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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY WAKE

PROJECT DESCRIPTION BRIDGE NO. 126 ON -L-(LIGON MILL ROAD) OVER SMITH CREEK BETWEEN MAIN STREET AND US 401

STATE PROJECT REFERENCE NO. B-531816

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IMP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NINCLATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MOY LAVE CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

J. WESSELL

F. WESCOTT

M. EDWARDS

C. BUTLER

N. MOORE

C. WALKER

CAROLINA DRILLING

INVESTIGATED BY _F. WESCOTT

DRAWN BY __C. BUTLER

CHECKED BY J. WESSELL

SUBMITTED BY SCHNABEL ENG.

DATE __**JULY 2021**



7/28/2021

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PROJECT REPERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

Column C				
March 1	SOIL DESCRIPTION	GRADATION		TERMS AND DEFINITIONS
The content of the				ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
Maintain Company Com	ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIFICATION		SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	I ———
## 1	CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK.	[·
1			E0/7720/7/25	
Column C			ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	
The column	GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERANC MATERIALS			WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
The content of the	ULASS. (≤ 35% PASSING "2000) (> 35% PASSING "2000)		POCK (CD) WOULD FIELD SPI REFUSAL IF TESTED. RUCK TIPE INCLUDES GRANITE.	
The content is a part of the content is part of the content is part of the content is a part o			NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	I -
The content of the	000000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	
Part	6666666666666			
	#10 FG MY SIL1- MICK			
Second S	#40 30 MX 50 MX 51 MN	GRANULAR SILT - CLAY		
The control of the				
The case 1	PASSING #40 SOILS WITH			
	LL — — 46 MX 41 MN 46 MX 41 MN 46 MX 41 MN 48 MX 41 MN LITTLE OR LIGHT			
The part of the control of the con	CROIP INDEX 9 9 9 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC	GROUND WATER		
Part	LISUAL TYPES STONE FRACS			
Company Comp	OF MAJOR GRAVEL, AND SAND CRAVEL AND SAND SOLIS SOLIS			
Table 19	CEN PATING FAIR TO	∇PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	
Constitution Cons				FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
Compared			MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	
Part Company		MISCELLANEOUS SYMBOLS		
This	PRIMARY SOIL TYPE COMPRESSIVE PENETRATION RESISTENCE COMPRESSIVE STRENGTH			<u> </u>
Column C	(N-VALUE) (TUNS/FT-)	┫ ╚┦ '		
## ## ## ## ## ## ## ## ## ## ## ## ##	GENERALLY LOOSE 4 TO 10		TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	
Second Column 1985	MATERIAL MEDIUM DENSE 10 TO 30 N/A		<u> </u>	
Concession		THAN ROADWAY EMBANKMENT TEST		
Second S		— INFERRED SOIL BOUNDARY — CORE BORING ● SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	
STATES 10 10 10 10 10 10 10 1				I
Texture Description Property Color Property Property Color Property Prop	MATERIAL STIFF 8 TO 15 1 TO 2	A DIEZOMETED	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	
Married State St		INSTALLATION SPT N-VALUE		1
MACHINE 19	TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS		
Secretary Color		UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION -		SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
MILLION CLICAL		SHALLOW SHALLOW STEET OF		
MATERIAL	BUULDER CUBBLE GRAVEL SAND SAND SILI CLAY	UNDERCUT ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL		
SOLL MOSTURE SOLL MITS THE PROPERTY OF TERMS SOLL MOSTURE - CORRELATION OF THE MOSTURE - CORRELATION OF		ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	
SOIL MOISTURE CORRELATION OF TERMS SOIL MOISTURE CORRELATION OF THE TRANSPORT OF TERMS SOIL MOISTURE CORRELATION OF THE TRANSPORT OF TERMS SOIL MOISTURE CORRELATION OF THE TRANSPORT OF THE TRANSPOR				
SOLI MOISTURE SCALE FIELD MOISTURE DESCRIPTION COURS POR FILED MOISTURE DESCRIPTION COURSE FOR FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DESCRIPTION STATE AND FILED MOISTURE DURS OF A FICE FOR HIS SERVICE AS A PRECISION STATE AND FILED MOISTURE DESCRIPTION STATE AND FILED MOIST		CL CLAY MOD MODERATELY γ - UNIT WEIGHT		WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL
OHT - ONLY FIRST CASE CA	COTI MOISTINE CON E FIELD MOISTINE	70		
SATURATED - USUALLY LIDUID VEPT WET_USUALLY - VIDID BRTD - SAN-SARRY VET_USUALLY - VIDID BRTD - VIDID BRD - VIDID BRTD - VIDID BRTD - VIDID BRTD - VIDID BRTD - VIDID BRD		DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>		TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
FROM BELOW THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CROUND MATER TABLE FOR SET OF SET IN STATE OF THE CONTROL OF THE	- SATURATED - USUALLY LIQUID: VERY WET. USUALLY		PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
PLASTIC UNIT OF THIM MOISTURE SLEW THICKLY BEDDING TO ATTAIN OPTIMUM MOISTURE SLEW THICKLY BEDDING TO A T	(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE		THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
AND PLASTIC LIMIT PLASTICITY NON PLASTIC SIGNIFICATION PTIMOM MOISTURE ON PLASTICITY NON PLASTIC SIGNIFICATION PTIMOM MOISTURE ON PLASTICITY NON PLASTIC SIGNIFICATION PTIMOM MOISTURE ON PLASTICITY NON PLASTIC SIGNIFICATION SIGNIFICATION SIGNIFICATION SOLIDATOR NEAR OPTIMOM MOISTURE ON PLASTICITY NON PLASTIC SIGNIFICATION SIGNIFICATION SIGNIFICATION SOLIDATOR NEAR OPTIMOM MOISTURE ON PLASTICITY NON PLASTIC SIGNIFICATION SIGNIFI	PLASTIC CEMTED TO DESCRIPTION OF TO			TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
Fig.	RANGE Z - WET - (W) SEMISOLID; REGUIRES DRYING TO	FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING		BENCH MARK: BM-2, N 793495 E 2139827 -BL- STA 14+65.00
OM OPTIMUM MOISTURE SHRINKAGE LIMIT OF LORY - (ID) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE ON PLASTICITY PLASTICITY PLASTICITY PLASTICITY ON PLASTICI ON PLASTICI ON SOLIDATOR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT ON PLASTICI ON	"" PL L + PLASTIC LIMIT			OFFSET 76'LT, R/R SPIKE IN II" GUM
SL SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (LOSE SIZE) - DRY - (LOSE SIZE)	OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE		WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 201.04 FEET
PLASTICITY PLASTICITY INDEX (PI) SUIGHT SADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE PLASTICITY INDEX (PI) SUIGHT SADDITION NON PLASTIC SUIGHT SADDITION ORGANIC SUBSTAN ORGAN				NOTES:
PLASTICITY PLASTICITY INDEX (P)) ORY STRENGTH NON PLASTIC SLIGHTY PLASTIC O-5 VERY LOW SLIGHTY PLASTIC O-5 SLIGHTY OWNERATELY PLASTIC O-5 SLIGHTY OWNE SHEAR TEST OWNEROUS GRAINS, CASING W/ ADVANCER ODERATELY PLASTIC OF OWNERATELY INDURATED OF SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNERATELY PLASTIC OF SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNERATELY PLASTIC OF OWNERATELY INDURATED OF SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNERATELY INDURATED OF SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNERATELY INDURATED OF SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. OWNER SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, TO SUBJECT OF THE ADDRESS OF THE ADDRESS OF THE ADDRESS OF THE		G' CONTINUOUS ELIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD = FILLED IN AFTER DRILLING
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 9-5 VERY LOW SLIGHTLY PLASTIC 16-25 MEDIUM MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE 10-25 MEDIUM DESCRIPTIONS MAY INCLUDE COLOR OR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). 1 CORE BIT 1 CME-550 HARD FACED FINGER BITS Unng,-CARBIDE INSERTS Unng,-CARBIDE INSERTS TUNG,-CARB. INDURATED FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIBLE GENTLE BITM HOND TOOLS: CRAINS CAN BE SEPARATED SAMPLE. FRIBLE GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER. GRAIN ARE DIFFICULT TO BREAK WITH HAMMER. GRAINS ARE DIFFICULT TO BREAK WITH HAMMER. GRAIN CAN BE COMMEN TO THE ARE SUMMED		CME-555 □		
NON PLASTIC 0-5 VERY LOW SLIGHT VANE SHEAR TEST VANE SHEAR TES				
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH PORTABLE HOIST COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). SLIGHTLY PLASTIC 6-15 SLIGHT VANE SHEAR TEST HAND TOOLS: POST HOLE DIGGER HODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL P		TUNG,-CARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
HIGHLY PLASTIC 26 OR MORE HIGH PORTABLE HOIST X TRICONE STEEL TEETH HAND AUGER SOUNDING ROD TRICONE TR	SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST X CASING WY ADVANCER HAND TOOLS:	GENILE BLUW BY HAMMER DISINTEGRATES SAMPLE.	
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). TRICONE TUNGCARB SOUNDING ROD INDURATED GRAY WITH HAMMER. UNABLE SHEAR TEST SUBSPICIOUS TO BREAK WITH HAMMER.		POST HOLE DIGGER		
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). CORE BIT VANE SHEAR TEST CHARACTER CHAR	COLOR	TOUGHT TOUGHT	CRAINC ARE DISCILLET TO SERABATE WITH STEEL PROBE.	
CHARD HANNED DI ONC DEGITION TO DEGAY CANDLE.		I		
DATE: 8-15-1-				
		L] [A] NOLLOW SIEM AUGENS L]	SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-

