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DRAWN BY : N. D'AIUTO CHECKED BY : J.T. KELVINGTON DATE : 05/01/18 DESIGN DATE : 12/30/22 OF RECORD: J.T. KELVINGTON DATE : 04/21/23

Stantec

ZERO CAMBER BASE LINE —	€ BEARING ← @ END BENT 2
0       11       12       13       14       15       16       17       18       19       20       21       22       23       24       25       26       27       28       29       30       31       32       33       34       3         50       0.275       0.300       0.325       0.350       0.375       0.400       0.425       0.450       0.475       0.500       0.525       0.550       0.575       0.600       0.625       0.650       0.675       0.700       0.725       0.775       0.800       0.825       0.850       0.400         92       0.206       0.219       0.230       0.241       0.249       0.256       0.262       0.268       0.269       0.268       0.266       0.241       0.230       0.219       0.206       0.192       0.176       0.160       0.142       0.124       0.124       0.125       0.161       0.163       -0.163       -0.161       -0.155       -0.151       -0.146       -0.140       -0.132       -0.125       -0.116       -0.107       -0.096       -0.086       -0.075       0.140       -0.132       -0.125       -0.116       -0.163       -0.163       -0.163       -0.163       -0.155	5  36  37  38  39  40 $5  36  37  38  39  40$ $375  0.900  0.925  0.950  0.975  1.000$ $04  0.085  0.064  0.043  0.022  0.000$ $063 - 0.049 - 0.039 - 0.026 - 0.013  0.000$
$\frac{1}{16^{n}} + 1^{n}}{11^{n}} + \frac{1}{16^{n}} + $	<sup>1</sup> /2" +0 <sup>7</sup> / <sub>16</sub> " +0 <sup>5</sup> / <sub>16</sub> " +0 <sup>3</sup> / <sub>16</sub> " +0 <sup>1</sup> / <sub>8</sub> " +0" € BEARING © END BENT 2
FINAL CAMBER FINAL CAMBER FI	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
1/4"       +0 <sup>13</sup> / <sub>16</sub> "       +0 <sup>15</sup> / <sub>16</sub> "       +0 <sup>15</sup> / <sub>16</sub> "       +1"       +1"       +1 <sup>1</sup> / <sub>16</sub> "       +1 <sup>1</sup> / <sub>16</sub>	$\frac{1}{16''} + \frac{1}{3} + \frac{1}{4''} + \frac{1}{4} + \frac{1}{16''} + \frac{1}{16''} + \frac{1}{16''} + \frac{1}{16''} + \frac{1}{16''}$
SCHEMATIC CAMBER ORDINATES (SPAN A) All values are shown in decimals of a foot except final camber (or deflection) which is shown in inches. (+) final camber indicates net upward displacement. (-) final camber indicates net downward displacement.	PROJECT NO. <u>R-2707D</u> <u>CLEVELAND</u> COUNTY STATION: <u>810+00.00 -L-</u>
DOCUMENT NOT CONSIDERE FINAL UNLESS ALL SIGNATURES COMPLETED	STATE OF NORTH CAROLINA         DEPARTMENT OF TRANSPORTATION         RALEIGH         SUPERSTRUCTURE         DEAD LOAD         DEFLECTIONS         COLSPANS         ONO. BY: DATE: NO. BY: DATE: NO. BY: DATE: SOCIETAL         ONO. BY: DATE: NO. BY: DATE: SOCIETAL







THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE  $1 \frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE  $\frac{3}{4}$ " Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.



SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>R-2707D</u> <u>CLEVELAND</u> COUNTY 810+00.00 -L-STATION:\_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RAI FTGH STANDARD GUARDRAIL ANCHORAGE SEAL 13406 FOR BARRIER RAIL (LL) Joseph T. Kelvington SHEET NO. REVISIONS 4/21/2023780774AF. S8-18 NO. BY: DATE: DATE: BY: DOCUMENT NOT CONSIDERED total sheets 30 FINAL UNLESS ALL SIGNATURES COMPLETED



	RE BA	INF R S(	ORC Chei	ING Dule
BAR	NO.	SIZE	TYPE	LENGTH
<b>*</b> A1	217	#5	STR	40′-9″
Α2	217	#5	STR	40'-9"
<b>₩</b> B1	58	#5	STR	60'-0"
B2	82	#5	STR	59′-10″
<b>₩</b> B3	54	#6	STR	22'-11"
<b>₩</b> B4	54	#6	STR	25′-9″
<b>米</b> G1	2	#5	STR	40′-9″
K1	28	#4	STR	21'-3"
К2	6	#4	STR	7'-0"
K3	30	#4	STR	9'-10"
Κ4	6	#4	STR	8'-6″
К5	4	#4	3	5′-1″
К6	20	#4	3	6'-6″
К7	4	#4	3	5'-10"
K8	12	#4	STR	6′-8″
<b>*</b> S1	60	#4	2	10'-3"
<b>*</b> S2	80	#4	2	11'-8″
U1	60	#4	1	14'-8"
U2	16	#4	1	10'-10"
REIN STEEL	FORC:	ING		15,87

	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL					
	(CU.YDS.)	(LBS.)	(LBS_)					
POUR #1	140.2							
POUR #2	81.2							
TOTALS**	221.3	15,870	17,920					

\*\*QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE										
FOLLOWING MINIMUM SPLICE LENGTHS										
BAR SIZE	SUPERSTF EXCEPT A SLABS, P AND BARR	RUCTURE APPROACH ARAPET, IER RAIL	APPROAC	PARAPET AND BARRIER						
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL					
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"					
#5	2'-5"	2'-0"	2'-5"	2'-0"	3'-1"					
#6	2'-10"	2'-5"	3'-7"	2'-5"	3'-8"					
#7	4'-2"	2'-9"								
#8	4'-9"	3'-2"								



GROOVING	BRIDGE	FL	OORS
APPROACH SLABS	S	1692	SQ.FT.
BRIDGE DECK		4116	SQ.FT.
TOTAL		5808	SQ.FT.

		CLEV	<u>/El</u>	<u>_AND</u>	CO	UNTY		
	STATI	DN:	81	0+00	).00 -l			
	DEPA	DEPARTMENT OF TRANSPORTA						
TH CARO								
Pression Pression	SUPERSTRUCTURE							
SEAL 13406	BTIL OF MATERTAL							
MG INE K								
KELVINIII								
Joseph T. telvington								
6/22/2023		REVISIONS						
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	N0.	BY:	DATE:	28-12		
FINAL UNLESS ALL	1		3			SHEETS		
SIGNATURES CUMPLETED	2		43			30		

PROJECT NO. <u>R-2707D</u>

![](_page_4_Figure_1.jpeg)

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![](_page_5_Figure_1.jpeg)

Boolsigned by: Noseph t. Kelwin aton					(LL)		
4/ <del>21</del> /2 <b>9</b> 2 <b>3</b> 780774AF			REVI	ISION	S		SHEET NO.
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	58-21
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	ст	D #8					

![](_page_6_Figure_1.jpeg)

![](_page_7_Figure_1.jpeg)

![](_page_7_Figure_2.jpeg)

