NOTES:

FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS WITH CAST-IN-PLACE (CIP) CONCRETE FACING, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS WITH CAST-IN-PLACE CONCRETE FACING SPECIAL PROVISION.

A FENCE IS REQUIRED ON TOP OF RETAINING WALL #19C. SEE ROADWAY PLANS FOR FENCE ATTACHMENT DETAILS.

A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP REINFORCED CONCRETE FACE FOR RETAINING WALL #19C. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP REINFORCED CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, AND STONE TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK. FORM LINER ARCHITECTURAL FINISH WILL ABUT AND BLEND INTO LAND BRIDGE FORM LINER ARCHITECTURAL FINISH.FOR FORM LINER ARCHITECTURAL FINISH, SEE THE SIMULATED STONE FORM LINER FINISH SPECIAL PROVISION.

BEFORE BEGINNING MSE RETAINING WALL WITH CIP CONCRETE FACING DESIGN FOR RETAINING WALL #19C, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DO NOT USE FINE AGGREGATE IN THE REINFORCED ZONE OF RETAINING WALL #19C.

A SEPARATION GEOTEXTILE IS REQUIRED AT THE BACK OF THE REINFORCED ZONE FOR RETAINING WALL #19C.

A DRAIN IS REQUIRED FOR RETAINING WALL NO. #19C.

DO NOT USE AN MSE WALL SYSTEM WITH SEGMENTAL RETAINING WALL UNITS FOR THIS RETAINING WALL.

DESIGN THIS RETAINING WALL FOR WALL HEIGHTS EQUAL TO THE DESIGN HEIGHT (DIFFERENCE BETWEEN GRADE ELEVATION AND BOTTOM OF WALL ELEVATION) PLUS EMBEDMENT (DIFFERENCE BETWEEN BOTTOM OF WALL ELEVATION AND TOP OF LEVELING PAD ELEVATION).

DESIGN RETAINING WALL NO. #19C FOR THE FOLLOWING: 1) DESIGN HEIGHT (H) = WALL HEIGHT + WALL EMBEDMENT

2) DESIGN LIFE = 75 YEARS

3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 7350 PSF 4) MINIMUM REINFORCEMENT LENGTH (L) = 0.7×H OR 6.0 FT, WHICHEVER IS LONGER

5) MINIMUM EMBEDMENT DEPTH = 2.0 FT, SEE TABLE ON SHEET W19C-1 AND SPECIAL PROVISION 6) RETNEORCED ZONE AGGREGATE PARAMETERS.

6) REINFORCED ZONE AGG	REGATE FARAMETERS		
AGGREGATE TYPE*	UNIT WEIGHT (y) PCF	FRICTION ANGLE (ф) DEGREES	COHESION (c) PSF
COARSE	110	38	0
FINE	115	34	0

*SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE MATERIAL REQUIREMENTS.

7) IN-SITU ASSUMED MATERIAL PARAMETERS:

MATERIAL TYPE	UNIT WEIGHT (y) PCF	FRICTION ANGLE (ф) DEGREES	COHESION (c) PSF
BACKFILL	120	30	0
FOUNDATION	120	30	0

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH REINFORCEMENT FOR RETAINING WALL #19C.

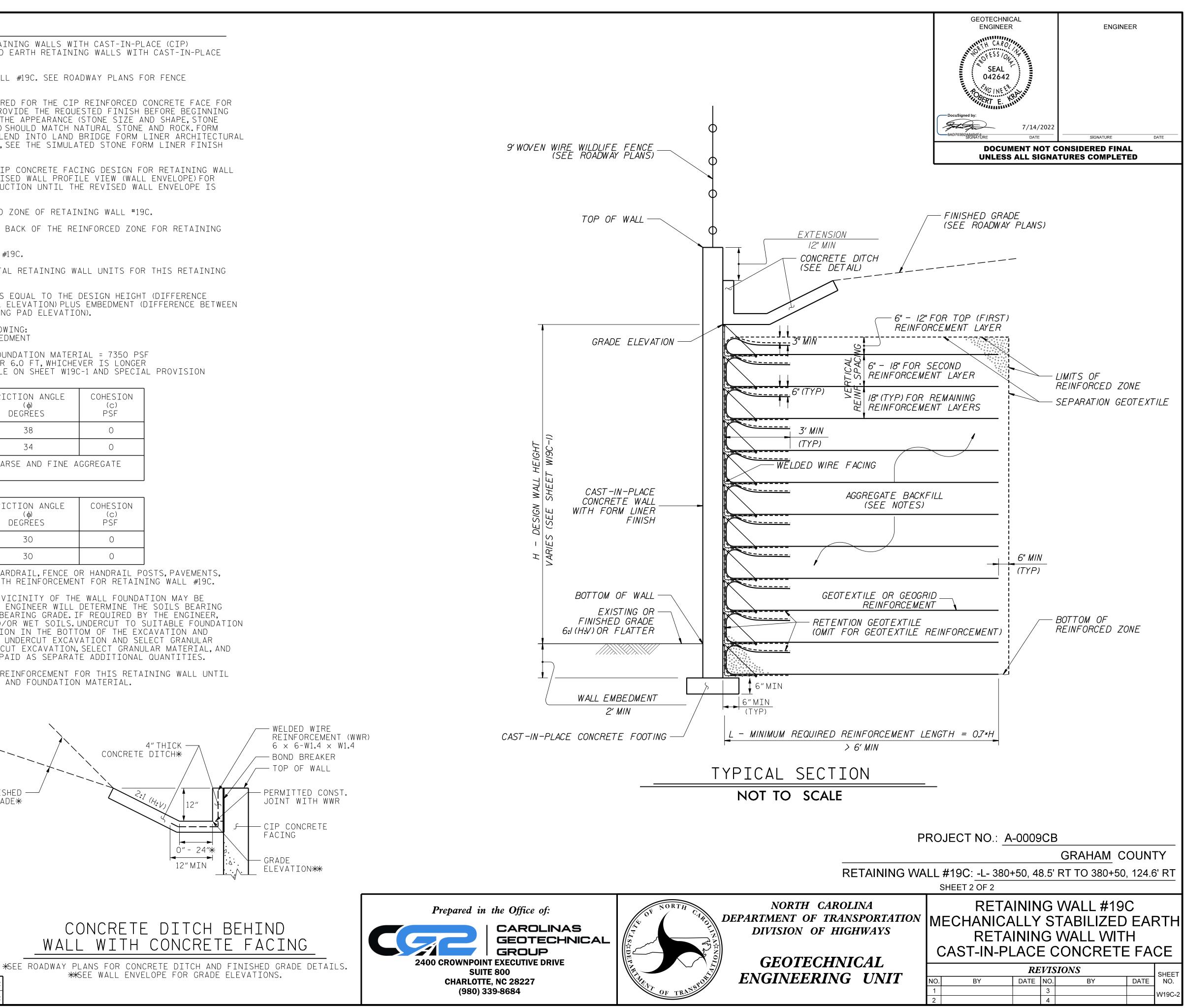
UNDERCUTTING SOFT AND/OR WET SOILS IN THE VICINITY OF THE WALL FOUNDATION MAY BE REQUIRED TO IMPROVE BEARING RESISTANCE. THE ENGINEER WILL DETERMINE THE SOILS BEARING RESISTANCE AFTER THE WALL IS EXCAVATED TO BEARING GRADE. IF REQUIRED BY THE ENGINEER, USE UNDERCUT EXCAVATION TO REMOVE SOFT AND/OR WET SOILS. UNDERCUT TO SUITABLE FOUNDATION SOILS. PLACE GEOTEXTILE FOR SOIL STABILIZATION IN THE BOTTOM OF THE EXCAVATION AND BACKFILL WITH SELECT GRANULAR MATERIAL.FOR UNDERCUT EXCAVATION AND SELECT GRANULAR MATERIAL SEE STANDARD SPECIFICATIONS. UNDERCUT EXCAVATION, SELECT GRANULAR MATERIAL, AND GEOTEXTILE FOR SOIL STABILIZATION WILL BE PAID AS SEPARATE ADDITIONAL QUANTITIES.

DO NOT PLACE CONCRETE, COARSE AGGREGATE, OR REINFORCEMENT FOR THIS RETAINING WALL UNTIL OBTAINING APPROVAL OF THE EXCAVATION DEPTH AND FOUNDATION MATERIAL.

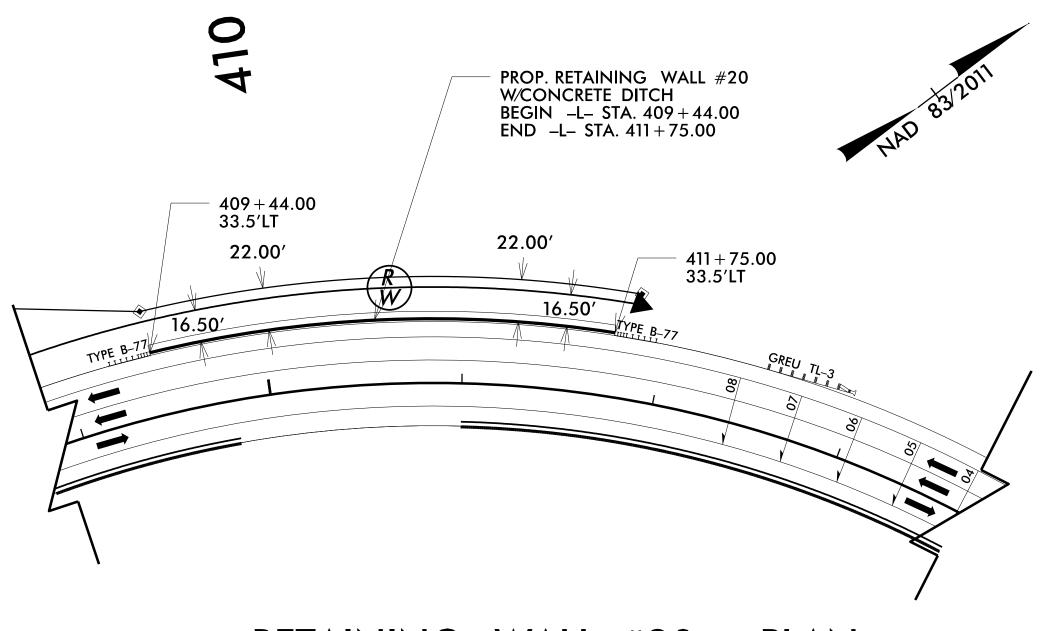
4" THICK -CONCRETE DITCH* FINISHED -GRADE米

CONCRETE DITCH BEHIND WALL WITH CONCRETE FACING

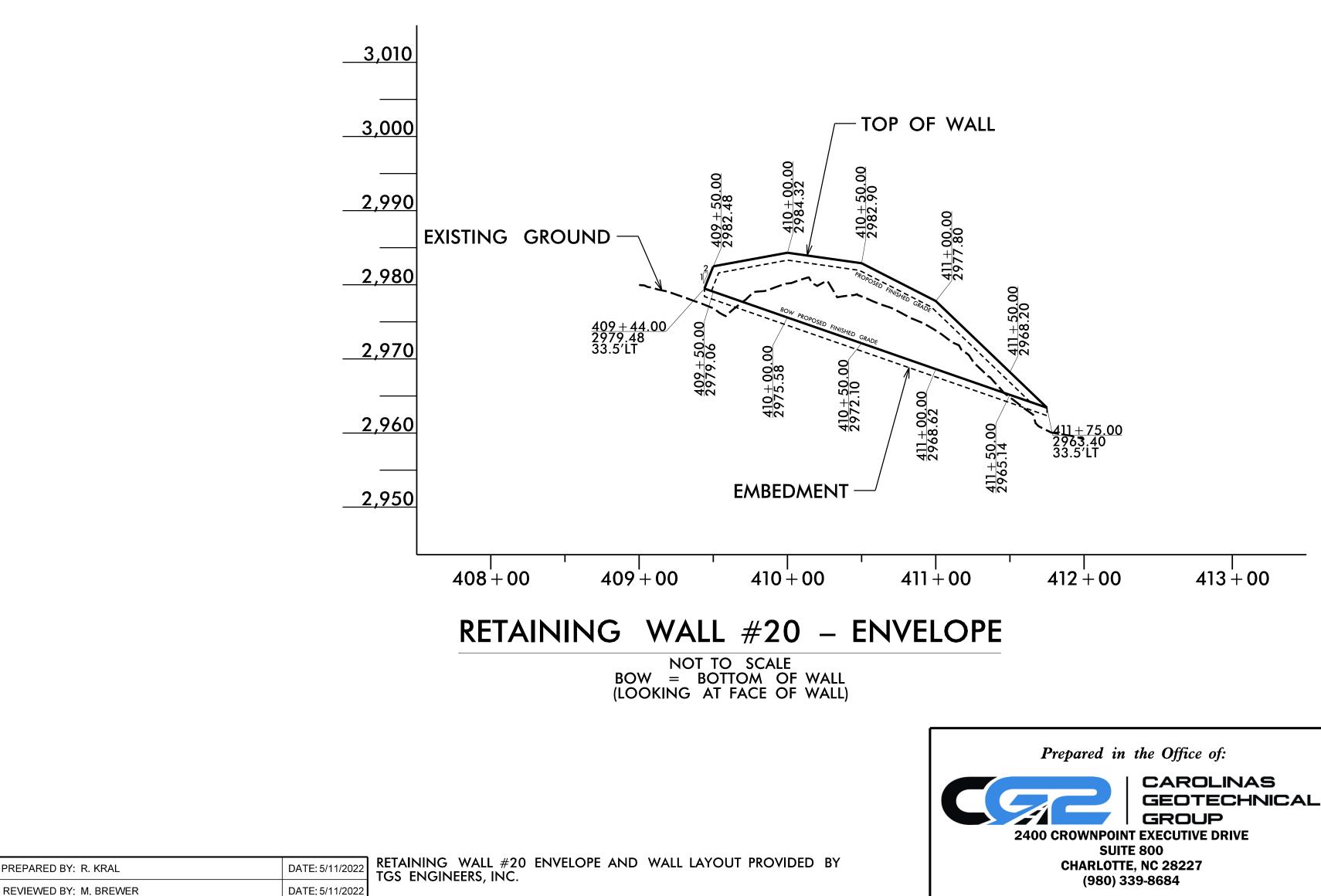
		AD L L	NUADWAI	***SEE	ı د W
PREPARED BY: R. KRAL	DATE: 7/14/2022			ANOLL	11 /
REVIEWED BY: M. BREWER	DATE: 7/14/2022				



RETAINING WALL #20:

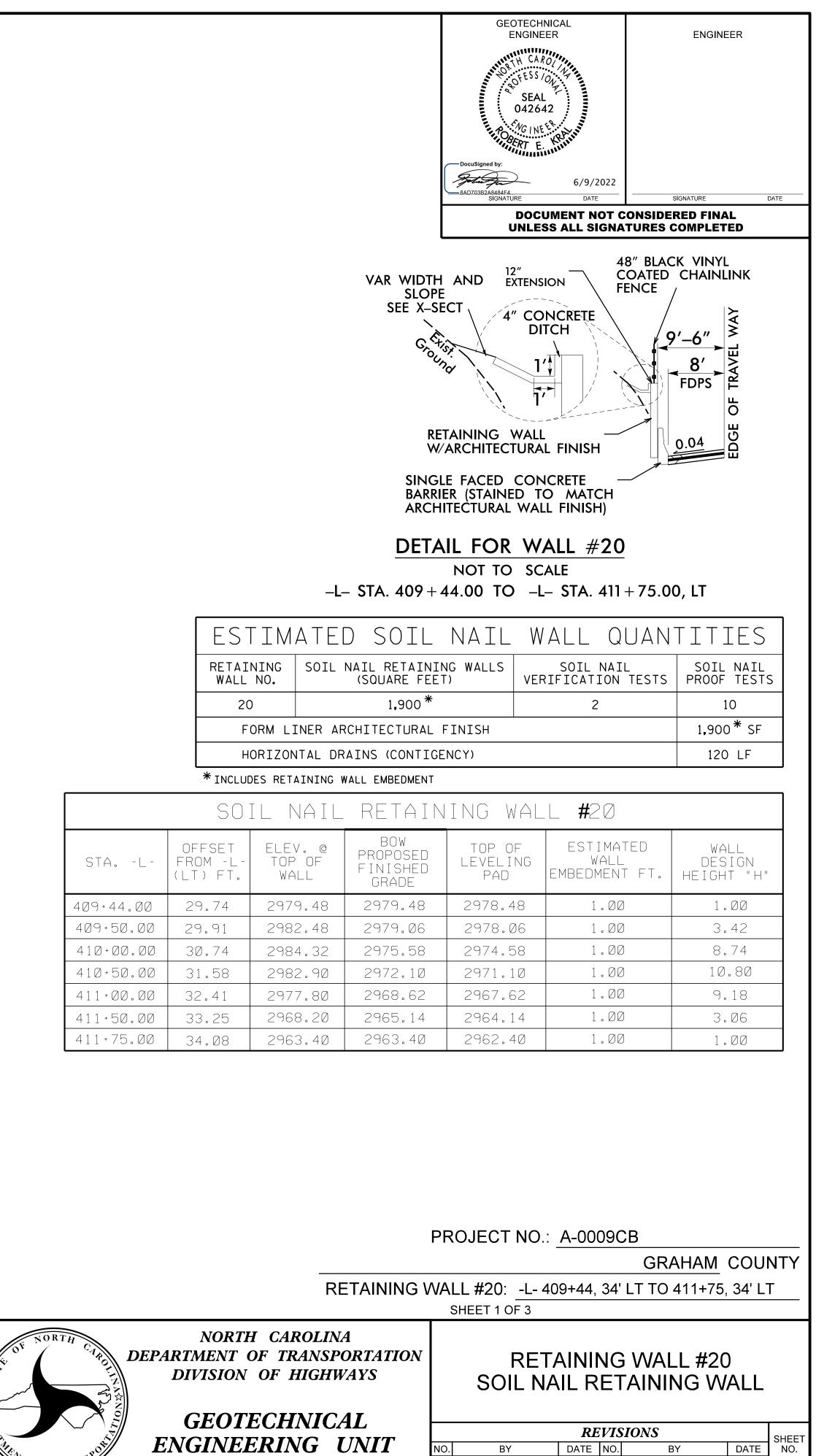








NOT TO SCALE



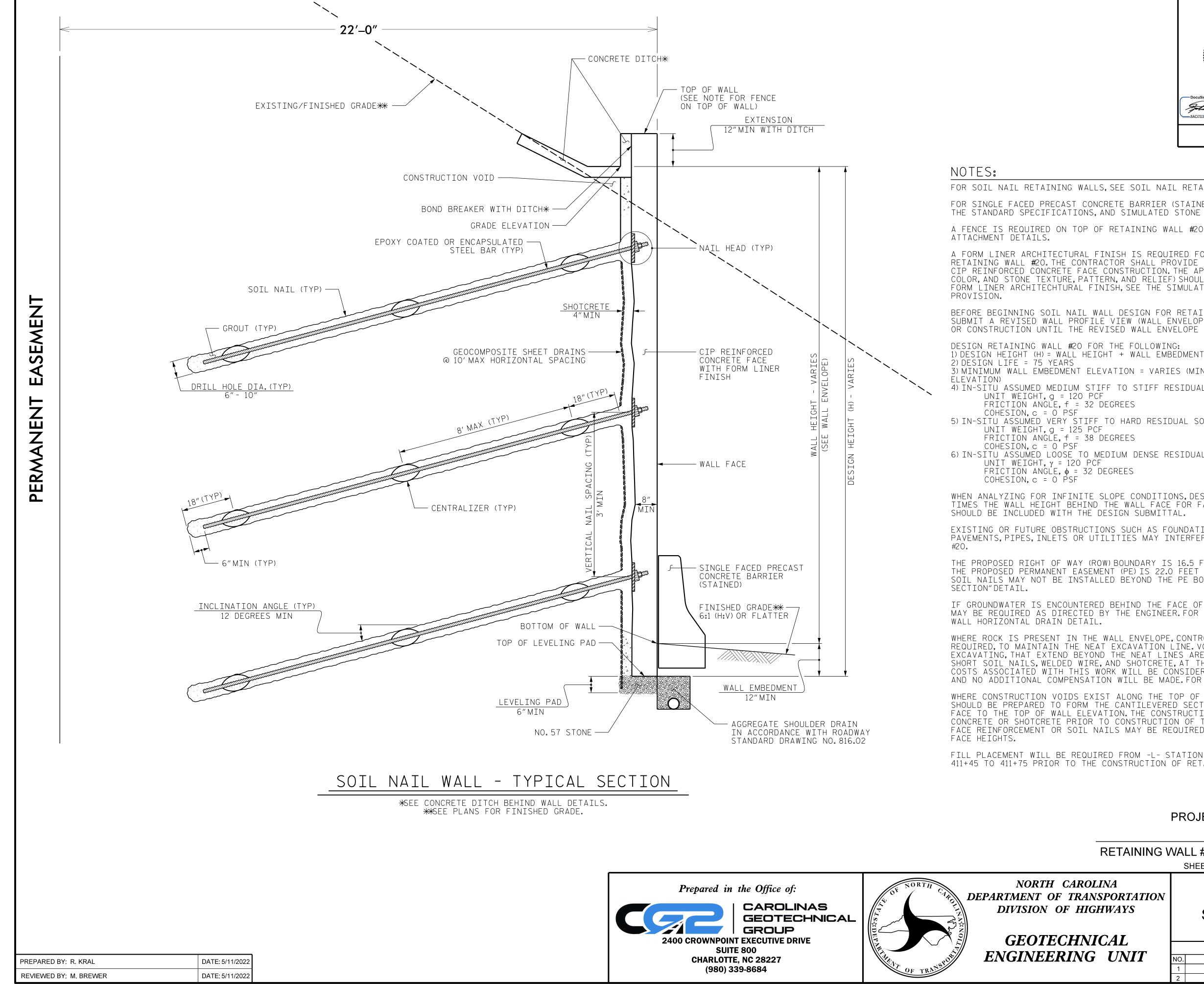
3

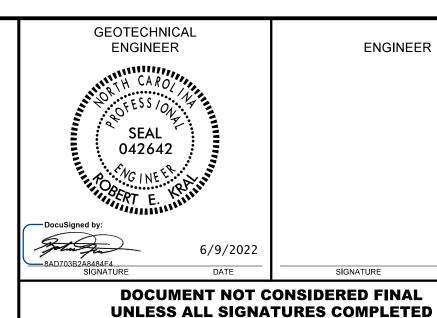
4

W20-1

F
*

STAL-	OFFS FROM (LT)
409+44.00	29.7
409+50.00	29.9
410+00.00	30.7
410+50.00	31.5
411+00.00	32.4
411+50.00	33.2
411+75.00	34.(





ENGINEER

SIGNATURE

DATE

FOR SOIL NAIL RETAINING WALLS, SEE SOIL NAIL RETAINING WALLS PROVISION.

