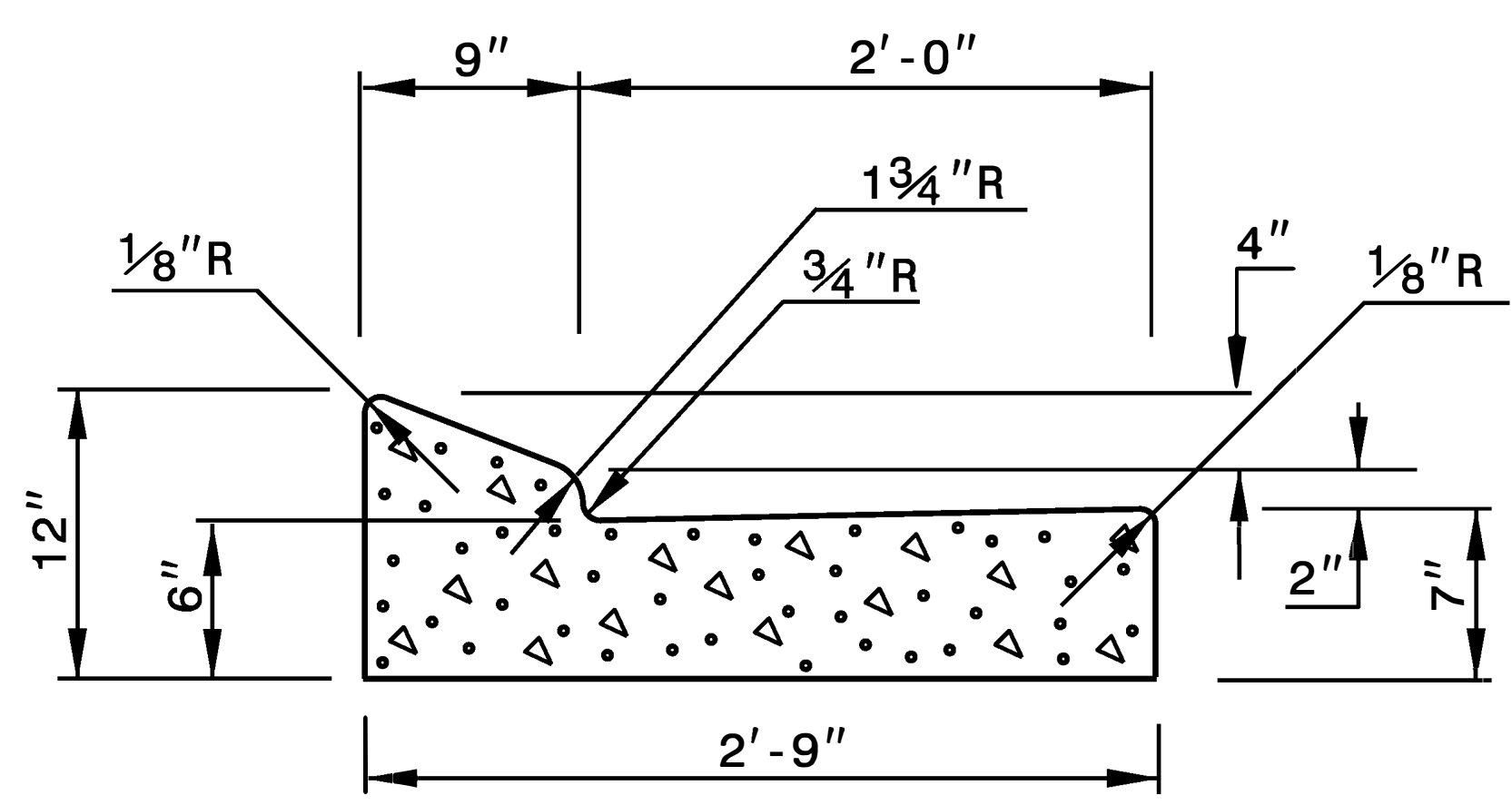
DOCUMENT NOT CONSIDERED FINAL
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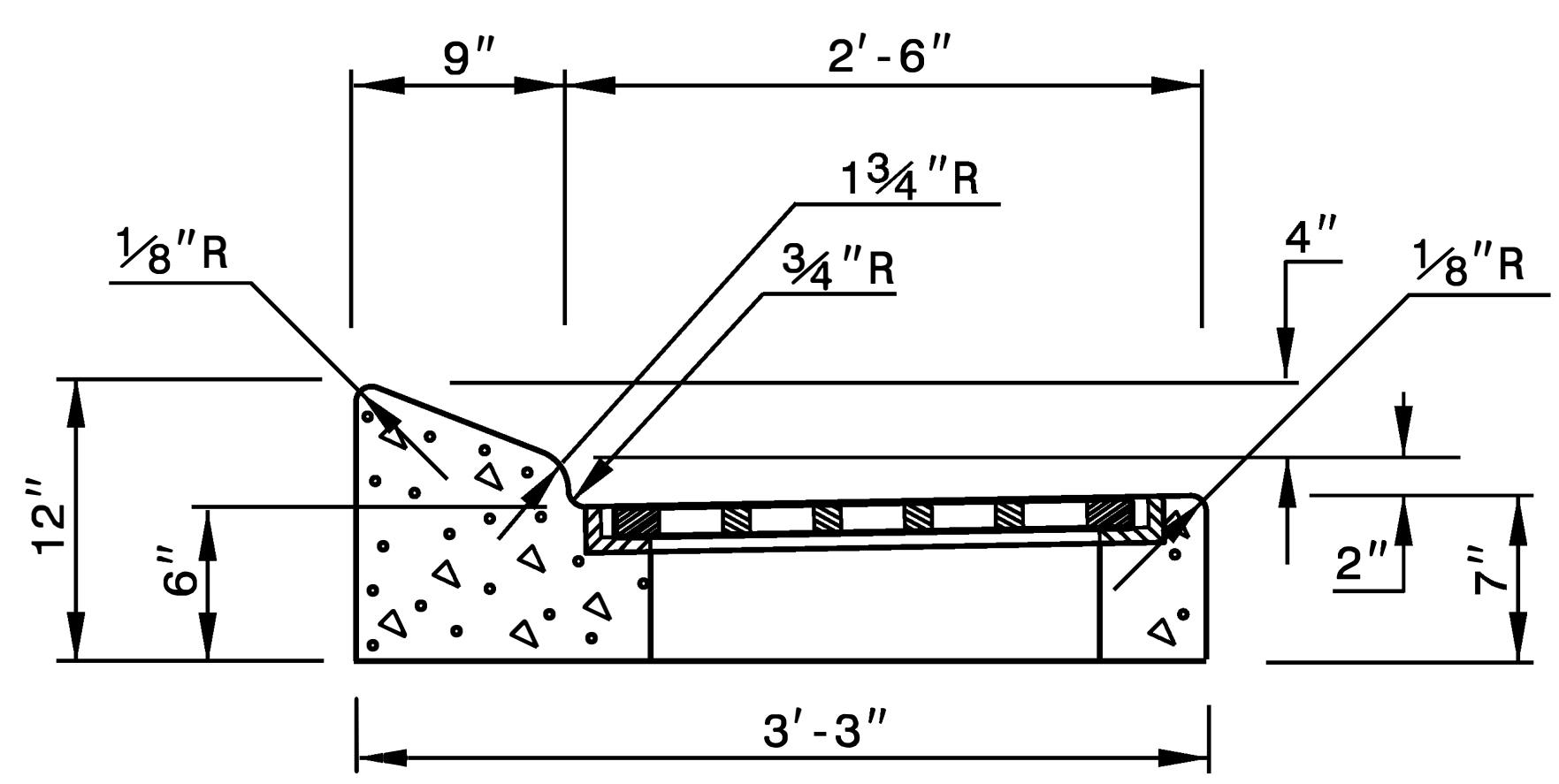
**DETAIL OF 1'-6"
TO 2'-9" CURB & GUTTER
TRANSITION SECTION**

ORIGINAL BY: T.S. SPELL DATE: NOV. 26, 2001
 MODIFIED BY: T.S. SPELL DATE: JAN. 23, 2007
 CHECKED BY: DATE:
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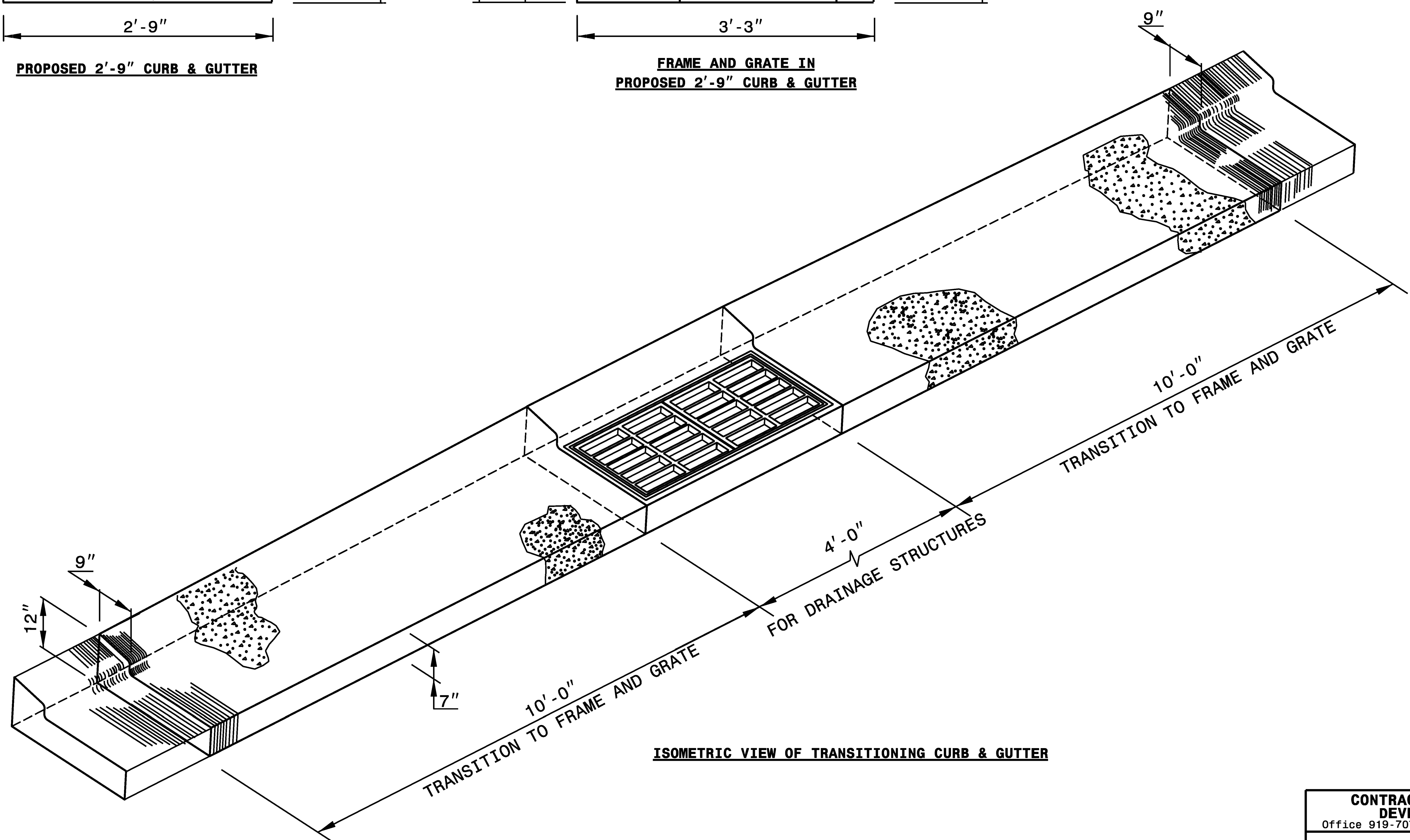
5/14/99



PROPOSED 2'-9" CURB & GUTTER



FRAME AND GRATE IN PROPOSED 2'-9" CURB & GUTTER



ISOMETRIC VIEW OF TRANSITIONING CURB & GUTTER



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DEVELOPMENT UNIT**
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**DETAIL OF 2'-9"
TO FRAME AND GRATE**

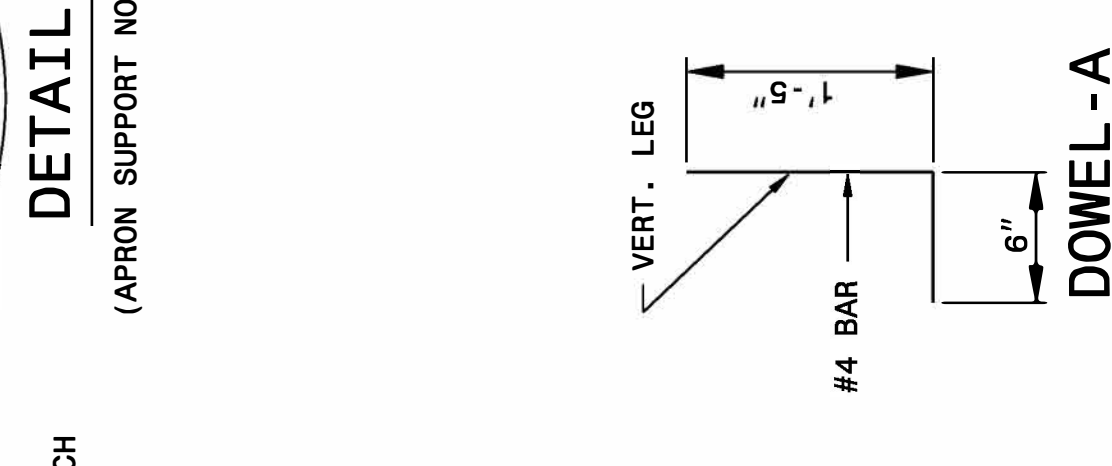
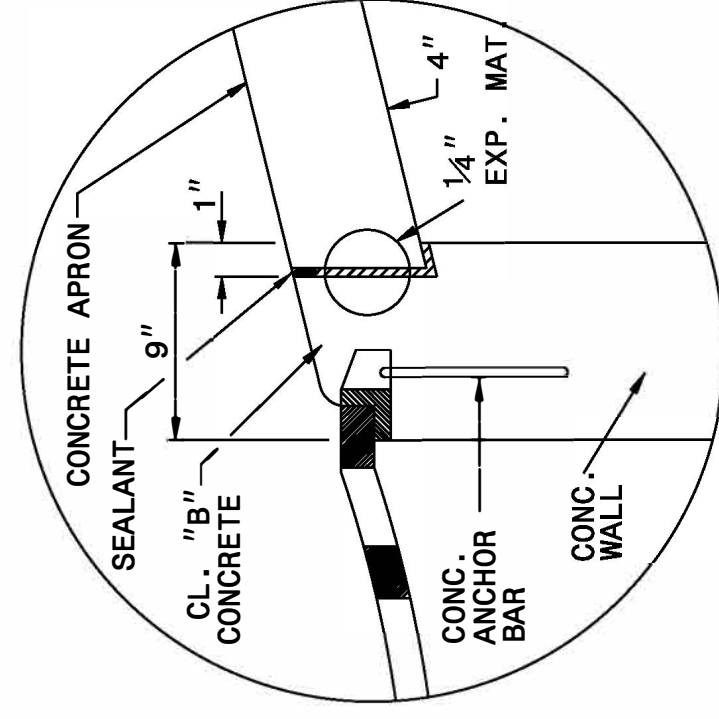
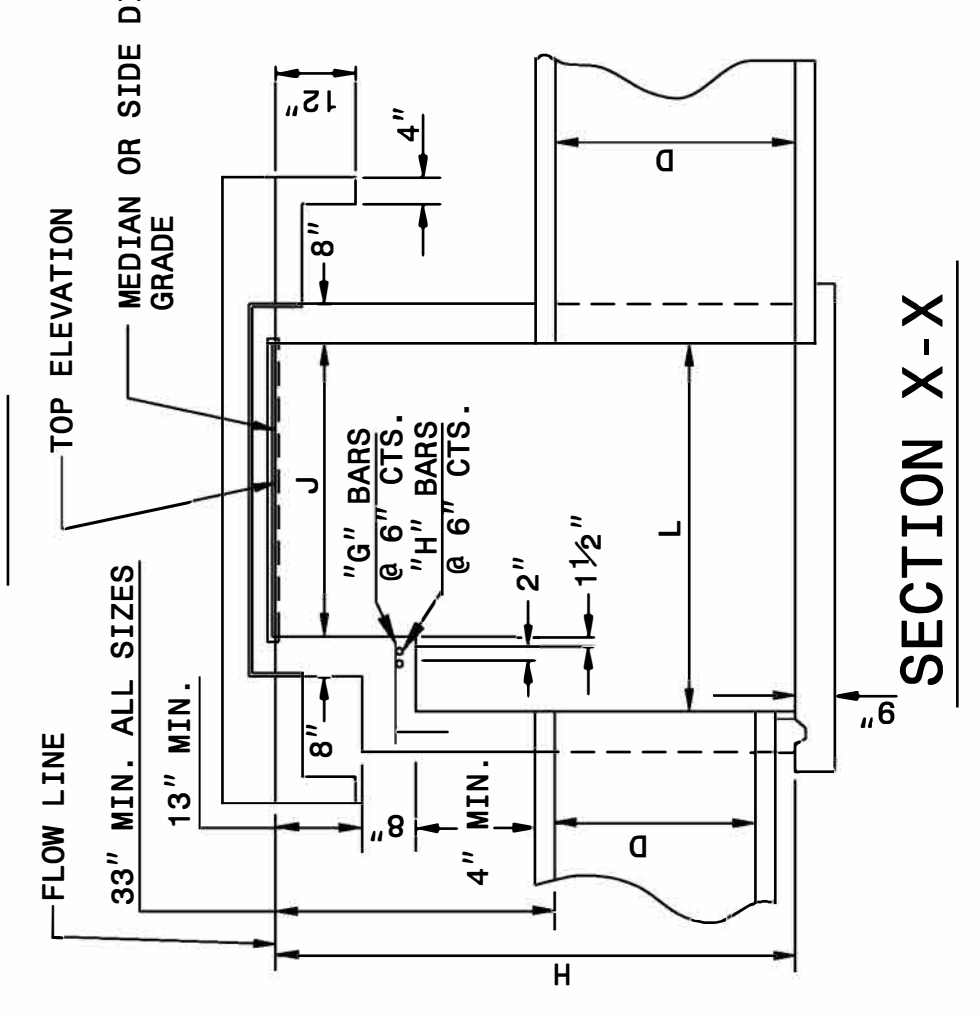
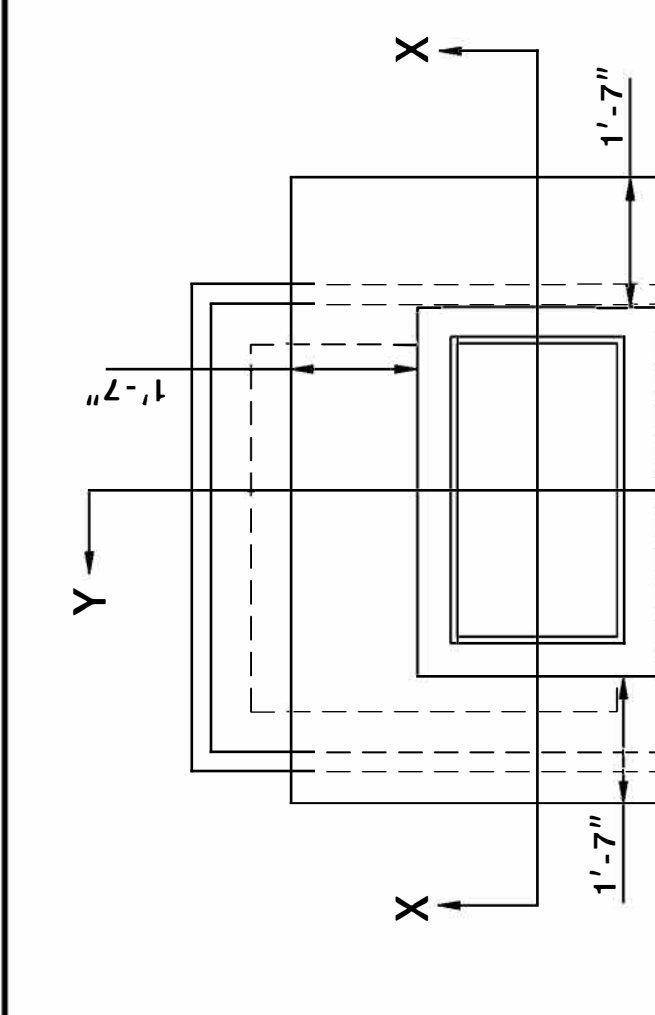
ORIGINAL BY: _____	DATE: _____
MODIFIED BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____
FILE SPEC.: <u>kkempf/english/curb gutter transition.dgn</u>	

01-MAR-2018 07:33
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 J:\overton AT CSU-252595

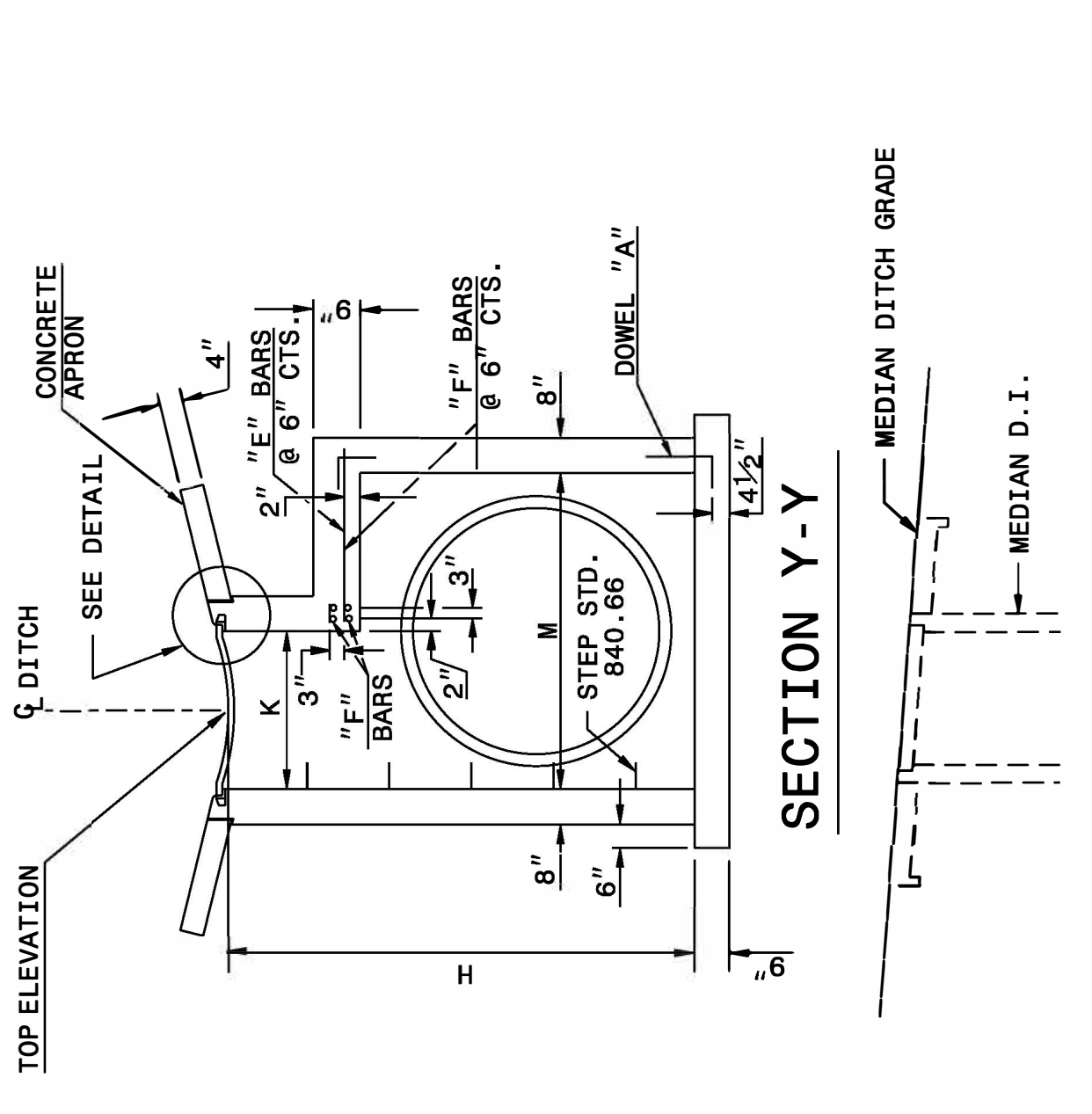
STATE OF
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DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
CONCRETE MEDIAN DROP INLET TYPE 'A'
EXTRA DEPTH OVER 12' TO 25'
12" THRU 72" PIPE

SHEET 1 OF 2
840D17



GENERAL NOTES:
 USE CLASS "B" CONCRETE THROUGHOUT.
 PROVIDE DROP INLETS WITH STEPS 12" ON CENTER. USE STEPS WHICH COMPLY WITH STD. DRAWING 840.66.
 OPTIONAL CONSTRUCTION - MONOLITHIC FOUR, 2" KEYWAY, OR #4 BAR DOWELS AT 12" CENTERS AS DIRECTED BY THE ENGINEER.
 USE FORMS FOR THE CONSTRUCTION OF THE BOTTOM SLAB.
 IF REINFORCED CONCRETE PIPE IS SET IN BOTTOM SLAB OF BOX, ADD TO SLAB WHEN PAYMENT FOR THE DROP INLET IS MADE ON A PER EACH BASTS, THE CONCRETE APRON WILL BE CONSIDERED PART OF THE DROP INLET.
 CONSTRUCT WITH PIPE CROWNS MATCHING.
 USE STANDARD FRAMES AND GRATES 840.22 (SHOWN), 840.24 (SHOWN), 840.20, 840.29, AND 840.35.
 SEE STANDARD DRAWING 840.25 FOR ATTACHMENT OF FRAMES AND GRATES NOT SHOWN.
 CHAMFER ALL EXPOSED CORNERS 1".
 DRAWING NOT TO SCALE.
 MAX. DEPTH OF THIS STRUCTURE FROM TOP OF BOTTOM SLAB TO TOP ELEVATION IS 25 FEET.



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RALEIGH, N.C.

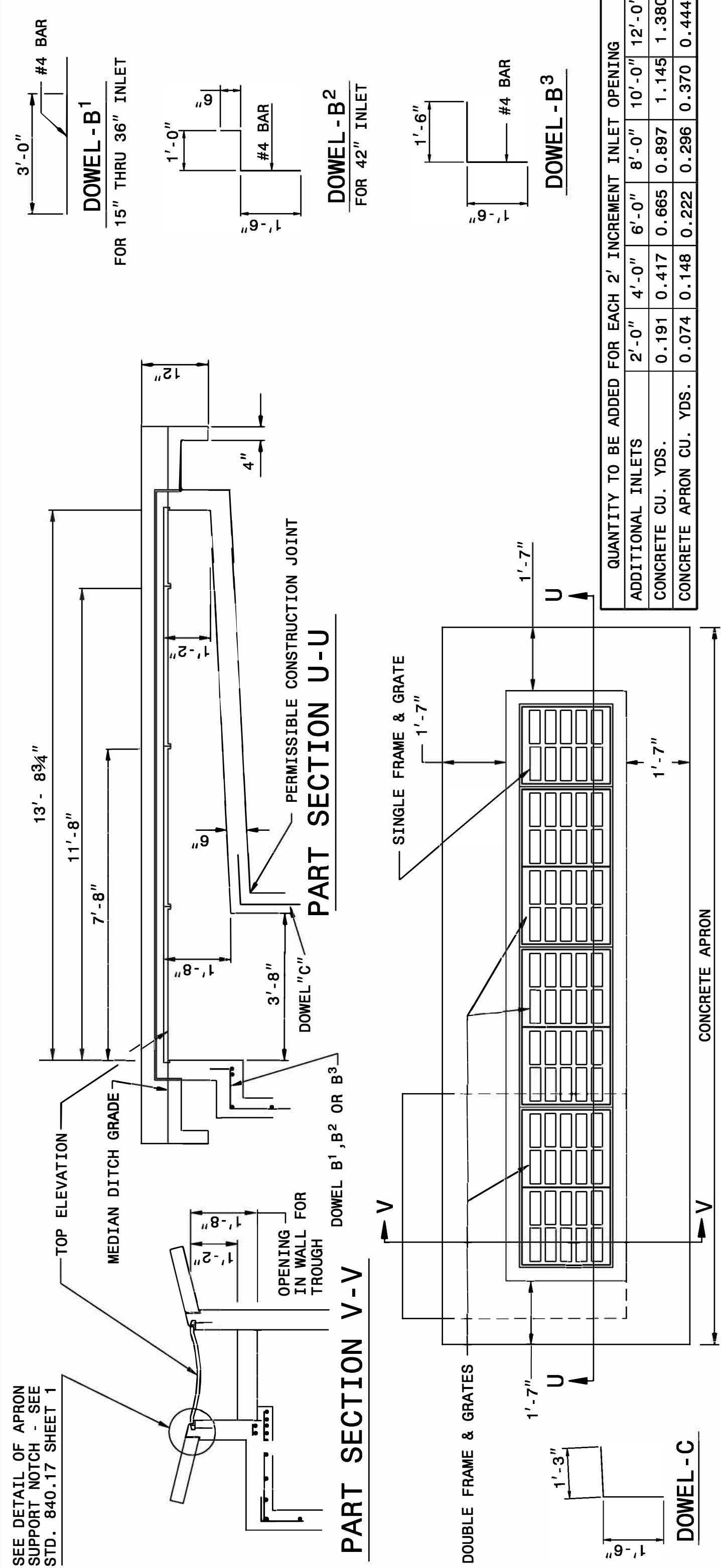
ENGLISH DETAIL DRAWING FOR
CONCRETE MEDIAN DROP INLET TYPE 'A'
EXTRA DEPTH OVER 12' TO 25'
12" THRU 72" PIPE

SHEET 1 OF 2
840D17

STATE OF
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DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
CONCRETE MEDIAN DROP INLET TYPE 'A'
EXTRA DEPTH OVER 12' TO 25'
12" THRU 72" PIPE

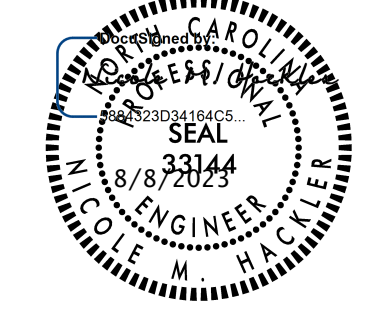
SHEET 2 OF 2
840D17



QUANTITY TO BE ADDED FOR EACH 2' INCREMENT INLET OPENING

ADDITIONAL INLETS	2'-0"	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"
CONCRETE CU. YDS.	0.191	0.417	0.665	0.897	1.145	1.380
CONCRETE APRON CU. YDS.	0.074	0.148	0.222	0.296	0.370	0.444

PIPE DIAMETER	DIMENSIONS OF BOX AND PIPE		REINFORCING STEEL - NO. 4 BARS															TOTAL		DEDUCTIONS FOR ONE PIPE				
	SPAN	WIDTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	H PER FT	APRON TOTAL	C.S.	R.C.		
12"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	0.926	0.247	0.395	1.683	0.015	0.024
15"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	0.988	0.247	0.395	1.745	0.023	0.036
18"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	1.050	0.247	0.395	1.807	0.033	0.049
24"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	1.362	0.278	0.444	2.201	0.069	0.085
30"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	1.644	0.288	0.502	2.541	0.092	0.127
36"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	1.931	0.321	0.560	2.920	0.132	0.178
42"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	2.500	0.370	0.704	3.677	0.180	0.243
48"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3.013	0.407	0.823	4.315	0.235	0.317
54"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3.589	0.444	0.951	5.072	0.297	0.401
60"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	4.539	0.494	1.311	6.170	0.367	0.495
66"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	5.061	0.537	1.136	6.901	0.444	0.599
72"	3'-8"	2'-0"	3	3'-8"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	3	3'-9"	5.860	0.560	1.500	7.868	0.528	0.713



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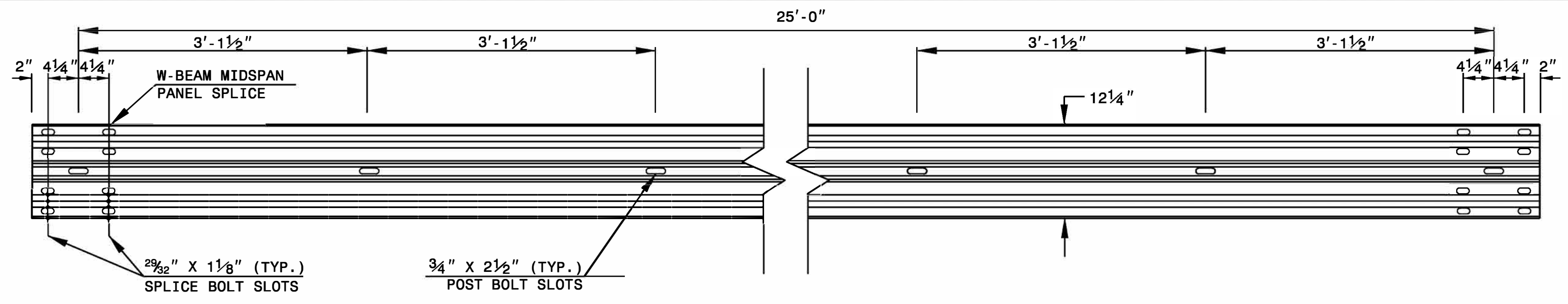
CONTRACT STANDARDS AND DEVELOPMENT UNIT
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SEE PLATE FOR TITLE

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 MODIFIED BY: K.A. KEMPF DATE: 07-06-09
 CHECKED BY: _____ DATE: _____
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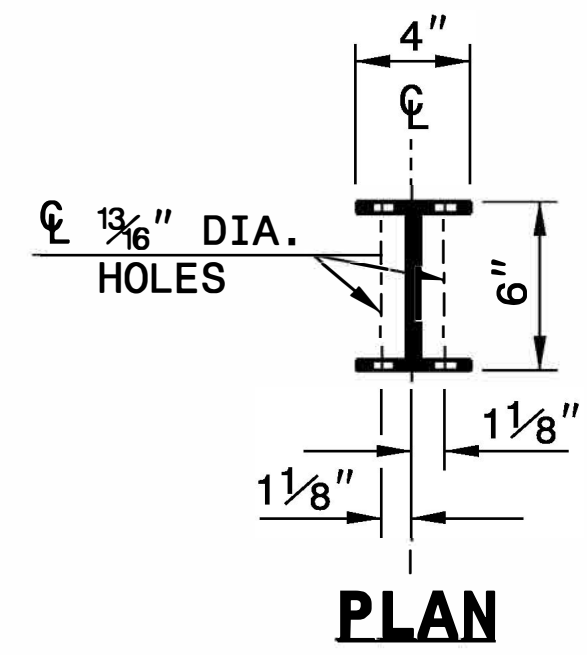
STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
GUARDRAIL INSTALLATION

