-003 B REFERENCE

67033

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

CONTENTS

SHEET NO. **DESCRIPTION** TITLE SHEET LEGEND (SOIL & ROCK) SITE PLAN 4-6 CROSS SECTIONS 7-10 BORE LOGS & CORE REPORTS 11-13 CORE PHOTOGRAPHS

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY MCDOWELL
PROJECT DESCRIPTION <u>REPLACE BRIDGE</u> #84
ON SR 1234 (PARKER PADGETT RD.) OVER I-40
SITE DESCRIPTION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
V.C.	67033	1	13

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE DESCREED AND AND THE STANDARD TEST METHOD. THE DISSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

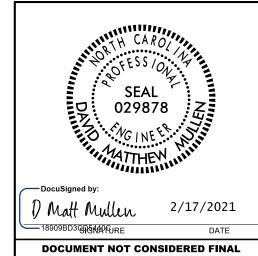
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION OF HIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED OF AN PREADON RESULTING FROM THE ACTUAL CONDITIONS OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

	D JOHNSON
	о снеек
_ C	J COFFEY
INVESTIGATED BY .	DMM
DRAWN BY <i>DM</i>	
CHECKED BY JC.	
SUBMITTED BY	CK .
DATE 2/16/2021	



UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, CRYSTALLINE ROCK (CR)	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNEISS, CHEBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	BOOK (NICE) SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.
7. PASSING	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR SIL1- MUCK,	PERCENTAGE OF MATERIAL	CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 38 MX 58 MX 51 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3%, 3 - 5%, TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL - - 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN LITTLE OR HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CROUP INDEX A A A AMY S MY 12 MY 16 MY NO MY AMOUNTS OF ORGANIC	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
INCIDENT TYPES STONE FRACS ORGANIC SUILS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAYEL, AND FAINE SILIT OF CLATET SILIT CLATET MATTER	▼ STATIC WATER LEVEL AFTER 24 HOURS	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SANU		MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABL		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
P1 OF A-7-5 SUBGROUP IS ≤ LL - 30 ; P1 OF A-7-6 SUBGROUP IS > LL - 30	- O-M⊶ SPRING OR SEEP	WITH FRESH ROCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNIESS OR RANGE OF STANDARD RANGE OF UNCONFINED	TT 25,425	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LOOSE (4	SPT C SURE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL OPT DMT TEST BORING INSTALLATION INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10/10/30/ N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	→ INFERRED SOIL BOUNDARY → CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MN MONITORING WELL	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2	WITH CORE	SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4	→→→→→ ALLUVIAL SOIL BOUNDARY \(\triangle \) INSTALLATION \(\triangle \) SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNSUITABLE WASTE ACCEPTABLE, BUT NOT TO BE	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOLI MOISTURE SCALE FIELD MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC γ_d - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRACT - FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING	BENCH MARK: BL-I
(PI) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 1369.96 FEET
SL _ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO		VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	
ATTAIN OPTIMUM MOISTURE	CME-55 6° CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	X 8* HOLLOW AUGERS	INDURATION	-
	X CME-550 HARD FACED FINGER BITS X -N XWL	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
PLASTICITY INDEX (PI) DRY STRENGTH		RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
PLASTICITY INDEX (PI) DRY STRENGTH	VANE SHEAR TEST TUNG-CARBIDE INSERTS HAND TOOLS:	GENTLE BLUW BY HAMMER DISINIEGRATES SAMPLE.	
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 YERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS HAND TOOLS: CASING X W/ ADVANCER POST HOLE DIGGER	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
PLASTICITY INDEX (PI) DRY STRENGTH	VANE SHEAR TEST TUNGCARBIDE INSERTS HAND TOOLS: CASING X W/ ADVANCER POST HOLE DIGGER TRICONE STEEL TEETH TRICONE STEEL TEETH TAYLOGUS STEEL TEETH TOURGE STEEL TEETH TOURGE STEEL TEETH TOURGE STEEL TEETH	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
PLASTICITY INDEX (PI) DRY STRENGTH	VANE SHEAR TEST TUNGCARBIDE INSERTS X CASING X W/ ADVANCER POST HOLE DIGGER HAND TOOLS: POST HOLE DIGGER HAND AUGER TRICONE STEEL TEETH TRICONE TRICONE SOUNDING ROD	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	VANE SHEAR TEST TUNGCARBIDE INSERTS HAND TOOLS: CASING X W/ ADVANCER POST HOLE DIGGER TRICONE STEEL TEETH TRICONE STEEL TEETH TAYLOGUS STEEL TEETH TOURGE STEEL TEETH TOURGE STEEL TEETH TOURGE STEEL TEETH	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	DATE: 8-15-1

