



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

J. ERIC BOYETTE
SECRETARY

October 12, 2021

MEMORANDUM TO: Clark Morrison PhD, P.E.
State Pavement Design Engineer

Tatia L. White, P.E., PLS
State Roadway Design Engineer

FROM: J. L. Pilipchuk, P.E., L.G.
State Geotechnical Engineer

DocuSigned by:
John Pilipchuk
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STATE PROJECT: 38332.1.FS1 (B-3186, B-5898)

COUNTY: Haywood

DESCRIPTION: US 23/US 74/US 19 (Great Smoky Mountain Hwy) from
west of NC 209 (Crabtree Rd.) to east of Russ Ave.

SUBJECT: Pavement and Subgrade Investigation Report

The Geotechnical Engineering Unit has completed the evaluation of the pavement and subgrade investigation for this project and presents the following.

The proposed work consists of widening the inside and outside of the existing four-lane divided roadway to construct a six-lane roadway with full-depth paved shoulders.

The subgrade beneath the existing roadway consists of residual and roadway embankment soils. Predominant soil types are micaceous, sandy silt (A-4) and silty clays (A-7-5, A-7-6).

Anticipated borrow will likely consist of sandy silt (A-4) and silty clay (A-7).

The length of this project is 1.2579 miles.

The existing pavement is in good condition with low to moderate severity transverse and longitudinal cracking present.

The project mainline is approximately 25 percent embankment.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

A. Highly Plastic Clays:

Cohesive soils with a PI of 26 or greater were not encountered on this project.

B. Ground Water or Trapped Water within the Pavement:

Neither ground water nor trapped water were encountered during this investigation.

C. Soils with a High Moisture Content:

Locations of soils that were classified as wet to saturated.

LINE	STATION AND OFFSET	MOISTURE CONTENT
-L-	35+34 EB OSS	25.4%

D. Auger Refusal

Locations of Auger refusal within 6 feet of proposed subgrade.

LINE	STATION AND OFFSET
-L-	20+09 WB OSS
-L-	76+90 EB OSS, EB OSL
-Y1LT-	13+66 WB OSL

JLP/JBB/LMH

ATTACHMENT 1:	Pavement and Subgrade Inventory	43
ATTACHMENT 2:	DCP Graphs	20
ATTACHMENT 3:	Pavement Core Evaluation	03



Jeffrey Brian Barfield

DocuSigned by:
Jeffrey Brian Barfield 10/13/2021
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REFERENCE: B-3186/B-5898

PROJECT: 38332

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3186/B-5898	1	42

**ROADWAY
SUBSURFACE INVESTIGATION**

COUNTY HAYWOOD
PROJECT DESCRIPTION US 23-74 BRIDGE 155 & 158
OVER RICHLAND CREEK ON US 23-74
NORTHBOUND LANE (COMBINE WITH B-5898)
PAVEMENT AND SUBGRADE INVESTIGATION

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2, 2A	LEGEND (SOIL & ROCK), ABBREVIATIONS
3	ROADWAY TITLE SHEET
4-10	PLAN SHEETS
11-13	PAVEMENT INVESTIGATION DATA SHEETS
14-17	DUAL MASS DYNAMIC CONE PENETROMETER DATA SHEETS (-L- EB)
18	PAVEMENT CORE PHOTOS (-L- EB)
19-23	DUAL MASS DYNAMIC CONE PENETROMETER DATA SHEETS (-L- WB)
24	PAVEMENT CORE PHOTOS (-L- WB)
25-26	DUAL MASS DYNAMIC CONE PENETROMETER DATA SHEETS (-YI- US 19)
27	PAVEMENT CORE PHOTOS (-YI- US 19)
28-42	LABORATORY TEST RESULTS

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

M. BREWER
D. UNDERWOOD
R. KRAL

INVESTIGATED BY CG2
DRAWN BY M. BREWER, P.E.
CHECKED BY R. KRAL, P.E.
SUBMITTED BY CG2
DATE SEPTEMBER 2021

Prepared in the Office of:



**CAROLINAS
GEOTECHNICAL
GROUP**
2400 CROWNPOINT EXECUTIVE DRIVE
SUITE 800
CHARLOTTE, NC 28227
(980) 339-8684



DocuSigned by:
D. Matthew Brewer 10/13/2021
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SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION					GRADATION					ROCK DESCRIPTION					TERMS AND DEFINITIONS									
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 208, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>					WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.					HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL, SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:					ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.									
SOIL LEGEND AND AASHTO CLASSIFICATION					ANGULARITY OF GRAINS					MINERALOGICAL COMPOSITION					COMPRESSION									
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS					THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.					MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.					SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50									
MINERALOGICAL COMPOSITION					COMPRESSION					PERCENTAGE OF MATERIAL					GROUND WATER									
GROUP CLASS. A-1, A-2, A-3, A-4, A-5, A-6, A-7					ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS MUCK, PEAT					TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC > 10%					WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP									
CONSISTENCY OR DENSENESS					MISCELLANEOUS SYMBOLS					ROCK HARDNESS					RECOMMENDATION SYMBOLS									
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)					ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRERD SOIL BOUNDARY INFERRERD ROCK LINE ALLUVIAL SOIL BOUNDARY					25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES SPT DMT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION					UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK									
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270					AR - AUGER REFUSAL BT - BORING TERMINATED CL. - CLAY CPT - COARSE PENETRATION TEST CSE. - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY					MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY					VST - VANE SHEAR TEST WE. - WEATHERED UNIT WEIGHT DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO									
TEXTURE OR GRAIN SIZE					ABBREVIATIONS					FRACTURE SPACING					BEDDING									
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)					DRILL UNITS: CME-45C CME-55 CME-550 VANE SHEAR TEST PORTABLE HOIST MOBILE B29					ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 4" SOLID STEM AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE STEEL TEETH TRICONE TUNG-CARB. CORE BIT (4-INCH DIAMETER)					VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY CLOSE LESS THAN 0.16 FEET					VERY THICKLY BEDDED 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET				
SOIL MOISTURE - CORRELATION OF TERMS					EQUIPMENT USED ON SUBJECT PROJECT					INDURATION					BENCH MARK: N/A									
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION					HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B H N HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST DUAL MASS DCP					FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.					ELEVATION: FEET									
PLASTICITY					NOTES:					ROADWAY DESIGN FILES DATED 7/29/2021 PROVIDED BY NCDOT					PAVEMENT CORE WITH DCP BULK SAMPLE									
NON PLASTIC SLIGHTLY PLASTIC MODERATELY PLASTIC HIGHLY PLASTIC PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH					DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																			

Tip No. B-3186/B-5898 | WBS No. 38332.1.FS1 | Haywood County

ABBREVIATIONS

RT LN = Right Lane

LT LN = Left Lane

OSL = Outside Lane

ISL = Inside Lane

PS = Paved Shoulder

LTL = Left Turn Lane

RTL = Right Turn Lane

MID = Middle Lane

CTL - Center Turn Lane

ISWP = Inside Wheel Path

OSWP = Outside Wheel Path

PS = Paved Shoulder

FW = From White Line

FY = From Yellow Line

RT = Right

LT = Left

(I) = Inside

(O) = Outside

BOC = Back of Curb

C&G = Curb and Gutter

EOP = Edge of Pavement

CR = Crown

S = Super

C = Cut

F = Fill

DCP = Dynamic Cone Penetrometer

M = Moist

W = Wet

N/A = Not Observed

NSR = No Sample Recovered

Ref- = Soil Reference Sample

S- = Soil Grab Sample

SS- = Split Spoon Sample

RE = Roadway Embankment

F. = Fine

Cse. = Coarse

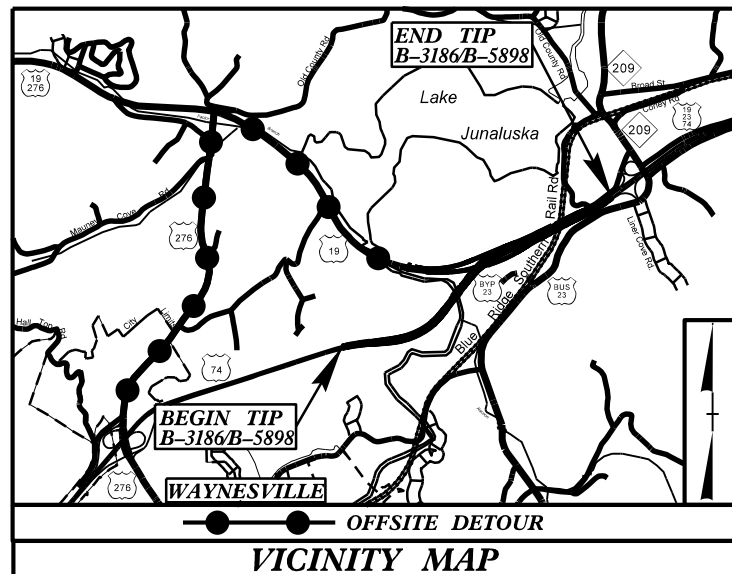
ABC = Aggregate Base Course

STBC = Soil Type Base Course

09/08/99

TIP PROJECT: B-3186 / B-5898

See Sheet 1-A For Conventional Symbols



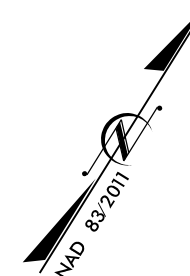
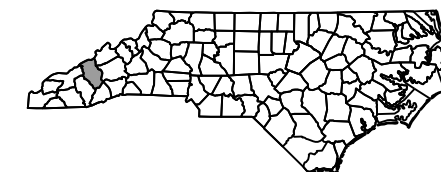
75% PLANS

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

HAYWOOD COUNTY

LOCATION: US 23/US 74/US 19 (GREAT SMOKY MOUNTAIN HWY)
FROM WEST OF NC 209(CRABTREE RD.) TO EAST OF RUSS AVE.
TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURES
AND UTILITIES.

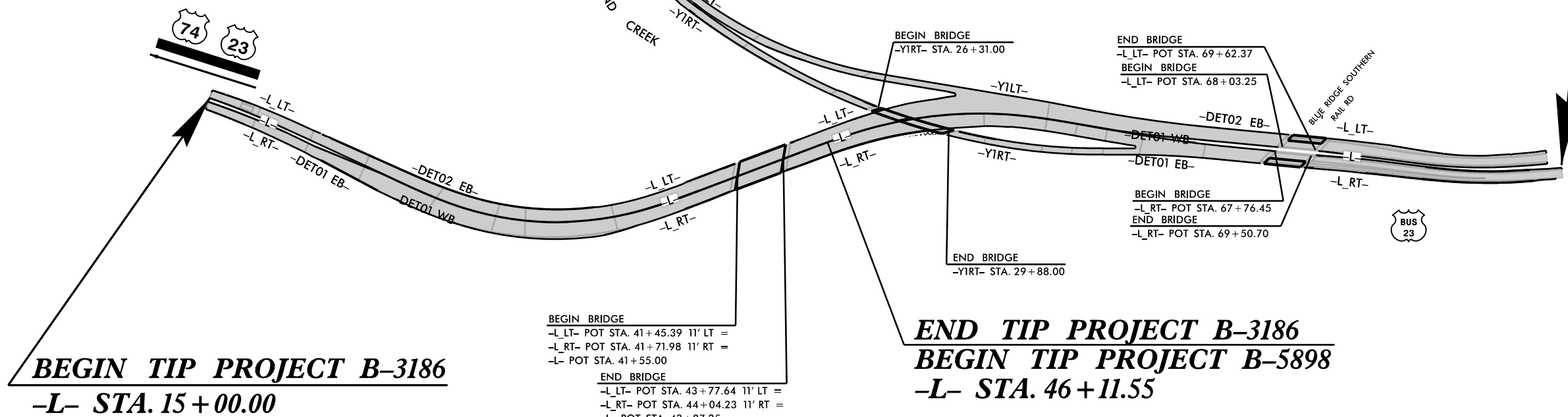
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3186 / B-5898	3	42
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
38332.1.FS.1	BRNHP-0023(32)	P.E.	
48030.1.FS.1	BRSTP-0019(49)	P.E.	
38332.2.1	N/A	RW/UTILITY	
48030.2.1	N/A	RW/UTILITY	



BEGIN CONSTRUCTION
-YIRT- STA. 12 + 00.00

BEGIN CONSTRUCTION
-YILT- STA. 11 + 99.83

END TIP PROJECT B-5898
-L- STA. 81 + 35.95

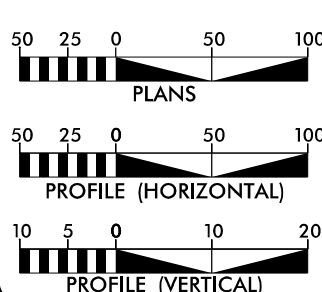


CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II

THIS IS A CONTROLLED-ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2022 = 47,300
ADT 2042 = 59,400
K = 8 %
D = 55 %
T = 5 % *
V = 65 MPH
* TTST = 2% DUAL 3%
FUNC CLASS = FREEWAY
STATEWIDE TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-3186 / B-5898 = 1.181 miles
TOTAL STRUCTURES TIP PROJECT B-3186 / B-5898 = 0.076 miles
TOTAL LENGTH TIP PROJECT B-3186 / B-5898 = 1.2579 miles
(LENGTHS BASED ON L_RT ALIGNMENT)



Prepared In the Office of:
HDR Engineering, Inc. of the Carolinas
555 Fayetteville St, Suite 900 Raleigh, N.C. 27601
N.C.B.E.L.S. License Number: F-0116

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JULY 8, 2021

LETTING DATE:
JANUARY 18, 2022

PHILLIP E. ROGERS, PE
PROJECT ENGINEER

HENRY W. BARE
PROJECT DESIGN ENGINEER

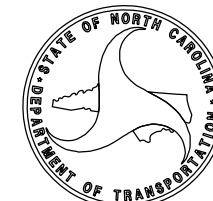
GARRETT HIGDON
NCDOT CONTACT

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

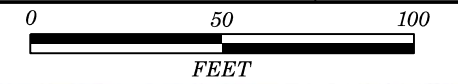


PLOT DRIVER: \$PLTDVRS\$
USER: \$USER\$
FILE: \$P\WVAULT\PATHDESC\$

DATE: \$DATE\$

PENTABLE: \$PENTBL\$
TIME: \$TIME\$

CONTRACT:



C-25 -L- 15+13 WB OSS	
5.5 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.25 in
STBC	16.75 in

C-26 -L- 20+09 WB OSS	
1.2 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.50 in
STBC	17.50 in
C-28 -L- 20+09 WB ISS	
2.4 FT LT FY	
PAVEMENT SECTION	
ASPHALT	5.75 in
STBC	16.25 in



C-1 -L- 15+32 EB OSS	
5.0 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.25 in
STBC	18.75 in

-L- CS Sta. 17+47.64



C-29 -L- 25+50 WB OSS	
5.7 FT RT FW	
PAVEMENT SECTION	
ASPHALT	4.75 in
STBC	17.25 in
C-30 -L- 25+50 WB ISS	
1.8 FT LT FY	
PAVEMENT SECTION	
ASPHALT	6.25 in
STBC	16.75 in

C-32 -L- 30+33 WB ISL	
5.5 FT RT FY	
PAVEMENT SECTION	
ASPHALT	8.75 in
STBC	12.25 in
C-33 -L- 30+33 WB ISS	
2.0 FT LT FY	
PAVEMENT SECTION	
ASPHALT	7.00 in
STBC	13.00 in

-L- SC Sta. 27+88.34

25+00

30+00

- C-29 WB OSS
- C-30 WB OSS
- C-5 EB ISS
- C-6 EB OSS

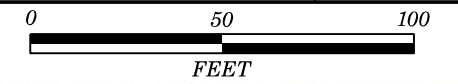
- C-32 WB ISL
- C-33 WB ISS

- C-7 EB ISS
- C-8 EB ISL

C-5 -L- 25+35 EB ISS	
1.4 FT LT FY	
PAVEMENT SECTION	
ASPHALT	4.00 in
STBC	18.00 in
C-6 -L- 25+35 EB OSS	
5.4 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.00 in
STBC	19.00 in

C-7 -L- 29+86 EB ISS	
1.3 FT LT FY	
PAVEMENT SECTION	
ASPHALT	5.50 in
STBC	14.50 in
C-8 -L- 29+86 EB ISL	
8.0 FT RT FY	
PAVEMENT SECTION	
ASPHALT	9.75 in
STBC	12.75 in

-L- TS Sta. 24+52.34



INC GRID
NAD 83 NA 2011

-L- CS Sta. 36+26.50

-L- ST Sta. 39+62.50

C-34 -L- 35+34 WB OSS	
6.0 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.00 in
STBC	17.00 in
C-35 -L- 35+35 WB ISS	
2.3 FT LT FY	
PAVEMENT SECTION	
ASPHALT	8.75 in
STBC	14.25 in

BULK-2

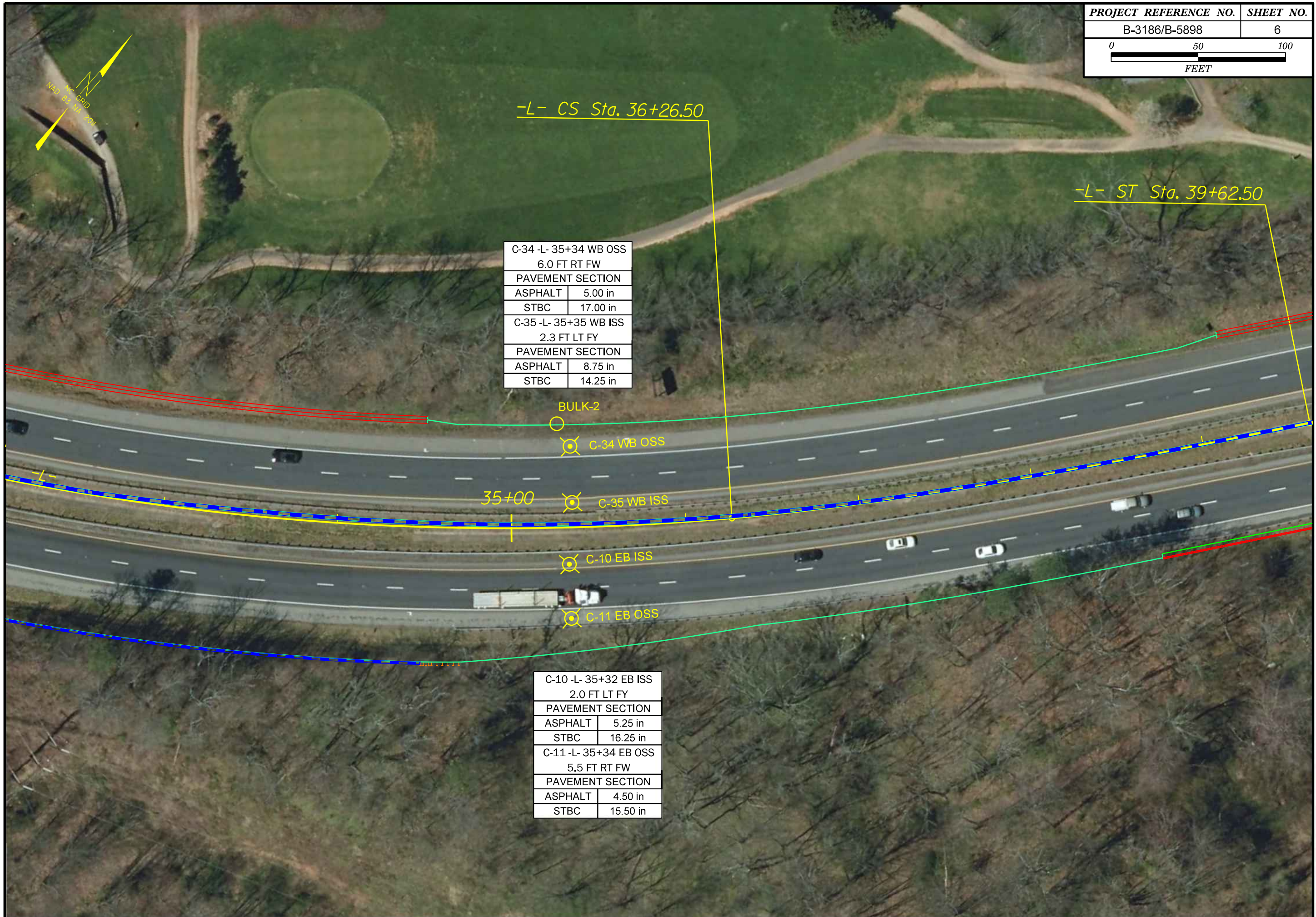
C-34 WB OSS

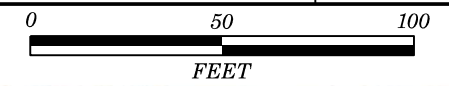
35+00 C-35 WB ISS

C-10 EB ISS

C-11 EB OSS

C-10 -L- 35+32 EB ISS	
2.0 FT LT FY	
PAVEMENT SECTION	
ASPHALT	5.25 in
STBC	16.25 in
C-11 -L- 35+34 EB OSS	
5.5 FT RT FW	
PAVEMENT SECTION	
ASPHALT	4.50 in
STBC	15.50 in





-YILT- POT Sta. 33+16.69 =
-L_LT- POS Sta. 56+11.71 36' LT

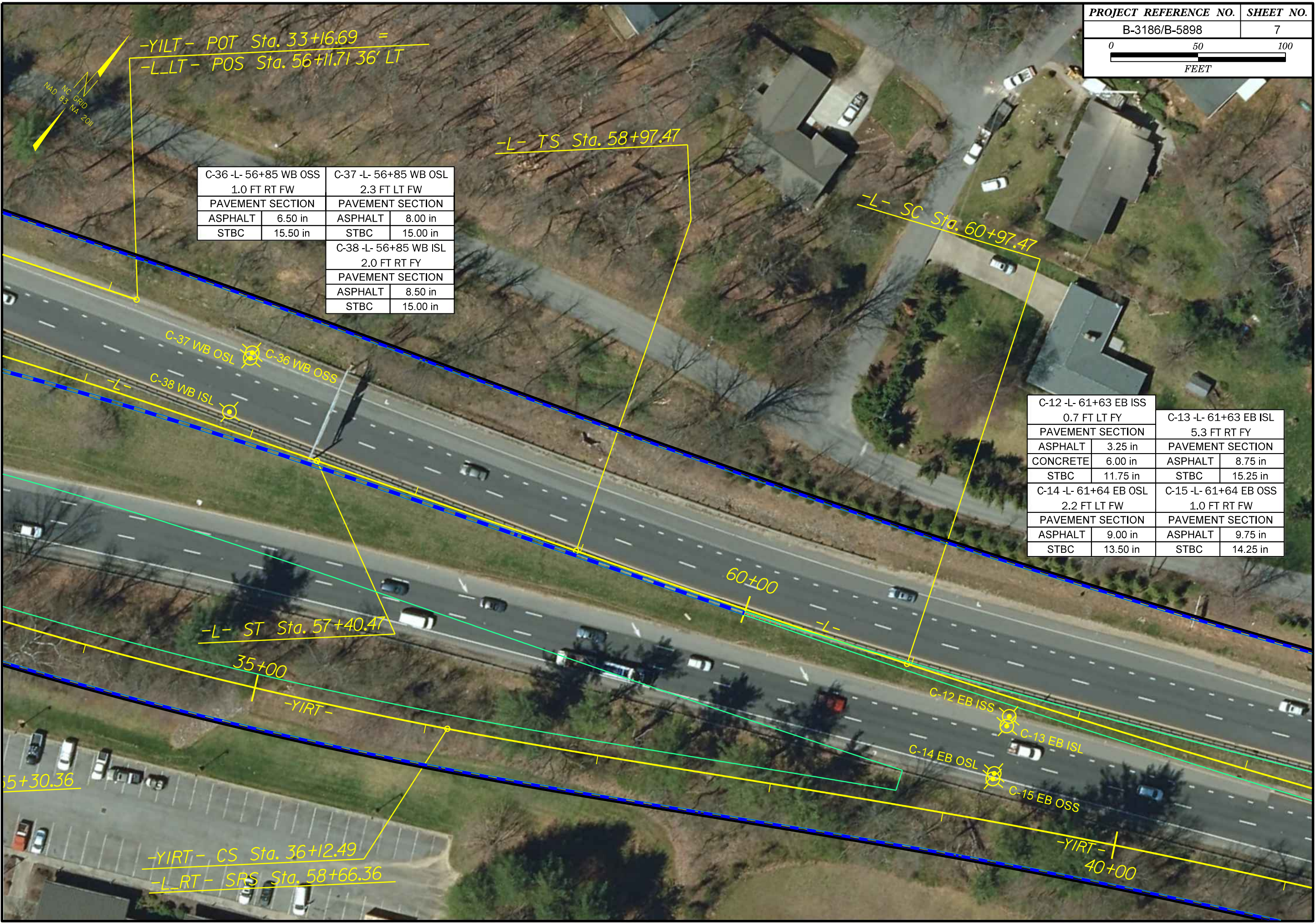
NC GRID
NAD 83 NA 2011

-L- TS Sta. 58+97.47

-L- SC Sta. 60+97.47

C-36 -L- 56+85 WB OSS 1.0 FT RT FW		C-37 -L- 56+85 WB OSL 2.3 FT LT FW	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	6.50 in	ASPHALT	8.00 in
STBC	15.50 in	STBC	15.00 in
		C-38 -L- 56+85 WB ISL 2.0 FT RT FY	
		PAVEMENT SECTION	
ASPHALT	8.50 in		
STBC	15.00 in		

C-12 -L- 61+63 EB ISS 0.7 FT LT FY		C-13 -L- 61+63 EB ISL 5.3 FT RT FY	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	3.25 in	ASPHALT	8.75 in
CONCRETE	6.00 in	STBC	15.25 in
STBC	11.75 in		
C-14 -L- 61+64 EB OSL 2.2 FT LT FW		C-15 -L- 61+64 EB OSS 1.0 FT RT FW	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	9.00 in	ASPHALT	9.75 in
STBC	13.50 in	STBC	14.25 in



-L- ST Sta. 57+40.47

35+00

-YIRT-

60+00

C-12 EB ISS

C-13 EB ISL

C-14 EB OSL

C-15 EB OSS

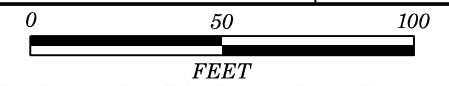
5+30.36

-YIRT- CS Sta. 36+12.49

-L_RT- SRS Sta. 58+66.36

-YIRT-

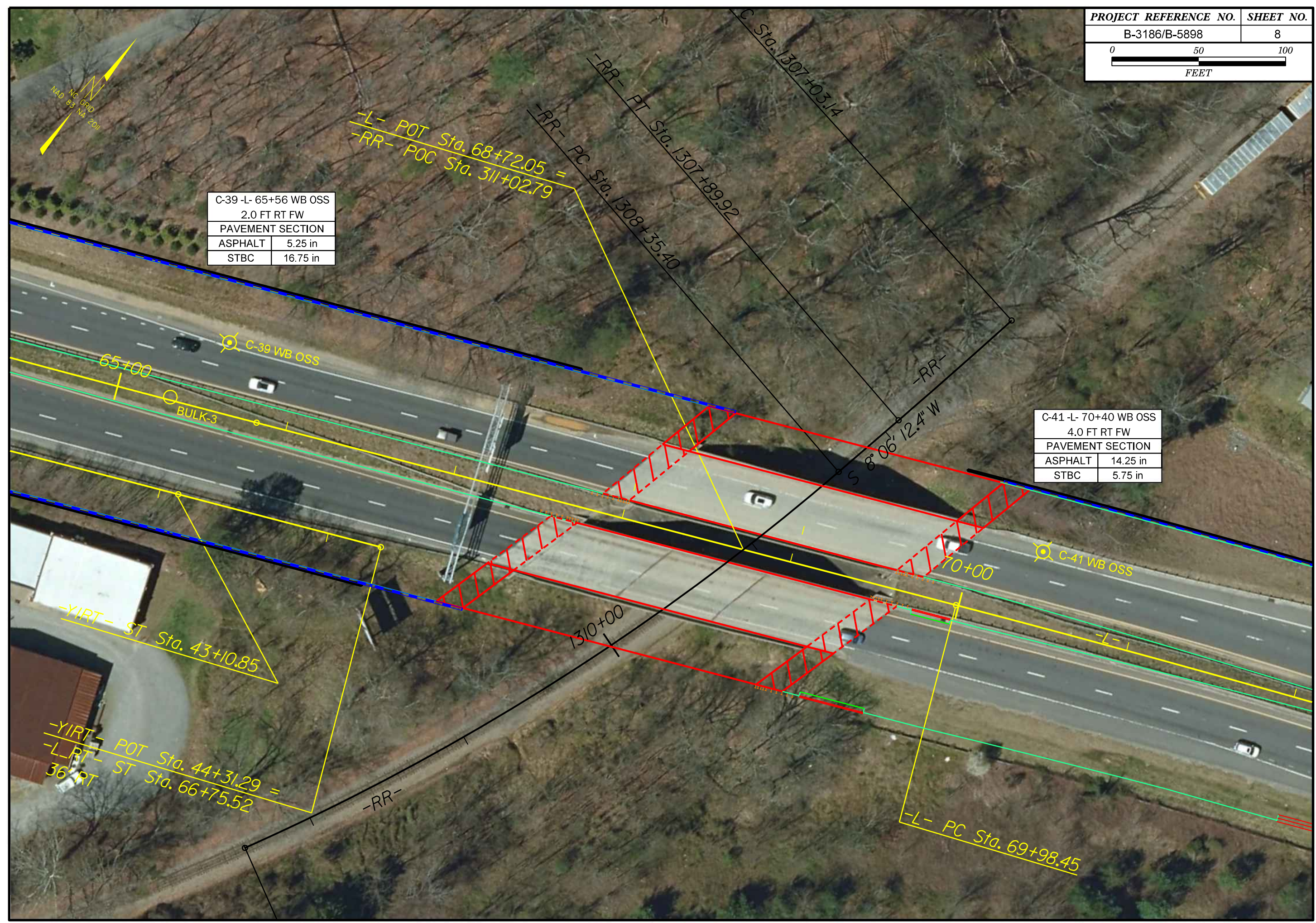
40+00



NC GRID
NAD 83 NA 2011

C-39 -L- 65+56 WB OSS	
2.0 FT RT FW	
PAVEMENT SECTION	
ASPHALT	5.25 in
STBC	16.75 in

C-41 -L- 70+40 WB OSS	
4.0 FT RT FW	
PAVEMENT SECTION	
ASPHALT	14.25 in
STBC	5.75 in



-L- POT Sta. 68+72.05 =
-RR- POC Sta. 311+02.79

-RR- PC Sta. 1308+35.40
-RR- PT Sta. 1307+89.92
C Sta. 1307+03.14

S 8° 06' 12.4" W

65+00

BULK-3

C-39 WB OSS

70+00

C-41 WB OSS

-YIRT- ST Sta. 43+10.85

1310+00

-YIRT- POT Sta. 44+31.29 =
-L- RT ST Sta. 66+75.52
36' RT

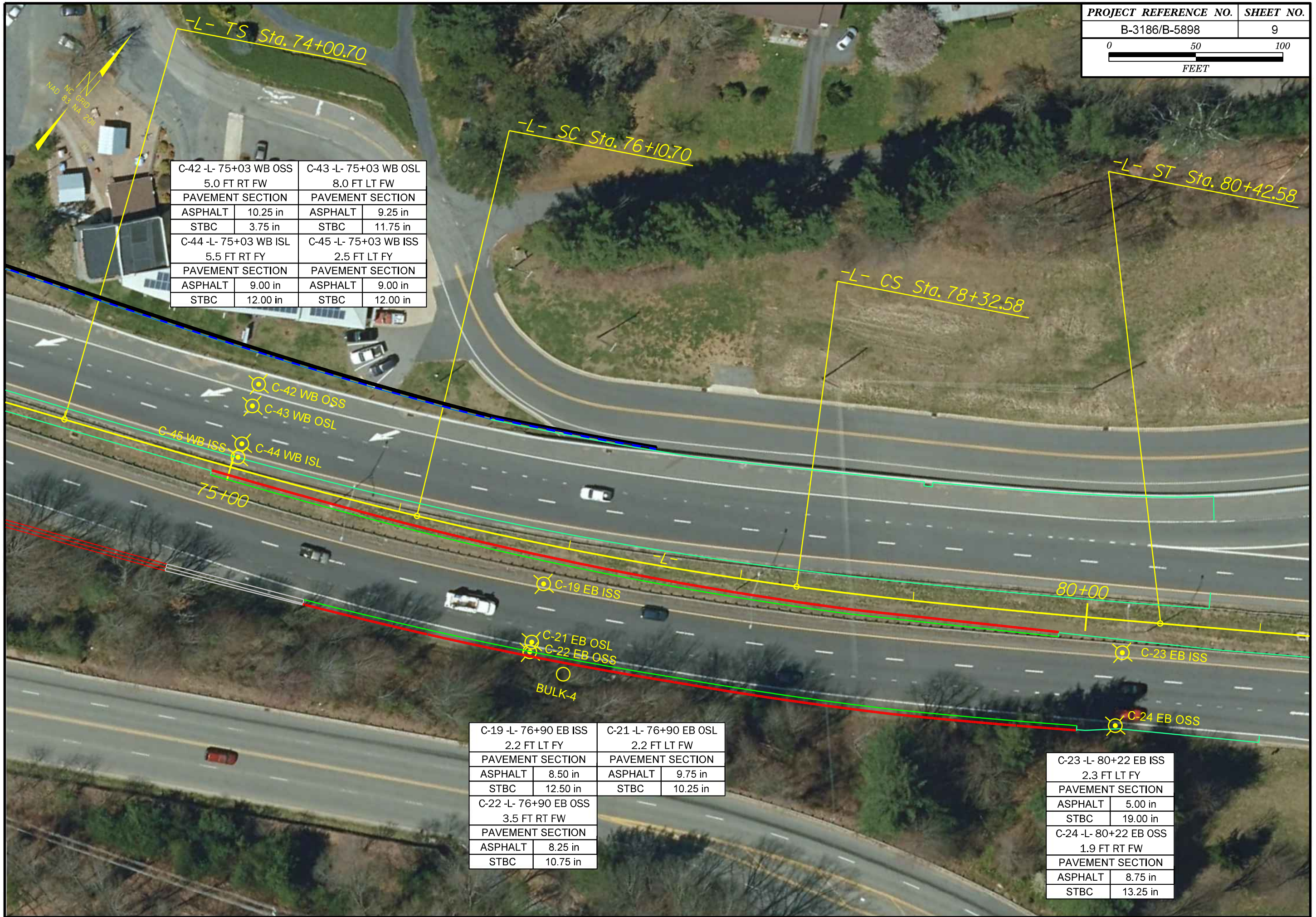
-RR-

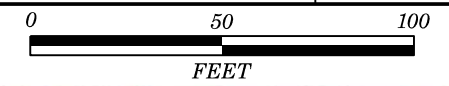
-L- PC Sta. 69+98.45

C-42 -L- 75+03 WB OSS 5.0 FT RT FW		C-43 -L- 75+03 WB OSL 8.0 FT LT FW	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	10.25 in	ASPHALT	9.25 in
STBC	3.75 in	STBC	11.75 in
C-44 -L- 75+03 WB ISL 5.5 FT RT FY		C-45 -L- 75+03 WB ISS 2.5 FT LT FY	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	9.00 in	ASPHALT	9.00 in
STBC	12.00 in	STBC	12.00 in

C-19 -L- 76+90 EB ISS 2.2 FT LT FY		C-21 -L- 76+90 EB OSL 2.2 FT LT FW	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	8.50 in	ASPHALT	9.75 in
STBC	12.50 in	STBC	10.25 in
C-22 -L- 76+90 EB OSS 3.5 FT RT FW			
PAVEMENT SECTION			
ASPHALT	8.25 in		
STBC	10.75 in		

C-23 -L- 80+22 EB ISS 2.3 FT LT FY	
PAVEMENT SECTION	
ASPHALT	5.00 in
STBC	19.00 in
C-24 -L- 80+22 EB OSS 1.9 FT RT FW	
PAVEMENT SECTION	
ASPHALT	8.75 in
STBC	13.25 in





-YILT- POT Sta. 10+00.00

-YILT- TS Sta. 12+06.91

-YILT- SC Sta. 14+06.91

C-47 -Y1LT- 13+66 US 19		C-48 -Y1LT- 13+66 US 19	
WB RTL, 6.0 FT RT C&G		WB OSL, 15.5 FT LT C&G	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	7.25 in	ASPHALT	8.50 in
STBC	-	STBC	13.50 in

C-49 -Y1LT- 13+67 US 19	
WB ISL, 2.8 FT RT C&G FACE	
PAVEMENT SECTION	
ASPHALT	11.50 in
STBC	10.50 in

10+00

10+00

C-47 US 19 WB RTL
C-48 US 19 WB OSL
C-49 US 19 WB ISL

-YILT-

15+00

15+00

C-50 US 19 EB ISL

-Y1RT-

C-51 US 19 EB OSL
C-52 US 19 EB OSS

-Y1RT- TS Sta. 11+12.00

-Y1RT- POT Sta. 10+00.00

C-50 -Y1RT- 13+67 US 19		C-51 -Y1RT- 14+30 US 19	
EB ISL, 1.0 FT RT FY		EB OSL, 1.5 FT LT FW	
PAVEMENT SECTION		PAVEMENT SECTION	
ASPHALT	12.50 in	ASPHALT	7.00 in
STBC	-	STBC	14.00 in

C-52 -Y1RT- 14+30 US 19	
EB OSS, 1.8 FT RT FW	
PAVEMENT SECTION	
ASPHALT	4.00 in
STBC	19.00 in

-Y1RT- SC Sta. 13+12.00

PAVEMENT INVESTIGATION DATA SHEET

Project:	38332.1.FS1
TIP:	B-3186/B-5898

Route:	US 23-74 Bridge 155 and 158 over Richland Creek on US 23-74 Northbound Lane
County:	Haywood County

Date Performed:	8/4 to 8/6/2021 (2 nights)
Field Personnel:	M. Brewer, D. Underwood, R. Kral

Test Location	Cut/Fill (Est. of Amount) (ft)	Width (ft)		Offset Distance (See Notes)	(ft)	(in)	Thickness (in)					Pavement Layering	Subgrade					Asphalt Notes	GPS Coordinates	
		Lane	Shoulder				Gross to Top of Soil	Asphalt	STBC	Stabilized Subgrade	Concrete		Description (Depth - ft)	Soil Sample Number	AASHTO Classification	Soil Moisture	Boring Depth (ft)		Northing	Easting
C-1 -L- 15+32 EB OSS	7.0 Fill	11.0	9.5 PS Asphalt Curb	5.0 RT FW	Cr	24.00	5.25	18.75	-	-	Asphalt STBC Soil Subgrade	2.0-5.0: RE - Brown, Silty Fine to Coarse SAND	S-1	A-2-4	M	5	Low to Moderate Severity Transverse Cracking, No Observable Distress in OSL	665,229	816,598	
C-5 -L- 25+35 EB ISS	10.0 Fill	11.5	3.0 PS	1.4 LT FY	Cr	22.00	4.00	18.00	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: RE - Brown-Red, Fine to Coarse Sandy SILT	S-5	A-4	M	5	No Observable Distress	665,399	817,583	
C-6 -L- 25+35 EB OSS	15.0 Fill	11.0	9.5 PS	5.4 RT FW	Cr	24.00	5.00	19.00	-	-	Asphalt STBC Soil Subgrade	2.0-5.0: RE - Brown-Red, Silty CLAY, Slightly Plastic *STBC - Silty, Fine to Coarse Sandy GRAVEL (A-1-a)	S-6 STBC-1	A-7-5 A-1-a	M	5	No Observable Distress	665,368	817,587	
C-7 -L- 29+86 EB ISS	20.0 Cut	11.5	2.5 PS	1.3 LT FY	S	20.00	5.50	14.50	-	-	Asphalt STBC Soil Subgrade	1.7-5.0: Residual - Brown-Tan, Fine Sandy Micaceous SILT	Ref-8	A-4	M	5	No Observable Distress 3-inch Bottom Up Crack in Core	665,502	818,025	
C-8 -L- 29+86 EB ISL	20.0 Cut	11.5	2.5 PS	8.0 RT FY	S	22.50	9.75	12.75	-	-	Asphalt STBC Soil Subgrade	1.9-5.0: Residual - Brown-Tan, Fine to Coarse Sandy Micaceous SILT	S-8	A-4	M	5	Low Severity Transverse Cracking and Logitudinal Cracking (OSWP & ISWP)	665,493	818,029	
C-10 -L- 35+32 EB ISS	4.0 Cut	11.0	4.0 PS	2.0 LT FY	S	21.50	5.25	16.25	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Orange, Fine Sandy Micaceous SILT	Ref-11A	A-4	M	5	Low Severity Transverse Cracking and Logitudinal Cracking (OSWP & ISWP) Full Depth Bottom Up Crack in Core	665,793	818,493	
C-11 -L- 35+34 EB OSS	3.5 Fill	11.0	9.5 PS	5.5 RT FW	S	20.00	4.50	15.50	-	-	Asphalt STBC Soil Subgrade	1.7-3.5: RE - Red, Silty CLAY, Moderately Plastic 3.5-5.0: Residual - Brown-Red, Fine to Coarse Sandy Micaceous SILT	S-11 S-11A	A-7-6 A-4	M	5	Low to Moderate Severity Transverse Cracking Full Depth Bottom Up Crack in Core	665,771	818,515	
C-12 -L- 61+63 EB ISS	1.9 Fill	11.5	1.5 PS	0.7 LT FY	Cr	21.00	3.25	11.75	-	6.00	Asphalt Concrete STBC / SG	1.8-5.0: Residual - Red-Tan, Silty Fine to Coarse SAND Possibly 2-3 inches of contaminated drainage sand beneath concrete	S-12	A-2-4	M	5	Low Severity Transverse Cracking Full Depth Bottom Up Crack in Asphalt Core	667,474	820,429	
C-13 -L- 61+63 EB ISL	2.0 Fill	11.5	1.5 PS	5.3 RT FY	Cr	24.00	8.75	15.25	-	-	Asphalt STBC Soil Subgrade	2.0-5.0: Residual - Brown, Fine to Coarse Sandy Micaceous SILT	S-13	A-4	M	5	Low Severity Transverse Cracking 7-inch Top Down Crack in Core	667,469	820,431	
C-14 -L- 61+64 EB OSL	4.0 Fill	11.0	4.5 PS	2.2 LT FW	Cr	22.50	9.00	13.50	-	-	Asphalt STBC Soil Subgrade	1.9-4.0: RE - Red-Brown, Fine Sandy SILT 4.0-5.0: Residual - Brown, Fine to Coarse Sandy Micaceous SILT	Ref-15 Ref-11A	A-4 A-4	M	5	No Observable Distress	667,444	820,444	
C-15 -L- 61+64 EB OSS	4.0 Fill	11.0	4.5 PS	1.0 RT FW	Cr	24.00	9.75	14.25	-	-	Asphalt STBC Soil Subgrade	2.0-4.0: RE - Brown, Fine to Coarse Sandy Micaceous SILT 4.0-5.0: Residual - Brown, Fine to Coarse Sandy Micaceous SILT	S-15 Ref-11A	A-4 A-4	M	5	No Observable Distress	667,441	820,445	
C-19 -L- 76+90 EB ISS	20.0 Cut	11.0	4.0 PS	2.2 LT FY	S	21.00	8.50	12.50	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Tan, Silty, Gravelly Fine to Coarse SAND	S-19	A-1-b	M	5	No Observable Distress	668,133	821,809	
C-21 -L- 76+90 EB OSL	20.0 Cut	10.0	6.0 PS	2.2 LT FW	S	20.00	9.75	10.25	-	-	Asphalt STBC Soil Subgrade	1.7-2.1: Residual - Brown, Fine Sandy Micaceous SILT 2.0-3.0: Weathered Rock - GNEISS Auger Refusal at 3.0 ft	Ref-22	A-4 WR	M	3 A/R	No Observable Distress	668,103	821,825	
C-22 -L- 76+90 EB OSS	20.0 Cut	10.0	6.0 PS	3.5 RT FW	S	19.00	8.25	10.75	-	-	Asphalt STBC Soil Subgrade	1.6-2.3: Residual - Brown, Fine to Coarse Sandy Micaceous SILT 2.3-3.9: Weathered Rock - GNEISS Auger Refusal at 3.9 ft	S-22	A-4 WR	M	3.9 A/R	No Observable Distress	668,098	821,828	
C-23 -L- 80+22 EB ISS	2.0 Fill	11.5	4.0 PS	2.3 LT FY	S	24.00	5.00	19.00	-	-	Asphalt STBC Soil Subgrade	2.0-5.0: Residual - Brown, Fine Sandy Micaceous SILT	Ref-22	A-4	M	5	No Observable Distress	668,322	822,085	

Notes:
Offset Distance: Left and Right Relative to the Direction of Travel

Prepared by: DMB
Reviewed by: REK

PAVEMENT INVESTIGATION DATA SHEET

Project:	38332.1.FS1
TIP:	B-3186/B-5898

Route:	US 23-74 Bridge 155 and 158 over Richland Creek on US 23-74 Northbound Lane
County:	Haywood County

Date Performed:	8/4 to 8/6/2021 (2 nights)
Field Personnel:	M. Brewer, D. Underwood, R. Kral

Test Location	Cut/Fill (Est. of Amount) (ft)	Width (ft)		(ft)	(in)	Thickness (in)					Pavement Layering	Subgrade					Asphalt Notes	GPS Coordinates	
		Lane	Shoulder			Offset Distance (See Notes)	Crown "C" or Super "S"	Gross to Top of Soil	Asphalt	STBC		Stabilized Subgrade	Concrete	Description (Depth - ft)	Soil Sample Number	AASHTO Classification		Soil Moisture	Boring Depth (ft)
C-24 -L- 80+22 EB OSS	8.0 Cut	12.0	5.0 PS	1.9 RT FW	Cr	22.00	8.75	13.25	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown, Micaceous, Silty Fine to Coarse SAND	S-24	A-2-4	M	5	No Observable Distress	668,288	822,109
C-25 -L- 15+13 WB OSS	20.0 Fill	12.0	11.0 PS Asphalt Curb	5.5 RT FW	Cr	22.00	5.25	16.75	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: RE - Brown-Red, Silty CLAY, Slightly Plastic	S-25	A-7-5	M	5	Low Severity Transverse Cracking, Polished Aggregate	665,309	816,560
C-26 -L- 20+09 WB OSS	20.0 Cut	12.0	11.0 PS	1.2 RT FW	Cr	23.00	5.50	17.50	-	-	Asphalt STBC Soil Subgrade	1.9-2.5: Residual - Brown-Tan, Fine Sandy Micaceous SILT Auger Refusal at 2.5 ft	Ref-28	A-4	M	2.5 A/R	Low Severity Transverse Cracking	665,394	817,054
C-28 -L- 20+09 WB ISS	20.0 Cut	11.5	3.5 PS	2.4 LT FY	Cr	22.00	5.75	16.25	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Orange, Fine to Coarse Sandy Micaceous SILT	S-28	A-4	M	5	Low Severity Transverse Cracking	665,367	817,057
C-29 -L- 25+50 WB OSS	25.0 Cut	12.0	10.5 PS	5.7 RT FW	Cr	22.00	4.75	17.25	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Red-Orange-Brown, Silty CLAY, Slightly Plastic	S-29	A-7-5	M	5	Low to Moderate Severity Transverse Cracking, Polished Aggregate Full Depth Bottom Up Crack in Core	665,467	817,589
C-30 -L- 25+50 WB ISS	10.0 Cut	12.0	3.5 PS	1.8 LT FY	Cr	23.00	6.25	16.75	-	-	Asphalt STBC Soil Subgrade	1.9-5.0: Residual - Brown-Orange, Silty CLAY, Slightly Plastic	S-30	A-7-5	M	5	Low to Moderate Severity Transverse Cracking Full Depth Bottom Up Crack in Core	665,436	817,593
C-32 -L- 30+33 WB ISL	25.0 Cut	11.5	4.0 PS	5.5 RT FY	S	21.00	8.75	12.25	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Tan-Brown, Fine to Coarse Sandy Micaceous SILT	S-32	A-4	M	5	Low Severity Longitudinal Cracking (ISWP)	665,559	818,053
C-33 -L- 30+33 WB ISS	25.0 Cut	11.5	4.0 PS	2.0 LT FY	S	20.00	7.00	13.00	-	-	Asphalt STBC Soil Subgrade	1.7-5.0: Residual - Tan-Brown, Fine Sandy Micaceous SILT	Ref-32	A-4	M	5	Low Severity Longitudinal Cracking (ISWP)	665,552	818,056
C-34 -L- 35+34 WB OSS	10.0 Cut	11.5	10.5 PS	6.0 RT FW	S	22.00	5.00	17.00	-	-	Asphalt STBC Soil Subgrade	1.8-3.0: RE - Red, Fine to Coarse Sandy SILT 3.0-5.0: Residual - Brown-Red, Fine Sandy Micaceous SILT	S-34 Ref-32	A-4 A-4	M M	5 5	Low Severity Longitudinal (OSWP & ISWP) and Transverse Cracking at Shoulder / OSL	665,844	818,449
C-35 -L- 35+35 WB ISS	20.0 Cut	11.5	4.0 PS	2.3 LT FY	S	23.00	8.75	14.25	-	-	Asphalt STBC Soil Subgrade	1.9-3.5: RE - Red, Fine to Coarse Sandy SILT	S-35	A-4	M	5	No Observable Distress	665,821	818,471
C-36 -L- 56+85 WB OSS	20.0 Cut	12.0	4.0 PS	1.0 RT FW	Cr	22.00	6.50	15.50	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown, Micaceous, Fine to Coarse Sandy, Clayey SILT	Ref-37	A-5	M	5	Low Severity Transverse Cracking 2-inch Top Down Crack in Core	667,345	819,966
C-37 -L- 56+85 WB OSL	20.0 Cut	12.0	4.0 PS	2.3 LT FW	Cr	23.00	8.00	15.00	-	-	Asphalt STBC Soil Subgrade	1.9-5.0: Residual - Brown, Micaceous, Fine to Coarse Sandy, Clayey SILT	S-37	A-5	M	5	Low Severity Transverse Cracking	667,342	819,967
C-38 -L- 56+85 WB ISL	20.0 Cut	11.5	2.0 PS Asphalt over C&G	2.0 RT FY	Cr	23.50	8.50	15.00	-	-	Asphalt STBC Soil Subgrade	1.9-3.5: RE - Red, Silty CLAY, Slightly Plastic 3.5-5.0: Residual - Brown, Fine to Coarse Sandy, Clayey SILT	S-38 Ref-37	A-7-5 A-5	M M	5 5	Low Severity Transverse Cracking Full Depth Bottom Up Crack in Core	667,311	819,978
C-39 -L- 65+56 WB OSS	15.0 Cut	11.5	4.5 PS	2.0 RT FW	Cr	22.00	5.25	16.75	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-White, Fine Sandy Micaceous SILT	Ref-41	A-4	M	5	No Observable Distress	667,688	820,762
C-41 -L- 70+40 WB OSS	15.0 Fill	11.5	7.0 PS Concrete Exp Gutter	4.0 RT FW	Cr	20.00	14.25	5.75	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: RE - Brown-Red, Fine to Coarse Sandy Micaceous SILT	S-41	A-4	M	5	Low Severity Longitudinal Cracking (OSWP) and Transverse Cracking	667,906	821,194

Notes:
Offset Distance: Left and Right Relative to the Direction of Travel

Prepared by: DMB
Reviewed by: REK

PAVEMENT INVESTIGATION DATA SHEET

Project:	38332.1.FS1
TIP:	B-3186/B-5898

Route:	US 23-74 Bridge 155 and 158 over Richland Creek on US 23-74 Northbound Lane
County:	Haywood County

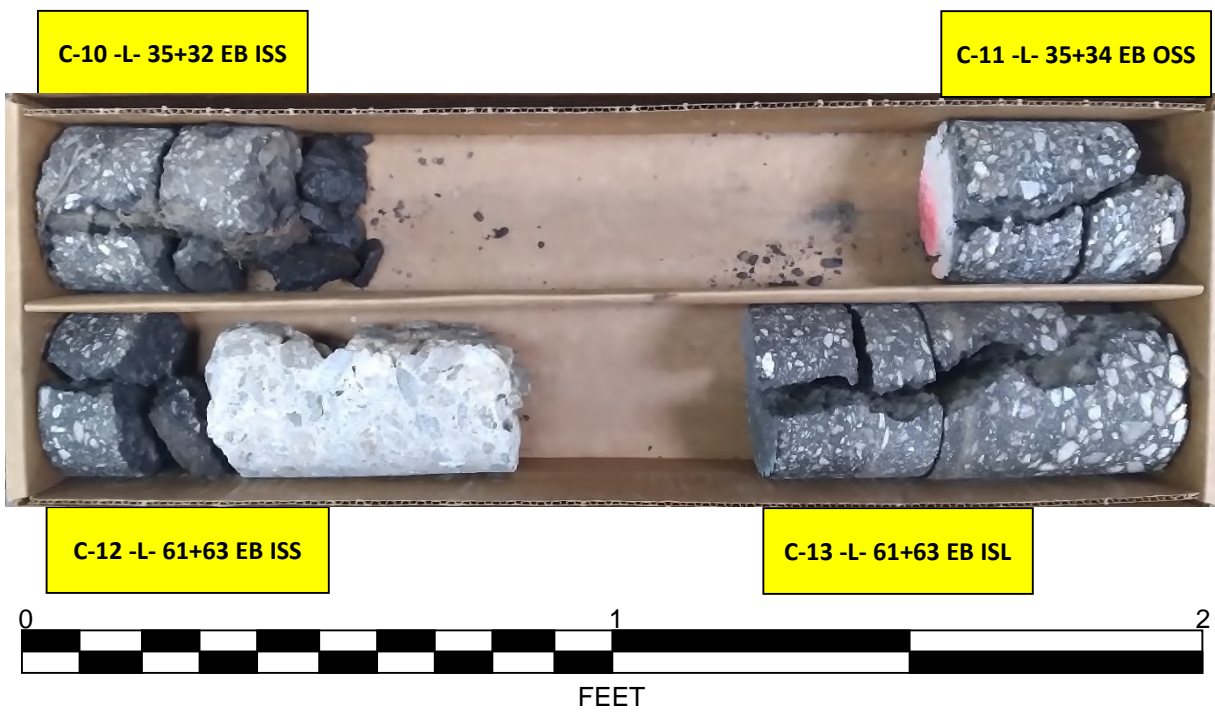
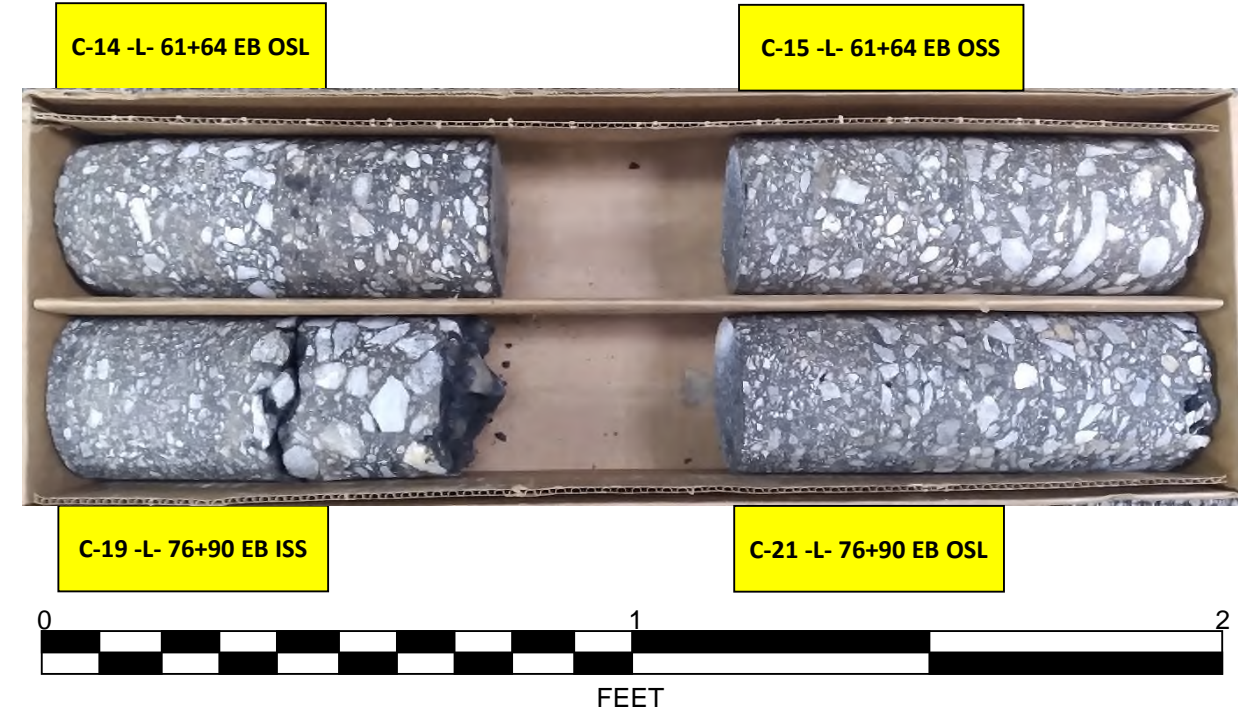
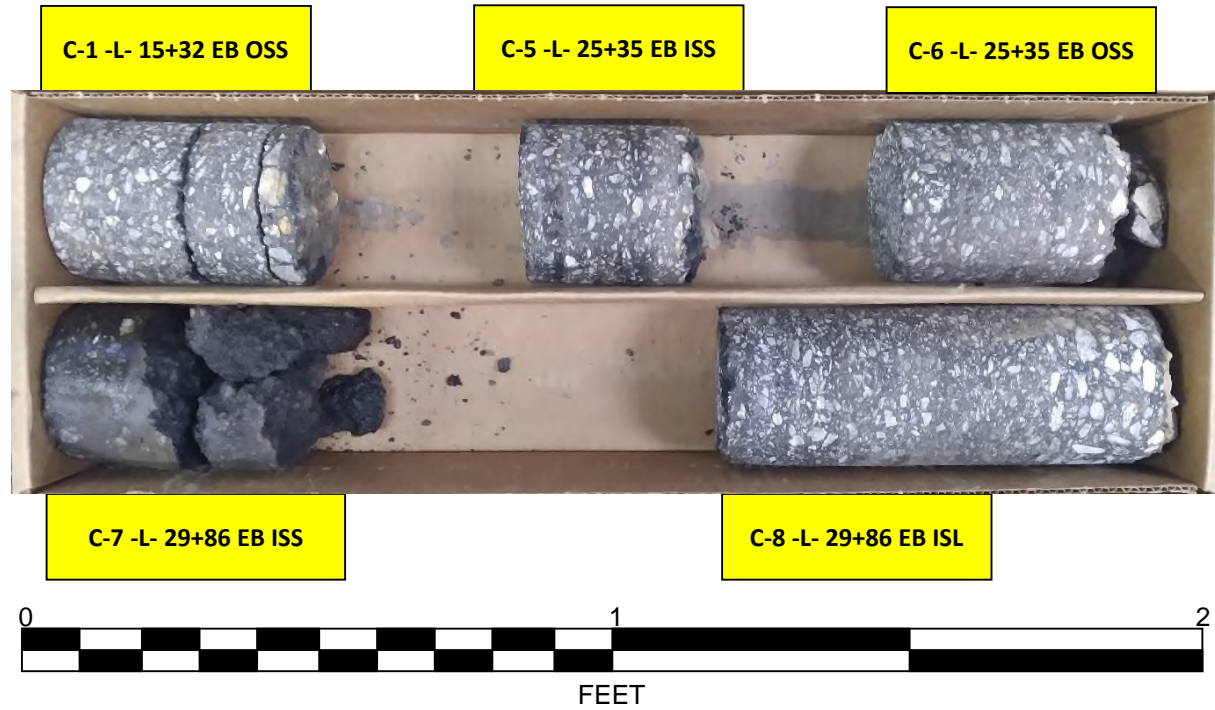
Date Performed:	8/4 to 8/6/2021 (2 nights)
Field Personnel:	M. Brewer, D. Underwood, R. Kral

Test Location	Cut/Fill (Est. of Amount) (ft)	Width (ft)		(ft)	(in)	Thickness (in)					Pavement Layering	Subgrade					Asphalt Notes	GPS Coordinates		
		Lane	Shoulder			Offset Distance (See Notes)	Crown "C" or Super "S"	Gross to Top of Soil	Asphalt	STBC		Stabilized Subgrade	Concrete	Description (Depth - ft)	Soil Sample Number	AASHTO Classification		Soil Moisture	Boring Depth (ft)	Northing
C-42 -L- 75+03 WB OSS	4.0 Cut	12.0	11.5 PS	5.0 RT FW	S	14.00	10.25	3.75	-	-	Asphalt STBC Soil Subgrade	1.2-5.0: Residual - Brown-Tan, Fine Sandy Micaceous SILT	Ref-43	A-4	M	5	Low Severity Longitudinal Cracking (OSWP) and Transverse Cracking	668,111	821,610	
C-43 -L- 75+03 WB OSL	4.0 Cut	12.0	11.5 PS	8.0 LT FW	S	21.00	9.25	11.75	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Tan, Fine to Coarse Sandy Micaceous SILT	S-43	A-4	M	5	Low Severity Longitudinal Cracking (OSWP) and Transverse Cracking Full Depth Bottom Up Crack in Core	668,099	821,615	
C-44 -L- 75+03 WB ISL	8.0 Cut	11.0	5.0 PS	5.5 RT FY	S	21.00	9.00	12.00	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Tan, Fine Sandy Micaceous SILT	S-44	A-4	M	5	Low Severity Transverse Cracking	668,079	821,625	
C-45 -L- 75+03 WB ISS	8.0 Cut	11.0	5.0 PS	2.5 LT FY	S	21.00	9.00	12.00	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Brown-Tan-Gray, Fine to Coarse Sandy Micaceous SILT	S-45	A-4	M	5	Low Severity Transverse Cracking Full Depth Bottom Up Crack in Core	668,072	821,628	
C-47 -Y1LT- 13+66 US 19 WB RTL	6.0 Cut	11.5	Concrete C&G	6.0 RT C&G	S	7.25	7.25	0.00	-	-	Asphalt Soil Subgrade	0.6-4.0: Residual - Brown-Tan, Fine Sandy SILT Auger Refusal at 4.0 ft	Ref-48	A-4	M	4 A/R	Low Severity Longitudinal Cracking (OSWP & ISWP) and Transverse Cracking, Polished Aggregate	666,937	818,029	
C-48 -Y1LT- 13+66 US 19 WB OSL	6.0 Cut	11.0	N/A	15.5 LT C&G (RT)	S	22.00	8.50	13.50	-	-	Asphalt STBC Soil Subgrade	1.8-5.0: Residual - Tan-Red, Fine to Coarse Sandy SILT	S-48	A-4	M	5	Polished Aggregate	666,928	818,027	
C-49 -Y1LT- 13+67 US 19 WB ISL	3.5 Fill	12.0	Asphalt over C&G	2.8 RT C&G Face	S	22.00	11.50	10.50	-	-	Asphalt STBC Soil Subgrade	1.8-3.5: RE - Gray-Tan, Silty CLAY, Moderately Plastic 3.5-5.0: Residual - Tan-Brown, Fine Sandy SILT	S-49 Ref-48	A-7-6 A-4	M	5	1/2-inch Rutting (ISWP), Low Severity Longitudinal Cracking (ISWP), Low to Moderate Severity Transverse Cracking, Polished Aggregate	666,910	818,024	
C-50 -Y1RT- 13+67 US 19 EB ISL	4.0 Fill	12.0	Asphalt over C&G	1.0 RT FY	S	12.50	12.50	0.00	-	-	Asphalt Soil Subgrade	1.0-4.0: RE - Red, Silty CLAY, Slightly Plastic 4.0-5.0: Residual - Tan-Brown, Fine Sandy SILT	S-50 Ref-48	A-7-6 A-4	M	5	Low to Moderate Severity Longitudinal Cracking (OSWP) and Transverse Cracking, Ravelling Polished Aggregate	666,896	818,021	
C-51 -Y1RT- 14+30 US 19 EB OSL	10.0 Fill	11.0	4.0 PS	1.5 LT FW	S	21.00	7.00	14.00	-	-	Asphalt STBC Soil Subgrade	1.8-4.0: RE - Brown, Fine Sandy Micaceous SILT 4.0-5.0: RE - Brown-Red, Silty CLAY, Slightly Plastic	Ref-52 Ref-50	A-4 A-7-5	M	5	Low to Moderate Severity Longitudinal Cracking (OSWP) and Transverse Cracking, Ravelling Polished Aggregate	666,865	818,080	
C-52 -Y1RT- 14+30 US 19 EB OSS	10.0 Fill	11.0	4.0 PS	1.8 RT FW	Cr	23.00	4.00	19.00	-	-	Asphalt STBC Soil Subgrade	1.9-5.0: RE - Brown, Fine to Coarse Sandy Micaceous SILT	S-52	A-4	M	5	Low to Moderate Severity Longitudinal Cracking (OSWP) and Transverse Cracking, Ravelling Polished Aggregate	666,862	818,079	

Notes:
Offset Distance: Left and Right Relative to the Direction of Travel

Prepared by: DMB
Reviewed by: REK

**B-3186/B-5898 - US 23-74 Bridge 155 & 158 over Richland Creek on US 23-74 Northbound Lane
(Combine with B-5898)
Pavement Core Photographs**

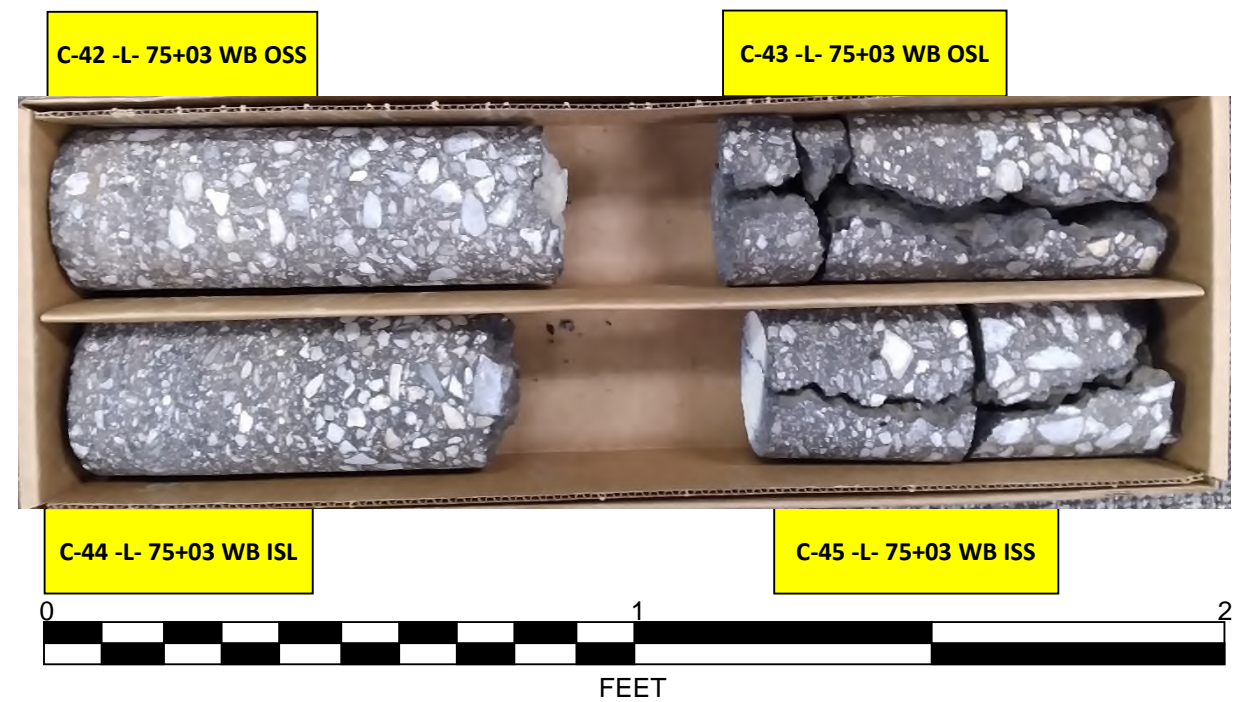
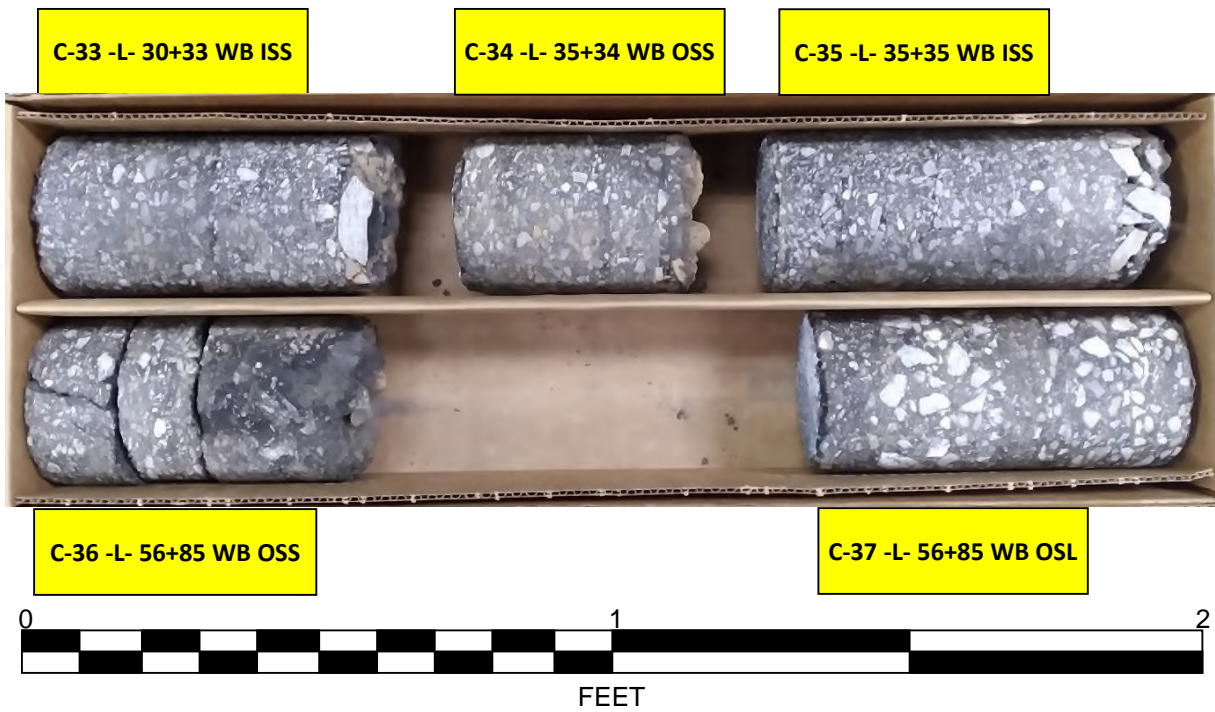
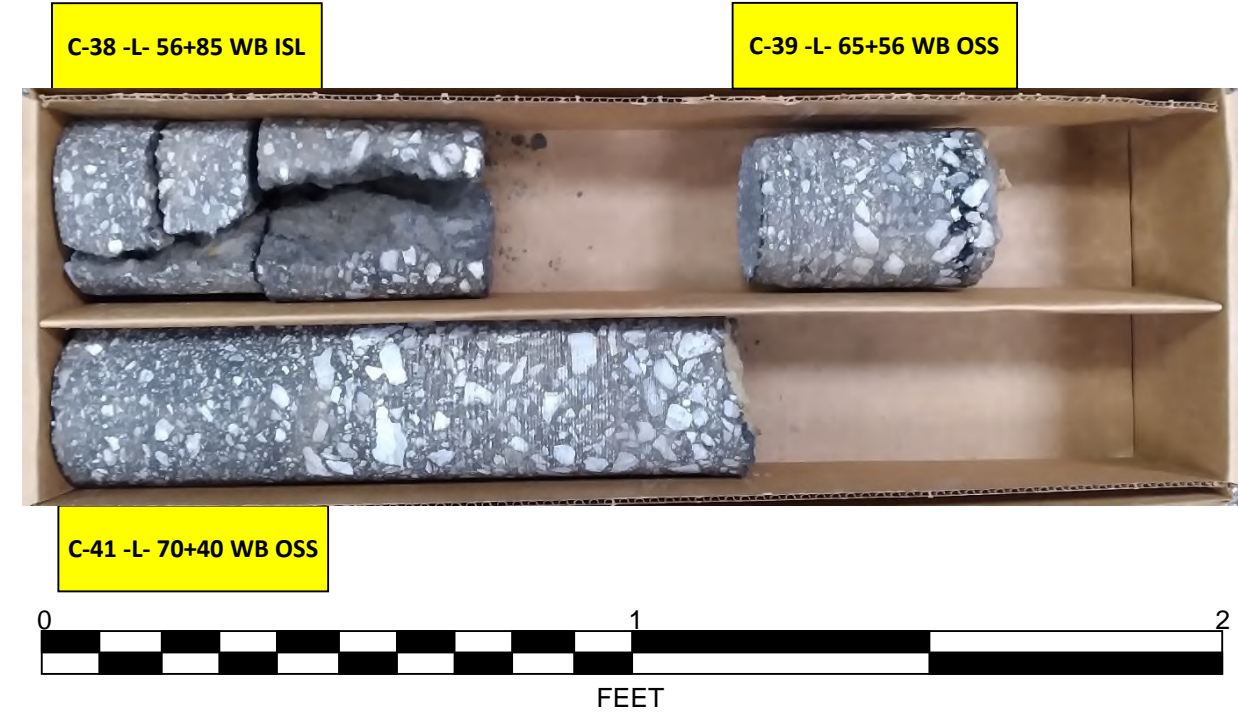
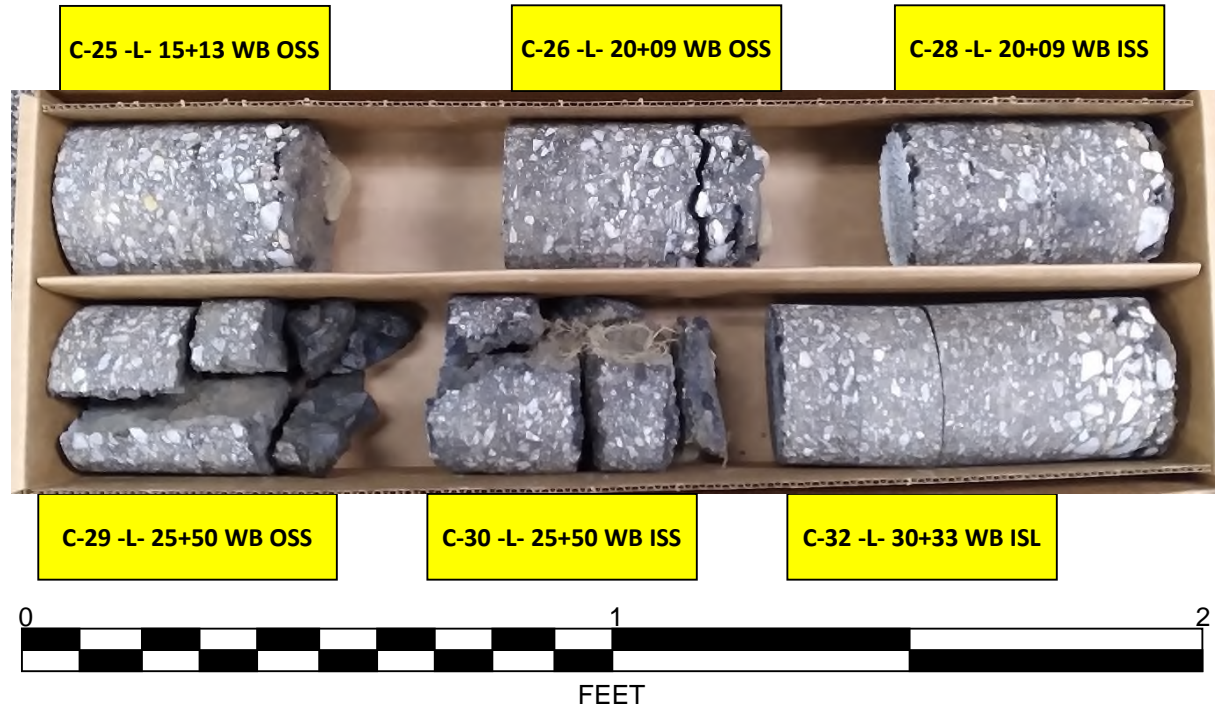




DUAL MASS DYNAMIC CONE PENETROMETER DATA SHEET. Includes metadata (WBS NO., PROJECT TIP I.D., ROUTE, COUNTY, FIELD PROFESSIONAL, FIELD CREW, Date Run) and a data table with columns for Test Location (Type, Test Interval, Datum, Cut/Fill) and Date Run (8/4 to 8/6/2021). Test locations include C-25, C-26, and various depth intervals (-L- 15+13, WB OSS, 5.5 FT RT FW).