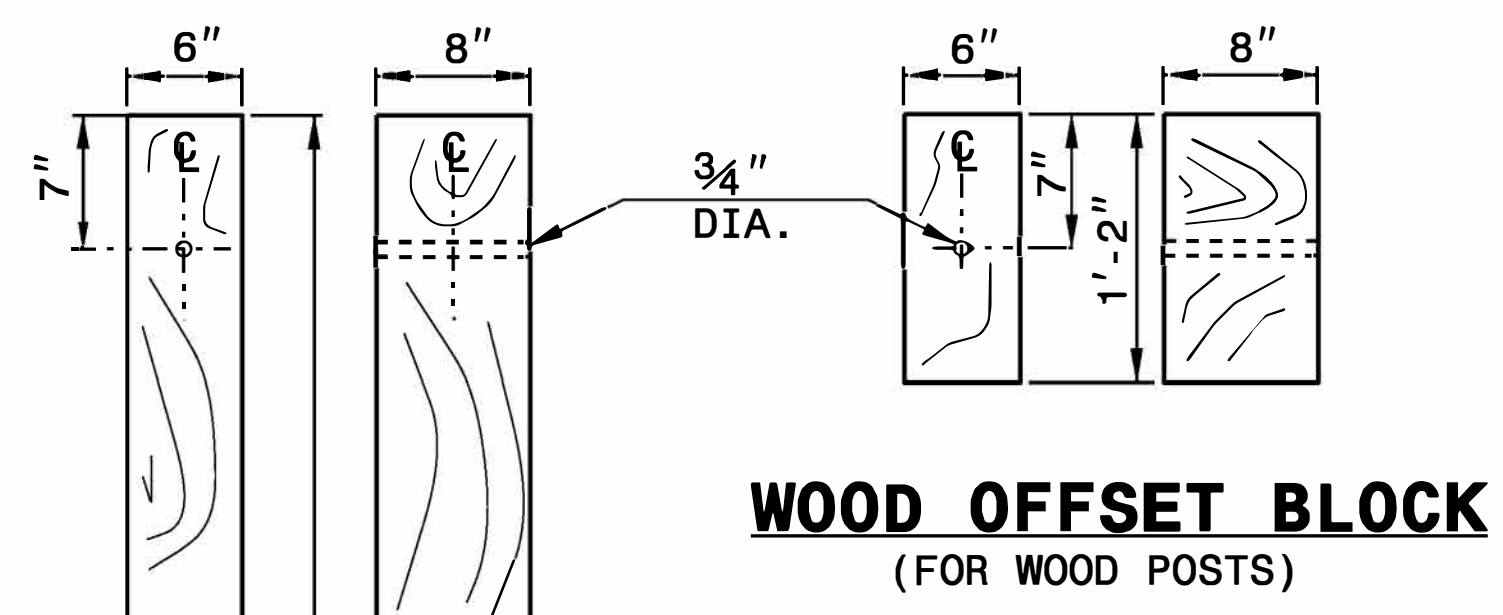
SHEET 6 OF 8
862D02



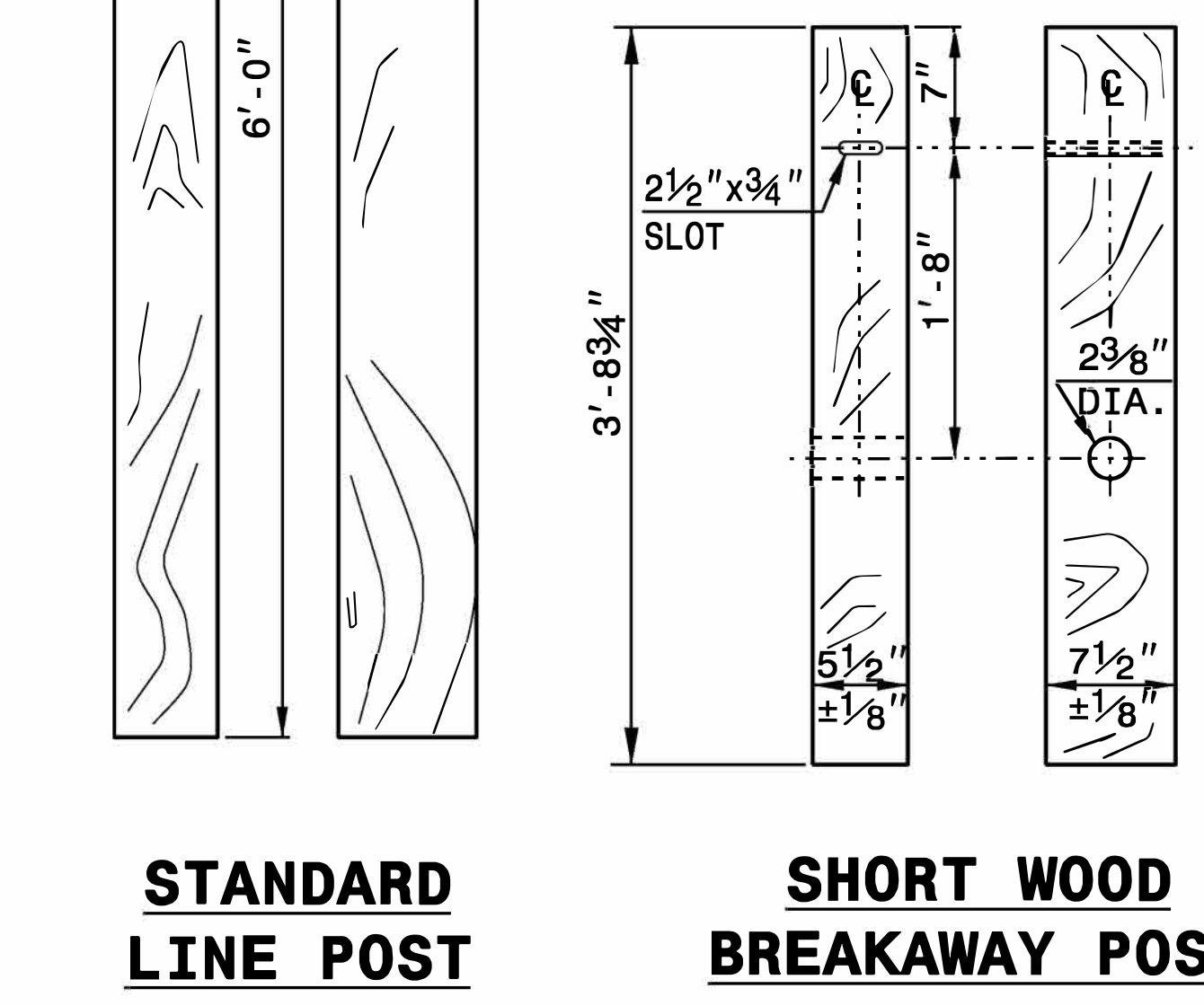
STANDARD W-BEAM GUARDRAIL



PLAN

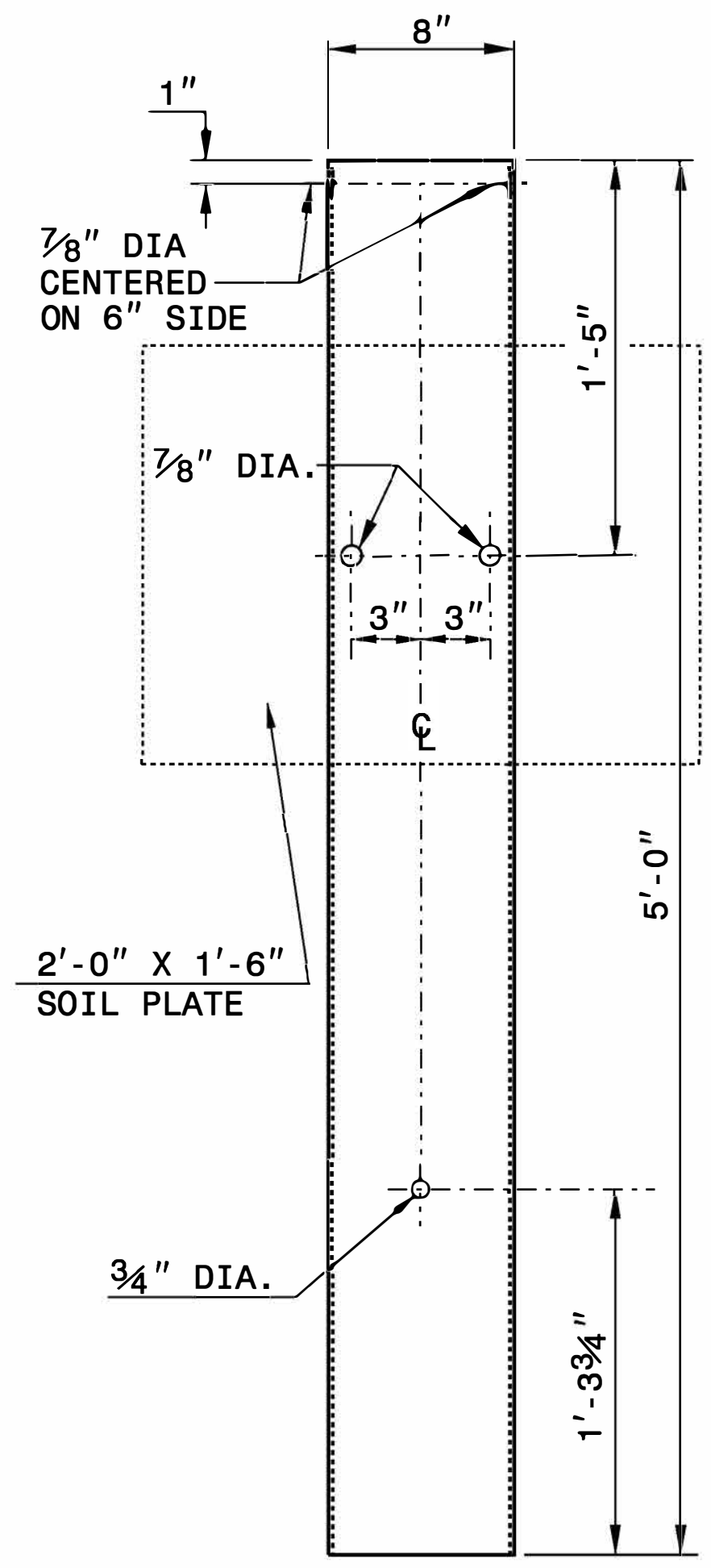


**WOOD OFFSET BLOCK
(FOR WOOD POSTS)**

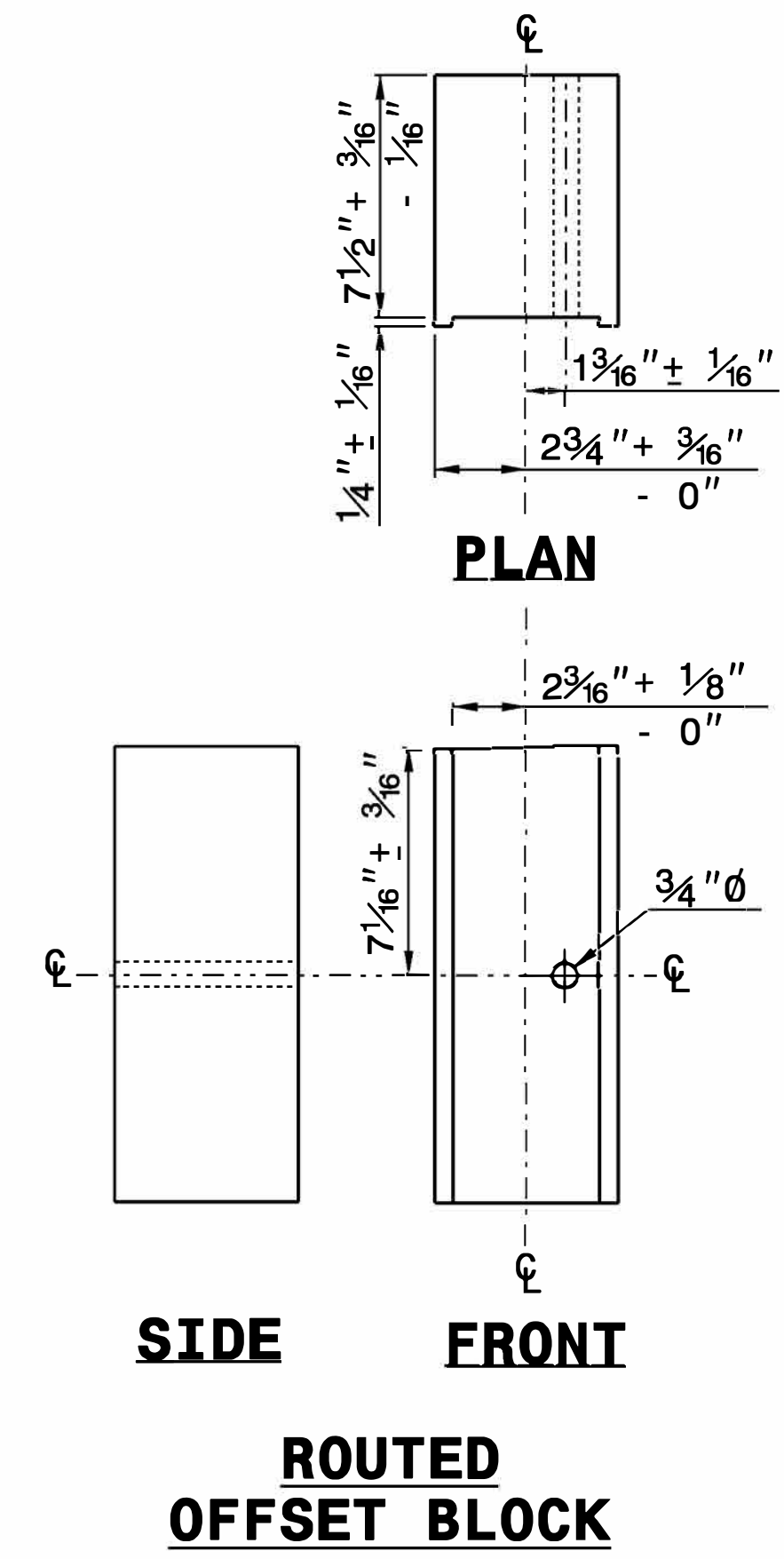


**STANDARD
LINE POST**

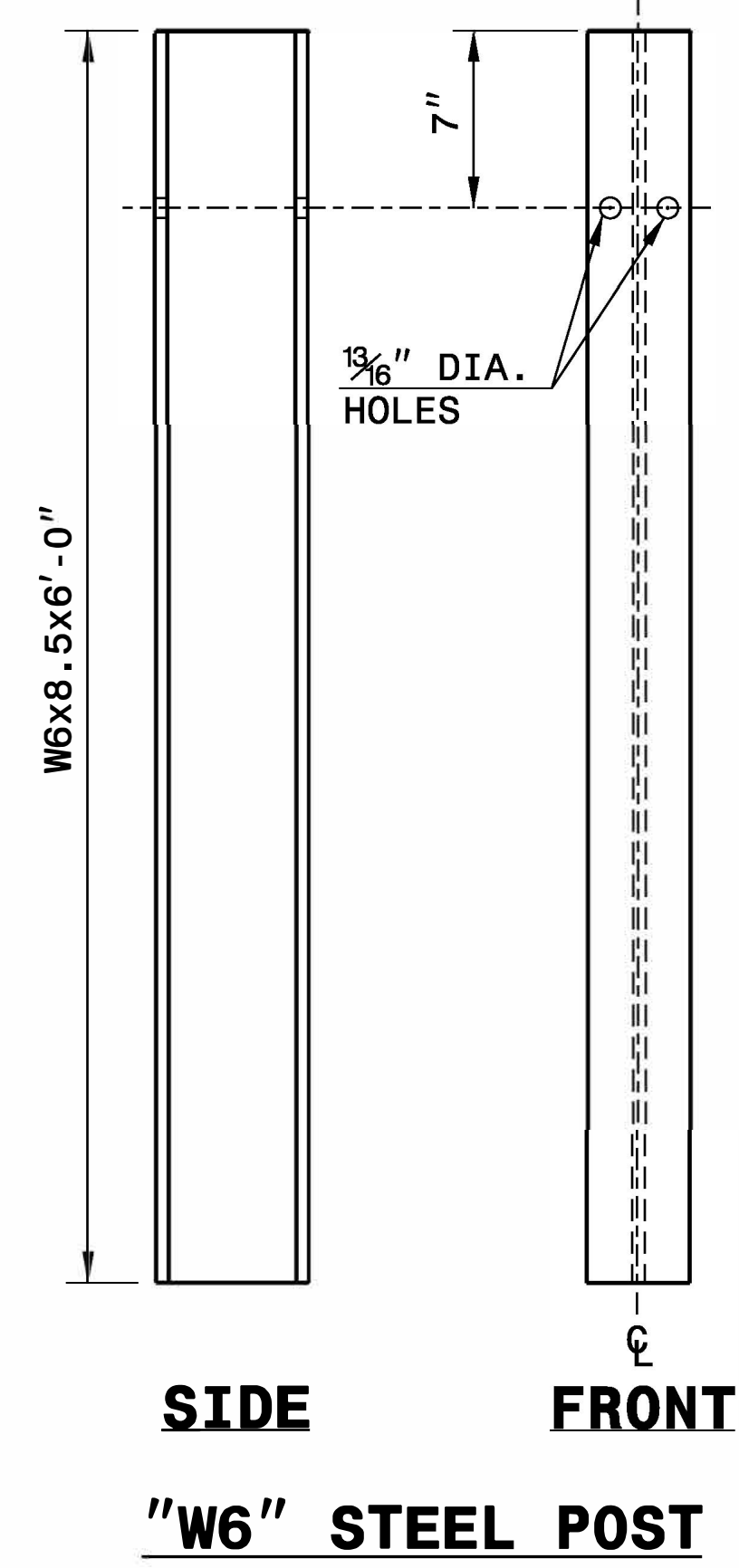
**SHORT WOOD
BREAKAWAY POST**



**STEEL TUBE
TS 6"x8"x0.1875"**



**ROUTED
OFFSET BLOCK**



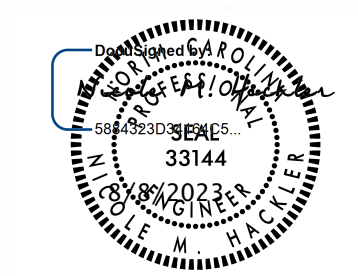
"W6" STEEL POST

SYSTEM PARTS

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DEPT. OF TRANSPORTATION
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RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
GUARDRAIL INSTALLATION

SHEET 6 OF 8
862D02

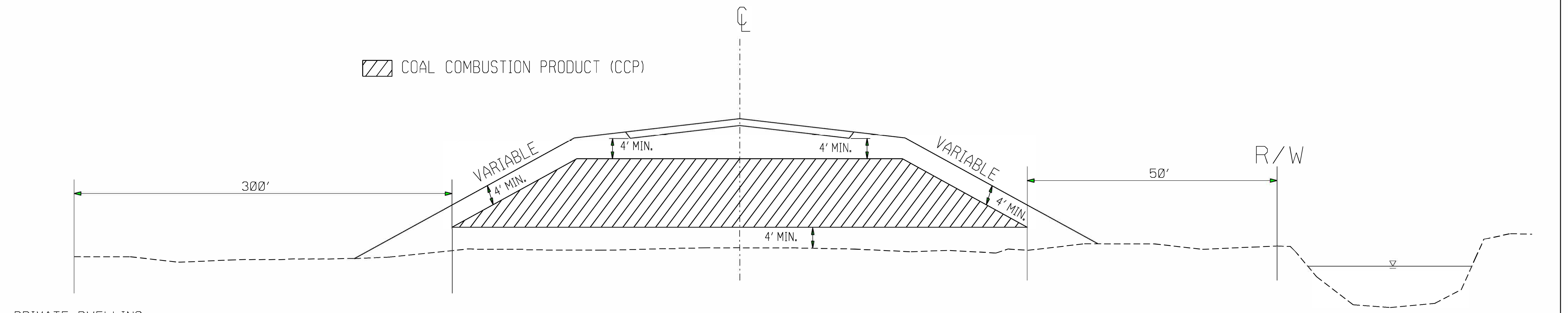


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ORIGINAL BY: J. HOWERTON DATE: 3-7-2018
MODIFIED BY: DATE: _____
CHECKED BY: DATE: _____
FILE SPEC.: _____

COAL COMBUSTION PRODUCT PLACEMENT



PRIVATE DWELLING OR WELL

PERENNIAL STREAM, OTHER SURFACE WATER BODY OR *WETLAND

*(OBTAIN PERMISSION FROM ARMY CORPS OF ENGINEERS)

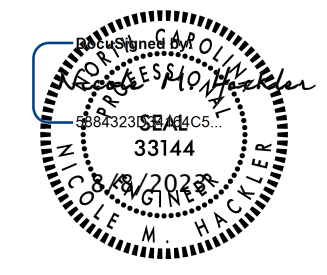
PLACE CCP IN HATCHED AREA IN ACCORDANCE WITH THE PROJECT SPECIAL PROVISIONS

PLACE CCP A MINIMUM OF 5' ABOVE SEASONAL HIGH GROUND WATER

PLACE AT LOCATIONS AS APPROVED BY THE ENGINEER

PLACE SOIL BORROW MATERIAL ON THE OUTSIDE OF CCP AS EACH LIFT OF CCP IS PLACED

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



CONTRACT STANDARDS AND DEVELOPMENT UNIT	
Office 919-707-6950 FAX 919-250-4119	
COAL COMBUSTION PRODUCT PLACEMENT DETAIL	
ORIGINAL BY: J.S.H.	DATE: 3/16/15
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC: joel/coal combustion material detail.dgn	

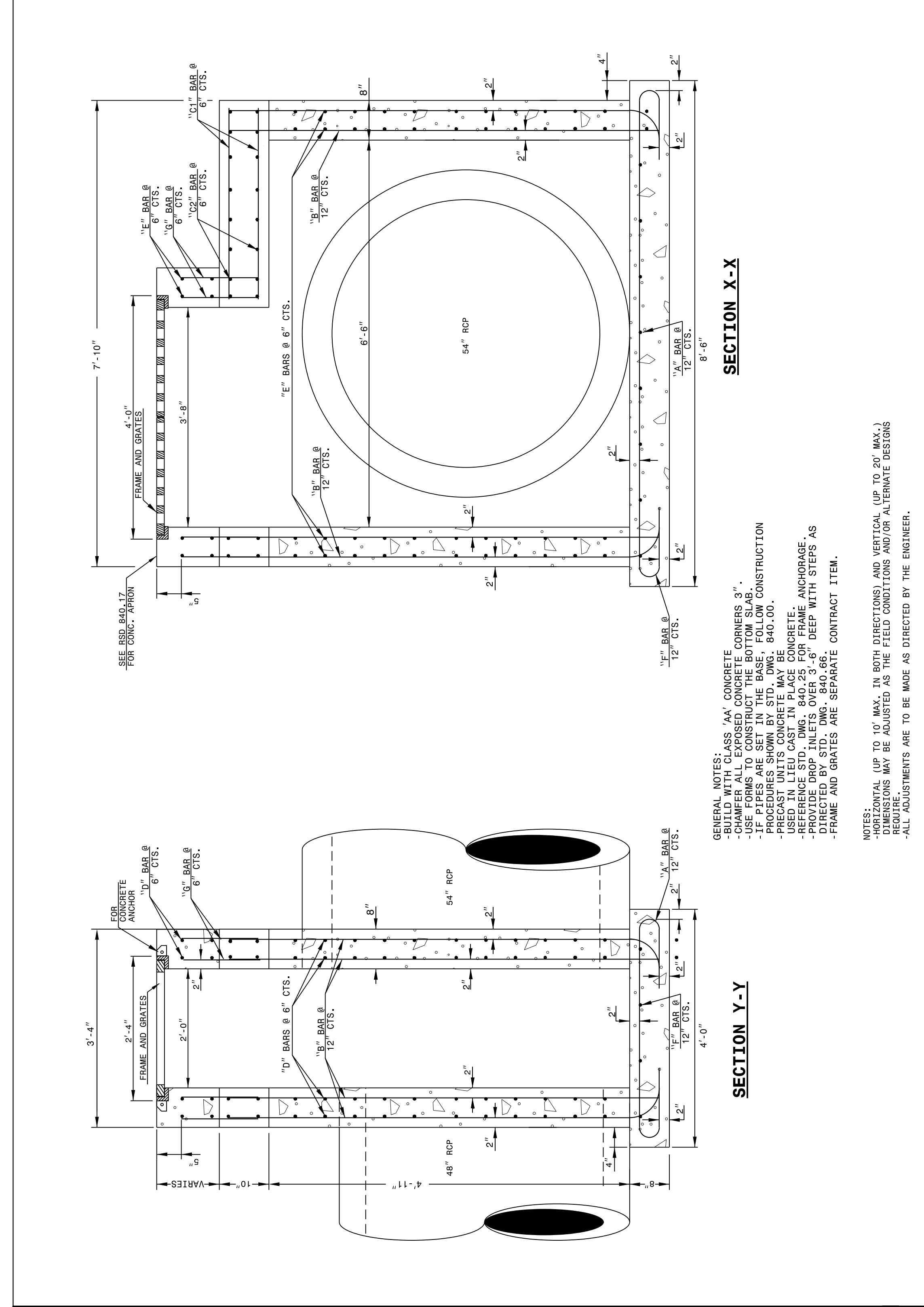
07-SEP-2017 08:21 S:\Contracts\Special Details\Jhewerton\Coal Combustion Product Detail.dgn Jhewerton AT CSU-292895

I3-AUG-2018 09:00 S:\Contracts\Contractors\Special Details\Jhewerton\840d35 TB01 Up to 54in.dgn Jhewerton AT USD-292595

STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR TRAFFIC BEARING GRATED INLET FOR PIPES UP TO 54"

SHEET 1 OF 2
840D35



STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

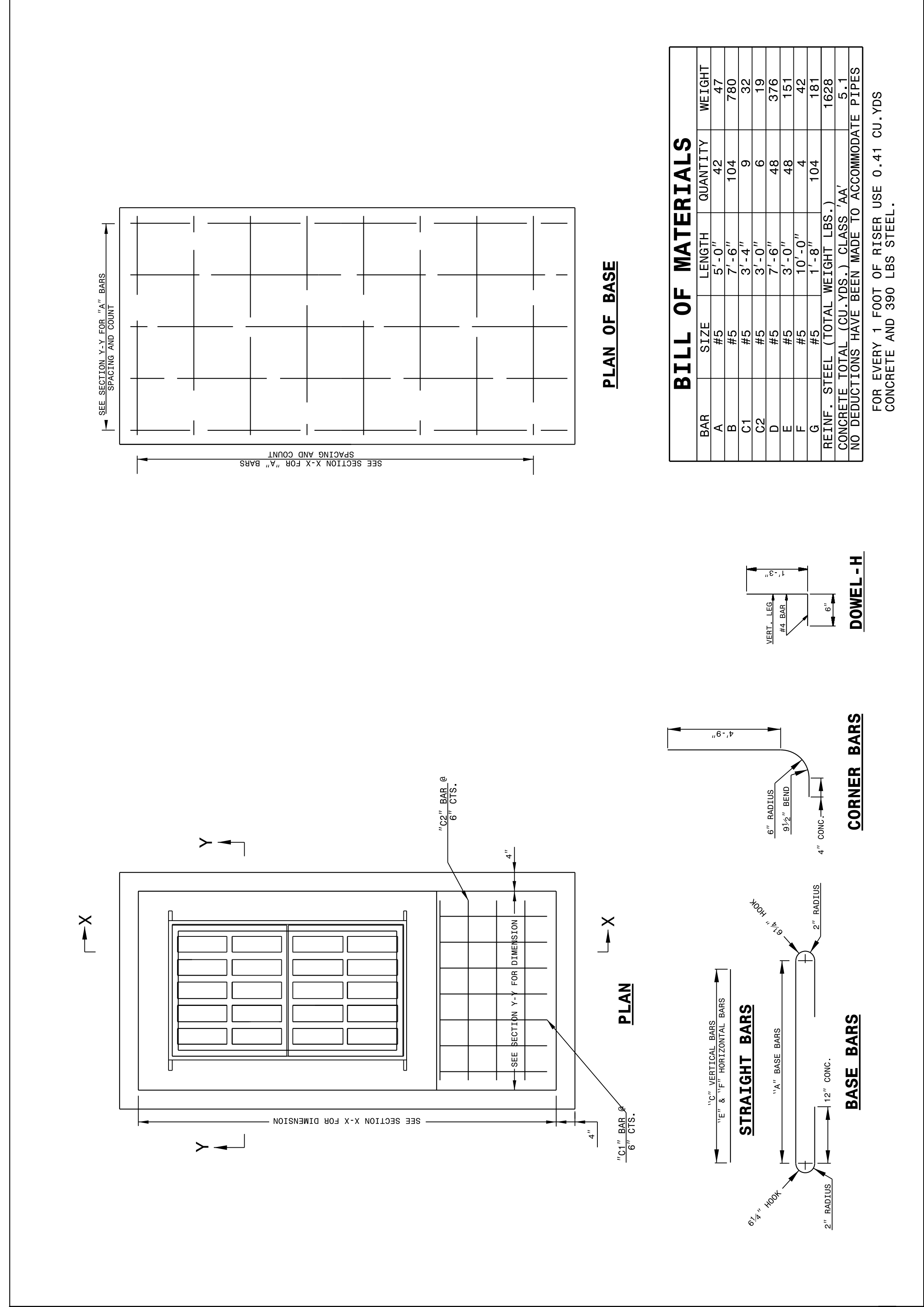
ENGLISH DETAIL DRAWING FOR TRAFFIC BEARING GRATED INLET FOR PIPES UP TO 54"

SHEET 1 OF 2
840D35

STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR TRAFFIC BEARING GRATED INLET FOR PIPES UP TO 54"

SHEET 2 OF 2
840D35



STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR TRAFFIC BEARING GRATED INLET FOR PIPES UP TO 54"

SHEET 2 OF 2
840D35

SECTION X-X

GENERAL NOTES:
 -BUILD WITH CLASS 'AA' CONCRETE
 -CHAMFER ALL EXPOSED CONCRETE CORNERS 3".
 -USE FORMS TO CONSTRUCT THE BOTTOM SLAB.
 -PIPE ANCHORS IN THE BASE, FOLLOW CONSTRUCTION PRACTICES SHOWN IN THE DRAWING.
 -PRECAST UNITS CONCRETE MAY BE USED IN LIEU CAST IN PLACE CONCRETE.
 -REFERENCE STD. DWG. 840.25 FOR FRAME ANCHORAGE.
 -FRAME AND GRATES ARE SEPARATE CONTRACT ITEM.
 -FRAME AND GRATES ARE SEPARATE CONTRACT ITEM.

NOTES:
 -HORIZONTAL (UP TO 10' MAX. IN BOTH DIRECTIONS) AND VERTICAL (UP TO 20' MAX.) DIMENSIONS MAY BE ADJUSTED AS THE FIELD CONDITIONS AND/OR ALTERNATE DESIGNS REQUIRE.
 -ALL ADJUSTMENTS ARE TO BE MADE AS DIRECTED BY THE ENGINEER.

SECTION Y-Y

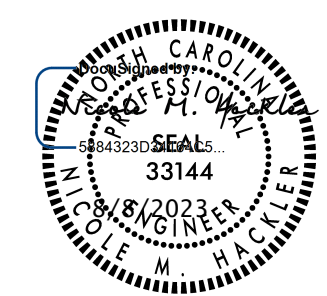
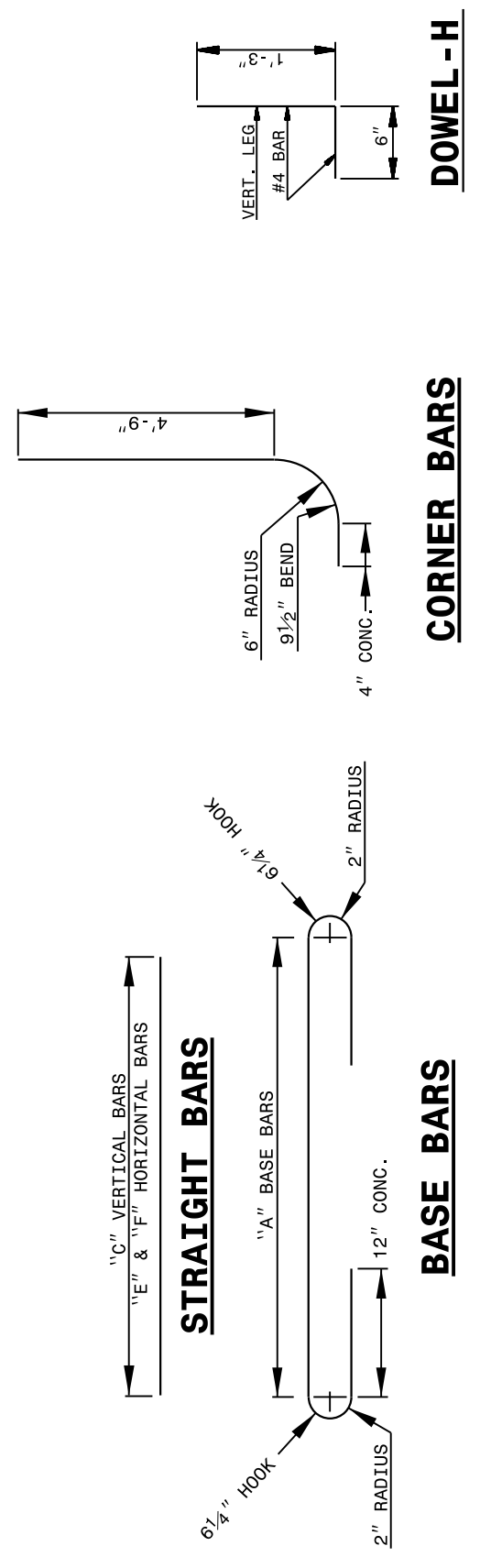
GENERAL NOTES:
 -BUILD WITH CLASS 'AA' CONCRETE
 -CHAMFER ALL EXPOSED CONCRETE CORNERS 3".
 -USE FORMS TO CONSTRUCT THE BOTTOM SLAB.
 -PIPE ANCHORS IN THE BASE, FOLLOW CONSTRUCTION PRACTICES SHOWN IN THE DRAWING.
 -PRECAST UNITS CONCRETE MAY BE USED IN LIEU CAST IN PLACE CONCRETE.
 -REFERENCE STD. DWG. 840.25 FOR FRAME ANCHORAGE.
 -FRAME AND GRATES ARE SEPARATE CONTRACT ITEM.
 -FRAME AND GRATES ARE SEPARATE CONTRACT ITEM.

NOTES:
 -HORIZONTAL (UP TO 10' MAX. IN BOTH DIRECTIONS) AND VERTICAL (UP TO 20' MAX.) DIMENSIONS MAY BE ADJUSTED AS THE FIELD CONDITIONS AND/OR ALTERNATE DESIGNS REQUIRE.
 -ALL ADJUSTMENTS ARE TO BE MADE AS DIRECTED BY THE ENGINEER.

BILL OF MATERIALS

BAR	SIZE	LENGTH	QUANTITY	WEIGHT
A	#2	5'-0"	42	47
B	#2	7'-6"	104	790
C1	#3	3'-0"	9	32
C2	#3	3'-0"	6	19
D	#5	3'-6"	48	376
E	#5	3'-0"	48	151
F	#5	1'-0"	4	42
G	#5	1'-0"	104	181
REFIN. STEEL (TOTAL WEIGHT LBS.)				1626
CONCRETE TOTAL (CU. YDS.) CLASS 'AA'				5.1
NO DEDUCTIONS HAVE BEEN MADE TO ACCOMMODATE PIPES				

FOR EVERY 1 FOOT OF RISER USE 0.41 CU. YDS CONCRETE AND 390 LBS STEEL.