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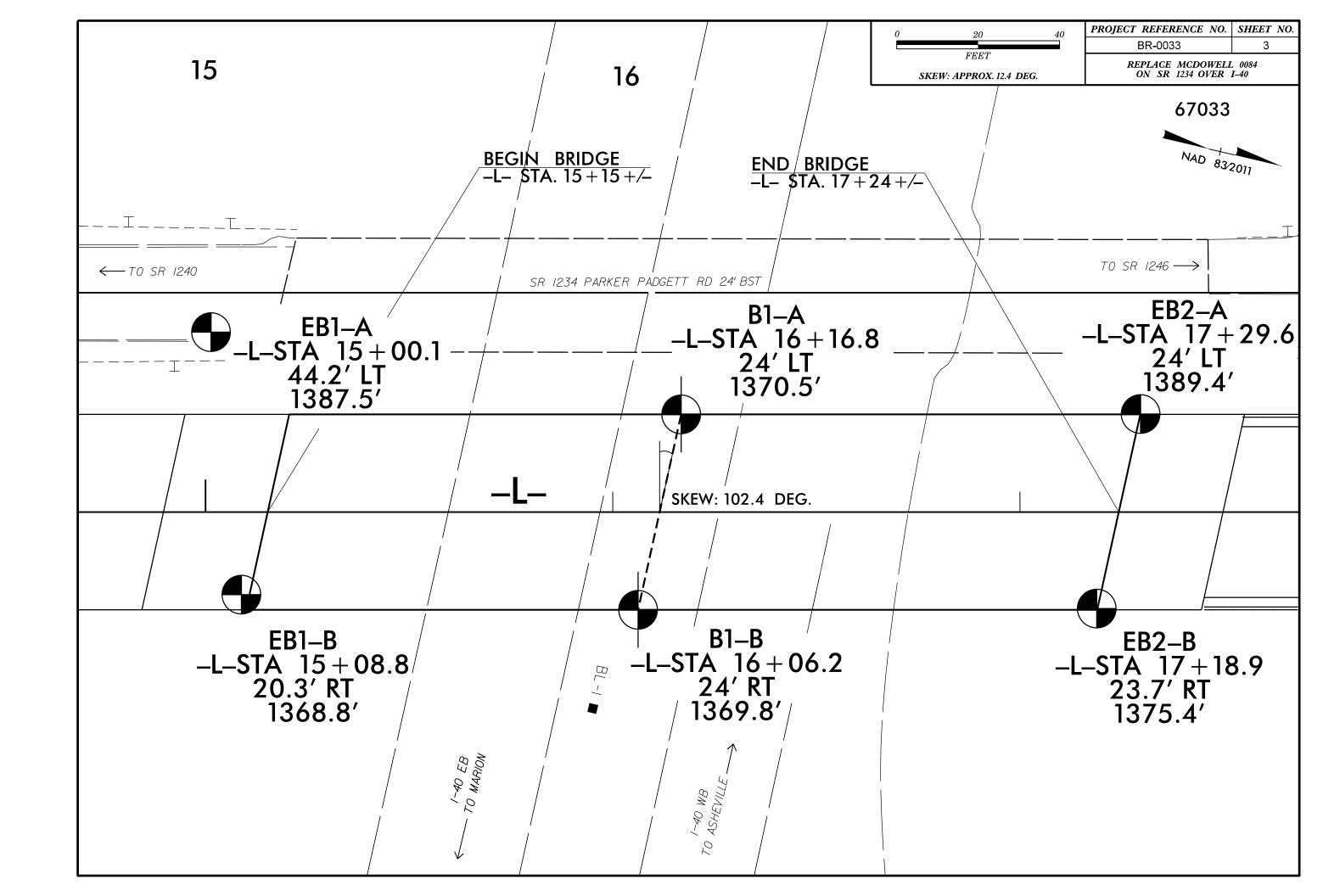
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