

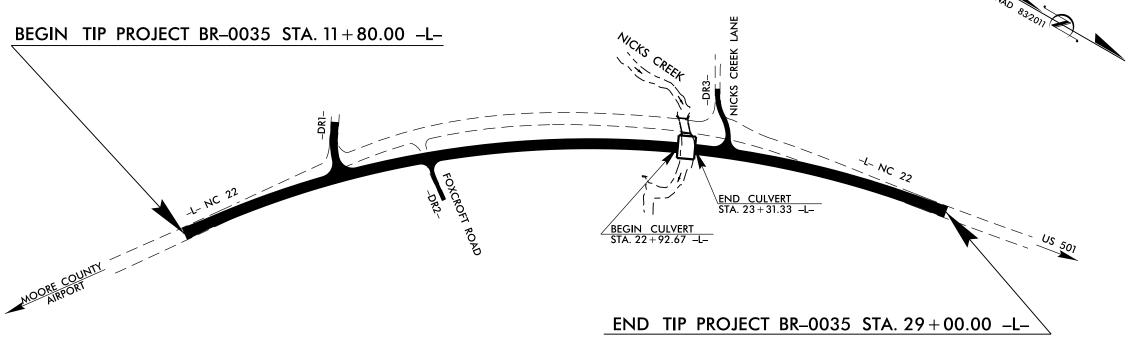
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

MOORE COUNTY

LOCATION: BRIDGE NO. 24 ON NC 22 OVER NICKS CREEK

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND CULVERT

STATE	STATE	PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS	
N.C.	В	R-0035		1		
STAT	B PROJ. NO.	F. A. PROJ. NO.		DESCRIPTION		
49	073.1.1	_		P.E. ROW/UTIL		
49	073.2.1	_				
49	073.3.1	0022015		CONS	T.	
	•					



CULVERT



DESIGN DATA

ADT 2020 = 6650

ADT 2040 = 8000

K = 11 %

D = 60 %

T = 4 % *

V = 60 MPH* (TTST=1% + DUAL=3%)

FUNC CLASS =

MINOR ARTERIAL STATEWIDE TIER

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT BR-0035 = 0.319 MI

LENGTH OF STRUCTURE TIP PROJECT BR-0035 = 0.007 MI

TOTAL LENGTH OF TIP PROJECT BR-0035 = 0.326 MI

LETTING DATE : JANUARY 18, 2022

2018 STANDARD SPECIFICATIONS

Prepared in the Office of:

DIVISION OF HIGHWAYS

STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. RALEIGH. N.C. 27610

W. KEVIN FISCHER, P.E.

KRISHNA P. SEDAI, P.E.

PROFILE ALONG & CULVERT

= 315.22 FT.

GRADE DATA

GRADE PT. EL. @ STA. 23+12.00 -L-= 318.91

= 304.80' BED EL.@ STA.23+12.00 -L-ROADWAY SLOPES = 3:1

A. SORSENGINH DATE : 8/2021 DRAWN BY : _ DATE : 8/2021 M. G. SHAIKH CHECKED BY : DESIGN ENGINEER OF RECORD: A. YASMEEN DATE: 8/2021

HYDRAULIC DATA

DESIGN DISCHARGE = 2,050 CFS = 50 YEARS FREQUENCY OF DESIGN FLOOD = 314.6 FT. DESIGN HIGH WATER ELEVATION

DRAINAGE AREA = 26.7 SQ. MI. BASE DISCHARGE (Q100) = 2,370 CFS

DESIGN FILL------MAX. 6.38 FT. MIN. 3.69 FT.

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

ASSUMED LIVE LOAD ------HL-93 OR ALTERNATE LOADING.

