GEOTECHNICAL REPORT DUAL BRIDGES ON U.S. 401 OVER NEUSE RIVER WAKE COUNTY, NORTH CAROLINA N.C.DOT NO. 8.1402103 (R-2425C) LAW JOB NO. 30720-6-1415





June 21, 1996

North Carolina Department of Transportation P.O. Box 25201 Geotechnical Unit Raleigh, North Carolina 27611

Attention:

Mr. W.L. Moore, III, State Engineering Geologist

DESCRIPTION:

DUAL BRIDGES ON U.S. 401 OVER NEUSE RIVER

SUBJECT:

GEOTECHNICAL REPORT

BRIDGE FOUNDATION INVESTIGATION FOR

DUAL BRIDGES ON U.S. 401 OVER NEUSE RIVER

WAKE COUNTY, NORTH CAROLINA N.C.DOT NO. 8.1402103 (R-2425C) LAW PROJECT NO. 30720-6-1415

Dear Mr. Moore:

Law Engineering and Environmental Services, Inc. (Law) is pleased to submit the attached geotechnical report for the above-referenced project. A Geotechnical Report Review Checklist for Site Investigations and other supportive documents are also attached.

Our services were provided in accordance with Law Engineering and Environmental Services, Inc. Proposal No. 30720-6-03550 dated May 6, 1996. We performed our services under the terms and conditions of the Engineering Agreement made and entered into on November 13, 1995 by and between the N.C.DOT and Law Engineering and Environmental Services.

We are available to discuss our recommendations with you and to provide additional studies or services necessary to complete the project. We have enjoyed assisting you on this project and look forward to serving as your geotechnical consultant on future projects.

Very truly yours,

LAW ENGINEERING AND ENVIRONMENTAL SERVICES, INC

Brian K. Banks, G.I.T.

BKB/JAT/bkb/pap/tag

Staff Geologist

J. Allan Tice, P.E.

Corporate Geotechnical Consultan

AUG

Registered, North Carolina

Attachments

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1.0 PROJECT DESCRIPTION

Project information has been provided by the Geotechnical Unit of N.C.DOT. We have received the following information:

1. County map of site

- 2. Bridge Survey and Hydraulic Design Reports dated January 31, 1996 with attached preliminary roadway plan and profiles.
- Location and surveys unit property contact report dated April 25, 1994.
- Preliminary roadway plan dated December 11, 1995.

The bridges are to be 91 meters long with each bridge having 2 span lengths of 28 meters and 2 span lengths of 17.5 meters. The bridges are skewed at 90 degrees to the Neuse River. Interior bents 1 and 2 fall within the channel of the Neuse River. A future 3.6 meter greenway is proposed between Interior Bent 3 and End Bent 2.

The purpose of this geotechnical investigation was to provide the general subsurface investigation data for the design and construction of the new structures. In addition, this investigation was to provide soil properties and geologic data for the 100 year scour determination. This report, including the profile and cross sections, is based on all data obtained from our investigation performed during May, 1996.



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2.0 SITE DESCRIPTION & GEOLOGY

2.1 Site Description

The bridge site is located on U.S. 401 where it crosses the Neuse River, approximately 5.5 kilometers northeast of the Raleigh, North Carolina city limits. A Site Location map is provided as Drawing No. 1 in the Appendix.

The land immediately surrounding the site is undeveloped and wooded, covered by large pines and hardwoods with a thin understory. The area around End Bent 2 of the existing bridge is clear and open, providing easy access for most vehicles via a dirt access road that runs from Mitchell Mill Road southwestward along U.S. 401 to the Neuse River. The bridge site lies within the flood plain of the Neuse River with the end bents on an embankment approximately 3.0 meters above the flood plain. Natural ground surface elevations within the flood plain in the immediate vicinity of the bridge are between 56 and 58 meters above MSL.

Photographs of the site are provided in the Appendix. These photographs show site conditions at each boring location prior to the start of the field exploration.

2.2 Geology

The bridge site is located in the Raleigh Belt of the eastern Piedmont Physiographic Province, an area underlain by ancient igneous and metamorphic rock. The Piedmont rocks of the Carolinas are marked by strongly directional zones or belts trending to the northeast. The belts contain rocks with similar properties but contrast sharply with one another in terms of petrology, grade of metamorphism and age.

The hard rock encountered at the bridge site is from the group of granitic rocks known as the Rolesville Suite. The hard rock cored during our investigation is classified as massive equigranular granitic rock, which is consistent with the Geologic Map of North Carolina compiled by The North Carolina Geologic Survey in 1985.

Neuse River has a well defined channel that, under normal rainfall conditions, is confined by moderately steep to near vertical river banks. The river depth is normally approximately 1.5 to 2.0 meters at its deepest point in the channel. During periods of



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flooding, the Neuse River flows over its banks to deposit sediments on its roughly 200 meter-wide flood plain. Deposition of principally silt is occurring in the flood plain during these periodic episodes of flooding. The alluvial soil deposited by flood waters of the Neuse River is composed of weathered and transported Piedmont material previously existing upstream from the site.

The soils at the bridge site can be characterized into two classes. First, alluvial soils, mentioned above, make up a substantial part of the soils encountered at the site. The alluvial soils exist at the ground surface in the flood plain and in the channel of the Neuse River, extending to depths of 4.5 meters to 7.0 meters in the flood plain and between 2.0 and 2.7 meters in the channel. The alluvial soils existing in the flood plain consist of mostly silt (A-4) with interbeds of clay (A-6/A-7) and often exhibit a basal gravel zone marking the boundary between alluvial soils and the more resistant residual soils below. There are thin erratic interbeds of organic material consisting of leaves and wood fragments existing in the alluvial soil horizon in the flood plain.

In the channel of the Neuse River the alluvial soil is much coarser, existing chiefly of coarse sand (A-3) and gravel (A-1) with little or no fines. The velocity of the water in the channel is sufficient to carry away most smaller grain sizes. Also included in the sand and gravel channel material are zones of wood fragments ranging in size from sticks to logs.

Residual soils lie unconformably below the alluvial material across the site. Residual soils are derived from the in-situ weathering of the underlying parent rock type, in this case, granite. As a hard rock weathers in place, it leaves behind a distinctive stratigraphic profile characterized by residual soil overlying soft weathered rock and hard weathered rock overlying hard rock. Soft Weathered Rock and Hard Weathered Rock represent the transition between residual soil and hard rock. The residual soil in this sequence exhibits the same structure as its parent rock but lacks the strength of rock as a result of the weathering process.

Weathering along fractures and joints has produced seams of weathered rock within the granitic rock. It is not unusual to core through competent rock, re-enter weathered rock and return to competent rock. Core losses noted on the coring records are most likely due to the presence of these weathered zones. Weathering along discontinuities can also create boulders of hard rock within the residual soil or weathered rock zone. This is common in association with granitic bodies although it was not readily apparent in the borings performed at this bridge site.



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The cross section and profile drawings of subsurface conditions and the test boring records included in the Appendix provide descriptions of the residual soil, weathered rock, and hard rock encountered at the site.

3.0 FIELD TESTING

A total of 18 borings was performed at the bridge site. The boring locations are shown on Drawing No. 2 in the Appendix. Six of the borings were performed at the proposed end bent locations and included only soil testing with a split spoon sampler. The remaining 12 borings were performed at each end of the 3 interior bents of each structure and involved soil testing and rock coring. Eight of the interior bent borings fell in the water and thus were drilled from a barge using a CME 450 drill rig. The land borings were advanced using a Mobile B-56 and a CME 550, both mounted on all-terrain vehicles.

The borings were advanced using wash drilling techniques to the depth of rock. Rock coring was performed at each of the interior bent borings with diamond tip core barrels. Water used during coring was taken directly from the Neuse River. Casing was advanced when necessary.

Baseline -L- surveyed along the center of the existing bridge was used to lay out boring locations by measuring distances and offsets. Ground surface and water surface elevations at each land boring were determined by Law personnel by direct measurement relative to a known benchmark. Collar elevations of the water borings were calculated by subtracting the water depth at the borings from the water surface elevation. Water surface elevations were measured daily and monitored throughout the day for fluctuation.

4.0 LABORATORY TESTING

Laboratory testing was conducted on selected soil samples to aid in classification of the on-site soils. AASHTO classification tests were conducted on 16 split-spoon samples which were considered representative of the soil stratigraphy encountered across the site. Sample numbers are shown on the boring logs beside the appropriate split spoon interval. Grain size distribution curves were generated for samples numbers SS-4, SS-5 and SS-6, which are representative of the Neuse River channel bed material for use in determining the critical scour depth. Three rock samples, representative of the rock



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type found at the site, were tested for unconfined compression strength with Young's Modulus and Possion's Ratio.

All testing was performed in general accordance with applicable ASTM/AASHTO/N.C.DOT specifications. The results of these tests are included in the Appendix of this report.

5.0 SUBSURFACE CONDITIONS

5.1 General

Included in the Appendix are two generalized profiles parallel to the proposed bridge. The first profile represents the subsurface conditions encountered at the southbound lane and the second profile depicts the conditions at the northbound lane. The cross sections represent the conditions along the line of each bent, which are skewed at 90° to the -L- alignment. Also included in the Appendix are boring logs describing soil conditions at each of the boring locations, and core boring reports describing rock core in terms of color, weathering, continuity and hardness. Joint orientations are given relative to the axis of the core with 90° being parallel to the axis and 0° being perpendicular to the axis. Joint spacing, ADS, is given when it is determinable.

5.2 Subsurface Conditions - Southbound Lane - End Bent 1

Two borings were drilled to characterize the subsurface conditions at End Bent 1 on the Southbound Lane bridge. These borings encountered 1.23 to 2.10 meters of loose to medium dense (N values from 4 to 11) silty fine sandy alluvial material (A-2-4). A 2.87 to 3.40 meter-thick zone of medium stiff (N values from 6 to 8) alluvial silty sandy clay (A-6, A-7-6) and sandy silt (A-4) exists below the upper sand layer. Very dense (N value of 100+) alluvial silty sandy gravel marks the base of the alluvium, extending below the silt and clay to an approximate elevation of 51.0 meters. Weathered granite classified as Soft Weathered Rock (SWR) and Hard Weathered Rock (HWR) sampled as silty fine to coarse sand lies below the alluvium to the termination of the borings. This rock is characterized by N values greater than 100. The borings were terminated on hard rock at depths ranging from 7.91 to 9.93 meters.



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5.3 Subsurface Conditions - Southbound Lane - Bent 1

At the location of Bent 1 on the Southbound Lane bridge, our borings encountered 1.70 to 2.20 meters of alluvial channel material existing as very loose to loose (N values of 1 to 5) silty fine sand (A-2-4) with a thin interbed of clay and fine to coarse sand (A-3) with wood fragments. Below the alluvial material exists 0.4 to 1.5 meters of residual soil classified as very dense (N value of 63) silty fine to coarse sand (A-2-4), weathered granite. Soft Weathered Rock (SWR) sampled as silty fine to coarse sand exists below the residual soil to depths of 5.80 to 6.70 meters. Hard Weathered Rock (HWR) exists below the Soft Weathered Rock, extending to elevations 45.76 and 45.01 meters in the two borings. Hard Rock (HR), was cored below the Hard Weathered Rock and is described as orange, tan, pink and gray medium to coarse grained, severely weathered to fresh, extremely fractured to sound, soft to very hard granite.

5.4 Subsurface Conditions - Southbound Lane - Bent 2

The two borings at this location encountered 1.71 to 2.16 meters of alluvial channel material consisting of very loose to dense (N values of 2 to 34) silty sand and gravel (A-2-4, A-3, A-1-b) with wood fragments. In Boring SBLB2-A, a 0.30 meter-thick granite boulder lies on top of residual soil. The boulder could be in place or it could have been transported by waters of the Neuse River. A 1.39 meter-thick zone of residual soil in this boring overlies Soft Weathered Rock (SWR) which begins at elevation 49.46 meters. The Soft Weathered Rock changes to Hard Rock (HR) at elevation 40.84 meters. Boring SBLB2-A was terminated in Hard Rock (HR) at elevation 35.67 meters.

In Boring SBLB2-B residual soil underlies the alluvial soil and exists from elevation 51.63 meters to 50.99 meters where it changes to Soft Weathered Rock (SWR). Another zone of residual soil exists from elevation 49.49 meters to 48.29 meters where it again changes to Soft Weathered Rock. Hard Rock (HR) exists from elevation 45.35 meters to the termination of the boring at 39.42 meters.

The residual soils, Soft Weathered Rock, and Hard Rock at this bent location are similar to those described above.



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5.5 Subsurface Conditions - Southbound Lane - Bent 3

The two borings at this bent encountered 5.30 meters to 6.25 meters of alluvial material consisting of very soft to medium stiff (N values of 2 to 6) fine sandy clayey silt (A-4), very soft (N value of 1) silty sandy clay (A-6) and medium dense to dense (N values of 13 to 37) fine to coarse sand and gravel (A-2-4, A-1-b). The alluvial material overlies 2.8 to 3.25 meters of residual soils consisting of dense to very dense (N values of 31 to 67) silty gravely fine to coarse sand (A-2-4). The residual soil changes to Soft Weathered Rock (SWR) at elevations 46.86 meters and 48.22 meters. In Boring SBLB3-B there exists a 1.4 meter-thick zone of residual soil within the Soft Weathered Rock zone. A thin 0.6 to 0.7 meter-thick zone of Hard Weathered Rock (HWR) separates the Soft Weathered Rock from Hard Rock (HR). The borings were terminated in Hard Rock. The Soft Weathered Rock, Hard Weathered Rock and Hard Rock at this bent location are similar to that described above.

5.6 Subsurface Conditions - Southbound Lane - End Bent 2

At this end bent location alluvial material exists to elevations 50.61 meters and 48.18 meters. The alluvium consists of loose to medium dense (N values of 4 to 24) and medium stiff to stiff (N values of 5 to 12) silty fine to coarse sand (A-2-4), silty fine sandy clay (A-7), and fine sandy clayey silt (A-4). Residual material extends below the alluvium to elevations of 45.94 meters to 45.18 meters. Soft Weathered Rock (SWR) and Hard Weathered Rock (HWR) exist below the residual soil to the termination of the borings on the top of Hard Rock (HR) at elevations 44.08 and 41.54 meters. Residual soils and weathered rock at this location are similar to those described above.

5.7 Subsurface Conditions - Northbound Lane - End Bent 1

The two borings drilled at End Bent 1 on the Northbound Lane bridge encountered 4.5 to 5.8 meters of alluvial material consisting of sand, silt, clay and gravel. The upper 0.95 to 1.23 meters of soil is characterized as loose to medium dense (N values of 6 to 11) silty fine sand (A-2-4). Medium stiff to very stiff (N values of 6 to 24) fine sandy silt (A-4) and silty sandy clay (A-6) exist below the upper sand layer to elevations of 52.23 and 53.02 meters respectively. A 0.7 to 1.0 meter layer of basal gravel marks the lower limits of the alluvial material and overlies residual soil and weathered rock.



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In Boring EB1-C, the basal gravel lies directly on top of Soft Weathered Rock (SWR) sampled as silty fine to coarse sand. The boring encountered 2.11 meters of (SWR) and was terminated on Hard Rock (HR) at a depth of 7.91 meters. Boring NBLEB1-B encountered 5.0 meters of residual soil below the basal gravel layer. This soil is characterized as very dense (N values of 67 to 96) silty sand (A-2-4) and gravel (A-1-b) and is the result of in-situ weathering of granite. Weathered rock sampled as silty fine to coarse sand (SWR, HWR) exists below the residual soil in boring NBLEB1-B to the termination of the boring on Hard Rock (HR) at a depth of 13.19 meters.

5.8 Subsurface Conditions - Northbound Lane - Bent 1

At this location 0.70 to 2.10 meters of alluvial channel material exists above the residual soils. The alluvial soil is very loose to loose (N values of 0 to 7) fine to coarse sand and gravel (A-3, A-1-b) with wood fragments. Below the alluvial material is 1.4 to 2.9 meters of residual soil classified as dense to very dense (N values of 30 to 53) silty fine to coarse sand and gravel (A-2-4, A-1-b) extending to the top of Soft Weathered Rock.

In Boring NBLB1-A Soft Weathered Rock (SWR) begins at elevation 48.11 meters and changes over to Hard Weathered Rock (HWR) at elevation 47.41 meters. Hard Rock (HR) begins at elevation 47.50 meters and extends to the termination of the bore hole at elevation 40.73 meters. In boring NBLB1-B Soft Weathered Rock (SWR) begins at elevation 50.63 and changes over to Hard Weathered Rock (HWR) at elevation 45.64 meters. Hard Rock (HR) is encountered at elevation 45.57 meters in NBLB1-B and extends to the termination of the boring at elevation 38.65 meters. The Soft Weathered Rock, Hard Weathered Rock and Hard Rock at this location is similar to that described above.

5.9 Subsurface Conditions - Northbound Lane - Bent 2

Alluvial channel material in the two borings at this location extends to depths of 2.50 to 2.70 meters below the river bottom. The alluvial channel material consists of very loose to medium dense (N values of 2 to 13) fine to coarse sand and gravel (A-3, A-1-b) with wood fragments. Below the alluvial soils lies 0.75 to 1.60 meters of residual soil. Soft Weathered Rock extends from elevation 50.08 meters to 44.83 meters in NBLB2-A and from elevation 49.46 meters to 46.96 meters in NBLB2-B. A thin 0.25 to 0.93 meter-thick zone of Hard Weathered Rock (HWR) exists below the Soft Weathered



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Rock in these borings below which extends Hard Rock (HR) to the termination of the borings. The residual soils, Soft Weathered Rock, Hard Weathered Rock and Hard Rock at this bent location are similar to that described above.

5.10 Subsurface Conditions - Northbound Lane - Bent 3

The upper 4.70 meters to 5.40 meters of these two borings consists of alluvial material existing as very soft to soft and loose (N values of 1 to 6) silty fine sandy clay (A-6, A-7), silty sand (A-2-4), fine sandy clayey silt (A-4) and gravel (A-1-b). Below the alluvium exists 3.3 to 4.2 meters of residual soils consisting of dense to very dense (N values of 32 to 99) silty fine to coarse sand. Residual soils change to Soft Weathered Rock (SWR) at elevations 48.49 and 47.10 meters. Soft Weathered Rock changes to Hard Weathered Rock (HWR) at elevations 40.99 and 43.80 meters. In boring NBLB3-B there exists a 1.5 meter-thick zone of residual soil within the Soft Weathered Rock zone. Hard Rock (HR) begins below the Hard Weathered Rock at elevations 40.17 meters and 43.47 meters and continues to the termination of the borings. The Soft Weathered Rock, Hard Weathered Rock and Hard Rock at this bent location are similar to that described above.

5.11 Subsurface Conditions - Northbound Lane - End Bent 2

These borings encountered 5.73 to 8.50 meters of alluvium consisting of loose to medium dense and medium stiff to stiff (N values of 4 to 24) silty fine sand (A-2-4), clayey fine sandy silt (A-4) and silty fine sandy clay (A-7). Residual soils extend below the alluvium to boring depths of 8.00 to 11.50 meters. Residual soils change to Weathered Rock at elevations 45.18 meters and 48.93 meters. Borings were terminated on Hard Rock (HR) at elevations of 41.54 to 42.60 meters. Residual soils and weathered rock at this location are similar to those described above.

6.0 GROUNDWATER

Groundwater level observations were generally obtained immediately after drilling and 24 hours after completion at each land boring location. Groundwater readings are shown on the boring logs. On the south side of the river, 24 hour groundwater was observed at depths of 2.44 meters, 0.90 meters and 0.90 meters. On the north side of the river, 24 hour groundwater was observed at depths of 2.42 meters, 2.13 meters,



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2.50 meters, 3.08 meters, 2.59 meters, 1.98 meters and 3.35 meters. These measured groundwater depths are somewhat inconsistent with the water elevation in the Neuse River. This could be due to the existence of a dam approximately 10 miles upstream of the bridge site. When water is released at the dam above and beyond the normal levels, it takes time for groundwater levels to reflect water levels in the river, especially when the majority of the flood plain soils are fine grained silts and clays. Therefore, measured groundwater levels in the flood plain can sometimes be lower than the elevation of the water surface in the Neuse River.

7.0 SCOUR

Detail scour analysis for the proposed structure was not part of the scope of services for the bridge foundation investigation, but limited scour information concerning the bridge site was obtained from site observations as required by the N.C. Department of Transportation for all water crossings. A Field Scour Report form describing the observed site conditions related to bridge scour is included in the Appendix.

