SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>PROFILE</u>
-L-	II+00 to 26+00	4-5	6
-DET-	10+00 to 24+83	4-5	

CROSS SECTIONS

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	12+00 to 16+00	7-11
-L-	18+00 to 23+00	12-16
-DET-	II+00 to I5+02	7-11
-DET-	17+02 to 21+99	12-16

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY PAMLICO

PROJECT DESCRIPTION BRIDGE NO. 38 ON -L- (NC 55) **OVER TRENT CREEK**

INVENTORY – **REVISED**

4 Ŕ REFERENCE

 \mathbf{m}

59.

38422 PROJEC

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4593	1	16

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNIKG AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVEWED OR INSPECTED IN RALEICH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSIFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARIL REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-FLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOLI MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOLI MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OF CONTRACTOR IS CAUTONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOS NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY THINSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REDUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAVES ANY CLAIMS FOR INCREASED COMPENSATION OR STETNSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. 2.

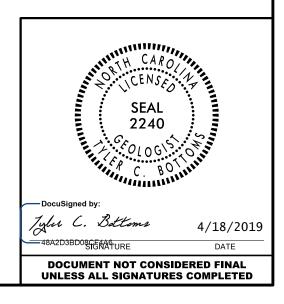
PERSONNEL

S.N. ZIMARINO

R.E. SMITH

J.M. EDMONDSON

INVESTIGATED BY _____. BOTTOMS DRAWN BY _T.C. BOTTOMS CHECKED BY ______. D.N. ARGENBRIGHT SUBMITTED BY ______. ARGENBRIGHT DATE ______ 2019



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT** SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLICHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AGSHTO T 206, ASTM DISB6). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MONISTURE, AASHTO L CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH MON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) IN INCLUSE A ROOM IN AN ANTERIAL THAT WOULD YIELD SPT N VALUES A ROCK (WR)
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS OUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7.5 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEOS.ETC.
10 50 MX 10 50	PERCENTAGE OF MATERIAL	
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	ORGANIC MATERIAL GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER
MATERIAL PASSING *40 LL 40 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 50ILS WITH LL 40 MX 11 MN 18 MX 11 MN 18 MX 11 MN	TRACE OF ORGANIC MATTER 2 -3% 3 -5% TRACE 1 10% LITTLE ORGANIC MATTER 3 -5% 5 12% LITTLE 10 20% MODERATELY ORGANIC 5 10% 12 -20% SOME 20 -35% HIGHLY ORGANIC 5 10% 2 20% HIGHLY 35% AND ABOVE	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF
CROLIP INDEX 0 0 0 4 MY 12 MY 16 MY NO MY AMULIATE OF ORGANIC	GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO
USUAL TYPES STONE FRAGS. DE MAIND CRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		(SLIJ) I INCK DEREJ THESH, SHALS AND DESCRIPTION EXTENSION EXTENSION AND THE STATES AND THE STAT
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER 24 HOURS ▽PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS
GEN.RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY (N-VALUE) COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP LIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT
GENERALLY VERY LOOSE < 4 GRANULAR LOOSE 4 TO 10 MEDIUM DENSE 10 TO 30 N/A	SOIL SYMBOL	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, MOULD YELD SPT IN VALUES > 100 BPF
MATERIAL DENSE 30 TO 50 (NON-COHESIVE) VERY DENSE > 50	ARTIFICIAL FILL (AF) DIHER THAN ROADWAY EMBANKMENT HAN ROADWAY EMBANKMENT CORE BORING SOUNDING ROD	VERY SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	\downarrow	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0 MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERV STIFF 15 TO 300 2 TO 4		COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE ON DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.
HARD > 30 > 4		ROCK HARDNESS
TEXTURE OR GRAIN SIZE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
BOULDER COBBLE GRAVEL COARSE FINE SAND SAND SAND	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.
(BLDR.) (COB.) (GR.) (GR.) (F SD.) (SL.) (CL.) GRAIN MM 305 75 2.0 0.25 0.05 0.005	ABBRE VIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACOUS WEA WEATHERED CL CLAY MOD MODERATELY γ - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SWALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETR TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u> DPT - DVNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS SPLIT SPOON F - FINE SL SILT, SILTY ST SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY
	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.
RANGE - WET - (W) SEMISCLIF, RECORDS DATING TO (P) PL PLASTIC LIMIT	FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING HI HIGHLY V - VERY RATIO	FRACTURE SPACING BEDDING
	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET
	CME-45C CLAY BITS AUTOMATIC MANUAL	CLOSE 0.16 TO YERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET
- DRY - (D) ATTAIN OPTIMUM MOISTURE	CME-55 6' CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET
PLASTICITY	В HOLLOW AUGERS	INDURATION
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ET
NON PLASTIC Ø-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	X VANE SHEAR TEST	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH		MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;
COLOR		BREAKS EASILY WHEN HIT WITH HAMMER.
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED. YELLOW-BROWN, BLUE-GRAY),		INDURATED DIFFICULT TO BREAK WITH HAMMER.
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: SAMPLE BREAKS ACROSS GRAINS.





TERMS AND DEFINITIONS

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

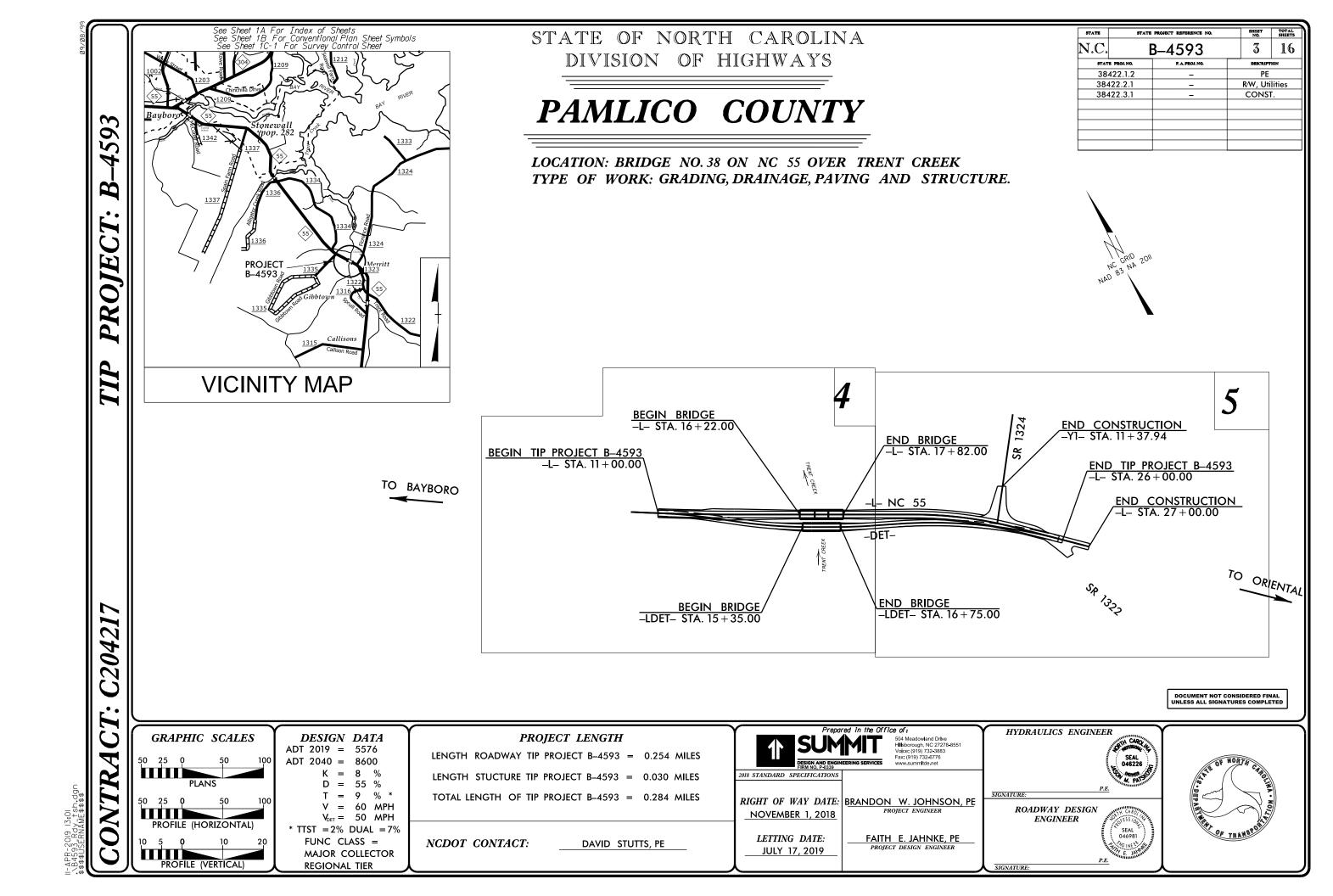
ADUIFER - A WATER BEARING FORMATION OR STRATA.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. N VALUES > ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND CK THAT SURFACE. CLUDES GRANITE, CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. AL PLAIN IF TESTED. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. MAY NOT YIELD STONE, CEMENTED CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. $\underline{\text{DIKE}}$ - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. RINGS UNDER DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL . NATINGS IF OPEN. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. AMMER BLOWS IF

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE ІСК ИР ТО SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FELDSPAR BLOWS. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. Y. ROCK HAS AS COMPARED FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. ELDSPARS DULL OSS OF STRENGTH WHEN STRUCK. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO VIDENT BUT ITS LATERAL EXTENT. ARE KAOLINIZED LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

RE DISCERNIBLE PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE STRONG ROCK T ONLY MINOR VALUES < 100 BPF ROCK OUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SECMENTS EQUAL TO OR CREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE IN SMALL AND SAPROLITE IS RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT S REQUIRES SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO LOWS REQUIRED THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. $\underline{\text{SLICKENSIDE}}$ - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. EEP CAN BE ETACHED STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL R PICK POINT WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL BLOWS OF THE TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. $\underline{STRATA CORE RECOVERY (SREC.)}$ - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. FRAGMENTS IT. SMALL. THIN STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK DUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. PIECES 1 INCH ED READILY BY TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: ELEVATIONS OBTAINED FROM B4593_Is_tin.tin FILE THICKNESS DATED 06/28/2017 4 FEET ELEVATION: FEET .5 - 4 FEET 6 - 1.5 FEET NOTES: 3 - Ø.16 FEET 08 - 0.03 FEET VST = VANE SHEAR TEST 0.008 FEET AT, PRESSURE, ETC. TEEL PROBE:

DATE: 8-15-14





STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER GOVERNOR

April 11, 2019

State Project: 38422.1.2 (B-4593) F.A. Project: BRSTP-55(34) County: Pamlico Description: Bridge No. 38 on -L- (NC 55) over Trent Creek

Subject: Geotechnical Inventory Report - Revised

Project Description

This project begins outside of Oriental in Pamlico County, 680 feet southeast of the intersection of Gibbtown Road and NC 55 and extends southeast along NC 55 for approximately 1500 feet across Trent Creek. This geotechnical investigation was confined to the areas of proposed construction.

