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PROJECT LE	NGTH	STRUCTURE	repared for: MANAGEMENT UNIT
LENGTH ROADWAY PROJEC	T = 0.114 MILES	NORTH CAROLINA D	EPARTMENT OF TRANSPORTATION
LENGTH STRUCTURES PROJE	CT = 0.019 MILES		
TOTAL LENGTH PROJECT =	0.133 MILES		PROJECT ENGINEER
CDOT CONTACT:	DAVID STUTTS, PE PROJECT MANAGER	LETTING DATE: APRIL 19, 2022	PATRICK HOLDER, PE PROJECT DESIGN ENGINEER

DocuSign Envelope ID: E1727B46-06BF-4FEB-98F0-BC9CB6EFEFF2



DN1 SLOPE		
ONT SLOPE 157,51: GRADE DATA +0.30007 +4.77697 APPROX.EXISTING GROUND LINE PI SIA: 14470.00 -1- EXTENDED PI SIA: 14470.00 -1- EXTENDED PI SIA: 14470.00 -1- EXTENDED PROJECT NO. BR-0032 MADISON PROJECT NO. BR-0032 MADISON PROJECT NO. BR-0032 MADISON STATION: 13+50.00 -L- SHET 10*3 SHET 10*3 REPLACES BRIDGE *84 PROJECT NO. BR-0032 MADISON COUNTY STATION: 13+50.00 -L- SHET 10*3 SHET 10*3 REPLACES BRIDGE *84 COUNTY STATION: SHET NOT SUBJOR COUNTY TANGENT SHET NOT SUBJOR COUNTY TANGENT SHET NOT SUBJOR SHET 10*3 SHET NOT SUBJOR COUNTY TANGENT SHET NOT SUBJOR COUNTY TANGENT SHET NOT SUBJOR SHET 10*3 SHET NOT SUBJOR	14+50.00	
ONT SLOPE 157, 21*** GRADE DATA		
ONT SLOPE 167.4.3 GRADE DATA APPROX.EXISTING GROUND LINE HORIZONTAL CURVE DATA MORIZONTAL CURVE DATA PI STA. + 15+01.80 -L- W.C. + 1200 MORIZONTAL CURVE DATA PI STA. + 15+01.80 -L- C. + 22*-50-44.8" (U.T.) C. + 126.00 -L- L + 144.52 - L- L + 144.52 - L- L + 144.52 - L- L + 144.52 - L- L + 144.52 - L- R + 480.00 - R + 155.00 LEARTH BERM I HEREBY CERTIFY THESE PLANS MADISON TANGENT PROJECT NO. <u>BR-0032</u> MADISON _ COUNTY STATION: 13+50.00 -L- SHEET 10* 3 - REPLACES BRIDGE +64 USE PRANSPORTATION BEADOW FORK CREEK BETWEEN SR 1175 - SR 1171 MEET NOT CONSIDERED TANGENT NOT CONSIDERED TANGENT LINESS ALL STATE ON NC 209 OVER MEADOW FORK CREEK BETWEEN SR 1171 - SR 1171		
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APPROX.EXISTING EL. = 1867.73" V.C. = 120" PI STA.= 12-02.09 -L-	<u></u>	+0.3000% +4.7769% PI STA. = 14+70.00 -L-
HORIZONTAL CURVE DATA PISTA - 12402.09 -L- G = 26°-50′-14.9″ (LT) D = 114.52″ T = 112.68″ R = 480.00 R = 155.00 LEARTH BERM LEARTH BERM	APPROX.EXISTING GROUND LINE	EL. = 1867.78' V.C. = 120'
HORIZONTAL CURVE DATA PI STA. = 12+02.09 - L-		
HORIZONTAL CURVE DATA PI STA. = 12+02.09 - L- A = 26°-50'-4.8" L. D = 11°-56'-1.6" L. D = 11°-56'-1.6" L. D = 194.67" J. D = 100.60" J.		
PI STA = 12+02.09 -L- 2 = 26°-50'-48'' UT.) D = 36°-57'-54.1'' L = 224.37'' L = 24.452'' T = 114.52'' T = 114.52'' T = 112.68'' R = 480.00 R = 155.00 L EARTH BERM L EARTH BERM L EARTH BERM L EARTH BERM	`	HORIZONTAL CURVE DATA
EARTH BERM	$\begin{array}{ccc} PI S^{-1} & PI S^{-1} \\ C & C & D \\ C$	TA. = $12+02.09 - L -$ PI STA. = $15+01.80 - L 26^{\circ}-50'-14.8''$ (LT.) $\triangle = 72^{\circ}-02'-02.8''$ (LT.) $11^{\circ}-56'-11.8''$ $D = 36^{\circ}-57'-54.1''$ $224.83'$ $L = 104.83'$
LEARTH BERM TI HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS TO SR 1171 EXTENDED TANGENT PE TT.) PROJECT NO. <u>BR-0032</u> MADISON COUNTY STATION: <u>13+50.00 -L-</u> SHEET 1 OF 3 REPLACES BRIDGE *84 STATE OF MOTING CANALINA DEPARTMENT OF TRANSPORTATION SHEET 1 OF 3 REPLACES BRIDGE *84 STATE OF MOTING CANALINA DEPARTMENT OF TRANSPORTATION SALETON DOCUMENT NOT CONSIDERED DOCUMENT NOT CONSIDERED DOCUMENT NOT CONSIDERED STATE OF MOTING CANAL SIGNATURES COMPLETED NOT A DATE: NO. <u>S-1</u> <u>SHEET 1 OF 3 REPLACES BRIDGE *84</u> STATE OF MOTING CANALINA SR 1175 & SR 1171 SHEET NO. <u>S-1</u> <u>SHEET NO. S-1</u> <u>SHEET NO. S-1</u>	$\frac{1}{1} \frac{1}{1} \frac{1}$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
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PROJECT NO. <u>BR-0032</u> <u>MADISON</u> <u>COUNTY</u> <u>STATION: 13+50.00 -L-</u> <u>SHEET 1 OF 3</u> <u>REPLACES BRIDGE *84</u> <u>STATE OF MORTH CAROLINA</u> <u>DEPARTMENT OF TRANSPORTATION</u> <u>RALEIGH</u> <u>BENERAL DRAWING</u> FOR BRIDGE ON NC 209 OVER <u>MEADOW FORK CREEK BETWEEN</u> <u>SR 1175 & SR 1171</u> <u>SHEET NO.</u> <u>SHEET NO.</u>	.) TO SR IIII	
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PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: <u>13+50.00 -L-</u> SHEET 1 OF 3 REPLACES BRIDGE *84 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING FOR BRIDGE ON NC 209 OVER MEADOW FORK CREEK BETWEEN SR 1175 & SR 1171 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED)PE RT.)	
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALLNO.BY:DATE:NO.BY:DATE:SIGNATURESS	A089F83FD9D/4F8 2/22/2022	REVISIONS SHEET NO.
	DOCUMENT NOT CONSIDER FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: S-1 1 3 3 TOTAL SHEETS 2 4 34 34



DRAWN BY :	D. A. GLADDEN	_ DATE :	09/21
CHECKED BY :	E.E. MURRAY	DATE :	12/21
DESIGN ENGINEER	OF RECORD: E.E. MURRAY	_ DATE :	1/22

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	REVISIONS SHEET NO.								
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S-2				
SIGNATURES COMPLETED	2		4		34				

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bopt/					Driven Piles			Predrilling for Piles*			Drilled-In Piles		
Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Lenth per Pile FT	Scour Critical Elevation FT	Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile Exc In Soil per Pile Lin FT
End Bent No.1 Piles 1-5	54	See Structure	20	N/A		90					1838.0	5.0	14.2
End Bent No.1 Piles 6-8	54	Plans	20	N/A		90]				1840.0	5.7	11.1
							Į						
							ļ						

*Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

SUMMARY OF DRILLED PIER INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pier(s) #-# (e.g., "Bent 1, Piers 1-3")	Factored Resistance per Pier TONS	Minimum Pier Tip (Tip No Higher Than) Elevation FT	Required Tip Resistance per Pier TSF	Scour Critical Elevation FT	Minimum Drilled Pier Penetration Into Rock per Pier Lin FT	Drilled Pier Length per Pier Lin FT	Drilled Pier Length Not In Soil per Pier Lin FT	Drilled Pier Length In Soil per Pier Lin FT	Permanent Steel Casing Required? YES or MAYBE	Permanent Steel Casing Tip Elevation (Elev Not To Extend Casing Below) FT	Permanent Steel Casing Length* per Pier Lin FT
Bent No. 1 Piers 1-2	395	1823.5	15	1838	17.0	27.7	17.0	10.7	MAYBE	1840.5	11.0
Bent No. 1 Piers 3	395	1831.0	15	1838	10.0	20.2	10.0	10.2	MAYBE	1842.2	9.0

*Permanent Steel Casing Length equals the difference between the ground line or top of drilled pier elevation, whichever is higher, and the permanent casing tip elevation.

