

REFERENCE: B-5670

PROJECT: 45625

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

**STRUCTURE**  
**SUBSURFACE INVESTIGATION**

COUNTY NASH  
PROJECT DESCRIPTION BRIDGE NO. 29 ON -L-  
(US 64 ALT.) OVER TAR RIVER AT STA. 16+98

**CONTENTS**

<u>SHEET NO.</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4-5	PROFILE(S)
6-8	CROSS SECTION(S)
9-15	BORE LOG(S) & CORE REPORT(S)
16	ROCK TEST RESULTS
17-18	CORE PHOTOGRAPH(S)
19	SITE PHOTOGRAPH(S)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5670	1	19

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 TOT-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
N. O. MOORE  
\_\_\_\_\_  
A. N. KINTNER  
\_\_\_\_\_  
D. G. PINTER  
\_\_\_\_\_  
J. E. DEAN

INVESTIGATED BY J. L. LOVE  
DRAWN BY J. L. LOVE  
CHECKED BY N. T. ROBERSON  
SUBMITTED BY N. T. ROBERSON  
DATE AUGUST 2019



DocuSigned by:  
Jaime L. Love 11/16/2021  
B93571039B88245 SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED**

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
**GEOTECHNICAL ENGINEERING UNIT**  
**SUBSURFACE INVESTIGATION**  
 SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

**SOIL DESCRIPTION**

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

**SOIL LEGEND AND AASHTO CLASSIFICATION**

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)							ORGANIC MATERIALS			
	A-1	A-3	A-2		A-2-6		A-2-7	A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5	A-6, A-7	A-3	A-4, A-5	A-6, A-7	
GROUP CLASS.	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7					A-7-5, A-7-6							
SYMBOL																		
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX 10 MX	51 MN 35 MX 35 MX	35 MX 35 MX 35 MX	35 MX 35 MX 35 MX	35 MX 35 MX 35 MX	36 MN 36 MN 36 MN	36 MN 36 MN 36 MN	36 MN 36 MN 36 MN									
MATERIAL PASSING #40 LL PI	— 6 MX	— NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN								
GROUP INDEX	0	0	0	4 MX	8 MX	12 MX	16 MX	NO MX										
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS	CLAYEY SOILS										
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD						FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE					

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

**CONSISTENCY OR DENSENESS**

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

**TEXTURE OR GRAIN SIZE**

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.76	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE, SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						
GRAIN SIZE	305	75	2.0	0.25	0.05	0.005
SIZE IN.	12	3				

**SOIL MOISTURE - CORRELATION OF TERMS**

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL - LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
PL - PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL - SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

**PLASTICITY**

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

**COLOR**

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

**GRADATION**

**WELL GRADED** - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  
**UNIFORMLY GRADED** - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  
**GAP-GRADED** - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

**ANGULARITY OF GRAINS**

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

**MINERALOGICAL COMPOSITION**

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

**COMPRESSIBILITY**

SLIGHTLY COMPRESSIBLE LL < 31  
 MODERATELY COMPRESSIBLE LL = 31 - 50  
 HIGHLY COMPRESSIBLE LL > 50

**PERCENTAGE OF MATERIAL**

	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE

**GROUND WATER**

- ∇ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- ▽ STATIC WATER LEVEL AFTER 24 HOURS
- ▽PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

**MISCELLANEOUS SYMBOLS**

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT DMT TEST BORING
- AUGER BORING
- CORE BORING
- MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

**RECOMMENDATION SYMBOLS**

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

**ABBREVIATIONS**

- AR - AUGER REFUSAL
- BT - BORING TERMINATED
- CL - CLAY
- CPT - CONE PENETRATION TEST
- CSE - COARSE
- DMT - DILATOMETER TEST
- DPT - DYNAMIC PENETRATION TEST
- e - VOID RATIO
- F - FINE
- FOSS. - FOSSILIFEROUS
- FRAC. - FRACTURED, FRACTURES
- FRAGS. - FRAGMENTS
- HI. - HIGHLY
- MED. - MEDIUM
- MICA - MICACEOUS
- MOD. - MODERATELY
- NP - NON PLASTIC
- ORG. - ORGANIC
- PMT - PRESSUREMETER TEST
- SAP. - SAPROLITIC
- SD. - SAND, SANDY
- SL. - SILTY, SILTY
- SLI. - SLIGHTLY
- TCR - TRICONE REFUSAL
- w - MOISTURE CONTENT
- V - VERY
- VST - VANE SHEAR TEST
- WEA. - WEATHERED
- γ - UNIT WEIGHT
- γ<sub>d</sub> - DRY UNIT WEIGHT
- SAMPLE ABBREVIATIONS
- S - BULK
- SS - SPLIT SPOON
- ST - SHELBY TUBE
- RS - ROCK
- RT - RECOMPACTED TRIAXIAL
- CBR - CALIFORNIA BEARING RATIO

**EQUIPMENT USED ON SUBJECT PROJECT**

- DRILL UNITS:
  - CME-45C
  - CME-55
  - CME-550
  - VANE SHEAR TEST
  - PORTABLE HOIST
  - Water Truck
- ADVANCING TOOLS:
  - CLAY BITS
  - 6" CONTINUOUS FLIGHT AUGER
  - 8" HOLLOW AUGERS
  - HARD FACED FINGER BITS
  - TUNG-CARBIDE INSERTS
  - CASING  w/ ADVANCER
  - TRICONE \*STEEL TEETH
  - TRICONE \*TUNG-CARB.
  - CORE BIT
- HAMMER TYPE:
  - AUTOMATIC  MANUAL
- CORE SIZE:
  - B  -H
  - N  -XWL
- HAND TOOLS:
  - POST HOLE DIGGER
  - HAND AUGER
  - SOUNDING ROD
  - VANE SHEAR TEST

**ROCK DESCRIPTION**

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

- WEATHERED ROCK (WR)**: NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
- CRYSTALLINE ROCK (CR)**: FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
- NON-CRYSTALLINE ROCK (NCR)**: FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
- COASTAL PLAIN SEDIMENTARY ROCK (CP)**: COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

**WEATHERING**

- FRESH**: ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
- VERY SLIGHT (V SL.)**: ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
- SLIGHT (SL.)**: ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
- MODERATE (MOD.)**: SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
- MODERATELY SEVERE (MOD. SEV.)**: ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
- SEVERE (SEV.)**: ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
- VERY SEVERE (V SEV.)**: ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
- COMPLETE**: ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

**ROCK HARDNESS**

- VERY HARD**: CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
- HARD**: CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
- MODERATELY HARD**: CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
- MEDIUM HARD**: CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
- SOFT**: CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
- VERY SOFT**: CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.

**FRACTURE SPACING**

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

**BEDDING**

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

**INDURATION**

- FRIABLE**: RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
- MODERATELY INDURATED**: GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
- INDURATED**: GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
- EXTREMELY INDURATED**: SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

**TERMS AND DEFINITIONS**

- ALLUVIUM (ALLUV.)**: SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
- AQUIFER**: A WATER BEARING FORMATION OR STRATA.
- ARENACEOUS**: APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
- ARGILLACEOUS**: APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
- ARTESIAN**: GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
- CALCAREOUS (CALC.)**: SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
- COLLUVIUM**: ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
- CORE RECOVERY (REC.)**: TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
- DIKE**: A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
- DIP**: THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
- DIP DIRECTION (DIP AZIMUTH)**: THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
- FAULT**: A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
- FISSILE**: A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
- FLOAT**: ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.
- FLOOD PLAIN (FP)**: LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
- FORMATION (FM)**: A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
- JOINT**: FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
- LEDGE**: A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
- LENS**: A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
- MOTTLED (MOT.)**: IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
- PERCHED WATER**: WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
- RESIDUAL (RES.) SOIL**: SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
- ROCK QUALITY DESIGNATION (RQD)**: A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
- SAPROLITE (SAP.)**: RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
- SILL**: AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
- SLICKENSIDE**: POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
- STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)**: NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
- STRATA CORE RECOVERY (SREC.)**: TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- STRATA ROCK QUALITY DESIGNATION (SROD)**: A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
- TOPSOIL (TS.)**: SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BL-105, PIN & CAP AT -L- STA. 18+74, 23' LT

