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H	
R	

CONTENTS

DESCRIPTION

SUPPLEMENTAL LEGEND (GSI)

BORE LOG(S) & CORE REPORT(S)

TITLE SHEET LEGEND (SOIL & ROCK)

SITE PLAN

PROFILE(S)

CROSS SECTION(S)

ROCK TEST RESULTS

CORE PHOTOGRAPH(S)

SITE PHOTOGRAPH(S)

SHEET NO.

2Α

4-5

6-8

9-15

16

17-18

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY NASH

PROJECT DESCRIPTION BRIDGE NO. 29 ON -L-(US 64 ALT.) OVER TAR RIVER AT STA. 16+98 STATE PROJECT REFERENCE NO. 19 B-5670

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-8850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE OR INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

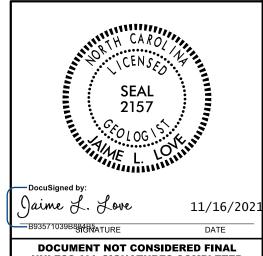
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED OF AN PREASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL N. O. MOORE A. N. KINTNER D. G. PINTER J. E. DEAN INVESTIGATED BY J. L. LOVE DRAWN BY J. L. LOVE CHECKED BY _N. T. ROBERSON SUBMITTED BY N. T. ROBERSON DATE _AUGUST 2019



UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUYIUM (ALLUY.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	SU//2SU//A	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED VILLE NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERANC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE CRYSTA	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
ULASS. (≤ 35% PASSING =200) (> 35% PASSING =200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1- A-1- A-1- A-2- A-2-5 A-2-6 A-2-7 A-1- A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7. PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
■10 50 MX GRANULAR SIL1- MUCK,	PERCENTAGE OF MATERIAL	CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 30 MX 50 MX 51 MN PEAT SOILS SOILS SOILS SOILS SOILS	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING #40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL 48 MX 41 MN 48 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN LITTLE OR LITTLE OR MODERATE HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOULS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
CEN PATING.		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	<u> </u>	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD RANGE OF UNCONFINED COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION WITH SOUL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-YHLOE) (TONS/FT)	SPI SOL SESSION TON SPI	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE	SOIL SYMBOL Opt ont test boring SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A ALLINIAL COLL POLINDARY A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTALLATION SPIN-VALUE	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND (SL) (CL)		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(LSE, SU.) (F SU.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
COLL MOISTURE COALE FIELD MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) OBSCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PL PLASTIC LIMIT	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	BENCH MARK: BL-105, PIN & CAP AT -L- STA. 18+74, 23' LT
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 170.84 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
PEGUIDES ADDITIONAL MATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
- DRY - (D) ATTAIN OPTIMUM MOISTURE	X CME-55 6 CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	TOP OF RAIL @ EBI-L- STA. 15+34, 15' RT ELEV. = 174.5'
PLASTICITY	X 8* HOLLOW AUGERS	INDURATION	TOP OF RAIL @ EB2 -L- STA. 18+52, 15' RT ELEV. = 174.5'
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS X-N XWL	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	ELEV. = 1/4.5'
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST X TUNGCARBIDE INSERTS HAND TOOLS:	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING X W/ ADVANCER POST HOLE DIGGER	CDAING CAN BE CEDADATED EDOM CAMBLE WITH CTEEL DROPE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED ORALING CHIN BE SCHARLED FRUIT SHIFTLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	X Water Truck Truck Sounding ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER,	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST	SUADD HAMMED DIDN'S DECILIBED TO DDEAK SAMPLE.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-