FOR SINGLE FACED PRECAST CONCRETE BARRIER (STAINED), SEE ROADWAY PLANS, SECTION 857 OF THE STANDARD SPECIFICATIONS, AND SIMULATED STONE FORM LINER SPECIAL PROVISION.

A FENCE IS REQUIRED ON TOP OF RETAINING WALL #20. SEE ROADWAY PLANS FOR FENCE

A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP REINFORCED CONCRETE FACE FOR RETAINING WALL #20. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP REINFORCED CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, AND STONE TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK. FOR FORM LINER ARCHITECHTURAL FINISH, SEE THE SIMULATED STONE FORM LINER FNISH SPECIAL

BEFORE BEGINNING SOIL NAIL WALL DESIGN FOR RETAINING WALL #20, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL #20 FOR THE FOLLOWING:

3) MINIMUM WALL EMBEDMENT ELEVATION = VARIES (MIN.1 FT BELOW PROPOSED FINISHED GRADE 4) IN-SITU ASSUMED MEDIUM STIFF TO STIFF RESIDUAL SOIL PARAMETERS:

UNIT WEIGHT, g = 120 PCF FRICTION ANGLE, f = 32 DEGREES

5) IN-SITU ASSUMED VERY STIFF TO HARD RESIDUAL SOIL PARAMETERS:

UNIT WEIGHT,g = 125 PCF Friction Angle,f = 38 degrees

6) IN-SITU ASSUMED LOOSE TO MEDIUM DENSE RESIDUAL SOIL PARAMETERS:

UNIT WEIGHT, γ = 120 PCF FRICTION ANGLE, ϕ = 32 DEGREES

COHESION.c = O PSF

WHEN ANALYZING FOR INFINITE SLOPE CONDITIONS, DESIGNERS SHOULD ANALYZE UP TO TWO (2) TIMES THE WALL HEIGHT BEHIND THE WALL FACE FOR FAILURE PLANE SEARCHES. THIS INFORMATION SHOULD BE INCLUDED WITH THE DESIGN SUBMITTAL.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH SOIL NAILS FOR RETAINING WALL

THE PROPOSED RIGHT OF WAY (ROW) BOUNDARY IS 16.5 FT FROM THE FACE OF RETAINING WALL #20. THE PROPOSED PERMANENT EASEMENT (PE) IS 22.0 FEET FROM THE FACE OF RETAINING WALL #20. SOIL NAILS MAY NOT BE INSTALLED BEYOND THE PE BOUNDARY.SEE "SOIL NAIL WALL - TYPICAL

IF GROUNDWATER IS ENCOUNTERED BEHIND THE FACE OF RETAINING WALL #20, HORIZONTAL DRAINS MAY BE REQUIRED AS DIRECTED BY THE ENGINEER. FOR HORIZONTAL DRAINS, SEE THE RETAINING

WHERE ROCK IS PRESENT IN THE WALL ENVELOPE, CONTROLLED BLASTING IS RECOMMENDED, BUT NOT REQUIRED, TO MAINTAIN THE NEAT EXCAVATION LINE. VOIDS, RESULTING FROM BLASTING OR EXCAVATING, THAT EXTEND BEYOND THE NEAT LINES ARE TO BE FILLED WITH A COMBINATION OF SHORT SOIL NAILS, WELDED WIRE, AND SHOTCRETE, AT THE DISCRETION OF THE ENGINEER. THE COSTS ASSOCIATED WITH THIS WORK WILL BE CONSIDERED INCIDENTAL TO WALL CONSTRUCTION AND NO ADDITIONAL COMPENSATION WILL BE MADE. FOR BLASTING, SEE THE BLASTING PROVISION.

WHERE CONSTRUCTION VOIDS EXIST ALONG THE TOP OF RETAINING WALL #20, THE CONTRACTOR SHOULD BE PREPARED TO FORM THE CANTILEVERED SECTION OF THE CIP REINFORCED CONCRETE FACE TO THE TOP OF WALL ELEVATION. THE CONSTRUCTION VOID SHOULD BE FILLED WITH CONCRETE OR SHOTCRETE PRIOR TO CONSTRUCTION OF THE CONCRETE DITCH. ADDITIONAL WALL FACE REINFORCEMENT OR SOIL NAILS MAY BE REQUIRED FOR TALLER THAN TYPICAL CANTILEVER

FILL PLACEMENT WILL BE REQUIRED FROM -L- STATION 409+00 TO 409+70 AND -L- STATION 411+45 TO 411+75 PRIOR TO THE CONSTRUCTION OF RETAINING WALL #20.

PROJECT NO .: A-0009CB

GRAHAM COUNTY RETAINING WALL #20: -L- 409+44, 34' LT TO 411+75, 34' LT SHEET 2 OF 3

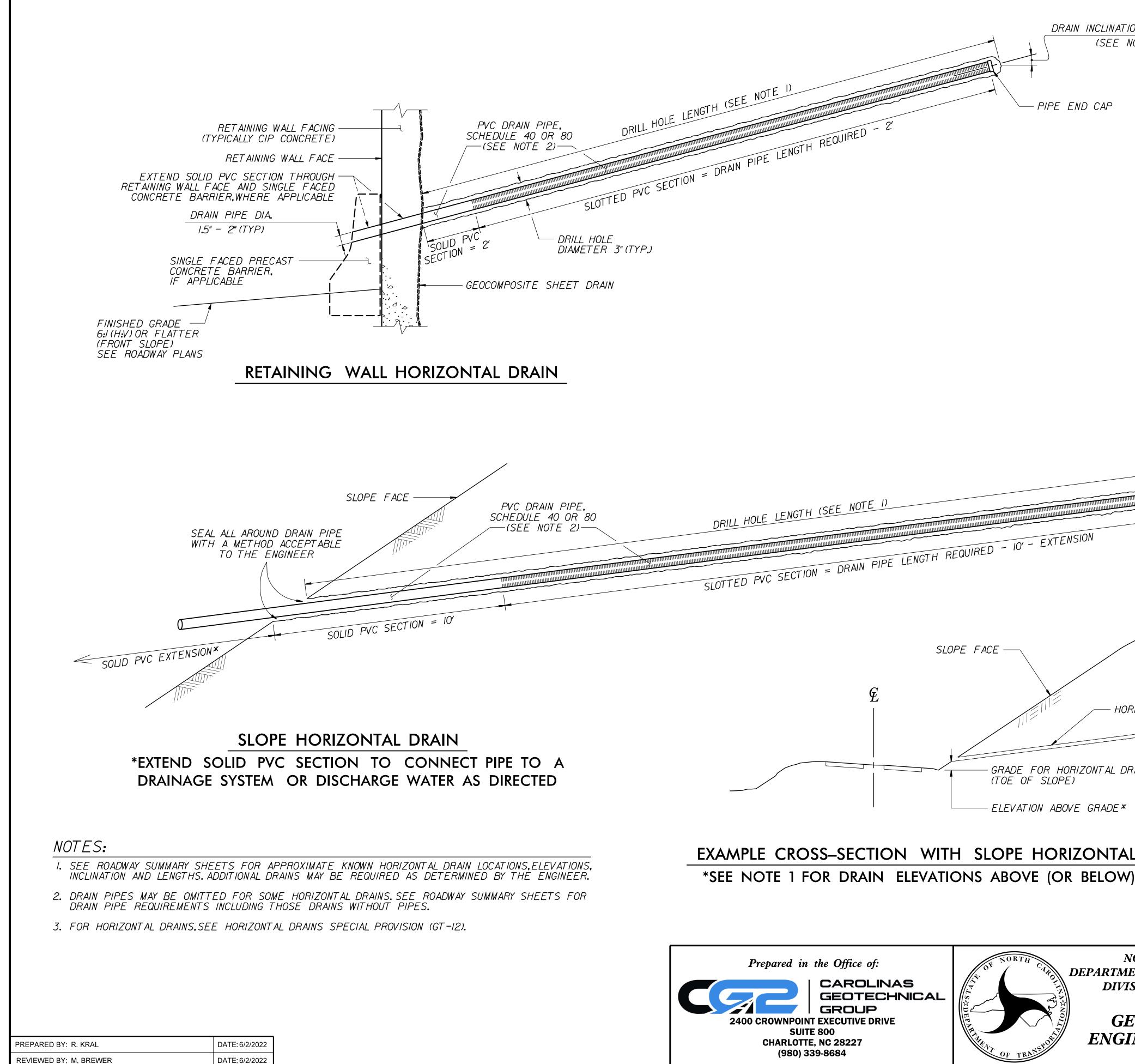
NORTH CAROLINA **DEPARTMENT OF TRANSPORTATION** DIVISION OF HIGHWAYS

RETAINING WALL #20 SOIL NAIL RETAINING WALL

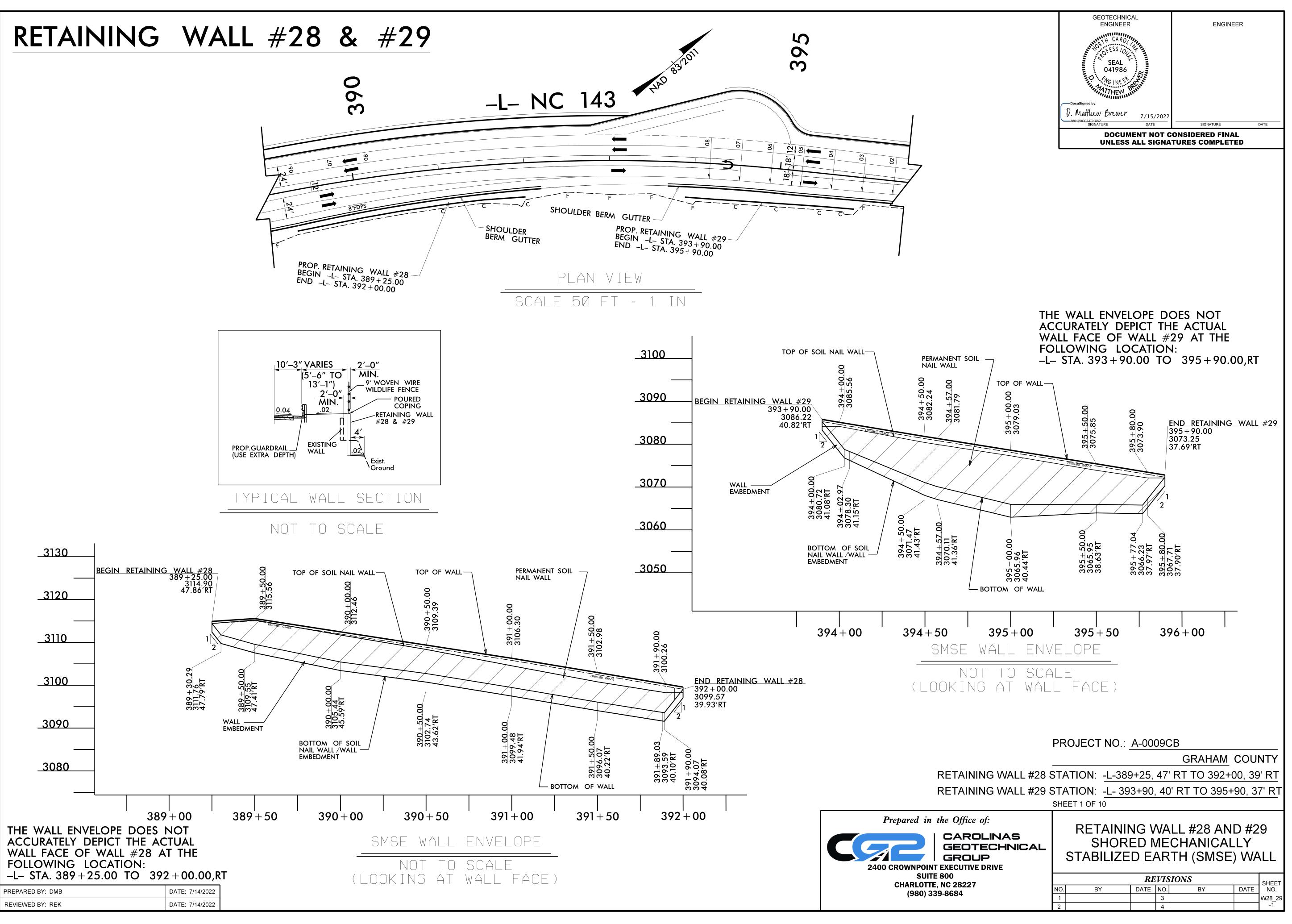
GEOTECHNICAL ENGINEERING UNIT

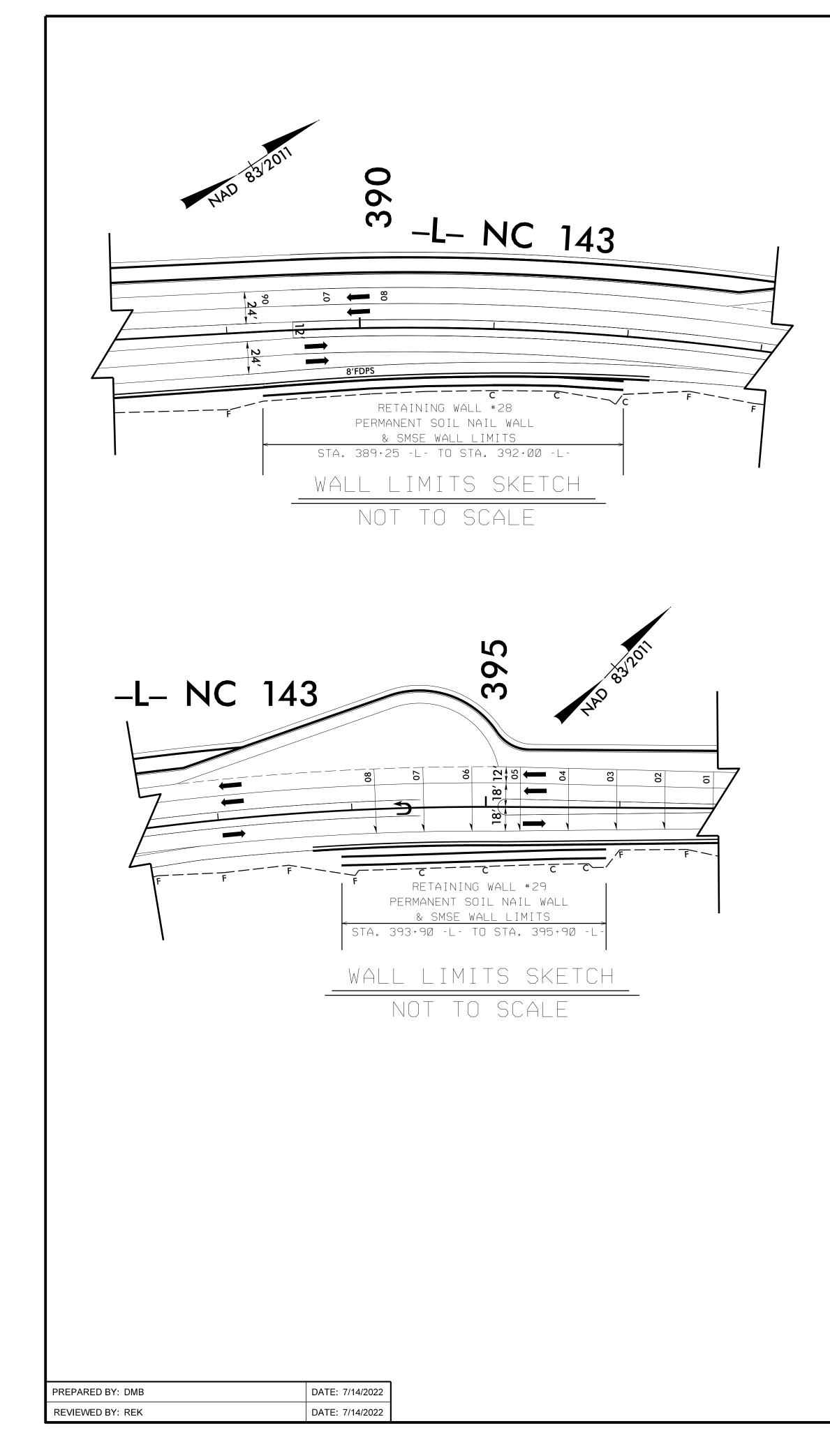
REVISIONS SHEET NO. BY DATE NO. BY DATE 3 W20-2

4



	GEOTECHNICAL ENGINEER	ENGINEER
ON ANGLE (TYP)	SEAL 042642	
NOTE I)	042642	
	DocuSigned by: 6/9/2022 8AD703B2A8484E4	
	DOCUMENT NOT C	SIGNATURE DATE CONSIDERED FINAL TURES COMPLETED
	DRAIN INCLINATION	
	PIPE END CAP	
RIZONTAL DRAIN		
RAIN		
L DRAIN		
	PROJECT NO.: A-00090	СВ
		GRAHAM COUNTY
	VALL #20:L- 409+44, 34' SHEET 3 OF 3	LI IU 4I I+75, 34 [°] LI
IORTH CAROLINA ENT OF TRANSPORTATION	RETAINING	
SION OF HIGHWAYS	SOIL NAIL RET	AINING WALL
EOTECHNICAL INEERING UNIT	REVIS	BY DATE NO.
	1 3 2 4	W20-3