ELEVATION: 170.84 FEET

NOTES:  
 TOP OF RAIL @ EBI-L- STA. 15+34, 15' RT  
 ELEV. = 174.5'

TOP OF RAIL @ EB2 -L- STA. 18+52, 15' RT  
 ELEV. = 174.5'

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL ENGINEERING UNIT

**SUBSURFACE INVESTIGATION**

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES  
 FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

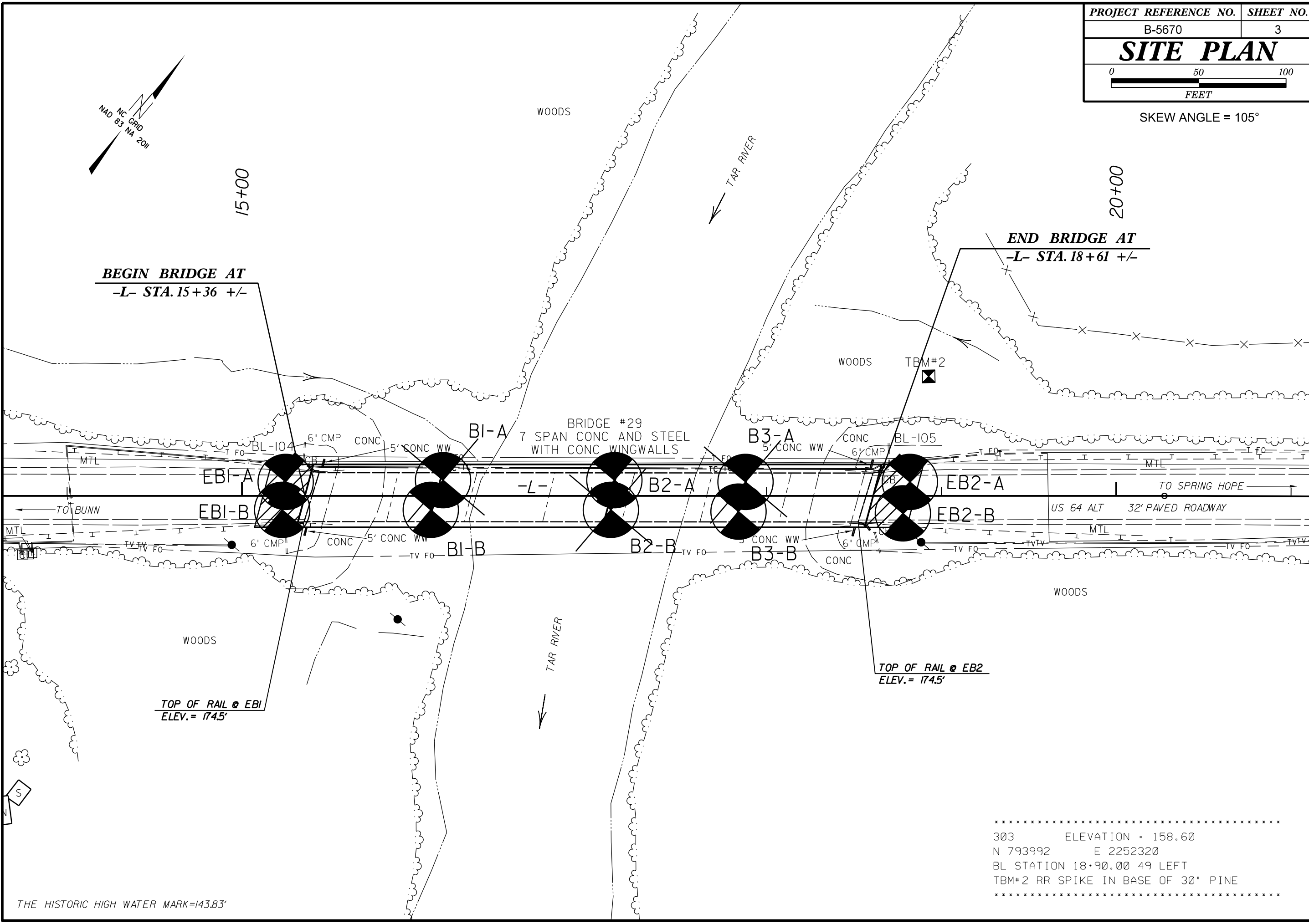
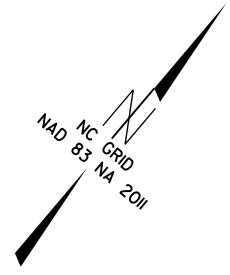
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		SURFACE CONDITIONS					GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)		SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)				
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.		VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings
STRUCTURE		DECREASING SURFACE QUALITY →					COMPOSITION AND STRUCTURE						
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities		90			N/A	N/A	<b>A.</b> Thick bedded, very blocky sandstone. The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70					
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets		80					<b>B.</b> Sandstone with thin inter-layers of siltstone	60					
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets			70				<b>C.</b> Sandstone and siltstone in similar amounts		50				
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity			60				<b>D.</b> Siltstone or silty shale with sandstone layers			40			
DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces				50			<b>E.</b> Weak siltstone or clayey shale with sandstone layers				30		
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes					40		<b>F.</b> Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure					20	
					30		<b>G.</b> Undisturbed silty or clayey shale with or without a few very thin sandstone layers						10
					20		<b>H.</b> Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.						
					10								
		N/A	N/A										

→ Means deformation after tectonic disturbance

SKEW ANGLE = 105°



**BEGIN BRIDGE AT**  
-L- STA. 15+36 +/-

**END BRIDGE AT**  
-L- STA. 18+61 +/-

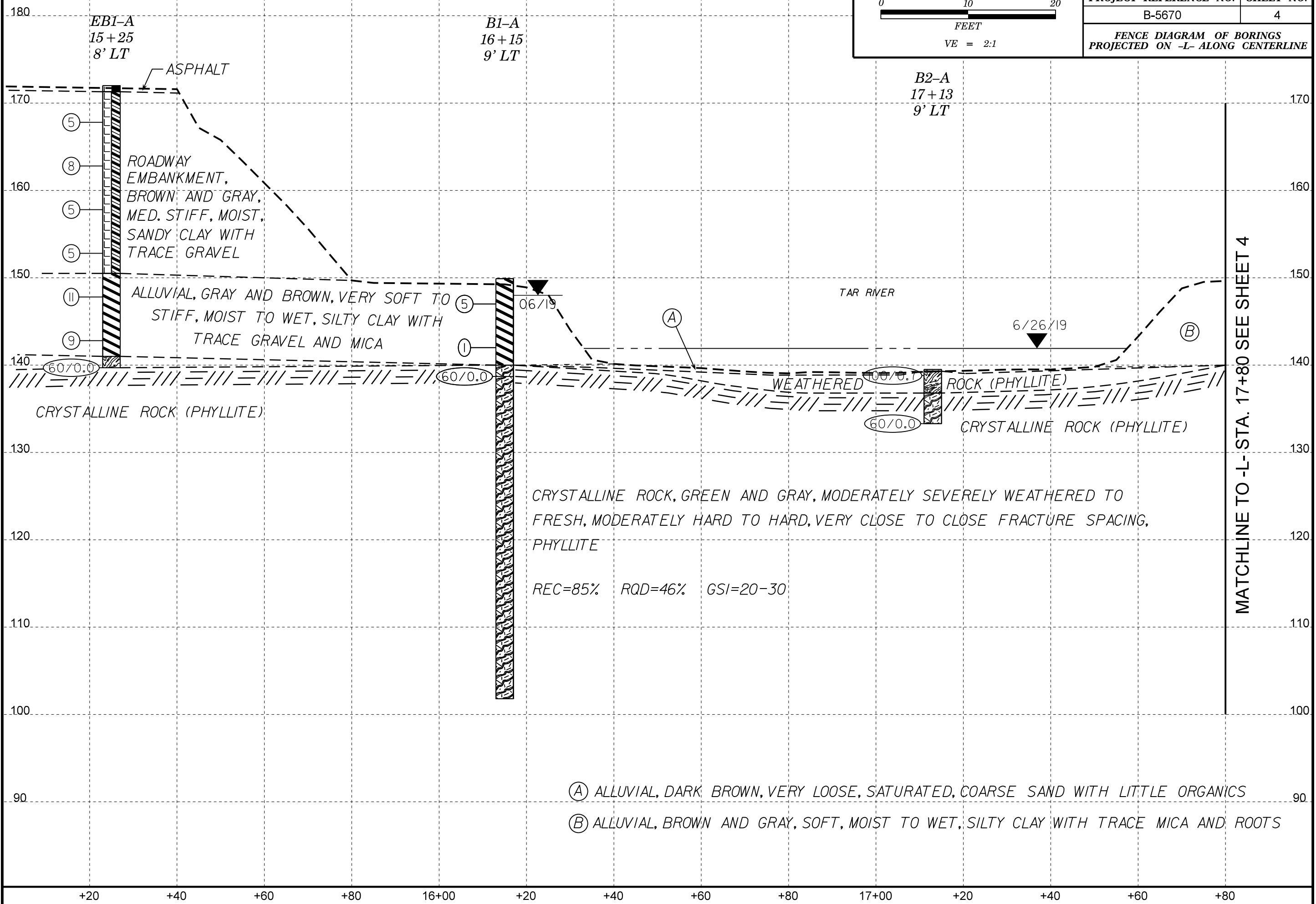
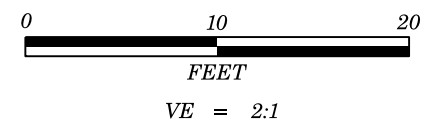
BRIDGE #29  
7 SPAN CONC AND STEEL  
WITH CONC WINGWALLS