3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN STAGE I/STAGE II CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

- 1. WING FOOTING, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS IN STAGE I (PHASE I).
- 2. THE REMAINING PORTIONS OF THE WALLS, SILL AND WING FULL HEIGHT IN STAGE I (PHASE I).
- 3. WING FOOTING, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS IN STAGE I (PHASE II).
- 4. THE REMAINING PORTIONS OF THE WALLS, SILLS AND WING FULL HEIGHT IN STAGE I (PHASE II) FOLLOWED BY ROOF SLAB AND HEADWALL IN STAGE I.
- 5. WING FOOTING, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS IN STAGE II (PHASE III).
- 6. THE REMAINING PORTIONS OF THE WALLS, SILL AND WING FULL HEIGHT IN STAGE II (PHASE III).
- 7. WING FOOTING, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS IN STAGE II (PHASE IV).
- 8. THE REMAINING PORTIONS OF THE WALLS, SILLS AND WING FULL HEIGHT IN STAGE II (PHASE IV) FOLLOWED BY ROOF SLAB AND HEADWALL IN STAGE II.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FT. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTNG OF RC DECK ON CONCRETE ENCASED STEEL PLATES WIDENED WITH SINGLE 19'X 6'RCBC ON ABUTMENTS: REINFORCED CONCRETE STUB AND LOCATED AT THE SAME LOCATION AS THE PROPOSED CULVERT SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE STRUCTURE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED CULVERT, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED. AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL. TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FORM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE. PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

TOTAL STRUCTURE QUANTITIES								
FOUNDATION CLASS A REINFORCING MATERIAL (TONS) (CY) (LBS)								
STAGE I	PHASE I	49	51.1	6,361				
STAGE I	PHASE II	78	131.4	15,889				
STAGE II	PHASE III	31	36.7	4,196				
STAGE II	PHASE IV	50	88.4	10,149				
TOTAL	TOTAL 208 307.6 36,595							
REMOVAL OF EXISTING STRUCTURE @ STA.23+12.00 -L- LUMP SUM								
CULVERT EXCAVATION @ STA. 23+12.00 -L- LUMP SUM								
ASBESTOS A	SSESSMENT			LUMP SUM				

OVERTOPPING DATA

OVERTOPPING DISCHARGE = +3,160 CFS FREQUENCY OF OVERTOPPING FLOOD = 500+ YEARS = 319.4 FT. OVERTOPPING FLOOD ELEVATION

NOTES:

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR

THE REINFORCED CONCRETE BOX CULVERT SHALL BE PLACED ON THE STANDARD 1.0 FOOT BLANKET OF FOUNDATION CONDITIONING MATERIAL, SEE SECTION 414 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

FOR CONSTRUCTION SEQUENCE, SEE EROSION CONTROL PLANS.

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING. SEE PLANS AND TEMPORARY SHORING PROVISION.

DESIGN TEMPORARY SHORING FROM STATION 18+00 ± -L-, 37 FT. LT. TO STATION 24+00 ± -L- 15 FT. LT. FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION: UNIT WEIGHT OF SOIL ABOVE WATER TABLE, y=120 PCF UNIT WEIGHT OF SOIL BELOW WATER TABLE, y=60 PCF FRICTION ANGLE, \emptyset = 30° COHESION, c = 0 PSF GROUNDWATER ELEVATION = 312 FT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION 18+00 ± -L-, 37 FT.LT. TO STATION 24+00 ± -L-, 15 FT. LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION 18+00 ± -L-, 37 FT.LT. TO STATION 24+00 ± -L-, 15 FT.LT. SEE GEOTECHNICAL STANDARD DETAIL 1801.02 FOR STANDARD TEMPORARY WALLS.

DO NOT USE CANTILEVER, BRACED, OR ANCHORED SHORING FOR TEMPORARY SHORING FROM STATION 18+00 ± -L-, 37 FT. LT. TO STATION 24+00 ± -L-, 15 FT.LT.

FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

CONTROL PLANS.

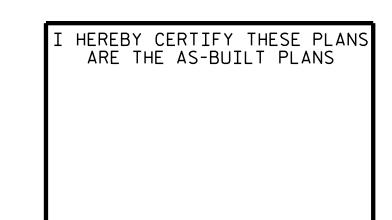
FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

SIZE LENGTH #3 6'-2" #4 7'-4"
#4 7'-4"
· · · · · · · · · · · · · · · · · · ·
5 8'-6"
#6 9'-8"
7 10′-10″
8 12'-0"
9 13'-2"
10 14′-6″
11 15′-10″

F.A. PROJECT NO.: 0022015

SAMPLE BAR REPLACEMENT LENGTHS BASED ON 30" (SAMPLE LENGTH) PLUS TWO SPLICE LENGTHS AND Fy =60 KSI.