Fieldwork was conducted in February 2018. Hand auger borings and push probes were completed at various offsets along the project corridor. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignments were investigated. A subsurface profile and selected cross sections of these alignments are included in this report.

Line	<u>Station(\pm)</u>
-L-	11+00 to 27+00
-DET-	10+00 to 24+83

Areas of Special Geotechnical Interest

- 1) The entire project was found to exhibit seasonal high ground water.
- 2) The following sections contain organic soils which have the potential to cause embankment/subgrade and or slope stability problems during construction.

Line		<u>Station(\pm)</u>
-L-		12+25 to 22+75
-DET-		11+25 to 21+75
	Telephone: (919) 662-4710	

Mailing Address: NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT -EASTERN REGIONAL OFFICE 1570 MAIL SERVICE CENTER RALEIGH, NC 27699-1570

Customer Service: 1-877-368-4968

Website: www.ncdot.gov

Location 3301 JONES SAUSAGE RD. SUITE 100 GARNER, NC 27529

JAMES H. TROGDON, III

SECRETARY

Physiography and Geology

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Natural ground elevations ranged from $-10\pm$ to $4\pm$ feet above sea level. Existing roadway embankment lies at a maximum elevation of 11± feet.

Surficial soils in this area are generally classified as alluvial.

Ground water data was collected in February of 2018. Ground water elevations ranged from $0\pm$ to $5\pm$ feet above sea level.

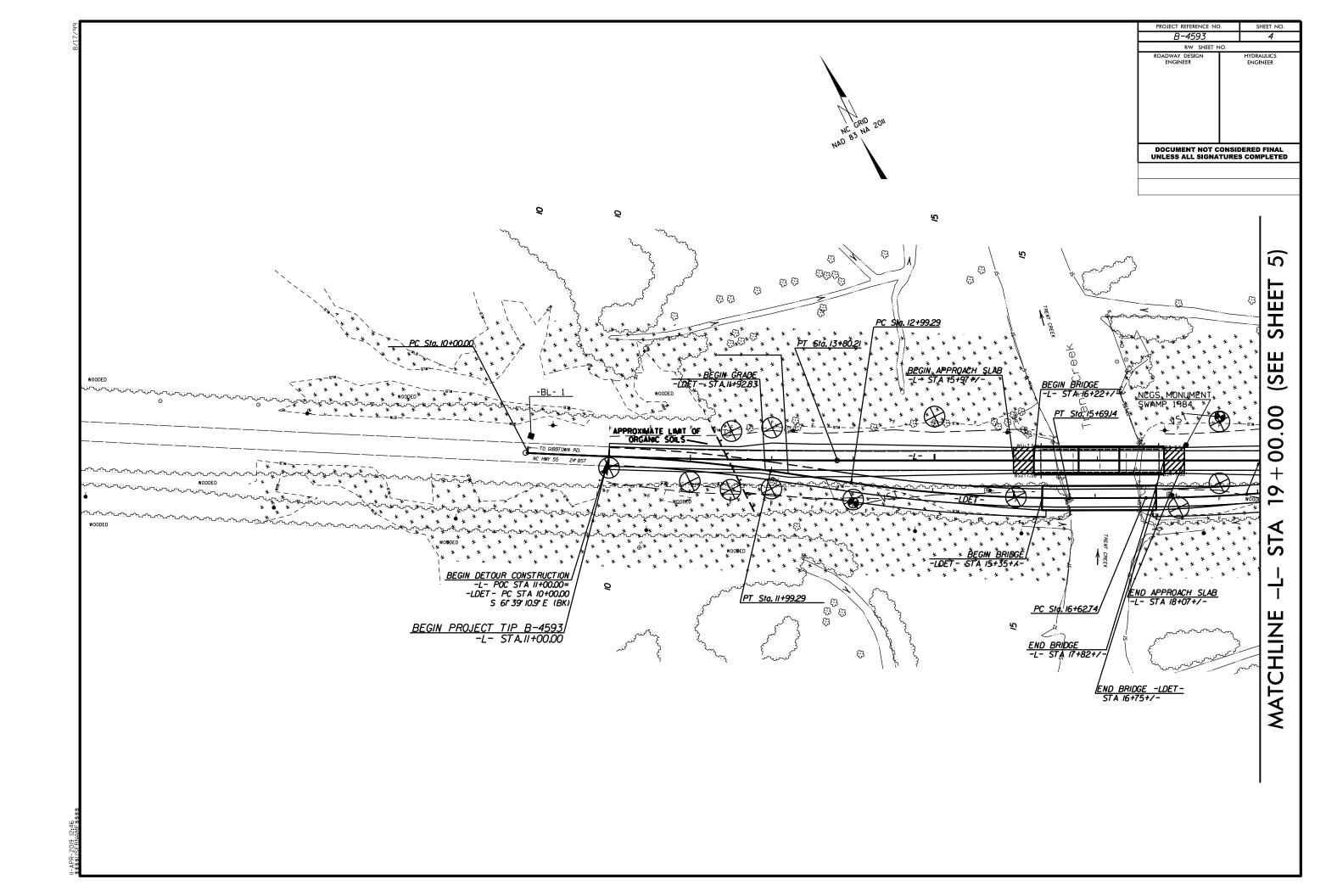
Soils encountered within this project area have been divided into two categories: Roadway Embankment and alluvial.

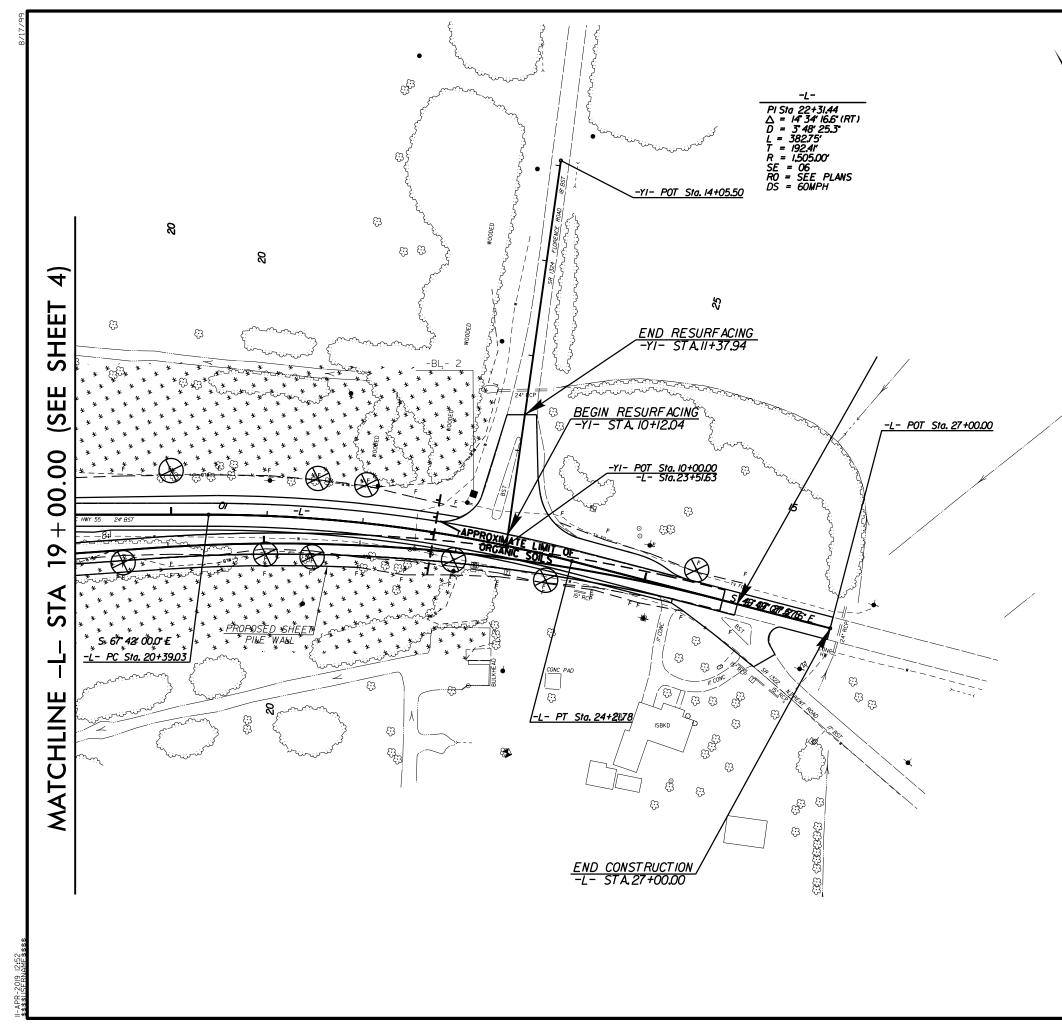
Roadway embankment soils were found along the existing NC 55 corridor. Where encountered it was composed of $1\pm$ to $10\pm$ feet of loose to medium dense sand (A-2-4).

Soils identified as alluvial are composed of $2\pm$ feet or more of loose to medium dense sand (A-2-4). Organic soils consist of $1\pm$ to $11\pm$ feet of loose to medium dense trace to moderately organic sand (A-2-4), $3\pm$ to $12\pm$ feet of soft little to moderately organic silt (A-4), and $1\pm$ to $9\pm$ feet of soft muck. Moisture samples taken within these organic units returned a natural moisture content ranging from 36% to 410%. Vane Shear Tests were completed at -L- Sta. 14+00 and -L- Sta. 18+50 and returned shear strength (psf) values between 0 and 1963. An overabundance of roots and wood fragments resulted in higher than average shear strength values.