TOP OF RAIL @ EB1  
ELEV. = 174.5'

TOP OF RAIL @ EB2  
ELEV. = 174.5'

THE HISTORIC HIGH WATER MARK=143.83'

\*\*\*\*\*  
 303            ELEVATION = 158.60  
 N 793992       E 2252320  
 BL STATION 18+90.00 49 LEFT  
 TBM#2 RR SPIKE IN BASE OF 30" PINE  
 \*\*\*\*\*



EBI-A  
15+25  
8' LT

BI-A  
16+15  
9' LT

B2-A  
17+13  
9' LT

ROADWAY  
EMBANKMENT,  
BROWN AND GRAY,  
MED. STIFF, MOIST,  
SANDY CLAY WITH  
TRACE GRAVEL

ALLUVIAL, GRAY AND BROWN, VERY SOFT TO  
STIFF, MOIST TO WET, SILTY CLAY WITH  
TRACE GRAVEL AND MICA

TAR RIVER

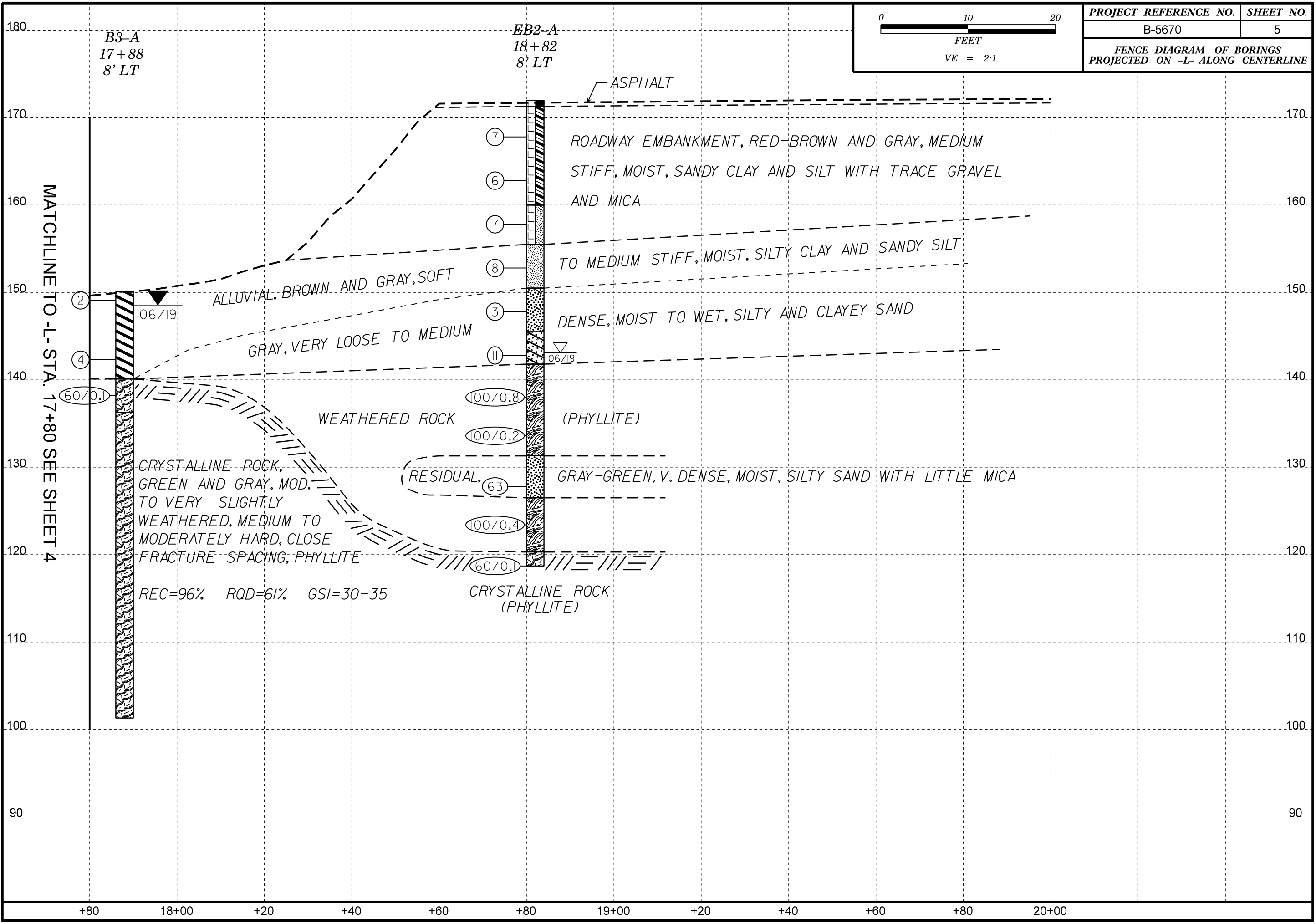
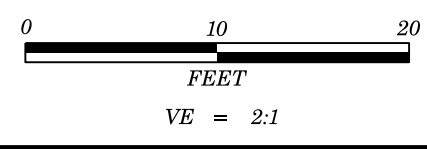
WEATHERED ROCK (PHYLLITE)  
CRYSTALLINE ROCK (PHYLLITE)

