

REFERENCE: B-3186/B-5898

PROJECT: 38332/48030

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3186/B-5898	1	17

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SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND
3	SITE PLAN
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8-16	BORE LOGS, CORE REPORTS & CORE PHOTOGRAPHS
17	ROCK TEST RESULTS

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY HAYWOOD
PROJECT DESCRIPTION US 23/US 74/US 19 (GREAT SMOKY MOUNTAIN HWY) FROM WEST OF NC 209 (CRABTREE RD.) TO EAST OF RUSS AVE.
SITE DESCRIPTION BRIDGE NO. 430110 & 430107 ON -L LT- (US 1923/74 WB) OVER THE BLUE RIDGE SOUTHERN RAILROAD (BLU) BETWEEN US 276 AND NC 209

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

C. SWAFFORD

J. CRENSHAW

GEOTECHNOLOGY, INC.

INVESTIGATED BY C. SWAFFORD

DRAWN BY T. LYNN

CHECKED BY P. ZHANG

SUBMITTED BY HDR

DATE AUGUST 2021



SIGNATURE _____ DATE _____

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION								GRADATION								ROCK DESCRIPTION								TERMS AND DEFINITIONS																																																																																																																																																
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>								<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>								<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>								<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENISE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																																																																																																																
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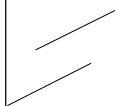
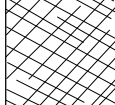
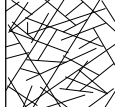


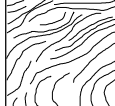
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
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GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

<p>GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)</p> <p>From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.</p> <p>STRUCTURE</p>	<p>SURFACE CONDITIONS</p> <p>VERY GOOD Very rough, fresh unweathered surfaces</p> <p>GOOD Rough, slightly weathered, iron stained surfaces</p> <p>FAIR Smooth, moderately weathered and altered surfaces</p> <p>POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments</p> <p>VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings</p> <p align="center">DECREASING SURFACE QUALITY →</p>					<p>GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)</p> <p>From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.</p> <p>SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)</p> <p>VERY GOOD - Very Rough, fresh unweathered surfaces</p> <p>GOOD - Rough, slightly weathered surfaces</p> <p>FAIR - Smooth, moderately weathered and altered surfaces</p> <p>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</p> <p>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</p>					
<p>INTERLOCKING OF ROCK PIECES</p> <p align="center">DECREASING INTERLOCKING OF ROCK PIECES ↓</p>	<p>COMPOSITION AND STRUCTURE</p>										
 <p>INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities</p>	90	80	70	60	50	N/A	70	60	50	40	30
 <p>BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets</p>	80	70	60	50	40	N/A	60	50	40	30	20
 <p>VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets</p>	70	60	50	40	30	N/A	50	40	30	20	10
 <p>BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity</p>	60	50	40	30	20	N/A	40	30	20	10	0
 <p>DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces</p>	50	40	30	20	10	N/A	30	20	10	0	0
 <p>LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes</p>	N/A	N/A	N/A	N/A	N/A	N/A	20	10	0	0	0

A. Thick bedded, very blocky sandstone
The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.

B. Sandstone with thin inter-layers of siltstone **C. Sandstone and siltstone in similar amounts** **D. Siltstone or silty shale with sandstone layers** **E. Weak siltstone or clayey shale with sandstone layers**

C, D, E, and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to **F** and **H**.

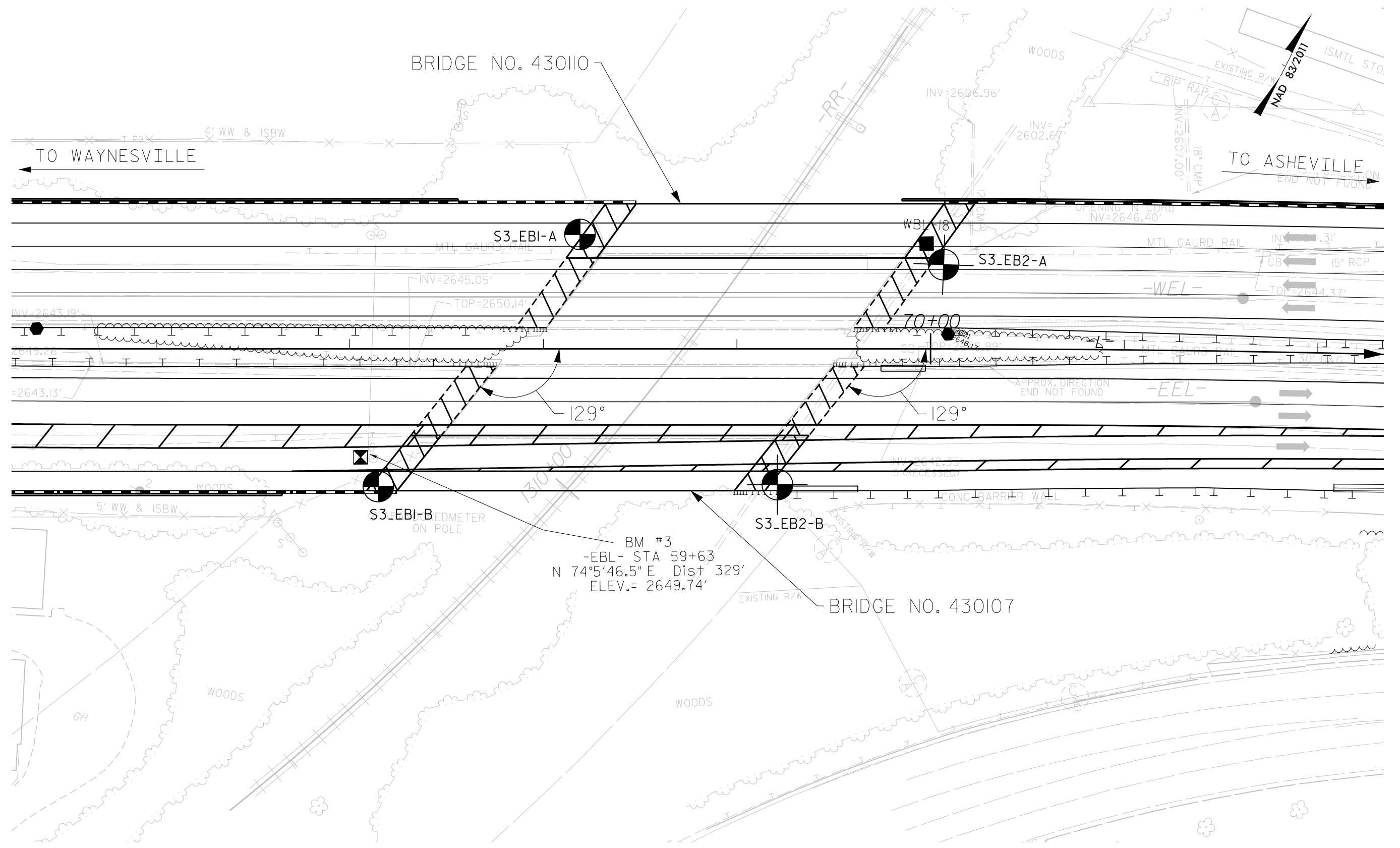
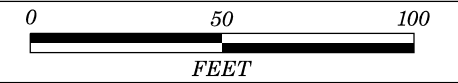
F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure

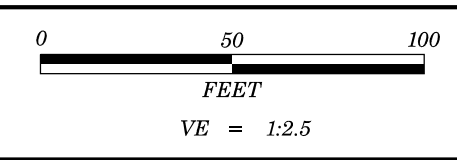
G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers

H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.

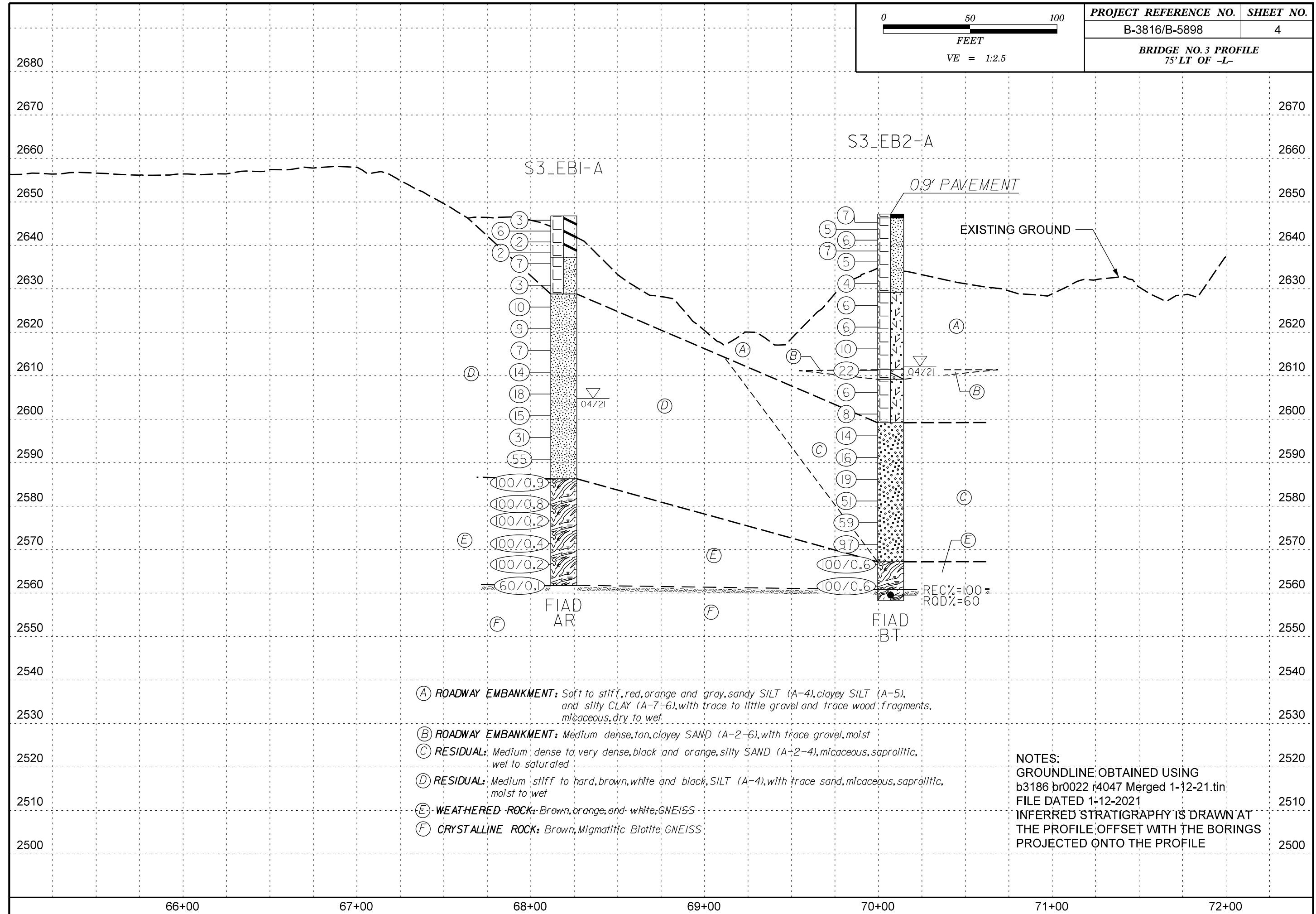
→ Means deformation after tectonic disturbance

SITE PLAN





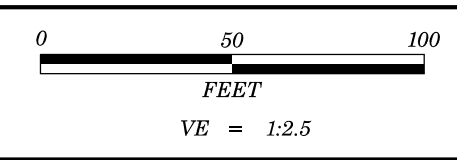
PROJECT REFERENCE NO.	SHEET NO.
B-3816/B-5898	4
BRIDGE NO. 3 PROFILE 75' LT OF -L-	



