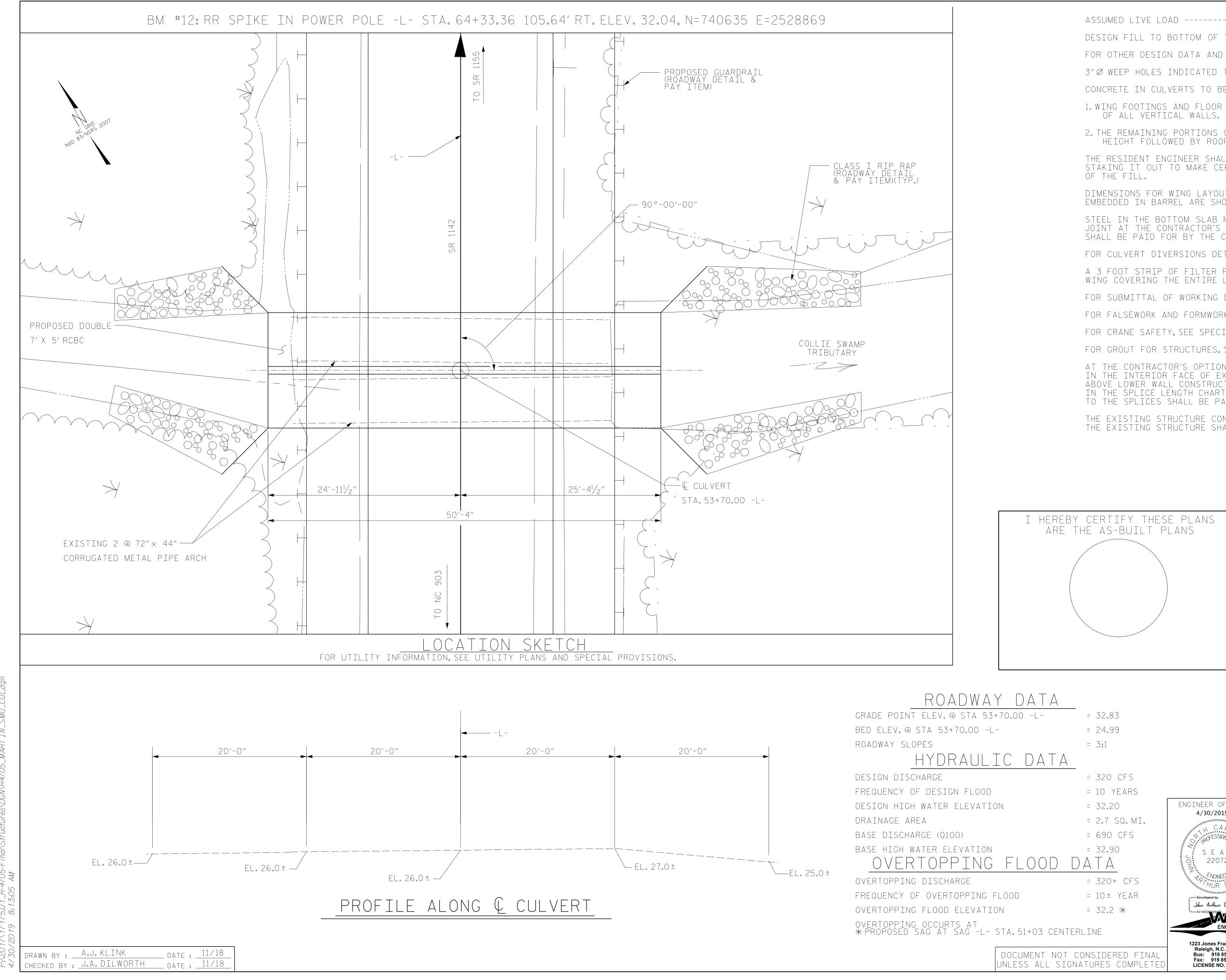
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NOTES
UMED LIVE LOAD HL-93 OR ALTERNATE LOADING.
IGN FILL TO BOTTOM OF TOP SLAB 2.02'(MIN.)AND 2.84'(MAX)
OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.
WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.
CRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:
ING FOOTINGS AND FLOOR SLAB INCLUDING 4″ OF ALL VERTICAL WALLS.
HE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.
RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE KING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE THE FILL.
IENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EDDED IN BARREL ARE SHOWN ON WING SHEET.
EL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION NT AT THE CONTRACTOR'S OPTION.EXTRA WEIGHT OF STEEL DUE TO THE SPLICES LL BE PAID FOR BY THE CONTRACTOR.
CULVERT DIVERSIONS DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.
FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE IG COVERING THE ENTIRE LENGHT OF EXPANSION JOINT.
SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
CRANE SAFETY, SEE SPECIAL PROVISIONS.
GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
THE CONTRACTOR'S OPTION HE MAY SPITCE THE VERTICAL RETNEORCING STEEL