Ground Water

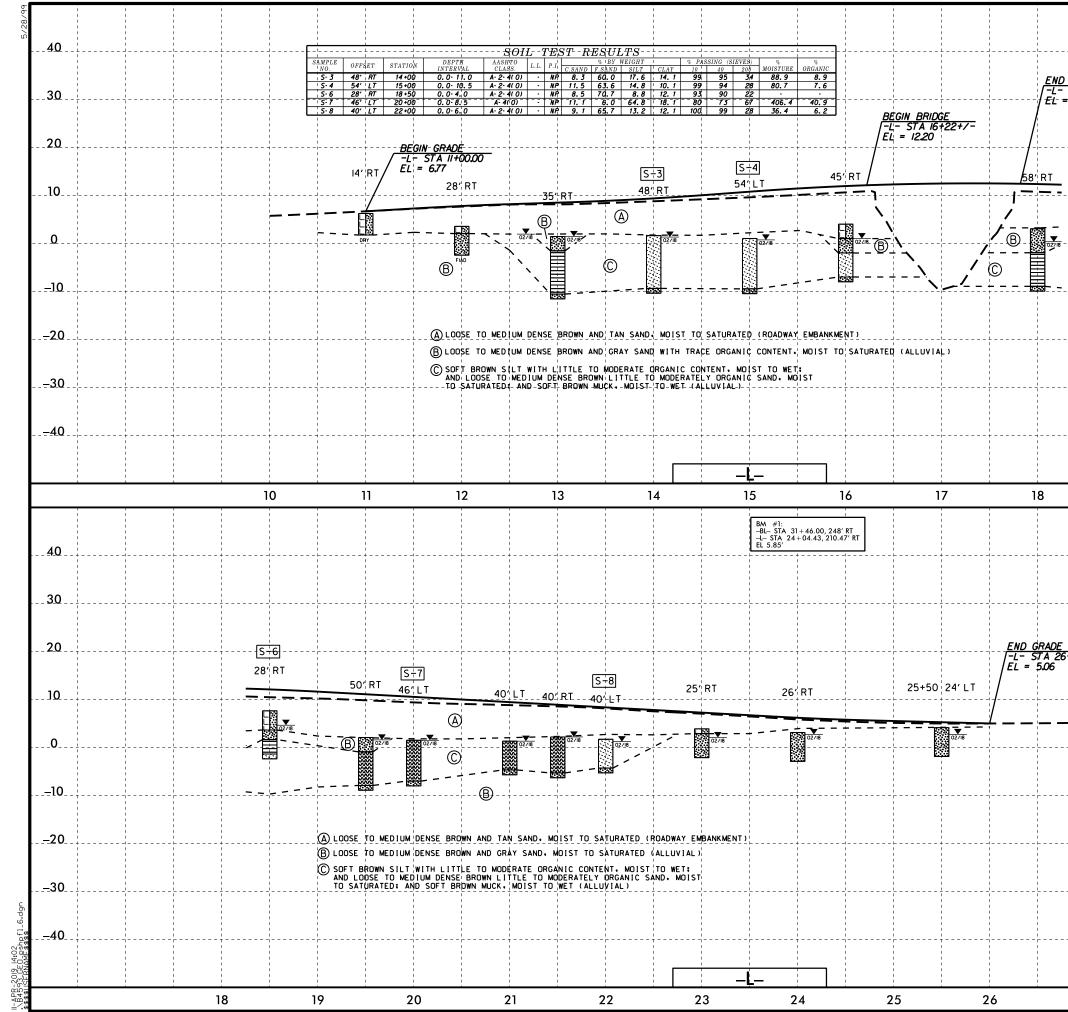
Soils



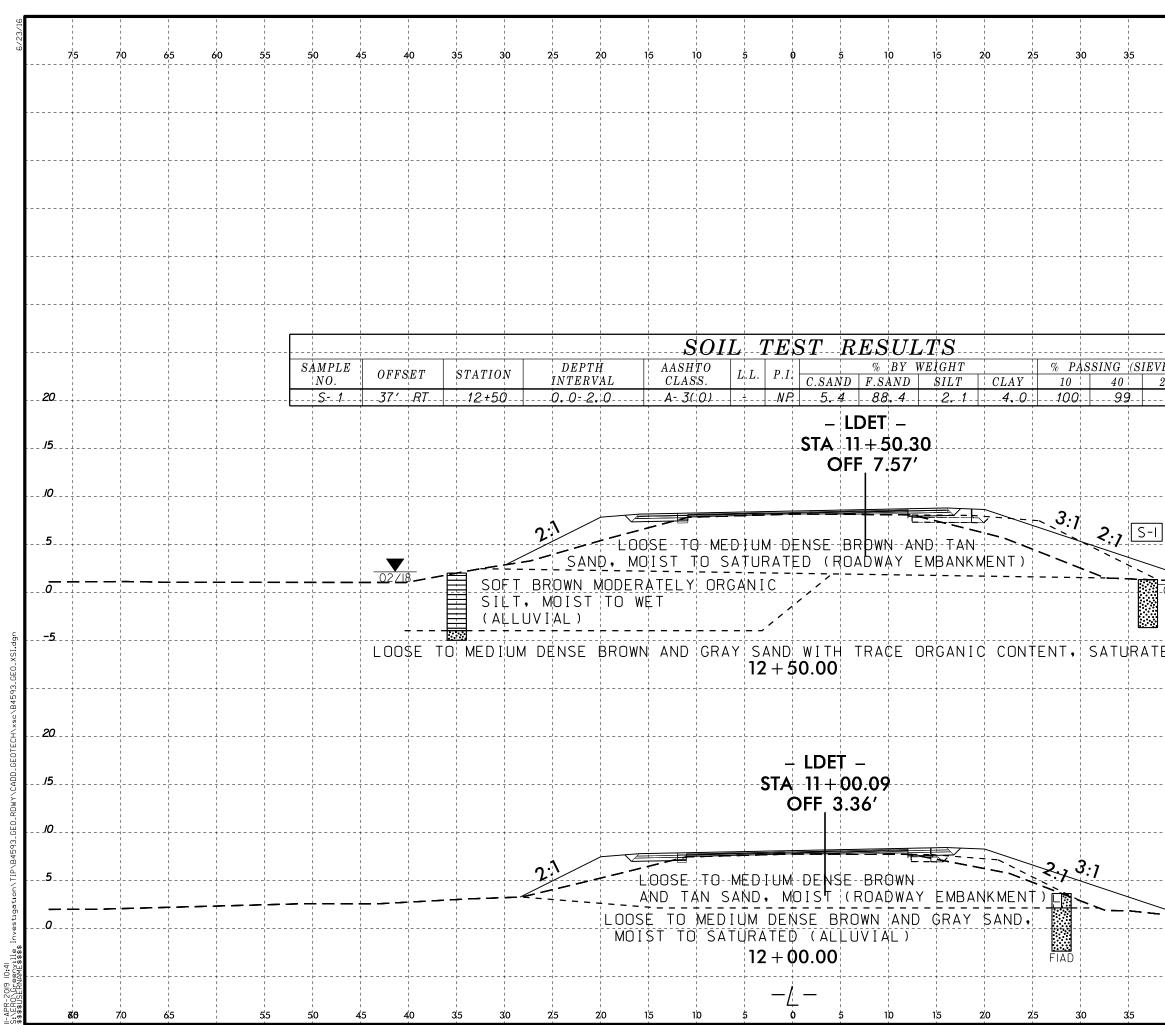




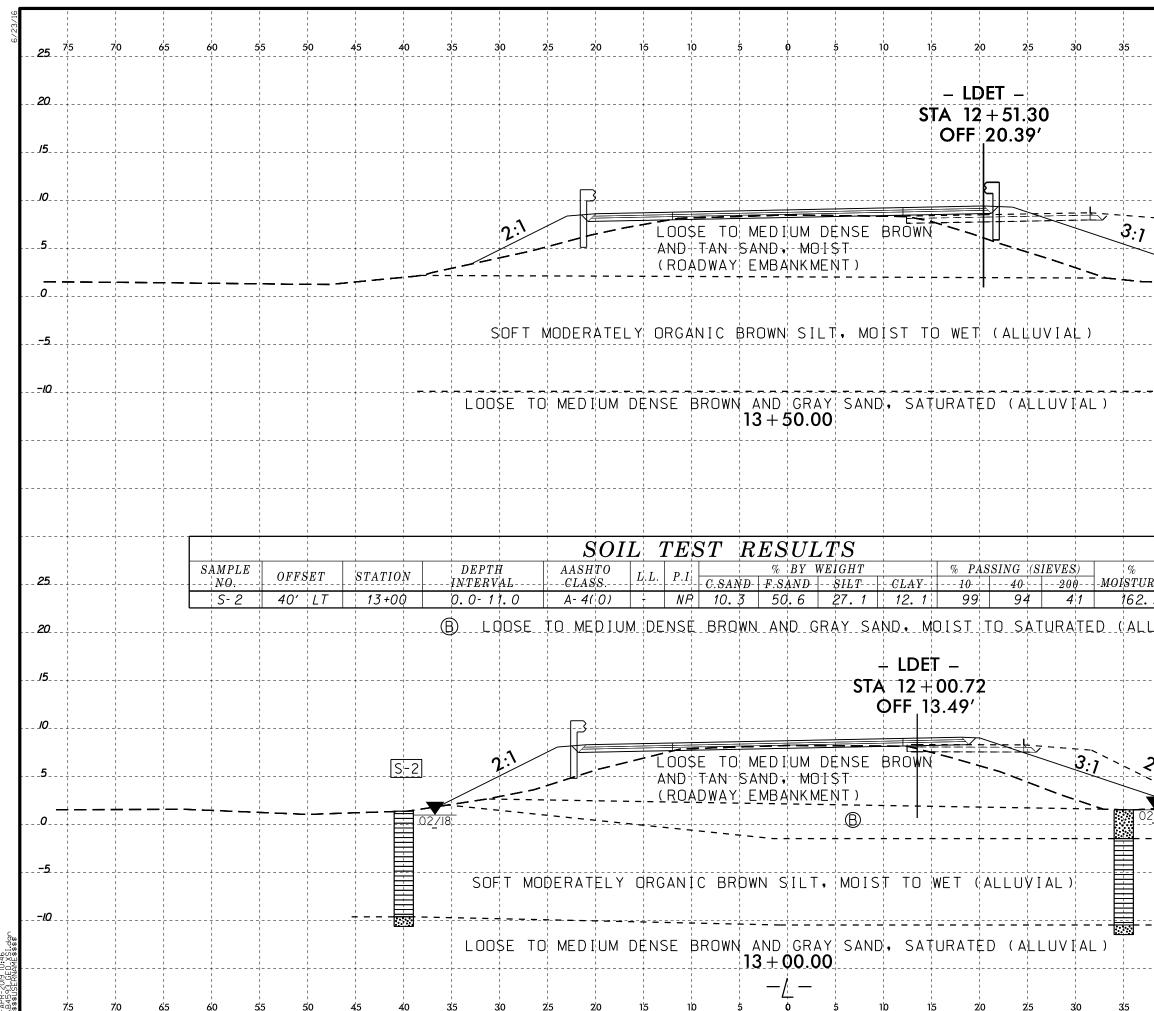
PROJECT REFERENCE NC).	SHEET NO.		
B-4593		5		
R/W SHEET N	10.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER		
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED				