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CONTRACT STANDARDS AND DEVELOPMENT UNIT
 Office 919-707-6950 FAX 919-250-4119

SEE PLATE FOR TITLE

ORIGINAL BY: K. KEMPF DATE: 03-03-2015
 MODIFIED BY: DATE:
 CHECKED BY: DATE:
 FILE SPEC.: jhewerton/840d35 TB01 Up to 54in.dgn

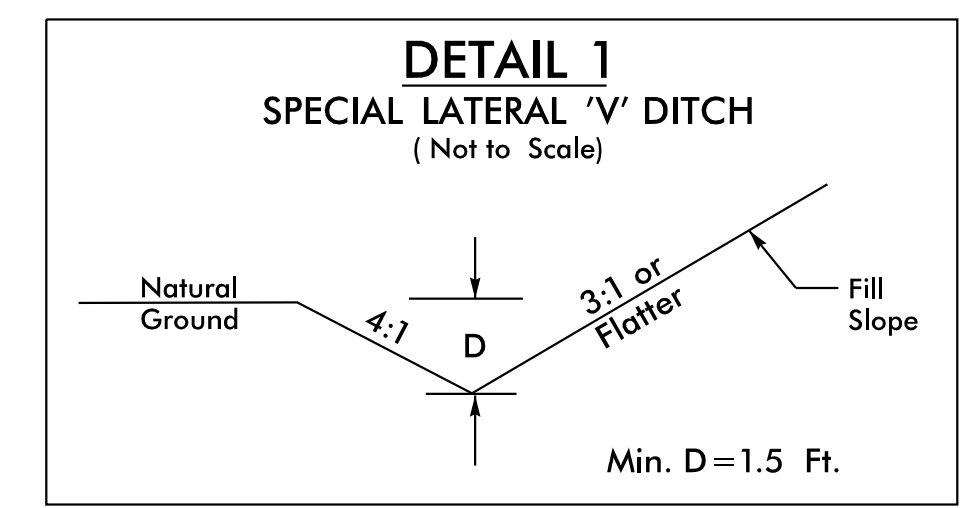
5/14/99

REVISIONS

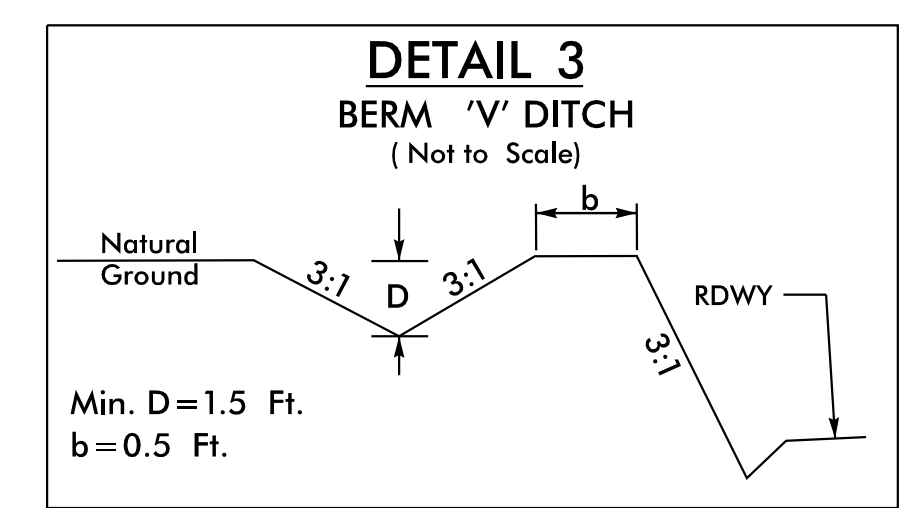
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P.O. BOX 33068 • RALEIGH, N.C. 27636-3068

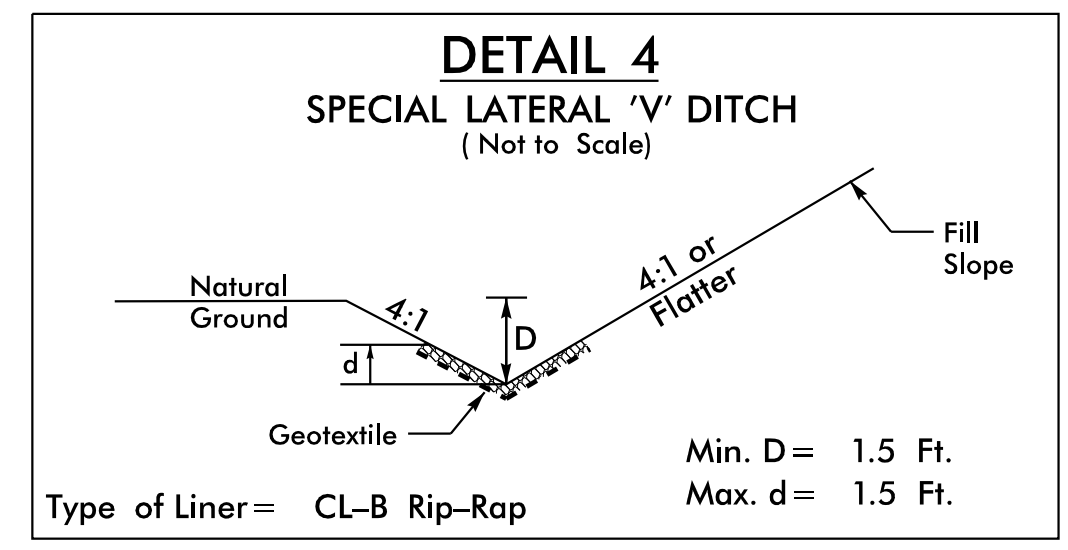
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2D-1
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



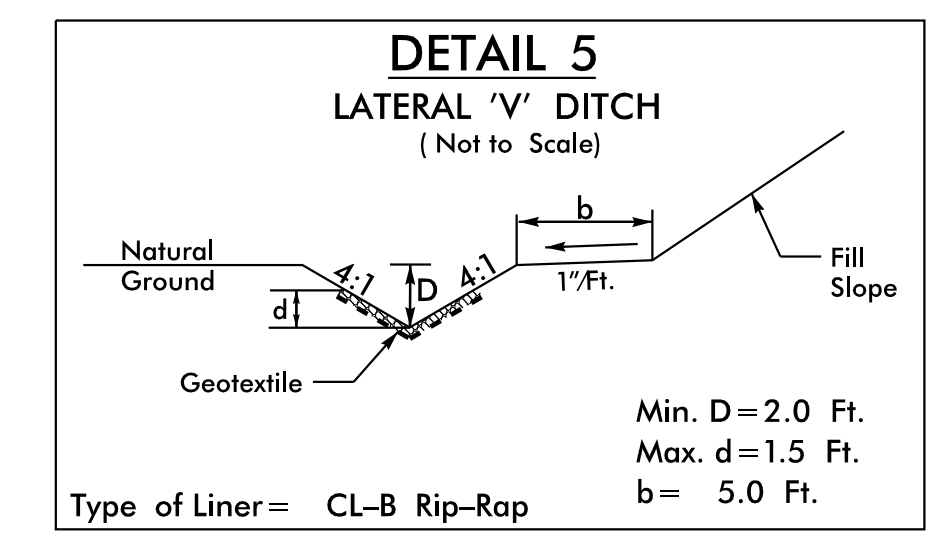
FROM STA. 21+00 TO STA. 23+00 -L- RT
 FROM STA. 26+85 TO STA. 28+75 -L- LT
 FROM STA. 26+70 TO STA. 31+00 -L- RT
 FROM STA. 53+50 TO STA. 59+30 -L- RT
 FROM STA. 53+09 TO STA. 57+00 -L- LT
 FROM STA. 63+30 TO STA. 64+81 -L- RT
 FROM STA. 68+50 TO STA. 72+50 -L- RT
 FROM STA. 76+00 TO STA. 77+20 -L- RT
 FROM STA. 74+50 TO STA. 76+75 -L- LT
 FROM STA. 78+50 TO STA. 80+50 -L- RT
 FROM STA. 91+50 TO STA. 94+50 -L- RT
 FROM STA. 93+00 TO STA. 96+00 -L- LT
 FROM STA. 104+50 TO STA. 109+50 -L- RT
 FROM STA. 108+40 TO STA. 110+00 -L- LT
 FROM STA. 123+18 TO STA. 124+85 -L- LT
 FROM STA. 132+50 TO STA. 133+50 -L- LT
 FROM STA. 133+00 TO STA. 134+80 -L- RT
 FROM STA. 134+58 TO STA. 140+00 -L- LT
 FROM STA. 136+20 TO STA. 142+50 -L- RT
 FROM STA. 151+00 TO STA. 153+20 -L- LT
 FROM STA. 164+00 TO STA. 164+80 -L- RT
 FROM STA. 178+50 TO STA. 180+50 -L- RT
 FROM STA. 182+50 TO STA. 186+20 -L- LT
 FROM STA. 198+25 TO STA. 201+50 -L- LT
 FROM STA. 233+35 TO STA. 234+40 -L- LT
 FROM STA. 233+90 TO STA. 235+00 -L- RT
 FROM STA. 216+74 TO STA. 221+50 -L- RT
 FROM STA. 224+50 TO STA. 227+30 -L- RT
 FROM STA. 13+55 TO STA. 14+05 -Y1- LT
 FROM STA. 10+70 TO STA. 13+50 -Y2- RT
 FROM STA. 19+76 TO STA. 21+73 -Y5- LT
 FROM STA. 13+50 TO STA. 15+40 -Y6- LT
 FROM STA. 24+70 TO STA. 27+45 -Y7- RT
 FROM STA. 29+00 TO STA. 31+95 -Y7- LT



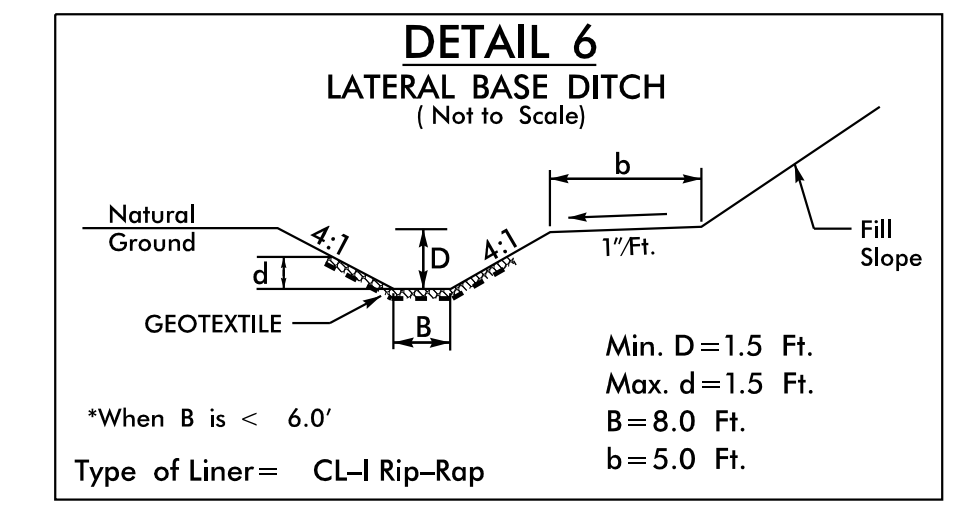
FROM STA. 16+28 TO STA. 17+55 -L- RT
 FROM STA. 186+50 TO STA. 189+80 -L- RT



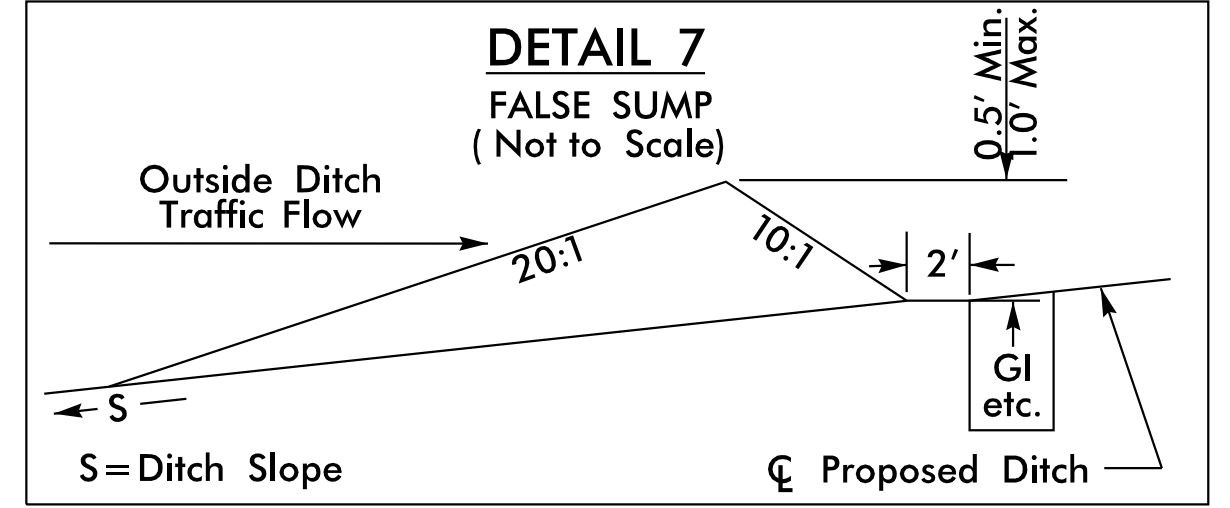
FROM STA. 90+50 TO STA. 91+50 -L- RT
 FROM STA. 27+45 TO STA. 29+00 -Y7- LT



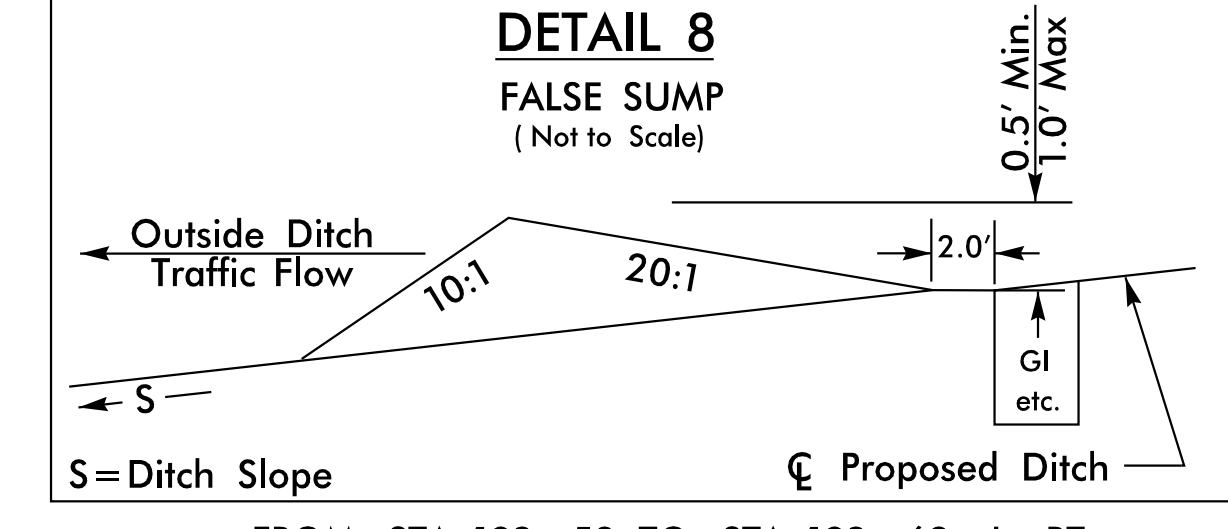
FROM STA. 24+06 TO STA. 26+30 -L- LT.



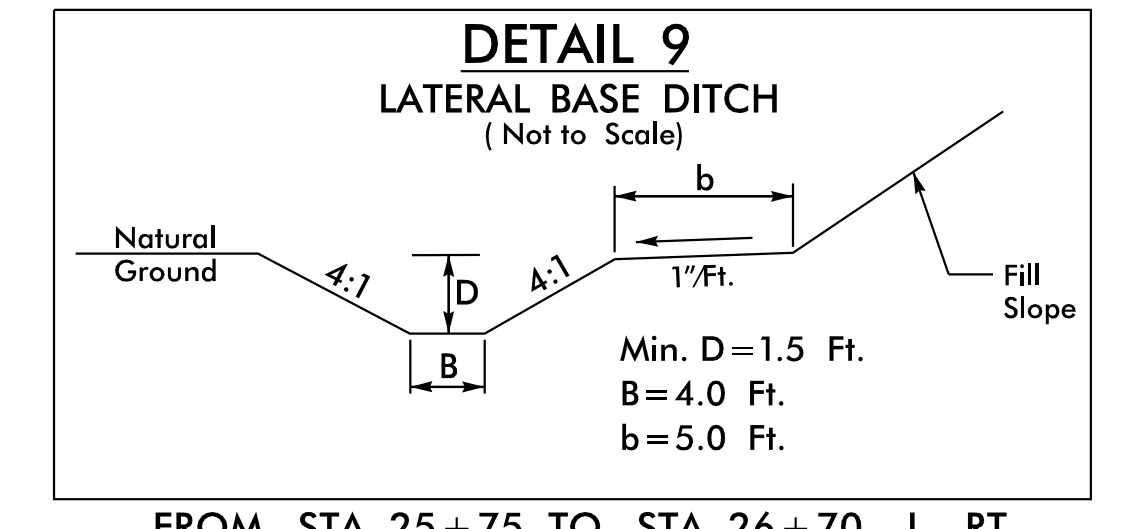
FROM STA. 23+00 TO STA. 24+88 -L- RT.



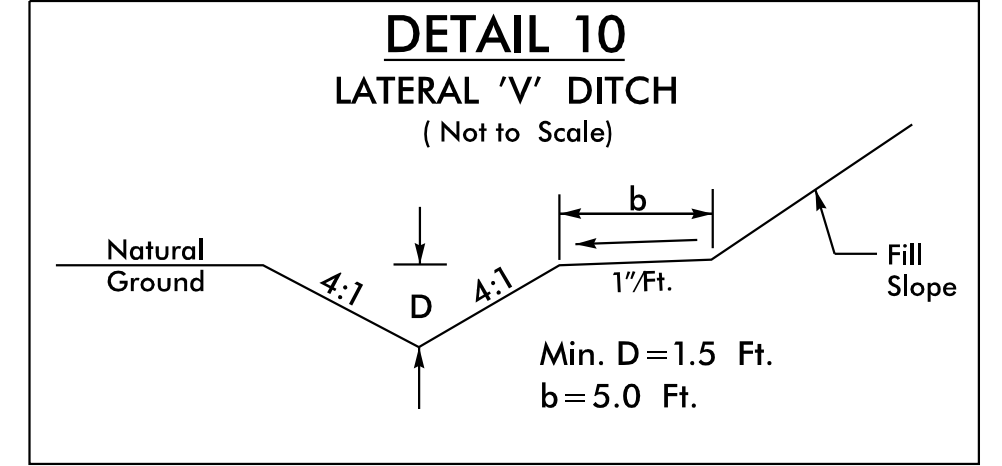
FROM STA. 47+33 TO STA. 47+48 -L- RT
 FROM STA. 51+40 TO STA. 51+55 -L- RT
 FROM STA. 50+00 TO STA. 50+15 -L- RT
 FROM STA. 81+54 TO STA. 81+69 -L- LT
 FROM STA. 83+54 TO STA. 83+69 -L- LT
 FROM STA. 84+02 TO STA. 84+17 -L- LT
 FROM STA. 86+04 TO STA. 86+19 -L- LT
 FROM STA. 183+83 TO STA. 183+98 -L- RT



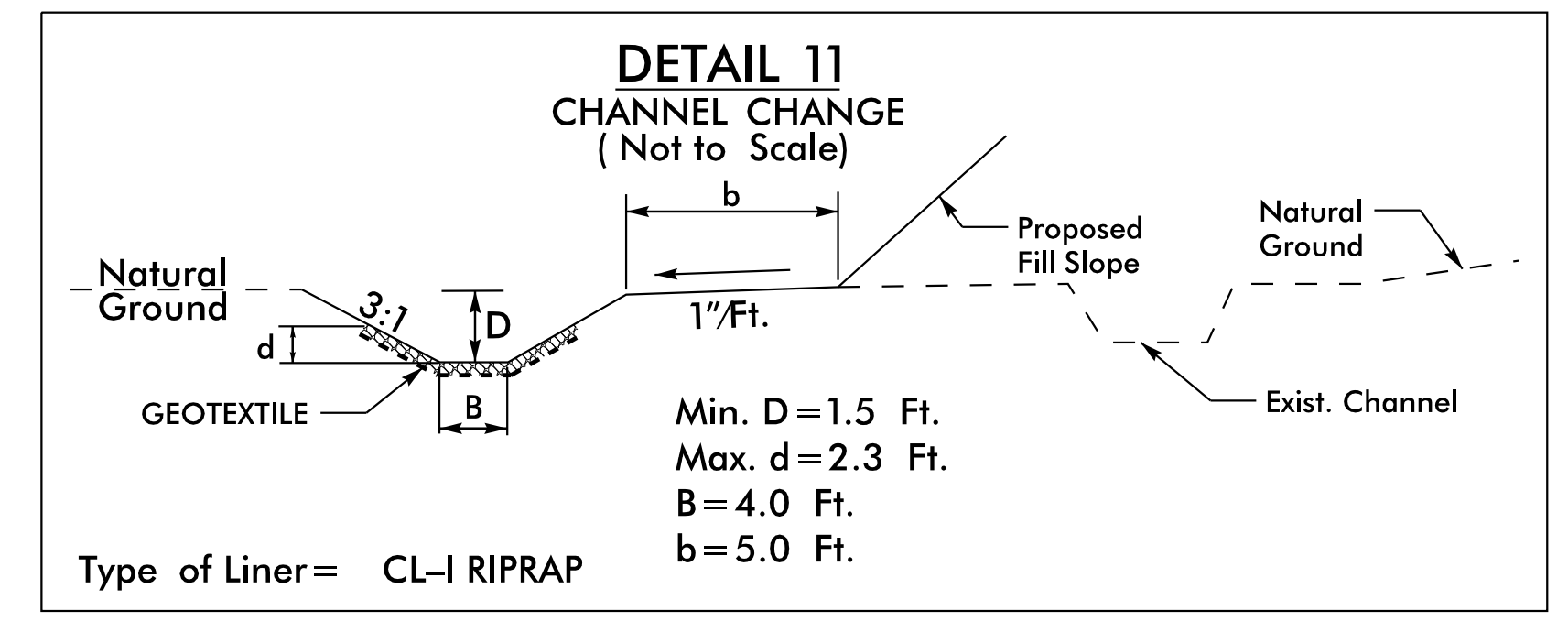
FROM STA. 192+53 TO STA. 192+68 -L- RT
 FROM STA. 194+52 TO STA. 194+67 -L- RT
 FROM STA. 207+33 TO STA. 207+48 -L- LT
 FROM STA. 204+83 TO STA. 204+98 -L- LT
 FROM STA. 11+24 TO STA. 11+39 -Y2- LT
 FROM STA. 12+00 TO STA. 12+15 -Y2- LT



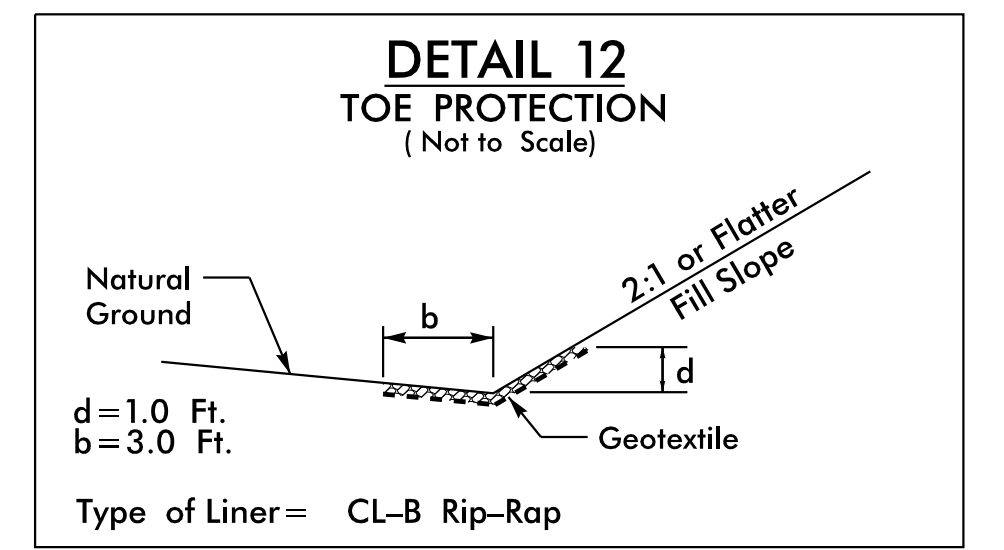
FROM STA. 25+75 TO STA. 26+70 -L- RT.



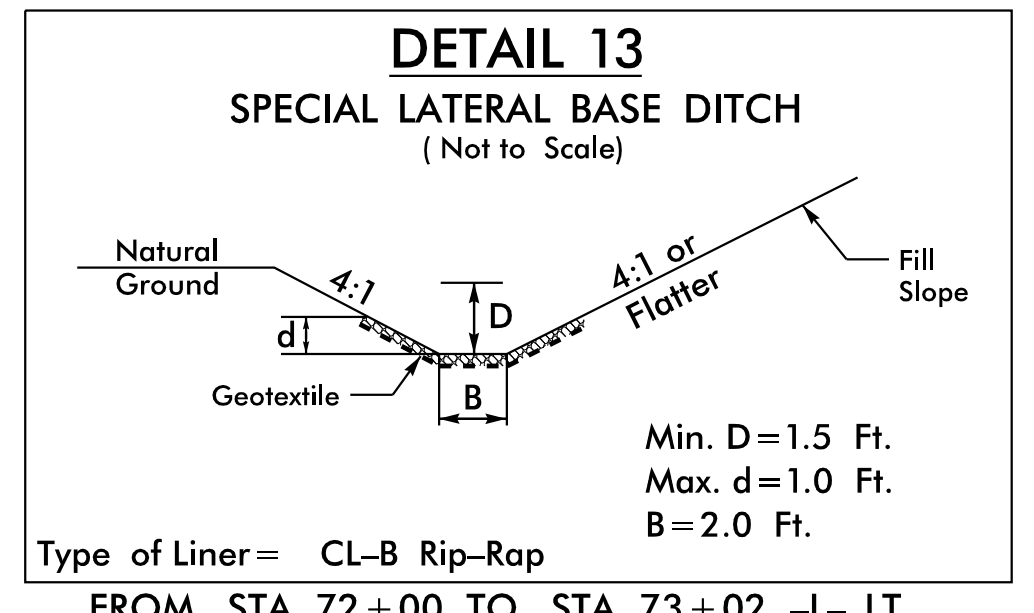
FROM STA. 153+20 TO STA. 154+75 -L- RT.



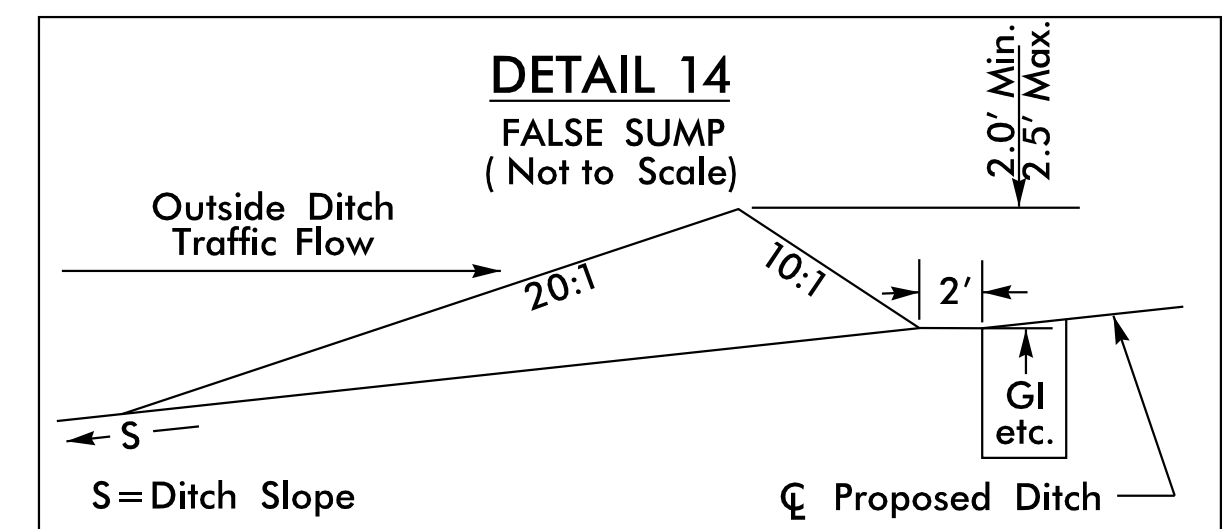
FROM STA. 42+50 TO STA. 43+50 -L- RT



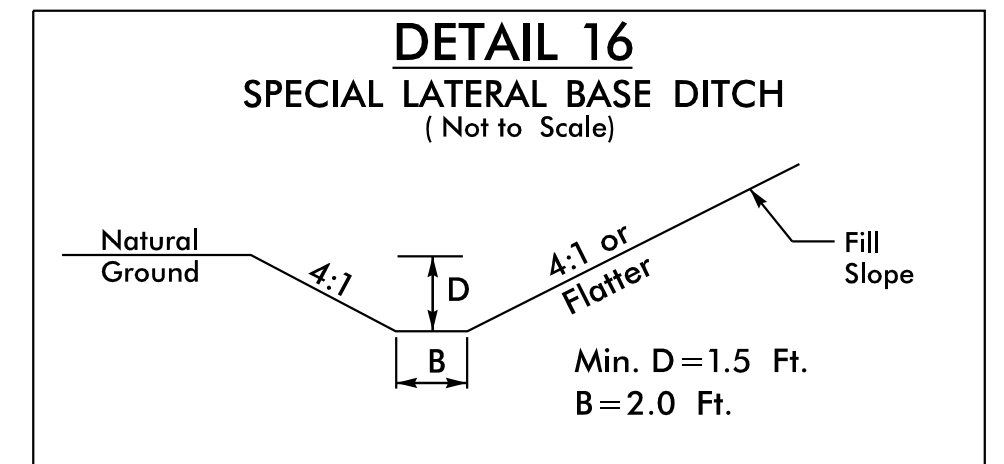
FROM STA. 44+00 TO STA. 45+68 -L- RT.
 FROM STA. 161+07 TO STA. 162+43 -L- LT
 FROM STA. 196+65 TO STA. 197+50 -L- RT
 FROM STA. 227+95 TO STA. 230+45 -L- RT



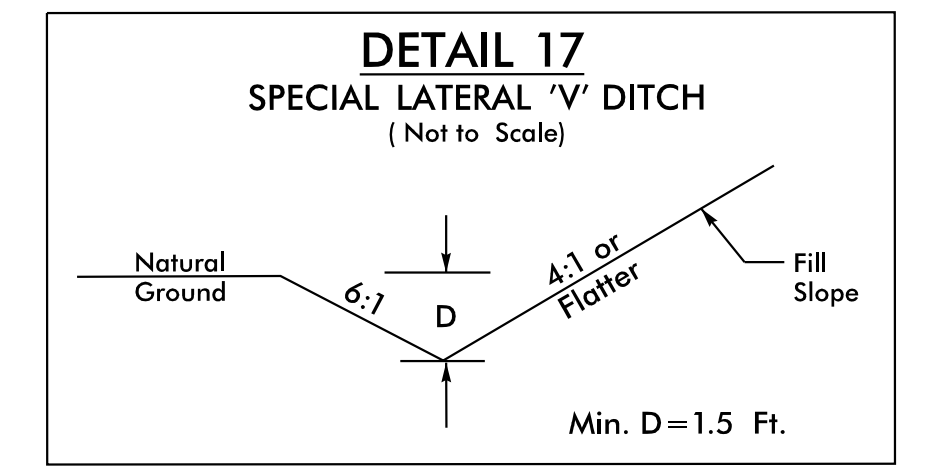
FROM STA. 72+00 TO STA. 73+02 -L- LT.