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS.EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

THE EXISTING STRUCTURE CONSISTS OF 2 @ 72″X 44″CORRUGATED METAL PIPE ARCH. THE EXISTING STRUCTURE SHALL BE REMOVED.

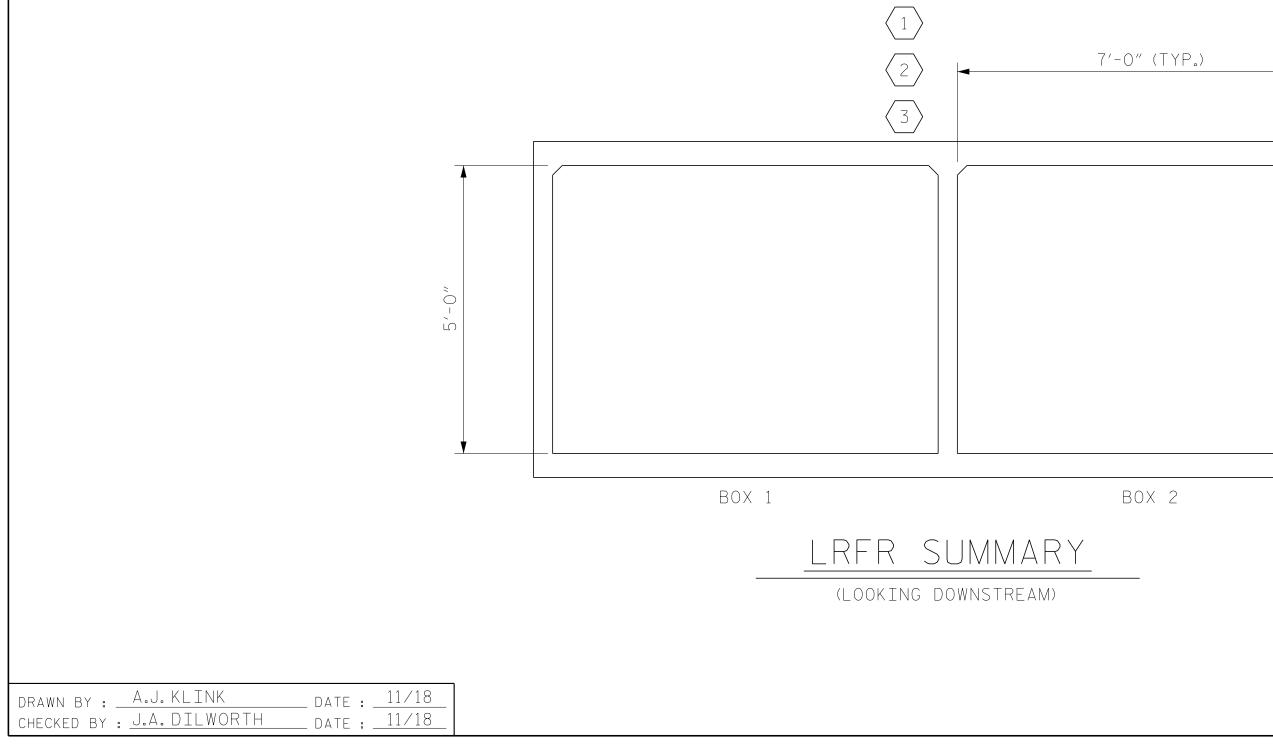
TOTAL STRUCTURE QUAN	TITIES
REMOVAL OF EXISTING STRUCTURE	LUMP SUM
CULVERT EXCAVATION	LUMP SUM
FOUNDATION CONDITIONING MATERIAL	71 TONS
CLASS A CONCRETE	
BARREL @1.539_CY/FT	77.5 C.Y.
WING ETC	20.4 <u></u> C.Y.
TOTAL	97.9 <u></u> C.Y.
REINFORCING STEEL	
BARREL	<u>10110</u> LBS.
WINGS ETC	<u>1200</u> LBS.
TOTAL	<u>11310</u> LBS.