Kevin Fischer

* FESSION:"

30878 * NGINEER

BR-0035 PROJECT NO. ___ MOORE COUNTY STATION: 23+12.00 -L-

SHEET 1 OF 8

REPLACES BRIDGE NO. 24

Krishna P. Sedai TOWNSORFHER CARC MOR GESSION A 031583

CACINETY & PRASAD

RALEIGH TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT 90° SKEW

STATE OF NORTH CAROLINA

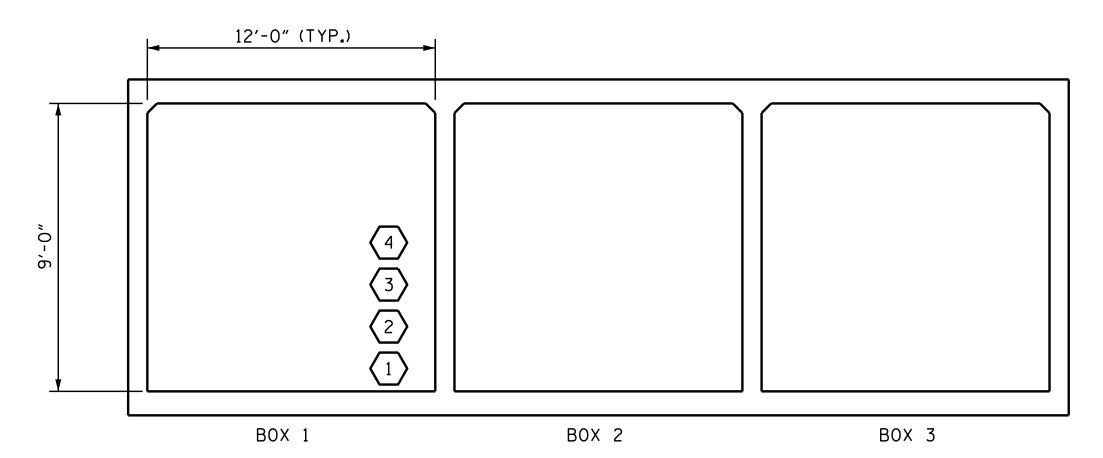
DEPARTMENT OF TRANSPORTATION

SHEET NO **REVISIONS** NO. DATE: C-1 DATE: BY: BY: DOCUMENT NOT CONSIDERED TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED

BASE HIGH WATER ELEVATION

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

									STRENGTH	I LIM	IT ST	ATE				
										MOMENT				SHEAR		
ΓΕνει		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.37		1.75	1.42	1	TOP SLAB	5.70	1.37	1	BOTTOM SLAB	11.67	
DESIGN		HL-93 (OPERATING)	N/A		1.78		1.35	1.84	1	TOP SLAB	5.70	1.78	1	BOTTOM SLAB	11.67	
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.37	49.35	1.75	1.93	1	TOP SLAB	5.38	1.37	1	BOTTOM SLAB	11.67	
		HS-20 (OPERATING)	36.000		1.78	63.98	1.35	2.50	1	TOP SLAB	5.38	1.78	1	BOTTOM SLAB	11.67	
		SNSH	13.500		3 . 33	44.91	1.40	3 . 51	1	TOP SLAB	5.38	3.33	1	TOP SLAB	11.56	
	1.1	SNGARBS2	20.000		3.02	60.46	1.40	3 . 29	1	TOP SLAB	5.38	3.02	1	TOP SLAB	11.56	
	VEHICLE ;V)	SNAGRIS2	22.000		2.79	61.28	1.40	3 . 19	1	BOT. CORNER WALL	9.73	2.79	1	BOTTOM SLAB	11.67	
	VEH.	SNCOTTS3	27.250		1.78	48.57	1.40	1.78	1	TOP SLAB	5.38	1.93	1	TOP SLAB	11.56	
	(/)	SNAGGRS4	34.930		1.77	61.82	1.40	2.10	1	TOP SLAB	5.38	1.77	1	BOTTOM SLAB	11.67	
	SINGLE	SNS5A	35.550		1.75	62.27	1.40	1.97	1	TOP SLAB	5.70	1.75	1	BOTTOM SLAB	11.67	
	0)	SNS6A	39.950		1.56	62.41	1.40	1.96	1	TOP SLAB	5.70	1.56	1	BOTTOM SLAB	11.67	
LEGAL		SNS7B	42.000		1.48	62.14	1.40	2.03	1	TOP SLAB	5.38	1.48	1	BOTTOM SLAB	11.67	
LOAD RATING	ER	TNAGRIT3	33.000		1.87	61.85	1.40	2.70	1	BOT. CORNER WALL	9.73	1.87	1	BOTTOM SLAB	11.67	
	SEMI-TRAILER T)	TNT4A	33.080		1.88	62.06	1.40	2.12	1	TOP SLAB	5.38	1.88	1	BOTTOM SLAB	11.67	
	II-II	TNT6A	41.600		1.49	62.03	1.40	2.13	1	TOP SLAB	5.38	1.49	1	BOTTOM SLAB	11.67	
	L (2)	TNT7A	42.000		1.52	63.69	1.40	2.22	2	BOTTOM SLAB	0.95	1.52	1	BOTTOM SLAB	11.67	
	TOR (TT	TNT7B	42.000		1.52	63 . 95	1.40	2.02	1	TOP SLAB	5.38	1.52	1	BOTTOM SLAB	11.67	
	TRAC	TNAGRIT4	43.000		1.45	62.48	1.40	2.03	1	TOP SLAB	5.38	1.45	1	BOTTOM SLAB	11.67	
		TNAGT5A	45.000		1.39	62 . 65	1.40	2.03	2	BOTTOM SLAB	11.72	1.39	1	BOTTOM SLAB	11.67	
	TRUCK	TNAGT5B	45.000	3	1.39	62.64	1.40	2.04	1	BOTTOM SLAB	11.72	1.39	1	BOTTOM SLAB	11.67	
EMERGENC'	Y	EV2	28.750		2.30	66.15	1.30	2.48	1	TOP SLAB	5.38	2.30	1	BOTTOM SLAB	11.67	
VEHICLE (EV3	43.000	4	1.54	66.21	1.30	1.54	1	TOP SLAB	5.70	1.54	1	BOTTOM SLAB	11.67	



LRFR SUMMARY

DRAWN BY: A. SORSENGINH
CHECKED BY: M. G. SHAIKH
DESIGN ENGINEER OF RECORD: A. YASMEEN

DATE: 08/2021
DATE: 08/2021 (LOOKING DOWNSTREAM)

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

DESIGN EGAD NATING TACTOR								
LOAD TYPE	MAX FACTOR	MIN FACTOR						
DC	1.25 0.90							
DW	1.50	0.65						
EV	1.30	0.90						
EH	1.35	0.90						
ES	1.35	0.90						
LS	1.75							
WA	1.00							