FOUNDATION NOTES

1 FOR PILES, SEE PILES PROVISION AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

2 FILL HOLES FOR PILE EXCAVATION AT END BENT NO. 1 WITH CONCRETE.

3 FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

- 4 CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 5 KEY IN SPREAD FOOTINGS AT END BENT NO. 2 AT LEAST 6" INTO ROCK/12" INTO WEATHERED ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE PLANS.
- 6 FOR BLASTING, SEE SECTION 220 OF THE STANDARD SPECIFICATIONS.
- 7 BLAST ALL PHASES OF THE PROJECT AT THE SAME TIME. PATCH/REPAIR ROADWAY AS DIRECTED BY THE ENGINEER.
- 8 CONTRACTOR IS TO PROVIDE A BLASTING CONSULTANT TO REVIEW SITE CONDITIONS, SCOPE OF WORK AND PROVIDE A BLASTING DESIGN/PLAN
- THE COST FO R THIS WORK WILL BE CONSIDERED INCIDENTAL TO THE BLASTING WITH NO ADDITIONAL COST TO THE DEPARTMENT FOR DESIGN OR IMPLEMENTATION

NOTES:

9 DEPENDING ON THE APPROVED BLASTING PLAN, ADDITIONAL TRAFFIC CONTROL MEASURES INCLUDING TEMPORARY CLOSURES, DETOURS, OR OTHER MEASURES MAY BE REQUIRED AND WILL BE THE RESPONSIBILITY OF THE CONTRACTOR.

1. The Pile and Drilled Pier Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Shiping Yang, PE #031661) on 11-05-2021. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
 The Engineer will determine the need for PDA Testing, Pipe Pile Plates, Permanent Steel Casing, SPTs, CSL Testing, SID Inspections and PITs when these items may be required.

End Bent/ Bent No, Pier(s) #-# (e.g., "Bent 1, Piers 1-3")	Factored Bearing Resistance TSF	Footing Dimensions (Length x Width) FT x FT	Required Bearing Resistance TSF	Scour Critical Elevation FT	Minimum Bottom of Footing (Footing No Higher Than) Elevation FT
End Bent No. 2	15	47.6X 7.0	25	1855.9	1855.9



*CSL Tubes are required if CSL Testing is or may be required. The number of CSL Tubes per drilled pier is equal to one tube per foot of design pier diameter with at least 4 tubes per pier. The length of each CSL Tube is equal to the drilled pier length plus 1.5 ft.

SEAL

2/22/2022

025516

SUMMARY OF SPREAD FOOTING INFORMATION

(Blank entries indicate item is not applicable to structure)

SUMMARY OF DRILLED PIER TESTING

(Blank entries indicate item is not applicable to structure)

Standard enetration Test (SPT) equired? YES or MAYBE	Crosshole Sonic Logging (CSL) Required?* YES or MAYBE	Total CSL Tube Length (For All Tubes) per Pier Lin FT	Shaft Inspection Device (SID) Required? YES or MAYBE	Pile Integrity Test (PIT) Required? MAYBE
No	Maybe	233	Maybe	No
No	Maybe	87	Maybe	No
	1	320	2	
	1			

PROJECT NO. BR-0032

MADISON

_COUNTY

STATION:

13+50.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

PILE, DRILLED PIER AND SPREAD FOOTING FOUNDATION TABLES

SIGNATURE DATE			SHEET NO. S-3				
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
				4			34



							TOTAL	BILL OF I	MATERIA	AL.								
	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	FOUNDATION EXCAVATION FOR END BENT 2	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	3'-O"DIA DRILLED PIERS IN SOIL	3'-O"DIA DRILLED PIERS NOT IN SOIL	PERMANAENT STEEL CASING FOR 3'-O"DIA DRILLED PIERS	CSL TESTING	SID INSPECTIONS	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PILE DRIVING EQUIPMENT SET UP FOR HP 12 X 53 STEEL PILES	HP STEI	12 X 53 EL PILES
	LUMP SUM	LUMP SUM	LUMP SUM	LN.FT.	LN.FT.	LN.FT.	LN.FT.	LN.FT.	EA.	EA.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	LBS.	EA.	NO.	LN.FT.
SUPERSTRUCTURE																		
END BENT NO.1				104.3	42.1							20.9		3027		8	8	160
BENT NO.1						31.6	44.0	31.0				23.1		11899	1876			
END BENT NO.2			LUMP SUM									76.3		11985				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	104.3	42.1	31.6	44.0	31.0	1	2	LUMP SUM	120.3	LUMP SUM	26911	1876	8	8	160

TOTAL BILL OF MATERIAL

	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARING	3'- PRE C(COR	O″X 1'-9″ STRESSED DNCRETE ED SLABS
	LN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LN.FT.
SUPERSTRUCTURE	200.58				12	360
END BENT NO.1		23	26			
BENT NO.1						
END BENT NO.2		157	175			
TOTAL	200.58	180	201	LUMP SUM	12	360

DRAWN BY :	D. A. GLADDEN	DATE : 09/21	
CHECKED BY :	E.E. MURRAY	DATE : 12/21	
DESIGN ENGIN	NEER OF RECORD:E.E.MURRAY	DATE :1/22	

+

PLANS AND SPECIAL PROVISIONS.

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WI THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHE

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISI

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISION

FOR TEMPORARY ANCHORED PORTABLE CONCRETE MEDIA SEE TRAFFIC MANAGEMENT PLANS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AT END SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT.LEF AND 50 FT.RIGHT SIDE OF CENTERLINE ROADWAY AS BY THE ENGINEER. THE MATERIAL SHOWN IN THE CRO AREA AT END BENT 2 SHALL BE EXCAVATED FOR A DI 23 FT.LEFT SIDE AND 30 FT.RIGHT SIDE OF CENTERL ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF A SINGLE STEEL PLANK DECK ON I-BEAMS SUPPORTED ON MASON AND A CLEAR ROADWAY WIDTH 19'-5' LOCATED AT THE STRUCTURE SITE SHALL BE REMOVED. THE EXISTING B PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE INTEGRITY OF THE BRIDGE DETERIORATE DURING CON OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POS BE REDUCED AS FOUND NECESSARY DURING THE LIFE PROJECT.



840

12

HYDRAULIC DATA

DESIGN DISCHARGE
FREQUENCY OF DESIGN FLOOD
DESIGN HIGH WATER ELEVATION
DRAINAGE AREA
BASE DISCHARGE (Q100)
BASE HIGH WATER ELEVATION

- = 4500 C.F.S.
- = 50 YEARS
- = 1858.5 FT.
- = 22.8 SQ.MI.
- = 5400 C.F.S.
- = 1859.9 FT.