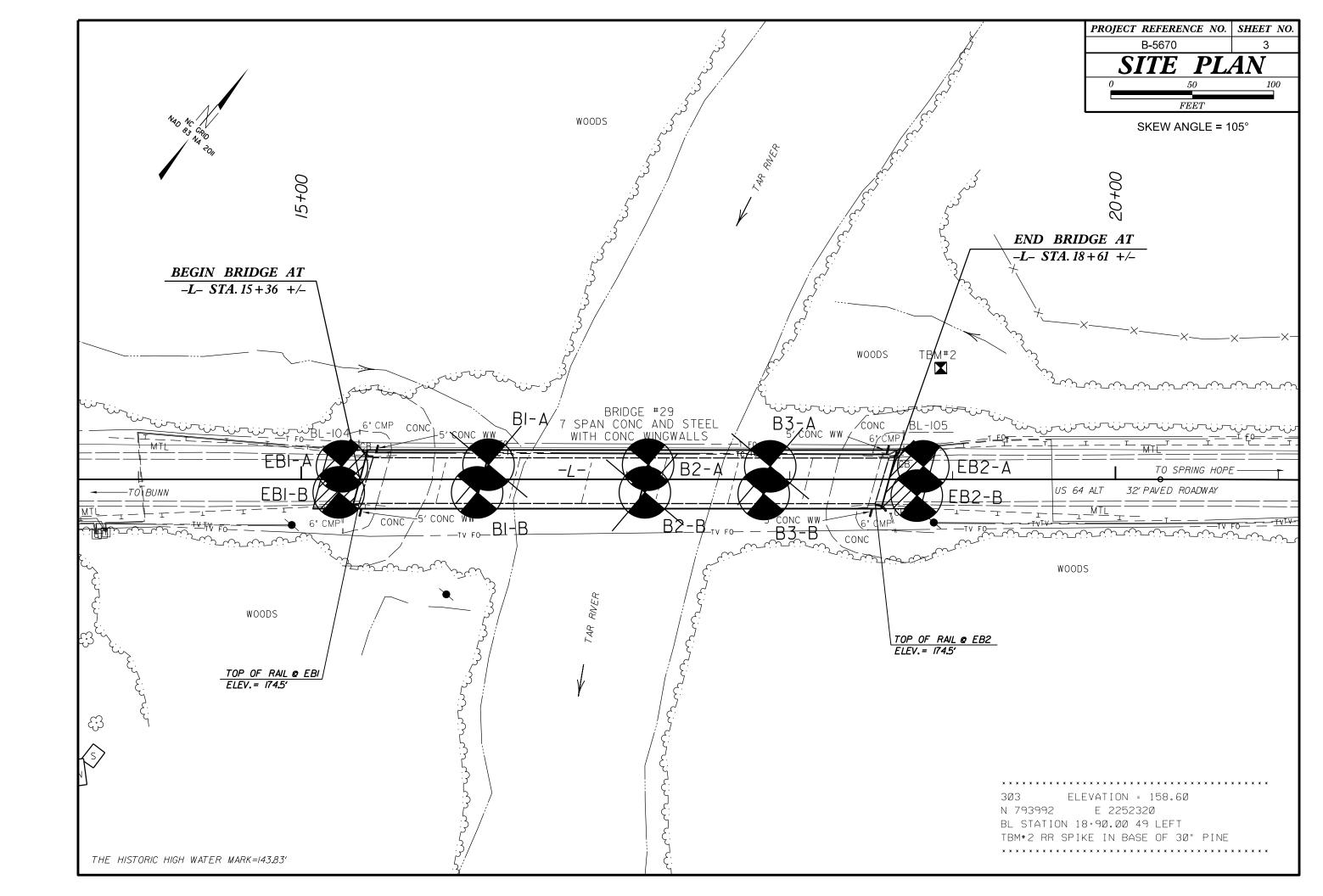
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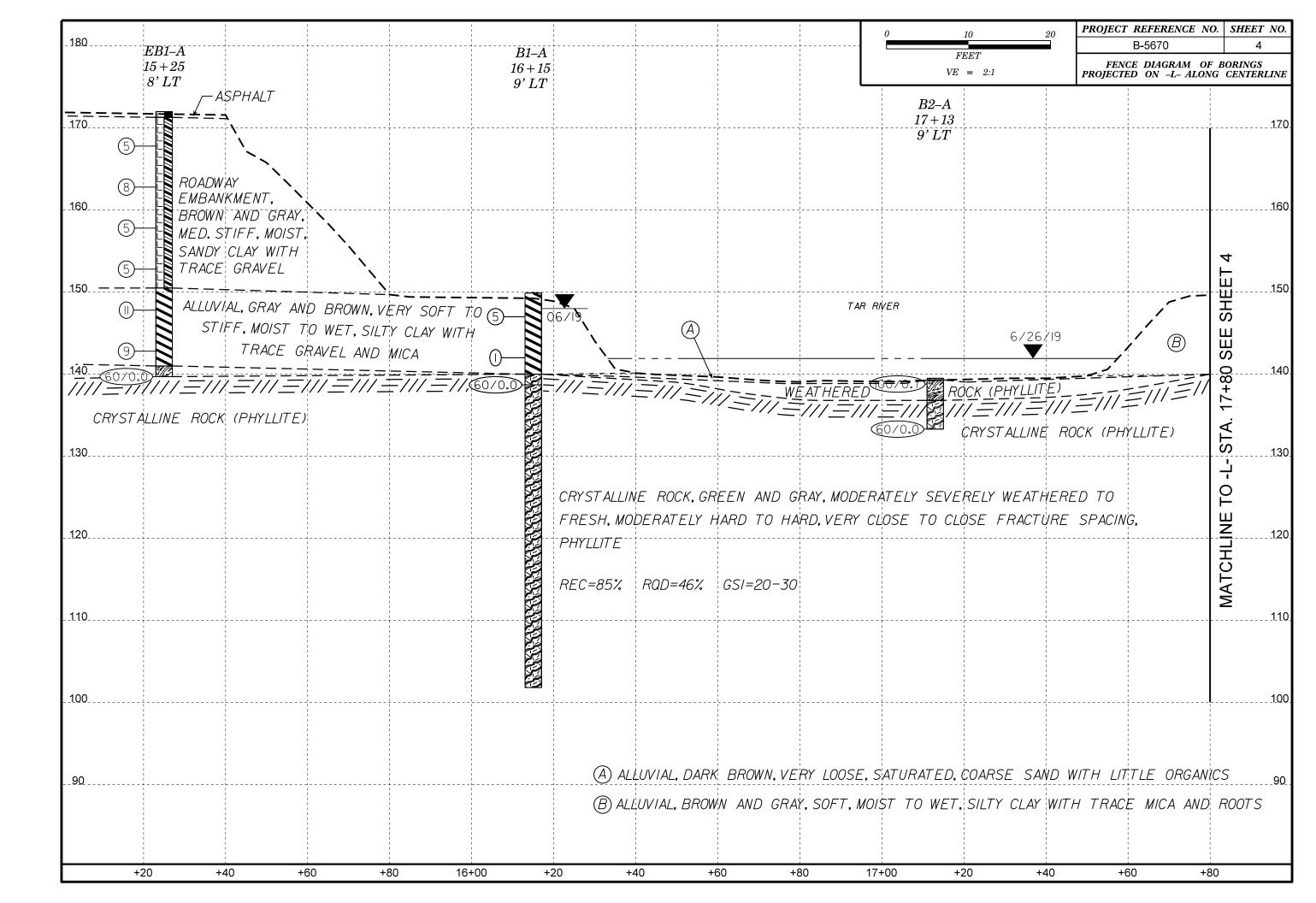
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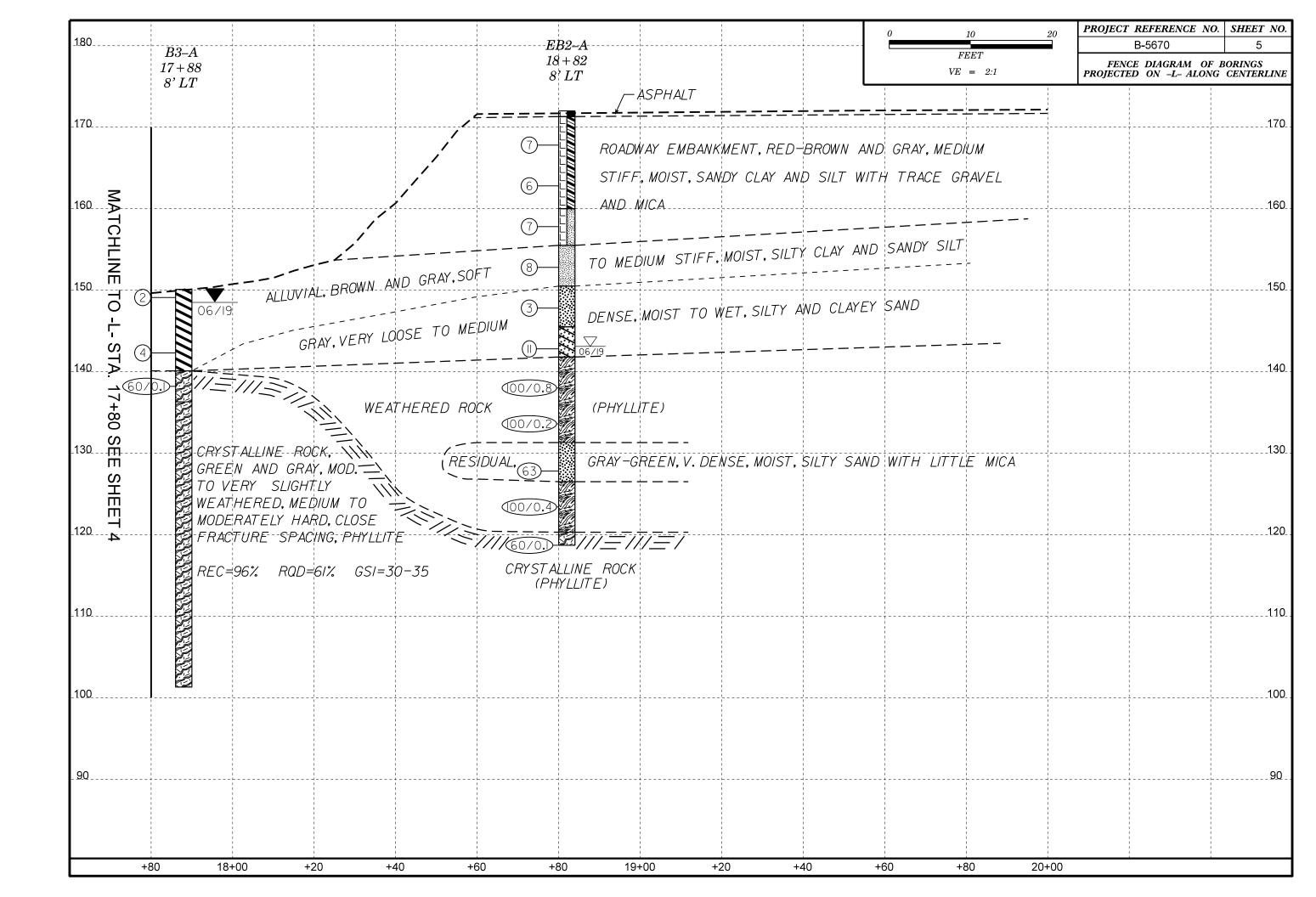
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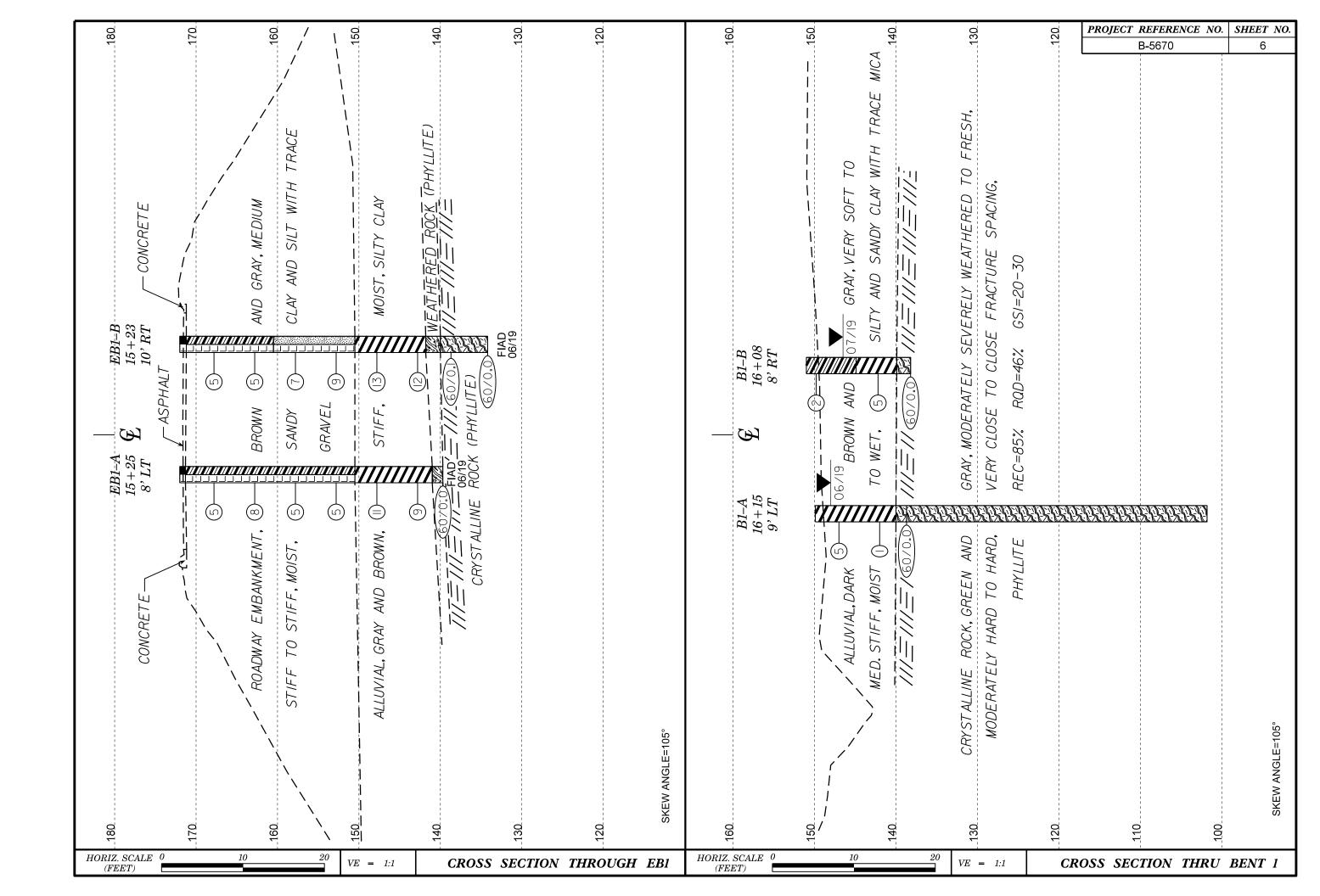
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos, P and Hoek E., 2000) From a description of the lithology, structure and From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to POOR - Very smooth, slicken-l or highly weathered surfaces soft clay coatings or fillings athered surf or fillings and surface conditions (particularly of the bedding smooth, occasionally surfaces with compar fillings with angular planes), choose a box in the chart. Locate the ed s be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not planes) 0 position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too ed, apply to structurally controlled failures. Where weak planar structural planes are precise. Quoting a range from 33 to 37 is more weather realistic than giving GSI = 35. Note that the slightly present in an unfavorable orientation SURFACE CONDITIONS (DISCONTINUITIES (Predominantly beddir highly coating Hoek-Brown criterion does not apply to structurally with respect to the excavation face, CONDITIONS these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, POOR kensided, hig soft clay d Smooth, ed and in rocks that are prone to deterioration slightly es these will dominate the behaviour of the rock mass. Rough, POOR Slickensided, h with compact or angular fra as a result of changes in moisture content will be reduced if water is POOR - Very s slickensided coatings or f fragments The strength of some rock masses is reduced by the 1 0 G000 rough, G00D thered presence of groundwater and this can be allowed for present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. by a slight shift to the right in the columns for