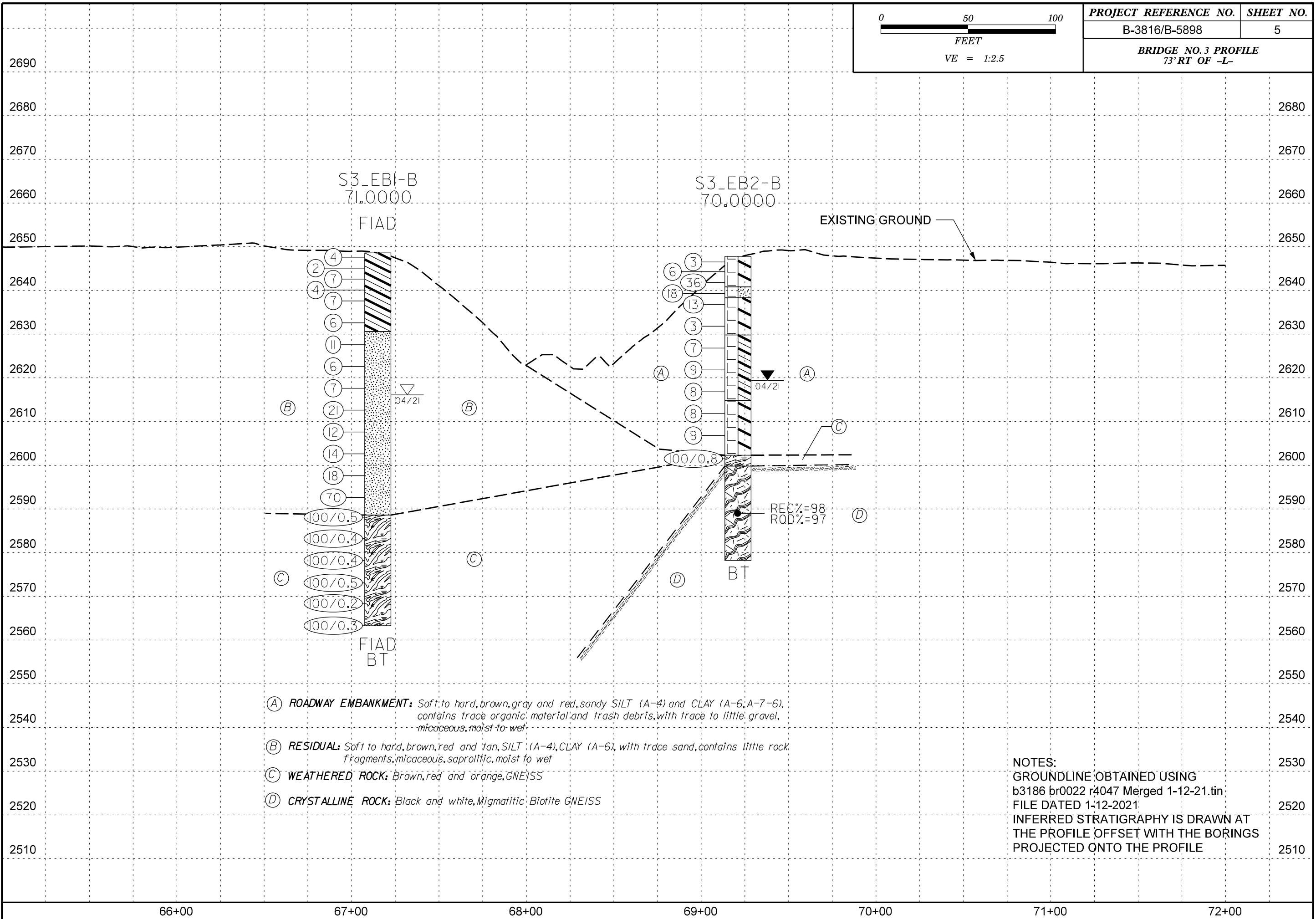
- (A) ROADWAY EMBANKMENT: Soft to stiff, red, orange and gray, sandy SILT (A-4), clayey SILT (A-5), and silty CLAY (A-7-6), with trace to little gravel and trace wood fragments, micaceous, dry to wet.
- (B) ROADWAY EMBANKMENT: Medium dense, tan, clayey SAND (A-2-6), with trace gravel, moist
- (C) RESIDUAL: Medium dense to very dense, black and orange, silty SAND (A-2-4), micaceous, saprolitic, wet to saturated.
- (D) RESIDUAL: Medium stiff to hard, brown, white and black, SILT (A-4), with trace sand, micaceous, saprolitic, moist to wet
- (E) WEATHERED ROCK: Brown, orange, and white, GNEISS
- (F) CRYSTALLINE ROCK: Brown, Migmatitic Biotite, GNEISS

NOTES:
GROUNDLINE OBTAINED USING
b3186 br0022 r4047 Merged 1-12-21.tin
FILE DATED 1-12-2021
INFERRED STRATIGRAPHY IS DRAWN AT
THE PROFILE OFFSET WITH THE BORINGS
PROJECTED ONTO THE PROFILE