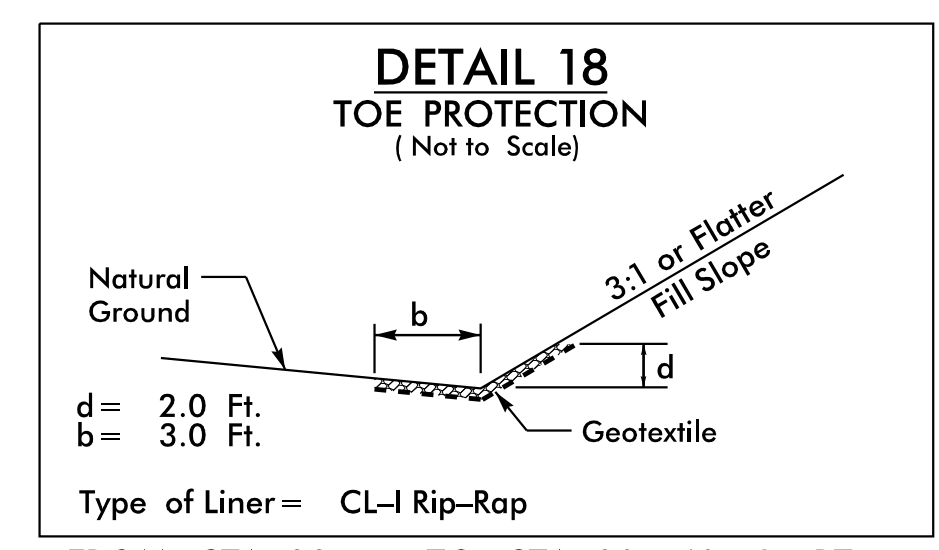
FROM STA. 77+22 TO STA. 77+82 -L- LT
 FROM STA. 122+18 TO STA. 122+78 -L- LT
 FROM STA. 124+85 TO STA. 125+45 -L- LT
 FROM STA. 126+00 TO STA. 126+60 -L- LT



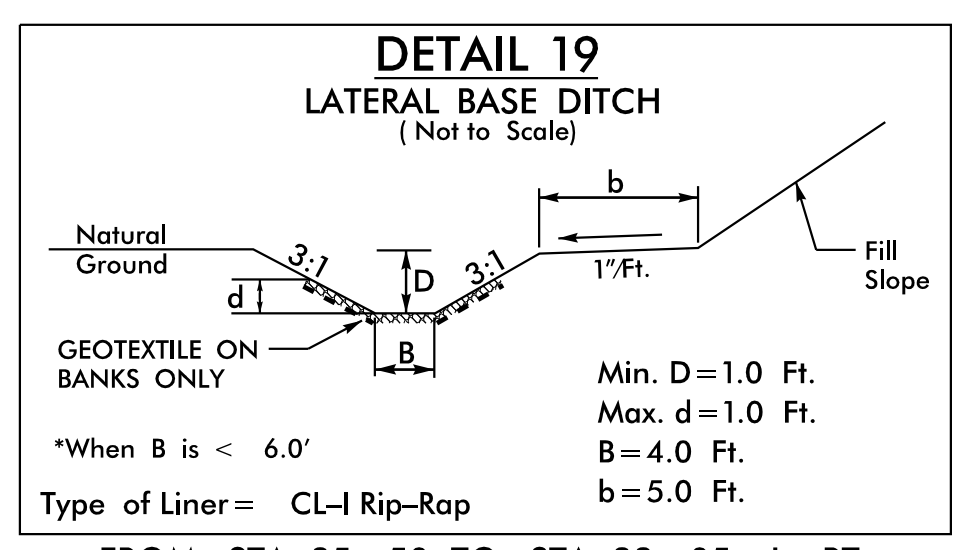
FROM STA. 72+50 TO STA. 73+25 -L- RT
 FROM STA. 151+50 TO STA. 153+75 -L- RT
 FROM STA. 154+05 TO STA. 156+50 -L- RT
 FROM STA. 178+00 TO STA. 179+00 -L- LT



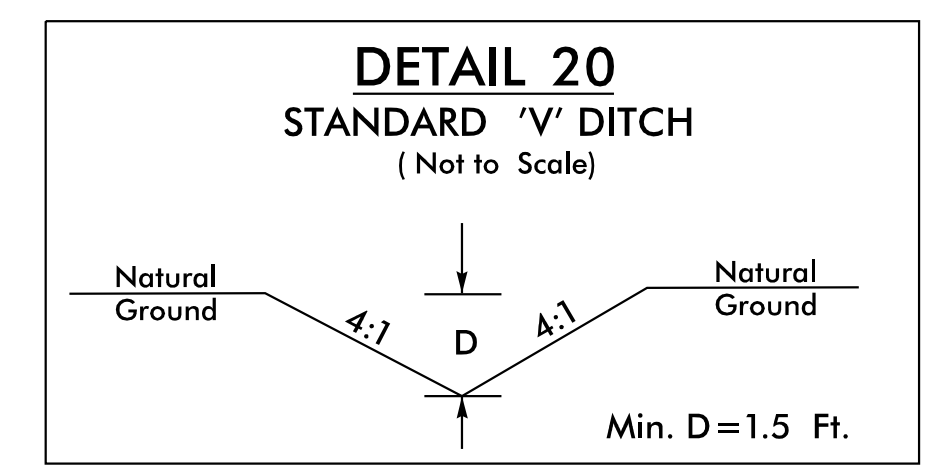
FROM STA. 82+50 TO STA. 85+00 -L- LT.
 FROM STA. 146+50 TO STA. 148+50 -L- LT



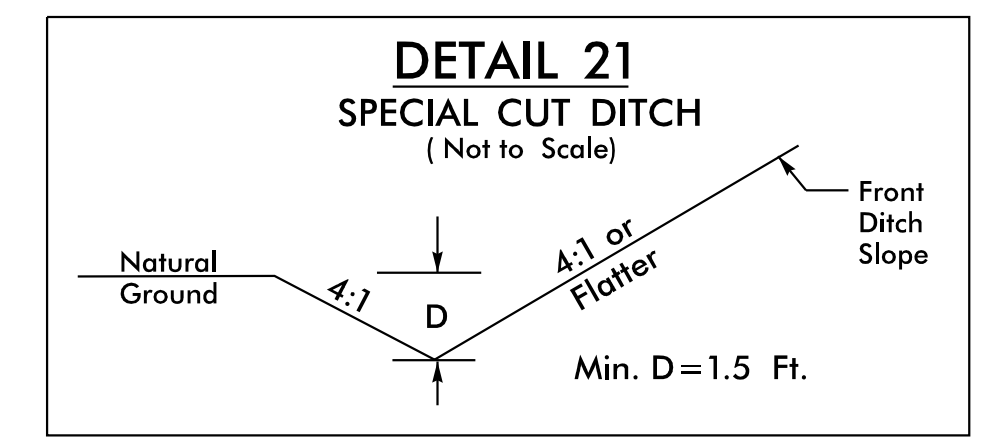
FROM STA. 88+15 TO STA. 90+40 -L- RT.
 FROM STA. 90+48 TO STA. 90+58 -L- RT.



FROM STA. 85+50 TO STA. 88+05 -L- RT



FROM STA. 100+25 TO STA. 101+50 -L- RT
 AT STA. 64+81 RT

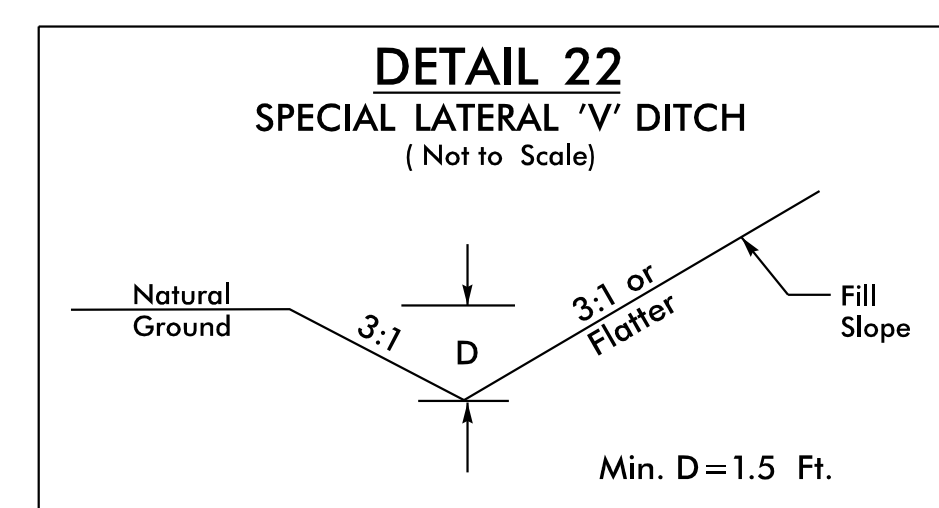


FROM STA. 103+50 TO STA. 104+50 -L- RT
 FROM STA. 159+50 TO STA. 160+50 -L- RT
 FROM STA. 177+50 TO STA. 178+50 -L- RT
 FROM STA. 209+65 TO STA. 211+00 -L- RT
 FROM STA. 212+34 TO STA. 212+50 -L- LT
 FROM STA. 23+50 TO STA. 24+50 -Y4- LT
 FROM STA. 10+30 TO STA. 11+00 -Y6A- RT
 FROM STA. 34+00 TO STA. 36+00 -Y7- LT

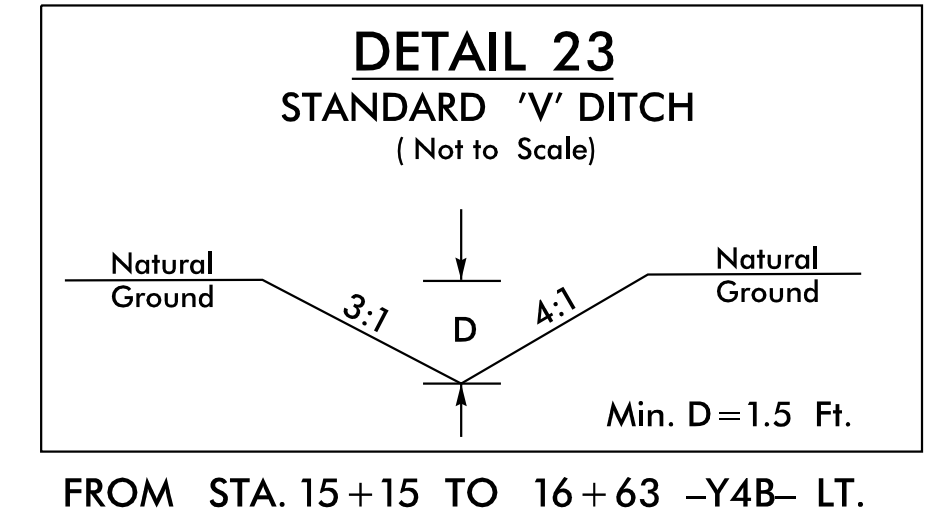
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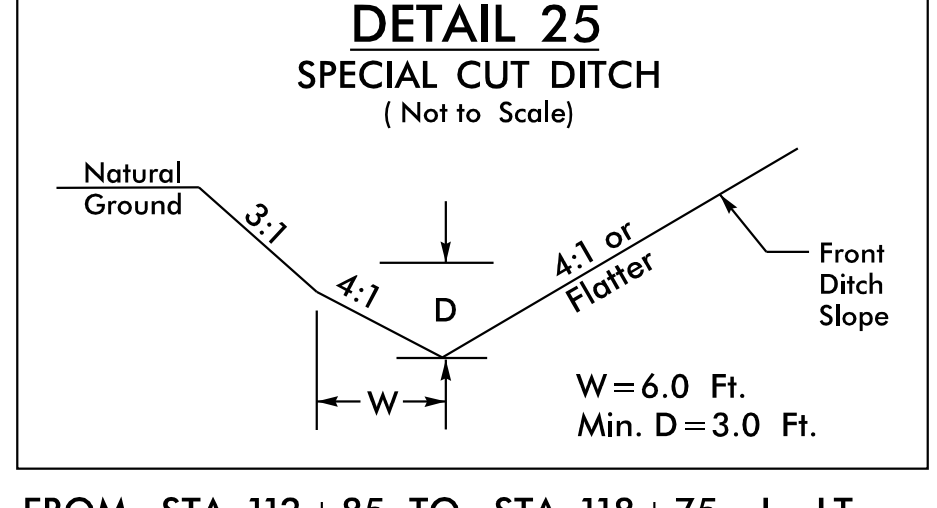
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2D-2
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



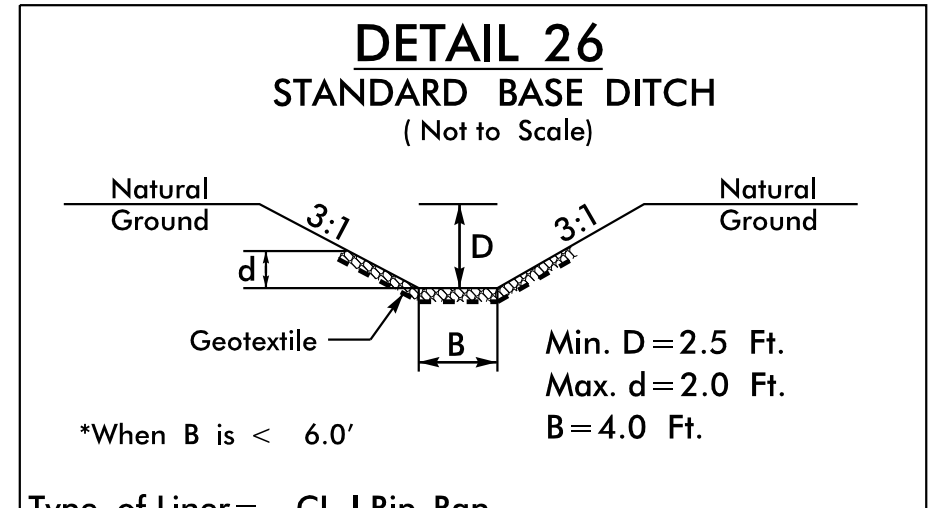
FROM STA. 130+09 TO STA. 132+50 -L- LT
 FROM STA. 10+88 TO STA. 12+85 -Y3- RT
 FROM STA. 10+88 TO STA. 12+95 -Y3- LT
 FROM STA. 13+13 TO STA. 15+00 -Y4- LT
 FROM STA. 16+00 TO STA. 16+63 -Y4B- LT
 FROM STA. 10+65 TO STA. 13+50 -Y5A- RT
 FROM STA. 17+65 TO STA. 22+44 -Y6- RT
 FROM STA. 17+50 TO STA. 22+00 -Y6- LT
 FROM STA. 13+94 TO STA. 14+80 -Y7C- LT
 FROM STA. 15+00 TO STA. 17+00 -Y7C- LT
 FROM STA. 13+88 TO STA. 14+70 -Y7C- LT
 FROM STA. 13+50 TO STA. 16+00 -Y7D- LT



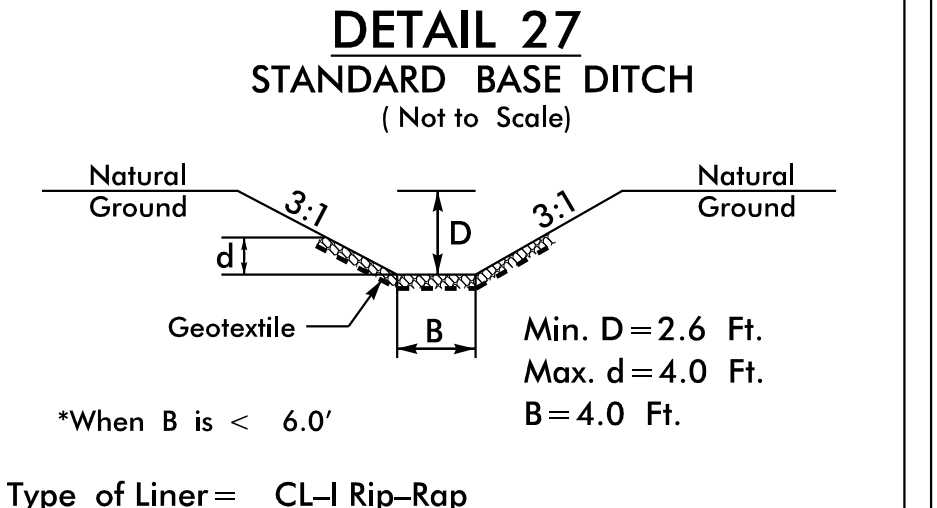
FROM STA. 15+15 TO STA. 16+63 -Y4B- LT.



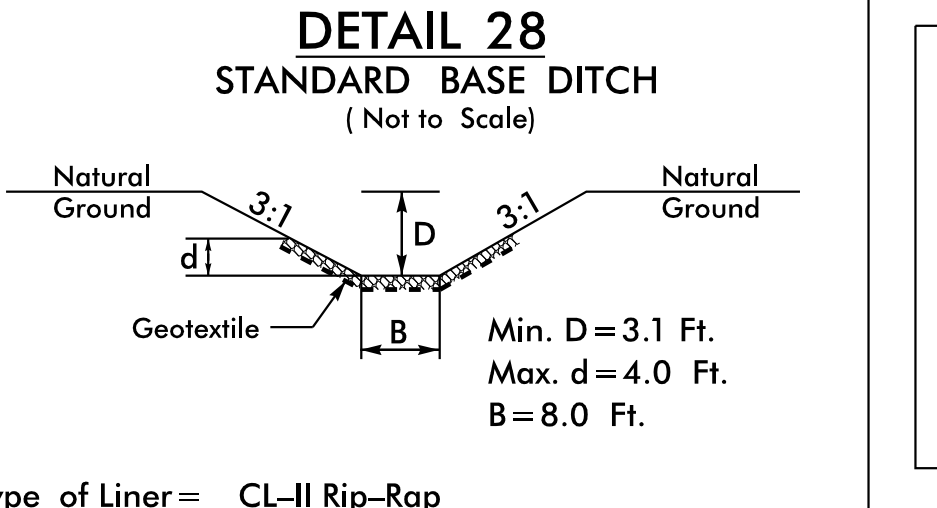
FROM STA. 112+85 TO STA. 118+75 -L- LT.
 FROM STA. 121+00 TO STA. 123+18 -L- LT.
 FROM STA. 190+40 TO STA. 192+50 -L- RT.
 FROM STA. 211+00 TO STA. 212+00 -L- RT.



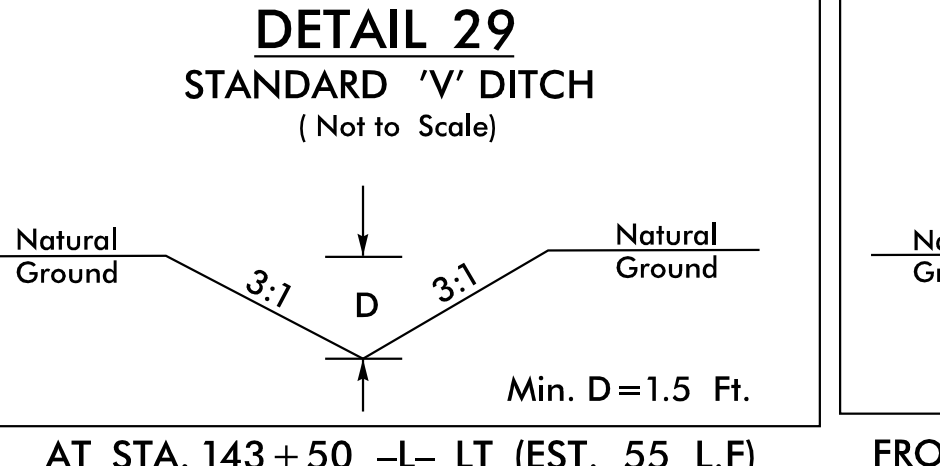
Type of Liner = CL-I Rip-Rap
 AT STA. 45+00 -L- LT (EST. 70 L.F.)
 AT STA. 108+30 -L- LT (EST. 90 L.F.)



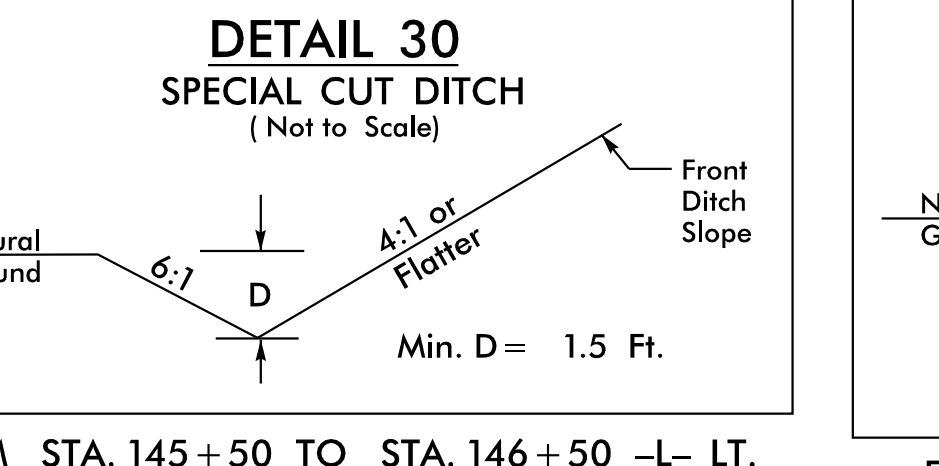
Type of Liner = CL-I Rip-Rap
 AT STA. 22+30 -Y4- RT. (EST. 50 L.F.)



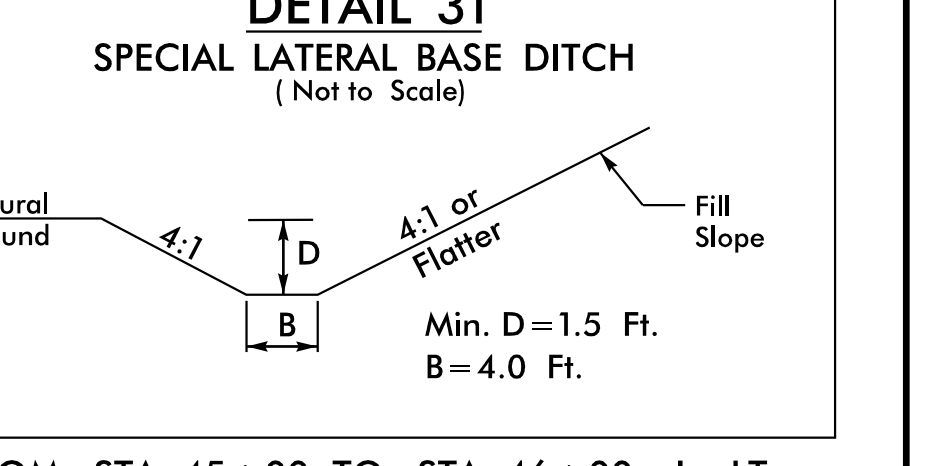
Type of Liner = CL-II Rip-Rap
 AT STA. 19+25 -Y4A- RT. (EST. 50 L.F.)



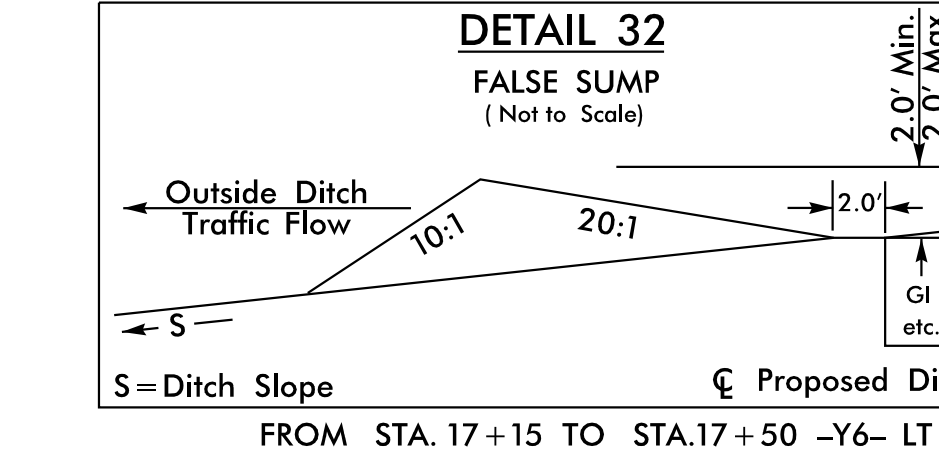
AT STA. 143+50 -L- LT (EST. 55 L.F.)
 AT STA. 18+50 -Y6A- LT (EST. 50 L.F.)



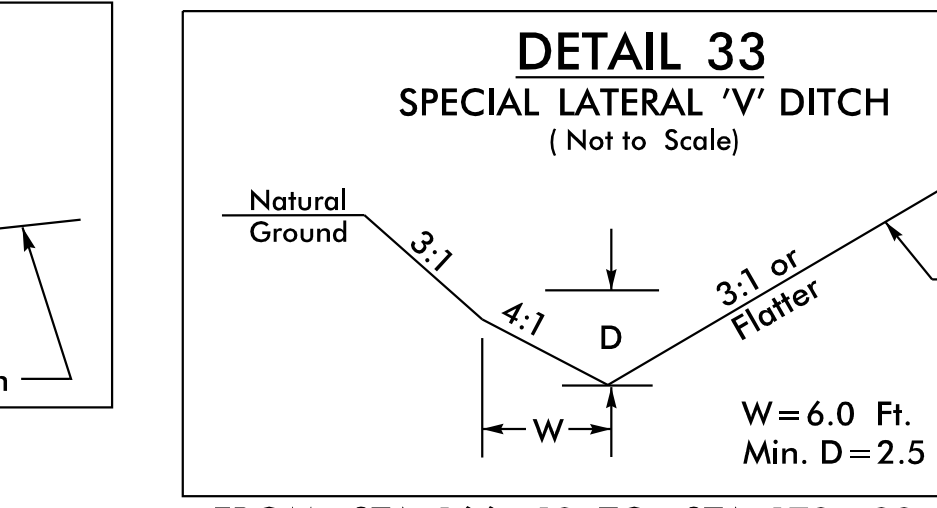
FROM STA. 145+50 TO STA. 146+50 -L- LT.



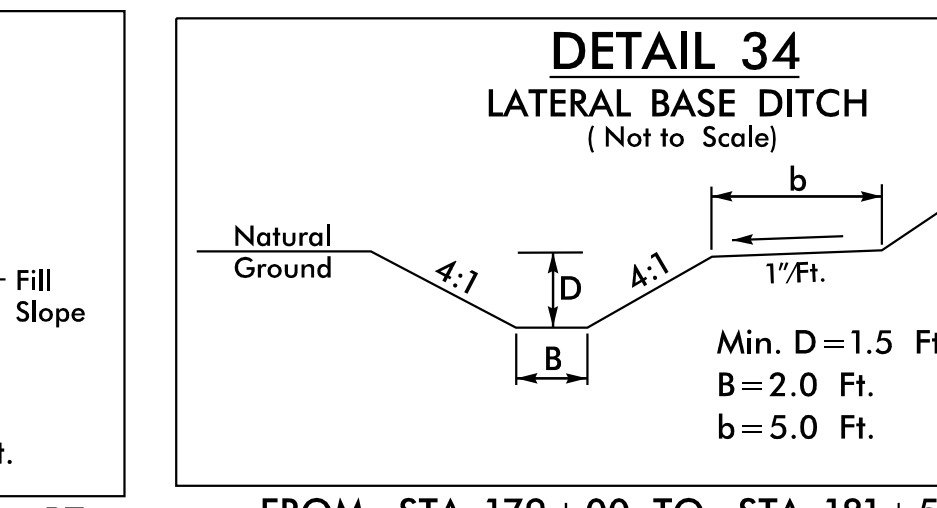
FROM STA. 45+00 TO STA. 46+00 -L- LT
 FROM STA. 92+25 TO STA. 93+00 -L- LT
 FROM STA. 160+50 TO STA. 161+80 -L- RT
 FROM STA. 161+80 TO STA. 164+00 -L- RT
 FROM STA. 173+00 TO STA. 173+65 -L- RT
 FROM STA. 202+50 TO STA. 203+80 -L- LT
 FROM STA. 203+80 TO STA. 205+00 -L- LT
 FROM STA. 10+50 TO STA. 13+15 -Y1- LT
 FROM STA. 22+00 TO STA. 23+50 -Y4- LT



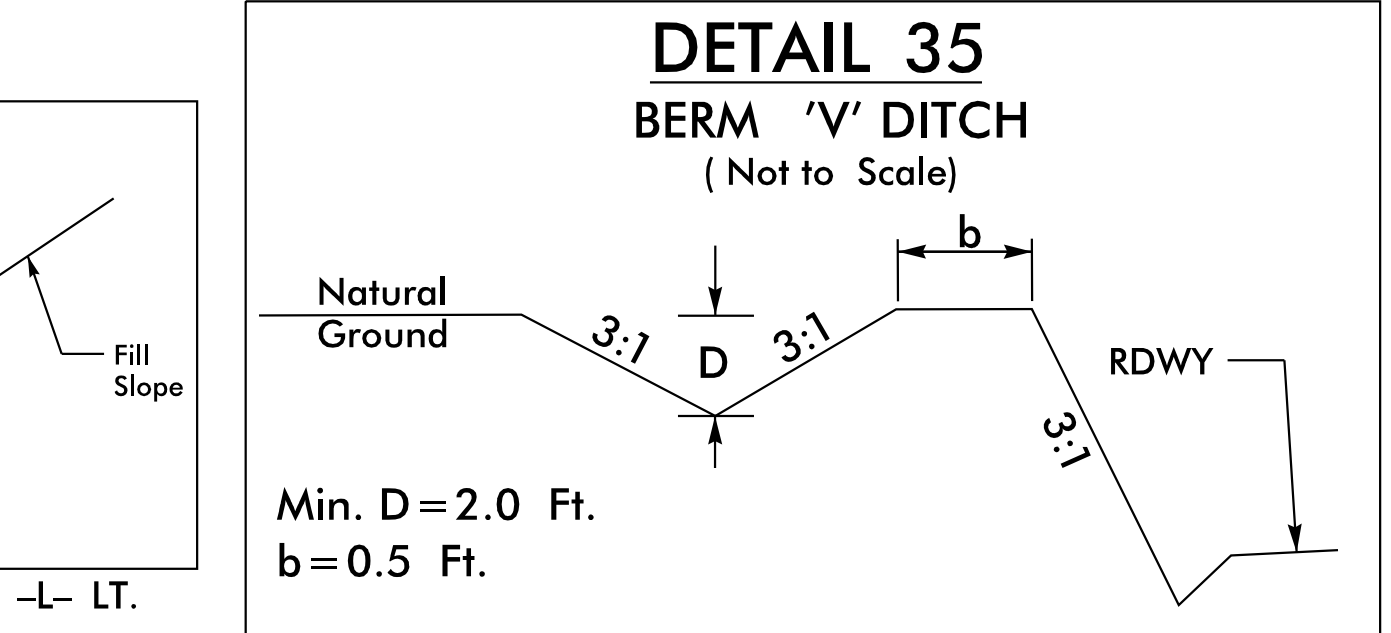
FROM STA. 17+15 TO STA. 17+50 -Y6- LT



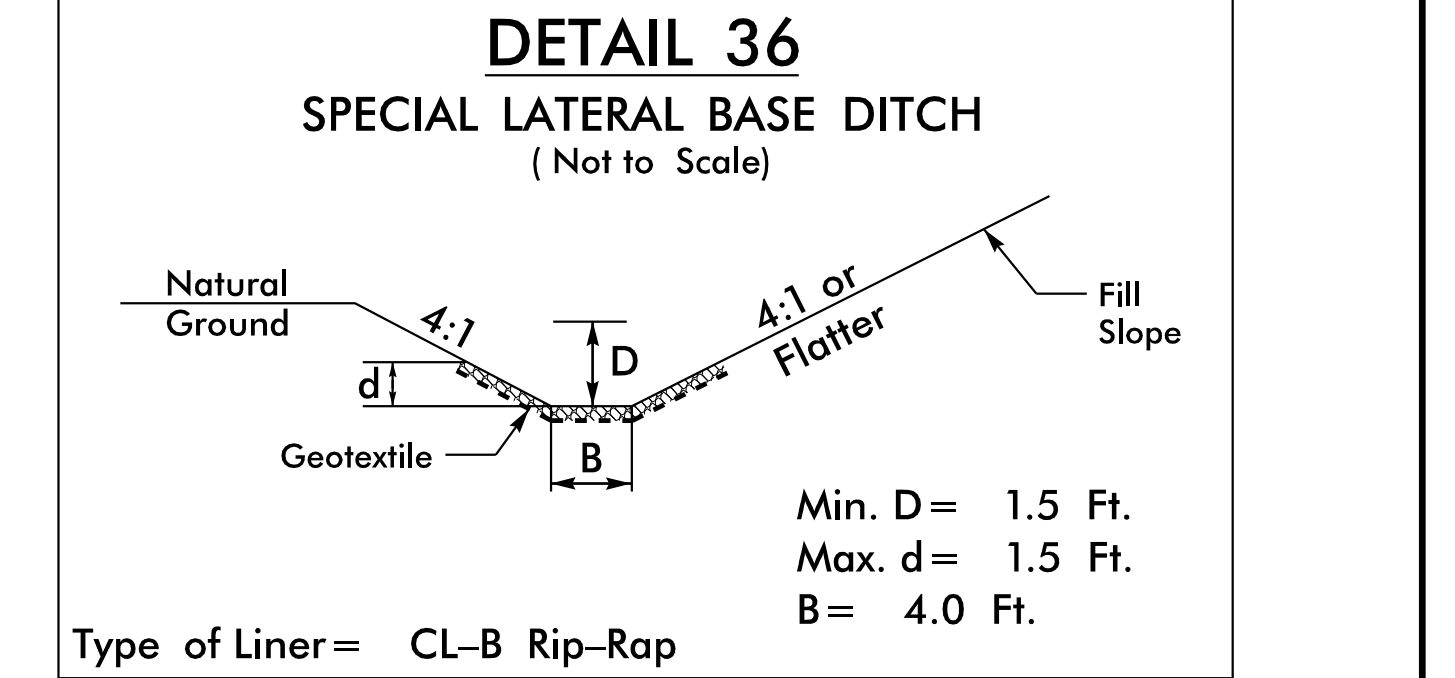
FROM STA. 166+10 TO STA. 170+00 -L- RT
 FROM STA. 194+50 TO STA. 195+50 -L- RT



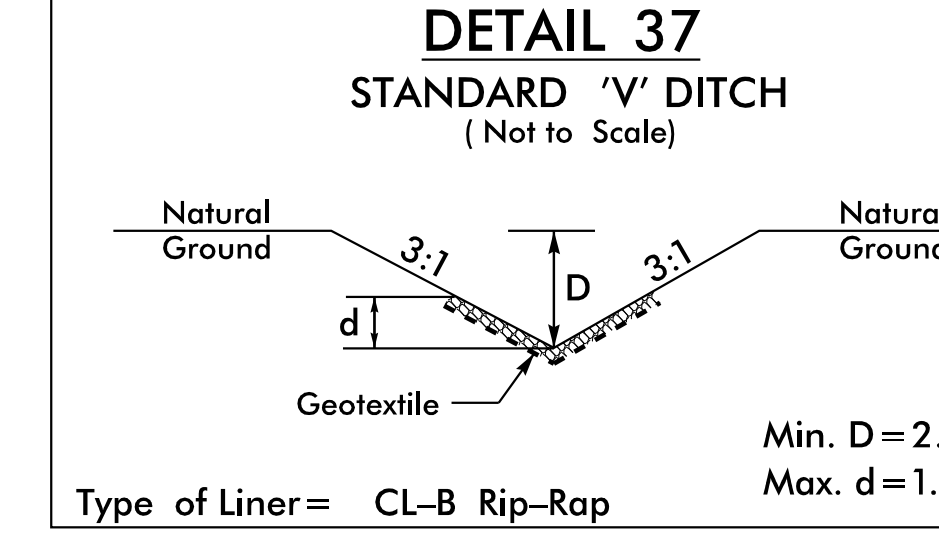
FROM STA. 179+00 TO STA. 181+50 -L- LT.



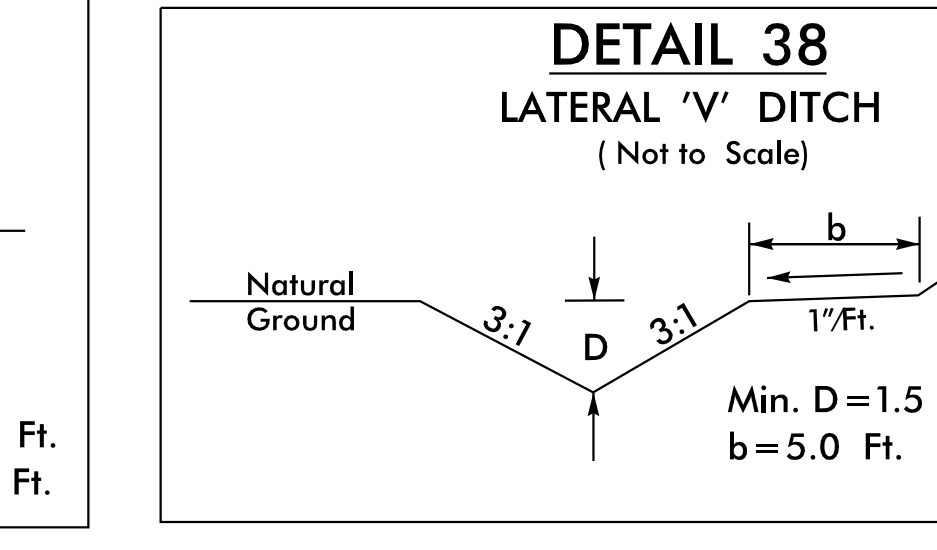
FROM STA. 190+30 TO STA. 192+50 -L- RT.