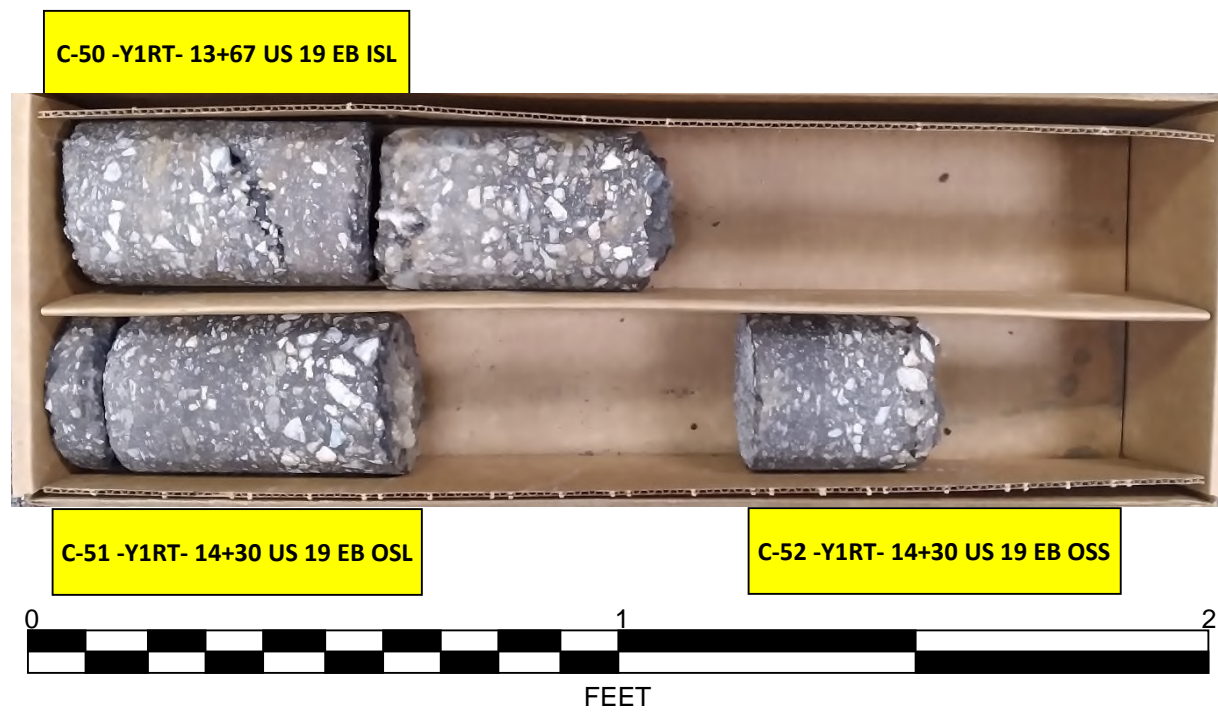
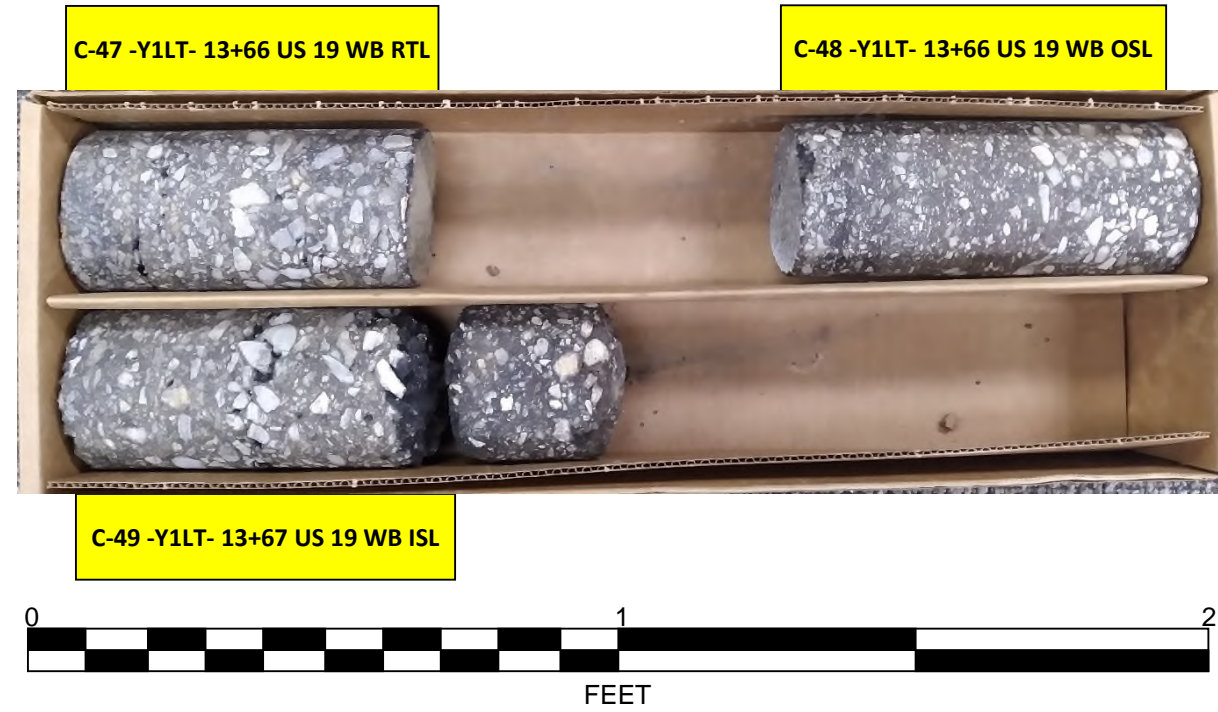
DUAL MASS DYNAMIC CONE PENETROMETER DATA SHEET. Includes metadata (WBS NO., PROJECT TIP I.D., ROUTE, COUNTY, FIELD PROFESSIONAL, FIELD CREW, Date Run) and a data table with columns for Test Location (Type, Test Interval, Datum, Cut/Fill) and Date Run (8/4 to 8/6/2021). Test locations include C-28, C-29, and various depth intervals (-L- 20+09, WB ISS, 2.4 FT LT FY, -L- 25+50, WB OSS, 5.7 FT RT FW).

**B-3186/B-5898 - US 23-74 Bridge 155 & 158 over Richland Creek on US 23-74 Northbound Lane
(Combine with B-5898)
Pavement Core Photographs**





**B-3186/B-5898 - US 23-74 Bridge 155 & 158 over Richland Creek on US 23-74 Northbound Lane
(Combine with B-5898)
Pavement Core Photographs**



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION

LABORATORY TEST RESULTS

REFERENCE: B-3186/B-5898

PROJECT: 38332

Prepared in the Office of:

F&ME CONSULTANTS, INC.
COLUMBIA, SOUTH CAROLINA
NCDOT LAB CERT. NO. 132-0212

SOIL TEST RESULTS

SAMPLE NO.	STATION	LOCATION	OFFSET*	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-1	-L- 15+32	EB OSS	5' RT FW	2.0 - 5.0'	A-2-4	34	NP	30.9	28.9	28.1	12.1	68.7	54.1	31.8	23.9	-
S-5	-L- 25+35	EB ISS	1.4' LT FY	1.8 - 5.0'	A-4(2)	38	5	19.5	25.5	30.8	24.2	87.7	75.9	54.0	23.5	-
S-6	-L- 25+35	EB OSS	5.4' RT FW	2.0 - 5.0'	A-7-5(9)	45	13	14.7	17.4	23.1	44.8	93.7	84.8	67.5	25.8	-
STBC-1	-L- 25+35	EB OSS	5.4' RT FW	0.4 - 2.0'	A-1-a	26	NP	43.5	30.3	20.1	6.0	41.9	28.4	13.3	5.2	-
S-8	-L- 29+86	EB ISL	8' RT FY	1.9 - 5.0'	A-4(0)	32	4	25.5	25.9	32.4	16.1	90.2	74.1	50.3	11.5	-
S-11	-L- 35+34	EB OSS	5.5' RT FW	1.7 - 3.5'	A-7-6(10)	47	22	20.9	17.7	19.0	42.4	86.4	73.9	56.3	25.4	-
S-11A	-L- 35+34	EB OSS	5.5' RT FW	3.5 - 5.0'	A-4(2)	37	7	24.1	25.6	24.1	26.2	95.7	80.2	53.8	19.8	-
S-12	-L- 61+63	EB ISS	0.7' LT FY	1.8 - 5.0'	A-2-4	35	NP	28.1	31.1	26.7	14.1	68.8	56.0	32.6	25.0	-
S-13	-L- 61+63	EB ISL	5.3' RT FY	2.0 - 5.0'	A-4(0)	30	NP	29.3	32.7	25.9	12.0	67.1	54.3	56.7	8.5	-
S-15	-L- 61+64	EB OSS	1' RT FW	2.0 - 4.0'	A-4(0)	31	4	22.3	29.1	28.5	20.1	82.2	69.6	46.3	15.7	-
S-19	-L- 76+90	EB ISS	2.2' LT FY	1.8 - 5.0'	A-1-b	23	NP	43.9	27.2	24.8	4.0	70.9	46.6	24.9	4.4	-
S-22	-L- 76+90	EB OSS	3.5' RT FW	1.6 - 2.3'	A-4(3)	40	10	17.5	30.2	32.1	20.2	80.4	71.9	47.8	16.9	-
S-24	-L- 80+22	EB OSS	1.9' RT FW	1.8 - 5.0'	A-2-4	39	NP	26.4	33.7	31.9	8.0	65.0	53.2	31.1	16.5	-
S-25	-L- 15+13	WB OSS	5.5' RT FW	1.8 - 5.0'	A-7-5(9)	50	15	17.7	25.5	38.7	18.1	94.9	84.2	60.4	30.4	-
S-28	-L- 20+09	WB ISS	2.4' LT FY	1.8 - 5.0'	A-4(5)	40	7	11.3	31.5	37.1	20.1	97.3	90.8	65.2	21.2	-
S-29	-L- 25+50	WB OSS	5.7' RT FW	1.8 - 5.0'	A-7-5(6)	50	12	17.0	32.0	32.9	18.1	94.5	84.4	56.1	28.5	-
S-30	-L- 25+50	WB ISS	1.8' LT FY	1.9 - 5.0'	A-7-5(15)	54	14	7.1	17.2	49.4	26.2	99.0	94.8	81.0	27.9	-
S-32	-L- 30+33	WB ISL	5.5' RT FY	1.8 - 5.0'	A-4(2)	37	8	20.6	30.7	32.6	16.1	90.9	78.2	52.4	7.4	-
S-34	-L- 35+34	WB OSS	6' RT FW	1.8 - 3.0'	A-4(3)	37	9	21.0	24.6	26.2	28.2	92.2	80.0	55.3	22.7	-
S-35	-L- 35+35	WB ISS	2.3' LT FY	1.9 - 3.5'	A-4(0)	39	NP	25.0	24.2	26.7	24.1	83.9	69.2	47.4	17.8	-
S-37	-L- 56+85	WB OSL	2.3' LT FW	1.9 - 5.0'	A-5(0)	43	NP	19.4	35.3	29.2	16.1	95.2	85.4	50.7	16.7	-
S-38	-L- 56+85	WB ISL	2' RT FY	1.9 - 3.5'	A-7-5(2)	46	13	29.1	28.6	24.2	18.1	82.7	66.7	39.9	16.9	-
S-41	-L- 70+40	WB OSS	4' RT FW	1.8 - 5.0'	A-4(2)	39	8	17.8	29.8	34.3	18.1	86.0	76.9	51.4	26.1	-
S-43	-L- 75+03	WB OSL	8' LT FW	1.8 - 5.0'	A-4(1)	35	9	31.4	30.7	25.9	12.1	93.9	73.8	41.6	17.0	-
S-44	-L- 75+03	WB ISL	5.5' RT FY	1.8 - 5.0'	A-4(0)	23	NP	27.6	41.5	24.9	6.0	99.3	85.4	37.9	8.2	-
S-45	-L- 75+03	WB ISS	2.5' LT FY	1.8 - 5.0'	A-4(5)	39	10	15.1	30.6	40.2	14.1	99.1	91.0	63.1	13.1	-
S-48	-Y1LT- 13+66	WB OSL	15.5' LT C&G (RT)	1.8 - 5.0'	A-4(1)	34	9	28.3	28.0	23.5	20.1	87.6	70.5	43.3	18.0	-
S-49	-Y1LT- 13+67	WB ISL	2.8' RT C&G FACE	1.8 - 3.5'	A-7-6(8)	44	17	18.7	23.6	23.4	34.3	93.6	81.7	59.3	25.1	-
S-50	-Y1RT- 13+67	EB ISL	1' RT FY	1.0 - 4.0'	A-7-6(7)	43	14	18.1	23.4	20.3	38.2	95.4	84.8	60.5	21.5	-
S-52	-Y1RT- 14+30	EB OSS	1.8' RT FW	1.9 - 5.0'	A-4(0)	40	5	26.7	26.7	26.4	20.1	81.5	66.6	42.5	22.5	-

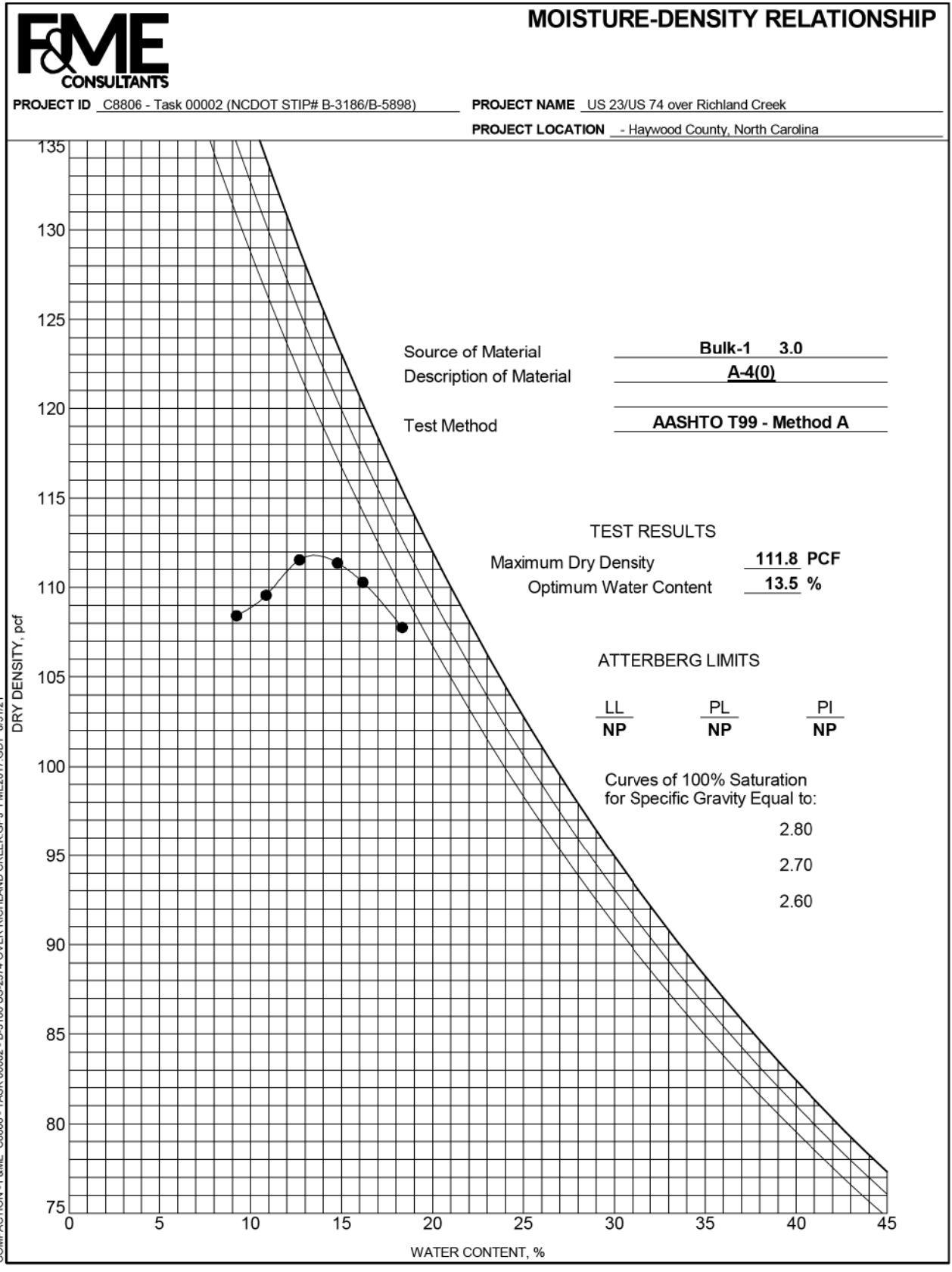
*FROM WHITE LINE (FW)

*FROM YELLOW LINE (FY)

SOIL TEST RESULTS

SAMPLE NO.	STATION	LOCATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
BULK-1	-L- 20+26	CUT SLOPE	68' RT	1.0 - 3.0'	A-4(0)	39	NP	36.3	23.6	31.9	8.1	96.7	69.6	43.4	12.9	-
BULK-2	-L- 35+27	CUT SLOPE	60' LT	1.0 - 3.0'	A-4(1)	37	NP	12.7	16.7	46.3	24.3	90.5	82.9	68.0	21.7	-
BULK-3	-L- 65+31	AT GRADE	2' LT	1.0 - 2.0'	A-2-4	30	4	32.3	27.5	20.7	19.6	77.2	61.7	64.3	11.4	-
BULK-4	-L- 77+10	CUT SLOPE	71' RT	1.0 - 3.0'	A-4(1)	33	7	22.0	28.7	31.7	17.6	83.2	72.1	45.7	8.9	-

*NP = NON-PLASTIC



REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

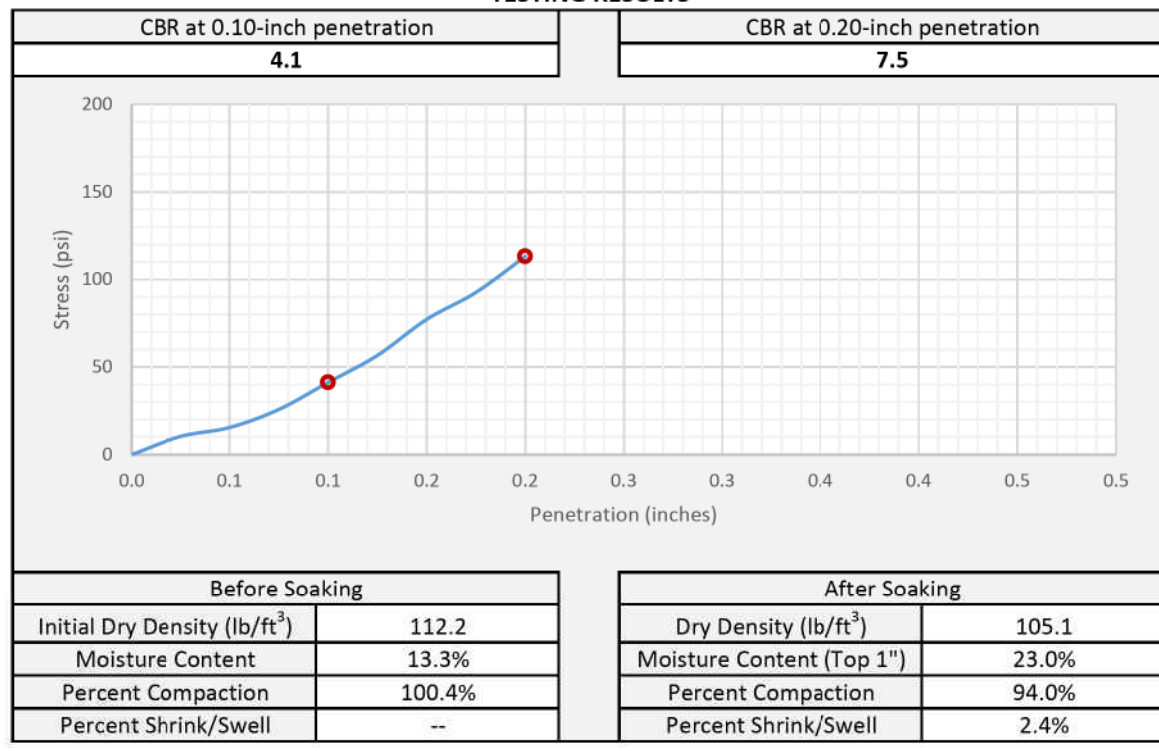
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-1		FME Lab ID	21-1758	
Soil Description	A-4(0)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	0%
Max Dry Density (lb/ft ³)	111.8	Optimum Moisture Content (%)	13.5
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=39, PL=NP, PI=NP, %Passing #200 Sieve = 43.4
20+26 - 68' RT

	F&ME Consultants, Inc. 3112 Devine Street, Columbia, SC 29205		130-04-0212
		Reviewed By	NCDOT Certification No. 8/18/2021 Date

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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

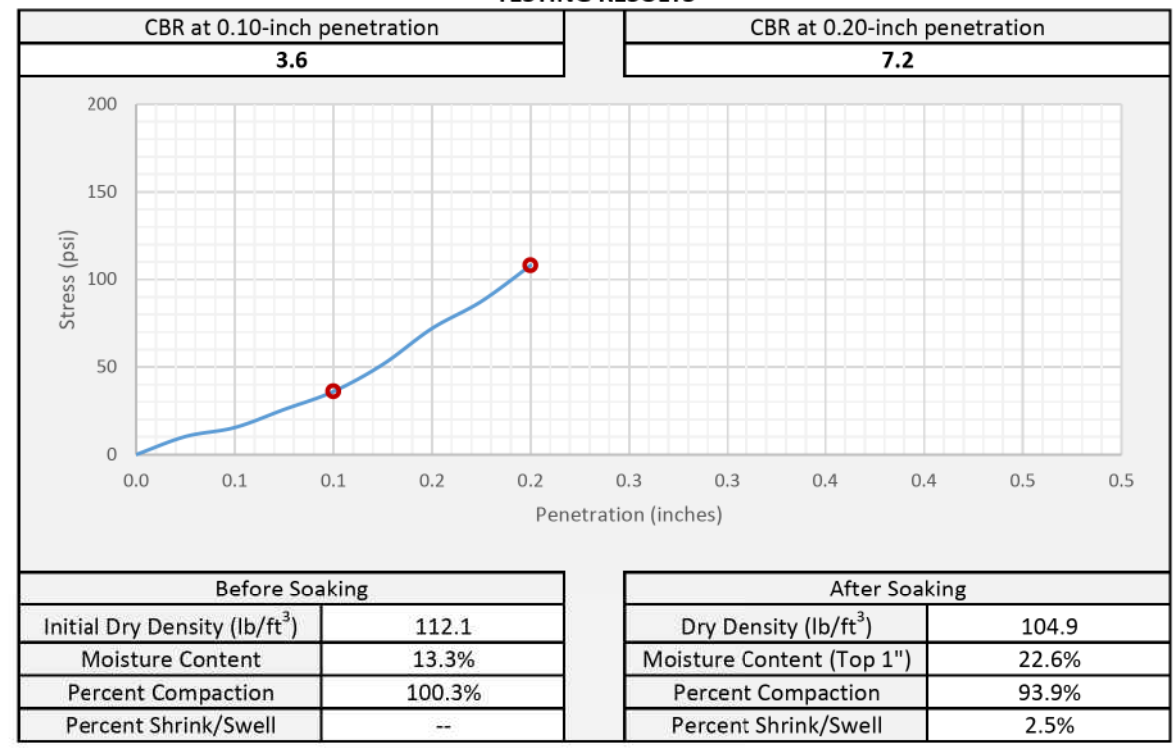
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-1		FME Lab ID	21-1758	
Soil Description	A-4(0)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	0%
Max Dry Density (lb/ft ³)	111.8	Optimum Moisture Content (%)	13.5
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=39, PL=NP, PI=NP, %Passing #200 Sieve = 43.4
20+26 - 68' RT

	F&ME Consultants, Inc. 3112 Devine Street, Columbia, SC 29205		130-04-0212
		Reviewed By	NCDOT Certification No. 8/18/2021 Date

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REV 08/2021

**COMPRESSIVE STRENGTH OF MOLDED SOIL-CEMENT CYLINDERS
ASTM D-1633**

SAMPLE INFORMATION

Project Name	US 23 / US 74 over Richland Creek	NCDOT STIP #	B-3186/B-5898
Sample Location	Bulk-1	FME Lab ID	21-1758
Soil Description	A-4(0)	Depth/Elev.	1.0 - 3.0 ft.
Station	20+26	Offset	68' RT
Date Sampled	7/28/2021	Sampled By:	CG2
Date Molded	8/19/2021	Date Tested	8/26/2021
		Tested By	M. Johnson

MOLDING CHARACTERISTICS

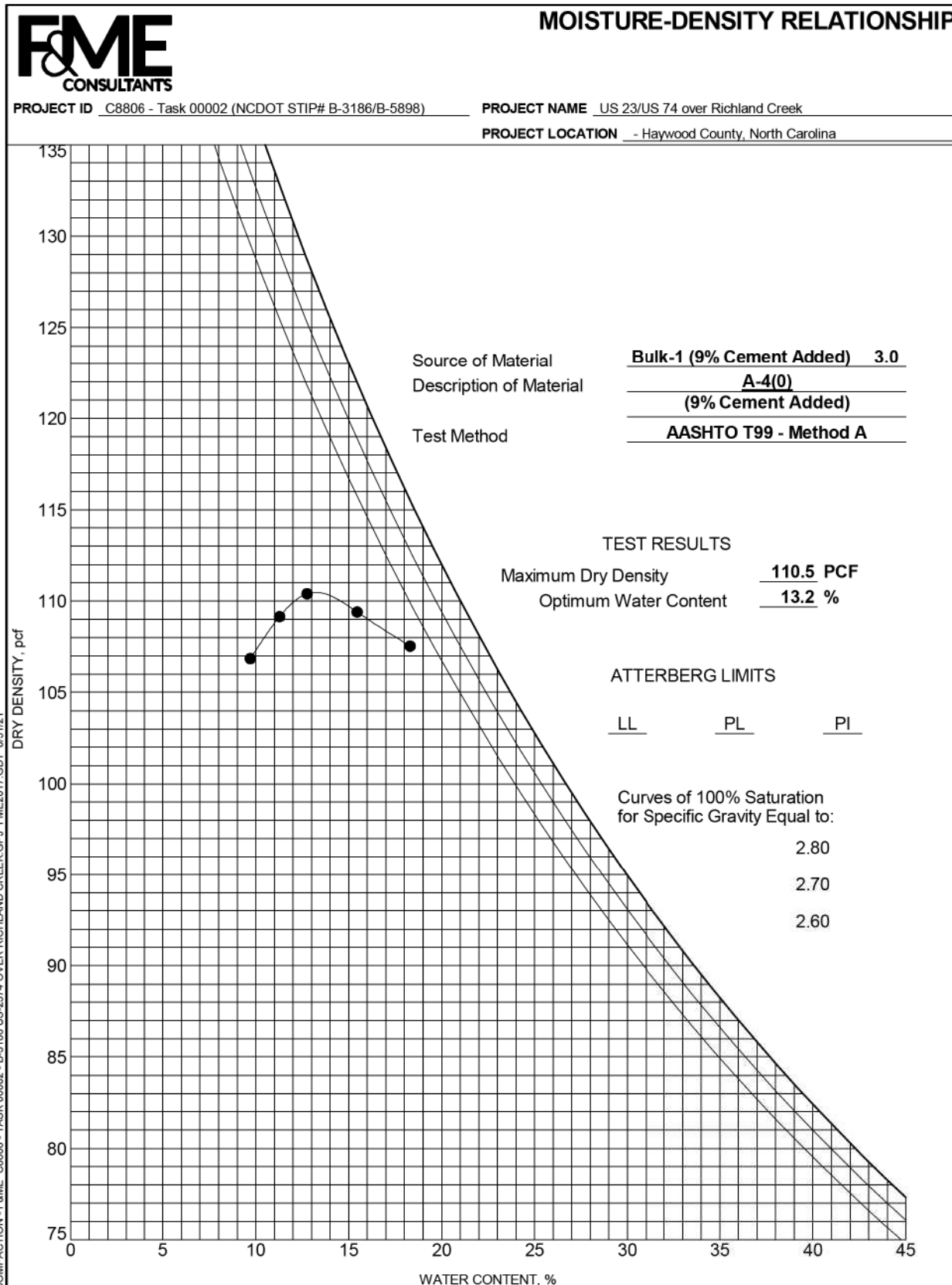
Method	AASHTO T99 - Method A	% Cement Added to Proctor	9%
Max Dry Density (lb/ft ³)	110.5	Optimum Moisture Content (%)	13.2

TESTING RESULTS

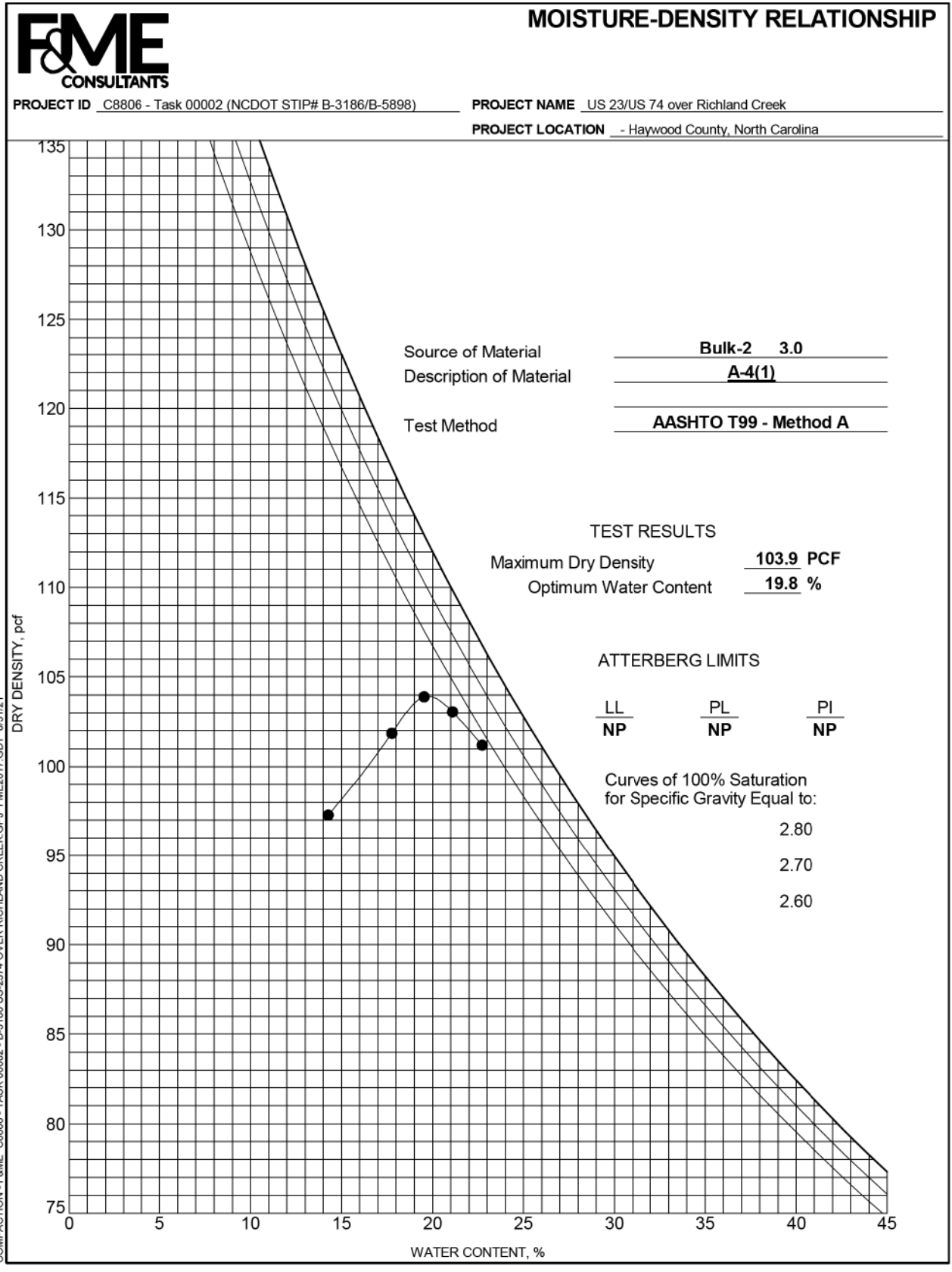
% Cement	Age (Days)	Moisture Content	Height (in.)	Diameter (in.)	Area (in. ²)	Maximum Load (lbf)	Compressive Strength (psi)	Average Compressive Strength (psi)
8%	7	13.0%	4.599	4.005	12.60	3990	315	260
8%	7	13.0%	4.607	4.005	12.60	2450	195	
10%	7	12.9%	4.63	4.003	12.59	5170	410	350
10%	7	12.9%	4.617	3.996	12.54	3640	290	

ADDITIONAL COMMENTS

Bulk Soil Sample Data (without Cement Added):
 LL=39, PL=NP, PI=NP
 %Passing #200 Sieve = 43.4%
 As-Received Natural Moisture Content = 12.9%



<p>F&ME Consultants, Inc. 3112 Devine Street, Columbia, SC 29205</p>	 Reviewed By	130-04-0212 NCDOT Certification No. 8/31/2021 Date
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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

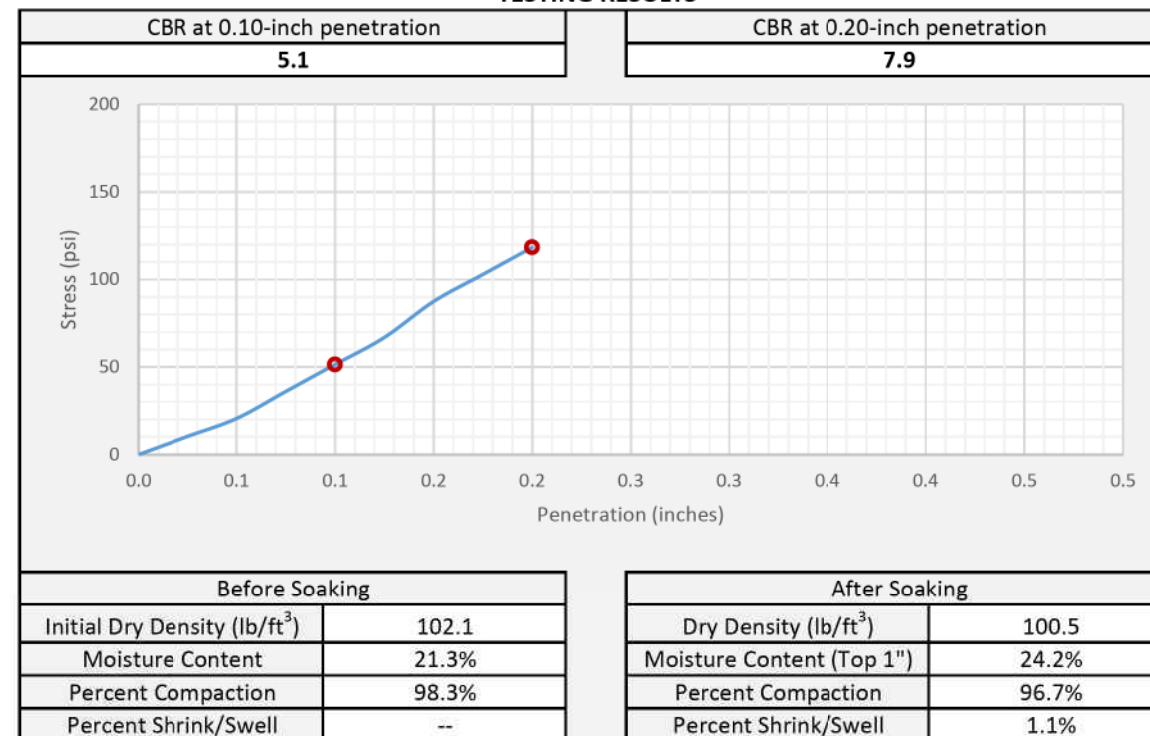
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-2		FME Lab ID	21-1759	
Soil Description	A-4(1)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	0%
Max Dry Density (lb/ft ³)	103.9	Optimum Moisture Content (%)	19.8
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=37, PL=NP, PI=NP, %Passing #200 Sieve = 68.0
35+27, 60' LT



F&ME Consultants, Inc.
3112 Devine Street, Columbia, SC 29205

Jerry P. Davis
Reviewed By

130-04-0212
NCDOT Certification No.
8/18/2021
Date

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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

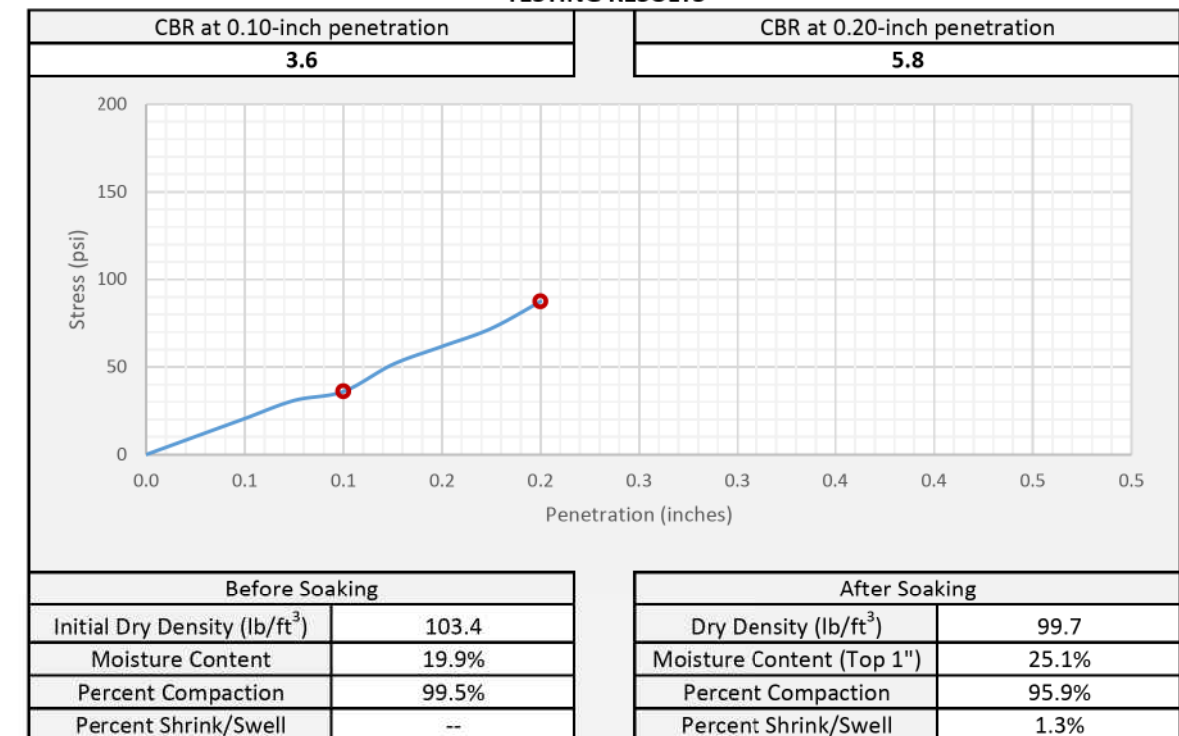
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-2		FME Lab ID	21-1759	
Soil Description	A-4(1)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	0%
Max Dry Density (lb/ft ³)	103.9	Optimum Moisture Content (%)	19.8
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=37, PL=NP, PI=NP, %Passing #200 Sieve = 68.0
35+27, 60' LT

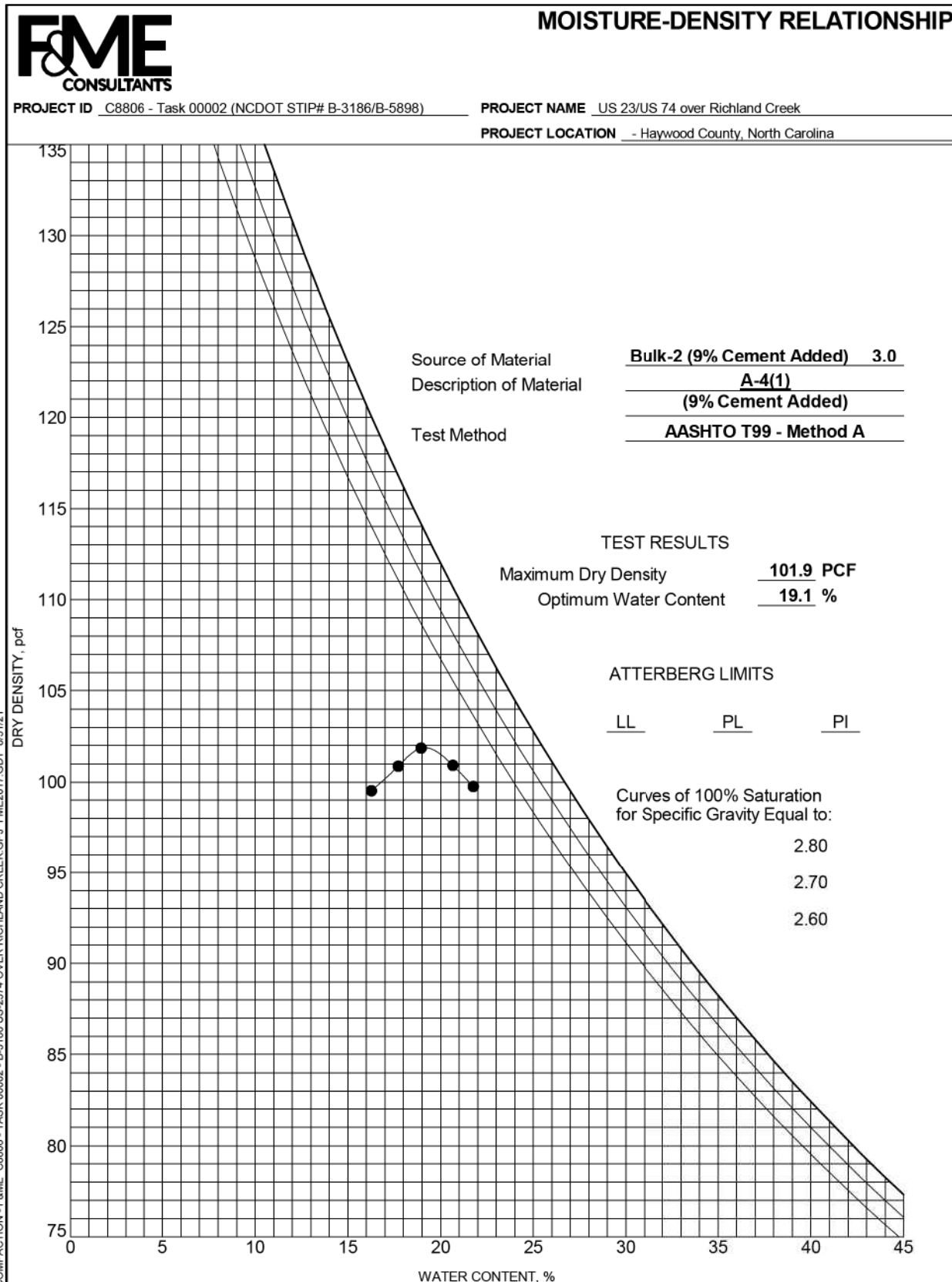


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3112 Devine Street, Columbia, SC 29205

Jerry P. Davis
Reviewed By

130-04-0212
NCDOT Certification No.
8/18/2021
Date

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REV 08/2021

**COMPRESSIVE STRENGTH OF MOLDED SOIL-CEMENT CYLINDERS
 ASTM D-1633**

SAMPLE INFORMATION

Project Name	US 23 / US 74 over Richland Creek	NCDOT STIP #	B-3186/B-5898
Sample Location	Bulk-2	FME Lab ID	21-1759
Soil Description	A-4(1)	Depth/Elev.	1.0 - 3.0 ft.
Station	35+27	Offset	60' LT
Date Sampled	7/28/2021	Sampled By:	CG2
Date Molded	8/19/2021	Date Tested	8/26/2021
		Date Received	8/4/2021
		Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Cement Added to Proctor	9%
Max Dry Density (lb/ft ³)	101.9	Optimum Moisture Content (%)	19.1

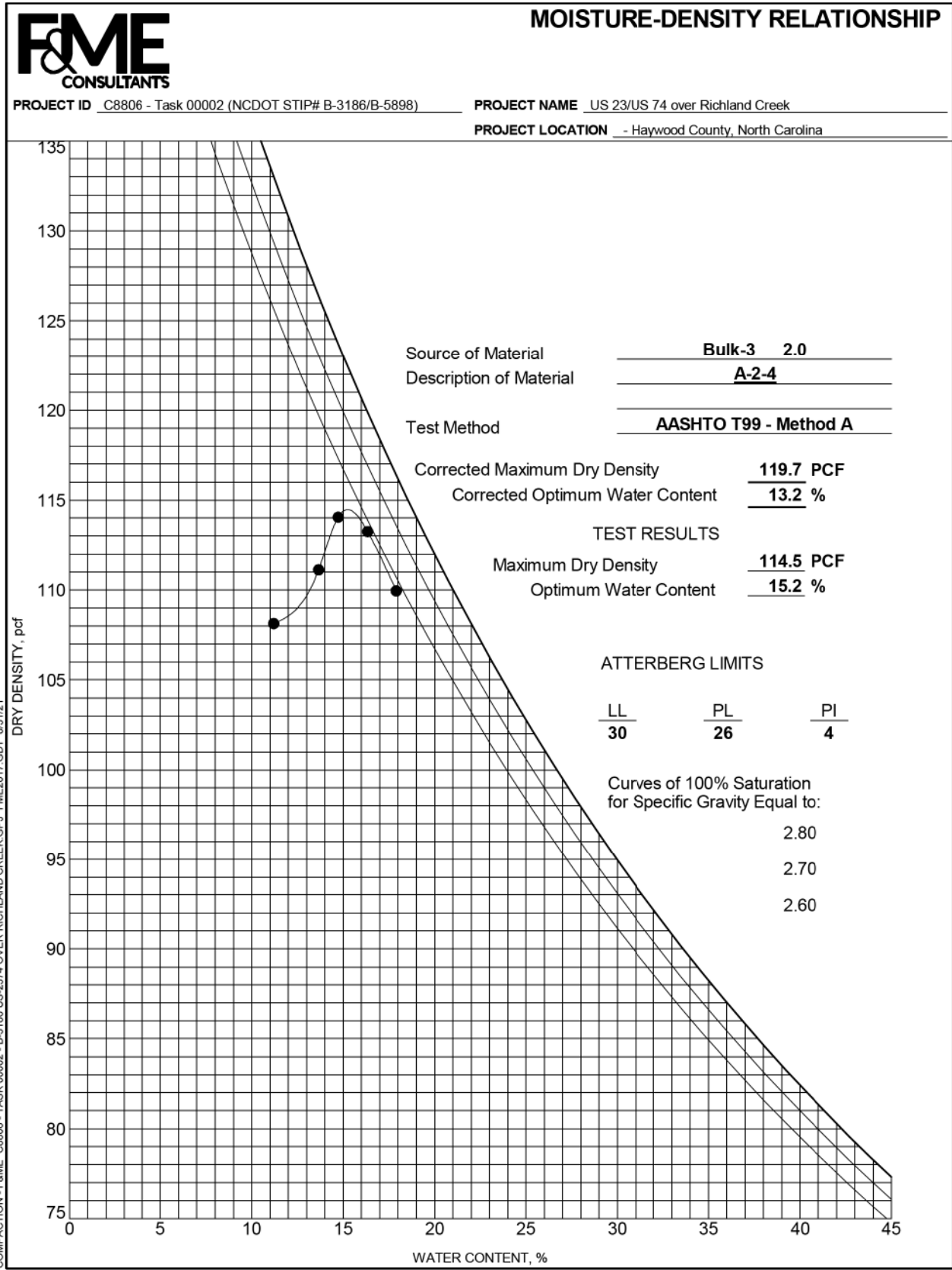
TESTING RESULTS

% Cement	Age (Days)	Moisture Content	Height (in.)	Diameter (in.)	Area (in. ²)	Maximum Load (lbf)	Compressive Strength (psi)	Average Compressive Strength (psi)
8%	7	19.8%	4.626	4.002	12.58	3720	295	280
8%	7	19.8%	4.619	3.999	12.56	3270	260	
10%	7	19.1%	4.614	4.002	12.58	4160	330	325
10%	7	19.1%	4.619	4.002	12.58	3950	315	

ADDITIONAL COMMENTS

Bulk Soil Sample Data (without Cement Added):
 LL=37, PL=NP, PI=NP
 %Passing #200 Sieve = 68.0%
 As-Received Natural Moisture Content = 21.7%

<p>F&ME Consultants, Inc. 3112 Devine Street, Columbia, SC 29205</p>	 Reviewed By	130-04-0212 NCDOT Certification No. 8/31/2021 Date
	Date	



REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

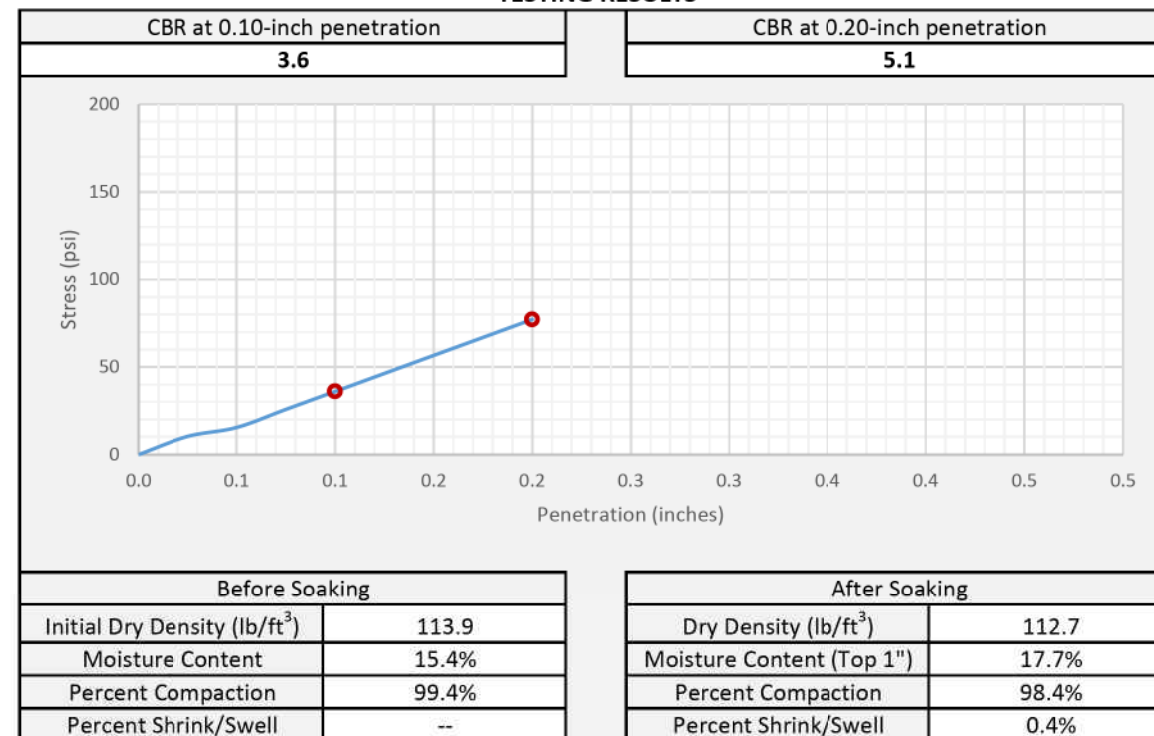
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-3		FME Lab ID	21-1760	
Soil Description	A-2-4		Depth/Elev.	1.0' - 2.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	5%
Max Dry Density (lb/ft ³)	114.5	Optimum Moisture Content (%)	15.2
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=30, PL=26, PI=4, %Passing #200 Sieve = 34.3
63+31, 2' LT



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8/18/2021

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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

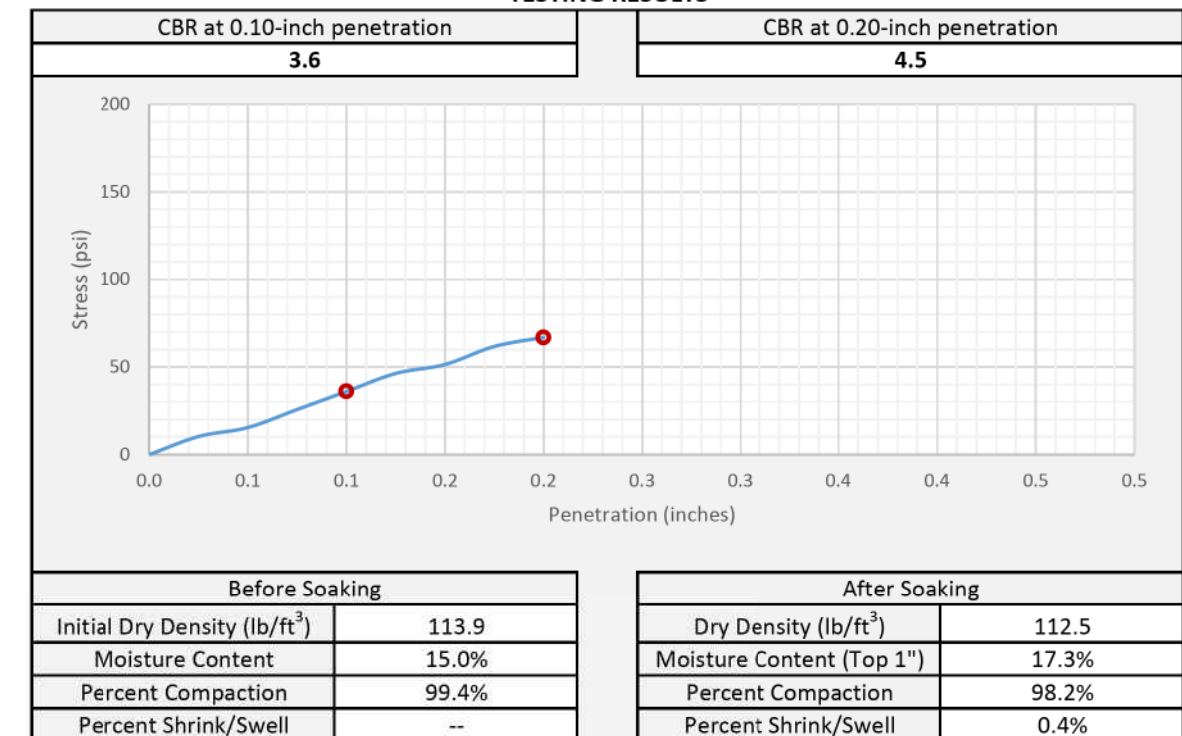
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-3		FME Lab ID	21-1760	
Soil Description	A-2-4		Depth/Elev.	1.0' - 2.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	5%
Max Dry Density (lb/ft ³)	114.5	Optimum Moisture Content (%)	15.2
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=30, PL=26, PI=4, %Passing #200 Sieve = 34.3
63+31, 2' LT



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Jerry P. Davis

Reviewed By

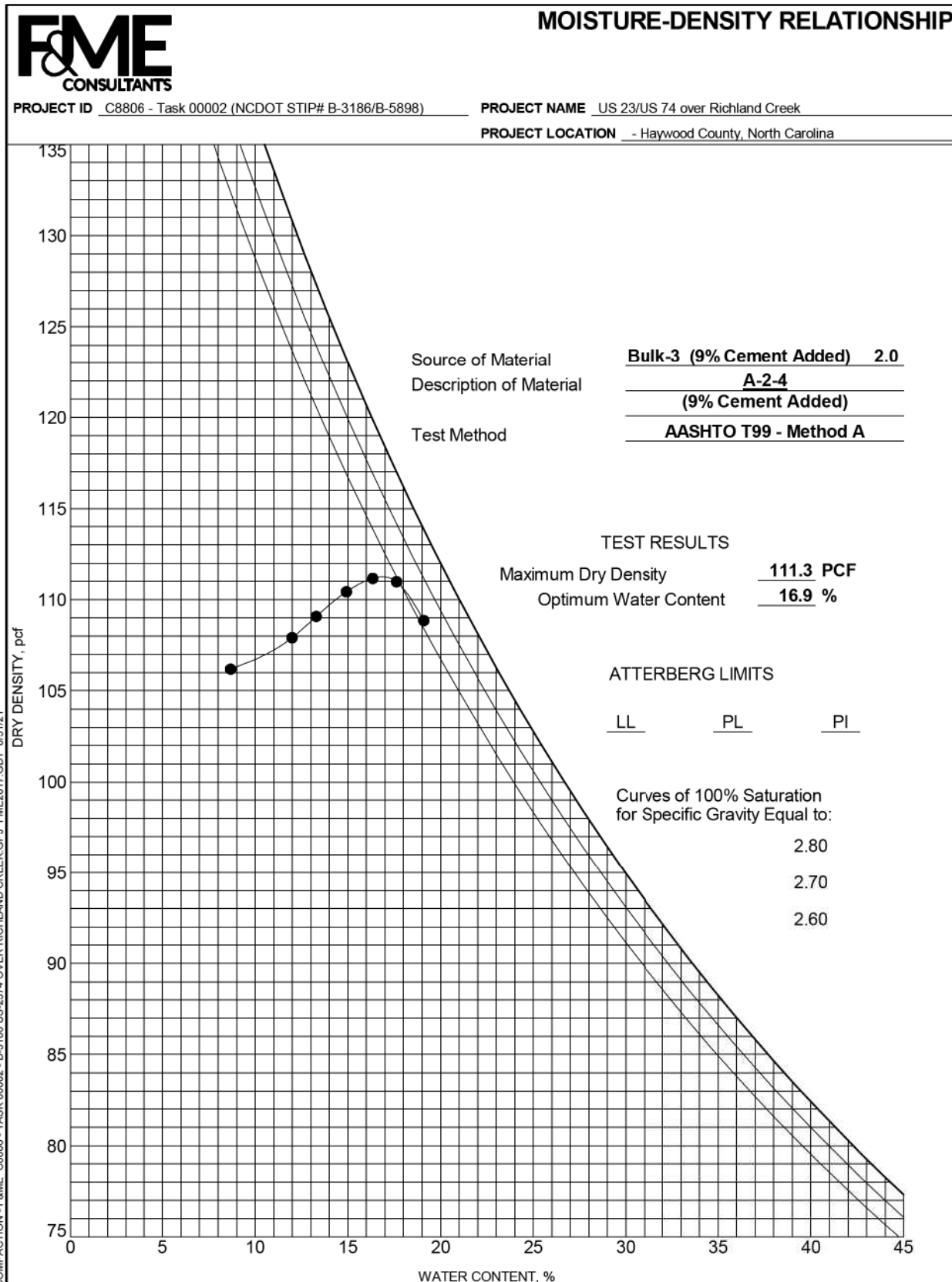
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NCDOT Certification No.