	SMSE RETAINING WALL #28 INFORMATION								
STAL-	OFFSET RT FROM CL TO WALL FACE	TOP OF WALL	BOTTOM OF WALL	MINIMUM TOP OF LEVELING PAD	ESTIMATED MINIMUM SMSE WALL EMBEDMENT	* DESIGN SMSE WALL HEIGHT "H"	TOP OF Soil nail Wall	APPROX. Soil nail Wall Height	WALL REINFORCEMENT (MSE STRAP) LENGTH 'L'
389+25.00	47.86	3114.90	3114.40	3112.40	2.00	2.00	3115.00	2.6Ø	Ø.5XH OR 6 FT (MIN)
389+50.00	47.41	3115.56	3109.55	3107.55	2.00	7.51	3115.24	7.69	Ø.5XH OR 6 FT (MIN)
390+00.00	45.59	3112.46	3105.44	3102.44	3.00	9.52	3111.72	9.28	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
390+50.00	43.62	3109.39	3102.74	3100.74	2.00	8.15	31Ø8.37	7.63	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
391+00.00	41.94	3106.30	3099.48	3097.48	2.00	8.32	3104.54	7.Ø6	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
391+50.00	40.22	3102.98	3096.07	3094.07	2.00	8.41	3100.79	6.72	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
391+90.00	40.08	3100.26	3094.07	3092.07	2.00	7.69	3098.25	6.18	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
392+00.00	39.93	3099.57	3099.07	3097.07	2.00	2.00	3098.00	0.93	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)

* FOR DESIGN WALL HEIGHT "H" AND ADDITIONAL CONSTRUCTION DETAILS, SEE ADDITIONAL SHEETS ALL TABLE DIMENSIONS ARE GIVEN IN FEET

	SMSE RETAINING WALL #29 INFORMATION								
STAL-	OFFSET RT FROM CL TO WALL FACE	TOP OF WALL	BOTTOM OF WALL	MINIMUM TOP OF LEVELING PAD	ESTIMATED MINIMUM SMSE WALL EMBEDMENT	* DESIGN SMSE WALL HEIGHT "H"	TOP OF Soil nail Wall	APPROX. Soil nail Wall Height	WALL REINFORCEMENT (MSE STRAP) LENGTH 'L'
393+90.00	40.82	3Ø86.22	3085.72	3Ø83.72	2.00	2.00	3084.15	Ø.43	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
394+00.00	41.08	3085.56	3080.72	3078.72	2.00	6.34	3084.15	5.43	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
394+50.00	41.43	3Ø82.24	3071.47	3068.48	3.ØØ	13.27	3080.39	11.92	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
394+57.00	41.36	3081.79	3070.11	3Ø67.11	3.00	14.18	3079.90	12.79	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
395+00.00	40.44	3079.03	3065.96	3062.96	3.00	15.57	3077.29	14.33	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
395+50.00	38.63	3075.85	3065.95	3063.95	2.00	11.40	3073.90	9.95	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
395+77.04	37.97	3074.09	3066.23	3064.23	2.00	9.36	3072.30	8.07	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
395+80.00	37.90	3073.90	3068.71	3066.71	2.00	6.69	3Ø72.12	5.41	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)
395+90.00	37.69	3073.25	3072.75	3070.75	2.00	2.00	3Ø72.12	1.37	LOWER: Ø.5XH OR 6 FT (MIN) UPPER: Ø.7XH OR 10 FT (MIN)

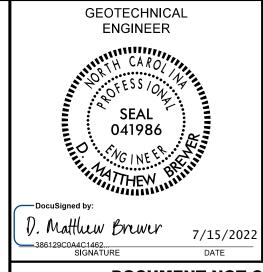
* FOR DESIGN WALL HEIGHT "H" AND ADDITIONAL CONSTRUCTION DETAILS, SEE ADDITIONAL SHEETS

ALL TABLE DIMENSIONS ARE GIVEN IN FEET



ENGINEER

DATE



SIGNATURE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO.: A-0009CB

GRAHAM COUNTY

RETAINING WALL #28 STATION: -L- 389+25, 47' RT TO 392+00, 39' RT RETAINING WALL #29 STATION: -L- 393+90, 40' RT TO 395+90, 37' RT SHEET 2 OF 10

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

RETAINING WALL #28 AND #29 SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL

	REVISIONS					
NO.	BY	DATE	NO.	BY	DATE	SHEET NO.
1			3			W28_29
2			4			-2

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ES	TIMATED SMSE W	'ALL QUANTI	TIES
SHORE	ED MSE RETAINING WAL	L #28 2,31Ø	SQ. FT.
SHORE	ED MSE RETAINING WAL	L #29 2,37Ø	SQ. FT.
ESTIM	ATED SOIL NAIL	WALL QUAN	TITIES
RETAINING WALL #	SOIL NAIL RETAINING WALLS (SQUARE FEET)	SOIL NAIL VERIFICATION TESTS	SOIL NAIL PROOF TESTS
28	1 , 960 *	2	7
29	2,110 *	2	7

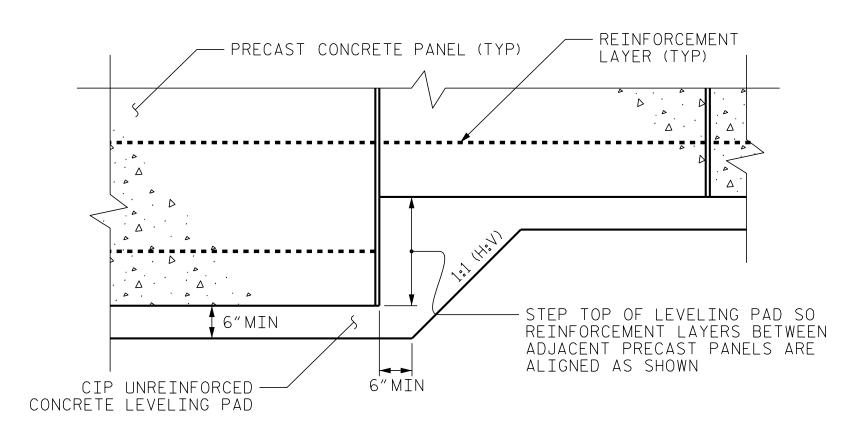
* INCLUDES RETAINING WALL EMBEDMENT

FRO	NT SLOPE WALL EME	BEDMENT
SLOPE IN FRO	ONT OF STRUCTURES	MINIMUM EMBEDMENT DEPTH
	FOR WALLS	H/2Ø
HORIZONTAL	FOR ABUTMENTS	H/1Ø
3.ØH:1.ØV	WALLS	H/1Ø
2.5H:1.ØV	WALLS	H/8.5
2.ØH:1.ØV	WALLS	H/7
1.5H:1.ØV	WALLS	H/5
1.25H:1.ØV	WALLS	H/4
1.ØH:1.ØV	WALLS	H/3

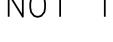
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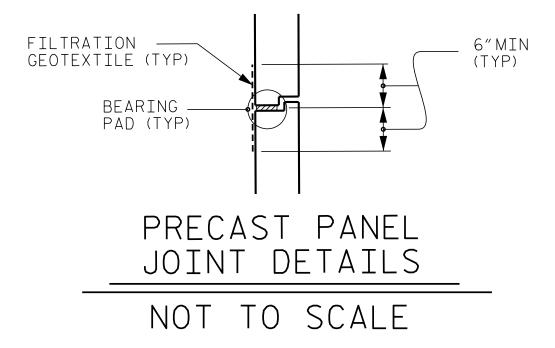
1) MAINTAIN A MINIMUM BENCH WIDTH OF 4 FT IN FRONT OF THE WALL FOR ITS ENTIRE LENGTH. 2) MINIMUM EMBEDMENT DEPTH OF 2 FT, UNLESS LARGER DEPTHS DICTATED BY ABOVE TABLE. 3) MAXIMUM SLOPE OF 1H:1V WILL BE MAINTAINED ON FRONT SLOPES FOR THE ENTIRE LENGTH OF THE WALL. 4) SUBMIT WITH THE WALL DESIGN INTERNAL, EXTERNAL, AND GLOBAL STABILITY ANALYSES. REFERENCE SPECIAL PROVISION GT-7 FOR SMSE WALL.

PREPARED BY: DMB	DATE: 7/14/2022
REVIEWED BY: REK	DATE: 7/14/2022

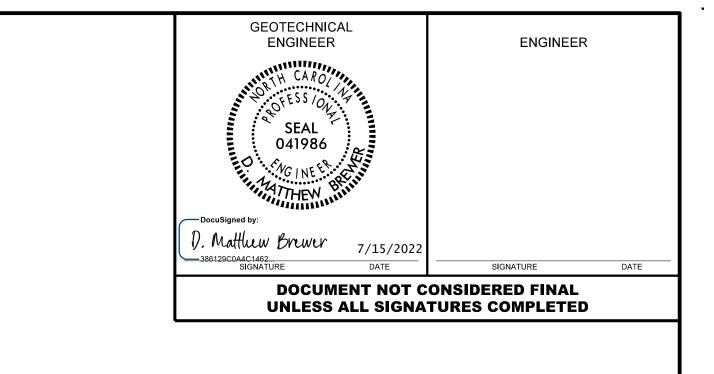












GRAHAM COUNTY

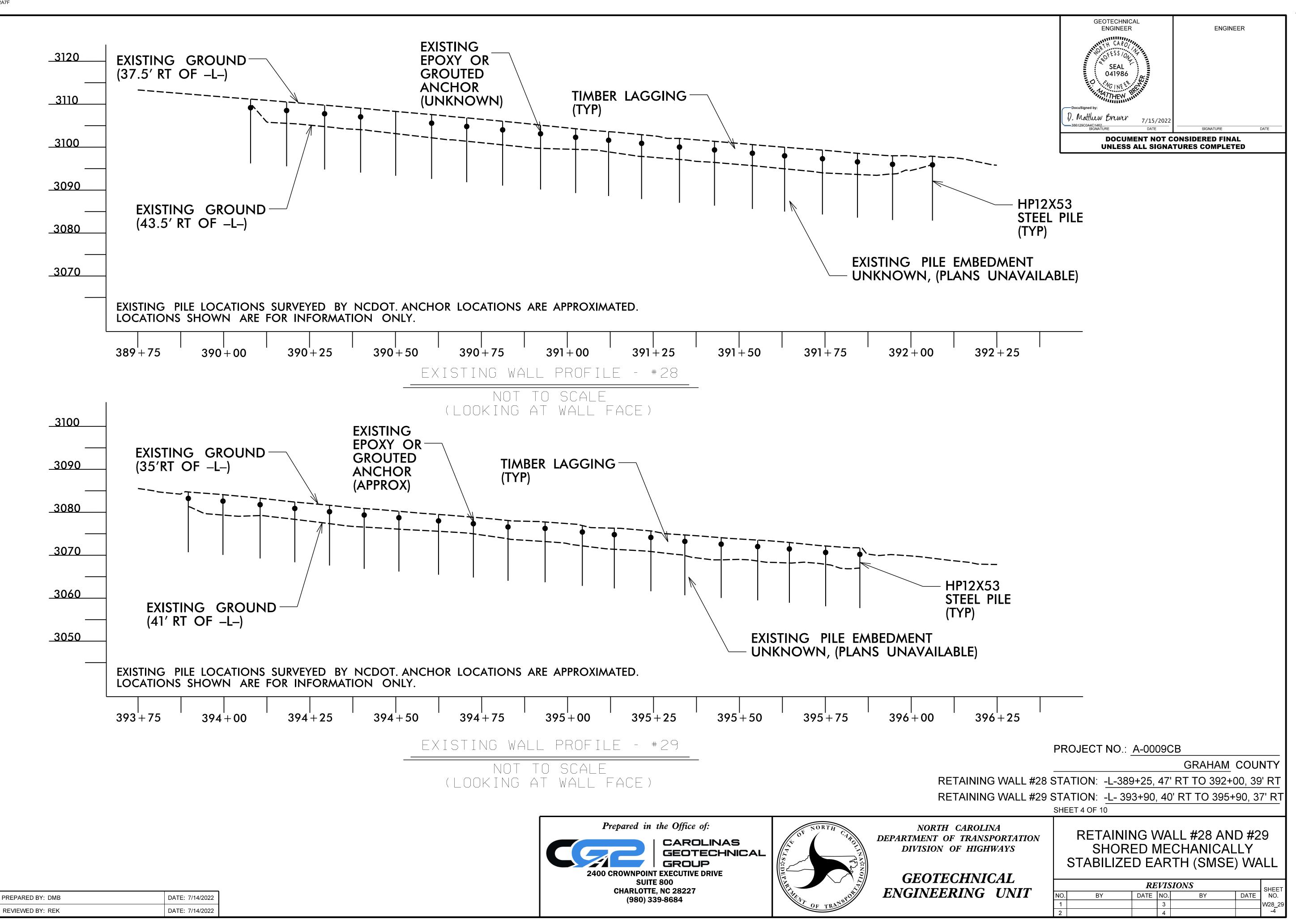
RETAINING WALL #28 STATION: -L-389+25, 47' RT TO 392+00, 39' RT RETAINING WALL #29 STATION: -L- 393+90, 40' RT TO 395+90, 37' RT SHEET 3 OF 10

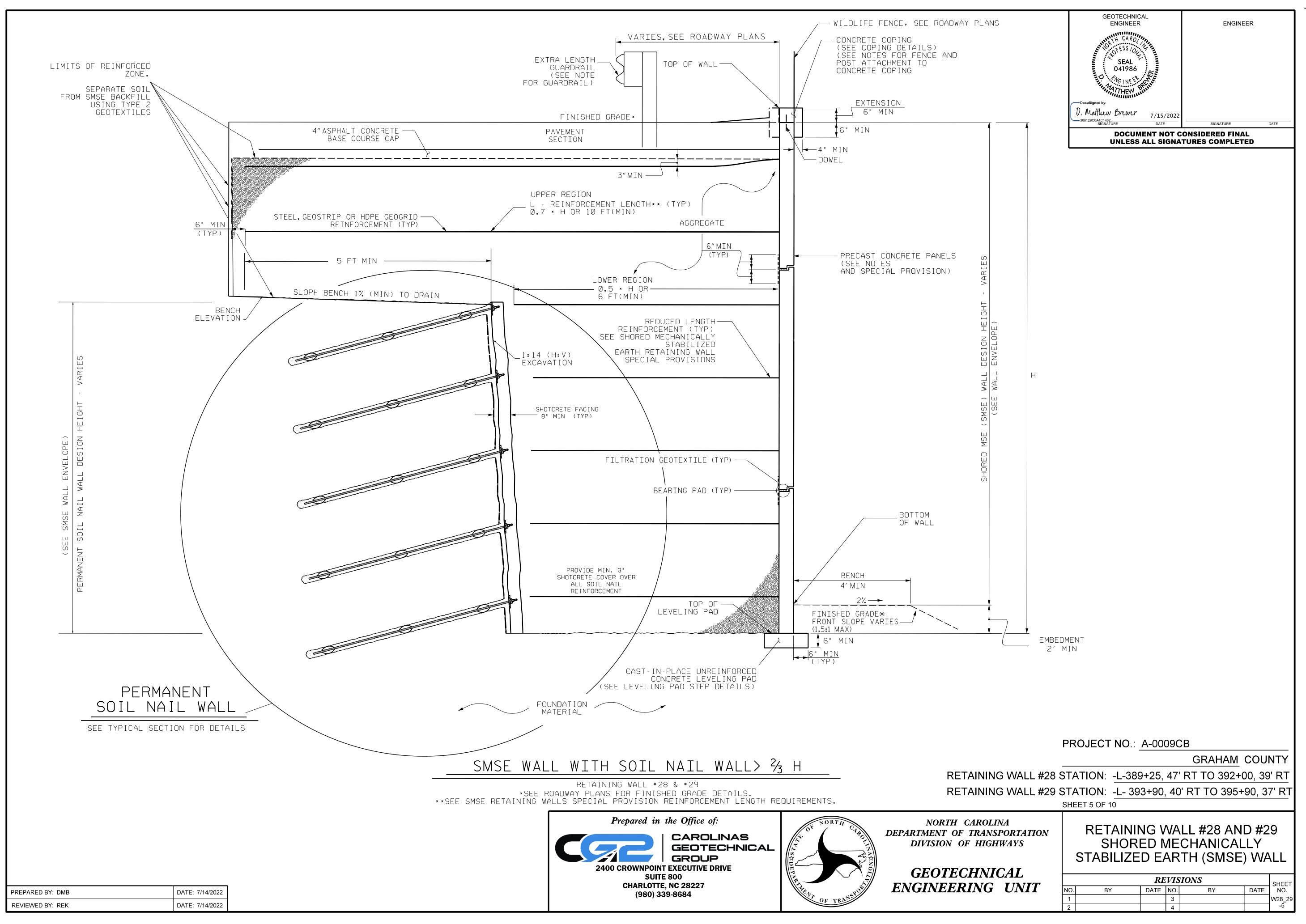
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

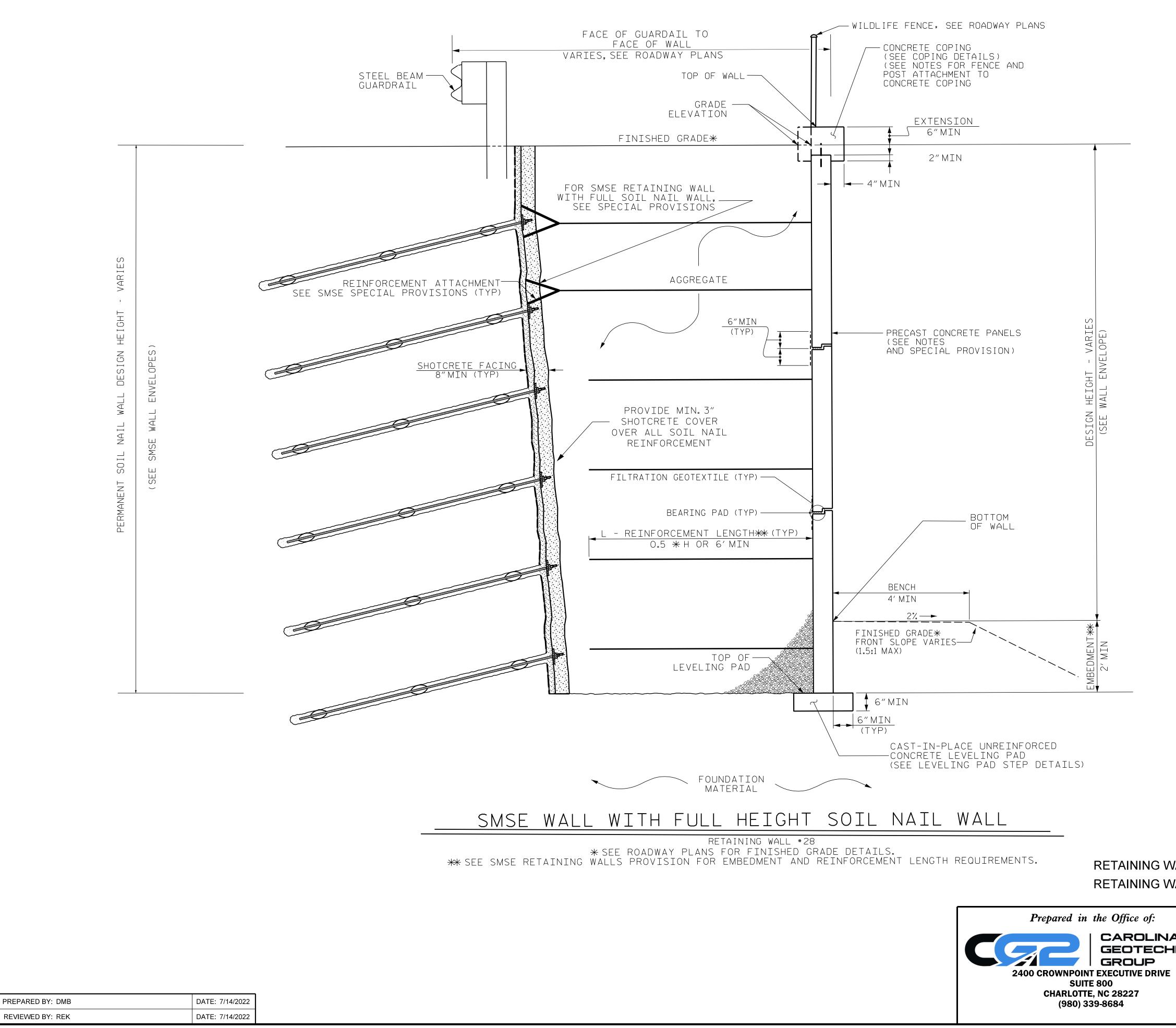
GEOTECHNICAL ENGINEERING UNIT

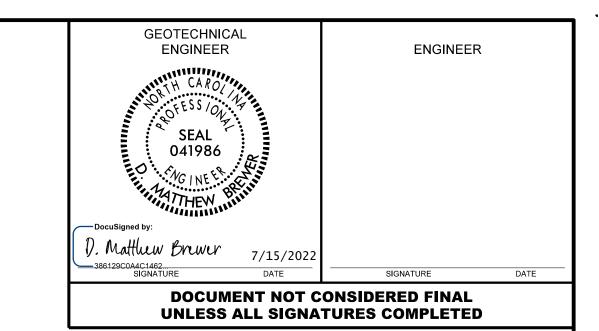
RETAINING WALL #28 AND #29 SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL

	REVISIONS						
NO.	BY	DATE	NO.	BY	DATE	SHEET NO.	
1			3			W28_29	
2			4			-3	









PROJECT NO.: A-0009CB

GRAHAM COUNTY

RETAINING WALL #28 STATION: -L-389+25, 47' RT TO 392+00, 39' RT RETAINING WALL #29 STATION: -L- 393+90, 40' RT TO 395+90, 37' R1 SHEET 6 OF 10

in	the Office of:
	CAROLINAS

GEOTECHNICAL GROUP

RETAINING WALL #28 AND #29 SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL

SHE		DNS	EVIS	RE		
	DATE	BY	NO.	DATE	BY	NO.
W28_			3			1
-6			4			2

NOTES:

FOR SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL RETAINING WALLS, SEE SHORED MECHANICALLY STABILIZED EARTH RETAINING WALLS SPECIAL PROVISION. FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS PROVISION. FOR STEEL BEAM GUARDRAIL, SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS. USE AN SMSE WALL SYSTEM WITH PRECAST PANELS FOR THIS RETAINING WALL. DO NOT USE FINE AGGREGATE IN THE REINFORCED ZONE OF RETAINING WALL *28 AND *29. A SMOOTH ARCHITECTURAL FINISH IS REQUIRED FOR PRECAST CONCRETE PANELS FOR RETAINING WALL *28 AND *29.