		BI	ELL	OF N	1ATERI	AL
°ES			ΕN	ID BE	ent 1	
	BAR	N0.	SIZE	TYPE	LENGTH	WEIGHT
	B1	6	#10	1	50′-7″	1306
	B2	2	#10	STR	47'-3"	407
$ +$ $( \times )$	B3	8	#4	STR	24'-10"	133
	B4	6	#6	STR	47'-9"	430
	B5	6	#9	1	50'-3"	1025
, M	B6	19	#4	STR	3'-4"	42
		6	#4	SIR	30'-5"	122
3'_1"	B8	b C	#4	SIR	19 <sup>°</sup> -5 <sup>°</sup>	18
		Ø	<sup></sup> 4	SIR	C- 0	54
	H1	15	#7	6	20'-3"	621
I-J LAP	H2	11	#7	6	18'-0"	405
	Н3	13	#5	6	20'-3"	275
	H4	13	#5	6	18'-0"	244
	Н5	8	#7	6	17'-11"	293
	Н6	6	#7	6	15'-8″	192
	Н7	8	#7	6	9′-2″	150
	Н8	6	#7	6	9'-2"	112
	Н9	2	#7	6	19'-8"	80
	H10	2	#7	6	17'-5"	71
	K 1	0	# 1	СТР	21-9"	15
	K1 K2	28	#4	STR	5'-10"	109
	S1	64	#5	2	11′-6″	768
	S2	56	#5	3	4'-3"	248
	<u>S3</u>	32	#4	4	7'-7"	162
		30	#⊿	5	6'-11"	139
	U2	15	#4	5	4'-0"	40
		7 4		C T D	7/ 5/	<b>F</b> 70
		74	#5 #5	SIR	11/-10"	572
_		20	#5	SIR	11'-10	3/7
		11	#5	STR	11 <sup>-</sup> 1 11′-9″	135
	V <del>1</del>	11	#5	STR	11'-0"	126
RE OUT TO OUT.						
	REIN	FORC	ING S <sup>-</sup>	TEEL		LBS. 9,002
	REIN	FORC:	ING S <sup>-</sup>	TEEL	EAKDOWN:	LBS. 9,002
	REIN CLAS POUR	FORC SA #1: CA	ING S <sup>-</sup> Concre	TEEL Ete br Lars, 8	EAKDOWN:	LBS. 9,002 C.Y. 36.3
	REIN CLAS POUR POUR	FORC S A ( #1: C4 #2: UI	ING S <sup>-</sup> Concre Ap,coli PPER V	TEEL Ete br Lars, & Wings	EAKDOWN: WINGS	LBS. 9,002 C.Y. 36.3 C.Y. 18.8

- STATION: 810+00.00 -L-
- SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE

![](_page_7_Picture_7.jpeg)

MGINER CONTRACTOR					(LL)		
4/21/2003780774AF			REVI	SIO	NS		SHEET NO.
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	N0.	BY:	DATE:	S8-23
FINAL UNLESS ALL	1			ଭ			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	SТ	R #8					

SEAL 13406

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![](_page_8_Figure_1.jpeg)

![](_page_9_Figure_1.jpeg)

Joseph T. Kelminaton					(LL)		
4/21/2023780774AF		REVISIONS					
DOCUMENT NOT CONSTDERED	N0.	BY:	DATE:	NO.	BY:	DATE:	S8-2
FINAL UNLESS ALL	1			ଞ			TOTA SHEE
SIGNATURES COMPLETED	2			4			30
	ST	R. #8					

![](_page_10_Figure_1.jpeg)

![](_page_11_Figure_1.jpeg)

![](_page_11_Figure_2.jpeg)

### BILL OF MATERIAL

D	F	$\subset$
	L	J

DJ	
B1	

![](_page_11_Figure_6.jpeg)

![](_page_11_Figure_7.jpeg)

![](_page_11_Figure_8.jpeg)

DOCUMENT NOT

FND RENT 2										
			U DE		WETCHT					
	NU.	512E		LENGIH	WEIGHT					
		#10	I STD	50 - 1 17'- 3"	1306					
	2	#10	STR	2/1/-10"	133					
	6	#6	STR	24 IU 17'-9"	133					
B5	6	#9	1	50'-3"	1025					
B6	19	#4	STR	3'-4"	42					
B7	6	#4	STR	30'-5"	122					
B8	6	#4	STR	19'-5"	78					
B9	6	#4	STR	8'-5"	34					
H1	17	#7	6	20'-9"	721					
H2	15	#7	6	18′-6″	567					
H3	13	#5	6	20'-9"	281					
H4	13	#5	6	18'-6"	251					
H5	9	#7	6	18′-5″	339					
H6	9	#5	6	16'-2"	152					
H7	9	#7	6	9'-2"	169					
H8	9	#7	6	9′-2″	169					
Н9	2	#7	6	20'-2"	82					
H10	2	#7	6	17'-11"	73					
K1	8	#4	STR	2'-9"	15					
K2	28	#4	STR	5′-10″	109					
S1	64	#5	2	11'-6″	768					
S2	56	#5	3	4'-3"	248					
<u>S3</u>	32	#4	4	7'-7"	162					
	30	#4	5	6'-11"	142					
02	18	#4	5	4'-0"	142					
1/1		+-			<b>F 7 0</b>					
		1 #5 #r		( <sup>*</sup> -5 <sup>*</sup>	512					
	120	C"		11'-10"	JZ1 40C					
	4 <u>/</u>   11	C " ] #۲		11'-0"	400					
		". #F		11'-9	100					
				11 -0	120					
	L RF	L TNFOR(	L CING S		BS, 9.510					
	SA	CONCR	ETE BR	EAKDOWN						
POUR	(#1: C.4		LARS 8	WINGS	C.Y. 32.6					
POUR	(#2: U	PPER V	VINGS		C.Y. 21.1					
CLAS	S A	CONCRE	ETE TC	TAL	C.Y. 58.7					
				_						

	PROJEC	CT NO.	<u> </u>	707D	
		CLEV	<u>'ELAND</u>	CO	UNTY
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	<u>Sheet 4 C</u>	F 4			
	DEPA	STAT RTMENT	e of north car OF TRAI raleigh	OLINA NSPORTA	TION
SEAL 13406 KELV	EN[	D BEN	NT 2	DETA	ILS
Docusigned by: Asept. t. Kelsuin aton.			(LL)		
4/21/2023 <sup>780774AF</sup>		REVIS	SIONS		SHEET NO.
OCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY: ନ୍ଥ	DATE:	TOTAL
SIGNATURES COMPLETED	2		<u>୭</u>		sheets 30

![](_page_12_Figure_1.jpeg)

### GENERAL NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. THE CONTRACTOR, AT HIS OPTION, MAY USE ALTERNATE ``B'' ONLY FOR HIGHWAY OVER HIGHWAY GRADE SEPARATIONS WITH 2:1 END BENT SLOPE IN RURAL, UNPOPULATED AREAS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH.SEE GENERAL DRAWING.

## ALTERNATE ``A''

ALTERNATE ``A'' SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.810+00.00 -L-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE		
	SQUARE YARDS	APPROX.L.F.		
END BENT 1	259	467		
END BENT 2	275	496		

\* QUANTITY SHOWN IS BASED ON 5' POURS.