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

4 EMERGENCY VEH. LOAD RATING ** ** SEE CHART FOR VEHICLE TYPE

> PROJECT NO. BR-0035 MOORE COUNTY STATION: 23+12.00 -L-

SHEET 2 OF 8

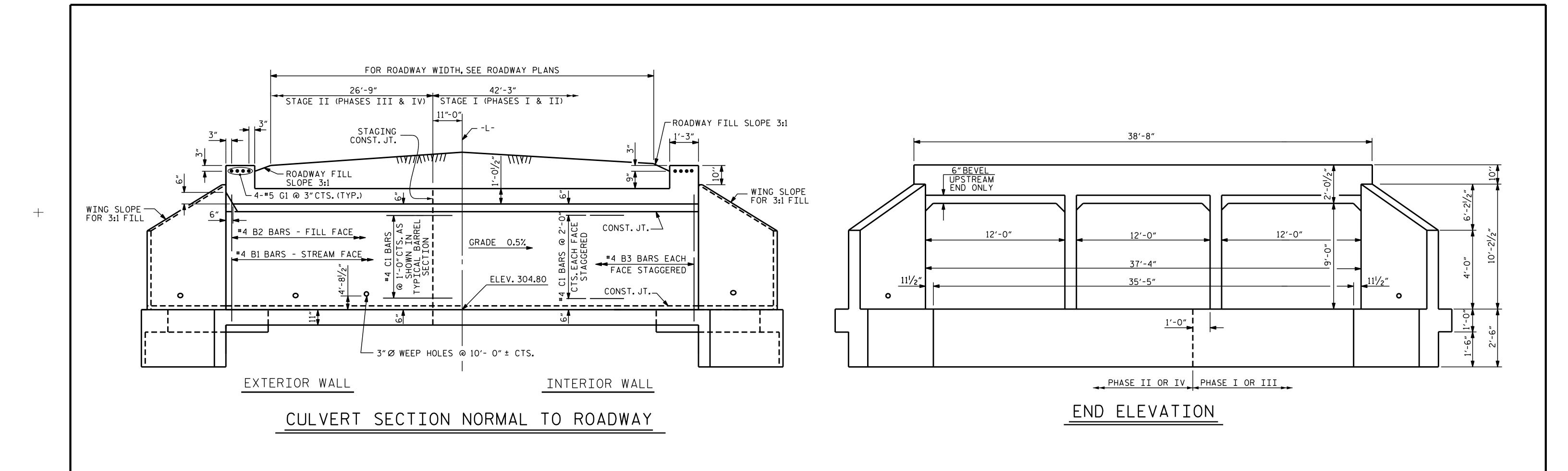


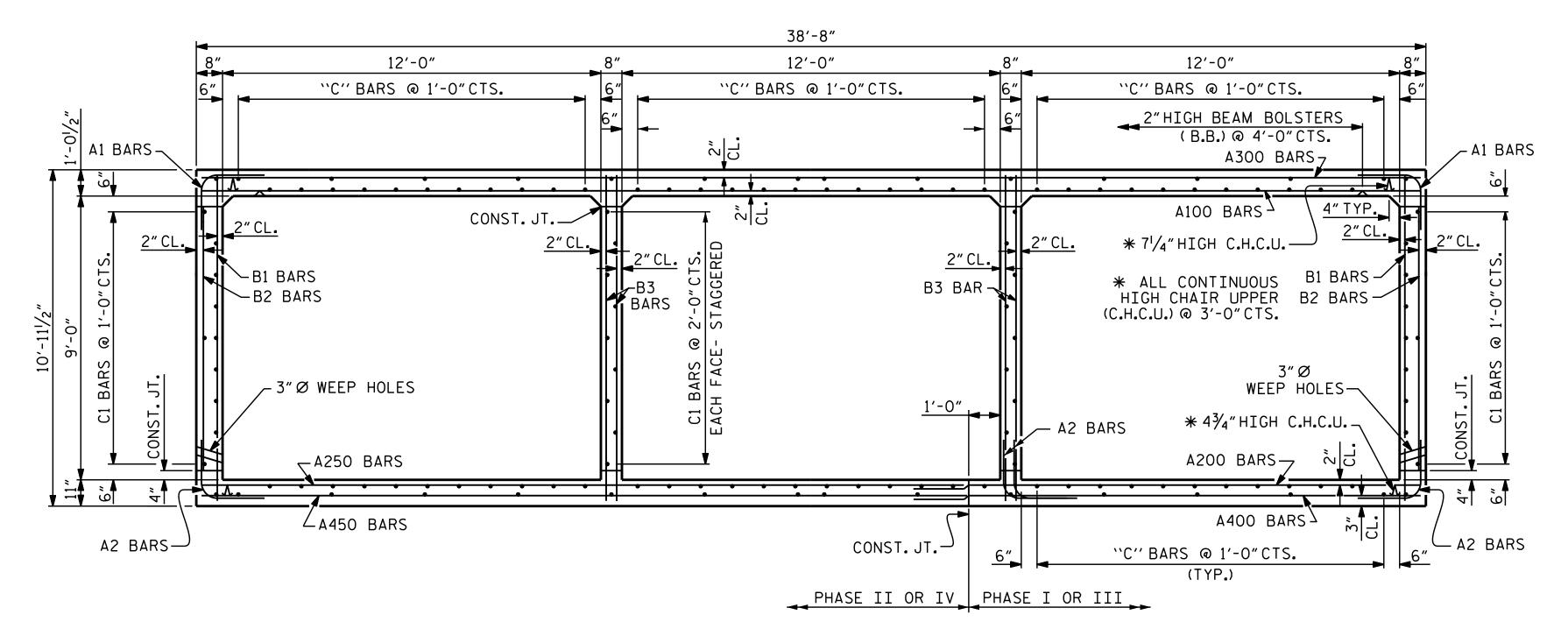
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)