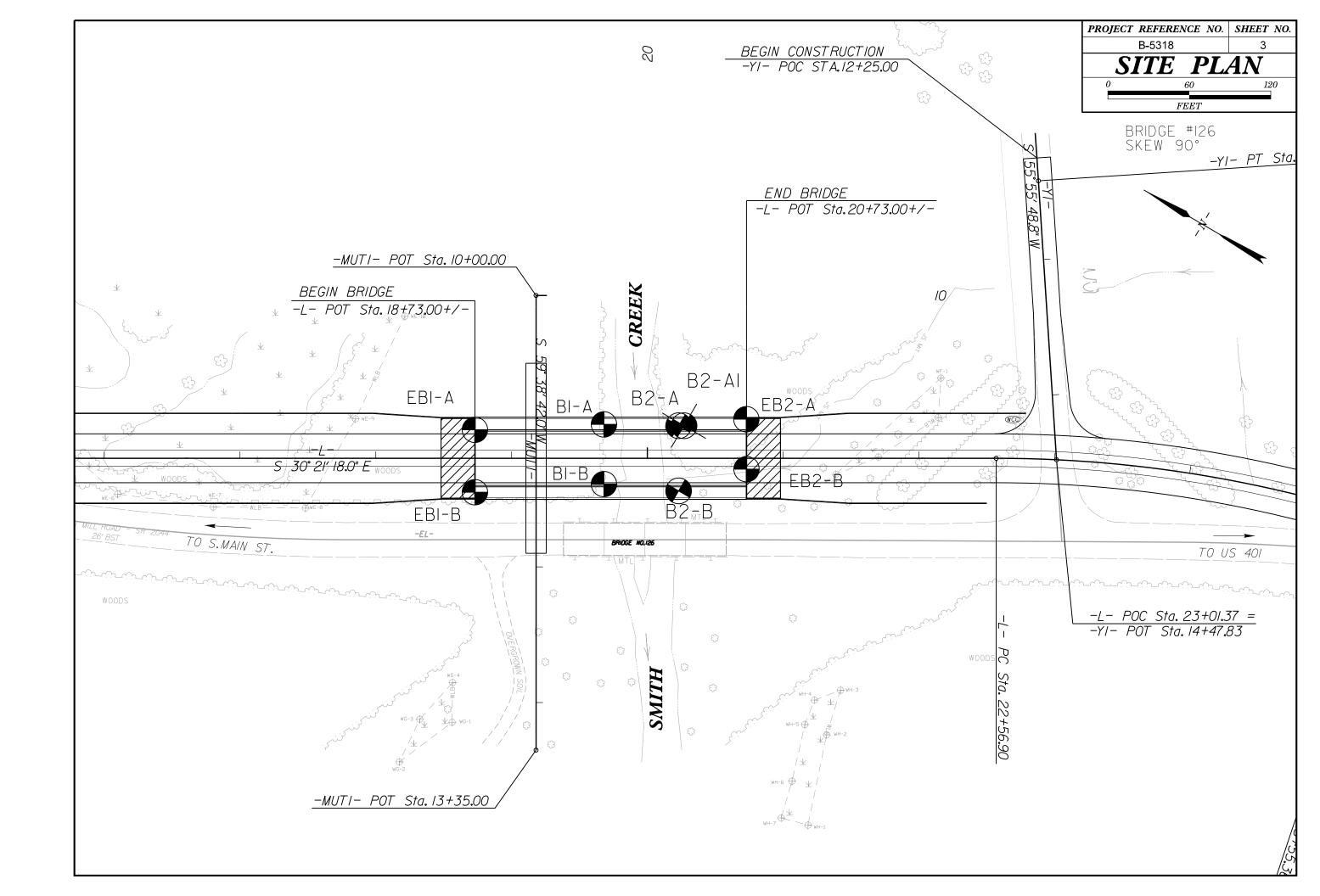
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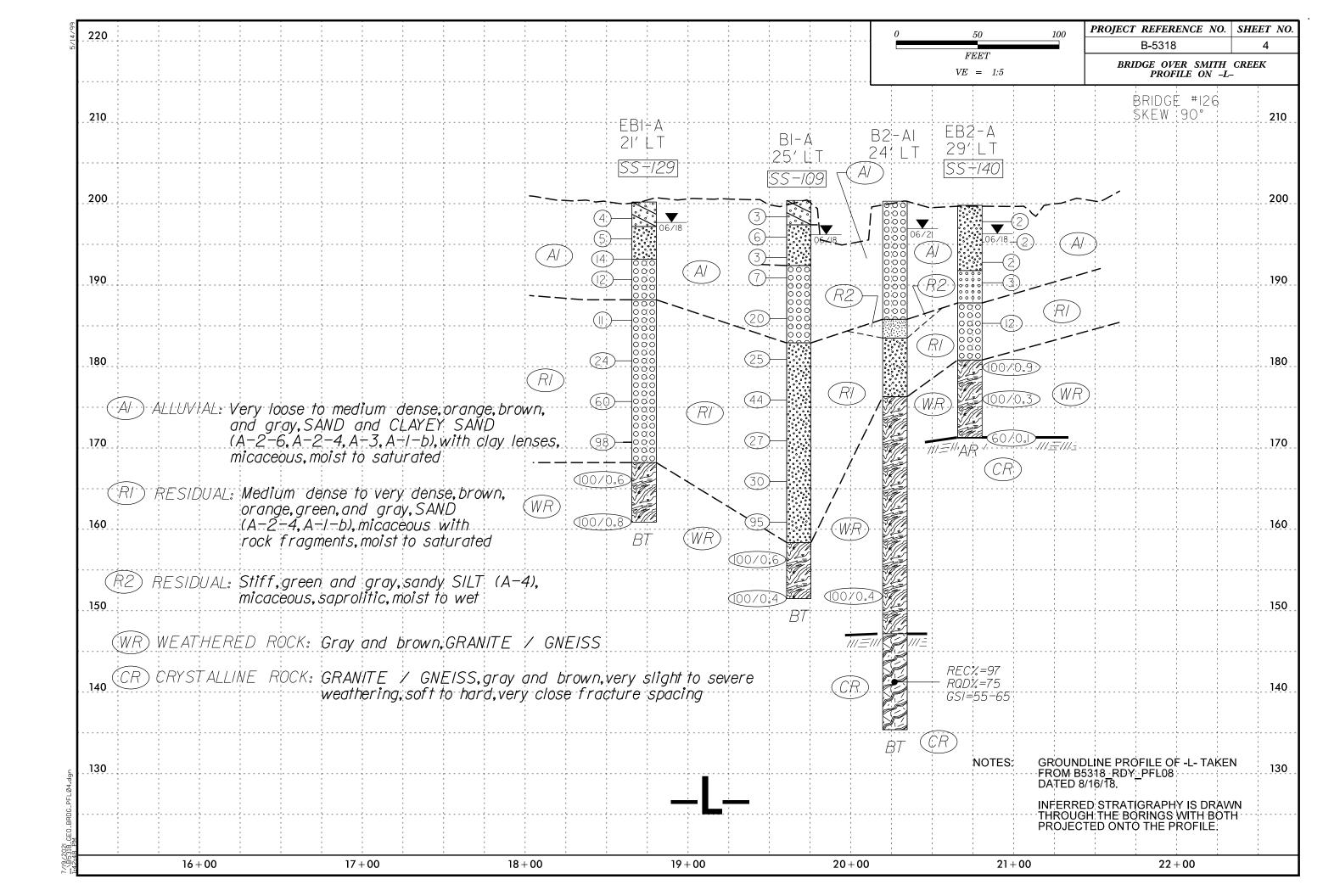
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