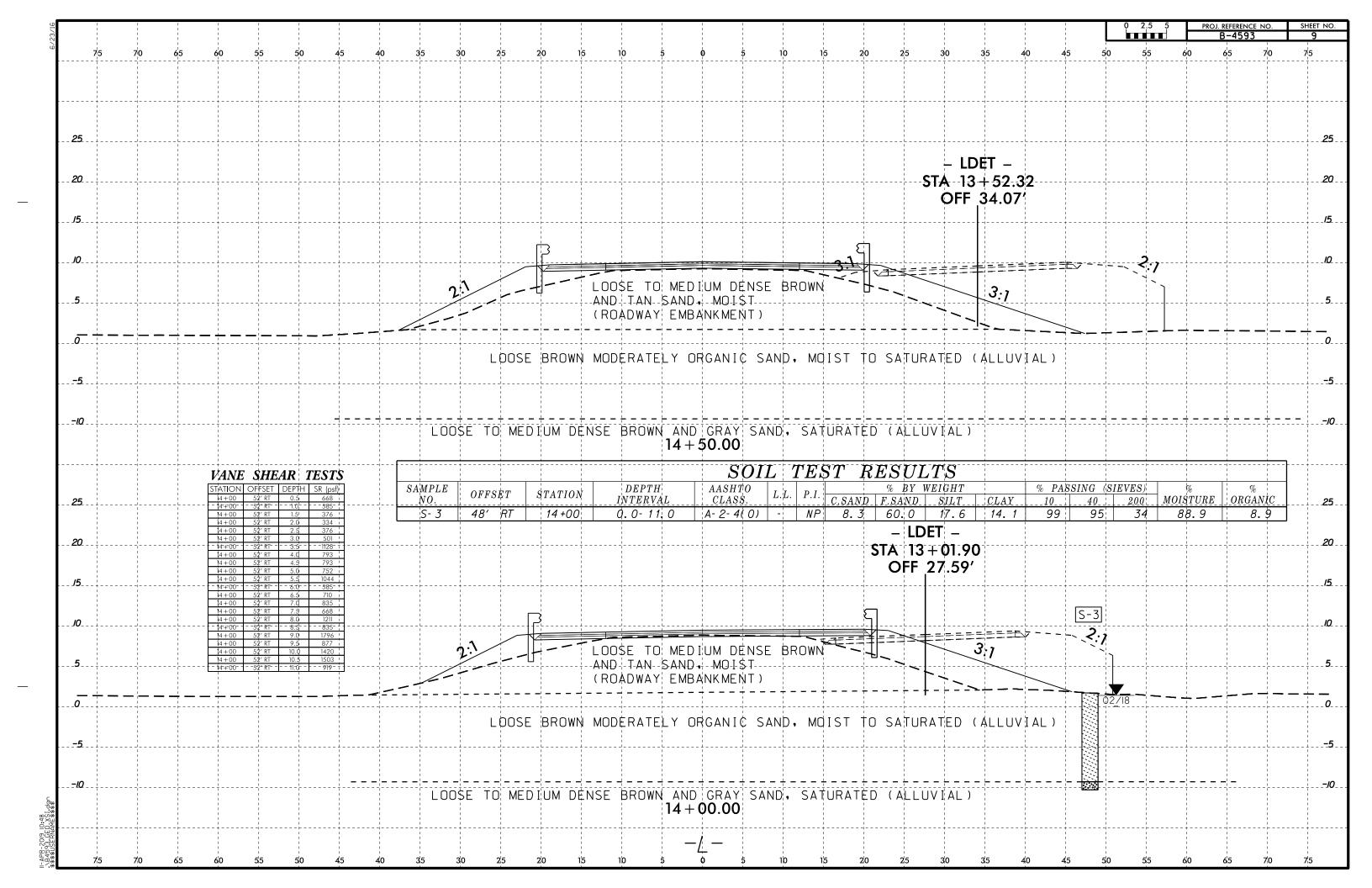
VANE SHEAR TESTS	1	1	-			PROJECT REFERENCE NO.	SHEET NO.
BRIDGE STATTADZYT- TZST DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED STATICAL OFFSCT 12 ST DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED STATICAL OFFSCT 14 100 SR (pSI) 14 100 SR (pSI) 14 100 O. 14 100 SZ RT D. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 10. STATICAL OFFSCT 14 100 SR (pSI) 14 100 SR (pSI) 14 100 O. 14 100 SZ RT D. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 14 100 SZ RT D. D. O. 14 100 SZ RT D. D. D.							
BY OF TREE DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 10	·		 				
BY OF TREE DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 10			1			INCOMPLE DO NOT USE FOR	TE PLANS
P224 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 10. 10. VANE SHEAR TESTS 0. 14:00 52:01 0. <	<u>BRIDGI</u> STÁ 17	<u>E</u> '+82+/+					
VANE SHEAR TESTS	= 12.51 -						
VANE SHEAR TESTS							
VANE SHEAR TESTS .0. STATION OFFSET, DEPTH SR (sf) -0. $\frac{14+03}{14+03}$ $22/R1$ $58/6$ $\frac{14+03}{14+03}$ $22/R1$ $53/6$ $\frac{14+03}{14+03}$ $22/R1$ $23/6$ $\frac{14+03}{14+03}$ $22/R1$ $23/6$ $\frac{14+03}{14+03}$ $22/R1$ $45/7$ $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 6.5 $\frac{14+03}{14+03}$ $52/R1$ 7.5 6.86 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.55 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.55 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.56 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.56 $\frac{14+03}{14+03}$ $52/R1$ $10.$ $10.$ $\frac{14+03}{14+03}$ $52/R1$ $10.$ $10.$ $\frac{10}{14+03}$ $52/R1$							
VANE SHEAR TESTS .0. STATION OFFSET, DEPTH SR (sf) -0. $\frac{14+03}{14+03}$ $22/R1$ $58/6$ $\frac{14+03}{14+03}$ $22/R1$ $53/6$ $\frac{14+03}{14+03}$ $22/R1$ $23/6$ $\frac{14+03}{14+03}$ $22/R1$ $23/6$ $\frac{14+03}{14+03}$ $22/R1$ $45/7$ $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 5.5 $\frac{14+03}{14+03}$ $22/R1$ 6.5 $\frac{14+03}{14+03}$ $52/R1$ 7.5 6.86 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.55 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.55 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.56 $\frac{14+03}{14+03}$ $52/R1$ 8.5 8.56 $\frac{14+03}{14+03}$ $52/R1$ $10.$ $10.$ $\frac{14+03}{14+03}$ $52/R1$ $10.$ $10.$ $\frac{10}{14+03}$ $52/R1$							
STATION OFSET: DEPTH SR (pdf)							
STATION OFSET: DEPTH SR (pdf)							
STATION OFSET: DEPTH SR (pdf)		VANE	SHE	AR 1	TESTS		0
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		STATION	OFFSET	DEPTH	\$R (psf)		
$ \frac{ 4+00 52' \text{ kr} 20, 334, 100 $							
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$							_10
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		14 + 00	52'RT !	2.5	376		<u>-</u> 10
$ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{5}{50} & \frac{752}{1044} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{5}{50} & \frac{1544}{1640} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{5}{50} & \frac{1325}{1648} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{150}{1211} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{150}{1211} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{150}{1211} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{100}{105} & \frac{1120}{160} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 5 } & \frac{100}{105} & \frac{1120}{160} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{1120}{1105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{1120}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{1120}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{105} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{10} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{10} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{10} \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & \frac{52' \text{ RT}}{ 100 } & \frac{105}{ 100 } \\ \frac{ 4+00 }{ 4+00 } & 1$		14+00	52′RT !	4.0	793		
$ \frac{ 4+00 }{ 4+00 } & \frac{52^{\circ}}{52^{\circ}} \text{II} & \frac{4}{65} & \frac{586}{700} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{2}{65} & \frac{700}{756} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{2}{75} & \frac{586}{835} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{2}{75} & \frac{585}{835} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{8}{80} & \frac{1211}{835} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{9}{25} & \frac{587}{835} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{9}{25} & \frac{587}{835} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{100}{10^{\circ}} & \frac{1420}{1400} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{100}{10^{\circ}} & \frac{1803}{1400} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{100}{10^{\circ}} & \frac{1803}{1400} \\ \hline 4+00 & \frac{52^{\circ}}{62^{\circ}} \text{II} & \frac{100}{10^{\circ}} & \frac{199}{19} \\ \hline \\ $!					· · · · · · · · · · · · · · · · · · ·	
$ \frac{ 4+00 }{ 4+00 } \frac{52' \text{ RT} 1}{22} \left(\frac{45}{236} - \frac{70}{200} - \frac{14'+00}{22' \text{ RT} 1} - \frac{75}{25} - \frac{668}{68} - \frac{121}{1211} - \frac{14'+00}{14'+00 } \frac{52' \text{ RT} 1}{22' \text{ RT} 1} - \frac{85}{20} - \frac{13}{1210} - \frac{14'+00}{14'+00 } - \frac{52' \text{ RT} 1}{22' \text{ RT} 1} - \frac{85}{20} - \frac{17}{1210} $							
= -30. $= -30.$ $= -30.$ $= -30.$ $= -30.$ $= -40.$ $=$							
$\frac{ 4+00 }{ 4+00 } \frac{52' \text{ RT} 8.6 121 }{ 34+00 } \frac{122' \text{ RT} 8.5 835}{ 14+00 } \frac{122' \text{ RT} 8.6 827}{ 14+00 } \frac{122' \text{ RT} 9.0 1796}{ 14+00 } \frac{122' \text{ RT} 9.0 1796}{ 14+00 } \frac{122' \text{ RT} 10.5 1503}{ 14+00 } \frac{122' \text{ RT} 10.5 1203}{ 14+00 } \frac{122' \text{ RT} 1203}{ 1203 } \frac{122' \text{ RT} 1203}{ 1203 } \frac{122' \text{ RT} 1203}{ 1203 } \frac{122' \text{ RT} 1203}{ 1000 } \frac{122' \text{ RT} 1203 }$		l4±00	52′RI_	ZQ	835		30
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1						
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		14+00	52′RT !	8.5			
$\frac{14+00}{14+00} \frac{52' \text{ RT}}{52' \text{ RT}} \frac{10.0}{10.5} \frac{1420}{1503} \frac{1420}{199} \frac{1420}{144+00} \frac{1420}{52' \text{ RT}} \frac{10.0}{10.5} \frac{1503}{1903} \frac{1420}{919} \frac{14400}{144+00} \frac{152' \text{ RT}}{10.0} \frac{10.5}{919} \frac{1420}{919} \frac{1400}{919} $	i						-40
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		14+00	52'RT !	10.0	1420		
	1						
$\begin{array}{c} 30 \\ \hline & 30 \\ \hline \\ $							
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1		1				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							40
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	5+00,00						
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	5+00,00	17 A NT			TESTS		30 20
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	5+00,00						30 20
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5+00,00	STATION 18+50	OFFSET 55' LT	DEPTH 0.5	SR (psf)		30 20
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	5+00100	STATION 18+50 18+50 18+50	OFFSET 55' LT 55' LT 55' LT	DEPTH 0.5 1.0 1.5	SR (psf) 42 167 0		30 20 10
-10 -10	5+00D0	STATION 18+50 18+50 18+50 - 18+50	OFFSET 55' LT 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 -	SR (psf) 42 167 0 42 		30 20 10
-10 - 10 - 10 - 10 - 10 - 10 - 10 - 10	5+00,00	STATION 18+50 18+5b 18+5b - 18+50 - 18+50 18+5b 18+5b	OFFSET 55' LT 55' LT - 55' LT - 55' LT 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0	SR (psf) 42 167 0 <u>1</u> - 42 251 125		30 20 10
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5+00.D0 -	STATION 18+50 18+50 - 18+50 - 18+50 - 18+50 18+50 18+50 18+50	OFFSET 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.5 3.0 3.5	SR (psf) - 42 - 167 - 0 - 251 - 251 - 125 - 1963		30 20 10
-20	5+00.00	STATION 18 + 50 18 + 50	OFFSET 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0 3.5 4.0 4.5 -	SR (psf) 42 167 201 251 125 1963 585 		30 20 10 0
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	5+00D0 -	STATION 18+50 18+50 - 18+50 - 18+50 18+50 18+50 18+50 - 18+50 - 18+50 - 18+50 - 18+50 - 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.5 3.0 3.5 4.0 4.5 - 5.0	SR (psf) 42 167 - 2:167 - 1:251 - 1:125 - 1:963 - - - - - - - - - - - - - - - - - - - - - - - - - - - - -		30 20 10 0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		STATION 18+50 18+50 - 18+50 - 18+50 18+50 18+50 18+50 - 18+50 - 18	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0 3.5 4.0 4.5 - 5.0 5.5 6.0	SR (psf) 42 167 251 125 1963 585 1253 1253 1253 1253 1253 1253 1253 376		30 20 10 0
	5+00.D0 -	STATION 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0 3.5 4.0 4.5 - 5.5 6.0 6.5	SR (psf) - 1 42 - 1 167 - 2 251 - 1 251 - 1 125 - 585 - - - 877 543 376 459		30 10 0 10
	5+00.00 	STATION 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.5 3.0 3.5 4.0 5.0 5.5 6.0 6.5 7.5	SR (psf) 42 167 251 125 1963 585 -1253 376 459 -2543 -376		30 10 0 10
18+5b 55' LT 10.0 585 18+5b 55' LT 10.5 209 18+50 55' LT 11.0 710 18+5b 55' LT 11.0 710 18+5b 55' LT 11.5 167	5+00.00 -	STATION 18+50 18+50 - 18+50 18+50 18+50 18+50 18+50 - 18+50 - 18+5	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0 3.5 4.0 4.5 5.0 5.5 6.0 6.5 7.0 - 7.5 8.0	$\begin{array}{c c c c c c c c c c c c c c c c c c c $		30 10 0 10
18+50 55' LT ! 10.5 ! 209 18+50 55' LT ! 11.0 ! 710 18+50 55' LT ! 11.5 ! 167	5+00,00	STATION 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.0 - 2.5 3.0 3.5 4.0 5.5 6.0 6.5 5.5 6.0 6.5 - 7.6 8.0 8.0 8.0 8.0 8.0	$\begin{array}{c c} & & - & - \\ & & SR (psf) \\ & & 42 \\ & & 167 \\ & & 0 \\ & & 251 \\ & & 125 \\ & & 125 \\ & & 1253 \\ & & 585 \\ & & -1253 \\ & & 585 \\ & & -1253 \\ & & 585 \\ & & -1253 \\ & & 585 \\ & & -1253 \\ & & 585 \\ & & -1253 \\ & & 387 \\ & & 376 \\ & & 459 \\ & & 459 \\ & & 459 \\ & & 459 \\ & & 459 \\ & & -292 \\ & & -543 \\ & & 1754 \\ & & 334 \\ & & 334 \\ & & 501 \end{array}$		30 20 10 10 10 20
18+50 55'LT 11.5 167	5+00,00	STATION 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.2 3.0 3.5 4.0 4.5 5.5 6.0 6.5 5.5 6.0 6.5 7.0 8.0 8.5 9.0 0 9.5	SR (psf) 42 167 251 125 1963 585 -1253 1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1253 -1543 376 4292 -1543 1754 334 501 -376		30 20 10 10 10 20
	5+00D0 -	STATION 18+50	OFFSET 55' LT : 55' L	DEPTH 0.5 1.0 1.5 2.0 2.5 3.0 3.5 4.0 5.5 4.0 5.5 6.0 5.5 6.0 5.5 6.0 5.5 8.0 8.5 7.5 8.0 8.5 9.0 10.5			30 20 10 10 10 20
	5+00D0 -	STATION 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 2.20 2.5 3.0 3.5 4.0 4.5 5.0 5.5 6.0 6.5 7.0 7.5 8.0 8.5 9.0 7.0 - 7.5 10.0 10.0 10.0	$\begin{array}{c c c c c c c c c c c c c c c c c c c $		
	5+00)00	STATION 18+50 18+50 - J8+50 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 - 2.0 2.5 3.0 3.5 4.0 5.5 6.0 5.5 6.0 6.5 - 7.0 7.5 8.0 8.5 9.0 8.5 9.0 10.5 11.0 10.5 11.			
	5+00,00	STATION 18+50 18+50 - J8+50 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 - 2.0 2.5 3.0 3.5 4.0 5.5 6.0 5.5 6.0 6.5 - 7.0 7.5 8.0 8.5 9.0 8.5 9.0 10.5 11.0 10.5 11.5 11.5			
	5+00.00 -	STATION 18+50 18+50 - J8+50 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 - 2.0 2.5 3.0 3.5 4.0 5.5 6.0 5.5 6.0 6.5 - 7.0 7.5 8.0 8.5 9.0 8.5 9.0 10.5 11.0 10.5 11.5 11.5			
	5+00D0 -	STATION 18+50 18+50 - J8+50 18+50	OFFSET 55' LT 55' LT	DEPTH 0.5 1.0 1.5 - 2.0 2.5 3.0 3.5 4.0 5.5 6.0 5.5 6.0 6.5 - 7.0 7.5 8.0 8.5 9.0 8.5 9.0 10.5 11.0 10.5 11.5 11.5			