8.0 FOUNDATION RECOMMENDATIONS

The soft and loose alluvial soils will not provide suitable support for spread foundations. We recommend HP 310mm x 79 kg/m steel piles terminating in the soft to hard weathered rock be used for the two end bents. At End Bent 1, piles driven to tip elevations ranging from about 45 meters to about 49 meters are expected to develop a vertical load capacity of 400 to 450 kilonewtons. The deeper pile tip elevations are expected on the eastern side of the NBL. At End Bent 2, piles driven to an average tip elevation of 45 meters are expected to develop a vertical load capacity of 400 to 450 kilonewtons.

For the interior bents, we recommend drilled piers bearing on the hard rock. Lateral resistance can be developed by the residual soil and soft weathered rock that is present above the hard rock if the scour analysis determines these materials will remain. Otherwise, the drilled piers can be socketed 2 to 3 meters into competent rock to develop lateral resistance. The required pier size is expected to depend on the lateral capacity analysis. Driven piles are not considered due to the potential reduction in lateral resistance if the alluvial soils are scoured and the difficulty of driving into the rock.



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9.0 EMBANKMENTS

Based on the standard penetration resistances and soil types in the end bent borings, we recommend side and end slopes of 1.5(H):1(V) for the approach fills. Similar slopes are present on the approach fills for the existing bridge and do not show indications of excessive settlement or lack of stability. The medium stiff silts (A-4) in some end bent borings, are expected to experience some initial settlement under the weight of the fill. We recommend a one month delay period after placing the fill before pile driving.

10.0 CONSTRUCTION CONSIDERATIONS

Drilled pier construction should use temporary casing installed through the alluvium. The residual soils and soft weathered rock are sandy textured and water inflows into an unprotected hole may cause side softening or collapse. Either temporary casing or slurry drilling should be used to advance the drilled pier through the residual soil and into the soft weathered rock.

While it is possible to have lenses of hard rock present within the residual soil or the soft weathered rock, none of the end bent borings encounter such conditions. Therefore pile tip reinforcement is not recommended.



APPENDIX

N.C.DOT GEOTECHNICAL UNIT SOIL AND ROCK CLASSIFICATION SHEET AND LEGEND SUPPLEMENT

SITE LOCATION MAP (DRAWING NO. 1)

BORING LOCATION PLAN (DRAWING NO. 2)

SOUTHBOUND LANE CROSS SECTIONS THROUGH BENTS (DRAWINGS NO. 3 THROUGH 7)

NORTHBOUND LANE CROSS SECTIONS THROUGH BENTS (DRAWINGS NO. 9 THROUGH 13)

SOUTHBOUND LANE BORING LOGS AND CORE REPORTS WITH CORE PHOTOS NORTHBOUND LANE BORING LOGS AND CORE REPORTS WITH CORE PHOTOS

SCOUR REPORT

SITE PHOTOGRAPHS

AASHTO/ASTM LABORATORY TEST RESULTS

PROFILE ALONG SOUTHBOUND LANE,
APPROXIMATELY 18 METERS LEFT OF -L- (DRAWING NO. 8)

PROFILE ALONG NORTHBOUND LANE,
APPROXIMATELY 21 METERS RIGHT OF -L- (DRAWING NO. 14)

NORTH CAROLINA DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

SOIL AND ROCK CLASSIFICATION, LEGEND, AND ABBREVIATIONS

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	ERG LIMITS	- ,	DESCRIPTION	אנ	<u> </u>							\Box	ARTIFIC	IAL FILL OTHER TH	an -C	- CORE BORD	
			-SATURATE		USUA	LLY,	LIQUID:	VER	Y WET	. USUAL	LY		ROADWA	A EMBANCHENIS		PIEZOMETER	SAMPLE
LL,	FIGNIO FINI	IT _	(SAT.)					_				 	INFERE	EO SOIL BOUNDAR	IES C	INSTALLATI	DN UC-UNCONFINED
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	PLASTIC LIM	41T			UP 11	MOM	1401210	<i>,</i> ,,,,				-	STRIDE	E AND OIP	_		
OM T	OPTIMUM MOIS	TISSE	-MOIST- (M)	SOL	ID: A	T OR NE	EAR	OPTIM	UM MOI	STURE	-	APPAPI	ENT DIP		D- SPT H-YALL	E
	SHRINKAGE I											1			() HONITORING	VELL
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IS ARRITRAR	Y. TRANSITION	BE TWEEN	SOIL AND R	OCK 15	OFTEN	I REPF	ESENTED	81 /	A ZONE	OF YEA	THERED	CAL		CALCAREOUS			SAMPLE TAKEN
BOCK, EUB	THE PURPOSE	OF THIS	INVESTIGAT	ION, TH	ESE M	ATERI/	ALS ARE	DIVIO	DED AS	FOLLOW:	S:	CL.		CLAY		01101	RGANIC EFER TO
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TERM	SY	MBOLS		\mathbb{I}_{-}			OESC					CSE		COARSE		s. S	OFT
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	11 V/G		EATHERED	OUFFL	CULTY	USING	POVER AL	IGERS	AND YE	ELOS SPT	REFUSAL	FRA		FRACTURED	•	SL. S	ILT. SILTY
WEATHERED ROCK		RO	OCK (HAN)				CAN BE						G(S).	FRAGMENT(S)		7.	LIGHTLY Fandard Penetration Test
(WR)		₹1	SOFT	OICE	TOTAL TY	HISTN	n POWER	AUG	ERS AN	D YIELUS	3	GR.		GRAVEL	+		OPSOIL
l K			OCK (SWR)	SPT	VALUE	5 > 16	e BLOVS	BUT	< SPT	REFUSA	<u> </u>	GS		SPECIFIC GRA	VIII ED	TS. T	ANE SHEAR TEST
1	ar - 2211 / 11											GW	١.	MEDIUM		v. v	ERY
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<i> </i> € .	dir	4	: 12									000	LIECT OF	EOLOGIST BK	Bank	S_ SUBMETTED	BY <u>law</u>
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N.C.DOT LEGEND SUPPLEMENT

addition to the terms and abbreviations listed on the N.C.DOT Legend Sheet, the following is used to further describe rock quality on this project.

WEATHERING

Rock fresh, crystals bright, few joints may show slight staining. Rock rings under hammer is crystalline. Fresh Rock generally fresh, joints stained, some joints may show thin clay coatings if open, crystals on a Very Slight broken specimen face shine brightly. Rock rings under hammer blows if or a crystalline nature. (V.SLI.) Rock generally fresh, joints stained and discoloration extends into rock up to 25 mm (1 in.). Open joints Slight

may contain clay. In granitoid rocks some occasional feldspar crystals are dull and discolored.

Crystalline rocks ring under hammer blows.

Comparable to soil

Significant portions of rock show discoloration and weathering effects. In granitoid rocks, most Moderate feldspars are dull and discolored, some show clay. Rock has dull sound under hammer blows and show (MOD.) significant loss or strength as compared with fresh rock.

All rock except quartz discolored or stained. In granitoid rocks, all feldspars dull and discolored and a Moderately majority show kaolinization. Rock shows sever loss of strength and can be excavated with geologist's Severe pick. Rock gives "clunk" sound when struck. Comparable to hard weathered rock (MOD.SEV.)

All rocks except quartz discolored or stained. Rock "fabric" clear and evident but reduced in strength to Severe strong soil. In granitoid rocks all feldspars are kaolinized to some extent. Some fragments of strong (SEV.) rock usually remain. Comparable to soft weathered rock

All rock except quartz discolored or stained. Rock fabric elements are discernible but the mass is 'ery Severe effectively reduced to soil status, with only fragments of strong rock remaining. Saprolite is an example √.SEV.) of rock weathered to a degree such that only minor vestiges of the original rock fabric remain.

Rock reduced to soil. Rock fabric not discernible only in small and scattered concentrations. Quartz may be present as dikes or stringers. Saprolite is also an example. Comparable to soil

ROCK CONTINUITY

Core pieces larger than 15 cms. Sound-Core pieces between 10 and 20 cms. Slightly Fractured (SLI.FRACT.)-Core pieces between 2.5 and 10 cms. Moderately Fractured (MOD.FRACT.)-Core pieces less than 2.5 cms. Extremely Fractured (EXT.FRACT.)-

JOINT SPACING

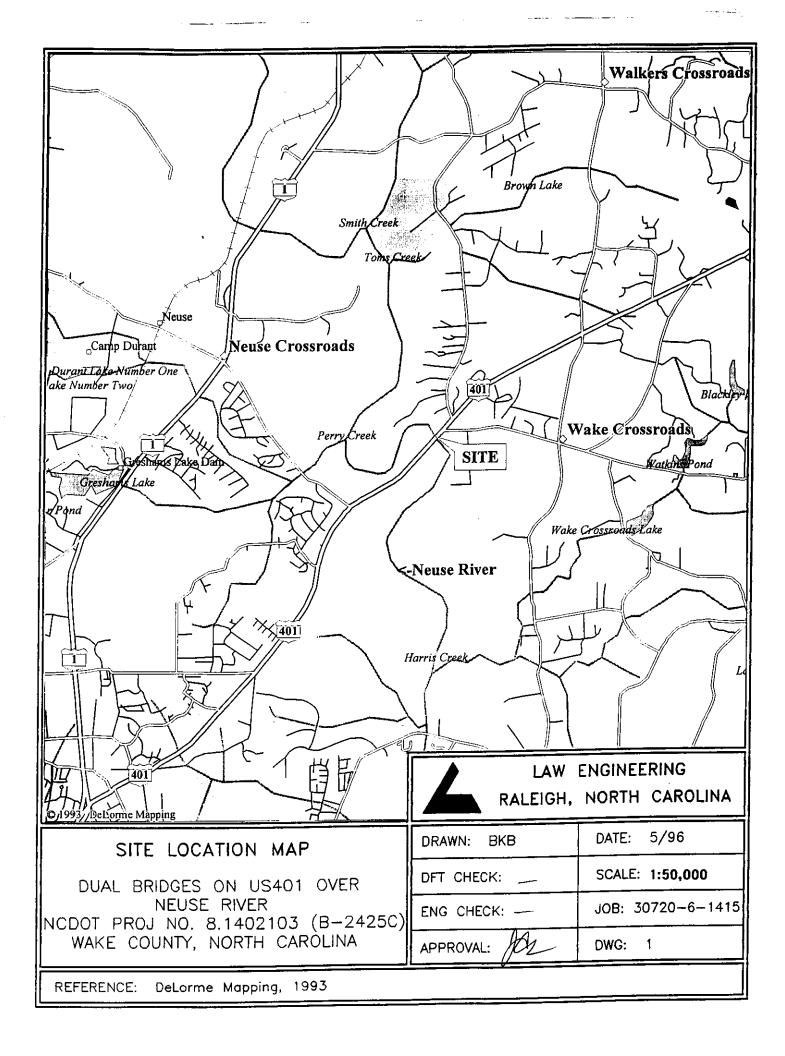
Average Discontinuity Spacing (ADS) The average measured distance (in centimeters) between joints in the same set. Will not apply to individual joints.

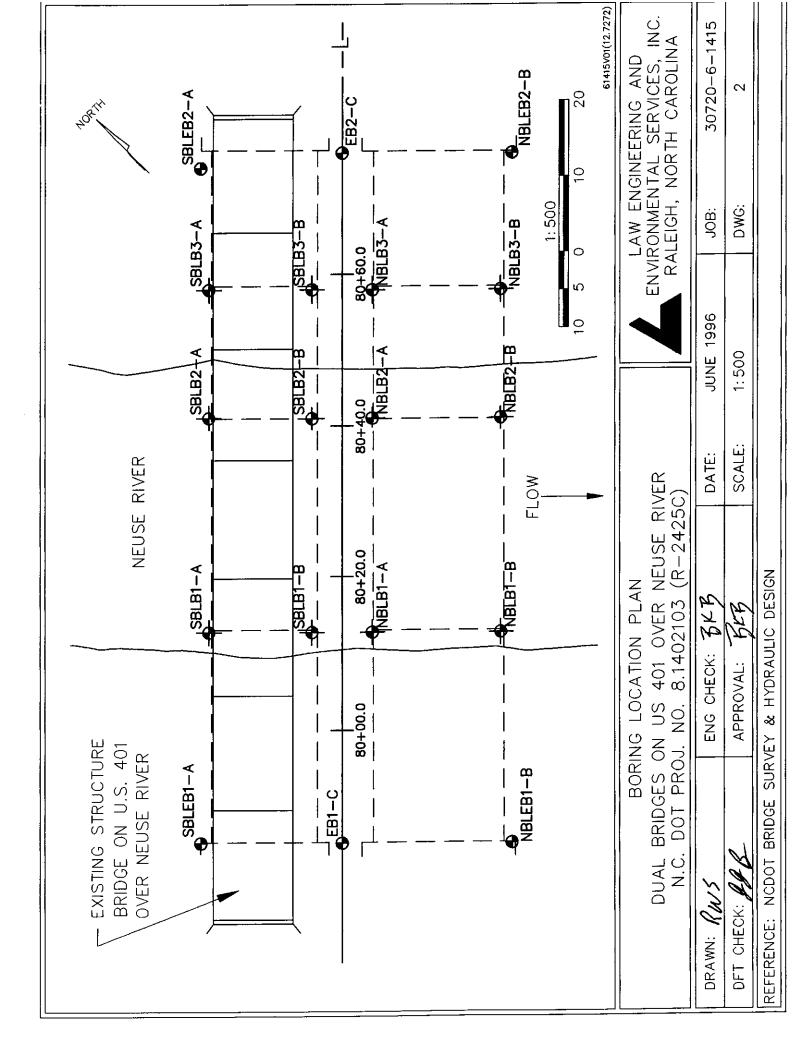
JOINT THICKNESS

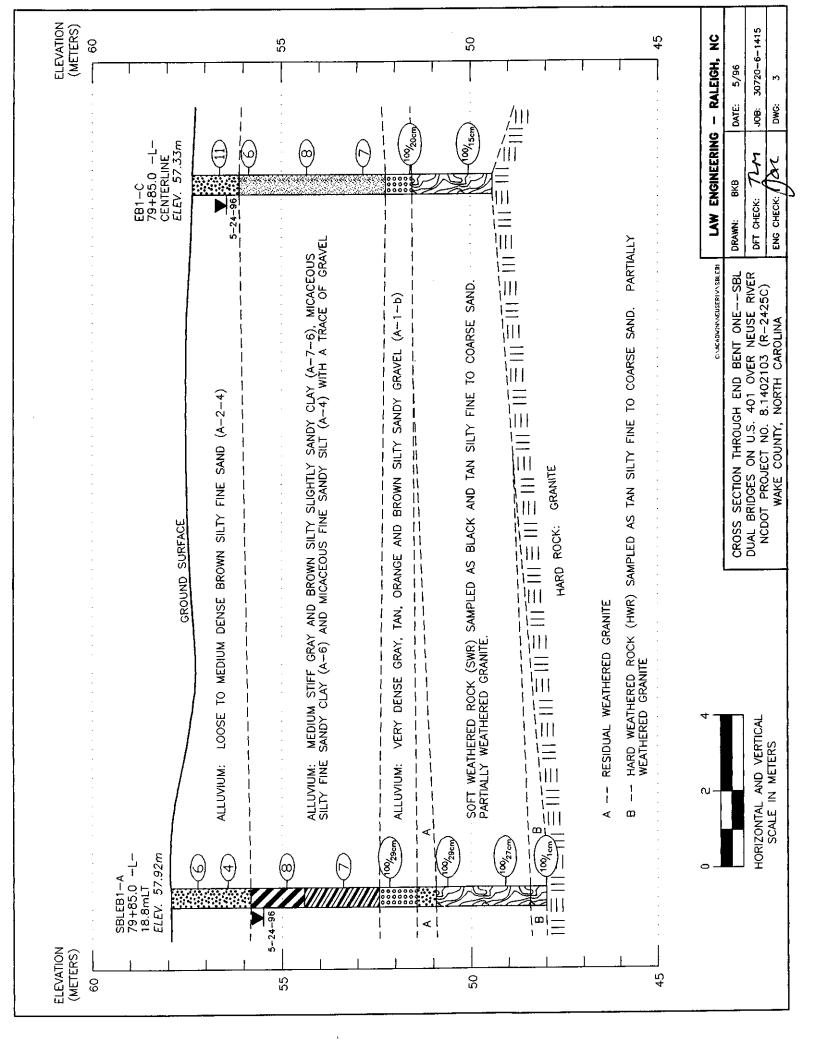
Average Discontinuity Thickness (ADT) The average thickness or width of gap in the joint.

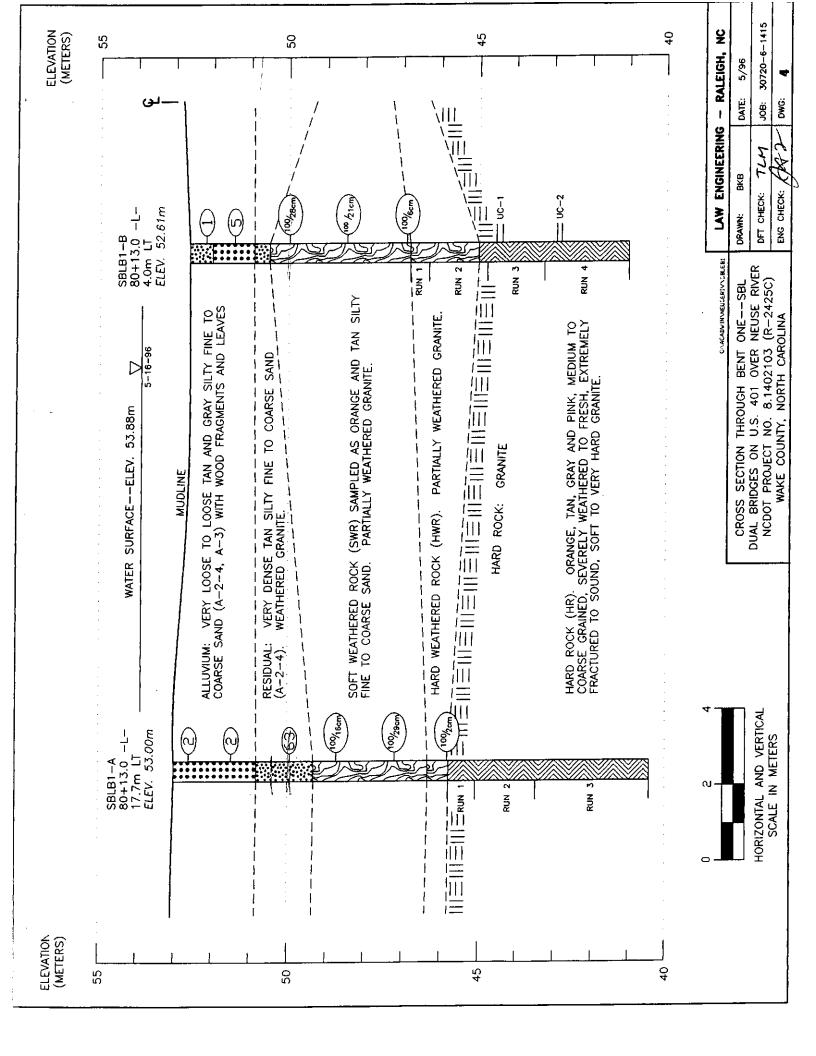
(SLI.)