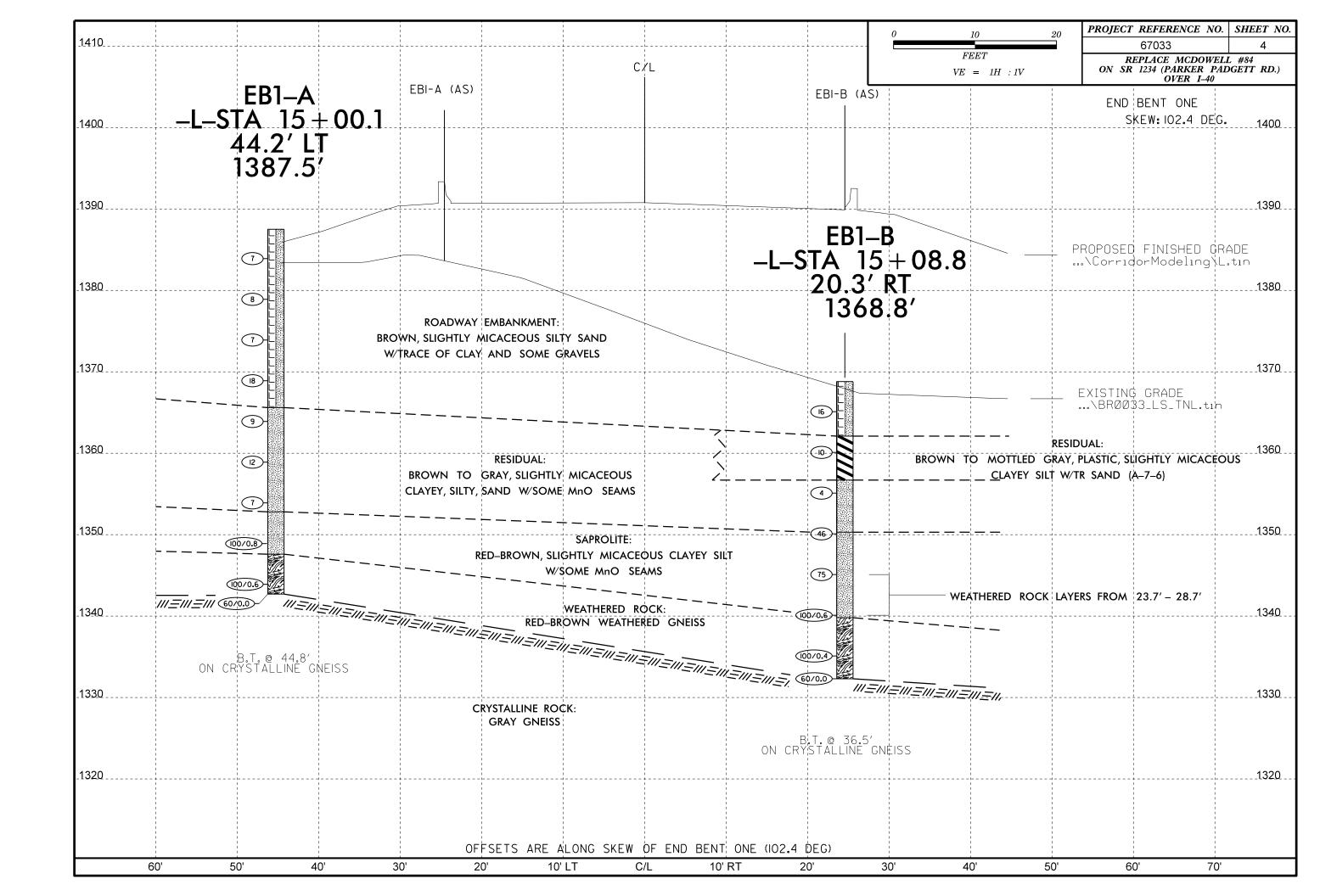
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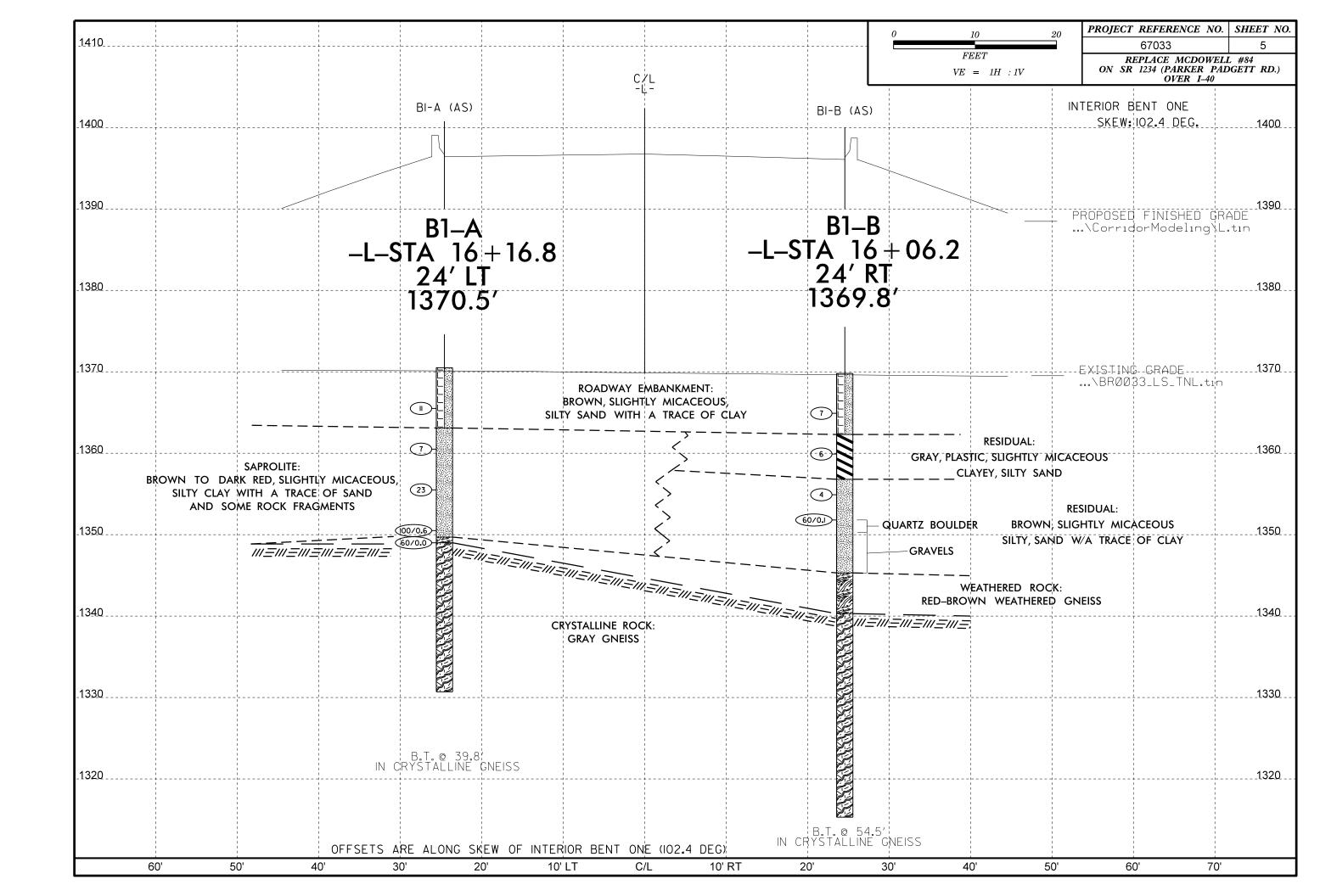
SUBSURFACE INVESTIGATION