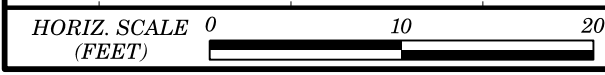
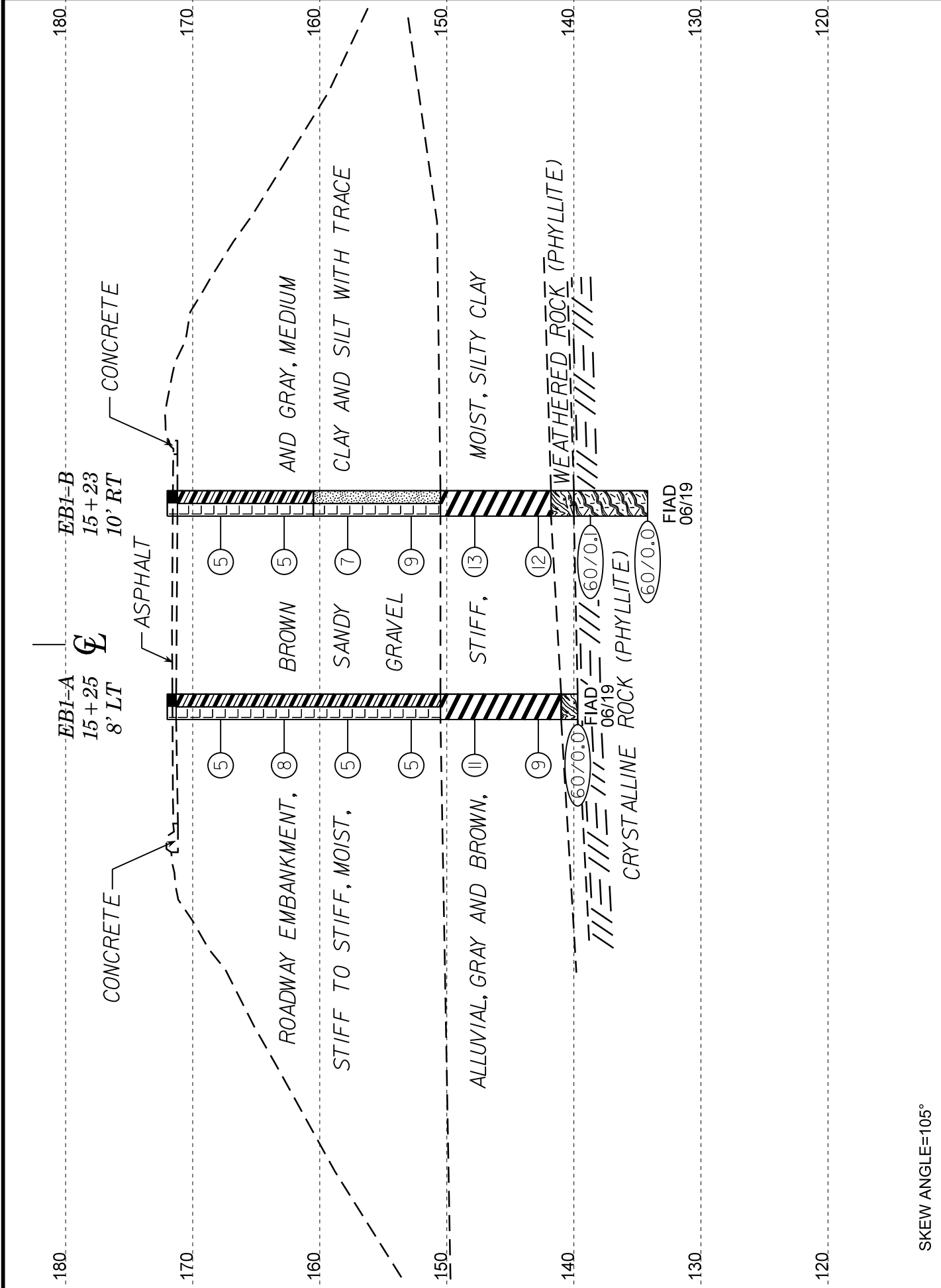
CRYSTALLINE ROCK, GREEN AND GRAY, MODERATELY SEVERELY WEATHERED TO  
FRESH, MODERATELY HARD TO HARD, VERY CLOSE TO CLOSE FRACTURE SPACING,  
PHYLLITE

REC=85% RQD=46% GSI=20-30

MATCHLINE TO -L- STA. 17+80 SEE SHEET 4

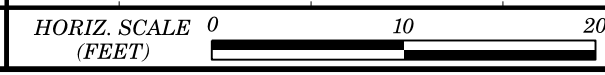
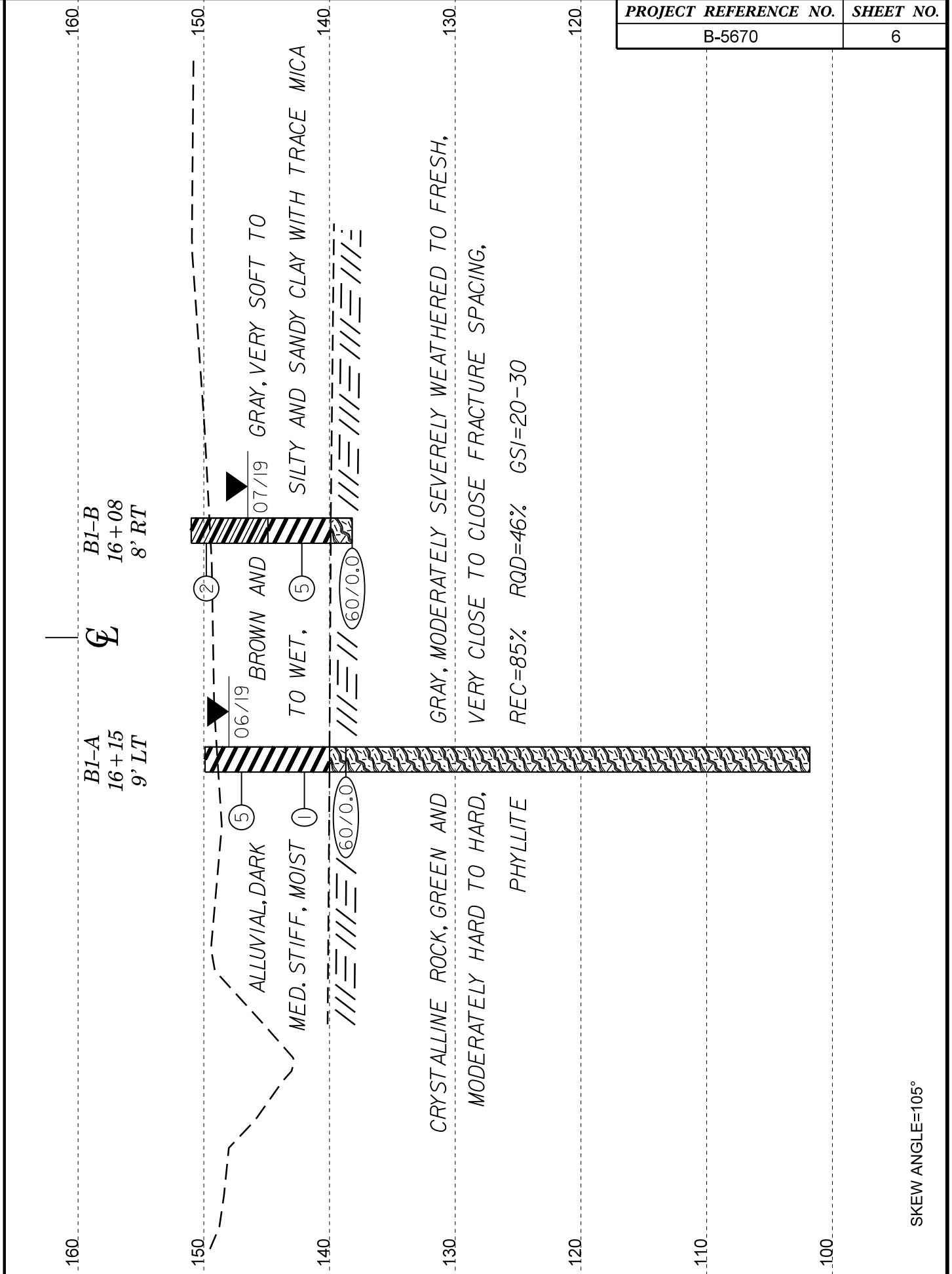
- Ⓐ ALLUVIAL, DARK BROWN, VERY LOOSE, SATURATED, COARSE SAND WITH LITTLE ORGANICS
- Ⓑ ALLUVIAL, BROWN AND GRAY, SOFT, MOIST TO WET, SILTY CLAY WITH TRACE MICA AND ROOTS