OV<u>ERTOPPING FLOOD DATA</u>

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD = 500+ YEARS OVERTOPPING FLOOD ELEVATION OVERTOPPING FLOOD STATION

- = 12100 C.F.S.
- = 1867.7 FT. = 12+92.30 -L-
- 5430 Wade Park Boulevard, Suite 410 Raleigh, NC 27607 Tel. 919-854-0344 Fax. 919-854-0355 NC License No. F-0765

KERT

TH IEET SN.	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND
PROVISIONS.	THE ACTUAL CONDITIONS AT THE PROJECT SITE. REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER.
	THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
NS. An barrier,	THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ``HEC 18-EVALUATING SCOUR AT BRIDGES.''
	FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
) BENT 1 T SIDE	FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.
DIRECTED SS-HATCHED STANCE OF LINE WILL R 412	INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+50.00 -L"
41'-O" SPAN IRY ABUTMENTS, PROPOSED	FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.
RIDGE IS STRUCTURAL STRUCTION STED AND OF THE	ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

	PROJECT NO	<u>BR-0</u>	032
	MADIS	<u>NC</u>	COUNTY
	STATION: 13	+50.00	-L-
	SHEET 3 OF 3		
CAROLANA SESSIO: A	STATE OF DEPARTMENT O	T NORTH CAROLINA	RTATION
SEAL 025516	GENERAI	_ DRAW	ING

FOR BRIDGE ON NC 209 OVER MEADOW FORK CREEK BETWEEN SR 1175 & SR 1171

			REV	ISION	S		SHEET NO.
DOCUMENT NOT CONSTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			34

CINEE

689F83FD9D74F8 3/3/2022

Emily E.

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	LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																							
										STRE	ENGTH	ILIN	NIT ST	ΓΑΤΕ				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	COMMENT NUMBER
		HL-93(Inv)	N⁄A	1	1.202		1.75	0.256	2.04	30'	EL	14.423	0.655	1.2	30′	EL	1.442	0.80	0.256	1.75	30′	EL	14.423	
DESIGN		HL-93(0pr)	N⁄A		1.558		1.35	0.256	2.64	30′	EL	14.423	0.655	1.56	30′	EL	1.442	NZA						
		HS-20(Inv)	36.000	2	1.365	49.124	1.75	0.256	2.82	30′	EL	11.538	0.655	1.36	30′	EL	1.442	0.80	0.256	2.45	30′	EL	11.538	
RATING		HS-20(0pr)	36.000		1.769	63 . 679	1.35	0.256	3.65	30′	EL	11.538	0.655	1.77	30′	EL	1.442	NZA						
		SNSH	13.500		3.333	45.002	1.4	0.256	5.76	30′	EL	14.423	0.655	3.33	30′	EL	1.442	0.80	0.256	3.95	30′	EL	14.423	
		SNGARBS2	20.000		2.581	51.624	1.4	0.256	5.04	30′	EL	11.538	0.655	2.58	30′	EL	1.442	0.80	0.256	3.50	30′	EL	11.538	
		SNAGRIS2	22.000		2.487	54.723	1.4	0.256	5.13	30′	EL	11.538	0.655	2.49	30′	EL	1.442	0.80	0.256	3.56	30′	EL	11.538	
		SNCOTTS3	27.250		1.684	45.891	1.4	0.256	2.89	30′	EL	14.423	0.655	1.68	30′	EL	1.442	0.80	0.256	1.99	30′	EL	14.423	
	S S	SNAGGRS4	34.925		1.551	54.185	1.4	0.256	2.79	30′	EL	14.423	0.655	1.55	30′	EL	1.442	0.80	0.256	1.91	30′	EL	14.423	
		SNS5A	35.550		1.645	58.469	1.4	0.256	2.7	30′	EL	14.423	0.655	1.64	30′	EL	1.442	0.80	0.256	1.85	30′	EL	14.423	
		SNS6A	39.950		1.547	61.791	1.4	0.256	2.55	30′	EL	14.423	0.655	1.55	30′	EL	1.442	0.80	0.256	1.75	30′	EL	14.423	
I FGAI		SNS7B	42.000		1.578	66.285	1.4	0.256	2.48	30′	EL	14.423	0.655	1.58	30'	EL	1.442	0.80	0.256	1.70	30′	EL	14.423	
LOAD		TNAGRIT3	33.000		1.838	60.67	1.4	0.256	3.31	30′	EL	14.423	0.655	1.84	30′	EL	1.442	0.80	0.256	2.27	30′	EL	14.423	
RATING		TNT4A	33.075		1.71	56.559	1.4	0.256	3.13	30′	EL	14.423	0.655	1.71	30′	EL	1.442	0.80	0.256	2.15	30′	EL	14.423	
		TNT6A	41.600		1.652	68.714	1.4	0.256	2.85	30′	EL	14.423	0.655	1.65	30′	EL	1.442	0.80	0.256	1.96	30′	EL	14.423	
	ST	TNT7A	42.000		1.573	66.067	1.4	0.256	2.94	30′	EL	14.423	0.655	1.57	30'	EL	1.442	0.80	0.256	2.02	30′	EL	14.423	
	= [TNT7B	42.000		1.536	64.525	1.4	0.256	2.77	30′	EL	14.423	0.655	1.54	30'	EL	1.442	0.80	0.256	1.90	30′	EL	14.423	
	[TNAGRIT4	43.000		1.486	63.9	1.4	0.256	2.87	30'	EL	14.423	0.655	1.49	30'	EL	1.442	0.80	0.256	1.97	30′	EL	14.423	
	[TNAGT5A	45.000		1.594	71.736	1.4	0.256	2.79	30'	EL	14.423	0.655	1.59	30'	EL	1.442	0.80	0.256	1.92	30'	EL	14.423	
		TNAGT5B	45.000	3	1.399	62.946	1.4	0.256	2.68	30'	EL	11.538	0.655	1.4	30′	EL	1.442	0.80	0.256	1.85	30'	EL	11.538	



LRFR SUMMARY

FOR SPAN `A'

ASSEMBLED BY : [CHECKED BY : E.). A. GLADDEN E. MURRAY	DATE : DATE :	6/21 11/21
DRAWN BY : CVC CHECKED BY : DNS	6/10 6/10		

16-FEB-2022 09:58 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ brent.barnhill AT C-1000010

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD	STRENGTH I	1 . 25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1.
- 2. 3.
- 4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
(3) LEGAL LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER



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		LOAD AN	ID RE	SIST	ANCE	E FA(CTOR	RAT	ING	(LRF	ED) S	UMMA	ry f	FOR F	PRES	TRES	SSED	CON	CRET	E GI	RDEF	RS					
										STRE	ENGTH	I LIN	IT S	TATE				SE	ERVICE	III	LIMI	T STA	TE				
										MOMENT					SHEAR						MOMENT			-			
LEVEL		VEHICLE	VEHICLE		HL-93(Inv)	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	L I VELOAD F ACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	COMMENT NUMBER
		HL-93(Inv)	N⁄A	1	1.06		1.75	0.248	1.14	70′	EL	34.423	0.655	1.06	70'	EL	6.885	0.80	0.248	1.11	70′	EL	34.423				
DESIGN		HL-93(0pr)	N/A		1.374		1.35	0.248	1.48	70′	EL	34.423	0.655	1.37	70′	EL	6.885	N/A									
LOAD RATING		HS-20(Inv)	36.000	2	1.32	47.508	1.75	0.248	1.48	70′	EL	34.423	0.655	1.32	70′	EL	6.885	0.80	0.248	1.44	70′	EL	34.423				
		HS-20(0pr)	36.000		1.711	61.585	1.35	0.248	1.91	70′	EL	34.423	0.655	1.71	70′	EL	6.885	N/A									
		SNSH	13.500		3.204	43.258	1.4	0.248	4.12	70′	EL	34.423	0.655	3.9	70′	EL	6.885	0.80	0.248	3.20	70′	EL	34.423				
		SNGARBS2	20.000		2.403	48.063	1.4	0.248	3.09	70'	EL	34.423	0.655	2.78	70′	EL	6.885	0.80	0.248	2.40	70′	EL	34.423				
		SNAGRIS2	22.000		2.282	50 . 21	1.4	0.248	2.94	70'	EL	34.423	0.655	2.58	70′	EL	6.885	0.80	0.248	2.28	70′	EL	34.423				
		SNCOTTS3	27.250		1.595	43.463	1.4	0.248	2.05	70'	EL	34.423	0.655	1.95	70′	EL	6.885	0.80	0.248	1.59	70′	EL	34.423				
	S S	SNAGGRS4	34.925		1.339	46.755	1.4	0.248	1.72	70'	EL	34.423	0.655	1.62	70′	EL	6.885	0.80	0.248	1.34	70′	EL	34.423				
		SNS5A	35.550		1.309	46.526	1.4	0.248	1.68	70'	EL	34.423	0.655	1.65	70′	EL	6.885	0.80	0.248	1.31	70′	EL	34.423				
		SNS6A	39.950		1.203	48.069	1.4	0.248	1.55	70'	EL	34.423	0.655	1.5	70′	EL	6.885	0.80	0.248	1.20	70′	EL	34.423				
I FGAI		SNS7B	42.000		1.146	48.129	1.4	0.248	1.47	70'	EL	34.423	0.655	1.48	70'	EL	6.885	0.80	0.248	1.15	70′	EL	34.423				
LOAD		TNAGRIT3	33.000		1.468	48.444	1.4	0.248	1.89	70′	EL	34.423	0.655	1.79	70'	EL	6.885	0.80	0.248	1.47	70′	EL	34.423				
RAIING		TNT4A	33.075		1.475	48.79	1.4	0.248	1.9	70′	EL	34.423	0.655	1.74	70'	EL	6.885	0.80	0.248	1.48	70′	EL	34.423				
		TNT6A	41.600		1.208	50.272	1.4	0.248	1.55	70′	EL	34.423	0.655	1.58	70′	EL	6.885	0.80	0.248	1.21	70′	EL	34.423				
	ST	TNT7A	42.000		1.216	51.061	1.4	0.248	1.56	70′	EL	34.423	0.655	1.55	70′	EL	6.885	0.80	0.248	1.22	70′	EL	34.423				
		TNT7B	42.000		1.261	52.955	1.4	0.248	1.62	70′	EL	34.423	0.655	1.44	70'	EL	6.885	0.80	0.248	1.26	70′	EL	34.423				
		TNAGRIT4	43.000		1.197	51.476	1.4	0.248	1.54	70′	EL	34.423	0.655	1.4	70'	EL	6.885	0.80	0.248	1.20	70′	EL	34.423				
		TNAGT5A	45.000		1.128	50.745	1.4	0.248	1.45	70′	EL	34.423	0.655	1.39	70'	EL	6.885	0.80	0.248	1.13	70′	EL	34.423				
		TNAGT5B	45.000	3	1.113	50.088	1.4	0.248	1.43	70′	EL	34.423	0.655	1.33	70′	EL	6.885	0.80	0.248	1.11	70′	EL	34.423	1			



LRFR SUMMARY

FOR SPAN 'B'

ASSEMBLED BY : [CHECKED BY : E.). A. GLADDEN E. MURRAY	N DATE : DATE :	6/21 11/21
DRAWN BY : CVC CHECKED BY : DNS	6710 6710		

16-FEB-2022 09:59 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ brent.barnhill AT C-1000010

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. 2.
- 3.
- 4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
$\sqrt{3}$ LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER



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	PROJEC	CT NO. MADIS ON: 1 F 12	<u>BF</u> SON 3+50	<u>-003</u> co .00 -	2 UNTY L-
Bould and B. Murray Adores 1907478 2/22/2022	DEPA	SUPER CONS	e of NORTH CAR OF TRAI RALEIGH RSTRUC TRUC	NSPORTA TURE TION CE	TION
		REVIS	SIONS		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S- 7
FINAL UNLESS ALL SIGNATURES COMPLETED	า 2		জ ধ্ব		SHEETS 34



DRAWN BY :	D. A. G	LADDEN	DATE :	4/21
CHECKED BY :	D . R.	.SMITH	DATE :	11/21
DESTGN ENGINEER	OF RECORD:	P. N. HOLDER	DATE :	5/21





STD. NO. 21" PCS_36_120S_30L



PLAN OF
#4 S11 BARS MAY BE S
1" CLEAR TO GROUTED
POST-TENSIONING STR

CHECKED BY :D.R.SMITHDATE :11/21DESIGN ENGINEER OF RECORD:P.N.HOLDERDATE :5/21	DRAWN BY :	D. A. G	LADDEN	DATE : _	4/21
DESIGN ENGINEER OF RECORD:	CHECKED BY :	D. R.	SMITH	DATE : _	11/21
	DESIGN ENGINEER	OF RECORD: _	P. N. HOLDER	DATE : .	5/21







PLAN OF TYPE II INTERIOR CORED SLAB UNIT



ANCHURS FUR PURIAB	L
TO ROADWAY STANDAF	? 1
ANCHORS. IF THE POR	1
STANDARD'S DETAILS	
PROPOSED BARRIERS	4

DRAWN BY :	D. A. GL	ADDEN	DATE :	4/21
CHECKED BY :	D. R.	SMITH	DATE :	11/21
DESIGN ENGINEER	OF RECORD:	D.R.SMITH	DATE :	11/21

PLAN OF TYPE I EXTERIOR CORED SLAB UNIT

	PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: 13+50.00 -L-
BOCKSTONE DE MARTINA BOCKSTONE	BATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH PLAN OF 30'-0'' PRESTRESSED CONCRETE CORED SLAB UNIT TYPE I, TYPE II, AND TYPE III (SPAN A)
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-12 1 3 3 5 5 5 5 2 4 34 34
•	STD. NO. 21" PCS_36_120S_30L





DRAWN BY :	D.A.GLA	ADDEN	DATE :	4/21
CHECKED BY :	D. R. S	MITH	DATE : _	11/21
DESIGN ENGINEER	OF RECORD:	D.R.SMITH	DATE : _	11/21





	PROJEC MA STATIC	T NO ADISON N:13+50	<u>BR-00</u> co 0.00 -l	<u>32</u> UNTY
SEAL O25516 BOCKEER HOMELY CONECTION CONTRACTO	DEPAF DEPAF PRES CC TYP	STATE OF NORTH RTMENT OF TI RALEIC PLAN OF TRESSED ORED SLA E IV AN (SPAN	A CAROLINA RANSPORTA 30'-0' CONCI ABUNI ABUNI IDTYP	TION , RETE T E V
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: 1 2	REVISIONS DATE: NO. BY: 3 4 4	: DATE:	SHEET NO. S-13 TOTAL SHEETS 34
	STD.	NO. 21" PCS	_36_120S	_30L

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ANCHORS FOR PORTABLE CONCRETE BARRIER HAVE BEEN SPACED ACCORDING TO ROADWAY STANDARD 1170.01. THE CONTRACTOR SHALL CONFIRM THE USE OF BARRIERS MATCHING THIS STANDARD AND THE LOCATIONS OF THE ANCHORS. IF THE PORTABLE CONCRETE BARRIER DOES NOT MEET THE STANDARD'S DETAILS AND THE SPACING OF THE ANCHORS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING ANCHORS PROPERLY SPACED FOR THE PROPOSED BARRIERS AS APPROVED BY THE ENGINEER.