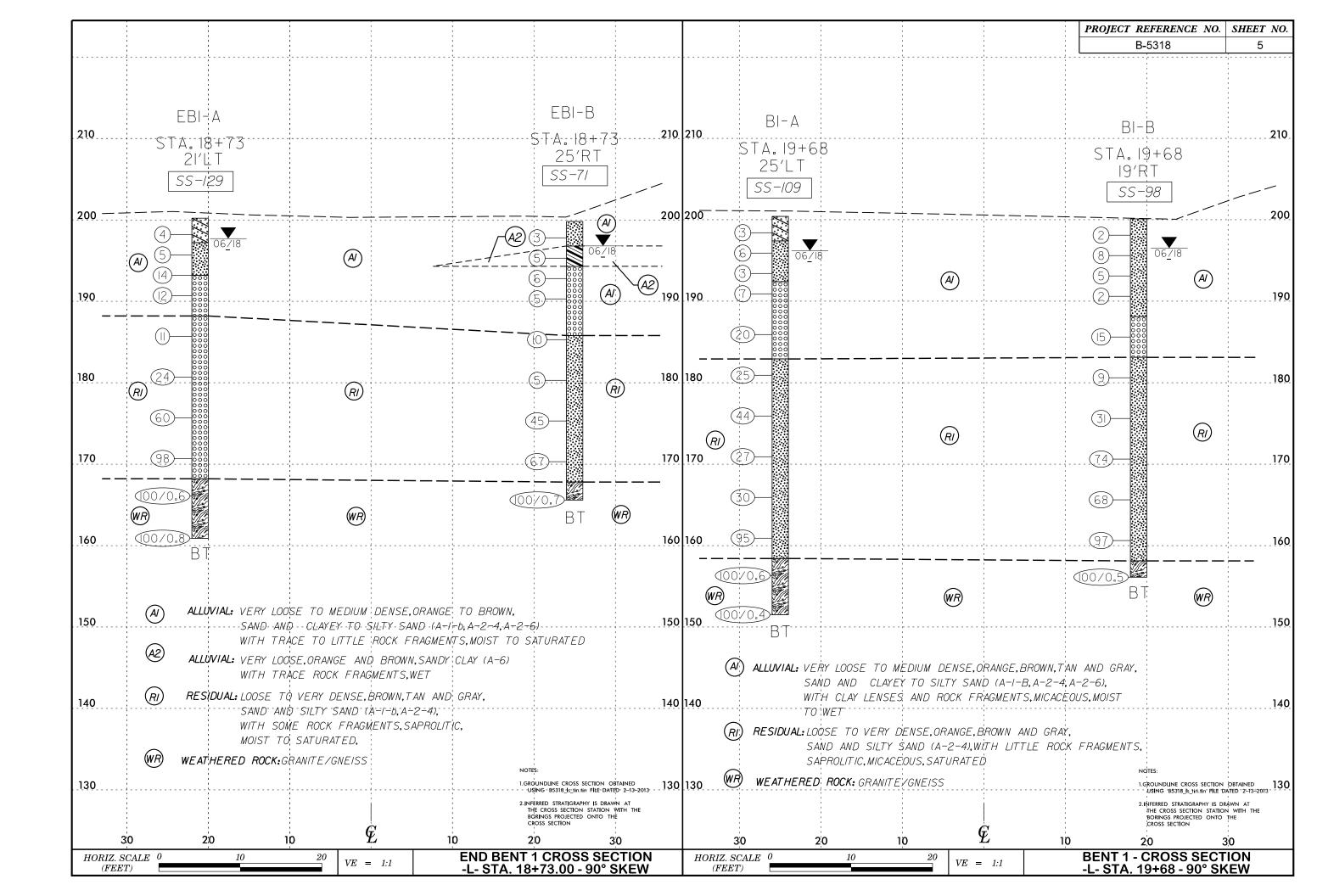
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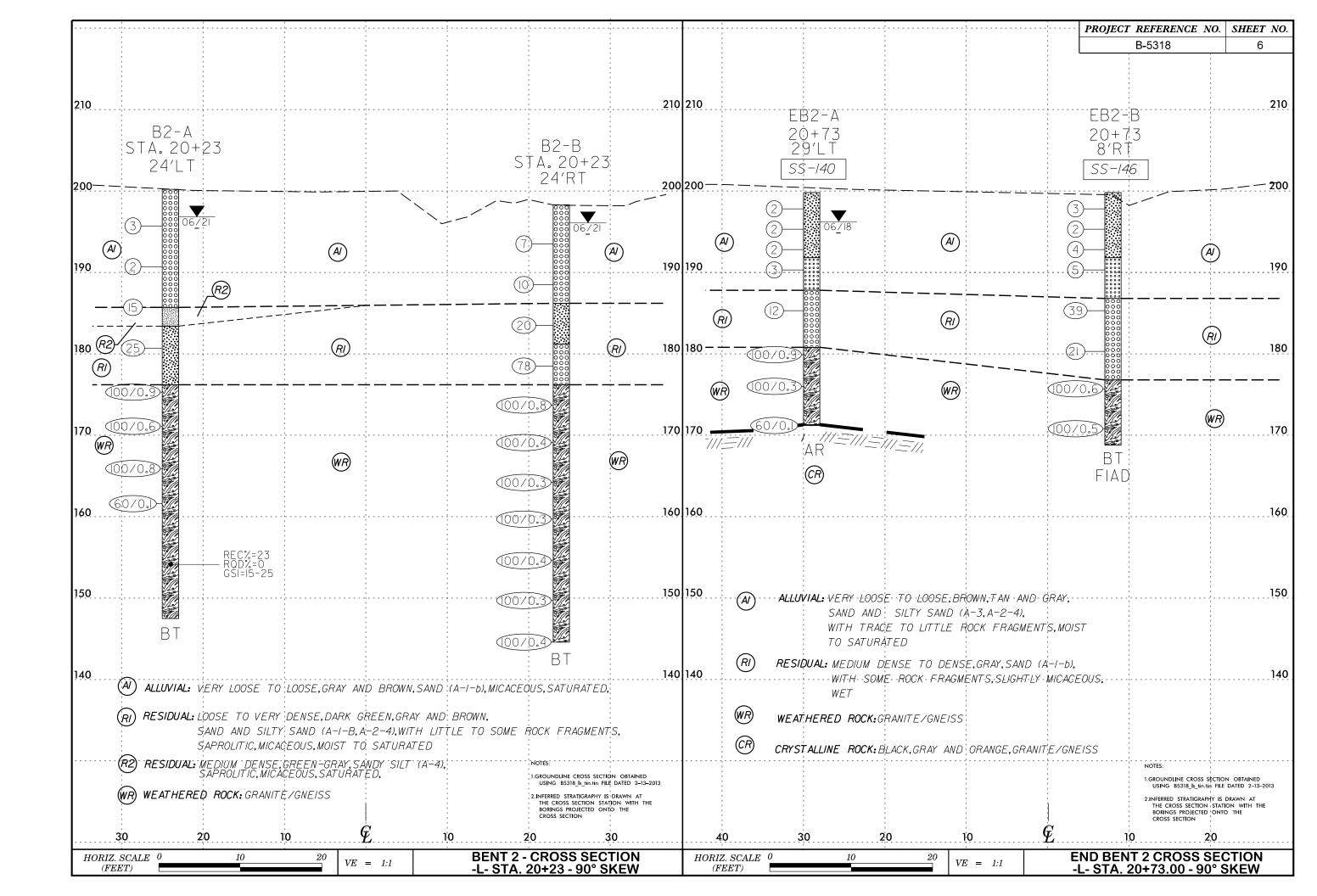
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