fair, SURFACE th, r GOOD Rough, s surface poor and very poor conditions. Water pressure does VERY I sided with s VERY I FAIR -VERY Very VERY Slicke With FAIR Smoo-alter Water pressure is dealt with by effective not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. COMPOSITION AND STRUCTURE STRUCTURE DECREASING SURFACE QUALITY -INTACT OR MASSIVE - intact A. Thick bedded, very blocky sandstone
The effect of pelitic coatings on the bedding .90 rock specimens or massive in N/A N/A situ rock with few widely spaced planes is minimized by the confinement of discontinuities the rock mass. In shallow tunnels or slopes PIECE these bedding planes may cause structurally controlled instability. 60 BLOCKY - well interlocked un-70[′] disturbed rock mass consisting of cubical blocks formed by three B. Sand-stone wi 50 intersecting discontinuity sets C. Sand D. Siltstone E. Weak 60 or silty shale siltstone stone with stone and siltstone with sandor clayey С thin inter layers of ın sımılar stone layers shale with VERY BLOCKY - interlocked. OCKING sands tone siltstone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets INTERL C. D. E. and G - may be more or F. Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but ntensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and ith broken and deformed CREASING discontinuity sets. Persistence loss of continuity moves these sandstone layers forming an 30 categories to F and H. of bedding planes or schistosity almost chaotic structure 20 DISINTEGRATED - poorly interlocked, heavily broken rock mass 20 **G.** Undisturbed silty H. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a 10 rounded rock pieces or without a few very chaotic structure with pockets of clay. Thin layers of thin sandstone layers sandstone are transformed nto small rock pieces. 10 LAMINATED/SHEARED - Lack of blockiness due to close spacing N/A N/A → Means deformation after tectonic disturbance of weak schistosity or shear planes DATE: 8-19-16