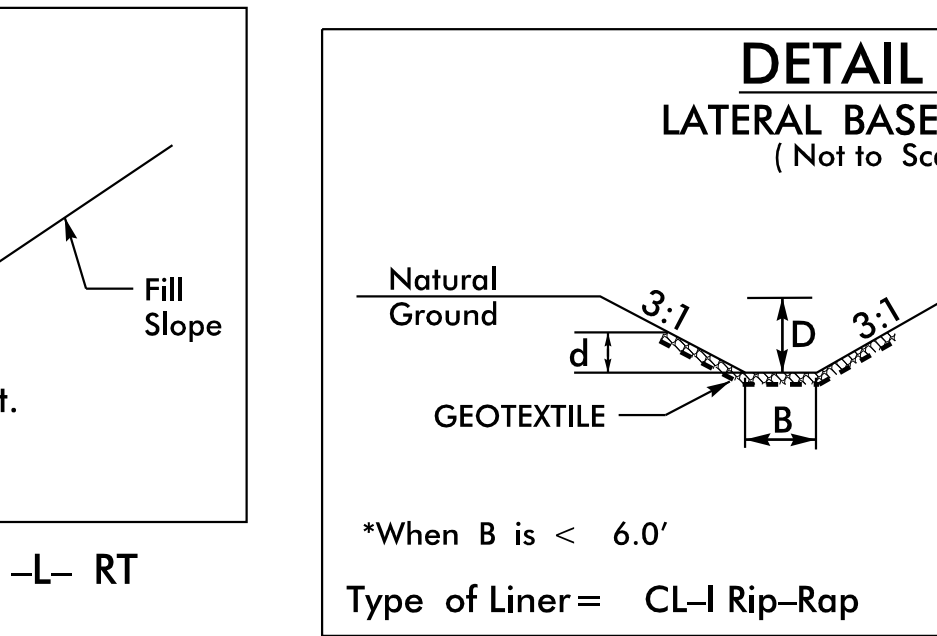
Type of Liner = CL-B Rip-Rap
 FROM STA. 20+00 TO STA. 22+00 -Y4- LT



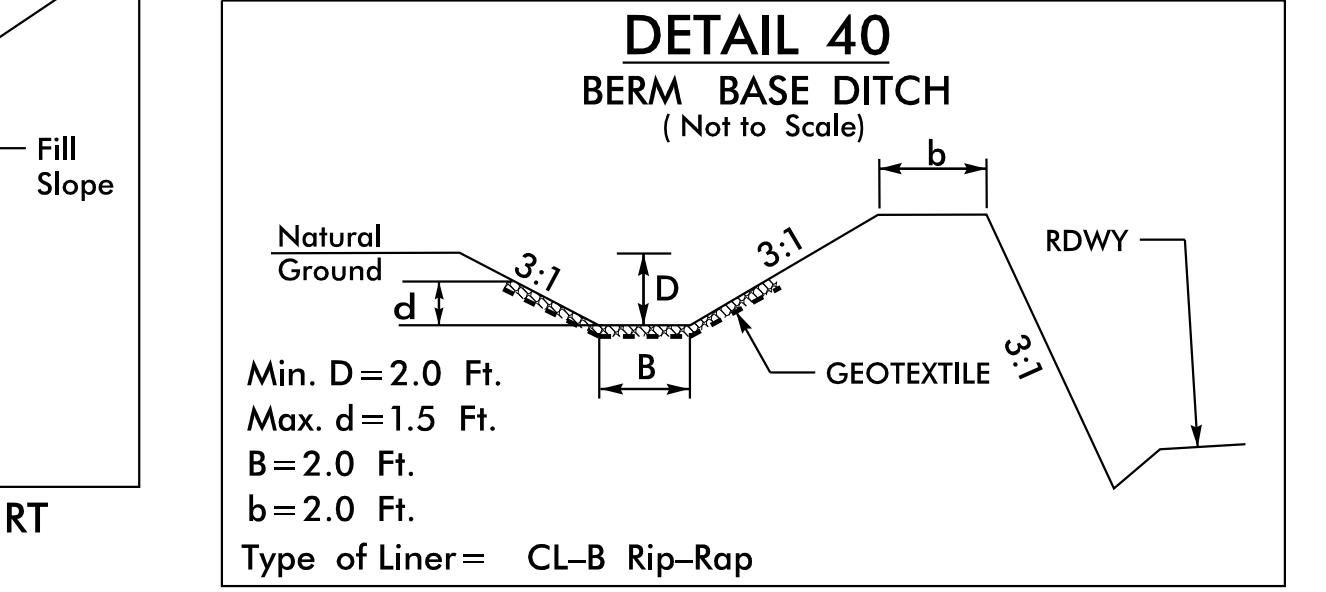
Type of Liner = CL-B Rip-Rap
 FROM -L- STA. 132+50 LT. TO
 -Y5B- STA. 13+90 LT. (EST. 64 L.F.,
 HEAD DITCH THROUGH -Y5B-)



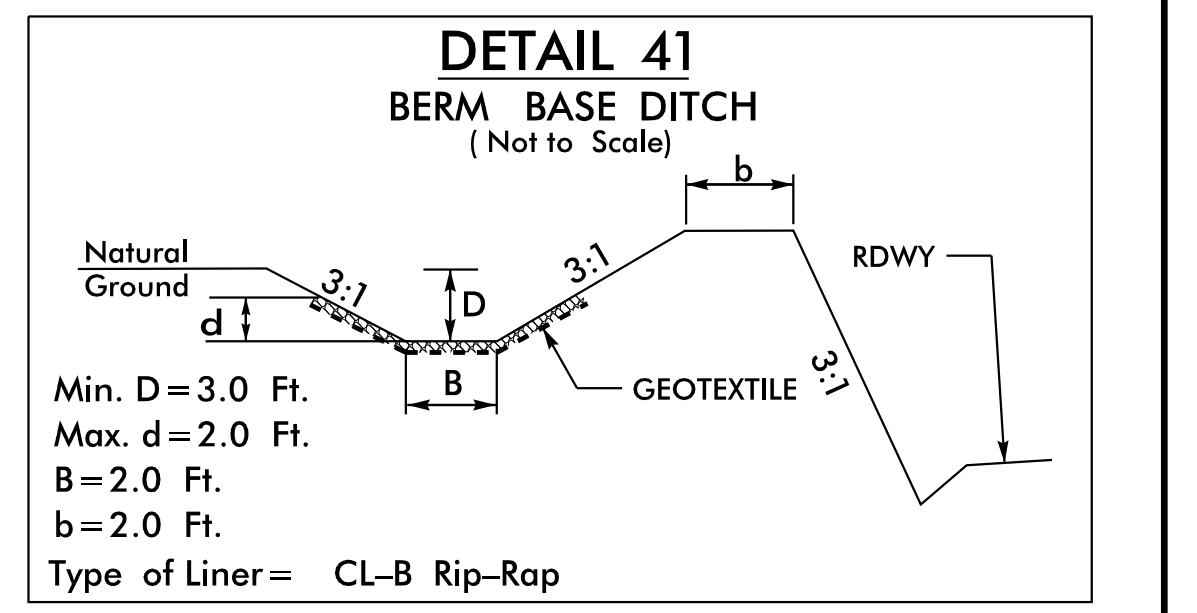
FROM STA. 132+10 TO STA. 133+00 -L- RT



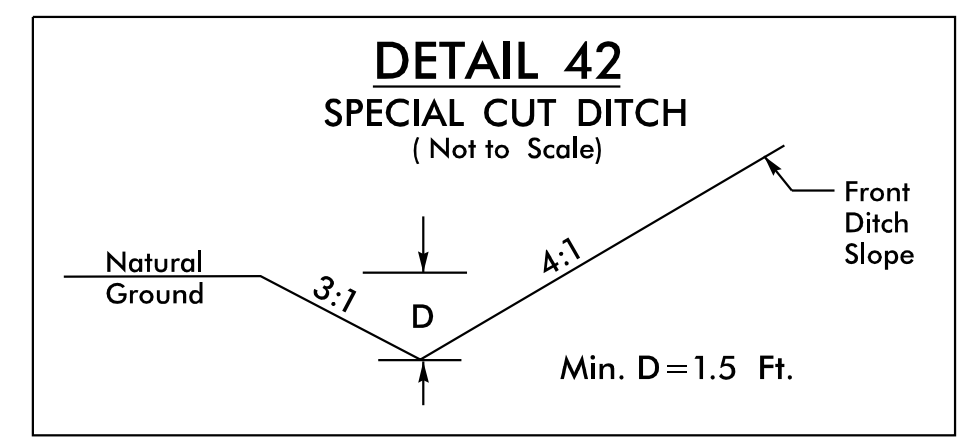
*When B is < 6.0'
 Type of Liner = CL-I Rip-Rap
 FROM STA. 197+50 TO STA. 198+90 -L- RT



Type of Liner = CL-B Rip-Rap
 FROM STA. 205+00 TO STA. 205+50 -L- LT.
 FROM STA. 208+50 TO STA. 211+30 -L- LT.



Type of Liner = CL-B Rip-Rap
 FROM STA. 207+50 TO STA. 208+50 -L- LT.



FROM STA. 16+50 TO STA. 17+30 -Y7D- LT
 FROM STA. 15+00 TO STA. 19+20 -Y7D- RT
 FROM STA. 17+76 TO STA. 19+20 -Y7D- LT

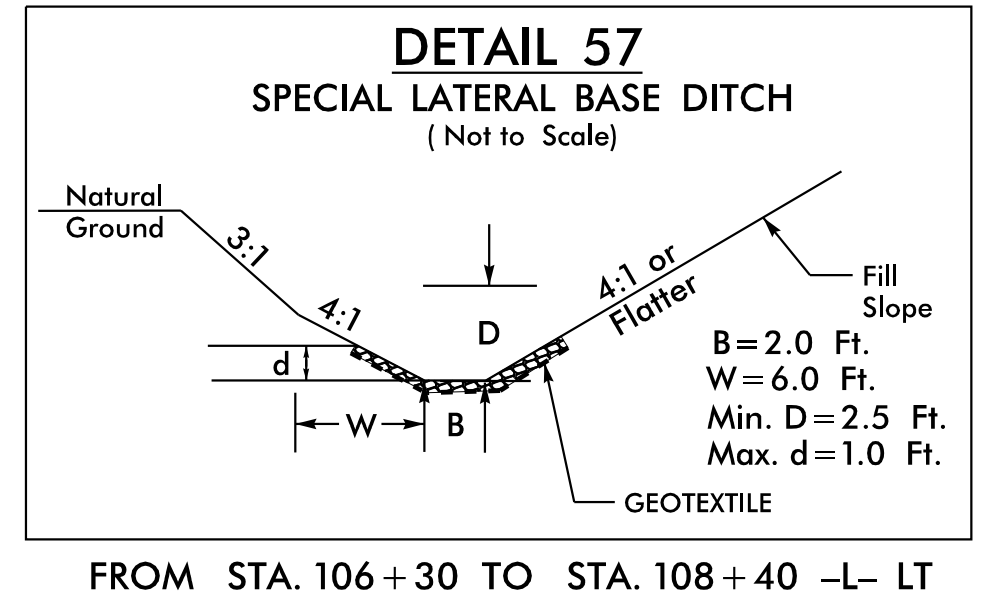
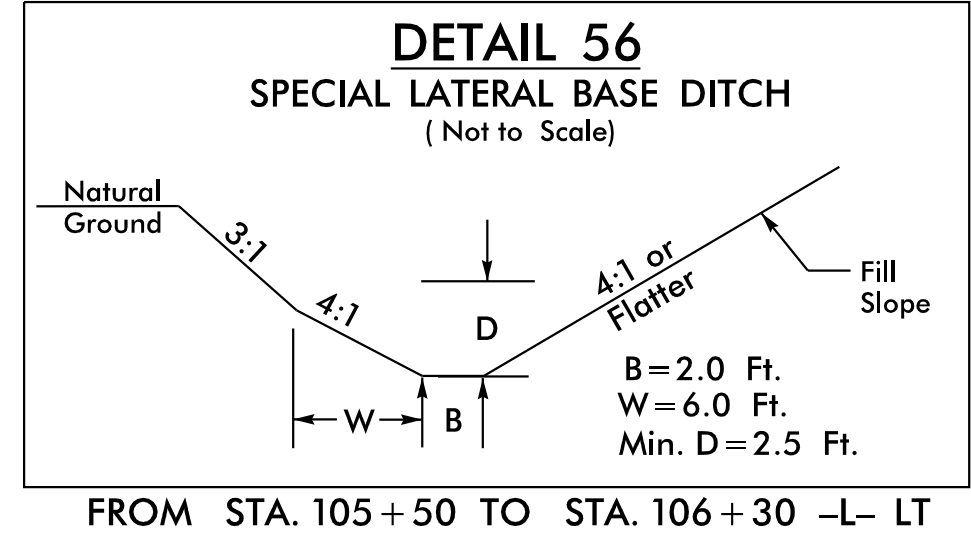
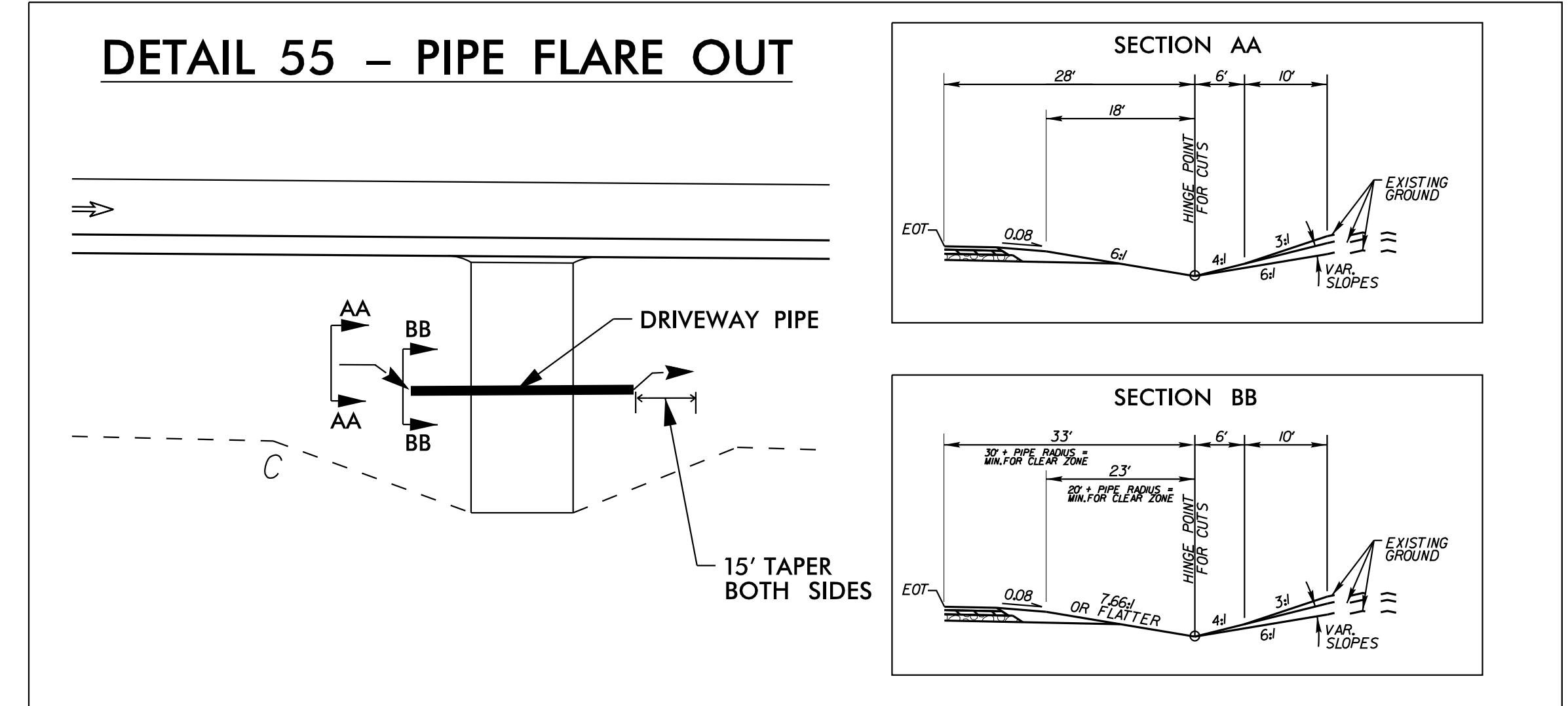
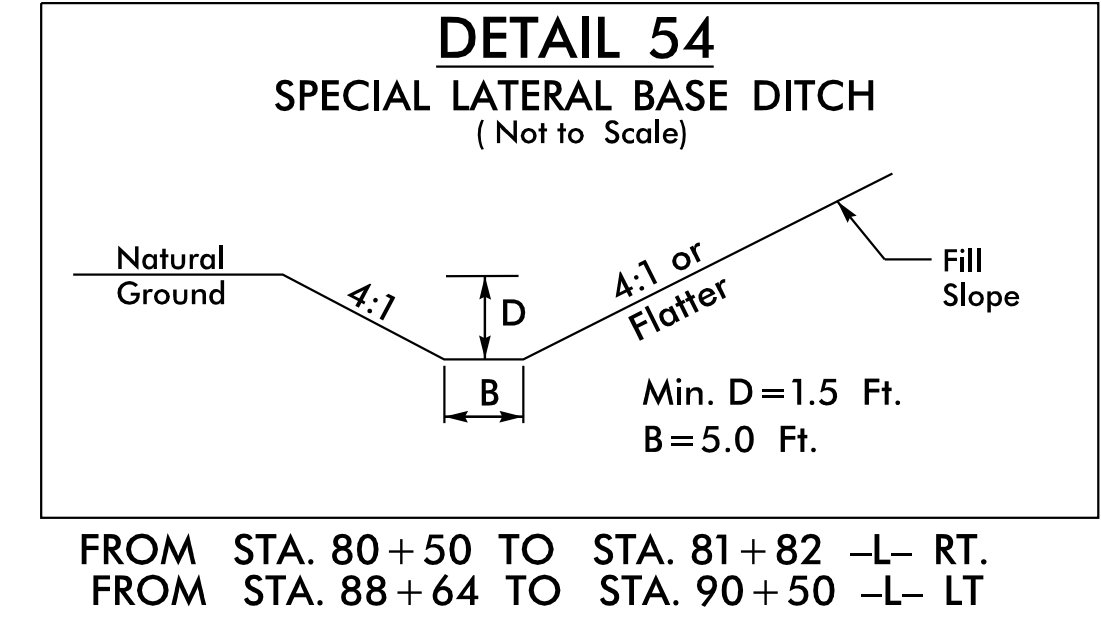
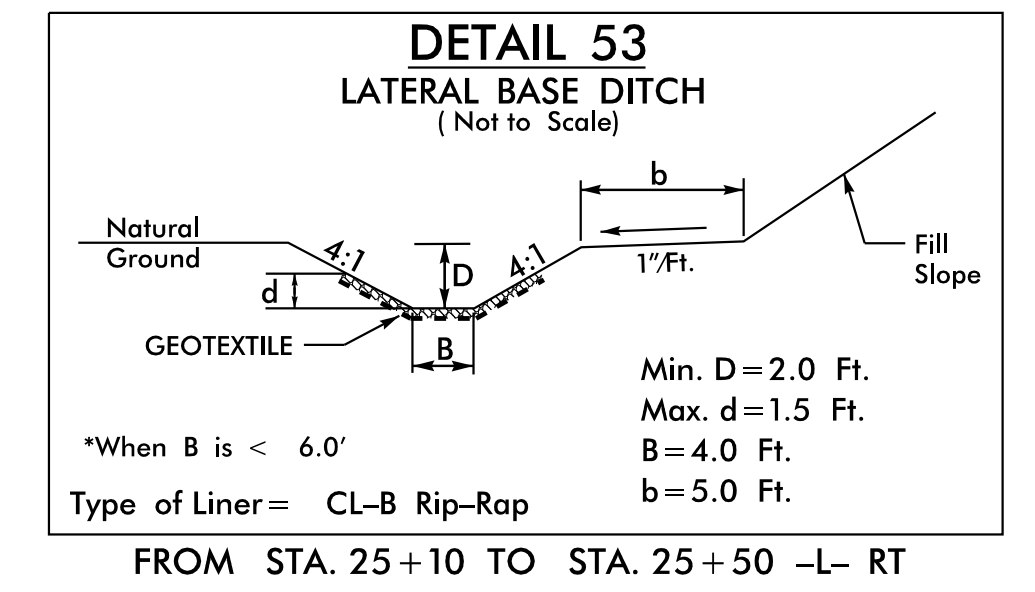
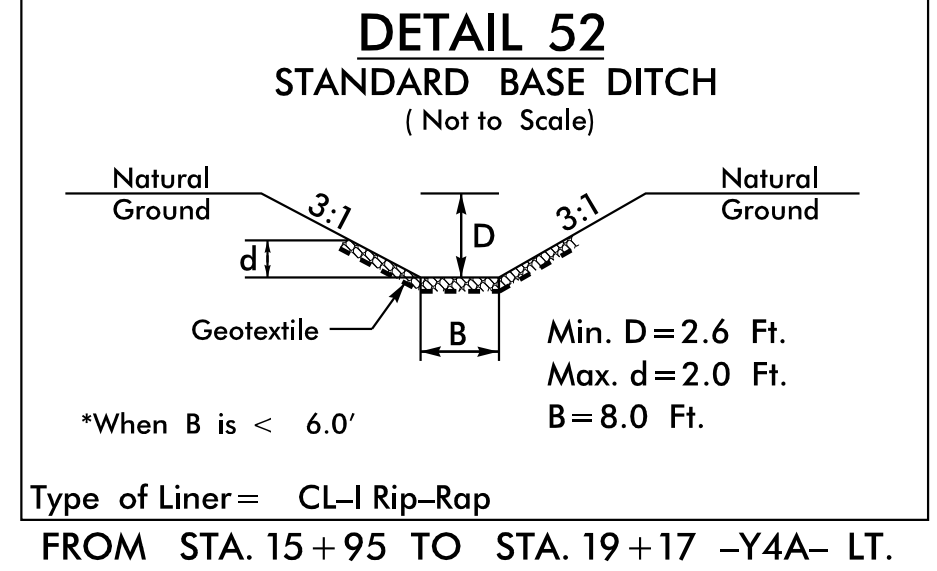
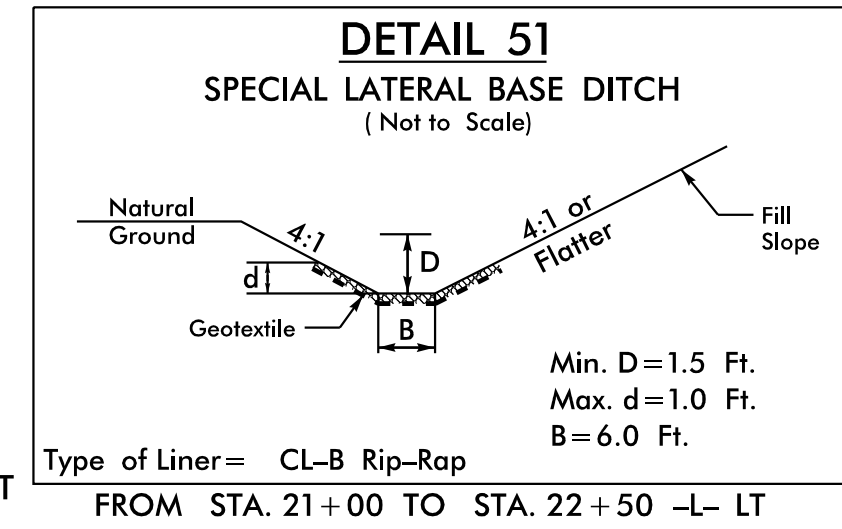
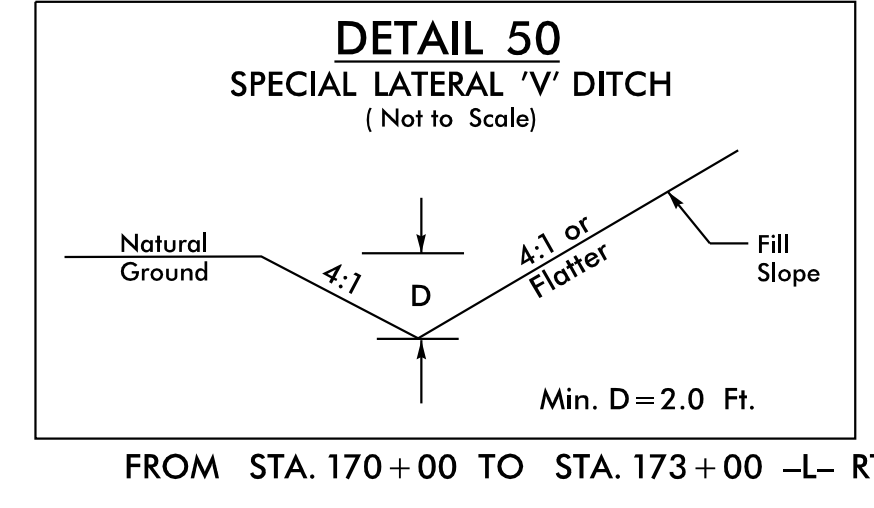
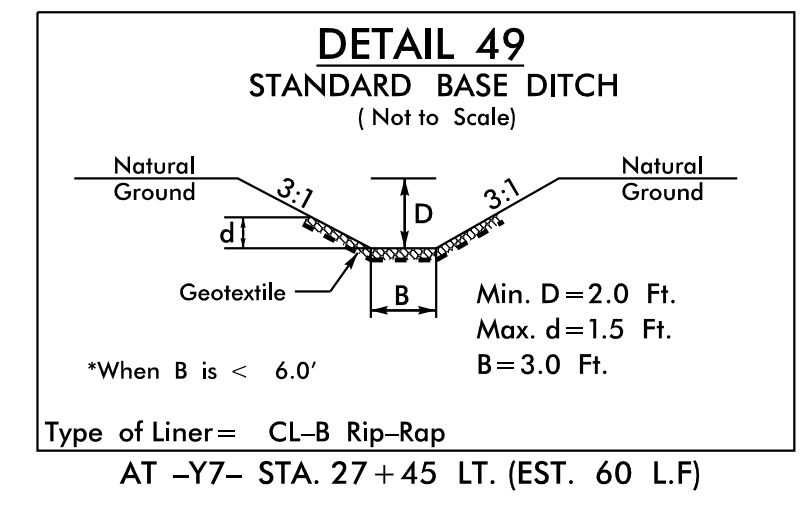
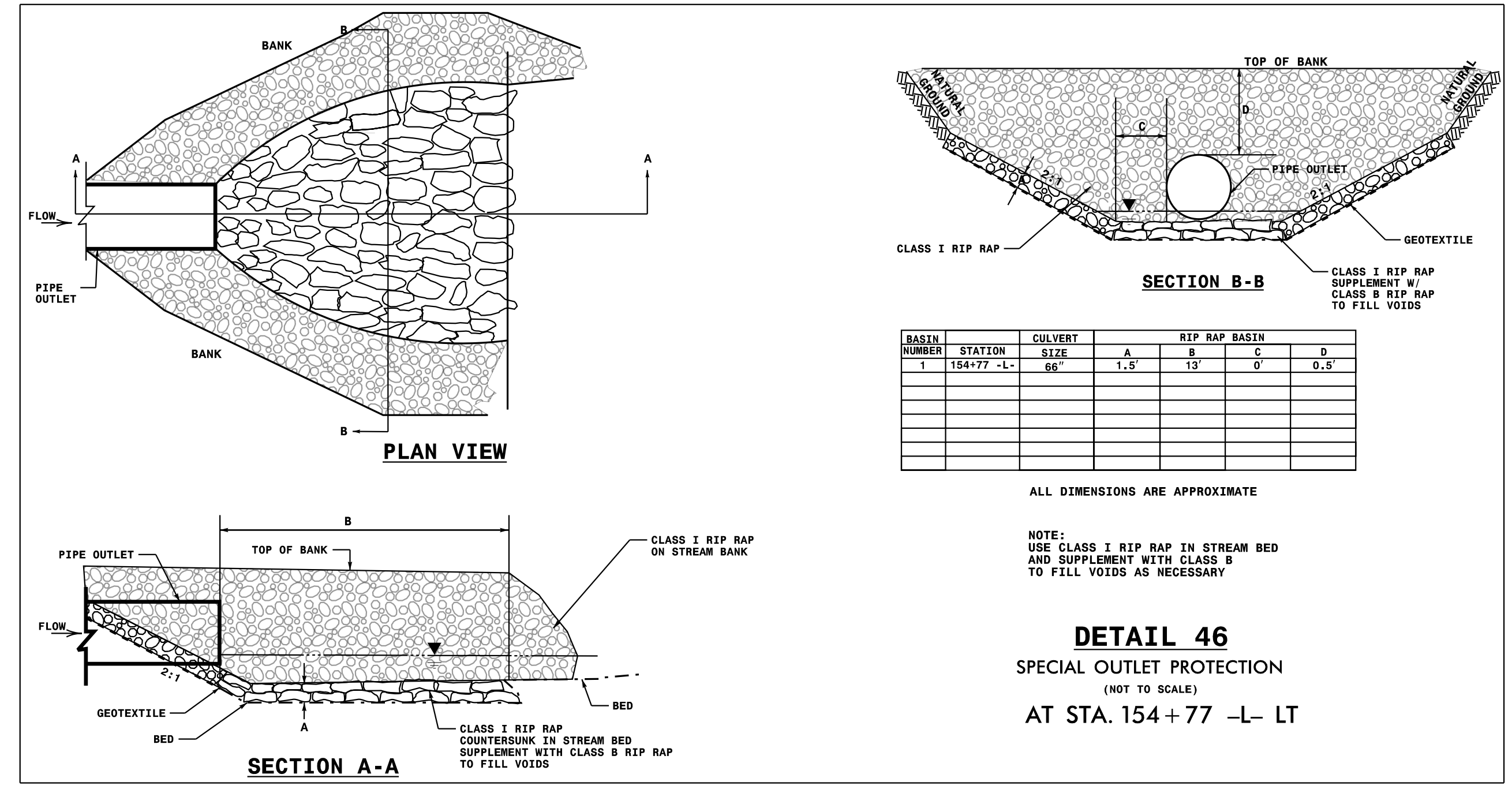
REVISIONS

8/8/2023

5/14/99

REVISIONS

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2D-3
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



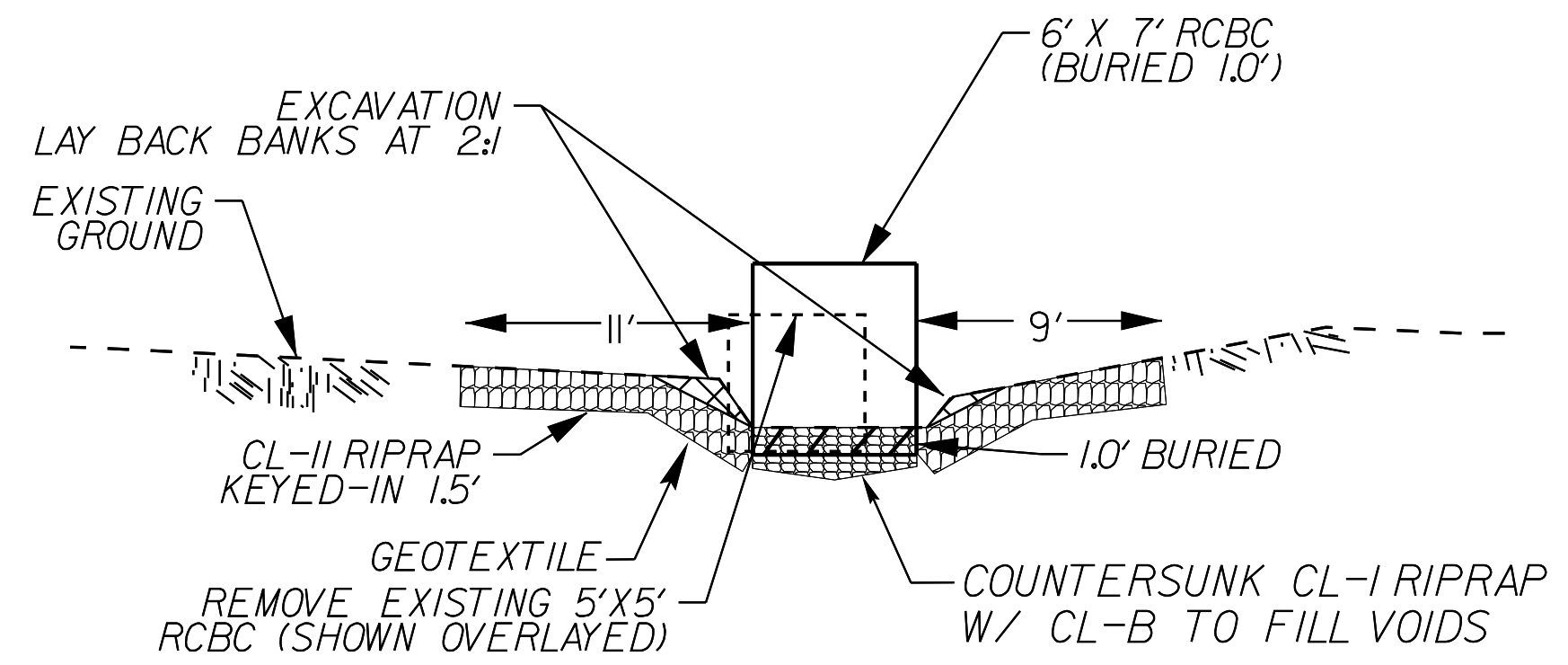
- AT STA. 95+06 -L- RT
- AT STA. 98+24 -L- LT
- AT STA. 104+45 -L- LT
- AT STA. 119+50 -L- LT
- AT STA. 147+00 -L- RT
- AT STA. 157+55 -L- RT
- AT STA. 177+08 -L- RT
- AT STA. 186+69 -L- LT
- AT STA. 235+90 -L- RT