8/18/2021

Date

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REV 08/2021

**COMPRESSIVE STRENGTH OF MOLDED SOIL-CEMENT CYLINDERS
 ASTM D-1633**

SAMPLE INFORMATION

Project Name	US 23 / US 74 over Richland Creek	NCDOT STIP #	B-3186/B-5898
Sample Location	Bulk-3	FME Lab ID	21-1760
Soil Description	A-2-4	Depth/Elev.	1.0 - 2.0 ft.
Station	65+31	Offset	2' LT
Date Sampled	7/28/2021	Sampled By:	CG2
Date Molded	8/19/2021	Date Tested	8/26/2021
		Date Received	8/4/2021
		Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Cement Added to Proctor	9%
Max Dry Density (lb/ft ³)	111.3	Optimum Moisture Content (%)	16.9

TESTING RESULTS

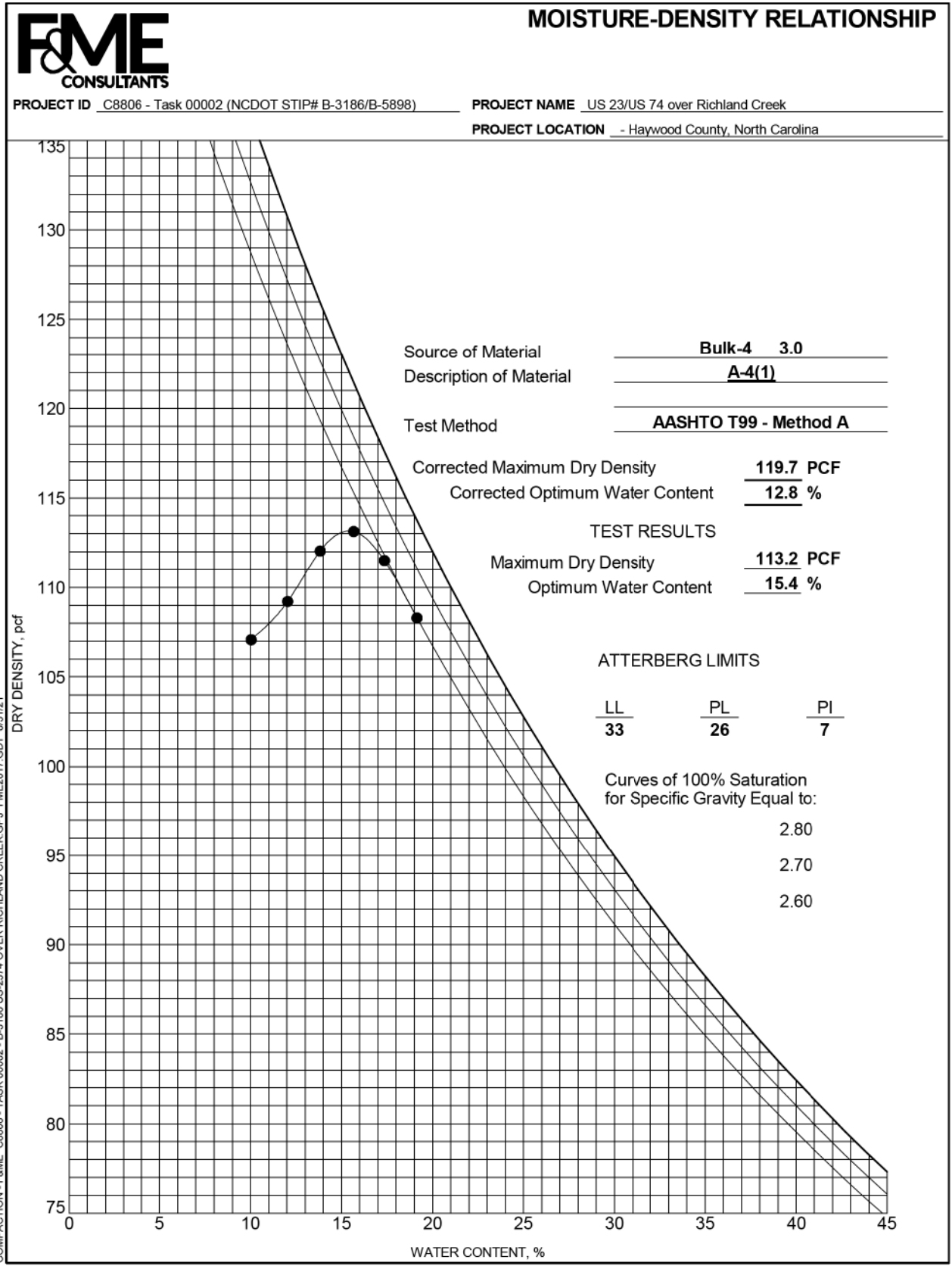
% Cement	Age (Days)	Moisture Content	Height (in.)	Diameter (in.)	Area (in. ²)	Maximum Load (lbf)	Compressive Strength (psi)	Average Compressive Strength (psi)
8%	7	16.3%	4.623	4.001	12.57	4440	355	345
8%	7	16.3%	4.638	3.998	12.55	4160	330	
10%	7	16.4%	4.612	4.000	12.57	5440	435	435
10%	7	16.4%	4.613	4.001	12.57	5410	430	

ADDITIONAL COMMENTS

Bulk Soil Sample Data (without Cement Added):
 LL=30, PL=26, PI=4
 %Passing #200 Sieve = 34.3%
 As-Received Natural Moisture Content = 11.4%

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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

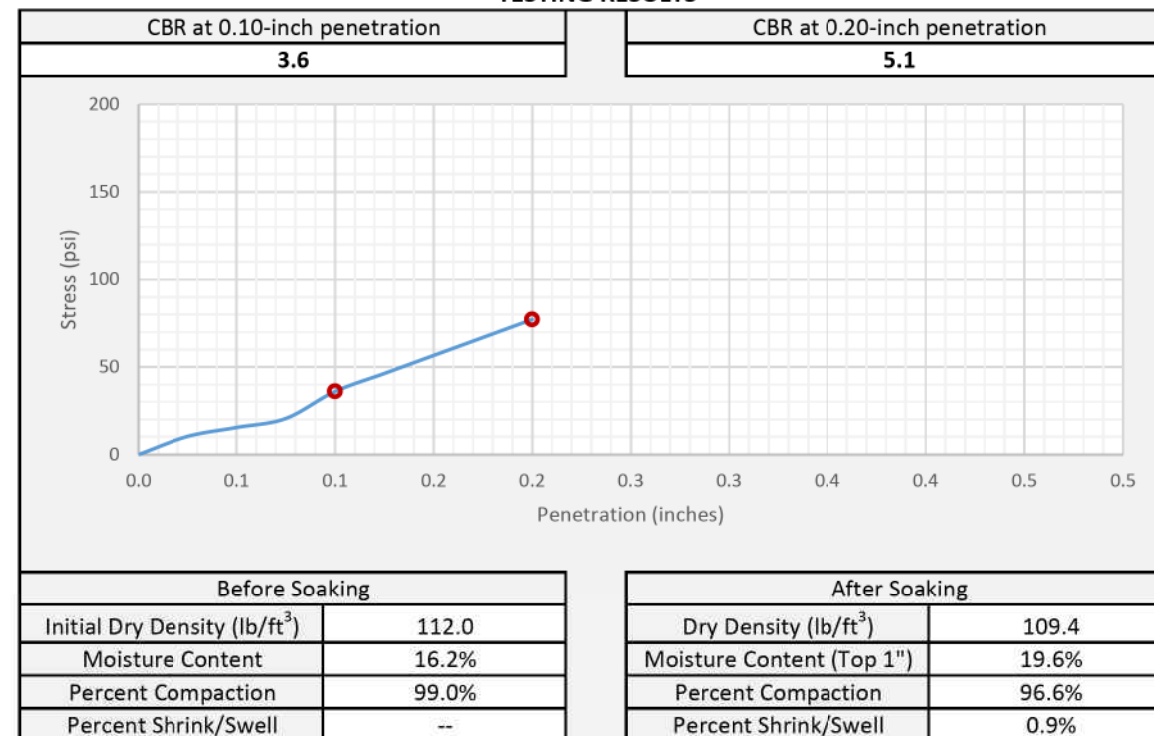
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-4		FME Lab ID	21-1761	
Soil Description	A-4(1)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	6%
Max Dry Density (lb/ft ³)	113.2	Optimum Moisture Content (%)	15.4
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=33, PL=26, PI=7, %Passing #200 Sieve = 45.7
77+10, 71' RT



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3112 Devine Street, Columbia, SC 29205

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130-04-0212
NCDOT Certification No.
8/10/2021
Date

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REV 08/2021

**CALIFORNIA BEARING RATIO (CBR)
AASHTO T193**

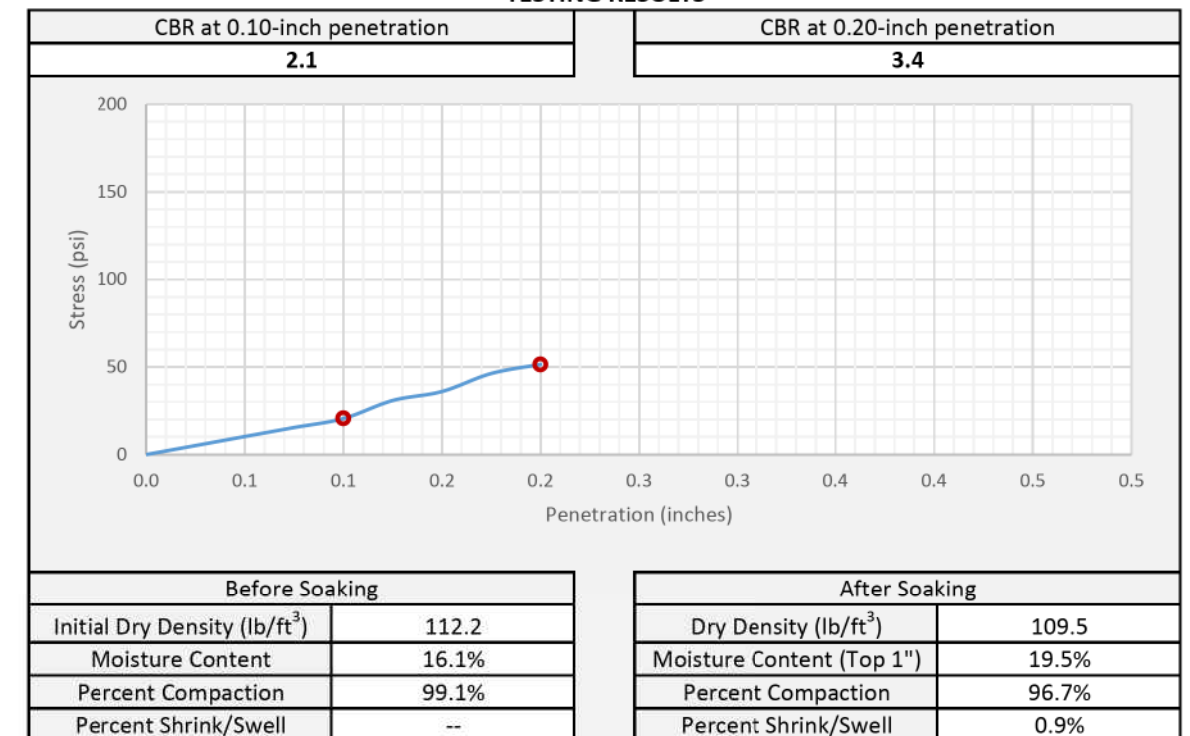
SAMPLE INFORMATION

Project Name	US 23 / US 74 Over Richland Creek		NCDOT STIP #	B-3186/B-5898	
Sample Location	Bulk-4		FME Lab ID	21-1761	
Soil Description	A-4(1)		Depth/Elev.	1.0' - 3.0'	
Date Sampled	7/28/2021	Sampled By:	CG2	Date Received	8/4/2021
Date Test Began	8/13/2021	Date Completed	8/17/2021	Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Retained on 3/4" Sieve	6%
Max Dry Density (lb/ft ³)	113.2	Optimum Moisture Content (%)	15.4
Soak Time (hr)	96	Surcharge Weight (lb)	10.0

TESTING RESULTS



ADDITIONAL COMMENTS

LL=33, PL=26, PI=7, %Passing #200 Sieve = 45.7
77+10, 71' RT

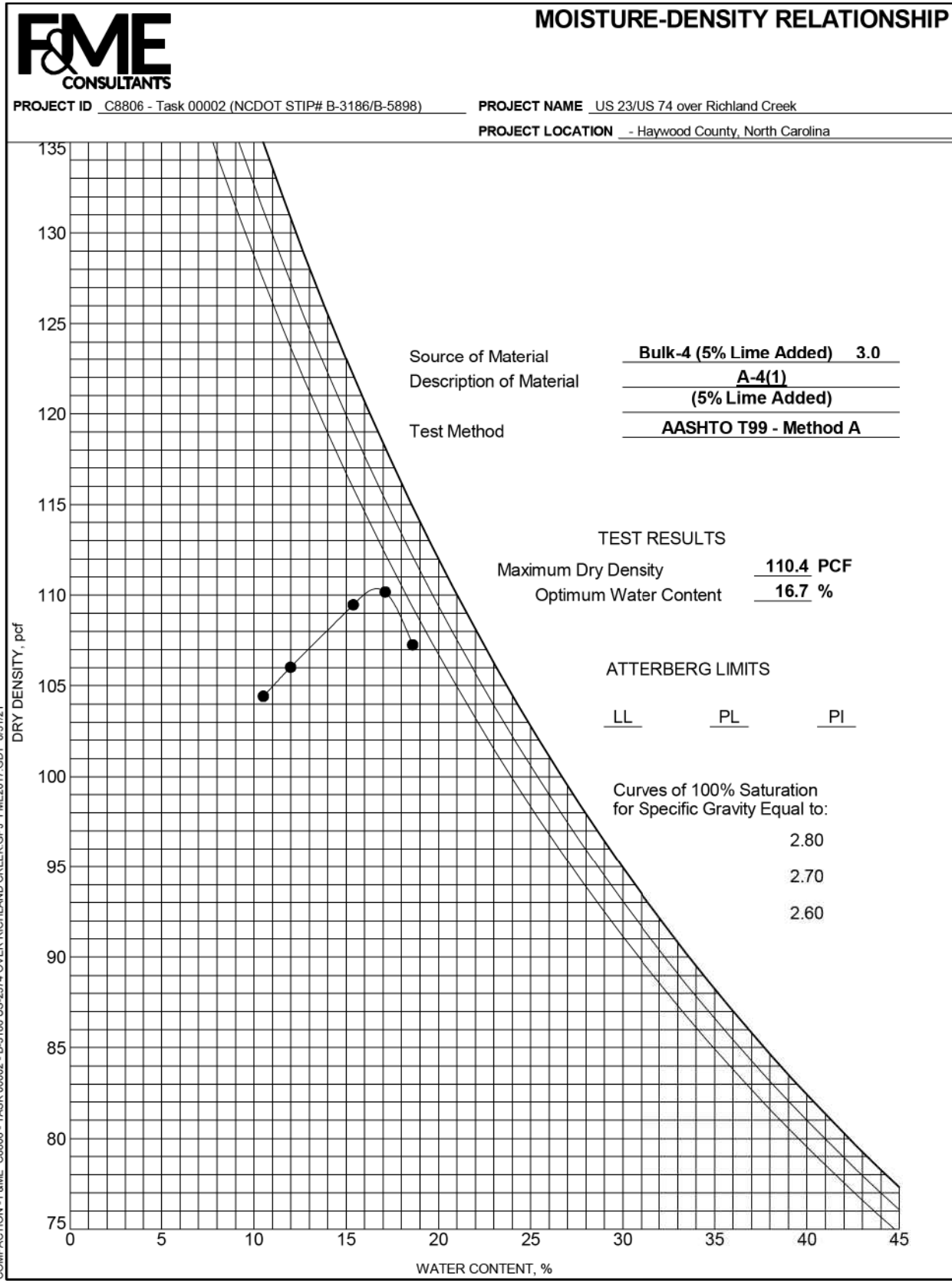


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REV 08/2021

COMPRESSIVE STRENGTH OF MOLDED SOIL-LIME CYLINDERS
ASTM D-1633

SAMPLE INFORMATION

Project Name	US 23 / US 74 over Richland Creek	NCDOT STIP #	B-3186/B-5898
Sample Location	Bulk-4	FME Lab ID	21-1761
Soil Description	A-4(1)	Depth/Elev.	1.0 - 3.0 ft.
Station	77+10	Offset	71' RT
Date Sampled	7/28/2021	Sampled By:	CG2
Date Molded	8/19/2021	Date Tested	9/3/2021
		Date Received	8/4/2021
		Tested By	M. Johnson

MOLDING CHARACTERISTICS

Method	AASHTO T99 - Method A	% Lime Added to Proctor	5%
Max Dry Density (lb/ft ³)	110.1	Optimum Moisture Content (%)	16.7

TESTING RESULTS

% Lime	Age (Days)	Moisture Content	Height (in.)	Diameter (in.)	Area (in. ²)	Maximum Load (lbf)	Compressive Strength (psi)	Average Compressive Strength (psi)
4%	7	16.8%	4.628	3.993	12.52	730	60	60
4%	7	16.7%	4.608	3.995	12.53	750	60	
6%	7	16.9%	4.635	3.992	12.52	770	60	60
6%	7	16.7%	4.630	3.997	12.55	770	60	

ADDITIONAL COMMENTS

Bulk Soil Sample Data (without Lime Added):
 LL=33, PL=24, PI=7
 %Passing #200 Sieve = 45.7%
 As-Received Natural Moisture Content = 8.9%

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			9/8/2021 Date

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**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-1 -L- 15+32 EB OSS

5.0 FT RT FW

Datum = STBC

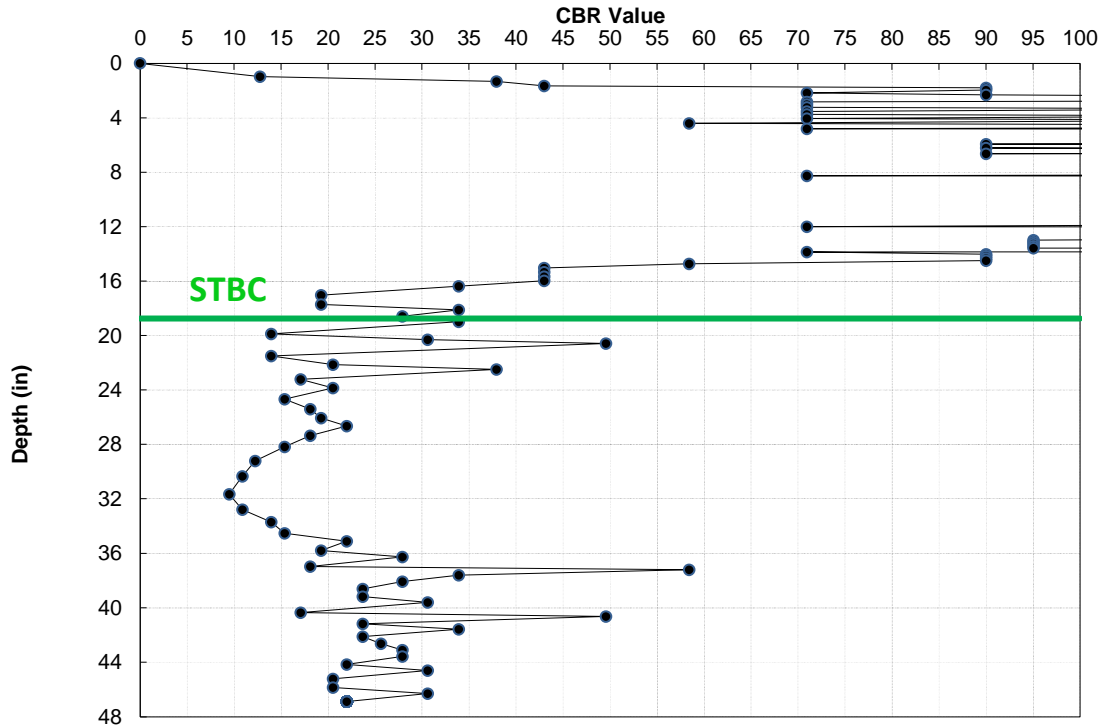
RAW

Fill

08/04/21

Interval	
0.0	to 18.6
# of Values	148
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	12.8

Interval	
18.6	to 46.9
# of Values	45
Avg CBR	24.0
Wghtd Avg.	20.6
Max CBR	58.4
Min CBR	9.5



1

C-5 -L- 25+35 EB ISS

1.4 FT LT FY

Datum = STBC

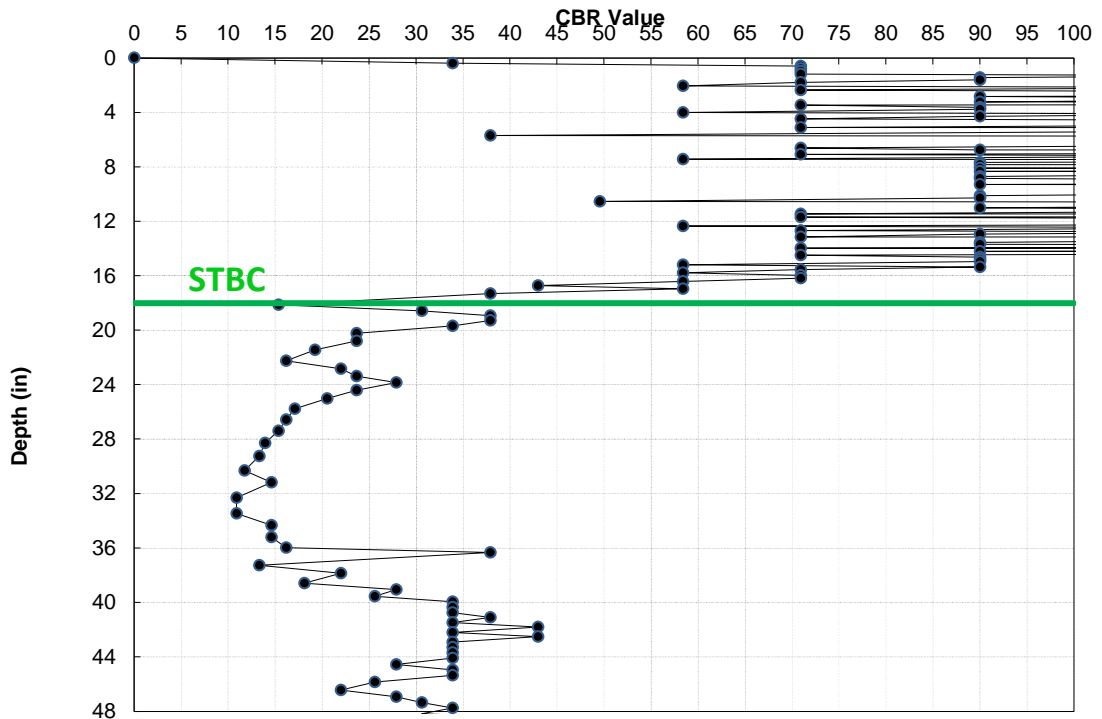
RAW

Fill

08/04/21

Interval	
0.0	to 18.1
# of Values	122
Avg CBR	100+
Wghtd Avg.	95.6
Max CBR	100+
Min CBR	15.4

Interval	
18.1	to 56.9
# of Values	73
Avg CBR	28.0
Wghtd Avg.	24.7
Max CBR	43.0
Min CBR	10.9



2

**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

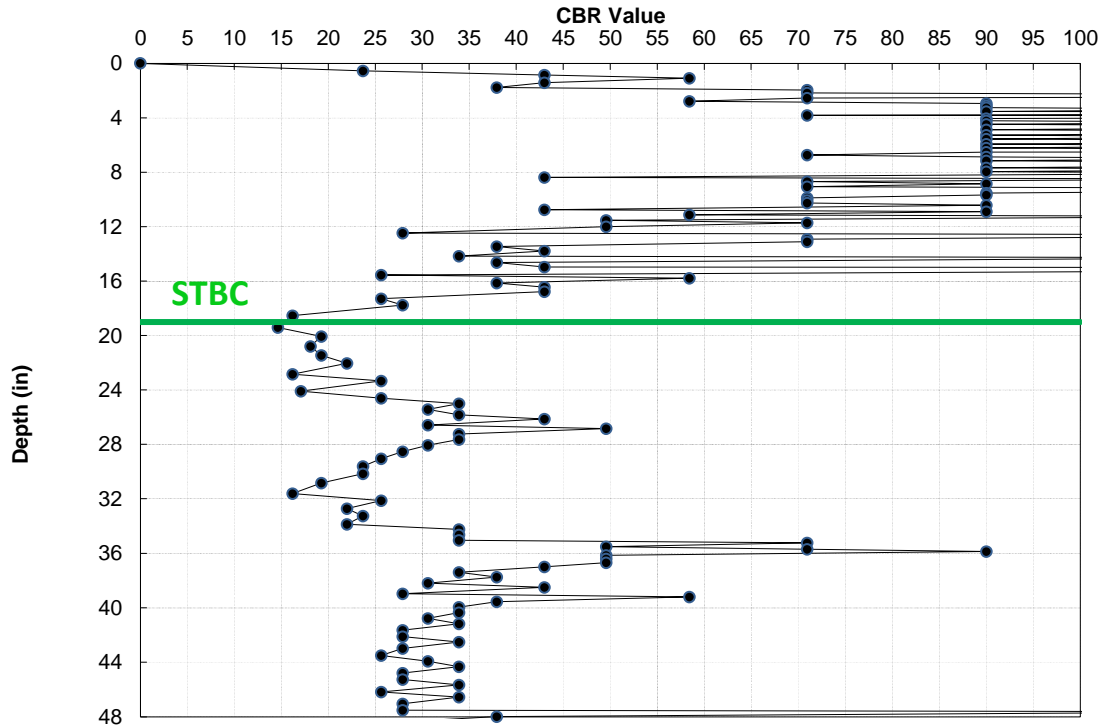
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-6 -L- 25+35 EB OSS
5.4 FT RT FW
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 19.4
# of Values	92
Avg CBR	90.4
Wghtd Avg.	65.9
Max CBR	100+
Min CBR	14.6

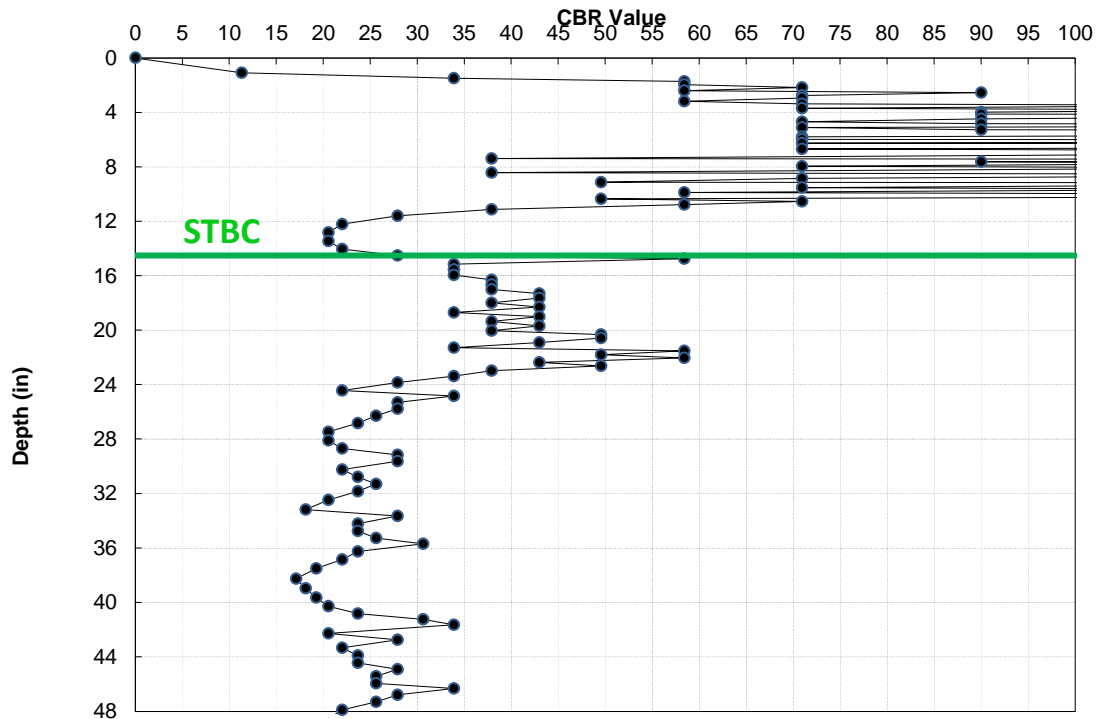
Interval	
19.4	to 56.3
# of Values	87
Avg CBR	35.3
Wghtd Avg.	31.3
Max CBR	100+
Min CBR	16.2



C-7 -L- 29+86 EB ISS
1.3 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 14.5
# of Values	67
Avg CBR	96.8
Wghtd Avg.	64.0
Max CBR	100+
Min CBR	11.3

Interval	
14.5	to 57.2
# of Values	87
Avg CBR	29.5
Wghtd Avg.	26.8
Max CBR	58.4
Min CBR	17.1



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

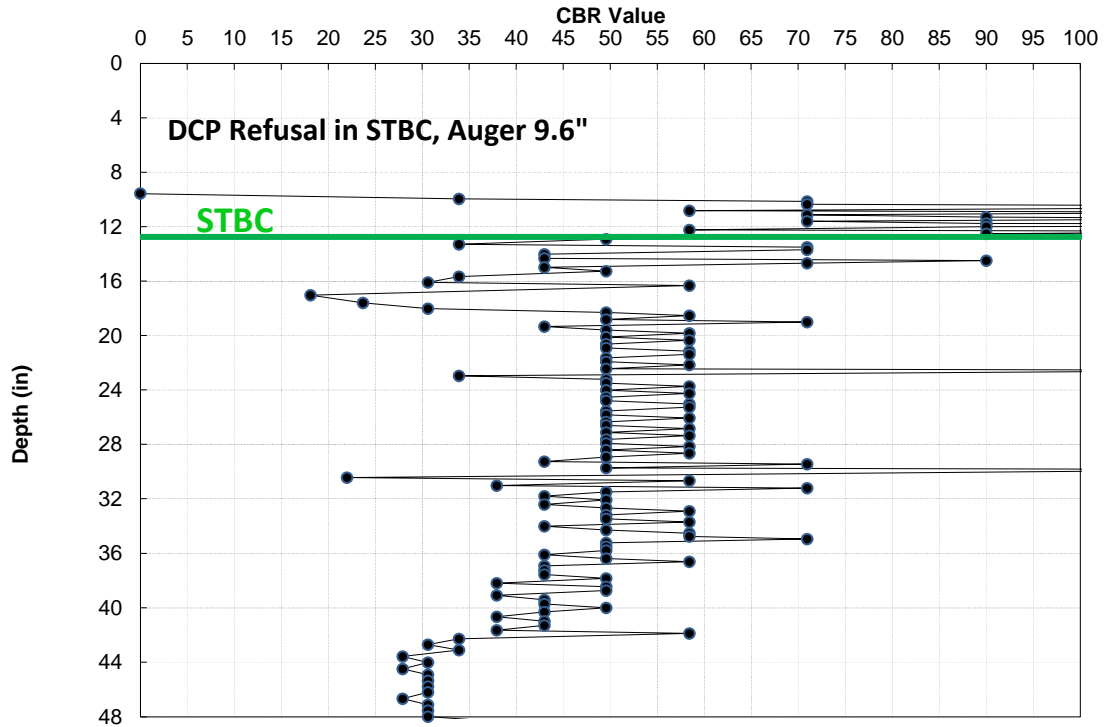
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-8 -L- 29+86 EB ISL
8.0 FT RT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
9.6	to 12.9
# of Values	19
Avg CBR	93.0
Wghtd Avg.	79.9
Max CBR	100+
Min CBR	33.9

Interval	
12.9	to 51.5
# of Values	127
Avg CBR	48.6
Wghtd Avg.	44.7
Max CBR	100+
Min CBR	18.1

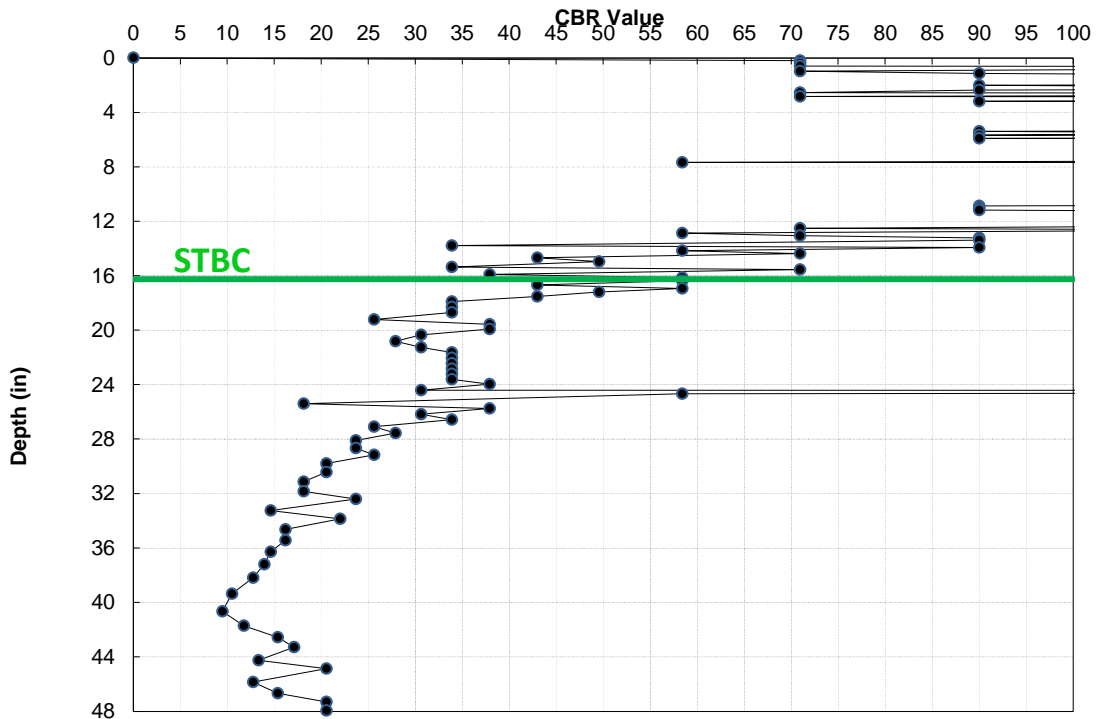


5

C-10 -L- 35+32 EB ISS
2.0 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 16.4
# of Values	151
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	33.9

Interval	
16.4	to 60.1
# of Values	74
Avg CBR	31.4
Wghtd Avg.	22.0
Max CBR	100+
Min CBR	9.5



6

**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

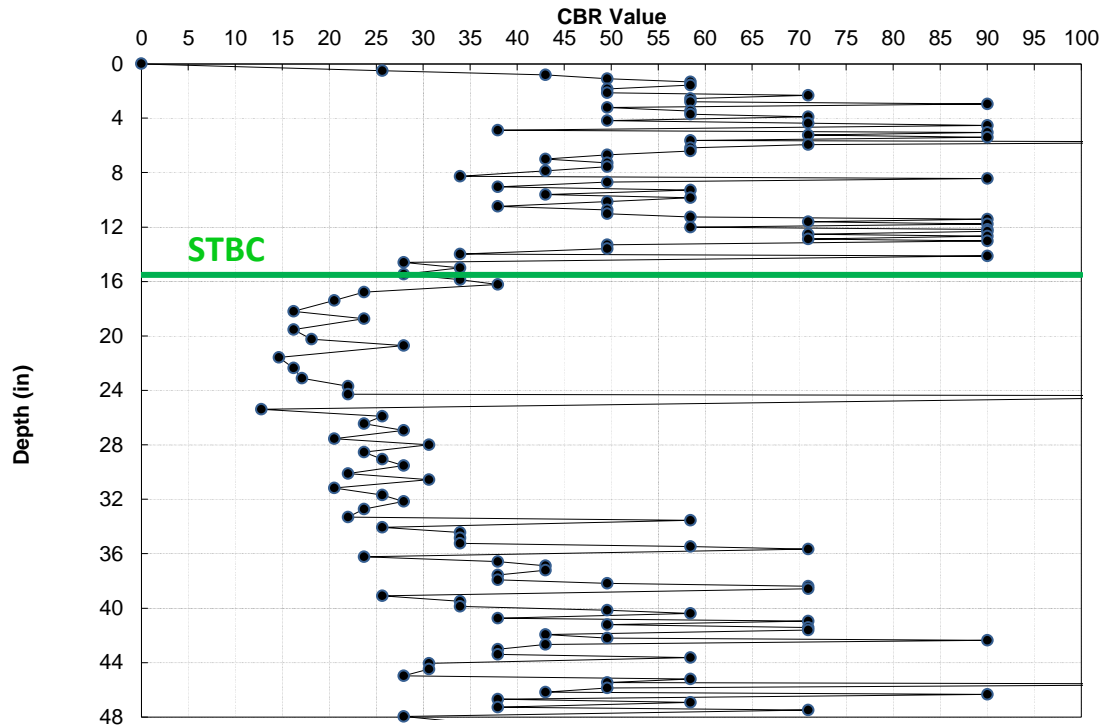
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-11 -L- 35+34 EB OSS
5.5 FT RT FW
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 15.5
# of Values	61
Avg CBR	60.5
Wghtd Avg.	54.1
Max CBR	100+
Min CBR	25.6

Interval	
15.5	to 62.2
# of Values	127
Avg CBR	46.1
Wghtd Avg.	36.4
Max CBR	100+
Min CBR	12.8

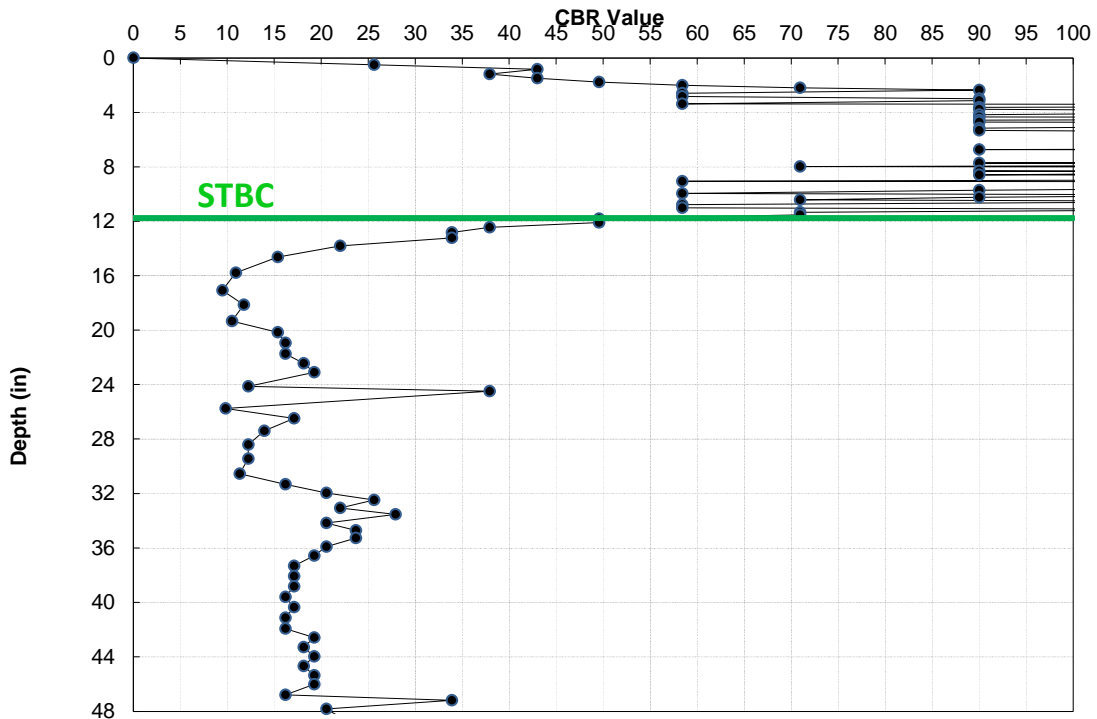


7

C-12 -L- 61+63 EB ISS
0.7 FT LT FY
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 11.8
# of Values	81
Avg CBR	60.5
Wghtd Avg.	54.1
Max CBR	100+
Min CBR	25.6

Interval	
11.8	to 55.4
# of Values	62
Avg CBR	20.4
Wghtd Avg.	18.2
Max CBR	49.6
Min CBR	9.5



8

**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

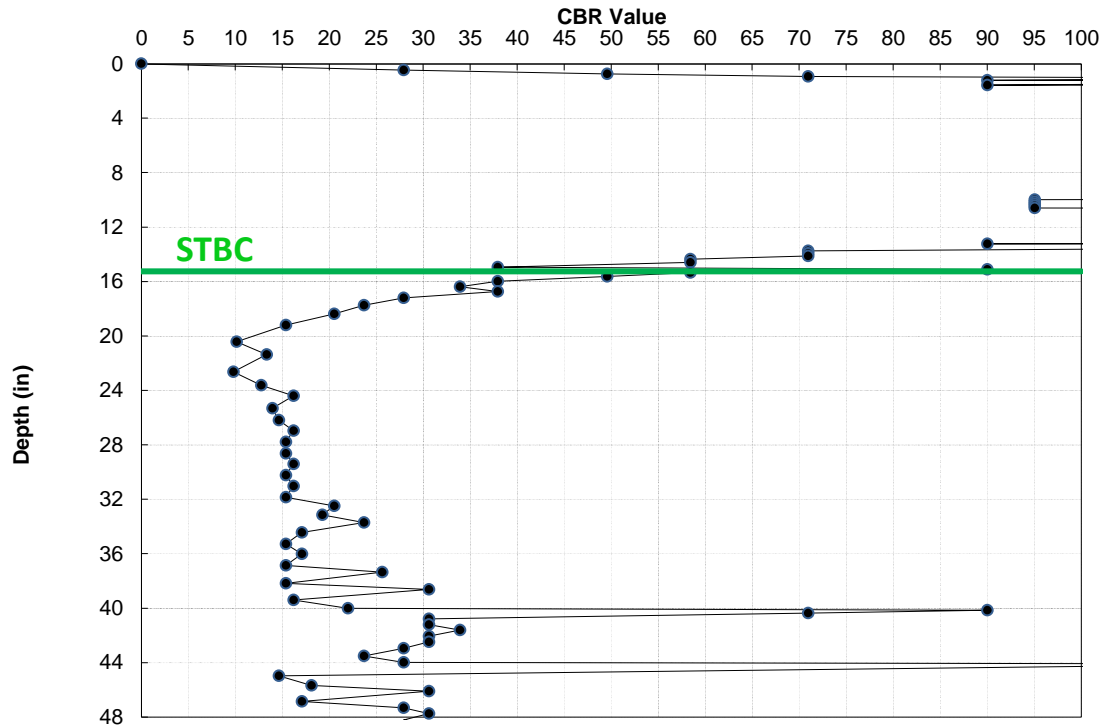
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-13 -L- 61+63 EB ISL
5.3 FT RT FY
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 15.1
# of Values	189
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	27.9

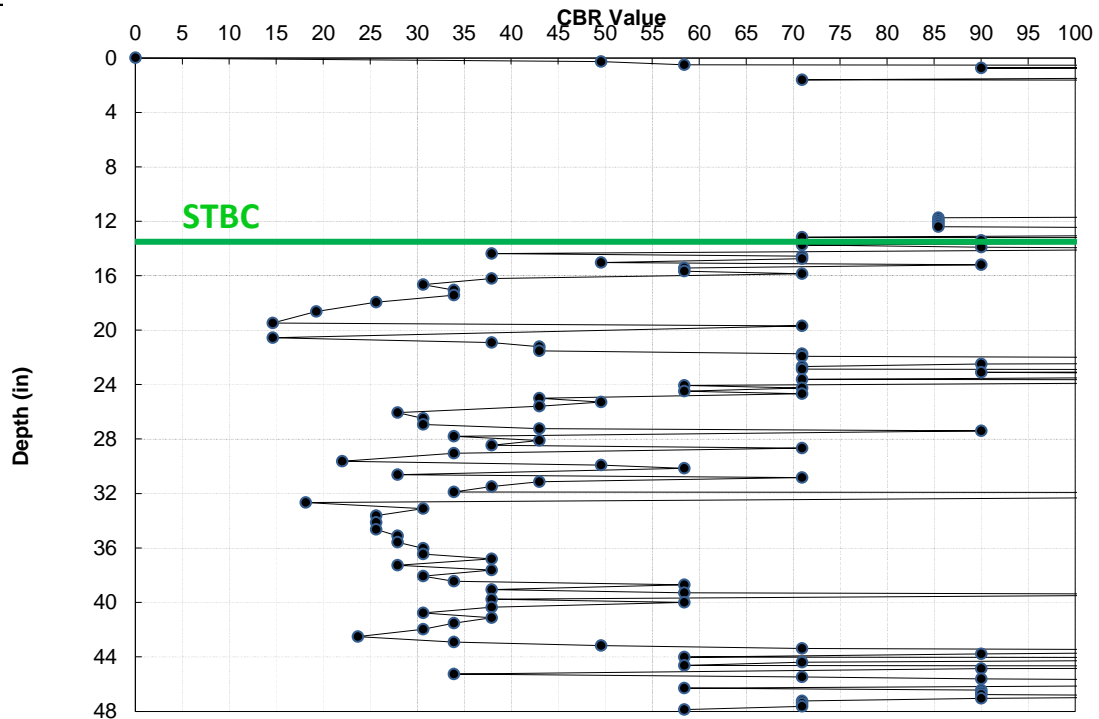
Interval	
15.1	to 55.8
# of Values	72
Avg CBR	29.0
Wghtd Avg.	23.0
Max CBR	100+
Min CBR	9.8



C-14 -L- 61+64 EB OSL
2.2 FT LT FW
Datum = STBC
RAW
Fill
8/4/2021

Interval	
0.0	to 13.5
# of Values	228
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	49.6

Interval	
13.5	to 59.4
# of Values	157
Avg CBR	63.8
Wghtd Avg.	46.5
Max CBR	100+
Min CBR	14.6



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

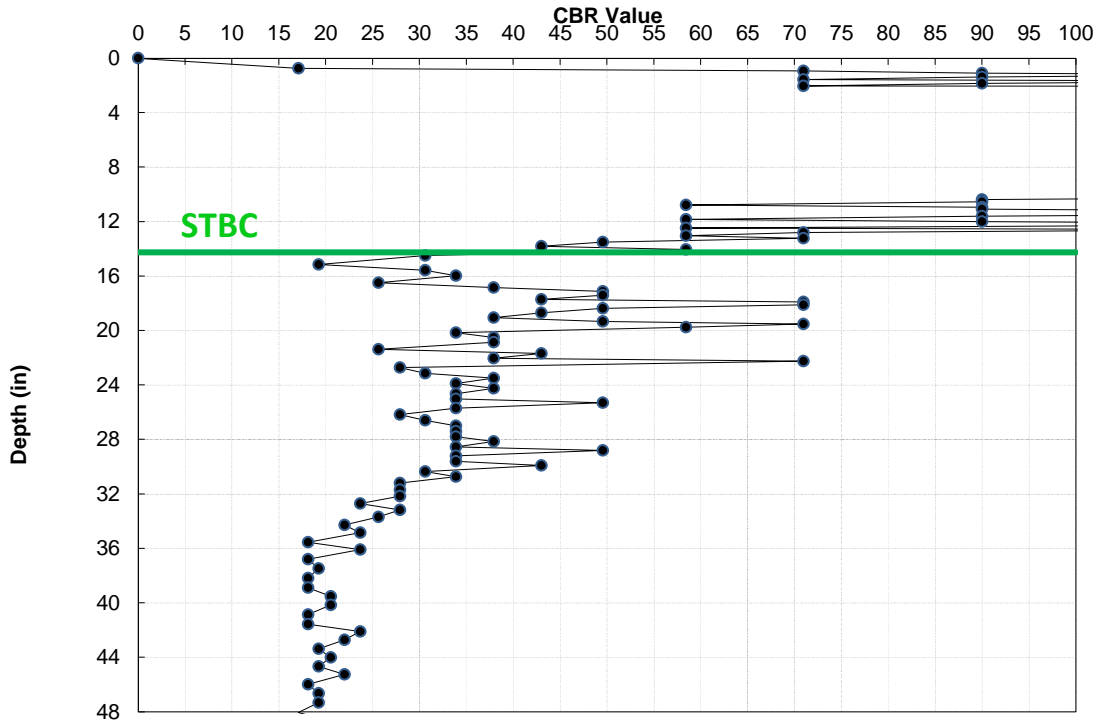
FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-15 -L- 61+64 EB OSS

1.0 FT RT FW
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 14.1
# of Values	122
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	17.1

Interval	
14.1	to 53.5
# of Values	83
Avg CBR	31.8
Wghtd Avg.	27.7
Max CBR	70.9
Min CBR	17.1



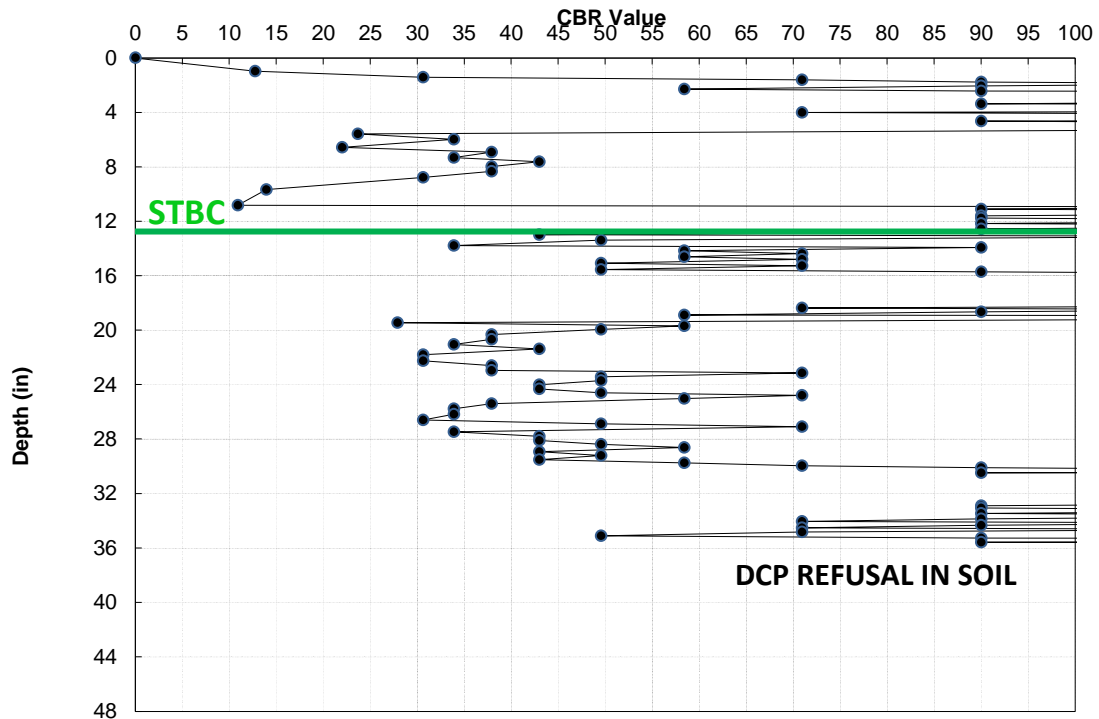
11

C-19 -L- 76+90 EB ISS

2.2 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 12.7
# of Values	58
Avg CBR	100+
Wghtd Avg.	63.5
Max CBR	100+
Min CBR	10.9

Interval	
12.7	to 36.7
# of Values	214
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	27.9



12

**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

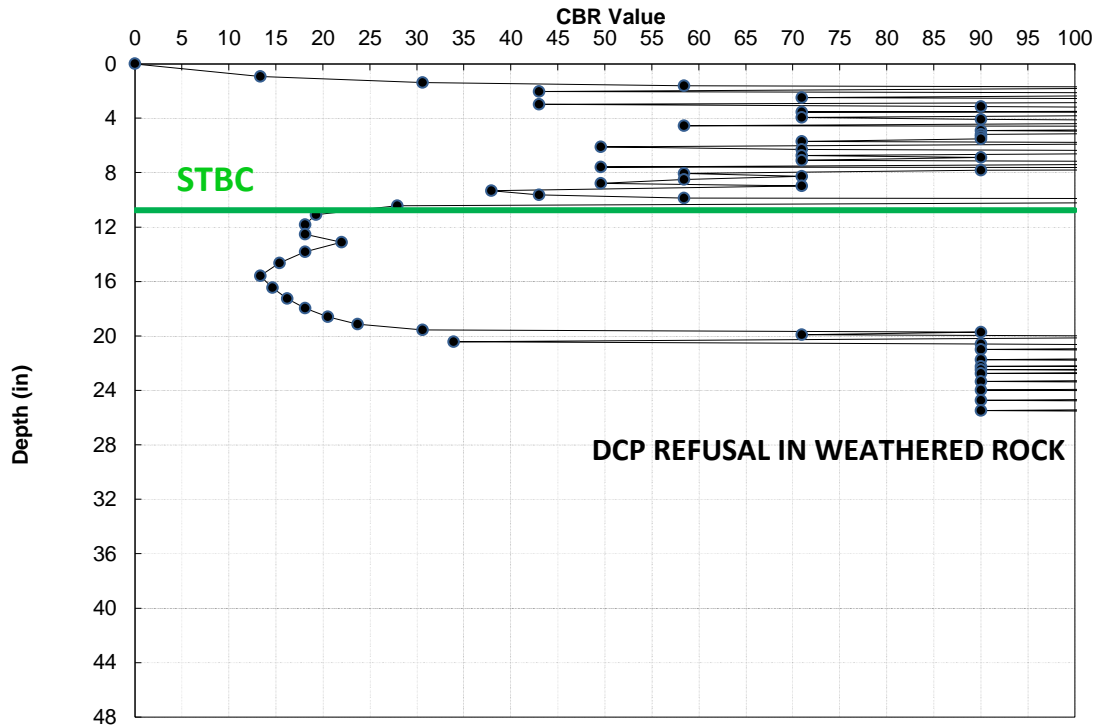
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-21 -L- 76+90 EB OSL
2.2 FT LT FW
Datum = STBC
RAW
Cut
08/04/21

Interval 0.0 to 11.1	
# of Values	54
Avg CBR	93.0
Wghtd Avg.	67.7
Max CBR	100+
Min CBR	13.3

Interval 11.1 to 25.8	
# of Values	59
Avg CBR	95.1
Wghtd Avg.	55.1
Max CBR	100+
Min CBR	13.3

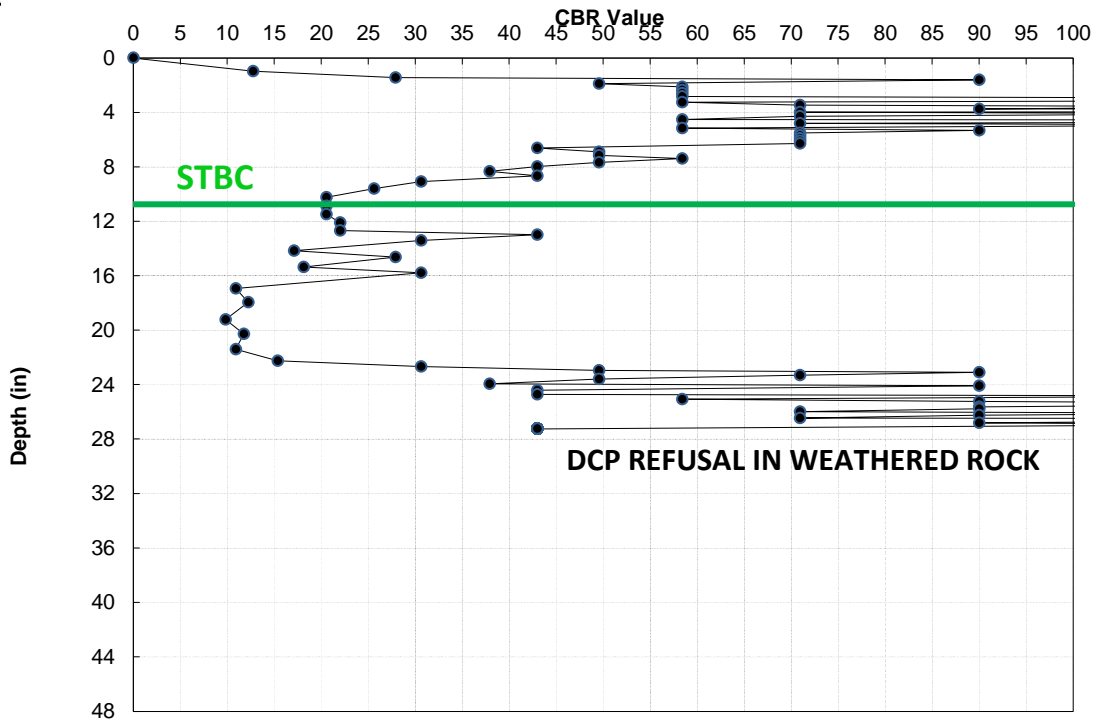


13

C-22 -L- 76+90 EB OSS
3.5 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval 0.0 to 10.9	
# of Values	41
Avg CBR	76.6
Wghtd Avg.	51.7
Max CBR	100+
Min CBR	12.8

Interval 10.9 to 27.2	
# of Values	40
Avg CBR	60.6
Wghtd Avg.	32.5
Max CBR	100+
Min CBR	9.8



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**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

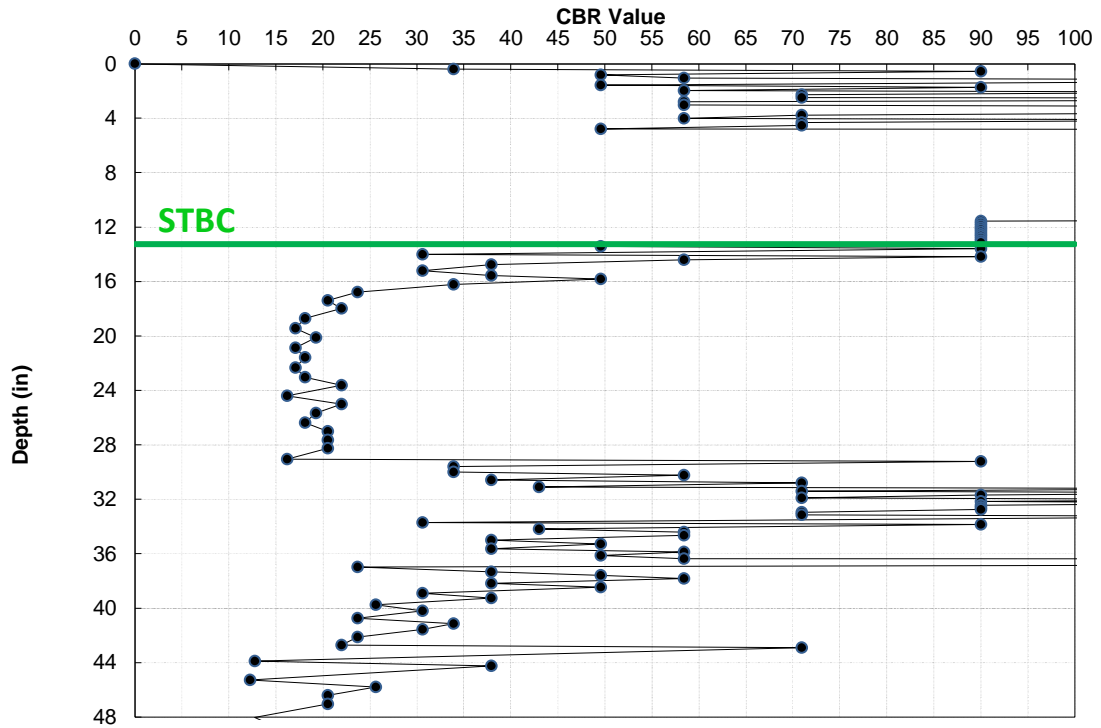
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-23 -L- 80+22 EB ISS
2.3 FT LT FY
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 13.1
# of Values	132
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	33.9

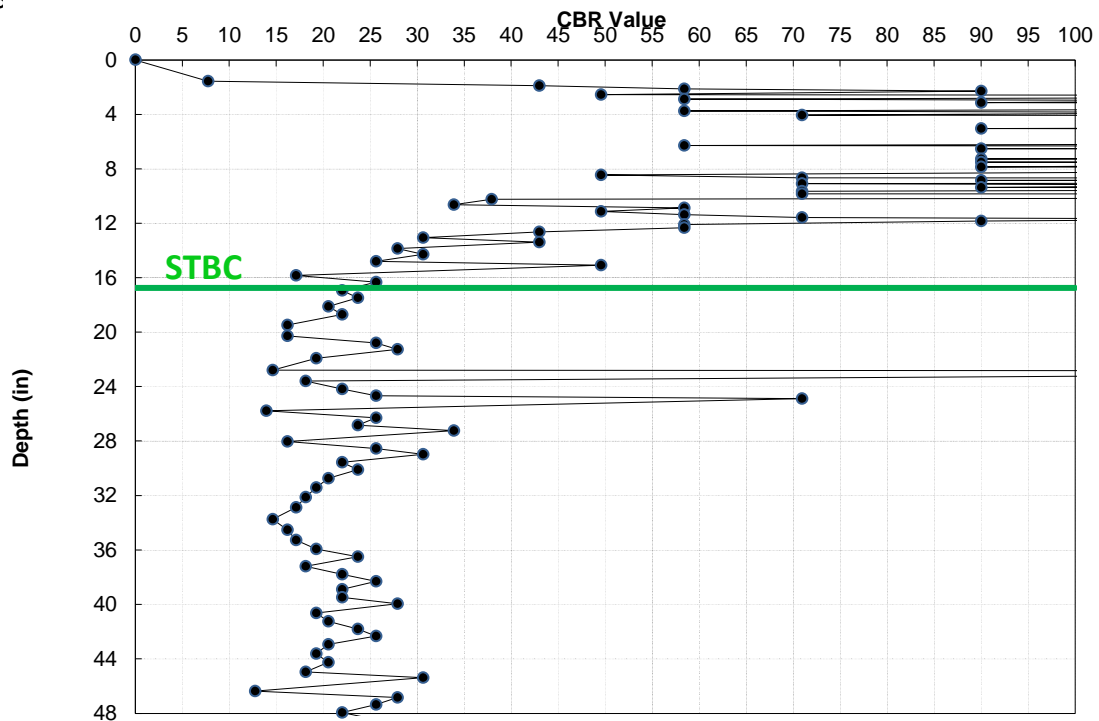
Interval	
13.1	to 60.2
# of Values	106
Avg CBR	45.4
Wghtd Avg.	29.8
Max CBR	100+
Min CBR	12.2



C-24 -L- 80+22 EB OSS
1.9 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 16.9
# of Values	89
Avg CBR	100+
Wghtd Avg.	73.6
Max CBR	100+
Min CBR	7.7

Interval	
16.9	to 56.2
# of Values	68
Avg CBR	26.2
Wghtd Avg.	22.5
Max CBR	100+
Min CBR	12.8



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

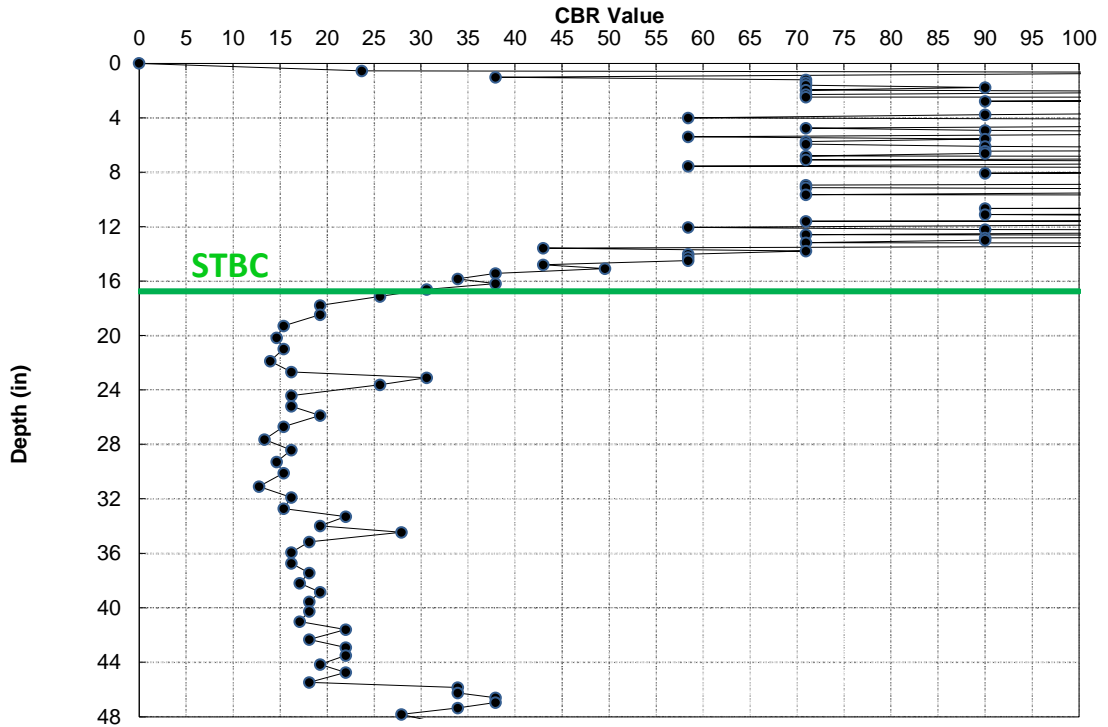
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-25 -L- 15+13 WB OSS
5.5 FT RT FW
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 16.6
# of Values	109
Avg CBR	100+
Wghtd Avg.	93.2
Max CBR	100+
Min CBR	23.7

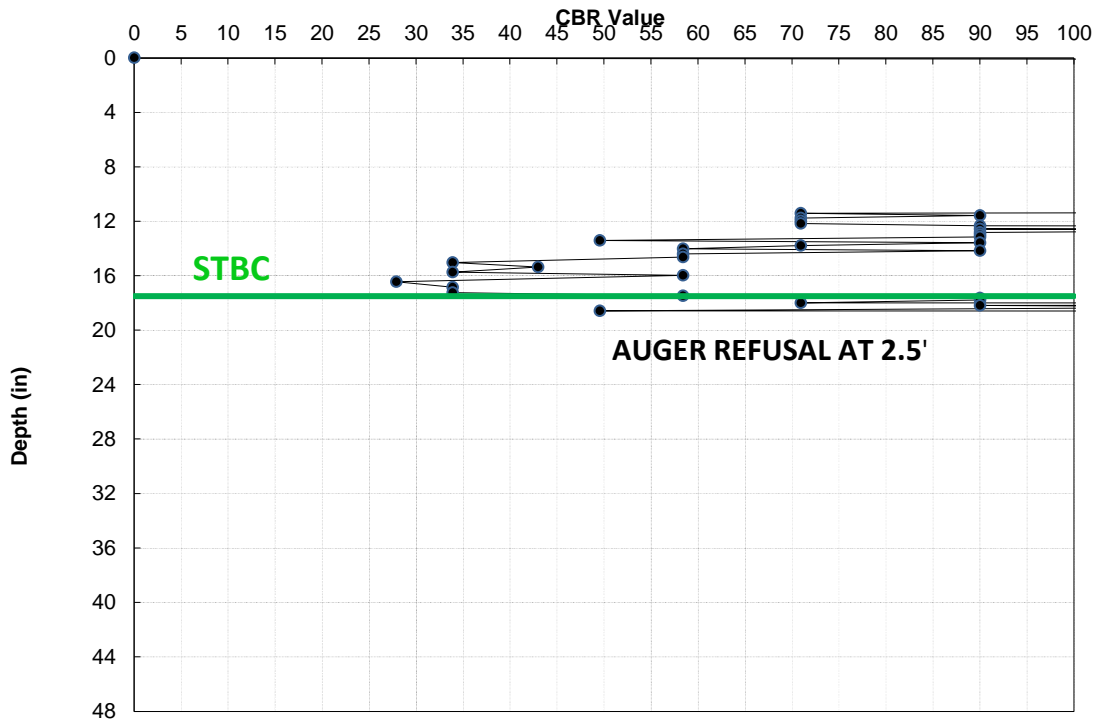
Interval	
16.6	to 62.4
# of Values	125
Avg CBR	86.0
Wghtd Avg.	36.6
Max CBR	100+
Min CBR	12.8



C-26 -L- 20+09 WB OSS
1.2 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 17.5
# of Values	167
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	27.9

Interval	
17.5	to 20.4
# of Values	57
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	49.6



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

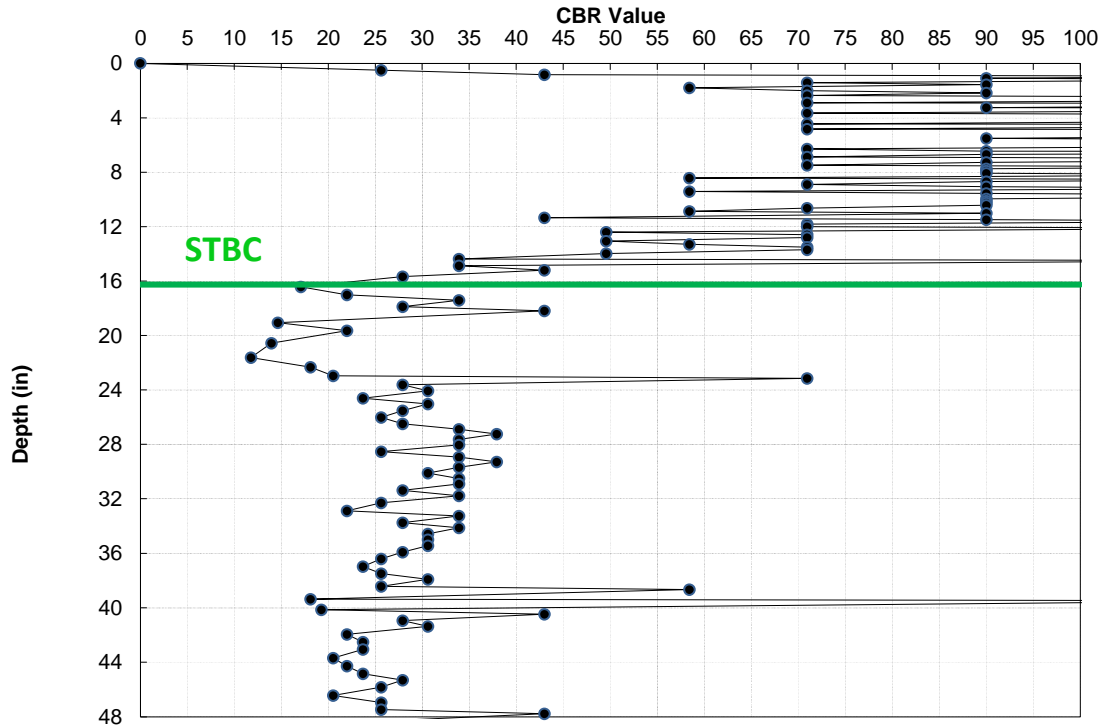
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-28 -L- 20+09 WB ISS
2.4 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 16.4
# of Values	92
Avg CBR	96.0
Wghtd Avg.	78.8
Max CBR	100+
Min CBR	17.1

Interval	
16.4	to 60.7
# of Values	89
Avg CBR	29.1
Wghtd Avg.	26.4
Max CBR	100+
Min CBR	11.8

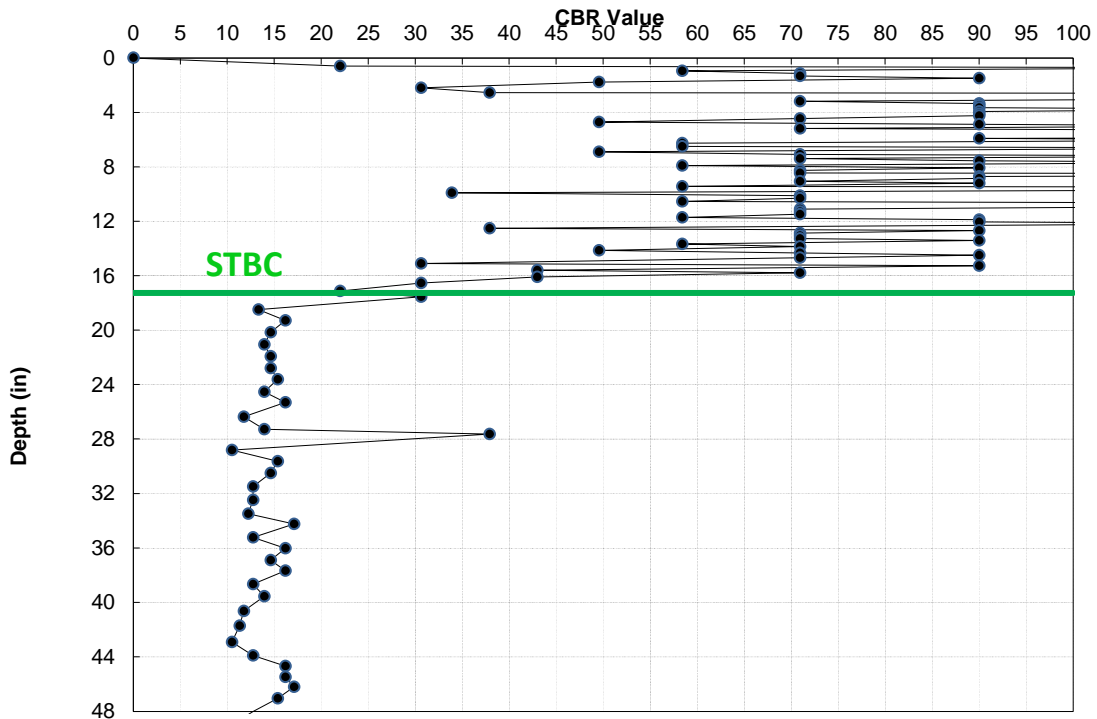


3

C-29 -L- 25+50 WB OSS
5.7 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 17.1
# of Values	86
Avg CBR	85.0
Wghtd Avg.	70.1
Max CBR	100+
Min CBR	22.0

Interval	
17.1	to 60.0
# of Values	49
Avg CBR	15.4
Wghtd Avg.	14.5
Max CBR	37.9
Min CBR	9.5



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**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