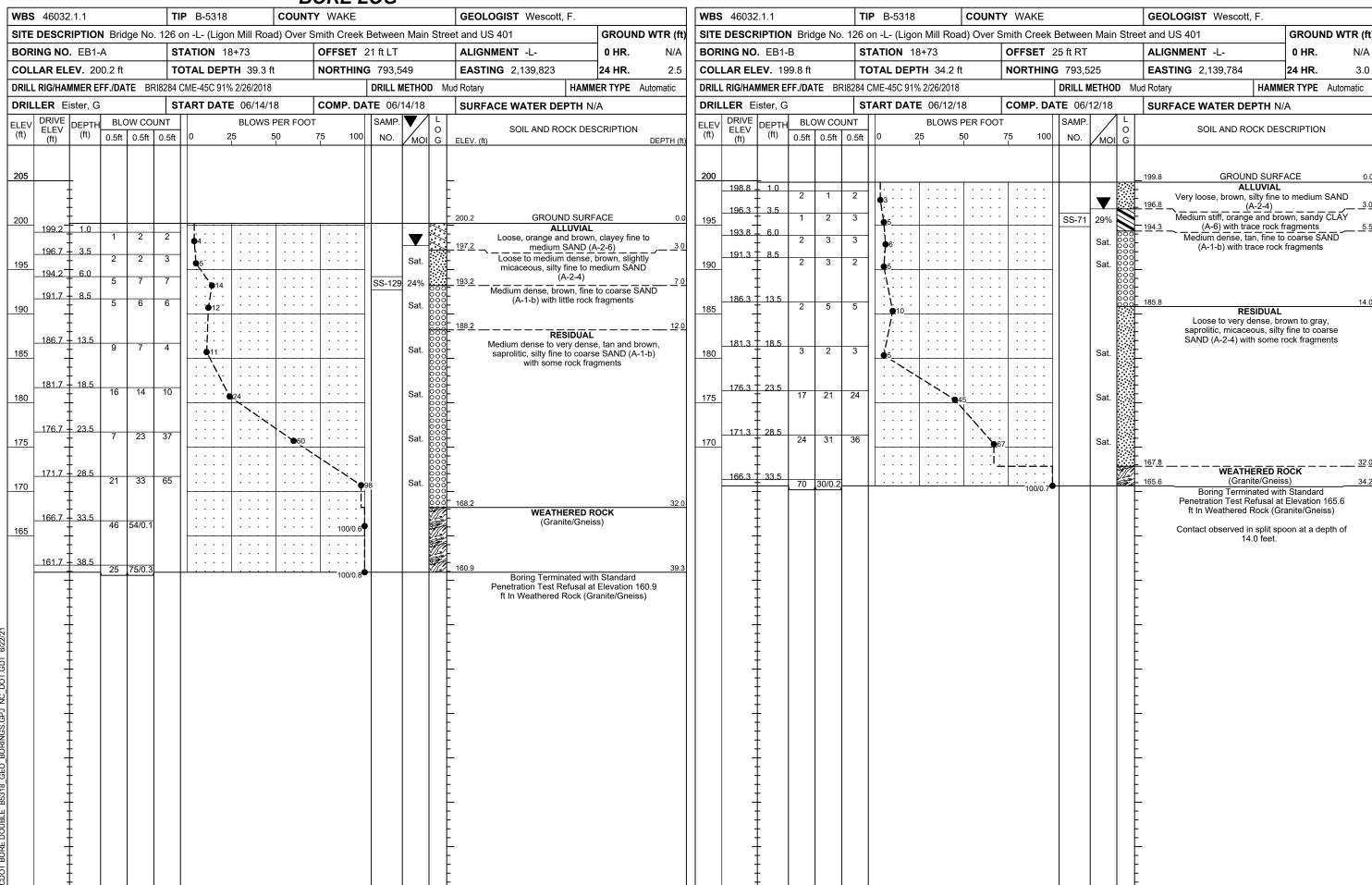
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos. P and Hoek E., 2000) From a description of the lithology, structure and ,occasionally es with compact s with angular POOR - Very smooth, slicken-l or highly weathered surfaces soft clay coatings or fillings From the lithology, structure and surface athered sur or fillings conditions of the discontinuities, estimate the average value of GSI. Do not try to surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the planes) be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too apply to structurally controlled failures. Where weak planar structural planes are precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the slightly present in an unfavorable orientation smooth, c surface fillings highly coating Hoek-Brown criterion does not apply to structurally with respect to the excavation face, these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, in rocks that are prone to deterioration slightly es these will dominate the behaviour of the rock mass. Rough, blickensided, houth compact of angular fra as a result of changes in moisture content will be reduced if water is The strength of some rock masses is reduced by the 1 0 GOOD rough, presence of groundwater and this can be allowed for present. When working with rocks in the by a slight shift to the right in the columns for fair, fair to very poor categories, a shift to th, r poor and very poor conditions. Water pressure does the right may be made for wet conditions. GOOD Rough, s surface VERY | sided with s FAIR -weath VERY Slick with VERY Very VERY Water pressure is dealt with by effective FAIR Smooralter not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. STRUCTURE DECREASING SURFACE QUALITY COMPOSITION AND STRUCTURE INTACT OR MASSIVE - intact A. Thick bedded, very blocky sandstone 90 rock specimens or massive in 7Ó N/A N/A The effect of pelitic coatings on the bedding situ rock with few widely spaced planes is minimized by the confinement of PIECES discontinuities the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally 80 controlled instability. 60 BLOCKY - well interlocked un-70[′] disturbed rock mass consisting of cubical blocks formed by three D. Siltstone B. Sand-stone wi thin inte intersecting discontinuity sets THINIA E. Weak 50 🛭 C. Sand-60 stone and stone with or silty shale siltstone thin inter siltstone with sandor clayey С shale with layers of an similar stone layers VERY BLOCKY - interlocked, siltstone amounts sands tone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets INTERL C. D. E. and G - may be more or F. Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but intensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and with broken and deformed CREASING loss of continuity moves these discontinuity sets. Persistence sandstone layers forming an 30 categories to F and H. of bedding planes or schistosity almost chaotic structure 20 DISINTEGRATED - poorly interlocked, heavily broken rock mass 20 H. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a rounded rock pieces or without a few very chaotic structure with pockets thin sandstone layers of clay. Thin layers of sandstone are transformed into small rock pieces 10 LAMINATED/SHEARED - Lack of blockiness due to close spacing N/A N/A → Means deformation after tectonic disturbance of weak schistosity or shear planes DATE: 8-19-16