REVISIONS SHEET NO C-2 DATE: NO. BY: DOCUMENT NOT CONSIDERED NOT FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 8





RIGHT ANGLE SECTION OF BARREL

THERE ARE 136 "C" BARS IN SECTION OF BARREL.

PROJECT NO. <u>BR-0035</u>

<u>MOORE</u> county

STATION: <u>23+12.00</u> -L-

SHEET 3 OF 8

SEAL 031583 -

DEPARTMENT OF TRANSPORTATION
RALEIGH

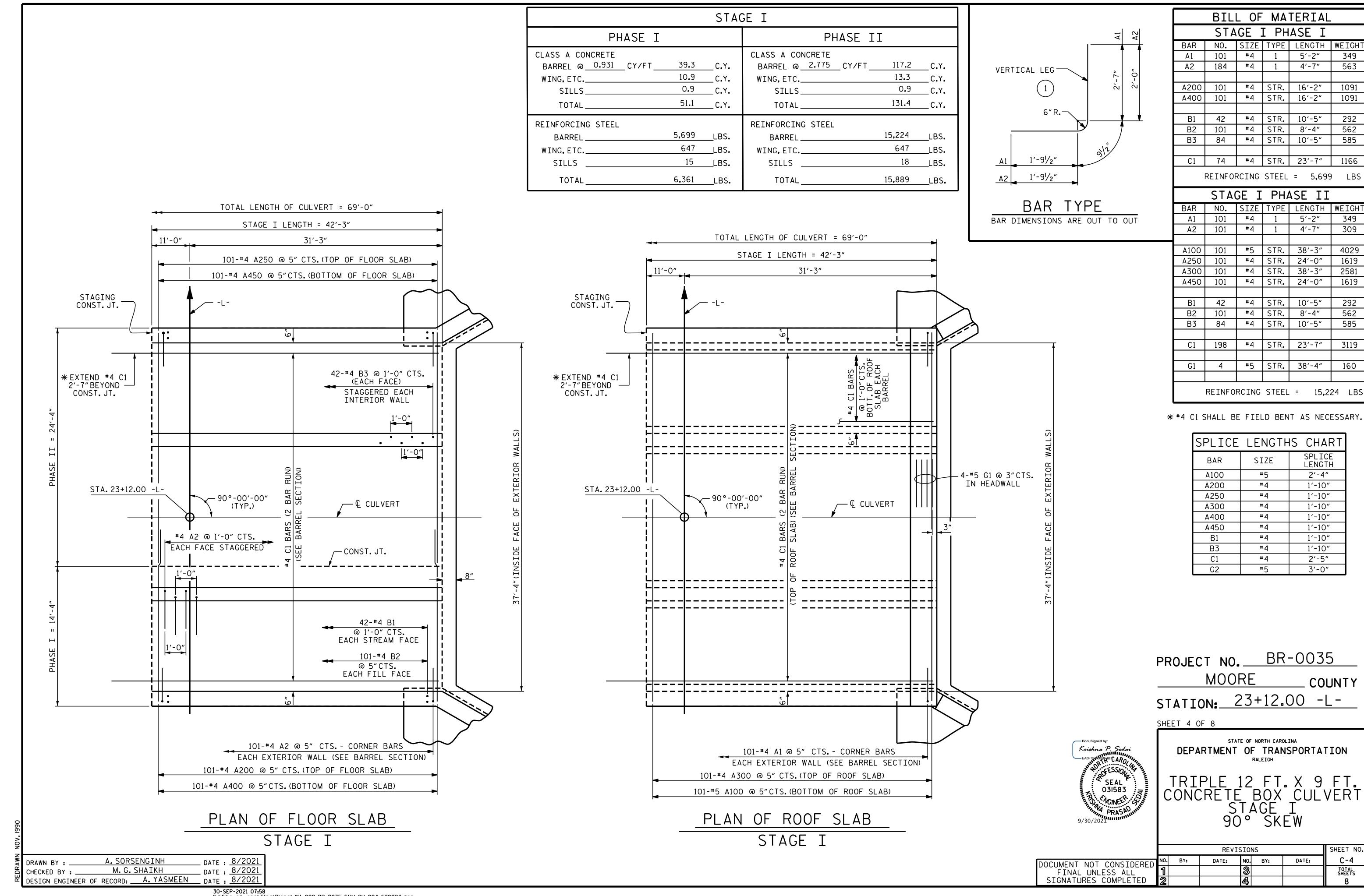
TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT 90° SKEW