VE = 1:1

CROSS SECTION THROUGH EBI



VE = 1:1

CROSS SECTION THRU BENT 1







# GEOTECHNICAL BORING REPORT

## BORE LOG

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Kintner, A. N.									
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)								
BORING NO. EB1-A		STATION 15+25		OFFSET 8 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 172.0 ft		TOTAL DEPTH 32.3 ft		NORTHING 793,708		EASTING 2,252,078									
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER Pinter, D. G.		START DATE 06/24/19		COMP. DATE 06/24/19		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
175															
170	168.8	3.2	2	2	3								M	GROUND SURFACE 0.0 ROADWAY EMBANKMENT ASPHALT 0.7 BROWN AND GRAY, SANDY CLAY WITH TRACE GRAVEL	
165	163.8	8.2	2	4	4								M		
160	158.8	13.2	3	2	3								M		
155	153.8	18.2	3	2	3								M		
150	148.8	23.2	3	5	6								M	ALLUVIAL GRAY AND BROWN, SILTY CLAY WITH SOME MOTTLING	21.5
145	143.8	28.2	5	4	5								M		
140	139.7	32.3	60/0.0										WEATHERED ROCK (PHYLITITE) 31.0 139.7 32.3 Boring Terminated with Standard Penetration Test Refusal at Elevation 139.7 ft ON CRYSTALLINE ROCK (PHYLITITE)		

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Kintner, A. N.									
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)								
BORING NO. EB1-B		STATION 15+23		OFFSET 8 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 172.0 ft		TOTAL DEPTH 37.8 ft		NORTHING 793,695		EASTING 2,252,087									
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER Pinter, D. G.		START DATE 06/24/19		COMP. DATE 06/24/19		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
175															
170	168.8	3.2	2	2	3								M	GROUND SURFACE 0.0 ROADWAY EMBANKMENT ASPHALT 0.8 BROWN AND GRAY, SANDY CLAY WITH TRACE GRAVEL	
165	163.8	8.2	2	2	3								M		
160	158.8	13.2	1	3	4								M	ORANGE AND GRAY, SANDY SILT WITH TRACE ANGULAR GRAVEL	11.5
155	153.8	18.2	3	4	5								M		
150	148.8	23.2	4	6	7								M	ALLUVIAL GRAY AND BROWN, MOTTLED SILTY CLAY	21.5
145	143.8	28.2	4	6	6								M		
140	138.8	33.2	60/0.1										WEATHERED ROCK (PHYLITITE) 30.2 140.0 32.0 CRYSTALLINE ROCK (PHYLITITE)		
135	134.2	37.8	60/0.0										Boring Terminated with Standard Penetration Test Refusal at Elevation 134.2 ft IN CRYSTALLINE ROCK (PHYLITITE)	37.8	

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/27/19

# GEOTECHNICAL BORING REPORT

## BORE LOG

# GEOTECHNICAL BORING REPORT

## CORE LOG

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.										
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)									
BORING NO. B1-A		STATION 16+15		OFFSET 9 ft LT		ALIGNMENT -L-										
COLLAR ELEV. 149.9 ft		TOTAL DEPTH 48.1 ft		NORTHING 793,767		EASTING 2,252,146										
DRILL RIG/HAMMER EFF./DATE RFO0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing WSPT & Core		HAMMER TYPE Automatic										
DRILLER Pinter, D. G.		START DATE 06/25/19		COMP. DATE 06/26/19		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
150														149.9	GROUND SURFACE	0.0
	148.0	1.9	WOH	2	3							M			ALLUVIAL DARK BROWN, SILTY CLAY WITH TRACE MICA AND GRAVEL	
145																
	143.0	6.9	WOH	WOH	1							M				
140																
	138.7	11.2								60/0.0				140.0	CRYSTALLINE ROCK (PHYLLITE)	9.9
														138.7	GREEN AND GRAY MODERATELY SEVERELY WEATHERED TO FRESH, MODERATELY HARD TO HARD, VERY CLOSE TO CLOSE FRACTURE SPACING, PHYLLITE	11.2
135												RS-1				
130																
125																
120																
115												RS-2				
110																
105																
														101.8	Boring Terminated at Elevation 101.8 ft IN CRYSTALLINE ROCK (PHYLLITE)	48.1

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.						
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)					
BORING NO. B1-A		STATION 16+15		OFFSET 9 ft LT		ALIGNMENT -L-						
COLLAR ELEV. 149.9 ft		TOTAL DEPTH 48.1 ft		NORTHING 793,767		EASTING 2,252,146						
DRILL RIG/HAMMER EFF./DATE RFO0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing WSPT & Core		HAMMER TYPE Automatic						
DRILLER Pinter, D. G.		START DATE 06/25/19		COMP. DATE 06/26/19		SURFACE WATER DEPTH N/A						
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	TOTAL RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (%)	RQD (%)		REC. (%)	RQD (%)			
138.7												
	138.7	11.2	2.0	N=60/0.0 1:31/1.0 2:42/1.0	(1.7) 85%	(1.4) 70%		(31.2) 85%	(17.1) 46%		Begin Coring @ 11.2 ft	
	136.7	13.2									GREEN AND GRAY MODERATELY SEVERELY WEATHERED TO FRESH, MODERATELY HARD TO HARD, VERY CLOSE TO CLOSE FRACTURE SPACING, PHYLLITE	11.2
135			5.0	1:55/1.0 1:47/1.0 1:46/1.0 1:18/1.0	(5.0) 100%	(2.4) 48%				RS-1	GSI=20-30	
	131.7	18.2										
130			5.0	1:17/1.0 1:24/1.0 1:31/1.0 1:35/1.0	(5.0) 100%	(3.1) 62%						
	126.7	23.2										
125			5.0	1:35/1.0 2:06/1.0 2:25/1.0	(5.0) 100%	(1.9) 38%						
	121.7	28.2										
120			5.0	2:08/1.0 2:22/1.0	(5.0) 100%	(2.2) 44%						
	116.7	33.2										
115			4.5	1:32/1.0 1:43/1.0 1:40/1.0	(4.2) 93%	(1.8) 40%				RS-2		
	112.2	37.7										
	111.4	38.5	0.8	3:45/0.5 -0.8	(0.8) 100%	(0.8) 100%						
110			4.6	1:30/1.0 1:34/1.0 1:20/1.0	(3.5) 76%	(2.9) 63%						
	106.8	43.1										
105			5.0	1:34/1.0 0:35/0.6	(1.0) 20%	(0.6) 12%						
	101.8	48.1		1:14/1.0 1:26/1.0 1:08/1.0 1:22/1.0 1:11/1.0							Boring Terminated at Elevation 101.8 ft IN CRYSTALLINE ROCK (PHYLLITE)	48.1

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/28/19

# GEOTECHNICAL BORING REPORT

## BORE LOG

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.									
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)								
BORING NO. B1-B		STATION 16+08		OFFSET 8 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 151.0 ft		TOTAL DEPTH 12.8 ft		NORTHING 793,750		EASTING 2,252,152									
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing w/ SPT		HAMMER TYPE Automatic									
DRILLER Pinter, D. G.		START DATE 07/01/19		COMP. DATE 07/01/19		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
155															
150	150.8	0.2	WOH	WOH	2							M	GROUND SURFACE	0.0	
145	143.2	7.8	WOH	2	3							M	ALLUVIAL BROWN, SANDY CLAY WITH TRACE MICA AND ROOTS	6.1	
140	138.2	12.8											CRYSTALLINE ROCK (PHYLLITE)	12.8	
Boring Terminated with Standard Penetration Test Refusal at Elevation 138.2 ft IN CRYSTALLINE ROCK (PHYLLITE)															

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.									
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)								
BORING NO. B2-A		STATION 17+13		OFFSET 9 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 139.5 ft		TOTAL DEPTH 6.2 ft		NORTHING 793,831		EASTING 2,252,221									
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing w/ SPT		HAMMER TYPE Automatic									
DRILLER Pinter, D. G.		START DATE 06/26/19		COMP. DATE 06/26/19		SURFACE WATER DEPTH 2.5ft									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
140	139.5	0.0													
135	133.3	6.2													
Boring Terminated with Standard Penetration Test Refusal at Elevation 133.3 ft IN CRYSTALLINE ROCK (PHYLLITE)															