A SEPARATION GEOTEXTILE IS REQUIRED AT THE BACK OF THE REINFORCED ZONE FOR RETAINING WALL *28 AND *29. BEFORE BEGINNING SMSE WALL DESIGN FOR RETAINING WALL *28 AND *29, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED. DESIGN RETAINING WALL *28 AND *29 FOR WALL HEIGHTS EQUAL TO THE DESIGN HEIGHT (DIFFERENCE BETWEEN FINISHED GRADE/TOP OF WALL LEEVATION AND BOTTOM OF WALL ELEVATION) PLUS EMBEDMENT (DIFFERENCE BETWEEN BOTTOM OF WALL ELEVATION AND TOP OF LEVELING PAD ELEVATION).

DESIGN RETAINING WALL #28 AND #29 FOR THE FOLLOWING: 1) H = DESIGN HEIGHT + EMBEDMENT 2) DESIGN LIFE = 75 YEARS

3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL: RETAINING WALL #28: 2,000 PSF

RETAINING WALL #29:2,850 PSF

4) MINIMUM MSE REINFORCEMENT LENGTH (L) = VARIES, SEE TABLE ON SHEET W28_29-2 5) MINIMUM SOIL NAIL REINFORCEMENT LENGTHS ARE BASED ON SNAIL. 6) MINIMUM EMBEDMENT DEPTH = VARIES, SEE TABLE ON SHEET W28_29-2

7) REINFORCED ZONE AGGREGATE PARAMETERS:

AGGREGATE TYPE *	UNIT WEIGHT (y) PCF	FRICTION ANGLE (¢) DEGREES	COHESION (C) PSF		
COARSE	110	38	Ø		
FINE	115	34	Ø		
* SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE					

* SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE MATERIAL REQUIREMENTS.

9) IN-SITU ASSUMED MATERIAL PARAMETERS:

MATERIAL TYPE	UNIT WEIGHT (y) PCF	FRICTION ANGLE (ф) DEGREES	COHESION (C) PSF
BACKFILL	120	32	0
FOUNDATION	120	32	0

DESIGN RETAINING WALL #28 AND #29 FOR A LIVE LOAD (TRAFFIC) SURCHARGE. EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH MSE AND SOIL NAIL REINFORCEMENT FOR RETAINING WALL #28 AND #29. DO NOT PLACE LEVELING PAD CONCRETE, AGGREGATE OR REINFORCEMENT FOR MSE WALL PORTION OF RETAINING WALL #28 AND #29 UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.

FOR WILDLIFE FENCES ON THE TOP OF THE RETAINING WALL, SEE ROADWAY PLANS FOR FENCE ATTACHMENT DETAILS. FOR SOIL NAIL RETAINING WALLS, SEE SMSE RETAINING WALL SPECIAL PROVISION. THE SMSE WALL DESIGNER SHALL CONSULT WITH THE SOIL NAIL WALL DESIGNER TO VERIFY LOCATIONS WHERE "TEMPORARY SHORING" MAY BE REQUIRED FOR THE RETAINING WALL IN ACCORDANCE WITH THE TEMPORARY SHORING PROVISION. SEE TRAFFIC CONTROL PLANS. IN LOCATIONS WHERE "PERMANENT SOIL NAIL WALL" IS USED, PAYMENT WILL NOT BE MADE FOR "TEMPORARY SHORING" FOR TRAFFIC CONTROL. THE PERMANENT SOIL NAIL WALL HEIGHT IS AN ESTIMATE ONLY, THAT IS BASED ON THE ANTICIPATED EXCAVATION PLUS THE MINIMUM EMBEDMENT LISTED.

WHERE APPLICABLE, DESIGN SOIL NAIL WALL REINFORCEMENT INCLINATION TO ACCOUNT FOR EXISTING OR FUTURE UTILITY CONFLICTS BEHIND THE SOIL NAIL WALL. VERIFY UTILITY LOCATION AND ELEVATION BEFORE BEGINNING SOIL NAIL WALL DESIGN OR CONSTRUCTION. "TOP OF SOIL NAIL WALL" AS SHOWN IN THE WALL ENVELOPE REPRESENTS THE APPROXIMATE GRADE ELEVATION AT A DISTANCE OF Ø.5 TIMES THE PROPOSED WALL HEIGHT ("H") AT THAT STATION OR ELEVATION AT THE TOP OF THE EXISTING WALL.

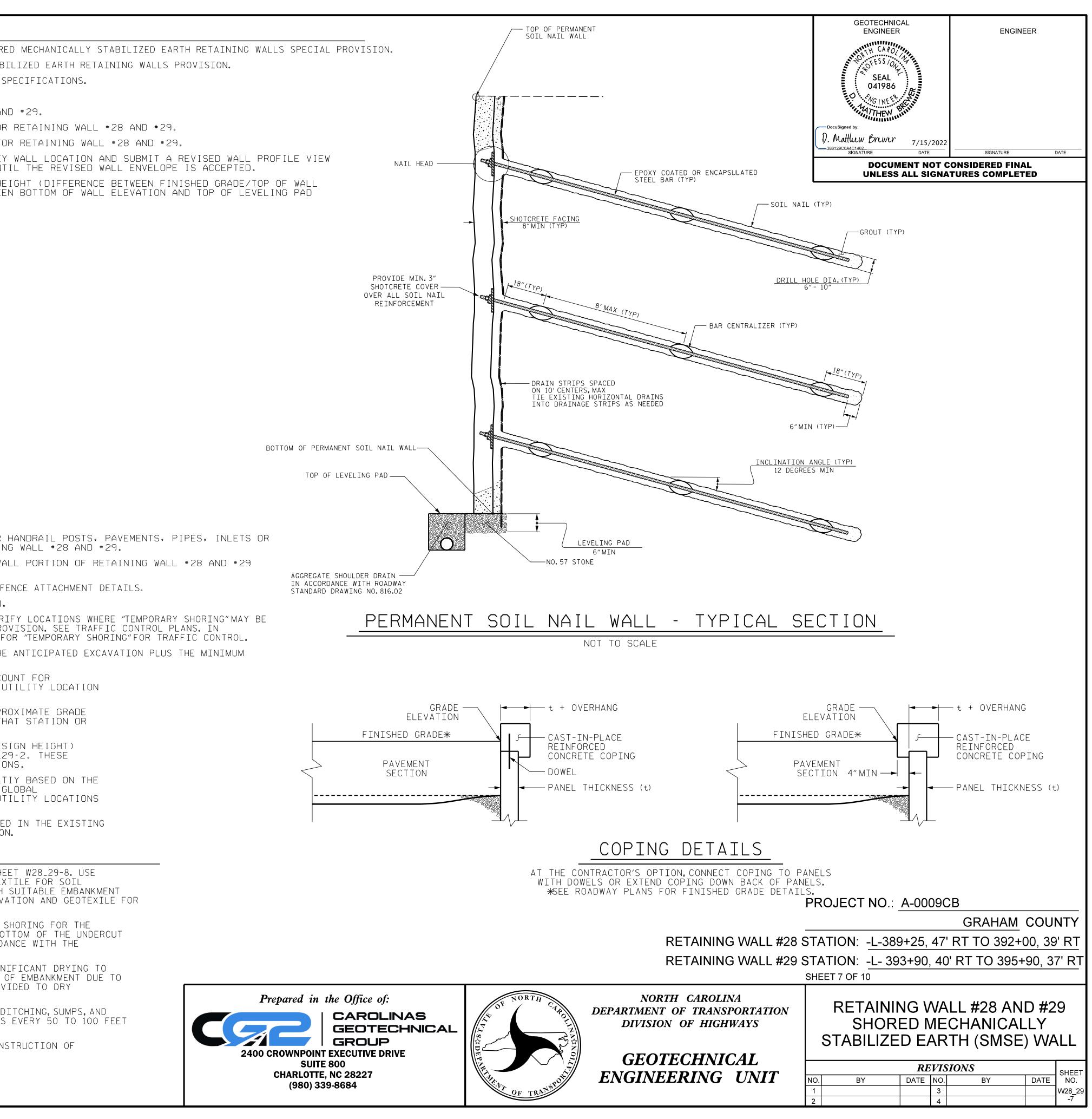
THE ESTIMATED SOIL NAIL WALL QUANTITY IS BASED ON 0.5 TIMES "H" (SMSE DESIGN HEIGHT) INCLUDING THE MINIMUM EMBEDMENT LISTED IN THE DESIGN TABLE ON SHEET W28_29-2. THESE VALUES ARE PROVIDED AS AN ESTIMATE ONLY AND MAY VARY DUE TO SITE CONDITIONS. THE SOIL NAIL WALL DESIGNER IS RESPONSIBLE FOR DETERMINING GLOBAL STABILTLY BASED ON THE FINISHED SMSE WALL. A MINIMUM FACTOR OF SAFETY OF 1.35 IS REQUIRED FOR GLOBAL STABILITY. SUBMIT THESE RESULTS WITH THE WALL DESIGN PACKAGE. VERIFY UTILITY LOCATIONS AND ELEVATIONS BEFORE BEGINNING MSE WALL DESIGN OR CONSTRUCTION.

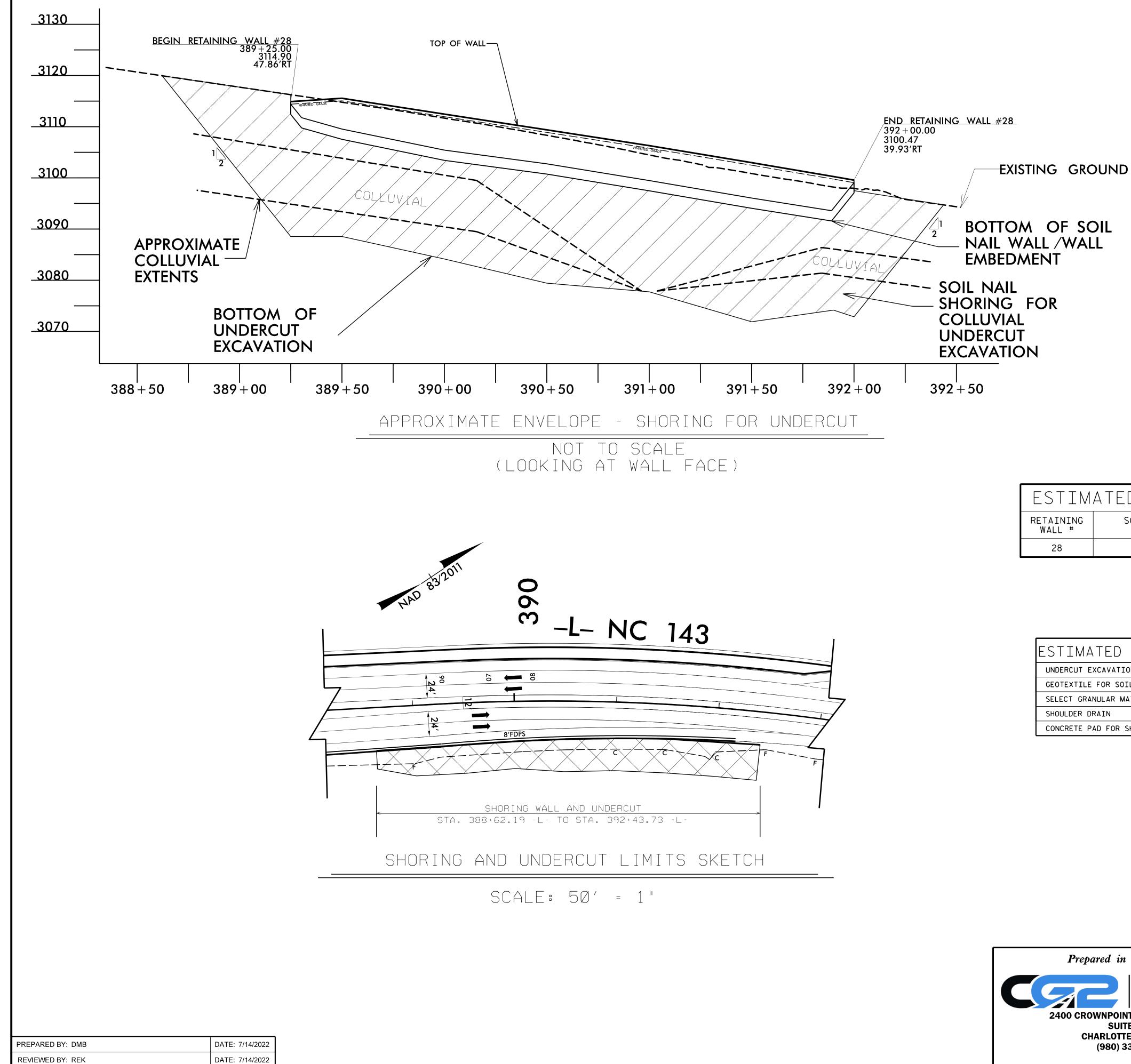
CONTRACTOR SHALL BE MADE AWARE THAT GRAVELLY SOILS AND BOULDER FILL WERE USED IN THE EXISTING ROADWAY EMBANKMENT AND MAY BE ENCOUNTERED DURING SOIL NAIL WALL CONSTRUCTION.

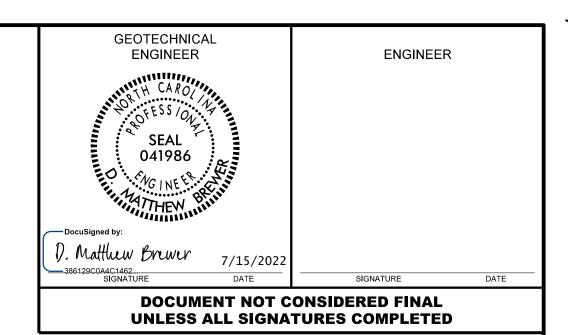
SPECIAL NOTES:

UNDERCUTTING COLLUVIAL SOILS BELOW THE SMSE WALL IS REQUIRED AS SHOWN ON SHEET W28_29-8. USE UNDERCUT EXCAVATION TO REMOVE SOILS AS DIRECTED BY THE ENGINEER. PLACE GEOTEXTILE FOR SOIL STABILIZATION WHEN NEEDED IN THE BOTTOM OF THE EXCAVATION AND BACKFILL WITH SUITABLE EMBANKMENT MATERIAL.FOR UNDERCUT EXCAVATION SEE STANDARD SPECIFICATIONS. UNDERCUT EXCAVATION AND GEOTEXILE FOR SOIL STABILIZATION WILL BE PAID AS SEPARATE ADDITIONAL QUANTITIES. EXTEND SOIL NAIL SHORING TO BOTTOM OF UNDERCUT EXCAVATION. DESIGN SOIL NAIL SHORING FOR THE HEIGHT EQUAL TO THE DIFFERENCE BETWEEN THE EXISTING ROADWAY GRADE AND THE BOTTOM OF THE UNDERCUT EXCAVATION. SOIL NAIL SHORING FOR UNDERCUT EXCAVATION WILL BE PAID IN ACCORDANCE WITH THE TEMPORARY SOIL NAIL SHORING FOR COLLUVIAL UNDERCUT SPECIAL PROVISION. THE COLLUVIAL SOILS ARE SUITABLE FOR USE AS EMBANKMENT BUT WILL REQUIRE SIGNIFICANT DRYING TO ACHIEVE THE REQUIRED DENSITY. DO NOT USE COLLUVIAL SOILS IN THE UPPER 3 FEET OF EMBANKMENT DUE TO THE PRESENCE OF BOULDERS AND COBBLES. NO ADDITIONAL COMPENSATION WILL BE PROVIDED TO DRY COLLUVIAL SOILS OR FOR DOUBLE-HANDLING SOILS. CONTROL GROUNDWATER DURING AND AT THE BOTTOM OF UNDERCUT EXCAVATION USING DITCHING, SUMPS, AND PERMANENT SHOULDER DRAINS AS DIRECTED BY THE ENGINEER. OUTLET SHOULDER DRAINS EVERY 50 TO 100 FEET AS DIRECTED BY THE ENGINEER. THE EXISTING ANCHORED PILE PANEL WALLS WILL BE DEMOLISHED AS PART OF THE CONSTRUCTION OF RETAINING WALL #28 AND #29.