8/8/2023

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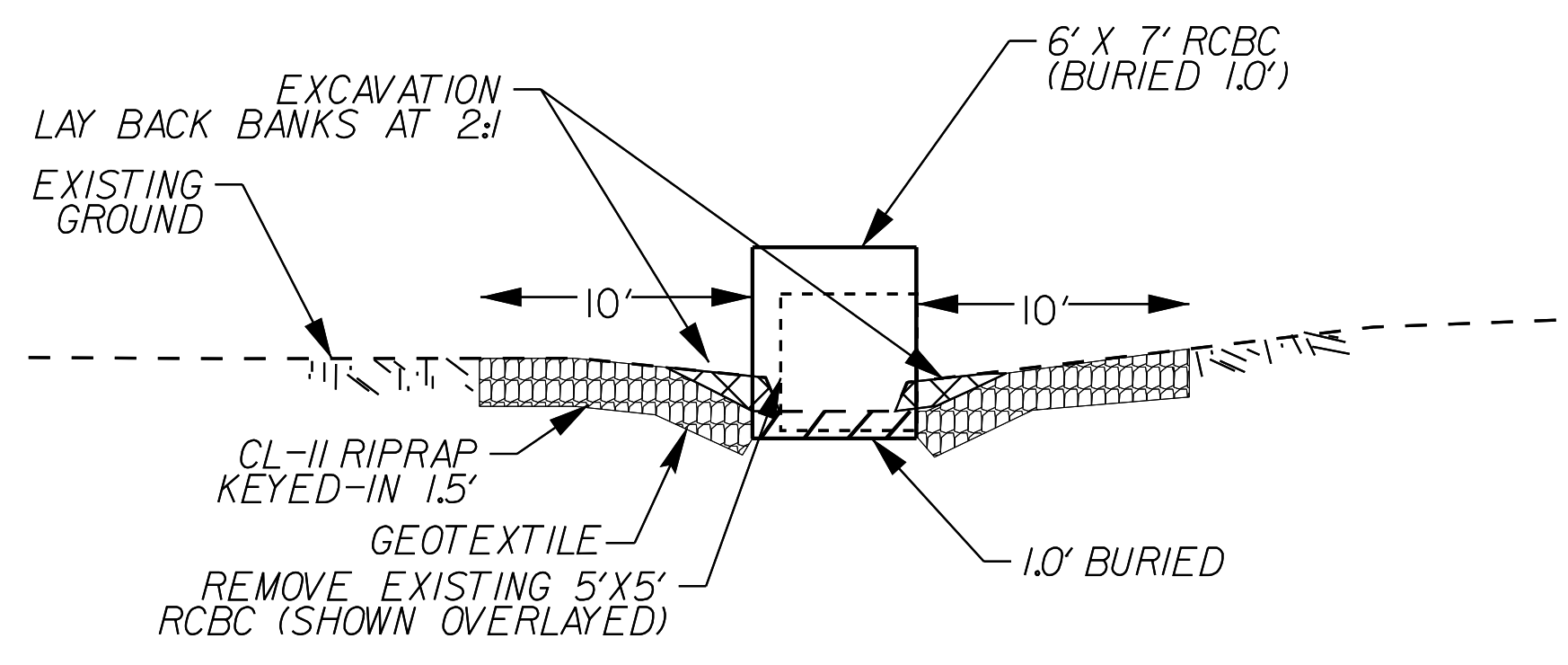
PROJECT REFERENCE NO. R-5705A	SHEET NO. 2D-4
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

**DETAIL 58 TYPICAL OUTLET CHANNEL IMPROVEMENTS
LOOKING DOWNSTREAM
NOT TO SCALE**



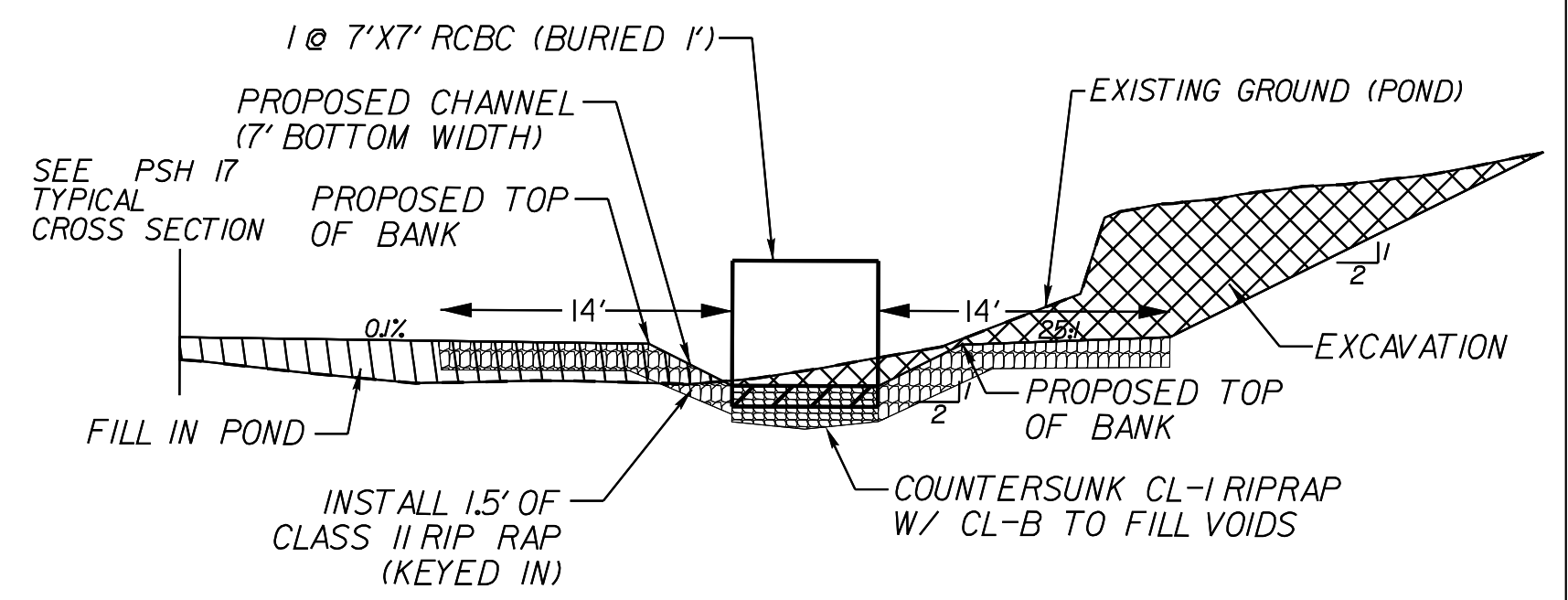
APPROX. STA. 24+10 -L- LT

**DETAIL 59 TYPICAL - INLET CHANNEL IMPROVEMENTS
LOOKING DOWNSTREAM
NOT TO SCALE**



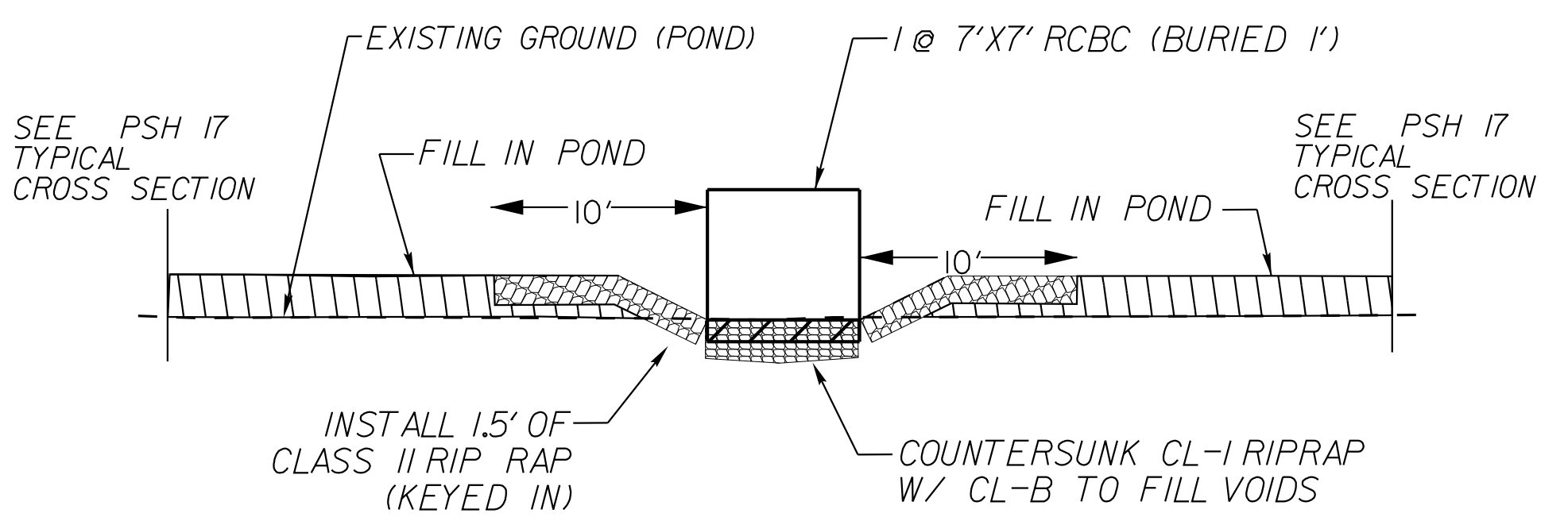
APPROX. STA. 24+70 TO 25+00 -L- RT

**DETAIL 60 TYPICAL OUTLET CHANNEL IMPROVEMENTS
LOOKING DOWNSTREAM
NOT TO SCALE**



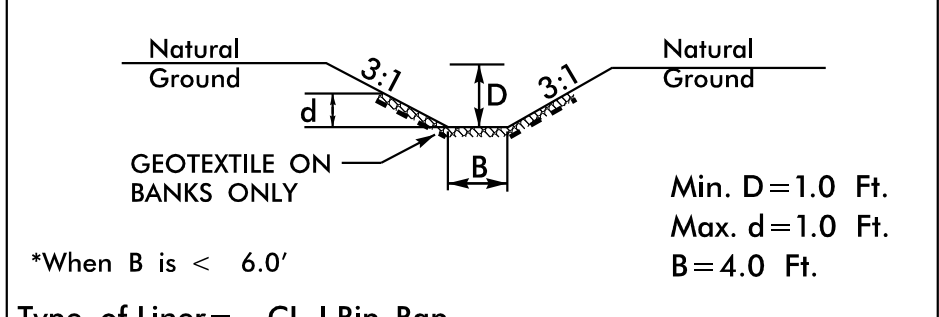
APPROX. STA. 24+10 -L- LT

**DETAIL 61 TYPICAL - INLET CHANNEL IMPROVEMENTS
LOOKING DOWNSTREAM
NOT TO SCALE**



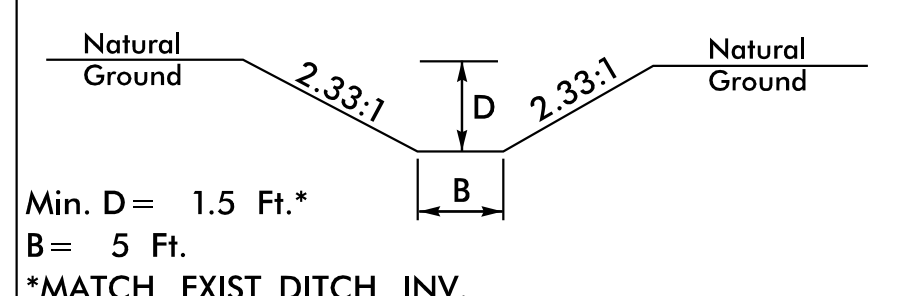
APPROX. STA. 24+10 -L- LT

**DETAIL 62
STANDARD BASE DITCH
(Not to Scale)**



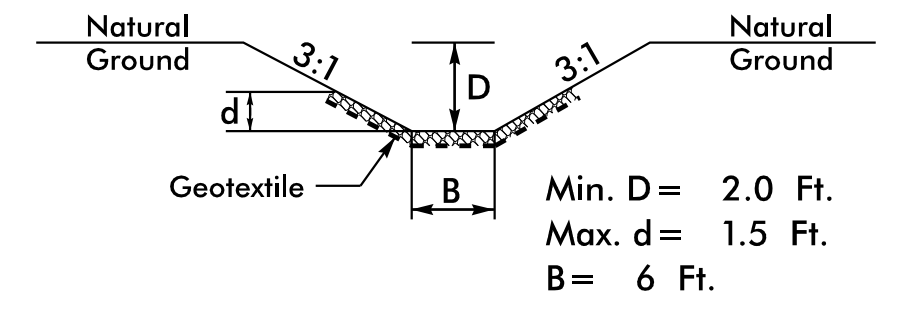
FROM STA. 86+50 TO STA. 86+87 -L- RT

**DETAIL 63
STANDARD BASE DITCH
(Not to Scale)**



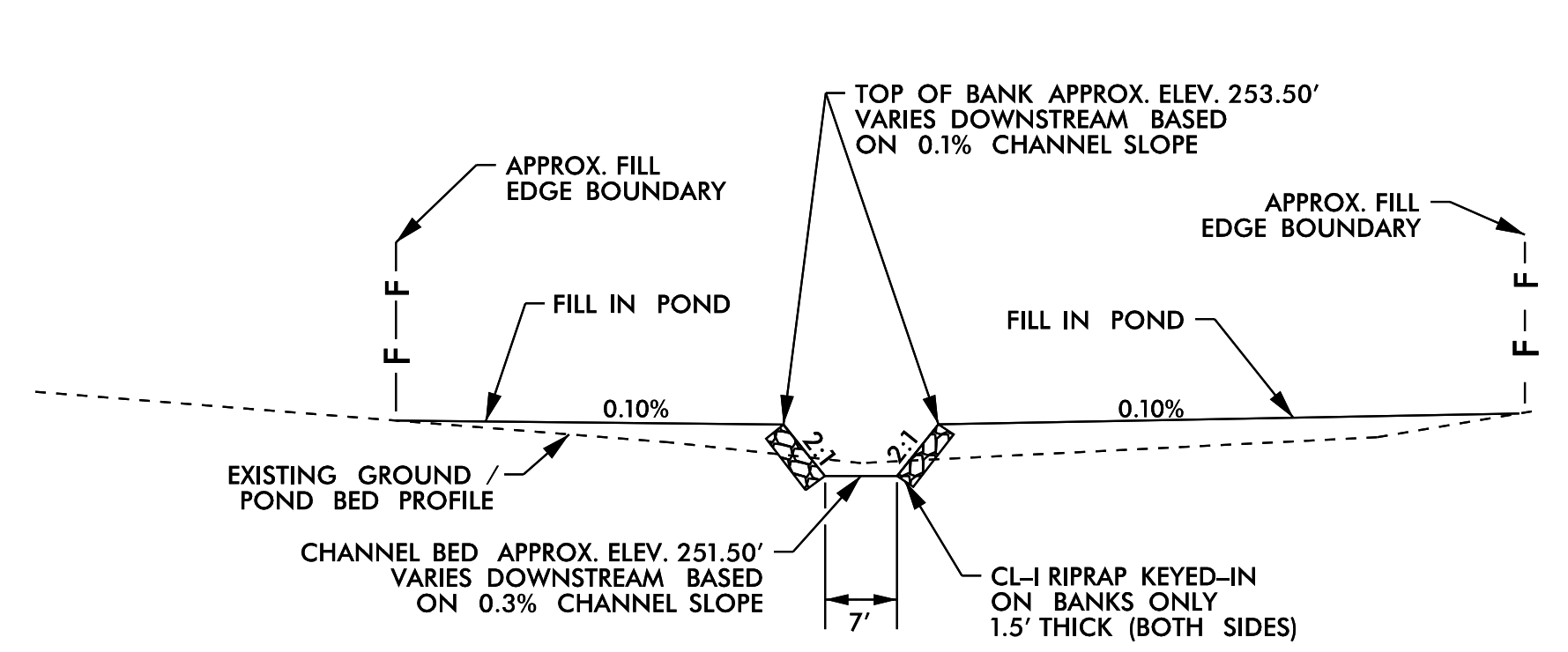
STA. 174+10 -L- LT (EST. 78 L.F.)

**DETAIL 64
STANDARD BASE DITCH
(Not to Scale)**



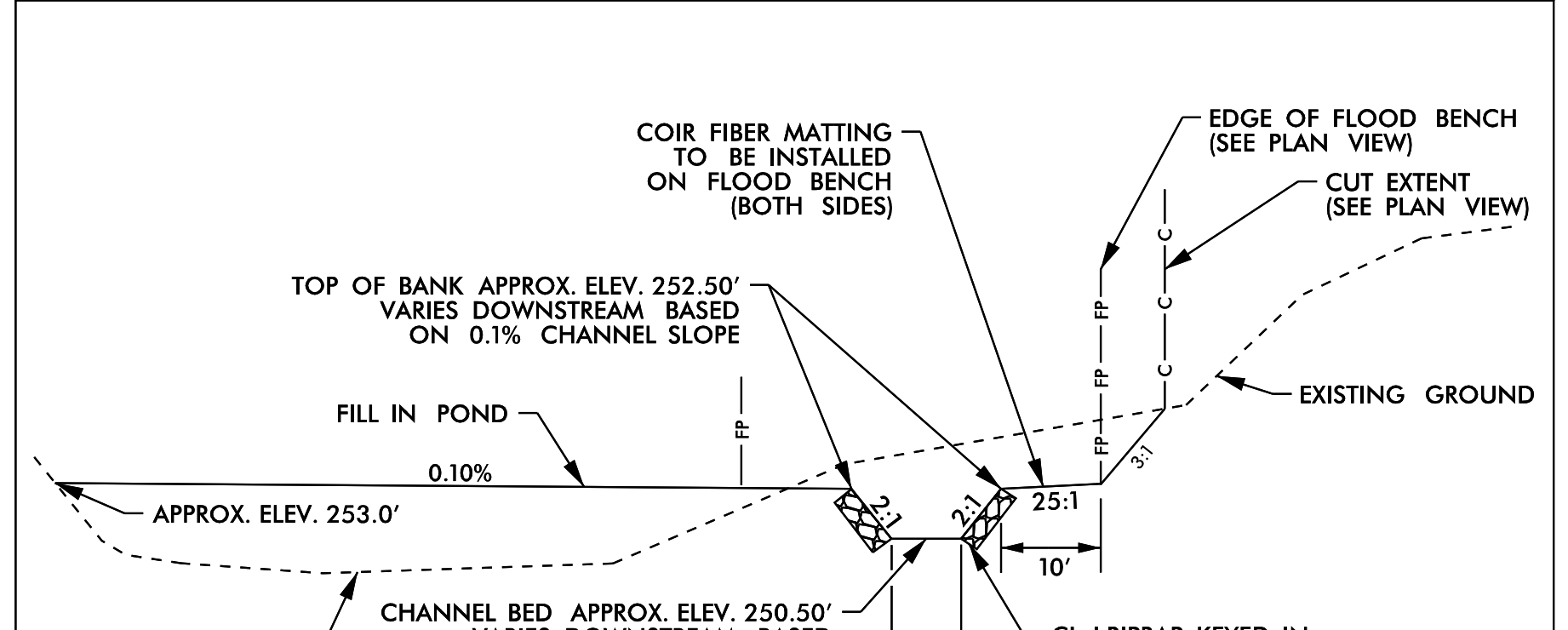
STA. 162+53 -L- LT (EST. 73 L.F.)

DETAIL TYPICAL SECTION P1



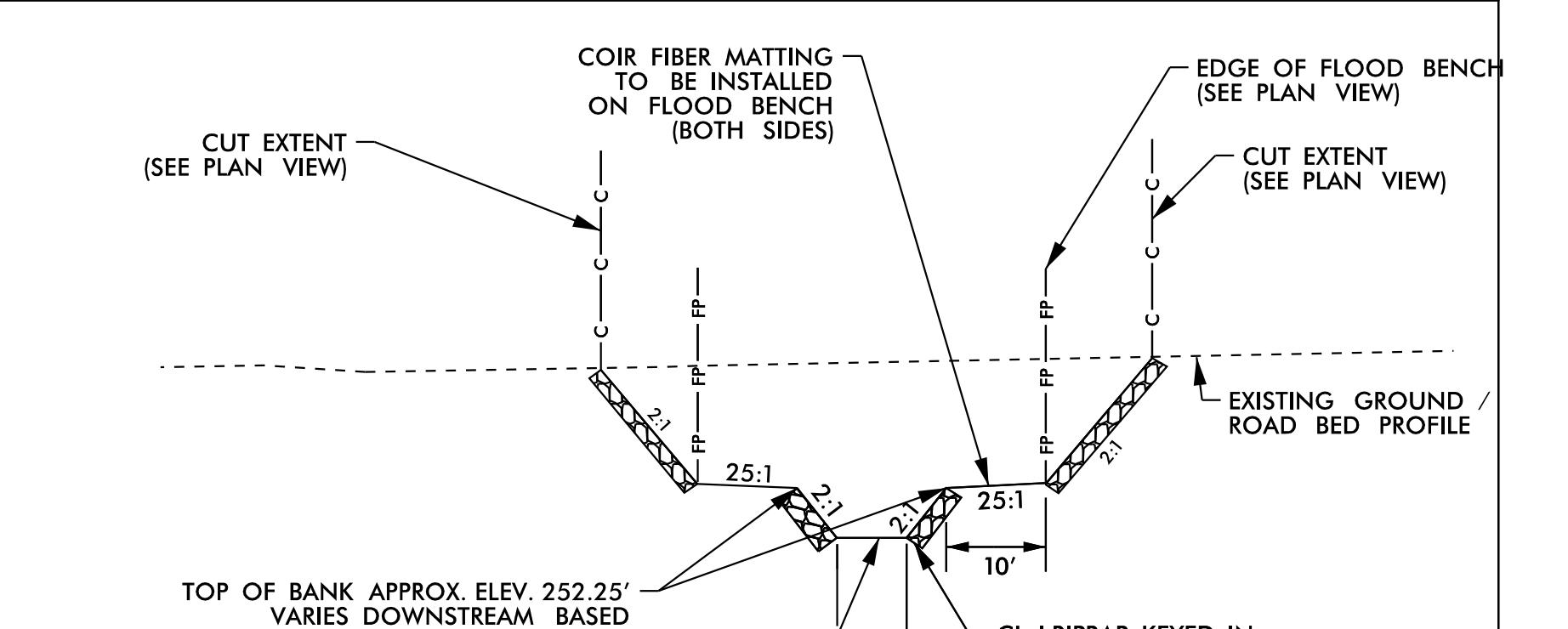
NOT TO SCALE

DETAIL TYPICAL SECTION P2



NOT TO SCALE

DETAIL TYPICAL SECTION P3



NOT TO SCALE

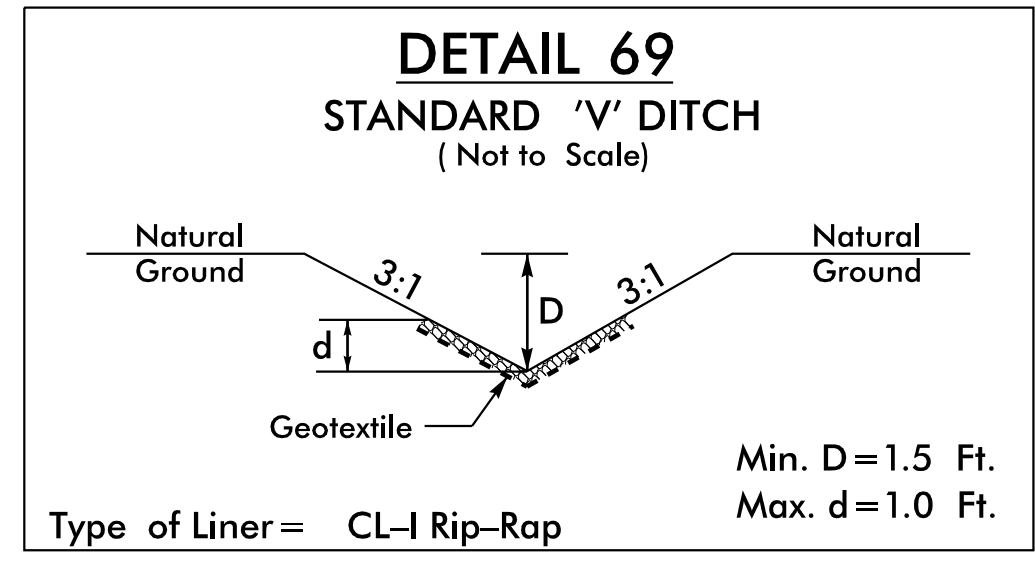
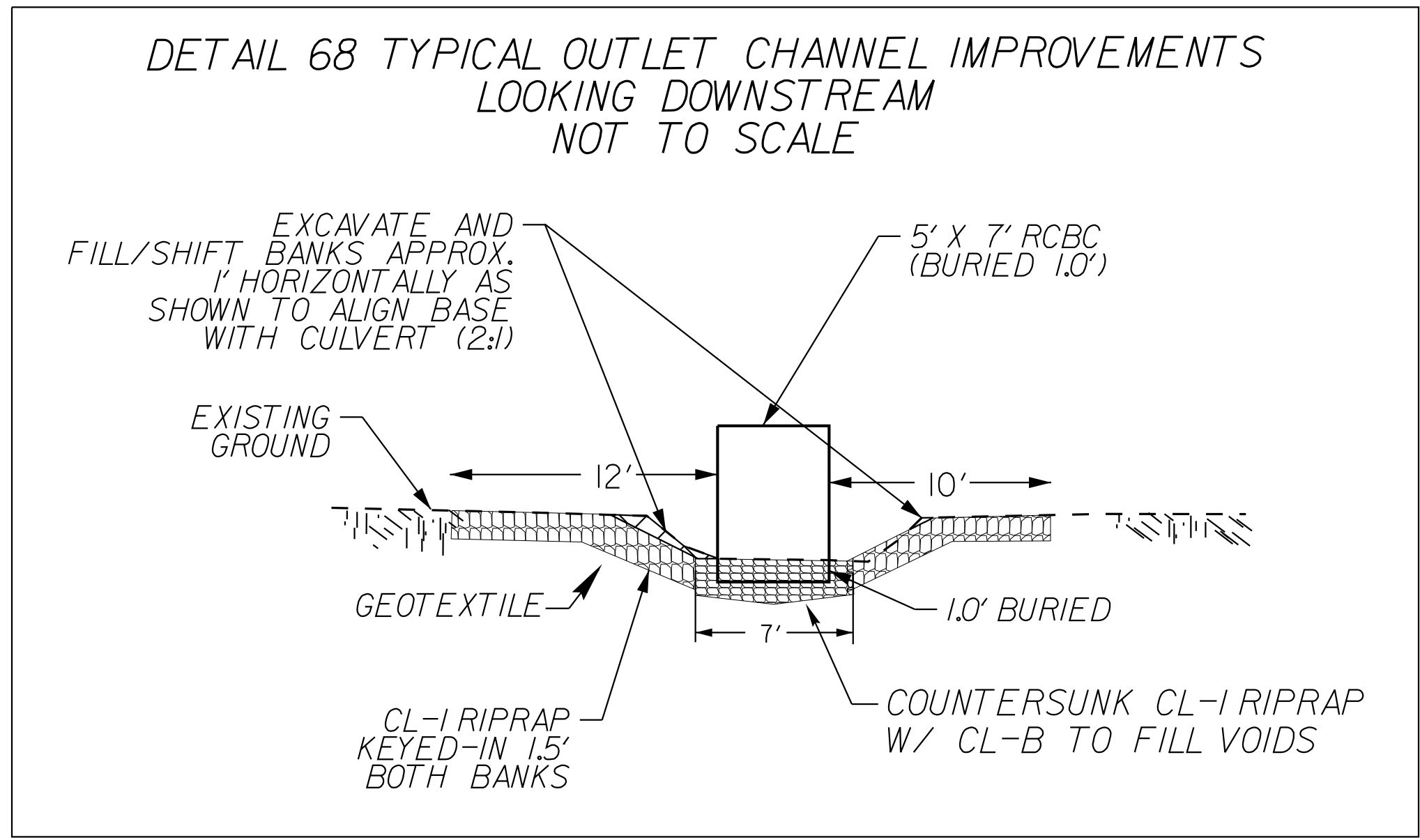
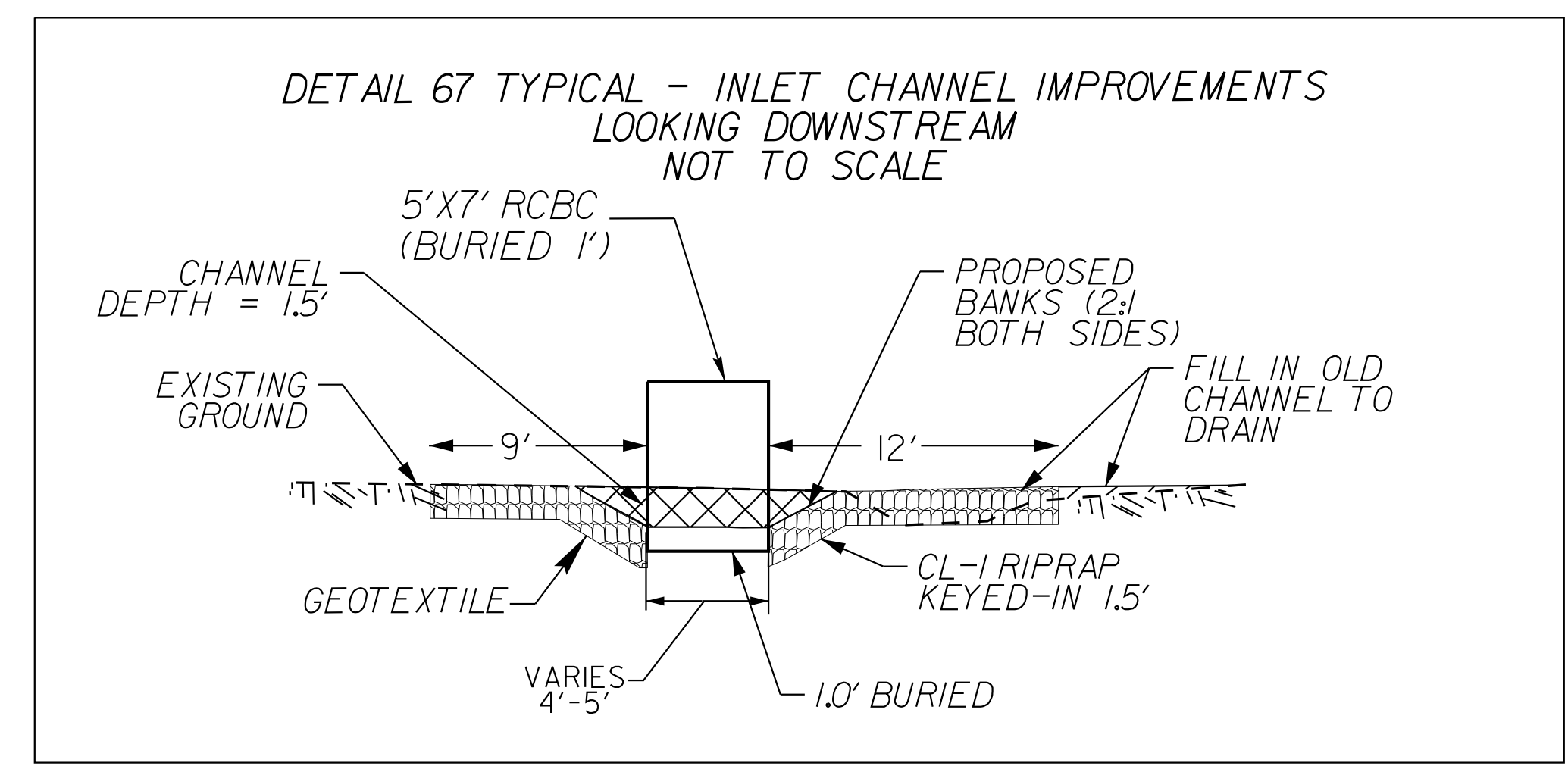
REVISIONS

8/8/2023

5/14/99

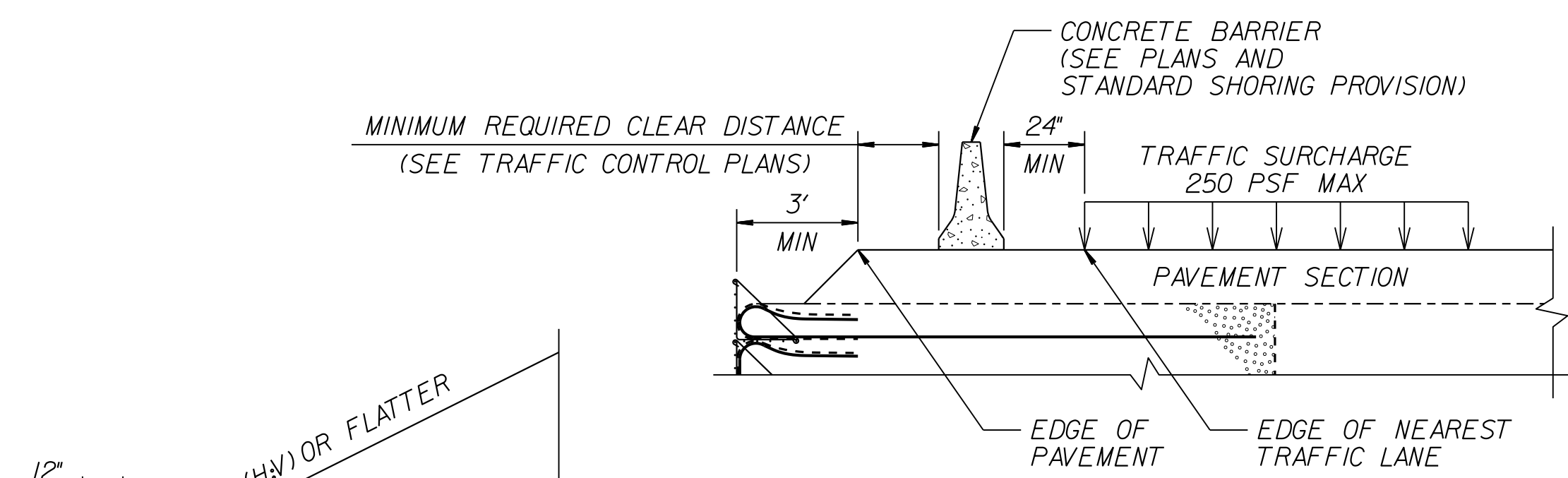
REVISIONS

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2D-5
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

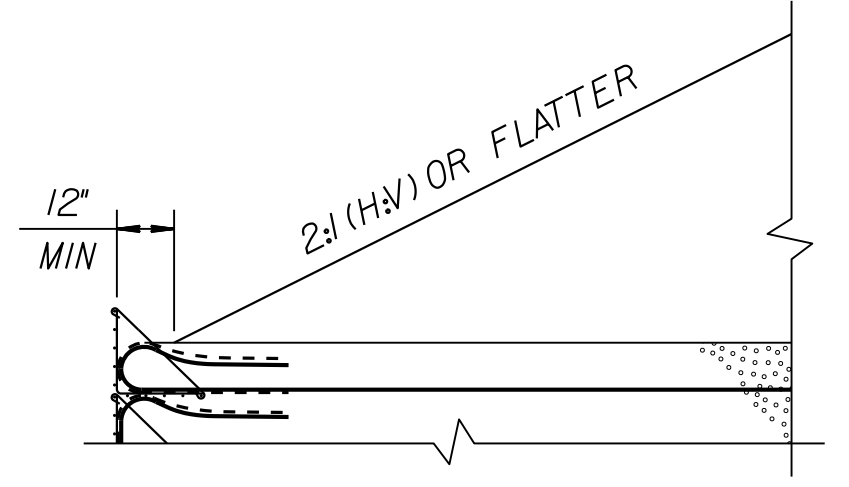


FROM STA. 20+80 TO STA. 23+30 -Y7B- LT
 (EQUATES TO: -L- LT. STA. 195+00 TO STA. 197+26)