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/27/19



# GEOTECHNICAL BORING REPORT

## BORE LOG

# GEOTECHNICAL BORING REPORT

## CORE LOG

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.											
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)										
BORING NO. B3-A		STATION 17+88		OFFSET 8 ft LT		ALIGNMENT -L-											
COLLAR ELEV. 150.1 ft		TOTAL DEPTH 48.8 ft		NORTHING 793,878		EASTING 2,252,279											
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing WSPT & Core		HAMMER TYPE Automatic											
DRILLER Pinter, D. G.		START DATE 06/27/19		COMP. DATE 06/27/19		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION				
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft)	DEPTH (ft)			
155																	
150	150.1	0.0												150.1	0.0	GROUND SURFACE	
145			WOH	WOH	2											ALLUVIAL BROWN AND GRAY, SILTY CLAY TRACE MICA AND ROOTS	
140	143.3	6.8	1	2	2												
135	138.3	11.8	60/0.1														CRYSTALLINE ROCK (PHYLLITE)
130																	GREEN AND GRAY, SLIGHTLY TO VERY SLIGHTLY WEATHERED, MODERATELY HARD TO HARD, CLOSE FRCATURE SPACING, PHYLLITE
125																	REC=96% RQD=61% GSI=30-35
120																	
115																	
110																	
105																	
																	Boring Terminated at Elevation 101.3 ft IN CRYSTALLINE ROCK (PHYLLITE)

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.						
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)					
BORING NO. B3-A		STATION 17+88		OFFSET 8 ft LT		ALIGNMENT -L-						
COLLAR ELEV. 150.1 ft		TOTAL DEPTH 48.8 ft		NORTHING 793,878		EASTING 2,252,279						
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing WSPT & Core		HAMMER TYPE Automatic						
DRILLER Pinter, D. G.		START DATE 06/27/19		COMP. DATE 06/27/19		SURFACE WATER DEPTH N/A						
CORE SIZE NXWL			TOTAL RUN 35.0 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	
					REC. (%)	RQD (%)		REC. (%)	RQD (%)			ELEV. (ft)
136.3												Begin Coring @ 13.8 ft
135	136.3	13.8	5.0	1:25/1.0 1:12/1.0 1:03/1.0 1:07/1.0 1:15/1.0	(5.0) 100%	(2.4) 48%		(33.5) 96%	(21.4) 61%		136.3	GREEN AND GRAY, SLIGHTLY TO VERY SLIGHTLY WEATHERED, MODERATELY HARD TO HARD, CLOSE FRCATURE SPACING, PHYLLITE
130	131.3	18.8	5.0	1:45/1.0 1:42/1.0 1:42/1.0 2:24/1.0 2:11/1.0	(4.9) 98%	(3.8) 76%						GSI=30-35
125	126.3	23.8	5.0	1:54/1.0 1:58/1.0 2:01/1.0 2:06/1.0 2:00/1.0	(5.0) 100%	(4.0) 80%						
120	121.3	28.8	5.0	0:50/1.0 1:46/1.0 2:00/1.0 2:02/1.0 1:50/1.0	(5.0) 100%	(3.7) 74%						
115	116.3	33.8	5.0	2:09/1.0 2:23/1.0 1:47/1.0 1:18/1.0 1:41/1.0	(4.4) 88%	(2.8) 56%						
110	111.3	38.8	5.0	1:08/1.0 0:57/1.0 1:12/1.0 1:17/1.0 1:42/1.0	(4.2) 84%	(1.1) 22%						
105	106.3	43.8	5.0	1:18/1.0 1:22/1.0 1:20/1.0 1:11/1.0 1:46/1.0	(5.0) 100%	(3.6) 72%						
	101.3	48.8									101.3	Boring Terminated at Elevation 101.3 ft IN CRYSTALLINE ROCK (PHYLLITE)

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/28/19

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/28/19

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.								
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)							
BORING NO. B3-B		STATION 17+84		OFFSET 9 ft RT		ALIGNMENT -L-	0 HR. N/A							
COLLAR ELEV. 150.5 ft		TOTAL DEPTH 12.2 ft		NORTHING 793,863		EASTING 2,252,287	24 HR. 4.7							
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019				DRILL METHOD NW Casing w/ SPT		HAMMER TYPE Automatic								
DRILLER Pinter, D. G.		START DATE 06/25/19		COMP. DATE 06/26/19		SURFACE WATER DEPTH N/A								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
155														
150	150.5	0.0	WOH	2	2	4						M	150.5	GROUND SURFACE 0.0
145	143.4	7.1	WOH	WOH	WOH							M	145.1	ALLUVIAL BROWN AND GRAY, SANDY CLAY WITH TRACE MICA AND ROOTS
140	138.4	12.1											139.3	GRAY, SANDY SILT WITH TRACE MICA AND ROOTS
													138.7	
													138.3	WEATHERED ROCK (PHYLLITE)
													138.3	CRYSTALLINE ROCK (PHYLLITE)
														Boring Terminated with Standard Penetration Test Refusal at Elevation 138.3 ft IN CRYSTALLINE ROCK (PHYLLITE)

# GEOTECHNICAL BORING REPORT

## BORE LOG

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.											
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)										
BORING NO. EB2-A		STATION 18+82		OFFSET 8 ft LT		ALIGNMENT -L-											
COLLAR ELEV. 172.0 ft		TOTAL DEPTH 53.3 ft		NORTHING 793,939		EASTING 2,252,351											
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019			DRILL METHOD H.S. Augers			HAMMER TYPE Automatic											
DRILLER Pinter, D. G.		START DATE 06/20/19		COMP. DATE 06/20/19		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
175																	
170	168.8	3.2	3	3	4								M	172.0 171.3	0.0 0.7	GROUND SURFACE ROADWAY EMBANKMENT ASPHALT BROWN AND GRAY, SANDY CLAY WITH TRACE GRAVEL AND MICA	
165	163.8	8.2	2	3	3								M				
160	158.8	13.2	3	3	4								M	160.0	12.0	ORANGE AND GRAY, SANDY SILT WITH TRACE MICA	
155	153.8	18.2	7	5	3								M	155.5	16.5	ALLUVIAL GRAY, SANDY SILT WITH TRACE ROOTS	
150	148.8	23.2	1	2	1								M	150.5	21.5	GRAY, SILTY SAND WITH TRACE CLAY LENSES AND MICA	
145	143.8	28.2	2	3	8								M	145.5	26.5	ORANGE ANE GRAY CLAYEY SAND WITH TRACE MICA AND GRAVEL	
140	138.8	33.2	47	53/0.3									M	141.8	30.2	WEATHERED ROCK (PHYLLITE)	
135	133.8	38.2	100/0.2										M				
130	128.8	43.2	15	22	41								M	131.3	40.7	GRAY-GREEN, SILTY SAND WITH LITTLE MICA	
125	123.8	48.2	100/0.4										M	126.5	45.5	WEATHERED ROCK (PHYLLITE)	
120	118.8	53.2	60/0.1										M	120.3 118.7	51.7 53.3	CRYSTALLINE ROCK (PHYLLITE)	
																	Boring Terminated with Standard Penetration Test Refusal at Elevation 118.7 ft IN CRYSTALLINE ROCK (PHYLLITE)