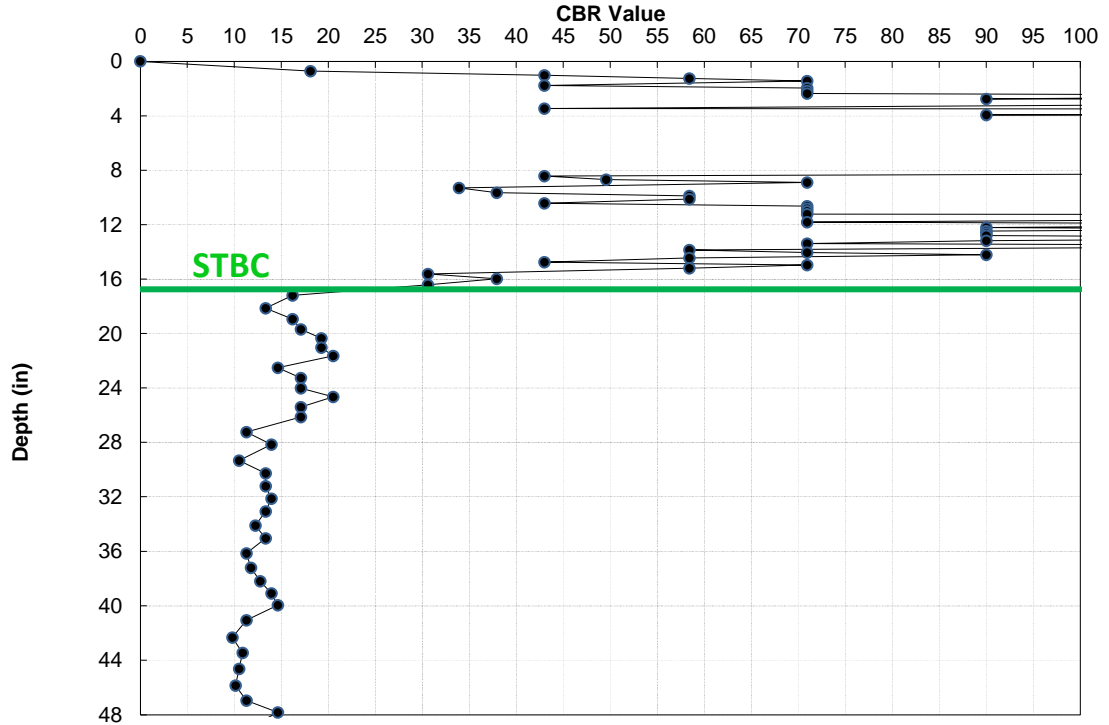
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-30 -L- 25+50 WB ISS
1.8 FT RT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 16.4
# of Values	104
Avg CBR	100+
Wghtd Avg.	89.8
Max CBR	100+
Min CBR	18.1

Interval	
16.4	to 58.4
# of Values	45
Avg CBR	14.0
Wghtd Avg.	13.5
Max CBR	20.5
Min CBR	9.8

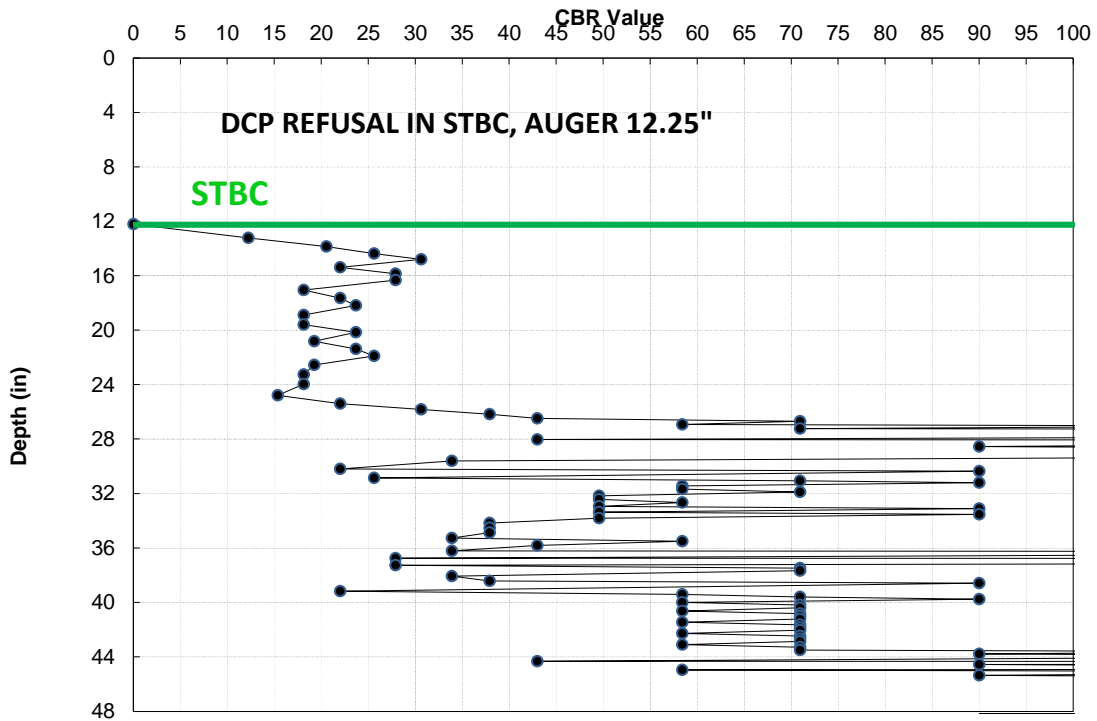


5

C-32 -L- 30+33 WB ISL
5.5 FT RT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
12.2	to 0.0
# of Values	0
Avg CBR	#DIV/0!
Wghtd Avg.	#DIV/0!
Max CBR	0.0
Min CBR	0.0

Interval	
12.2	to 59.1
# of Values	216
Avg CBR	100+
Wghtd Avg.	63.9
Max CBR	100+
Min CBR	12.2



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**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

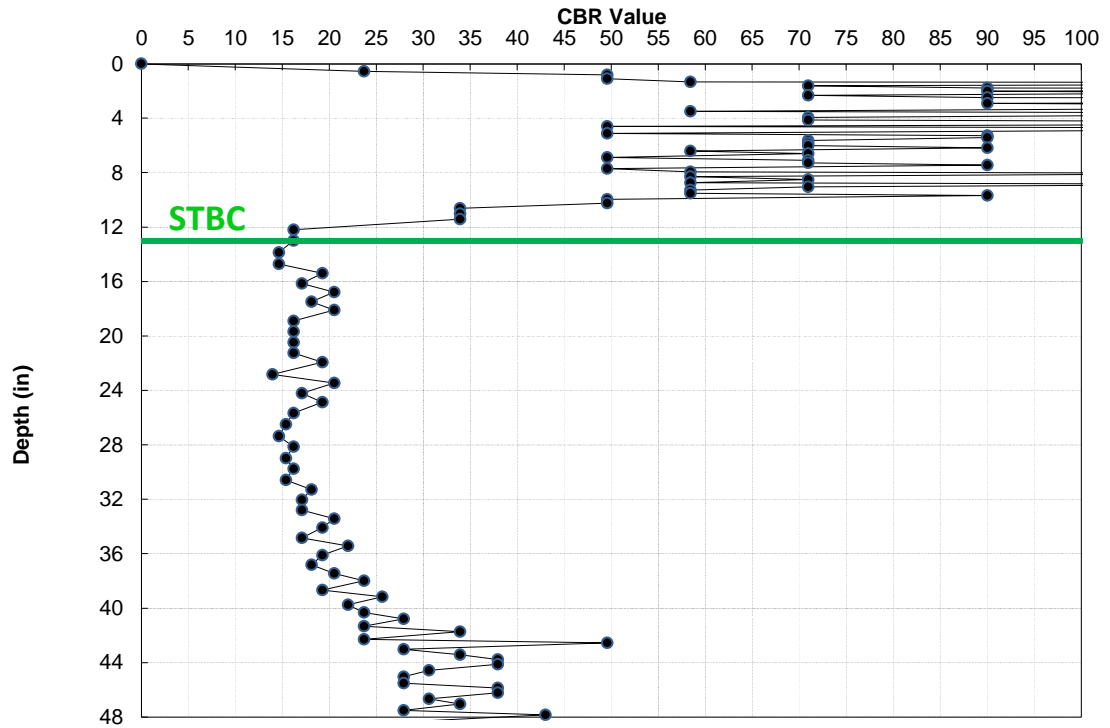
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-33 -L- 30+33 WB ISS
2.0 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 13.0
# of Values	60
Avg CBR	84.9
Wghtd Avg.	64.1
Max CBR	100+
Min CBR	16.2

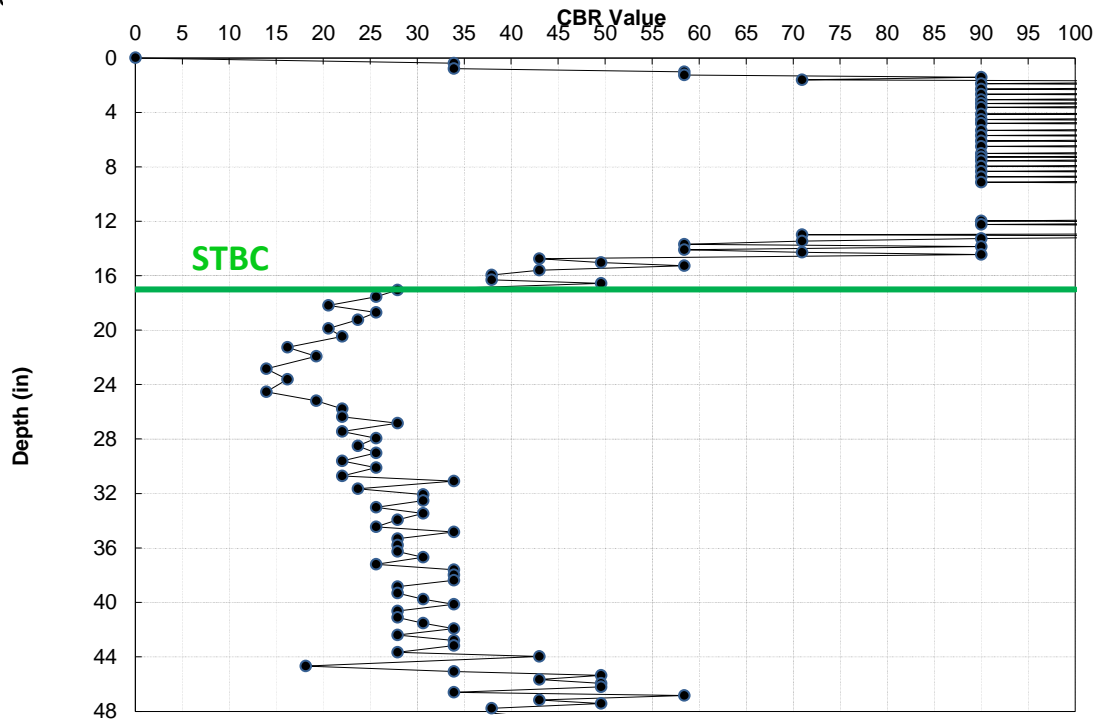
Interval	
13.0	to 59.7
# of Values	84
Avg CBR	26.1
Wghtd Avg.	23.4
Max CBR	49.6
Min CBR	13.9



C-34 -L- 35+34 WB OS^c
6.0 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 17.0
# of Values	122
Avg CBR	84.9
Wghtd Avg.	64.1
Max CBR	100+
Min CBR	16.2

Interval	
17.0	to 63.1
# of Values	111
Avg CBR	35.7
Wghtd Avg.	32.0
Max CBR	70.9
Min CBR	13.9



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

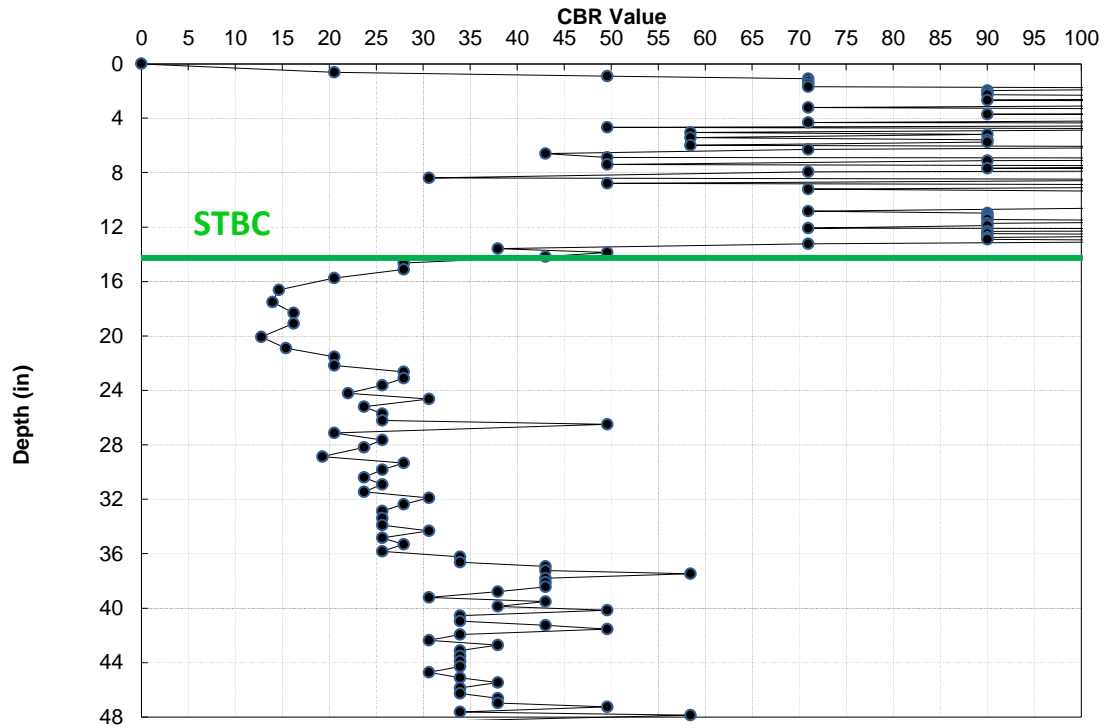
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-35 -L- 35+35 WB ISS
2.3 FT LT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 14.2
# of Values	83
Avg CBR	98.8
Wghtd Avg.	82.6
Max CBR	100+
Min CBR	20.5

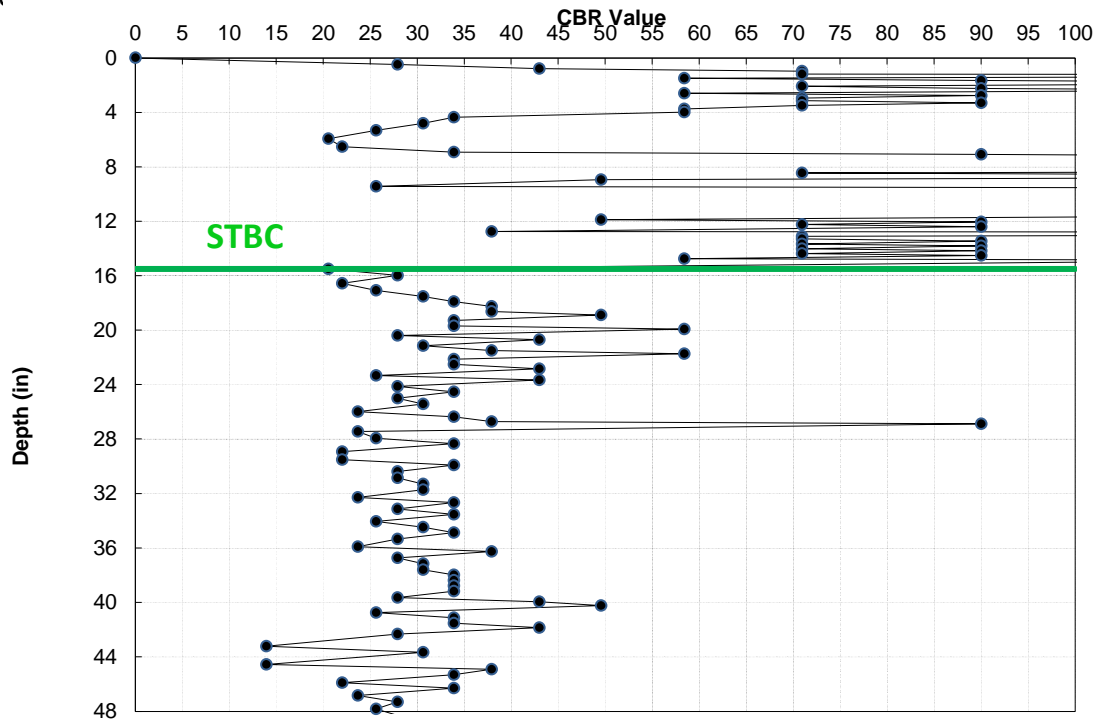
Interval	
14.2	to 55.3
# of Values	100
Avg CBR	39.5
Wghtd Avg.	32.3
Max CBR	100+
Min CBR	12.8



C-36 -L- 56+85 WB OS^c
1.0 FT RT FW
Datum = STBC
RAW
Cut
8/4/2021

Interval	
0.0	to 15.5
# of Values	85
Avg CBR	100+
Wghtd Avg.	76.9
Max CBR	100+
Min CBR	20.5

Interval	
15.5	to 56.1
# of Values	92
Avg CBR	32.2
Wghtd Avg.	30.0
Max CBR	90.0
Min CBR	13.9



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

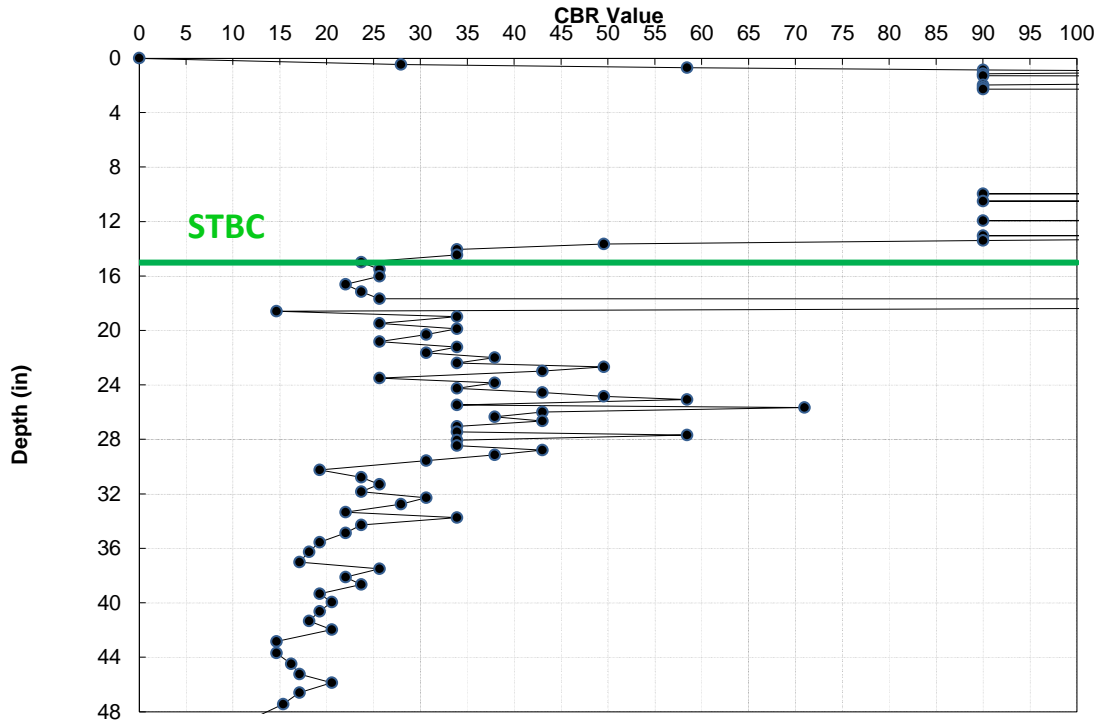
FILE	B-3186_B-5898 DCP Graphs
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C-37 -L- 56+85 WB OSL

2.3 FT LT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 15.0
# of Values	172
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	23.7

Interval	
15.0	to 55.9
# of Values	72
Avg CBR	32.7
Wghtd Avg.	22.9
Max CBR	100+
Min CBR	8.4



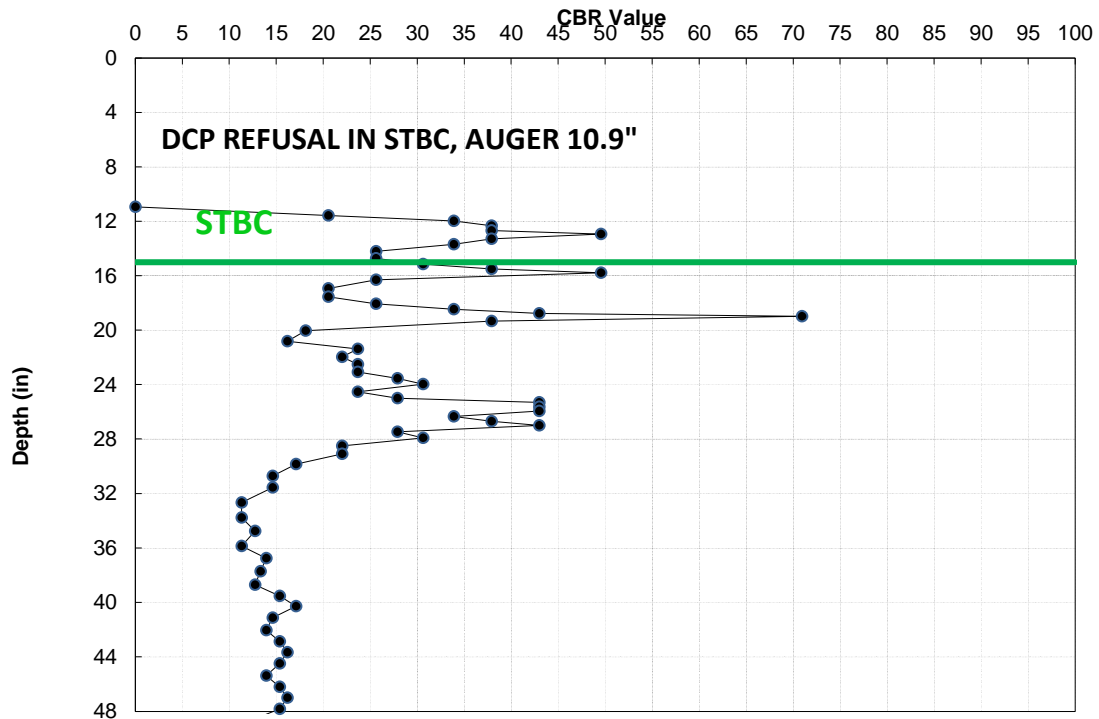
11

C-38 -L- 56+85 WB ISL

2.0 FT RT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
10.9	to 15.2
# of Values	10
Avg CBR	33.3
Wghtd Avg.	31.5
Max CBR	49.6
Min CBR	20.5

Interval	
15.2	to 54.0
# of Values	58
Avg CBR	23.4
Wghtd Avg.	19.2
Max CBR	70.9
Min CBR	11.3



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**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

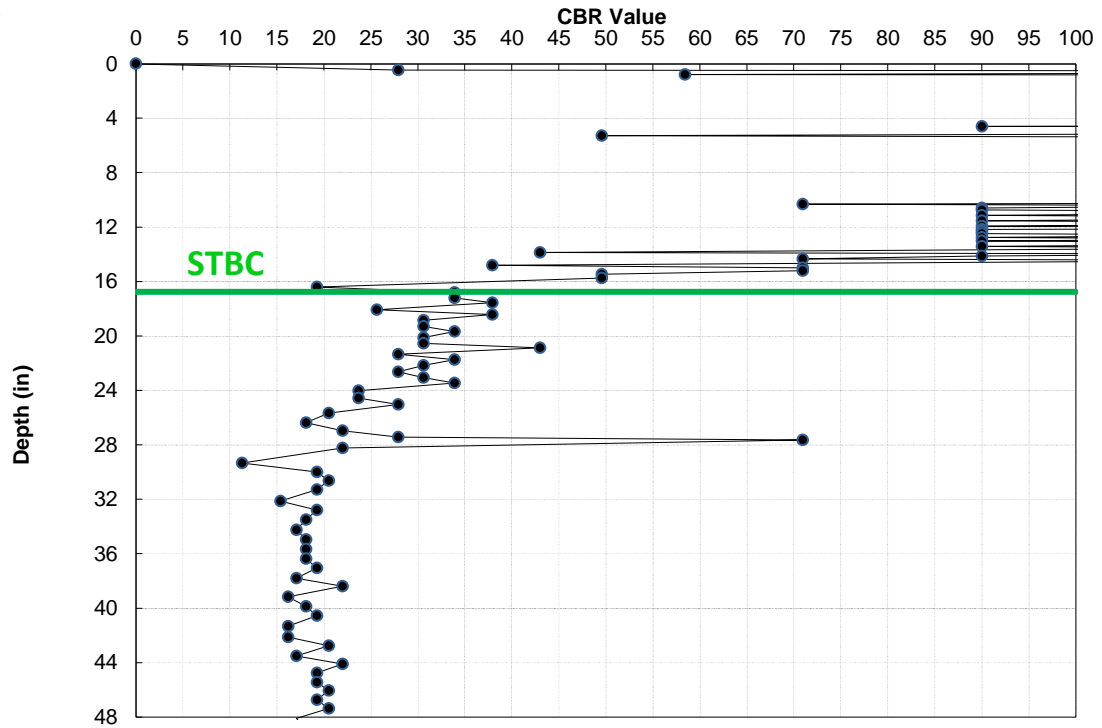
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-39 -L- 65+56 WB OSS
2.0 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 16.8
# of Values	148
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	19.2

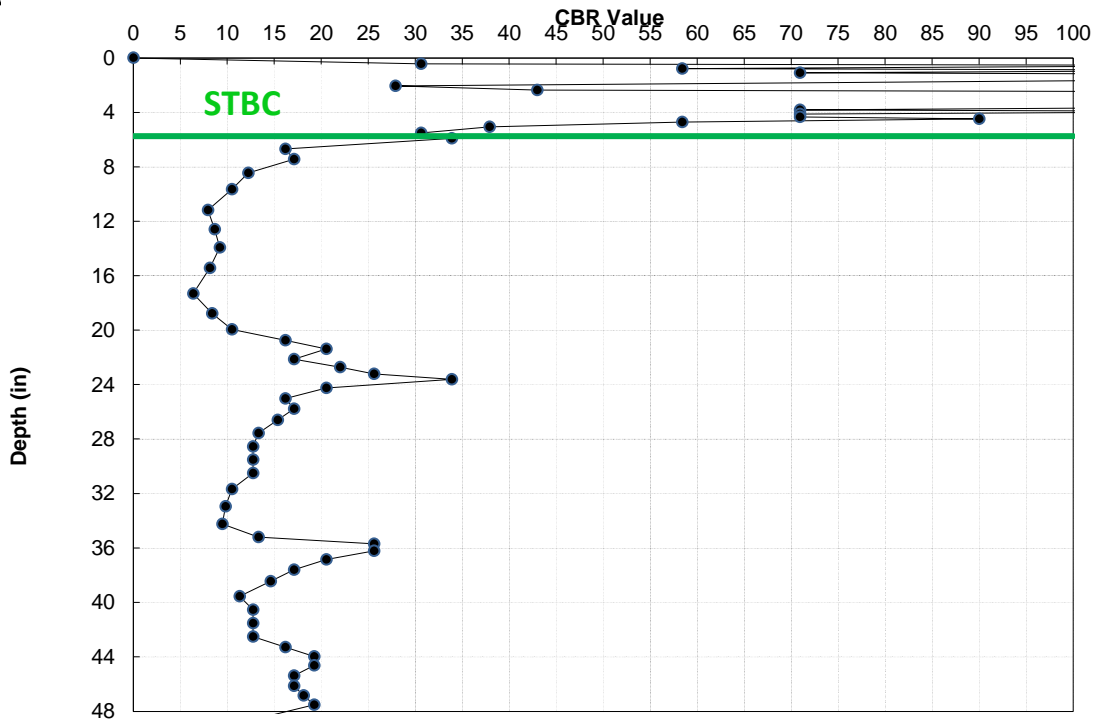
Interval	
16.8	to 59.9
# of Values	69
Avg CBR	22.8
Wghtd Avg.	20.7
Max CBR	70.9
Min CBR	9.5



C-41 -L- 70+40 WB OSS
4.0 FT RT FW
Datum = STBC
RAW
Fill
08/04/21

Interval	
0.0	to 5.9
# of Values	30
Avg CBR	89.7
Wghtd Avg.	70.9
Max CBR	100+
Min CBR	27.9

Interval	
5.9	to 54.7
# of Values	54
Avg CBR	15.5
Wghtd Avg.	14.0
Max CBR	33.9
Min CBR	6.4



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

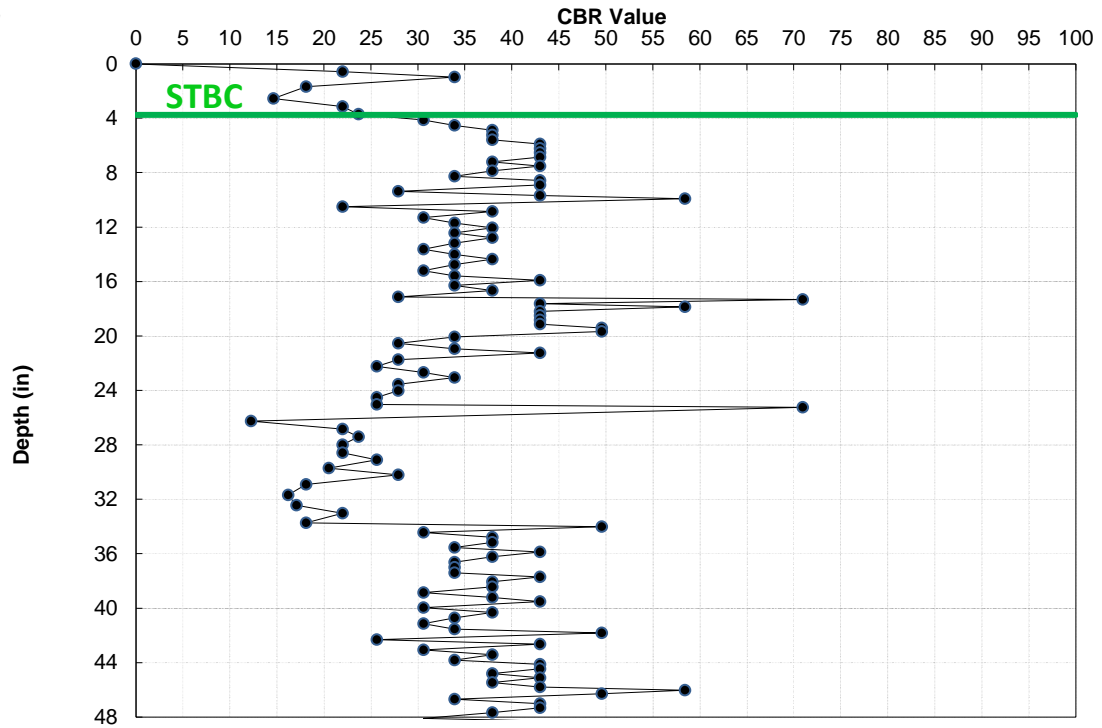
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-42 -L- 75+03 WB OSS
5.0 FT RT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 3.7
# of Values	6
Avg CBR	22.4
Wghtd Avg.	21.0
Max CBR	33.9
Min CBR	14.6

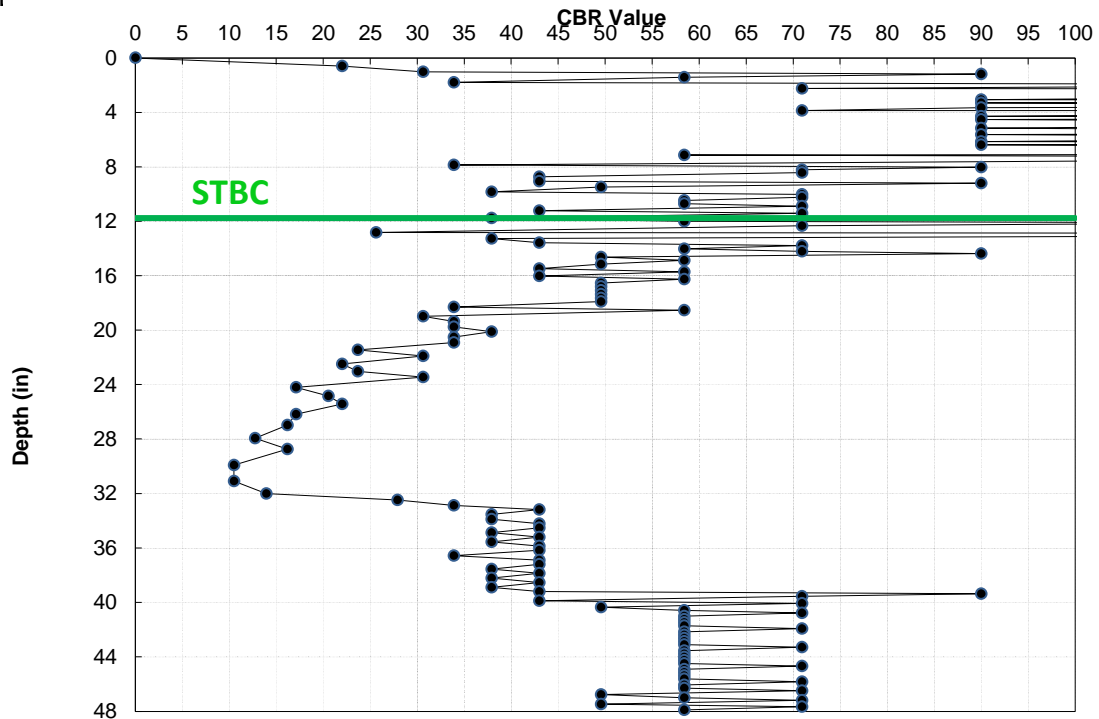
Interval	
3.7	to 53.1
# of Values	133
Avg CBR	43.5
Wghtd Avg.	36.0
Max CBR	100+
Min CBR	12.2



C-43 -L- 75+03 WB OSI
8.0 FT LT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 11.8
# of Values	68
Avg CBR	100+
Wghtd Avg.	81.4
Max CBR	100+
Min CBR	22.0

Interval	
11.8	to 53.0
# of Values	128
Avg CBR	51.8
Wghtd Avg.	42.0
Max CBR	100+
Min CBR	10.5



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

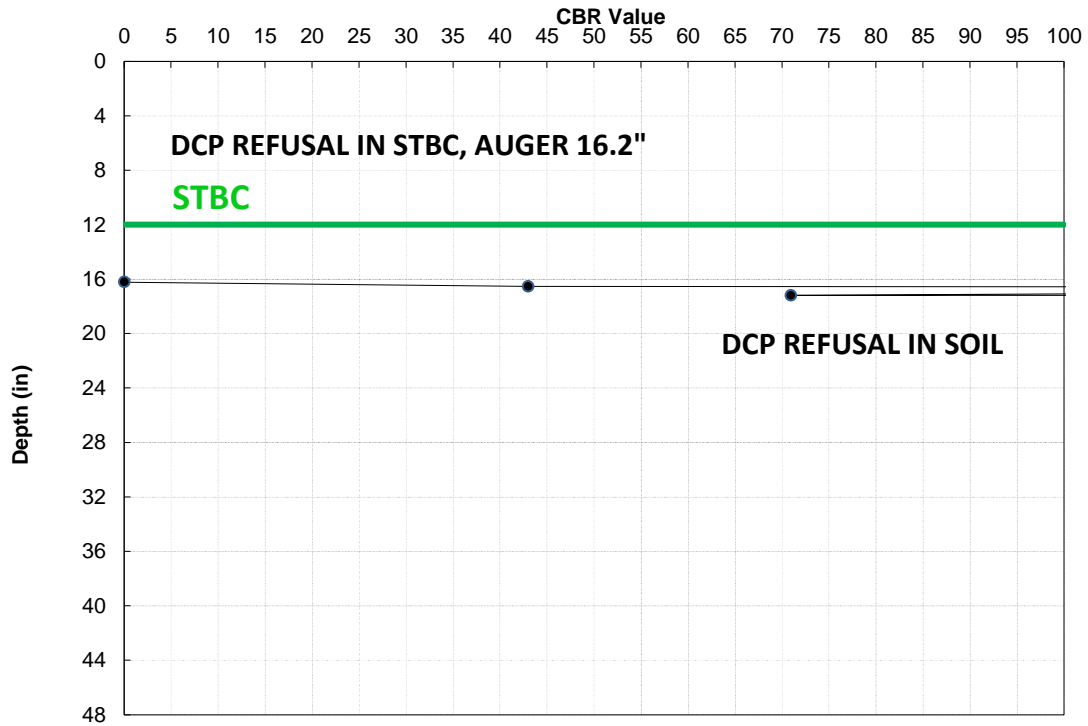
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
------	--------------------------

C-44 -L- 75+03 WB ISL
5.5 FT RT FY
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 0.0
# of Values	0
Avg CBR	#DIV/0!
Wghtd Avg.	#DIV/0!
Max CBR	0.0
Min CBR	0.0

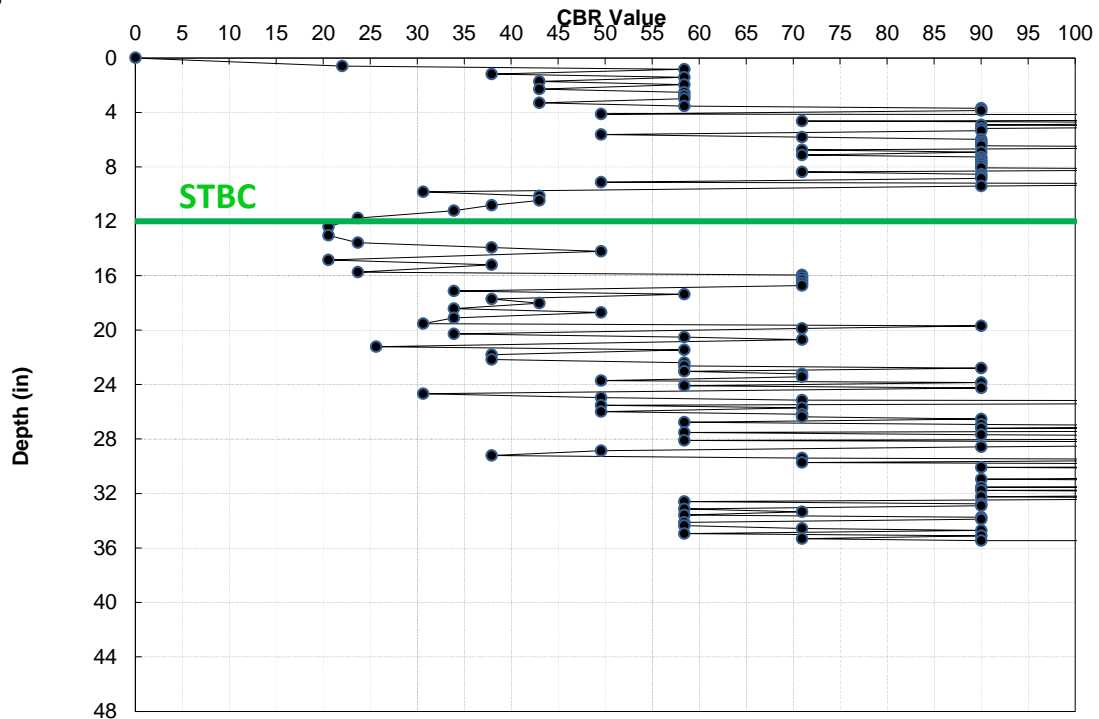
Interval	
16.2	to 18.7
# of Values	71
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	43.0



C-45 -L- 75+03 WB ISS
2.5 FT LT FW
Datum = STBC
RAW
Cut
08/04/21

Interval	
0.0	to 11.8
# of Values	55
Avg CBR	81.4
Wghtd Avg.	64.9
Max CBR	100+
Min CBR	22.0

Interval	
11.8	to 36.5
# of Values	167
Avg CBR	100+
Wghtd Avg.	96.3
Max CBR	100+
Min CBR	20.5



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

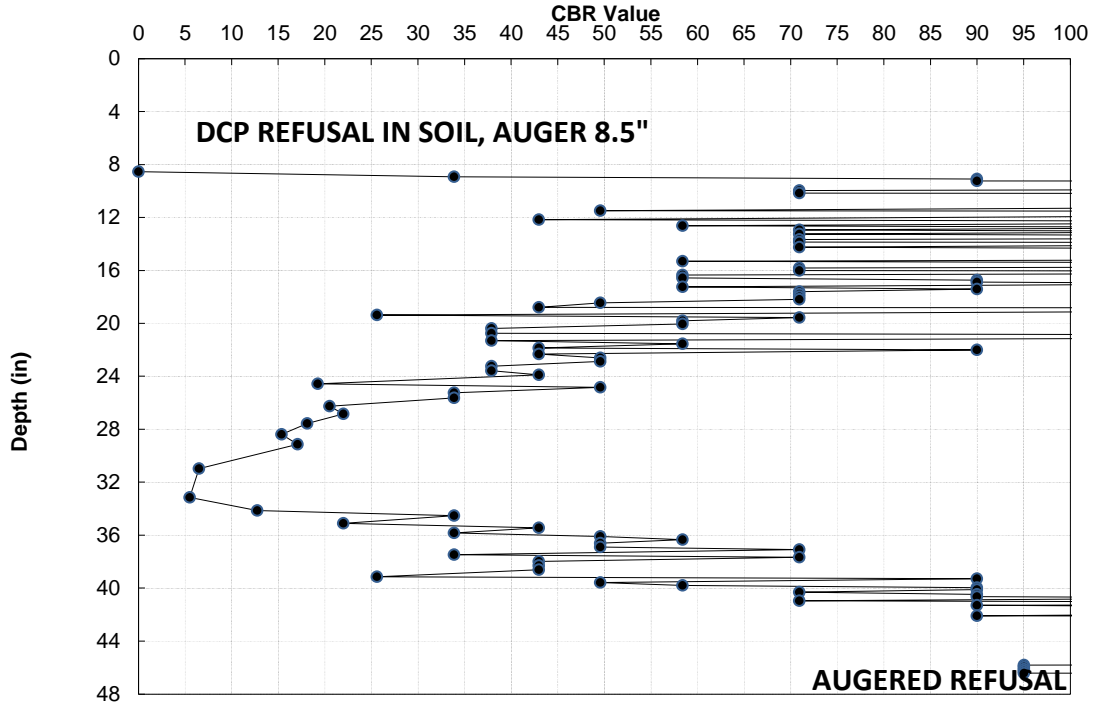
GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-47 -Y1LT- 13+66 US 19 WB RTL
6.0 FT RT C&G
Datum = SG
RAW
Cut
08/06/21

Interval	
0.0	to 0.0
# of Values	0
Avg CBR	#DIV/0!
Wghtd Avg.	#DIV/0!
Max CBR	0.0
Min CBR	0.0

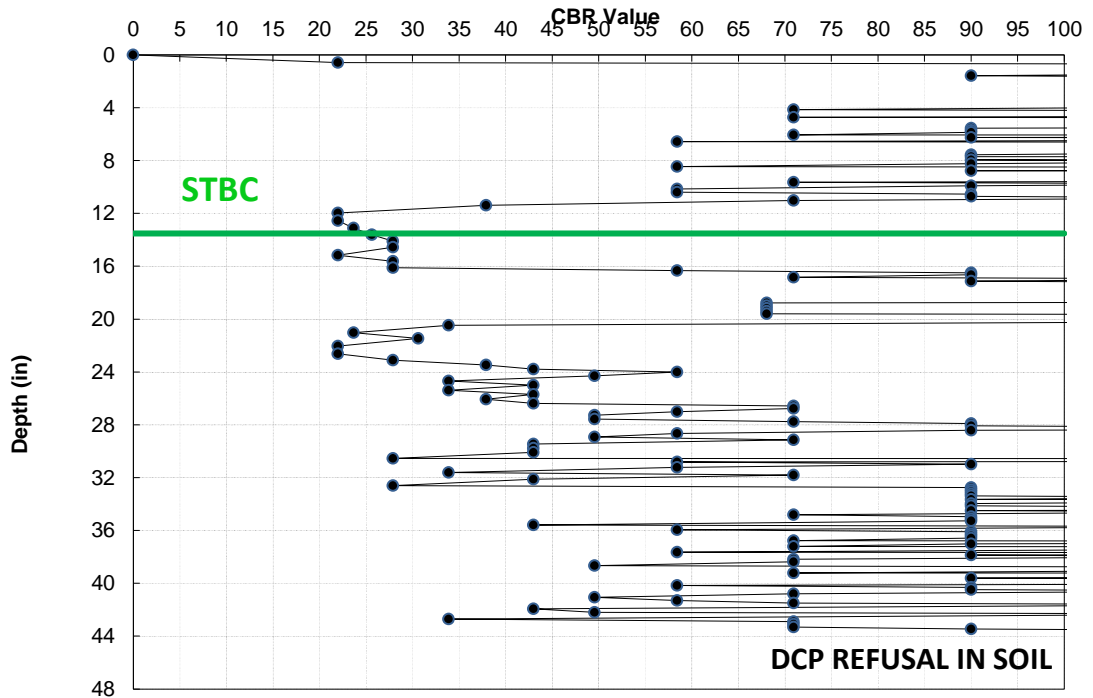
Interval	
8.5	to 52.0
# of Values	245
Avg CBR	100+
Wghtd Avg.	79.2
Max CBR	100+
Min CBR	5.5



C-48 -Y1LT- 13+66 US 19 WB OSL
15.5 FT LT C&G (RT)
Datum = STBC
RAW
Cut
08/06/21

Interval	
0.0	to 13.6
# of Values	101
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	22.0

Interval	
13.6	to 45.8
# of Values	204
Avg CBR	100+
Wghtd Avg.	89.9
Max CBR	100+
Min CBR	22.0



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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C-49 -Y1LT- 13+67 US 19 WB ISL

2.8 FT RT C&G (FACE)

Datum = STBC

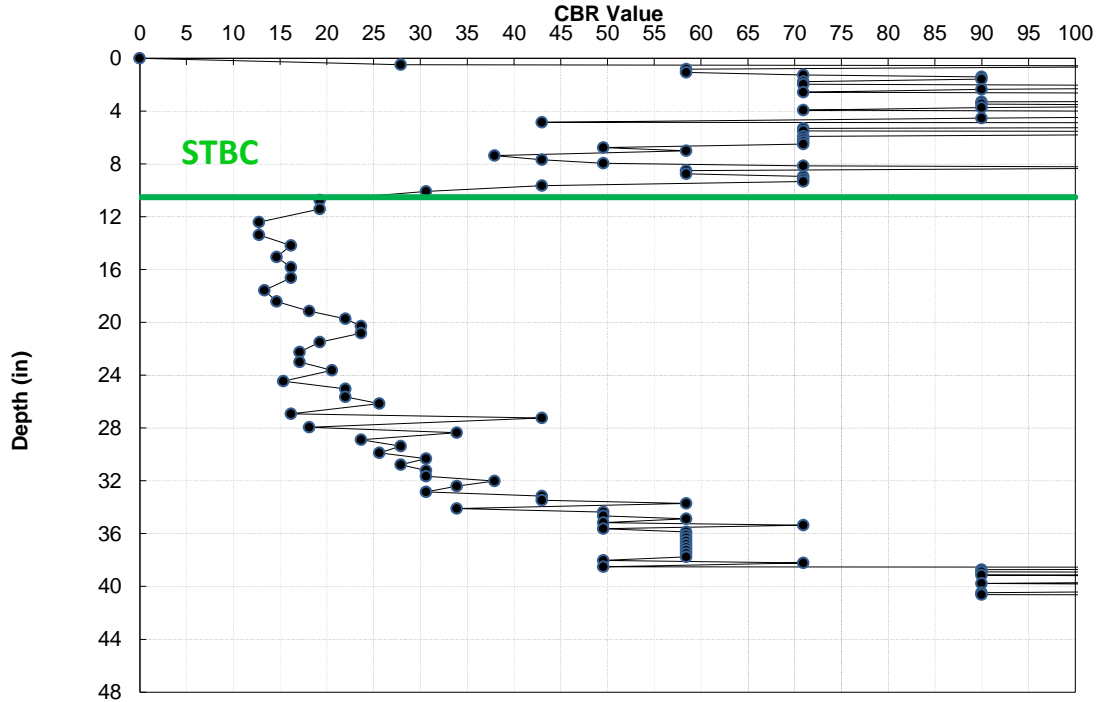
RAW

Fill

08/06/21

Interval	
0.0	to 10.7
# of Values	56
Avg CBR	93.5
Wghtd Avg.	72.9
Max CBR	100+
Min CBR	19.2

Interval	
10.7	to 45.3
# of Values	135
Avg CBR	100+
Wghtd Avg.	53.6
Max CBR	100+
Min CBR	12.8



C-50 -Y1RT- 13+67 US 19 EB ISL

1.0 FT RT FY

Datum = SG

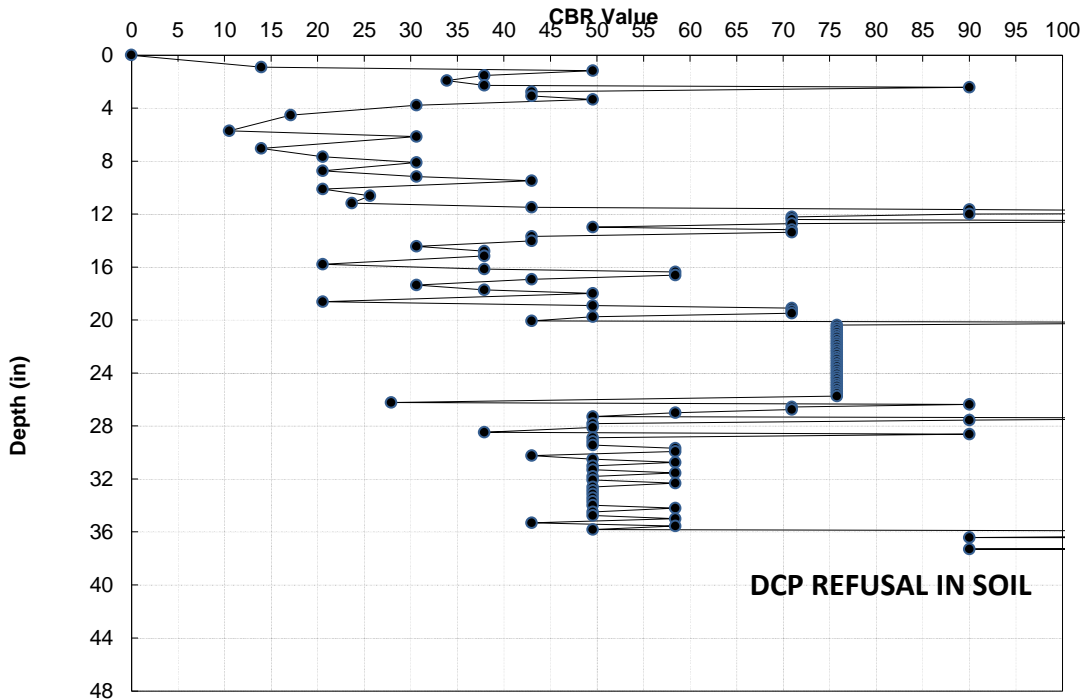
RAW

Fill

08/06/21

Interval	
0.0	to 0.0
# of Values	0
Avg CBR	#DIV/0!
Wghtd Avg.	#DIV/0!
Max CBR	0.0
Min CBR	0.0

Interval	
0.0	to 42.1
# of Values	245
Avg CBR	100+
Wghtd Avg.	81.9
Max CBR	100+
Min CBR	10.5



**CONE PENETROMETER RESULTS
NCDOT, GEOTECHNICAL ENGINEERING UNIT**

PROJECT NO.	38332.1.FS1
PROJECT ID	B-3186_B-5898
ROUTE	US 23-74
COUNTY	HAYWOOD

GEOLOGIST	J. B. BARFIELD
GEOTECHS	CG2

FILE	B-3186_B-5898 DCP Graphs
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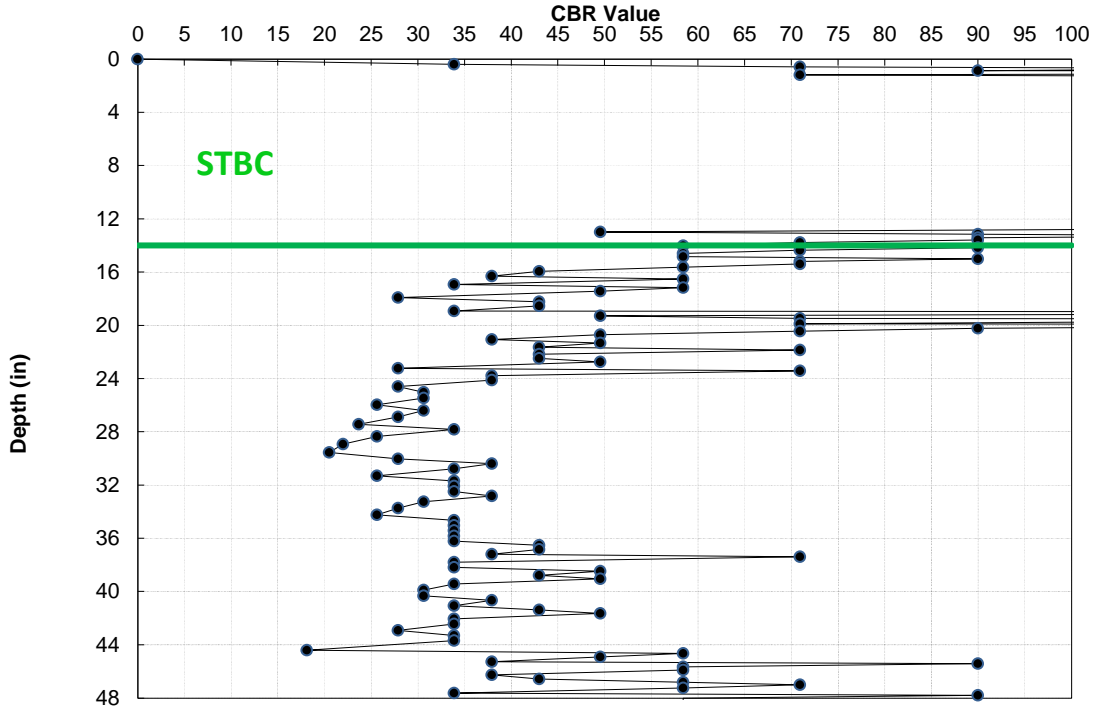
C-51 -Y1RT- 14+30 US 19 EB OSL

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1.5 FT LT FW
Datum = STBC
RAW
Fill
08/06/21

Interval	
0.0	to 14.0
# of Values	293
Avg CBR	100+
Wghtd Avg.	100+
Max CBR	100+
Min CBR	33.9

Interval	
14.0	to 55.2
# of Values	142
Avg CBR	62.5
Wghtd Avg.	46.9
Max CBR	100+
Min CBR	18.1



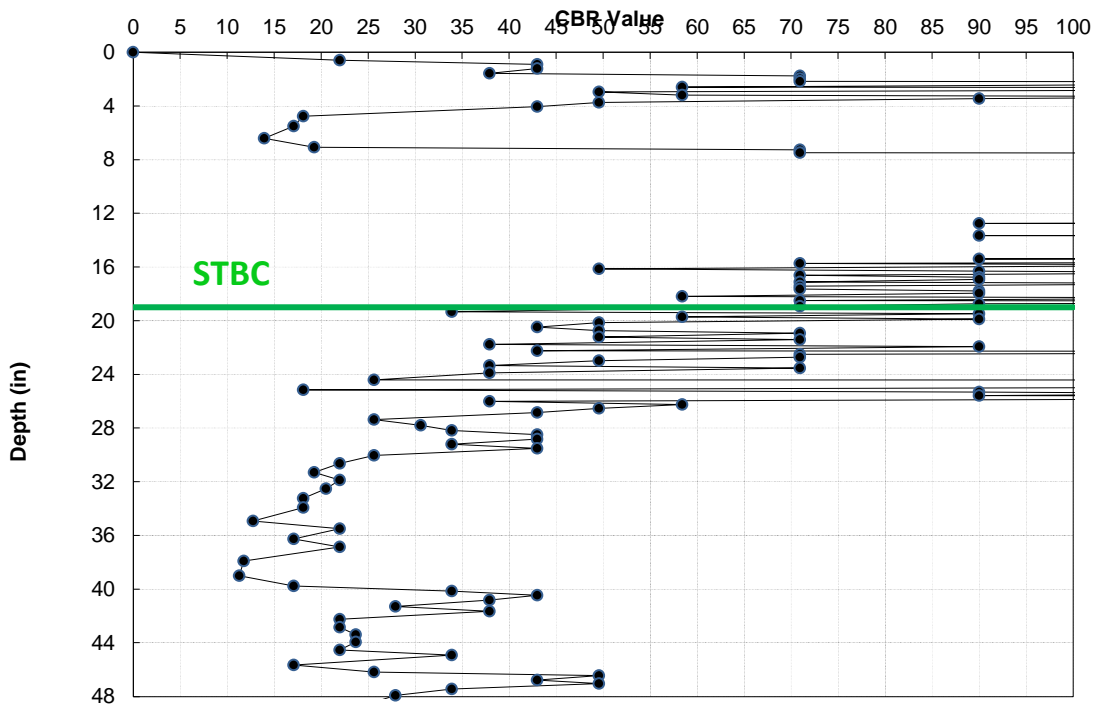
C-52 -Y1RT- 14+30 US 19 EB OSS

6

1.8 FT RT FW
Datum = STBC
RAW
Fill
08/06/21

Interval	
0.0	to 18.9
# of Values	128
Avg CBR	100+
Wghtd Avg.	96.2
Max CBR	100+
Min CBR	13.9

Interval	
18.9	to 60.9
# of Values	100
Avg CBR	46.9
Wghtd Avg.	31.6
Max CBR	100+
Min CBR	11.3



PAVEMENT CORE EVALUATION
38332.1.FS1 (B-3186, B-5898) Haywood County

LINE	STATION	ABC (in)	LAYER THICKNESS (in)	LAYER	LIFT(S)	REMARKS
-L-	15+32 EB OSS	-	4.50	S	3	lift 2 delaminated from lift 3 (mechanical break)
	Asphalt 5.25"		0.75	B	1	delaminated from upper lift
-L-	25+35 EB ISS	-	4.00	S	4	low severity oxidation in lift 1
	Asphalt 4"		0.50	B	1	
-L-	25+35 EB OSS	-	4.50	S	4	
	Asphalt 5"		0.50	B	1	aggregate missing, weathered, some pieces missing (possibly mechanical)
-L-	29+86 EB ISS	-	5.00	S	5	last 2 lifts, highly oxidized, delaminated, rubble, bottom up crack from lift 5 to lift 2
	Asphalt 5.50"					
-L-	29+86 EB ISL	-	7.00	S	5	
	Asphalt 9.75"		2.75	B	1	
-L-	35+32 EB ISS	-	4.50	S	2	top down crack through full core, roots through core, lifts delaminated
	Asphalt 5.25"					
-L-	35+34 EB OSS	-	4.50	S	4	full depth top down crack, initial signs of delamination between lifts 2 & 3, missing aggregate at base
	Asphalt 4.5"					
-L-	61+63 EB ISS	-	3.25	S	2	full depth crack
	Asphalt 3.25"		6.25	C	1	old concrete, moderate to high severity stripping, some large aggregate sections missing
-L-	61+63 EB ISL	-	5.00	S	4	6.5" top down crack, delaminated between lift 2 & 3, lift 3 & 4
	Asphalt 8.75"		3.50	B	1	
-L-	61+64 EB OSL	-	1.50	S	1	
	Asphalt 9"		3.00	I	1	
			1.50	S	1	low severity oxidation in lift 1
			3.00	I	1	
-L-	61+64 EB OSS	-	1.25	S	1	
	Asphalt 9.75"		3.00	I	1	
			5.00	B	1	
-L-	76+90 EB ISS	-	4.00	S	4	
	Asphalt 8.50"		4.50	B	1	mechanical break, missing aggregate at base
-L-	76+90 EB OSL	-	2.75	S	2	
	Asphalt 9.75"		7.00	B	2	low severity stripping with some small voids
-L-	76+90 EB OSS	-	5.00	S	4	
	Asphalt 8.25"		3.25	I	1	very low severity stripping with few small voids
-L-	80+20 EB ISS	-	4.50	S	4	low stripping between lifts 2 & 3
	Asphalt 5"		0.75	B	1	
-L-	80+20 EB OSS	-	3.75	S	3	low stripping in lift 1
	Asphalt 8.75"		3.00	I	2	
			1.75	S	1	some pieces of aggregate missing
-L-	15+13 WB OSS	-	4.25	S	4	
	Asphalt 5.25"		0.75	B	1	low severity stripping with some medium to small voids

PAVEMENT CORE EVALUATION
38332.1.FS1 (B-3186, B-5898) Haywood County

LINE	STATION	ABC (in)	LAYER THICKNESS (in)	LAYER	LIFT(S)	REMARKS
-L-	20+09 WB OSS	-	1.50	S	1	
	Asphalt 5.50"		3.50	I	1	bottom 1.5" mechanical break, low to moderate severity stripping with some small voids
-L-	20+09 WB ISS	-	4.75	S	3	low severity stripping with some small voids between lift 2 & 3
	Asphalt 5.75"		0.75	B	1	low severity stripping with some small voids
-L-	25+50 WB OSS	-	4.00	S	3	full-depth crack, half of lift 3 delaminated from lift 2, bottom lift of core rubble
	Asphalt 4.75"					
-L-	25+50 WB ISS	-	6.00	S	4	full-depth crack with roots, on one side of crack lift 1 delaminated, lift 2 partially delaminated, lift 4 rubble & delaminated, 5 delaminated, highly stripped
	Asphalt 6.25"					
-L-	30+33 WB ISL	-	6.00	S	4	mechanical break between lift 2 & 3
	Asphalt 8.75"		2.50	B	1	low severity stripping with some small voids
-L-	30+33 WB ISS	-	6.00	S	4	low severity stripping with few small voids between lift 2 & 3
	Asphalt 7"		1.00	B	1	low severity stripping with some small voids
-L-	35+34 WB OSS	-	4.00	S	3	
	Asphalt 5"		1.00	B	1	
-L-	35+35 WB ISS	-	7.25	S	5	low severity stripping and oxidation on top and bottom border of lift 2
	Asphalt 8.75"		1.25	B	1	low to moderate severity stripping with many medium to small voids
-L-	56+85 WB OSS	-	6.75	S	3	2" top down crack through lift 1, lift 2 delaminated, lift 3 highly oxidized
	Asphalt 6.5"					
-L-	56+85 WB OSL	-	1.25	S	1	
	Asphalt 8"		2.50	I	1	
			1.00	S	1	
			3.00	I	1	
-L-	56+85 WB ISL	-	6.00	S	3	full-depth crack, half of lift 1 delaminated, lift 3 delaminated from lift 2, lift 3 oxidized
	Asphalt 8.5"		3.00	I	1	
-L-	65+56 WB OSS	-	1.50	S	1	
	Asphalt 5.25"		3.25	I	1	last 1" low to moderate severity stripping with some small voids
-L-	70+40 WB OSS	-	5.50	S	3-4	lifts indistinguishable
	Asphalt 14.25"		8.75	B	1	
-L-	75+03 WB OSS	-	1.50	S	1	
	Asphalt 10.25"		3.50	I	1	
			5.25	B	1	
-L-	75+03 OSL	-	5.75	S	4	moderate oxidation, low severity bleeding, full-depth crack, lift 1 delaminated, part of lift 2 rubble
	Asphalt 9.25"		3.50	B	1	moderate oxidation, low severity bleeding
-L-	75+03 WB ISL	-	5.50	S	3	
	Asphalt 9"		3.50	I	1	
-L-	75+03 WB ISS	-	2.25	S	2	full-depth crack
	Asphalt 9"		2.25	I	1	intermediate delaminated from base (possible mechanical break)
			4.00	B	1	some larger chunks of aggregate missing in bottom 1"

PAVEMENT CORE EVALUATION
38332.1.FS1 (B-3186, B-5898) Haywood County

LINE	STATION	ABC (in)	LAYER THICKNESS (in)	LAYER	LIFT(S)	REMARKS
-Y1LT-	13+66 US19 WB RTL	-	3.50	S	3-4	lifts indistinguishable, low severity stripping with some small voids
	Asphalt 7.25"		3.50	B	1	low severity stripping, few small voids
-Y1LT-	13+66 US19 WB OSL	-	5.00	S	4	
	Asphalt 8.5"		3.50	I	1	
-Y1LT-	13+67 US19 WB ISL	-		S		
	Asphalt 11.5"					
-Y1RT-	13+67 US19 EB ISL	-	6.00	S	4	some intermediate mix wedging in lift 3, low severity stripping, few small voids
	Asphalt 12.5"		6.00	I/B	1	delaminated from surface
-Y1RT-	14+30 US19 EB OSL	-	4.50	S	4	low severity oxidation in lift 1, delaminated
	Asphalt 7"		2.75	I	1	
-Y1RT-	14+30 US19 EB OSS	-	3.00	S	2	
	Asphalt 4"		1.00	I	1	low to moderate severity stripping with some small voids

REFERENCE: B-3186/B-5898

PROJECT: 38332/48030

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3816/B-5898	1	66

CONTENTS

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>PROFILE</u>
-L-	20+00.00-81+35.95	4-8	10-13
-YIRT-	10+00.00-44+31.29	6, 7 & 9	14
-YILT-	10+00.00-33+16.69	6 & 9	

CROSS SECTIONS

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	20+00.00-81+00.00	15-51
-YIRT-	12+50.00-26+00.46	52-63

APPENDICES

<u>TITLE</u>	<u>SHEETS</u>
BORE LOGS	64
SOIL TEST RESULTS	65-66

ROADWAY SUBSURFACE INVESTIGATION

COUNTY HAYWOOD
PROJECT DESCRIPTION US 23/US 74/US 19 (GREAT
SMOKY MOUNTAIN HWY) FROM WEST OF NC
209 (CRABTREE RD.) TO EAST OF RUSS AVE.

INVENTORY

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

C. SWAFFORD

R. DUGGER

N. YACOBI

GEOTECHNOLOGY, INC.