DRAWN BY :	D. A. GL	ADDEN	DATE :	4/21
CHECKED BY :	D. R.	SMITH	DATE : _	11/21
DESIGN ENGINEER	OF RECORD:	D.R.SMITH	DATE : .	11/21

PLAN OF TYPE VI EXTERIOR CORED SLAB UNIT

	23'-4" (TYP. EA. FACE)	
→	21'-8 ³ /8" (TYP.EA.FACE)	
]
<u>4"</u> <u>1'-3"</u> <u>17'-1</u>	2 ¹ /2″(TYP.EA.VOID)	3'-0
		23'-4" (TYP. EA. FACE) 21'-8 ³ / ₈ " (TYP. EA. FACE) 1'-3" 4" 4" 1'-3" 1'-3" 1'-3" 1'-3" 1''-2 ¹ / ₂ " (TYP. EA. VOID)

PLAN OF TYPE VII INTERIOR CORED SLAB UNIT

70'-0''			
23'-4" (TYP. EA. FACE)		23'-4" (TYP. EA. FACE)	
21'-8 <mark>%</mark> "(TYP.EA.FACE)	► _ ◄	21'-8 ³ / ₈ " (TYP. EA. FACE)	
4" -0			-
+ + + +			
<u>3"</u> 19'-7 <mark>%</mark> " (TYP.EA. VOID)		17'-2 ¹ /2" (TYP.EA.VOID)	3'-C
5'-1" 5'-0" 5'-1"	5'-0" 5'-1" 5	5'-0" 5'-1"	5'-0"

7-ANCHORED PORTABLE CONCRETE BARRIERS (14-STRUCTURAL CONCRETE INSERTS REQ'D.) SEE SHEET 11 OF 13 FOR SECTION OF CONCRETE INSERT LOCATION.

PLAN OF TYPE VIII INTERIOR CORED SLAB UNIT





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DRAWN BY : D.A.GLADD	<u>EN</u> DATE :	4/21	
CHECKED BY : D.R.SMI	<u>TH</u> DATE :	11/21	
DESIGN ENGINEER OF RECORD:	D.R.SMITH DATE : _	11/21	
	16-FEB- \$\$\$\$\$ brent.t	9-2022 10:28 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$.barnhill AT C-1000010	\$

PLAN OF TYPE IX INTERIOR CORED SLAB UNIT

PLAN OF TYPE X EXTERIOR CORED SLAB UNIT



	PROJEC	CT NO.	BF	2-003	2	
	MADISON COUNTY					
	STATION: 13+50.00 -L-					
	SHEET 8 C)F 13				
SEAL 025516 Brily E. Murray A689F83FD9D74F8	DEPA PRES C TYF	RTMENT PLAN STRES ORED PE IX	E OF NORTH CAR OF TRAI RALEIGH OF 7 SED SED SLAE AND	NSPORTA NSPORTA CONCR UNI TYPE R)	TION RETE T E X	
2/22/2022	REVISIONS SHEET NO.					
OCUMENT NOT CONSTDERED	NO. BY:	DATE:	NO. BY:	DATE:	S-15	
FINAL UNLESS ALL SIGNATURES COMPLETED	1		3 4		TOTAL SHEETS 34	

DESIGN ENGINEER OF RECORD: ______D.R.SMITH _____DATE : _____11/21







SECTION I-I FILL RECESSES WITH GROUT







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DRAWN BY :	D. A. G	LADDEN	DATE :	4/21
CHECKED BY :	D. R.	SMITH	DATE :	11/21
DESIGN ENGINEE	R OF RECORD: _	D.R.SMITH	DATE :	11/21
DESIGN ENGINEE			DAIL : .	



TYPE VIII INTERIOR CORED SLAB UNIT

GROUTED RECESS AT END OF POST TENSIONED STRAND CORED SLABS (SPAN B)



TYPE X EXTERIOR CORED SLAB UNIT

	PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: 13+50.00 -L-					
	SHEET 10 OF 13					
WWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWW	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SEAL 025516 WCNEER Emily E. Murray A689F83FD9D74F8 2/22/2022	3'-0' PRESTRESS CORED (SF	' X 2'-O'' SED CONCF SLAB UNI PAN B)	RETE T			
	REVISIO	NS	SHEET NO.			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. 1 3 3 2 4 4	BY: DATE:	TOTAL SHEETS 34			

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SECTION OF CONCRETE INSERT LOCATION (SPAN A)

(TYPE III CORED SLAB UNIT)



SECTION OF CONCRETE INSERT LOCATION (SPAN B)

(TYPE VIII CORED SLAB UNIT)

DRAWN BY :	D. A. GLADDEN			4/21
CHECKED BY :	D . R.	.SMITH	DATE :	11/21
DESIGN ENGINEER	OF RECORD:	D.R.SMITH	DATE :	11/21

101/2

NOTES

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- Β. ENGINEER.
- WITH THE STANDARD SPECIFICATIONS.

THE COST OF THE STRUCTURAL CONCRETE INSERT ASSEMBLY, COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-O"X 1'-9" AND 3'-O"X 2'-O" PRESTRESSED CONCRETE CORED SLABS.

TO FACILITATE PLACEMENT OF STRUCTURAL CONCRETE INSERT ASSEMBLIES, #3 BARS MAY BE TIED TO THE #4 "B' BARS IN THE CORED SLAB UNIT. THE COST OF THE #3 BARS SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-O" X 1'-9" AND 3'-O" X 2'-O" PRESTRESSED SLABS.

STIRRUPS IN THE CORED SLAB UNITS ME BE SHIFTED SLIGHTY AS NECESSARY TO CLEAR STRUCTURAL CONCRETE INSERT ASSSEMBLIES.

FERRULES SHALL BE PLUGGED DURING CASTING OF THE CORED SLAB UNIT AS RECOMMENDED BY THE MANUFACTURER.

SEE TRAFFIC CONTROL PLANS FOR PAY LIMITS OF THE ANCHORED PORTABLE CONCRETE BARRIER. AFTER REMOVAL OF TEMPORARY BARRIER RAIL, THE STRUCTURAL CONCRETE INSERTS SHALL BE FILLED WITH GROUT.



A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{5}{8}$ ".

1 - $\frac{7}{8}$ " Ø X 8 $\frac{1}{2}$ "BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325.BOLT AND WASHER SHALL BE GALVANIZED.AT THE CONTRACTORS OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø X 8 $\frac{1}{2}$ " GALVANIZED BOLT AND WASHER.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A325.THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI.

D. STRUCTURAL CONCRETE INSERT ASSEMBLIES SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE



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NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1\!/}_{2}{}'' \varnothing$ dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

RELEASE		STRENGTH
		PSI
		4000

ADE 270 S	TRANDS	PROJECT NO. BR-0032		
	0.6″ØL.R.	MADISON COUNTY		
RE INCHES)	0.217	5747001 - 13 + 50.00 - 1 - 10000 - 10000 - 10000 - 10000 - 10000 - 100000 - 100000 - 100000 - 100000000		
ATE STRENGTH PER STRAND)	58,600	STATION: 15 50:00 L		
D PRESTRESS FR STRAND)	43,950	SHEET 12 OF 13		
Emily A689F83F 2,	CAROL ESSION SEAL SEAL CONEER CONEER CONEER CONEER CONEER CONEER CONEER CONEER CONEER CONEER CONEER CONER CO	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 120° SKEW (SPAN A)		
		REVISIONS SHEET NO.		
DOCUMENT N FINAL U SIGNATURE	OT CONSIDERE JNLESS ALL S COMPLETED	NO. BT: DATE: NO. BY: DATE: STIS 1 3		
STD. NO. 21" PCS3_36_120S				

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NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1\!/}_{2}{}'' \varnothing$ dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

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THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

ELEASE S	STRENGTH	
	PSI	
	5500	
		PROJECT NO. DR-UUJZ
ADE 270	STRANDS	MADISON COUNTY
	0.6″ØL.R.	CTATION 13+50 00 -1 -
RE INCHES)	0.217	STATION: 13, 30.00 L
ATE STRENGT PER STRAND	H 58,600	SHEET 13 OF 13
ED PRESTRESS PER STRAND	⁵ 43,950	STATE OF NORTH CAROLINA
Emily A689FE	SEAL O25516 CONECTION CONTROL CONTROL CONTROL CONTROL CONTROL CONTROL CONTRO	STANDARD 3'-O"X 2'-O" PRESTRESSED CONCRETE CORED SLAB UNIT 120° SKEW (SPAN B)
		REVISIONS SHEET N
	NOT CONSIDERE	DATE: NO. BY: DATE: S-20
FINAL SIGNATU	RES COMPLETED	2 4 34 34
		STD. NO. 24PCS3_36_60&120S

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MAA/THC



THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION. THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 76" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.