SEAL

13406

S, MGINE

5'-0″

![](_page_12_Figure_9.jpeg)

OPTIONAL POURING DETAIL

POURING DETAIL

5′-0″ I

PROJECT NO. <u>R-2707D</u> <u>CLEVELAND</u> COUNTY 810+00.00 -L-STATION:\_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

# SLOPE PROTECTION DETAILS

Boseph t. Kelwin atom					(LL)		
4/21/20923780774AF			REVI	SION	IS		SHEET NO.
DOCUMENT NOT CONSTDERED	N0.	BY:	DATE:	N0.	BY:	DATE:	S8-28
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	стг	) #0					

![](_page_13_Figure_1.jpeg)

# NOTES

### APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE,6"Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 6"Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

![](_page_13_Figure_13.jpeg)

![](_page_13_Figure_15.jpeg)

BILL OF MATERIAL									
FOR ONE APPROACH SLAB (2 REQ'D)									
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
<b>₩</b> A1	52	#4	STR	20'-6"	712				
Α2	52	#4	STR	20'-5"	709				
<b>米</b> B1	78	#5	STR	24'-2"	1966				
B2	78	#6	STR	24'-8"	2890				
REINFO	ORCING	STEE	L	LBS.	3599				
* EPOXY COATED REINFORCING STEEL LBS. 2,678									
CLASS	AA CC	NCRET	E	C.Y.	42.5				

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

![](_page_13_Picture_18.jpeg)

![](_page_13_Figure_21.jpeg)

PROJECT N	0. <u>R-270</u>	)7D
CL	EVELAND	COUNT
STATION:_	810+00.0	)0 -L-
SHEET 1 OF 2		

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT WITH FLEXIBLE PAVEMENT

Joseph T. Kelvington					(LL)		
4/21/2993780774AF			REVI	SION	IS		SHEET NO.
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S8-29
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	STR	# <u>Q</u>					

SEAL

13406

![](_page_14_Figure_1.jpeg)

![](_page_15_Figure_1.jpeg)

![](_page_16_Figure_1.jpeg)

		CLEV	<u>'ELAND</u>	CO	UNTY		
	STATI	DN: <u>11+</u>	80.00	-SRVR	D_5-		
	SHEET 2 A	FЗ					
	SHEET 2 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION						
HOR FESSION	FO	UNDA <sup>-</sup>	TION	LAYO	UT		
SEAL 13406	FO	R BRIDG OVER -S SR 2047	E ON US RVRD_5- (BORDERS	74 BYPA BETWEEN RD.) AND	SS )		
Joseph T. Kelvington	US	74 BUSI	NESS (E. (RL)	MARION :	ST_)		
4/21842023780774AF		REVIS	SIONS		SHEET NO.		
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	59-02		
FINAL UNLESS ALL SIGNATURES COMPLETED	1		3 4		total sheets 30		
	STR.#9						

PROJECT NO. R-2707D

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		SL	JMMAR	Y OF (blank entri	PILE es indicate	T ITEN
END BENT/ BENT NO. PILE (s) #-# (e.g., "BENT 1, PILES 1-5")	FACTORED RESISTANCE PER PILE TONS	PILE CUT-OFF (TOP OF PILE) ELEVATION FT	ESTIMATED PILE LENGTH PER PILE FT	SCOUR CRITICAL ELEVATION FT	MIN.PILE TIP (TIP NO HIGHER THAN)ELEV FT	R [ RE (RDR)
END BENT 1, PILES 1-4	120	725.53	20			
END BENT 1, PILES 5-8	120	725.53	20			
END BENT 2, PILES 1-4	140	725.49	20			
END BENT 2, PILES 5-8	140	725.49	15			
* PREDRILLING FOR THE CONTRACTOR'S * * RDR= <u>FACTORED</u>	PILES IS RE OPTION FOR E RESISTANCE +	QUIRED FOR EN ND BENTS/ BEN FACTORED DOW	D BENTS/ BEN <sup>-</sup> TS WITH PREDI INDRAY LOAD +	T WITH A PRE RILLING INFO FACTORED D	EDRILLING LE DRMATION BU EAD LOAD +	ENGTH T NO NORM
	[	DYNAMIC RESIS	TANCE FACTOR			
SUMMARY (BLANK	OF F	DA/P	S NOT APPLICA	RDER		
PILE [	DRIVING ANAL	YZER (PDA)	τοται	PILE O	RDER LENGTH	S
END BENT/ TES BENT NO. YE MA	TING IRED? 5 OR YBE	PDA TEST PILE LENGTH FT	PDA TESTING QUANTITY EACH	END BENT/ BENT NO(s)	PILE O LENG BASI EST OR	RDER TH S <del>*</del> PDA
				I		
* EST=PILE ORDER L PDA TESTING.FOR G TESTING,THE FIRST BENT WITH THE PDA	ENGTHS FROM ROUPS OF END END BENT/ B	ESTIMATED PIL BENTS/BENTS ENT NO.LISTED	LE LENGTHS; PD WITH PILE ORE FOR EACH GRO	A=PILE ORDEF DER LENGTHS DUP IS THE R	R LENGTHS BA BASED ON PE EPRESENTATI	ASED )A EVE EI
* EST=PILE ORDER L PDA TESTING.FOR G TESTING,THE FIRST BENT WITH THE PDA	ENGTHS FROM ROUPS OF END END BENT/ B	ESTIMATED PIL BENTS/BENTS ENT NO.LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN	A=PILE ORDEP DER LENGTHS DUP IS THE R INFOR OT APPLICABI	R LENGTHS BA BASED ON PE EPRESENTATI	ASED DA EVE EI ON TURE)
* EST=PILE ORDER L PDA TESTING.FOR G TESTING, THE FIRST BENT WITH THE PDA END BENT/ BENT NO. PILE (s) *-* (e.g., "BENT 1, PILES 1-5")	ENGTHS FROM ROUPS OF END END BENT/ B (BLANK E FACTORED AXIAL LOAD PER PILE TONS	ESTIMATED PIL BENTS/BENTS ENT NO.LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN TE ITEM IS NO FACTORED DEAD LOAD* PER PILE TONS	A=PILE ORDEF DER LENGTHS DUP IS THE R THE R OT APPLICABI DYNAMIC RESISTANCE FACTOR	R LENGTHS BA BASED ON PE EPRESENTATI	ASED A EVE EI ON TURE) E
* EST=PILE ORDER L PDA TESTING.FOR G TESTING, THE FIRST BENT WITH THE PDA END BENT/ BENT NO. PILE (s) *-* (e.g., "BENT 1, PILES 1-5") END BENT 1, PILES 1-4	ENGTHS FROM ROUPS OF END END BENT/ B (BLANK E FACTORED AXIAL LOAD PER PILE TONS	ESTIMATED PIL BENTS/BENTS ENT NO.LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN TE ITEM IS NO FACTORED DEAD LOAD* PER PILE TONS	A=PILE ORDEF DER LENGTHS DUP IS THE R TAPPLICABI DYNAMIC RESISTANCE FACTOR 0.60	R LENGTHS BA BASED ON PE EPRESENTATI	ASED A EVE EI ON TURE) E
* EST=PILE ORDER L PDA TESTING.FOR G TESTING, THE FIRST BENT WITH THE PDA END BENT/ BENT NO. PILE (s) #-* (e.g., "BENT 1, PILES 1-5") END BENT 1, PILES 1-4 END BENT 1, PILES 5-8	ENGTHS FROM ROUPS OF END END BENT/ B BLANK E FACTORED AXIAL LOAD PER PILE TONS 120 120	ESTIMATED PIL BENTS/BENTS ENT NO. LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN TE ITEM IS NO FACTORED DEAD LOAD* PER PILE TONS	A=PILE ORDEP DER LENGTHS DUP IS THE R THE R DYNAMIC RESISTANCE FACTOR 0.60 0.60	R LENGTHS BA BASED ON PE EPRESENTATI	ASED A EVE EI TURE) E
* EST=PILE ORDER L PDA TESTING.FOR G TESTING, THE FIRST BENT WITH THE PDA END BENT/ BENT NO. PILE (s) *-* (e.g., "BENT 1, PILES 1-5") END BENT 1, PILES 1-4 END BENT 1, PILES 5-8 END BENT 2, PILES 1-4	ENGTHS FROM ROUPS OF END END BENT/ B BLANK E FACTORED AXIAL LOAD PER PILE TONS 120 120 140	ESTIMATED PIL BENTS/BENTS ENT NO. LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN TE ITEM IS NO FACTORED DEAD LOAD* PER PILE TONS	A=PILE ORDEF DER LENGTHS DUP IS THE R TAPPLICABI DYNAMIC RESISTANCE FACTOR 0.60 0.60 0.60	R LENGTHS BA BASED ON PE PRESENTATI	ASED ASED VE EI TURE) E S
* EST=PILE ORDER L PDA TESTING.FOR G TESTING, THE FIRST BENT WITH THE PDA END BENT/ BENT NO. PILE (s) *-* (e.g., "BENT 1, PILES 1-5") END BENT 1.PILES 1-4 END BENT 1.PILES 5-8 END BENT 2.PILES 1-4 END BENT 2.PILES 5-8	ENGTHS FROM ROUPS OF END END BENT/ B BLANK E FACTORED AXIAL LOAD PER PILE TONS 120 120 140 140	ESTIMATED PIL BENTS/BENTS ENT NO. LISTED	E LENGTHS; PD WITH PILE ORE FOR EACH GRO SIGN TE ITEM IS NO FACTORED DEAD LOAD* PER PILE TONS	A=PILE ORDEP DER LENGTHS DUP IS THE R DUP IS THE R DYNAMIC RESISTANCE FACTOR 0.60 0.60 0.60 0.60	R LENGTHS BA BASED ON PE EPRESENTATI	ASED ASED VE EI TURE) E S