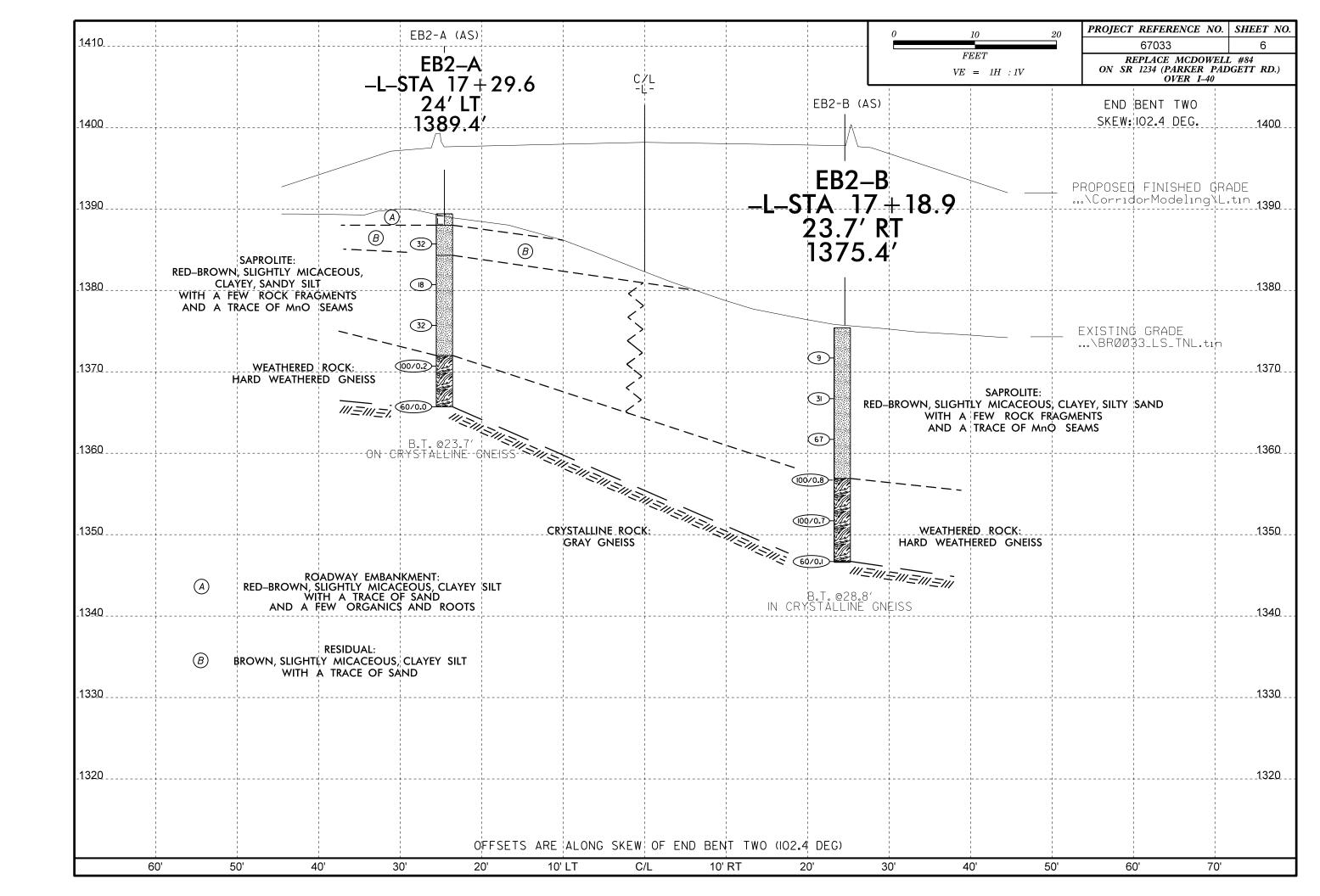
SUPPLEMENTAL LEGEND GEOLOGICAL STRENGTH INDEX (GSI) TARLES

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	nted R			SHTO LRI	FD BRID	AL STRENGTH INDEX (GSI) TABLES GE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		s e e		S O O O	S O O S	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS	VERY GOOD Very rough, fresh unweathered surface: GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surf with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surf with soft clay coatings or fillings	Exercise of Bright Park Process of the strength of Salight shift to the right in the columns (by a slight shift to the right wooder as the strength of son to change the value of Controlled the strength of son to the strength of the streng
STRUCTURE		DECREASING	SURFACE QU	JALITY =	⇒	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities BLOCKY - well interlocked un-	PIECES -	90 80		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability. A. Thick bedded, very blocky sandstone TO A
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	OF ROCK	70 60				B. Sand- stone with stone and still inter- layers of C. Sand- stone and stone and still shale or silty shale or clayey stone layers B. C. D E. Weak or clayey shale with
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	LOCKING		50			siltstone amounts state lagers 40
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	 DECREASING INTERL		40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H. F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	- DECRE			20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	Ÿ	N/A N/A		///	10	Into small rock pieces. → Means deformation after tectonic disturbance