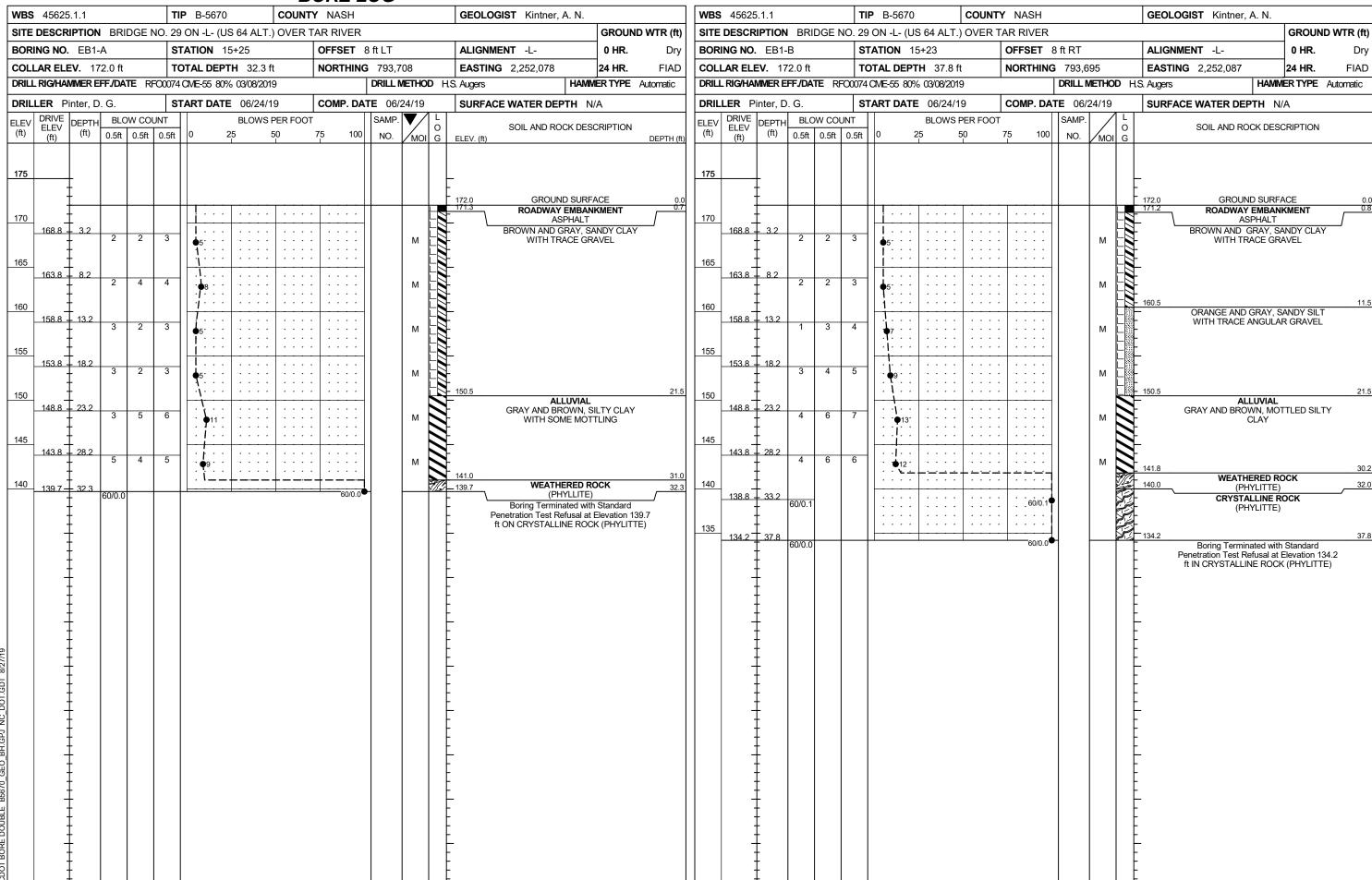








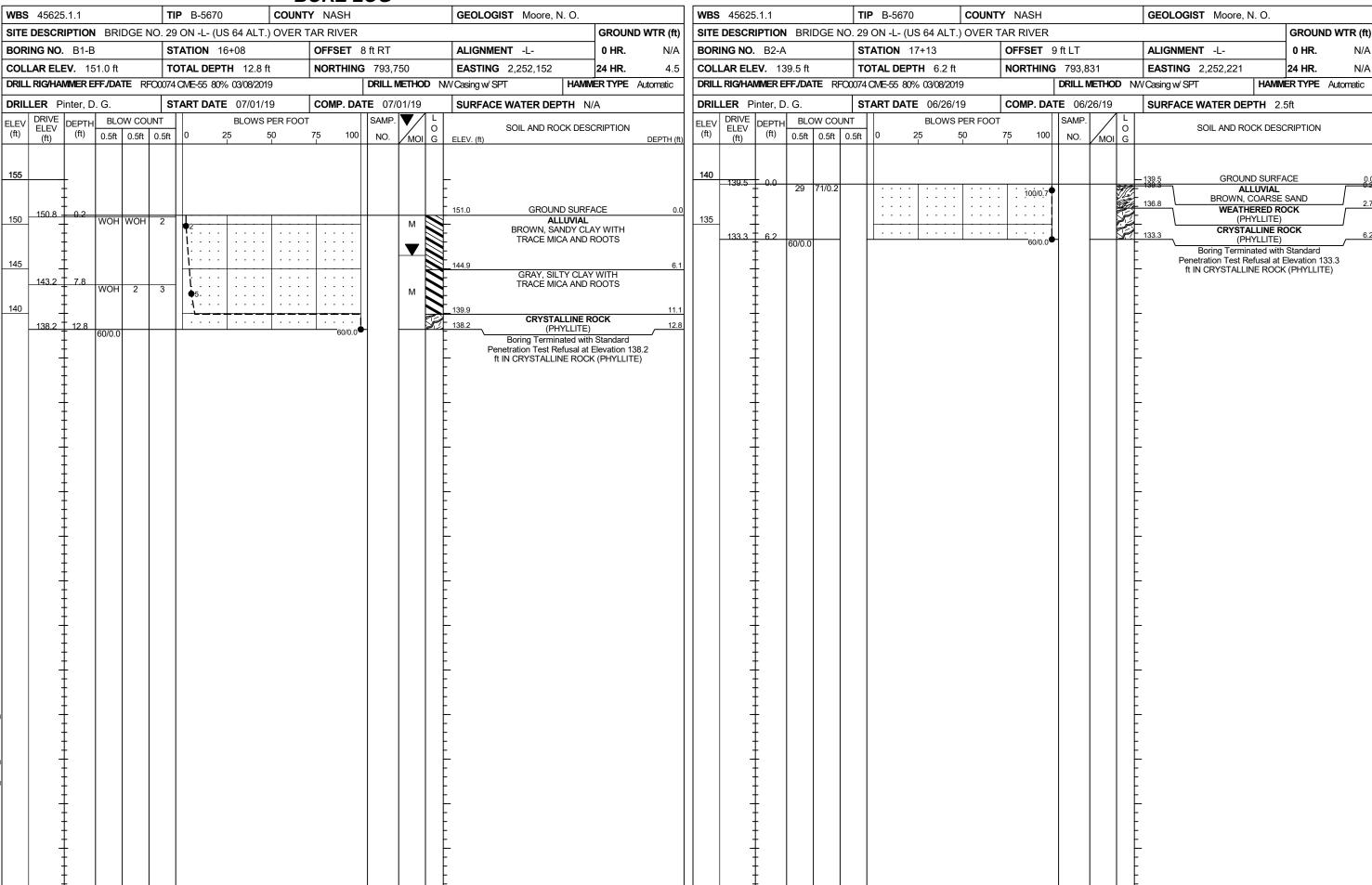
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	130	
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WBS 45625.1.1 **TIP** B-5670 **COUNTY** NASH GEOLOGIST Moore, N. O. SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER GROUND WTR (ft) ALIGNMENT -L-**STATION** 16+15 OFFSET 9 ft LT BORING NO. B1-A 0 HR. N/A COLLAR ELEV. 149.9 ft TOTAL DEPTH 48.1 ft **NORTHING** 793,767 **EASTING** 2,252,146 24 HR. 1.9 **DRILL RIG/HAMMER EFF./DATE** RF00074 CWE-55 80% 03/08/2019 **DRILL METHOD** NW Casing W/SPT & Core **HAMMER TYPE** Automatic DRILLER Pinter, D. G. **START DATE** 06/25/19 **COMP. DATE** 06/26/19 SURFACE WATER DEPTH N/A SAMP. **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G ELEV. (ft) DEPTH (ft 150 **GROUND SURFACE** ALLUVIAL • DARK BROWN, SILTY CLAY 148.0 1.9 WOH WITH TRACE MICA AND GRAVEL 145 143.0 I 6.9 woн woн Μ 140 CRYSTALLINE ROCK 138.7 + 11.2 60/0.0 GREEN AND GRAY MODERATELY SEVERELY WEATHERED TO FRESH, 135 MODERATELY HARD TO HARD, VERY RS-1 CLOSE TO CLOSE FRACTURE SPACING, PHYLLITE REC=85% RQD=46% GSI=20-30 130 125 120 RS-2 115 110 105 Boring Terminated at Elevation 101.8 ft IN CRYSTALLINE ROCK (PHYLLITE)

GEOTECHNICAL BORING REPORT CORE LOG

									<u>C</u>	<u>OI</u>	RE LOG					
WBS	45625	5.1.1			TIP	B-567	' 0	C	OUNT	Y 1	ASH	GEOLOGI	ST Moore,	N. O.		
SITE	DESCR	RIPTION	I BRI	DGE NO	. 29 O	N -L- (US 64 AI	_T.) O	VER 1	ΓAR	RIVER				GROUN	D WTR (ft)
BOR	ORING NO. B1-A STATION					ΓΙΟΝ	16+15			OF	SET 9ftLT	ALIGNMENT -L-			0 HR.	N/A