INVESTIGATED BY C. SWAFFORD

DRAWN BY T. LYNN

CHECKED BY K. BUSSEY

SUBMITTED BY HDR

DATE NOVEMBER 2021

HDR HDR Engineering, Inc. of the Carolinas
555 Fayetteville St, Suite 900 Raleigh, N.C. 27601
N.C.B.E.L.S. License Number: F-01116



Kenneth R. Bussey, Jr. 11/22/21
SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

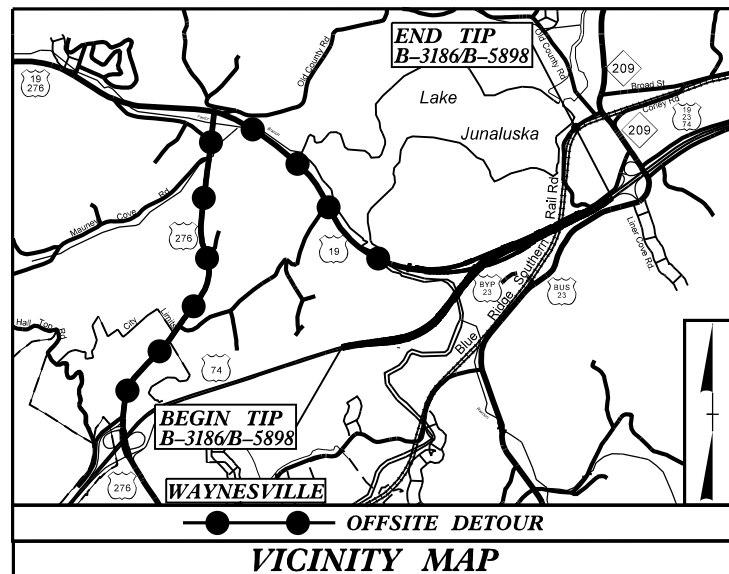
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TIP PROJECT: B-3186 / B-5898

CONTRACT: C204684

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See Sheet 1-A For Conventional Symbols



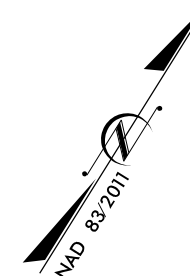
90% PLANS

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

HAYWOOD COUNTY

**LOCATION: US 23/US 74/US 19 (GREAT SMOKY MOUNTAIN HWY)
FROM WEST OF NC 209(CRABTREE RD.) TO EAST OF RUSS AVE.
TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURES
AND UTILITIES.**

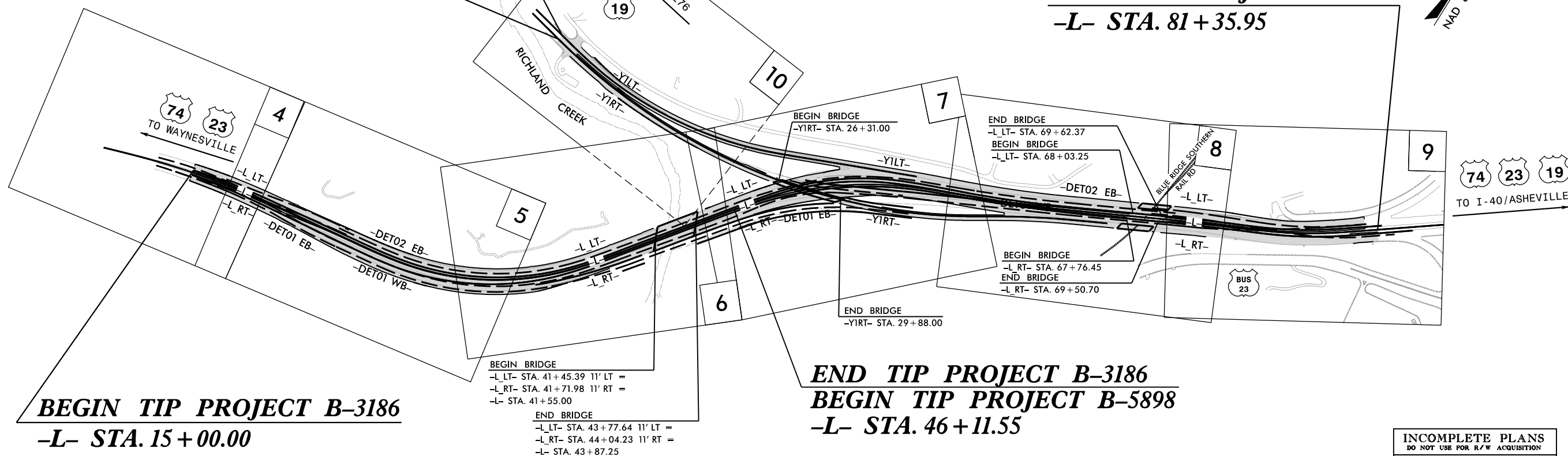
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3186 / B-5898	3	66
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
38332.1.FS.1	BRNHP-0023(32)	P.E.	
48030.1.FS.1	BRSTP-0019(49)	P.E.	
38332.2.1	N/A	RW/UTILITY	
48030.2.1	N/A	RW/UTILITY	



**BEGIN CONSTRUCTION
-YIRT- STA. 12 + 00.00**

**BEGIN CONSTRUCTION
-YILT- STA. 11 + 99.83**

**END TIP PROJECT B-5898
-L- STA. 81 + 35.95**



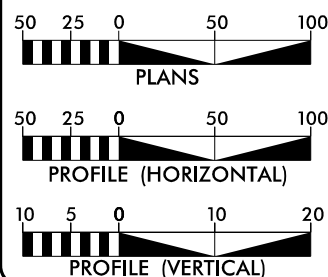
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-L- STA. 15 + 00.00**

**END TIP PROJECT B-3186
BEGIN TIP PROJECT B-5898
-L- STA. 46 + 11.55**

**INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

THIS IS A CONTROLLED-ACCESS PROJECT WITH
ACCESS BEING LIMITED TO INTERCHANGES

GRAPHIC SCALES



DESIGN DATA

ADT 2022 = 47,300
 ADT 2042 = 59,400
 K = 8 %
 D = 55 %
 T = 5 % *
 V = 65 MPH
 * TTST = 2% DUAL 3%
 FUNC CLASS = FREEWAY
 STATEWIDE TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-3186 = 0.545 miles
 TOTAL STRUCTURES TIP PROJECT B-3186 = 0.044 miles
 TOTAL LENGTH TIP PROJECT B-3186 = 0.589 miles
 LENGTH ROADWAY TIP PROJECT B-5898 = 0.636 miles
 TOTAL STRUCTURES TIP PROJECT B-5898 = 0.033 miles
 TOTAL LENGTH TIP PROJECT B-5898 = 0.669 miles
 (LENGTHS BASED ON L_RT ALIGNMENT)

Prepared in the Office of:
HDR HDR Engineering, Inc. of the Carolinas
 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601
 N.C.B.E.L.S. License Number: F-0116

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JULY 8, 2021

LETTING DATE:
MARCH 15, 2022

PHILLIP E. ROGERS, PE
PROJECT ENGINEER

HENRY W. BARE
PROJECT DESIGN ENGINEER

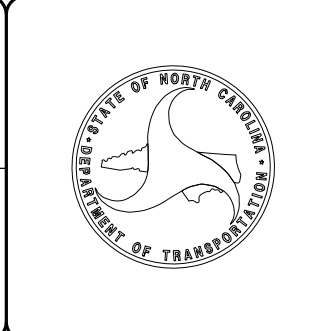
GARRETT HIGDON
NCDOT CONTACT

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



November 3, 2021

STATE PROJECT: B-3186/B-5898
 FEDERAL PROJECT: N/A
 COUNTY: HAYWOOD
 DESCRIPTION: US 23/US 74/US 19 (Great Smoky Mountain Hwy.) from West of NC 209 (Crabtree Rd.) to East of Russ Ave.
 SUBJECT: Geotechnical Inventory Report

Project Description

The project area lies in the town of Lake Junaluska, NC between Highway 276 and NC 209. This project consists of upgrading US 74 to a six-lane, median divided facility from east of the US 276 Interchange to the US 23 Business Interchange. Additionally, US 19 will be upgraded with full depth paved, 10-ft shoulders from east of Holston Village Road to the US 74 Interchange. In total, approximately 1.2 miles of roadway will be upgraded. Finally, a two-span bridge on US 19 over US 74/US 23, a three-span bridge on US 19/US 23/US 74 over Richland Creek, a dual, three-span bridges on US 19/US 23/US 74 over the Blue Ridge Southern Railroad, and nine retaining walls are included with this project, but are covered under separate reports.

The geotechnical field investigation was conducted from January 2021 to April 2021. Three drill rigs mounted on a rubber tracked all-terrain carrier and equipped with an automatic hammer (CME 55, CME 550X, and CME 75) were used to advance borings for the subsurface exploration. Hollow stem auger drilling procedures were used to advance borings to the required depths. Standard Penetration Tests were performed at approximately 2.5-foot to 5.0-foot intervals to termination in selected borings. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis.

The following alignments were explored. Subsurface profiles and/or cross sections of these alignments are included in this report.

<u>Line</u>	<u>Stations (±)</u>
-L-	20+00 – 81+35.95
-Y1RT-	10+00 – 44+31.29
-Y1LT-	10+00 – 33+16.69

Areas of Special Geotechnical Interest

1) Loose/Soft Soils: Very soft or very loose soils were encountered during the investigation. Such soils (N-value < 4) could have the potential to cause embankment/subgrade and/or slope stability problems during construction. These soils were encountered along the following intervals:

<u>Line</u>	<u>Station (±)</u>	<u>Offset (ft)</u>
-L-	54+50	RT
-L-	57+00 – 59+50	RT
-L-	66+50	RT
-Y1RT-	20+00	RT
-Y1RT-	25+50	RT

- 2) Highly Plastic Soils: Highly plastic soils were encountered as part of the investigation. However, these soils were encountered at depths great enough to not adversely impact embankment/subgrade and/or slope stability. However, if encountered during construction, these soils have the potential to cause such problems.
- 3) Organic Soils: Though not encountered during the investigation, organic matter and wood debris could be present along the project limits, especially near streams and creeks.
- 4) Artificial Fill: Artificial fill was encountered at the following locations.

<u>Line</u>	<u>Station (±)</u>	<u>Offset (ft)</u>
-L-	38+00 – 41+00	LT to RT
-L-	44+50 – 46+50	LT

Several smaller areas of artificial fill may be present throughout the project corridor and are related to business developments, gravel, and soil driveways, as well as previous construction of utility lines.

5) Crystalline Rock: Crystalline rock was encountered within 6 feet of proposed grade at the following locations:

<u>Line</u>	<u>Station (±)</u>	<u>Offset (ft)</u>
-L-	32+00	RT
-L-	75+00 – 77+00	LT to RT
-Y1RT-	17+00 – 18+00	LT to RT

- 6) Groundwater: High groundwater was not encountered during the investigation. However, especially around streams and creeks, seasonal high ground water or the potential for groundwater related construction problems could be present.
- 7) Ponds: No ponds were found or identified on or within close proximity of right of way on this project. These were noted at the following locations:
- 8) Water Wells: Water wells were not found or identified within or in close proximity to the proposed right of way.

Physiography and Geology

The project is located in the Blue Ridge Physiographic Province. Land use along the project corridor consists of residential, agricultural, commercial businesses and woods. Geologically, the project is located within the Blue Ridge Belt. Bedrock generally consists of rocks from the Coweeta Group (**ZYbn**), consisting of migmatitic Biotite Gneiss interlayered and gradational with biotite-garnet gneiss and amphibolite, with intrusive metamorphosed gabbro and diorite.

Soil Properties

Soils encountered at the project site include roadway embankment, artificial fill, alluvial, residual, weathered metamorphic rock and crystalline metamorphic rock.

Roadway Embankment soils were mainly encountered along the existing sections of US 74 and US 19 and consisting of gray, red, and brown, very soft to very stiff silt and clay (A-4, A-5, A-6, A-7) and loose to medium dense, clayey and silty sand and gravel (A-2-6, A-2-4, A-3, A-1-b).

Artificial fill soils consist of brown, orange, and gray, very soft to stiff, silt and clay (A-4, A-6, A-7-6), and loose to medium dense, sand and gravel (A-3, A-1-b). The artificial fill is underlain by residual soils.

Alluvial deposits are located within the floodplains of Richland Creek and nearby streams within the project limits. These soils are black, gray, and brown, very soft to medium stiff, silt and clay (A-4, A-5, A-7), and very loose to dense, sand and gravel and silty sand (A-1-b, A-2-4, A-2-6, A-3).

Residual soils were encountered throughout the project. These soils consist primarily of red, tan, and brown, soft to hard silt and clay (A-4, A-5, A-7), and loose to very dense, silty and clayey sand (A-2-4, A-2-5, A-2-6, A-2-7, A-3).

Rock Properties

Weathered rock was encountered during the roadway investigation at elevations ranging from approximately 2638 to 2551 feet. It originates from the underlying metamorphic rock, specifically Gneiss.

Crystalline rock was encountered during the roadway investigation at elevations ranging from approximately 2653 to 2523 feet and consists of Gneiss. Refer to the "Areas of Special Geotechnical Interest" for areas of rock encountered within 6 feet of proposed grade.


Ground Water

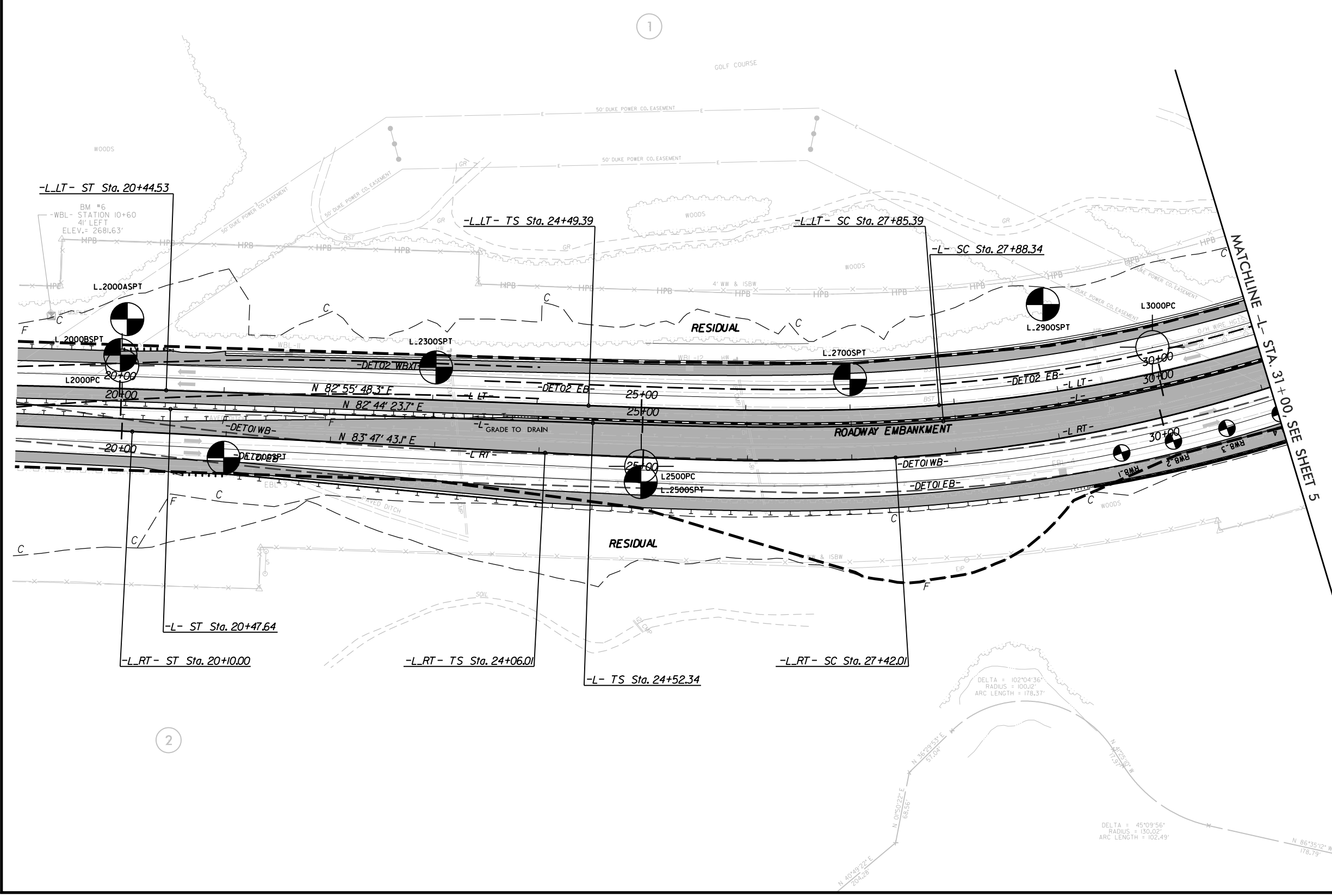
Groundwater was encountered in multiple borings and ranges in elevation from approximately 2620 to 2567 feet. Groundwater may fluctuate with seasonal precipitation.

Prepared By,
HDR Engineering, Inc., of the Carolinas



Kenneth R. Bussey, Jr., P.E.
Senior Geotechnical Engineer

PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St. Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116	




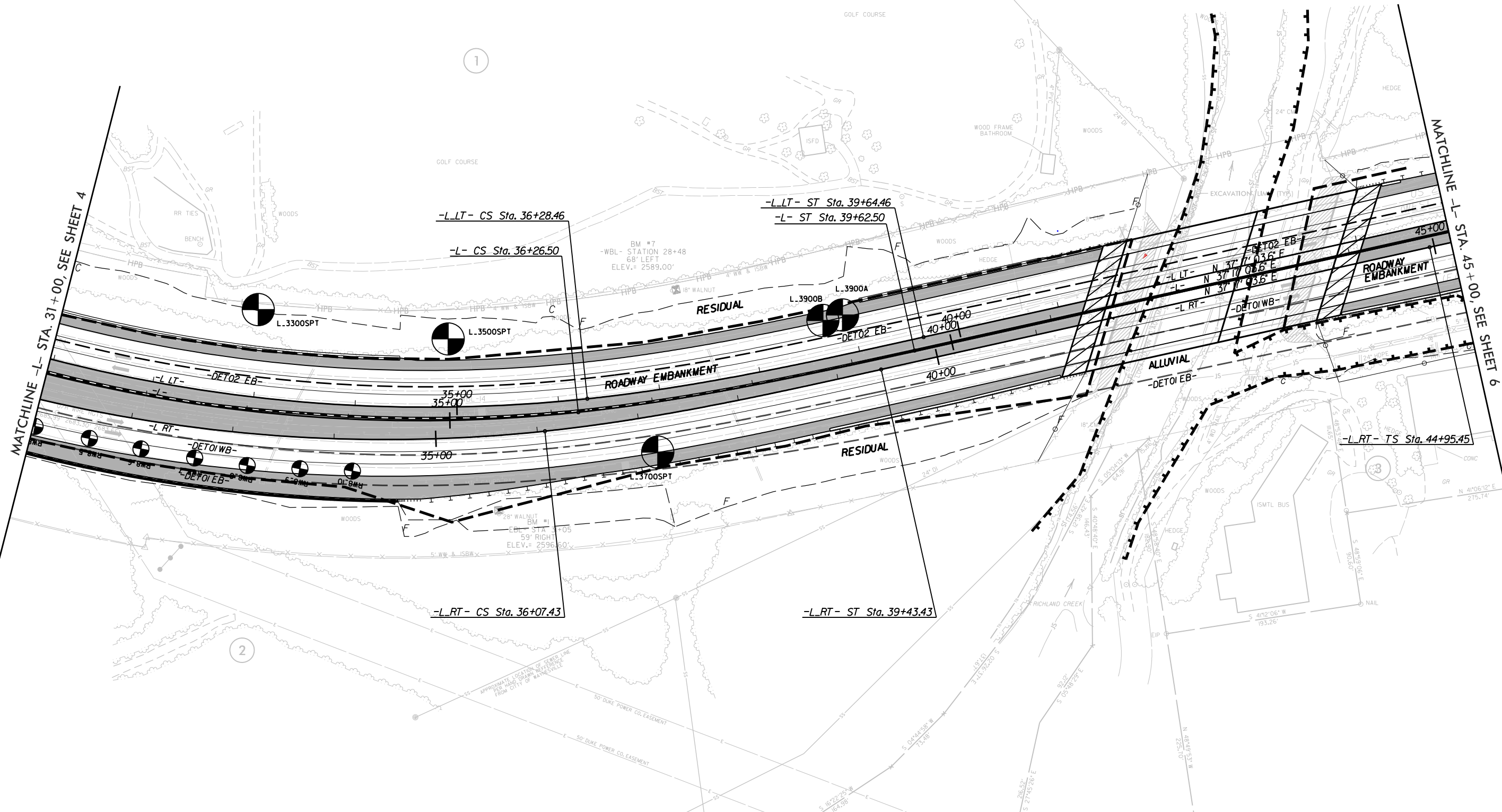
REVISIONS

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2

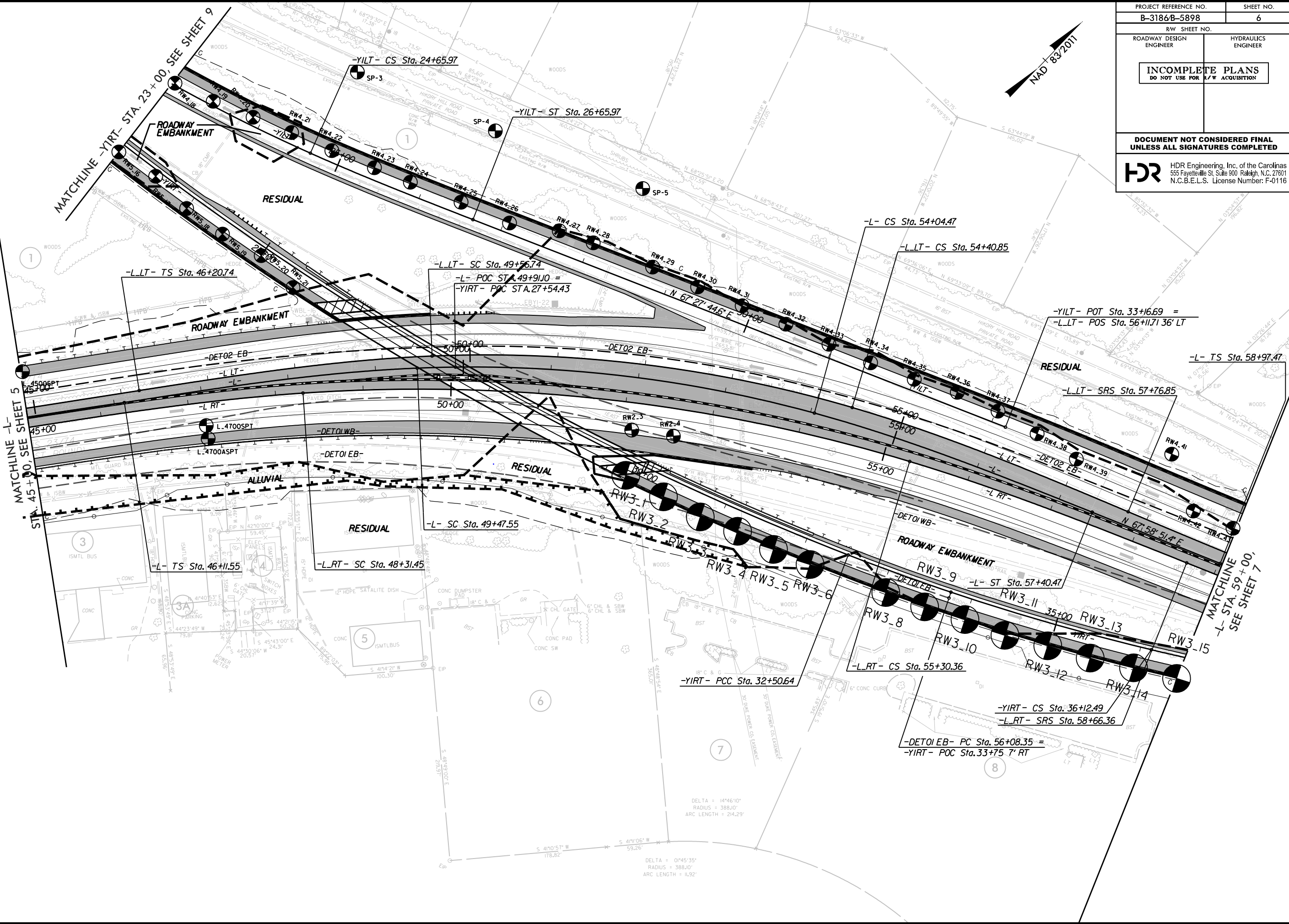
PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116	



REVISIONS

PLOT DRIVER: NCDOT_color_eng_100.plt
 USER: TLYNN
 DATE: 11/3/2021
 TIME: 11:47:24 AM
 FILE: \

PROJECT REFERENCE NO.		SHEET NO.	
B-3186B-5898		6	
RW SHEET NO.		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER			
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			
 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116			




REVISIONS

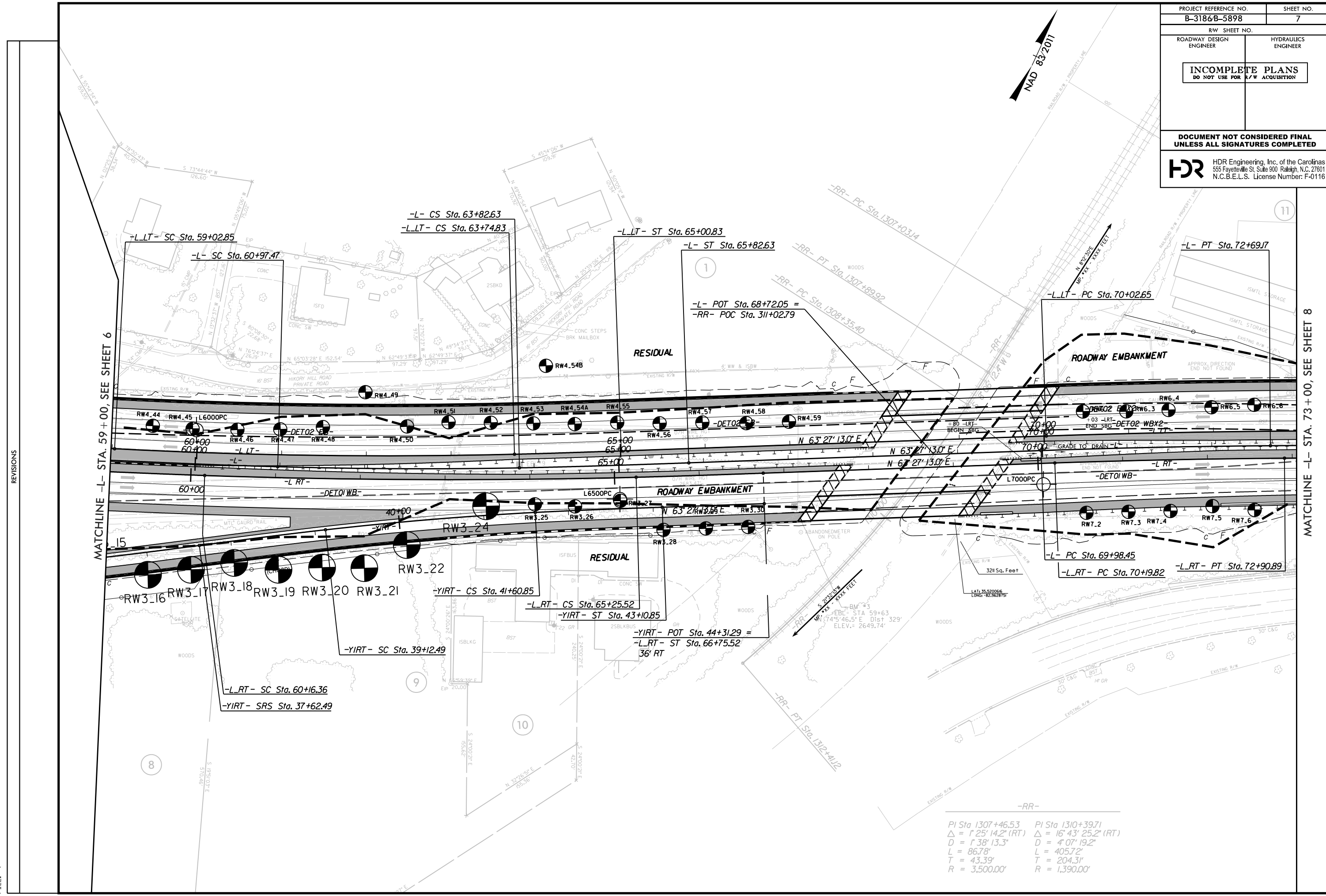
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 TIME: 11:47:27 AM

PLOT DRIVER: NCDOT_color_eng_100.plt
 USER: TLYNN
 DATE: 11/3/2021
 TIME: 11:47:27 AM

DELTA = 14°46'10"
 RADIUS = 388.10'
 ARC LENGTH = 242.29'

DELTA = 0°45'35"
 RADIUS = 388.10'
 ARC LENGTH = 11.92'

PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
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MATCHLINE -L- STA. 59+00, SEE SHEET 6

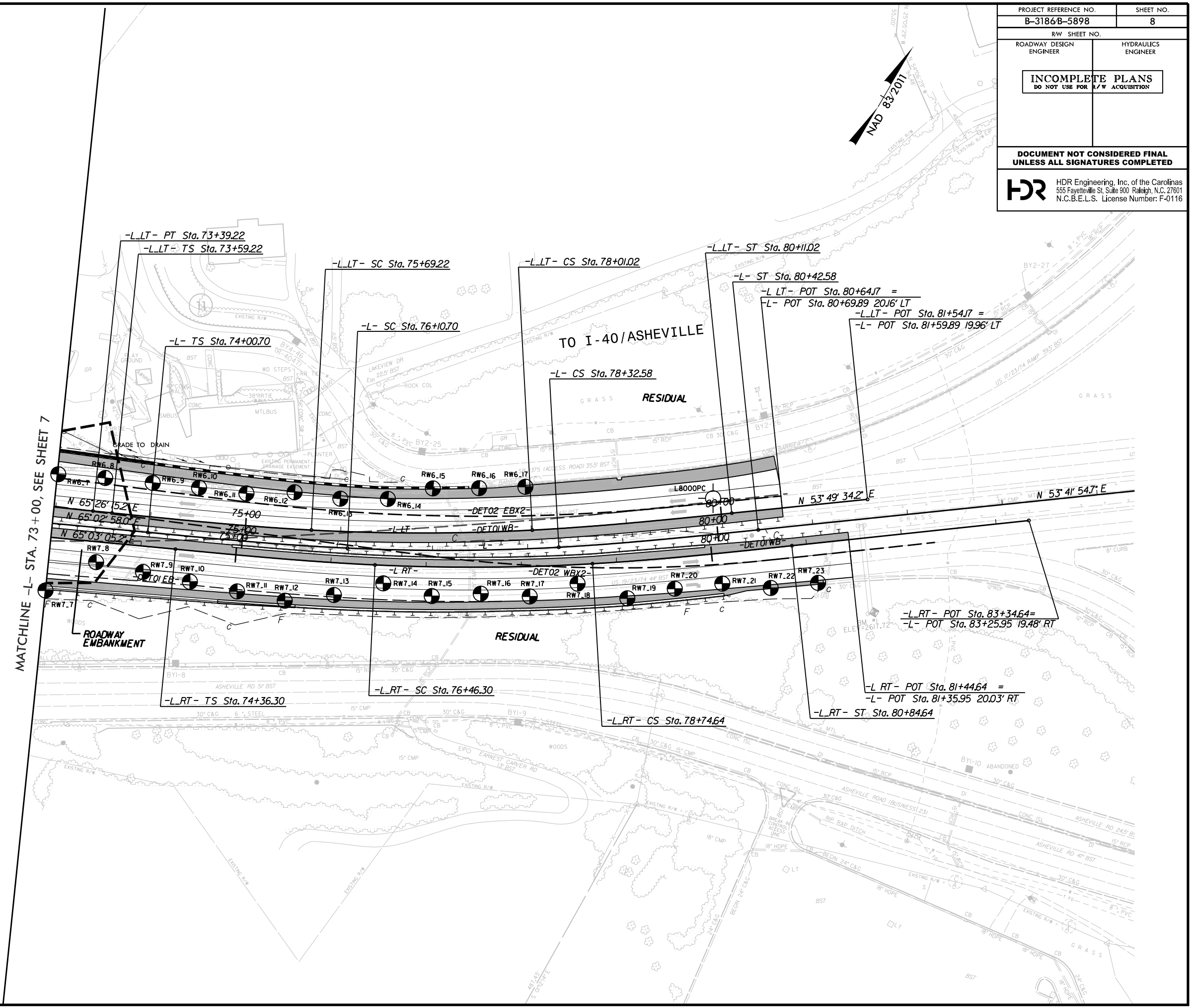
MATCHLINE -L- STA. 73+00, SEE SHEET 8

REVISIONS

PLOT DRIVER: NCDOT_color_eng_100.plt
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 DATE: 11/3/2021
 TIME: 11:47:30 AM
 FILE:

$PI\ Sta\ 1307+46.53$ $\Delta = 1'25'14.2'' (RT)$ $D = 1'38'13.3''$ $L = 86.78'$ $T = 43.39'$ $R = 3,500.00'$	$PI\ Sta\ 1310+39.71$ $\Delta = 16'43'25.2'' (RT)$ $D = 4'07'19.2''$ $L = 405.72'$ $T = 204.31'$ $R = 1,390.00'$
--	---


PROJECT REFERENCE NO. B-3186B-5898		SHEET NO. 8	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			
 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116			



REVISIONS

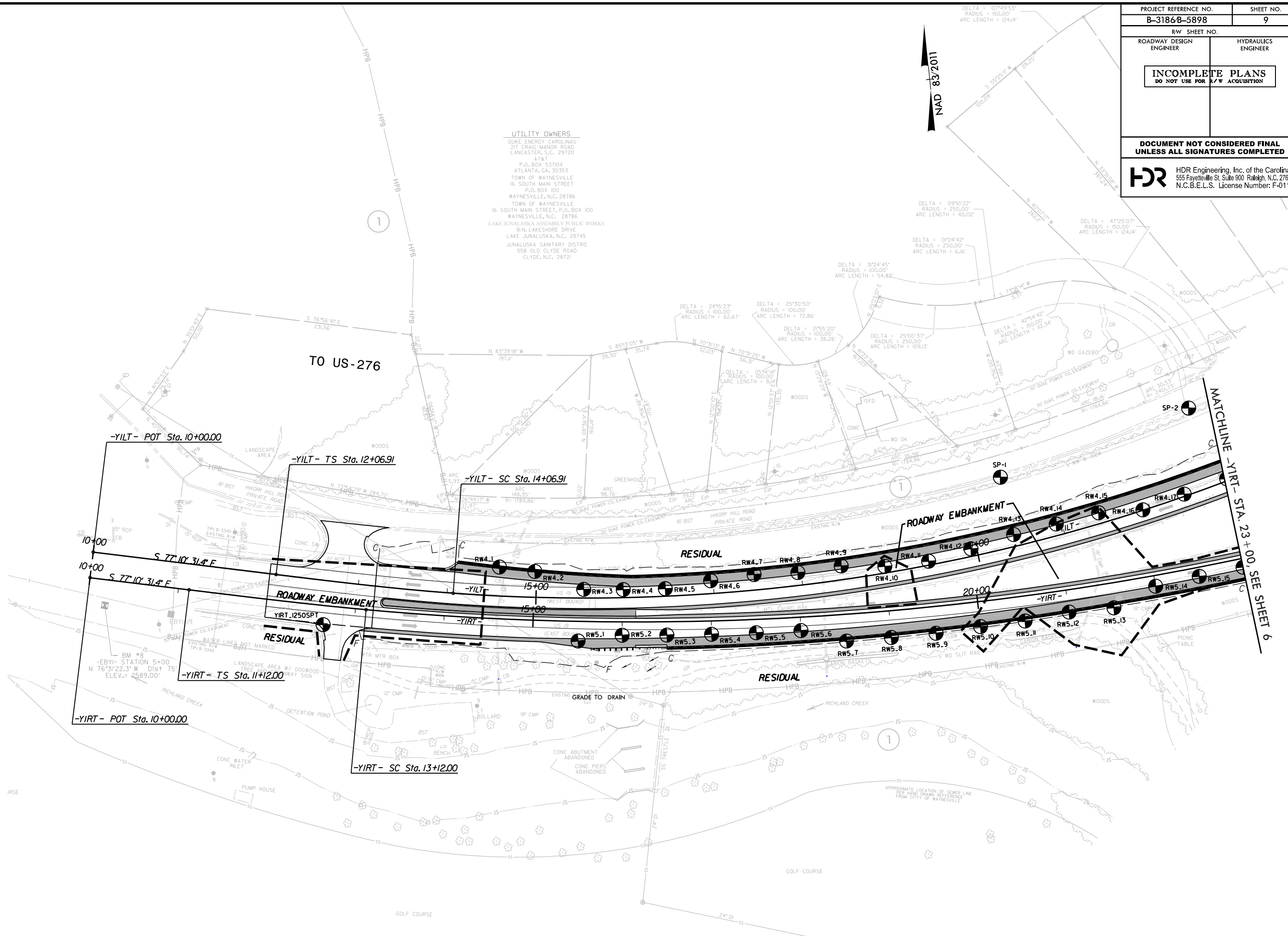
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DATE: 11/3/2021
TIME: 11:47:33 AM

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USER: TLYNN
DATE: 11/3/2021
TIME: 11:47:33 AM

PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
 HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116	

UTILITY OWNERS
 DUKE ENERGY CAROLINAS
 217 CRAIG MANOR ROAD
 LANCASTER, S.C. 29720
 AT&T
 P.O. BOX 537104
 ATLANTA, GA. 30353
 TOWN OF WAYNESVILLE
 16 SOUTH MAIN STREET
 P.O. BOX 100
 WAYNESVILLE, N.C. 28786
 TOWN OF WAYNESVILLE
 16 SOUTH MAIN STREET, P.O. BOX 100
 WAYNESVILLE, N.C. 28786
 LAKE JUNALUSKA ASSEMBLY PUBLIC WORKS
 91 N. LAKESHORE DRIVE
 LAKE JUNALUSKA, N.C. 28745
 JUNALUSKA SANITARY DISTRICT
 558 OLD CLYDE ROAD
 CLYDE, N.C. 28721

NAD 83/2011

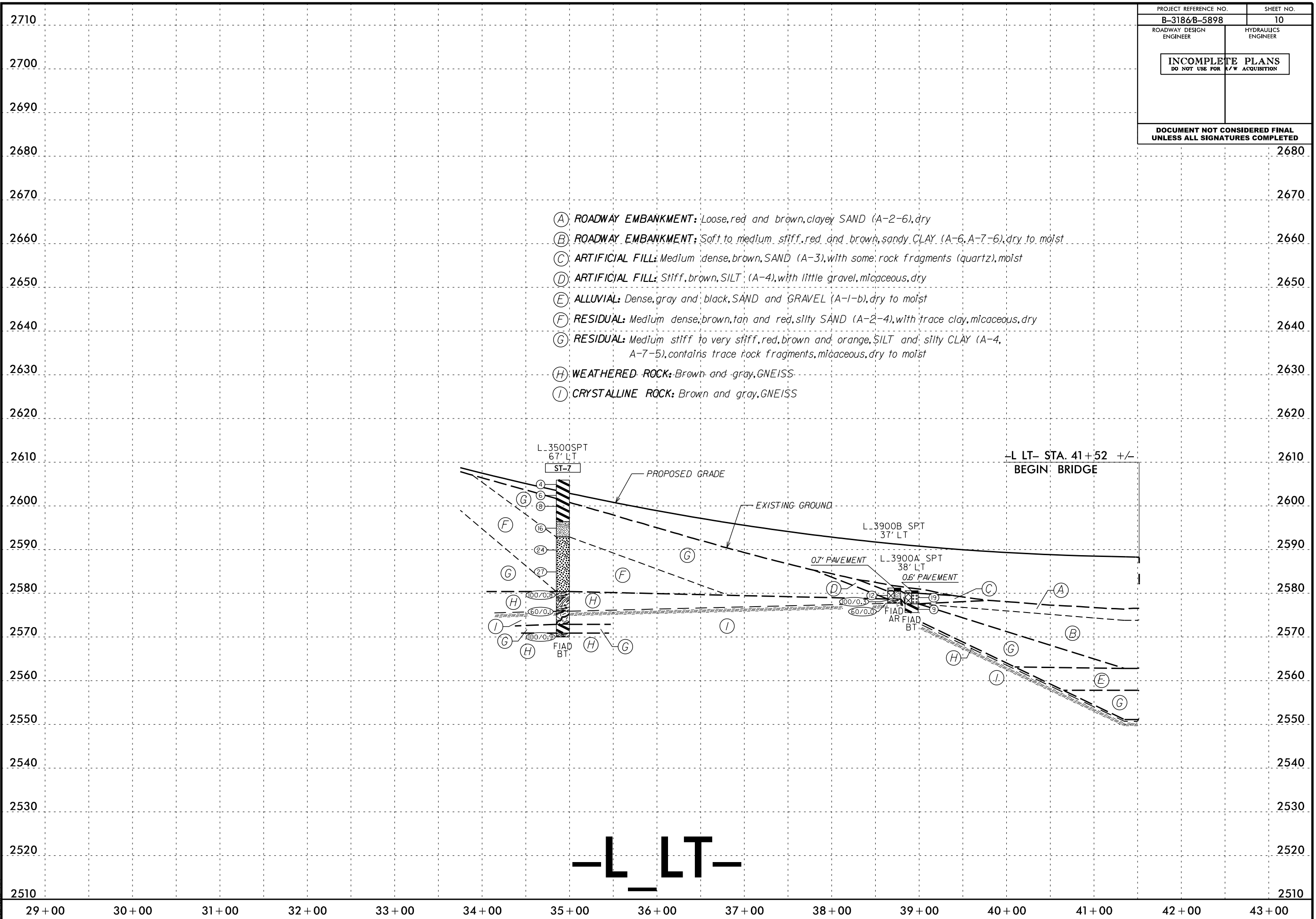


REVISIONS

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 DATE: 11/3/2021
 TIME: 11:47:36 AM
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5/14/99

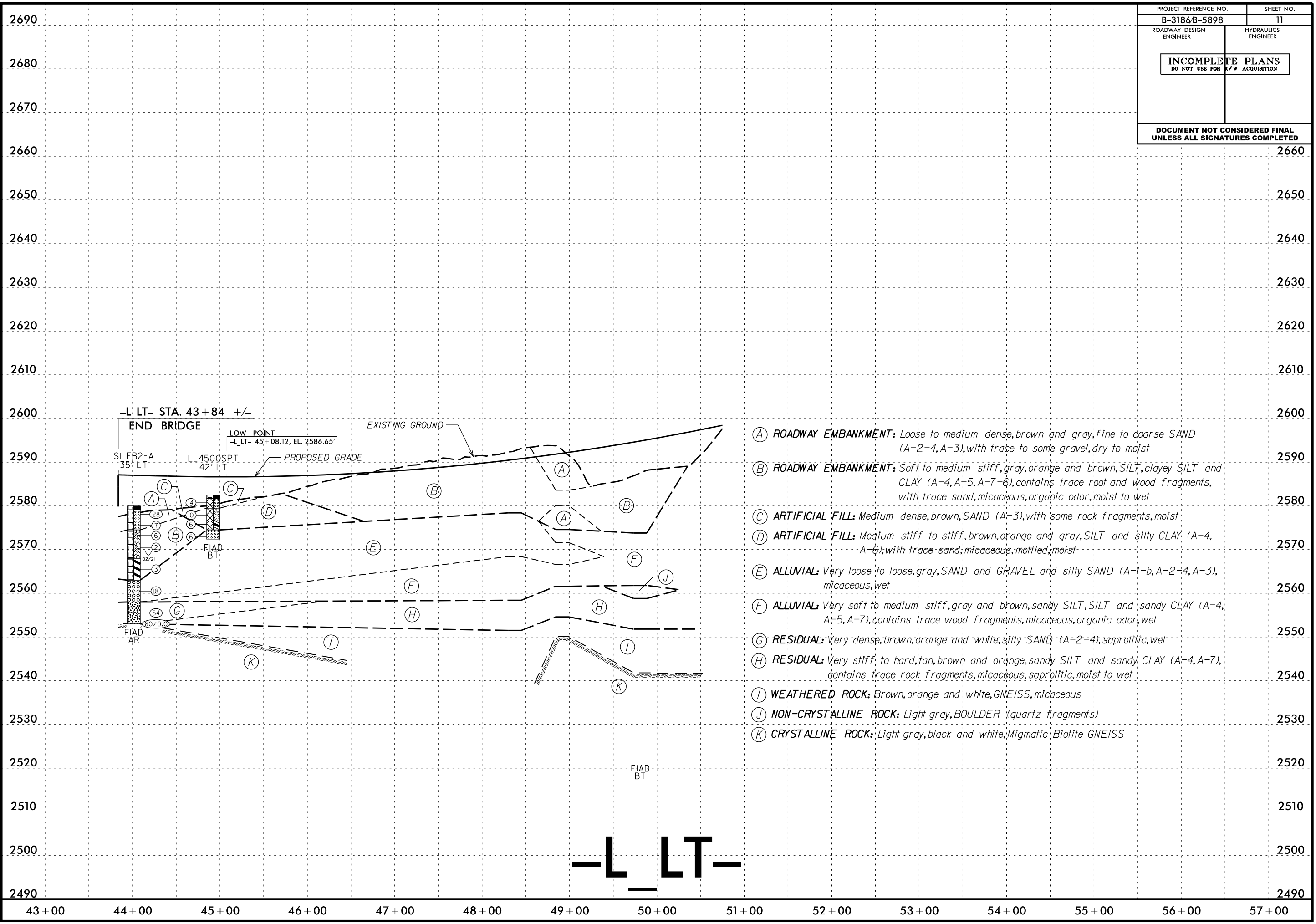
PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 10
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



10/22/2021
B-3186B-5898_GEO_RDY_PFL-LT.dgn
2:41:50 AM

5/14/99

PROJECT REFERENCE NO. B-3186/B-5898	SHEET NO. 11
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



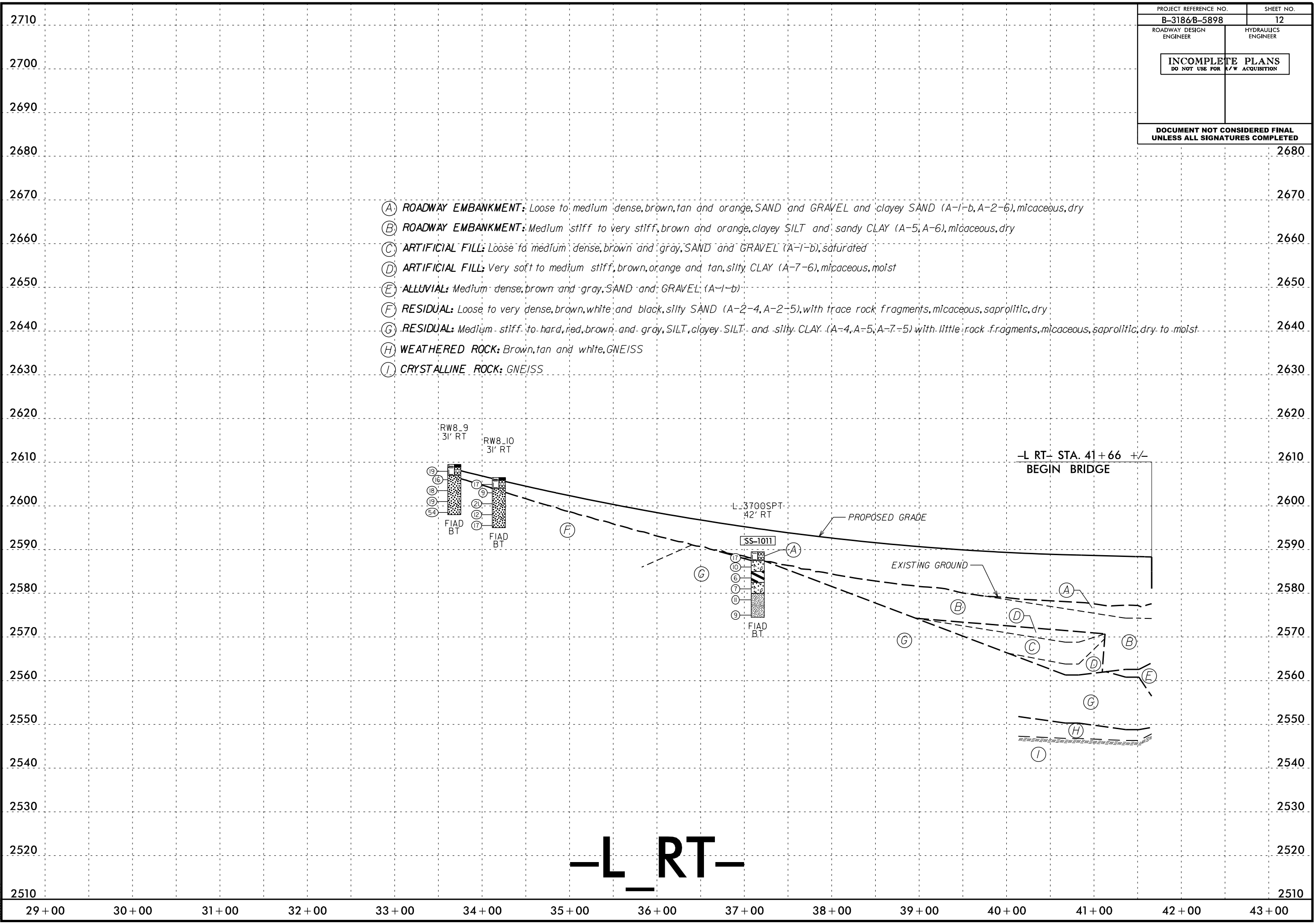
- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown and gray, fine to coarse SAND (A-2-4, A-3), with trace to some gravel, dry to moist
- (B) ROADWAY EMBANKMENT: Soft to medium stiff, gray, orange and brown, SILT, clayey SILT and CLAY (A-4, A-5, A-7-6), contains trace root and wood fragments, with trace sand, micaceous, organic odor, moist to wet
- (C) ARTIFICIAL FILL: Medium dense, brown, SAND (A-3), with some rock fragments, moist
- (D) ARTIFICIAL FILL: Medium stiff to stiff, brown, orange and gray, SILT and silty CLAY (A-4, A-6), with trace sand, micaceous, mottled, moist
- (E) ALLUVIAL: Very loose to loose, gray, SAND and GRAVEL and silty SAND (A-1-b, A-2-4, A-3), micaceous, wet
- (F) ALLUVIAL: Very soft to medium stiff, gray and brown, sandy SILT, SILT and sandy CLAY (A-4, A-5, A-7), contains trace wood fragments, micaceous, organic odor, wet
- (G) RESIDUAL: Very dense, brown, orange and white, silty SAND (A-2-4), saprolitic, wet
- (H) RESIDUAL: Very stiff to hard, tan, brown and orange, sandy SILT and sandy CLAY (A-4, A-7), contains trace rock fragments, micaceous, saprolitic, moist to wet
- (I) WEATHERED ROCK: Brown, orange and white, GNEISS, micaceous
- (J) NON-CRYSTALLINE ROCK: Light gray, BOULDER (quartz fragments)
- (K) CRYSTALLINE ROCK: Light gray, black and white, Migmatite, Biotite GNEISS

-L LT-

10/22/2021
B3186-B5898_GEO_RDY_PFL.L.L.T.dgn
2021.6

5/14/99

PROJECT REFERENCE NO. B-3186B-5898	SHEET NO. 12
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

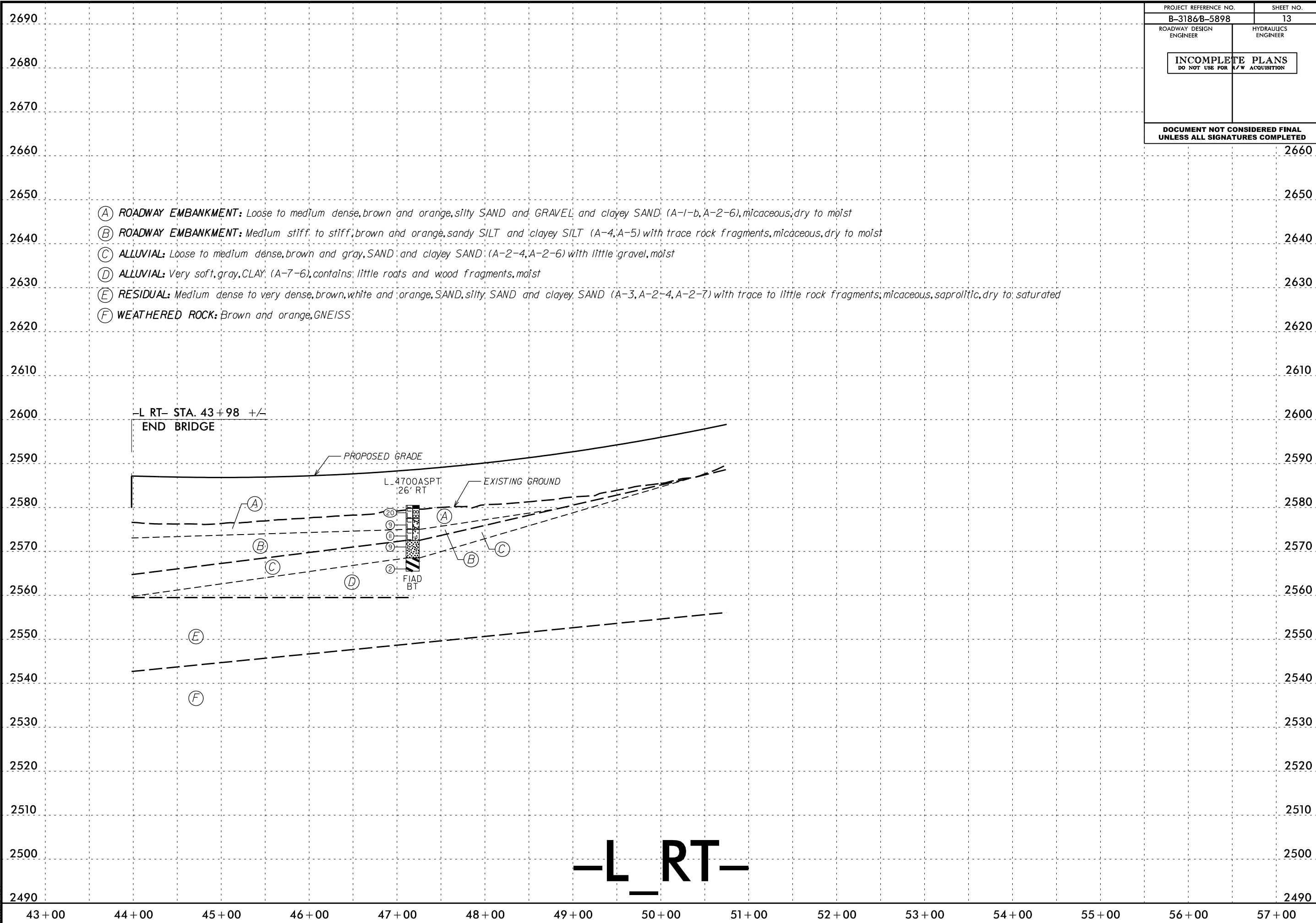


-L_RT-

10/22/2021 10:22:20 AM B:\3186B-5898_GEO_RDY_PFL_L_RT.dgn

5/14/99

PROJECT REFERENCE NO. B-3186/B-5898	SHEET NO. 13
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

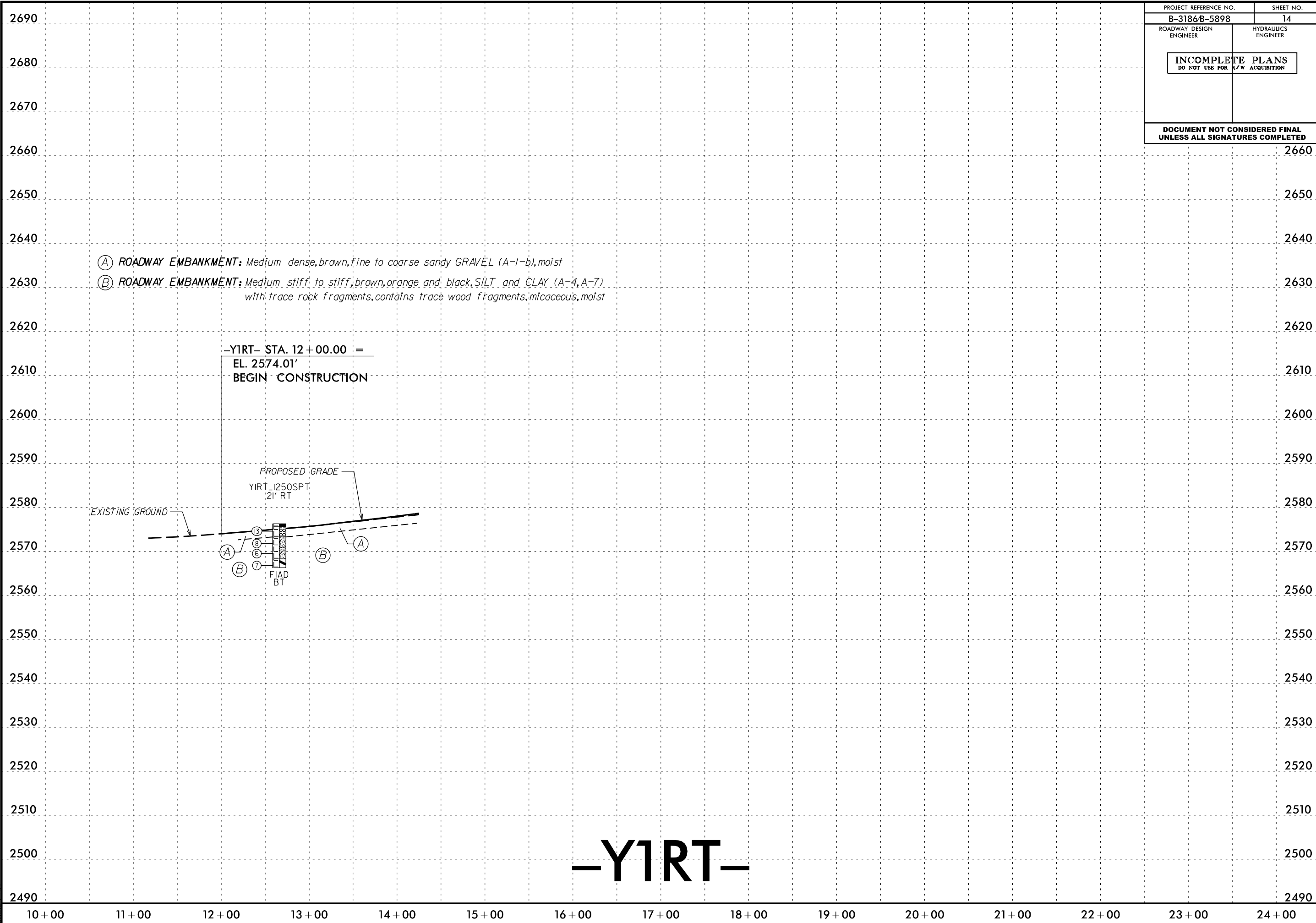


10/22/2021 10:22:20 AM B:\3186\B-5898_GEO_RDY_PFL_L_RT.dgn

-L_RT-

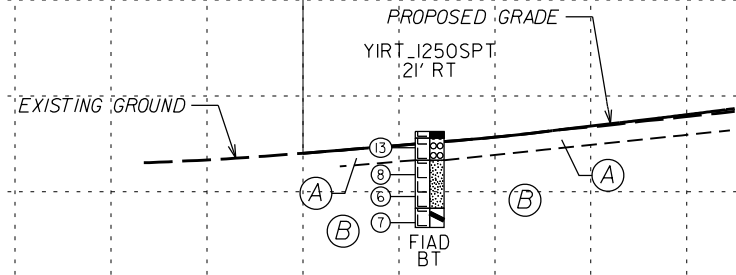
5/14/99

PROJECT REFERENCE NO. B-3186/B-5898	SHEET NO. 14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



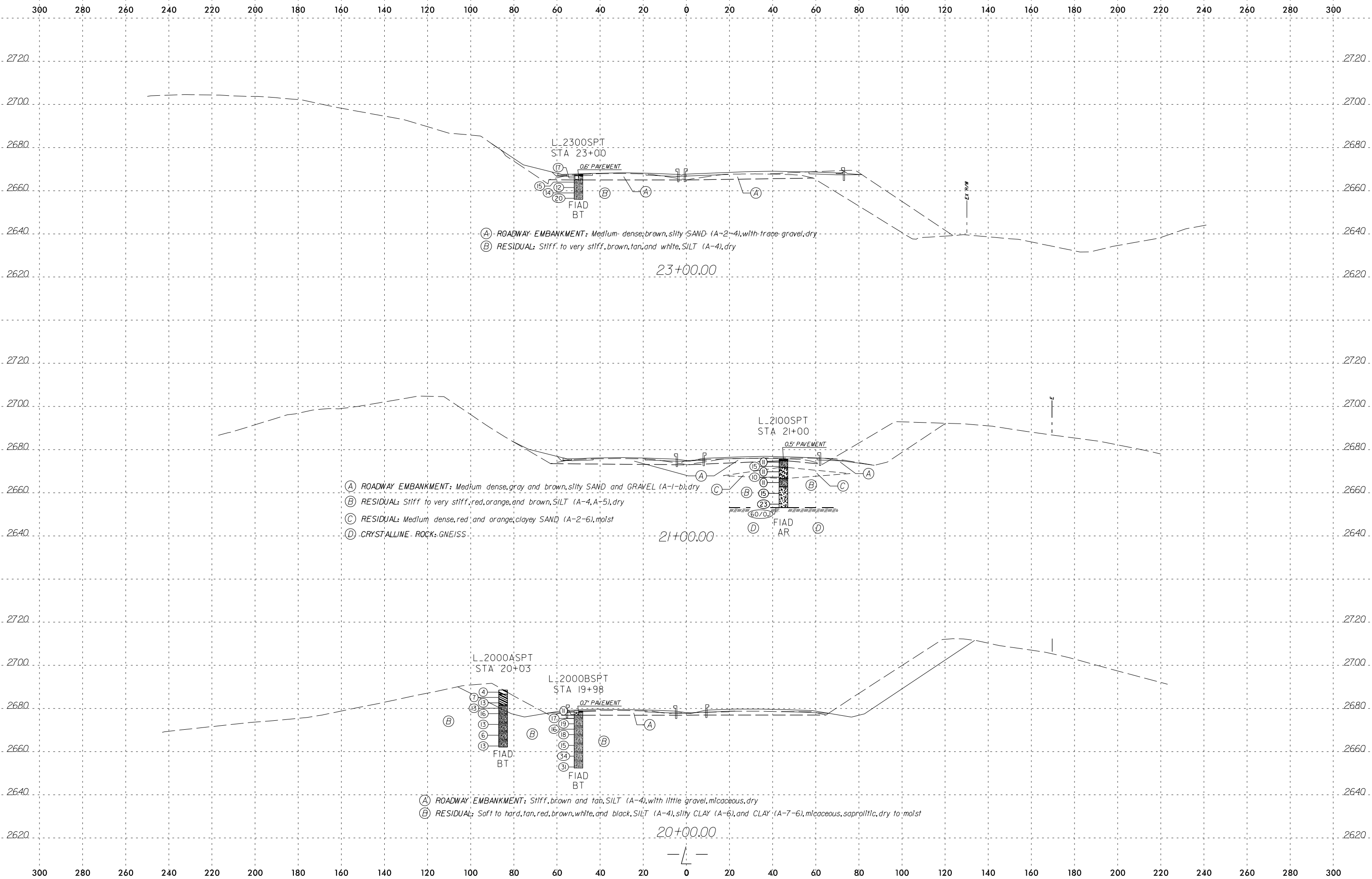
- (A) ROADWAY EMBANKMENT: Medium dense, brown, fine to coarse sandy GRAVEL (A-1-b), moist
- (B) ROADWAY EMBANKMENT: Medium stiff to stiff, brown, orange and black, SILT and CLAY (A-4, A-7) with trace rock fragments, contains trace wood fragments, micaceous, moist

-Y1RT- STA. 12+00.00 =
EL. 2574.01'
BEGIN CONSTRUCTION



-Y1RT-

10/22/2021 10:22:02 AM B:\3186-B5898-GEO-RDY-PFL-Y1RT.dgn



L_2300SPT
STA 23+00
0.6' PAVEMENT
FIAD
BT

- (A) ROADWAY EMBANKMENT: Medium dense, brown, silty SAND (A-2-4), with trace gravel, dry
- (B) RESIDUAL: Stiff, to very stiff, brown, tan, and white, SILT (A-4), dry

23+00.00

L_2100SPT
STA 21+00
0.5' PAVEMENT
FIAD
AR

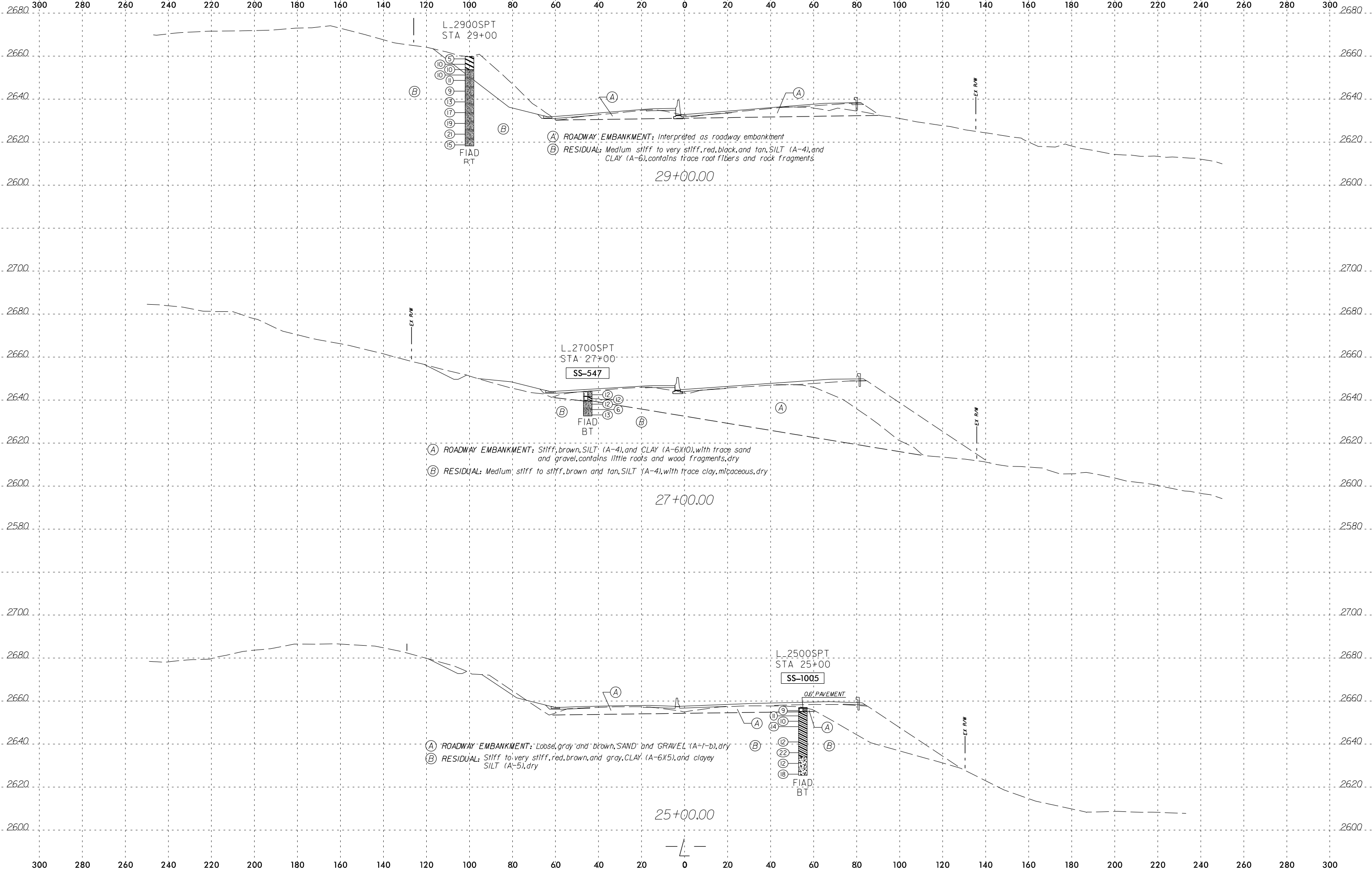
- (A) ROADWAY EMBANKMENT: Medium dense, gray and brown, silty SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff to very stiff, red, orange, and brown, SILT (A-4, A-5), dry
- (C) RESIDUAL: Medium dense, red and orange, clayey SAND (A-2-6), moist
- (D) CRYSTALLINE ROCK: GNEISS

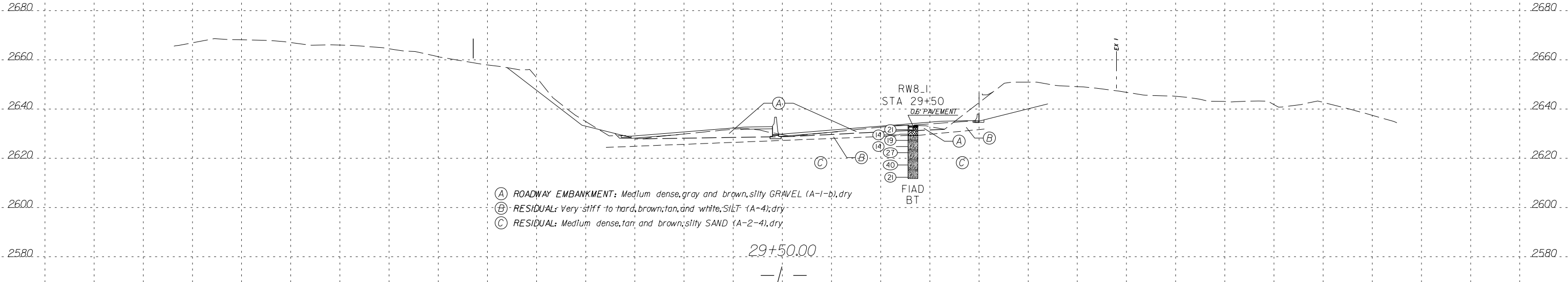
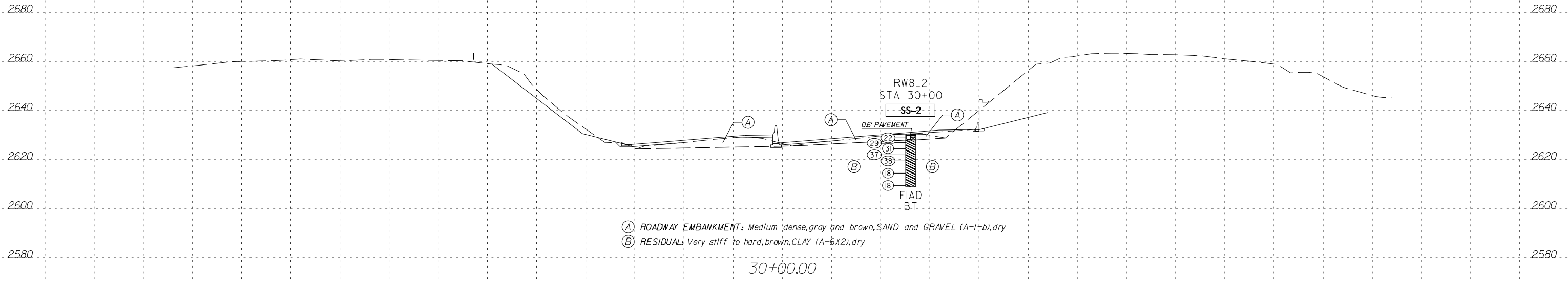
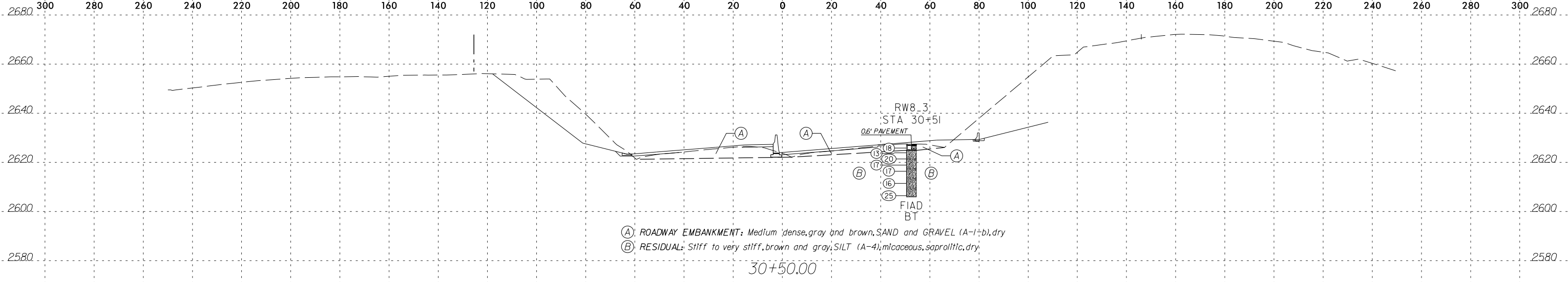
21+00.00