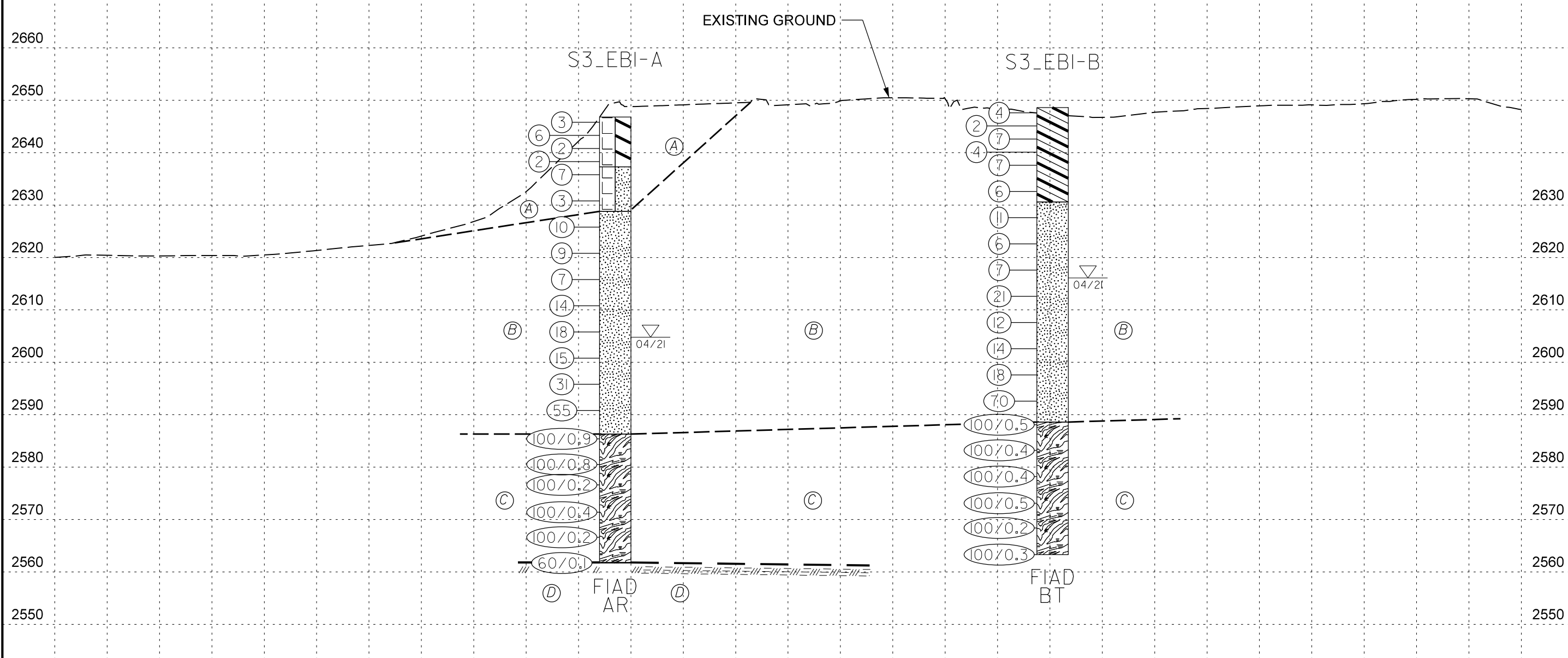
66+00 67+00 68+00 69+00 70+00 71+00 72+00



PROJECT REFERENCE NO.	SHEET NO.
B-3816/B-5898	5
BRIDGE NO. 3 PROFILE 73' RT OF -L-	



66+00 67+00 68+00 69+00 70+00 71+00 72+00



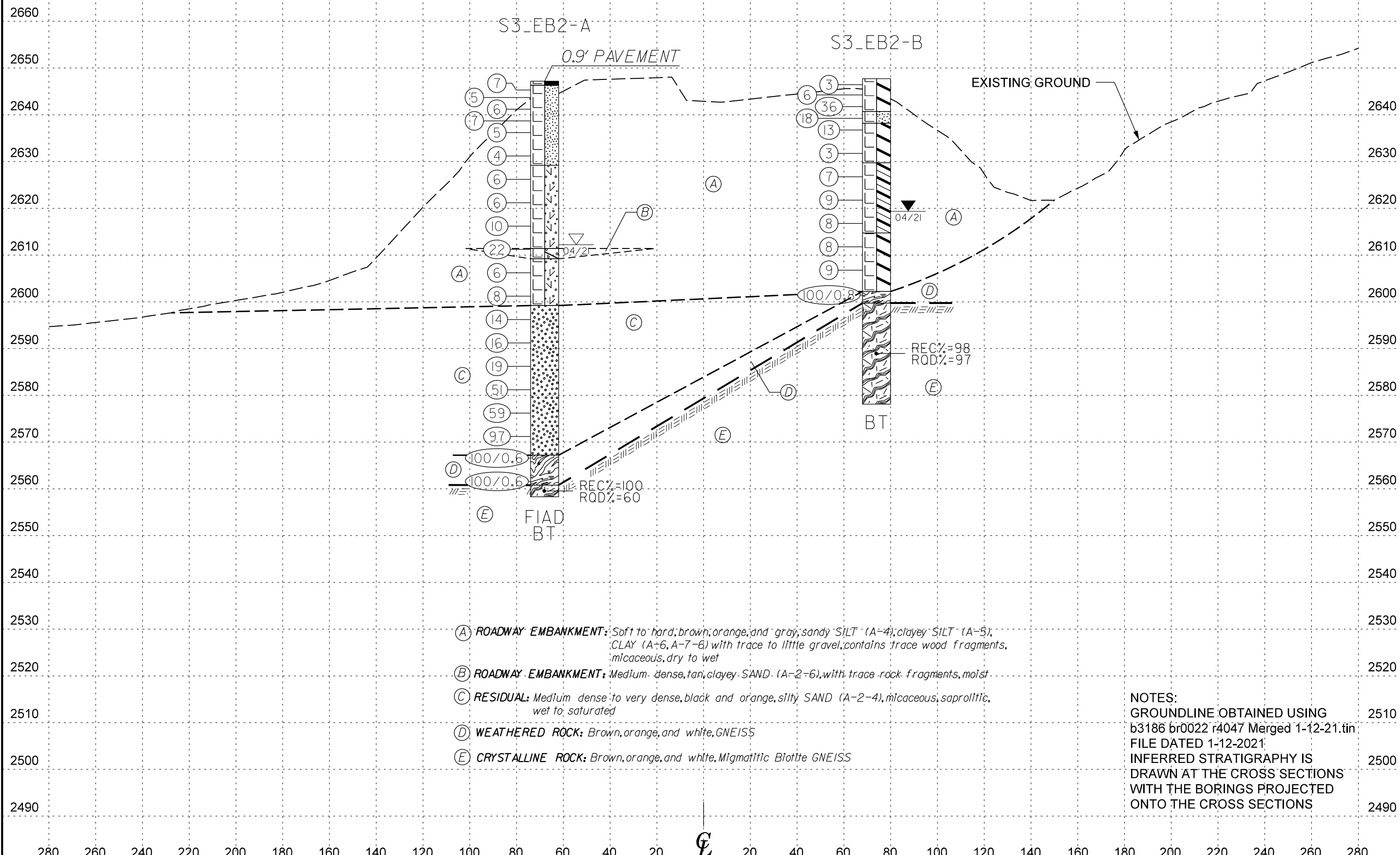
- (A) **ROADWAY EMBANKMENT:** Soft to medium stiff, red and tan, SILT (A-4) and silty CLAY (A-7-6), with trace gravel, micaceous, moist to wet
- (B) **RESIDUAL:** Soft to hard, brown, red and white, SILT (A-4), CLAY (A-6), with trace sand, contains little rock fragments, micaceous, saprolitic, moist to wet
- (C) **WEATHERED ROCK:** Brown, orange, and white, GNEISS
- (D) **CRYSTALLINE ROCK:** Brown, orange, and white, GNEISS