WBS 67033.1.1	TIP BR-0033 COUNTY MG	AcDOWELL GE	EOLOGIST Johnson, C. D.	WBS 67033.1.1	TIP BR-0033 COUN	NTY McDOWELL	GEOLOGIST Johnson, C. D.
	BRIDGE 84 ON SR-1234 OVER I-40		· · · · · · · · · · · · · · · · · · ·		ACE BRIDGE 84 ON SR-1234 OVER		GROUND WTR (ft)
BORING NO. EB1-A		FSET 44 ft LT ALI		BORING NO. EB1-B	STATION 15+09	OFFSET 20 ft RT	ALIGNMENT L 0 HR. Caved
COLLAR ELEV. 1,387.5 ft	TOTAL DEPTH 44.8 ft NOR	PRTHING 702,131 EA	ASTING 1,065,920 24 HR . FIAD	COLLAR ELEV. 1,368.8 ft	TOTAL DEPTH 36.5 ft	NORTHING 702,153	EASTING 1,065,981 24 HR . N/A
DRILL RIG/HAMMER EFF./DATE AF	O8963 CME-550X 94% 04/08/2019	DRILL METHOD H.S. Aug	gers HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATI	AFO8963 CME-550X 94% 04/08/2019	DRILL METHOD	H.S. Augers HAMMER TYPE Automatic
DRILLER Cheek, D. O.	START DATE 12/09/20 CON	OMP. DATE 12/09/20 SUI	JRFACE WATER DEPTH N/A	DRILLER Cheek, D. O.	START DATE 12/08/20	COMP. DATE 12/08/20	SURFACE WATER DEPTH N/A
ELEV COUCHT (ft) DRIVE (ft) DEPTH BLOW COUCHT (ft) 0.5ft 0.5ft		100 NO. MOI G ELEV	SOIL AND NOON DESCRIPTION		V COUNT BLOWS PER FO 0.5ft 0.5ft 0 25 50	OT SAMP. L O NO. MOI G	
1390			7.5 GROUND SURFACE 0.0	1370			1,368.8 GROUND SURFACE 0.0
1,383.9 3.6 4 3	4		ROADWAY EMBANKMENT BROWN, SL MIC, SILTY SAND w/TR CLAY & SOME GRVLS	1365 1,365.1 3.7	9 7		BROWN, SANDY SILT w/TR CLAY, SOME GRVLS/RK FRAGS 1,362.1 6,7
1,378.9 8.6 3 4	1 .1			1360 1,360.1 8.7	4 6	SS-SS-1 M	RESIDUAL BROWN TO MOTTLED GREY, PLASTIC, SL MIC, CLAYEY-SILT W/TR SAND (A-7-6)
1,373.9 13.6 1 3	4 7	M L E		1355 1,355.1 13.7	2 2	SS-SS-2 M	1,356.7
1,368.9 18.6 5 11	. \			1350 1,350.1 18.7	18 28	м	18.5 SAPROLITE BROWN-RED, SL MIC, SANDY SILT W/TR CLAY, FEW RK FRAGS
1365	5	M -	RESIDUAL BROWN-GREY TO LT GREY, SL MIC, CLAYEY SILTY SAND w/FEW PEBS T/O	1345 1,345.1 23.7 24	37 38		IN/OUT WR LAYERS @23.7'-28.7'
1,358.9 28.6 2 5	7 12	 		1340 1,340.1 28.7	2/0.1		1,339.8 29.0 WEATHERED ROCK BROWN WEA GNEISS
1,353.9 33.6 WOH 3	4	M	SAPROLITE	1335 1,335.1 33.7 100/0.4 1,332.3 36.5 60/0.0		100/0.4	1,332.3 36.5 CRYSTALLINE ROCK
1,348.9 38.6 23 30		100/0.8	RED-BROWN, SL MIC, CLAYEY-SILT W/SOME MnO SEAMS 7.6 WEATHERED ROCK	+ 0000.0			DARK GREY GNEISS Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 1,332.3 ft ON CRYSTALLINE
1345		100/0.8	CRYSTALLINE ROCK				ROCK (GNEISS)
684.6PJ NC_DOT			DARK GREY GNEISS Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 1,342.7 ft ON CRYSTALLINE ROCK (GNEISS)				- - - - - -
							- - - - -
67033.1.1_M(- - - -
JBLE BR0033							- - - -
OT BORE DOI							
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WBS	67033	3.1.1			TI	P BR-0033	COUNT	Y McDOWI	ELL			GEOLOGIST Johnson, C. D.	.
SITE	DESCR	IPTION	N RE	PLAC	E BRID	GE 84 ON SR-12	234 OVER I-4	10					GROUND WTR (ft)
BORI	NG NO.	B1-A	\		S	TATION 16+17		OFFSET 2	24 ft LT			ALIGNMENT L	0 HR. N/A
OLL	AR ELE	EV. 1,	370.5	i ft	T	OTAL DEPTH 39).8 ft	NORTHING	702,2	48		EASTING 1,065,913	24 HR . N/A
RILL	RIG/HAI	MMER E	FF./D/	ATE /	\FO8963	CME-550X 94% 04	08/2019		DRILL N	/IETHOD) W	V Casing W/SPT & Core HAMM	ER TYPE Automatic
DRII I	LER C	heek [2 0		S	TART DATE 12/	15/20	COMP. DA	TF 12/	15/20		SURFACE WATER DEPTH N/	Δ
LEV	DRIVE	DEPTH	1	OW CC			WS PER FOOT	L	SAMP.	V /	L	OOR AGE WATER BEI III 10	
(ft)	ELEV (ft)	(ft)	0.5ft		1	0 25		75 100	NO.	MOI	O G	SOIL AND ROCK DESC	CRIPTION DEPTH (f
	()									, wor	Ť	LLL V. (II)	DEI III (I
1075													
375		<u> </u>									F	-	
	-	ł									Ŀ		
370	-	<u> </u>										1,370.5 GROUND SURFA	
	-	ļ						1		L		- ROADWAY EMBANI *INCOMPLETE SAMPLE, BR	
	-	‡								L	-	SILT, GRVLS T	
365	1,365.5	5.0	3	4	7	1 1 1 1 1 1		1		L	-NE	_	
	-	ł				· • • · · · · · · · · · · · · · · · ·				Ĺ	-WE	1,363.1	. 7.
						.					F	SAPROLITE BROWN-DK RED, SL MIC,	SILTY CLAY
360	1,360.5	10.0	2	3	4	7		+			F	w/TR SAND, w/SOME R	
	-	‡									**		
	- 1,355.5	15.0											
355	-,000.0	10.0	6	6	17	• 23		+				=	
	-	+				· · · · i · ·					∭-		
350 -	- 1,350.5	20.0	L		1					200000	₩F	4.040.7	00
	1,349.0	21.5	11 60/0.0	52	48/0.1			100/0.6	:		477	- 1,349.7 - 1,349.0 WEATHERED RC	
	-	‡	00/0.0	~				60/0.0				BROWN, WEA GNEIS CRYSTALLINE RO	
345	_	ţ										GREY GNEISS	
	-	ł									看		
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340	_	F										-	
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225	-	‡											
335	-	<u> </u>					 	+				-	
	-	ł											
-	-		-						4			1,330.7 Boring Terminated at Elevation	39.
	-	F									F	CRYSTALLINE ROCK	(GNEISS)
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GEOTECHNICAL BORING REPORT CORE LOG