	LAR ELI				TOTAL DEPTH 48.1 ft NORTHING 793,767						·		2,252,146		24 HR.	1.9
DRILI	L RIG/HA	MMER E	FF./DA	TE RFOO	074 CN	1E -55 8	03/08/	2019			DRILL METHOD NV	V Casing W/SP	T & Core	HAMIV	ER TYPE	Automatic
DRIL	LER P	inter, D). G.		STAI	RT DA	TE 06/2	5/19		CC	MP. DATE 06/26/19	SURFACE	WATER DE	PTH N	/A	
COR	E SIZE	NXWL	-		TOTA	AL RU	N 36.9 f	t								
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RATA RQD (ft) %	L O G	ELEV. (ft)	DESCRIPTION	AND REMAR	KS		DEPTH (ft)
138.7		44.0										Begin Cori	ng @ 11.2 ft			
135	138.7 .	13.2	5.0	N=60/0.0 1:31/1.0 2:42/1.0 1:55/1.0 1:47/1.0 1:46/1.0 1:18/1.0	(1.7) 85% (5.0) 100%	(1.4) 70% (2.4) 48%	RS-1	(31.2) 85%	(17.1) 46%		. 138.7 GREEN AND GRAY MODERATELY HAR	RD TO HARD, ' SPACING				
130	131.7 .	-	5.0	1:34/1.0 1:17/1.0 1:24/1.0 1:31/1.0 1:35/1.0 1:28/1.0	(5.0) 100%	(3.1) 62%					-					
125	121.7	28.2	5.0	1:35/1.0 2:06/1.0 2:25/1.0 2:08/1.0 2:22/1.0 1:45/1.0	(5.0) 100% (5.0)	(1.9) 38% (2.2)					: - - - -					
120 115	116.7	33.2	4.5	2:02/1.0 2:12/1.0 2:07/1.0 1:50/1.0 1:32/1.0	100%	(1.8)	RS-2				- - -					
110	117:3	37. <u>7</u> - 38.5	0.8	1:43/1.0 1:40/1.0 6:31/1.0 3:45/0.5 -/0.8	93% (0.8) (100%)	(0.8)					-					
105	106.8	43.1	5.0	1:30/1.0 1:34/1.0 1:20/1.0 1:34/1.0 0:35/0.6 1:14/1.0 1:26/1.0 1:08/1.0	(3.5) 76% (1.0) 20%	(2.9) 63% (0.6) 12%										
	101.8	48.1		1:08/1:0 1:22/1.0 1:11/1.0	2070	1270					. 101.8 Boring Terminat		n 101.8 ft IN CF YLLITE)	RYSTALLI	NE ROCK	48.1
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GEOTECHNICAL BORING REPORT