.83 .99 0 CFS YEARS		PROJEC STATIC	MART dn:5	IN	CO) UNTY
.20 7 SQ.MI. 0 CFS .90 <u>A</u> 0+ CFS ± YEAR .2 *	ENGINEER OF RECORD 4/30/2019 H. C.A.R.O. FROFESSION S.E.A.L. 22072 H.H. T. S. KOINEER Joursson Docusigned by: Jdus A.Hur Disorth. 621382200228420 ETHERILL	DOL	RTMENT BARRE JBLE NCRETI	raleigh LSTA 7 FT.	NSPORTA ANDAR X 5 F CULN	D T.
ERED FINAL	ENGINEERING 1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107	NO. ВҮ:	REVIS DATE:	NO. BY:	DATE:	SHEET NO. C1 Total sheets
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						STRENGTH I LIMIT STATE										
LEVEL								MOMENT			SHEAR					
		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM Rating factors (RF)	X VCT	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM Left end of element (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	
		HL-93 (INVENTORY)	N/A	$\langle 1 \rangle$	1.05		1.75	1.35	1	TOP SLAB	3.40	1.05	1	TOP SLAB	6.88	
DESIGN LOAD	HL-93 (OPERATING)	N/A		1.36		1.35	1.75	1	TOP SLAB	3.40	1.36	1	TOP SLAB	6.88		
RATING		HS-20 (INVENTORY)	36.000	2	1.19	42.84	1.75	1.41	1	TOP SLAB	3.40	1.19	1	TOP SLAB	6.88	
		HS-20 (OPERATING)	36.000		1.54	55.44	1.35	1.83	1	TOP SLAB	3.40	1.54	1	TOP SLAB	6.88	
		SNSH	13.500		2.21	29.84	1.40	2.58	1	TOP SLAB	3.40	2.21	1	TOP SLAB	6.88	
		SNGARBS2	20.000		2.07	41.40	1.40	2.41	1	TOP SLAB	3.40	2.07	1	TOP SLAB	6.88	
		SNAGRIS2	22.000		2.21	48.62	1.40	2.57	1	TOP SLAB	3.40	2.21	1	TOP SLAB	6.88	
	<pre>// CHI</pre>	SNCOTTS3	27.250	$\langle 3 \rangle$	1.33	36.24	1.40	2.08	1	TOP SLAB	3.40	1.33	1	TOP SLAB	6.88	
	ы S	SNAGGRS4	34.925		1.69	59.02	1.40	2.30	1	BOTTOM SLAB	7.67	1.69	1	TOP SLAB	6.88	
	INGL	SNS5A	35.550		1.55	55.10	1.40	2.16	1	BOTTOM SLAB	7.67	1.55	1	TOP SLAB	6.88	
	S I	SNS6A	39.950		1.54	61.52	1.40	2.09	1	BOTTOM SLAB	7.67	1.54	1	TOP SLAB	6.88	
LEGAL		SNS7B	42.000		1.53	64.26	1.40	2.09	1	BOTTOM SLAB	7.67	1.53	1	TOP SLAB	6.88	
LOAD Rating	ER	TNAGRIT3	33.000		2.14	70.62	1.40	2.57	1	TOP SLAB	3.40	2.14	1	TOP SLAB	6.88	
	RAIL	TNT4A	33.075		1.58	52.26	1.40	2.24	1	BOTTOM SLAB	7.67	1.58	1	TOP SLAB	6.88	
	EMI-T	TNT6A	41.600		1.55	64.48	1.40	2.30	1	BOTTOM SLAB	7.67	1.55	1	TOP SLAB	6.88	
		ΤΝΤ7Α	42.000		1.57	65.94	1.40	2.27	1	BOTTOM SLAB	7.67	1.57	1	TOP SLAB	6.88	
	TOR (TT	TNT7B	42.000		1.56	65.52	1.40	2.10	1	BOTTOM SLAB	7.67	1.56	1	TOP SLAB	6.88	
	TRAC	TNAGRIT4	43.000		1.58	67.94	1.40	1.92	1	BOTTOM SLAB	7.67	1.58	1	TOP SLAB	6.88	
		TNAGT5A	45.000		1.58	71.10	1.40	1.92	1	BOTTOM SLAB	7.67	1.58	1	TOP SLAB	6.88	
	TRUC	TNAGT5B	45.000		1.58	71.10	1.40	1.95	1	BOTTOM SLAB	7.67	1.58	1	TOP SLAB	6.88	