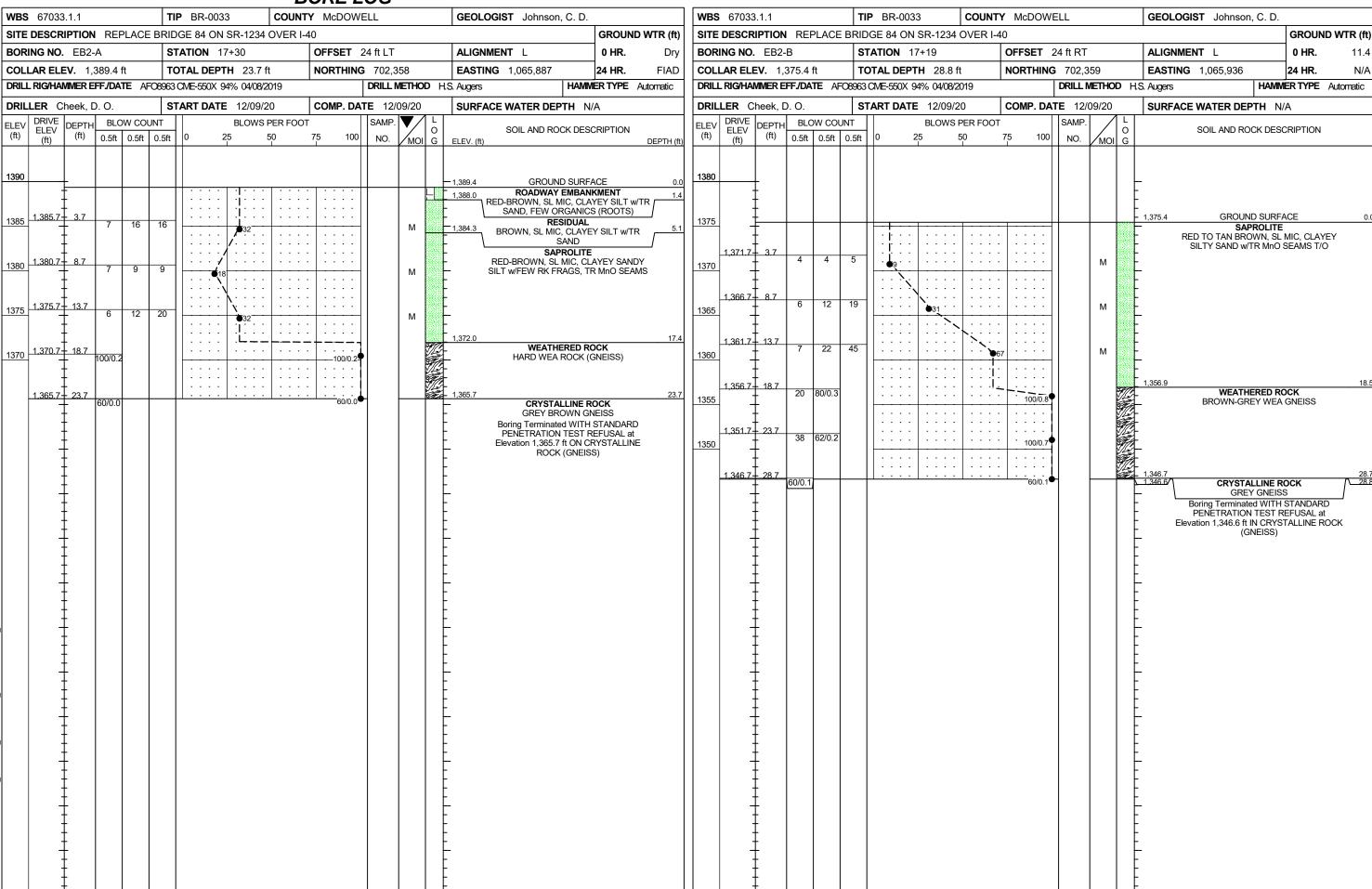
									(\mathcal{O}	RE I	_OG						
WBS	67033	3.1.1			TIP	BR-00	033	С	OUNT	ΥN	/cDOWE	ELL		GEOLOGIS	T Johnson	n, C. D.		
SITE	DESCR	IPTION	REF	PLACE B	RIDGI	E 84 O	N SR-12	34 OV	ER I-	40				•			GROUN	D WTR (ft)
BOR	ING NO.	B1-A	ı		STA	TION	16+17			OF	FSET 2	24 ft LT		ALIGNMEN	IT L		0 HR.	N/A
COL	LAR ELI	EV. 1,	370.5	ft	тот	AL DE	PTH 39	.8 ft		NO	RTHING	702,248		EASTING	1,065,913		24 HR.	N/A
DRIL	L RIG/HA	MMER E	FF./DA	TE AFO8	963 CIV	/E-550X	94% 04/	08/2019)			DRILL METHO	D NW	Casing W/SP1	Core	HAMIN	MER TYPE	Automatic
DRIL	LER C	heek, [D. O.		STA	RT DA	TE 12/1	5/20		СО	MP. DA	TE 12/15/20		SURFACE	WATER DE	PTH N	/A	
COR	E SIZE	NXWL			тот	AL RU	N 18.31	ft										
LEV	RUN	DEPTH	RUN	DRILL RATE	REC.	JN RQD	SAMP.	STF REC.	RATA	Ļ				FECRIPTION		/C		
(ft)	ELEV (ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	O G	ELEV. (f	t)	D	ESCRIPTION	AND REMARI	15		DEPTH (ft)
1349														Begin Corir	ng @ 21.5 ft			
	1,349.0	21.5	3.3	N=60/0.0 1:38/1.0 1:59/1.0 1:45/1.3	(2.9) 88%	(1.3) 39%					_ 1,349.0			CRYSTAL	LINE ROCK			21.5
1345	1,345.7	24.8	5.0	1:59/1.0	(4.6)	GSI: \40-50/					_							
	-	F	0.0	1:14/1.0 1:42/1.0 1:09/1.0 1:31/1.0	92%	(3.0)					-							
	1,340.7	29.8		1:31/1.0 1:43/1.0		GSI:					_							
1340	1,040.7	20.0	5.0	1.22/1 0	(5.0)	(5.0)	1				_							
		Ŧ		1:12/1.0 1:10/1.0 1:07/1.0	100%	100% GSI:					-							
1335	1,335.7	34.8		1.07/1.0	1	80-90					-							
1000	-	‡	5.0	1:38/1.0 1:22/1.0	(5.0) 100%						-							
		†		1:38/1.0 1:22/1.0 1:08/1.0 1:15/1.0		GSI: 85-90					-							
	1,330.7	39.8		1:46/1.0							_ 1,330.7 _	Boring Ter	minated	d at Elevation	1,330.7 ft IN C	RYSTALI	LINE ROCK	39.8
		‡									-			(GN	IEISS)			
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WBS	6703	3.1.1			TI	P BR-0033	СО	UNTY	McDOWE	≣LL			GEOLOGIST Johnson, C. D.	