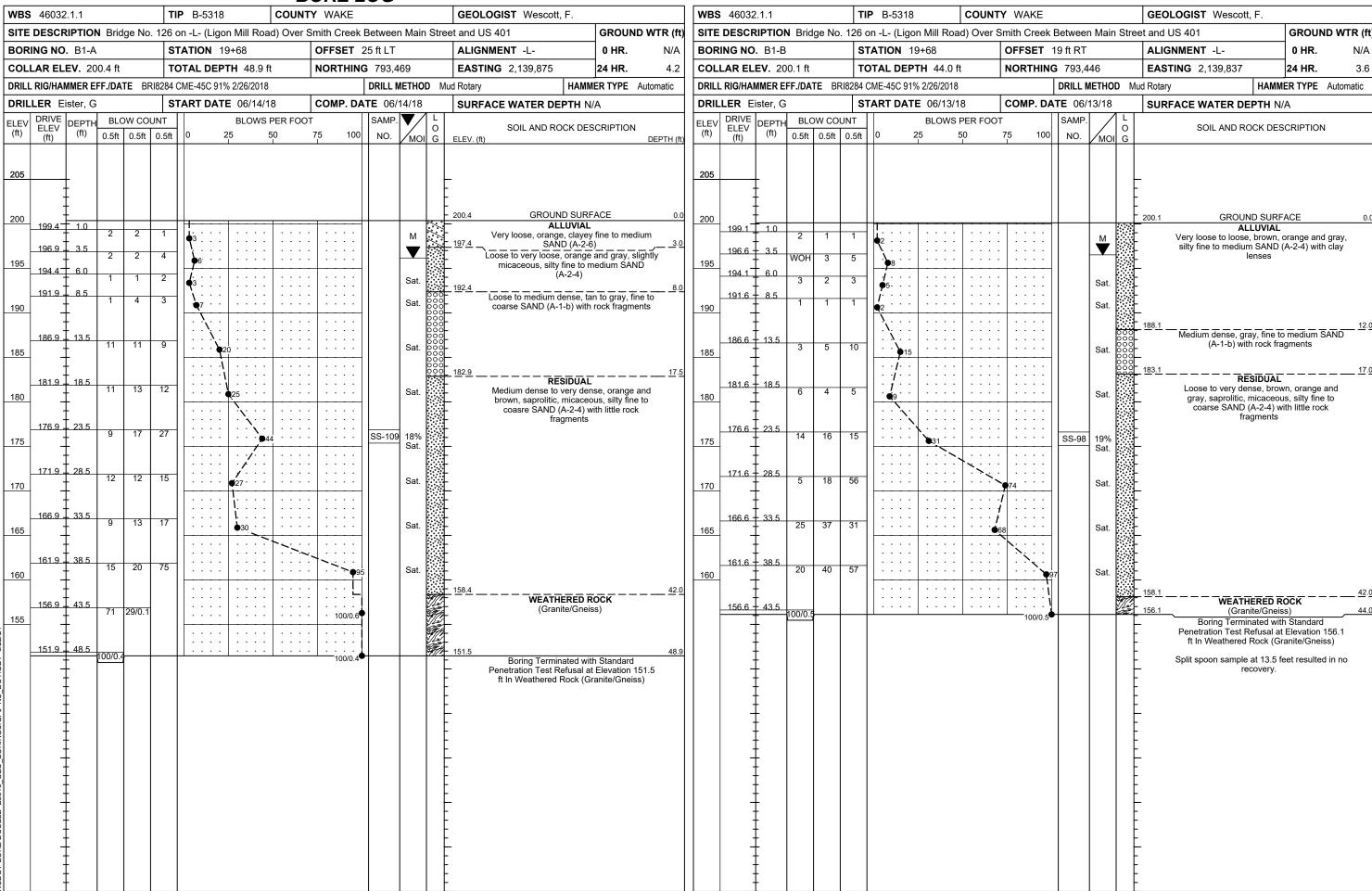










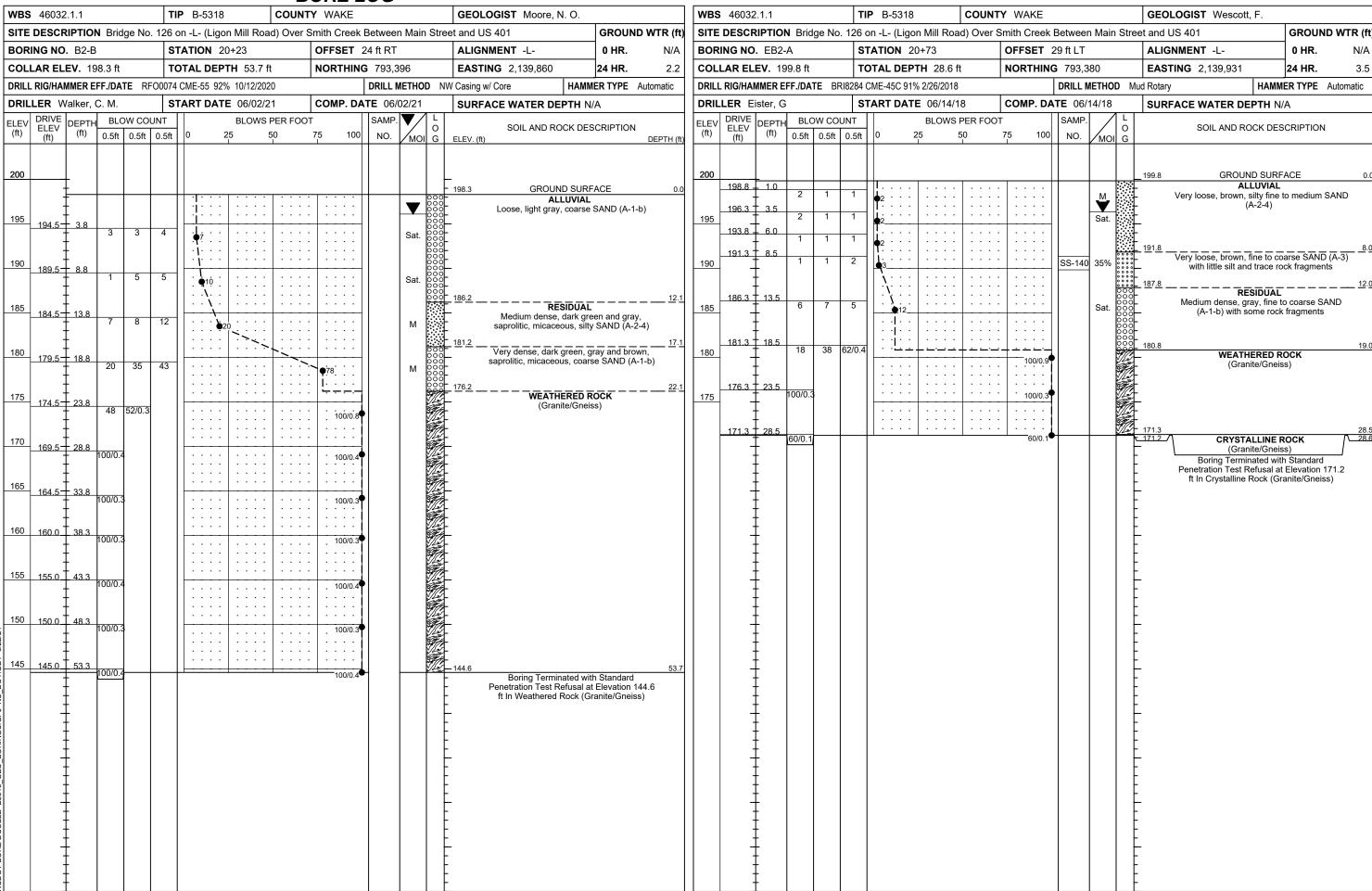


GEOTECHNICAL BORING REPORT CORE LOG

BORE LOG						CORE LOG																									
WBS 4603				TIP B										46032.1.1			P B-5318 COUNTY						EOLOGIST Moore, N. O.								
SITE DESC	RIPTIO	N Brido	je No. 1	1						SITE	SITE DESCRIPTION Bridge No. 126 on -L- (Ligon M				•	oad) Ove						ND WTR									
BORING NO). B2-A	. B2-A STATION 20+23 OFFSET 24 ft LT ALIGNMENT -L- 0 HR. N		N/A	BORING NO. B2-A			STATION 20+23				С	OFFSET 24 ft LT	ALIGNMI	ALIGNMENT -L- 0 H	0 HR.															
COLLAR EL				l		H 52.7 f		NORT					l	NG 2,139	,901	24 F	łR.	N/A		LAR ELEV. 20				PTH 52.		N	ORTHING 793,420		3 2,139,901	24 HR.	١
DRILL RIG/HA	MMER E	FF./DAT	E RFO	0074 CME-	55 92%	10/12/202	20			DRILL I	METHO	D NW	Casing w	/ Core	Н	AMMER TY	PE Auto	matic	DRILI	L RIG/HAMMER E	FF./DATE RFO	0074 CMI	E-55 92	% 10/12/2	020		DRILL METHO	NW Casing w/ Co	ore H.	AMMER TYPE	Automati
DRILLER V					DATE	05/26/2	21	COMF					SURFA	ACE WATI	ER DEPT	H N/A			DRIL	LER Walker,	C. M.	STA	RT DA	TE 05/2	6/21	С	OMP. DATE 05/27/21	SURFAC	E WATER DEPT	H N/A	
ELEV DRIVE ELEV			W COUN				PER FOO			SAMP.	1 7	0		SOIL A	AND ROCK	DESCRIP	TION		COR	E SIZE NQ				IN 14.1 f							
(II) (ft)	(ft)	0.5ft	0.5ft 0	.5ft U	2		50	75 	100	NO.	MOI	G E	ELEV. (ft)				D	EPTH (ft)	ELEV (ft)	RUN DEPTH	I /ft/ I KAIL	REC.	RUN RQD (ft) %	SAMP. NO.	STRATA REC. RC (ft) (ft) % %	V C		DESCRIPTIO	N AND REMARKS		
																			(11)	(ft) (II)	(III) (Min/fi	t) (11)	(II) %	INO.	(11)	' G	ELEV. (ft)				DEPT
205	+											-							161.6	161.6 + 38.6	2.6 00:26/0	0 6 (0 1)	(0.0)		(3.2) (0	0) 7	7-	Begin Cor	ring @ 38.6 ft D ROCK (continued)		
	Ŧ											F							160	159.0 41.2	00:43/1	0.6 (0.1) .0 4%	0%		(3.2) (0. 23% 09	6	Granite/Gneis	s, gray-brown, very	y slight to severe we e fracture spacing	athering, soft	to hard,
200	Ŧ											- 2	200.2	(GROUND S			0.0		1 1	5.0 01:11/1 01:02/1	.0 (0.0) .0 0%	(0.0) 0%					·	SI = 15-25		
	Ŧ							1				000		Very loos	ALLU\ se to mediu	m dense, d	ray and		155	1,5,0	00:47/1 00:36/1	.0						90	31 - 13-23		
196.7	3.5		2	i:								000 -		brown, mic	aceous, co	arse SAND) (Å-1-b) ,			154.0 46.2	01:17/1 5.0 00:57/1	.0 .0 (3.1) .0 62%	(0.0)								
195	Ŧ	2	2	· •3							Sat.								1.50	‡	01:32/1	.0	0%								
404.7	8.5										1								150	149.0 51.2	01:30/1 01:26/1	01	/2								