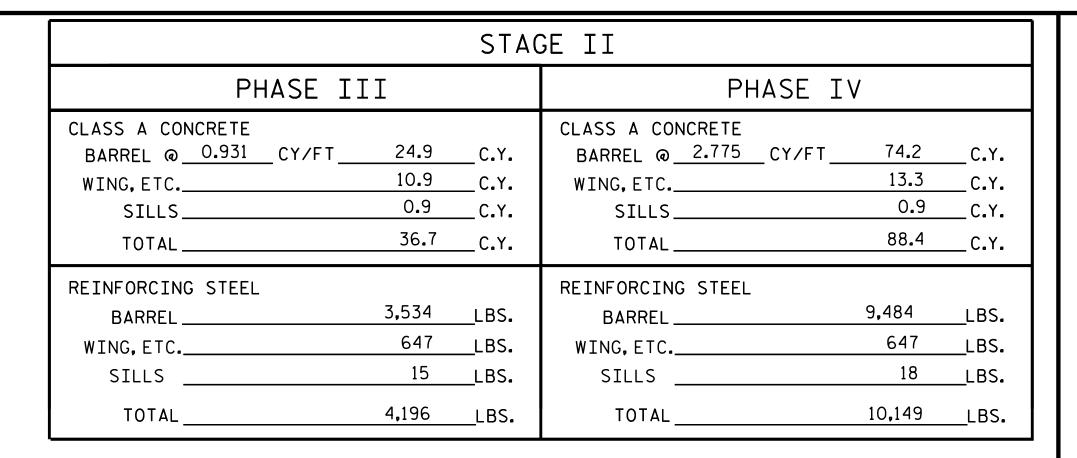
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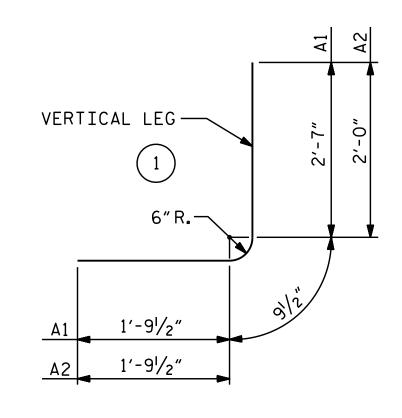
DRAWN BY: A. SORSENGINH
CHECKED BY: M. G. SHAIKH
DESIGN ENGINEER OF RECORD: A. YASMEEN

DATE: 8/2021
DATE: 8/2021

30-SEP-2021 07:58 K:\Structures\FinalPlans\411_007_BR-0035_SMU_CU_003_620024.dgn ksedai







	BAR	ΤΥ	PE		
BAR	DIMENSIONS	ARE	OUT	ТО	OUT

A400	64	#4	STR.	16'-2"	691
B1	27	#4	STR.	10'-5"	188
B2	64	#4	STR.	8'-4"	356
В3	54	#4	STR.	10'-5"	376
C2	37	#4	STR.	26′-5″	653
	REINFO	RCING	STEEL	= 3,534	LBS
	STAG	EI	I PH	ASE I\	/
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	64	#4	1	5′-2″	221
Α2	64	#4	1	4'-7"	196
A100	64	#5	STR.	38′-3″	2553
A250	64	#4	STR.	24'-0"	1026
A300	64	#4	STR.	38'-3"	1635
A450	64	#4	STR.	24'-0"	1026
B1	27	#4	STR.	10′-5″	188
B2	64	#4	STR.	8'-4"	356
В3	54	#4	STR.	10′-5″	376
C2	99	#4	STR.	26′-5″	1747