SITE	DESCF	RIPTION	N REI	PLAC	E BRID	OGE 84 ON S	SR-1234 OVE	R I-40)				•	GROUND WTR (f
BOR	ING NO). B1-E	3		s [.]	TATION 16	+06		OFFSET 2	24 ft RT			ALIGNMENT L	0 HR. N/
COLI	LAR EL	. EV . 1	369.8	ft		OTAL DEPTH			NORTHING		49		EASTING 1,065,962	
						CME-550X 94						D N		
DRII	LER (Cheek	D 0		S	TART DATE	12/15/15		COMP. DA				SURFACE WATER DEPTH	
ELEV	DRIVE		1	DW CC			BLOWS PER F		JOHN . BA	SAMP.	10,20	1 L	OOK AGE WATER DEFINE	1 //-1
(ft)	ELEV (ft)	(ft)	0.5ft	1		0 25			75 100	NO.	MOI	O G	SOIL AND ROCK DES	SCRIPTION
	(1-)										, wiei	Ĭ		
1370													1,369.8 GROUND SURI	FACE
1370		‡											ROADWAY EMBA	NKMENT
		‡											BROWN, SL MIC, SILTY	SAND w/TR CL
1365	1,364.9	4.9			<u> </u>	·j· · ·							- -	
		‡	3	3	4	7							- - 1,362.3	7
		<u> </u>										1990	RESIDUAL	_
1360	1,359.9	9.9	WOH	3	3	6							GREY, PLASTIC, SL MIC, SAND	CLAYEY SILTY
		±											_ _ 1,356.8	13
1355	1 354 9	I 14.9											RESIDUAL BROWN, SL MIC, SILTY	
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Ŧ	2	2	2	• 4 · · ·							- V. HARD BLDR (@18.0'
	1,351.8	T 18.0	60/0.1	1				 	60/0.1	,			-	
1350	-	Ŧ	00/0.1										- -	
		Ŧ											- -	
1345		‡											- - 1,345.3	24
10-10	-	‡											WEATHERED F HIGHLY WEA G	ROCK
		‡											-	
1340	-	‡											- 1,340.3 CRYSTALLINE	ROCK 29
		‡											GREY GNEI	
1005		‡							: : : :				- -	
1335	-	‡											_ -	
		<u> </u>											- -	
1330	_	<u> </u>											<u>-</u>	
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1320		Ŧ											-	
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	-	‡—			+					-			1,315.3 Boring Terminated at Eleva	tion 1.315.3 ft IN
		‡											CRYSTALLINE ROCK	(GNEISS)
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GEOTECHNICAL BORING REPORT