LOAD FACTORS:

LOAD TYPE	.OAD TYPE FACTOR			
DC	1.25	0.90		
DW	1.50	0.65		
ΕV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WΑ	1.00			

DESIGN LOAD RATING FACTORS

NOTE:

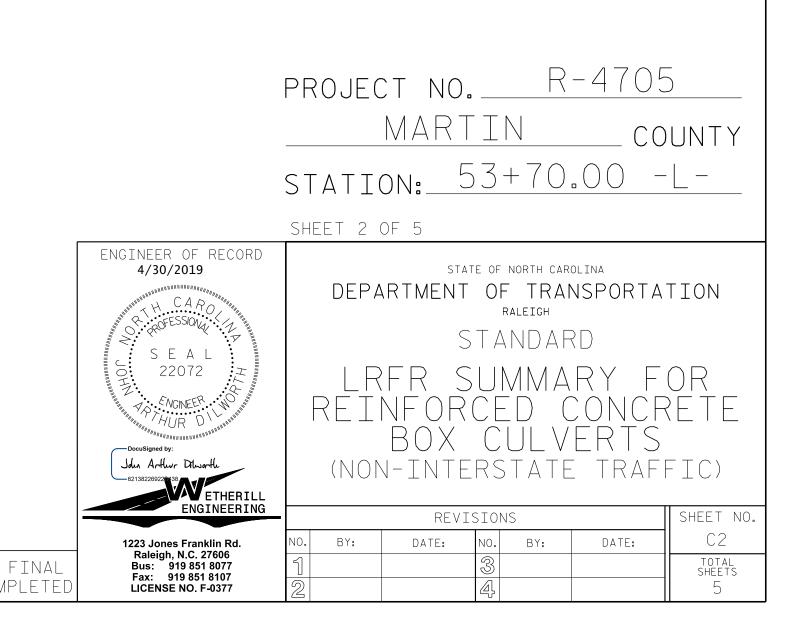
RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

