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BRIDGE @ STA. 315+72.39 -L-	BRIDGE @ STA. 315+72.39 -L- RIP RAP CLASS II (2'-0" THICK)				
	TONS	SQUARE YARDS			
END BENT 1 - STAGE I	256	284			
END BENT 1 - STAGE II	77	86			
END BENT 1 - STAGE III	457	508			
END BENT 2 - STAGE I	585	650			
END BENT 2 - STAGE II	131	145			
END BENT 2 - STAGE III	464	516			

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DRAWN BY :	D.R.	DRUM	DATE :	04/2022
CHECKED BY :	J.C. M	DATE :	05/2022	
DESIGN ENGINEER	OF RECORD:	J.E. SLOAN	DATE :	05/2022

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## **NOTES:**

THE COST OF THE CONCRETE MEDIAN BARRIER ON THE APPROACH SLAB SHALL BE INCLUDED IN THE LINEAR FOOT CONTRACT PRICE

THE CONCRETE MEDIAN BARRIER ON EACH APPROACH SLAB SHALL NOT BE CAST UNTIL ALL APPROACH SLAB CONCRETE HAS BEEN COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN CONCRETE MEDIAN BARRIERS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE MEDIAN BARRIER AND IN ACCORDA STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN MEDIAN BARRIER EXPANS JOINT IS REQUIRED AT MIDPOINT OF MEDIAN BARRIER SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE THAN 10 FEET IN LENGTH.

FOR ADDITIONAL NOTES, SEE SHEET 5 OF 6.





## SECTION THRU CONCRETE MEDIAN BARRIER

CONCRETI	
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CHECKED BY : J.C. MORRISON DATE : 05/2022 DESIGN ENGINEER OF RECORD: J.E. SLOAN DATE : 05/2022	DRAWN BY :	B.T. L	DATE :	05/2022	
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	DESIGN ENGINEER	OF RECORD:	J.E. SLOAN		05/2022

SIDE VIEW

# E MEDIAN BARRIER DETAILS

			r				
		EUD		VDDDU	Г МАТ ЛСН СТ	CRIAL	ר חי א
E BID FOR "CONCRETE MEDI	AN BARIER".	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
CAST AND HAS REACHED A	MINIMUM	<b>*</b> B15	20	5	STR	24'-8"	515
		ΨC11	25	4	2	01.11	125
		*511 *S12	50	5	1	<u>8 - 1</u> 3'-6"	183
DANCE WITH ARTICLE 825-10	)(B) OF THE						
ISION JOINTS. ONLY ONE CON E REQUIRED FOR THOSE SEC	NTRACTION GMENTS LESS						
·		* EPOX		ED			833 LBS.
		REIN	FORCIN	G STEEL			
		CLAS	S AA C				8.6 C.Y.
		CON		BAF	R TYPE	=5	JU L.I .
CTS.	<u>6"</u>		7	7 <sup>3</sup> ⁄4"		5½" HK	;
/		Ì		7-1-	<b>I</b>		135° HOOK
		3'-0"			3'-0"	5'-0"	(1)
				2			69°12'56"
	·	<u> </u>	_/	\_	<u> </u>		6"
N N N N N N N N N N N N N N N N N N N							←
END APPROACH SLAB							
						D 1110	
		PR	OJEC	T NO.		D-4442	
			BL	INCO	MBF	C(	DUNTY
		ST	ATIC	)N:	315+	72.39 -l	-
		SHE	ET 4 O	F 6			
			_	STAT	E OF NORTH	CAROLINA	
	AECOM TECHNICAL SERVICES OF NC, IN 5438 WADE PARK BOULEVARD, SUITE 20 RALEIGH, NC 27607 (919) 854-6200 www.aecom AECOM License No. F-0342	C. O	DEPAI	RTMENT	OF TR RALEIGH	ANSPORTA	TION
	THE CAROL MAN		BRI	DGE AP	PROAC	H_SLAB_	FOR
	CEAL CEAL		τN	I EGRAL FLEXIE	ABUT BLE PA	MENI WI AVEMENT	I H
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	FILL ON F SLOATIN			DEVIT	STONS		
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APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 6 OF 6 FOR DETAILS.