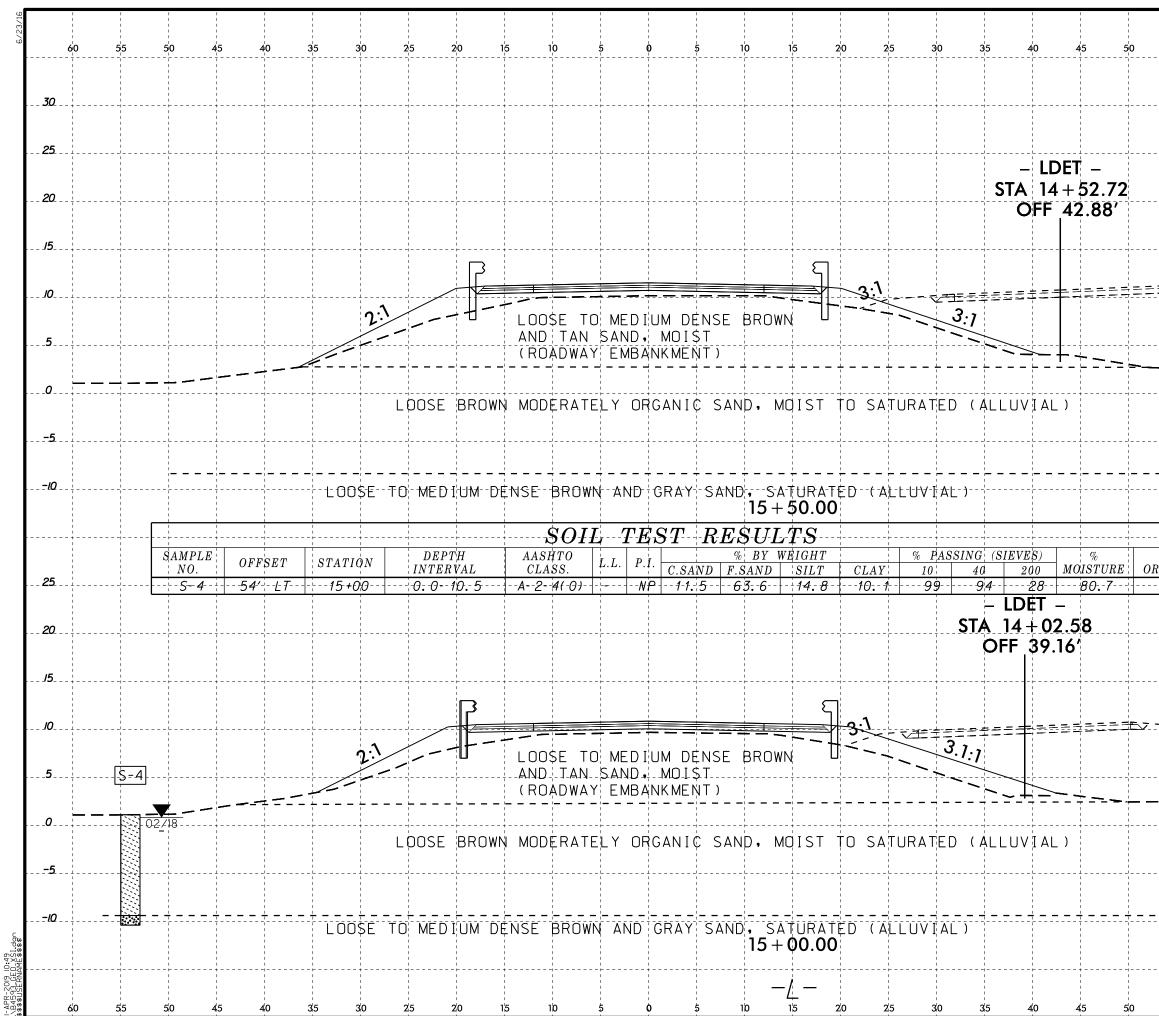


1		0	2.5 5 PROJ. REFERENCE NO. B-4593		ce no. 3	SHEET NO. 7	
40	45	1	55	60	65	70	75
	·	!	·	+	·		
		!	L			 I I	
	·			+			
		!	· L		·	L	<u>-</u>
VES)	1%		%		·		
200 8	MOISTURE		GANIC 225	+	·		
		1					
		!	L		·	 I	
	·						
ה							
	·		·	+	·		5
-02./-1	<u> </u>						<u> </u>
	D	! 	L		· 	L	
	 						-5
ΓΕĎ	(ALLUVI	AL)					
 		 			·		
		! !	· L		·		
	·				·		
		!					<i>IU</i>
, , , , ,	, , , , , , , , , , , , , , , , , , ,	, , , , ,	L	, , , , , ,		L	
	 :			 ¦	=	 <u> </u>	
40	45	50	55	60	65	70	75
40	40	D _I U	20	o U	co	10	C'V