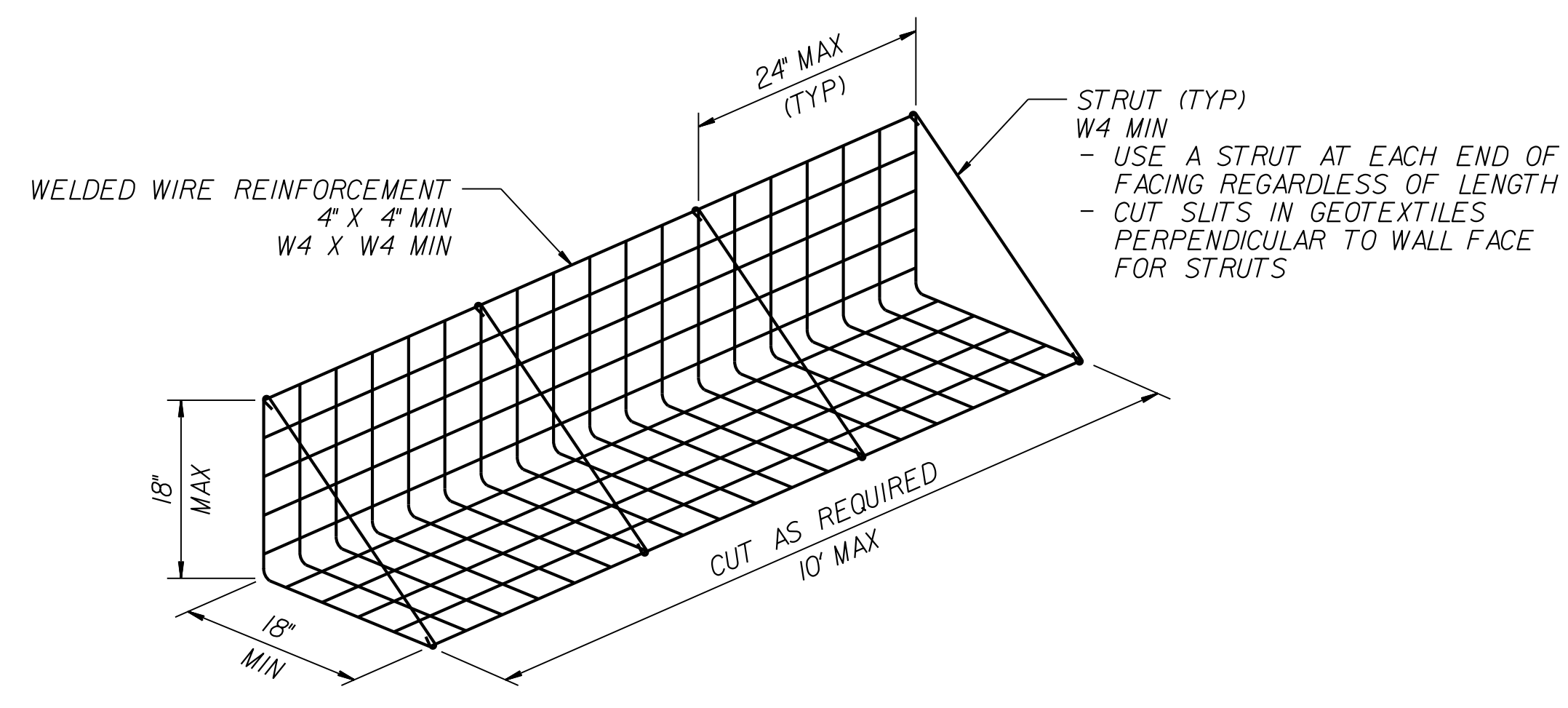
8/8/2023



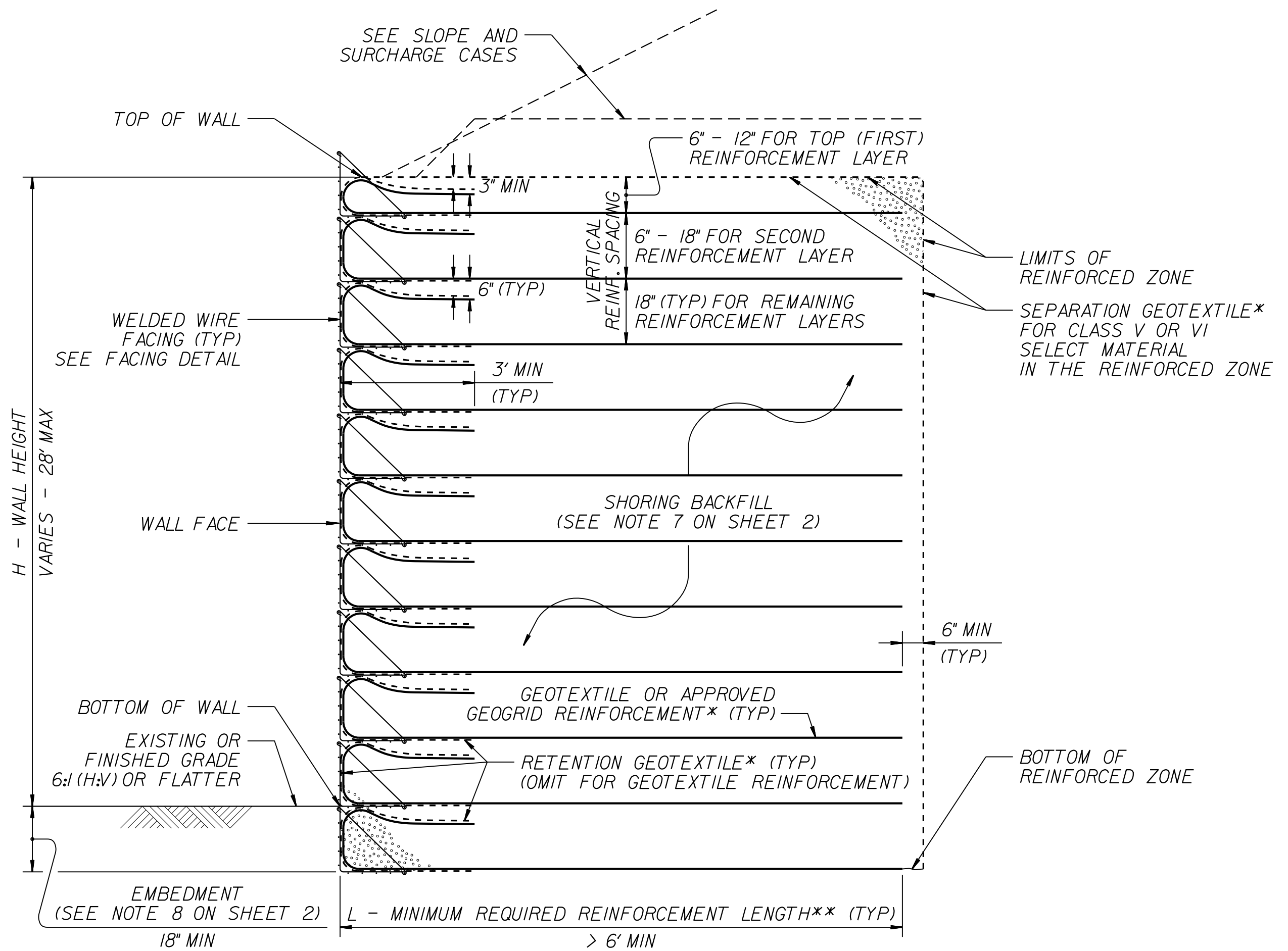
SURCHARGE CASE



SLOPE CASE

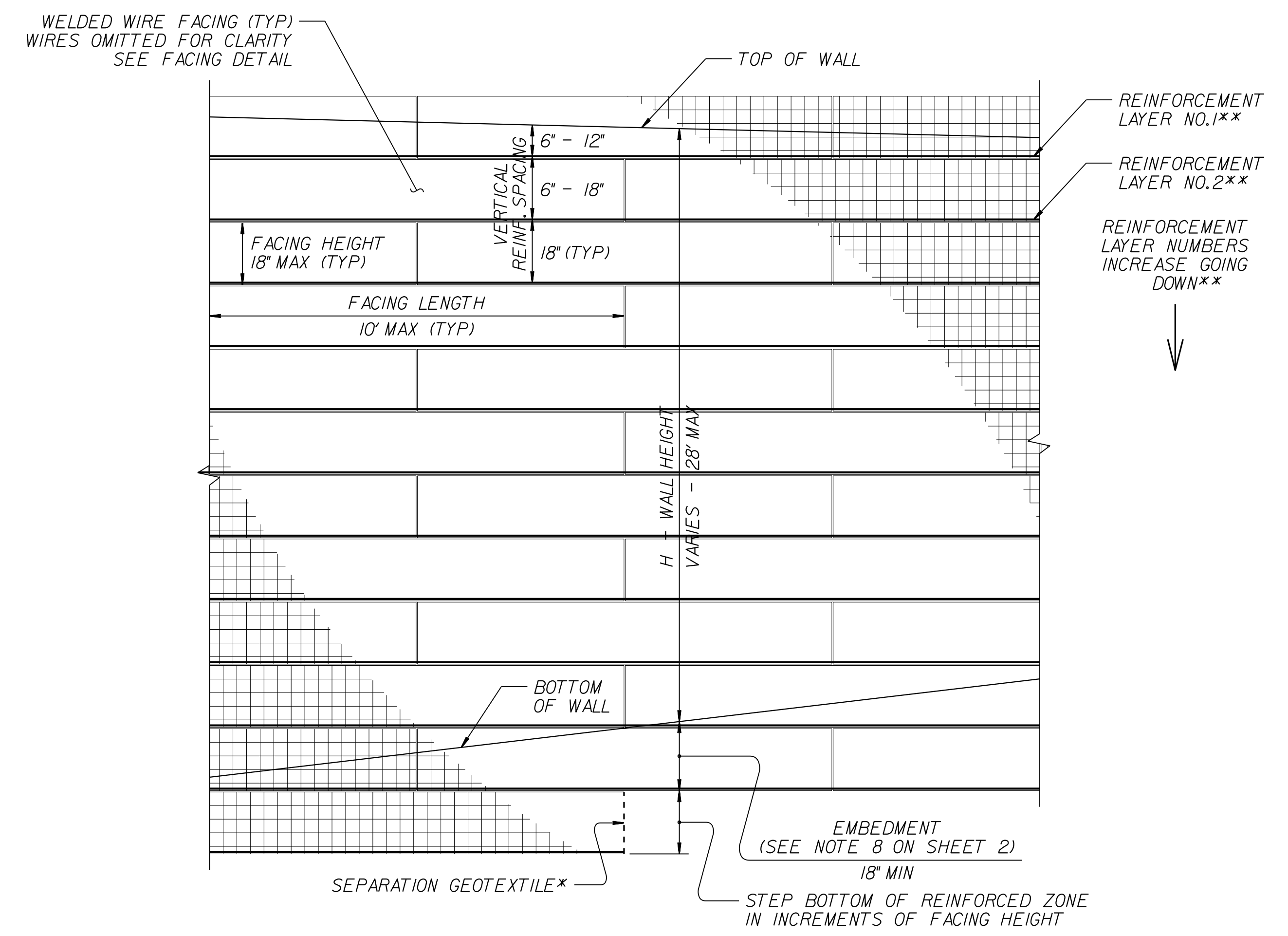


FACING DETAIL



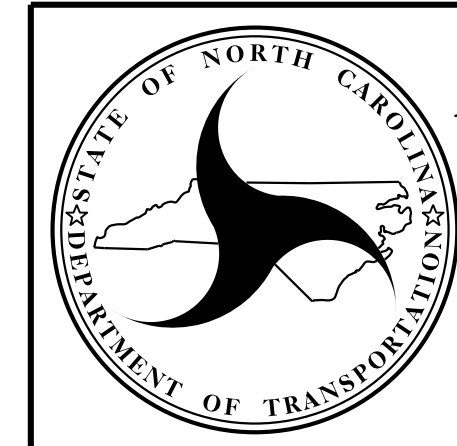
STANDARD TEMPORARY WALL

(FOR STANDARD TEMPORARY WALLS ON STRUCTURES, SEE TEMPORARY WALL ON STRUCTURE DETAIL ON SHEET 2.)
 *SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.
 **SEE REINFORCEMENT TABLES ON SHEET 3.



STANDARD TEMPORARY WALL – PARTIAL ELEVATION

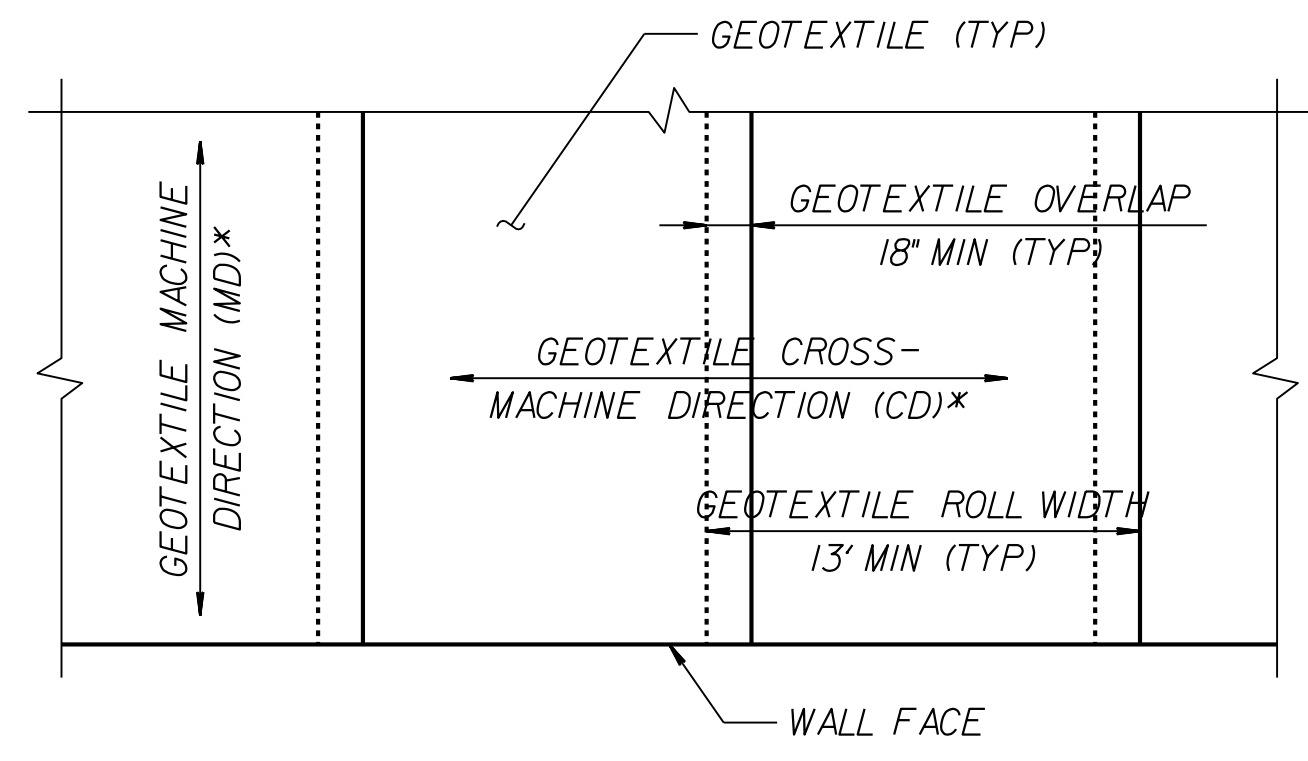
*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.
 **SEE REINFORCEMENT TABLES ON SHEET 3.



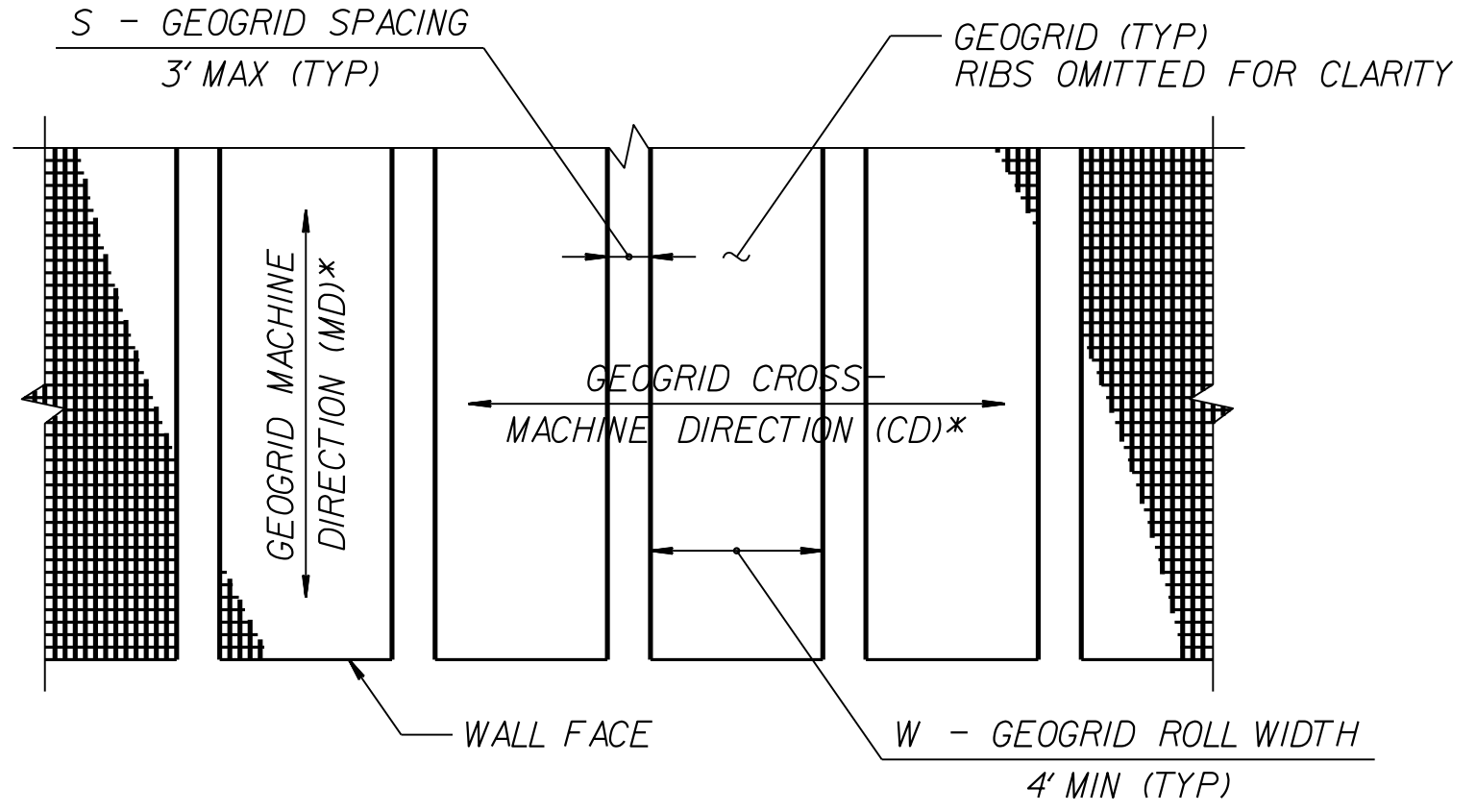
NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
**GEOTECHNICAL
 ENGINEERING UNIT**

STANDARD DETAIL NO. 1801.02

STANDARD
 TEMPORARY WALL
 SHEET 1 OF 3

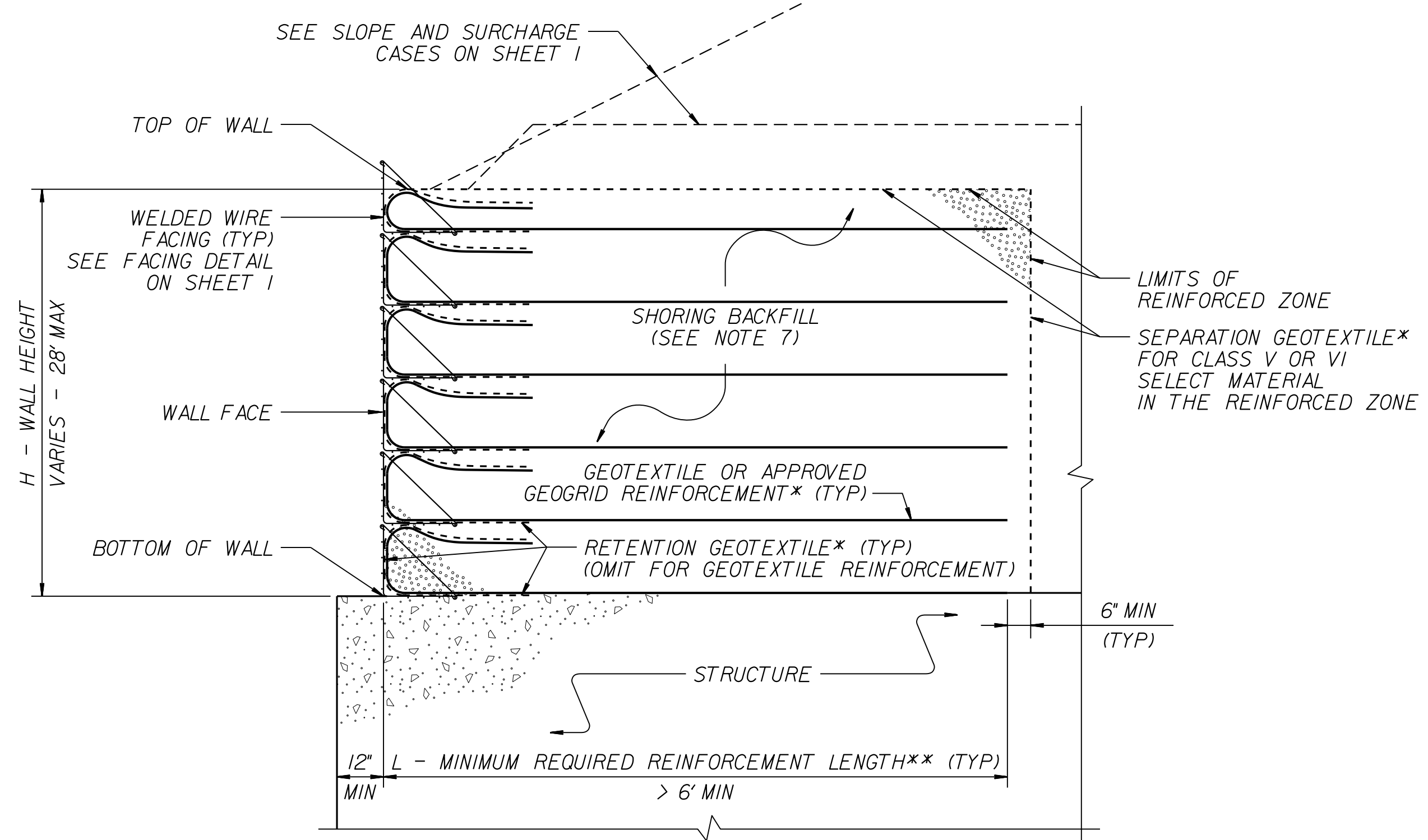


GEOTEXTILE PLACEMENT
(100% COVERAGE MIN FOR GEOTEXTILE REINFORCEMENT)



GEOGRID PLACEMENT
(80% COVERAGE MIN FOR GEOGRID REINFORCEMENT - $\frac{W}{W+S} \times 100 \geq 80\%$, SEE NOTE 11)

GEOSYNTHETIC PLACEMENT DETAILS
(PLAN VIEW)
*SEE NOTE 12.



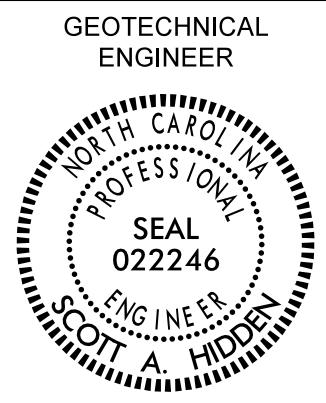
TEMPORARY WALL ON STRUCTURE DETAIL
*SEE GEOSYNTHETIC PLACEMENT DETAILS.
**SEE REINFORCEMENT TABLES ON SHEET 3.

NOTES:

- AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY WALLS AS NOTED IN THE PLANS.
- FOR STANDARD TEMPORARY WALLS, SEE STANDARD SHORING PROVISION.
- STANDARD TEMPORARY WALLS ARE BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS:
UNIT WEIGHT, $\gamma = 120$ PCF
FRICTION ANGLE, $\phi = 30$ DEGREES
COHESION, $c = 0$ PSF
- DO NOT USE STANDARD TEMPORARY WALLS IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
- DO NOT USE STANDARD TEMPORARY WALLS WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS BELOW TEMPORARY WALLS.
- USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, ASSUME GROUNDWATER DEPTH IS LESS THAN 7' BELOW BOTTOM OF REINFORCED ZONE. DO NOT USE STANDARD TEMPORARY WALLS IF GROUNDWATER IS ABOVE BOTTOM OF REINFORCED ZONE.
- DO NOT USE A-2-4 SOIL FOR STANDARD TEMPORARY WALLS AROUND CULVERTS OR IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS FOR SLOPE CASES. DO NOT USE CLASS VI SELECT MATERIAL IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS WITH GEOTEXTILE REINFORCEMENT.
- EMBEDMENT IS NOT REQUIRED FOR STANDARD TEMPORARY WALLS ON STRUCTURES OR ROCK AS DETERMINED BY THE ENGINEER.
- DO NOT USE MORE THAN 4 DIFFERENT REINFORCEMENT STRENGTHS FOR EACH STANDARD TEMPORARY WALL.
- GEOGRIDS ARE TYPICALLY APPROVED FOR ULTIMATE TENSILE STRENGTHS IN THE MACHINE DIRECTION (MD) AND CROSS-MACHINE DIRECTION (CD) OR SHORT-TERM DESIGN STRENGTHS FOR A 3-YEAR DESIGN LIFE IN THE MD BASED ON MATERIAL TYPE. THE LIST OF APPROVED GEOGRIDS WITH DESIGN STRENGTHS IS AVAILABLE FROM: connect.ncdot.gov/resources/Materials/Pages/Materials-Manual-by-Manual.aspx. DEFINE MATERIAL TYPE FROM THE WEBSITE ABOVE FOR SHORING BACKFILL AS FOLLOWS:

MATERIAL TYPE	SHORING BACKFILL
BORROW	A-2-4 SOIL
FINE AGGREGATE	CLASS II, TYPE I OR CLASS III SELECT MATERIAL
COARSE AGGREGATE	CLASS V OR VI SELECT MATERIAL

- IF THE WEBSITE DOES NOT LIST A SHORT-TERM DESIGN STRENGTH FOR AN APPROVED GEOGRID, USE A SHORT-TERM DESIGN STRENGTH EQUAL TO THE ULTIMATE TENSILE STRENGTH DIVIDED BY 3.5 FOR THE GEOGRID REINFORCEMENT.
- FOR GEOGRID REINFORCEMENT WITH LESS THAN 100% COVERAGE, STAGGER REINFORCEMENT SO GEOGRIDS ARE CENTERED OVER GAPS IN THE REINFORCEMENT LAYER BELOW.
 - AT THE CONTRACTOR'S OPTION, REINFORCEMENT MAY BE INSTALLED WITH THE MD PARALLEL TO THE WALL FACE IF BOTH OF THE FOLLOWING CONDITIONS OCCUR:
- W (REINFORCEMENT ROLL WIDTH) \geq (MINIMUM REQUIRED REINFORCEMENT LENGTH) + 4.5' AND
- REINFORCEMENT STRENGTH IN CD \geq MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD.
 - SUBMIT A "STANDARD TEMPORARY WALL SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING TEMPORARY WALL CONSTRUCTION. STANDARD SHORING SELECTION FORMS ARE AVAILABLE FROM: connect.ncdot.gov/resources/Geological/Pages/Geotech_Forms_Details.aspx
 - DO NOT PLACE SHORING BACKFILL OR REINFORCEMENT UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.
 - FOR STANDARD TEMPORARY WALLS WITH PILE FOUNDATIONS IN THE REINFORCED ZONE, DRIVE PILES THROUGH REINFORCEMENT AFTER CONSTRUCTING TEMPORARY WALLS.
 - DO NOT SPLICE OR OVERLAP REINFORCEMENT SO SEAMS ARE PARALLEL TO THE WALL FACE.
 - CONTACT THE ENGINEER WHEN EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT.
 - FOR STANDARD TEMPORARY WALLS WITH INTERIOR ANGLES LESS THAN 90 DEGREES, WRAP GEOSYNTHETICS AT ACUTE CORNERS AS DIRECTED BY THE ENGINEER.
 - FOR STANDARD TEMPORARY WALLS WITH TOP OF WALL WITHIN 5' OF FINISHED GRADE, REMOVE TOP FACING AND INCORPORATE TOP REINFORCEMENT LAYER INTO FILL WHEN PLACING FILL IN FRONT OF WALL.

PROJECT REFERENCE NO. R-5705A	SHEET NO. 2G-3
	ENGINEER
Designated by: <i>Scott A. Hidden</i> 10/22/2018 <small>DATE</small>	<small>SIGNATURE</small>
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

SLOPE OR SURCHARGE CASE	GROUNDWATER DEPTH BELOW BOTTOM OF REINFORCED ZONE (SEE NOTE 6 ON SHEET 2) (FT)	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)	H - WALL HEIGHT (FT)																									
			< 4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	
SLOPE CASE	> 0	CLASS II, TYPE I, CLASS III, CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	8	9	11	12	13	13	14	15	16	17	18	19	20	21	22	23	24	24	25	26	27	27	
SURCHARGE CASE	> 0 TO 7 FOR H < 20' > 0 TO 10 FOR H ≥ 20'	ALL SHORING BACKFILL TYPES	6	7	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	17	17	18	19	19	20	21	22	
		A-2-4 SOIL	6	6	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	16	17	18	18	19	20	20	21	
		CLASS II, TYPE I OR CLASS III SELECT MATERIAL	6	6	7	7	8	8	9	10	10	11	11	12	12	13	14	15	15	16	16	17	17	18	18	19	20	
	> 7 FOR H < 20' > 10 FOR H ≥ 20'	CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	7	8	8	9	9	10	10	11	12	13	13	14	14	15	15	16	17	17	18	19	19		

L - MINIMUM REQUIRED REINFORCEMENT LENGTH (FT)
(FOR ALL REINFORCEMENT TYPES)

WALL HEIGHT (H) + EMBEDMENT (FT)	NUMBER OF REINFORCEMENT LAYERS*
2.5 - 4	3
4 - 5.5	4
5.5 - 7	5
7 - 8.5	6
8.5 - 10	7
10 - 11.5	8
11.5 - 13	9
13 - 14.5	10
14.5 - 16	11
16 - 17.5	12
17.5 - 19	13
19 - 20.5	14
20.5 - 22	15
22 - 23.5	16
23.5 - 25	17
25 - 26.5	18
26.5 - 28	19
28 - 29.5	20

*BASED ON VERTICAL REINFORCEMENT SPACING SHOWN ON SHEET 1.

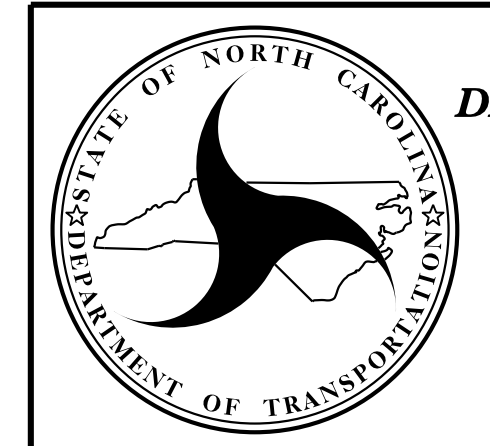
REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL
1	2400	2400	2400	2400	2400
2	2400	2400	2400	2400	2400
3	2400	2400	2400	2400	2400
4	2400	2400	2500	2400	2400
5	2500	2400	3000	2400	2400
6	3000	2400	3500	2800	2400
7	3500	2700	4000	3200	2600
8	4000	3100	4500	3600	2900
9	4500	3500	5000	4000	3200
10	5000	3900	5500	4400	3500
11	5500	4300	6000	4800	3800
12	6000	4700	6500	5200	4100
13	6500	5100	7000	5600	4400
14	7000	5400	7500	6000	4700
15	7500	5800	8000	6400	5000
16	8000	6200	8500	6800	5300
17	8500	6600	9000	7200	5600
18	9000	7000	9500	7600	5900
19	9500	7400	10000	8000	6200
20	10000	7800	10500	8400	6500

GEOTEXTILE REINFORCEMENT
ULTIMATE TENSILE STRENGTH (LB/FT)

REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL
1	240	200	340	290	240
2	380	310	520	430	350
3	530	420	700	570	460
4	690	550	870	720	570
5	860	690	1050	860	680
6	1030	830	1220	1000	790
7	1200	970	1400	1150	900
8	1370	1110	1580	1290	1010
9	1550	1240	1750	1430	1120
10	1720	1380	1930	1580	1230
11	1890	1520	2100	1720	1340
12	2060	1660	2280	1860	1450
13	2240	1800	2450	2010	1560
14	2410	1940	2630	2150	1670
15	2580	2080	2800	2290	1780
16	2750	2220	2980	2440	1890
17	2930	2360	3160	2580	2000
18	3100	2500	3330	2720	2110
19	3270	2640	3510	2860	2220
20	3440	2780	3690	3000	2330

GEOGRID REINFORCEMENT
SHORT-TERM DESIGN STRENGTH (LB/FT)
(SEE NOTE 10 ON SHEET 2.)

MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD
(SEE NOTE 9 ON SHEET 2.)
*SEE PARTIAL ELEVATION ON SHEET 1 FOR REINFORCEMENT LAYER NUMBERING.



NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

**GEOTECHNICAL
ENGINEERING UNIT**

STANDARD DETAIL NO. 1801.02
STANDARD TEMPORARY WALL SHEET 3 OF 3
DATE: 11-19-13



SUMMARY OF EARTHWORK
 IN CUBIC YARDS

LOCATION	EXCAVATION		EMBANKMENT	BORROW	TOTAL WASTE
	TOTAL UNCLASSIFIED	UNDERCUT			
PHASE I					
SECTION 1 (PHASE I)					
-L- 21+21.92 TO -L- 26+55.00 (TEMP PVMT)	56		484	428	
SUBTOTAL	56		484	428	
SECTION 2 (PHASE I)					
-L- 15+58.00 (RT) TO 24+50.00 (RT)	2,816		3,929	1,763	650
SUBTOTAL	2,816		3,929	1,763	650
SECTION 3 (PHASE I)					
-L- 24+50.00 (RT) TO 52+50.00 (RT)	3,990		30,810	26,820	
SUBTOTAL	3,990		30,810	26,820	
SECTION 4 (PHASE I)					
-L- 52+50.00 (RT) TO 82+50.00 (RT)	3,122		13,553	10,431	
-Y2- 10+38.01 TO 13+49.88	310		373	63	
SUBTOTAL	3,432		13,925	10,493	
SECTION 5 (PHASE I)					
-L- 82+50.00 (RT) TO 94+50.00 (RT)	962		13,503	12,541	
SUBTOTAL	962		13,503	12,541	
SECTION 6 (PHASE I)					
-LDET- 94+50.58 TO 100+14.37	664		268		397
SUBTOTAL	664		268		397
SECTION 7 (PHASE I)					
-L- 106+00.00 TO 136+00.00	16,128	8039	50,241	36,523	10,449
POND AT -L- 110+50.00			525	525	
-Y4- 13+61.00 (LT) TO 18+42.99 (LT)	727		153		575
-Y4- 19+20.49 (LT) TO 25+77.00 (LT)	1,423		7,559	6,136	
-Y4A- 19+50.00 (LT) TO 20+08.00 (LT)			851	851	
-Y4B- 15+50.00 (RT) TO 17+00.00 (RT)	19		45	26	
-Y5- 19+76.00 TO 22+22.97	108		206	98	
-Y5A- 10+42.91 TO 13+64.00	68		1,228	1,160	
-Y5DET- 11+69.18 TO 12+94.56	9		75	66	
-Y5DET- 14+10.49 TO 15+88.94	56		204	148	
SUBTOTAL	18,538	8,039	61,086	45,533	11,024
SECTION 8 (PHASE I)					
-L- 136+00.00 TO 165+00.00	9,988	10,071	59,039	50,731	11,751
SUBTOTAL	9,988	10,071	59,039	50,731	11,751
SECTION 9 (PHASE I)					
-Y7DET1- 12+46.51 TO 27+86.59	233		649	416	
-Y7- 23+30.00 (LT) TO 32+60.28 (LT)	1,294		4,149	4,015	1,160
-Y7- 33+40.63 (LT) TO 37+60.00 (LT)	1,142		283		860
-Y7A- 46+91.77 TO 47+90.00	5		74	69	
SUBTOTAL	2,674		5,154	4,499	2,020
PHASE II					
SECTION 10 (PHASE II)					
-L- 15+58.00 (LT) TO 24+50 (LT)	1,311		930		381
SUBTOTAL	1,311		930		381
SECTION 11 (PHASE II)					
-L- 24+50.00 (LT) TO 52+50.00 (LT)	4,509		1,001		3,508
-Y1- 10+50.00 TO 14+50.00	224		240	16	
SUBTOTAL	4,733		1,241	16	3,508

QUANTITIES ARE APPROXIMATE ONLY. THE RESIDENT ENGINEER WILL RE-CROSS SECTION THE WORK ACCURATELY WHEN THE PROJECT IS STAKED OUT. THESE CROSS SECTION NOTES WILL BE USED IN COMPUTING THE FINAL QUANTITIES FOR WHICH THE CONTRACTOR WILL BE PAID.