GEOTECHNICAL BORING REPORT **BORE LOG CORE LOG**

WDC 45005 4.4		V NACII	CEOLOGIST Massa N. O.	WDC 45005 4.4		TV NACH	CEOLOGIST Maara N. C.
		Y NASH	GEOLOGIST Moore, N. O. GROUND WTR (ft)	WBS 45625.1.1 SITE DESCRIPTION BRIDGE NO		TY NASH	GEOLOGIST Moore, N. O. GROUND WTR (ft)
SITE DESCRIPTION BRIDGE NO. 29 BORING NO. B2-B ST	ATION 17+11	OFFSET 8 ft RT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B2-B	STATION 17+11	OFFSET 8 ft RT	ALIGNMENT -L- 0 HR. N/A
	OTAL DEPTH 36.7 ft					NORTHING 793,816	
COLLAR ELEV. 139.7 ft TO DRILL RIG/HAMMER EFF./DATE RF000740		NORTHING 793,816	WCasing WSPT & Core HAMMER TYPE Automatic	COLLAR ELEV. 139.7 ft DRILL RIG/HAMMER EFF./DATE RFC	TOTAL DEPTH 36.7 ft	· ·	WCasing WSPT & Core HAMMER TYPE Automatic
		ļ				1	
	TART DATE 07/02/19 BLOWS PER FOOT	COMP. DATE	SURFACE WATER DEPTH 2.0ft	DRILLER Pinter, D. G. CORE SIZE NXWL	START DATE 07/02/19 TOTAL RUN 28.5 ft	COMP. DATE 07/02/19	SURFACE WATER DEPTH 2.0ft
ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUNT (ft) 0.5ft 0.5ft 0.5ft		75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV RUN DEDTH BLIN DRILL			
(1.9)		▼ William	, ,	ELEV ELEV (ft) DEPTH RUN RATE (Min/ft)	RUN STRATA REC. RQD (ft) (ft) (ft) NO. (ft) (O O G ELEV. (ft)	DESCRIPTION AND REMARKS DEPTH (ft)
140			WATER SURFACE (07/02/19)	131.5	70 70 70 70 70	ELEV. (III)	Begin Coring @ 8.2 ft
139.5 0.2 90 10/0.1		100/0.6	139.7 GROUND SURFACE 0.0	131.5 + 8.2 4.8 1:03/1.0 N=60/0) (4.4) (2.6) (25.9) (18.	7) 131.5 GREEN AND GRA MODERATEL	Y, MODERATELY TO VERY SLIGHTLY WEATHERED, 8.2 Y HARD TO HARD, CLOSE FRACTURE SPACING,
		. 109/0:0	BROWN, COARSE SAND WEATHERED ROCK 2.7	1:03/7.0	0 (4.4) (2.6) (25.9) (18. 1 92% 54% 92% 67%	WODERATEL	PHYLLITE
135			CRYSTALLINE ROCK	126.7 13.0 1:17/1.0 1:19/1.0 1 4.7 0:56/0.8	3/ (3.7) (1.8)		GSI=25-30
			(PHYLLITE)	125 T 4.7 (0.36/0.8 1:03/1.0 1:19/1.0	(1.8) (3.7) (1.8) (79% 38% RS-3		
130 + 8.2 60/0.1		60/0.1	- 131.5 8.2 - GREEN AND GRAY, MODERATELY	1:26/1:0 122.0 17.7 1:26/1:0 2:17/1.0			
 			SEVERELY WEATHERED TO FRESH, MODERATELY HARD TO HARD, VERY	120 ± 5.0 (2:17/0.7 2:28/1.0 2:17/1.0	(4.7) (3.1) 94% 62%		
			CLOSE TO CLOSE FRACTURE SPACING, PHYLLITE	122.0 17.7 2.17/1.0 2.17/1.0 2.17/1.0 2.17/1.0 2.17/1.0 2.17/1.0 2.17/1.0 1.126/1.0 1.14/1.0			
125		RS-3	- - REC=92% RQD=66% GSI=25-30	117.0 1 22.7 1 1:34/1.0	0 (4.8) (4.0) 0 96% 80%		
			-	0:57/1.0	0 96% 80%	MODERATEL* 103.4 103.0 GREEN AND G	
120			-	112.0 27.7 1:29/1.0 1:30/1.0			
 +			-	110 5.0 1:15/1.0 1:18/1.0	0 (4.9) (4.0) 0 98% 80%		
			-	1:30/1.0			
115			- -	107.0	0 (3.8) (3.2) 0 95% 80%		
			-	0:54/1.0	95% 80% RS-4	103.4	36.3
110 7			-	103.0	(0.4)	103.0 GREEN AND G	WEATHERED ROCK GRAY, SEVERELY WEATHERED, SOFT, PHYLLITE
		RS-3 RS-4 RS-4	-		100 /6		nated at Elevation 103.0 ft IN WEATHERED ROCK
‡			-	‡			(PHYLLITE)
105		RS-4	-				
				‡			
			GREEN AND GRAY, SEVERELY WEATHERED, SOFT, PHYLLITE				
			- REC=100%				
			Boring Terminated at Elevation 103.0 ft IN WEATHERED ROCK (PHYLLITE)	‡		-	
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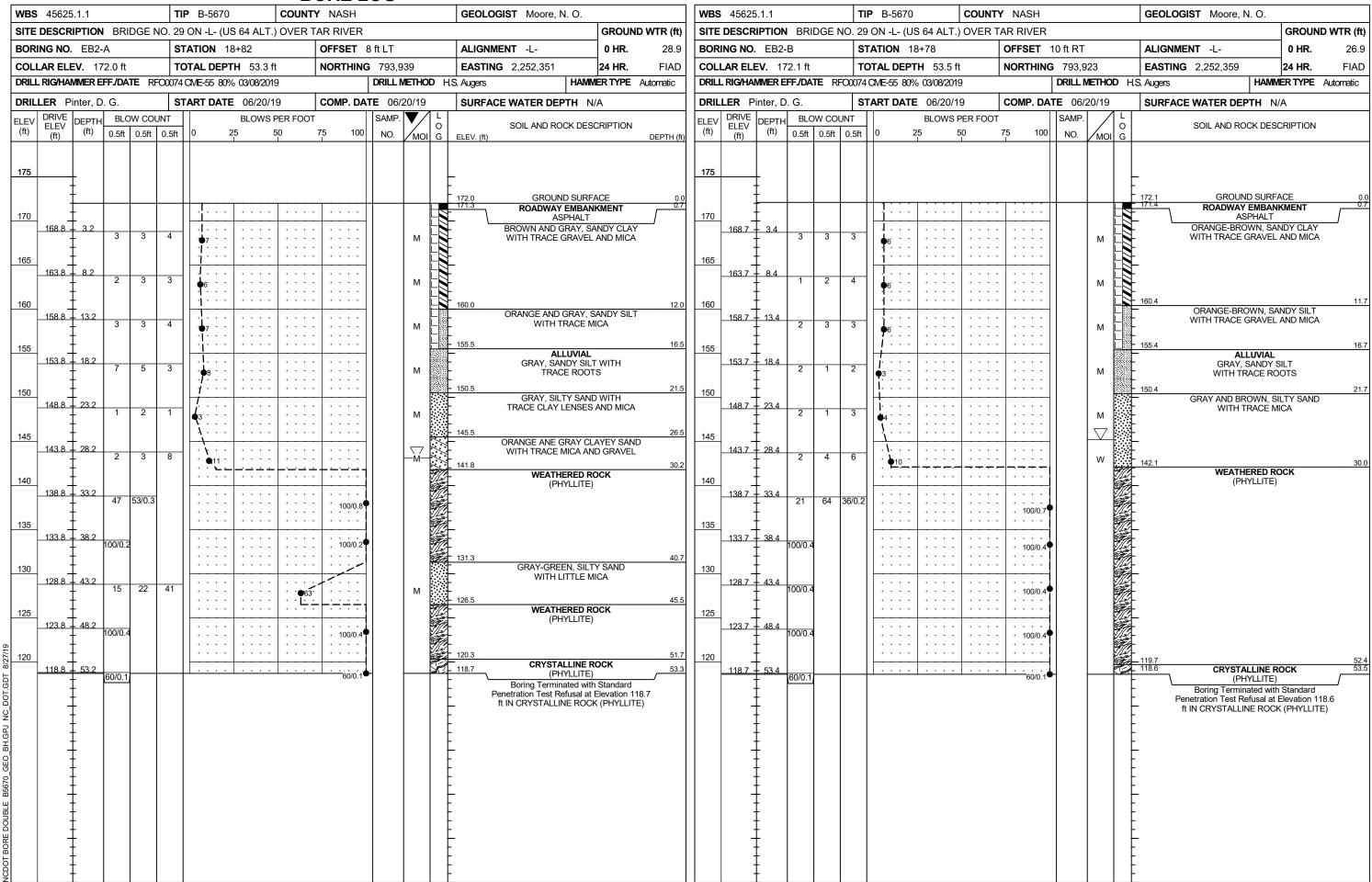
GEOTECHNICAL BORING REPORT GEOTECHNICAL BORING REPORT **BORE LOG CORE LOG**