PREPARED BY: DMB	DATE: 7/14/2022
REVIEWED BY: REK	DATE: 7/14/2022







ED	SOIL	NAIL	WALL	QUAN	TITIES
	NAIL SHOP		SOIL VERIFICATI		SOIL NAIL PROOF TESTS
	7,000		3		23

D QUANTITIES - RETAI	NING WALL #28
ATION	6,900 CY
SOIL STABILIZATION	1,350 SY
R MATERIAL	1,350 CY
	800 LF
OR SHOULDER DRAIN PIPE OUTLET	5 EA

PROJECT NO.: A-0009CB

GRAHAM COUNTY

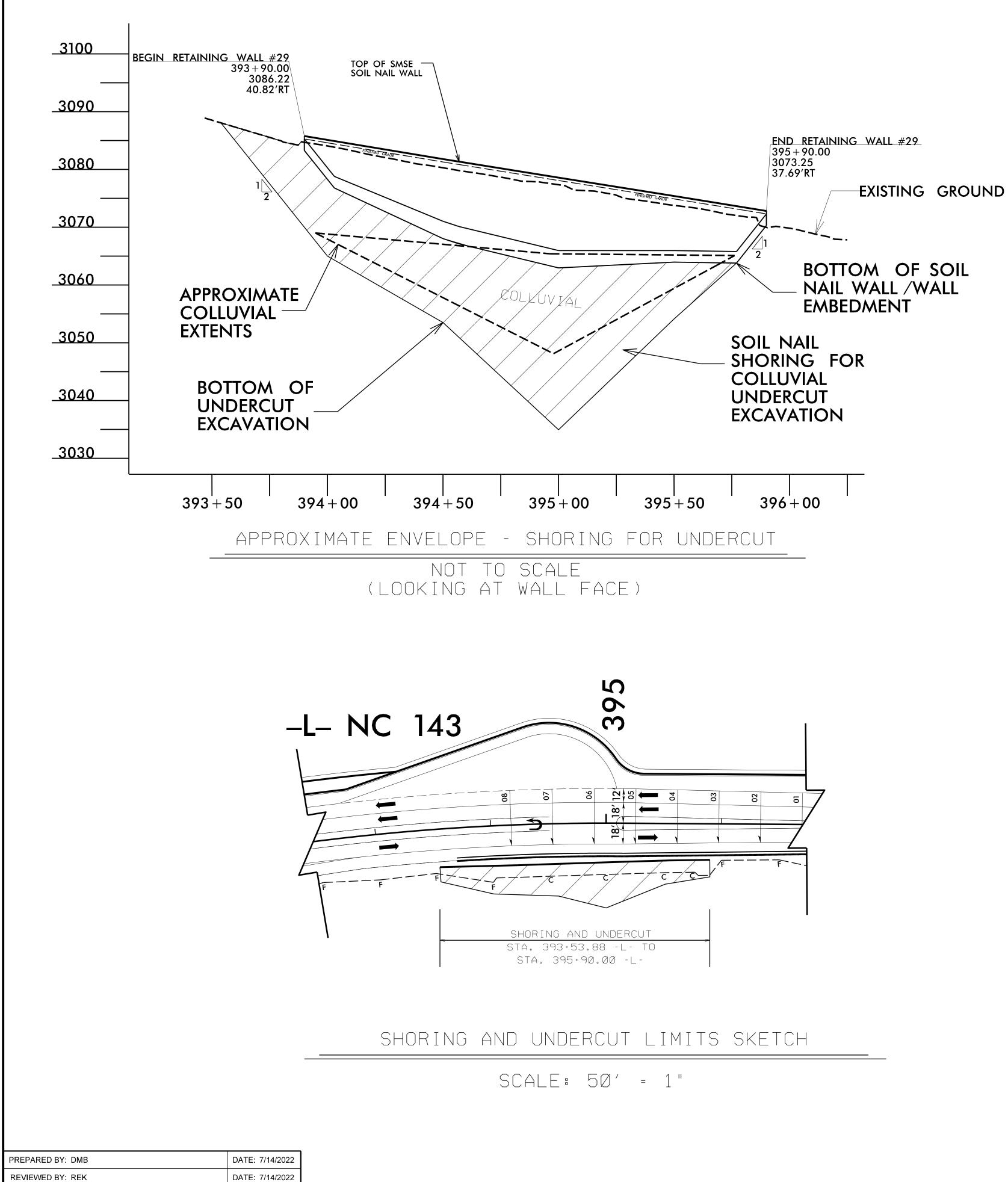
RETAINING WALL #28 STATION: -L-389+25, 47' RT TO 392+00, 39' RT RETAINING WALL #29 STATION: -L- 393+90, 40' RT TO 395+90, 37' R1 SHEET 8 OF 10

Prepared in the Office of:

	CAROLINAS
	GEOTECHNICAL
	GROUP
POIN	T EXECUTIVE DRIVE
SUIT	E 800
οττι	E, NC 28227
0) 3	39-8684

RETAINING WALL #28 AND #29 SOIL NAIL SHORING FOR UNDERCUT EXCAVATION

REVISIONS						SHEET
NO.	BY	DATE	NO.	BY	DATE	NO.
1			3			W28_29
2			4			-8



ESTIM	ATED SOIL
RETAINING WALL #	SOIL NAIL SH (SQUARE FE
29	3,200

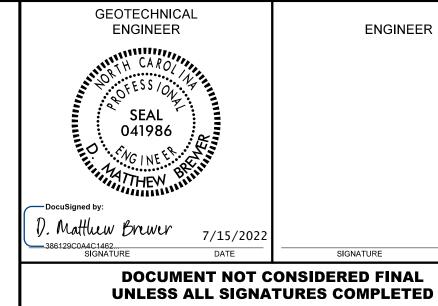
ESTIMATED QUANTITIES - RETAI	NING WALL #29
UNDERCUT EXCAVATION	3,400 CY
GEOTEXTILE FOR SOIL STABILIZATION	730 SY
SELECT GRANULAR MATERIAL	730 CY
SHOULDER DRAIN	500 LF
CONCRETE PAD FOR SHOULDER DRAIN PIPE OUTLET	3 EA



ENGINEER

SIGNATURE

DATE



11

OIL NAIL WALL QUANTITIES SOIL NAIL SOIL NAIL VERIFICATION TESTS IL SHORING RE FEET)

3

PROJECT NO.: A-0009CB

GRAHAM COUNTY

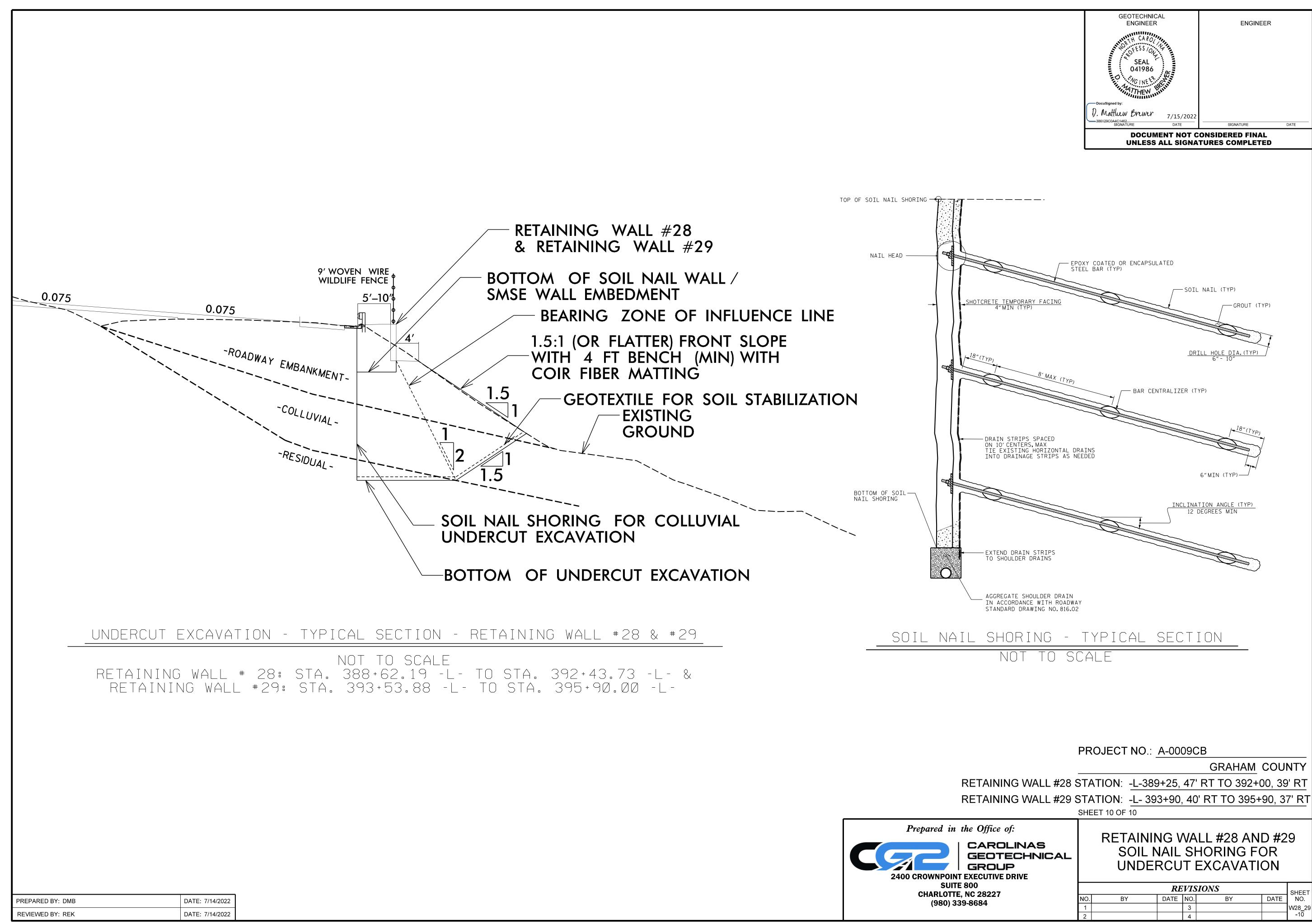
RETAINING WALL #28 STATION: -L-389+25, 47' RT TO 392+00, 39' RT RETAINING WALL #29 STATION: -L- 393+90, 40' RT TO 395+90, 37' RT SHEET 9 OF 10

Prepared in the Office of:

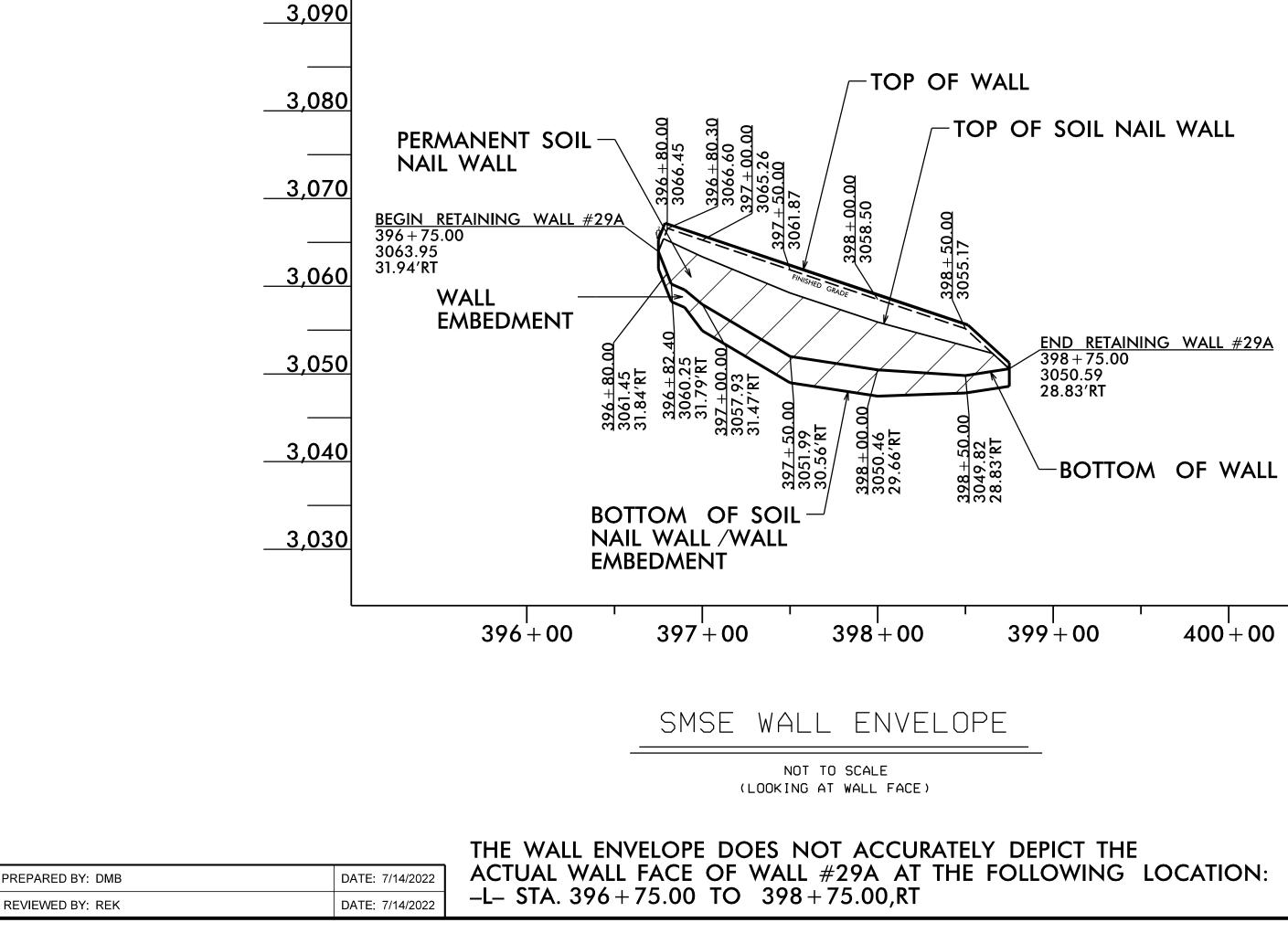
CAROLINAS GEOTECHNICAL GROUP 2400 CROWNPOINT EXECUTIVE DRIVE SUITE 800 CHARLOTTE, NC 28227

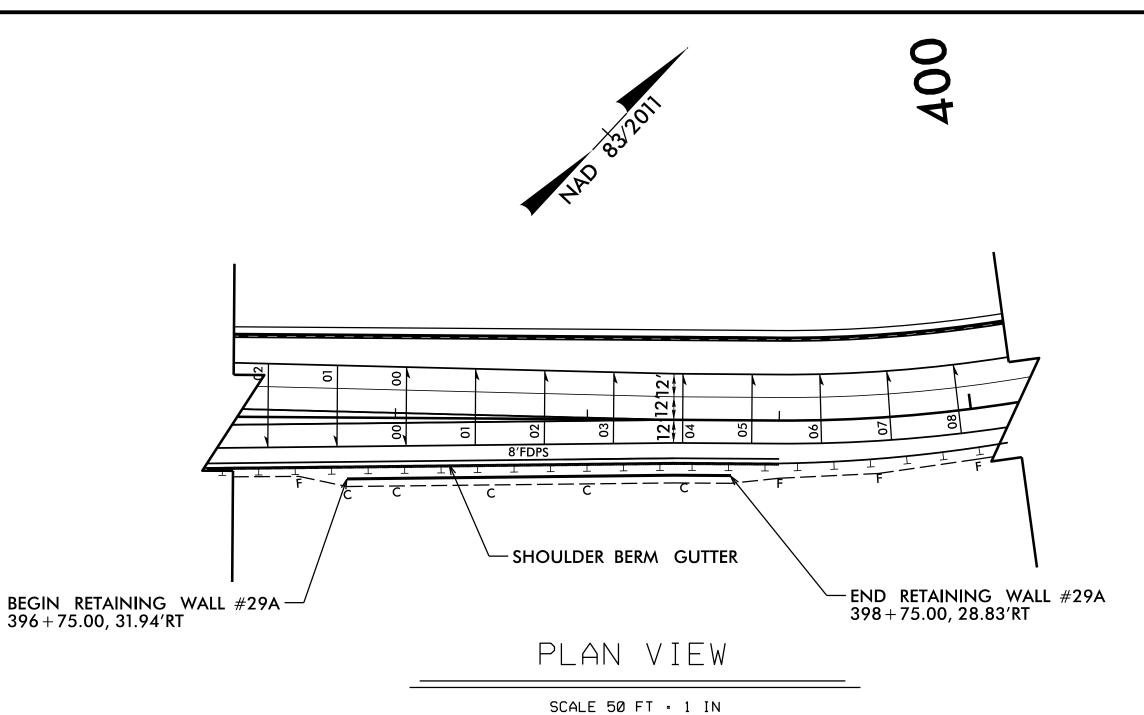
RETAINING WALL #28 AND #29 SOIL NAIL SHORING FOR UNDERCUT EXCAVATION

REVISIONS						SHEET
NO.	BY	DATE	NO.	BY	DATE	NO.
1			3			W28_29
2			4			-9

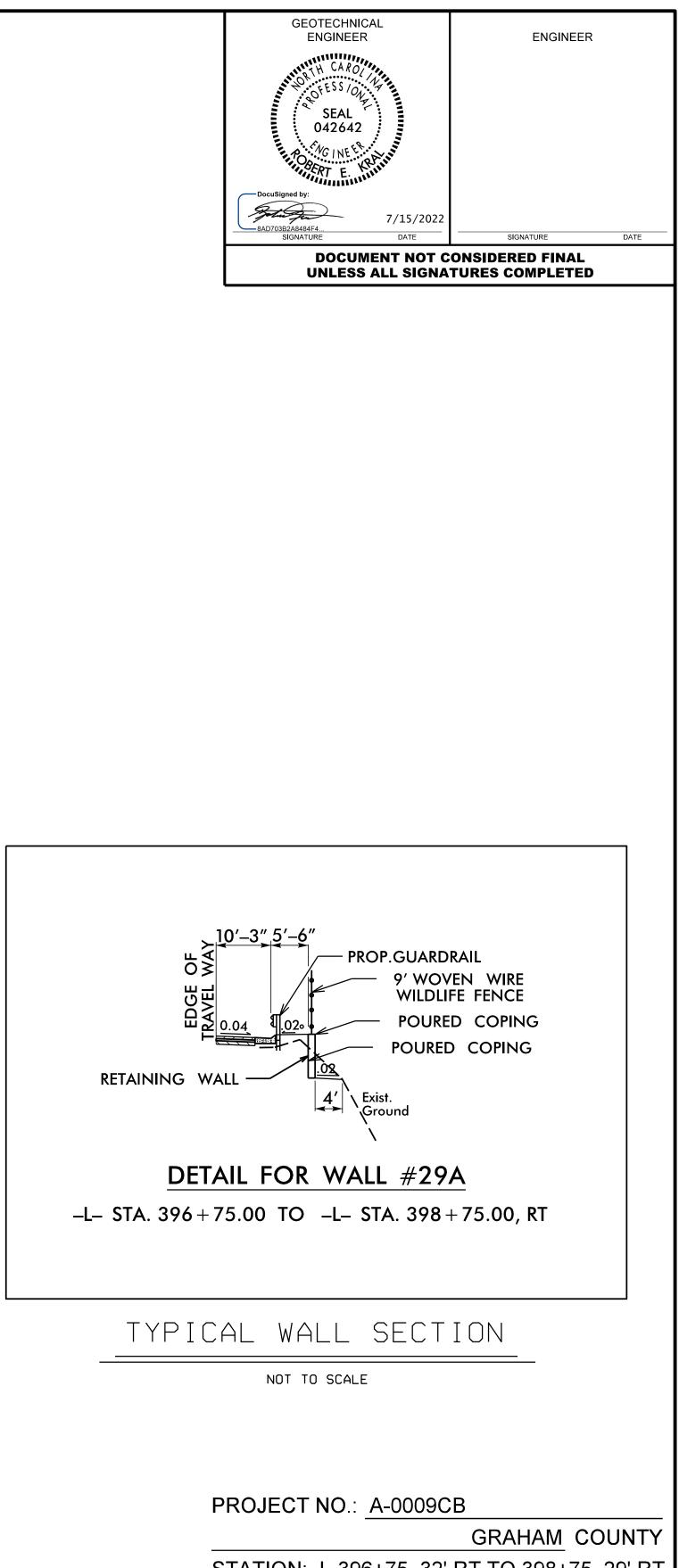


RETAINING WALL #29A









STATION: -L-396+75, 32' RT TO 398+75, 29' RT SHEET 1 OF 6

RETAINING WALL #29A SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL

	REVISIONS					
NO.	BY	DATE	NO.	BY	DATE	SHEET NO.
1			3			W29A-1
2			4			VVZ3A-1

Prepared in the Office of:

CAROLINAS GEOTECHNICAL GROUP 2400 CROWNPOINT EXECUTIVE DRIVE SUITE 800 CHARLOTTE, NC 28227

	SMSE RETAINING WALL #29A INFORMATION								
STAL-	OFFSET RT FROM CL TO WALL FACE	F I N I SHED GRADE	BOTTOM OF WALL	MINIMUM TOP OF LEVELING PAD	ESTIMATED MINIMUM SMSE WALL EMBEDMENT	* DESIGN SMSE WALL HEIGHT "H"	TOP OF SOIL NAIL WALL	APPROX. SOIL NAIL WALL HEIGHT	WALL REINFORCEMENT (MSE STRAP) LENGTH 'L'
396.75.00	31.94	3063.95	3063.95	3061.95	2.00	2.00	3063.95	2.00	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
396+80.00	31.84	3066.45	3061.45	3059.45	2.00	7.00	3065.23	5.78	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
396+80.30	31.82	3066.60	3061.30	3059.30	2.00	7.30	3065.20	5.90	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
396+82.40	31.79	3066.46	3060.25	3058.25	2.00	8.21	3065.00	6.75	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
397.00.00	31.47	3065.26	3057.93	3054.93	3.00	10.33	3063.36	8.43	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
397.50.00	30.56	3061.87	3051.99	3048.99	3.00	12.88	3059.23	10.24	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
398.00.00	29.66	3058.50	3050.46	3047.46	3.00	11.04	3055.98	8.52	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
398.50.00	28.83	3055.17	3049.82	3047.82	2.00	7.35	3053.19	5.37	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)
398.75.00	28.83	3050.59	3050.59	3048.59	2.00	2.00	3050.59	2.00	LOWER: 0.5XH OR 6 FT (MIN) UPPER: 0.7XH OR 10 FT (MIN)