SUMMARY OF EARTHWORK
 IN CUBIC YARDS

LOCATION	EXCAVATION		EMBANKMENT	BORROW	TOTAL WASTE
	TOTAL UNCLASSIFIED	UNDERCUT			
PHASE I					
SECTION 12 (PHASE II)					
-L- 52+50.00 (LT) TO 82+50.00 (LT)	3,727		2,033		1,695
SUBTOTAL	3,727		2,033		1,695
SECTION 13 (PHASE II)					
-L 82+50.00 (LT) TO 94+50.00 (LT)	1,164		2,391	1,227	
-Y3- 10+88.00 TO 13+36.74	240		524	284	
SUBTOTAL	1,404		2,915	1,511	
SECTION 14 (PHASE II)					
-L- 94+50.00 TO 100+00.00	2,110		1,013		1,098
-Y4B- TURN 10+02.00 TO 11+10.00	64				64
SUBTOTAL	2,174		1,013		1,162
SECTION 15 (PHASE II)					
-L- 100+00.00 TO 106+00.00	7,012	3,201	4,545		5,668
SUBTOTAL	7,012	3,201	4,545		5,668
SECTION 16 (PHASE II)					
-L- 165+00.00 TO 181+00.00	5,078	1,752	36,389	32,671	3,112
POND AT -L- 180+50.00			1,288	1,288	
-Y6- 13+41.00 TO 16+16.60	58		1,631	1,573	
-Y6- 16+95.86 TO 22+44.00	854		161		693
-Y6A- 10+12.69 TO 11+17.00	81		3		79
-DRW2- 10+43.00 TO 12+00.00	15		634	619	
SUBTOTAL	6,086	1,752	40,105	36,150	3,883
SECTION 17 (PHASE II)					
-L- 184+00.00 TO 198+50.00	21,055	3,142	27,804	11,549	7,942
POND AT -L- 197+00.00			3,875	3,875	
-DRW5- 10+43.00 TO 11+07.00	313				313
-DRW7- 10+43.00 TO 11+45.00	32		351	319	
SUBTOTAL	21,400	3,142	32,030	15,743	8,255
SECTION 18 (PHASE II)					
-L- 198+50.00 TO 233+00.00	19,939	5,175	91,571	76,992	10,535
-Y7- 25+50.00 (RT) TO 37+60.00 (RT)	40		5,466	5,426	
-Y7C- 13+73.45 TO 16+50.00	473		56		417
-Y7C- TURN 19+84.00 TO 20+84.00	18		20	2	
-Y7D- 13+30.00 TO 19+66.00	5,517		163		5,355
-DRW8- 10+28.00 TO 10+75.00	8		113	105	
-DRW10- 10+22.00 TO 10+85.00	9		93	84	
SUBTOTAL	26,004	5,175	97,481	82,609	16,306
SECTION 19 (PHASE II)					
-L- 233+00.00 TO 246+00.00	7,282	3,432	18,924	13,462	5,252
SUBTOTAL	7,282	3,432	18,924	13,462	5,252

REVISIONS

1/3/2023

SUMMARY OF EARTHWORK IN CUBIC YARDS



REVISIONS

LOCATION	EXCAVATION		EMBANKMENT	BORROW	TOTAL WASTE
	TOTAL UNCLASSIFIED	UNDERCUT			
PHASE III					
SECTION 20 (PHASE III)					
-L- 94+50.00 TO 103+50.00	634		475		159
SUBTOTAL	634		475		159
SECTION 21 (PHASE III)					
-L- 172+00.00 TO 181+00.00	2,240		10,999	8,759	
-Y6A- TURN 16+66.00 TO 18+19.00	38		25		13
-DRW3- 10+43.00 TO 12+38.00	100		303	203	
-DRW4- 10+43.00 TO 11+26.90	164		1		163
SUBTOTAL	2,542		11,328	8,961	176
SECTION 22 (PHASE III)					
-L- 181+00.00 TO 193+50.00	1,614		7,040	5,426	
-Y7B- TURN 18+00.00 TO 19+00.00	36		20		16
SUBTOTAL	1,650		7,060	5,426	16
SECTION 23 (PHASE III)					
-Y4- 13+61.00 (RT) TO 15+00.00 (RT)	20		13		8
-Y4- 23+00.00 (RT) TO 25+77.00 (RT)	7		531	524	
-Y4A- 17+50.00 (RT) TO 19+50.00 (RT)			458	458	
-Y4A- TURN 11+40.00 TO 12+90.74	72		25		47
-Y4B- 15+50.00 (LT) TO 16+50.00 (LT)	34				34
SUBTOTAL	133		1,026	982	89
SECTION 24 (PHASE III)					
-Y5- 21+00.00 TO 21+78.82	28		1		27
-Y5B- TURN 12+15.00 TO 13+19.00	75				75
SUBTOTAL	103		1		102
PHASE IV					
SECTION 25 (PHASE IV)					
-Y5- 21+00.00 (LT) TO 22+00.00 (LT)	48				48
-Y5A- 10+50 (RT) TO 12+00.00 (RT)	91				91
SUBTOTAL	139				139
SECTION 26 (PHASE IV)					
-Y7- 23+30.00 (LT) TO 24+50.00 (LT)	125				125
SUBTOTAL	125				125
TOTAL	129,579	34,812	409,305	317,668	72,758
MATERIAL FOR SHOULDER CONSTRUCTION			16,438	16,438	
LOSS DUE TO CLEARING AND GRUBBING	-12,000			12,000	
ADD'L UNDERCUT FROM GEOTECH RECS		2,250	2,813	2,813	2,250
SELECT GRANULAR MATERIAL IN LIEU OF BORROW			-58,750	-58,750	
WASTE IN LIEU OF BORROW				-17,686	-17,686
PROJECT TOTAL	117,579	37,062	369,805	272,482	57,322
EST 5% TO REPLACE TOP SOIL ON BORROW PIT				13,624	
GRAND TOTAL	117,579	37,062	369,805	286,106	57,322
SAY	118,000	37,500		286,500	
ESTIMATED DDE = 3,390 CY					
ESTIMATED SHALLOW UNDERCUT = 1,120 CY					
PAVEMENT STRUCTURE VOLUME = 6,600 SY					
ACCEPTABLE UNCLASSIFIED EXCAVATION NOT TO BE USED IN TOP 3FT OF EMBANKMENT OR BACKFILL = 8,000 CY					

QUANTITIES ARE APPROXIMATE ONLY. THE RESIDENT ENGINEER WILL RE-CROSS SECTION THE WORK ACCURATELY WHEN THE PROJECT IS STAKED OUT. THESE CROSS SECTION NOTES WILL BE USED IN COMPUTING THE FINAL QUANTITIES FOR WHICH THE CONTRACTOR WILL BE PAID.

1/3/2023



DIVISION OF HIGHWAYS
 STATE OF NORTH CAROLINA
GUARDRAIL SUMMARY

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL
 TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT
 FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL
 W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL

G = GATING IMPACT ATTENUATOR TL-3 or TL-2
 NG = NON-GATING IMPACT ATTENUATOR TL-3 or TL-2

SURVEY LINE	BEG. STA.	END STA.	LOCATION	LENGTH (L.F.)			WARRANT POINT		"N" DIST. FROM E.O.L.	TOTAL SHOULDER WIDTH	FLARE LENGTH (L.F.)		W		ANCHORS								IMPACT ATTENUATOR		REMOVE EXISTING GUARDRAIL (L.F.)			TERMINAL END SECTION (EA.)	REMARKS								
				STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END			APPROACH END	TRAILING END	APPROACH END	TRAILING END	Type III	B-77	GREU, TL-3	GREU, TL-2	CAT-1	AT-1	Type III SC	B-77 SC	IA-MASH TL-3	G	NG	SINGLE FACED CONCRETE BARRIER	REMOVE EXISTING GUARDRAIL			REMOVE & STOCKPILE EXISTING GUARDRAIL							
-L-	179+06.00	182+12.25	RT	306.25					10'	10'	50	6.25																									
-L-	181+13.75	184+45.00	LT	331.25					10'	10'	50	6.25																									
-L-	196+63.00	199+19.25	RT	256.25					10'	10'	50	6.25																									
-L-	198+06.75	201+63.00	LT	356.25					10'	10'	50	6.25																									
-Y4A-	11+35.00		LT/RT	25.00																																2	
-Y4B-	09+97.00		LT/RT	25.00																																2	
-Y5B-	13+24.00		LT/RT	25.00																																2	
-Y6A-	18+24.00		LT/RT	25.00																																2	
-Y7B-	19+05.00		LT/RT	25.00																																2	
-Y7B-	19+83.94	23+46.04	RT																																428		
-Y7B-	19+85.24	23+46.04	LT																																428		
-Y7C-	16+05.00		LT/RT	25.00																																2	
-Y7C-	19+79.00		LT/RT	25.00																																2	
SUBTOTAL				1425.00																																14	
LESS ANCHOR DEDUCTIONS																																					
GREU, TL-3				4 @ 50'	=	200.00																															
CAT-1				4 @ 6.25'	=	25.00																															
TOTAL				1200.00																																	14
SAY				1200																																	14

ADDITIONAL GUARDRAIL POSTS = 10 EA

LINE	STATION	STATION	LOCATION	SQUARE YARDS
-L-	63+85.00	65+85.00	RT	62
-L-	98+75.91	105+97.91	LT/RT	2679
-L-	131+96.57	137+32.98	LT/RT	2453
-L-	165+53.61	165+74.84	LT/RT	97
-L-	172+70.79	180+00.00	RT	1537
-L-	183+50.00	198+60.81	LT	3596
-Y4-	21+53.43	24+43.02	RT	677
-Y4-	14+21.25	16+38.64	LT	413
-Y6-	17+33.00	18+71.70	LT	182
-Y6-	18+91.88	21+68.91	LT	265
-Y6A-	17+76.03	18+02.59	LT	4
-Y7-	26+25.00	26+50.00	LT/RT	80
-Y7-	28+50.00	30+00.00	LT/RT	401
-Y7D-	13+30.00	17+86.19	LT/RT	969
-Y7C-	15+45.17	15+71.19	RT	7
-Y7C-	15+95.01	19+84.01	LT/RT	796
-LDET-	97+90.12	100+14.37	RT	46
-Y5DET-	11+69.18	12+94.56	LT	90
-Y5DET-	14+10.49	1588.94	LT	146
-Y7DET1-	12+46.51	2199.03	RT	683
-Y7DET1-	23+81.61	2786.59	RT	265
-Y7DET2-	22+52.23	2386.08	LT	73
TOTAL				15521
SAY				15600

LINE	STATION	STATION	LOCATION	SQUARE YARDS
-L-	180+00	183+50	L/R	776
-Y6-	15+25	16+55	L/R	296
-Y7-	26+50	28+50	L/R	643
-Y7-	29+00	29+50	L/R	89
-Y7-	30+00	34+25	L/R	1484
TOTAL				3289
SAY				3300

REVISIONS

LL2592

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROJECT NO. SHEET NO.
R-5705A 3D-1

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.
See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Table with columns for Line & Station, Offset, Structure Number, Top Elevation, Invert Elevation, Minimum Required Slope, Drainage Pipe (RCP, CSP, CAAP, HDPE, PVC, or PP Pipe), C.S. Pipe, R.C. Pipe Class IV, Endwalls, Reinforced Endwalls, Drainage Structure, Quantities for Drainage Structures, Frame, Grates, and Hood, Concrete Transitional Section, and Remarks. Includes a SHEET TOTALS row at the bottom.

ABBREVIATIONS
C.A.A. CORRUGATED ALUMINIUM ALLOY
C.B. CATCH BASIN
C.S. CORRUGATED STEEL
D.I. DROP INLET
G.D.I. GRATED DROP INLET
H.D.P.E. HIGH DENSITY POLYETHYLENE
J.B. JUNCTION BOX
M.H. MANHOLE
N.S. NARROW SLOT
P.V.C. POLYVINYL CHLORIDE
R.C. REINFORCED CONCRETE
T.B.D.I. TRAFFIC BEARING DROP INLET
T.B.J.B. TRAFFIC BEARING JUNCTION BOX
W.S. WIDE SLOT

LL2562

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROJECT NO. SHEET NO.
R-5705A 3D-2

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Table with columns for Line & Station, Offset, Structure Number, Invert Elevation, Minimum Required Slope, Pipe Size, Material (Drainage Pipe, C.S. Pipe, R.C. Pipe), Endwalls, Reinforced Endwalls, Drainage Structure, Quantities for Drainage Structures, Frame, Grates, and Hood, Grate Type, and Remarks. Includes a SHEET TOTALS row at the bottom.

ABBREVIATIONS table listing codes like C.A.A., C.B., C.S., D.I., G.D.I., H.D.P.E., J.B., R.C., T.B.D.I., T.B.J.B., W.S. and their corresponding material names.

SHEET TOTALS

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROJECT NO. SHEET NO.
R-5705A 3D-3

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.
See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Table with columns: LINE & STATION, OFFSET, STRUCTURE NUMBER, TOP ELEVATION, INVERT ELEVATION, MINIMUM REQUIRED SLOPE, Drainage Pipe (RCP, CSP, CAAP, HDPE, PVC, or PP PIPE), C. S. PIPE, R. C. PIPE CLASS IV, ENDWALLS, REINFORCED ENDWALLS, DRAINAGE STRUCTURE, QUANTITIES FOR DRAINAGE STRUCTURES, FRAME, GRATES, AND HOOD, CONCRETE TRANSITIONAL SECTION, GRATE TYPE, FLOWABLE FILL, CONCRETE COLLARS, PIPE REMOVAL, ABBREVIATIONS, and REMARKS.

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

PROJECT NO. SHEET NO.
R-5705A 3D-5

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.
See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Table with columns: LINE & STATION, OFFSET, STRUCTURE NUMBER, TOP ELEVATION, INVERT ELEVATION, MINIMUM REQUIRED SLOPE, Drainage Pipe (RCP, CSP, CAAP, HDPE, PVC, or PP PIPE), C. S. PIPE, R. C. PIPE CLASS IV, ENDWALLS, REINFORCED ENDWALLS, MASONRY, QUANTITIES FOR DRAINAGE STRUCTURES, FRAME, GRATES, AND HOOD, CONCRETE TRANSITIONAL SECTION, GRATE TYPE, FLOWABLE FILL, CONCRETE COLLARS, PIPE REMOVAL, ABBREVIATIONS, and REMARKS.

SHEET TOTALS

LL2592

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROJECT NO. SHEET NO.
R-5705A 3D-7

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.
See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Table with columns: LINE & STATION, SIZE, THICKNESS OR GAUGE, OFFSET, STRUCTURE NUMBER, TOP ELEVATION, INVERT ELEVATION, MINIMUM REQUIRED SLOPE, Drainage Pipe (RCP, CSP, CAAP, HDPE, PVC, or PP PIPE), C. S. PIPE, R. C. PIPE CLASS IV, ENDWALLS, REINFORCED ENDWALLS, DRAINAGE STRUCTURE, QUANTITIES FOR DRAINAGE STRUCTURES, FRAME, GRATES, AND HOOD, CONCRETE TRANSITIONAL SECTION, and REMARKS. Includes a SHEET TOTALS row at the bottom.

ABBREVIATIONS table listing codes and descriptions for materials like CORRUGATED ALUMINIUM ALLOY, CATCH BASIN, CORRUGATED STEEL, etc.

LL2582

COMPUTED BY: JCB DATE: 2022-01-03
CHECKED BY: LDR DATE: 2023-01-03

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROJECT NO. SHEET NO.
R-5705A 3D-8

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

Main data table with columns for Line & Station, Offset, Structure Number, Invert Elevation, Pipe Type (Drainage Pipe, C.S. Pipe, R.C. Pipe), Quantities for Drainage Structures, Frame, Grates, and Hood, and Remarks. Includes sub-totals for SHEET TOTALS and PROJECT TOTALS.

ABBREVIATIONS table listing codes like C.A.A., C.B., C.S., D.I., G.D.I., H.D.P.E., J.B., M.H., N.S., P.V.C., R.C., T.B.D.I., T.B.J.B., W.S. and their corresponding material descriptions.

SHEET TOTALS and PROJECT TOTALS summary rows at the bottom of the table.

COMPUTED BY: WPA DATE: 7/2023
 CHECKED BY: _____ DATE: _____

(2-3-23)

PROJECT NO. R-5705A	SHEET NO. 3G-1
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**STATE OF NORTH CAROLINA
 DIVISION OF HIGHWAYS**

SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTINGENCY			SD	2000
				TOTAL LF:	2000

*UD = Underdrain
 *BD = Blind Drain
 *SD = Subsurface Drain

SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type* ASU(1/2)/ AST	Aggregate Thickness INCHES [8" for ASU(2)]	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Soil Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
L	16+75	21+75	ASU (1)	12	390	1370	2200		
L	70+25	72+25	ASU (1)	12	55	220	350		
Y7	23+75	25+75	ASU (1)	12	20	235	380		
Y7	28+25	29+25	ASU (1)	12	50	100	170		
Y7	33+45	36+75	ASU (1)	12	105	875	1400		
	CONTINGENCY		ASU (1)	12	500	1000	1000		
	CONTINGENCY		AST	3					250
			TOTAL CY/TONS/SY:		1120	3800**	5500**	0	250

*ASU(1/2) = Aggregate Subgrade (Type 1 or 2)
 *AST = Aggregate Stabilization

**Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Soil Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

PARCEL INDEX SHEET

PARCEL No.	SHEET No.	PROPERTY OWNER NAME	DEED BOOK
1A	4	JAMES JOHNSON	DB 615 PG 21 PB 2016 PG 364
1	4	JAMES JOHNSON	DB 731 PG 732
2	4,5	JAMES JOHNSON	DB 730 PG 223
3	4	TONY WARREN	DB 3271 PG 355
4	4	HOWARD BRIDGES	DB 1754 PG 517
5	4,5	DANNY HEDGEPEETH	DB 926 PG 126 PC # E PG 76A
6	5,6	JESSE MABRY	DB 915 PG 464
7	5	JEFFREY WALTERS	DB 968 PG 612
8	5	DANNY HEDGEPEETH	DB 1241 PG 820 MB 2014 PG 9
9	5	CHARLES GRAY	DB 3252 PG 220
10	5,6	CHARLES GRAY	DB 1442 PG 909 MB 2004 PG 19
11	6	NANCY SILVERS	DB 1101 PG 767
12	6	CHARLES GRAY	DB 2830 PG 173
13	6	SHERRY OATES	DB 1822 PG 69 DB 2007 PG 518
14	6	SHERRILL MCLAMB	DB 786 PG 810
15	6	NANCY SILVERS	DB 1101 PG 767
15	6,7	NANCY SILVERS	DB 1101 PG 767
15	6	NANCY SILVERS	DB 1101 PG 767
18	6	SHERRILL MCLAMB	DB 738 PG 754
19	6	SHERRILL MCLAMB	DB 839 PG 291
20	6	VIRGINIA HOLLIDAY	DB 635 PG 530
21	6,7	SHERRILL MCLAMB	DB 738 PG 754
22	6,7	NANCY SILVERS	DB 1159 PG 680
23	7	NANCY SILVERS	DB 1229 PG 887
24	7	A AND L PROPERTIES LLC	DB 3350 PG 890
25	7	TONI GAINES	DB 3183 PG 524 MB 2013 PG 217
26	7	JEAN SMITH	DB 3175 PG 397 MB 2013 PG 271
27	7	CARRIE BULLARD	DB 502 PG 121
28	7,8	CAROL BLALOCK	DB 88E PG 258
29	7	CARRIE BULLARD	DB 695 PG 318
30	7	LUCILLE PATE	DB 634 PG 613
31	7,8	CURTIS ADAMS	DB 508 PG 169
32	8	DANIEL MORTON	DB 3441 PG 835 MB 2008 PG 1042
33	8	MARIO HERNANDEZ	DB 2857 PG 342 MB 2011 PG 303
34	8	JAMES GARNER	DB 1145 PG 360

PARCEL No.	SHEET No.	PROPERTY OWNER NAME	DEED BOOK
35	8	JAMES GARNER	DB 1602 PG 219 MB 2002 PG 303
35	8	JAMES GARNER	DB 1071 PG 120
36	8	BARBARA HENRIES	DB 2007 PG 3
37	8	D&Q MOTOR SPORTS LLC	DB 2803 PG 711
39	8	SYLVIA THOMPSON	NO DEED INFO PC # F PG 326C
40	8	JIMMY WALTERS	DB 1102 PG 980 PC # F PG 326C
41	8	BUCKEYE GAS PRODUCTS	DB 817 PG 394
42	8,9	JIMMY WALTERS	DB 606 PG 222
43	8	LYNN MUCHINSON	DB 1508 PG 838
44	8,9	DONALD GREGORY	DB 1665 PG 80 MB 2001 PG 1260
45	8,9	JEANETTE STRICKLAND	DB 1405 PG 740 MB 2000 PG 84
46	8,9	WADE BURT	DB 2967 PG 15 MB 2011 PG 317
47	9	MR CURRIN	DB 1315 PG 641
48	8,9	JRT MANAGING PROPERTIES, LLC	DB 2458 PG 329 MB 2006 PG 930
49	9,10	CLAY E. GARDNER & DENISE P. GARDNER	DB 3677 PG 997
50	9	FRANK D'ALESIO	DB 3604 PG 42 MB 2017 PG 390
51	9,10	ISABEL LOREDO	DB 3258 PG 986 MB 2014 PG 313
52	9,10	W E COLLIER	DB 2768 PG 34
53	10	RILDA COLLIER LIFE ESTATE	DB 3278 PG 600
54	10	RILDA COLLIER LIFE ESTATE	DB 3278 PG 598
55	10,11	CLAY E. GARDNER & DENISE P. GARDNER	DB 3636 PG 222 MB 2009 PG 275
56	10	JERRY TUCKER	DB 2208 PG 537 MB 2001 PG 641
57	10,11	RIVER RUN DEVELOPERS	DB 3454 PG 424
58	10,11	CLAY GARDNER	DB 3454 PG 422 MB 2009 PG 275
59	11	CLAY GARDNER	DB 3454 PG 424 MB 2009 PG 277
60	11,12	HERMAN WOOD	DB 904 PG 981
60	12	HERMAN WOOD	DB 612 PG 58
61	12	JAMES BURGIN	DB 1880 PG 98 MB 2000 PG 565
61A	12	JAMES BURGIN	DB 1321 PG 0296
62	12,13	TONY MABRY	DB 1417 PG 6 MB 2000 PG 279
63	13	CHRISTY JORDAN	DB 3524 PG 729 MB 2000 PG 466
64	13	JOSEPH LIPSCOMB	DB 1903 PG 67
65	13	LORI MCVEIGH PERRY & LAYTON ANDREW PERRY	DB 3743 PG 300
66	13,14	WILLIAM HAWLEY	DB 1889 PG 108 MB 2016 PG 67
67	13,14	DANIEL HONEYCUTT	DB 3182 PG 966

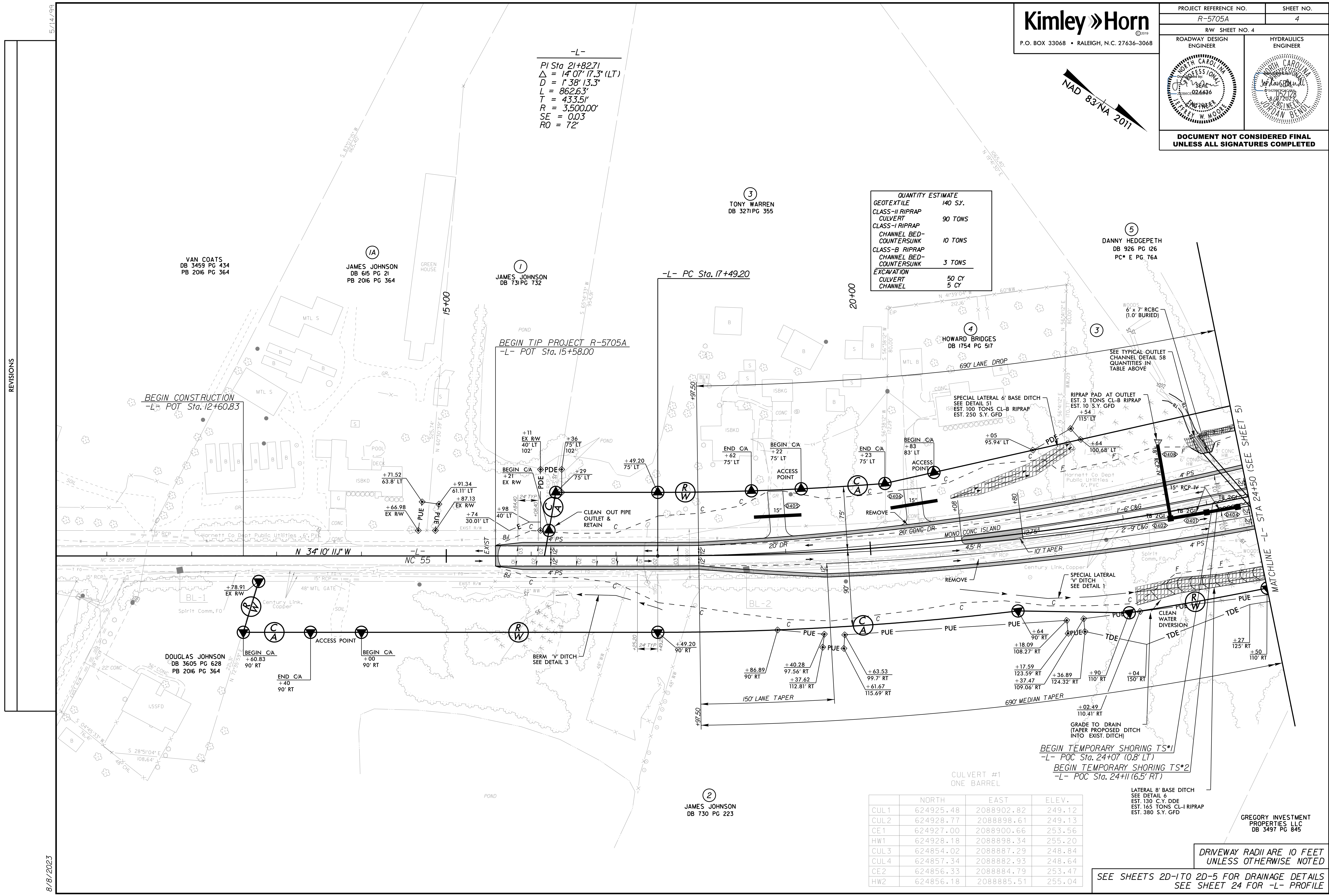
PROJECT REFERENCE NO. R-5705A	SHEET NO. 4
RW SHEET NO. 4	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



-L-
 PI Sta 21+82.71
 $\Delta = 14^{\circ}07'17.3''$ (LT)
 $D = 1^{\circ}38'13.3''$
 $L = 862.63'$
 $T = 433.5'$
 $R = 3,500.00'$
 $SE = 0.03$
 $RO = 72'$

QUANTITY ESTIMATE

GEOTEXTILE	140 SY.
CLASS-II RIPRAP	
CULVERT	90 TONS
CLASS-I RIPRAP	
CHANNEL BED-COUNTERSUNK	10 TONS
CLASS-B RIPRAP	
CHANNEL BED-COUNTERSUNK	3 TONS
EXCAVATION	
CULVERT	50 CY
CHANNEL	5 CY



REVISIONS

8/8/2023

② JAMES JOHNSON
DB 730 PG 223

③ TONY WARREN
DB 327 PG 355

⑤ DANNY HEDGEPEETH
DB 926 PG 126
PC* E PG 76A

CULVERT #1
ONE BARREL

	NORTH	EAST	ELEV.
CUL1	624925.48	2088902.82	249.12
CUL2	624928.77	2088898.61	249.13
CE1	624927.00	2088900.66	253.56
HW1	624928.18	2088898.34	255.20
CUL3	624854.02	2088887.29	248.84
CUL4	624857.34	2088882.93	248.64
CE2	624856.33	2088884.79	253.47
HW2	624856.18	2088885.51	255.04

LATERAL 8" BASE DITCH
 SEE DETAIL 6
 EST. 130 C.Y. DDE
 EST. 165 TONS CL-I RIPRAP
 EST. 380 S.Y. GFD

GREGORY INVESTMENT
PROPERTIES LLC
DB 3497 PG 845

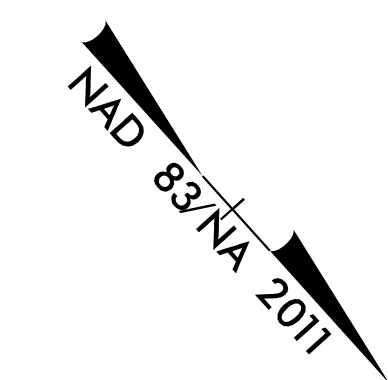
DRIVEWAY RADII ARE 10 FEET
 UNLESS OTHERWISE NOTED
 SEE SHEETS 2D-1 TO 2D-5 FOR DRAINAGE DETAILS
 SEE SHEET 24 FOR -L- PROFILE

5/14/99

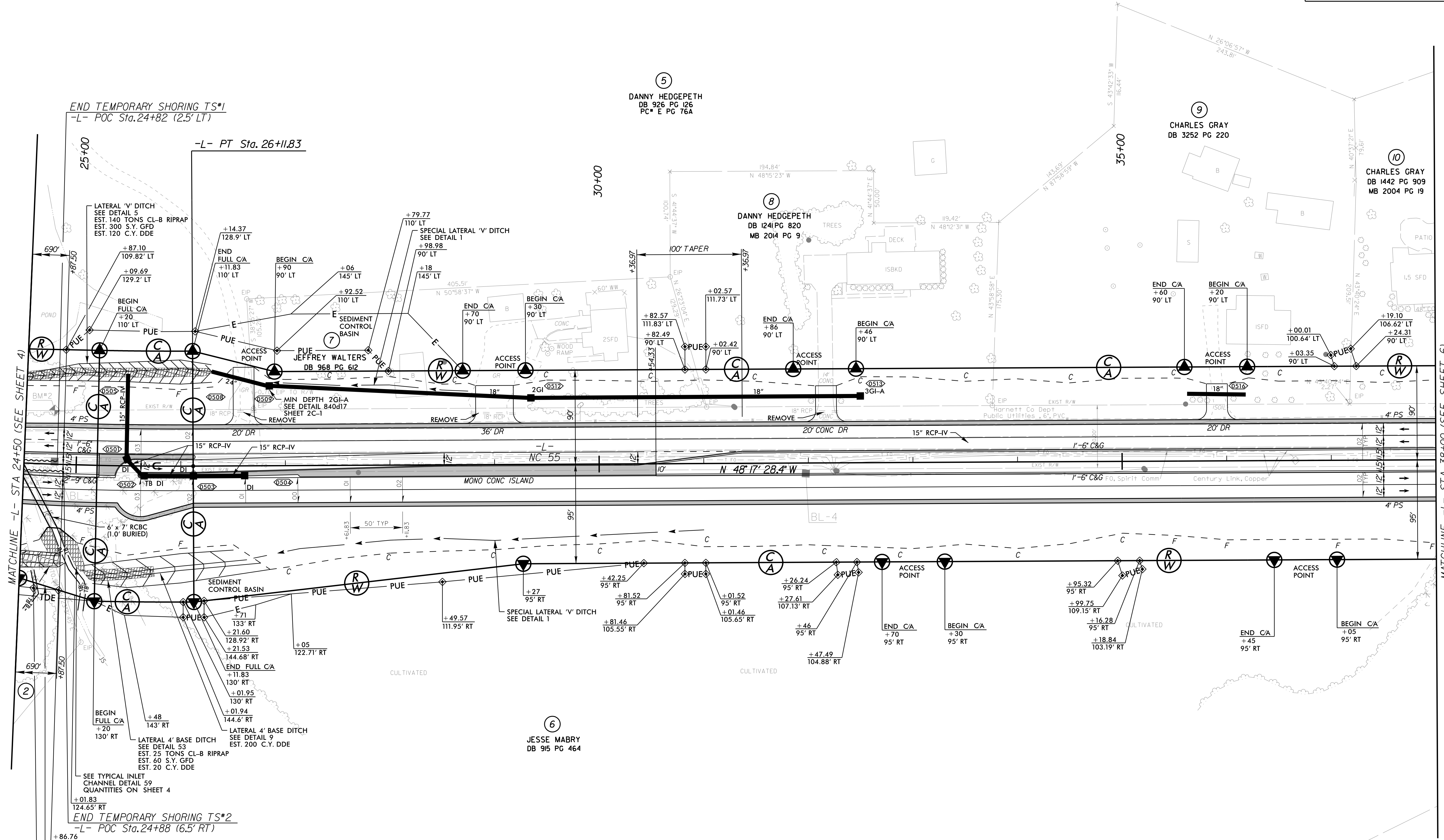
-L-

PI Sta 21+82.71
 $\Delta = 14' 07" 17.3" (LT)$
 $D = 1' 38" 13.3"$
 $L = 862.63'$
 $T = 433.51'$
 $R = 3,500.00'$
 $SE = 0.03$
 $RO = 150'$

PROJECT REFERENCE NO. R-5705A		SHEET NO. 5	
RW SHEET NO. 5		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



REVISIONS



MATCHLINE -L- STA 24+50 (SEE SHEET 4)

MATCHLINE -L- STA 38+00 (SEE SHEET 6)

DRIVEWAY RADII ARE 10 FEET UNLESS OTHERWISE NOTED

SEE SHEET 2B-1 FOR INTERSECTION DETAILS 1 AND 2
SEE SHEETS 2D-1 TO 2D-5 FOR DRAINAGE DETAILS
SEE SHEET 24 FOR -L- PROFILE

8/8/2023