NOTES:
GROUNDLINE OBTAINED USING
b3186 br0022 r4047 Merged 1-12-21.tin
FILE DATED 1-12-2021
INFERRED STRATIGRAPHY IS
DRAWN AT THE CROSS SECTIONS
WITH THE BORINGS PROJECTED
ONTO THE CROSS SECTIONS



VE = 1:1

BRIDGE NO. 3 - END BENT 1 -L- STA. 67+86.38 129° SKEW



- (A) ROADWAY EMBANKMENT: Soft to hard, brown, orange, and gray, sandy SILT (A-4), clayey SILT (A-5), CLAY (A-6, A-7-6) with trace to little gravel, contains trace wood fragments, micaceous, dry to wet
- (B) ROADWAY EMBANKMENT: Medium dense, tan, clayey SAND (A-2-6), with trace rock fragments, moist
- (C) RESIDUAL: Medium dense to very dense, black and orange, silty SAND (A-2-4), micaceous, saprolitic, wet to saturated
- (D) WEATHERED ROCK: Brown, orange, and white, GNEISS
- (E) CRYSTALLINE ROCK: Brown, orange, and white, Migmatitic Biotite GNEISS

NOTES:
GROUNDLINE OBTAINED USING
b3186 br0022 r4047 Merged 1-12-21.tin
FILE DATED 1-12-2021
INFERRED STRATIGRAPHY IS
DRAWN AT THE CROSS SECTIONS
WITH THE BORINGS PROJECTED
ONTO THE CROSS SECTIONS

GEOTECHNICAL BORING REPORT

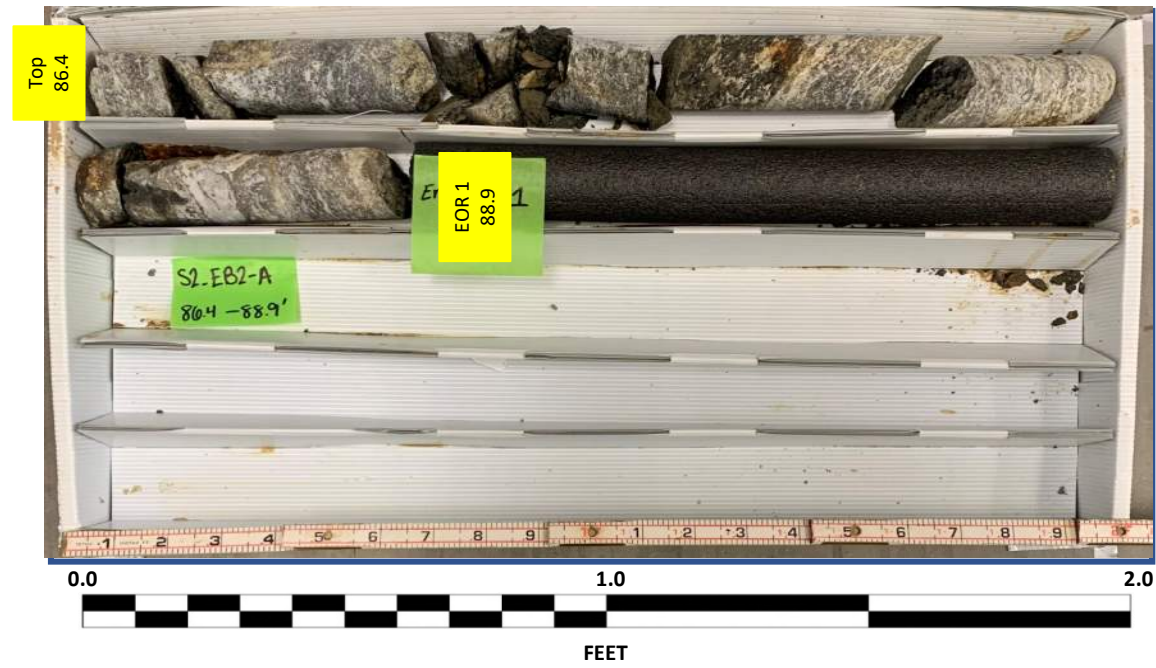
CORE LOG

WBS 38332.1.FS1		TIP B-3186 / B-5898		COUNTY HAYWOOD		GEOLOGIST J. Crenshaw					
SITE DESCRIPTION US 23/ US 74 (Great Smoky Mountain Highway)							GROUND WTR (ft)				
BORING NO. S3_EB2-A		STATION 70+07		OFFSET 43 ft LT		ALIGNMENT -L-					
COLLAR ELEV. 2,647.2 ft		TOTAL DEPTH 88.9 ft		NORTHING 667,892		EASTING 821,164					
DRILL RIGHAMMER EFF./DATE GTC8255 CME-55 93%(11/24/2020)				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic					
DRILLER L. Wanstrath		START DATE 04/12/21		COMP. DATE 04/13/21		SURFACE WATER DEPTH N/A					
CORE SIZE NQ2		TOTAL RUN 2.5 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (ft) %	RQD (ft) %	REC. (ft) %	RQD (ft) %			
2560.8	2560.8	86.4	2.5	1:11	(2.5)	(1.5)	(2.5)	(1.5)		Begin Coring @ 86.4 ft	86.4
	2558.3	88.9		1:56 2:35/0.5	100%	60%	100%	60%		CRYSTALLINE ROCK Brown, Migmatitic Biotite GNEISS, moderate to severe weathering, hard, close fracture spacing	88.9
										Boring Terminated at Elevation 2,558.3 ft in Crystalline Rock (GNEISS)	
										<p style="text-align: center;">NOTES</p> <p>Core barrel blocked off and wireline cable malfunction - Rock fell into hole when core barrel removed to retrieve core barrel</p> <p>Abandoned boring to allow for time to get off road before traffic closure stop time</p>	