SHEET 9

										C	ORE	LC)G								9
WBS	670	33.1.1			TIP	BR-00	033	С	OUNT	ΥN	ИcDOWE	ELL			GEOL	OGIST	Johns	son, C	. D.		
SITE	DES	CRIPTIO	N RE	PLACE B	RIDGI	E 84 O	N SR-12	34 OV	'ER I-	40										GROU	ND WTR (ft)
BOR	ING N	IO . B1-	В		STA	TION	16+06			OF	FSET 2	24 ft F	RT		ALIGN	MENT	. r			0 HR.	N/A
COL	LAR I	ELEV. 1	,369.8	ft	TOT	AL DE	PTH 54.	5 ft		NC	RTHING	702	2,249		EAST	ING 1	,065,96	2		24 HR.	N/A
DRIL	L RIG/	HAMMER	EFF./D/	ATE AFOS	3963 CN	1E-550X	94% 04/0	08/2019)			DRIL	L METHO	D NW	/Casing\	N/SPT 8	& Core	H	AMME	RTYPE	Automatic
DRIL	LER	Cheek,	D. O.		STA	RT DA	TE 12/1	5/15		CC	MP. DAT	TE 1	12/15/20		SURF	ACE W	ATER D	DEPTH	I N/A	١	
COR	E SIZ	E NXW	'L				N 36.5 f														
ELEV (ft)	RUI ELE (ft)	V DEFI	H RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STF REC. (ft) %	RQD (ft) %	L O G				D	ESCRIP [*]	TION AI	ND REMA	ARKS			
1351.8		10 100	<u> </u>			(==)									Begin	Coring	@ 18.0) ft			
1350	1,350 1,350	5.8 18.0 	5.0	N=60/0.1	(1.0) 67% (1.0) 20%	(0.0) 0% poulder (0.0)					- - - -				RES	IDUAL	(continue	d)			
1345	1,345	5.3 24.5	5.0	1:22/1.0 1:15/1.0 1:20/1.0	(2.1) 42%	0% gravel (0.0) 0%					1,345.3				WE	ATHER	ED ROCK	Κ			24.5
1340	1,340).3 + 29.5		1:14/1.0 1:32/1.0	(4.4)	GSI: 25					1,340.3				ODY	OTALL	INE DOG	14			29.5
	1 221	34.5	5.0	1:19/1.0 1:49/1.0 0:46/1.0 1:04/1.0	(4.4) 88%	(2.9) 58% GSI: 25-70					<u> </u>				CKY	SIALL	INE ROC	ĸ			
1335	1,335	5.3 + 34.5	5.0	1:03/1.0 1:12/1.0 1:08/1.0 1:51/1.0	(4.5) 90%	(3.0) 60% GSI:					<u>-</u>										
1330	1,330).3 + 39.5 	5.0	1:11/1.0 1:30/1.0 1:20/1.0 0:48/1.0	(5.1) 102%						<u> </u>										
1325	1,325	5.3 44.5 1	5.0	1:09/1.0 0:58/1.0 1:36/1.0 1:06/1.0	(5.0)	GSI: 80 (5.0)					<u> </u>										
1320	1,320).3 49.5		0:51/1.0 0:53/1.0 0:47/1.0 1:15/1.0	100%	GSI: 85					E E										
	4 04/	1 545	5.0	0/1.0 0/1.0 0/1.0 0/1.0	(5.1) 102%	(4.6) 92% GSI: 90					- - - - -										544
	1,315	5.3 + 54.5		0/1.0							1,315.3		Boring Te	rminated	d at Elev		315.3 ft IN	N CRYS	STALLI	NE ROCI	54.5 <
													BOILING TE	miliated	J at Elev	(GNE		N CATS	STALLI	NE ROOM	



CORE PHOTOGRAPHS

B1-A

BOX 1 OF 2: 21.5 - 31.6 FEET 21.5' - 27.5' GSI: 40 - 70 27.5' - 31.6' GSI: 80 - 90 **B1-A**

BOX 2 OF 2: 31.6 - 39.8 FEET GSI: 85 - 90



FEET





CORE PHOTOGRAPHS

B1-B

BOX 1 OF 3: 18.0 - 36.5 FEET

18' - 24.5' GSI: 0 24.5' - 31' GSI: 25 - 30 31' - 36.5' GSI: 50 - 75

B1-B

BOX 2 OF 3: 36.5 - 45.5 FEET 36.5' - 39.5' GSI 50 - 75 39.5' - 45.5' GSI 80 - 85



FEET





BR-0033 / MCDOWELL BRIDGE NO. 084

CORE PHOTOGRAPHS

B1-B

BOX 3 OF 3: 45.5 - 54.5 FEET GSI: 80 - 85