# NOTES

BILL OF MATERIAL																	
STAGE I STAGE II							STA	GE I	II								
F	OR O	NEA (2	PPRO REQ '	ACH S D)	LAB	F	FOR ONE APPROACH SLAB (2 REQ'D) FOR ONE APPRO (2 REQ'D)				ROACH SLAB Q'D)						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
<b>*</b> A1	26	#4	STR	38'-5"	667	<b>*</b> A1	26	#4	STR	38'-5"	667	A1	52	#4	STR	28'-3"	981
A2	26	#4	STR	38'-5"	667	A2	26	#4	STR	R 38'-5" 667		A2	52	#4	STR	28'-1"	976
<b>*</b> B1 B2	70 70	#5 #6	STR STR	24'-1" 24'-7"	1758 2585	* B1 B2	70 70	#5 #6	STR STR	24'-1" 24'-7"	1758 2585	B1 B2	106 106	#5 #6	STR STR	24'-1" 24'-7"	2663 3914
REINF	REINFORCING STEEL 3,252 LBS. F						REINFORCING STEEL3,252 LBS.				252 LBS.	REINF		STEEL		4,	890 LBS.
REINF	ORCING	STEEL		2,	425 LBS.	REIN	EPOXY COATED REINFORCING STEEL 2,425 LBS.				425 LBS.	REINF	ORCING	STEEL		3,	644 LBS.
		CLASS	AA CON	CRETE			CLASS AA CONCRETE							CLASS /	AA CON	CRETE	
POUR	#1 (SLA	AB)			37.9 C.Y.	POUR	POUR #1 (SLAB) 37.8 C.Y.			37.8 C.Y.	POUR	#1 (SLA	(B)			57.1 C.Y.	
CLASS		NCRETE			37.9 C.Y. CLASS AA CONCRETE 37.8 C.Y.				CLASS	AA CO	VCRETE			57.1 C.Y.			



SECTION N-N

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SPLICE LENGTHS					
BAR SIZE	EPOXY COATED	UNCOATED			
#4	1'-11"	1'-7"			
#5	2'-5"	2'-0"			
#6	3'-7"	2'-5"			

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# DETAIL "A"

		PROJEC BU STATIO	T NO. NCOI	B MBE 315+72	-4442 C0 2.39 -L	UNTY -
	AECOM TECHNICAL SERVICES OF NC, INC. 5438 WADE PARK BOULEVARD, SUITE 200 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	DEPAF	state RTMENT S	OF NORTH CARC OF TRAN RALEIGH	DLINA NSPORTA D	<b>FION</b>
	SEAL 035062	BRID INT	GE API EGRAL FLEXIB	PROACH ABUTME SLE PAV	SLAB F ENT WIT EMENT	OR H
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#### **DESIGN DATA:**

SPECIFICATIONS A.A.S.H.T.O. (CURRENT
LIVE LOAD
IMPACT ALLOWANCE SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 20,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50W 27,000 LBS. PER SQ. IN
- AASHTO M270 GRADE 50 27,000 LBS. PER SQ. IN
REINFORCING STEEL IN TENSION - GRADE 60 24,000 LBS. PER SQ. IN
CONCRETE IN COMPRESSION 1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS 1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER 375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS. PER CU. FT. (MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### **CONCRETE:**

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS. AND APPROACH SLABS: AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### **CONCRETE CHAMFERS:**

UNLESS OTHERWISE NOTED ON THE PLANS. ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1<sup>1</sup>/<sub>2</sub>" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A <sup>1</sup>/<sub>4</sub>" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REOUIRED ON PLANS.

#### **DOWELS:**

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# **STANDARD NOTES**

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

## ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

## **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

## STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - <sup>7</sup>/<sub>8</sub>" Ø STUDS FOR 4 - <sup>3</sup>/<sub>4</sub>" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - <sup>3</sup>/<sub>4</sub>" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST  $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS " BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY  $\frac{1}{16}$ INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

## HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REOUIRED FOR METAL RAILS AND POSTS.

## SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

# ENGLISH JANUARY, 1990

STD. NO. SN