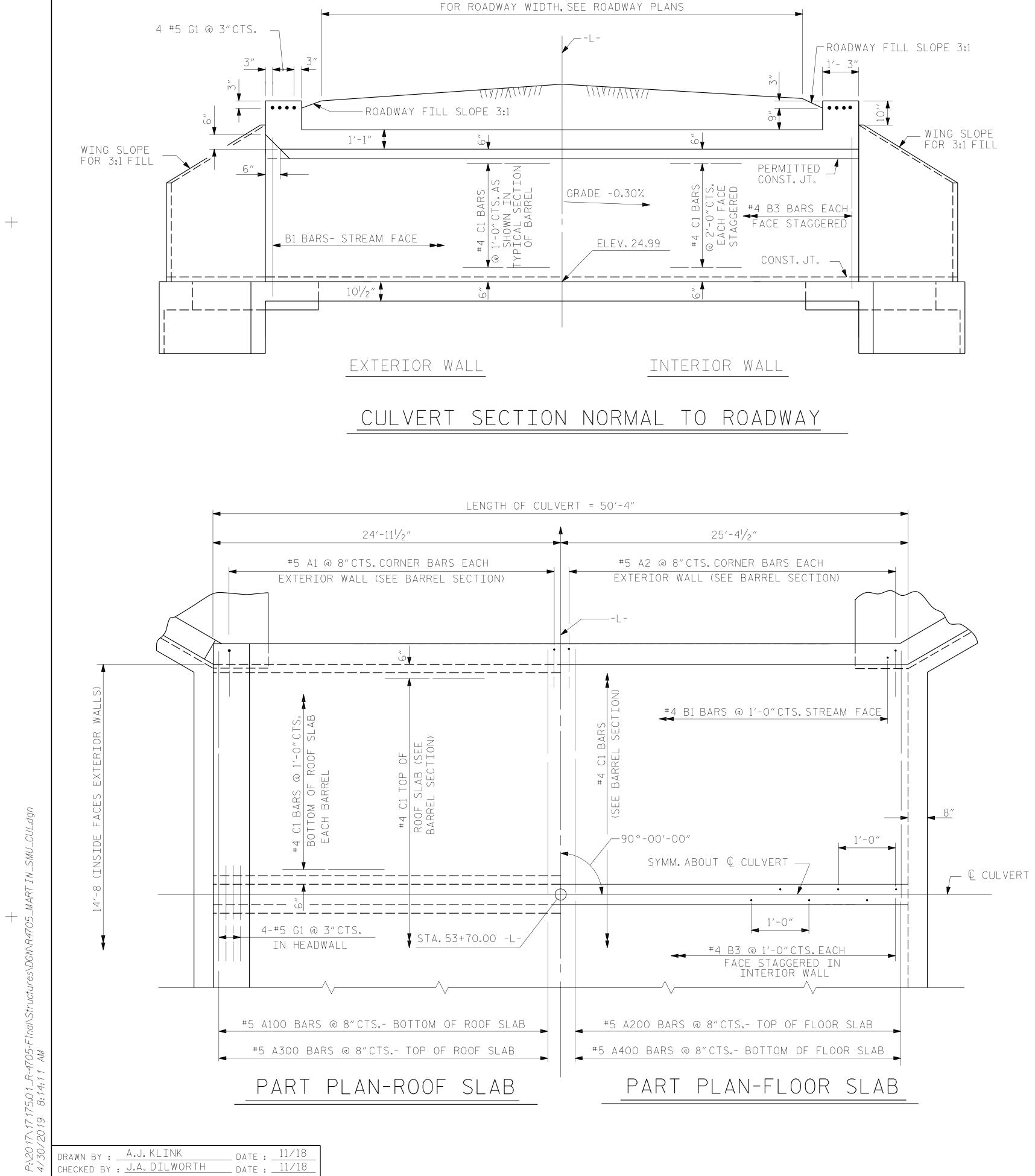
COMMENTS:

1.	
2.	
3.	

4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
* * SEE CHART FOR VEHICLE TYPE



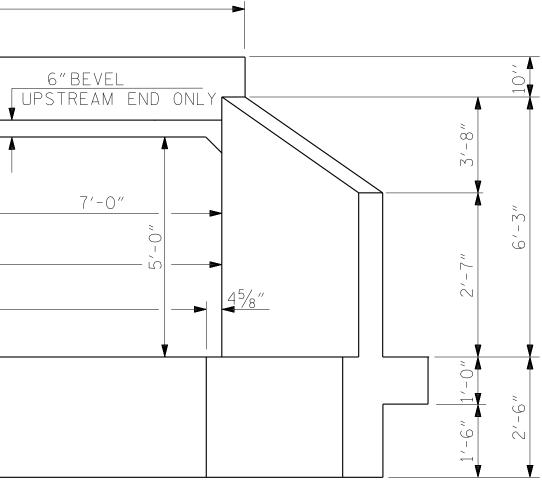


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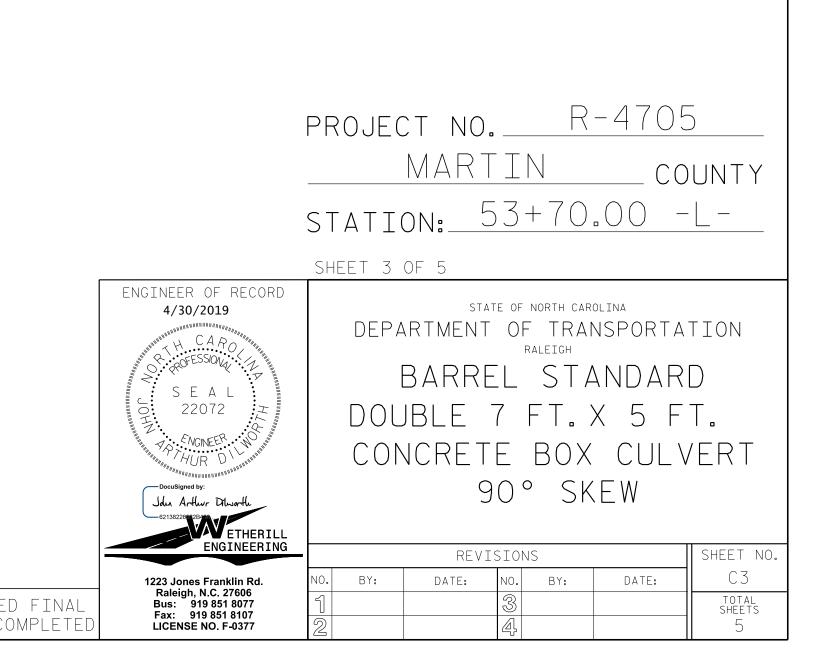
16′-0″ 7′-0″ 14′-8″ 13′-10¾″