WDC 45605 1.1		TV NACH	GEOLOGIST Maara N. O.	WPC AFGOE 4.4		TV NACH	GEOLOGIST Meers N. O.
WBS 45625.1.1 SITE DESCRIPTION BRIDGE NO		TY NASH	GEOLOGIST Moore, N. O. GROUND WTR (ft)	WBS 45625.1.1 SITE DESCRIPTION BRIDGE N		TY NASH	GEOLOGIST Moore, N. O. GROUND WTR (ft)
BORING NO. B3-A	STATION 17+88	OFFSET 8 ft LT	ALIGNMENT -L- GROUND WIR (II) 0 HR. N/A		STATION 17+88	OFFSET 8 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 150.1 ft DRILL RIG/HAMMER EFF./DATE RFC	TOTAL DEPTH 48.8 ft	NORTHING 793,878	EASTING 2,252,279 24 HR. 1.6 W Casing W/SPT & Core HAMMER TYPE Automatic	COLLAR ELEV. 150.1 ft DRILL RIG/HAMMER EFF./DATE RFG	TOTAL DEPTH 48.8 ft	NORTHING 793,878	EASTING 2,252,279 24 HR. 1.6 NW Casing W/SPT & Core HAMMER TYPE Automatic
	1				1		
DRILLER Pinter, D. G.	START DATE 06/27/19	COMP. DATE 06/27/19 ot SAMP.	SURFACE WATER DEPTH N/A	DRILLER Pinter, D. G.	START DATE 06/27/19	COMP. DATE 06/27/19	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COUNTY (ft) (ft) 0.5ft 0.5ft 0	I	75 400	SOIL AND ROCK DESCRIPTION	CORE SIZE NXWL	TOTAL RUN 35.0 ft		
(ft) (ft) (11) 0.5ft 0.5ft (5.511	75 100 NO. MOI G	ELEV. (ft) DEPTH (ft)	ELEV RUN DEPTH RUN RATE (ft) (ft) (ft) (Min/ft	RUN STRATA REC. RQD NO. (ft) (ft) (ft) (ft) (ft) (ft)	G ELEV. (ft)	DESCRIPTION AND REMARKS
455) % % % % %	G ELEV. (ft)	DEPTH (ft)
155			_	136.3 135 136.3 + 13.8 5.0 1:25/1.	0 (5.0) (2.4) (33.5) (21.4)	136.3 GREEN AND G MODERATEL	Begin Coring @ 13.8 ft GRAY,SLIGHTLY TO VERY SLIGHTLY WEATHERED, 13.8
				T 1:03/1.	0 100% 48% 96% 61%	MODERATEL	Y HARD TO HARD, CLOSE FRCATURE SPACING, PHYLLITE
150 150.1 - 0.0 WOH WOH			150.1 GROUND SURFACE 0.0	101.0 10.0 1.19/1.	0 0		GSI=30-35
		· · · · · · · · · · · · · · · · · · ·	ALLUVIAL BROWN AND GRAY, SILTY CLAY	1:42/1.	0 (4.9) (3.8) 0 98% 76%		
145			TRACE MICA AND ROOTS	1:42/1. 2:24/1. 126.3 7 23.8 2:11/1.	0 0 0 RS-5		
143.3 6.8		. 		125	0 (5.0) (4.0)		
143.3 + 0.8 1 2	2 4	: : : : : M			0 100% 80%		
140		· · · · · ·	140.1 10.0 CRYSTALLINE ROCK	2.00/1:	0 0 (5.0) (0.7)		
138.3 + 11.8 60/0.1		RS-5	(PHYLLITE)	1:46/1.	0 (5.0) (3.7) 0 100% 74%	101.3	
135			13.8 GREEN AND GRAY, SLIGHTLY TO VERY				
135 +			SLIGHTLY WEATHERED, MODERATELY HARD TO HARD, CLOSE FRCATURE	115	0 (4.4) (2.8)		
			SPACING, PHYLLITE	1:47/1.	0 88% 56%		
130			REC=96% RQD=61% GSI=30-35	1:18/1. 111.3	0 0 (4.0)		
				1 0:57/1.	0 (4.2) (1.1) 0 84% 22%		
125		RS-5		1:12/1. 1:17/1. 106.3 † 43.8 1:42/1.			
125			-	105	0 (5.0) (3.6)		
				1:20/1.	0 100% 72%		
120			_	101.3	0 RS-6		48.8
						Boring Termi	nated at Elevation 101.3 ft IN CRYSTALLINE ROCK (PHYLLITE)
115							
 113 + 			-			<u> </u>	
			-				
110			-				
			-			-	
105			-				
+			-			_	
		RS-6	- - - - - - 101.3 48.8				
8/28/10			Boring Terminated at Elevation 101.3 ft IN CRYSTALLINE ROCK (PHYLLITE)				
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WBS
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ELEV
(ft)
455
155
150
145
140