WBS 45625.1.1		TIP B-5670		COUNTY NASH		GEOLOGIST Moore, N. O.											
SITE DESCRIPTION BRIDGE NO. 29 ON -L- (US 64 ALT.) OVER TAR RIVER							GROUND WTR (ft)										
BORING NO. EB2-B		STATION 18+78		OFFSET 10 ft RT		ALIGNMENT -L-											
COLLAR ELEV. 172.1 ft		TOTAL DEPTH 53.5 ft		NORTHING 793,923		EASTING 2,252,359											
DRILL RIG/HAMMER EFF./DATE RFC0074 CME-55 80% 03/08/2019			DRILL METHOD H.S. Augers			HAMMER TYPE Automatic											
DRILLER Pinter, D. G.		START DATE 06/20/19		COMP. DATE 06/20/19		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
175																	
170	168.7	3.4	3	3	3								M	172.1 171.4	0.0 0.7	GROUND SURFACE ROADWAY EMBANKMENT ASPHALT ORANGE-BROWN, SANDY CLAY WITH TRACE GRAVEL AND MICA	
165	163.7	8.4	1	2	4								M				
160	158.7	13.4	2	3	3								M	160.4	11.7	ORANGE-BROWN, SANDY SILT WITH TRACE GRAVEL AND MICA	
155	153.7	18.4	2	1	2								M	155.4	16.7	ALLUVIAL GRAY, SANDY SILT WITH TRACE ROOTS	
150	148.7	23.4	2	1	3								M	150.4	21.7	GRAY AND BROWN, SILTY SAND WITH TRACE MICA	
145	143.7	28.4	2	4	6								M				
140	138.7	33.4	21	64	36/0.2								W	142.1	30.0	WEATHERED ROCK (PHYLLITE)	
135	133.7	38.4	100/0.4														
130	128.7	43.4	100/0.4														
125	123.7	48.4	100/0.4														
120	118.7	53.4	60/0.1														
																	Boring Terminated with Standard Penetration Test Refusal at Elevation 118.6 ft IN CRYSTALLINE ROCK (PHYLLITE)

NCDOT BORE DOUBLE B5670\_GEO\_BH.GPJ NC\_DOT.GDT 8/27/19



**PROJ. NO. - 45625.1.1**  
**ID NO. - B-5670**  
**COUNTY -NASH**

**B1-A -L-**

<b>ROCK TEST RESULTS</b>							
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ROCK TYPE	UNIT WT LB/FT <sup>3</sup>	UNCONFINED COMP. STRENGTH, KSI	SECTION MOD. @ 40% MPSI
RS-1	9 LT	16+15	15.4-16.0	PHYLLITE	172.3	2.44	2.4
RS-2	9 LT	16+15	33.6-34.1	PHYLLITE	179.8	9.52	6.8

**B2-B -L-**

<b>ROCK TEST RESULTS</b>							
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ROCK TYPE	UNIT WT LB/FT <sup>3</sup>	UNCONFINED COMP. STRENGTH, KSI	SECTION MOD. @ 40% MPSI
RS-3	8 RT	17+11	14.4-15.0	PHYLLITE		<b>CORE BROKE PRIOR TO TESTING</b>	
RS-4	8 RT	17+11	34.6-35.3	PHYLLITE	168.4	5.78	10.78

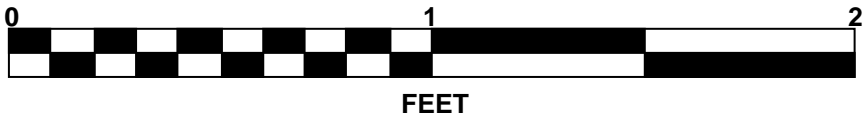
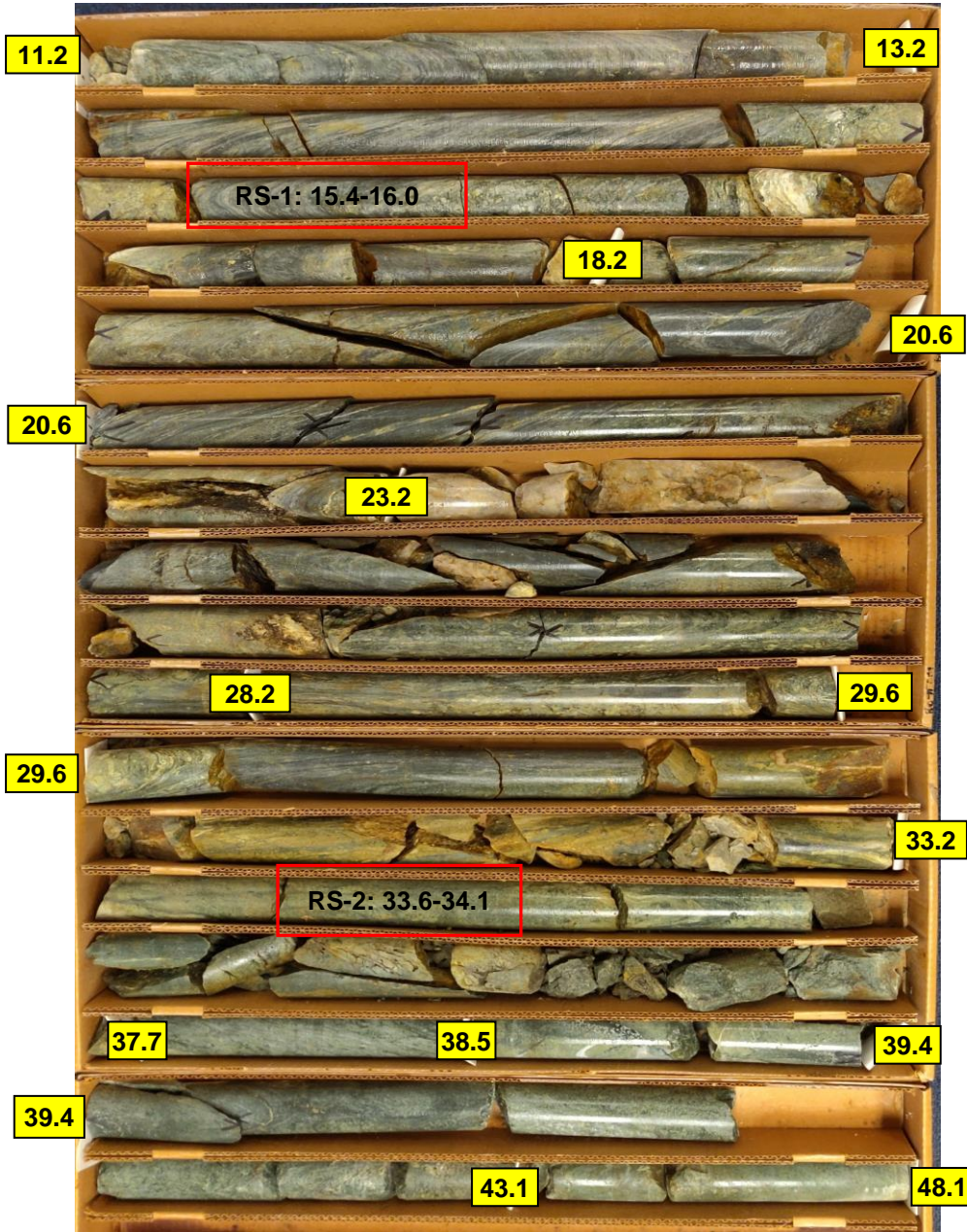
**B3-A -L-**

<b>ROCK TEST RESULTS</b>							
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ROCK TYPE	UNIT WT LB/FT <sup>3</sup>	UNCONFINED COMP. STRENGTH, KSI	SECTION MOD. @ 40% MPSI
RS-5	8 LT	17+88	22.6-23.1	PHYLLITE	169.6	16.2	-
RS-6	8 LT	17+88	46.9-47.3	PHYLLITE	173.2	5.53	9.22