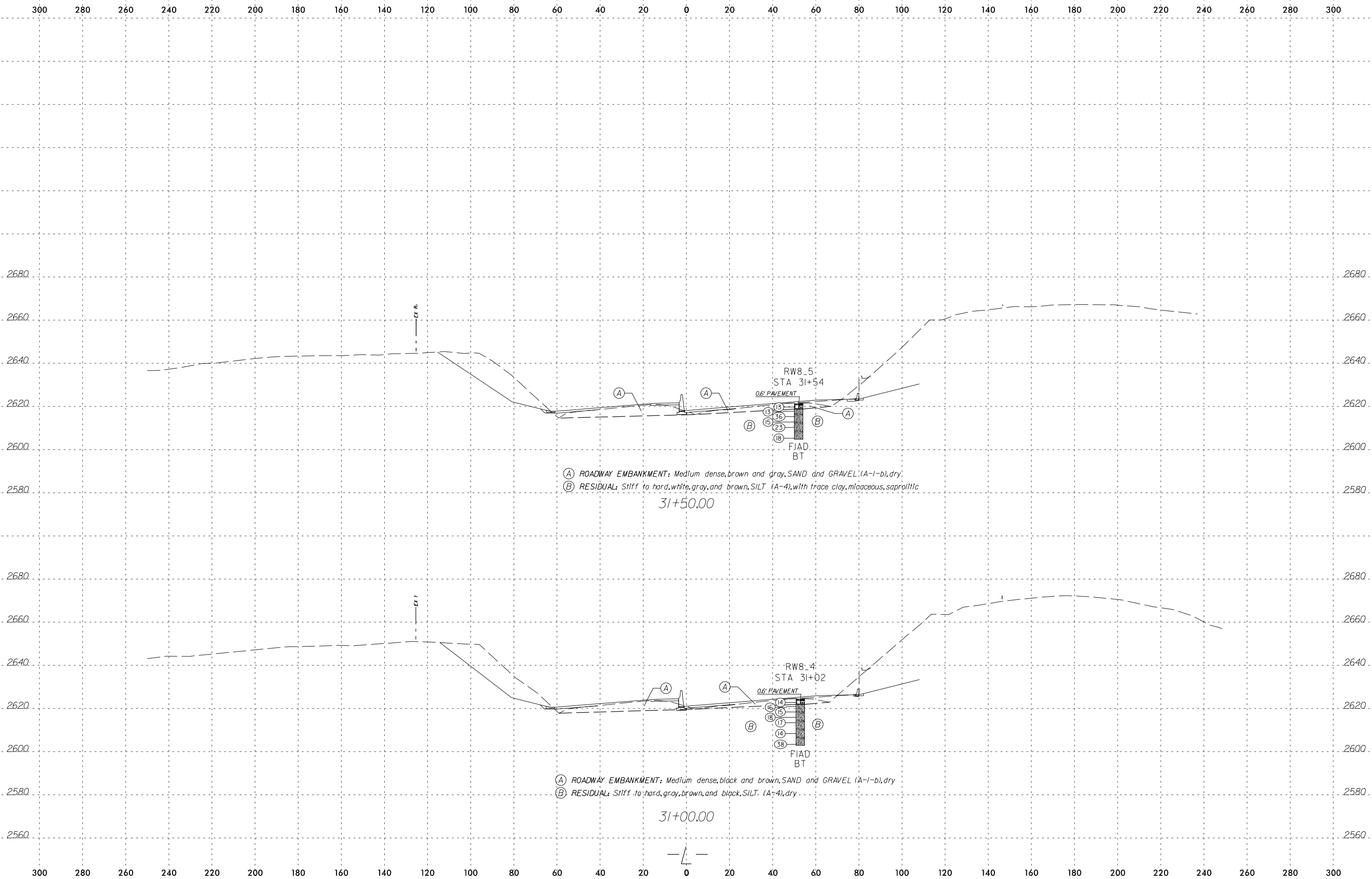
L_2000ASPT
STA 20+03
L_2000BSPT
STA 19+98
0.7' PAVEMENT
FIAD
BT

- (A) ROADWAY EMBANKMENT: Stiff, brown and tan, SILT (A-4), with little gravel, micaceous, dry
- (B) RESIDUAL: Soft to hard, tan, red, brown, white, and black, SILT (A-4), silty CLAY (A-6), and CLAY (A-7-6), micaceous, saprolitic, dry to moist

20+00.00

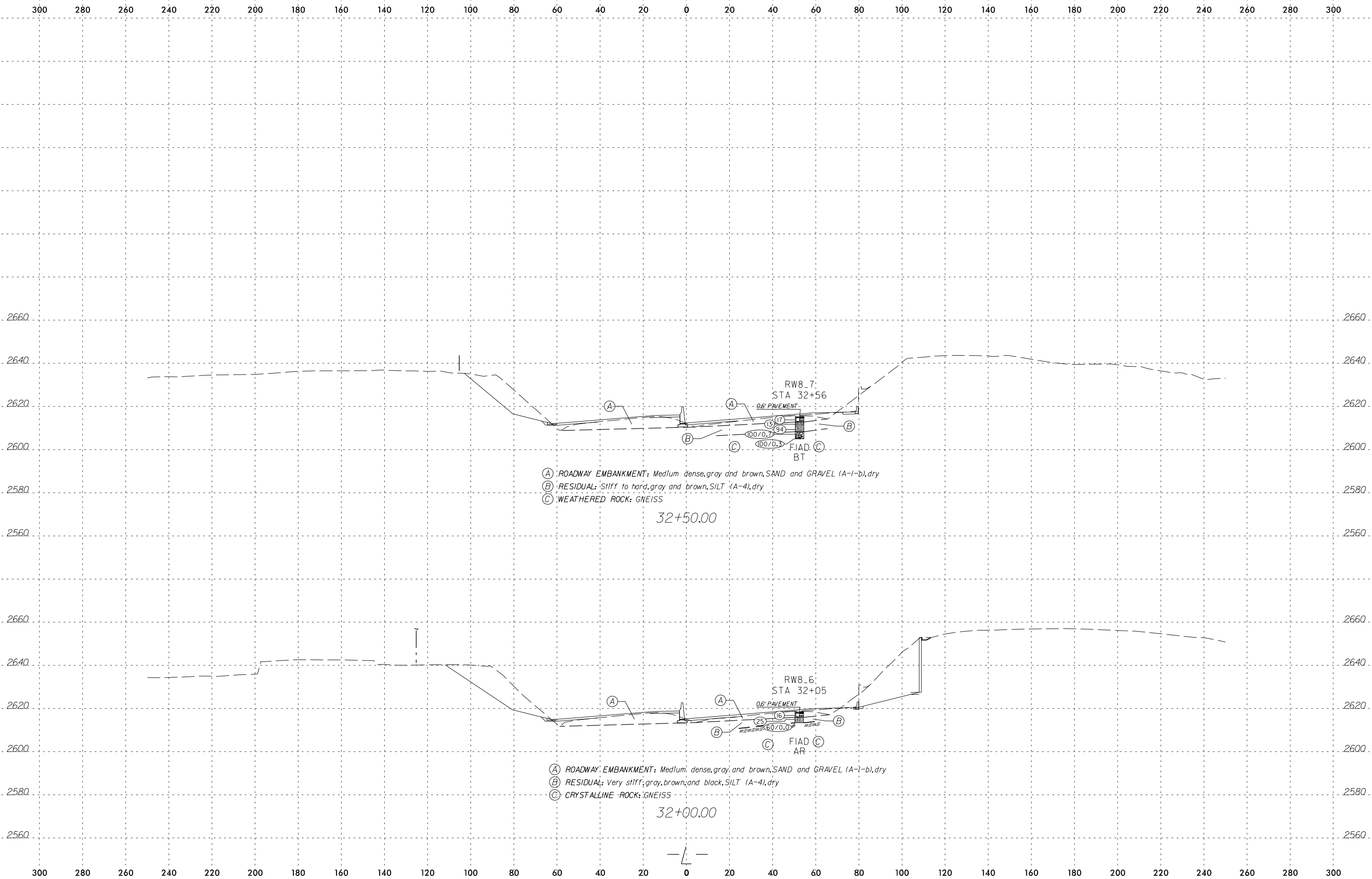






(A) ROADWAY EMBANKMENT: Medium dense, brown and gray, SAND and GRAVEL (A-I-b), dry.
 (B) RESIDUAL: Stiff to hard, white, gray, and brown, SILT (A-4), with trace clay, micaceous, saprolitic

(A) ROADWAY EMBANKMENT: Medium dense, black and brown, SAND and GRAVEL (A-I-b), dry.
 (B) RESIDUAL: Stiff to hard, gray, brown, and black, SILT (A-4), dry

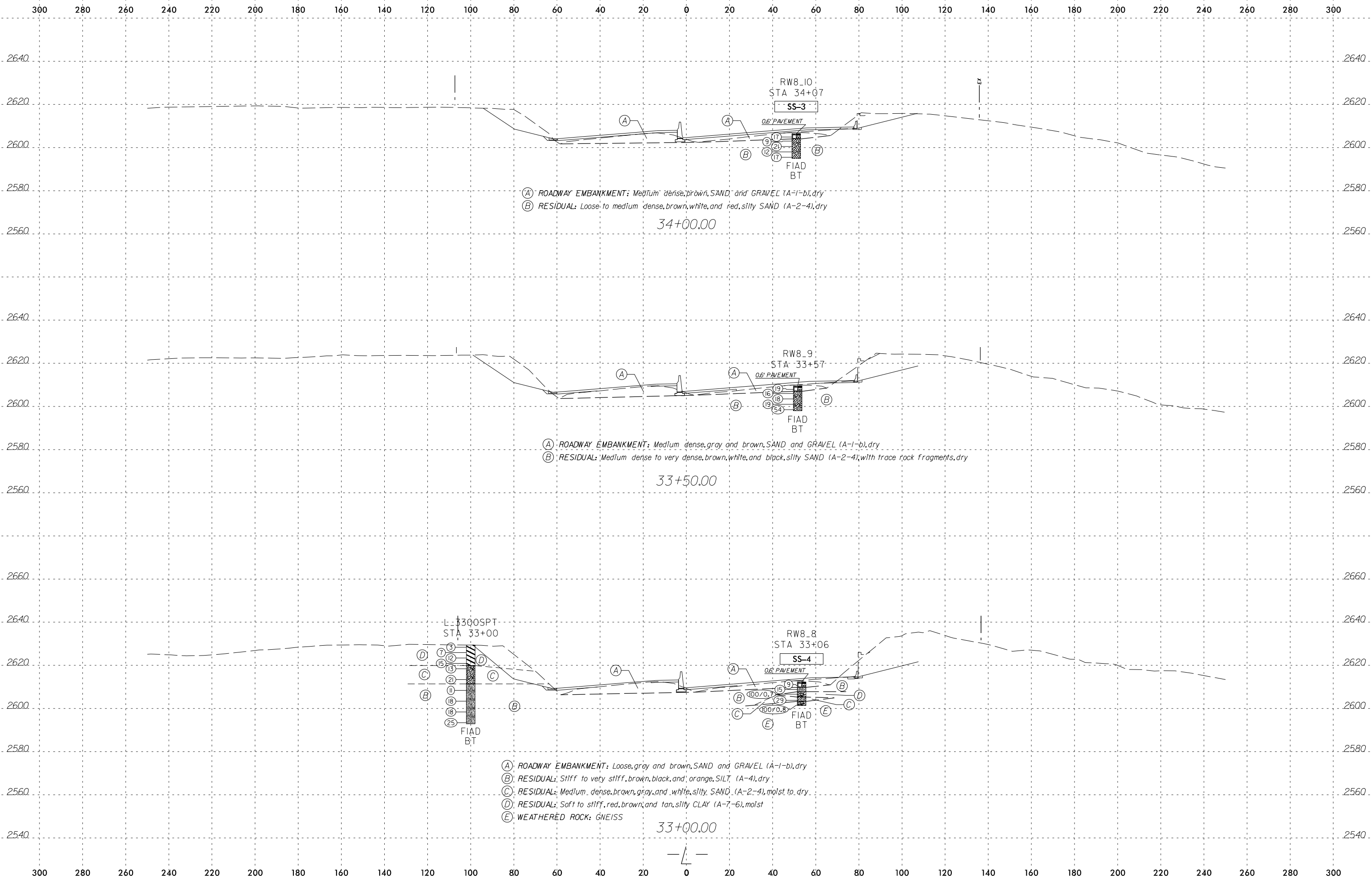


- (A) ROADWAY EMBANKMENT: Medium dense, gray and brown, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff to hard, gray and brown, SILT (A-4), dry
- (C) WEATHERED ROCK: GNEISS

32+50.00

- (A) ROADWAY EMBANKMENT: Medium, dense, gray, and brown, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Very stiff, gray, brown, and black, SILT (A-4), dry
- (C) CRYSTALLINE ROCK: GNEISS

32+00.00



- (A) ROADWAY EMBANKMENT: Medium dense, brown, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Loose to medium dense, brown, white, and red, silty SAND (A-2-4), dry

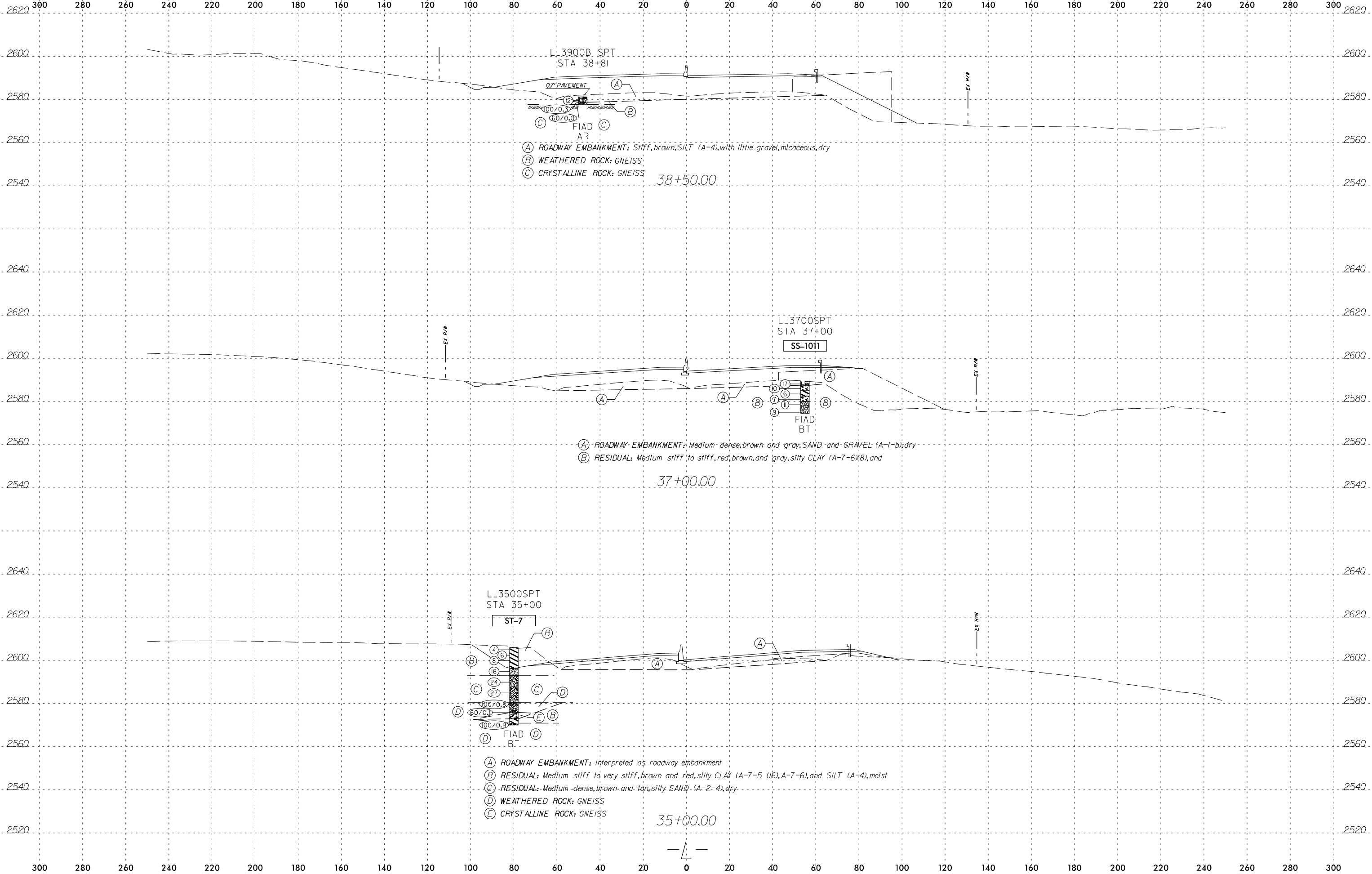
34+00.00

- (A) ROADWAY EMBANKMENT: Medium dense, gray and brown, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Medium dense to very dense, brown, white, and black, silty SAND (A-2-4), with trace rock fragments, dry

33+50.00

- (A) ROADWAY EMBANKMENT: Loose, gray and brown, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff to very stiff, brown, black, and orange, SILT (A-4), dry
- (C) RESIDUAL: Medium dense, brown, gray, and white, silty SAND (A-2-4), moist to dry
- (D) RESIDUAL: Soft to stiff, red, brown, and tan, silty CLAY (A-7-6), moist
- (E) WEATHERED ROCK: GNEISS

33+00.00



L_3900B_SPT
STA 38+81

0.7\"/>

- (A) ROADWAY EMBANKMENT: Stiff, brown, SILT (A-4), with little gravel, micaceous, dry
- (B) WEATHERED ROCK: GNEISS
- (C) CRYSTALLINE ROCK: GNEISS

38+50.00

L_3700SPT
STA 37+00

SS-1011

- (A) ROADWAY EMBANKMENT: Medium dense, brown and gray, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Medium stiff to stiff, red, brown, and gray, silty CLAY (A-7-6)(8), and

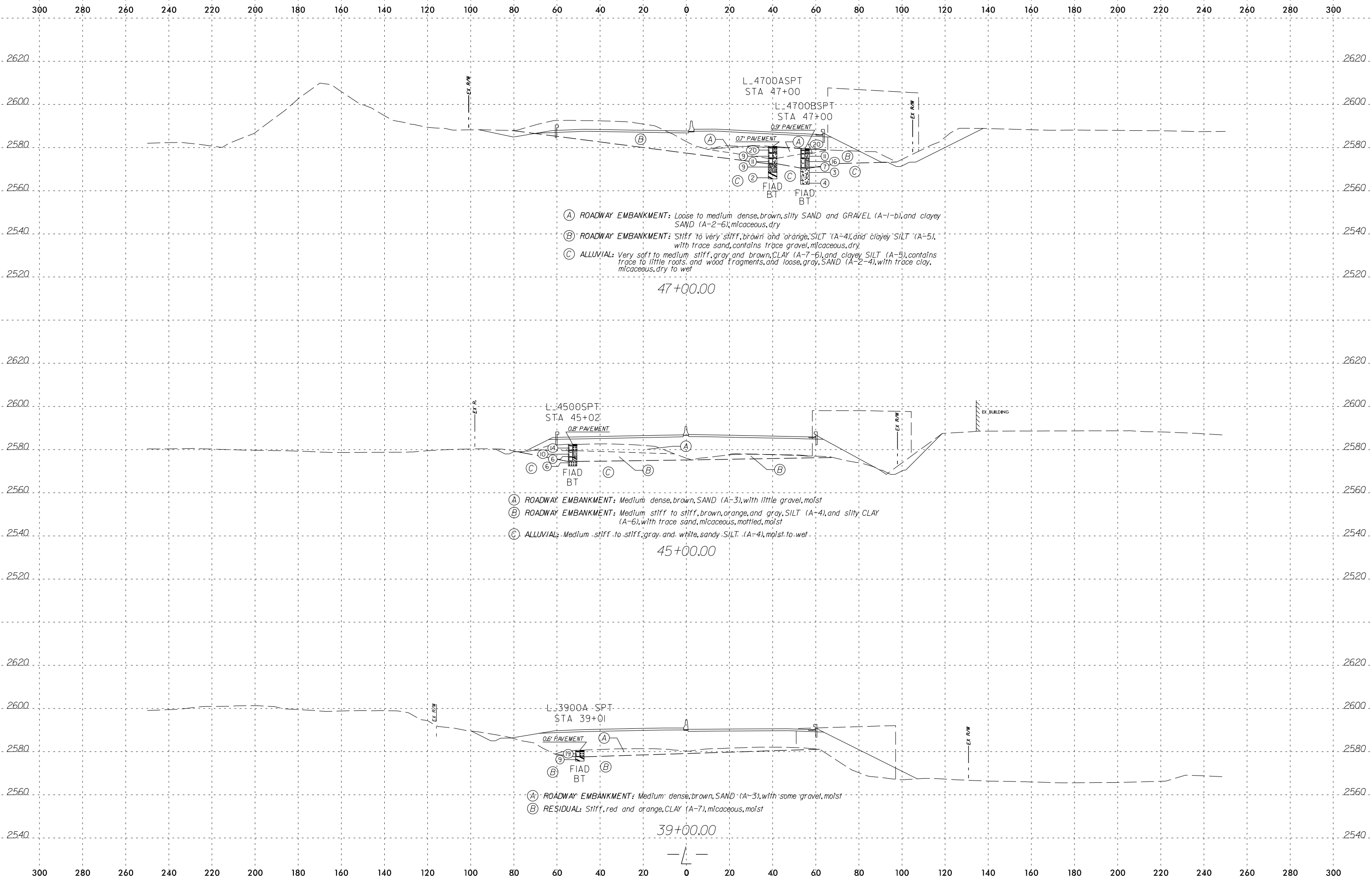
37+00.00

L_3500SPT
STA 35+00

ST-7

- (A) ROADWAY EMBANKMENT: Interpreted as roadway embankment
- (B) RESIDUAL: Medium stiff to very stiff, brown and red, silty CLAY (A-7-5 (16), A-7-6), and SILT (A-4), moist
- (C) RESIDUAL: Medium dense, brown, and tan, silty SAND (A-2-4), dry
- (D) WEATHERED ROCK: GNEISS
- (E) CRYSTALLINE ROCK: GNEISS

35+00.00



- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown, silty SAND and GRAVEL (A-1-b), and clayey SAND (A-2-6), micaceous, dry
- (B) ROADWAY EMBANKMENT: Stiff to very stiff, brown and orange, SILT (A-4), and clayey SILT (A-5), with trace sand, contains trace gravel, micaceous, dry
- (C) ALLUVIAL: Very soft to medium stiff, gray and brown, CLAY (A-7-6), and clayey SILT (A-5), contains trace to little roots and wood fragments, and loose, gray, SAND (A-2-4), with trace clay, micaceous, dry to wet

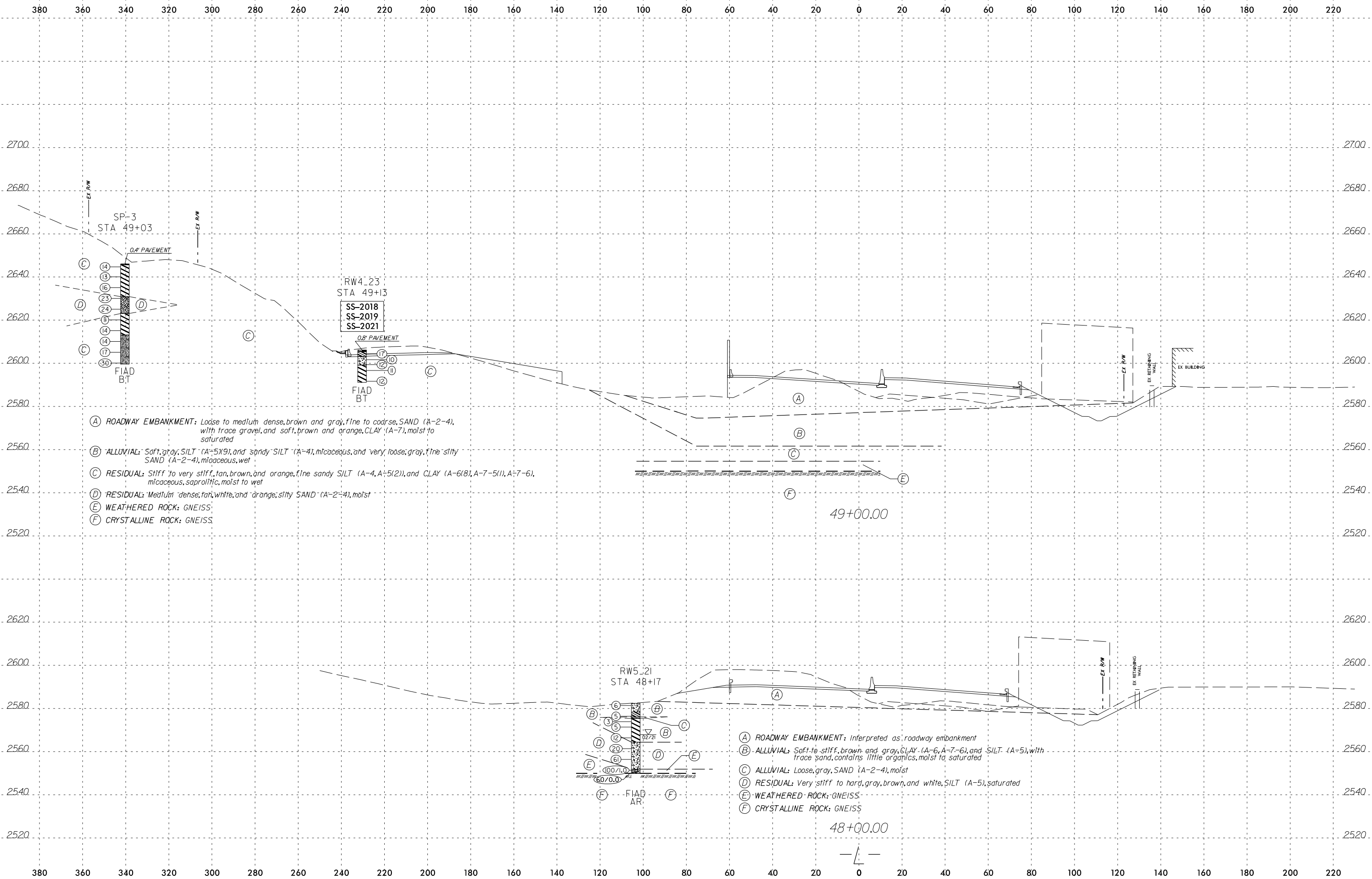
47+00.00

- (A) ROADWAY EMBANKMENT: Medium dense, brown, SAND (A-3), with little gravel, moist
- (B) ROADWAY EMBANKMENT: Medium stiff to stiff, brown, orange, and gray, SILT (A-4), and silty CLAY (A-6), with trace sand, micaceous, mottled, moist
- (C) ALLUVIAL: Medium stiff to stiff, gray and white, sandy, SILT (A-4), moist to wet

45+00.00

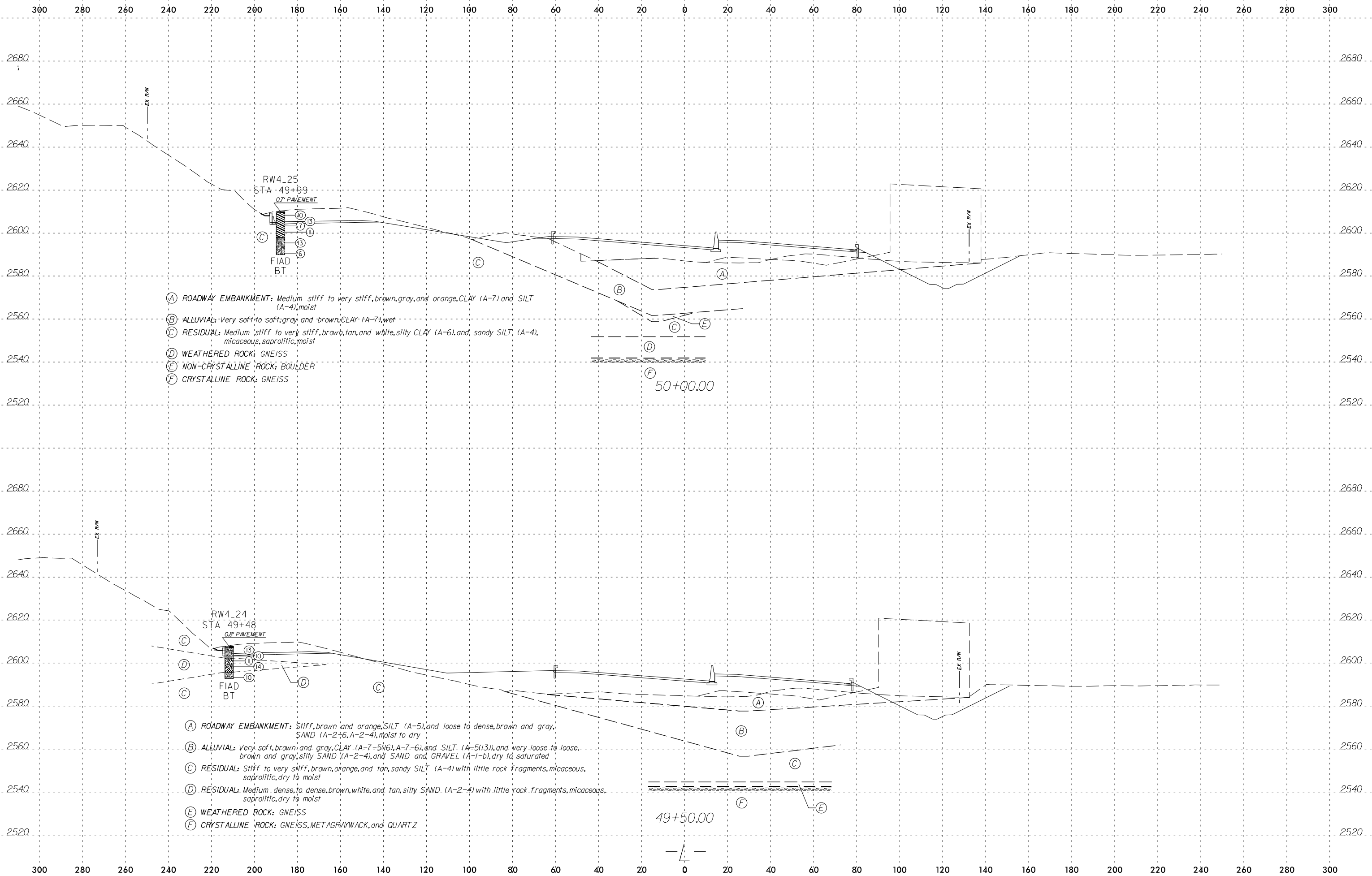
- (A) ROADWAY EMBANKMENT: Medium dense, brown, SAND (A-3), with some gravel, moist
- (B) RESIDUAL: Stiff, red and orange, CLAY (A-7), micaceous, moist

39+00.00



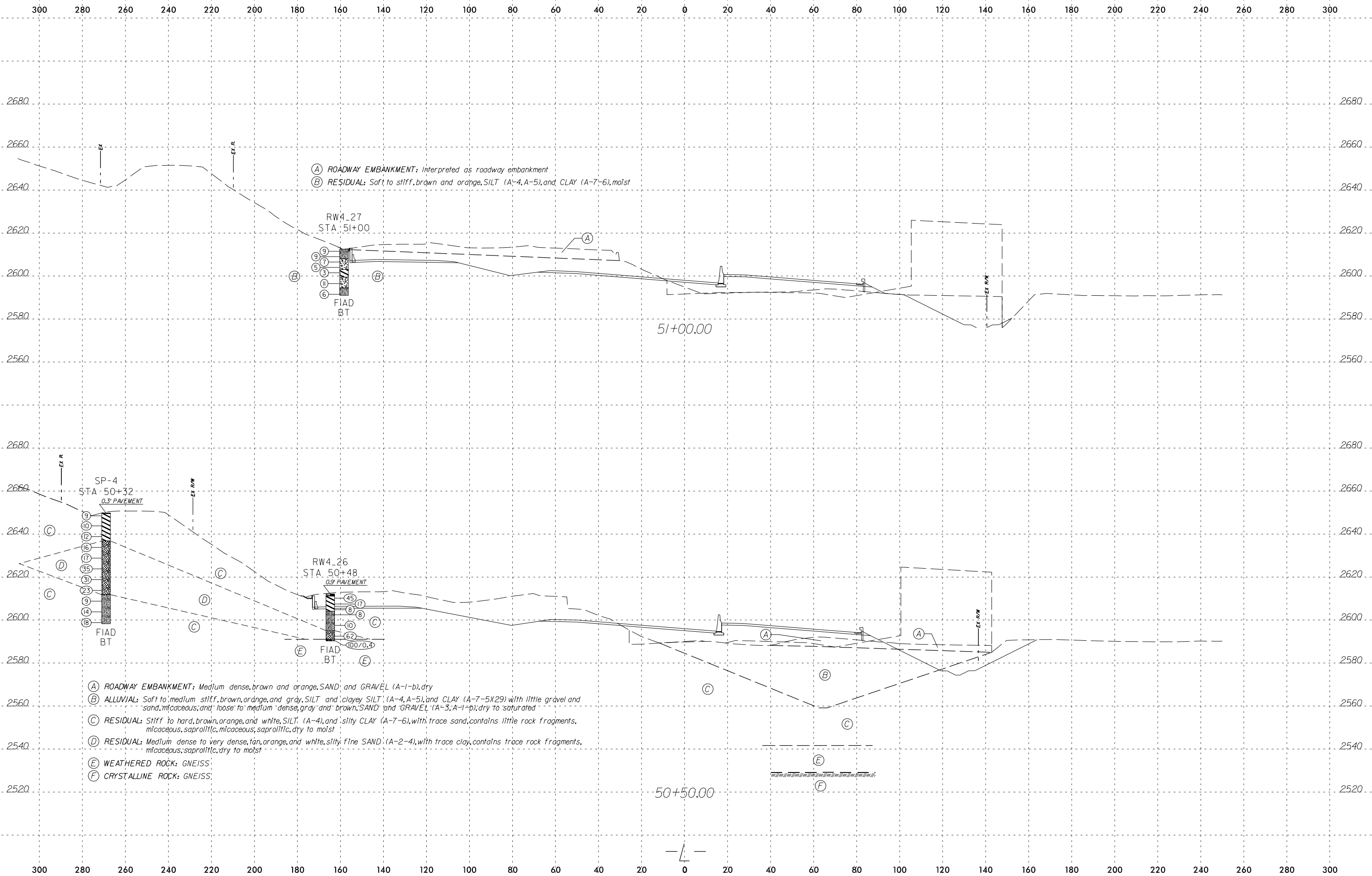
- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown and gray, fine to coarse, SAND (A-2-4), with trace gravel, and soft, brown and orange, CLAY (A-7), moist to saturated
- (B) ALLUVIAL: Soft, gray, SILT (A-5(9)), and sandy SILT (A-4), micaceous, and very loose, gray, fine silty SAND (A-2-4), micaceous, wet
- (C) RESIDUAL: Stiff to very stiff, tan, brown, and orange, fine sandy SILT (A-4, A-5(2)), and CLAY (A-6(8), A-7-5(1), A-7-6), micaceous, saprolitic, moist to wet
- (D) RESIDUAL: Medium dense, tan, white, and orange, silty SAND (A-2-4), moist
- (E) WEATHERED ROCK: GNEISS
- (F) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Interpreted as roadway embankment
- (B) ALLUVIAL: Soft to stiff, brown and gray, CLAY (A-6, A-7-6), and SILT (A-5), with trace sand, contains little organics, moist to saturated
- (C) ALLUVIAL: Loose, gray, SAND (A-2-4), moist
- (D) RESIDUAL: Very stiff to hard, gray, brown, and white, SILT (A-5), saturated
- (E) WEATHERED ROCK: GNEISS
- (F) CRYSTALLINE ROCK: GNEISS

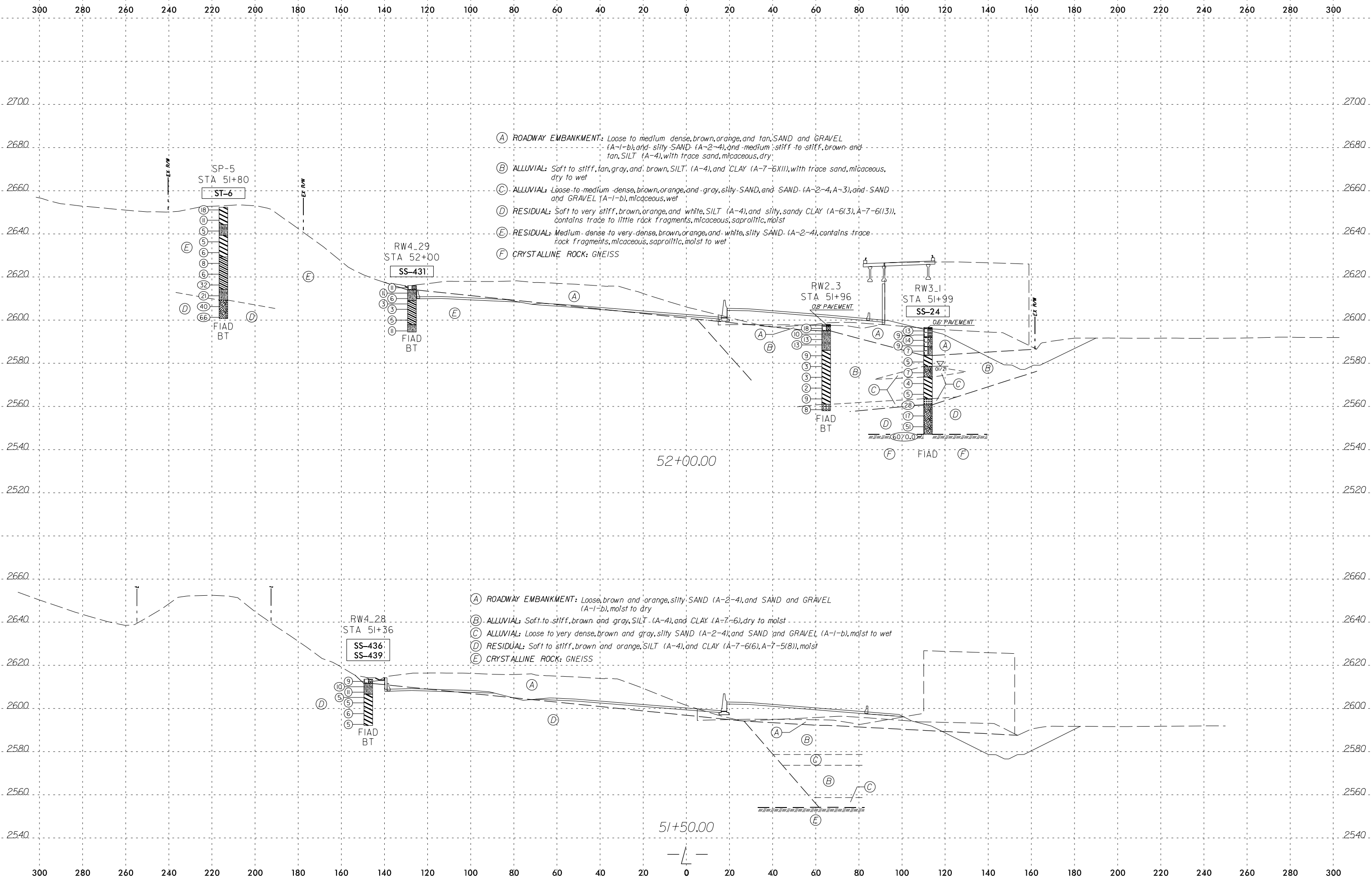


- (A) ROADWAY EMBANKMENT: Medium stiff to very stiff, brown, gray, and orange, CLAY (A-7) and SILT (A-4), moist
- (B) ALLUVIAL: Very soft to soft, gray and brown, CLAY (A-7), wet
- (C) RESIDUAL: Medium stiff to very stiff, brown, tan, and white, silty CLAY (A-6), and sandy SILT, (A-4), micaceous, saprolitic, moist
- (D) WEATHERED ROCK: GNEISS
- (E) NON-CRYSTALLINE ROCK: BOULDER
- (F) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Stiff, brown and orange, SILT (A-5), and loose to dense, brown and gray, SAND (A-2-6, A-2-4), moist to dry
- (B) ALLUVIAL: Very soft, brown and gray, CLAY (A-7-5(16), A-7-6), and SILT (A-5(13)), and very loose to loose, brown and gray, silty SAND (A-2-4), and SAND and GRAVEL (A-1-b), dry to saturated
- (C) RESIDUAL: Stiff to very stiff, brown, orange, and tan, sandy SILT (A-4) with little rock fragments, micaceous, saprolitic, dry to moist
- (D) RESIDUAL: Medium dense, to dense, brown, white, and tan, silty SAND (A-2-4) with little rock fragments, micaceous, saprolitic, dry to moist
- (E) WEATHERED ROCK: GNEISS
- (F) CRYSTALLINE ROCK: GNEISS, METAGRAYWACK, and QUARTZ

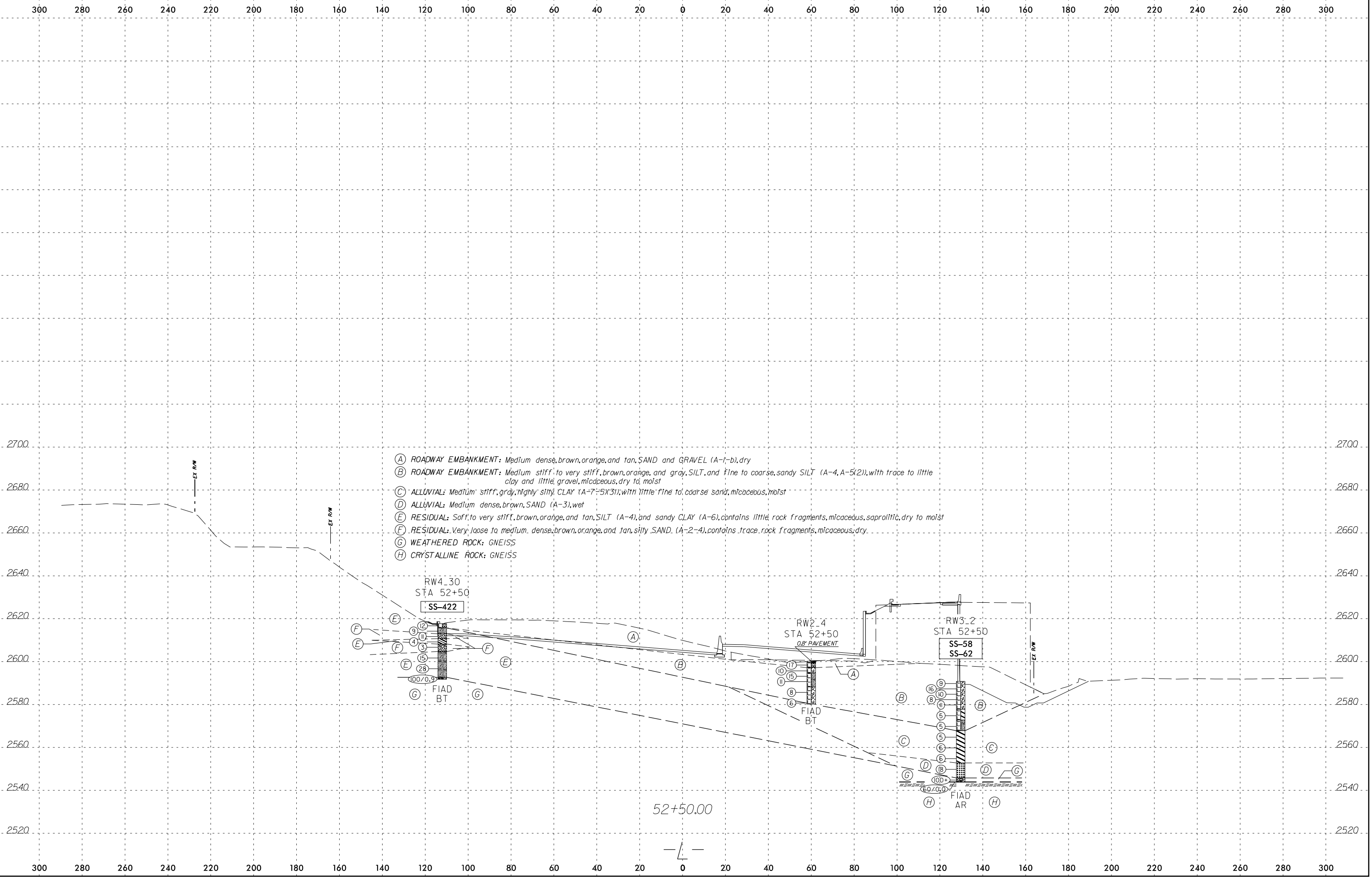


14:05 PM
B3186-B5898_RDY_XSI.L.dgn
B3186-B5898



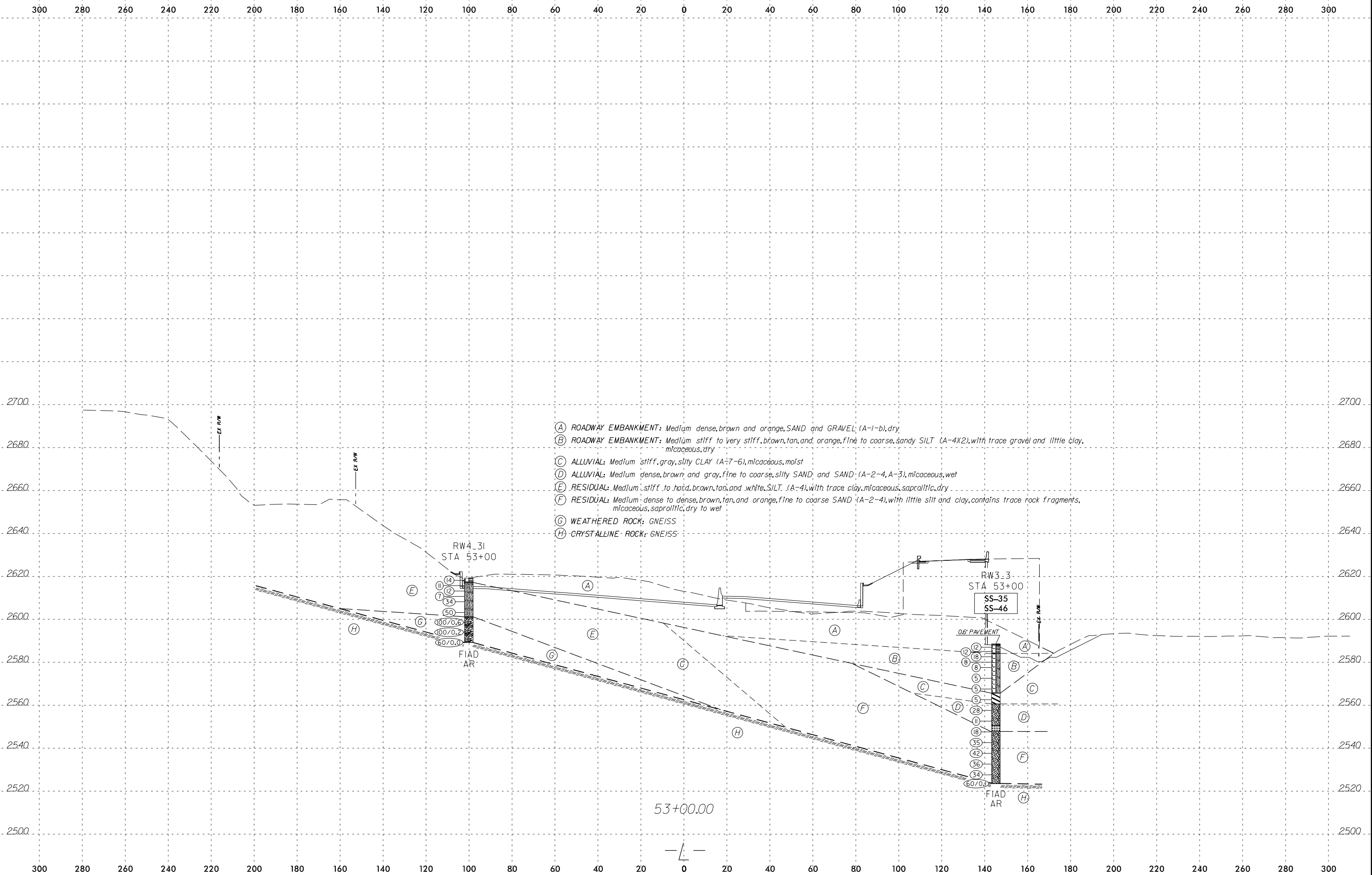
- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown, orange, and tan, SAND and GRAVEL (A-1-b), and silty SAND (A-2-4), and medium stiff to stiff, brown and tan, SILT (A-4), with trace sand, micaceous, dry
- (B) ALLUVIAL: Soft to stiff, tan, gray, and brown, SILT (A-4), and CLAY (A-7-6(XII)), with trace sand, micaceous, dry to wet
- (C) ALLUVIAL: Loose to medium dense, brown, orange, and gray, silty SAND, and SAND (A-2-4, A-3), and SAND and GRAVEL (A-1-b), micaceous, wet
- (D) RESIDUAL: Soft to very stiff, brown, orange, and white, SILT (A-4), and silty, sandy CLAY (A-6(3), A-7-6(13)), contains trace to little rock fragments, micaceous, saprolitic, moist
- (E) RESIDUAL: Medium dense to very dense, brown, orange, and white, silty SAND (A-2-4), contains trace rock fragments, micaceous, saprolitic, moist to wet
- (F) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Loose, brown and orange, silty SAND (A-2-4), and SAND and GRAVEL (A-1-b), moist to dry
- (B) ALLUVIAL: Soft to stiff, brown and gray, SILT (A-4), and CLAY (A-7-6), dry to moist
- (C) ALLUVIAL: Loose to very dense, brown and gray, silty SAND (A-2-4), and SAND and GRAVEL (A-1-b), moist to wet
- (D) RESIDUAL: Soft to stiff, brown and orange, SILT (A-4), and CLAY (A-7-6(6), A-7-5(8)), moist
- (E) CRYSTALLINE ROCK: GNEISS



- (A) ROADWAY EMBANKMENT: Medium dense, brown, orange, and tan, SAND and GRAVEL (A-1-b), dry
- (B) ROADWAY EMBANKMENT: Medium stiff to very stiff, brown, orange, and gray, SILT, and fine to coarse, sandy SILT (A-4, A-5(2)), with trace to little clay and little gravel, micaceous, dry to moist
- (C) ALLUVIAL: Medium stiff, gray, highly silty CLAY (A-7-5(3)), with little fine to coarse sand, micaceous, moist
- (D) ALLUVIAL: Medium dense, brown, SAND (A-3), wet
- (E) RESIDUAL: Soft to very stiff, brown, orange, and tan, SILT (A-4), and sandy CLAY (A-6), contains little rock fragments, micaceous, saprolitic, dry to moist
- (F) RESIDUAL: Very loose to medium dense, brown, orange, and tan, silty SAND (A-2-4), contains trace rock fragments, micaceous, dry
- (G) WEATHERED ROCK: GNEISS
- (H) CRYSTALLINE ROCK: GNEISS

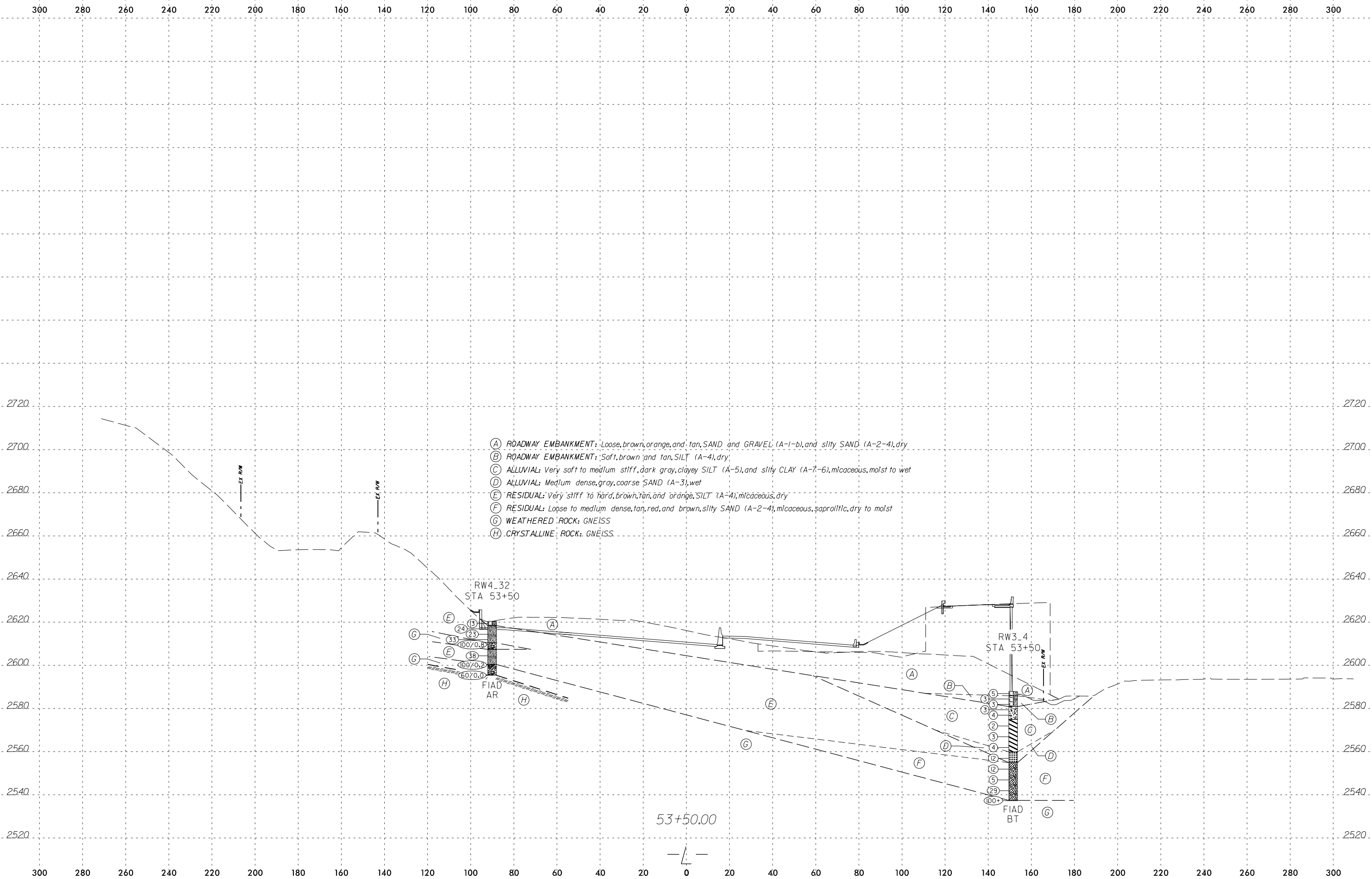
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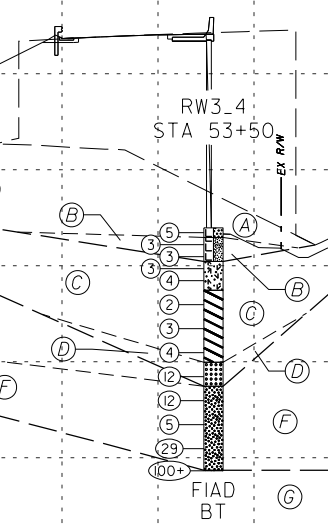
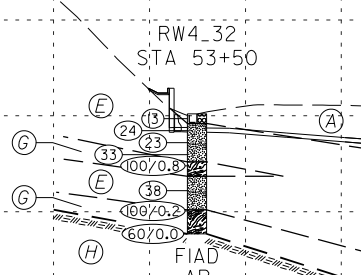
- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) ROADWAY EMBANKMENT: Medium stiff to very stiff, brown, tan, and, orange, fine to coarse, sandy SILT (A-4X2), with trace gravel and little clay, micaceous, dry
- (C) ALLUVIAL: Medium stiff, gray, silty CLAY (A-7-6), micaceous, moist
- (D) ALLUVIAL: Medium dense, brown and gray, fine to coarse, silty SAND, and SAND (A-2-4, A-3), micaceous, wet
- (E) RESIDUAL: Medium stiff to hard, brown, tan, and white, SILT (A-4), with trace clay, micaceous, saprolitic, dry
- (F) RESIDUAL: Medium dense to dense, brown, tan, and orange, fine to coarse SAND (A-2-4), with little silt and clay, contains trace rock fragments, micaceous, saprolitic, dry to wet
- (G) WEATHERED ROCK: GNEISS
- (H) CRYSTALLINE ROCK: GNEISS

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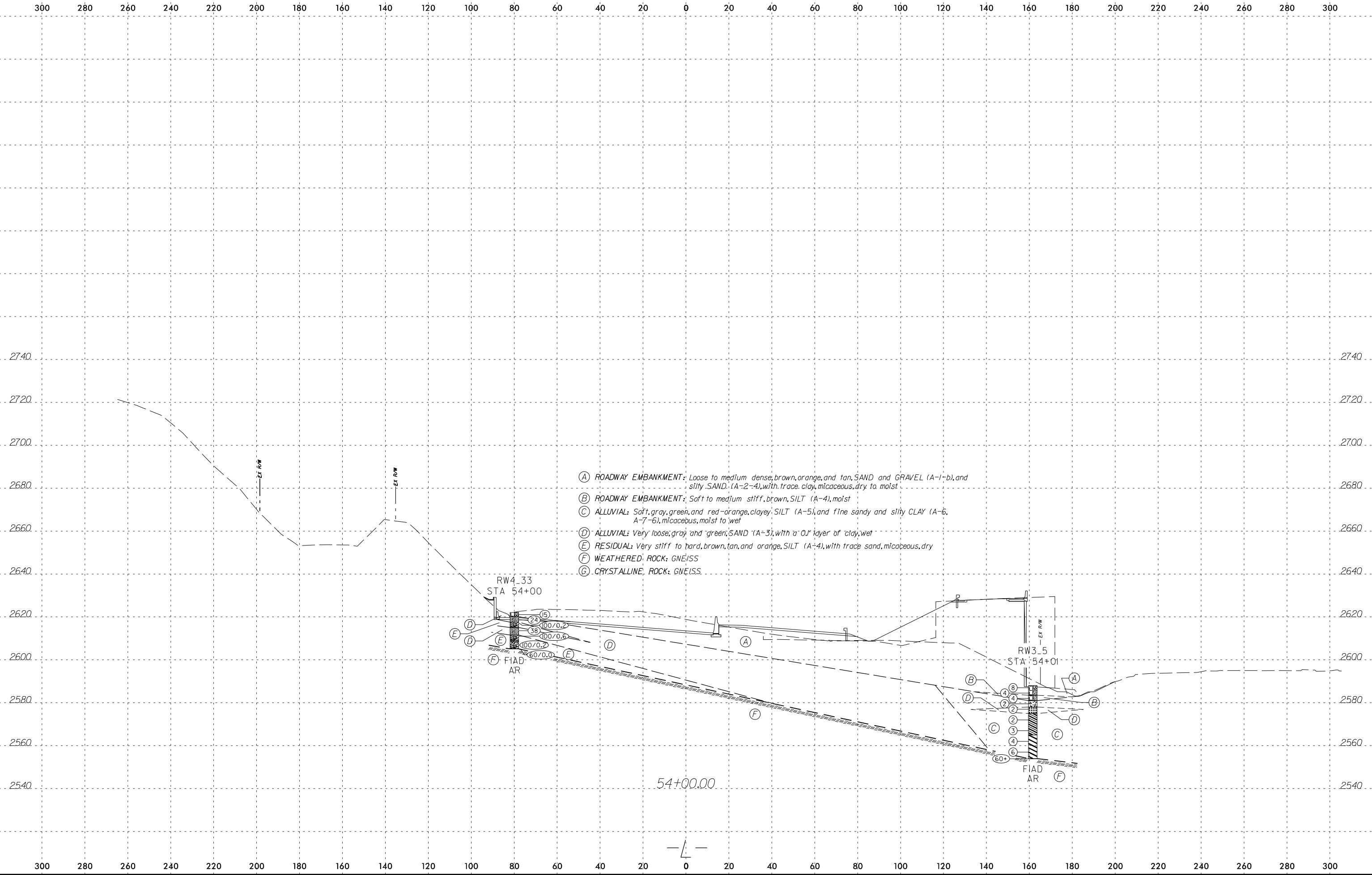
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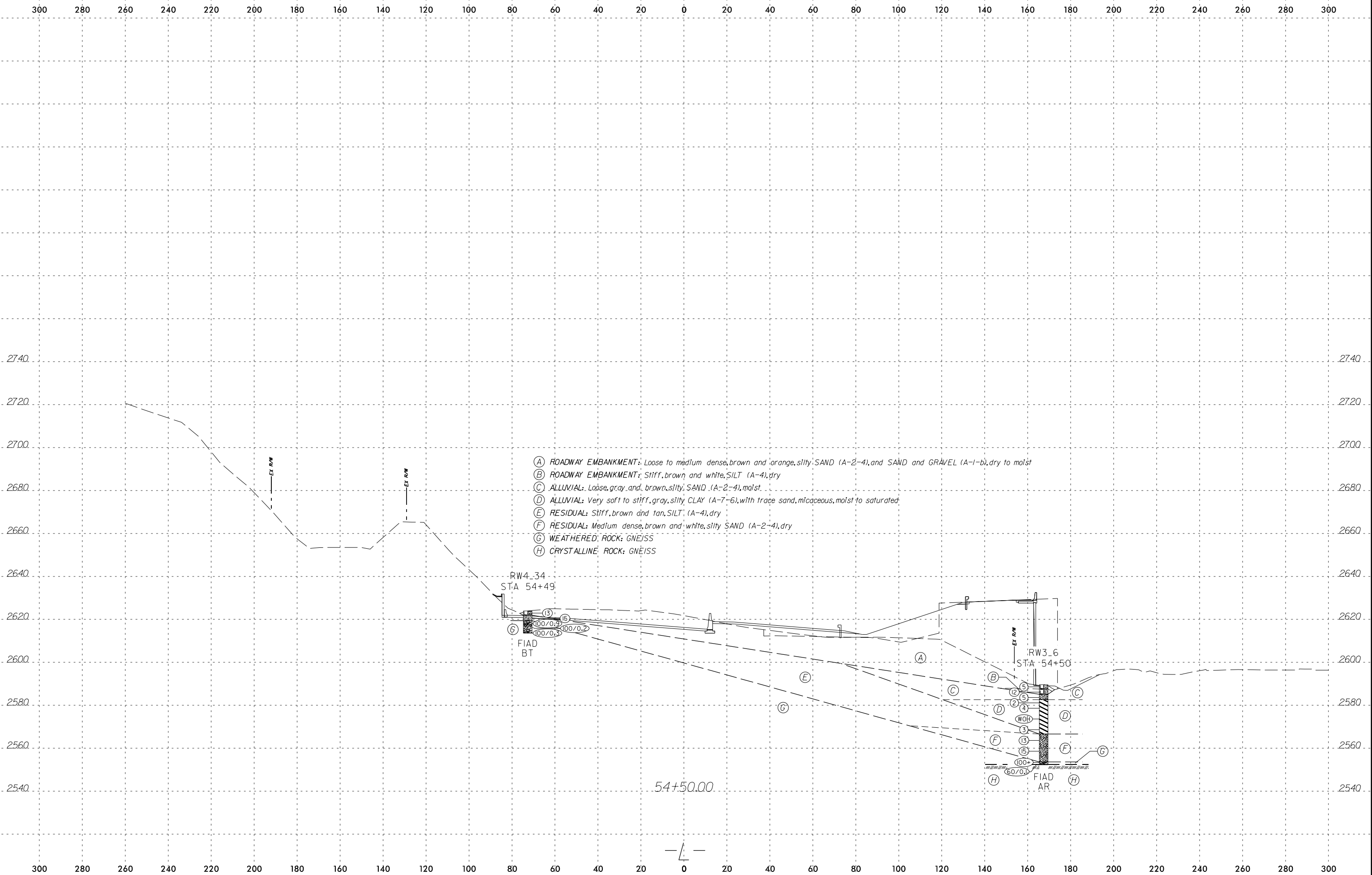
- (A) ROADWAY EMBANKMENT: Loose, brown, orange, and tan, SAND and GRAVEL (A-1-b), and silty SAND (A-2-4), dry
- (B) ROADWAY EMBANKMENT: Soft, brown and tan, SILT (A-4), dry
- (C) ALLUVIAL: Very soft to medium stiff, dark gray, clayey SILT (A-5), and silty CLAY (A-7-6), micaceous, moist to wet
- (D) ALLUVIAL: Medium dense, gray, coarse SAND (A-3), wet
- (E) RESIDUAL: Very stiff to hard, brown, tan, and orange, SILT (A-4), micaceous, dry
- (F) RESIDUAL: Loose to medium dense, tan, red, and brown, silty SAND (A-2-4), micaceous, saprolitic, dry to moist
- (G) WEATHERED ROCK: GNEISS
- (H) CRYSTALLINE ROCK: GNEISS



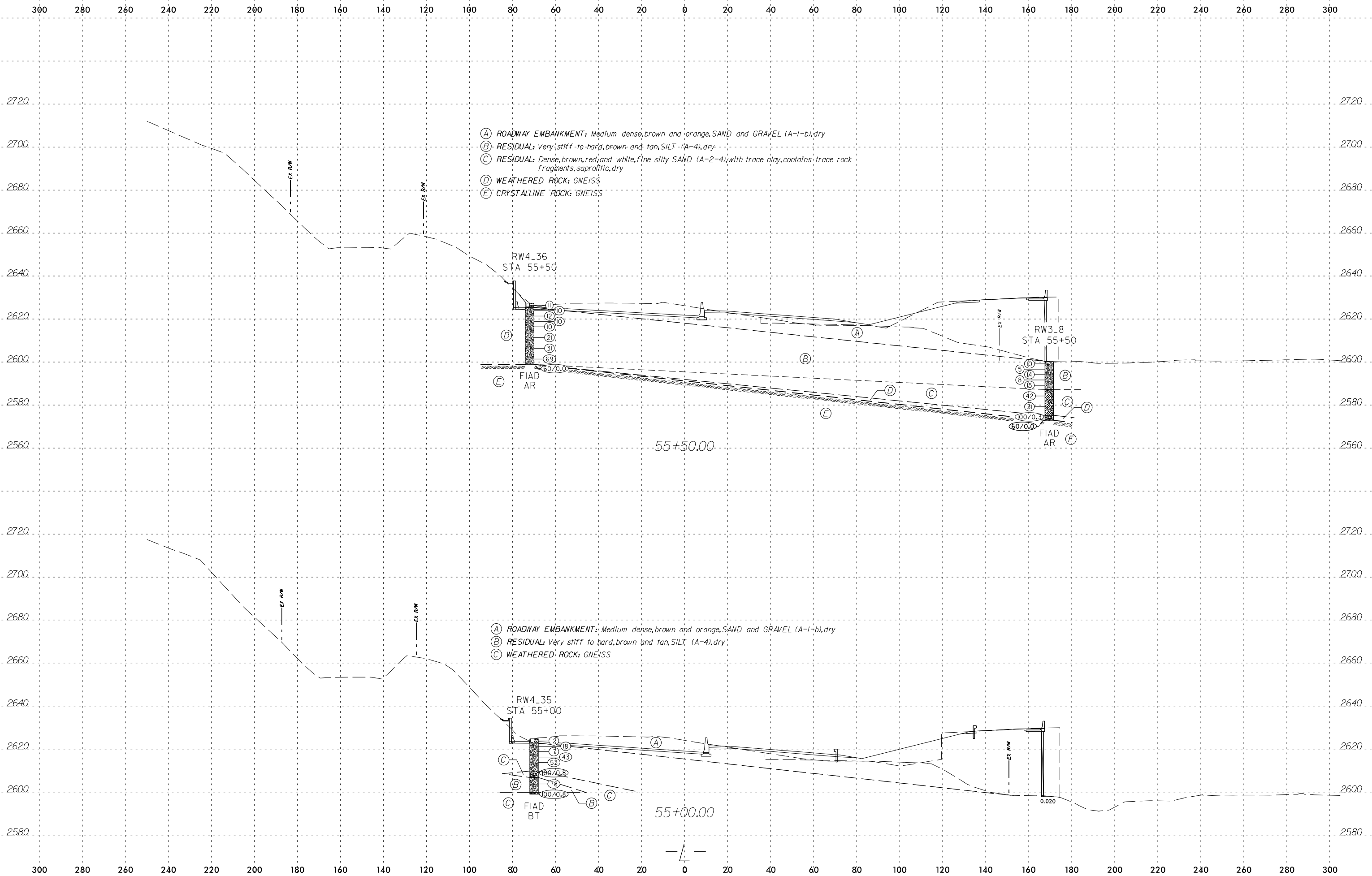
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- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown, orange, and tan, SAND and GRAVEL (A-1-b), and silty SAND, (A-2-4), with trace clay, micaceous, dry to moist
- (B) ROADWAY EMBANKMENT: Soft to medium stiff, brown, SILT (A-4), moist
- (C) ALLUVIAL: Soft, gray, green, and red-orange, clayey, SILT (A-5), and fine sandy and silty CLAY (A-6, A-7-6), micaceous, moist to wet
- (D) ALLUVIAL: Very loose, gray and green, SAND (A-3), with a "0.1" layer of clay, wet
- (E) RESIDUAL: Very stiff to hard, brown, tan, and orange, SILT (A-4), with trace sand, micaceous, dry
- (F) WEATHERED ROCK: GNEISS
- (G) CRYSTALLINE ROCK: GNEISS