END ELEVATION

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE



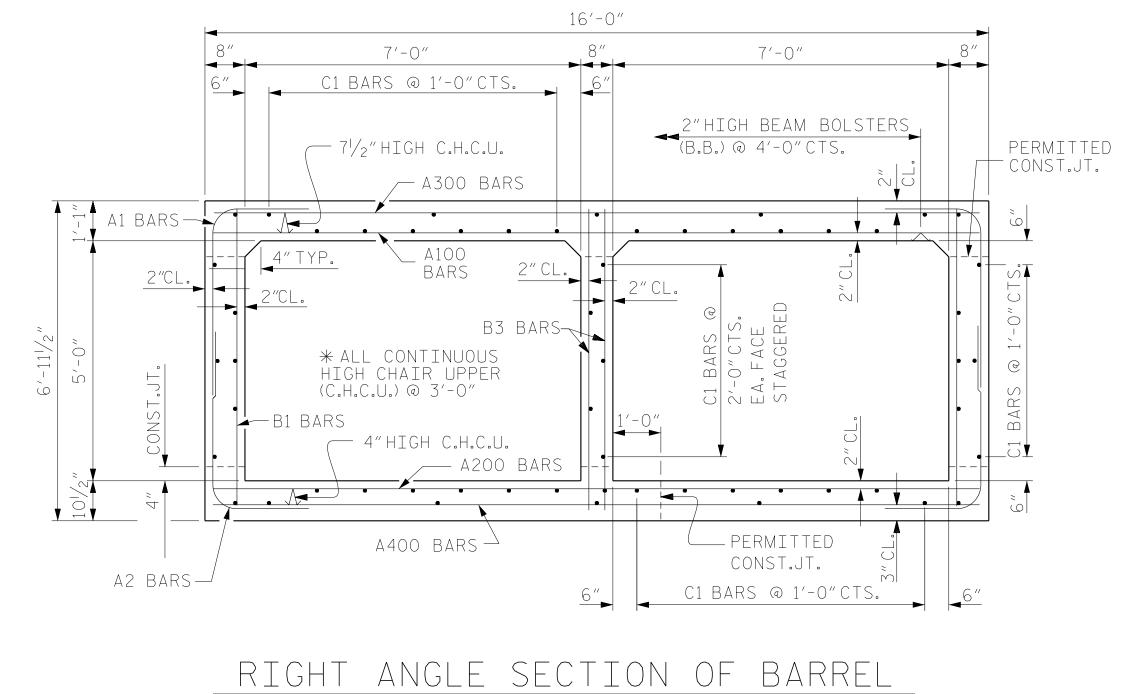




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4/	CHECKED BY : _	J.A. DILWORTH	_ DATE :	11/18