SHEET 14

WBS	45625	5.1.1			T	P E	3-5670		COUN	TY N	IASH				GEOLOGIST Moore, N. C	D.		
SITE	DESCR	IPTION	BRI	DGE I	NO. 29	ON	-L- (US	64 ALT	.) OVER	TAR	RIVER					G	ROUNE	WTR (ft)
BOR	ING NO.	В3-В			S	TATI	ON 17	'+84		OF	FSET	9 ft RT			ALIGNMENT -L-		HR.	N/A
	LAR ELE				- 1			H 12.2		NO	RTHIN	3 793,8			EASTING 2,252,287		4 HR.	4.7
DRILL	_ RIG/HAI	MMER E	FF./DA	TE R	FO0074	CME	-55 80%	03/08/20	19			DRILL	METHO	D N	V Casing w/ SPT H	IAMMER	TYPE	Automatic
DRIL	LER P	inter, D				TAR	T DATE	06/25/			MP. DA	TE 06/	26/19	<i>.</i>	SURFACE WATER DEPTH	l N/A		
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	UNT 0.5ft	0	2		PER FOO	75 	100	SAMP.	моі	O G	SOIL AND ROCK	DESCRI	PTION	
155		- - -												 - -	-			
150	150.5	0.0	WOH	2	2										150.5 GROUND S		<u> </u>	0.0
145	-	-	WOII	2	2	* *	4						M		BROWN AND GRA WITH TRACE MIC	Y, SAND	Y CLAY ROOTS	5.4
		- 7.1 -	WOH	WOH	WOH	0.							М		GRAY, SAN WITH TRACE MIC			
140	120 4	12.4				1			+==	- -					139.3 138.7 WEATHER	ED BOOK		11.2 11.8
	138.4	12.1	60/0.1			 	<u> </u>			- -	60/0.1				WEATHERE (PHYLL CRYSTALLI L Boring Terminated Penetration Test Refus ft IN CRYSTALLINE F	LITE) NE ROC LITE) d with Stated at Elever	K andard vation 13	11.8 12.2/ 8.3
															-			



PROJ. NO. - 45625.1.1 ID NO. - B-5670 COUNTY -NASH

<u> 31-A -L</u>

ROCK TEST RESULTS									
SAMPLE			DEPTH	ROCK	UNIT WT	UNCONFINED COMP.	SECTION MOD.		
NO.	OFFSET	STATION	INTERVAL	INTERVAL TYPE LB/FT ³ STRENGTH, KSI @ 40% MPSI					
RS-1	9 LT	16+15	15.4-16.0	PHYLLITE	172.3	2.44	2.4		
RS-2	9 LT	16+15	33.6-34.1	PHYLLITE	179.8	9.52	6.8		

SHEET 16

B2-B -L-

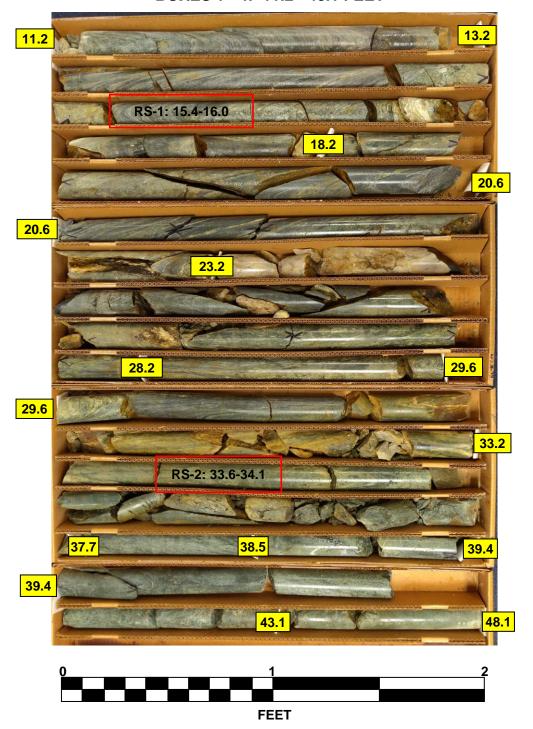
ROCK TEST RESULTS											
SAMPLE			DEPTH	ROCK	UNIT WT	UNCONFINED COMP.	SECTION MOD.				
NO.	OFFSET	STATION	INTERVAL	TYPE	LB/FT ³	STRENGTH, KSI	@ 40% MPSI				
RS-3	8 RT	17+11	14.4-15.0	PHYLLITE	CORE BROKE PRIOR TO TESTING						
RS-4	8 RT	17+11	34.6-35.3	PHYLLITE	168.4	5.78	10.78				

B3-A -L-

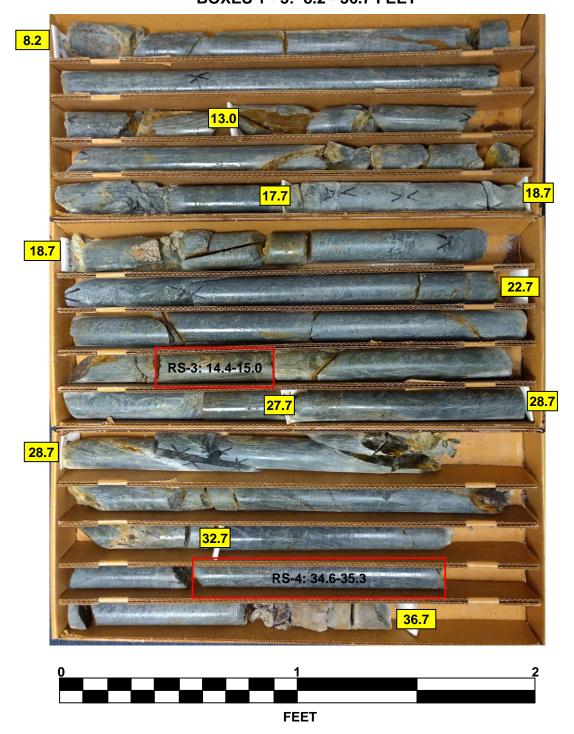
ROCK TEST RESULTS											
SAMPLE			DEPTH	ROCK	UNIT WT	UNCONFINED COMP.	SECTION MOD.				
NO.	OFFSET	STATION	INTERVAL	TYPE	LB/FT ³	STRENGTH, KSI	@ 40% MPSI				
RS-5	8 LT	17+88	22.6-23.1	PHYLLITE	169.6	16.2	-				
RS-6	8 LT	17+88	46.9-47.3	PHYLLITE	173.2	5.53	9.22				

CORE PHOTOGRAPHS

B1-ABOXES 1 - 4: 11.2 - 48.1 FEET



B2-BBOXES 1 - 3: 8.2 - 36.7 FEET



CORE PHOTOGRAPHS

B3-A





FEET

SITE PHOTOGRAPH

Bridge No. 29 on –L– (US 64 Alt.) over Tar River