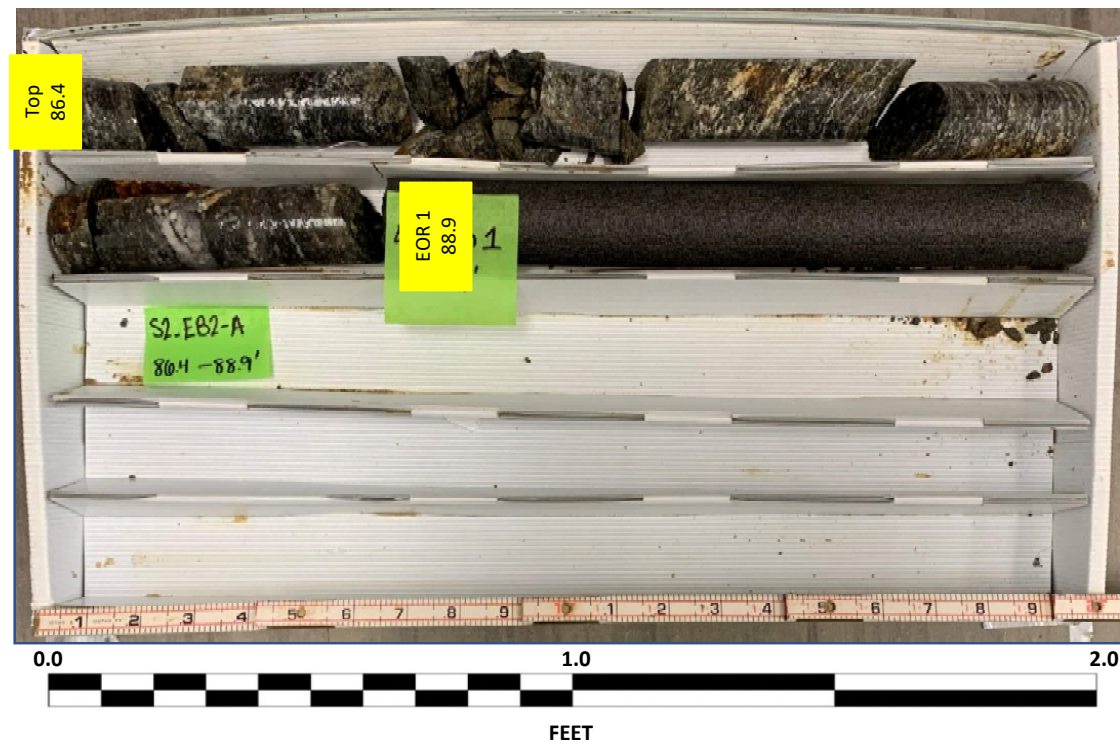
NCDOT CORE DOUBLE B3186_GEO_SPT.GPJ NC_DOT.GDT 8/5/21

CORE PHOTOGRAPHIC RECORD
38330.1.FS1 (B-3186/B-5898)
US 23/ US 74 Great Smokey Mountain Highway