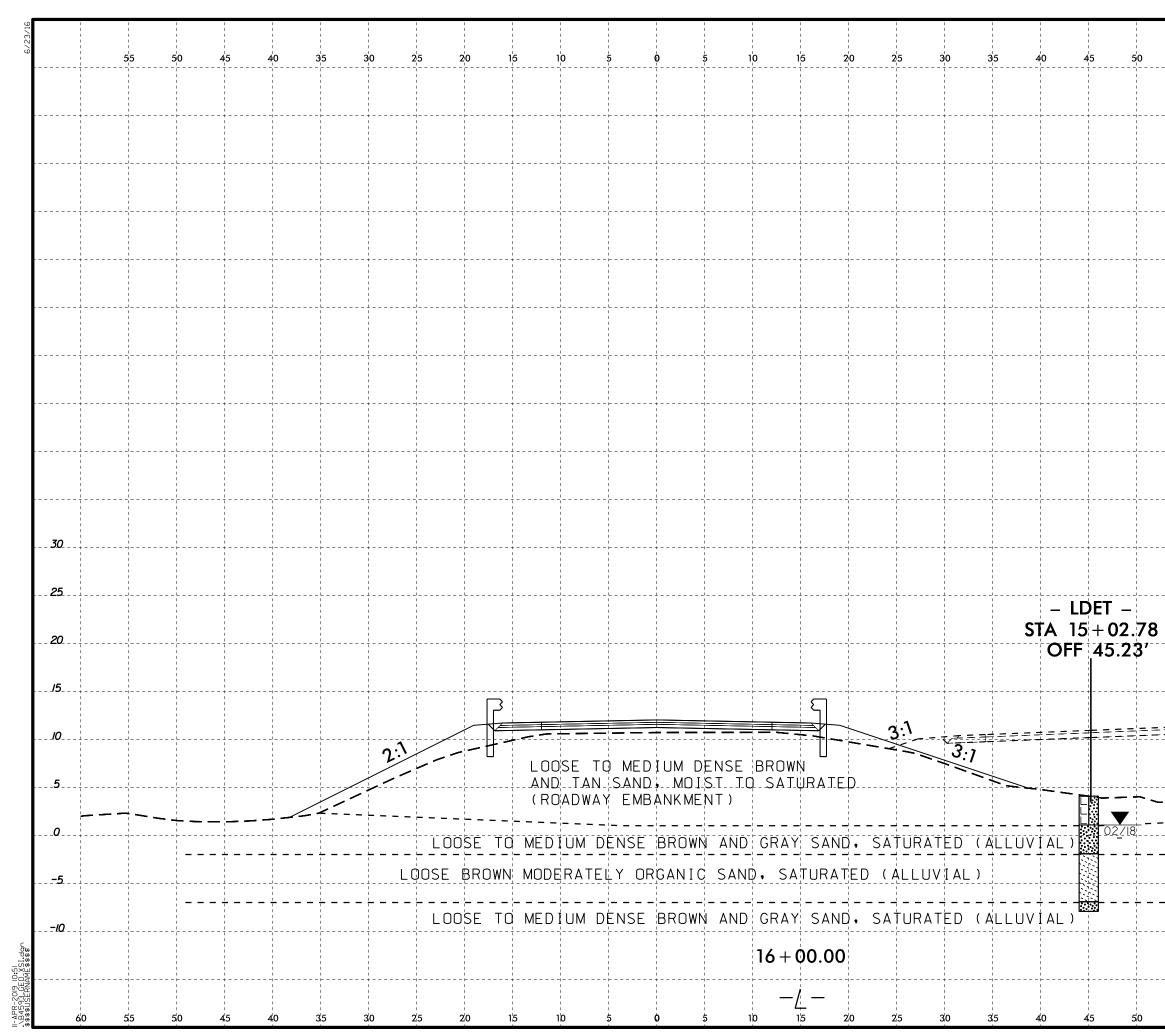


40 45 50 55 60 65 70 7	8 25 20 15 10 5 5 70
? ? ? <th>15 16 10 5 0 6</th>	15 16 10 5 0 6
RE - ORGANIC -	15 10 5 0 5
RE -ORGANIC- 3 13.7 LUVIAL)	10 5 0
RE -ORGANIC- 3 13.7 LUVIAL)	10 5 0
RE -ORGANIC- 3 13.7 LUVIAL)	5 5
RE -ORGANIC- 3 13.7 LUVIAL)	5 5
RE -ORGANIC- 3 13.7 LUVIAL)	 0 5
RE -ORGANIC- 3 13.7 LUVIAL)	 0 5
RE -ORGANIC- 3 13.7 LUVIAL)	5
<u>RE - ORGANIC-</u> <u>3</u> 13.7 LUVIAL)	5
<u>RE - ORGANIC-</u> <u>3</u> 13.7 LUVIAL)	
<u>RE - ORGANIC-</u> <u>3</u> 13.7 LUVIAL)	
<u>RE - ORGANIC-</u> <u>3</u> 13.7 LUVIAL)	// 0
<u>RE - ORGANIC-</u> <u>3</u> 13.7 LUVIAL)	
<u>3 13.7</u> LUVIAL)	
	20
2.7	20
2.,	
2.,	
2.,	
	_
2/18	0_
	5
	10
	_/0
40 45 50 55 60 65 70 7	_/0