191.7	T 8.5	1	1	1 2							Sat.	000								147.5 - 52.7	1.5 03:19/1 03:19/0	1.0] (0.0) <u>).5</u> /_0%	(0.0)			7/2		ninated due to core	e bit breaking off at E	levation 147	ft In
	Ŧ										[000								‡							L Donning Fen	Weathered R	Rock (Granite/Gneiss)	. 1. 111
186.7	13.5	1	5	10 ' '	λ							000 -	185.7					14 5		1 1							_	Hard drilling en	acountered at 20.5 fe	et.	
185	Ŧ	2	٦	' .	15		+				Sat.	- E		Ctiff ~~	RESID		niones - · ·	14.0		‡							ţ				
404.7	† ,, ,				: : /							1		Stiff, green	sandy SIL	_T (A-4)				+							-				
181.7	18.5	6	12	13 :	: : :\	25 · · ·					М			Medium de micad	ense, green ceous, silty	n and gray, SAND (A-2	saprolitic, 2-4)			1 1							-				
-	Ŧ			1 1			1													1 1							Ł				
176.7	23.5	10	28 72									1	176.2					24.0		1 1							_				
175	Ŧ	"	20 / 2	1					00/0.9					Granite/Gr	VEATHERI neiss. grav-		v sliaht to			1 1							_				
474.7	†			.	- 1			.	[severe wea	athering, so fracture s	oft to hard,	very close			+							-				
170	28.5	82	18/0.1						00/0.6						REC=239					1 1							-				
-	Ŧ			1 1			1								RQD=0% GSI = 1	6 (0.0')				1 1							Ł				
166.7	33.5	38	22/0.2												001 - 1	10-20				1 1							_				
165	Ŧ	36	52/0.5	1			+	10	00/0.8																		_				
404.7	38.5						1	.																			_				
160	38.5	60/0.1						۱ ^	 60/0.1											‡							-				
	Ŧ			11																‡							-				
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155	<u> </u>						+	_			1								_	‡							ţ				
	‡			-			1	.	[]		[9	7777	‡							F				
150	<u> </u>										[ļ		‡							ļ.				
	‡			11							[147 5					52.7	2	‡							ļ.				
	‡			1			'	'	•	1				Boring Terr	minated du	e to core bi	t breaking			‡							F				
-	‡										[on at Lieva	(Granite/	Gneiss)	5,54 I (OOK	. [‡							F				
	‡													Hard drill	ling encour	ntered at 20).5 feet.	Ğ	200	‡							F				
_	‡										1								2	‡							F				
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GEOTECHNICAL BORING REPORT CORE LOG

		BURE LUG				ORE LUG	
WBS 46032.1.1		NTY WAKE	GEOLOGIST Moore, N. O.	WBS 46032.1.1		Y WAKE	GEOLOGIST Moore, N. O.
SITE DESCRIPTION Bridge No.	126 on -L- (Ligon Mill Road) Ove	er Smith Creek Between Main Stre	eet and US 401 GROUND WTR (ft)	SITE DESCRIPTION Bridge No. 12	26 on -L- (Ligon Mill Road) Over Sr	mith Creek Between Main Stre	eet and US 401 GROUND WTR (1
BORING NO. B2-A1	STATION 20+27	OFFSET 24 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B2-A1	STATION 20+27	OFFSET 24 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 200.3 ft	TOTAL DEPTH 64.9 ft	NORTHING 793,417	EASTING 2,139,903 24 HR . 3.4	COLLAR ELEV. 200.3 ft	TOTAL DEPTH 64.9 ft	NORTHING 793,417	EASTING 2,139,903 24 HR. 3.4