# CORE PHOTOGRAPHS

## B1-A

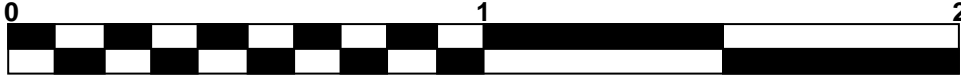
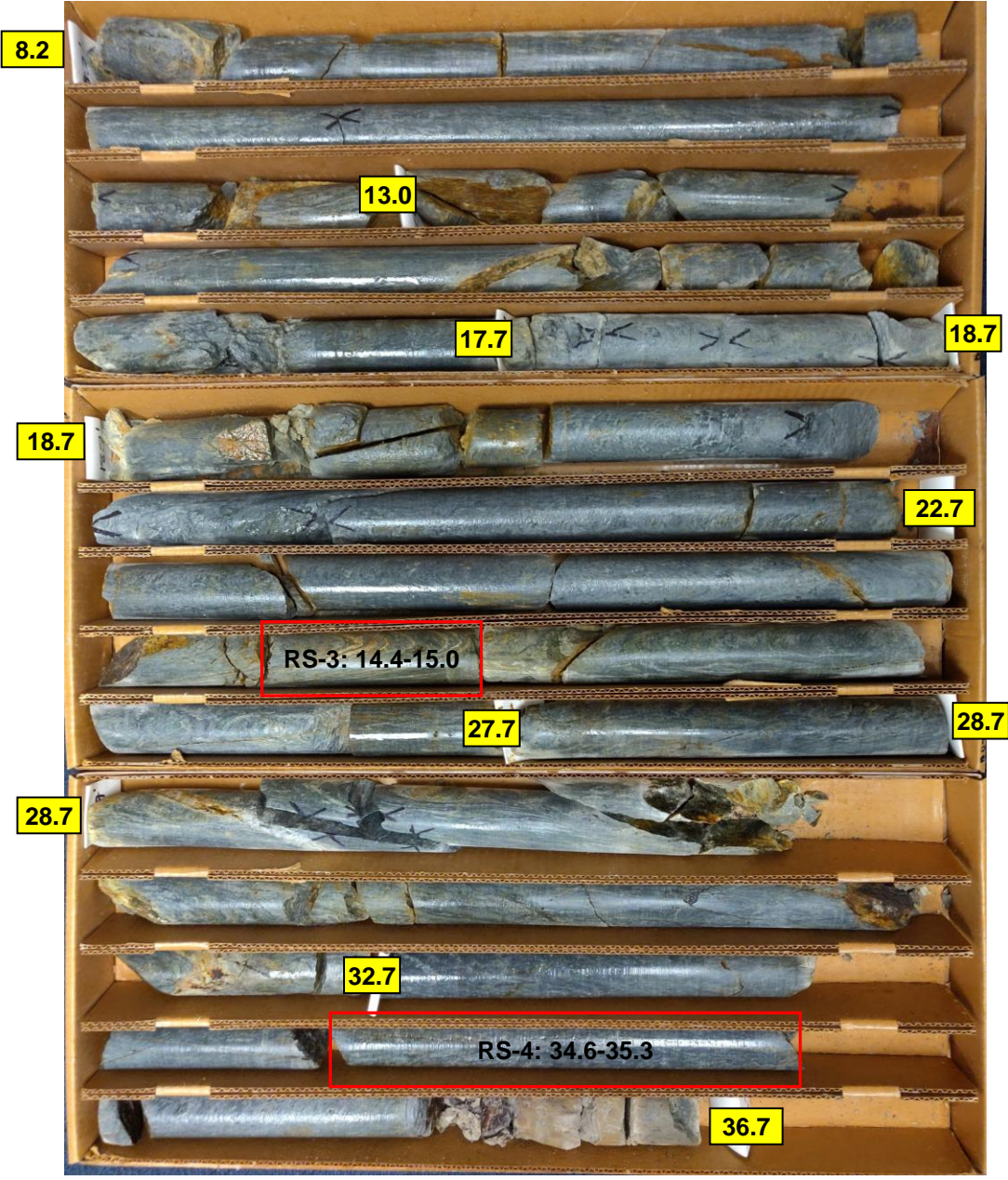
BOXES 1 - 4: 11.2 - 48.1 FEET



FEET

## B2-B

BOXES 1 - 3: 8.2 - 36.7 FEET



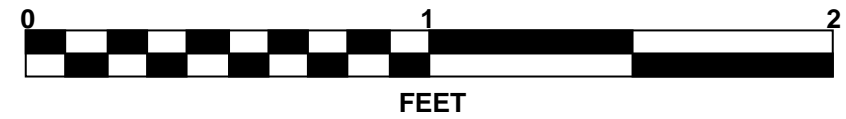
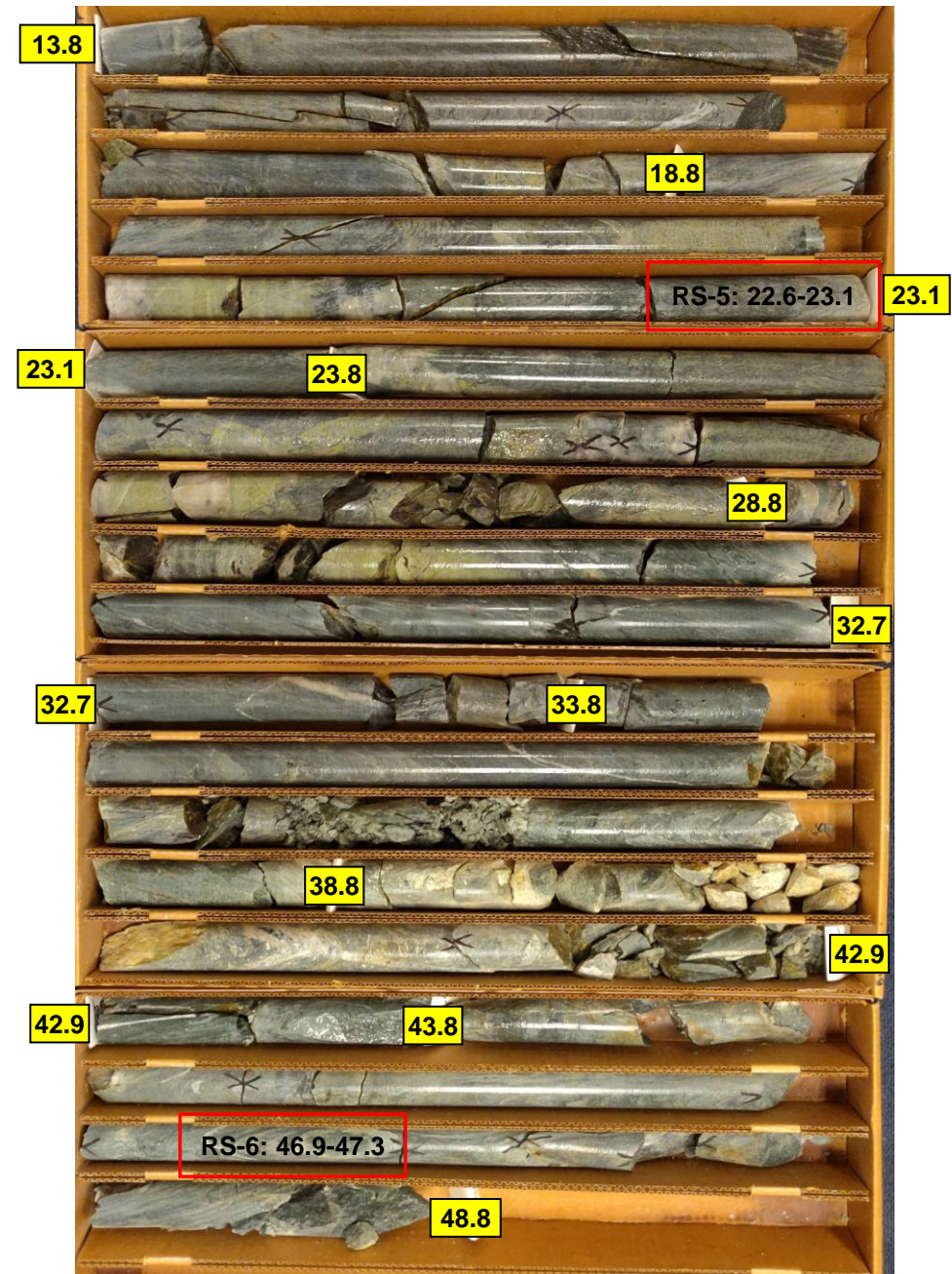
FEET



# CORE PHOTOGRAPHS

## B3-A

BOXES 1 - 4: 13.8 - 48.8 FEET





# SITE PHOTOGRAPH

Bridge No. 29 on -L- (US 64 Alt.) over Tar River



Looking North towards End Bent 2