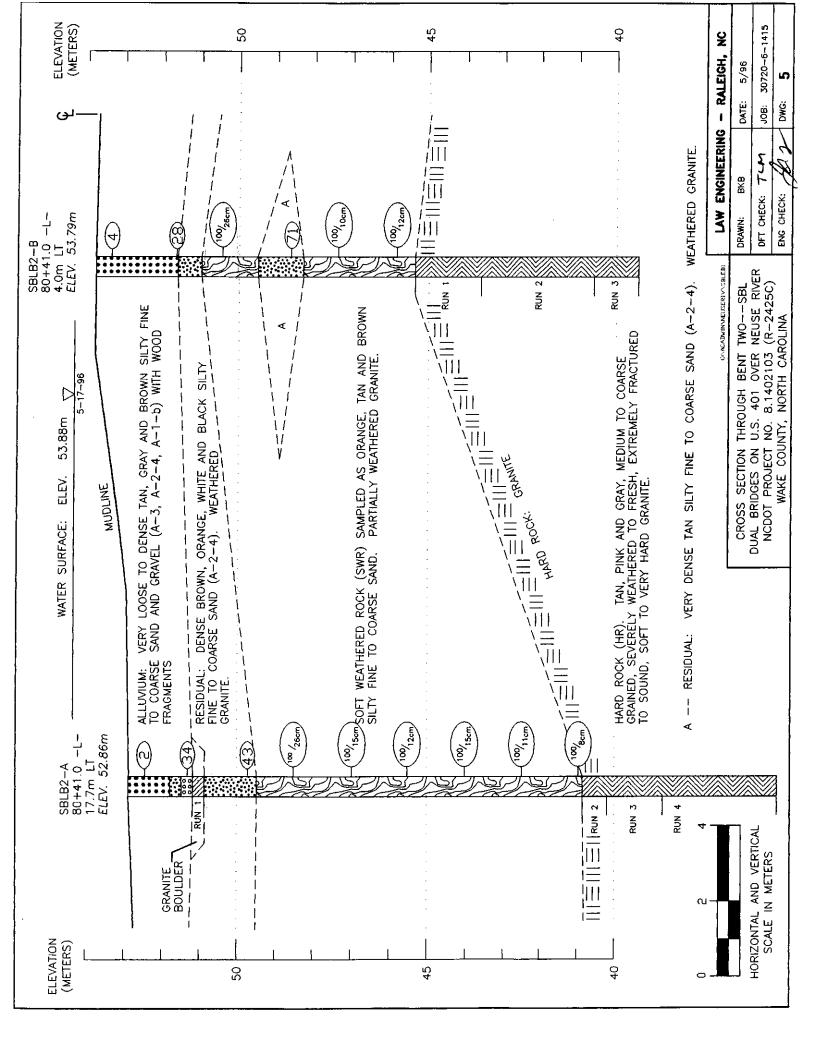
Complete

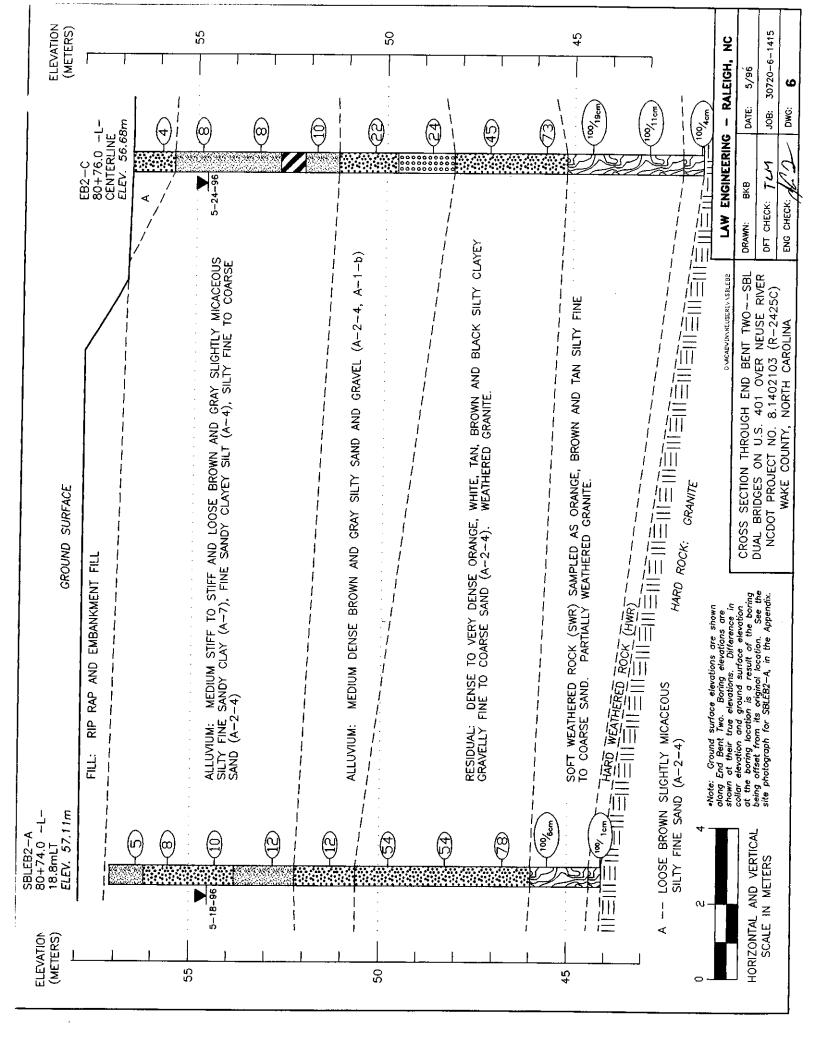


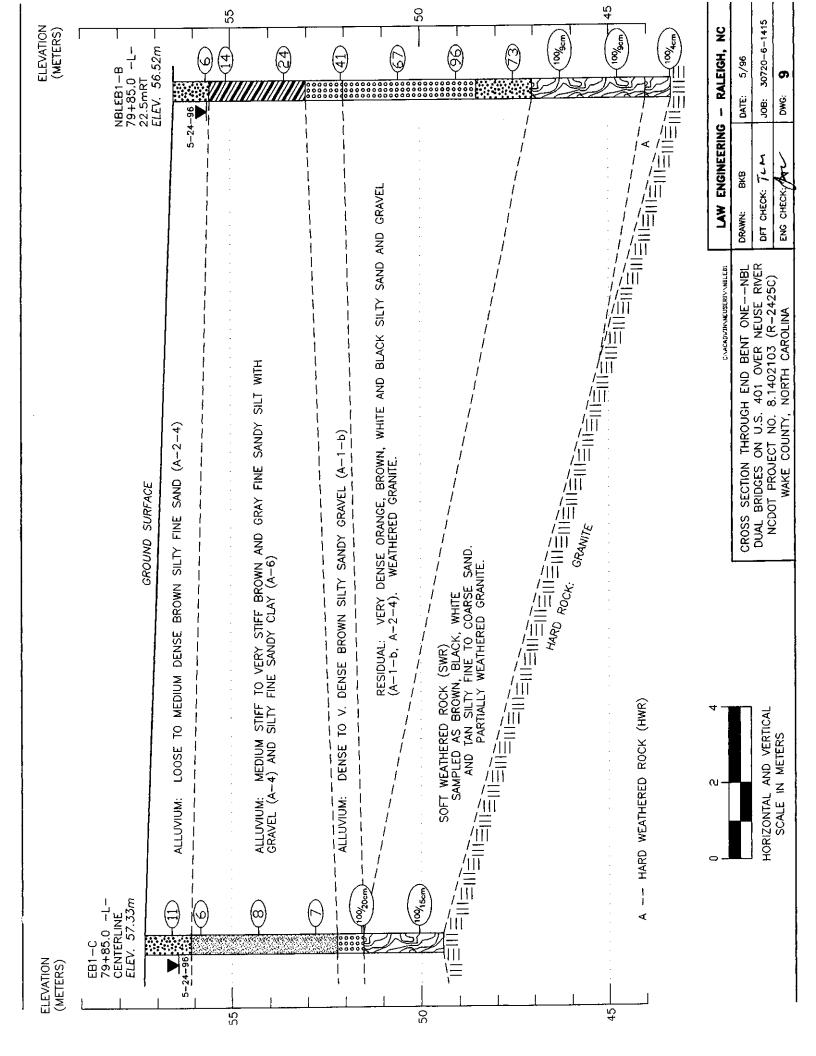


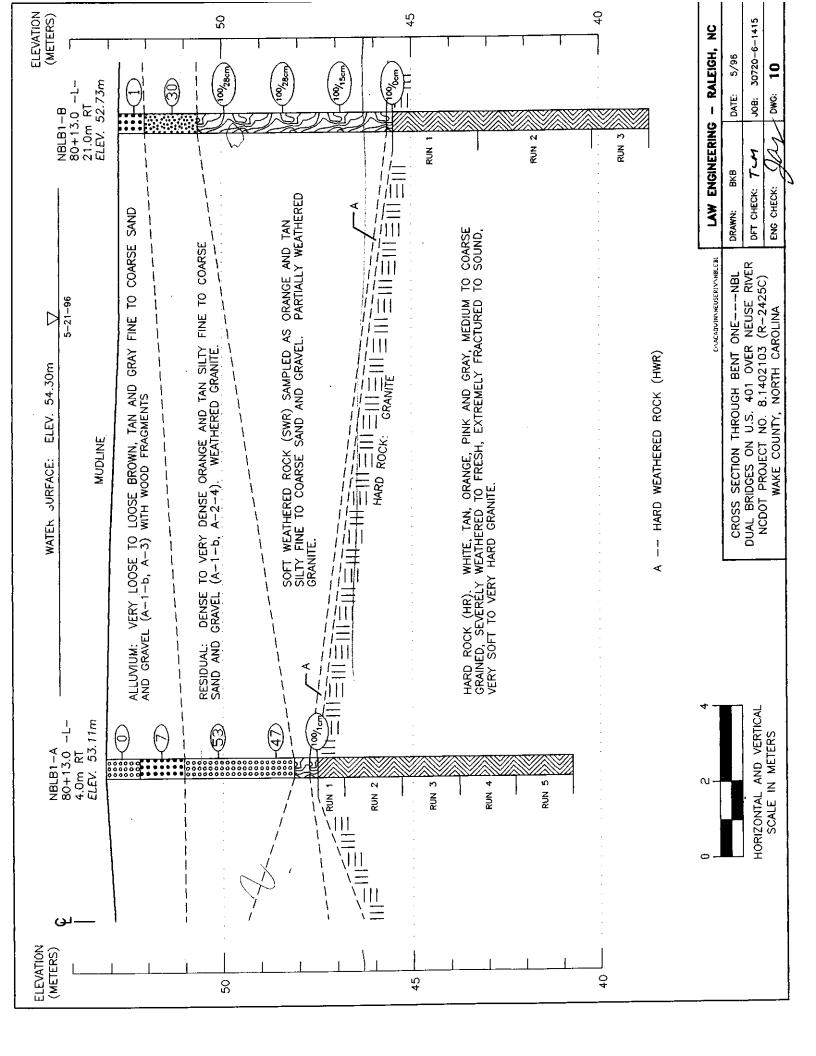


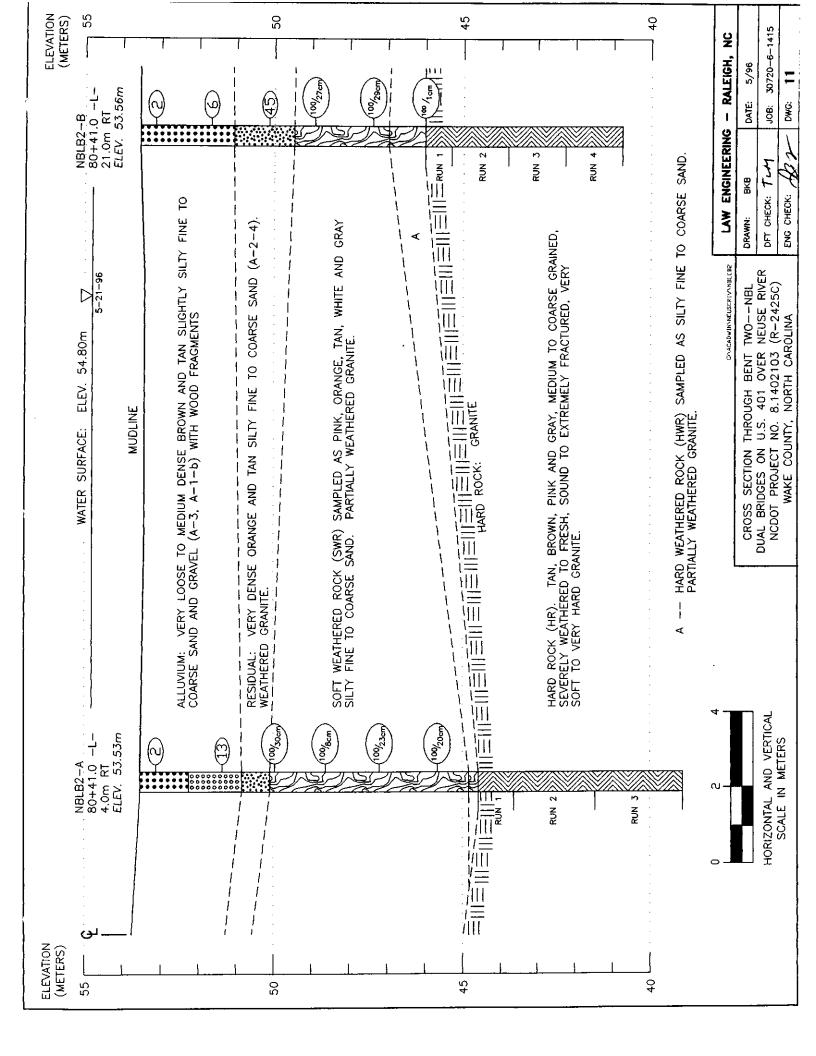


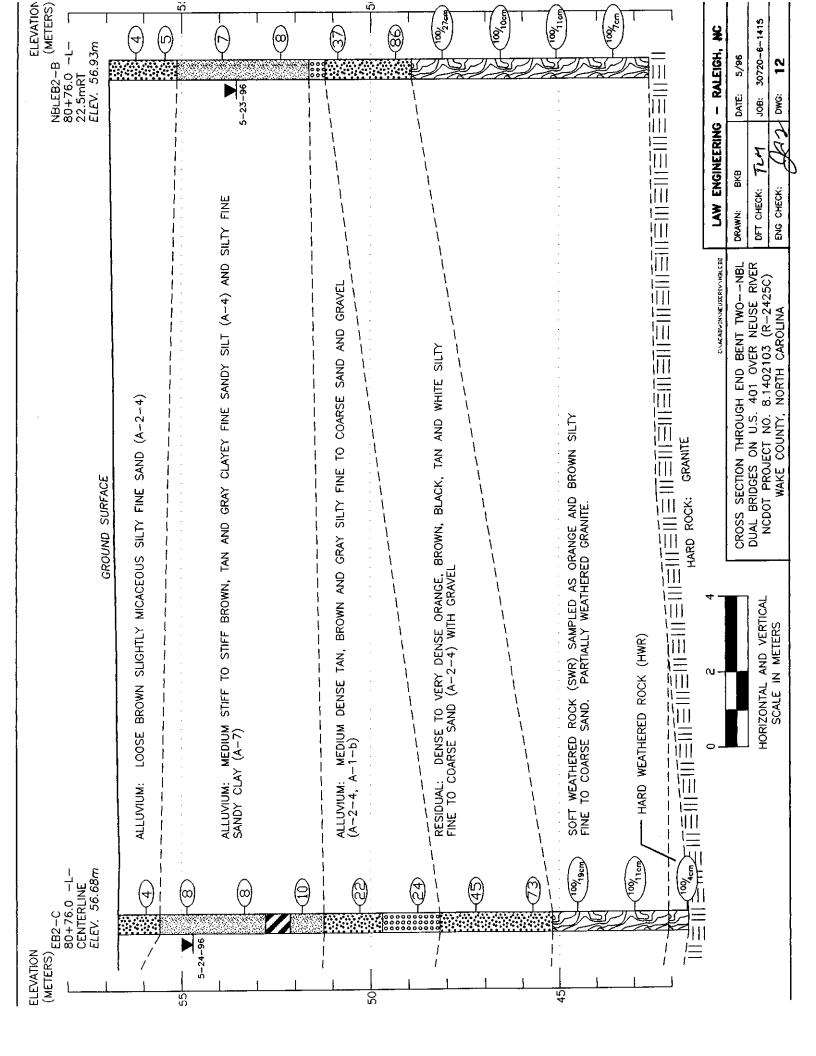












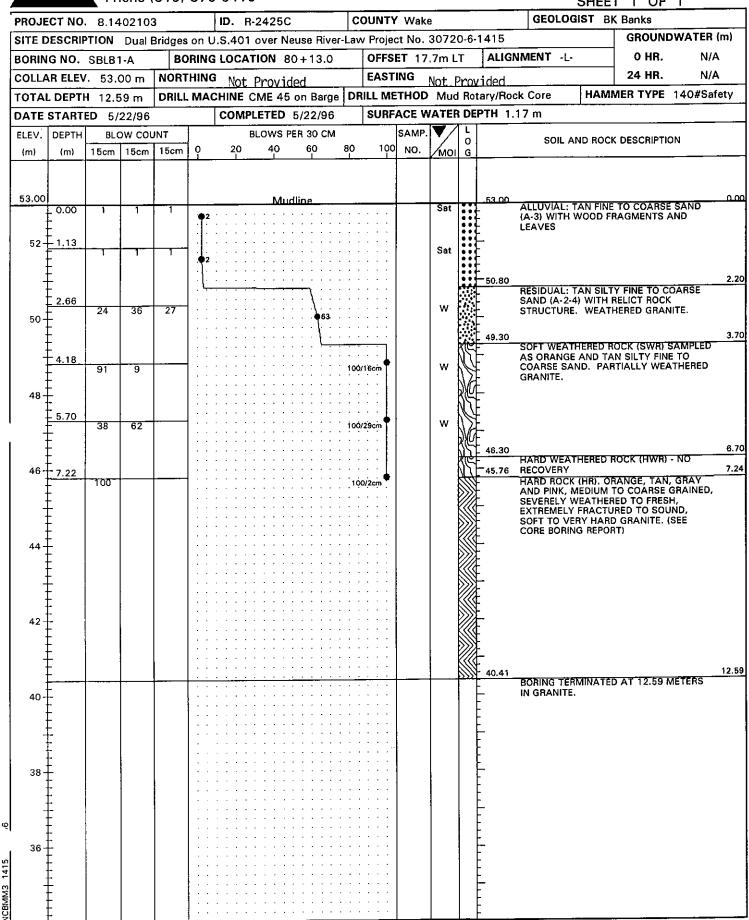


		Pr	ione	(919)	87	76-0416 						SHEET 1 OF 1
PROJE	CT NO	8.14	10210	3		ID. R-2425C	C	OUNTY	/ Wak	е		GEOLOGIST BK Banks
SITE D	ESCRI	PTION	Dual	Bridges	on	U.S.401 over Neuse River	-Lav	v Proje	ct No.	3072	0-6-	GROUNDWATER (m)
BORIN	G NO.	SBLEE	31-A	ВС	RIN	IG LOCATION 79+85.0		OFFS	ET 18	.8 m	LT	ALIGNMENT -L- 0 HR. 0.49
COLLA	R ELEV	/· 57.9	92 m	NORT	HIN	NG Not Provided		EAST	ING	Not	Pro	ovided 24 HR. 2.44
TOTAL	DEPT	H 9.93	3 m	DRILL	. MA	ACHINE CME 550	DR	RILL ME	THOD			
DATE	START	ED 5/	23/96			COMPLETED 5/23/96		SURF	ACE W	/ATEI	R DE	EPTH N/A
ELEV.	DEPTH	BLC	ow col	JNT		BLOWS PER 30 CM			SAMP.	V /	LO	SOIL AND ROCK DESCRIPTION
(m)	(m)	15cm	15cm	15cm	<u> </u>	20 40 60	80	100	NO.	МОІ		
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57.92	0.30					Ground Surface				∇		57.92 ALLUVIUM: BROWN SILTY FINE SAND
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56-	-									_		55.82 2 ALLUVIUM: GRAY AND BROWN MOTTLED
-	- - 2.62] : :		: :		66.1	N.	N	SILTY SLIGHTLY SANDY CLAY (A-7-6)
-	-	3	3	5		8			SS-1	М		\-
1	-				1				•			- 54.42 ALLUVIUM: GRAY AND BROWN MOTTLED
54-	- 4.14				: :							MICACEOUS SILTY FINE SANDY CLAY (A-6)
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52	- 5.00	18	60	40	1 : :	 	100/	/29cm		Sat	000	OL SILTY SANUT GRAVEL (A-1-D)
	_										000	or 51.426
]	_											RESIDUAL WEATHERED GRANITE (A-2-4)
1	_ 7.18 -	24	45	55	: :		100/	/29cm	i	w	77	SOFT WEATHERED ROCK (SWR) SAMPLED AS TAN SILTY FINE TO COARSE SAND.
50-	-				` :						1/3	PARTIALLY WEATHERED GRANITE.
307	-				: :							<u>}</u>
+	8.70	42	58				100/	/27cm		w	M	Ē
1	<u>-</u>										$ \zeta $	48,42
	9.92										77	HARD WEATHERED ROCK (HWR) SAMPLED 47.99 AS TAN SILTY FINE TO COARSE SAND. 9
48-	-	100				 	100)/1 cm				PARTIALLY WEATHERED GRANITE.
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		Ph	ione	(919)	87	6-0416							SHEET 1 OF 1			
PROJECT NO. 8.1402103 ID. R-2425C								C	YTNUC	Wake)		GEOLOGIST BK Banks			
SITE D	ESCRI	PTION	Dual	Bridges	on	U.S.401 over Ne	use River	-Law	Projec	t No.	3072	0-6-1	GROUNDWATER	(m)		
	G NO.					G LOCATION 75				T CL			ALIGNMENT -L- O HR. 0.2	27		
COLLAR ELEV. 57.33 m NORTHING Not Provided							EAST	NG	Not F	rov	ovided 24 HR. 0.9	0				
TOTAL DEPTH 7.91 m DRILL MACHINE CME 550								DR	ILL ME					fety		
	START			1		COMPLETED		1		ACE W						
.EV.	DEPTH		ow cou	INT	ļ	BLOWS PE				SAMP.	V /		AND DOOK PROCESSION			
(m)	(m)		15cm	15cm	o	20 40	60	80	100	NO.	мо	0 G	1			
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57.33				ļ		Ground	Surface				∇		57 33	0		
	0.30	5	6	5	, .						M		ALLUVIUM: BROWN SILTY FINE SAND (A-2-4)			
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1	-	3	4	4		.					М		£			
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52							- : : : : : : : : : : : : : : : : : : :	•	 i			000	ALLUVIUM: BROWN SILTY SANDY GRAVE			
	- 5.63 -	21	72	28	: :			100/	20cm		М	***	SOFT WEATHERED ROCK (SWR) SAMPLED			
1	- -				· ·							W	AS WHITE, BLACK AND TAN SILTY FINE TO COARSE SAND. PARTIALLY			
7	-								::			权	WEATHERED GRANITE.			
_	7.15	100						100/	15cm		м	PYN	华			
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL UNIT CORE BORING REPORT**

PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO: SBLB1-A

BORING LOCATION (STA): 80 + 13.0 -L-

OFFSET: 17.7m LT

COLLAR ELEV: 53.00m TOTAL DEPTH: 12.59m

DATE STARTED: 05-22-96

DRILL MACHINE: CME 45 on Barge

DATE COMPLETED: 05-22-96

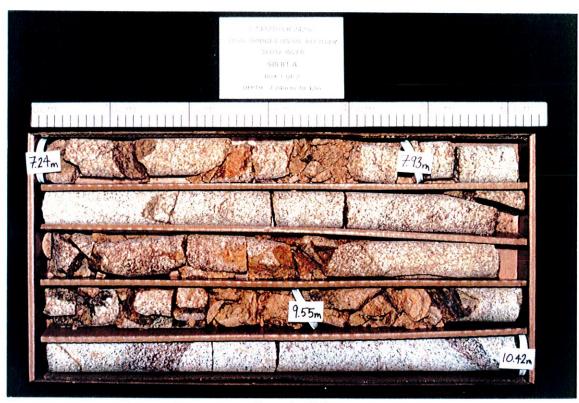
DRILL METHOD: Mud Rotary/Rock Core

CORE SIZE: NQ TOTAL RUN: 5.35m

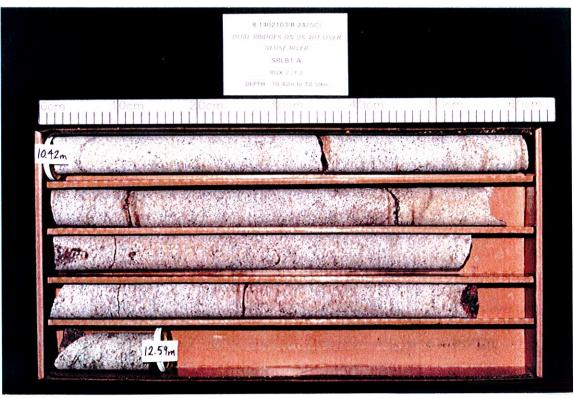
DRILLER: F.Cox/K.Pendley

CONL SIZE.			TOTAL NO				
ELEV.	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
(m)	(111)	(IVIIV/SOCIII)	(111)	/0	70	110.	Orange and tan medium to coarse grained moderately to
45.76	7.24	2:09					severely weathered, moderately to extremely fractured,
		2:36					
			0.69	0	0.44	Run 1	moderately hard to soft granite.
				(0%)	(64%)		2 Joints at 0-20 degrees
-							3 Joints at 40 degrees
45.07	7.93			•			4 Joints at 50-70 degrees
		0:55					Gray pink and tan medium to coarse grained, fresh to
		1:26					moderately severely weathered, sound to extremely fractured,
		1:42	1.62	0.39	1.62	Run2	very hard to medium hard granite.
		1:12		(24%)	(100%)		14 Joints at 0-10 degrees ADS = 3 to 18cm
43.45	9.55	1:06					6 Joints at 70-90 degrees
		2:20					Tan, gray and pink medium to coarse grained, moderately
		2:41					weathered to fresh, extremely fractured to sound, moderately
		3:11	3.04	2.90	3.04	Run 3	hard to very hard granite.
		3:08		(95%)	(100%)		8 Joints at 0-10 degrees ADS = 8 to 64cm
		3:30					4 Joints at 35-50 degrees ADS > 13cm
		3:07					
		4:14					
		4:31		:			
		4:58					
40.41	12.59	5:23					
							Boring terminated at 12.59 in granite
						,,	
11		1			<u> </u>	<u> </u>	<u></u>

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB1-A

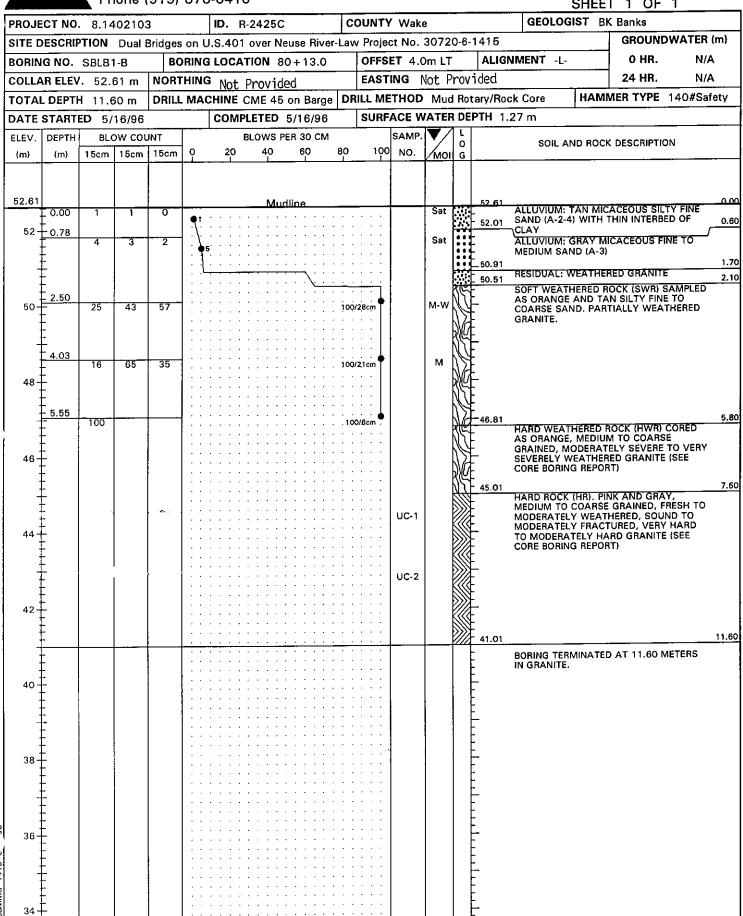


Box 1 of 2 7.24m to 10.42m



Box 2 of 2 10.42m to 12.59m





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL UNIT CORE BORING REPORT**

PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO: SBLB1-B

BORING LOCATION (STA): 80 + 13.0 -L-

OFFSET: 4.0m LT

DATE STARTED: 05-16-96

DRILL MACHINE: CME 45 on barge DRILL METHOD: Mud Rotary/Rock Core

COLLAR ELEV: 52.614m TOTAL DEPTH: 11.60m

DATE COMPLETED: 05-16-96 TOTAL BUN: 5.80m

DRILLER: F.Cox/K.Pendlev

CORE SIZE:	: NQ		TOTAL RUN	V: 5.80m			DRILLER: F.Cox/K.Pendley
		DRILL	B1 111	500	DEC	CALAD	FIELD CLASSIFICATION AND REMARKS
ELEV. (m)	DEPTH (m)	RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	LIEFA CEVROII IOVITION VIAN USINIMINA
		 	(1117	//	~	1,75,	No Recovery. Cuttings indicate hard weathered granite
46.81	5.80	2:05					(HWR)
		0:56/.2m					
			0.50	0	0	Run 1	
46.31	6.30						
		0:58_					Orange medium to coarse grained, moderately severe to
		2:08					severely weathered, soft to very soft granite.
		1:28	1.52	0	0.17	Run 2	
		1:32			(11%)		
44.79	7.82	1:56					
		2:10					Pink and gray medium to coarse grained, fresh to moderately
		3:30					weathered, sound to moderately fractured, very hard to
		4:23	1.52	0.96	1.14	Run 3	moderately hard granite.
		5:01		(63%)	(75%)		4 Joints at 0 degrees ADS ≈ 9 to 75cm
43.27	9.34	5:06					3 Joints at 45 degrees ADS = 38 to 73cm
		6:31]		Pink and gray medium to coarse grained, fresh to very slightly
		6:52					weathered, sound to moderately fractured, very hard granite.
		7:11	2.26	2.13	2.26	Run 4	2 Joints at 70 degrees ADS = 156cm
ļ		8:27		(94%)	(100%)		
į		10:15	ļ				
[]		19:33					
41.01	11.60	43:09					
41.01	1	1.5,05					
							Boring terminated at 11.60 meters in granite
:							
<u> </u>	1	ئــــــــــــــــــــــــــــــــــــــ		<u> </u>	<u> </u>		

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB1-B



Box 1 of 2 5.80m to 10.77m



Box 2 of 2 10.77m to11.60m



		Ph	one (919)	8/6	6-0416				_		SHEET 1 OF 1
PROJE	CT NO	8.14	0210	3		ID. R-2425C	CC	OUNTY	' Wake)		GEOLOGIST KD Trimble
SITE D	ESCRIF	TION	Dual 8	Bridges	on l	U.S.401 over Neuse River	-Law	/ Projec	ct No.	3072	0-6-1	-1415 GROUNDWATER (m
BORIN	G NO.	SBLB2	2-A	ВО	RINC	G LOCATION 80+41.0		OFFS	ET 17	.7m L	Т	ALIGNMENT -L- 0 HR. N/A
COLLA	R ELEV	. 52.8	36 m	NORT	HING	G Not Provided		EAST	ING	Not	Prov	ovided 24 HR. N/A
TOTAL	. DEPTI	ተ 17.1	9 m	DRILL	. MA	CHINE CME 45 on Barge	DR	ILL ME				
DATE	START	ED 5/.	22/96	<u> </u>		COMPLETED 5/22/96		SURF	ACE W	ATER	DE	EPTH 1.02 m
ELEV.	DEPTH	BLC	w cou	JNT		BLOWS PER 30 CM			SAMP.	$\mathbf{V}/$	L	SOIL AND ROCK DESCRIPTION
(m)	(m)	15cm	15cm	15cm	ρ	20 40 60	80	100	NO.	мог	O G	SOIL AND NOCK DESCRIPTION
52.86	0.00	1	1	1		Mudline				Sat	***	52 86 ALLUVIUM: TAN FINE TO COARSE SAND
1	_				•2						:::}	(A-3) WITH WOOD FRAGMENTS
52	_ _ 1.14									Sat		51.76 1 51.46 ALLUVIUM: BROWN AND GRAY SILTY 1
İ	-	1	1	33		₩34	<u> </u>	<u> </u>		Sat	000	51.15 SAND (A-2-4)
-	<u>-</u>					· · · · · · · · / · · · · · · · · · · ·						SANDY GRAVEL (A-1-b)
†	- - 2.74											GRANITE BOULDER RESIDUAL: BROWN, ORANGE, WHITE AND
50-	- 2./4 -	21	23	20		43	: :		SS-7	Sat	:::; <u>[</u>	BLACK SILTY FINE TO COARSE SAND
	_					\(\frac{1}{2} \cdot \frac{1}{2} \cdot \frac{1}{2	-: :				XIE!	49.46 (A-2-4), WEATHERED GRANITE. SOFT WEATHERED ROCK (SWR) SAMPLED
‡	_										W	AS ORANGE AND BROWN MICACEOUS SILTY FINE TO COARSE SAND. PARTIALLY
1	- 4.26 -	28	45	55			100/	26cm		Sat	$\langle \langle \rangle \rangle$	WEATHERED GRANITE.
48-	-										RKY	[_
1	-				` :			::			W	<u> </u>
_	5.78	100	_	_	; ;		100/	15cm		Sat	1)//+	 -
‡	-	''			: <i>:</i>			, .			17	<u> </u>
46-	- -				: :			::			181	<u>}</u>
1	7.22	100			::		100/	12cm		Sat	妼	f
†	_	100			: :						W	<u>[</u>
7	- -										{ {}	}
!	- - 8.74	400						•		Sat	8 (4	<u>(</u>
44	-	100					100/	1,5cm		Jat	1/1	<u> </u>
1	-						• •	, .) ///t	†
1	- -10.26										1/	-
1		100					100/	1,1 cm T		Sat	12/1	}
42	-										妼	F
1	- -11,78_										W	40.84 12
+		100					. 100	0/8cm .		Sat	//	HARD ROCK (HR), TAN, PINK AND GRAY,
1	_										>>>	MEDIUM TO COARSE GRAINED, SEVERELY WEATHERED TO FRESH, EXTREMELY
40	_											FRACTURED TO SOUND, SOFT TO VERY HARD GRANITE. (SEE CORE BORING
‡	_											REPORT)
- 1	_				: :						《《《	*
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]											₩	}
36-	<u>-</u>											35.67
]			-	ļ · -			· · · ·		<u> </u>		"	BORING TERMINATED AT 17.19 METERS
]	<u> </u>											IN GRANITE.
1]	``				1			
-	-]]				L		<u> </u>	

PROJECT NO :

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

BORING NO.: SBLB2-A

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING LOCATION (STA): 80+41.0 -L-

OFFSET: 17.7m LT

COLLAR ELEV.: 52.86m

DATE STARTED: 05-22-96

DRILL MACHINE: CME 45 on barge DRILL METHOD: Mud Rotary/Rock Core

COLLAR ELEV.: 52.86m DATE STARTED: 05-22-96
TOTAL DEPTH: 17.19m DATE COMPLETED: 05-22-96

DRILLER: F.Cox/K.Pendley

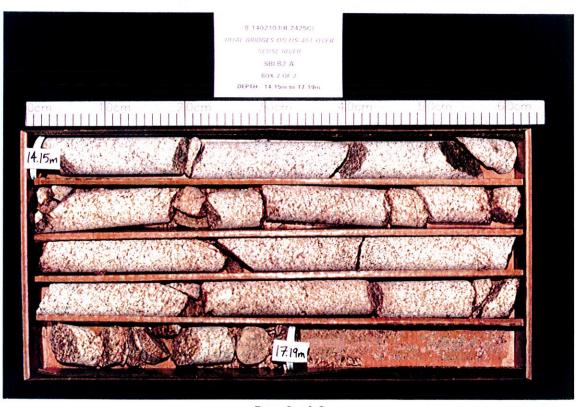
ORE SIZE: NO	TOTAL RUN: 5.17m
ORE SIZE: NU	TOTAL RUN: 5.17III

CORE SIZE	; NU		TOTAL RU	4. 5.17IR			DRIELLIN. T. COX/K.T Gridiey
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
51.15	1.71	13:00					Gray, medium grained, fresh, sound, very hard granite boulder
			0.30	0.22	0.22	Run 1	;
				(73%)	(73%)		
- 50.85	2.01						
40.84	12.02	2:30					Tan, pink and gray, medium to coarse grained, moderately
		2:30					weathered, extremely fractured, hard granite.
			0.61	0.14	0.51	Run 2	6 Joints at 30-40 degrees ADS = 3 to 6 cm
ļ				(23%)	(84%)		1 Joint at 70 degrees
40.23	12.63			<u>-</u>			
:		2:00					Tan, pink and gray medium to coarse grained, severely
		1:30				_	weathered to fresh, sound to extremely fractured, very hard to
		1:30	1.52	1.33	1.52	Run 3	soft granite. 9 Joints at 0-20 degrees ADS = 2 to 40cm
				(88%)	(100%)		•
38.71	14.15		·				4 Joints at 45 degrees ADS = 12 to 46cm Tan, pink and gray, medium to coarse grained, slightly
		3:00					weathered to fresh, sound to extremely fractured, very hard to
		2:30			2.05	Run 4	moderately hard granite.
		2:30	3.04	2.30	2.95	Run 4	13 Joints at 10-20 degrees ADS = 3 to 43cm
		1:30		(76%)	(97%)		9 Joints at 50-70 degrees ADS = 1 to 50cm
05.05	47.16	2:00					Boring terminated at 17.19 meters in granite
35.67	17.19	2:30				<u> </u>	Johns Committee at 17111

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB2-A



Box 1 of 2 1.71m to 2.01m 12.02m to 14.15m



Box 2 of 2 14.15m to 17.19m



					876-0416	T =					<u> 1 OF</u>	
	CT NO				ID. R-2425C		ITY Wa			GEOLOGIST BE		MATER 4
					on U.S.401 over Neuse River							WATER (m)
	G NO.			,——	PRING LOCATION 80+41.0		FSET 4			ALIGNMENT -L-	O HR.	N/A
OLLA	R ELEV	53.	79 m	NORT			STING			vided	24 HR.	N/A
TAL	DEPT	H 14.3	37 m	DRILL	MACHINE CME 45 on Barge						MER TYPE	140#Safet
ATE	START	ED 5/	17/96		COMPLETED 5/17/96	SU			R DEI	PTH 0.53 m		
LEV.	DEPTH	BLO	w cor	JNT	BLOWS PER 30 CM		SAM		0	SOIL AND ROCK	DESCRIPTION	N
(m)	(m)	15cm	15cm	15cm	0 20 40 60	80 1	100 NO.	MOI	G			
3.79					Mudline					53.79		
-	0.00	3	2	2				Sat		ALLUVIUM: GRAY AN SILTY, GRAVELLY FIF		
1										- (A-3)		
ŧ	<u> </u>						:		:::	• •		
52	- - 1.72	4.5		1.0			:	Sat	::-{	- - -		
327		10	10	18	28		:	380		51.63 RESIDUAL: WEATHER	ED GRANITE	<u></u>
ļ	<u>-</u>				\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\					50.99		
†	- 3.24			:]		179	SOFT WEATHERED R - AS ORANGE AND TA	N SILTY FINE	TO
Ī		54	46			100/28cm	1	W	M	COARSE SAND. PAR GRANITE.	IALLY WEA	THERED
50	-									49.49		
Ī	- - - 176						.		M	RESIDUAL: TAN SILT SAND (A-2-4) WITH F	Y FINE TO CO	DARSE IENTS.
‡	<u>4.76</u>	19	25	46	· , · · , · · · · · · · · · · · · · · ·			w		WEATHERED GRANIT		
‡	-				f_{i}		<u> </u>		أفهر	48,29 SOFT WEATHERED R	OCK (SWR) 3	AMPLED
48	_]					17/1	AS ORANGE AND TA	N SILTY FINE	: TO
ŧ	6.28	100		-		100/10cm	•	М	权	GRANITE.	IALLI WEA	
‡	<u>.</u> .								19 %	- 		
Ŧ	<u> </u>								W			
46	7.80	100				100/12cm	 		\\\ }	· ·		
ŧ	_	100							<u>W</u>	45.35	N- 515112 - 2212	CRAV
-	-									HARD ROCK (HR): TA MEDIUM TO COARSE	GRAINED, S	LIGHTLY
‡	_									WEATHERED TO FREE FRACTURED TO SOU	SH. EXTREM	ELY
44	_								₩	HARD TO VERY HARI CORE BORING REPOR	GRANITE.	SEE
"" Ţ	Ė į						:		₩	- COME DOMING HER OF	• •	
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7	-								 	- - -		
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40	_						:			- -		
Ŧ	_						-		777	- 39.42 BORING TERMINATED	ΔΤ 1/ 27 Ν	1 METERS
‡	_									IN GRANITE.	/AI 14.0/ N	76) EI 10
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38	-									. •		
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36	<u>-</u> -			,						- - -		

PROJECT NO.:

8,1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: SBLB2-B BORING LOCATION (STA): 80+41.0 -L-

OFFSET: 4.0m LT

COLLAR ELEV.: 53.79m TOTAL DEPTH: 14.37m

DATE STARTED: 05-17-96 DATE COMPLETED: 05-17-96 DRILL MACHINE: CME 45 on barge DRILL METHOD: Mud Rotary/Rock Core