# ENFORMATION/ INSTALLATION

ЕΜ	IS	NOT	APPLICABLE	ТΟ	STRUCTURE)	

LM IS NOT AT	ILICADEL I	5 STRUCTURE					
REQUIRED DRIVING RESISTANCE R) <del>%%</del> PER PILE TONS	TOTAL PILE REDRIVES QUANTITY EACH	PREDRILLING LENGTH PER PILE LIN FT	PREDRILLING ELEVATION (ELEV NOT TO PREDRILL BELOW) FT	MAXIMUM PREDRILLING DIA INCHES	PILE EXCAVATION (BOTTOM OF HOLE) ELEV FT	PILE EXC NOT IN SOIL PER PILE LIN FT	PILE EXC IN SOIL PER PILE LIN FT
200					706.5	8.1	7.9
200					707.5	8.2	6.8
235					708.5	13.3	5.7
235					709.5	10.0	4.0

H AND AT D PREDRILLING LENGTH.

RMAL DOWNDRAG RESISTANCE +

NORMAL SCOUR RESISTANCE SCOUR RESISTANCE FACTOR

IS

FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

ON END BENT/

N E)	
NOMINAL SCOUR RESISTANCE PER PILE TONS	SCOUR RESISTANCE FACTOR (DEFAULT=1.00)

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)									
END BENT/	PIPE PILE	ST	STEL						
BENT NO. PILE (s) #-# (e.g., "BENT 1, PILES 1-5")	PLATES REQUIRED YES OR MAYBE	PIPE PILE CUTTING SHOES REQUIRED? YES	PIPE PILE CONICAL POINTS REQUIRED? YES	H-PILE POINTS REQUIRED? YES	PILE TIPS REQUIRED? YES				
-				-					
TOTAL OTY.				-					

FOUNDATION TABLES ARE BASED ON THE BRIDGE SUBSTRUCTURE DESIGN AND FOUNDATION DATIONS SEALED BY A NORTH CAROLINA PROFESSIONAL ENGINEER (STEPHEN C.CROCKETT,048207)ON 01-04-2023. E DRIVING EQUIPMENT SETUP QUANTITY (NOT SHOWN IN PILE FOUNDATION TABLES)EQUALS THE NUMBER OF DRIVEN. "THE NUMBER OF PILES WITH A REQUIRED DRIVING RESISTANCE. HEER WILL DETERMINE NEED FOR PDA TESTING WHEN PDA'S MAY BE REQUIRED.

TH CAROLINA			PRI			NARI			
SEAL 5 13406 KELVING		PILE FOUNDATION TABLES							
Decessioned by: Jase plu. + ke huin atam					(RL)				
4/21 <sup>9/2</sup> 023 <sup>80774AF</sup>		REVISIONS							
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S9-03		
FINAL UNLESS ALL	1			3			TOTAL SHEETS		
SIGNATURES COMPLETED	2			4			30		
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PROJECT	NO. <u>R-270</u>	7D
(	CLEVELAND	_ COUNTY
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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION RALEIGH PRELIMINARY

![](_page_18_Figure_1.jpeg)

	TOTAL BILL OF MATERIAL														
	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MODI PRES CO GI	FIED 72″ STRESSED NCRETE RDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 STEEL PILES	HP STEE	14X73 El PILES	CONCRETE BARRIER RAIL	4″SLOPE PROTECTION	ELASTOMERIC BEARINGS
	LIN.FT.	LIN.FT.	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EA.	NO.	LIN.FT.	LIN.FT.	SQ.YDS.	LUMP SUM
SUPERSTRUCTURE			4,356	5,318		LUMP SUM		4	413.33				207.83		LUMP SUM
END BENT 1	59.0	65.0			53.3		8,744			8	8	160		215	
END BENT 2	39.0	93.0			54.2		8,822			8	8	140		230	
TOTAL	98.0	158.0	4,356	5,318	107.5	LUMP SUM	17,566	4	413.33	16	16	300	207.83	445	LUMP SUM

Stantec Consulting Services Inc. 801 Jones Franklin Road Suite 300 Raleigh, NC 27606 Tel. (919) 851-6866 Fax. (919) 851-7024 www.stantec.com License No. F-0672 DRAWN BY : J.E. HAGENBUSH CHECKED BY : J.T. KELVINGTON DATE : 12/01/22 CHECKED BY : J.T. KELVINGTON DATE : 12/05/22 DESIGN ENGINEER OF RECORD: J.T. KELVINGTON DATE : 04/21/23

## NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATION. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT STRUCTURES, SEE SPECIAL PROVISIONS. REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS. NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER. THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB. WORK SHALL NOT BE STARTED ON THIS BRIDGE UNTIL ROADWAY SECTION HAS BEEN EXCAVATED.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY IN PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

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	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH									
SEAL 13406 KEINING	GENERAL DRAWING FOR BRIDGE ON US 74 BYPASS OVER -SRVRD_5- BETWEEN SR 2047 (BORDERS RD.) AND									
Joseph T. Kelvinaton			(RL)							
4/21%2093%0774AF	NO. BY:		SIONS	DATE.	SHEET NO. \$9-04					
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								STRENGTH I LIMIT STATE					SE	SERVICE III LIMIT STATE										
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.08		1.75	0.936	1.26	А	EL	51.00	0.936	1.63	А	I	30.30	0.80	0.936	1.08	А	I	51.00	
DESIGN		HL-93 (OPERATING)	NZA		1.63		1.35	0.936	1.63	А	EL	51.00	0.936	2.16	А	I	6.60	NZA						
RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	1.52	54.7	1.75	0.936	1.76	А	EL	51.00	0.936	2.26	А	I	6.60	0.80	0.936	1.52	А	I	51.00	
		HS-20 (OPERATING)	36.000		2.29	82.4	1.35	0.936	2.29	А	EL	51.00	0.936	2.96	А	I	6.60	NZA					51.00	
		SNSH	13.500		3.62	48.9	1.40	0.936	5.25	А	EL	51.00	0.936	7.19	А	I	6.60	0.80	0.936	3.62	А	I	51.00	
		SNGARBS2	20.000		1.47	29.4	1.40	0.936	2.13	А	EL	51.00	0.936	2.85	А	I	6.60	0.80	0.936	1.47	А	I	51.00	
	ICLE	SNAGRIS2	22.000		1.31	28.8	1.40	0.936	1.90	А	EL	51.00	0.936	2.58	А	I	6.60	0.80	0.936	1.31	А	I	51.00	
		SNCOTTS3	27.250		1.44	39.2	1.40	0.936	2.09	А	EL	51.00	0.936	2.86	А	I	6.60	0.80	0.936	1.44	А	I	51.00	
	(S (S	SNAGGRS4	34.925		1.25	43.7	1.40	0.936	1.81	А	EL	51.00	0.936	2.50	А	I	6.60	0.80	0.936	1.25	А	I	51.00	
	ING	SNS5A	35.550		1.80	64.0	1.40	0.936	2.61	А	EL	51.00	0.936	3.53	А	I	6.60	0.80	0.936	1.80	А	I	51.00	
		SNS6A	39.950		2.44	97.5	1.40	0.936	3.54	А	EL	51.00	0.936	4.60	А	I	6.60	0.80	0.936	2.44	А	I	51.00	
LEGAL		SNS7B	42.000		2.61	109.6	1.40	0.936	3.79	А	EL	51.00	0.936	5.00	А	I	6.60	0.80	0.936	2.61	А	I	51.00	
RATING	ER	TNAGRIT3	33.000		1.59	52.5	1.40	0.936	2.31	А	EL	51.00	0.936	3.10	А	I	6.60	0.80	0.936	1.59	А	I	51.00	
	RAII	TNT4A	33.075		1.32	43.7	1.40	0.936	1.92	А	EL	51.00	0.936	2.46	А	I	6.60	0.80	0.936	1.32	А	I	51.00	
	1-IV	TNT6A	41.600		1.27	52.8	1.40	0.936	1.84	А	EL	51.00	0.936	2.39	А	I	6.60	0.80	0.936	1.27	А	I	51.00	
	SEA ST)	ΤΝΤ7Α	42.000		1.20	50.4	1.40	0.936	1.74	А	EL	51.00	0.936	2.34	А	I	6.60	0.80	0.936	1.20	А	I	51.00	
	TOR (TT)	TNT7B	42.000	$\langle 3 \rangle$	1.19	50.0	1.40	0.936	1.73	А	EL	51.00	0.936	2.27	А	I	6.60	0.80	0.936	1.19	А	I	51.00	
	TRAC	TNAGRIT4	43.000		1.59	68.4	1.40	0.936	2.31	А	EL	51.00	0.936	3.05	А	I	6.60	0.80	0.936	1.59	А	I	51.00	
	JCK	TNAGT5A	45.000		1.29	58.1	1.40	0.936	1.88	А	EL	51.00	0.936	2.62	А	I	6.60	0.80	0.936	1.29	А	I	51.00	
	TRL	TNAGT5B	45.000		1.29	58.1	1.40	0.936	1.88	А	EL	51.00	0.936	2.57	А	I	6.60	0.80	0.936	1.29	Α	I	51.00	

![](_page_19_Figure_4.jpeg)

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DRAWN BY :	N. D'AIUTO DATE : 05/01/18 BOYKIN JR. DATE : 12/08/22	DESIGN ENGINEER OF RECORD: <u>J.T.KELVINGTON</u> DATE : 04/21/23

+

![](_page_19_Figure_8.jpeg)

<u>LRFR SUMMARY</u>

END BENT 1

## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{\sf DW}$
LOAD RATING FACTORS	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

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СОМ

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN. SPAN LENGTH AND SUPPORT CONDITIONS SHOWN IN THE LRFR Sketch conforms to the analysis model used for all load conditions.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

	PROJEC	T NO.	R-2	707D					
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THE CAROL	STANDARD								
SEAL 13406		PRE	STRES	SSED	UK				
FRANK INE FRANK		)NCRE j-tntf	ETE G rstatf	IRDEI TRAFI	RS FTC)				
Docusigned by: Nose plu. t. tribuin atom	(RL)								
4/21 <sup>8</sup> /2023 <sup>80774AF</sup>		REVIS	SIONS		SHEET NO.				
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![](_page_20_Figure_1.jpeg)

![](_page_21_Figure_1.jpeg)

### <u>NOTES</u>

PROVIDE 11/4" HIGH BEAM BOLSTERS UPPER AT 4'-O"CTS.ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0"CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 21/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

NO CHAMFER IS REQUIRED ON CORNERS OF GIRDER BUILDUPS.

PREVIOUSLY CAST CONCRETE IN THE UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3.000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT EXCEPT AS NOTED IN THE PLANS.

#5 G1 BAR MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO CLEAR REINFORCING STEEL AND STIRRUPS. (2 BR) DENOTES 2 BAR RUN

![](_page_22_Figure_7.jpeg)

-

+

![](_page_22_Picture_10.jpeg)

![](_page_22_Figure_11.jpeg)

![](_page_23_Figure_1.jpeg)

![](_page_24_Figure_1.jpeg)

![](_page_24_Figure_2.jpeg)

## TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT

## NOTES:

FOR SECTION B-B, & ELEVATION A-A, SEE TYPICAL SECTION DETAILS, SHEET 3 OF 3.
 "A" BARS, "B" BARS & BARRIER RAIL REINFORCEMENT. ARE NOT SHOWN IN DECK SLAB FOR CLARITY.
 (2 BR) DENOTES 2 BAR RUN.
 #4 S2 BARS MAY BE REPOSITIONED AS FOUND NECESSARY TO CLEAR PRESTRESSED GIRDERS.
 DENOTES CONC. BLOCKOUT. SEE END BENT SHTS. AND SUPERSTRUCTURE TYPICAL SECTION DETAILS.
 1'-7" MIN. SPLICE FOR #4 K1.