* FOR DESIGN WALL HEIGHT "H" AND ADDITIONAL CONSTRUCTION DETAILS, SEE ADDITIONAL SHEETS ALL TABLE DIMENSIONS ARE GIVEN IN FEET

FRONT SLOPE WALL EMBEDMENT							
SLOPE IN FRONT OF STRUCTURES MINIMUM EMBEDMENT D							
	FOR WALLS	H/20					
HORIZONTAL	FOR ABUTMENTS	H/10					
3.0H:1.0V	WALLS	H/10					
2.5H:1.0V	WALLS	H/8.5					
2.0H:1.0V	WALLS	H/7					
1.5H:1.0V	WALLS	H/5					
1.25H:1.0V	WALLS	H/4					
1.0H:1.0V	WALLS	H/3					

NOTE:

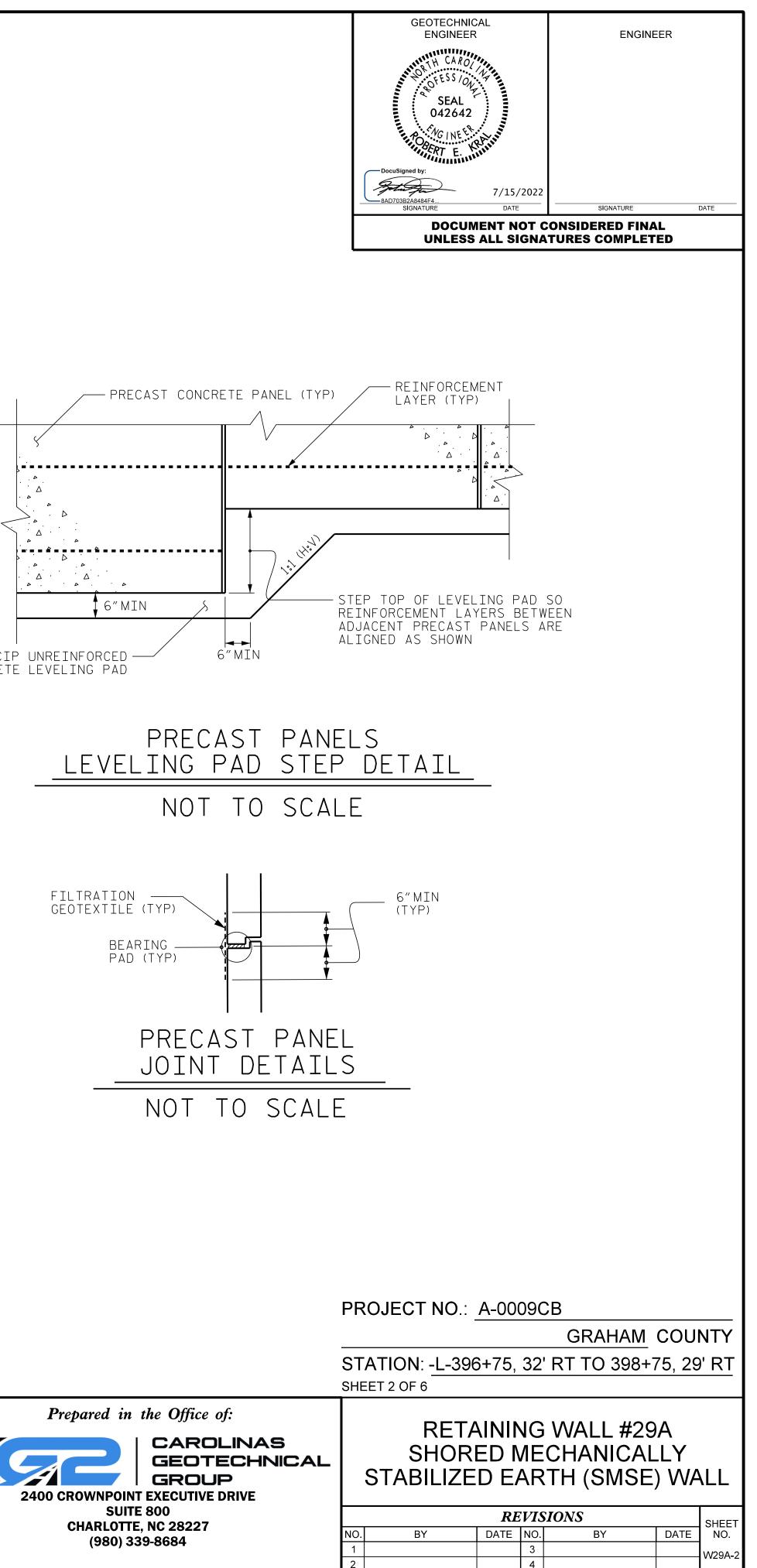
1) MAINTAIN A MINIMUM BENCH WIDTH OF 4.0 FT IN FRONT OF THE WALL FOR ITS ENTIRE LENGTH. 2) MINIMUM EMBEDMENT DEPTH OF 2 FT, UNLESS LARGER DEPTHS DICTATED BY ABOVE TABLE. 3) MAXIMUM SLOPE OF 1H:1V WILL BE MAINTAINED ON FRONT SLOPES FOR THE ENTIRE LENGTH OF THE WALL. 4) SUBMIT WITH THE WALL DESIGN INTERNAL, EXTERNAL, AND GLOBAL STABILITY ANALYSES. REFERENCE SPECIAL PROVISION GT-7 FOR SMSE WALL.

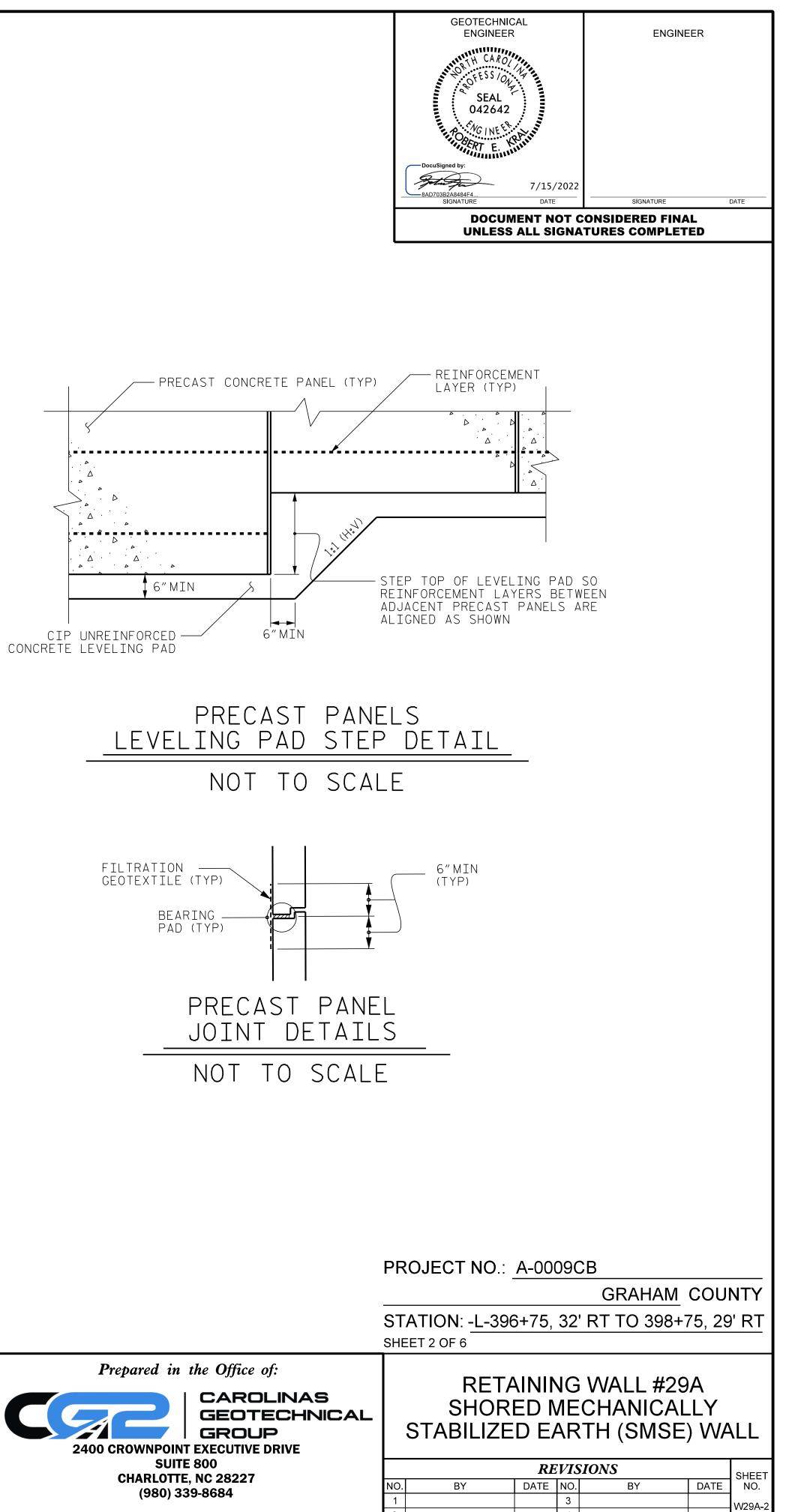
ESTIMATED SMSE WALL #29A	QUANTITIES
SHORED MSE RETAINING WALL #29A	<u>2,070</u> SQ. FT.

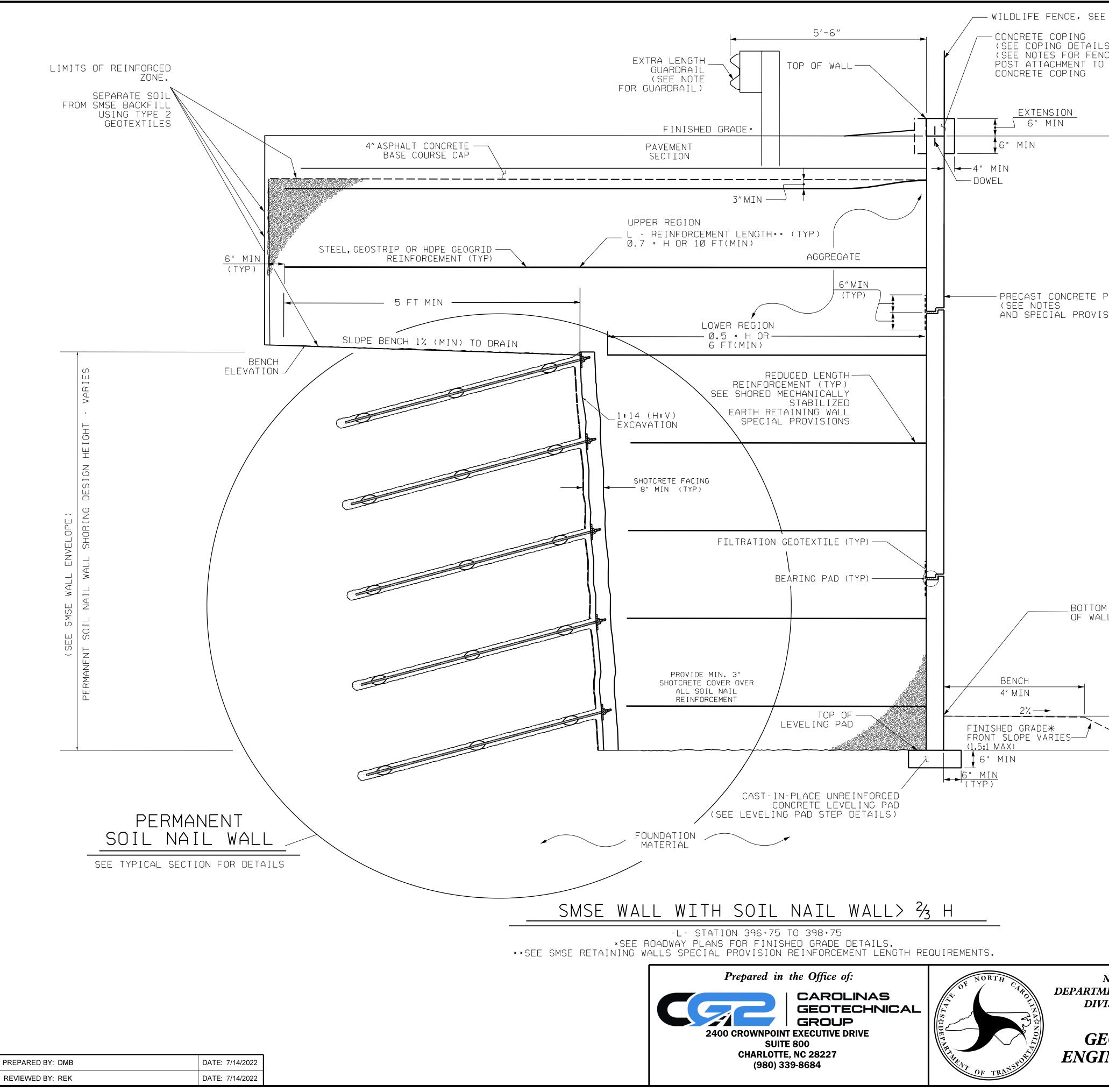
ESTIM	ATED SOIL NAIL	WALL QUAN	TITIES		
RETAINING WALL #	SOIL NAIL RETAINING WALLS (SQUARE FEET)	SOIL NAIL VERIFICATION TESTS	SOIL NAIL PROOF TESTS		
29A	1,550 *	2	5		
¥					

* INCLUDES RETAINING WALL EMBEDMENT

PREPARED BY: DMB	DATE: 7/14/2022
REVIEWED BY: REK	DATE: 7/14/2022







	E ROADWAY PLANS		ENGINEER
PPVLS 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 <th>_S) NCE AND D</th> <th>DocuSigned by: 7/15/2022</th> <th></th>	_S) NCE AND D	DocuSigned by: 7/15/2022	
PROJECT NO: A-0009CB Bit Bit Bit <th></th> <th>SIGNATURE DATE DOCUMENT NOT C</th> <th>ONSIDERED FINAL</th>		SIGNATURE DATE DOCUMENT NOT C	ONSIDERED FINAL
M FMEEDMENT EMEEDMENT EMEEDMENT 2' MIN FMEEDMENT 2' MIN GRAHAM COUNTY STATION: -1-396+75, 32' RT TO 398+75, 29' RT SHEET 3 OF 6 NORTH CAROLINA MENT OF TRANSPORTATION 755ION OF HIGHWAYS EOTECHNICAL INEERING UNIT NOTECHNICAL INTERNO UNIT	ISION) ARIE -		
PROJECT NO.: <u>A-0009CB</u> <u>GRAHAM</u> COUNTY STATION: -L-396+75, 32' RT TO 398+75, 29' RT SHEET 3 OF 6 NORTH CAROLINA MENT OF TRANSPORTATION VISION OF HIGHWAYS SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL EOTECHNICAL INEERING UNIT	MSE (SMSE) WALL (SEE WALL		
PROJECT NO.: <u>A-0009CB</u> <u>GRAHAM</u> COUNTY STATION: -L-396+75, 32' RT TO 398+75, 29' RT SHEET 3 OF 6 NORTH CAROLINA MENT OF TRANSPORTATION VISION OF HIGHWAYS SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL EOTECHNICAL INEERING UNIT	IM ILL		
GRAHAM_COUNTY GRAHAM_COUNTY STATION: -L-396+75, 32' RT TO 398+75, 29' RT SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL EOTECHNICAL INO. INO. BY DATE NO. INO. BY DATE NO. BY DATE NO. INO. INO. INO. INO. INO. INO. INO. INO. INO.			
GRAHAM_COUNTY GRAHAM_COUNTY STATION: -L-396+75, 32' RT TO 398+75, 29' RT SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHEET 3 OF 6 NORTH CAROLINA METAINING WALL #29A SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL EOTECHNICAL INO. INO. BY DATE NO. INO. BY DATE NO. BY DATE NO. INO. INO. INO. INO. INO. INO. INO. INO. INO.			
NORTH CAROLINA MENT OF TRANSPORTATION VISION OF HIGHWAYS EOTECHNICAL INEERING UNIT NO. BY DATE NO. BY DATE NO. W29A-3		STATION: -L-396+75, 32'	GRAHAM COUNTY
INEERING UNIT NO. BY DATE NO. BY DATE NO. W29A-3	MENT OF TRANSPORTATION VISION OF HIGHWAYS	RETAINING SHORED ME	CHANICALLY
		NO. BY DATE NO. 1 3 3	

NOTES:

FOR SHORED MECHANICALLY STABILIZED EARTH (SMSE) WALL RETAINING WALLS, SEE SHORED MECHANICALLY STABILIZED EARTH RETAINING WALLS SPECIAL PROVISION. FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS PROVISION. FOR STEEL BEAM GUARDRAIL, SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS. USE AN SMSE WALL SYSTEM WITH PRECAST PANELS FOR THIS RETAINING WALL. DO NOT USE FINE AGGREGATE IN THE REINFORCED ZONE OF RETAINING WALL *29A. A SMOOTH ARCHITECTURAL FINISH IS REQUIRED FOR PRECAST CONCRETE PANELS FOR RETAINING WALL *29A. A SEPARATION GEOTEXTILE IS REQUIRED AT THE BACK OF THE REINFORCED ZONE FOR RETAINING WALL *29A. BEFORE BEGINNING SMSE WALL DESIGN FOR RETAINING WALL *29A, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED. DESIGN RETAINING WALL *29A FOR WALL HEIGHTS EQUAL TO THE DESIGN HEIGHT (DIFFERENCE BETWEEN FINISHED GRADE/TOP OF WALL ELEVATION AND BOTTOM OF WALL ELEVATION) PLUS EMBEDMENT (DIFFERENCE BETWEEN BOTTOM OF WALL ELEVATION AND TOP OF LEVELING PAD ELEVATION).

DESIGN RETAINING WALL #29A FOR THE FOLLOWING: 1) H = DESIGN HEIGHT + EMBEDMENT

2) DESIGN LIFE = 75 YEARS 3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 2,400 PSF 4) MINIMUM MSE REINFORCEMENT LENGTH (L) = VARIES, SEE TABLE ON SHEET W29A-2 5) MINIMUM SOIL NAIL REINFORCEMENT LENGTHS ARE BASED ON SNAIL. 6) MINIMUM EMBEDMENT DEPTH = 5 FT (MIN), SEE TABLE ON SHEET W29A-2 7) DETNEODOCED ZONE ACCORECATE DADAMETERS.

7) REINFORCED ZONE AGGREGATE PARAMETERS:

AGGREGATE TYPE*	UNIT WEIGHT (y) PCF	FRICTION ANGLE (¢) DEGREES	COHESION (C) PSF
COARSE	110	38	Ø
FINE	115	34	Ø

* SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE MATERIAL REQUIREMENTS.

9) IN-SITU ASSUMED MATERIAL PARAMETERS:

MATERIAL TYPE	UNIT WEIGHT (y) PCF	FRICTION ANGLE (q) DEGREES	COHESION (C) PSF		
BACKFILL	120	32	0		
FOUNDATION	120	32	0		

DESIGN RETAINING WALL #29A FOR A LIVE LOAD (TRAFFIC) SURCHARGE.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH MSE AND SOIL NAIL REINFORCEMENT FOR RETAINING WALL #29A. DO NOT PLACE LEVELING PAD CONCRETE, AGGREGATE OR REINFORCEMENT FOR MSE WALL PORTION OF RETAINING WALL #29A UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.