TOTAL RUN: 5.93m CORE SIZE: NO

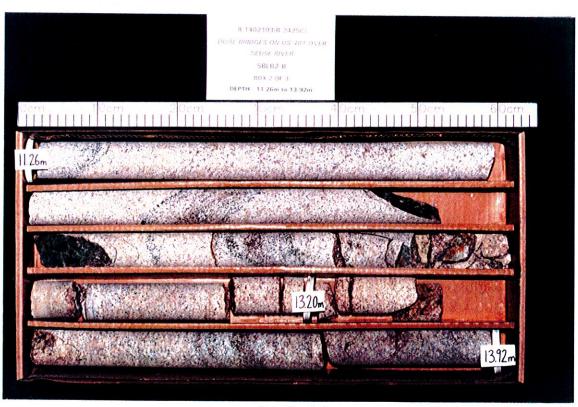
DRILLER: F.Cox/K.Pendley

CORE SIZE:	NQ		TOTAL RUI	v; 5.93m			DRILLER: F.Cox/K.Penaley
ELEV.	DEPTH	DRILL RATE	RUN	RQD	REC	SAMP	FIELD CLASSIFICATION AND REMARKS
(m)	(m)	(MN/30cm)	(m)	%	%	NO.	
45.35	8.44	1:19					Tan pink and gray, medium to coarse grained, slightly to
		1:47					moderately weathered, slightly to extremely fractured, hard
		2:40	1.72	0.82	1.60	Run 1	to moderately hard granite.
		1:45		(48%)	(93%)		8 Joints at 9-20 degrees ADS = 2 to 40cm
		1:41					8 Joints at 45-60 degrees ADS = 2 to 35cm
43.63	10.16	1:39					
		1:37					Pink and gray, medium to coarse grained, slightly weathered to
		3:10					fresh, sound to extremely fractured, very hard to hard granite.
		4:00		;			6 Joints at O degrees ADS = 3 to 18 cm
		4:12					9 Joints at 45 degrees ADS = 2 to 122cm
		4:15					1 Joint at 90 degrees
		4:04					
		4:36	3.04	2.05	2.98	Run 2	
		7:20		(67%)	(98%)		
		6:19					
40.59	13.20	5:58					
		5:31					Pink and gray, medium to coarse grained, fresh to slightly
		8:46	1.17	0.94	1.17	Run 3	weathered, sound to moderately fractured, very hard to hard
		29:10		(80%)	(100%)		granite.
		43:22					9 Joints at O-10 degrees ADS = 2 to 36cm
39.42	14.37						2 Joints at 45 degrees ADS = 44cm
							Boring terminated at 14.37 meters in granite
					<u> </u>		

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB2-B

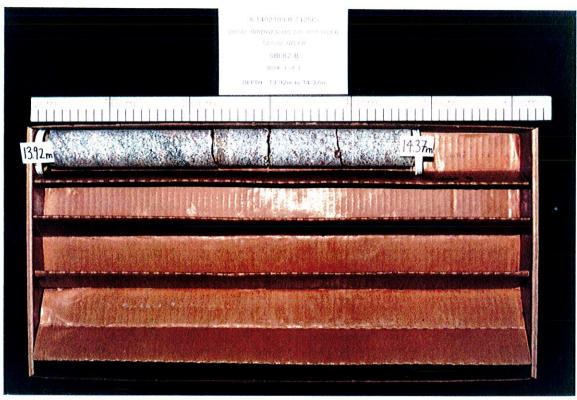


Box 1 of 3 8.44m to 11.26m



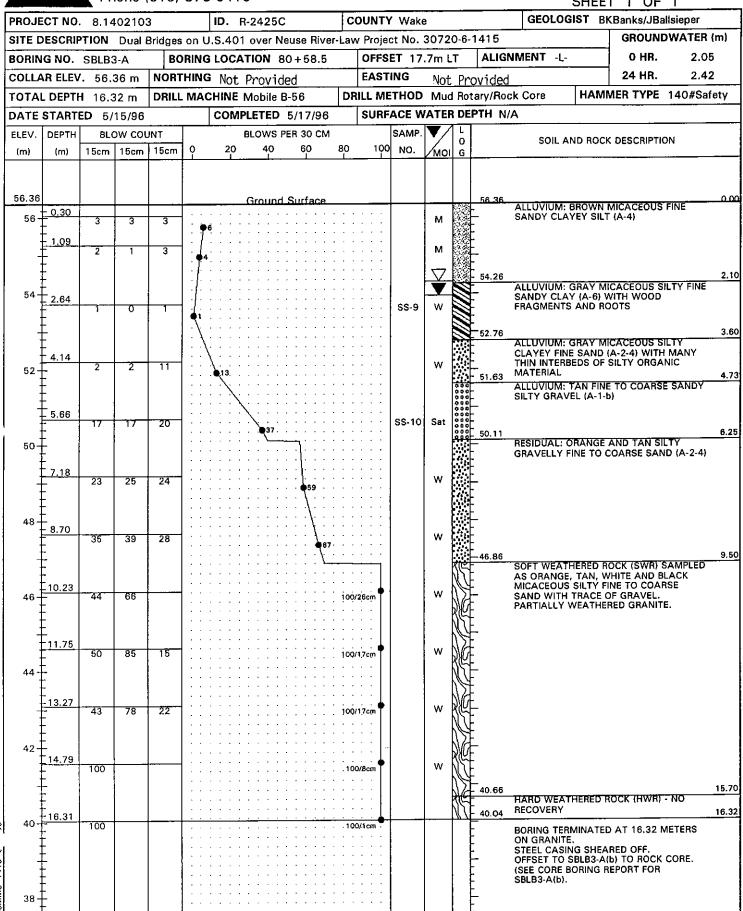
Box 2 of 3 11.26m to 13.92m

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB2-B



Box 3 of 3 13.92m to 14.37m





PROJECT NO.: 8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: J.Ballsieper

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

DRILL MACHINE: Mobile B-56

BORING NO.: SBLB3-A(b)

BORING LOCATION (STA): 80 + 58.5 -L
COLLAR ELEV.: 56.37m

DATE STARTED: 05-15-96

TOTAL DEPTH: 18.16m

DATE COMPLETED: 05-17-96

DRILL METHOD: Roc

DRILL METHOD: Rock Core

CORE SIZE	: NQ		TOTAL RUI	N: 2.94m			DRILLER: S.Hancock/J.Young
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
41.14	15.22	2:46 2:51 2:12	1.65	1.42	1.61	Run 1	Pink and gray, medium to coarse grained, fresh to moderately severely weathered, sound to extremely fractured, very hard to moderately hard granite.
		3:17 3:25	1,05	(86%)	(98%)	riuii i	4 Joints at 0 degrees ADS = 2 to 91cm 3 Joints at 20 degrees ADS = 10 to 52cm 10 Joints at 45 degrees
39.49	16.87	3:47 4:07					2 Joints at 80 degrees ADS = 10 to 52cm Pink and gray medium to coarse grained, fresh to slightly weathered, sound to moderately fractured, very hard to hard
38.20	18.16	4:22 4:13	1.29	1.21	1.29 (100%)	Run 2	granite. 5 Joints at 0 degrees ADS = 4 to 18cm 1 Joint at 45 degrees
00120							Boring terminated at 18.16 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB3-A

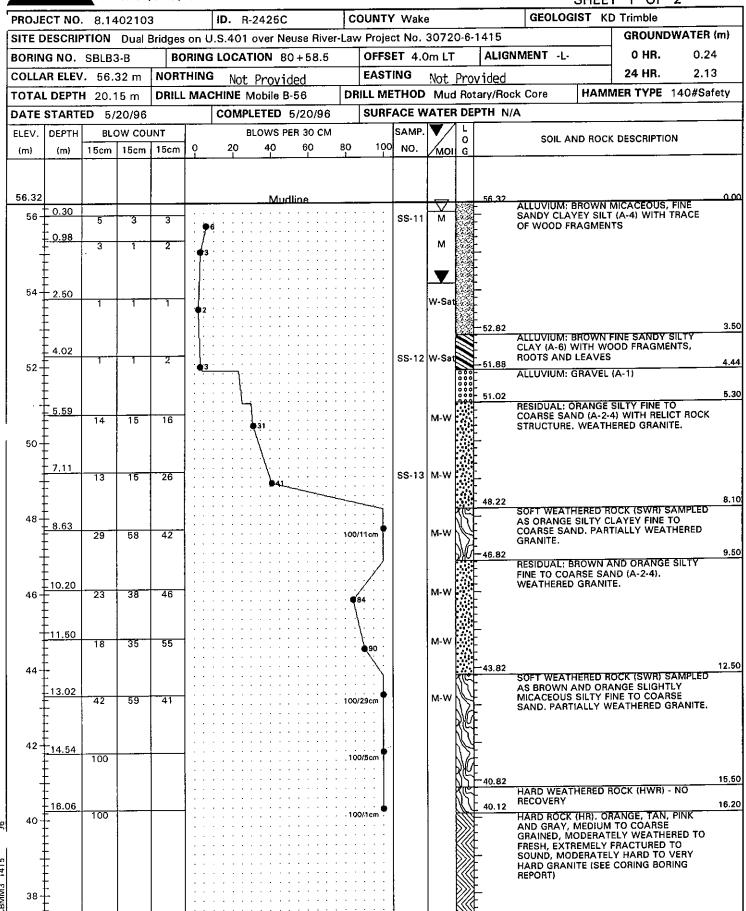


Box 1 of 2 15.22m to 17.96m



Box 2 of 2 17.96m to18.16m







PROJE	CT NO.	8.14	0210	3		ID.	R-2	425C		7	COUNTY	/ Wak	В			GEOLOG	IST K	D Trimble	
SITE D	ESCRIP	TION	Dual I	Bridges	on l	J.S.40)1 o	ver Ne	use R	iver-La	w Proje	ct No.	3072	0-6-	1415			GROUND	WATER (m
BORIN	G NO.	SBLB3	3-B	ВС	RINC	S LOC	ATIC	ON 80) + 58	.5	OFFS	ET 4.0	m LT		ALIG	NMENT -L-		O HR.	0.24
COLLA	AR ELEV	. 56.	32 m	NORT	THING	3 N/	nt P	rovio	led		EAST	ING	Not:	Prov	/ided			24 HR.	2.13
	L DEPTH			DRILL						D	RILL ME	THOD				ck Core	HAM	MER TYPE	140#Safet
	START						_	TED				ACE W		_			•		
ELEV.	DEPTH		ow cou	JNT		1	_)WS PI				SAMP.	lacksquare	Ľ	-	eou A	ND POC	/ DESCRIPTIO)N
(m)	(m)	15cm	15cm	15cm	ļ	20)	40	60	80	100	NO.	моі	0 G		50IL A	ND ROCI	C DESCRIPTIO	····
										•									
37.32	-	<u> </u>				Cont	inuec	1 from	previ	ous pa	ge			1111				RANGE, TAN,	
-	-				: :		•								-	GRAINED, M	ODERAT	TO COARSE	RED TO
-							· ·								36.17	FRESH, EXTI SOUND, MO	REMELY DERATE	FRACTURED LY HARD TO	TO 2.
36-	-						: :								-	HARD GRAN REPORT)	IITE (SEE	CORING BOR	RING
															-	BORING TER		D AT 20.15 N	ETERS
-	‡ į														-	IN GRANITE.			
-	‡ 1					: : :									- -				
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PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: SBLB3-B

BORING LOCATION (STA): 80+58.5 -L-

OFFSET: 4.0m LT DRILL MACHINE: Mobile B-56

COLLAR ELEV.: 56.32m COLLAR ELEV.: 56.32m TOTAL DEPTH: 20.15m

DATE STARTED: 05-20-96 DATE COMPLETED: 05-20-96

DRILL METHOD: Mud Rotary/Rock Core

CORE SIZE: NO

TOTAL RUN: 3.95m

DRILLER: S.Hancock/J.Young

CORE SIZE	NU		TOTAL RUI	v: 3.95m			DRILLER: 5. Harrock/3. Foung
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
40.12	16.20 17.11	3:00 2:30 2:30	0.91	O (O%)	0.36 40%	Run 1	Orange and tan, medium to coarse grained, moderately weathered, moderately to extremely fractured, moderately hard granite. Joints show iron staining and clay infilling
37.69	18.63	2:35 1:45 2:00 2:35 2:20	1.52	0.40 (26%)	0.76 (50%)	Run 2	Orange tan and gray, medium to coarse grained, moderately to very slightly weathered, slightly to extremely fractured, hard to moderately hard granite. 4 Joints at 45 degrees ADS = 4 to 17cm Joints show iron staining
36.17	20.15	2:45 4:36 6:00 6:30	1.52	1.45 (95%)	1.52 (100%)	Run 3	Pink and gray, medium to coarse grained, fresh, moderately fractured to sound, very hard granite. 5 Joints at 50-60 degrees ADS = 9 to 70cm 1 Joint at 80 degrees Joints show iron staining and mineralization
							Boring terminated at 20.15 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) SBLB3-B

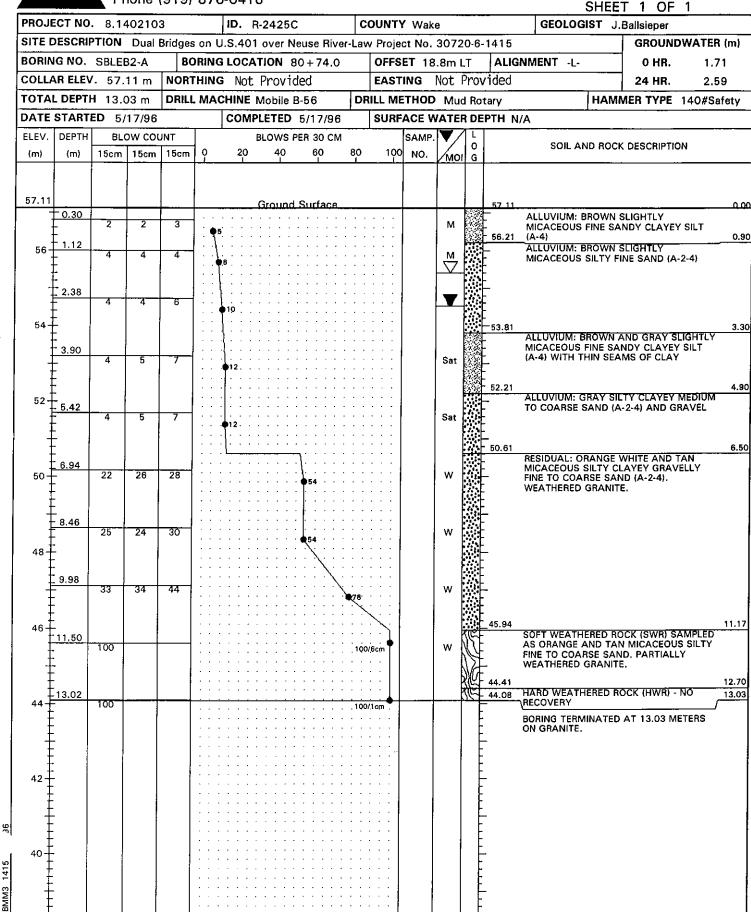


Box 1 of 1 16.20m to 20.15m

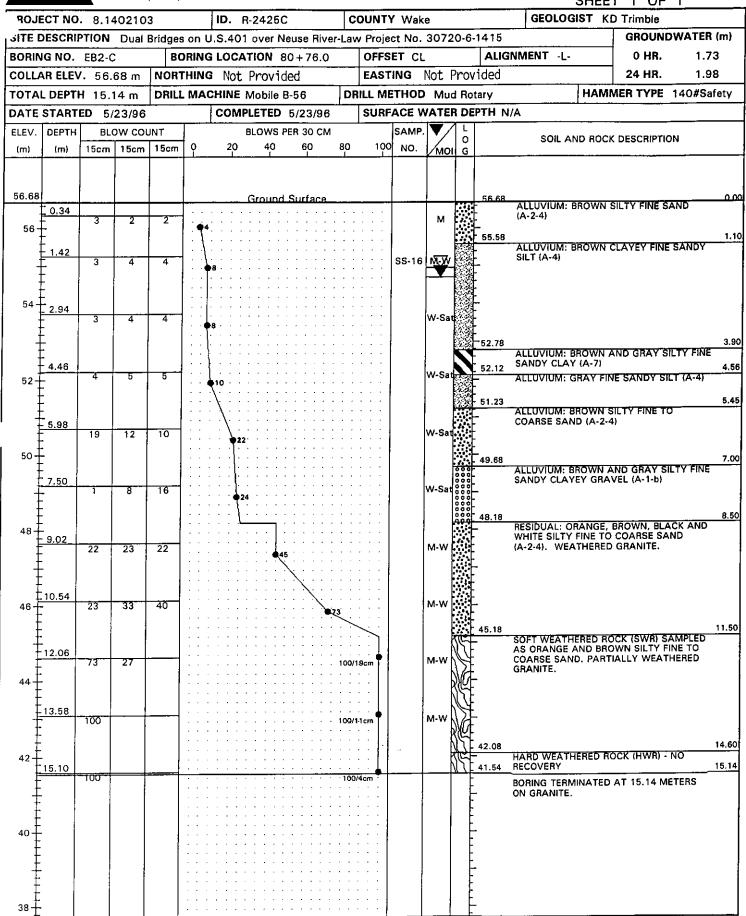


ICBMM3 1415

LAW ENGINEERING, INC. 3301 Atlantic Avenue Raleigh, North Carolina 27619 Phone (919) 876-0416