S3_EB2-A
Box 1 of 1: 86.4 – 88.9 FEET
DRY



S3_EB2-A
Box 1 of 1: 86.4 – 88.9 FEET
WET



GEOTECHNICAL BORING REPORT

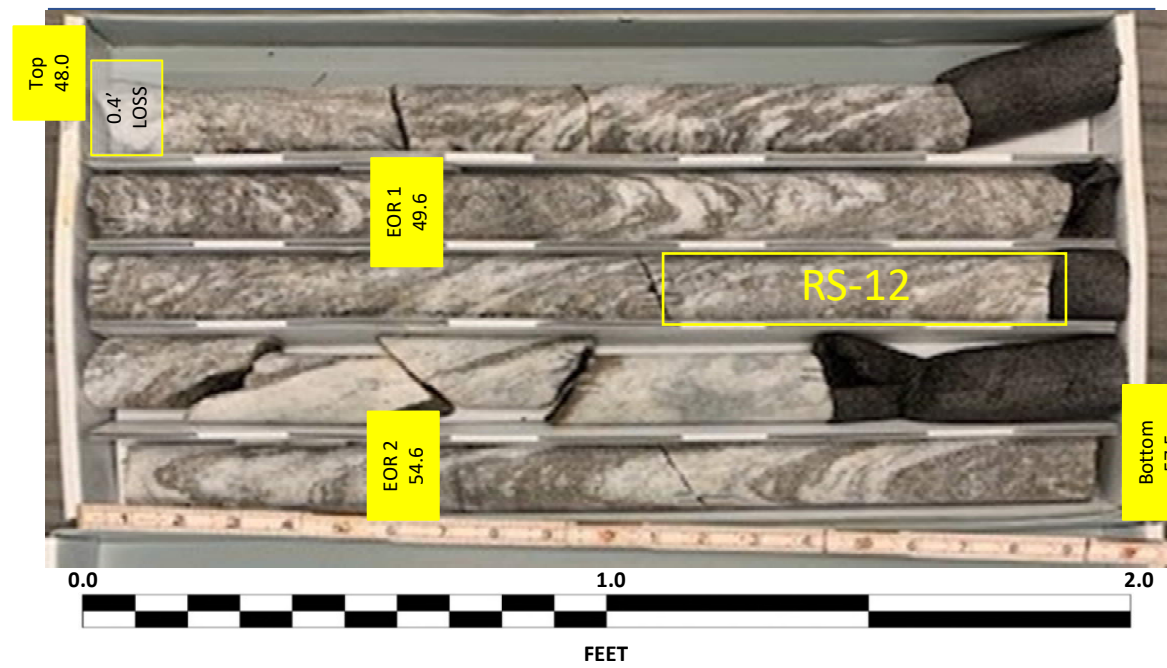
CORE LOG

WBS 38332.1.FS1		TIP B-3186 / B-5898		COUNTY HAYWOOD		GEOLOGIST C. Swafford						
SITE DESCRIPTION US 23/ US 74 (Great Smoky Mountain Highway)							GROUND WTR (ft)					
BORING NO. S3_EB2-B		STATION 69+21		OFFSET 70 ft RT		ALIGNMENT -L-						
COLLAR ELEV. 2,647.8 ft		TOTAL DEPTH 69.6 ft		NORTHING 667,752		EASTING 821,138						
DRILL RIGHAMMER EFF./DATE GTC9083 CME-550X 80%(11/24/2020)				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic						
DRILLER L. Wanstrath		START DATE 04/09/21		COMP. DATE 04/09/21		SURFACE WATER DEPTH N/A						
CORE SIZE NQ2		TOTAL RUN 21.6 ft										
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)	
					REC. (ft) %	RQD (ft) %	REC. (ft) %	RQD (ft) %				
2599.8	2,599.8	48.0	1.6	0:30/0.6	(1.2)	(1.2)	(21.2)	(21.0)		Begin Coring @ 48.0 ft CRYSTALLINE ROCK Black and white, Migmatitic Biotite GNEISS, fresh to very slight weathering, hard to very hard, close to wide fracture spacing; 0.4' core loss Epidote along healed fractures RS-12 52.1' - 52.8' GSI= 80 - 90 Qu= 11,009 psi	48.0	
	2,598.2	49.6	5.0	1:49 1:13 1:40 1:32 2:04	(5.0) 75%	(5.0) 75%						
2595					100%	100%						
	2,593.2	54.6	5.0	1:37 1:41 1:50 1:52 2:08	(5.0) 100%	(5.0) 100%						
2590												
	2,588.2	59.6	5.0	1:58 1:36 2:08 1:59 2:29	(5.0) 100%	(5.0) 100%						
2585												
	2,583.2	64.6	5.0	1:46 1:39 1:52 1:57 1:54	(5.0) 100%	(4.8) 96%						
2580												
	2,578.2	69.6										69.6
Boring Terminated at Elevation 2,578.2 ft in Crystalline Rock (GNEISS)												
NOTES 0.3' Topsoil Rig chatter and grinding encountered at 5.0' Rig chatter encountered at 10.0' Rig chatter and hard drilling encountered at 48.0' Auger refusal at 48.0'												

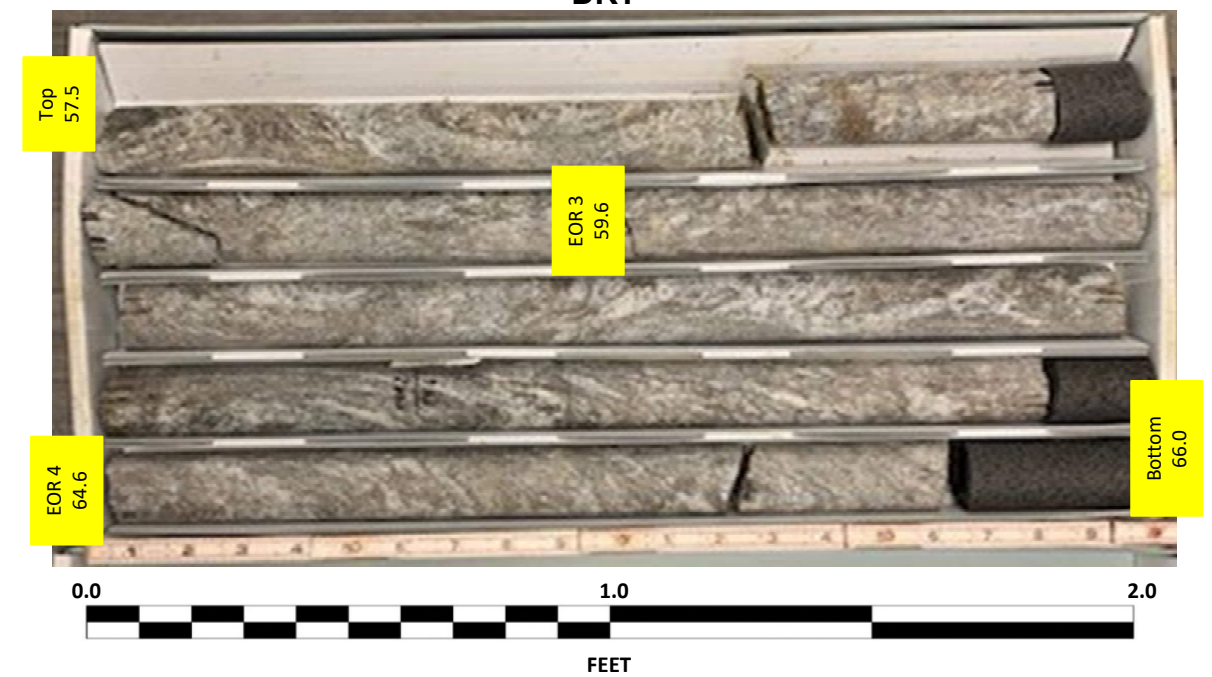
NCDOT CORE DOUBLE B3186_GEO_SPT.GPJ NC_DOT.GDT 8/5/21

CORE PHOTOGRAPHIC RECORD
38330.1.FS1 (B-3186/B-5898)
US 23/ US 74 Great Smokey Mountain Highway