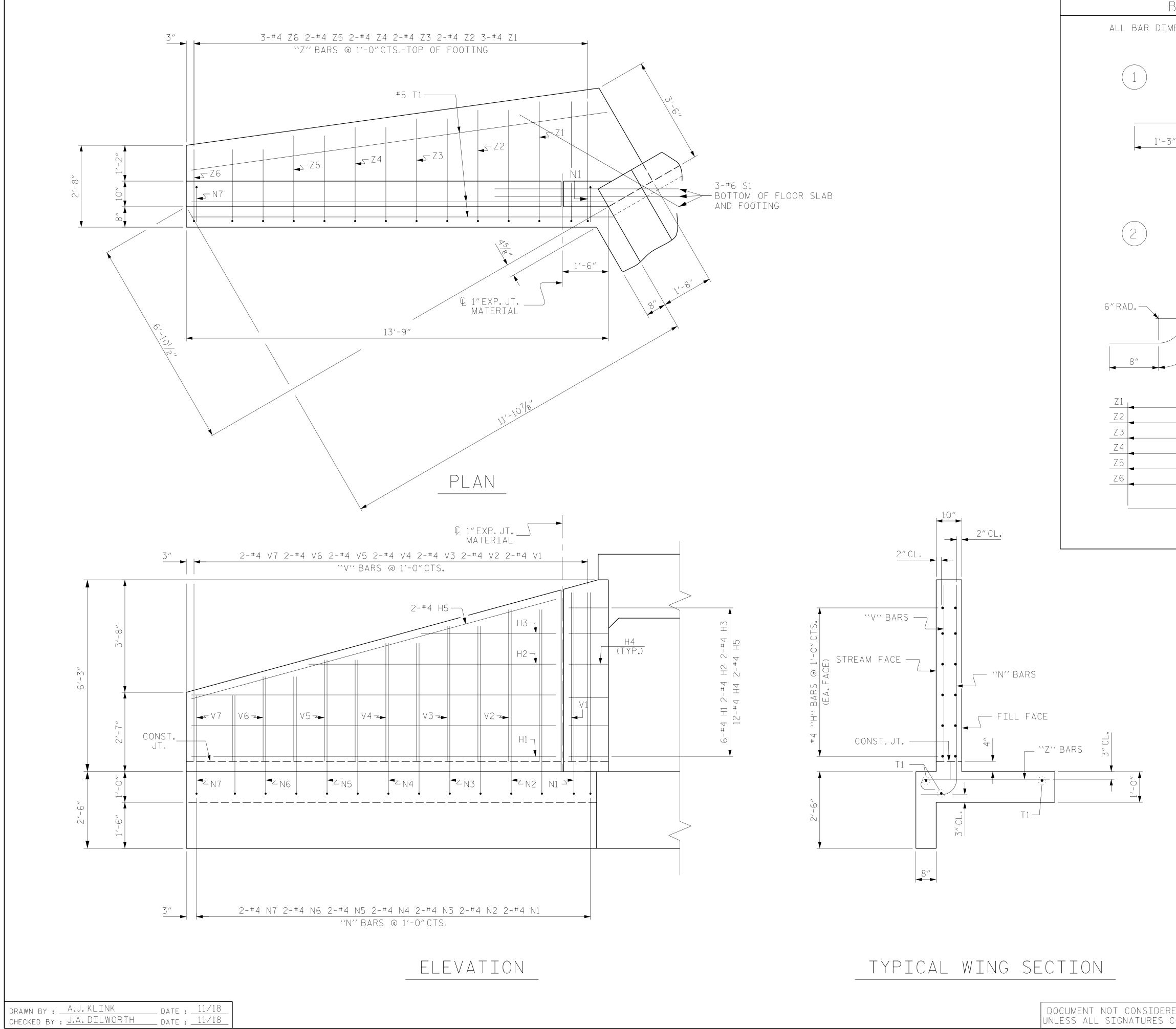
THERE ARE 55 ``C"BARS IN SECTION OF BARREL.

BAR TYPES		BILL OF MATERIAL							
	A1 A2	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
(1)		A1	152	#5 #5	1	7'-2"	1136		
		A2	152	#5	1	6'-10"	1083		
VERTICAL LEG 🔨		A100	76	#5	STR.	15′-8″	1242		
	4'-2 ¹ /2" 3'-10 ¹ /2"								
	4'-2 ¹ /2 3'-10 ¹ /	A200	76	#5	STR.	15′-8″	1242		
6″ R.		A300	76	#5	STR.	15′-8″	1242		
	¥	A400	76	#5	STR.	15′-8″	1242		
		B1	102	#4	STR.	6′-6″	443		
	$\sqrt{2}$	B3	102	#4	STR.	6'-6"	439		
A1 2'-2"	9 12 8.								
A2 2'-2"		C1	110	#4	STR.	26'-0"	1910		
		G1	8	#5	STR.	15′-8″	131		
					0 1 1 1	10 0	101		
ALL BAR DIMENSIONS ARE	OUT TO OUT.					4 7			
ALL BAR DIMENSIONS ARE OUT TO OUT. REINFORCING STEEL 10110 LBS									