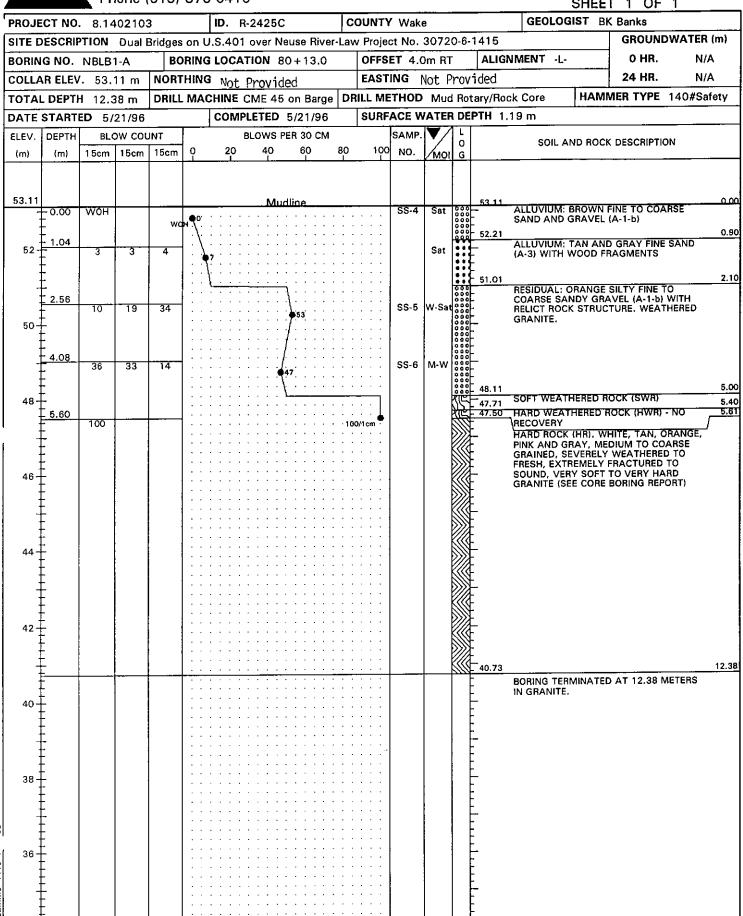


ROJE	CT NO	. 8.14	0210	3	ID.	R-2425C		C	DUNTY	Wake	,			GEOLOG	ST BK	Banks		
SITE D	ESCRI	PTION	Dual I	Bridges	on U.S.40	l over Ne	use Rive	r-Lav	/ Projec	t No.	3072	0-6-	1415			GROUND	WATER	(m
	G NO.				RING LOCA				OFFSI			•		IENT -L-		O HR.	0.27	7
	R ELEV			NORT	HING Not	Provid	ed		EAST	ING	Vot F	rov	/ided			24 HR.	0.90)
	DEPT			+	MACHINE			DR	ILL ME						HAMN	MER TYPE	140#Saf	et
	START			<u> </u>		PLETED			SURF								<u> </u>	_
LEV.	DEPTH		ow cou	TNL		BLOWS PI				SAMP.	V /	L						
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			ļ															
57.33	- 0.30					Ground	Surface				∇		57.33		00000	OIL TV FINE C	AND	
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]	1.07	2	2	4							М	: ·:	56.10					
56	-	4	2	"	, 🍎 6						,,,,		_ м	LLUVIUM: E ICACEOUS	FINE SA	AND GRAY NDY SILT (A-	4) WITH	
1													T F	RACE OF GR	RAVEL			
-	2.59				: : : : : :								-					
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54-	- -				: : : : : :								-					
‡	- 4.11												<u> </u>					
\pm	_	2	3	4	7			: : :			W		-					
‡	-				: : } : : :								- 52.23					
52	- - 5.63						- · · · ·		" .			000	[(A	LLUVIUM: B 1-b)	ROWN S	SILTY SANDY	GRAVEL	
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#	<u>.</u>				: : : : :							\mathcal{M}		S WHITE, BI COARSE S		ND TAN SILT` ARTIALLY	Y FINE	
‡	•								`			NA		EATHERED				
50-	7.15	100			· · · · · ·		: : : :	100/	15cm		М	RE	<u> </u>					
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		Pr	nome ((919)	8/	76-0416						SHEET 1 OF 1
PRO	ECT N	O. 8.14	10210	3		ID. R-2425C	C	DUNTY	' Wak	е		GEOLOGIST KD Trimble
SITE	DESCI	RIPTION	Dual	Bridges	on:	U.S.401 over Neuse River	Lav	/ Projec	et No.	3072	0-6-	GROUNDWATER (m)
BORI	NG NO	. NBLE	B1-B	ВС	RIN	NG LOCATION 79+85.0		OFFSI	E T 22	.5m F	₹T	ALIGNMENT -L- O HR. 2.97
COLL	AR EL	V. 56.	52 m	NORT	THIN	NG Not Provided		EAST	ING	Not	Prov	ovided 24 HR. 0.90
TOTA	AL DEP	TH 13.	19 m			ACHINE Mobile B-56	DR	ILL ME	THOD	Mud	Rot	otary HAMMER TYPE 140#Safety
		TED 5/				COMPLETED 5/23/96		SURF	ACE W	/ATEI	R DE	EPTH N/A
ELEV	. DEPT	H BL	ow cou	TNL		BLOWS PER 30 CM			SAMP.	$\mathbf{V}/$	1 L	SOIL AND ROCK DESCRIPTION
(m)	(m)	15cm	15cm	15cm	인	20 40 60	80	100	NO.	MOI		•
						-						
-0-												56.52
56.5	⇟	+	<u> </u>		<u> </u>	Ground Surface						ALLUVIUM: BROWN SILTY FINE SAND
56	L	3	3	3	1 :	6				₩		(A-2-4) - 55.570.95
	0.99	4	5	9	 					M-W		ALLUVIUM: BROWN AND GRAY SILTY FINE SANDY CLAY (A-6)
	‡	İ										SANDI CEAT (ATO)
	Ī				;							
54	2.51	3	3	21	:					<u>γ</u> 4-λγ/		-
	Ē				:	24				\vdash		.
ľ	‡										000	53.02 3.50 ALLUVIUM: BROWN FINE TO COARSE
	4.03	17	19	22						W-Sa	0000	SANDY SILTY GRAVEL (A-1-b)
52	‡	''	''		<u> </u>	• • • • • • • • • • • • • • • • • • • •	: :			·	000	of 52.02 4.30 RESIDUAL: ORANGE, BROWN, WHITE AND
	Ē		ļ								0000	TI DIACK CHITY CANDY COAVEL (A 1 5)
	5.55] :				SS-3	M-W	000	ö -
	‡	19	30	37	:		: :		33.3	141-44	000	6
50	£				:		<u> </u>				000	0 0 0
	7.07] :		: >	<u>````</u>			000	0 - 0
	Ī	43	46	50	:			996		M-W	000	\$ -
	‡				:		• •	<i>:</i> /:::			000	48.52 8.00
48	± 8.59				:		: :					SAND (A-2-4), WEATHERED GRANITE.
40	-	24	18	55	1:		73			M-W		
	Ī				1:		• •				, i	9.50 SOFT WEATHERED ROCK (SWR) SAMPLED
	10.1	,			:						TM	AS BROWN AND BLACK SILTY SAND. PARTIALLY WEATHERED GRANITE.
46	E	100			1 :		100	0/9cm		M-W	1/2	PARTIALLY WEATHERED GRANTE.
40	Ŧ				1						KK	<u>V</u>
	‡,,,	,			1:			: .]			16	<u>\f</u>
	-11.6	100	<u> </u>	1			100	0/9cm		M-W	M	Ćt.
	‡				:						77	44.02 12.50
44	+	_			:					ŀ	M	HARD WEATHERED ROCK (HWN) 1 NO RECOVERY 13.18
	13.1	100		-	 		10	0/4cm			100	BORING TERMINATED AT 13.19 METERS
	Ŧ				:				1			ON GRANITE.
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42	Ŧ											ļ.
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38	‡				:							E
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38	+				1		: :					F





PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO: NBLB1-A

OFFSET: 4.0m RT

BORING LOCATION (STA): 80+13.0 -L-

DRILL MACHINE: CME 45 on barge DRILL METHOD: Mud Rotary/Rock Core

COLLAR ELEV: 53.11m DATE STARTED: 05-21-96
TOTAL DEPTH: 12.38m DATE COMPLETED: 05-21-96

TOTAL DUNE 6 77

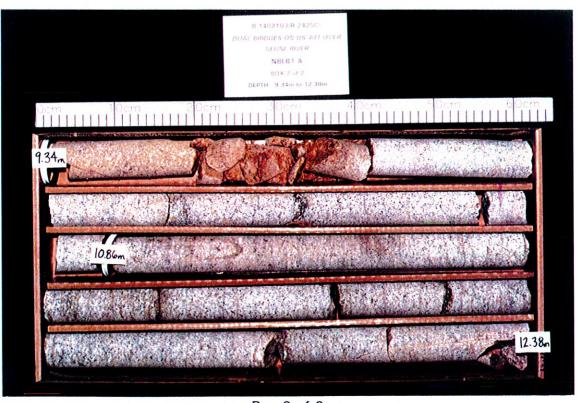
DRILLER: K Pendley

CORE SIZE:	NQ		TOTAL RUI	N: 6.77m			DRILLER: K.Pendley
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
47.50 46.81	5.61	2:56 1:54	0.69	O (0%)	0.53 (77%)	Run 1	White, tan and orange medium to coarse grained, moderately to severely weathered, moderately to extremely fractured, moderately hard to very soft granite
45.29	7.82	0:28 1:03 2:38 3:18	1.52	0.68 (45%)	0.84 (55%)	Run 2	(Recovered material) Pink and gray medium to coarse grained fresh, slightly fractured to sound, very hard granite
43.77	9.34	3:19 2:56 2:38 2:42 2:30	1.52	1.19 (78%	1.52	Run 3	Pink gray and orange, medium to coarse grained, fresh to moderately weathered, sound to extremely fractured, very hard to moderately hard granite. 5 Joints at 45 degrees ADS = 2cm 2 Joints at 0 degrees ADS = 2cm
42.25	10.86	1:32 2:28 2:48 2:52 2:58	1.52	1.20 (79%)	1.45 (95%)	Run 4	Pink gray and orange, medium to coarse grained, fresh to moderately weathered, sound to extremely fractured, very hard to moderately hard granite. 2 Joints at 70 degrees ADS = 8cm 3 Joints at 0-10 degrees ADS < 36cm
40.73	12.38	4:29 4:40 4:52 5:03 5:22	1.52	1.52 (100%)	1.52	Run 5	Pink and gray, medium to coarse grained, fresh, sound to slightly fractured, very hard granite 4 Joints at 0-10 degrees ADS = 13 to 22cm 2 Joints at 35 degrees ADS = 14cm Boring terminated at 12.38 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB1-A



Box 1 of 2 5.61m to 9.34m



Box 2 of 2 9.34m to 12.38m



		Ph	one	(919)	87	6-0416							SHEE	T 1 OF	1
PROJE	CT NO	. 8.14	0210	3		ID. R-242	5C	CO	UNTY	Wake	9		GEOLOGIST BI	(Banks	
SITE D	ESCRI	PTION	Dual	Bridges	on	U.S.401 over	Neuse River-	Law	Projec	et No.	3072	0-6-	1415	GROUND	WATER (m)
BORIN	G NO.	NBLB	1-B	ВС	RIN	G LOCATION	80 + 13.0		OFFS	ET 21	.0m F	ìΤ	ALIGNMENT -L-	O HR.	N/A
COLLA	R ELEV	/. 52.	73 m	NORT	HIN	G Not Pro	vided		EAST	ING P	Vot P	rov	ided	24 HR.	N/A
TOTAL	DEPT	H 14.0	08 m	DRILL	. MA	CHINE CME	45 on Barge	DRI	LL ME	THOD	Muc	Rot	tary/Rock Core HAMI	MER TYPE	140#Safety
DATE	START	ED 5/	15/96	٠		COMPLETE	D 5/15/96		SURF	ACE W	ATER	DE	PTH 1.14 m		
ELEV.	DEPTH	BLC	ow col	JNT		BLOWS	S PER 30 CM			SAMP.	\bigvee	г о	SOIL AND ROCI	DESCRIPTIO	N
(m)	(m)	15cm	15cm	15cm	Ŷ	20 40	60	80	100	NO.	MOI				
52.73						8.	fudline						52 73		0
1	- 0.00	WOH	1	0	•1						Sat		- ALLUVIUM: BROWN - SAND (A-3) WITH W		NTS
52 -	- - 1.01				:/:			: :					- 52.03 RESIDUAL: TAN SILT	Y FINE TO CO	ARSE
1	+ 1.01 -	10	14	16		30					w	: . [SAND (A-2-4), WEAT	HERED GRAN	ITE.
-	-					· · · · · · · · · · · · · · · · · · ·		: :				; }	- F0 63		2.
]	_		•						· · ·			रांप्री	50.63 SOFT WEATHERED R		AMPLED
50-	2.70	22	50	50	: :			100/2	· · ·		l w	W	AS ORANGE AND TA COARSE SAND AND	GRAVEL, PAF	RTIALLY
†	-	2.2	"	30				100/1			'	M	WEATHERED GRANIT	Έ.	
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†	4.22	19	4.1	59	, .				 •		w	KI.			
48		19	41	59				100/2			''	廵			
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1	- 5.74] : :				: : 		١	\mathbb{K}	<u> </u>		
]	-	100						100/1	1,5cm T		M	刿	- -		
46	- -											W	- - -		_
46	- 7.26	į										14	- 45.64 - 45.47 HARD WEATHERED I	OCK (HWR) -	7 7 7 7 70 7 7
1	-	100			: :			100	/0cm			M	RECOVERY HARD ROCK (HR). PII	UK GRAV AÑ	D TAN
1	-												MEDIUM TO COARSE WEATHERED TO FRE	GRAINED, SI	VERELY
†	-				: :							\gg	 EXTREMELY FRACTU 	RED, VERY S	OFT TO
44	_												VERY HARD GRANITI	e. (SEE CONE	BONING
†	<u>-</u>												- - -		
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40	-												<u>-</u>		
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34-							<u></u>		<u> </u>		l		_		

PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO: NBLB1-B

BORING LOCATION (STA): 80+13.0 -L-

OFFSET: 21.0m RT

COLLAR ELEV.: 52.73m

DATE STARTED: 05-15-96

DRILL MACHINE: CME 45 on barge

TOTAL DEPTH: 14.08m

DATE COMPLETED: 05-15-96 TOTAL BUN: 6.82m

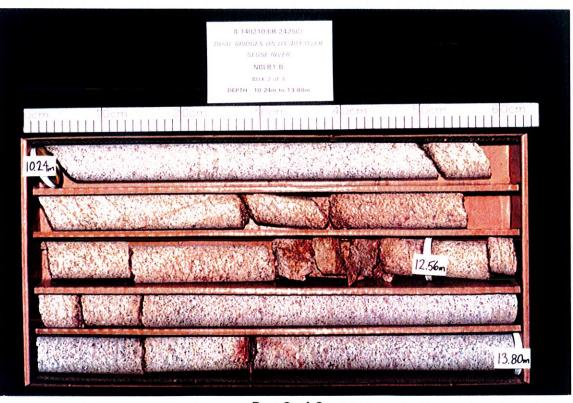
DRILL METHOD: Mud Rotary/Rock Core

CORE SIZE	: NQ		TOTAL RU	N: 6.82m			DRILLER: F.Cox/K.Pendley
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
45.47	7.26		2.26	1.35 (60%)	2.14 (95%)	Run 1	Pink and tan medium to coarse grained, moderately severe to severely weathered, sound to extremely fractured, moderately hard to very soft granite. 6 Joints at 45 degrees ADS = 4 to 9cm 2 Joints at 70 degrees 4 Joints at 0 degrees ADS = 13 to 55cm
43.21	9.52		3.04	1.82	2.24 (74%)	Run 2	Pink and tan medium to coarse grained, fresh to very severely weathered, sound to moderately fractured, very hard to very soft granite. 3 Joints at 0 degrees ADS = 6 to 32cm 1 Joint at 90 degrees 6 Joints at 45 degrees ADS = 6 to 46cm
38.65	14.08		1.52	1.35 (89%)	1.52	Run 3	Pink, gray and tan medium to coarse grained, fresh to slightly weathered, sound to moderately fractured, very hard to hard granite. 2 Joints at 20 degrees ADS = 3cm 4 Joints at 0 degrees ADS = 7 to 59cm
							Boring terminated at 14.08 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB1-B



Box 1 of 3 7.26m to 10.24m



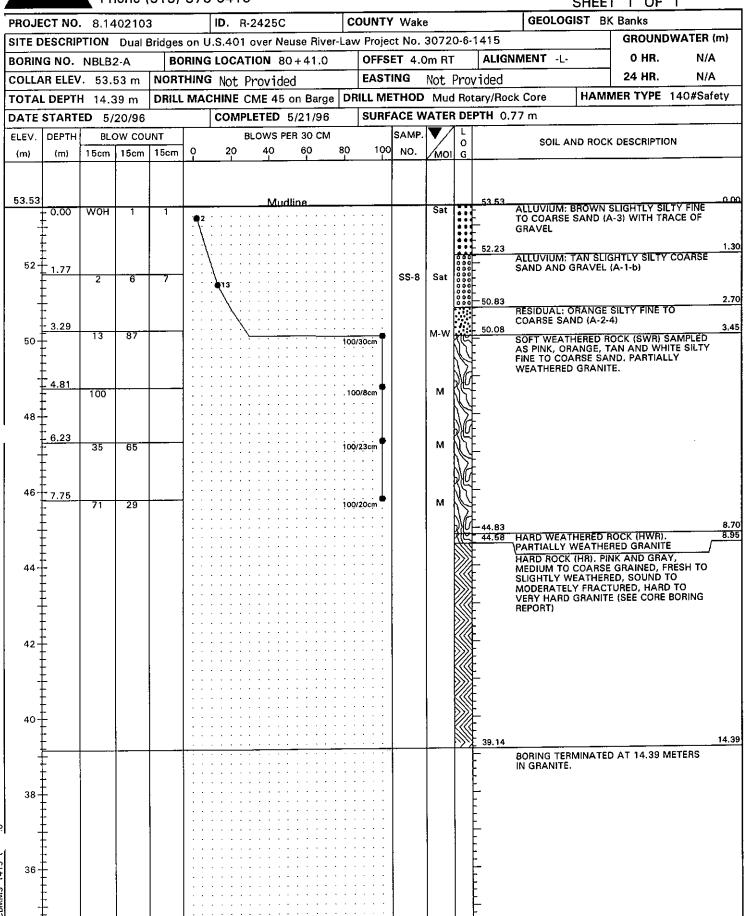
Box 2 of 3 10.24m to13.80m

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB1-B



Box 3 of 3 13.80m to 14.08m





PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: NBLB2-A

BORING LOCATION (STA): 80+41.0 -L-

OFFSET: 4.0m RT

DATE STARTED: 05-20-96 COLLAR ELEV.: 53.53m DATE STARTED: 05-20-96
TOTAL DEPTH: 14.39m DATE COMPLETED: 05-21-96 DRILL MACHINE: CME 45 on barge DRILL METHOD: Mud Rotary/Rock Core

DRILLER: F.Cox/K.Pendley

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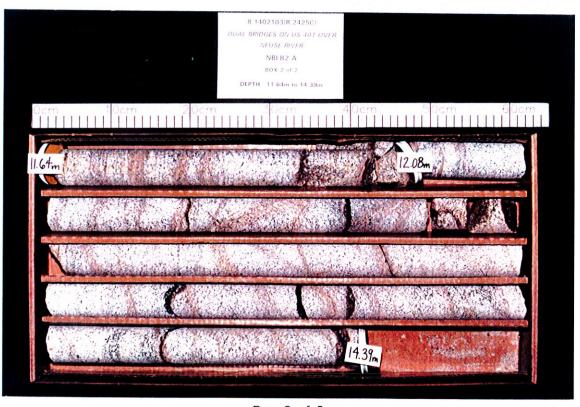
TOTAL RUN: 5).44ı	n
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ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
44.58 43.61	8.95 9.92	2:30 5:03 3:09	0.97	0.63 (65%)	0.93 (96%)	Run 1	Pink and gray, medium to coarse grained, fresh to slightly weathered, sound to moderately fractured, hard to very hard granite. 9 Joints at 0-10 degrees ADS = 1 to 29cm
41.45	12.08	2:57 5:13 5:11 5:52 6:19 13:22 27:43	2.16	1.75 (81%)	2.16 (100%)	Run 2	Pink and gray, medium to coarse grained, fresh, sound to moderately fractured, very hard granite. 9 Joints at 0-20 degrees ADS = 12 to 30cm 3 Joints at 40 degrees ADS = 1 to 16cm 2 Joints at 60 degrees ADS = 40cm
39.14	14.39	1:40 2:28 2:28 3:20 5:16 4:59 5:09	2.31	2.09 (90%)	2.31 (100%)	Run 3	Pink and gray, medium to coarse grained, fresh, sound to moderately fractured, very hard granite. 8 Joints at 0-20 degrees ADS = 2 to 95cm 4 Joints at 45 degrees ADS = 32 to 58cm Boring terminated at 14.39 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB2-A

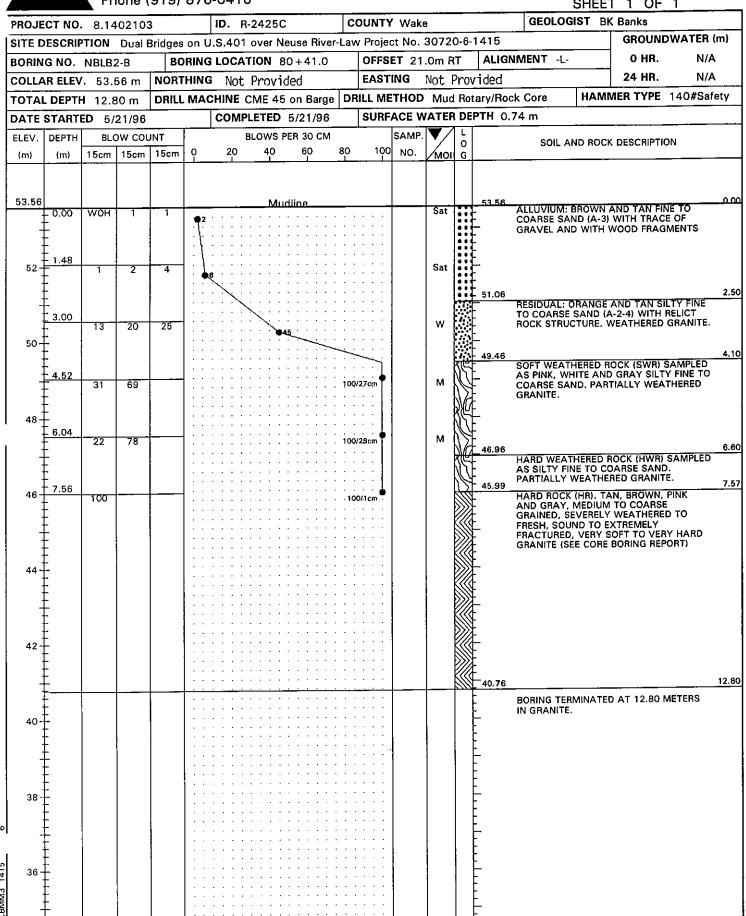


Box 1 of 2 8.95m to 11.64m



Box 2 of 2 11.64m to 14.39m





PROJECT NO :