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NOTES

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

TOP ELEV	OF PILE /ATIONS
	1862.95
2	1863.18
3	1863.41
4	1863.64
5	1863.90
6	1864.16
	1864.42

PRO STA 	JECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY TION: <u>13+50.00</u> -L-
TH CAROLINA	STATE OF NORTH CAROLINA EPARTMENT OF TRANSPORTATION RALEIGH
SEAL 025516 WCNEER WWWYAY A689F83ED9D74F8 2/22/2022	SUBSTRUCTURE END BENT #1
	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2	Y: DATE: NO. BY: DATE: S-22 3 3 TOTAL SHEETS 4 34

STD. NO. EB_36_120S



AR TYPES		BI	LL O	F MA	ATERIA	L
<u> </u>	END BENT 1					
			S	ΓAGE	Ι	
B1 $\sqrt{2}$	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B4	B1 B2	8	#9 #⊿	l STR	28'-1''	/64 154
<u>7'-7"</u> H1	B3	6	#4	STR	2'-5"	10
7'-2" H2	D1	13	#6	STR	1'-6"	29
$4^{1}/2^{\prime\prime}$ 2'-5" $4^{1}/2^{\prime\prime}$	НЗ	14	# 5	3	27'-4''	399
	К2	8	#4	STR	3'-5″	18
$(4) \checkmark (4)$	S1	32	#4	5	7'-5"	159
	S2	32	#4	4	3'-2"	68
1'-3'' LAP	S3 S4	8	#4 #6	6 8	6'-6" <u>/'-/''</u>	35 20
	S5	1	#6	7	8'-8''	13
	V1	63	#4	STR	4'-8"	196
$\left(\begin{array}{c} 6 \end{array}\right)$						
	REIN (STA	NFORCI GE I)	NG STE	EL	180	65 LBS.
<u> </u>			ST	AGE	II	
<u>1'-3"</u> <u>2'-0''</u> <u>1'-3"</u>	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	B3	6	#4	STR	2'-5"	10
нк.	B4 B5	8	#9 #4	1 STR	22'-5" 22'-0''	610 118
	D1	11	#6	STR	1'-6"	25
	H1	6	#4	2	8'-3"	33
· · ·	H2	6	#4	2	7'-10″	31
ENSTANS ARE AUT TO AUT	К1	6	#4	STR	3'-6″	14
LINSTONS AND OUT TO OUT.	S1	31	#4	5	7′-5″	154
	S2	31	#4	4	3'-2"	66
	S3	6	#4	6	6'-6"	26
	V1	24	#4	STR	4'-8"	75
	REIN (STA	NFORCI GE II)	ng ste	EL		1162
SS A CONCRETE BREAKDOWN	CLAS	S A C(ONCRET	E BREA	AKDOWN	
(STAGE I) JR #1 CAP,LOWER PART 10.3 C.Y. OF WINGS & COLLARS	POUF	() 1 #1 C 0	STAGE AP,LOV F WIN(II) VER PA GS & (ART COLLARS	6.8 C.Y.
JR #2 UPPER PART OF 2.8 C.Y.	POUF	2 #2 U W	PPER F	PART C)F	1.0 C.Y.
TAL CLASS A CONCRETE 13.1 C.Y.		L CLAS	SS A C	ONCRE	TE	7.8 C.Y.
	.	. -			2-003	32
PR	OJEC I		U TSC		<u>1-00.</u>	
		VIAU	TOC		CO	UNIY
ST	ΑΤΤΟ)N:	<u> 1</u> 3-	<u>+50</u>	.00 -	·L-
	· · · · · ·					
SHEE	1 3 0	- 3				
	DEPA	RTMEN	IT OF N	ORTH CARC	ISPORTA	TION
POPESSION AND SEAL		SL	JBSTF	RUCT	URE	
025516		ΓN	n R	FNT	F #1	
Emily E. Murray		• ، ت ا	DET	AIL	S	
A689F83FD9D74F8 2/22/2022		DF	VICIONS		[SHEFT NO
DOCUMENT NOT CONSTREES NO.	BY:	DATE:	NO.	BY:	DATE:	S-24
5 FINAL UNLESS ALL			3			TOTAL SHEETS
SIGNATURES COMPLETED 🙎			4			34

STD. NO. EB_36_120S

	NOTES
	STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.
	HOOKS ON ``V''BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
	FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
PAN E	ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR ``REINFORCING STEEL" AND ``SPIRAL
	COLUMN REINFORCING STEEL."
т	THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND
	LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT
v V	ONE FOOT BELOW THE GROUND LINE. DRILLED PIERS SHALL BE TERMINATED ONE FOOT ± ABOVE
	NORMAL WATER SURFACE ELEVATION FOR SHAFTS LOCATED IN WATER.
	THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.
YAN A	7
CAP	
A) 5 . 99	SLAB UNIT
CAP B) 5.68	E BEARING - 1'-10"
С	BENT (TYP.) (TYP.)
	2'-6" X 8" X 1"
	ELASTOMERIC BEARING PAD (TYPE I) (TYP.) #6 D1 DOWELS
	TO PROJECT 9" Above cap (typ.)
	DETAIL ``A''
اد	(DIMENSIONS ARE TYPICAL EACH BEARING)
	$\frac{1}{1} + \frac{1}{1} + \frac{1}$
	$-\frac{10}{3}$ STATION: <u>13+50.00</u> -L-
<u>'</u> -> .v.r	SHEET 1 OF 2
	<u>TUEIAIL</u> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
	RALEIGH
	SUBSTRUCTURE
	BENT No. 1
Γ	Emily E. MUNYAY A689EB3ED9D74EB 3/3/2022 DEVISIONS I SHEET NO
5	NO. BY: DATE: NO. BY: DATE: SHEET NO. SHEET NO. NO. BY: DATE: NO. BY: DATE: S-25
	SIGNATURES COMPLETED 26

STD.NO.DP_BT_36_120S_<50'

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BAR TYPES

BILL OF MATERIAL						BI	ill C)F MA	TERIAL	_
R	BEN	NT (S	STAGE	I)	F	OR	BEN	T (S	TAGE	II)
	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	# 11	1	27'-3''	1448	B3	10	#11	1	21'-9''	1156
	# 5	STR	27'-10''	174	B4	6	#5	STR	21'-0''	131
	# 5	STR	25'-8"	54	B6	2	# 5	STR	20'-2"	42
,	#6	STR	1'-6"	59	D1	22	#6	STR	1'-6"	50
										1501
)	#11	SIR	31'-6''	3985	M2	10	#11	SIR	30'-0''	1594
	# [0/ 0//	775	<u> </u>	70	#5		0/ 0//	757
<u>'</u>	#5	۷	90.	515	51	38	#5	2	90.	357
	# 1		 5/_0″	11	1.11	7	#1	7	<u> </u>	11
	4 #⊿	ך א	5'-6"	11		ר א	#⊿	ך א	5'-6"	11
	¬ #⊿	ך א	<u> </u>	70		21	 #⊿	<u>ן</u> ז	<u> </u>	58
<u>'</u>					05		'			50
,	#11	1	13'-11"	739	V3	10	#11	1	15'-0''	797
	#11	1	14'-5"	766				_		
<u> </u>					RETN	FORCT	NG STF	 Fl		
)			7	692 LBS.	(STA)	GE II)			4	207 LBS.
	*	4	444'-4''	927	SP-2	1	*	4	324'-6''	338
	**	5	290'-8''	194	SP-5	1	**	5	319'-0''	213
	**	5	305'-8''	204						
OLI	JMN RE	EINFORC	CING STEEL	-	SPIRA	AL COL	UMN RE	INFORC	ING STEE	L
			1	325 LBS.	(STAG	E II)				551 LBS.
⊃-1	SPIRA	AL REIN	FORCING S	STEEL	* T⊦	IE SP-2	2 SPIR	AL REIM	NFORCING	STEEL
BE	. W31 (DR D-31	COLD DRA		SH	IALL BI	E W31 (DR D-31	COLD DRA	
	JIL	AIN UN		DAN	** _		JIL	AIN UN		J DAN
ז⊂ ∟⊃	& SP	2-4 SPI	RAL REINF	ORCING	+++ T⊦	IE SP-	5 SPIR	AL REIN	NFORCING	STEEL
Sr W	IRE OF	E W20 7 #4 PL	AIN OR DE	FORMED	WIRE OR #4 PLAIN OR DEFORMED BAR					
٨с				M N I						W/NI
AD		STAGE	I)	WIN		CLAS		STAGE I	I)	WIN
					POUR	#2 (C()		2.1 C.Y.
(CA	P)			9.2 C.Y.	POUR	#3 (CA	AP)	,		7.9 C.Y.
						0.01				
ASS A CONCRETE 13.1 C.Y.					τοται	_ CLAS	S A CO	DNCRETE		10.0 C.Y.
	DRIL	LED PI	ERS:				DRIL	LED_PI	ERS:	
_	(S	STAGE I)				(S ⁻	IAGE I	L)	
PI	ER CON	ICRETE			DRILL	ED PI	ER CON	ICRETE		
DR:	ILLED	PIERS)		14.5 C.Y.	POUR	#1 (DR	ILLED	PIERS)		5.3 C.Y.