DRILL RIG/HAMMER EFF./DATE RFC	00074 CME-55 92% 10/12/2020	DRILL METHOD N	W Casing w/ Core HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE RFOOD	074 CME-55 92% 10/12/2020	DRILL METHOD N	W Casing w/ Core HAMMER TYPE Automatic
DRILLER Walker, C. M.	START DATE 05/27/21	COMP. DATE 06/01/21	SURFACE WATER DEPTH N/A	DRILLER Walker, C. M.	START DATE 05/27/21	COMP. DATE 06/01/21	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COU	NT BLOWS PER FO	DOT SAMP.	SOIL AND ROCK DESCRIPTION	CORE SIZE NQ	TOTAL RUN 11.8 ft		-
(ft) ELEV (ft) 0.5ft 0.5ft	0.5ft 0 25 50		SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV RUN DEPTH RUN DRILL	RUN SAMP. STRATA	L	DESCRIPTION AND DESMARKS
				ELEV RUN ELEV (ft) DEPTH RUN RATE (Min/ft)		O G ELEV. (ft)	DESCRIPTION AND REMARKS DEPTH
205				147.2			Begin Coring @ 53.1 ft
 				147.2	6 (2.6) (2.6) RS-1 (11.4) (8.9) 97% 75%	147.2 Granite/Gneiss bla	CRYSTALLINE ROCK ack, gray and orange, slight weathering, moderately hard
				145 144.6 55.7 02:39/1.0 - 4.2 01:59/1.0 02:04/1.0	0 (4.2) (2.8)	to I	hard, moderate to close fracture spacing
200			200.3 GROUND SURFACE 0.0	+ 101:29/1.0	0 100% 67% RS-2 RS-2		RS-1: 53.1'-54.1'
		- -	SEE B2-A FOR SOIL DESCRIPTIONS	1 140 4 1 50 0 1 101.15/1/	0 2 (4.6) (3.5)		RS-2: 57.7-58.1' GSI = 55-65
195				140 140.4 + 59.9 5.0 01:301.1	92% 70%	to I	Qu =
+		 	- -	135.4 + 64.9 01:30/1.0 00:55/1.0 01:29/1.0		135.4	٥
				100.4 04.9 01:29/1.0		Boring Terminated	at Elevation 135.4 ft In Crystalline Rock (Granite/Gneiss)
190			<u>-</u>				Hard drilling encountered at 21.2 feet.
						<u> </u>	
						F	
185			_	‡			
			-				
180							
] 32772	179.1 21.2				
			(Granite/Gneiss)			-	
175			-			_	
			-			<u> </u>	
		· · · · · ·					
$\frac{170}{1}$			_			<u> </u>	
±			-	‡			
165			-				
‡						-	
160			_				
155			-				
			- -	5			
152.2 48.1		1 1 PF///1	- -			-	
150		. 100/0.47	- 				
			L 1472			<u> </u>	
		RS-1	147.2 53.1 CRYSTALLINE ROCK			<u> </u>	
			Granite/Gneiss, black, gray and orange, slight weathering, moderately hard to hard,			[
			moderate to close fracture spacing				
140			REC=97% (11.4') RQD=75% (8.9')				
			- GSI = 55-65				
						-	
			Boring Terminated at Elevation 135.4 ft In				
			Crystalline Rock (Granite/Gneiss)			<u> </u>	
			Hard drilling encountered at 21.2 feet.				
			 - -			 -	



