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$\top \bigcirc \top$	AL BI	ILL OF	MATER		_S							
VING DGE ORS	CLASS A Concrete	BRIDGE APPROACH SLABS	REINFORCING STEEL	CON	54″F.I.B. PRESTRESSED NCRETE GIRDERS		PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 STEEL PILES			PILE DRIVING EQUIPMENT SETUP FOR PP 36"ØX 0.5" GALVANIZED STEEL PILES		
FT.	CU. YDS.	LUMP SUM	LBS.	NO.	D. LIN.FT.		EACH			EACH		
20				8	893.7		$\triangle$	$\sim\sim\sim$				
	67.3		7,856					, 12 k	)			
	45.3		5,056					, < <	)		5	
	61.0		7,205					. 10 <	)			
20	173.6	LUMP SUM	20,117	8	893.7			22 <	)		5	
YO BAR 1etal Rail	CONCRETE BARRIER RAIL	1'-2" X 3'- CONCRET PARAPE	-3" RIP RA E CLASS T (2'-0" TH	AP II ICK)	GEOTEXTILE FOR DRAINAGE	E	LASTON BEARII	MERIC NGS	EXPA Join <sup>-</sup>	NSION F SEAL		
IN. FT.	LIN.FT.	LIN. FT	. TONS		SQ.YDS.		LUMP S	SUM	LUMF	° SUM		
209.2	256.0	216.8										
			214		238							
			116		129							
209.2	256.0	216.8	330		367		LUMP S	ШM		SUM		

							TOT	TAL BI	ILL OF	$\mathbb{M}$	ATER	ΙΑI	_S					
	P TES	DA UN TING E	ICLASS STRUCI .XCAVA	IFIED URE TION	REIN CON DECI	IFORCED ICRETE K SLAB	GROOVING BRIDGE FLOORS	CLASS A Concrete	BRIDGE Approach Slabs	REINI S	FORCING TEEL	l Con	54″F.I.B. Prestressed Concrete Girders		PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 STEEL PILES		PILE DRIVING EQUIPMENT SETUP FOR PP 36"ØX 0.5" GALVANIZED STEEL PILE	
	E A	ACH	LUMP	SUM	SC	Q.FT.	SQ.FT.	CU.YDS.	LUMP SUM	L L	_BS.	NO.	LIN.FT.		EACH			EACH
SUPERSTRUCTURE					9	9,135	8,020					8	893.7		$\sim$			
END BENT NO.1								67.3		7	,856				\$ 12	$\langle$		
BENT NO.1								45.3		5	,056				{	$\langle$		5
END BENT NO.2								61.0		7	,205				\$ 10	3		
TOTAL	2	2	LUMP	SUM	9	9,135	8,020	173.6	LUMP SUM	2	0,117	8	893.7		22	$\langle$		5
	HP Stee	9 14X73 El PILES	PP 3 GAI STE	6″ØX _vaniz Tel pil	0.5″ ED ES	PILE REDRIVES	TWO BAR Metal Rail	CONCRETE BARRIER RAIL	1'-2" X 3'- CONCRET PARAPE	-3″ E T (	RIP RAI CLASS I 2'-0"THI	⊃ I CK)	GEOTEXTILE FOR DRAINAGE	ELASTOI BEARI	MERIC	EXPA Join	NSION T SEAL	
	NO.	LIN.FT.	NO.	LIN.	FT.	EACH	LIN.FT.	LIN.FT.	LIN.FT.		TONS		SQ.YDS.	LUMP	SUM	LUMF	⊃ SUM	
SUPERSTRUCTURE	1	·····					209.2	256.0	216.8									
END BENT NO.1	212	720.0	$\langle  $			6					214		238					
BENT NO.1	$\left  \right\rangle$		$\begin{cases} 5 \end{cases}$	600	.0	3												
END BENT NO.2	(10	700.0	$\langle  $			5					116		129					
TOTAL	22	1,420.0	\$ 5	600	.0	14	209.2	256.0	216.8		330		367	LUMP	SUM	LUMP	SUM	

DRAWN BY :	NSC		DATE :	03/2020
CHECKED BY :	MKC	1	DATE :	04/2021
DESIGN ENGINEER	OF RECORD:	RLB	DATE :	09/2021

3/10/2022 \\rsandh.com\files\Transportation\P\1031782004\_U-5798 (Gillis HillRoad)\_P&D\Design\Structures\CAD\Right Lane Bridge\402\_005\_U5798A\_SMU\_GD\_S-3\_250501.dgn CuanyN

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		HYDRAULI	IC DATA	
PI EQUI FOR P Alvani	LE DRIVING IPMENT SETUP PP 36″ØX0.5″ ZED STEEL PILES	DESIGN DISCHARGE FREQUENCY OF DESIGN DISCHAR DESIGN HIGH WATER ELEVATION DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION	= 710 CFS = 25 YRS = 152.1' = 16.1 SQ.MI. = 970 CFS = 153.6'	project no. <u>U-5798A</u>
	LACH	OVERTOPPING	FLOOD DATA	CUMBERLAND COUNTY
	5	OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING * OVERTOPPING ELEVATION	= 1,300+ CFS = 500+ YRS = 163.44′	STATION: 76+80.00 -L-
	5	* SAG @ ST	A. 80+55.33 -L-	SHEET 3 OF 3
SION SEAL SUM			TEGERIA ADDA TO THE PROVIDENCE OF THE PROVIDENCE	J. DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING RIGHT LANE BRIDGE ON SR 1102 OVER LITTLE ROCKFISH CREEK BETWEEN SR 1112 AND US 401
			DS8U Arabitatia Engineera Diannar	RIGHT LANE
<u>∕</u> RE	VISED HP 14X73 S	DOCUMENT NO FINAL U SIGNATURE	KS&H Architects-Engineers-Planner         8521 Six Forks Road, Suite 400         OT CONSIDERED         919-926-4100 FAX 919-846-9080         www.rsandh.com         S COMPLETED    North Carolina License Nos. 50073*F-0493*C:	Image: No.     BY:     DATE:     NO.     BY:     DATE:     SHEET NO.       No.     BY:     DATE:     NO.     BY:     DATE:     SHEET NO.       1     NSC     03/2022     3     TOTAL SHEETS       28     2     43

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIUE OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ``HEC 18 - EVALUATING SCOUR AT BRIDGES."

- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR INTERIOR BENT NO.1, ONLY PARTIAL GALVANIZING OF THE PILES IS REQUIRED.SEE INTERIOR BENT SHEETS FOR REQUIRED GALVANIZING LENGTHS. PAYMENT FOR PARTIALLY GALVANIZED PILES WILL BE MADE UNDER THE CONTRACT UNIT PRICE FOR GALVANIZED STEEL PILES.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S2-1 SHALL BE EXCAVATED FOR A DISTANCE OF 26 FT LEFT AND 52 FT RIGHT OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF THREE SPANS, ONE SPAN AT 30'-2", ONE SPAN AT 30'-1" AND ONE SPAN AT 30'-2" ON PRESTRESSED CONCRETE CORED SLABS, 32'-0" CLEAR ROADWAY WIDTH ON STEEL PILES AND LOCATED APPROXIMATELY 60' DOWNSTREAM FROM THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR REMOVAL OF EXISTING STRUCTURE AND ASBESTOS ASSESSMENT, SEE LEFT LANE BRIDGE.