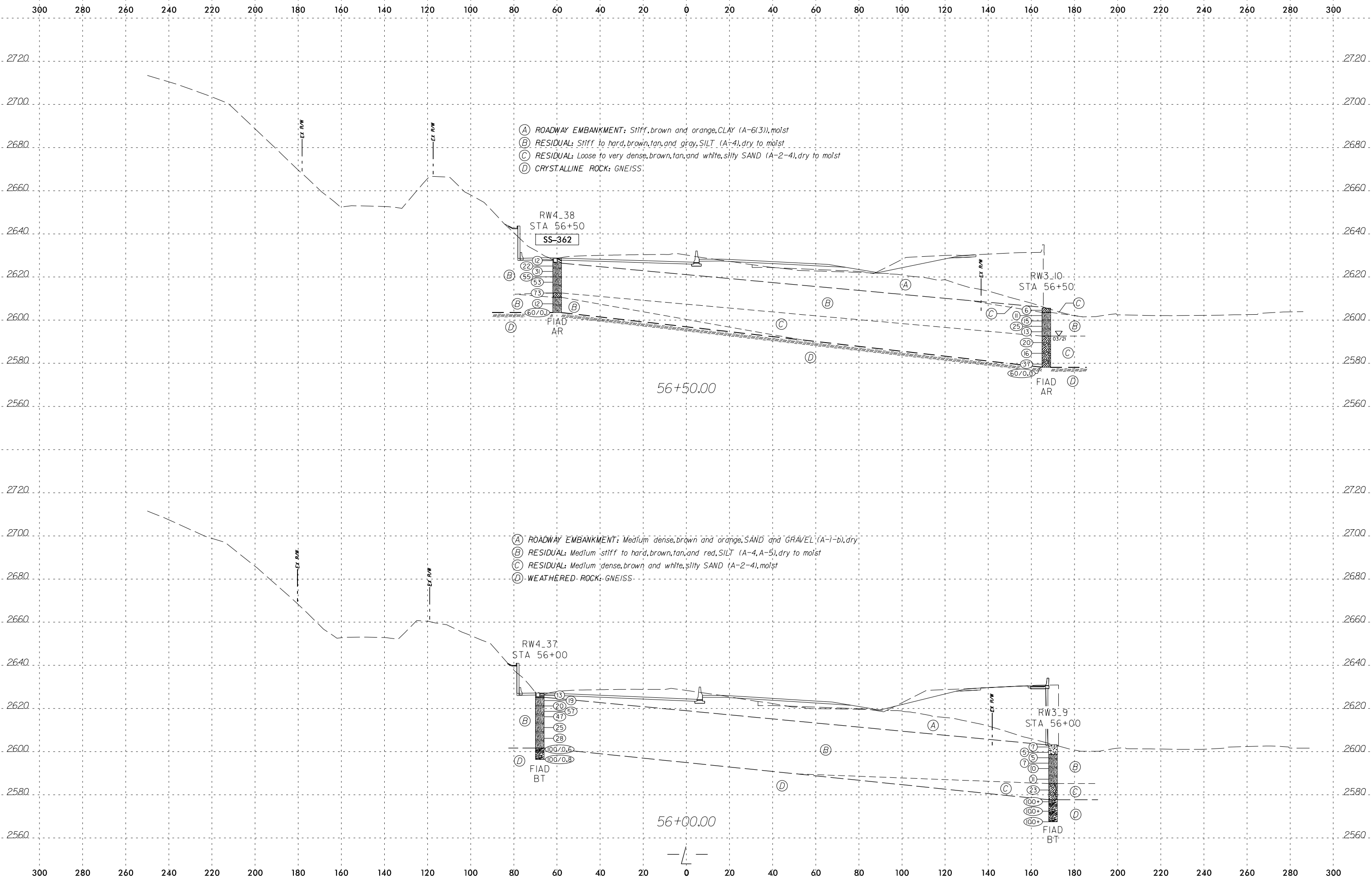


- (A) ROADWAY EMBANKMENT: Loose to medium dense, brown and orange, silty SAND (A-2-4), and SAND and GRAVEL (A-1-b), dry to moist
- (B) ROADWAY EMBANKMENT: Stiff, brown and white, SILT (A-4), dry
- (C) ALLUVIAL: Loose, gray and brown, silty SAND (A-2-4), moist
- (D) ALLUVIAL: Very soft to stiff, gray, silty CLAY (A-7-6), with trace sand, micaceous, moist to saturated
- (E) RESIDUAL: Stiff, brown and tan, SILT (A-4), dry
- (F) RESIDUAL: Medium dense, brown and white, silty SAND (A-2-4), dry
- (G) WEATHERED, ROCK: GNEISS
- (H) CRYSTALLINE, ROCK: GNEISS



- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Very stiff to hard, brown and tan, SILT (A-4), dry
- (C) RESIDUAL: Dense, brown, red, and white, fine silty SAND (A-2-4), with trace clay, contains trace rock fragments, saprolitic, dry
- (D) WEATHERED ROCK: GNEISS
- (E) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Very stiff to hard, brown and tan, SILT (A-4), dry
- (C) WEATHERED ROCK: GNEISS



- (A) ROADWAY EMBANKMENT: Stiff, brown and orange, CLAY (A-6(3)), moist
- (B) RESIDUAL: Stiff to hard, brown, tan, and gray, SILT (A-4), dry to moist
- (C) RESIDUAL: Loose to very dense, brown, tan, and white, silty SAND (A-2-4), dry to moist
- (D) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Medium stiff to hard, brown, tan, and red, SILT (A-4, A-5), dry to moist
- (C) RESIDUAL: Medium dense, brown and white, silty SAND (A-2-4), moist
- (D) WEATHERED ROCK: GNEISS

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RW3_10
STA 56+50

RW4_37
STA 56+00

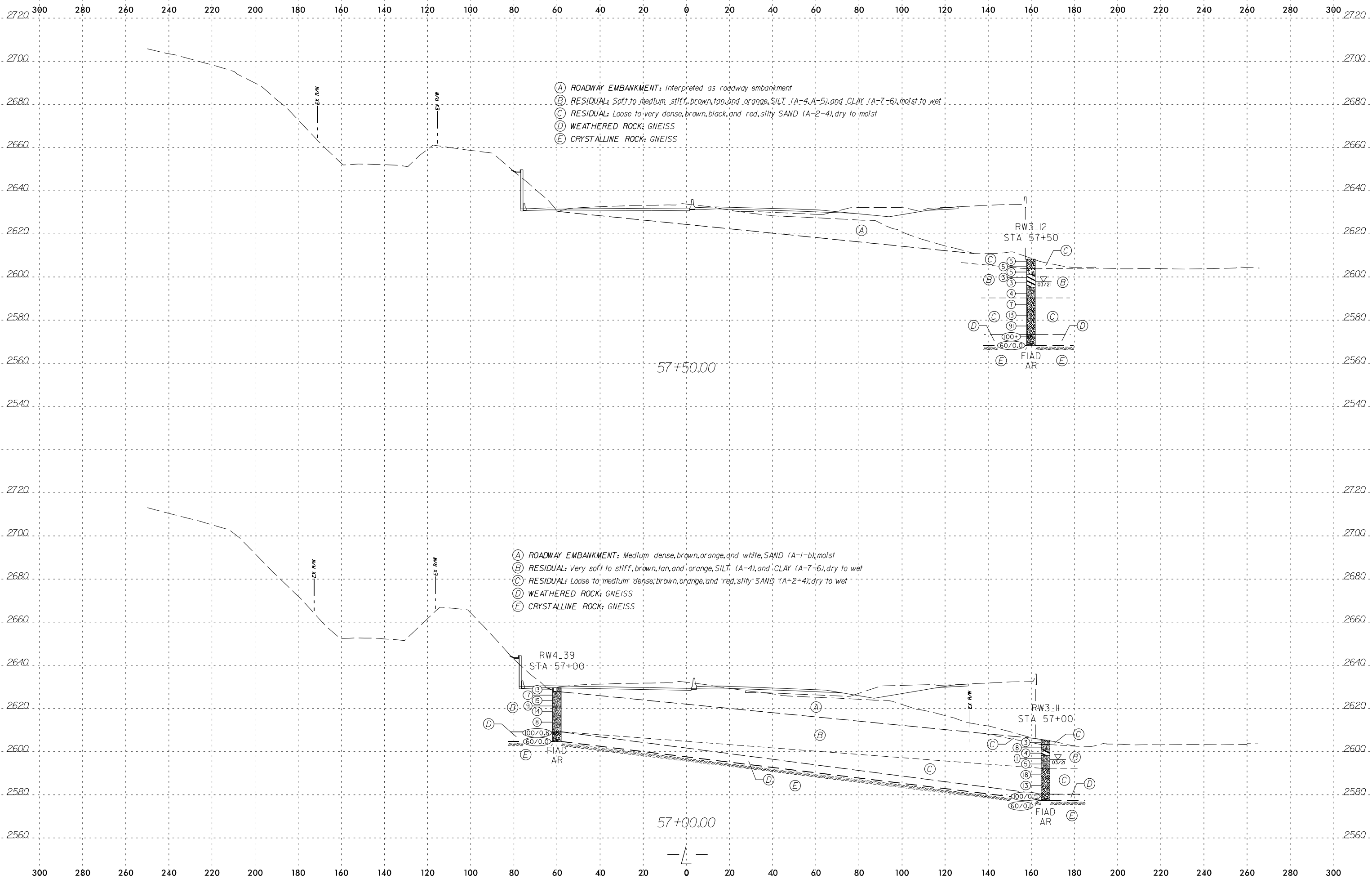
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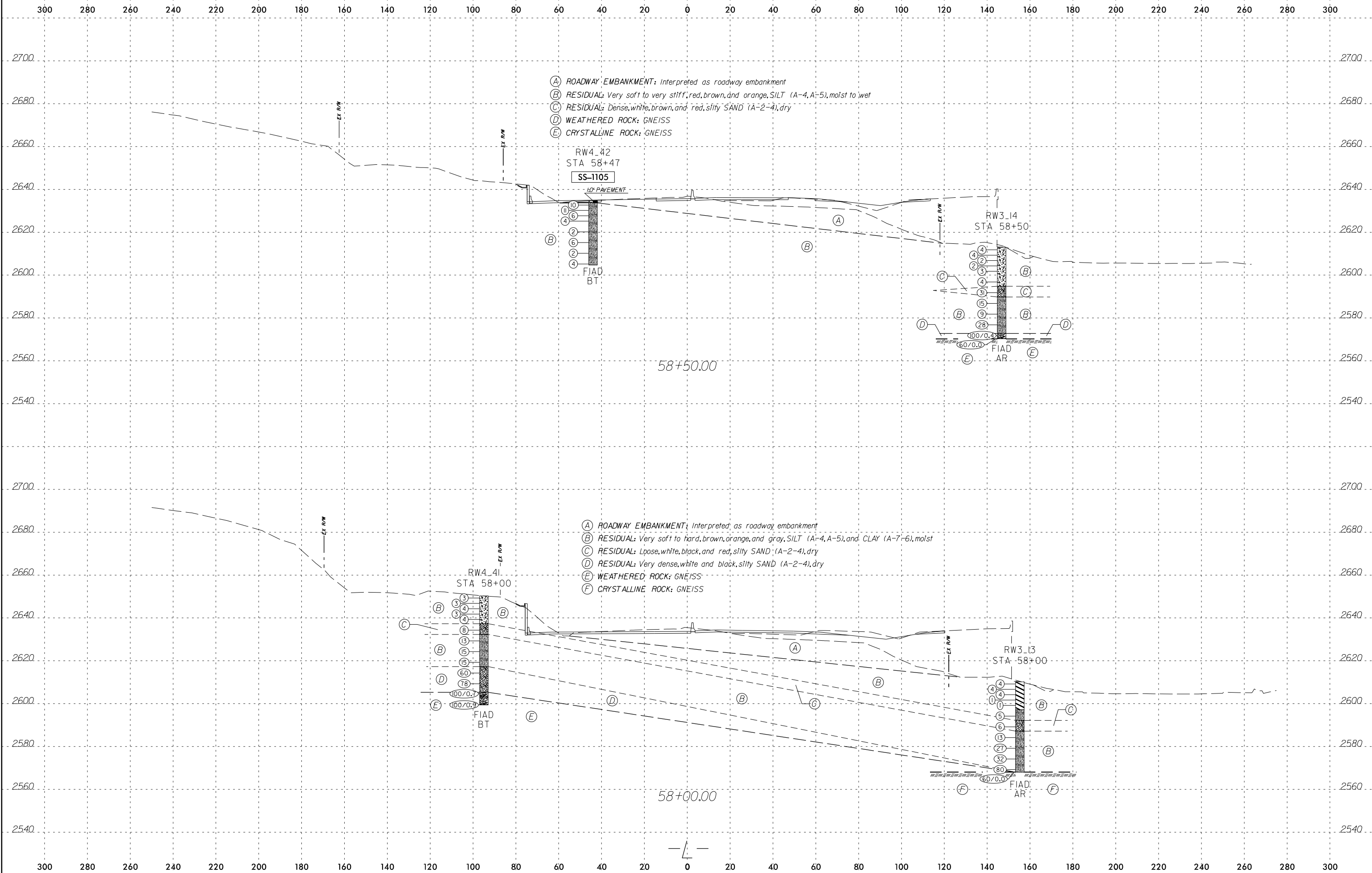
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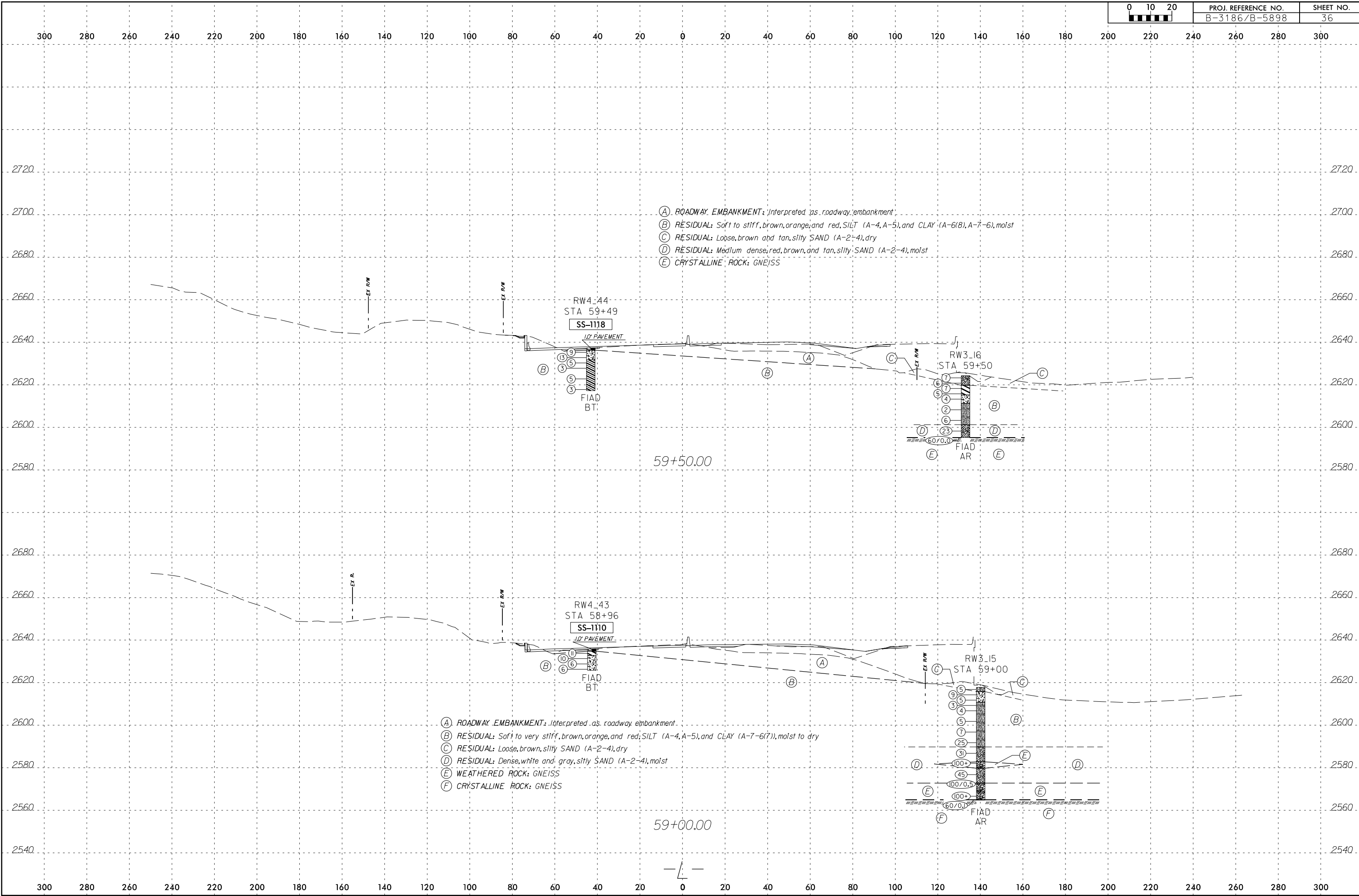
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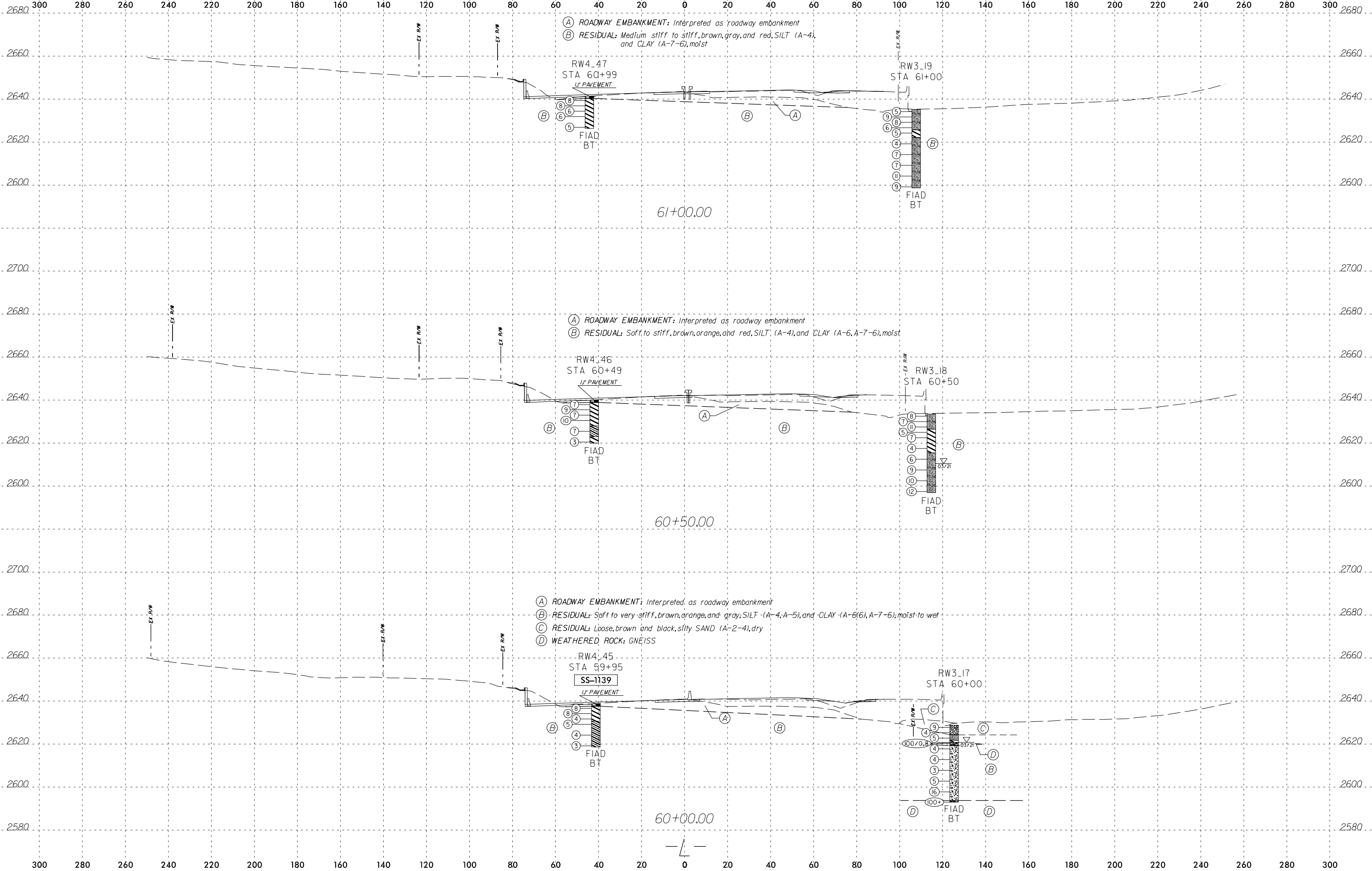
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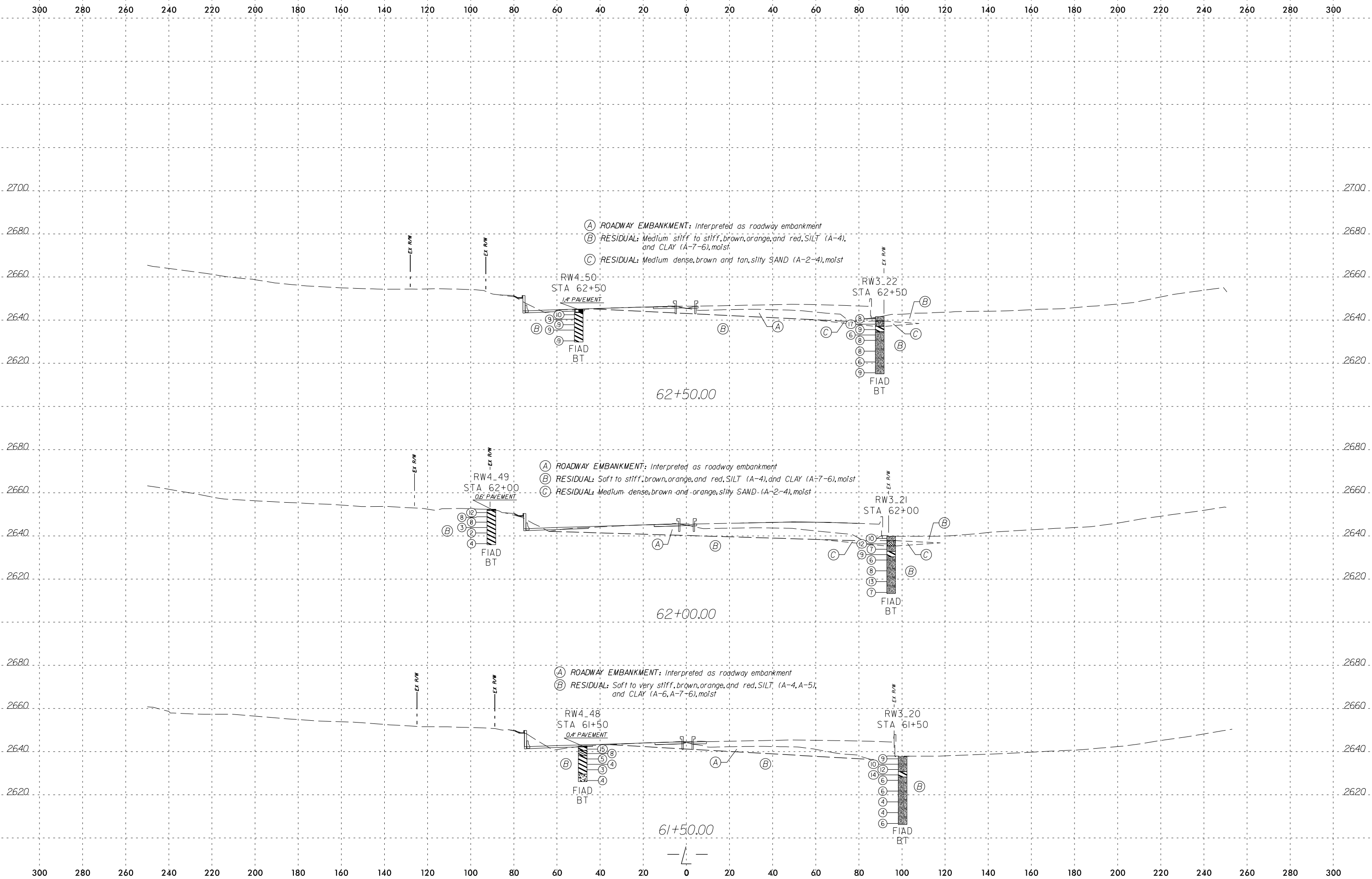




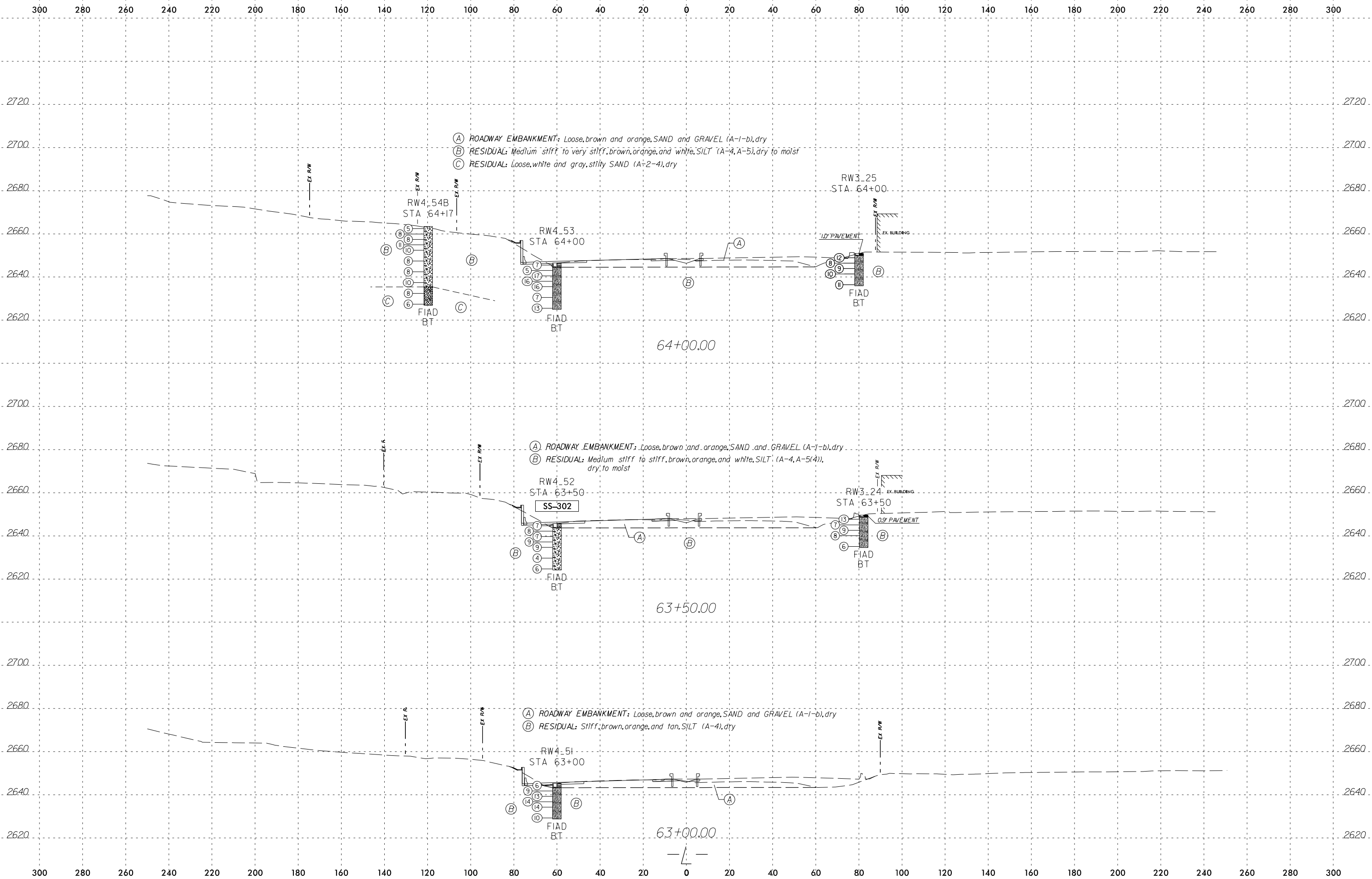
(A) ROADWAY EMBANKMENT: Interpreted as roadway embankment
 (B) RESIDUAL: Medium stiff to stiff, brown, gray, and red, SILT (A-4), and CLAY (A-7-6), moist

(A) ROADWAY EMBANKMENT: Interpreted as roadway embankment
 (B) RESIDUAL: Soft to stiff, brown, orange, and red, SILT (A-4), and CLAY (A-6, A-7-6), moist

(A) ROADWAY EMBANKMENT: Interpreted as roadway embankment
 (B) RESIDUAL: Soft to very stiff, brown, orange, and gray, SILT (A-4, A-5), and CLAY (A-6(6), A-7-6), moist to wet
 (C) RESIDUAL: Loose, brown and black, silty SAND (A-2-4), dry
 (D) WEATHERED ROCK: GNEISS



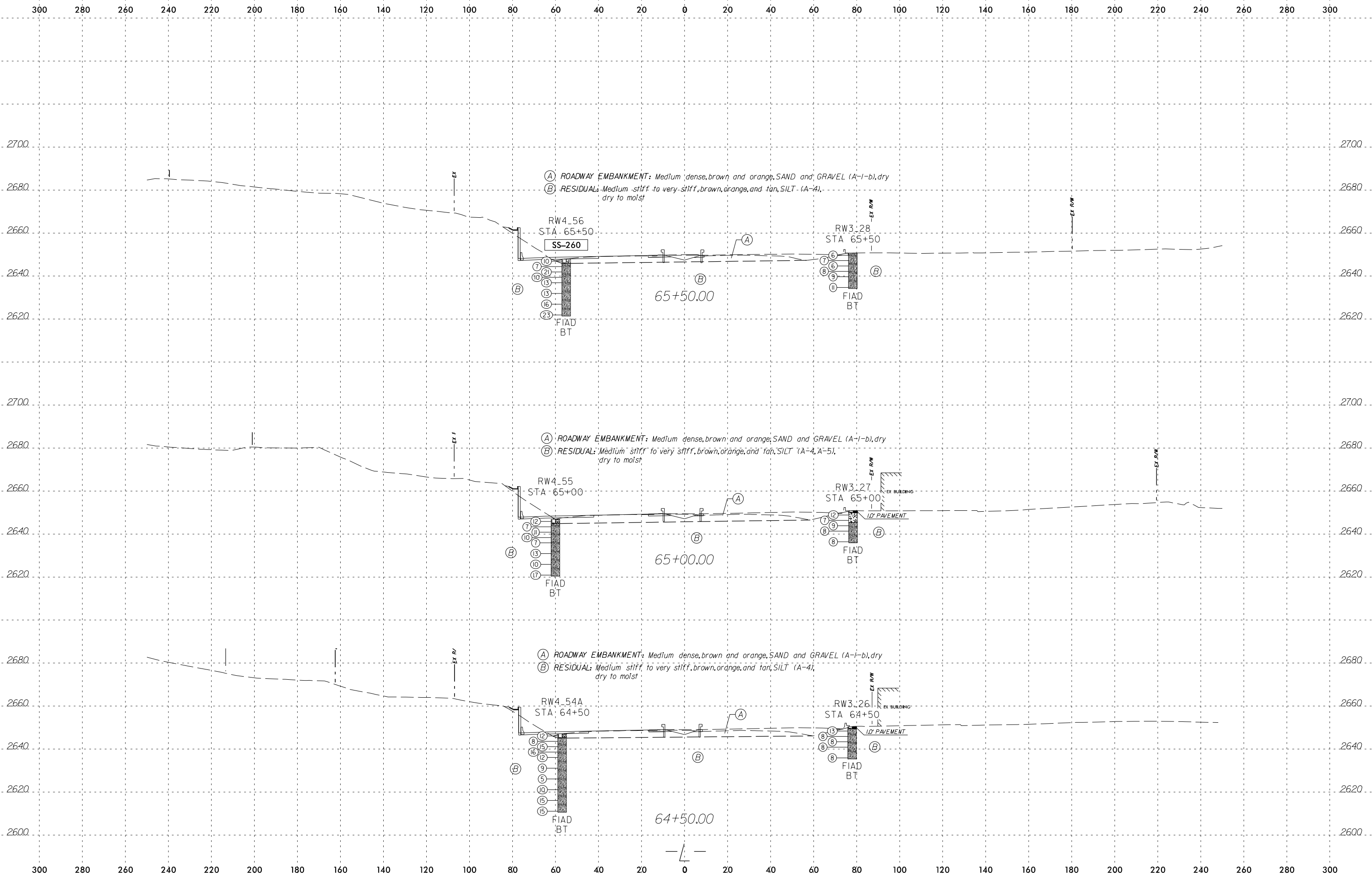
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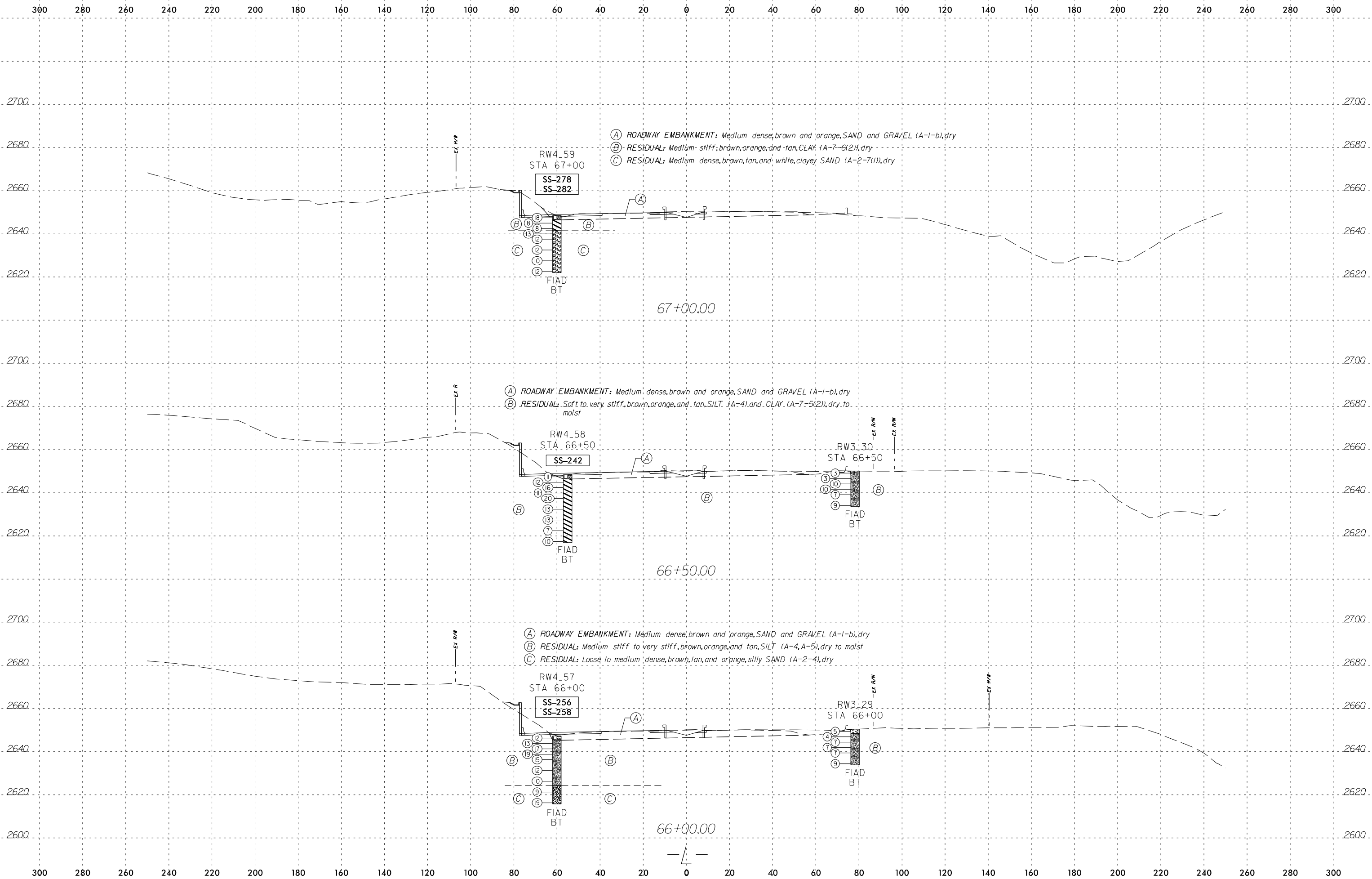


- (A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Medium stiff to very stiff, brown, orange, and white, SILT (A-4, A-5), dry to moist
- (C) RESIDUAL: Loose, white and gray, silty SAND (A-2-4), dry

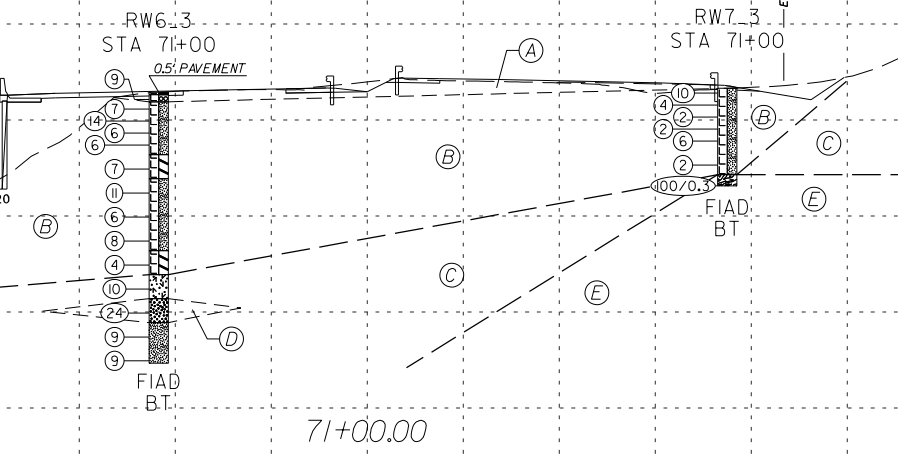
- (A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Medium stiff to stiff, brown, orange, and white, SILT (A-4, A-5(4)), dry, to moist

- (A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff, brown, orange, and tan, SILT (A-4), dry

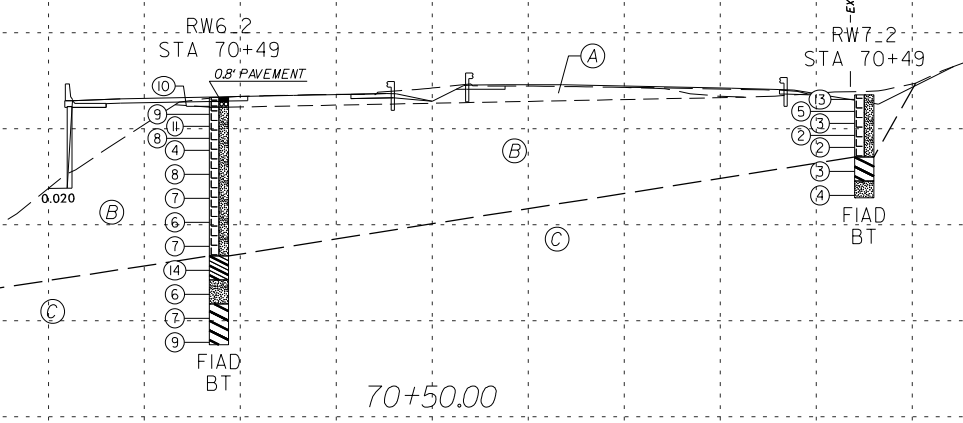


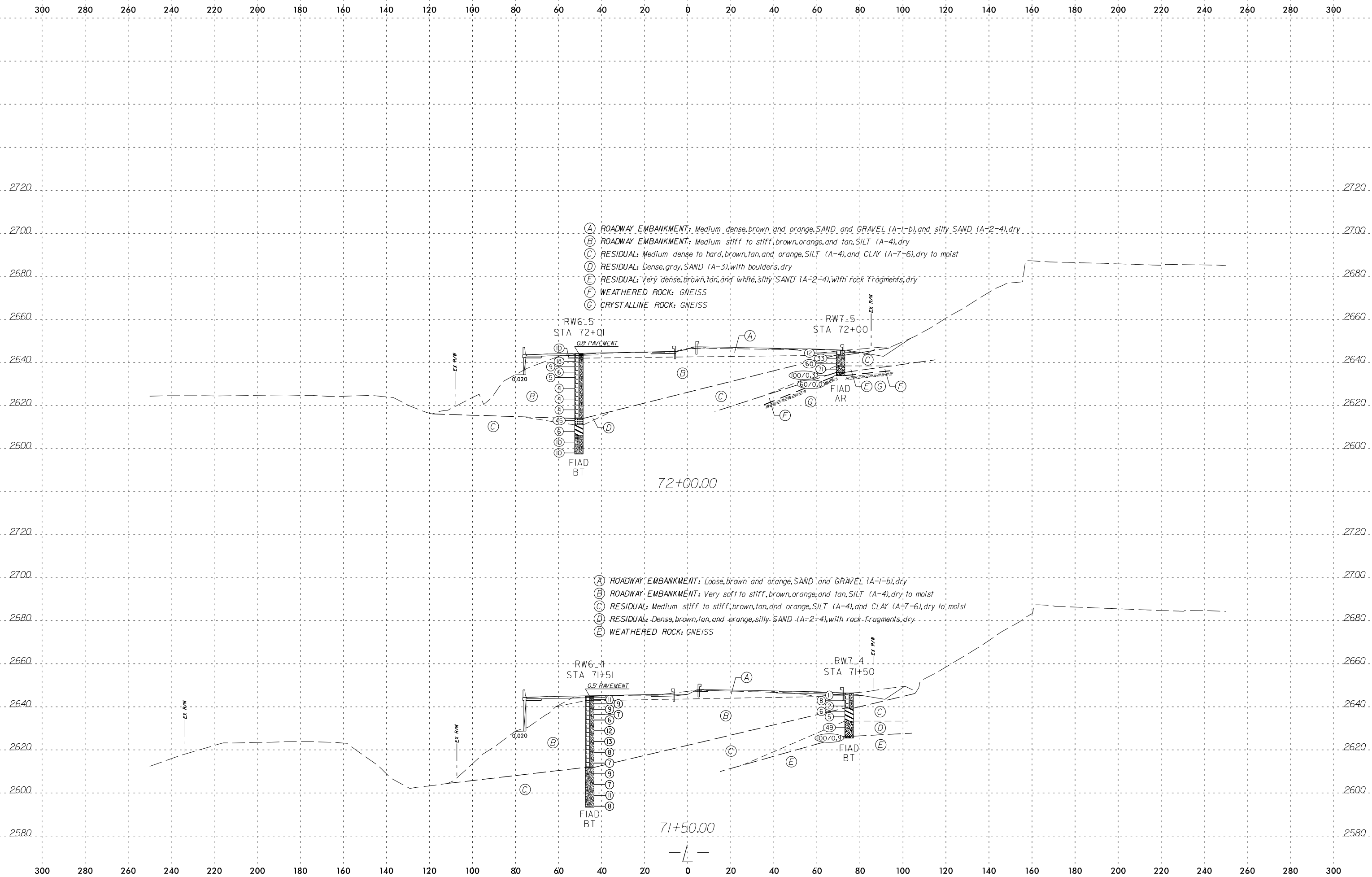


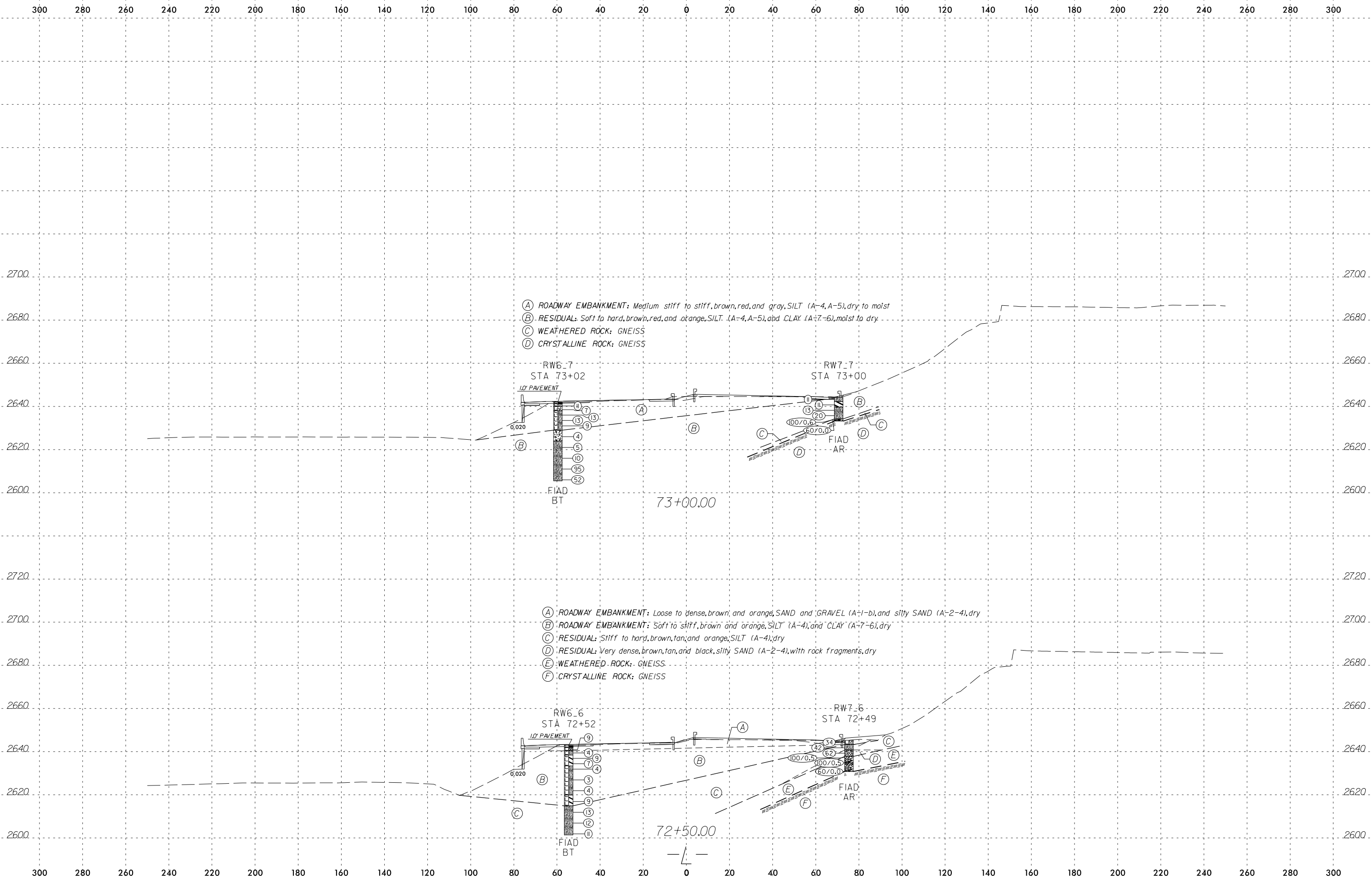
- (A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) ROADWAY EMBANKMENT: Soft to stiff, brown, orange, and tan, SILT (A-4), and CLAY (A-7-6), dry to moist
- (C) RESIDUAL: Stiff, brown, tan, and orange, SILT (A-5, A-4), dry to moist
- (D) WEATHERED ROCK: GNEISS
- (E) RESIDUAL: Medium dense, brown and orange, silty SAND (A-2-4), with boulders, dry



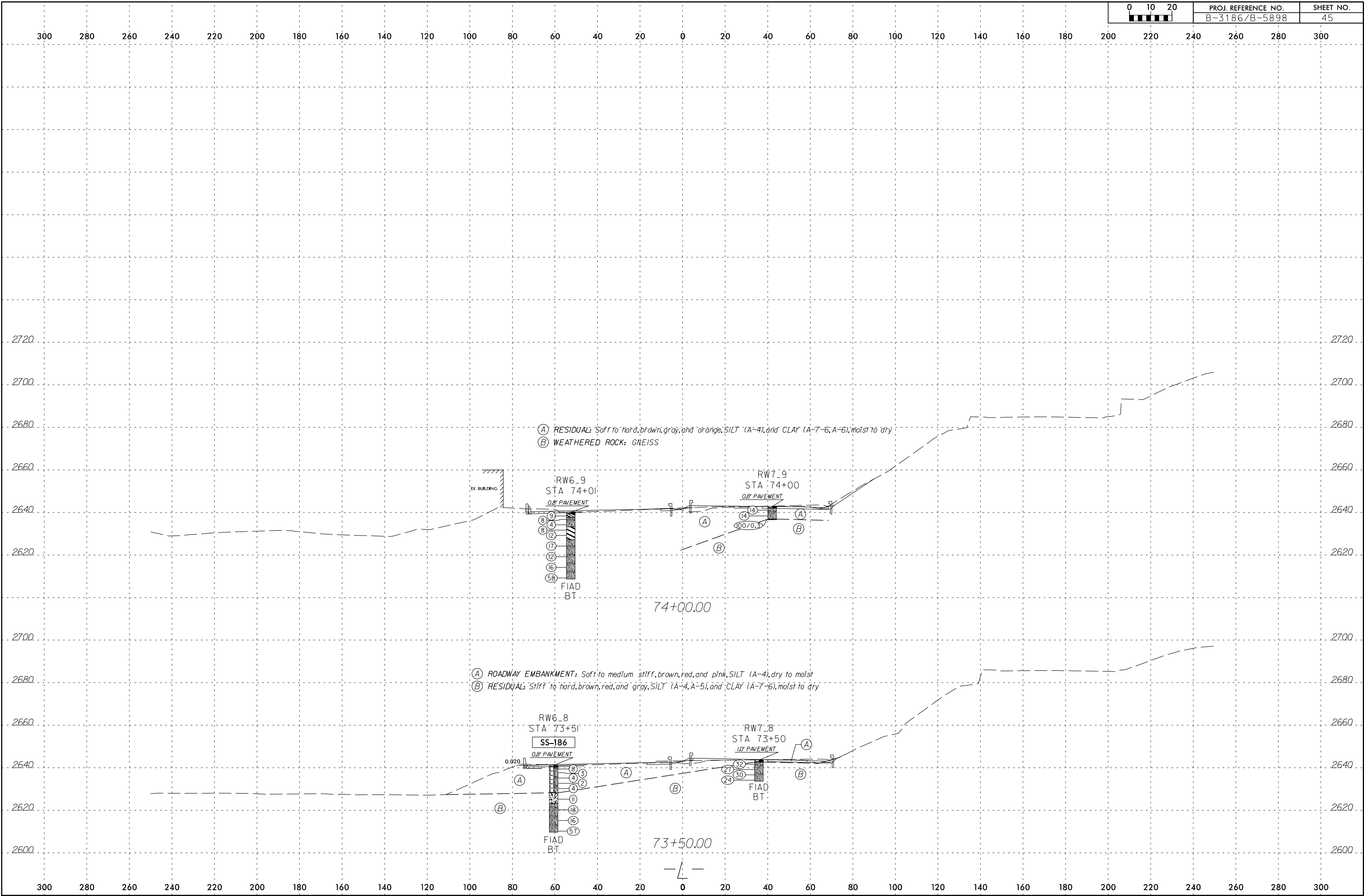
- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Soft to stiff, brown, orange, and tan, SILT (A-4), and CLAY (A-6), dry to moist
- (C) RESIDUAL: Soft, brown, orange, and tan, CLAY (A-7-6, A-6), and SILT (A-4), moist

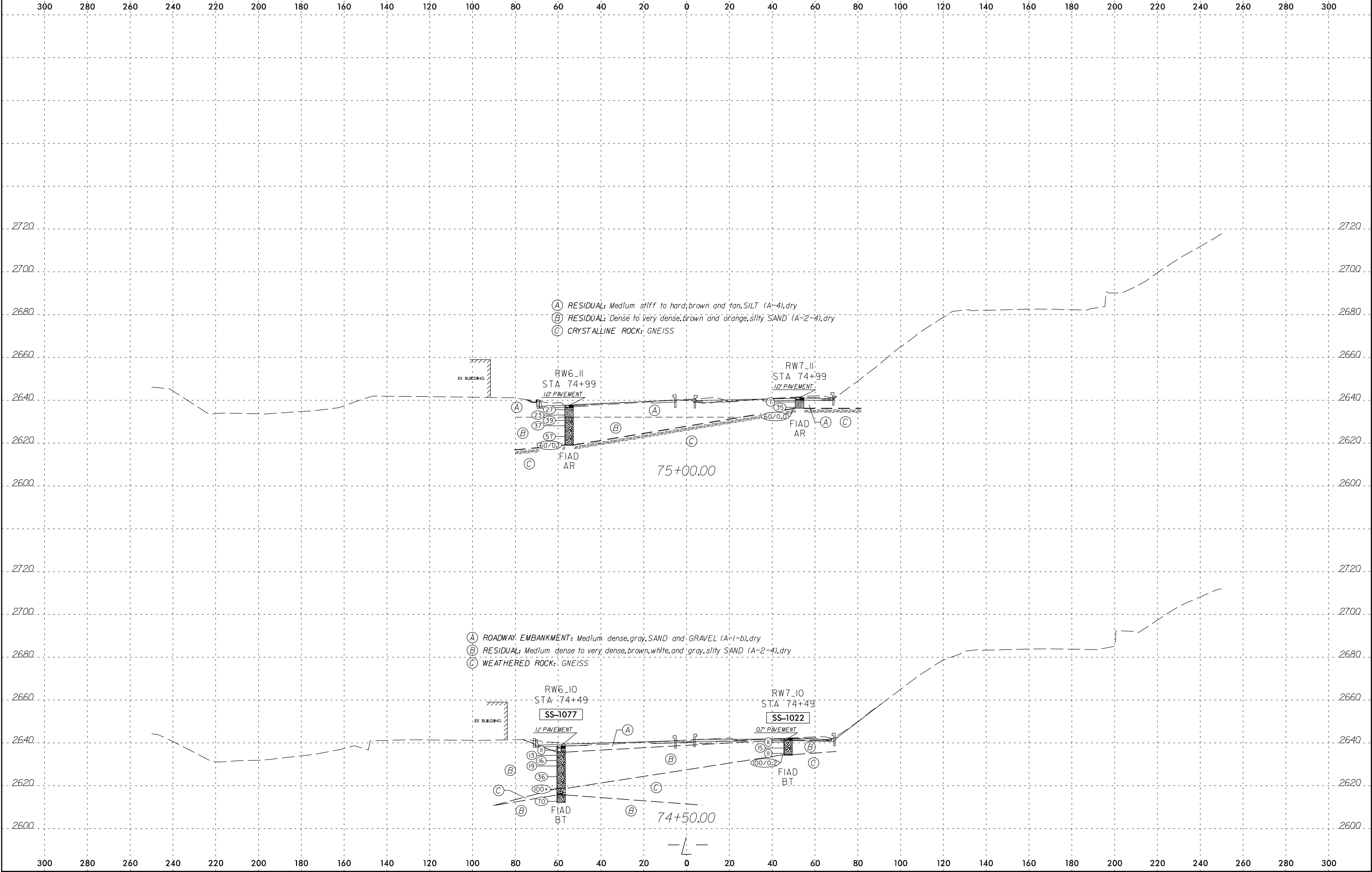


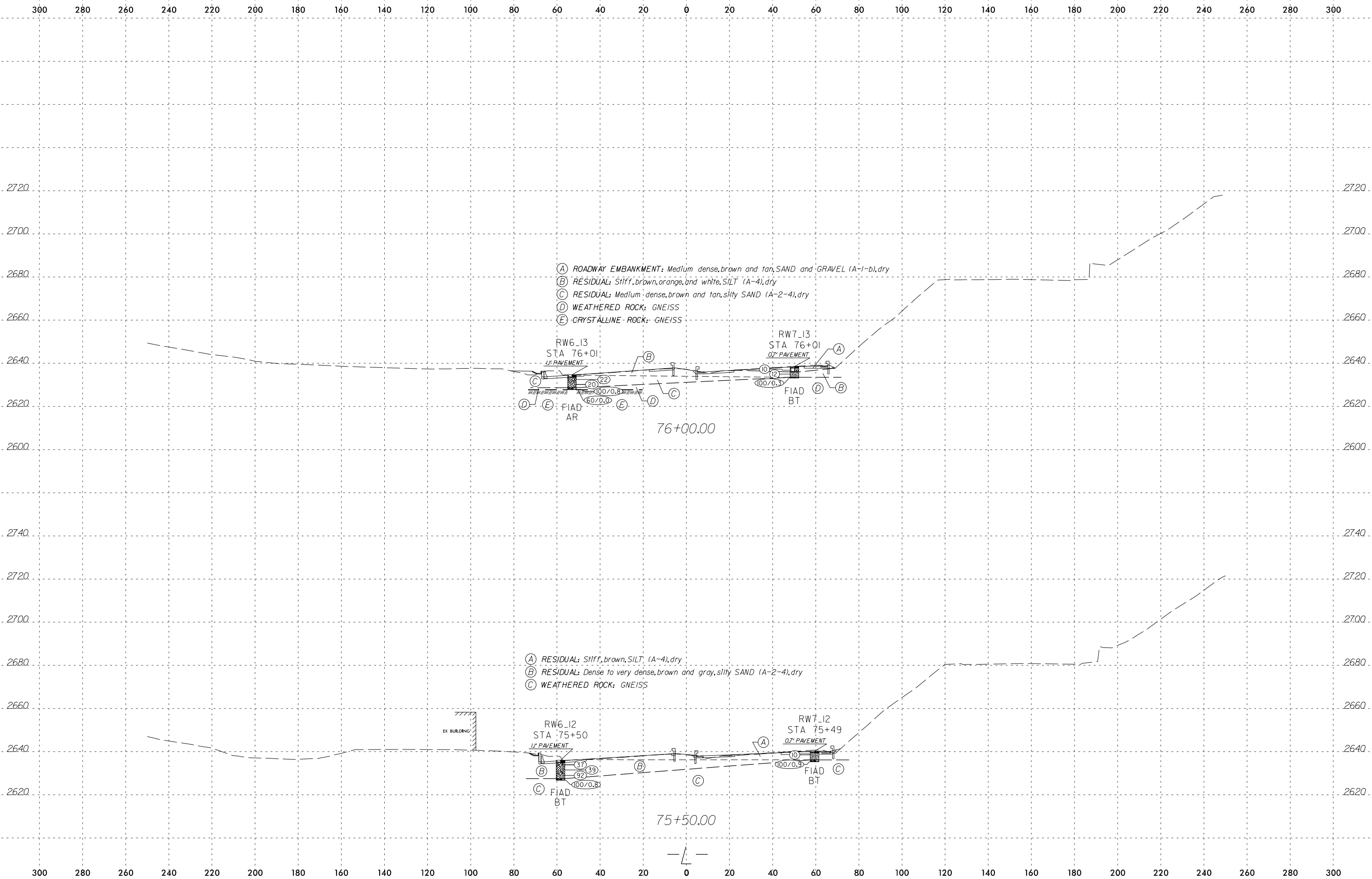




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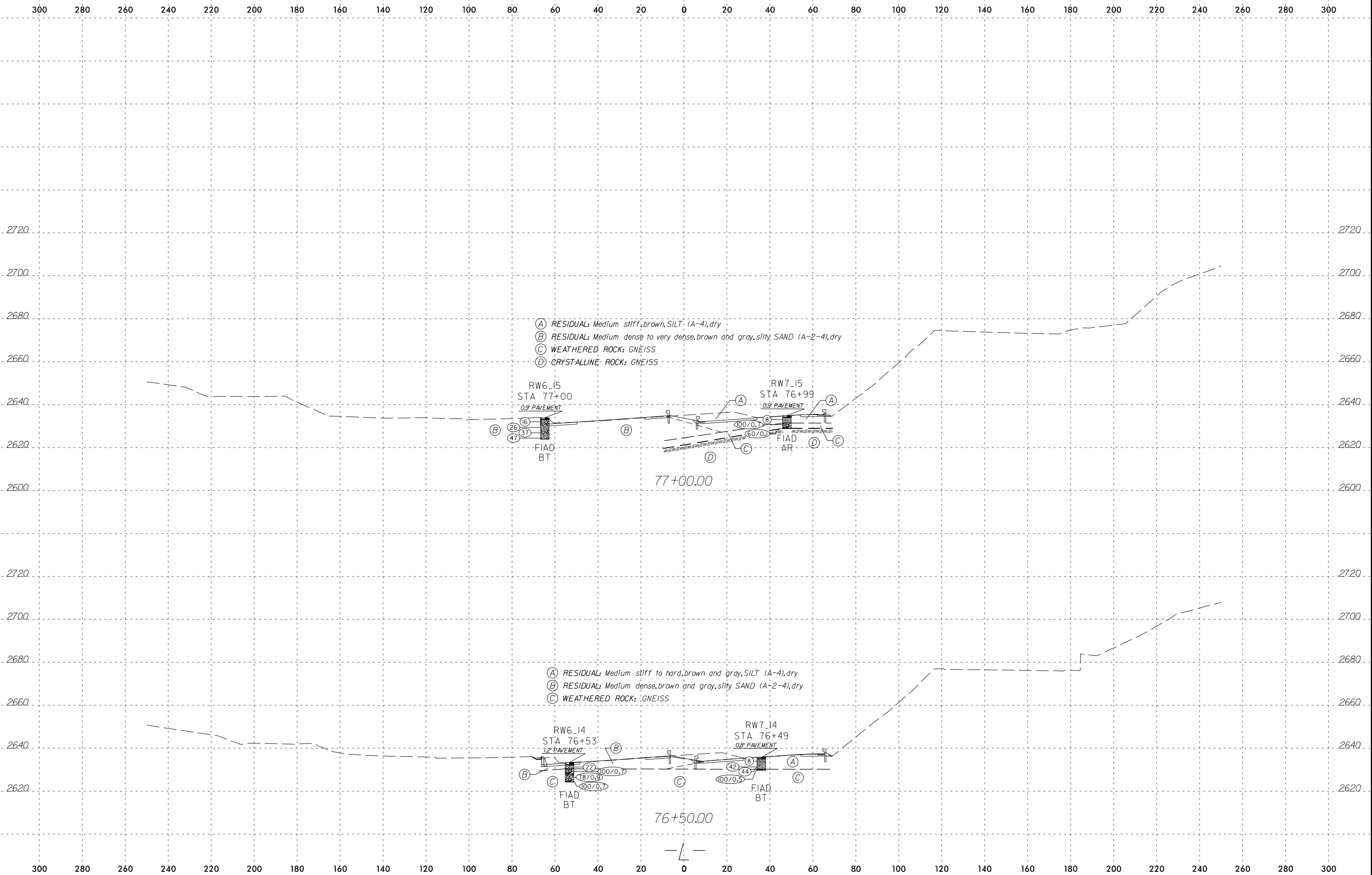




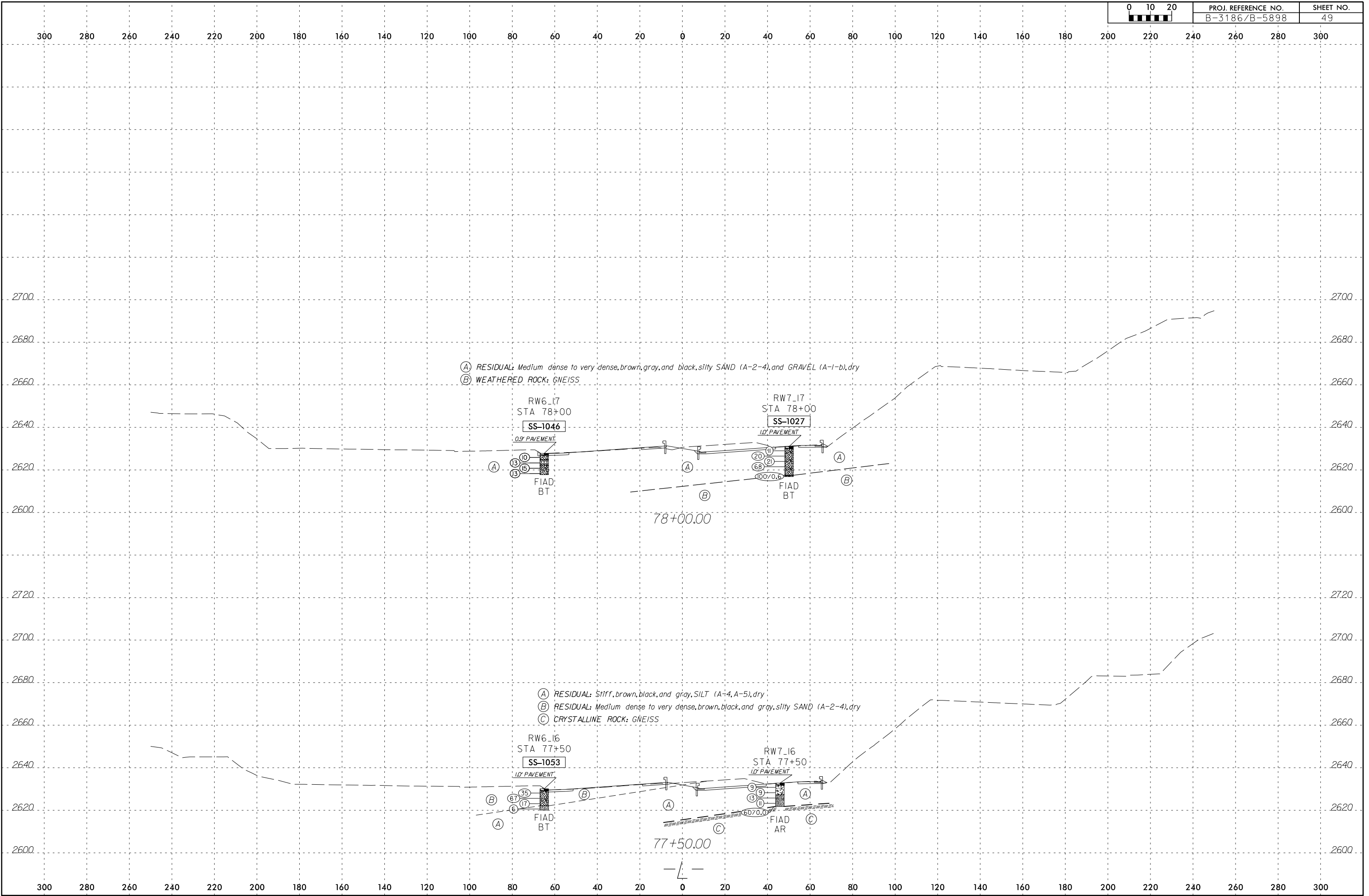
- (A) ROADWAY EMBANKMENT: Medium dense, brown and tan, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff, brown, orange, and white, SILT (A-4), dry
- (C) RESIDUAL: Medium dense, brown and tan, silty SAND (A-2-4), dry
- (D) WEATHERED ROCK: GNEISS
- (E) CRYSTALLINE ROCK: GNEISS

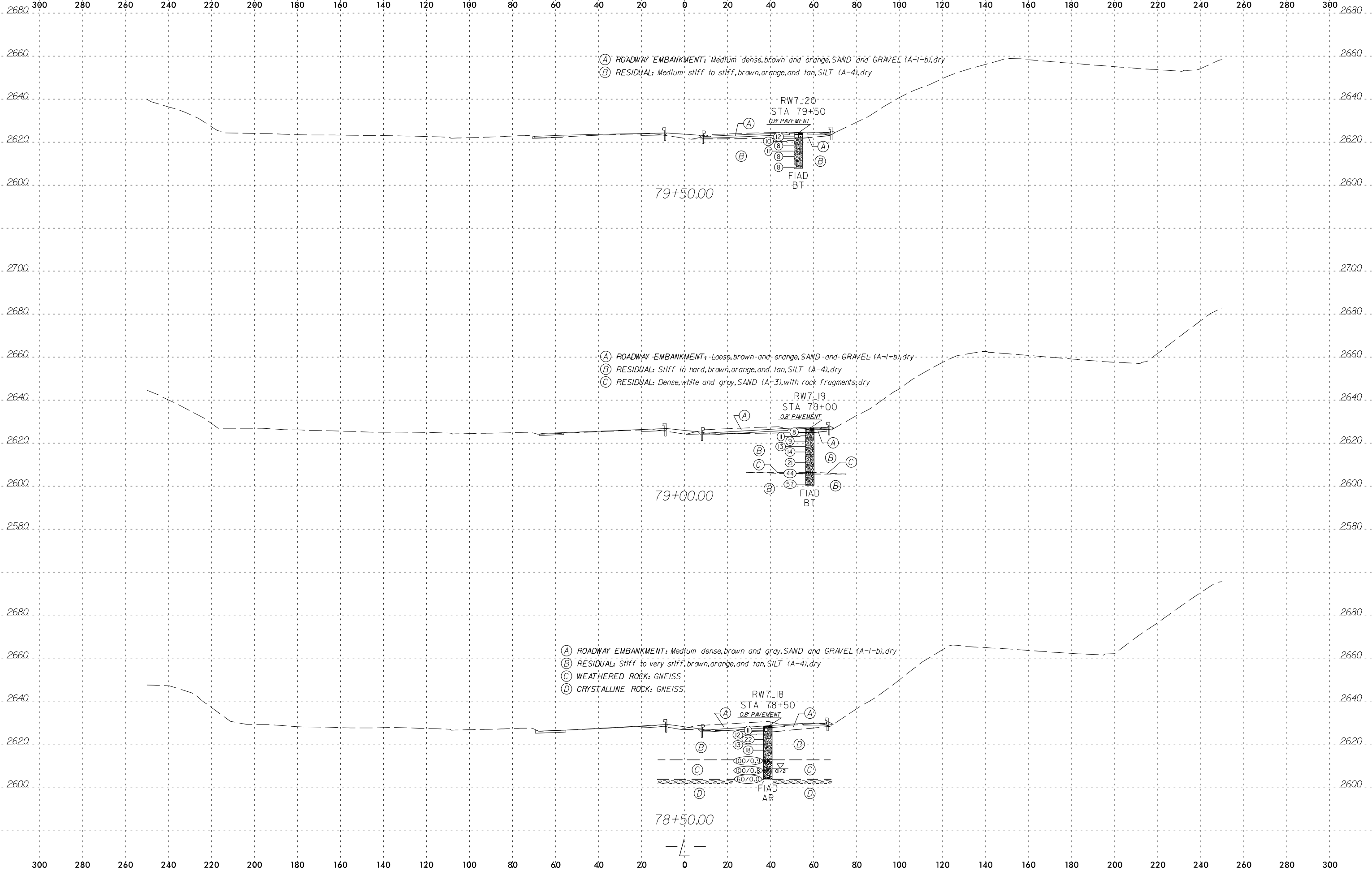
- (A) RESIDUAL: Stiff, brown, SILT (A-4), dry
- (B) RESIDUAL: Dense to very dense, brown and gray, silty SAND (A-2-4), dry
- (C) WEATHERED ROCK: GNEISS