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WHERE APPLICABLE, DESIGN SOIL NAIL WALL REINFORCEMENT INCLINATION TO ACCOUNT FOR EXISTING OR FUTURE UTILITY CONFLICTS BEHIND THE SOIL NAIL WALL. VERIFY UTILITY LOCATION AND ELEVATION BEFORE BEGINNING SOIL NAIL WALL DESIGN OR CONSTRUCTION.

"TOP OF SOIL NAIL WALL" AS SHOWN IN THE WALL ENVELOPE REPRESENTS THE APPROXIMATE GRADE ELEVATION AT A DISTANCE OF 0.5 TIMES THE PROPOSED WALL HEIGHT ("H") AT THAT STATION OR ELEVATION AT THE TOP OF THE EXISTING WALL.

THE ESTIMATED SOIL NAIL WALL QUANTITY IS BASED ON 0.5 TIMES "H" (SMSE DESIGN HEIGHT) INCLUDING THE MINIMUM EMBEDMENT LISTED IN THE DESIGN TABLE ON SHEET W29A-2. THESE VALUES ARE PROVIDED AS AN ESTIMATE ONLY AND MAY VARY DUE TO SITE CONDITIONS. THE SOIL NAIL WALL DESIGNER IS RESPONSIBLE FOR DETERMINING GLOBAL STABILTIY BASED ON THE FINISHED SMSE WALL. A MINIMUM FACTOR OF SAFETY OF 1.35 IS REQUIRED FOR GLOBAL STABILITY. SUBMIT THESE RESULTS WITH THE WALL DESIGN PACKAGE. VERIFY UTILITY LOCATIONS AND ELEVATIONS BEFORE BEGINNING MSE WALL DESIGN OR CONSTRUCTION.

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SPECIAL NOTES:

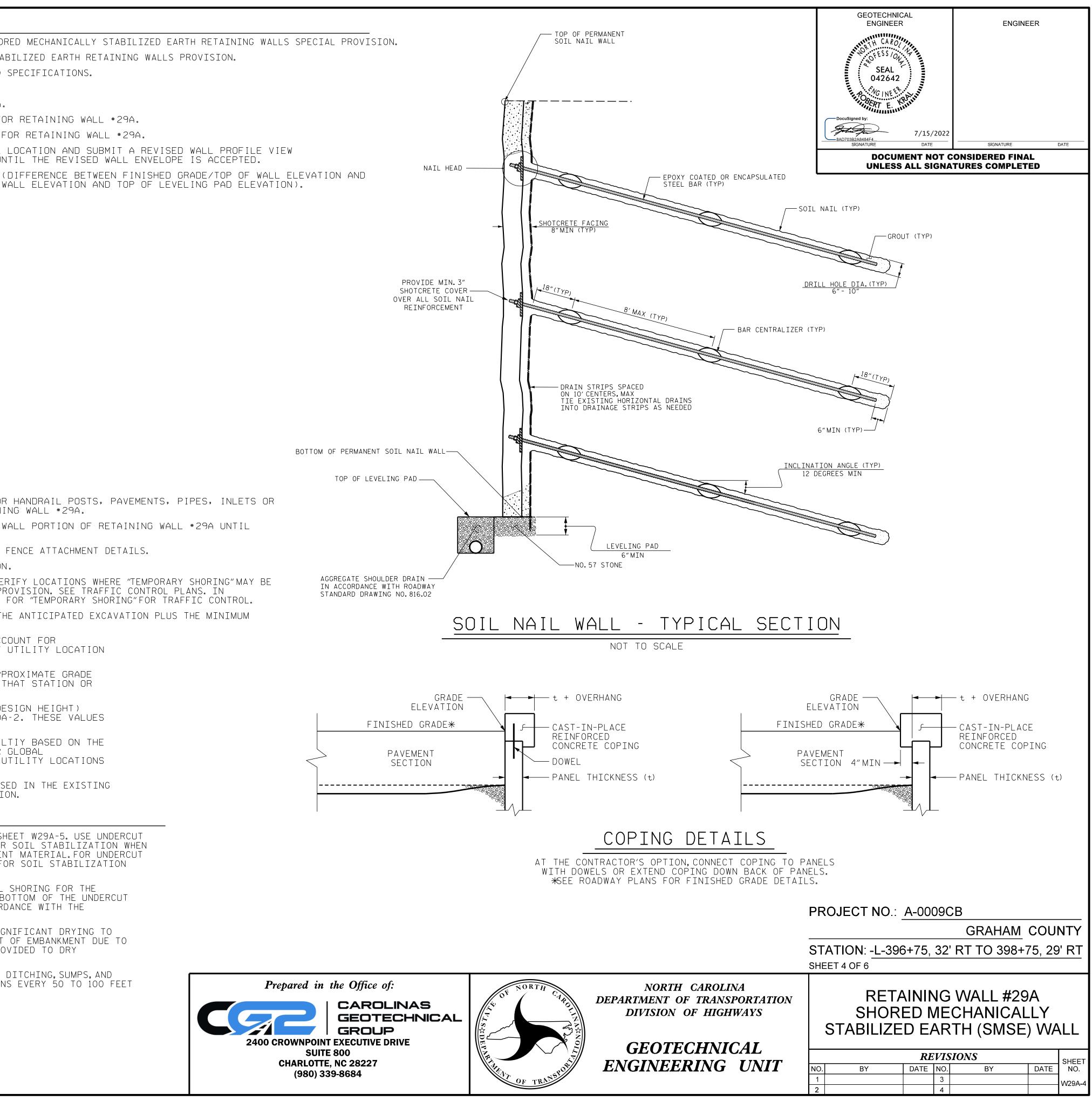
UNDERCUTTING COLLUVIAL SOILS BELOW THE SMSE WALL IS REQUIRED AS SHOWN ON SHEET W29A-5. USE UNDERCUT EXCAVATION TO REMOVE SOILS AS DIRECTED BY THE ENGINEER.PLACE GEOTEXTILE FOR SOIL STABILIZATION WHEN NEEDED IN THE BOTTOM OF THE EXCAVATION AND BACKFILL WITH SUITABLE EMBANKMENT MATERIAL.FOR UNDERCUT EXCAVATION SEE STANDARD SPECIFICATIONS.UNDERCUT EXCAVATION AND GEOTEXILE FOR SOIL STABILIZATION WILL BE PAID AS SEPARATE ADDITIONAL QUANTITIES.

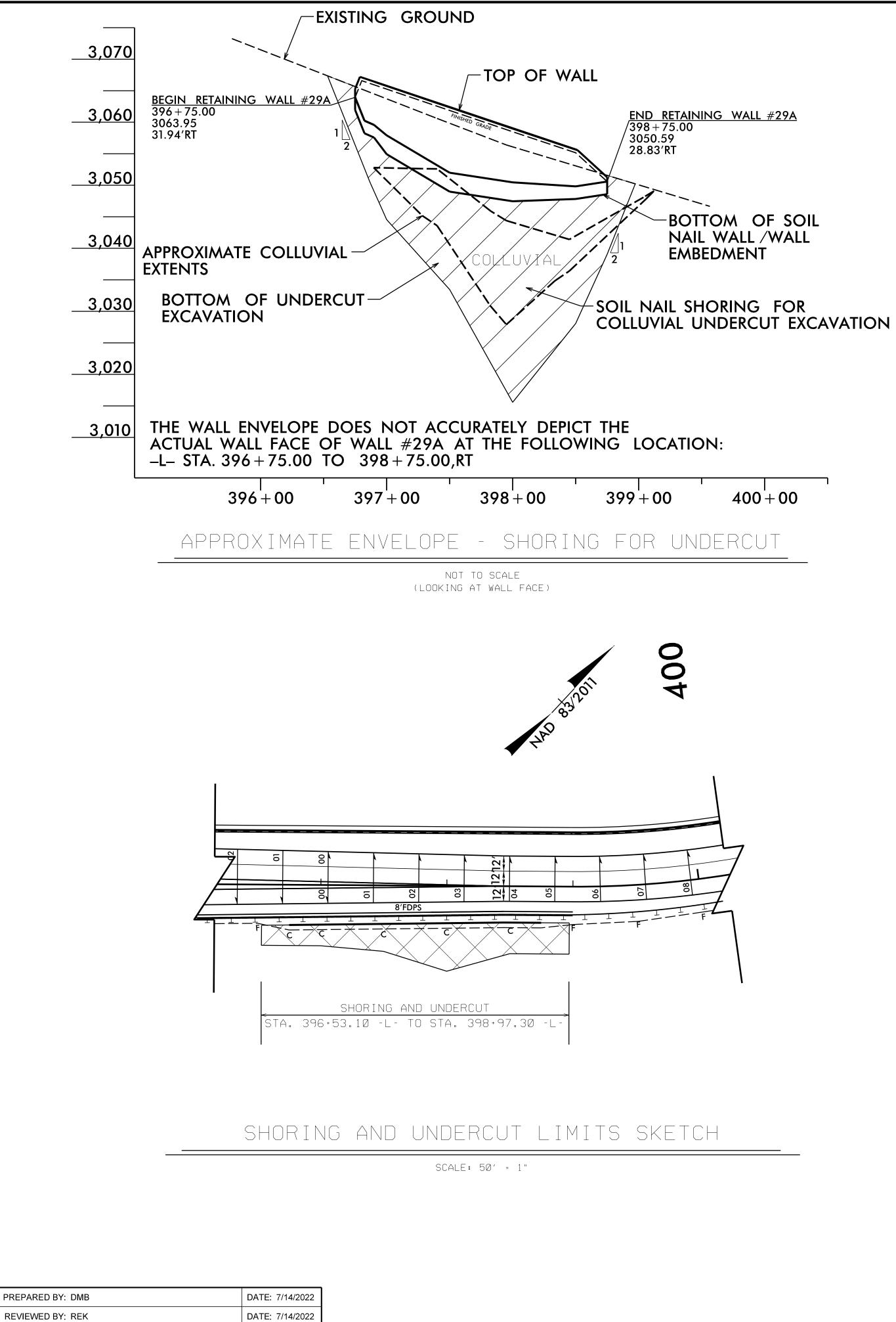
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THE COLLUVIAL SOILS ARE SUITABLE FOR USE AS EMBANKMENT BUT WILL REQUIRE SIGNIFICANT DRYING TO ACHIEVE THE REQUIRED DENSITY. DO NOT USE COLLUVIAL SOILS IN THE UPPER 3 FEET OF EMBANKMENT DUE TO THE PRESENCE OF BOULDERS AND COBBLES. NO ADDITIONAL COMPENSATION WILL BE PROVIDED TO DRY COLLUVIAL SOILS OR FOR DOUBLE-HANDLING SOILS.

CONTROL GROUNDWATER DURING AND AT THE BOTTOM OF UNDERCUT EXCAVATION USING DITCHING, SUMPS, AND PERMANENT SHOULDER DRAINS AS DIRECTED BY THE ENGINEER.OUTLET SHOULDER DRAINS EVERY 50 TO 100 FEET AS DIRECTED BY THE ENGINEER.

PREPARED BY: DMB	DATE: 7/14/2022	
REVIEWED BY: REK	DATE: 7/14/2022	



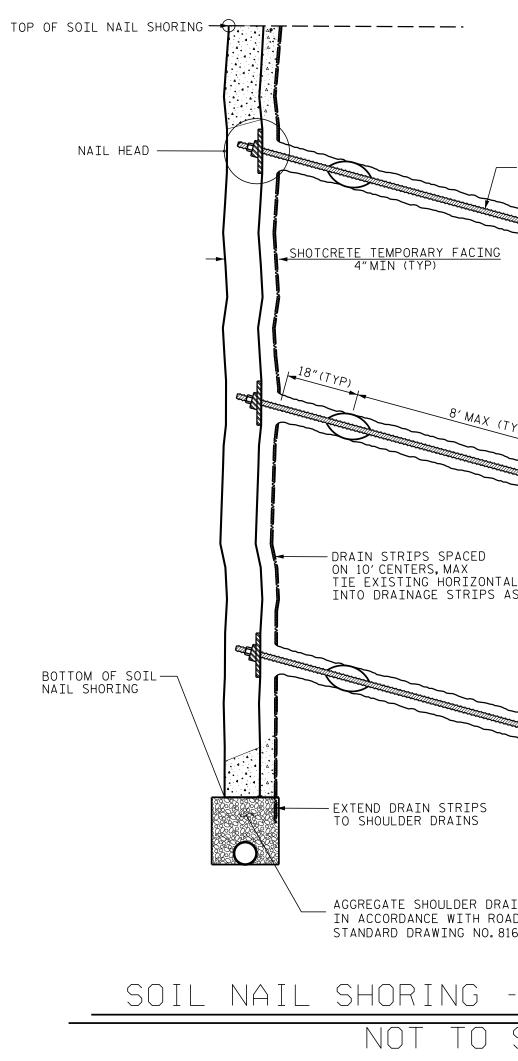


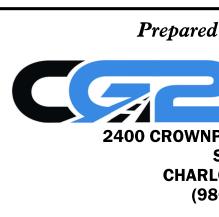
				GEOTECHNICAL ENGINEER	ENGINEER
<u></u>				CAROLINA NORTH CAROLINA	
ESTIMATE	D SOIL NAIL S	SHORING QUANTITI	ES	SEAL	
RETAINING WALL #	SOIL NAIL SHORING (SQUARE FEET)	SOIL NAIL SOIL VERIFICATION TESTS PROOF	NAIL TESTS	042642	
29A	3,930	3 10)	DocuSigned by	
				7/15/2022	2
			1		SIGNATURE DATE
	ESTIMATED QU			UNLESS ALL SIGN	ATURES COMPLETED
UNDERCUT EXCAVAT	DIL STABILIZATION	3,850 CY 800 SY	-		
SELECT GRANULAR N	MATERIAL	800 CY	-		
SHOULDER DRAIN CONCRETE PAD FOR	SHOULDER DRAIN PIPE OUTLET	500 LF 3 EA	-		
			-		
	OF SOIL	B' MAX (TYP) PRAIN STRIPS SPACED DN 10' CENTERS, MAX TIE EXISTING HORIZONTAL DRAINS INTO DRAINAGE STRIPS AS NEEDED EXTEND DRAIN STRIPS TO SHOULDER DRAINS AGGREGATE SHOULDER DRAIN IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 816.02 SHORING - TYPIC	SOIL N DRIL	TON ANGLE (TYP) GREES MIN	
		NOT TO SCALE			
				PROJECT NO.: A-00090	CB
					GRAHAM COUNTY
				STATION: -L-396+75, 32 SHEET 5 OF 6	' RT TO 398+75, 29' RT
		Prepared in the Office			
			DLINAS		WALL #29A
		GEOT	FECHNICAL JP		HORING FOR EXCAVATION
		2400 CROWNPOINT EXECUTIV SUITE 800			
		CHARLOTTE, NC 28227 (980) 339-8684	,	NO. BY DATE NO.	SIONS BY DATE NO.
				1 3	W29A-

					GEOTECHNICAL ENGINEER	ENGINEER
FSTTMA	TED SOIL NAIL	SHORT	NG OUANTTT	TES	NTH CARO	
RETAINING	SOIL NAIL SHORING	SC	DIL NAIL SOIL	NAIL	SEAL 042642	
WALL # 29A	(SQUARE FEET) 3,930	VERIFI	CATION TESTS PROOF		FIRST E WAITIN	
					DocuSigned by: PADTO2D2049464 7/15/2022	
[ттгс	1		SIGNATURE DATE
UNDERCUT E	ESTIMATED C		⊥ヒン 3,850 CY		UNLESS ALL SIGNA	TURES COMPLETED
	FOR SOIL STABILIZATION		800 SY 800 CY	-		
SHOULDER D	RAIN		500 LF	-		
CONCRETE F	AD FOR SHOULDER DRAIN PIPE OUTL		3 EA			
		DICRETE TEMPORA A" MIN (TY A" MIN (TY A	P)	SOIL N DRILL - BAR CENTRALIZER (TYP)	AIL (TYP) GROUT (TYP) HOLE DIA. (TYP) 6'' - 10'' MIN (TYP) ON ANGLE (TYP) GROUT (TYP) GROUT (TYP)	
					~	
		EXTEND DRAI TO SHOULDER	N STRIPS DRAINS			
		IN ACCORDAN	HOULDER DRAIN ICE WITH ROADWAY AWING NO.816.02			
	SOIL NAIL	SHOR	ING - TYPIC	AL SECTI		
		NOT	TO SCALE			
					PROJECT NO.: A-0009C	B
					<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>	GRAHAM COUNTY
					STATION: <u>-L-396+75, 32'</u> SHEET 5 OF 6	RT TO 398+75, 29' RT
	ſ		Prepared in the Office			
				DLINAS TECHNICAL	RETAINING SOIL NAIL SH	IORING FOR
		24	GROU	JP	UNDERCUT E	EXCAVATION
			SUITE 800 CHARLOTTE, NC 28227 (990) 329 8684	7	NO. BY DATE NO.	IONS BY DATE NO.
			(980) 339-8684			

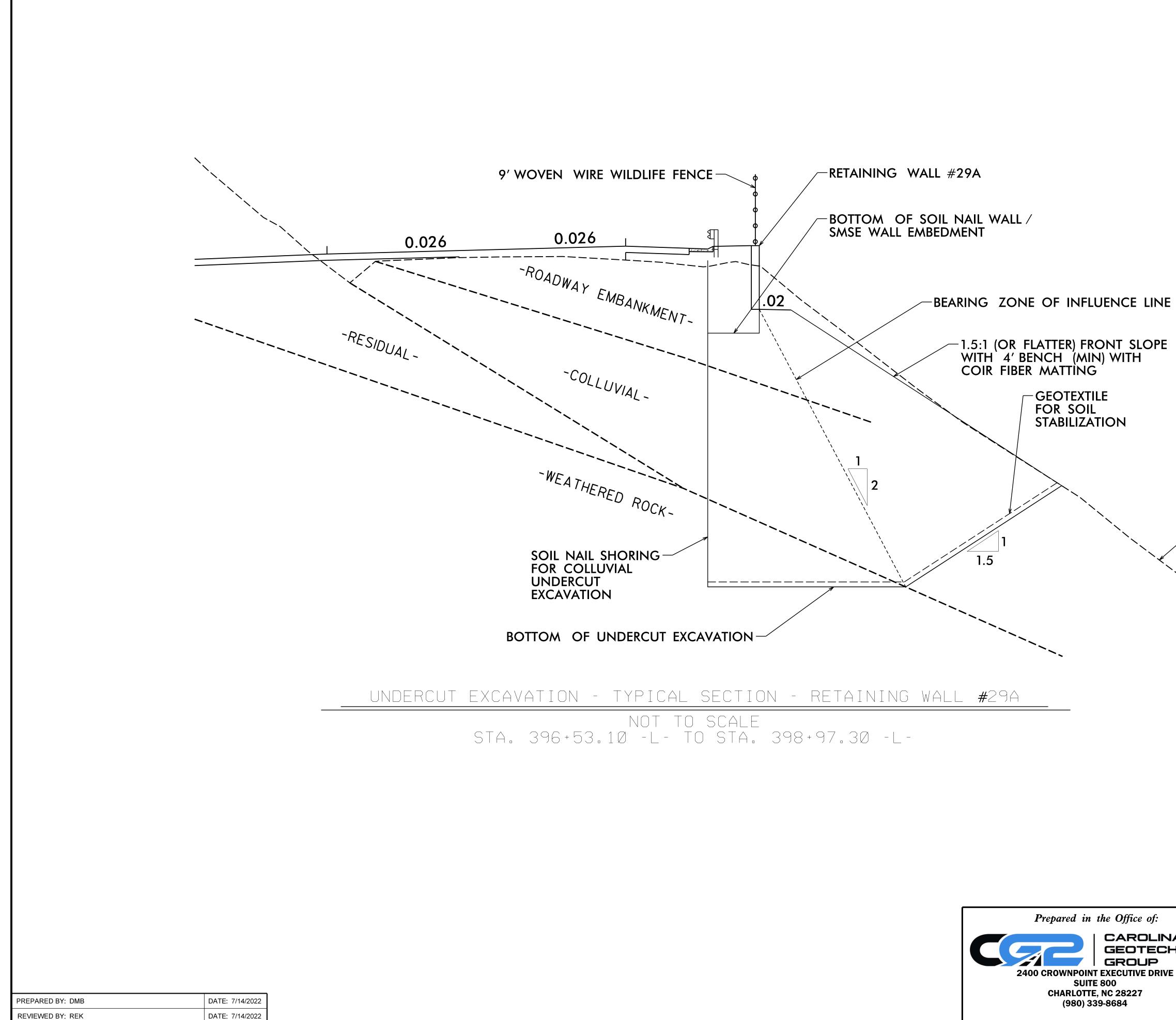
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4