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SEAL 13406		PLAN	OF S	SPANS				
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Joseph T. Lehnington 4777843699380074AF		REVI	SIONS		SHEET NO.			
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FINAL UNLESS ALL SIGNATURES COMPLETED	1		3 4		total sheets 30			
	STR.#9							

![](_page_25_Figure_1.jpeg)

-L--

101'-11"(@ BEARING TO @ BEARING) 33'-4" 35'-3" W.P. #2R-- & INTERMEDIATE STEEL DIAPHRAGM € BEARING — — € BOLT HOLES FOR INTERMEDIATE STEEL DIAPHRAGMS (TYP.) (E1) 105'-7"(W.P. #1R TO W.P. #2R)

### <u>SPAN A</u>

FRAMING PLAN

	CLEVELANDCOUNTY						
	STATION: 810+00.00 -L-						
	STATE OF NORTH CAROLINA						
	DEPARTMENT OF TRANSPORTATION						
TH CARO	SUPERSTRUCTURE						
SEAL 13406 KELVING	FRAMING PLAN						
Joseph T. Kelnington	(RL)						
4/219/2023 <sup>80774AF</sup>	REVISIONS SHEET NO.						
DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: \$9-11						
FINAL UNLESS ALL SIGNATURES COMPLETED	13Total sheets2430						
	STR. #9						

PROJECT NO. <u>R-2707D</u>

![](_page_25_Figure_7.jpeg)

1'-10"

![](_page_26_Figure_1.jpeg)

![](_page_27_Figure_1.jpeg)

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

DEPTH OF 1/4".

FLANGE.

![](_page_27_Figure_11.jpeg)

![](_page_27_Figure_12.jpeg)

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT.HEAT BENDING SHALL NOT BE ALLOWED.

# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

A 2" × 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM

	PROJEC	CT NO.		R-2	707D	
		CLEV	<u>/El</u>	_AND	C	OUNTY
	STATI	ON:	81	0+00	).00 -	-L-
	<u>Sheet 2 (</u>	)F 3				
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SEAL 13406 KELVINGINEER	PREST	MOE RESSEI I	) I   ) ( ) [	FIED CONC TAIL	72″ RETE S	GIRDER
Joseph T. Kelnington				(RL)		
4/21%202380774AF	NO. BY:	DATE:	NO.	BY:	DATE:	S9-13
FINAL UNLESS ALL SIGNATURES COMPLETED	1		3 4			TOTAL SHEETS 30

![](_page_28_Figure_1.jpeg)

STRUCTURAL STEEL NOTES
ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.
TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL $^{\prime}\!\!/_4$ TURN.
THE PLATES,BENT PLATES,AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION),SEE SPECIAL PROVISIONS.
FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.
GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ PROJECTION BEYOND THE NUT.
INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW,COMMENTS AND ACCEPTANCE. AFTER REVIEW,COMMENTS,AND ACCEPTANCE,SUBMIT SEVEN SETS FOR DISTRIBUTION.
IN THE EXTERIOR BAYS,PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS.STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM ``A''	DIM ``B''	DIM ``C''	DIM ``L''
MODIFIED 72" PRESTRESSED CONCRETE GIRDER	1′-6″	1'-8″	1′-8¾″	4'-2"

	PROJEC	CT NO.	R-2	707D		
	CLEVELAND COUNTY					
	STATION: 810+00.00 -L-					
SHEET 3 OF 3						
	DEPA	STAT RTMENT	e of north card OF TRAN Raleigh	NSPORTA	TION	
OFESSION	INTERMEDIATE STEEL DIAPHRAGMS					
SEAL 13406	PF	RESTRE	SSED C	E CONCRE	TE	
Joseph T. Kelvington		(	GIRDER	S		
4/21842023380774AF		REVIS	SIONS		SHEET NO.	
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STORATORES COMILETED	<u>८</u> STR.#9		(박)		30	

![](_page_29_Figure_1.jpeg)

![](_page_29_Figure_2.jpeg)

# NOTES

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

# MAXIMUM ALLOWABLE SERVICE LOADS D.L.+L.L. (NO IMPACT) TYPE IV 225 k

PROJECT NO. <u>R-2707D</u> CLEVELAND COUNTY STATION: 810+00.00 -L-

	DEF	PARTMENT	e of Of	NORTH CAR TRAN RALEIGH	NSPORTA	TION
OFESSION IF	EL#	ASTOME —— De	E R E T	RIC AIL	BEAR S ====	ING
13406 13406 13406 NG   NE P KELV	PRE	STRESSE SUPE	D ERS	CONCI STRUC (RL)	RETE G CTURE	IRDER
Joseph T. Kelmington. 1 /218439993280774AF		REVI	SIO	NS		SHEET NO.
DOCUMENT NOT CONSTDERED	NO. BY:	DATE:	NO.	BY:	DATE:	S9-15
FINAL UNLESS ALL SIGNATURES COMPLETED	1		3 4			total sheets 30
	STR.#9					

DocuSign Envelope ID: 028B4C68-1B97-428C-84AF-FC5A2FEC9514

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+

www.stantec.com

License No. F-0672

DRAWN BY : N. D'AIUTO CHECKED BY : J.T. KELVINGTON DATE : 12/09/22 DATE : 12/09/22 OF RECORD: J.T. KELVINGTON DATE : 04/21/23

Stantec

![](_page_30_Figure_1.jpeg)