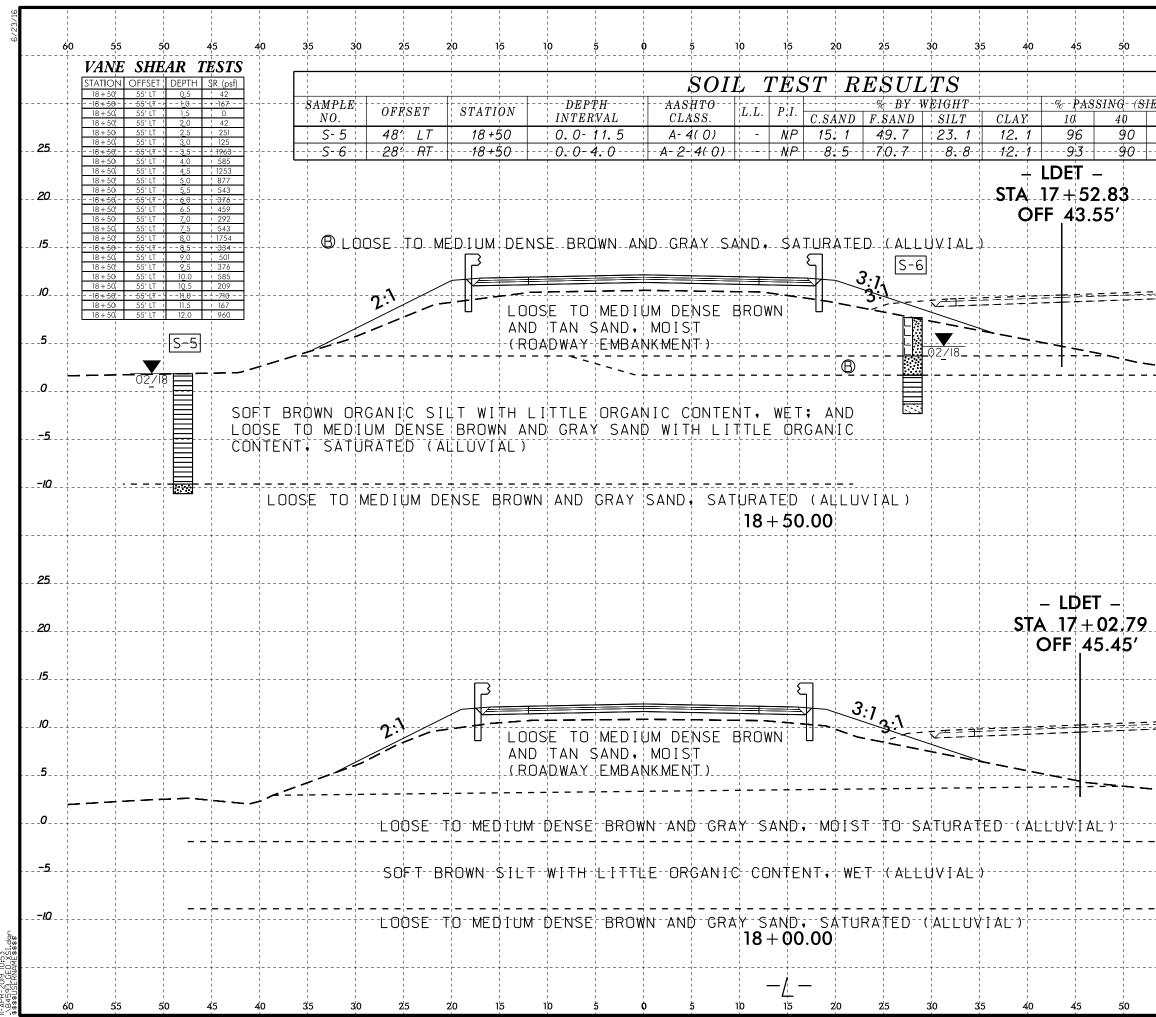




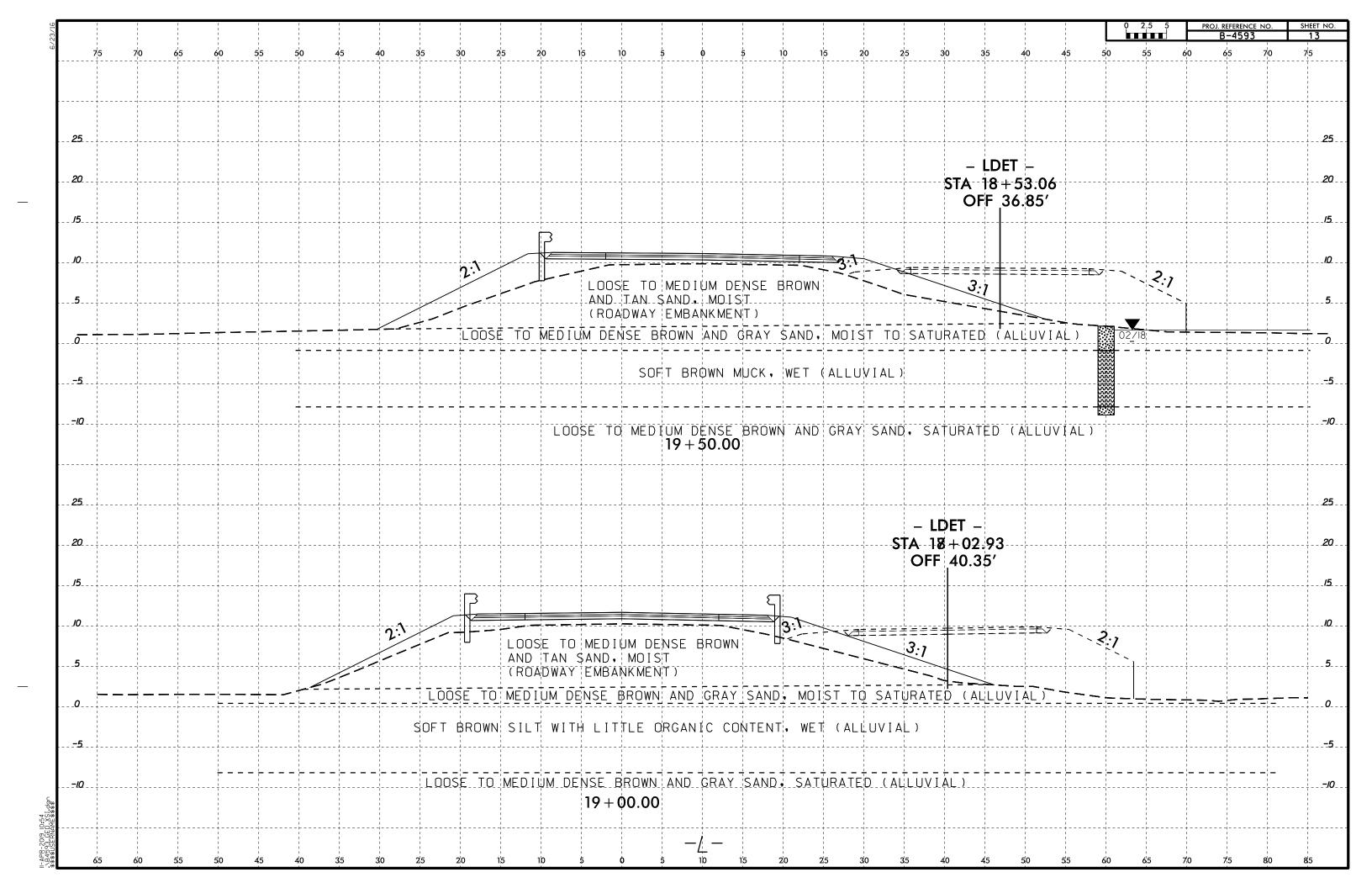
	1	1	02	.5 5	PROJ. RI B	FERENCE NO	D. S⊦	ieet no. 10
5	56	06	1	1	1	0 8		0
	 	T	 	 	T			
	, , , , , ,	+	, , , , ,					
	, , , , , , ,	, , , , , , , , , , , , , , , , , , ,	, , , , , , , , , , , , , , , , , , ,		- 			
	 	 	 	1 1 1	 			
	, 	, , , , , , ,	, , ,		, 			
	, 	, +	, 					15
: =="	¦ ≒,	<u>, ?:</u> 1	 	 	1 1 1			
			 ר					10
	 	 		1 1 1 1				5
		† 	;-+ ! !	 	† ! !			<i>.</i>
	·		¦	<mark></mark>	<u></u>	' 		0
	 	 	 	 				5
	, 	- 	, 	 	 			
		 	— — — — — 	' 	- 			
	 	' ' +	 	 	+	 		
% RGA	NIC	1 1 1	1 1 1		1 1 1			
7-	6	 	 	 	+			
	 	 	 	1 1 1	 			
	 	† 	; 		† ! !			2 0
	 	 +	 	 	 			
		1						
	, , , , , ,		, , , ,	, , , , ,				
		<u> </u>			<u></u>			
	, 	, 	, 		+	 		0
	1 1 1 1	1 1 1 1	1 1 1 1		1 1 1 1			_
	L I I	 	 	L				5
	¦ ¦ 	¦ ¦ 		¦ 				- 10
			 		<u>+</u>			U
	 	T 	1 - - - - - - - - - 	 	+			
5	5 6	0 6	5 7	o 7	75			

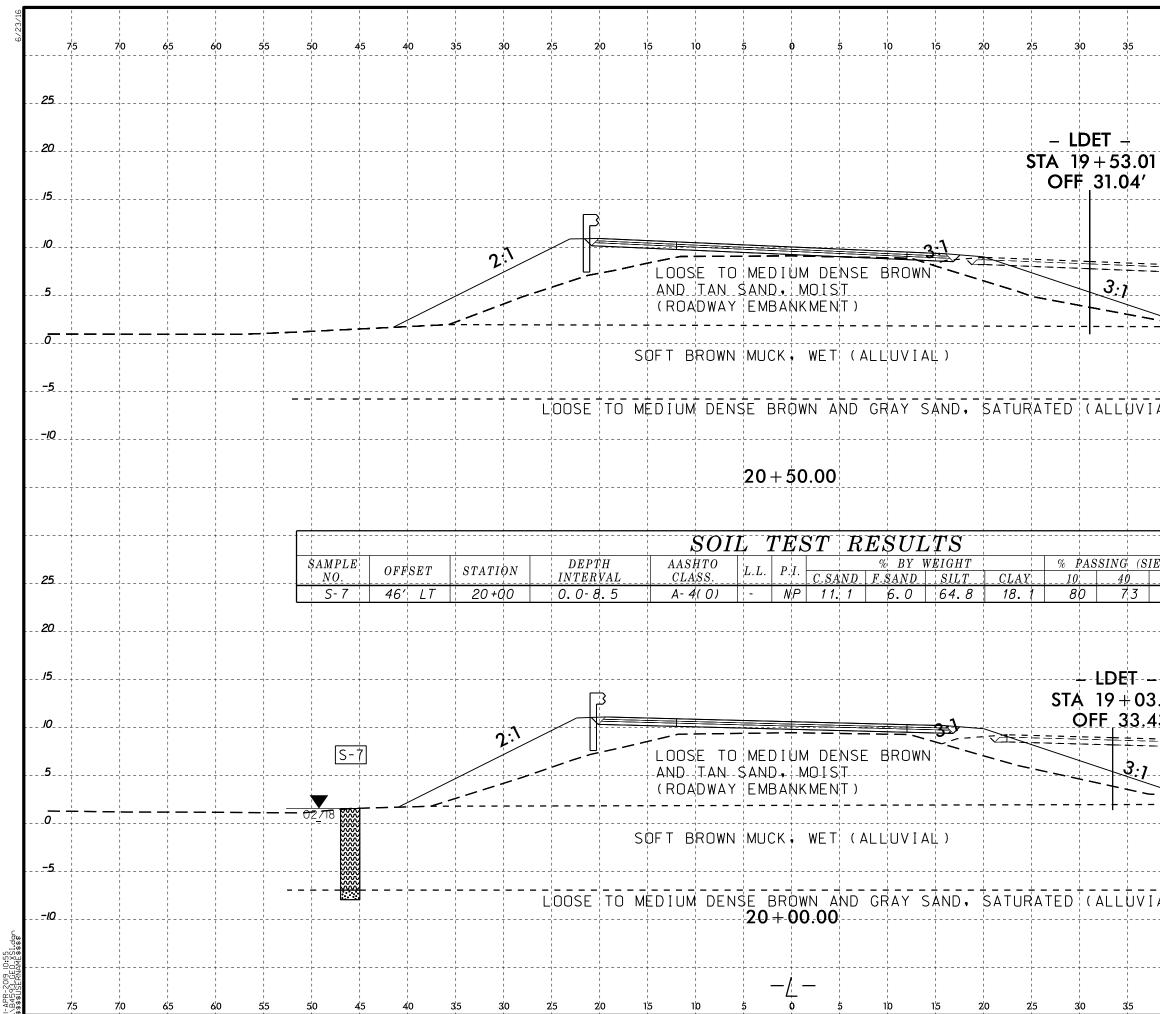


	1	1	0 2.5 5		2.5 5 PROJ. REFERENCE NO. B-4593			SHEET NO.	
	, , , ,	1 1 1		1	1	-4593		11	
5	5 6	0 6	5	7 <mark>0</mark>	7 [/] 5 +				
	1 1 1	1 1 1	1 1 1						
	1 1 1	1 1	1 1	1 1 1	1 1 1				
		+	1 1 1		+	 			
	1 1	, 	, 1 1		- 	, , , ,			
	 	 	 		<u> </u>				
		, 1 1 1	, 1 1		- - 				
	 	 	 	¦ 	 				
		, 	, 		- - 	, 			
	1 1 1	1 1 1	 			 			
		+	 	- -	+	 			
	1 	1 1 1	1 1 1	1 1 1	1 1 1	 			
	 L	 	 	 	<u> </u>	 			
	 	 	 	 	1 1 1				
	,	, - 	,						
	 	 	 	1					
		+	' 	+	+				
		1 1 1	 						
		, , +	; ! !		+				
	1 1 1	1 1 1	1 1 1						
	1 1 1	1 1 1	 		 	1 			
			 					70	
		+	' 	+	+				
	1 1 1	1 1 1	 						
		, , +	, 		+	 			
	1 1 1	1 1 1	1 1 1		 				
		 	 	¦ 	 				
	1 1 1	 	 		 	 			
	 	+ 	 		+			15	
			່. ວ	1	1 1 1	 			
==	:== <u>-</u>	Y ``	<u>?:</u> 1	 	 	 		10	
	1 1 1	1 1 1 1		1 1 1	 	 			
	 	 	¦	 	 	 			
		<u>-</u> ·	╧╶╱╌└─					0	
	 ! !	+ 	 	- -	+ 	 		<i>u</i>	
	<u> </u>		<u> </u>			_ 			
	 	 	 	 	 	 		5	
	- - - -	- 	, 				·		
	 	 	 	¦ 	1 1 1			10	
	, 1 1	, 	, 	- - - - -	, 	, 			
	1 1 1	 	 	1 1 1	1 1 1	 			
		+	1 1 1		+	 			
	1 1 1	 	 -		1 1 1	 			
5	5 6	0 6	5	70	75	1			