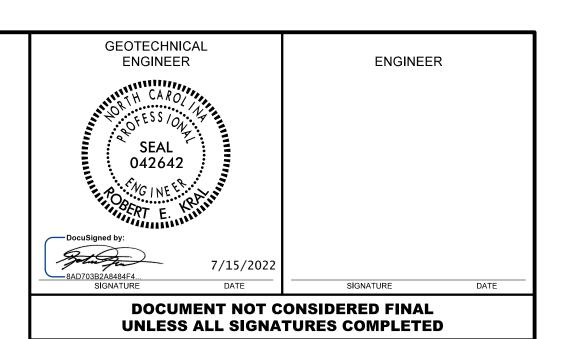


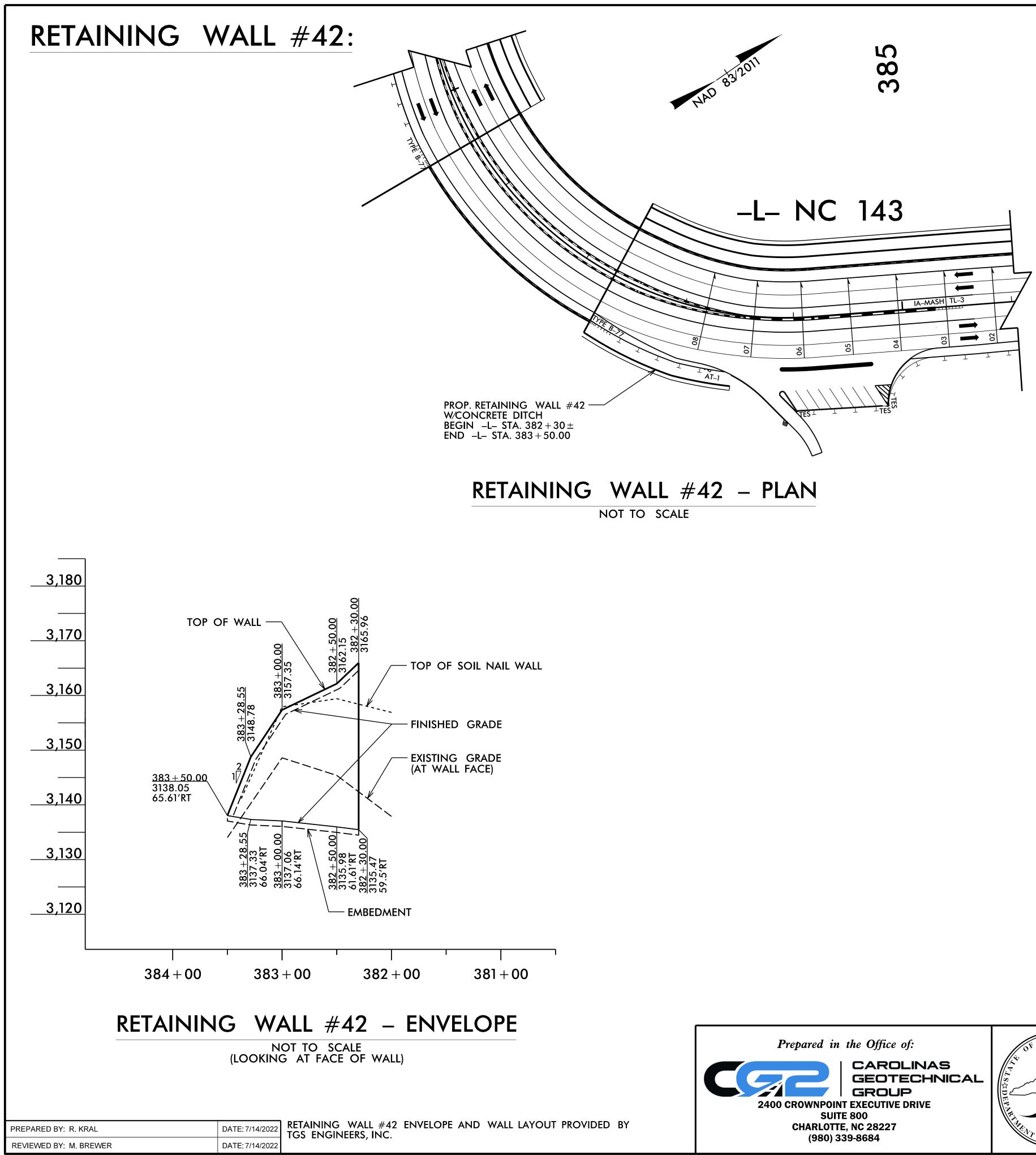
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	PROJECT NO.: A-0009CB						
	GRAHAM COUNTY						
	STAT	TON: - <u>L-396</u>	6+75,	32'	' RT TO 398+ ⁻	75, 29)' RT
	SHEET	6 OF 6					
A in the Office of: CAROLINAS GEOTECHNICAL GROUP NPOINT EXECUTIVE DRIVE CAROLINAS UNDERCUT EXCAVATION						ЭR	
SUITE 800 LOTTE, NC 28227	REVISIONS						
80) 339-8684	NO.	BY	DATE	NO. 3	BY	DATE	NO.
	2			4			W29A-6

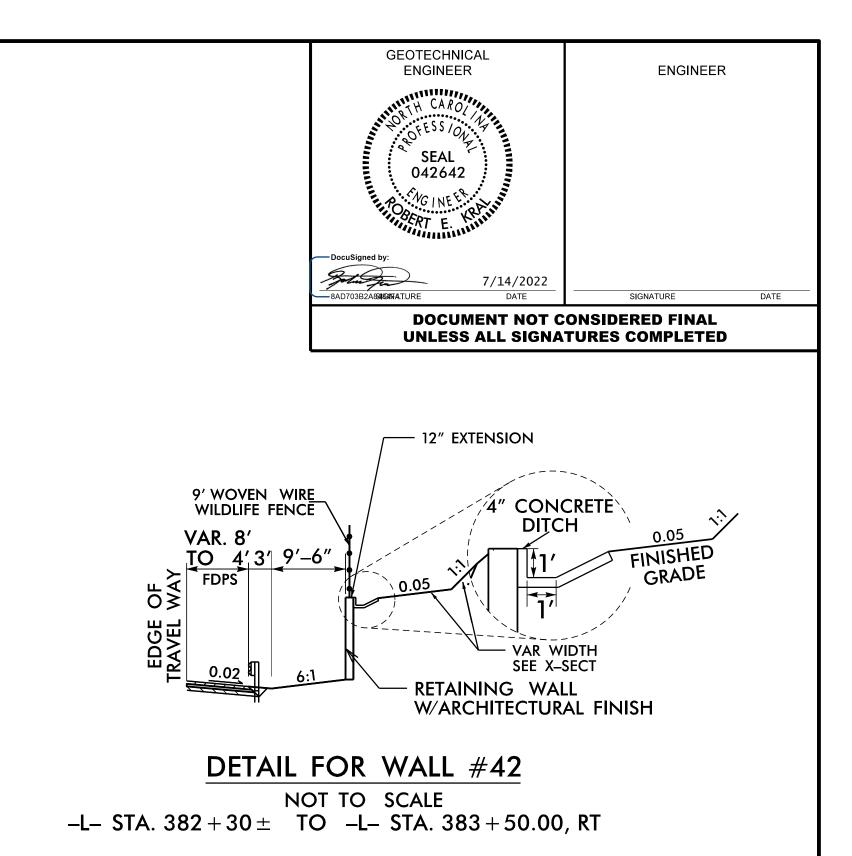
-EXISTING GROUND





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT**





ESTIN	ATED SOIL NAI	_ WALL QUAN	ITITIES
TAINING All NO.	SOIL NAIL RETAINING WALLS (SQUARE FEET)	SOIL NAIL VERIFICATION TESTS	SOIL NAIL PROOF TESTS
42	2,430*	2	5
SIMULATE	2,310 SF		
HORIZONT	60 LF		

* INCLUDES RETAINING WALL EMBEDMENT

RETAINING WALL # 42									
STAL-	OFFSET FROM -L- (RT) FT.	ELEV. @ Top of Wall ft.	* PROPOSED FINISHED GRADE FT.	* EXPOSED Wall Height ft.	** DESIGN WALL HEIGHT "H" FT.				
32+30.00	59.50	3165.96	3135.47	30.49	30.49				
32+50.00	61.61	3162.15	3135.98	26.17	26.17				
3 + 00 . 00	66.14	3157.35	3137.06	20.29	20.29				
33+28.55	66.Ø4	3148.78	3137.33	11.45	11.45				
3+50.00	65.61	3138.05	3138.05	0.00	1.00				

* ELEVATION @ PROPOSED FINISHED GRADE AND EXPOSED WALL HEIGHT DO NOT INCLUDE EMBEDMENT DEPTH

** FOR DESIGN WALL HEIGHT ``H" AND ADDITIONAL CONSTRUCTION DETAILS SEE SHEET W42-2

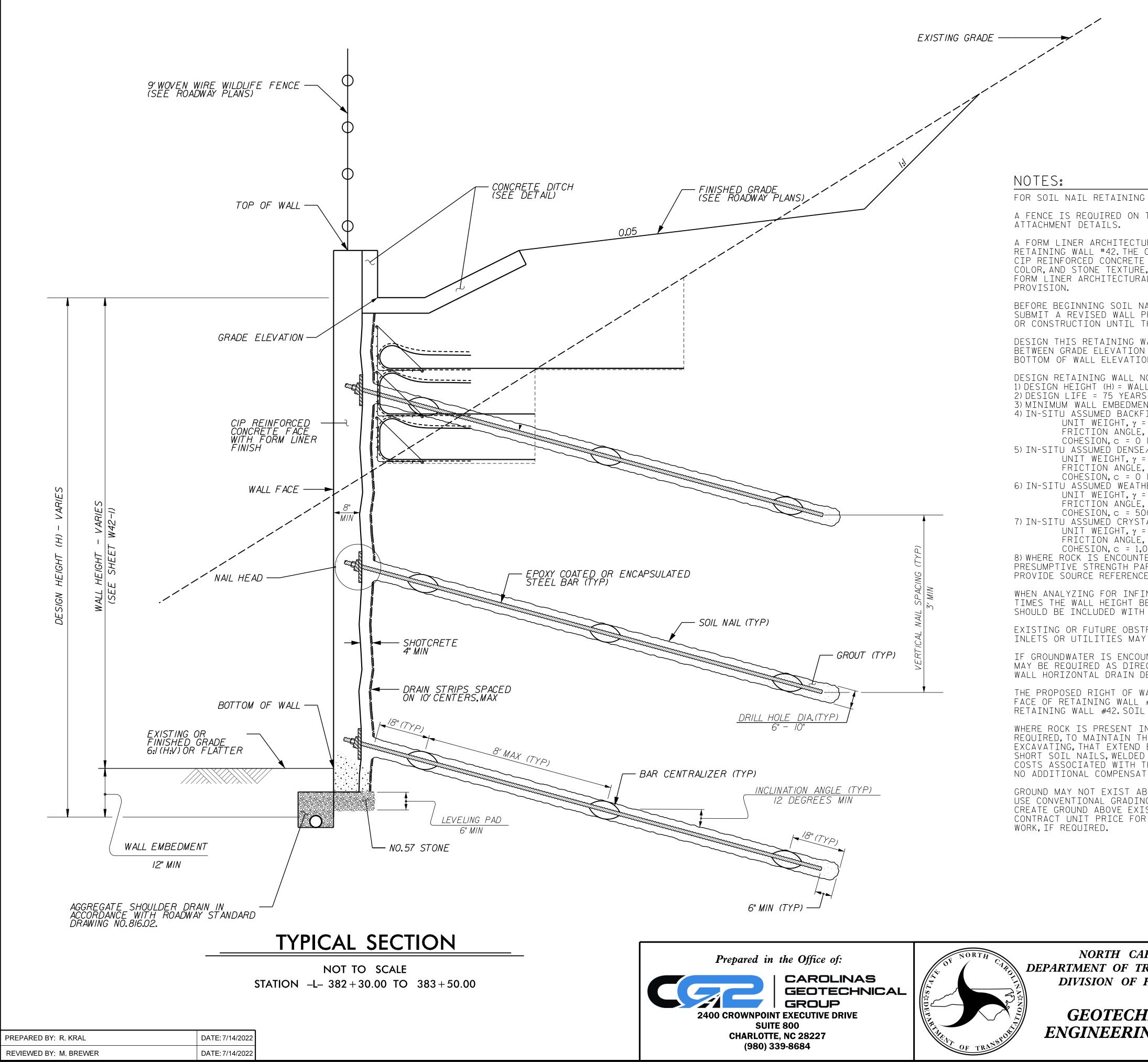
PROJECT NO.: A-0009CB

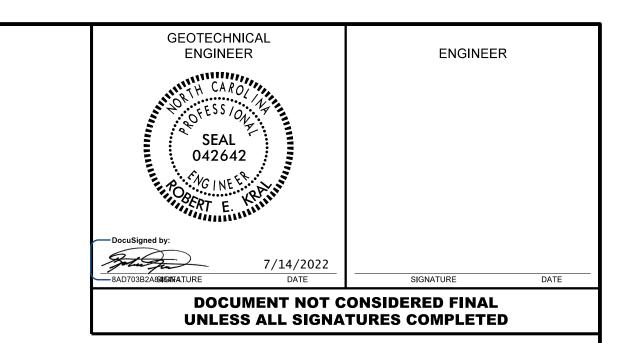
GRAHAM COUNTY

RETAINING WALL #42: -L- 382+30, 59.5' RT TO 383+50, 65.6' RT SHEET 1 OF 3

> RETAINING WALL #42 SOIL NAIL RETAINING WALL WITH CAST-IN-PLACE CONCRETE FACE

REVISIONS						
NO.	BY	DATE	NO.	BY	DATE	SHEET NO.
1			3			W42-1
2			4			v v-+2-1





FOR SOIL NAIL RETAINING WALLS, SEE SOIL NAIL RETAINING WALLS PROVISION.

A FENCE IS REQUIRED ON TOP OF RETAINING WALL #42. SEE ROADWAY PLANS FOR FENCE

A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP REINFORCED CONCRETE FACE FOR RETAINING WALL #42. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP REINFORCED CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, AND STONE TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK. FOR FORM LINER ARCHITECTURAL FINISH. SEE THE SIMULATED STONE FORM LINER FINISH SPECIAL

BEFORE BEGINNING SOIL NAIL WALL DESIGN FOR RETAINING WALL #42, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN THIS RETAINING WALL FOR WALL HEIGHTS EQUAL TO THE DESIGN HEIGHT (DIFFERENCE BETWEEN GRADE ELEVATION AND BOTTOM OF WALL ELEVATION) PLUS EMBEDMENT (DIFFERENCE BETWEEN BOTTOM OF WALL ELEVATION AND TOP OF LEVELING PAD ELEVATION).

DESIGN RETAINING WALL NO. #42 FOR THE FOLLOWING: 1) DESIGN HEIGHT (H) = WALL HEIGHT + WALL EMBEDMENT

3) MINIMUM WALL EMBEDMENT ELEVATION = VARIES (MIN.1 FT BELOW FINISHED GRADE)

4) IN-SITU ASSUMED BACKFILL SOIL PARAMETERS: UNIT WEIGHT, γ = 120 PCF FRICTION ANGLE, ϕ = 30 DEGREES

COHESION, c = 0 PSF

5) IN-SITU ASSUMED DENSE/VERY STIFF RESIDUAL SOIL PARAMETERS:

UNIT WEIGHT, γ = 125 PCF FRICTION ANGLE, ϕ = 36 DEGREES

COHESION, c = 0 PSF 6) IN-SITU ASSUMED WEATHERED ROCK (META-SANDSTONE) PARAMETERS:

UNIT WEIGHT, γ = 135 PCF FRICTION ANGLE, ϕ = 32 DEGREES

COHESION, c = 500 PSF

7) IN-SITU ASSUMED CRYSTALLINE ROCK (META-SANDSTONE) PARAMETERS:

UNIT WEIGHT, $\gamma = 170$ PCF FRICTION ANGLE, ϕ = 34 DEGREES

COHESION, c = 1,000 PSF 8) WHERE ROCK IS ENCOUNTERED IN THE WALL ENVELOPE, DESIGNERS SHOULD REFER TO THE FHWA PRESUMPTIVE STRENGTH PARAMETERS OR OTHER REPRESENTATIVE AND REPEATABLE VALUES AND PROVIDE SOURCE REFERENCES IN THEIR DESIGN SUBMITTAL.

WHEN ANALYZING FOR INFINITE SLOPE CONDITIONS, DESIGNERS SHOULD ANALYZE UP TO TWO (2) TIMES THE WALL HEIGHT BEHIND THE WALL FACE FOR FAILURE PLANE SEARCHES. THIS INFORMATION SHOULD BE INCLUDED WITH THE DESIGN SUBMITTAL.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH REINFORCEMENT FOR THE RETAINING WALL.

IF GROUNDWATER IS ENCOUNTERED BEHIND THE FACE OF RETAINING WALL #42, HORIZONTAL DRAINS MAY BE REQUIRED AS DIRECTED BY THE ENGINEER. FOR HORIZONTAL DRAINS, SEE THE RETAINING WALL HORIZONTAL DRAIN DETAIL.

THE PROPOSED RIGHT OF WAY (ROW) AND PERMANENT EASEMENT (PE) BOUNDARY VARIES FROM THE FACE OF RETAINING WALL #42.SEE THE ROADWAY PLANS FOR OFFSET DISTANCES FROM THE FACE OF RETAINING WALL #42. SOIL NAILS MAY NOT BE INSTALLED BEYOND THE PE BOUNDARY.

WHERE ROCK IS PRESENT IN THE WALL ENVELOPE, CONTROLLED BLASTING IS RECOMMENDED, BUT NOT REQUIRED, TO MAINTAIN THE WALL ENVELOPE, CONTROLLED BEASTING IS RECOMMENDED, BOT NOT REQUIRED, TO MAINTAIN THE NEAT EXCAVATION LINE. VOIDS, RESULTING FROM BLASTING OR EXCAVATING, THAT EXTEND BEYOND THE NEAT LINES ARE TO BE FILLED WITH A COMBINATION OF SHORT SOIL NAILS, WELDED WIRE, AND SHOTCRETE, AT THE DISCRETION OF THE ENGINEER. THE COSTS ASSOCIATED WITH THIS WORK WILL BE CONSIDERED INCIDENTAL TO WALL CONSTRUCTION AND NO ADDITIONAL COMPENSATION WILL BE MADE.FOR BLASTING, SEE THE BLASTING PROVISION.

GROUND MAY NOT EXIST ABOVE THE BOTTOM OF THE WALL IN SOME PORTIONS OF THE WALL ENVELOPE. USE CONVENTIONAL GRADING, TEMPORARY WALL, OR OTHER METHOD ACCEPTABLE TO THE ENGINEER TO CREATE GROUND ABOVE EXISTING GRADE IN ORDER TO CONSTRUCT THE SOIL NAIL WALL. THE CONTRACT UNIT PRICE FOR SOIL NAIL RETAINING WALLS WILL BE FULL COMPENSATION FOR THIS

PROJECT NO.: A-0009CB

GRAHAM COUNTY

RETAINING WALL #42: -L- 382+30, 59.5' RT TO 383+50, 65.6' RT SHEET 2 OF 3

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS**

GEOTECHNICAL ENGINEERING UNIT

RETAINING WALL #42 SOIL NAIL RETAINING WALL WITH CAST-IN-PLACE CONCRETE FACE

REVISIONS						
NO.	BY	DATE	NO.	BY	DATE	SHEET NO.
1			3			W42-2
2			4			V V-7Z-Z