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: NBLB2-B

BORING LOCATION (STA): 80+41.0 - L-

OFFSET: 21.0m RT

DRILL MACHINE: CME 45 on barge

COLLAR ELEV.: 53.56m DATE STARTED: 05-21-96
TOTAL DEPTH: 12.80m DATE COMPLETED: 05-21-96

DRILL METHOD: Mud Rotary/Rock Core

CORE SIZE: NO

TOTAL RUN: 5.23m

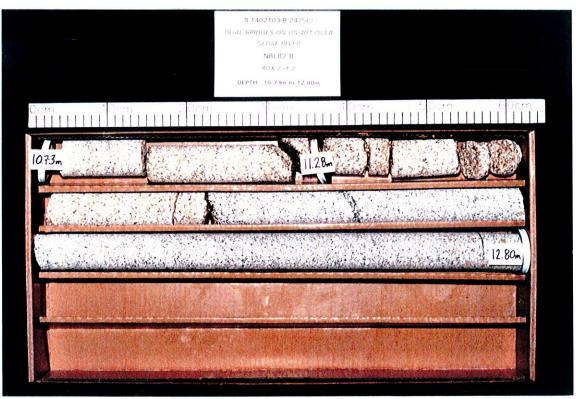
DRILLER: F.Cox/K.Pendley

CORE SIZE	, IVQ		TOTAL RUI	V. 9.23III			DRILLER. F.COX/K.I elidicy
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
45.99 45.32	7.57 8.24	1:23	0.67	0.63 (94%)	0.63 (94%)	Run 1	Tan and pink, medium to coarse grained, moderately weathered, sound to slightly fractured, hard to moderately hard granite. 3 Joints at O-10 degrees ADS = 4 to 42cm Joints show iron staining
43.80	9.76	1:01 1:09 1:08 1:03	1.52	0.80 (53%)	1.22 (80%)	Run 2	Pink, brown, tan, medium to coarse grained, moderately to severely weathered, slightly to extremely fractured, hard to very soft granite
42.28	11.28	1:19 1:26 1:32 1:22 1:34	1.52	1.27 (84%)	1.29 (85%)	Run 3	Tan and gray, medium to coarse grained, slightly weathered to fresh, sound to slightly fractured, hard to very hard granite. 4 4 Joints at 0-15 degrees ADS = 20 to 76cm 1 Joint at 45 degrees
40.76	12.80	1:31 1:47 2:03 2:14 3:41	1.52	1.17 (77%)	1.45 (95%)	Run 4	Pink and gray, medium to coarse grained, very slightly weathered to fresh, sound to moderately fractured, very hard granite. 7 Joints at 0-10 degrees ADS = 2 to 17cm
							Boring terminated at 12.80 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB2-B



Box 1 of 2 7.57m to 10.73m



Box 2 of 2 10.73m to 12.80m



PROJE	CT NO.				ID. R-2425C COUNTY Wake GEOLOGIST KD	Trimble
					on U.S.401 over Neuse River-Law Project No. 30720-6-1415	GROUNDWATER (m
	G NO.				RING LOCATION 80+58.5 OFFSET 4.0m RT ALIGNMENT -L-	0 HR. 0.00
OLLA	R ELEV	. 56.4	19 m	NORT	HING Not Provided EASTING Not Provided	24 HR. 2.50
OTAL	. DEPTH	1 18.7	'5 m			IER TYPE 140#Safet
ATE	STARTE	D 5/3	21/96	L	COMPLETED 5/21/96 SURFACE WATER DEPTH N/A	
LEV.	DEPTH	BLC	ow cou	JNT	BLOWS PER 30 CM SAMP. V L	DECORPTION
(m)	(m)	15cm	15cm	15cm	0 20 40 60 80 100 NO. MOI G SOIL AND ROCK	DESCRIPTION
		İ				
56.49	0.34				Ground Surface. 56.49 ALLUVIUM: BROWN N	
56-	_	3	3	3	D-M SAND (A-2-4) WITH V	VOOD FRAGMENTS
1	1.11	4	2	2	ALLUVIUM: BROWN M SANDY CLAYEY SILT	MICACEOUS FINE
-	-	7			of WOOD FRAGMENT	
‡	-					
54	2.63		4	1	V M-W M -W	
]	_	2	'	'	♦ 2	
†	-				52.59	
1	4.12				ALLUVIUM: BROWN S	
52	-	1	- 4	6	51.79	
1	-				RESIDUAL: ORANGE A	ID (A-2-4).
+	- - 5.64				WEATHERED GRANITI	I.
1	-	22	17	22	W-Sat	
50	-					
1	- - 7.16					
1	_	34	34	46	W-Sat	
†	-	Ì			48.49	CK (SWR) SAMPLED
48-	- - 8.68				AS ORANGE AND BRO	OWN SILTY CLAYEY
ŧ	-	23	27	73	M-W WEATHERED GRANITE	-
1	-					
1	- -10.20					
46	-10.20	100			M-W M-W	
	-					
1	-					
 	11.72	100			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
44	-					
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~	14.76	100			M-W	
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7	-				HARD WEATHERED R	US FINE SANDY
40	16.28	100			40.17 SILT. PARTIALLY WEA	
707	-				MEDIUM TO COARSE MODERATELY WEATH	GRAINED,
‡	-				EXTREMELY FRACTURE MODERATELY HARD	RED TO SOUND,
1	-				GRANITE. (SEE CORE	
1	-				BORING TERMINATED	AT 18.75 METERS
38-	-			<u></u>	IN GRANITE.	11

PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: NBLB3-A

BORING LOCATION (STA): 80+58.5 -L-

OFFSET: 4.0m RT

COLLAR ELEV.: 56.49m

DATE STARTED: 05-21-96

DRILL MACHINE: Mobile B-56

TOTAL DEPTH: 18.75m

DATE COMPLETED: 05-21-96

DRILL METHOD: Mud Rotary/Rock Core DRILLER: S.Hancock/J.Young

CORE SIZE: NQ TOTAL RUN: 2.43m

ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
40.17	16.32	2:22 2:45 5:15	0.91	0.51 (56%)	0.74 (81%)	Run 1	Tan, pink and gray, medium to coarse grained, moderately weathered to fresh, extremely to moderately fractured, very hard to moderately hard granite. 4 Joints at 0 degrees ADS = 4 to 22cm 2 Joints at 65 degrees ADS = 9cm Joints show iron staining.
39.26		7:48 8:00 7:15 7:45	1.52	1.12 (74%)	1.52 (100%)	Run 2	Pink and gray, medium to coarse grained, fresh, sound to moderately fractured, very hard granite. 12 Joints at 0-10 degrees ADS = 3 to 48cm Few joints show some iron staining
37.74	18.75	8:30					Boring terminated at 18.74 meters in granite

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB3-A



Box 1 of 1 16.32m to 18.75m



		Ph	ione	(919)	870	6-0416							T 1 OF	2	
	CT NO					ID. R-2425C		OUNTY					GEOLOGIST KD Trimble		
SITE D	ESCRI	MOIT	Dual	Bridges	on l	U.S.401 over Neus	e River-Lav	w Projec	ct No.	3072	0-6-1	1415	GROUND	WATER (m)	
BORIN	IG NO.	NBLB:	3-B	ВС	RIN	G LOCATION 80+	58.5	OFFS	ET 21	.Om F	RT	ALIGNMENT -L-	O HR.	2.71	
COLLA	R ELEV	. 56.	70 m	NORT	HIN	G Not Provided	J	EAST	ING _	Not	Prov	/ided	24 HR.	3.08	
TOTAL	L DEPTI	H 23.1	8 m	DRILL	МА	CHINE Mobile B-56	3 DI	RILL ME	THOD	Mud	Rot	ary/Rock Core HAM	MER TYPE	140#Safety	
DATE	START	ED 5/.	22/96			COMPLETED 5/2	22/96	SURF	ACE W	/ATEF	R DEF	PTH N/A	_		
ELEV.	DEPTH	BLC	ow col	JNT		BLOWS PER	30 CM		SAMP.	$\mathbf{V}/$	L	SOIL AND ROC	K DESCRIPTIO	n.	
(m)	(m)	15cm	15cm	15cm	Ŷ	20 40	60 80	100	NO.	MOI		OOIE AND NOO			
56.70							_							00	
36.70	0.30					Ground Su	rtace				1	ALLUVIUM: BROWN		SILTY	
56-		3	1	2	• 3				SS-14	М		FINE SANDY CLAY (FRAGMENTS, GRAS			
	1.06	1	1	1	<u> </u>					M-W		•			
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	2.58	4		0					SS-15	NZ.	Æ	FINE SANDY SILT (A FRAGMENTS	-4) WITH WO	OD	
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-	-											•			
-	4.10											-			
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52-	-				-						000	ALLUVIUM: GRAVEL			
]	- 5.62										***	RESIDUAL: ORANGE		SILTY 5.4	
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48-	8.66	26	46	53						M-W		-			
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-											777	SOFT WEATHERED F	OCK (SWR) S	SAMPLED	
-	.10.18	55	45				100)/25cm			1/4	SAND. PARTIALLY V	VEATHERED C	RANITE.	
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-											77	45.50 RESIDUAL: ORANGE	, BROWN, WI		
-	11.70	29	40	45				:				BLACK SILTY FINE S WEATHERED GRANI			
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	13.22	100						00/1cm			1//	HARD WEATHERED		- NO 13.2	
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42-												 EXTREMELY FRACTUM MODERATELY HARD 	TO VERY HA	ARD .	
	Ė							· · · ·			縱	GRANITE, (SEE CORI	NG BORING F	REPORT)	
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	17.63							<u>.</u>	UC-3		>>>	-			
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PROJECT NO. 8.1402103 ID. R-2425C SITE DESCRIPTION Dual Bridges on U.S.401 over Neuse River-La									<u> </u>						WATER (m	
											ET 21		_	ALIGNMENT -L-	0 HR.	2.71
BORING NO. NBLB3-B BORING LOCATION 80+58.5 COLLAR ELEV. 56.70 m NORTHING Not Provided								EAST				<u> </u>	24 HR.	3.08		
_				_					DD					rided ary/Rock Core HAMI	MER TYPE	
	L DEPTI			DKILL	. WA		Mobile							PTH N/A		140#0010
	START	T						5/22/96		<u> </u>	SAMP.		L	FIII N/A		
LEV.	DEPTH		W COL	15cm	0	20	40	PER 30 CM 60	80	100			0	SOIL AND ROC	C DESCRIPTION	ON
(m)	(m)	15cm	15cm	15011	<u> </u>		<u>'ř</u>				140.	/MOI	G			
37.70						Contin	ued fror	n previou	ıs pag	е					ALL SILLY ALV	SOBAV
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-	E													MODERATELY WEAT EXTREMELY FRACTU		
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PROJECT NO .:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: NBLB3-B

BORING LOCATION (STA): 80 + 58.5 -L-

OFFSET: 21.0m RT

COLLAR ELEV.: 56.70m DATE STARTED: 05-22-96
TOTAL DEPTH: 23.18m DATE COMPLETED: 05-22-96
CORE SIZE: NQ TOTAL RUN: 9.95m

DRILL MACHINE: Mobile B-56

DRILL METHOD: Mud Rotary/Rock Core

DRILLER: S.Hancock

CORE SIZE:	DΝ		TOTAL RUI	V: 9.95m			DRILLER: S.Hancock
ELEV. (m)	DEPTH (m)	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
43.47 42.63	13.23	1:30	0.84	0.33 (39%)	0.68 (81%)	Run 1	Tan and pink, medium to coarse grained, slightly to moderately weathered, sound to moderately fractured, hard to moderately hard granite 8 Joints at 0-10 degrees ADS = 2 to 33cm
39.59	17.10	2:00 2:00 1:45 1:36 1:59 1:02 1:19 1:06 0.96	3.04	0.39 {13%}	0.80 (26%)	Run 2	Tan and pink, medium to coarse grained, slightly to moderately weathered, sound to moderately fractured, hard to moderately hard granite. 9 Joints at 0-10 degrees ADS = 2 to 38cm
36.55	20.14	1:00 1:15 1:45 1:30 1:26 2:43 2:51 3:02	3.04	1.18 (39%)	2.45 {81%}	Run 3	Tan, pink and gray, medium to coarse grained, moderately to very slightly weathered, sound to moderately fractured, hard to very hard granite. 21 Joints at 0-10 degrees ADS = 3 to 50cm 9 Joints at 45 degrees ADS = 5 to 33cm 1 Joint at 70 degrees

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL UNIT CORE BORING REPORT**

PROJECT NO.:

8.1402103 ID:

R-2425C COUNTY: Wake

GEOLOGIST: BK Banks

SITE DESCRIPTION: Dual Bridges on U.S.401 over Neuse River

LAW JOB NO: 30720-6-1415

BORING NO.: NBLB3-B BORING LOCATION (STA): 80 + 58.5 -L- OFFSET: 21.0m RT

COLLAR ELEV.: 56.70m DATE STARTED: 05-22-96
TOTAL DEPTH: 23.18m DATE COMPLETED: 05-22-96

DRILL MACHINE: Mobile B-56

DRILL METHOD: Mud Rotary/Rock Core

DDILLED: C Hancack TOTAL BUN: 9.95

CORE SIZE: NO	T	OTAL RUN	l: 9.95m			DRILLER: S.Hancock
ELEV. DEPTH	DRILL RATE (MN/30cm)	RUN (m)	RQD %	REC %	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
36.55 20.14	3:10 3:19 3:18 3:31 3:32 3:22 3:14 3:51 3:49 3:42	3.04	2.01 (66%)	2.81 (92%)	Run 4	Tan, pink and gray, medium to coarse grained, very slightly weathered to fresh, extremely fractured to sound, very hard granite. 6 Joints at 0-10 degrees ADS = 5 to 21cm 10 Joints at 45 degrees ADS = 3 to 74cm 3 Joints at 70 degrees 1 Joint at 90 degrees Joints show clay infilling and mineralization Boring terminated at 23.18m in granite

H:\472\DOT\NBLB3B.XLS

Rev. 4/96

SHEET 2 OF 2

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB3-B

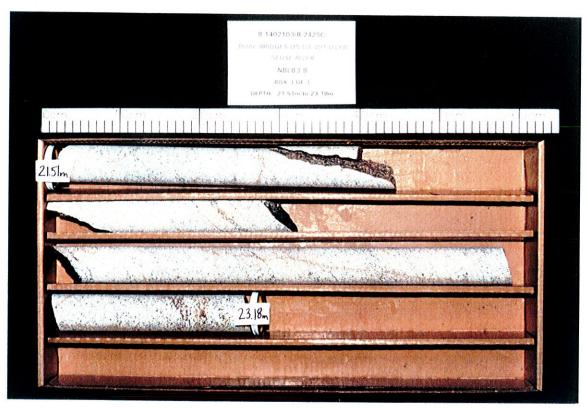


Box 1 of 3 13.23m to 18.26m



Box 2 of 3 18.26m to 21.51m

Dual Bridges on U.S. 401 Over Neuse River N.C.DOT Project No. 8.1402103 (R-2425C) NBLB3-B



Box 3 of 3 21.51m to 23.18m



LAW ENGINEERING, INC. 3301 Atlantic Avenue Raleigh, North Carolina 27619 Phone (919) 876-0416

SHEET 1 OF 1

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OTAL	. DEPTI	1 15.1	14 m	DRILL	MACH	IINE N	/lobile B	3-56	DR	ILL ME	THOD	Muc	Rot	ary		HAM	MER TYPE	140#Safe
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LAW ENGINEERING, INC. 3301 Atlantic Avenue Raleigh, North Carolina 27619 Phone (919) 876-0416

RO.IF	CT NO	<u>Ω</u> 2 1.	10210	3		ID. R-2	4250		CC	UNTV	/ Wak	9		SHEET 1 OF GEOLOGIST BK Banks	
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GEOTECHNICAL UNIT FIELD SCOUR REPORT

PROJECT: 8.1402103 ID: R-2425C COUNTY: Wake
L JCRIPTION (1): Dual Bridges on U.S. 401 over Neuse River
INFORMATION ON EXISTING BRIDGES Information obtained from X field inspection microfilm (Reel: Position:) other
COUNTY BRIDGE NO.131 BRIDGE LENGTH 107m NO.BENTS 8 NO.BENTS IN: CHANNEL 2 , FLOOD PLAIN 6
FOUNDATION TYPE: Spread footing on all bents
EVIDENCE OF SCOUR (2):
ABUTMENTS OR END BENT SLOPES: OK
INTERIOR BENTS: Wash out holes around interior bent piers
CHANNEL BED: None
CHANNEL BANKS: Western bank is nearly vertical indicating scour
EXISTING SCOUR PROTECTION:
TYPE (3): Rip-Rap along end bents
EXTENT (4): Wraps around end bents
EFFECTIVENESS (5): OK
OBSTRUCTIONS (6) (DAMS, DEBRIS, ETC.): Old concrete bridge piles exist a few meters upstream of the bridge
DESIGN INFORMATION
CHANNEL BED MATERIAL (7) (SAMPLE RESULTS ATTACHED): Silty fine to coarse sand (A-2-4), fine to coarse sand (A-3)
and fine to coarse sandy gravel (A-1-b)
CHANNEL BANK MATERIAL (8) (SAMPLE RESULTS ATTACHED): Silty fine sand (A-2-4), sandy clavey silt (A-4) and sandy
silty clay (A-6, A-7)
FOUNDATION BEARING MATERIAL (9): Hard rock (HR). Granite
CHANNEL BANK COVER (10): Grass, weeds; lightly vegetated to clear
F OD PLAIN WIDTH (11): 200 meter in immediate vicinity of bridge
FLOOD PLAIN COVER (12): Shrubs and trees exist on the southern floodplain. On the northern side of the river, the
bridge site is clear and open. Away, from the bridge on the northern floodplain, the area is lightly wooded.

DESIGN INFORMATION CONTINUED

STREAM IS X DEGRADING	AGGRADING (13)
LATER OBSERVATIONS AND COMMENTS: There is a	a dam approximately 10 miles upstream that, under normal
circumstances helps to control the flow of the river.	
CHANNEL MIGRATION TENDENCY (14): The migration	on tendency is southward in the immediate vicinity of the bridge.
CRITICAL SCOUR ELEVATIONS (15): 50.50 meters at	bove MSL
REPORTED BY: B.K. Banks	DATE: <u>May 23, 1996</u>

INSTRUCTIONS

- (1) GIVE THE DESCRIPTION OF THE SPECIFIC SITE GIVING ROUTE NUMBER AND BODY OF WATER CROSSED.
- (2) NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.).
- (3) NOTE ANY EXISTING SCOUR PROTECTION (RIPRAP, ETC.).
- (4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
- (5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
- (6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
- (7) DESCRIBE THE CHANNEL BED MATERIAL; A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- (8) DESCRIBE THE CHANNEL BANK MATERIAL; A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- (9) DESCRIBE THE FOUNDATION BEARING MATERIAL.
- (10) DESCRIBE THE BANK COVERING (GRASS, TREES, RIPRAP, NONE, ETC.).
- (11) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
- (12) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.).
- (13) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING.
- (14) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS).
- (15) GIVE THE CRITICAL SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE CRITICAL SCOUR ELEVATION. IF THE CRITICAL SCOUR ELEVATION IS DEPENDENT ON SCOUR COUNTER MEASURES, EXPLAIN. (RIPRAP ARMORING ON SLOPES, ETC.). THE CRITICAL SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY; CORE RECOVERY PERCENTAGE; PERCENT RQD; DIFFERENTIAL WEATHERING; SHEAR STRENGTH; OBSERVATIONS AT EXISTING STRUCTURES; OTHER TESTS DEEMED APPROPRIATE; AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

Site Photographs N.C.DOT Project 8.1402103(R-2425C)



PHOTO NO. 1 - View of SBLEB1-A, looking south along U.S.401

Dual Bridges on U.S.401 over Neuse River Law Project No. 30720-6-1415

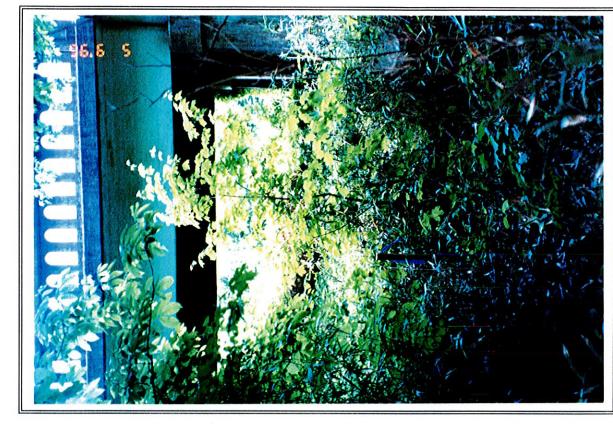


PHOTO NO. 2 - View of EB1-C, looking west along proposed End Bent One

Site Photographs N.C.DOT Project 8.1402103(R-2425C)

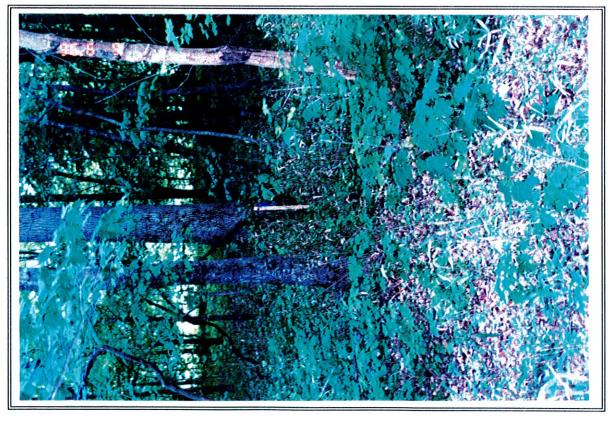


PHOTO NO. 3 - View of NBLEB1-B, looking east away from U.S.401

Dual Bridges on U.S.401 over Neuse River Law Project No. 30720-6-1415

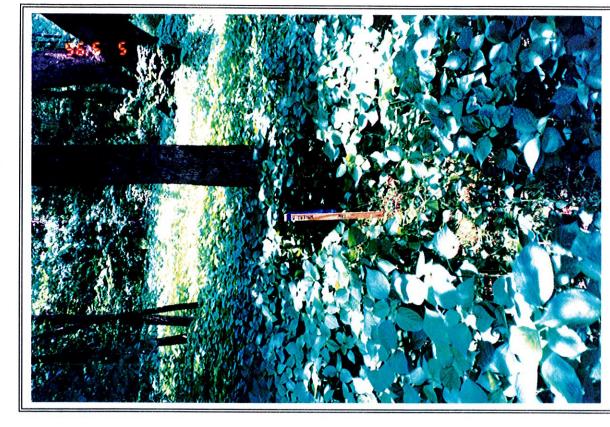


PHOTO NO. 4 - View of NBLEB2-B, looking east away from U.S.401

Site Photographs N.C.DOT Project 8.1402103(R-2425C)