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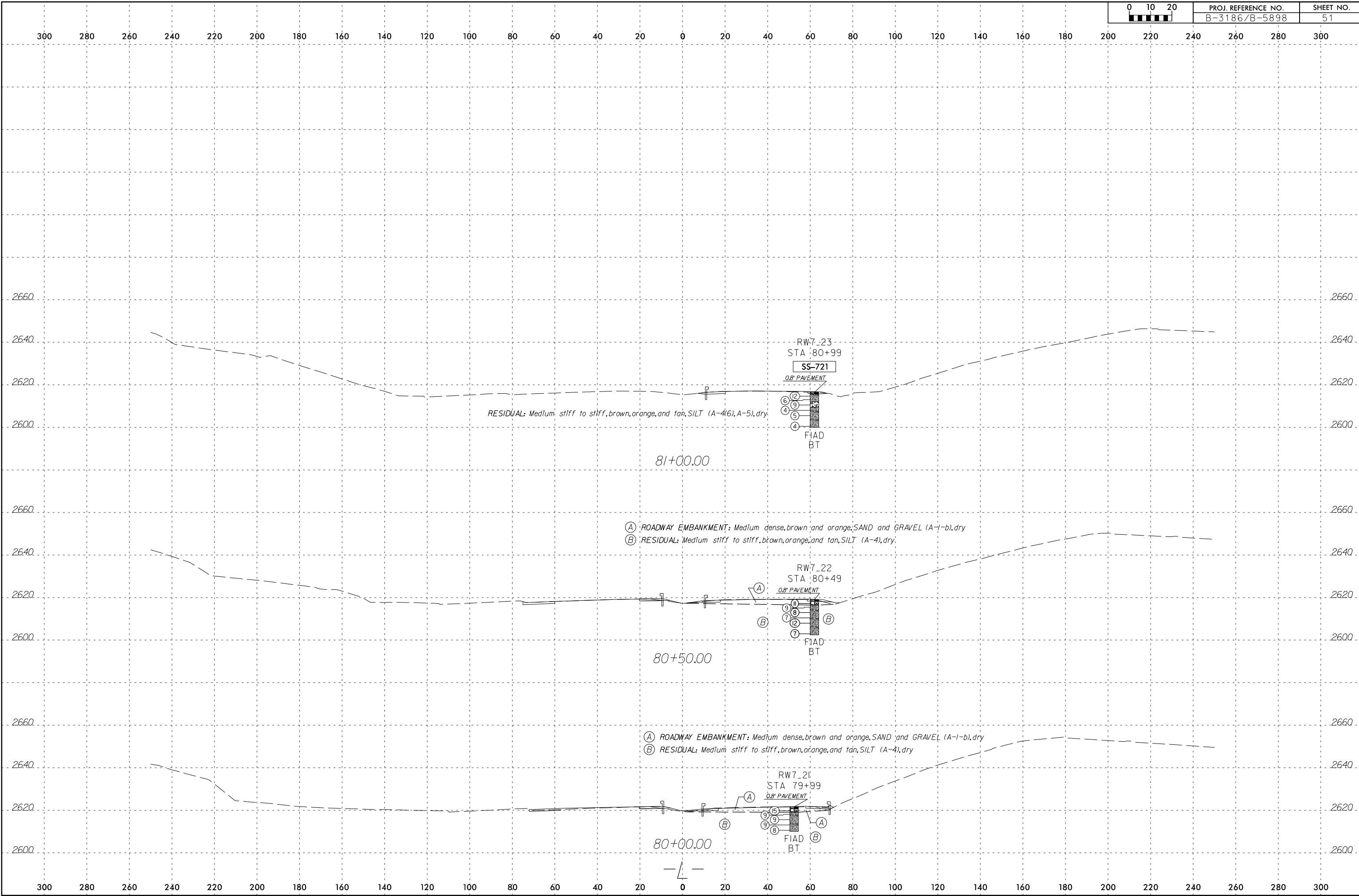


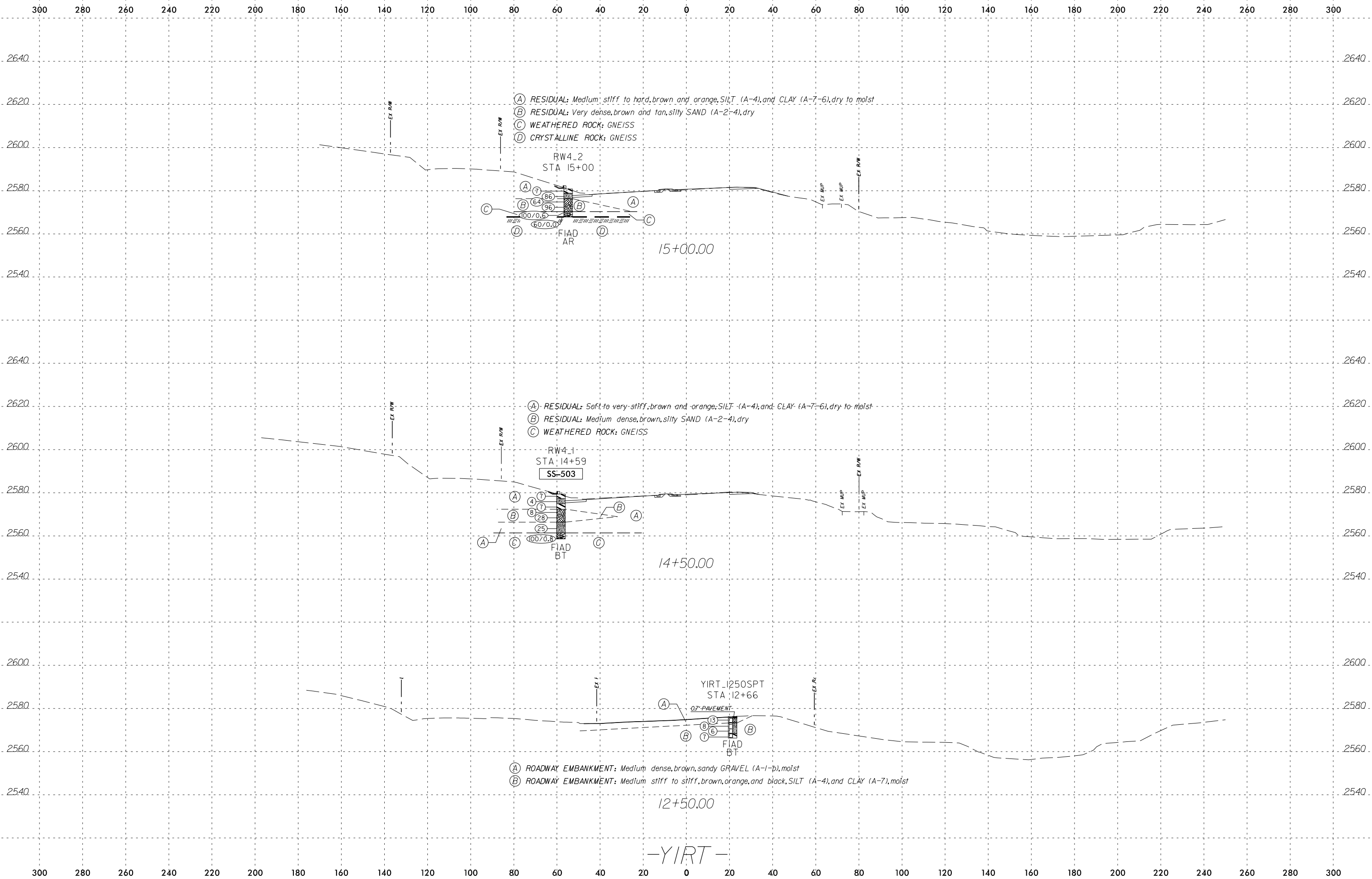


(A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
 (B) RESIDUAL: Medium stiff to stiff, brown, orange, and tan, SILT (A-4), dry

(A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), dry
 (B) RESIDUAL: Stiff to hard, brown, orange, and tan, SILT (A-4), dry
 (C) RESIDUAL: Dense, white and gray, SAND (A-3), with rock fragments, dry

(A) ROADWAY EMBANKMENT: Medium dense, brown and gray, SAND and GRAVEL (A-1-b), dry
 (B) RESIDUAL: Stiff to very stiff, brown, orange, and tan, SILT (A-4), dry
 (C) WEATHERED ROCK: GNEISS
 (D) CRYSTALLINE ROCK: GNEISS



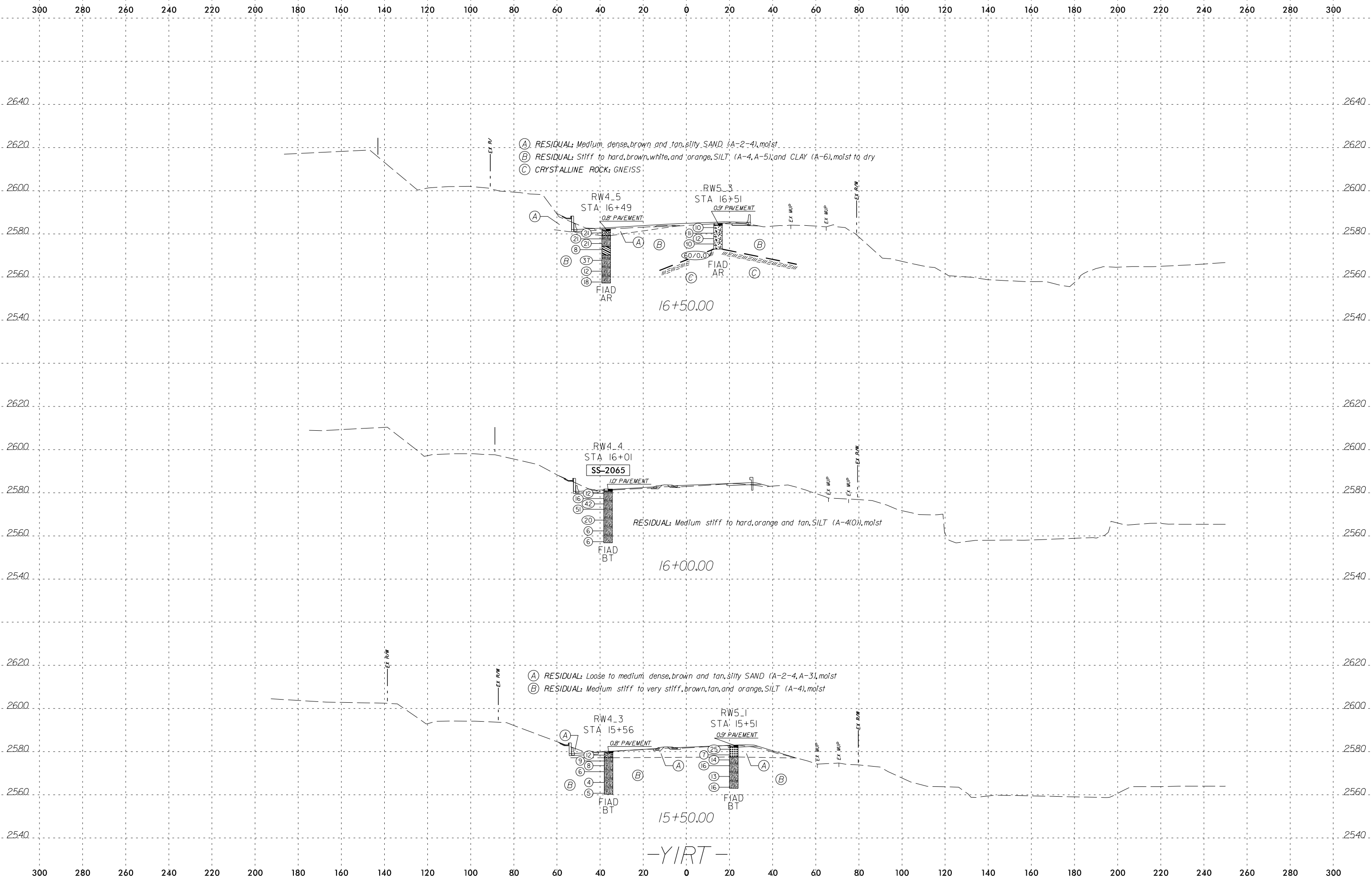


- (A) RESIDUAL: Medium, stiff to hard, brown and orange, SILT (A-4), and CLAY (A-7-6), dry to moist
- (B) RESIDUAL: Very dense, brown and tan, silty SAND (A-2-4), dry
- (C) WEATHERED ROCK: GNEISS
- (D) CRYSTALLINE ROCK: GNEISS

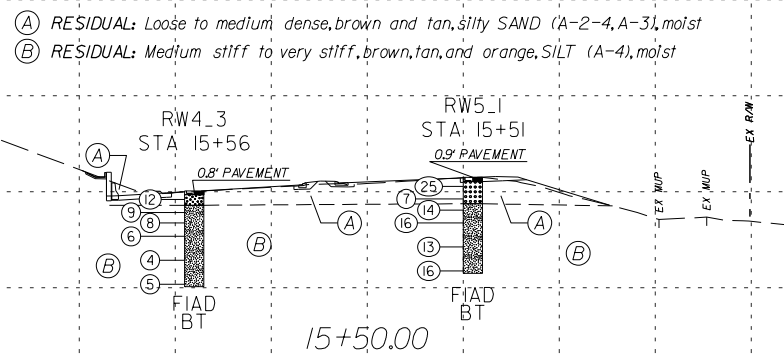
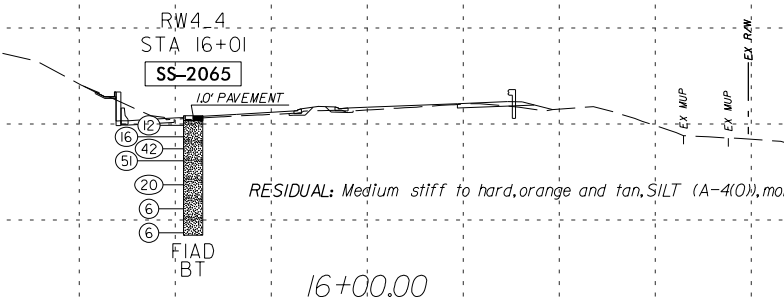
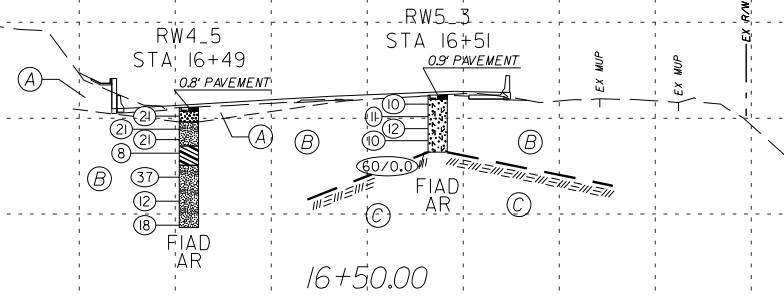
- (A) RESIDUAL: Soft to very stiff, brown and orange, SILT (A-4), and CLAY (A-7-6), dry to moist
- (B) RESIDUAL: Medium dense, brown, silty SAND (A-2-4), dry
- (C) WEATHERED ROCK: GNEISS

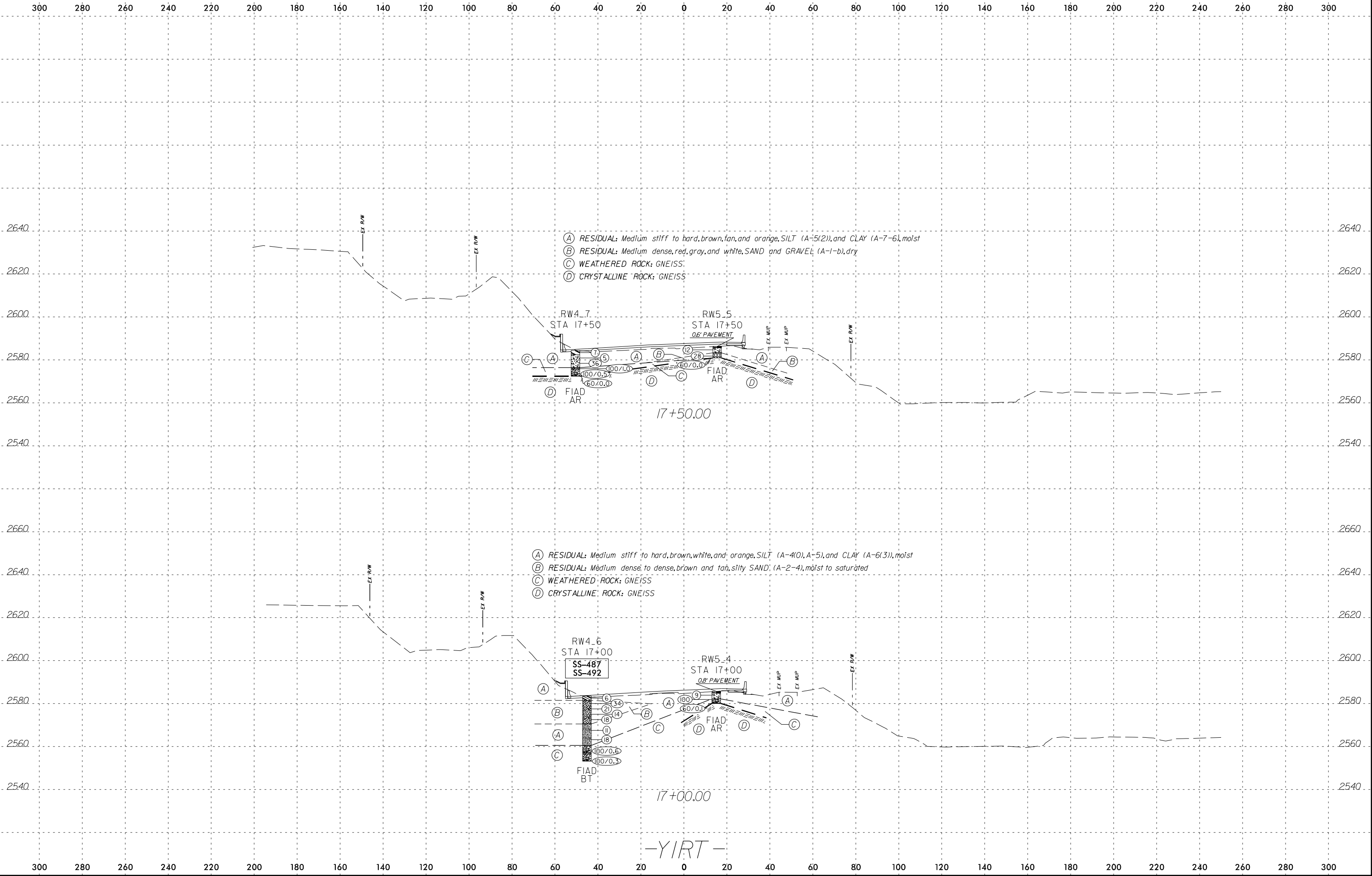
- (A) ROADWAY EMBANKMENT: Medium dense, brown, sandy GRAVEL (A-1-b), moist
- (B) ROADWAY EMBANKMENT: Medium stiff to stiff, brown, orange, and black, SILT (A-4), and CLAY (A-7), moist

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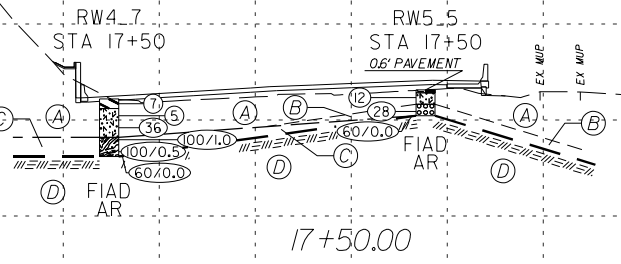


- (A) RESIDUAL: Medium dense, brown and tan, silty SAND (A-2-4), moist
- (B) RESIDUAL: Stiff to hard, brown, white, and orange, SILT (A-4, A-5); and CLAY (A-6), moist to dry
- (C) CRYSTALLINE ROCK: GNEISS

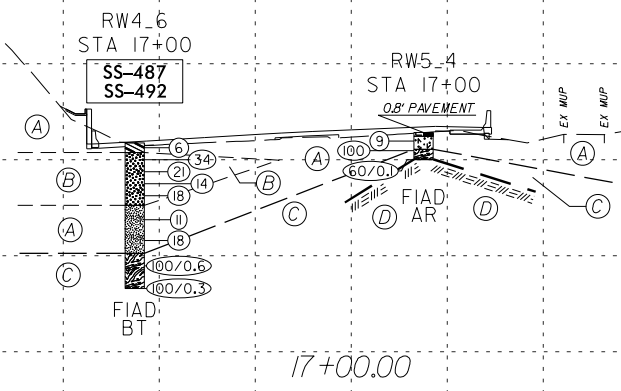




- (A) RESIDUAL: Medium stiff to hard, brown, tan, and orange, SILT (A-5(2)), and CLAY (A-7-6), moist
- (B) RESIDUAL: Medium dense, red, gray, and white, SAND and GRAVEL (A-1-b), dry
- (C) WEATHERED ROCK: GNEISS
- (D) CRYSTALLINE ROCK: GNEISS

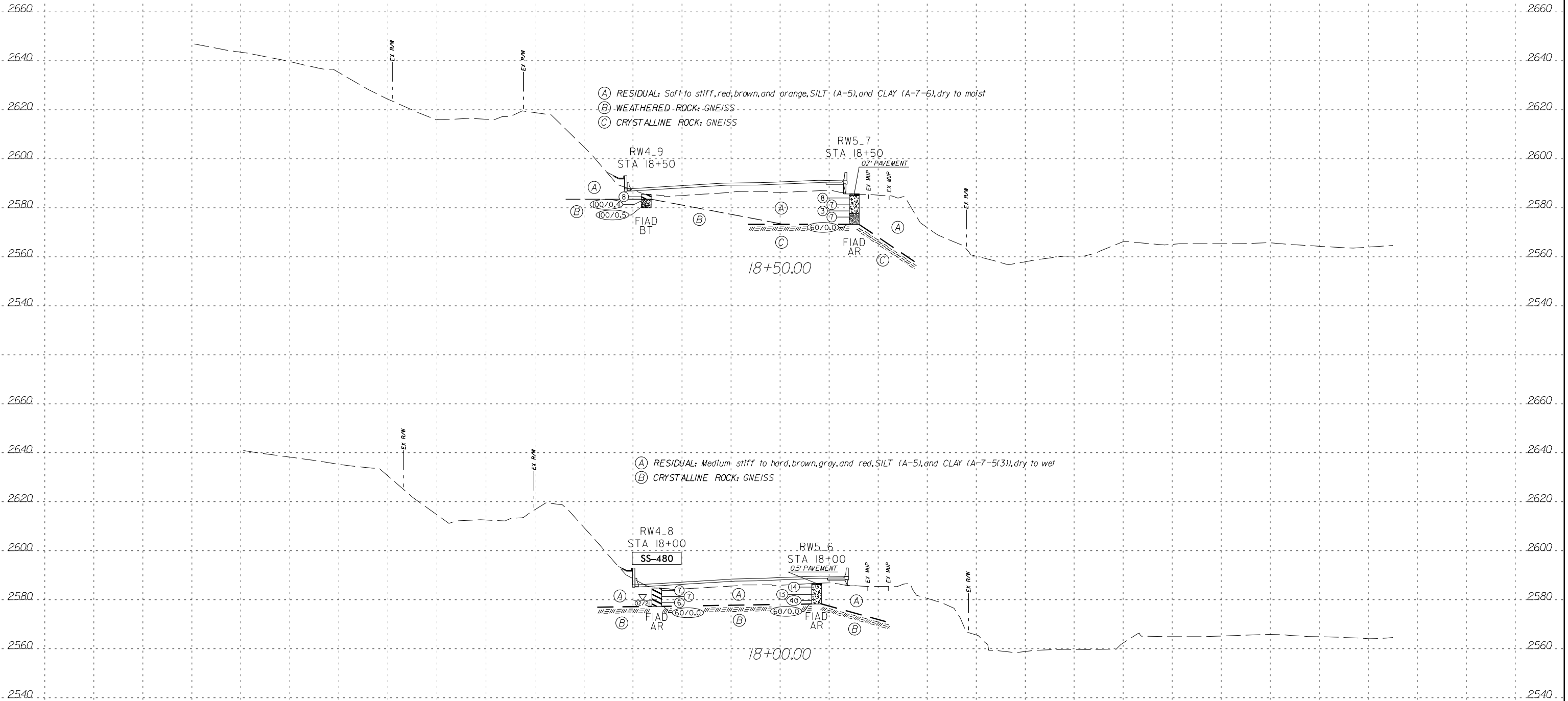


- (A) RESIDUAL: Medium stiff to hard, brown, white, and orange, SILT (A-4(0), A-5), and CLAY (A-6(3)), moist
- (B) RESIDUAL: Medium dense, to dense, brown and tan, silty SAND (A-2-4), moist to saturated
- (C) WEATHERED ROCK: GNEISS
- (D) CRYSTALLINE ROCK: GNEISS



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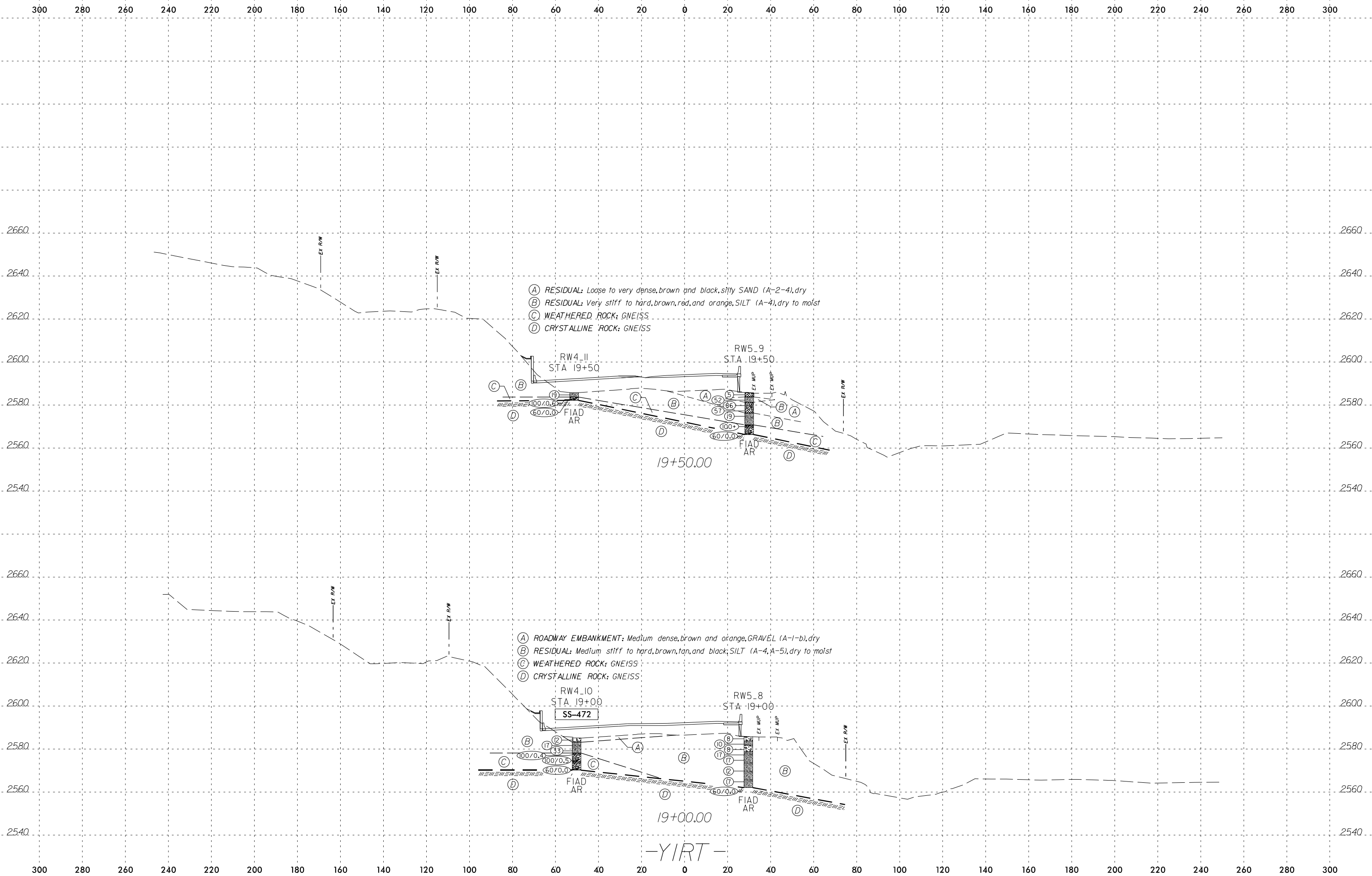
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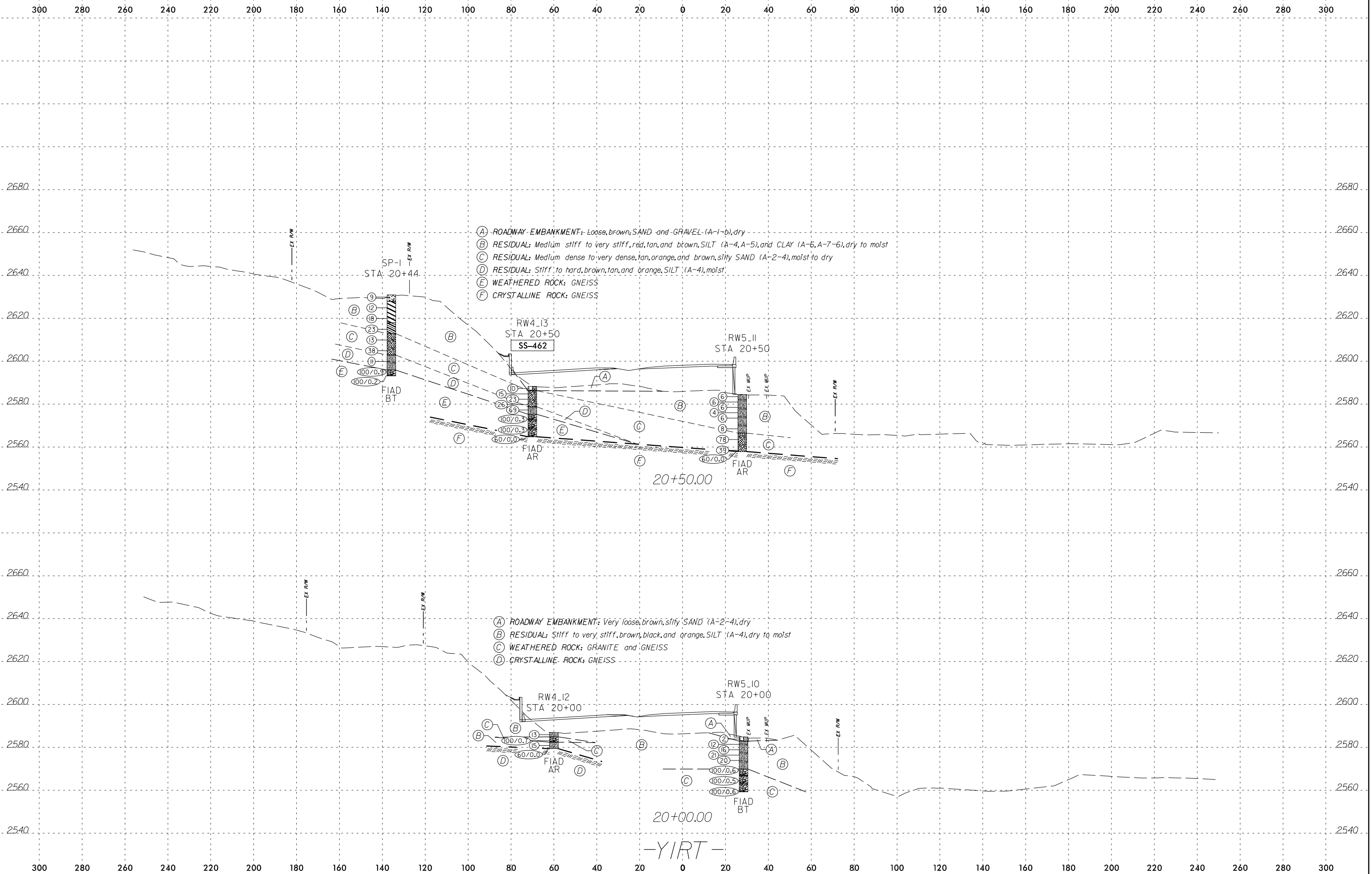


- (A) RESIDUAL: Soft to stiff, red, brown, and orange, SILT (A-5), and CLAY (A-7-6), dry to moist
- (B) WEATHERED ROCK: GNEISS
- (C) CRYSTALLINE ROCK: GNEISS

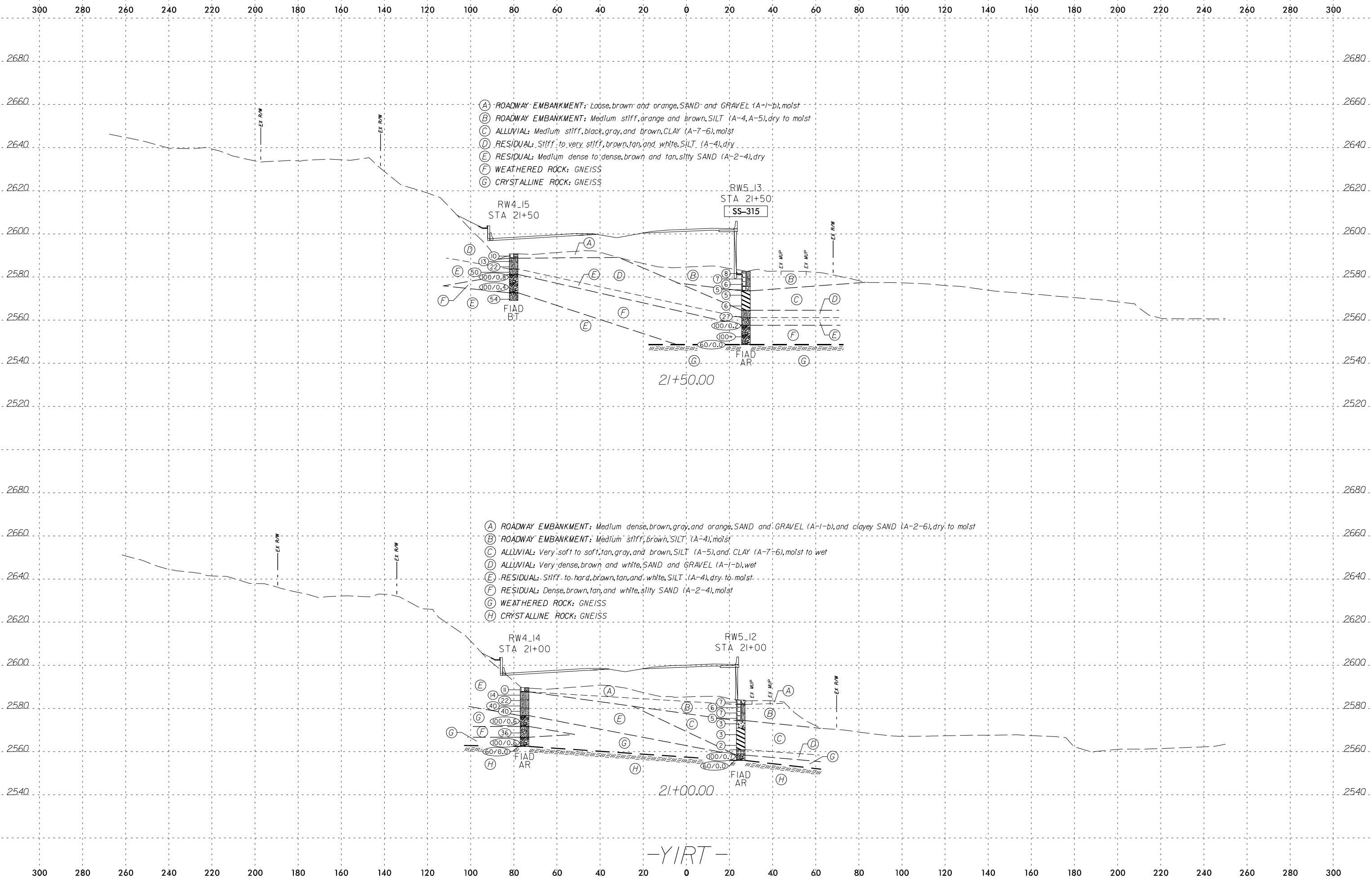
- (A) RESIDUAL: Medium, stiff to hard, brown, gray, and red, SILT (A-5), and CLAY (A-7-5(3)), dry to wet
- (B) CRYSTALLINE ROCK: GNEISS

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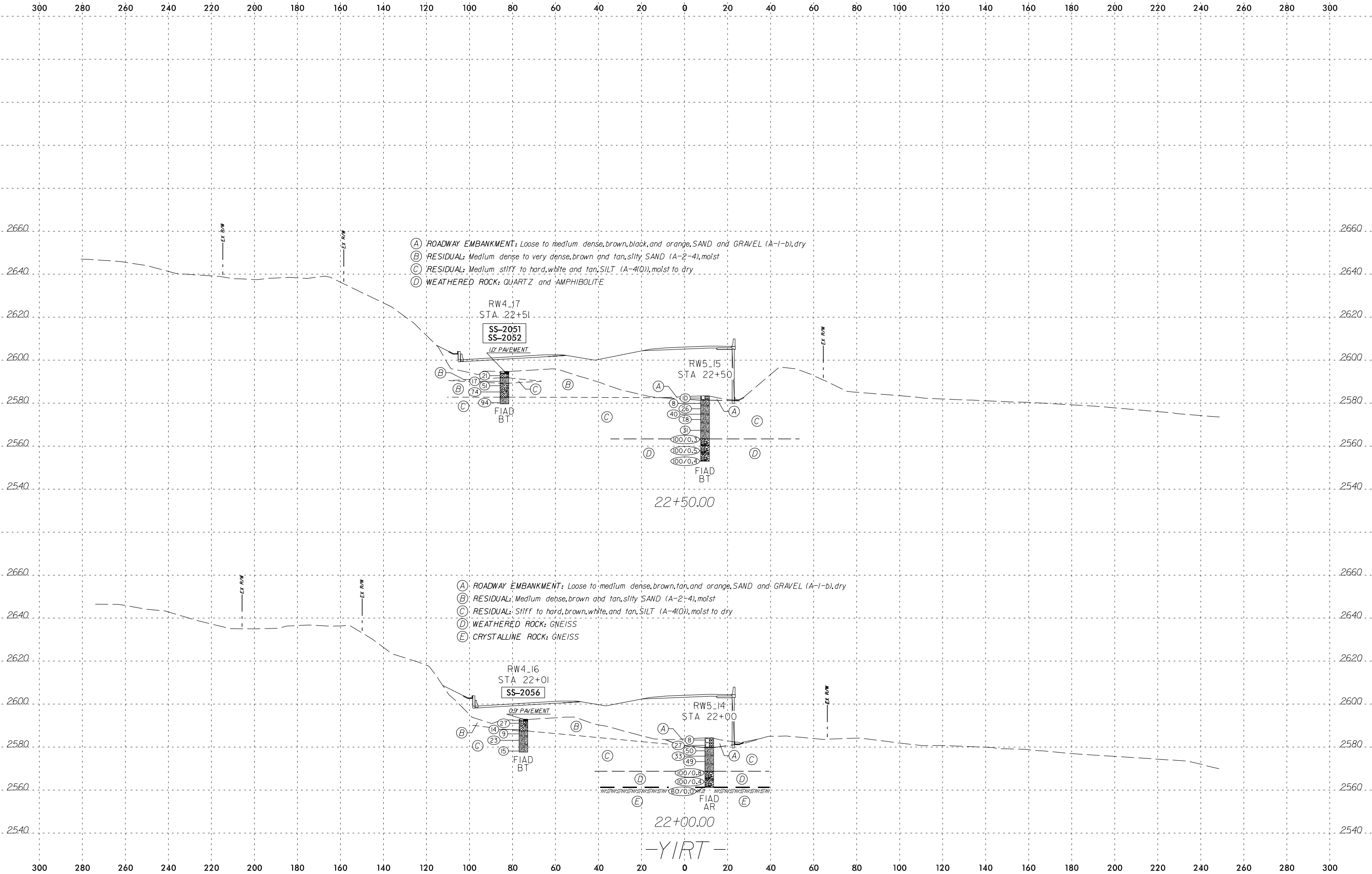
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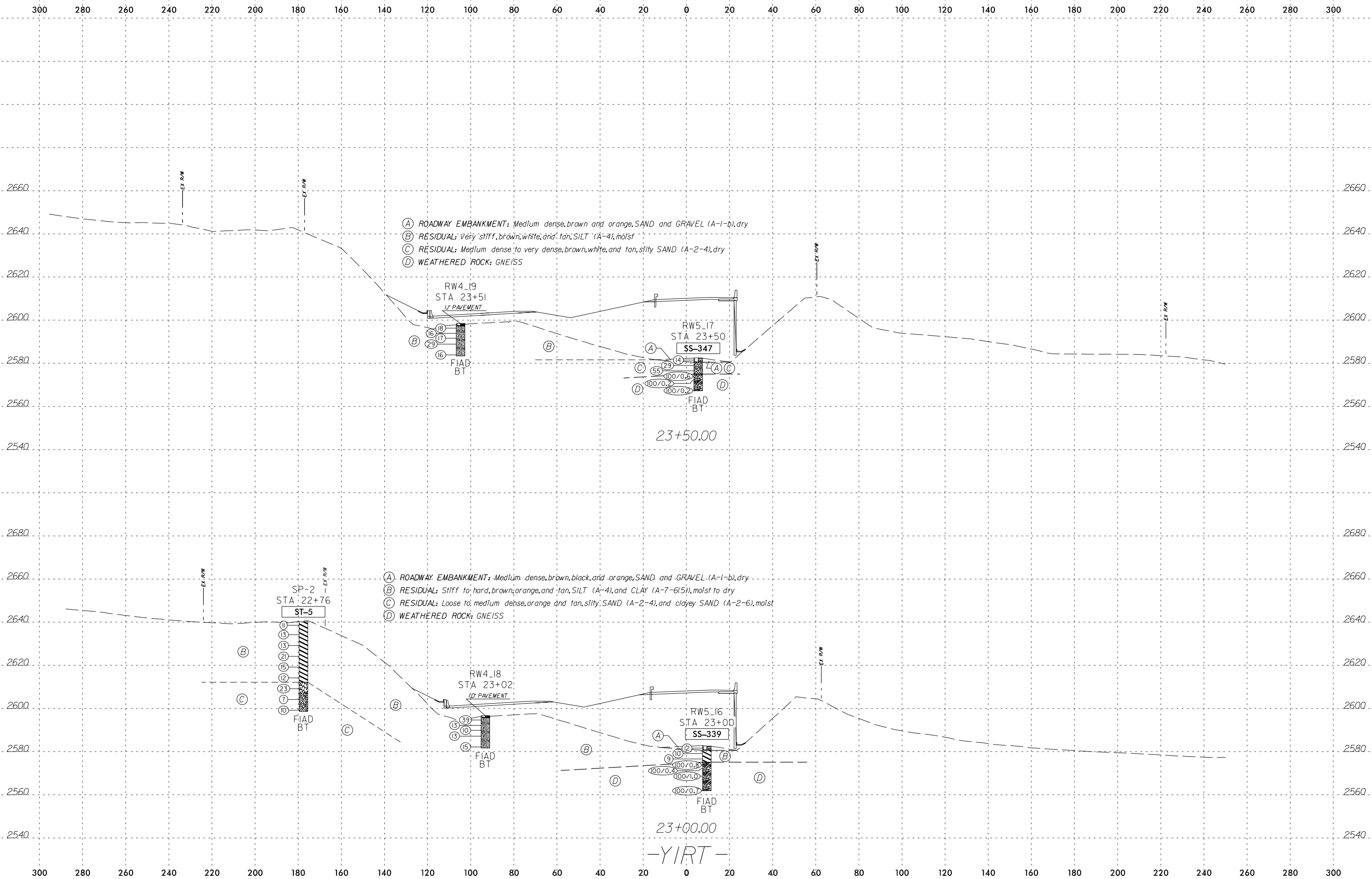
- (A) ROADWAY EMBANKMENT: Loose, brown and orange, SAND and GRAVEL (A-1-b), moist
- (B) ROADWAY EMBANKMENT: Medium stiff, orange and brown, SILT (A-4, A-5), dry to moist
- (C) ALLUVIAL: Medium stiff, black, gray, and brown, CLAY (A-7-6), moist
- (D) RESIDUAL: Stiff to very stiff, brown, tan, and white, SILT (A-4), dry
- (E) RESIDUAL: Medium dense to dense, brown and tan, silty SAND (A-2-4), dry
- (F) WEATHERED ROCK: GNEISS
- (G) CRYSTALLINE ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Medium dense, brown, gray, and orange, SAND and GRAVEL (A-1-b), and clayey SAND (A-2-6), dry to moist
- (B) ROADWAY EMBANKMENT: Medium stiff, brown, SILT (A-4), moist
- (C) ALLUVIAL: Very soft to soft, tan, gray, and brown, SILT (A-5), and CLAY (A-7-6), moist to wet
- (D) ALLUVIAL: Very dense, brown and white, SAND and GRAVEL (A-1-b), wet
- (E) RESIDUAL: Stiff to hard, brown, tan, and white, SILT (A-4), dry to moist
- (F) RESIDUAL: Dense, brown, tan, and white, silty SAND (A-2-4), moist
- (G) WEATHERED ROCK: GNEISS
- (H) CRYSTALLINE ROCK: GNEISS

-YIRT-



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\$\$\$\$\$USERNAME\$\$\$\$\$



- (A) ROADWAY EMBANKMENT: Medium dense, brown and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Very stiff, brown, white, and tan, SILT (A-4), moist
- (C) RESIDUAL: Medium dense to very dense, brown, white, and tan, silty SAND (A-2-4), dry
- (D) WEATHERED ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Medium dense, brown, black, and orange, SAND and GRAVEL (A-1-b), dry
- (B) RESIDUAL: Stiff to hard, brown, orange, and tan, SILT (A-4), and CLAY (A-7-6(5)), moist to dry
- (C) RESIDUAL: Loose to medium dense, orange and tan, silty, SAND (A-2-4), and clayey SAND (A-2-6), moist
- (D) WEATHERED ROCK: GNEISS

RW4_19
STA 23+51
1" PAVEMENT
FIAD BT

RW5_17
STA 23+50
SS-347
FIAD BT

SP-2
STA 22+76
ST-5
FIAD BT

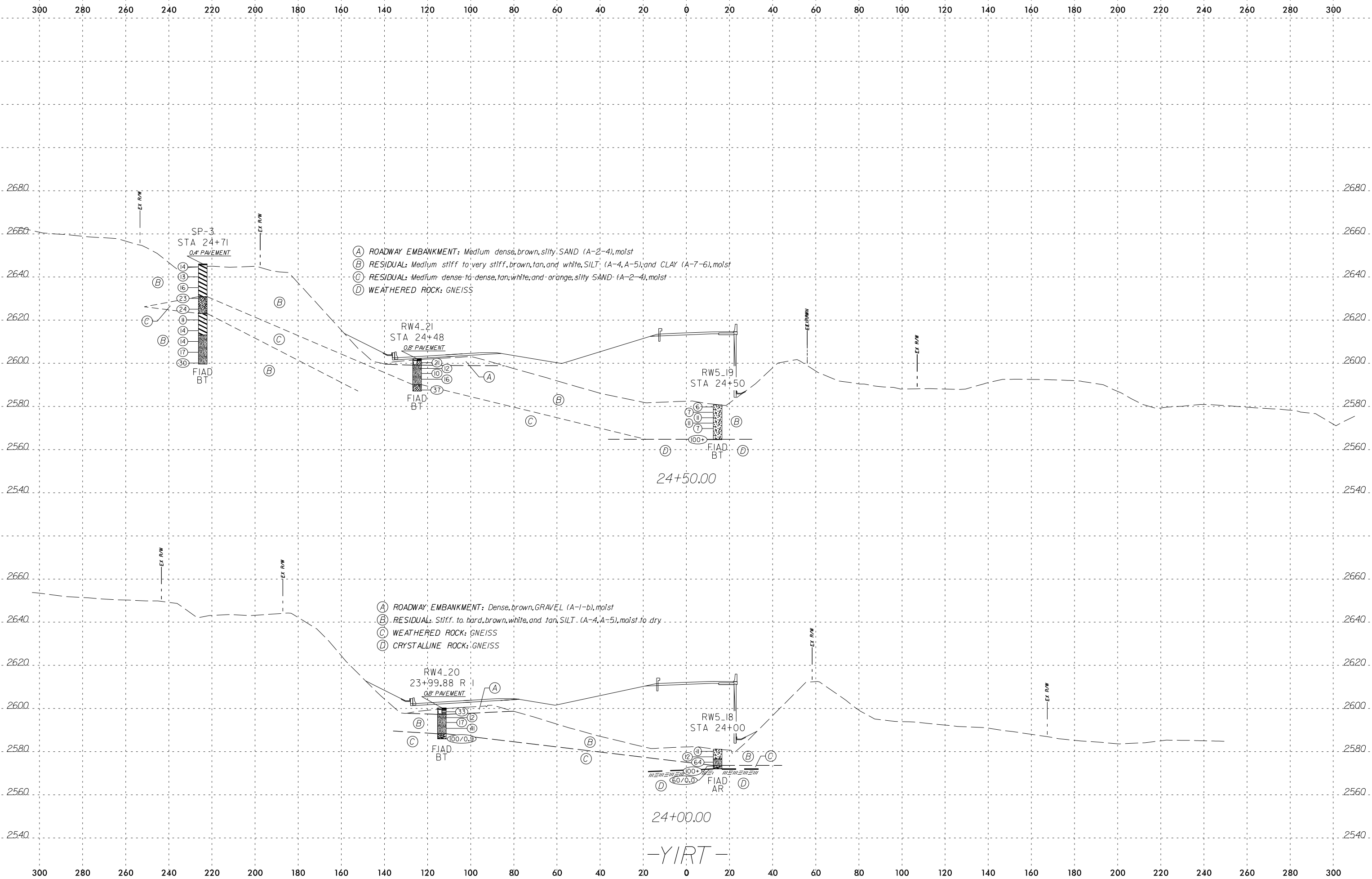
RW4_18
STA 23+02
1" PAVEMENT
FIAD BT

RW5_16
STA 23+00
SS-339
FIAD BT

23+50.00

23+00.00

-YIRT-



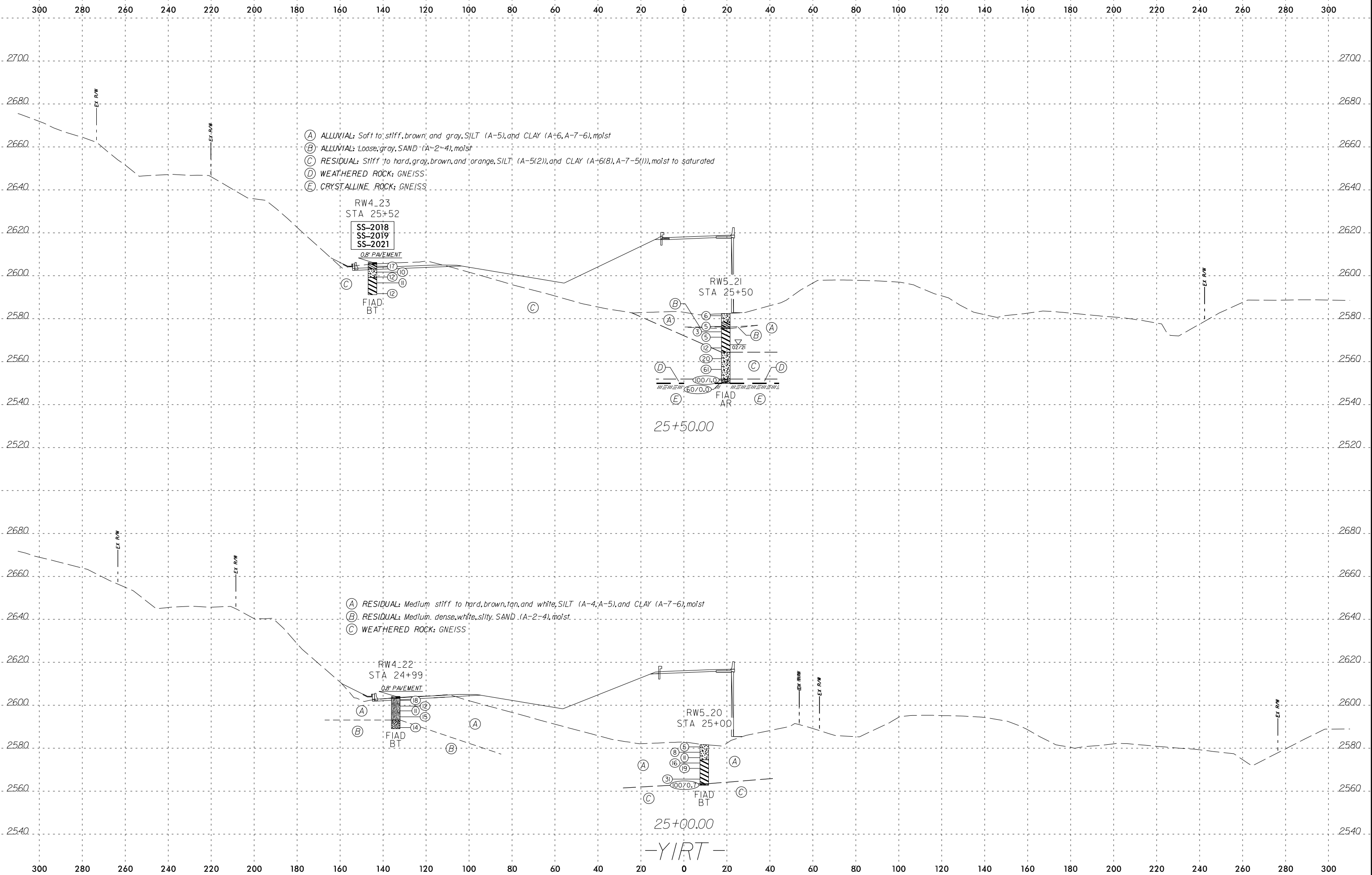
- (A) ROADWAY EMBANKMENT: Medium dense, brown, silty, SAND (A-2-4), moist
- (B) RESIDUAL: Medium stiff to very stiff, brown, tan, and white, SILT (A-4, A-5), and CLAY (A-7-6), moist
- (C) RESIDUAL: Medium dense to dense, tan, white, and orange, silty SAND (A-2-4), moist
- (D) WEATHERED ROCK: GNEISS

- (A) ROADWAY EMBANKMENT: Dense, brown, GRAVEL (A-1-b), moist
- (B) RESIDUAL: Stiff to hard, brown, white, and tan, SILT (A-4, A-5), moist to dry
- (C) WEATHERED ROCK: GNEISS
- (D) CRYSTALLINE ROCK: GNEISS

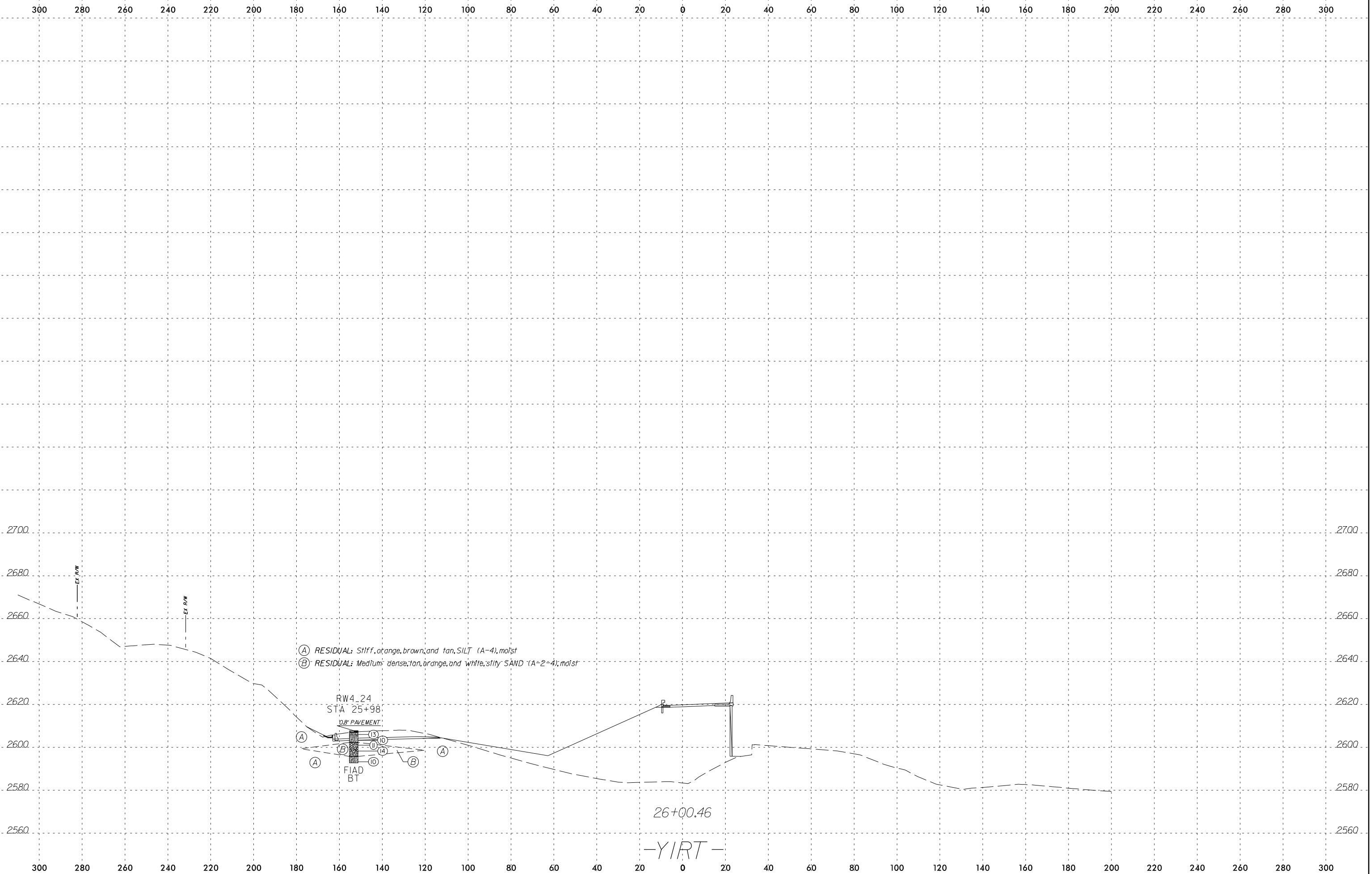
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-YIRT-



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\$\$\$\$\$USERNAME\$\$\$\$\$



GEOTECHNICAL BORING REPORT

BORE LOG

WBS 38332.1.FS1		TIP B-3186 / B-5898		COUNTY HAYWOOD		GEOLOGIST N. Yacobi									
SITE DESCRIPTION Retaining Wall No. 5 from -Y1RT- STA 15+25 to 26+13							GROUND WTR (ft)								
BORING NO. RW5_2		STATION 16+54		OFFSET 16 ft LT		ALIGNMENT -RW5-									
COLLAR ELEV. 2,583.8 ft		TOTAL DEPTH 20.0 ft		NORTHING 666,847		EASTING 818,249									
DRILL RIG/HAMMER EFF./DATE GTC3277 CME-75 83% (09/15/2020)				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER K. Boone		START DATE 02/13/21		COMP. DATE 02/13/21		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
2585														2,583.8 GROUND SURFACE 0.0	
	2,583.0	0.8	2	4	6								D	2,583.0 0.8' Pavement 0.8	
2580	2,580.3	3.5	5	6	7								D	2,580.8 ROADWAY EMBANKMENT 3.0	
	2,577.8	6.0	4	7	10								D	2,578.3 Loose to medium dense, red and brown, silty GRAVEL (A-1-b) 5.5	
2575	2,575.3	8.5	3	8	13								D	RESIDUAL	
													D	Stiff, red and brown, SILT (A-4), micaceous / Very stiff, gray, clayey SILT (A-5), micaceous, saprolitic	
2570	2,570.3	13.5	3	8	11								M		
2565	2,565.3	18.5	4	8	12								M		
													M	2,563.8 Boring Terminated at Elevation 2,563.8 ft in SILT 20.0	

NCDOT BORE DOUBLE B3186_GEO_SPT.GPJ NC_DOT.GDT 10/22/21

PROJECT REFERENCE NO. B-3186B-5898		SHEET NO. 65	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			
HDR		HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116	

-L- SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS- 1005	55' RT	25+00	15.0' - 16.5'	A-6 (5)	38	13	29.4	21.7	36.0	12.9	99.3	80.2	53.9	17	-
SS- 547	45' LT	27+00	2.5' - 4.0'	A-6 (10)	38	15	17.5	29.7	30.4	22.4	93.5	84.2	72.0	21	-
SS- 2	52' RT	30+00	2.5' - 4.0'	A-6 (2)	34	13	33.6	31.1	30.2	5.1	96.0	76.0	41.0	15	-
SS- 4	53' RT	33+06	7.5' - 9.0'	A-2-4 (0)	28	6	54.4	23.4	15.4	6.8	87.7	50.7	24.3	10	-
SS- 3	51' RT	34+07	2.5' - 4.0'	A-2-5 (0)	44	7	54.2	24.3	14.6	6.9	95.1	55.9	25.7	26	-
ST- 7	80' LT	35+00	3.5' - 5.5'	A-7-5 (16)	53	19	12.4	12.2	15.6	59.8	99.1	91.7	75.0	34	-
SS- 1011	55' RT	37+00	5.0' - 6.5'	A-7-5 (8)	49	17	23.9	20.6	42.7	12.8	90.6	76.4	54.6	28	-
SS- 2018	230' LT	49+13	0.8' - 2.3'	A-6 (8)	38	15	13.9	20.3	29.7	36.1	91.2	83.0	63.9	13	-
SS- 2019	230' LT	49+13	3.5' - 5.0'	A-5 (2)	45	6	23.5	31.4	38.6	6.5	92.9	77.9	49.7	12	-
SS- 2021	230' LT	49+13	8.5' - 10.0'	A-7-5 (1)	48	14	33.9	28.3	28.5	9.3	83.1	62.6	36.8	24	-
SS- 436	147' LT	51+36	7.5' - 9.0'	A-7-5 (8)	55	20	24.0	23.4	34.3	18.3	87.8	73.7	50.2	27	-
SS- 439	147' LT	51+36	20.0' - 21.5'	A-7-6 (6)	44	18	23.0	32.0	20.6	34.4	97.8	84.3	49.3	24	-
ST- 6	214' LT	51+80	18.0' - 20.0'	A-7-6 (13)	50	25	24.5	14.8	15.4	45.3	97.2	80.0	59.4	25	-
SS- 431	127' LT	52+00	15.0' - 16.5'	A-6 (3)	40	11	34.3	21.8	22.1	21.8	96.4	71.7	46.6	18	-
SS- 24	112' RT	51+99	15.0' - 16.1'	A-7-6 (11)	47	23	20.8	20.5	23.6	35.1	92.0	79.1	58.5	26	-
SS- 422	112' LT	52+50	10.0' - 11.5'	A-2-4(0)	33	9	47.9	18.0	20.2	13.8	88.9	54.7	33.4	17	-
SS- 58	130' RT	52+50	7.5' - 9.0'	A-5 (2)	44	10	26.6	29.4	23.9	20.1	88.8	73.7	44.4	28	-
SS- 62	130' RT	52+50	25.0' - 26.5'	A-7-5(31)	82	33	16.9	8.5	58.4	16.2	100	87.0	77.1	71	-
SS- 35	145' RT	53+00	10.0' - 11.5'	A-4 (2)	38	9	27.5	26.8	24.5	21.2	94.3	76.9	48.8	25	-
SS- 46	145' RT	53+00	55.0' - 56.5'	A-2-4(0)	32	NP	47.6	32.0	9.7	10.7	98.9	67.6	26.2	18	-
SS- 362	60' LT	56+50	0.0' - 1.5'	A-6 (3)	36	14	20.9	24.3	28.8	26.0	78.9	68.2	46.9	14	-
SS- 1105	44' LT	58+47	8.5' - 10.0'	A-4 (4)	40	9	17.8	28.7	36.5	17.0	99.3	88.9	59.4	29	-
SS- 1110	43' LT	58+96	1.0' - 2.5'	A-7-6 (7)	46	16	21.9	23.4	26.4	28.3	92.4	78.2	55.1	10	-
SS- 1118	43' LT	59+49	13.5' - 15.0'	A-6 (8)	38	15	15.7	24.8	46.0	13.5	98.6	89.3	63.5	28	-
SS- 1139	41' LT	59+95	8.5' - 10.0'	A-6 (6)	34	13	13.3	30.8	33.9	22.0	100.0	92.9	62.1	22	-
SS- 302	60' LT	63+50	2.5' - 4.0'	A-5 (4)	46	5	10.9	29.7	45.7	13.7	95.0	89.1	64.5	26	-
SS- 260	55' RT	65+50	0.0' - 1.5'	A-5 (5)	47	10	20.1	25.6	23.3	31.0	92.1	80.0	54.9	17	-
SS- 256	60' LT	66+00	15.0' - 16.5'	A-4 (1)	38	10	41.6	22.4	25.9	10.1	98.2	65.9	40.0	12	-
SS- 258	60' LT	66+00	25.0' - 26.5'	A-2-5(0)	42	10	47.2	26.8	24.5	1.5	92.0	60.9	28.4	34	-
SS- 242	55' LT	66+50	7.5' - 9.0'	A-7-5 (2)	52	13	33.8	32.5	25.1	8.6	98.6	76.8	39.8	28	-
SS- 278	60' LT	67+00	5.0' - 6.5'	A-7-6 (2)	42	13	21.7	44.4	31.8	2.1	100.0	91.3	42.2	22	-
SS- 282	60' LT	67+00	20.0' - 21.5'	A-2-7 (1)	53	16	38.6	36.2	22.5	2.7	95.9	73.1	29.7	18	-
SS- 186	61' LT	73+51	7.5' - 9.0'	A-4(0)	32	4	22.9	33.3	30.7	13.1	97.7	84.3	50.7	21	-
SS- 1077	59' LT	74+49	8.5' - 10.0'	A-2-4(0)	35	8	44.0	27.6	20.9	7.5	97.4	66.1	34.0	16	-
SS- 1022	47' RT	74+49	3.5' - 5.0'	A-2-4(0)	31	9	35.4	28.0	18.9	17.7	75.8	59.9	32.7	10	-
SS- 1053	65' LT	77+50	8.5' - 10.0'	A-4 (1)	39	10	39.1	23.4	19.6	17.9	96.5	69.9	41.0	30	-
SS- 1046	65' LT	78+00	0.9' - 2.4'	A-2-4(0)	39	9	21.0	45.2	20.8	13.0	80.7	74.1	33.9	15	-
SS- 1027	50' RT	78+00	8.5' - 10.0'	A-2-4(0)	29	NP	44.1	34.2	12.7	9.0	95.3	68.2	26.8	8	-
SS- 721	62' RT	80+99	10.0' - 11.5'	A-4 (6)	40	10	14.7	31.3	51.1	2.9	98.8	90.7	63.1	38	-

-Y1RT- SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS- 503	58' LT	14+59	2.5' - 4.0'	A-4(0)	36	4	30.1	31.9	21.0	17.0	88.4	70.9	39.3	27	-
SS- 2065	36' LT	16+01	1.0' - 2.5'	A-4(0)	31	9	33.4	31.2	33.9	1.5	87.3	68.3	36.8	16	-
SS- 487	45' LT	17+00	0.0' - 1.5'	A-6 (3)	39	14	23.2	23.9	27.6	25.3	79.8	67.3	45.9	20	-
SS- 492	45' LT	16+94	15.0' - 16.5'	A-4	35	7	34.3	25.0	26.4	14.3	83.0	62.5	38.6	21	-
SS- 480	50' LT	18+00	2.5- 4.0'	A-7-5 (3)	46	14	27.2	32.8	28.2	11.8	94.9	79.7	44.6	31	-
SS- 472	50' LT	19+00	0.0' - 1.5'	A-2-6	33	12	37.6	26.4	13.9	22.1	57.5	41.8	23.5	9	-

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REVISIONS

REVISIONS

-Y1RT- SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
SS- 462	70' LT	20+50	2.5' - 4.0'	A-2-4	40	9	44.0	25.3	19.7	11.0	94.6	62.5	34.0	11	-
SS- 315	28' RT	21+50	2.5' - 4.0'	A-4 (1)	38	8	28.5	29.9	34.3	7.3	94.2	75.9	45.9	21	-
SS- 2056	75' LT	22+01	8.8' - 10.3'	A-4	31	NP	34.8	27.9	34.3	3.0	98.2	73.2	42.6	19	-
SS- 2051	84' LT	22+51	8.6' - 10.1'	A-2-4	32	NP	37.1	25.9	33.1	3.9	59.6	43.3	25.7	8	-
SS- 2052	84' LT	22+51	13.6' - 15.1'	A-4	31	NP	39.5	22.7	34.4	3.4	98.5	70.5	41.7	13	-
ST- 5	178' LT	22+76	35.0' - 37.0'	A-2-4(0)	NP	NP	51.7	20.6	16.5	11.2	98.2	65.2	28.2	42	-
SS- 339	9' RT	23+00	2.5' - 4.0'	A-7-6 (5)	45	16	12.2	49.5	29.5	8.8	100.0	95.7	49.2	25	-
SS- 347	5' RT	23+50	5.0' - 6.5'	A-2-4	34	8	39.8	31.4	18.9	9.9	96.5	70.5	33.6	11	-
SS- 2018	145' LT	25+52	0.8' - 2.3'	A-6 (8)	38	15	13.9	20.3	29.7	36.1	91.2	83.0	63.9	13	-
SS- 2019	145' LT	25+52	3.5' - 5.0'	A-5 (2)	45	6	23.5	31.4	38.6	6.5	92.9	77.9	49.7	12	-
SS- 2021	145' LT	25+52	8.5' - 10.0'	A-7-5 (1)	48	14	33.9	28.3	28.5	9.3	83.1	62.6	36.8	24	-

PROJECT REFERENCE NO.		SHEET NO.	
B-3186B-5898		66	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER			HYDRAULICS ENGINEER
INCOMPLETE PLANS <small>DO NOT USE FOR A/W ACQUISITION</small>			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			
		HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116	