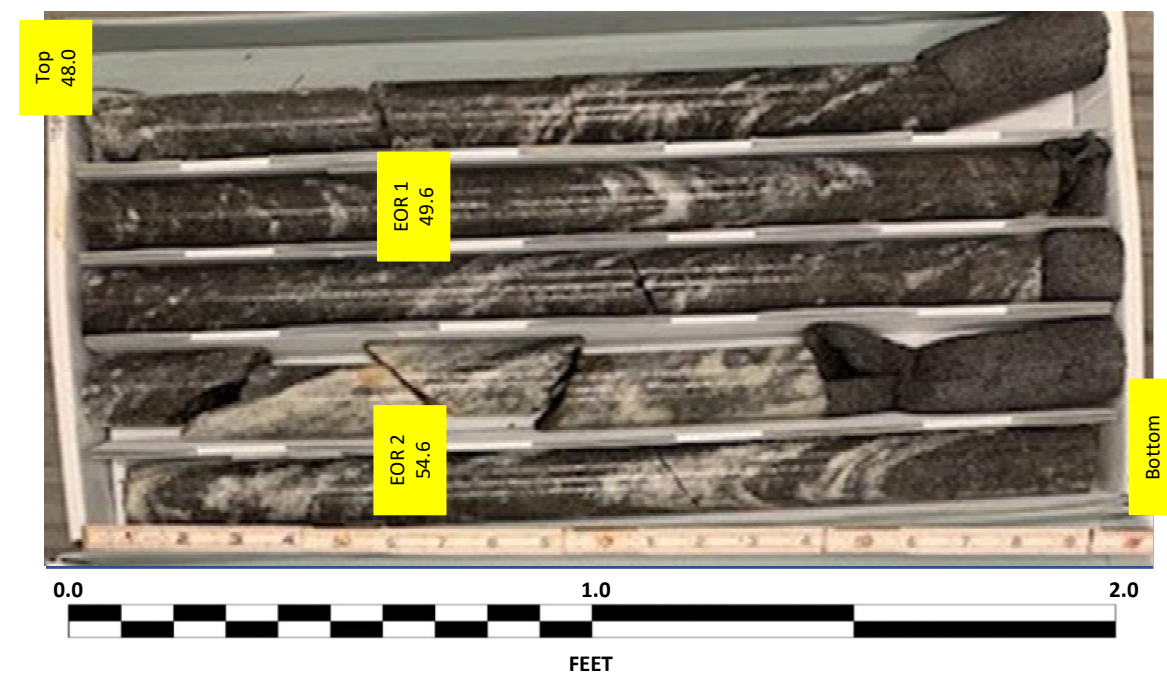
S3_EB2-B
Box 1 of 3: 48.0 – 57.5 FEET
DRY



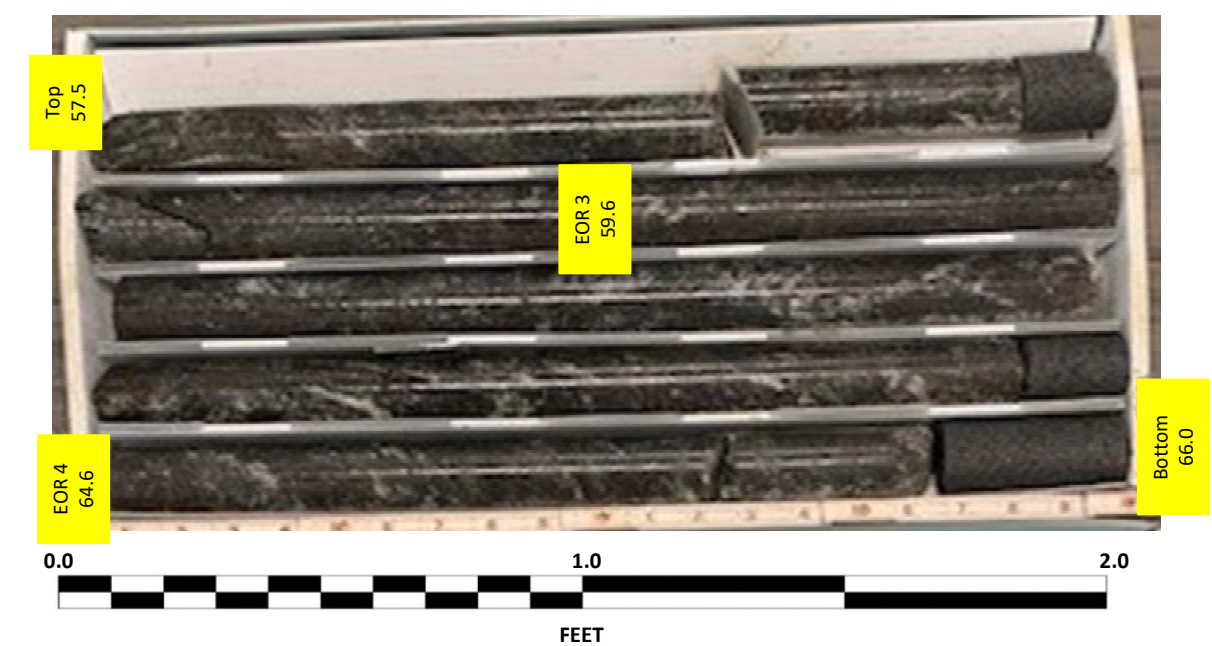
S3_EB2-B
Box 2 of 3: 57.5 – 66.0 FEET
DRY



S3_EB2-B
Box 1 of 3: 48.0 – 57.5 FEET
WET



S3_EB2-B
Box 2 of 3: 57.5 – 66.0 FEET
WET



CORE PHOTOGRAPHIC RECORD
38330.1.FS1 (B-3186/B-5898)
US 23/ US 74 Great Smokey Mountain Highway

S3_EB2-B
Box 3 of 3: 66.0 – 69.6 FEET
DRY



S3_EB2-B
Box 3 of 3: 66.0 – 69.6 FEET
WET





REPORT ON SAMPLES OF: Rock For Quality

PROJECT: B-3186 / B-5898
DATE SAMPLED: 05/11/2021
SAMPLED FROM: Test Borings
SUBMITTED BY: HDR

COUNTY: Haywood
RECEIVED: 5/11/2021
REPORTED: 5/12/2021
BY / CERT NO: Kevin E. Walker

BORING NO	SAMPLE	DEPTH (FT)	ROCK TYPE	LENGTH (IN)	DIAMETER (IN)	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH (PSI)
S3_EB2-B	RS-12	52.1-52.8	Biotite Gneiss	4.13	1.86	179.10	11,009