		<u> </u>	<u> </u>	
VBS 46032.1.1	TIP B-5318	COUNTY WAKE	GEOLOGIST Wescott	, F.
ITE DESCRIPTION Brid	No. 126 on -L- (Ligon M	ill Road) Over Smith Creek	Between Main Street and US 401	GROUND WTR (ft)
ORING NO. EB2-B	STATION 20+7	OFFSET 8	B ft RT ALIGNMENT -L-	0 HR. N/A
OLLAR ELEV. 199.8 ft	TOTAL DEPTH	31.0 ft NORTHING	EASTING 2,139,899	24 HR. FIAD
RILL RIG/HAMMER EFF./DA	BRI8284 CME-45C 91% 2/2	6/2018	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
RILLER Eister, G	START DATE	6/15/18 COMP. DA	TE 06/15/18 SURFACE WATER DE	PTH N/A
LEV DRIVE DEPTH BLO	COUNT B	LOWS PER FOOT	SAMP. V L	ACK DECODIDATION
(ft) ELEV (ft) 0.5ft	.5ft 0.5ft 0 25	50 75 100	NO. MOI G ELEV. (ft)	OCK DESCRIPTION DEPTH (ft
				·
200			199.8 GROUN	ID SURFACE 0.
198.8 = 1.0	1 2		I ∴ AL	LUVIAL
196.3 + 3.5	$\begin{vmatrix} 1 & 2 & & & & & & & & & $		fine to medium SAN	brown, tan and gray, silty ID (A-2-4) with trace clay,
95	1 1		SS-146 29% with little	rock fragments
193.8 + 6.0	2 2 2		Sat. Sat.	
191.3 + 8.5	$\frac{1}{3}$ $\frac{1}{2}$ $\frac{1}$		191.8 Loose, gray, fine	to coarse SAND (A-3)
$\frac{90}{1}$ $\frac{1}{2}$	⁵ ⁶ • • • • • • • • • • • • • • • • • • •	 	Sat.	
			186.8	13.0
85 186.3 13.5	21 18		RE	SIDUAL m dense, gray, slightly
□		<i>.</i>	micaceous, fine to o	coarse SAND (A-1-b) with
181.3 + 18.5	:::://.		some ro	ск тадтеліз
80 10	9 12		Sat. 000	
			000-	
75 176.3 + 23.5 79	/0.1 · · · · · · · ·	<u> </u>		ERED ROCK
$\frac{75}{1}$ $\frac{1}{1}$ $\frac{79}{1}$	70.1	100/0.6		ite/Gneiss)
+				
70 171.3 + 28.5 100/0.				
-			168.8	31.0 nated with Standard
			Elevation 168.8	st Tricone Refusal at ft On Weathered Rock ite/Gneiss)

SHEET 12

CORE PHOTOGRAPHS

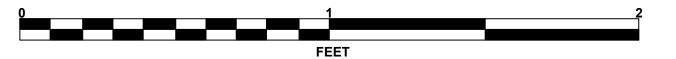
B2-ABOX 1: 38.6 - 52.7 FEET



B2-A1BOXES 1 & 2: 53.1 - 65.7 FEET







PROJECT REFERENCE NO.	SHEET NO.
B-5318	14

	SOIL TEST RESULTS														
SAMPLE	OFFSET	STATION	AASHTO	1 1	P.I.		% BY \	WEIGHT		% PASSING (SIEVES)			%	%	
NO.	OFFSEI	STATION	INTERVAL	CLASS.	L.L.	F.1.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS- 129	21' LT	18 +7 3	6.0-7.5	A- 1- b	-	NP	82.9	8.5	3. 1	5. 5	86	28	10	23.8	-
SS-71	25′ RT	18 +7 3	<i>3</i> . 5- 5. 0	A-6(6)	37	16	9.5	42.4	21.0	27.1	99	96	5 <i>4</i>	28.4	-
SS- 109	25′ LT	19+68	23. 5- 25. 0	A-2-4	-	NP	56.5	25.8	11.2	6.5	85	60	21	18.3	-
SS- 98	19′ RT	19+68	23. 5- 25. 0	A-2-4	-	NP	49.5	29.7	14.5	6.3	89	65	25	19.0	-
SS- 140	29′ LT	20 +7 3	8.5-10.0	A- 3	-	NP	57.4	36.4	3. 4	2.8	97	68	8	34.9	-
SS- 146	8′ RT	20 +7 3	3. 5- 5. 0	A-2-4	-	NP	48.8	28.8	9.6	12.7	98	64	25	29.0	-



PROJECT NO.: 43032.1.1 PROJECT ID: B-5318

REPORT ON SAMPLES OF: ROCK QUALITY

PROJECT DESCRIPTION: BRIDGE NO. 126 ON SR 2044 (LIGON MILL ROAD) OVER SMITH CREEK

DATE SAMPLED: 6/8/2021 COUNTY: WAKE

SUBMITTED BY: N.O. MOORE

BORING NO.	SAMPLE NO.	DEPTH (FT)	ROCK TYPE	GEOLOGIC MAP UNIT	LENGTH (IN)	DIAMETER (IN)	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH (PSI)	YOUNG'S MODULUS (PSI)	SPLITTING TENSILE STRENGTH (PSI)	REMARKS
B2-A1	RS-1	53.1-54.2	Injected Gneiss	CZig	13	1.86	164.2	29,700	-	-	-
B2-A1	RS-2	57.8-58.2	Injected Gneiss	CZig	5	1.86	154.2	6,430	-	-	-

SITE PHOTOGRAPHS BRIDGE NO. 126 OVER SMITH CREEK ON SR 2044



View of SR 2044 looking south.



View of Smith Creek looking west.