ZERO CAMBER BASE LINE	€ BEARING ◄ @ END BENT 2
0       11       12       13       14       15       16       17       18       19       20       21       22       23       24       25       26       27       28         50       0.275       0.300       0.325       0.350       0.375       0.400       0.425       0.450       0.475       0.500       0.525       0.550       0.575       0.600       0.625       0.650       0.675       0.7         10       124       0.124       0.123       0.145       0.151       0.152       0.152       0.151       0.155       0.600       0.625       0.650       0.675       0.7	3       29       30       31       32       33       34       35       36       37       38       39       40         00       0.725       0.750       0.775       0.800       0.825       0.850       0.875       0.900       0.925       0.950       0.975       1.000
$\frac{16}{0.124} 0.132 0.139 0.145 0.151 0.155 0.158 0.161 0.162 0.163 0.162 0.161 0.162 0.161 0.158 0.155 0.151 0.145 0.159 0.1}{0.1071 - 0.077 - 0.081 - 0.086 - 0.099 - 0.093 - 0.095 - 0.098 - 0.099 - 0.100 - 0.100 - 0.100 - 0.099 - 0.098 - 0.095 - 0.093 - 0.090 - 0.086 - 0.09}{0.091 - 0.091 - 0.091 - 0.099 - 0.098 - 0.095 - 0.093 - 0.090 - 0.086 - 0.09}{0.091 - 0.091 - 0.091 - 0.099 - 0.098 - 0.095 - 0.093 - 0.090 - 0.086 - 0.09}{0.091 - 0.093 - 0.093 - 0.095 - 0.095 - 0.098 - 0.099 - 0.100 - 0.100 - 0.100 - 0.099 - 0.098 - 0.095 - 0.093 - 0.090 - 0.086 - 0.09}{0.091 - 0.091$	$\frac{32}{981} - 0.077 - 0.071 - 0.066 - 0.059 - 0.053 - 0.046 - 0.039 - 0.030 - 0.024 - 0.016 - 0.008 0.000}{\frac{1}{8}''} + 0\frac{9}{16}'' + 0\frac{1}{2}'' + 0\frac{1}{16}'' + 0\frac{3}{8}'' + 0\frac{3}{8}'' + 0\frac{5}{16}'' + 0\frac{1}{4}'' + 0\frac{3}{16}'' + 0\frac{1}{8}'' + 0\frac{1}{16}'' + 0''$
June	Q END BENT 2 0
SCHEMATIC CAMBER ORDINATES (SPAN A) All values are shown in decimals of a foot except tinal camber (or deflection)" which is shown in inches. (+) final camber indicates net upward displacement. (-) final camber indicates net downward displacement.	PROJECT NO. <u>R-2707D</u> <u>CLEVELAND</u> COUNTY STATION: <u>810+00.00 -L-</u> STATION: <u>810+00.00 -L-</u> STATION: <u>810+00.00 -L-</u> STATION: <u>STATE OF NORTH CAROLINA</u> DEPARTMENT OF TRANSPORTATION RALEION SUPERSTRUCTURE DEFLECTIONS <u>REVISIONS</u> <u>REVISIONS</u> <u>REVISIONS</u> <u>REVISIONS</u> <u>SHEET NO.</u> <u>SHEET NO.</u> <u></u>

—

![](_page_31_Figure_1.jpeg)

(2)

987

1050

1489

46

4,930 LBS

boddsigned by: Nose plu T. Ke Livin atom					(RL)		
4/21 <sup>9/2</sup> 023 <sup>80774AF</sup>			REVI	SION	٩S		SHEET NO.
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S9-17
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	ST	R.#9		S	TD.N	IO. CBR1	_

![](_page_32_Figure_1.jpeg)

![](_page_32_Figure_2.jpeg)

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE  $1 \frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE  $\frac{3}{4}$ " Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

![](_page_32_Figure_13.jpeg)

SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>R-2707D</u> <u>CLEVELAND</u> COUNTY 810+00.00 -L-STATION:\_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RAI FTGH STANDARD GUARDRAIL ANCHORAGE SEAL 13406 FOR BARRIER RAIL (RL) Joseph T. Kelnington SHEET NO. REVISIONS 4**`21%2023**<sup>80774AF</sup> S9-18 NO. BY: DATE: DATE: BY: DOCUMENT NOT CONSIDERED TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED 30

![](_page_33_Figure_1.jpeg)

	RE BA	INF R S(	ORC Chei
BAR	NO.	SIZE	TYPE
<b>*</b> A1	191	#5	STR
Α2	191	<b>#</b> 5	STR
<b>*</b> B1	58	<b>#</b> 5	STR
B2	82	#5	STR
<b>₩</b> B3	54	#6	STR
<b>₩</b> B4	54	#6	STR
<b>*</b> G1	2	#5	STR
К1	28	#4	STR
К2	6	#4	STR
КЗ	30	#4	STR
К4	6	#4	STR
К5	4	#4	א
К6	20	#4	3
К7	4	#4	3
K8	12	#4	STR
<b>*</b> S1	60	#4	2
<b>*</b> S2	80	#4	2
U1	60	#4	1
U2	16	#4	1
REINF STEEL	FORC:	ING	

	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL					
	(CU.YDS.)	(LBS.)	(LBS.)					
POUR #1	122.9							
POUR #2	81.2							
TOTALS**	204.1	14,167	15,944					

\*\*QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS										
BAR	BAR SLABS, PARAPET, SIZE AND BARRIER RAIL					SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		H APPROACH SLABS		PARAPET AND BARRIER
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL					
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"					
#5	2'-5"	2'-0"	2'-5"	2'-0"	3'-1"					
#6	2'-10"	2'-5"	3'-7"	2'-5"	3'-8"					
#7	4'-2"	2'-9"								
#8	4'-9"	3'-2"								

![](_page_33_Figure_8.jpeg)

GROOVING	BRIDGE FL	OORS
APPROACH SLABS	1,692	SQ.FT.
BRIDGE DECK	3,626	SQ.FT.
TOTAL	5,318	SQ.FT.

PROJECT I	NO. <u>R-2707[</u>	)
CI	LEVELAND	COUNTY
STATION:	810+00.00	-L-

RALEIGH
SUPERSTRUCTUR

# RE BILL OF MATERIAL

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

Joseph T. Lelvington					(RL)		
6/22/2023			REVIS	510	٧S		SHEET NO.
DOCUMENT NOT CONSTDERED	N0.	BY:	DATE:	NO.	BY:	DATE:	S9-19
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	СТ	D #0					

FESS/ON

SEAL

13406

STO INEER

![](_page_34_Figure_1.jpeg)

![](_page_35_Figure_1.jpeg)

+

![](_page_36_Figure_1.jpeg)

![](_page_37_Figure_1.jpeg)

![](_page_37_Figure_2.jpeg)

![](_page_37_Figure_3.jpeg)

	BI	ILL (	OF M	1ATERI	AL
END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	6	#10	1	50′-7″	1306
B2	2	#10	STR	47'-3"	407
 	2	±∠	STR	24'-10"	133
	0			27 10	133
B4	0	#6 #0	SIR	47-11	432
B2	6	#Ч	1	50'-3"	1025
B6	19	#4	STR	3'-4"	42
Β7	6	#4	STR	30'-5″	122
B8	6	#4	STR	19′-5″	78
B9	6	#4	STR	8′-5″	34
Н1	16	#7	6	21′-9″	711
	0	+C	6	17/_7"	150
	0	"0 #F	0	13 - 3	109
H3	15	#5	6	21'-9"	295
H4	12	#5	6	13'-3"	166
H5	10	#7	6	19′-5″	397
Н6	4	<b>#</b> 6	6	10'-11"	66
Н7	10	#7	6	9'-2"	187
H8	4	#6	6	9'-2"	55
На	2	#7	6	21'-2"	87
	۷	ſ	0		
1/ 1	0	± 1	стр		1 ⊑
KI	× ک	#4	SIK	2 - Y"	15
K2	28	#4	STR	5'-10"	109
S1	64	#5	2	11'-6″	768
S2	56	#5	3	4'-3"	248
S3	32	#4	4	7'-7"	162
		•	· ·		102
1	30	# 1	5	7'-0"	140
	10	·· 4	ン -	1-0	140
UZ	12	#4	5	4'-0"	32
V1	74	#5	STR	7′-6″	579
V2	16	<b>#</b> 5	STR	11'-1"	185
٧3	44	#5	STR	11'-10″	543
V4	11	#5	STR	11'-0"	126
V5	11	#5	STR	11'-9"	135
<u> </u>			511	11 5	133
REIN	FORC	ING S	FEEL		LBS.8,744
CLAS	SA (	CONCRE	ETE BR	EAKDOWN:	
POUR	#1: CA	P,COLI	_ARS,V	VINGS	C.Y. 35.8
POUR	#2:IF	PPFR V	VINGS		C.Y. 17.5
CLAS	SA (		ETE TC	TAL	C.Y. 53.3
					ŋ