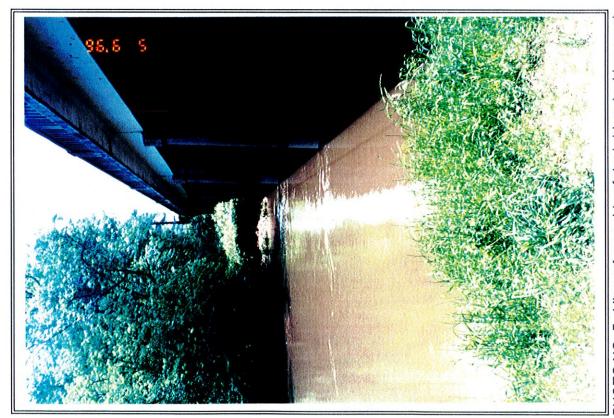


PHOTO NO. 5 - View of east side of the bridge, looking south showing locations of the water borings SBLB1-B, SBLB2-B, NBLB1-A, NBLB2-A, NBLB1-B, and NBLB2-B

Dual Bridges on U.S.401 over Neuse River Law Project No. 30720-6-1415

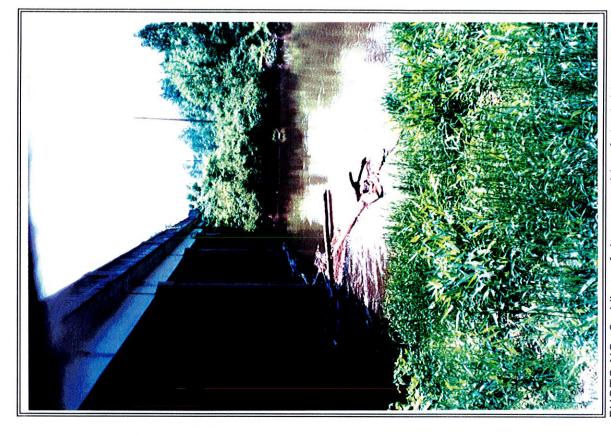


PHOTO NO. 6 - View of the west side of the bridge, looking south showing locations of the water borings SBLB1-A and SBLB2-A. The log jam shown in the picture has been removed to access Boring SBLB2-A

Site Photographs N.C.DOT Project 8.1402103(R-2425C)



PHOTO NO. 7 - View of NBLB3-A (Foreground), SBLB3-B (Centerground), looking west toward U.S.401

Dual Bridges on U.S.401 over Neuse River Law Project No. 30720-6-1415



PHOTO NO. 8 - View of NBLB3-B looking south toward the Neuse River

Site Photographs N.C.DOT Project 8.1402103(R-2425C)

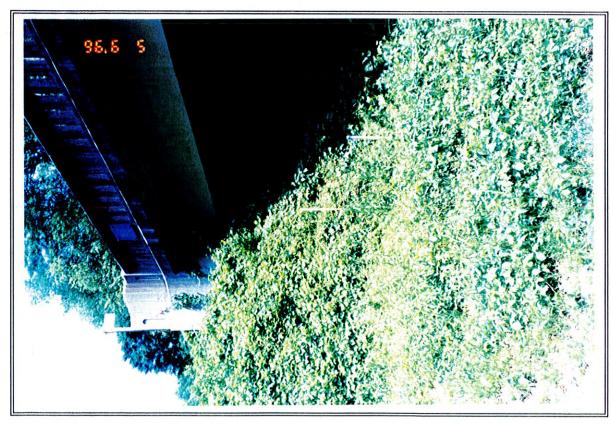


PHOTO NO. 9 - SBLEB2-A looking northeast toward End Bent Two. Stake at toe of slope indicates offset location of borehole

Dual Bridges on U.S.401 over Neuse River Law Project No. 30720-6-1415



PHOTO NO. 10 - View of EB2-C looking north toward End Bent Two

3301 ATLANTIC AVENUE RALEIGH, NC 27604

N.C.D.O.T./A.A.S.H.T.O. CLASSIFICATIONS

REPORT ON SAMPLES OF: SOILS FOR QUALITY

Law Project Name and Number: Neuse River Bridges 30720-6-1415

Project: Dual Bridges on US 401 over Neuse River

County: Wake

Owner: N.C.D.O.T.

Date Sampled: 05-15-96 to 05-23-96

Received: 05-28-96

Reported: 06-10-96

Sampled from: SBLEB1-A, NBLEB1-B, NBLB1-A

By: Banks, Trimble, Ballsieper

Submitted by: Law Engineering and Environmental Services, Inc.

1992 Standard Specifications

TEST RESULTS

Lab Sample No.		SS-1	SS-2	SS-3	SS-4	SS-5	SS-6
Retained 4.75 mm Sieve	%	0.0	0.0	40.1	1.5	14.2	0.0
Passing 2.00 mm Sieve	%	100.0	100.0	51.2	86.5	73.9	81.8
Passing 425 µm Sieve	%	99.7	99.5	25.1	26.5	38.3	34.4
Passing 75 µm Sieve	%	94.6	70.7	8.0	5.1	16.0	16.0

MINUS 2.00mm FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - 250 µm	%	0.7	1.9	82.4	84.2	71.4	73.6
Fine Sand Ret - 53 µm	%	7.9	27.5	11.3	11.6	14.1	12.3
Silt 0.05 - 0.005 mm	%	43.7	36.0	3.0	1.3	7.5	4.9
Clay < 0.005 mm	%	47.7	34.6	3.3	2.9	7.0	9.2

Moisture Content %	ND	ND	ND	ND	ND	ND
Liquid Limit, L.L.	55	35	21	25	28	28
Plasticity Index, P.I.	28	15	NP	NP	NP	NP
AASHTO Classification	A-7-6 (31)	A-6 (9)	A-1-b	A-1-b	A-1-b	A-1-b

						····	
Boring No.		SBLEB1-A	SBLEB1-A	NBLEB1-B	NBLB1-A	NBLB1-A	NBLB1-A
Station		79+85.0	79+85.0	79+85.0	80+13.0	80+13.0	80+13.0_
Offset	· · · · · · · · · · · · · · · · · · ·	18.8m LT	18.8m LT	22.5m RT	4.0m RT	4.0m RT	4.0m RT
Alignment		-L-	-L-	-L-	-L-	-L-	-L
Depth (m)	From	2.62	4.14	5.55	0.00	2.56	4.08
	to	3.08	4.60	6.01	0.46	3.02	4.54

ND=Not Determined.

Reviewed by:

3301 ATLANTIC AVENUE RALEIGH, NC 27604

N.C.D.O.T./A.A.S.H.T.O. CLASSIFICATIONS

REPORT ON SAMPLES OF: SOILS FOR QUALITY

Law Project Name and Number: Neuse River Bridges 30720-6-1415

Project: Dual Bridges on US 401 over Neuse River

County: Wake

Owner: N.C.D.O.T.

Date Sampled: 05-15-96 to 05-23-96

Received: 05-28-96

Reported: 06-10-96

Sampled from: SBLB2-A, NBLB2-A, SBLB3-A, SBLB3-B

By: Banks, Trimble, Ballsieper

Submitted by: Law Engineering and Environmental Services, Inc.

1992 Standard Specifications

TEST RESULTS

Lab Sample No.		SS-7	SS-8	SS-9	SS-10	SS-11	SS-12
Retained 4.75 mm Sieve	%	0.0	9.2	0.0	0.0	0.0	0.0
Passing 2.00 mm Sieve	%	92.5	65.9	99.7	85.0	99.4	99.8
Passing 425 µm Sieve	%	52.7	31.7	99.4	42.6	97.3	97.3
Passing 75 µm Sieve	%	18.8	6.8	75.3	17.3	45.9	66.3

MINUS 2.00mm FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - 250 µm	%	60.8	85.9	1.1	67.3	8.2	6.3
Fine Sand Ret - 53 µm	%	23.1	7.8	30.7	17.5	51.6	33.1
Silt 0.05 - 0.005 mm	%	10.0	0.0	30.4	8.3	14.1	23.7
Clay < 0.005 mm	%	6.1	6.3	37.8	6.9	26.1	36.9

Moisture Content	%	ND	ND	ND	ND	ND	ND
Liquid Limit, L.L.		31	22	37	31	30	35
Plasticity Index, P.I.		NP	NP	12	NP	7	12
AASHTO Classification		A-2-4	A-1-b	A-6 (9)	A-1-b	A-4 (1)	A-6(6)

Boring No.		SBLB2-A	NBLB2-A	SBLB3-A	SBLB3-A	SBLB3-B	SBLB3-B
Station		80+41.0	80+41.0	80+58.5	80+58.5	80+58.5	80+58.5
Offset		17.7m LT	4.0m RT	17.7m LT	17.7m LT	4.0m LT	4.0m LT
Alignment		-L-	-L-	-L-	-L-	-L-	-L-
Depth (m)	From	2.74	1.77	2.64	5.66	0.30	4.02
	to	3.20	2.23	3.09	6.12	0.76	4.48

ND=Not Determined.

Reviewed by:

3301 ATLANTIC AVENUE RALEIGH, NC 27604

N.C.D.O.T./A.A.S.H.T.O. CLASSIFICATIONS

REPORT ON SAMPLES OF: SOILS FOR QUALITY

Law Project Name and Number: Neuse River Bridges 30720-6-1415

Project: Dual Bridges on US 401 over Neuse River

County: Wake

Owner: N.C.D.O.T.

Date Sampled: 05-15-96 to 05-23-96

Received: 05-28-96

Reported: 06-10-96

Sampled from: SBLB3-B, NBLB3-B, EB2-C

By: Banks, Trimble, Ballsieper

Submitted by: Law Engineering and Environmental Services, Inc.

1992 Standard Specifications

TEST RESULTS

Lab Sample No.		SS-13	SS-14	SS-15	SS-16	
Retained 4.75 mm Sieve	%	0.0	0.0	0.0	0.0	
Passing 2.00 mm Sieve	%	85.3	99.3	100.0	100.0	
Passing 425 μm Sieve	%	36.4	98.6	99.1	99.7	
Passing 75 µm Sieve	%	16.4	67.8	60.3	36.7	

MINUS 2.00mm FRACTION

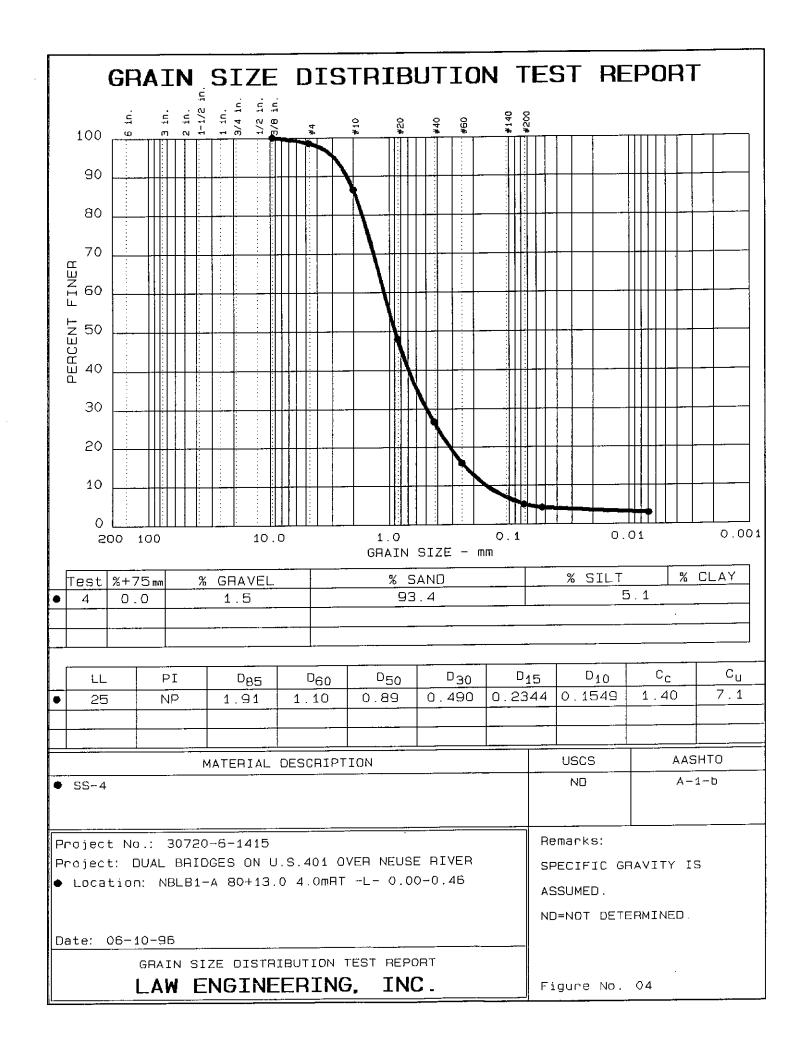
SOIL MORTAR - 100%						
Coarse Sand Ret - 250 µm	%	72.4	3.0	4.8	4.5	
Fine Sand Ret - 53 μm	%	12.5	34.1	39.6	65.2	
Silt 0.05 - 0.005 mm	%	6.0	29.0	26.5	16.6	
Clay < 0.005 mm	%	9.1	33.9	29.1	13.7	

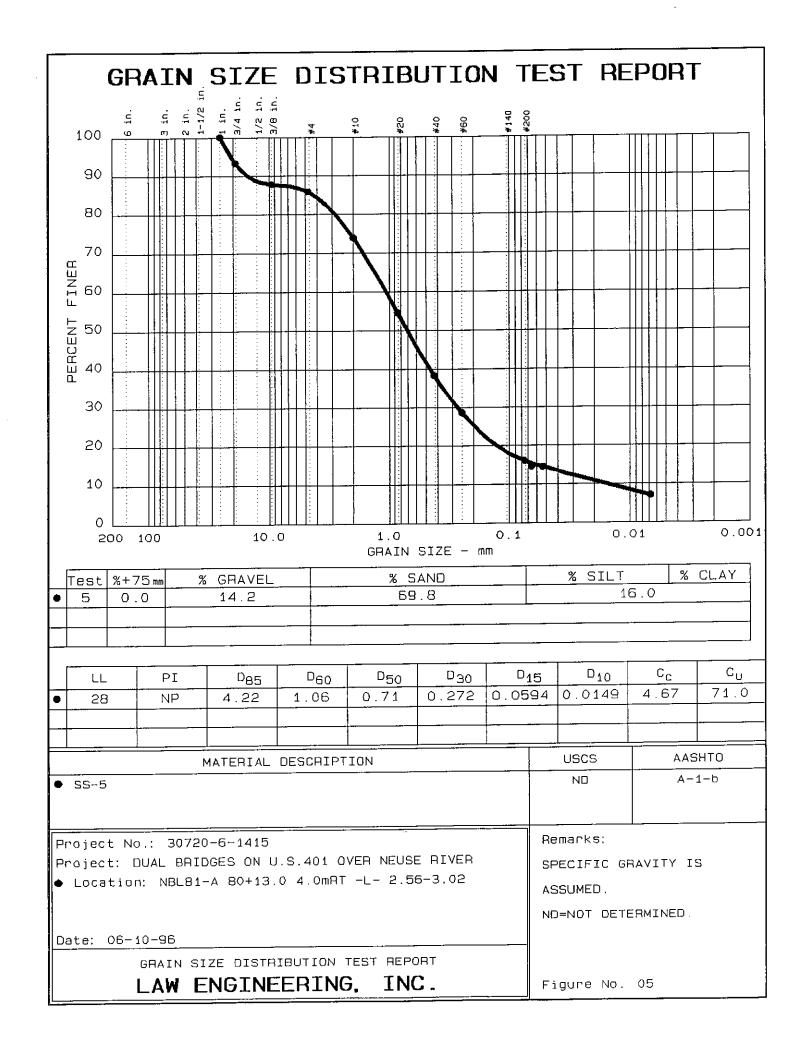
Moisture Content	%	ND	ND	ND	ND	 <u> </u>
Liquid Limit, L.L.		32	38	32	23	
Plasticity Index, P.I.	·	9	14	8	NP	
AASHTO Classification		A-2-4	A-6 (8)	A-4 (3)	A-4	

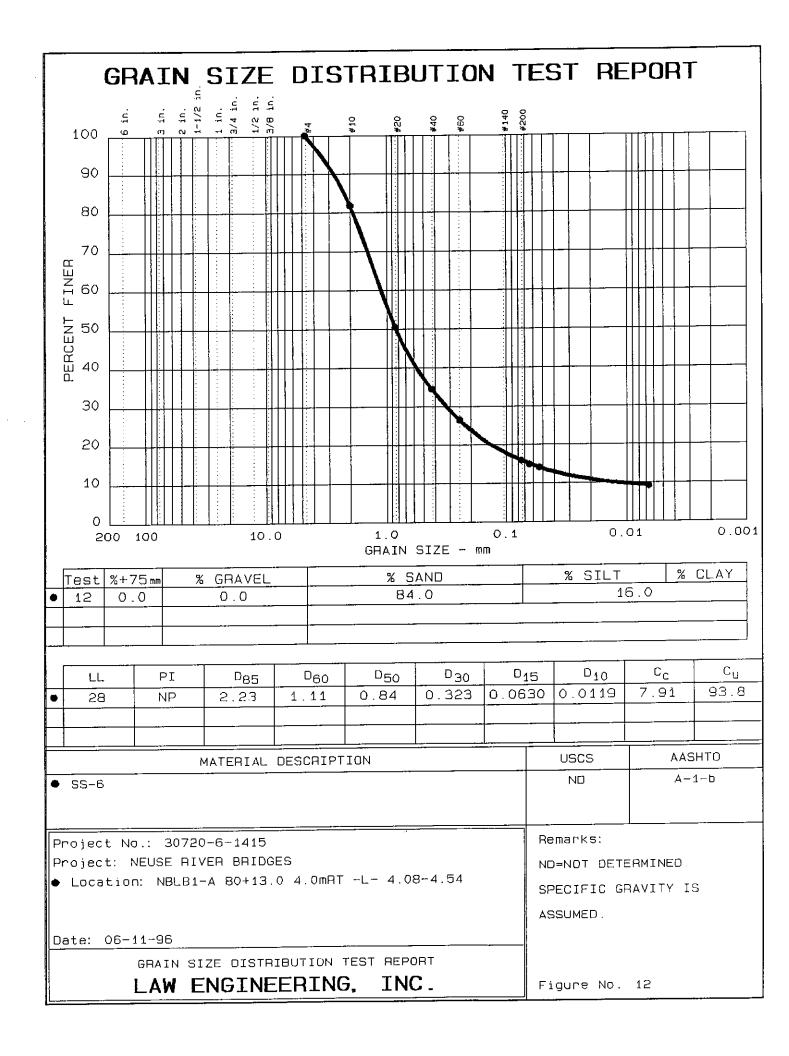
Boring No.		SBLB3-B	NBLB3-B	NBLB3-B	EB2-C	
Station	···	80+58.5	80+58.5	80+58.5	80+76.0	
Offset		4.0m LT	17.7mRT	17.7mRT	Centerline	
Alignment		-L-	-L-	-L-	-L-	
Depth (m)	From	7.11	0.30	2.58	1.42	
, , ,	to	7.57	0.76	3.04	1.88	

ND=Not Determined.

BK Fanhs Reviewed by:







SUMMARY OF LABORATORY TESTING

DUAL BRIDGES ON U.S. 401 OVER NEUSE RIVER LAW PROJECT NO. 30720-6-1415

ROCK CORE SPECIMENS									
SAMPLE ID	DEPTH (m)	ULTIMATE COMPRESSIVE STRENGTH (kPa)	YOUNG'S MODULUS (kPa X 10 ⁶)	CROSS SECTIONAL AREA (cm²)	POISSON'S RATIO				
UC-1 (SBLB1-B)	8.06-8.22	53,990	19.7	17.35	0.414				
UC-2 (SBLB1-B)	10.37-10.51	144,870	35.4	17.35	0.321				
UC-3 (NBLB3-B)	17.63-17.77	81,710	3.3	17.35	0.320				