#4 U2 (TYP.EA.END)	PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: <u>13+50.00</u> -L-
	SHEET 2 OF 2
STUTING TH CAROLAN	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL	SUBSTRUCTURE
V O25516	BENT No.1
A689F83FD9D74F8 3/3/2022	
DOCUMENT NOT CONSIDERED 5 FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-26 1 3 4 34 34
	STD NO DD DT 36 1905 /50/

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DRAWN BY :	D. A. GI	ADDEN	DATE :	8/21
CHECKED BY :	D. R.	SMITH	DATE :	12/21
DESIGN ENGINEER	OF RECORD: _	E.E.MURRAY	DATE :	1/22

STD. NO. EB_36_120S

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DRAWN BY :	D.A.GL	ADDEN	DATE :	8/21
CHECKED BY :	D. R. S	SMITH	DATE :	12/21
DESIGN ENGINEER	OF RECORD:	E.E.MURRAY	DATE :	1/22

STD. NO. EB_36_120S

STD. NO. EB_36_120S

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22-FEB-2022 15:54 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ emily.murray AT C-1000324

SID. NO. EB_36_120S

PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: <u>13+50.00</u> -L-	-
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT #2 DETAILS	
NO. BY: DATE: NO. BY: DATE: STORATURES COMPLETED SHEET NO.	10.

	BILL OF MATERIAL										
					END B	ENT	2				
		S1	ΓAGE	Ι				ST	AGE	II	
	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	18	# 5	STR	30'-0''	564	B2	18	#5	STR	22′-5″	421
$ \downarrow$	13	#6	STR	1'-6"	29	D1	11	#6	STR	1'-6"	25
_	47			4 7 / 44 / /	0.47		4.4		C T D	17/ 0//	1.40
+	13	#5 #5	2		243	H3	11	#5 #5	SIR	13'-0''	149
+	13	C	Ζ	17-4	235	4 5	11	#5	STR	12'-6''	145
╉	8	#4	STR	3'-9"	21	H6	1	#5	STR	9'-8''	10
╡		-				H7	1	# 5	STR	6'-10''	7
	65	#9	STR	6'-2''	1362	H8	1	# 5	STR	4'-1''	4
	34	#9	STR	7'-3''	838	Н9	1	#5	STR	12'-4''	13
	37	#9	1	11'-6"	1447	H10	1	# 5	STR	9'-6''	10
_			0.7.0	7.4. 0.4	4.0.7	H11	1	#5	STR	6'-8''	7
_	2	#6 #C	SIR	34'-2''	103	H12	1	#5 #5	SIR	3'-11''	4
_	2	#6 #6	SIR	32'-5'		HID	2	*5	SIR	9°-1°	19
+	2	#6	STR	31'-9''	96	M2	56	#Q	STR	7'-0''	1333
+	2	#6	STR	31'-0''	94	M3	31	#q	STR	7'-3''	764
╡	2	#6	STR	30'-2''	91	M4	33	#9	1	11'-6"	1290
	2	#6	STR	29′-5′′	89						
	2	#6	STR	28'-7''	86	Т9	16	#6	STR	22'-7''	543
	30	#6	STR	6'-8''	300	Т1О	28	#6	STR	6'-8''	280
\downarrow	16	#6	STR	2'-8''	64	T11	10	#6	STR	2'-8''	40
_	8	#6	STR	14'-0''	168	T13	8	#6	STR	11'-0''	132
+	4	#6	SIR	('-6''	45		6	+5	стр	10/ 5//	
+	12	#1	СТР	a'_0''	253	VZ V3	6 2	#5 #5	SIR	10'-5''	65
+	32	#6	STR	<u> </u>	192	V 3 V 4	2	#5	STR	10'-1''	22
+	52	0	<u> </u>		152	V5	2	#5	STR	9'-10''	21
╡						V6	2	# 5	STR	9'-7''	20
						٧7	2	# 5	STR	9'-4''	19
						V8	2	# 5	STR	9'-1''	19
\downarrow						V9	2	*5	STR	8'-9''	18
_						V10	2	#5 #5	STR	8'-6''	18
+						V11	2	#5 #5	SIR	8'-3''	
+						V12	Ζ	- 5	214	8-0	11
NF R	ORC: STAG	ING STE E I)	EL	6	519 LBS.	REIN (FOR	IFORCI STAG	ING ST E II)	EEL		5466 LBS.
NS:	5 A		TE BRE	AKDOWN		CLAS	SAC		TE BRE	AKDOWN	
JR	#1	FOOTIN	IG		17.0 C.Y.	POUR	#1	FOOTIN	G		13.8 C.Y.
JR	# 2	LOWER WINGS	PART & WAL	OF L	21.8 C.Y.	POUR	#2 I	LOWER WINGS	PART & WAL	OF L	20.6 C.Y.
JR	#3	UPPER WINGS	PART	OF	2.2 C.Y.	POUR	#3 I	UPPER WINGS	PART	OF	0.9 C.Y.
ΓΑΙ	L CL (FOF	ASS A (R STAGE	CONCRE I)	ETE	41.0 C.Y.	τοτα	LCLA	ASS A (CONCRE	ETE	35.3 C.Y.
UN E	DAT] ND E	ION EXC BENT	AVATI	ON	L.S.	FOUN AT E	DATIC ND BE	ON EXC.	AVATI	ON	L.S.

NOTES : For berm width dimensions, see general drawing.

— EL.1867.67

ESTIMATED QUANTITIES						
E @ 3+50.00 -L-	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
BENT 1	23	26				
BENT 2	157	175				

PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: <u>13+50.00</u> -L-						
NUMBERSON AND THE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SEAL 025516 CONEFRICATION FORMULY AG89F83FD9D74F8 2/22/2022	RIP RAP DE	TAILS				
	REVISIONS	SHEET NO.				
DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY:	DATE: S-31				
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 2 4	SHEETS 34				

STD. NO. RR1

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ASSEMBLED BY : R.G.BEAUC CHECKED BY : P. N. HOLI	CHAMP/ D.A.G. DER	DATE : DATE :	6/19 12/21
DRAWN BY :SHS/MAA 5-09 CHECKED BY : BCH 5-09	REV. 12-17		МАА/ТНС