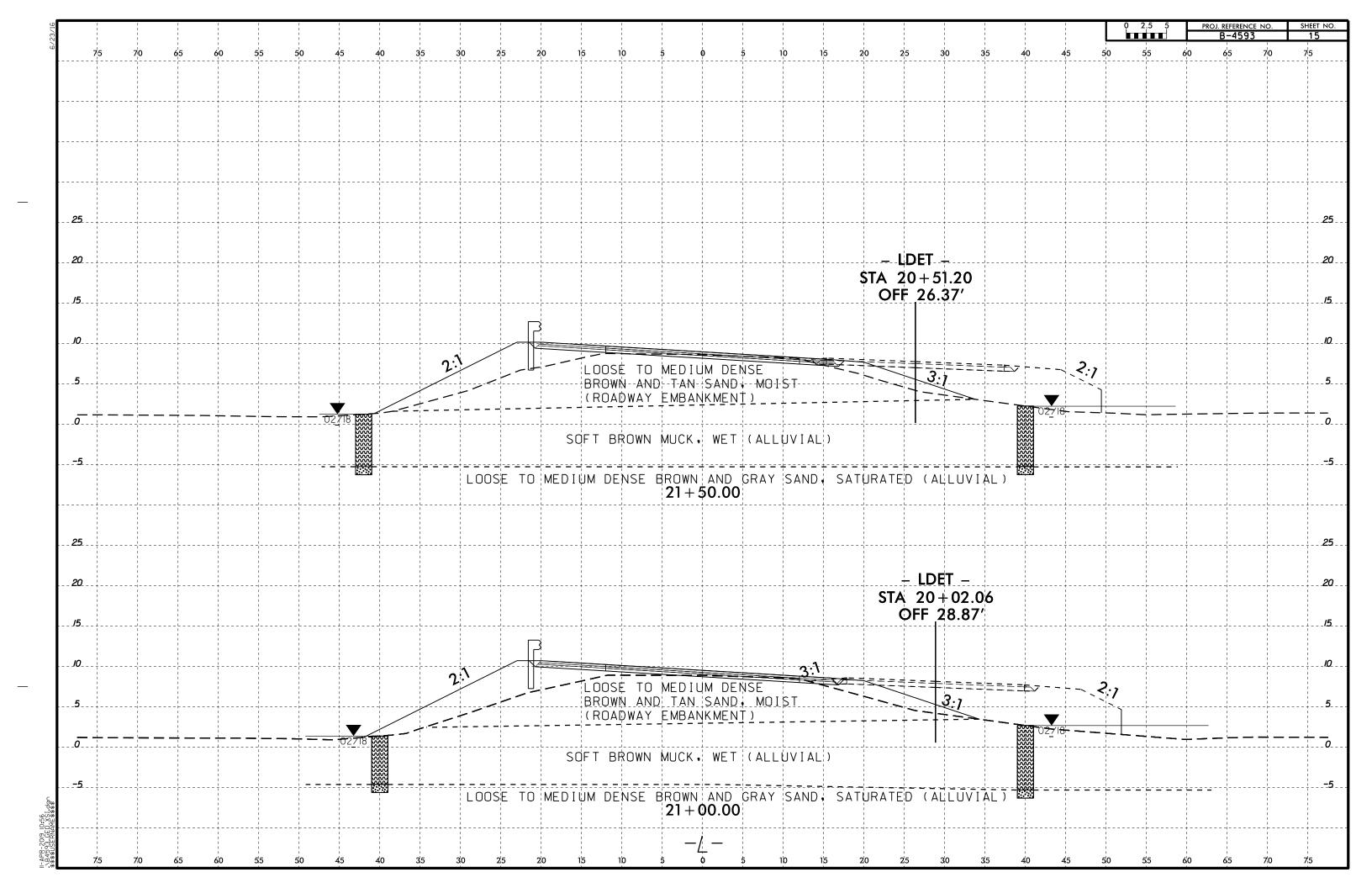


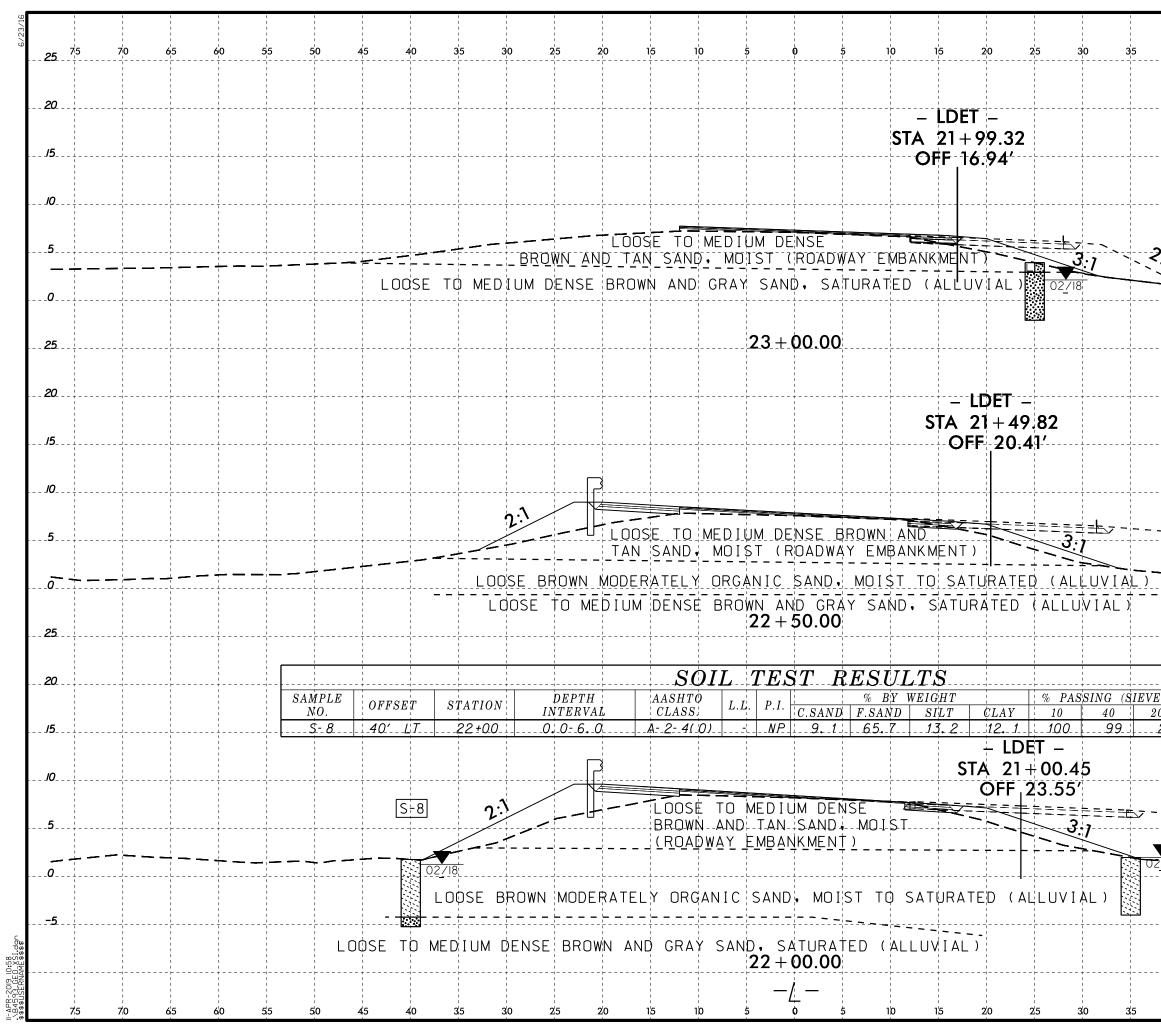
	1	0 2.5 5	PROJ. REFERENCE NO B-4593	SHEET NO.	
55	60 6	1 1	<u> </u>	12	
EVES)		OPCANIC			
<u>200</u> 37	MOISTURE				
2-2					
 		I I I I I I I I			
- EV		• • •		<i></i>	
		· · · · · · · · · · · · · · · · · · ·		5	
·	·			o	
		I I I I I I I I			
 		 		-5	
		I I I I I I I I			
				10	
	i	 			
		, , , , , , , , , , , , , , , , , , , ,			
		· · · · · · · · · · · · · · · · · · ·			
		, , , , , , , , , , , , , , , , , , ,			
:E≓£.	==,	:		<i>l0</i>	
				5	
	-02-/18	· · · · · · · · · · · · · · · · · · ·		0	
		1 1 1 1 1 1 1		5	
		· · · · · · · · · · · · · · · · · · ·			
		¦	·		
		· · · · · · · · · · · · · · · · · · ·			
		· · · · · · · · · · · · · · · · · · ·			
55	60 6	5 70	7.5		





		0 2.5 5		PROJ. R	PROJ. REFERENCE NO. B-4593		SHEET NO. 14
40	45 5	1	1	1		-	14 75
					 	/0 	
			-	- +			
					- - 		
ù ¦			- L 	- 1		- L	
			· ·				
		1 1 1	1 1 1	 	 	 	
		2	-	- +			
		2:1			 		
			1 1 1	1 	1 1 1	 	1 1 1
		╧╼╹	¦		<u></u>		<u> </u>
; ; ; ; ;		i 	i 	; ; ; ; ;	i 	; ; ; ;	-5
AL)				-			
							-10
		1 1 1	1 1 1	 	1 1 1	1 1 1	
			·	- + -		; 	
EVES)	%	9	/ 0				
<u>200N</u> 67	1 <u>0ISTURE</u> 406.4		A <u>NIC</u> D. 9	- <u>-</u>			
	100. 1			 	 	 	
				- 1			
		- 			- 		
 \	- +	 	- -	- 	 -	- 	15
3.17 13′							
+9 =======		- 2		- 1	 	- L	
		~					5
				- 			.
· - +			¦⊥	<u>-</u>	<u></u>		
			- 	- +			· •
, , , , ,		 	 	, , , , ,	 		-5
-						•	
AL)	 		 	 - 	1 1 1		
		 	 		1 1 1 -1	; ; ; ;	
		1					
40	45 5	50	55	60 0	65	70	75





1		(0 2.5 5		ROJ. REFEREN	ICE NO.	SHEET NO. 16
40	45	1	55	60	65	70	^{7/5} 25
 		1 1 1		 		 	
?:7				+	·		
··/							
							0
					·		
					·		20
 		·	 	 	·		
 		1 1 1		 		 	
					·		<i>10</i>
	<i>?:</i> 7				·		5
<mark> </mark> 		·¦ ! !	 		·		
ES)	 %	9	6		·		
200 N 28	% 10ISTURE 364	ORG.	ANIC 5.2			 	
20	0	<u> </u>	<u></u>	± 	·	L	<i></i>
							<i>10</i>
	 ! !	·			·		<u>-</u> <i>i</i> u
	. ? :7	 		 			5
				+ 	· 	 	
2/18			; ;				<u> </u>
		 		+			
		1 1 1		1 1 1			
40	45	50	55	60	65	70	75