PROJECT NO. R-2707D <u>CLEVELAND</u> COUNTY STATION: 810+00.00 -L-SHEET 4 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT 1 DETAILS SEAL 13406 (RL) Joseph T. Kelnington REVISIONS SHEET NO. 4**/21%2023**80774AF.. S9-23 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED BY: NO. TOTAL SHEETS 30

![](_page_38_Figure_1.jpeg)

![](_page_39_Figure_1.jpeg)

![](_page_39_Figure_15.jpeg)

![](_page_40_Figure_1.jpeg)

![](_page_41_Figure_1.jpeg)

![](_page_41_Figure_2.jpeg)

PES	

![](_page_41_Figure_4.jpeg)

![](_page_41_Figure_5.jpeg)

![](_page_41_Figure_6.jpeg)

Q

11

		END	BEN	T 2	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	6	<b>#</b> 10	1	50'-7"	1306
B2	2	<b>#</b> 10	STR	47'-3"	407
Β3	8	#4	STR	24'-4"	133
B4	6	#6	STR	46'-11″	432
B5	6	#9	1	50'-3"	1025
B6	20	#4	STR	3'-4"	45
Β7	6	#4	STR	29'-11"	122
B8	6	#4	STR	18'-11"	78
B9	6	#4	STR	7'-11"	34
H1	18	#7	6	22'-6″	828
H2	8	#6	6	13′-9″	165
Н3	13	#5	6	22'-6"	305
H4	12	#5	6	13′-9″	172
H5	10	#7	6	20'-2"	412
H6	4	#6	6	11′-5″	69
H7	10	#7	6	9′-2″	187
H8	4	#6	6	9′-2″	55
Н9	2	#7	6	21'-11"	90
K1	8	#4	STR	2'-9"	15
K2	28	#4	STR	5′-10″	109
S1	64	#5	2	11′-6″	768
S2	56	#5	3	4'-3"	248
S3	32	#4	4	7'-7"	162
U1	30	#4	5	7'-2″	144
U2	9	#4	5	4'-0"	24
V1	74	#5	STR	7′-6″	579
V2	20	#5	STR	11'-1"	231
V3	34	#5	STR	11'-10"	420
V4	11	#5	STR	11'-0"	126
V5	11	#5	STR	11'-9"	135
REIN	FORC	ING ST	LEEL		LBS. 8.822
					·
POUR	#1: C4	AP,COLI	_ARS,V	VINGS	C.Y. 36.1
I COLLAIN IN THOS CALA JUAL					

BILL OF MATERIAL

L			
I			
Γ	POUR#1:CAP,COLLARS,WINGS	C.Y.	36.1
ſ	POUR#2:UPPER WINGS	C.Y.	18.1
Γ	CLASS A CONCRETE TOTAL	C.Y.	54.2

		' <u>-</u>	12'-1"	_ ΠΖ, Π4	U2
1'-0"	U2		18'-6"	Н5	
3'-4"	U1	<u> </u>	9'-9"	H6	V1
1			7'-6"	► H7	V2
			7'-6"	► H8	V3
					V4
(5)			20'-9"	H9	V5
$\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{$		I		I	
ALL DAR D.		SARE UUT TU	001.		
					REIN
					POUR
					POUR
					CLASS

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	SHEET 4 C	)F 4			
	DEPA	STATE RTMENT	OF NORTH CAF	ROLINA NSPORTA	TION
SEAL 13406	EN[	) BEN	IT 2	DETA	ILS
Docusigned by:			(RL)		
4/2184202380774AF		REVIS	IONS		SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL	NO. ВҮ:	DATE:	NO. BY:	DATE:	TOTAL SHEETS

30

SIGNATURES COMPLETED

![](_page_42_Figure_1.jpeg)

![](_page_42_Figure_2.jpeg)

### GENERAL NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. THE CONTRACTOR, AT HIS OPTION, MAY USE ALTERNATE ``B'' ONLY FOR HIGHWAY OVER HIGHWAY GRADE SEPARATIONS WITH 2:1 END BENT SLOPE IN RURAL, UNPOPULATED AREAS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING.

## ALTERNATE ``A''

ALTERNATE ``A'' SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL' WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.810+00.00 -L-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
END BENT 1	215	388
END BENT 2	230	415

![](_page_42_Figure_9.jpeg)

![](_page_42_Figure_10.jpeg)

OPTIONAL POURING DETAIL

PROJECT N	NO	R-2707[	)
<u> </u>	_EVEL	AND	COUNTY
STATION:_	810	)+00.00	-L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SLOPE	PROTECTION
D	ETAILS

Toseph t. Kelinin atom					(RL)		
4/21942023 <sup>80774AF</sup>	REVISIONS				SHEET NO.		
DOCUMENT NOT CONSTDERED	N0.	BY:	DATE:	NO.	BY:	DATE:	S9-28
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			30
	стг	) #0					

SEAL 13406

![](_page_43_Figure_1.jpeg)

# NOTES

# APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE,6"Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 6"Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

![](_page_43_Figure_13.jpeg)

![](_page_43_Figure_15.jpeg)

BILL OF MATERIAL					
FOR ONE APPROACH SLAB (2 REQ'D)					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
<b>₩</b> A1	52	#4	STR	20'-6"	712
Α2	52	#4	STR	20'-5"	709
<b>米</b> B1	78	#5	STR	24'-2"	1966
B2	78	#6	STR	24'-8"	2890
REINFORCING STEEL			LBS.	3,599	
* EPOXY COATED REINFORCING STEEL				LBS.	2,678
CLASS AA CONCRETE			C.Y.	42.5	

SPLICE LENGTHS				
BAR SIZE	EPOXY COATED	UNCOATED		
#4	1'-11"	1'-7"		
#5	2'-5"	2'-0"		
#6	3'-7"	2'-5"		

![](_page_43_Picture_18.jpeg)

![](_page_43_Figure_21.jpeg)

PROJECT	NO. <u>R-2707</u>	)
(	CLEVELAND	COUNTY
STATION:	810+00.00	-L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

BRIDGE APPROACH SLAB

FOR INTEGRAL ABUTMENT WITH FLEXIBLE PAVEMENT

Joseph t. Kelwington	(RL)				
4/21%2%2%2% <sup>80774AF</sup>	REVISIONS				SHEET NO.
DOCUMENT NOT CONSTDERED	NO. BY:	DATE:	NO. BY:	DATE:	S9-29
FINAL UNLESS ALL	1		3		TOTAL SHEETS
SIGNATURES COMPLETED	2		<b>A</b>		30
	CTD #0	۲. ۲			

SEAL

13406

![](_page_44_Figure_1.jpeg)

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SO.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{1}{16}$ " Ø STUDS ALONG THE BEAM, AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{1}{16}$ " Ø STUDS FOR 4 - 🔏 Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

# ENGLISH JANUARY, 1990

STD. NO. SN