SECTION N-N

CURB DETAILS

FOR BRIDGE DRAINAGE P ROADWAY PL GEOTEXTILE STANDARD S SELECT MAT BE IN ACCO SECTION 101

SELECT MAT FILL FACE (EDGE OF API FOR THE 4" (STANDARD D AREA BETWE GRADED TO OF THE BRII

APPROACH

FOR ANCHOR INSERT LOC

NOTES	BILL OF MATERIAL					-
E APPROACH FILL INCLUDING GEOTEXTILE, 4"Ø PIPE, AND SELECT MATERIAL BACKFILL, SEE ANS.	A۱	PRC	ACH S	SLAE TAGE	3 AT EE I	3 #1
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
E SHALL BE TYPE 1 IN ACCORDANCE WITH THE SPECIFICATIONS SECTION 1056.	* A1	11	#4	STR	22'-7''	166
TERIAL BACKFILL (CLASS V OR CLASS VI) SHALL	AZ	11		518	21-5	163
016.	* B1	36	# 5	STR	11'-1"	416
	B2	36	# 6	STR	11'-7"	626
OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE PROACH SLAB.						
DRAINAGE FIFE OUTLET(S), SEE ROADWAT DRAWINGS.	REINF	ORCIN	G STEE	L	LBS.	789
EEN THE WINGWALL AND APPROACH SLAB SHALL BE DRAIN THE WATER AWAY FROM THE FILL FACE DGE AND SHALL BE PAVED.SEE ROADWAY PLANS.	* EPO REI	XY CO NFORC	ATED ING ST	EEL	LBS.	582
THE CROOVING IS NOT REQUIRED	CLASS	AA C	ONCRET	E	C.Y.	11.5
RED PORTABLE CONCRETE BARRIER AND CONCRETE CATIONS, SEE SHEET 3 OF 3.	A	PRC	ACH) ST	SLAE AGE	3 AT EE II	3 #1
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	* A3	11	#4	STR	20'-0''	147
	۵4	11	#4	STR	19'-8''	145
	* B1	35	# 5	STR	11'-1"	405
	B2	35	* 6	STR	11'-7"	609
	REINF	ORCIN	IG STEE	L	LBS.	754
	* EPO REI	XY CO NFORC	ATED ING ST	EEL	LBS.	552

SPL	SPLICE LENGTHS					
BAR SIZE	EPOXY COATED	UNCOATED				
#4	1'-11''	1'-7"				
# 5	2'-5″	2'-0"				
# 6	3'-7"	2'-5"				

C.Y.

11.5

CLASS AA CONCRETE

PROJECT NO. <u>BR-0032</u> <u>MADISON</u> COUNTY STATION: <u>13+50.00</u> -L-						
SEAL 025516 BUILD BUILD	SHEET TOP 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 120° SKFW					
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-32 1 3 3 34 34					

ASSEMBLED BY : R.G.BEAUC CHECKED BY : P. N. HOLI	CHAMP/ D.A.G. DER	DATE : DATE :	6/19 12/21
DRAWN BY :SHS/MAA 5-09 CHECKED BY : BCH 5-09	REV. 12-17		MAA/THC

(TYPE II - MODIFIED APPROACH FILL)

NOTES			BILL OF MATERIAL							
GE APPROAC PIPE, AND PLANS.	H FILL INCLUDING GEOTEXTI SELECT MATERIAL BACKFILL,	LE, SEE	4" Ø	۱A	PR(CH S ST/	L A E A G E	BATE I	B #2
LE SHALL BE	E TYPE 1 IN ACCORDANCE WIT	гн т	ΉE	BAR * A5	NO.	SI #	ZE T	YPE STR	LENGTH 25'-0''	WEIGHT
ATERTAL BA	CKETLL (CLASS V OR CLASS V			A6	11	#	4	STR	24'-8''	181
CORDANCE W	TTH STANDARD SPECIFICATIO	ONS	JHALL	* B1	32	#	² 5 5	STR	11'-1"	370
ATERIAL BA	CKFILL IS TO BE CONTINUOL	JS A	LONG	B2	32	#	6	STR	11'-7"	557
E OF BACKWA APPROACH SI	ALL FROM OUTSIDE EDGE TO LAB.	OUT	SIDE	₩ B3	14	#	6	STR	7′-9″	163
4″Ø DRAINA DRAWINGS.	GE PIPE OUTLET(S), SEE ROAD	WAY	,	REINF	ORCI	NG S	TEEL		LBS.	901
WEEN THE W O DRAIN TH	INGWALL AND APPROACH SLAE	3 S⊦ LL F	IALL BE FACE	REI	NFORC	ING	STEE	L	LBS.	660
RIDGE AND S	SHALL BE PAVED. SEE ROADWA	ΥP	LANS.	CLASS AA CONCRETE C.Y. 12.2						
ORED PORTA OCATIONS, S	SLAB GROOVING IS NOT REQUIRED. ORED PORTABLE CONCRETE BARRIER AND CONCRETE OCATIONS, SEE SHEET 3 OF 3.			APPROACH SLAB AT EB #2 STAGE II						
				BAR	NO.	SI	ZE T	YPE	LENGTH	WEIGHT
				* A7 A8	<u>11</u> 11	#	4 <u>9</u> 4 9	STR STR	19'-11'' 19'-7''	146
				<u>ч D1</u>	75		E (11/ 1//	405
				* B1 B2	35	#	6	STR	11 -1 11'-7"	405 609
				REINF	ORCI	NG S	TEEL		LBS.	753
				* EPO REI	NFORC	DATE SING	STEE	L	LBS.	551
				CLASS	5 44 (CONC	CRETE		C.Y.	11.5
						Г	SPI	тсі	E LENG	тнс
INUOUS AIR UPPER (CHCU) PROPOSED					┢	BAR			
TS. ACROSS	SLAB ASPHALT					F	512E			-7"
	6″ \ < ► 	\setminus				┢	#5	$\frac{1}{2}$	$\frac{-11}{-5''}$	- ('-0"
						┢	#6	3	<u> </u>	·-5″
-5,		5 2	<u> </u>	-ROADW	VAY					
		I			•					
	▼	``B	" BARS	"C-,	NIN.					
OVED WIRE	BAR /	U	DANS	~	J Z					
RTS @ 3'-0'	" CTS. —				<u> </u>					
/										
	- APPROXIMATE 1:1 SLOPE (TO BE DETERMINED									
	BY THE CONTRACTOR) (SOLID ROCK MAY BE LEFT									
тыс	INTACT IN APPROACH FILL AREA AT THE APPROVAL OF E ENCINEER PLACE A MINIMU	м								
C	DF 2'-O'' OF CLASS VI STONE UNDER APPROACH SLAB)	141								
							F		~~~	
D		Ρ	ROJE	CTI	NO.		<u>t</u>	зK	-003	
				MA	DI	<u>S(</u>	JN		COI	JNTY
AL TO END	BENT [†]	S	ΤΑΤΙ	ON:	1	3.	+50).(<u>- 0C</u>	L-
			SHEET 2	OF 3						
					STAT		NORTH C			
			DEP	ARIMI	ENI	OF R	IRA ALEIGH	ans	PORIAI	TON
	NITH CAROLANT		R	RTD	ςF	ΔP	PRC)∆(B
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				CO	REC) S	SLAI	3 ι	JNIT	
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-854-0355	FINAL UNLESS ALL	์ โ วิ)			3 A				TOTAL SHEETS 34
			1	I				1	1	

ASSEMBLED BY : R.G.BEAUC CHECKED BY : P. N. HOLD	CHAMP/ D.A.G. DATE : DER DATE :	6/19 12/21
DRAWN BY : SHS/MAA 5-09 CHECKED BY : BCH 5-09	REV. 12-17	MAA/THC

PART PLAN FOR BARRIER ANCHORAGE LOCATIONS ON APPROACH SLABS

		PROJECT	• NO IADIS N:13	BI ON 5+50	<u>R-003</u> co .00 -	32 UNTY ·L-
	SEAL O25516 FMILE E. Murray	DEPAR BRI FOR F (STATE OF TMENT O DGE AF PRESTRE CORED UB-REG 120	NORTH CARG F TRAN RALEIGH PPROA SSSED SLAB IONAI SKE	NSPORTA NSPORTA CONCI UNIT L TIER	TION AB RETE
K I`		REVISIONS				
410 854-0355	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: 1 2	DATE: NO. 3 4	BY:	DATE:	5-54 Total Sheets 34

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/2" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