SPLI	CE LEI	NGTHS CHART
BAR	SIZE	SPLICE LENGTH
A200	5	2'-2"
A400	5	2'-2"
B1	4	1'-9''
C1	4	1'-11"

		PROJEC 	MART		\backslash		5 DUNTY - L -	
		SHEET 4	OF 5					
	ENGINEER OF RECORD 4/30/2019	SHEET 4 OF 5 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH BARREL REINFORCEMEN DOUBLE 7 FT. X 5 FT CONCRETE BOX CULVER 90° SKEW						
	ENGINEERING		REVIS	SION	S		SHEET NO.	
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BAR TYPES	BILL OF MATERIAL								
DIMENSIONS ARE OUT TO OUT.	BAR NO. SIZE TYPE LENGTH W								
	H1 H2	24 8	#4 #4	STR STR	<u>11'-10''</u> 8'-0''	190 43			
	H3	8	#4	STR	4'-4''	23			
2'-0'	H4	48	#4	1	3'-3"	104			
	H5	8	#4	STR	12'-3"	65			
	N1	8	#4	2	7'-6"	40			
	N2	8	#4	2	6'-11"	37			
1'-3" 1'-8 ³ / ₄ "	N3 N4	8	#4 #4	2	6'-5" 5'-10"	34 31			
	N4 N5	8	#4	2	5'-4"	29			
	N6	8	#4	2	4'-9"	25			
N N N N N N N N N N N N N N N N N N N	N 7	8	#4	2	4'-2"	22			
	S1	12	#6	STR	6'-0"	108			
	— 1	1.0		<u>c</u> td	174 04	170			
	Τ1	12	#5	STR	13'-9"	172			
6'-0\/2" 5'-5\/2" 4'-11\/2" 3'-10\/2" 3'-3\/2" 2'-8\/2"	V1	8	#4	STR	5'-6"	29			
6 - 0 / 5 - 5 / 4 - 4 1 / 3 - 10 / 3 - 3 / 2 - 8 /	V2	8	#4	STR	4'-11"	26			
	V 3 V 4	8	#4 #4	STR STR	<u>4'-4"</u> <u>3'-10"</u>	23			
	V5	8	#4	STR	3'-3"	17			
	V6	8	#4	STR	2'-9"	15			
	V 7	8	#4	STR	2'-1"	11			
312	Z1	12	#4	3	4'-5"	35			
	Z2	8	#4	3	4'-1"	22			
	Z3 Z4	8	#4 #4	3	<u>3'-10"</u> <u>3'-7"</u>	20			
3'-11"	Z5	8	#4	3	3'-3"	17			
3'-7"	Z6	12	#4	3	2'-10"	23			
3'-4"									
<u>3'-1"</u>									
2'-9" 2'-4" 6"			CING S	TEEL	1.0				
		(4 W)	CONCR		12	00 LBS			
		4 W.	INGS		17	7.1 CY			
(3) HK.			EADWAL Curta			.5 CY .8 CY			
			0.011111).4 CY			
	PROJEC N STATIO	MAR		\backslash		UNTY			
ENGINEER OF RECORD 4/30/2019	STATIO	5	STATE OF N			<u> </u>			
$\frac{4/30/2019}{10000000000000000000000000000000000$	DEPAF		NT OF		NSPORTA [®]	TION			

		SHEET 5 (DF 5		
	ENGINEER OF RECORD 4/30/2019 4/30/2019 CARO POESSONA SEAL 22072 HILL SOURCER CARO CARO CARO CARO CARO CARO CARO COMPANY CARO C		state of North Car ARTMENT OF TRA Raleigh WING FOR FOR CRETE BOX '-0" 90° SKE	NSPORTA S CUL Slope	
	ENGINEERING	REVISIONS SHEET NO.			
	1223 Jones Franklin Rd.	NO. BY:	DATE: NO. BY:	DATE:	C5
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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	see plans
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾″∅ STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø studs for 4 - $\frac{3}{4}$ " Ø studs, and stud spacing changes SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø studs ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø studs based on the ratio of 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