BILL OF MATERIAL

#4 | STR. |

64

SIZE TYPE LENGTH WEIGH

117 #4 1 4'-7" 358

5′-2"

REINFORCING STEEL = 9,484 LBS

#5 STR. 38'-4" 160

STAGE II LENGTH = 26'-9" STAGING CONST.JT. 4-#5 G1 @ 3"CTS. IN HEADWALL STA. 23+12.00 -L-__90°-00′-00″ (TYP.) 64-#4 A1 @ 5" CTS. - CORNER BARS EACH EXTERIOR WALL (SEE BARREL SECTION) 64-#4 A300 @ 5" CTS.(TOP OF ROOF SLAB) 64-#5 A100 @ 5" CTS.(BOTTOM OF ROOF SLAB)

TOTAL LENGTH OF CULVERT = 69'-0"

PLAN OF ROOF SLAB
STAGE II

EAGET STATE OF ESSON

SEAL

031583

9/30/2021

PROJECT NO. BR-0035

MOORE COUNTY

STATION: 23+12.00 -L-

SHEET 5 OF 8

DEPARTMENT OF TRANSPORTATION
RALEIGH

TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT STAGE II 90° SKEW

REVISIONSSHEET NODOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETEDNO. BY: DATE: NO. BY: DATE: C-513TOTAL SHEETS248

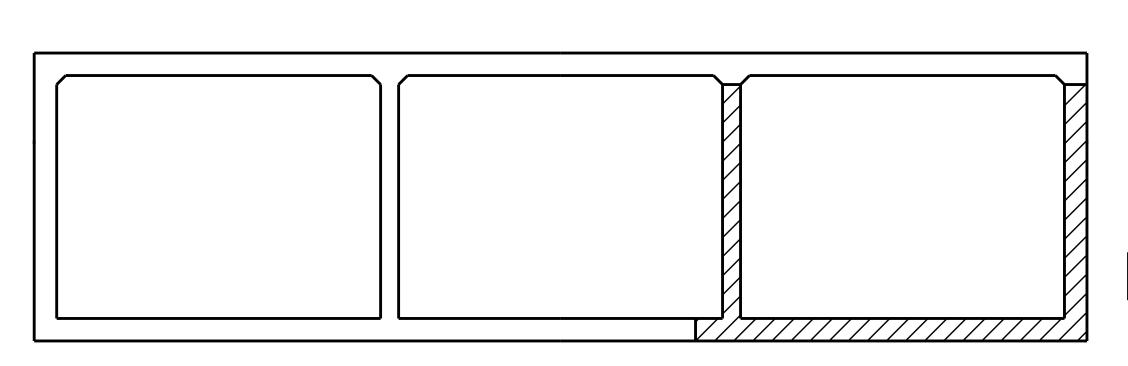
STAGE II LENGTH = 26'-9" 11'-0" 64-#4 A250 @ 5" CTS.(TOP OF FLOOR SLAB) 64-#4 A450 @ 5" CTS.(BOTTOM OF FLOOR SLAB) STAGING CONST. JT. 27-#4 B3 @ 1'-0" CTS. (EACH FACE) STAGGERED EACH INTERIOR WALL 1'-0" STA. 23+12.00 -L-90°-00′-00″__ (TYP.) CONST. JT. 27-#4 B1 @ 1'-0" CTS. EACH STREAM FACE #4 A2 @ 1'-0" CTS. 64-#4 B2 EACH FACE STAGGERED @ 5″CTS. EACH FILL FACE 64-#4 A2 @ 5" CTS. - CORNER BARS EACH EXTERIOR WALL (SEE BARREL SECTION) 64-#4 A200 @ 5" CTS.(TOP OF FLOOR SLAB) 64-#4 A400 @ 5" CTS.(BOTTOM OF FLOOR SLAB)

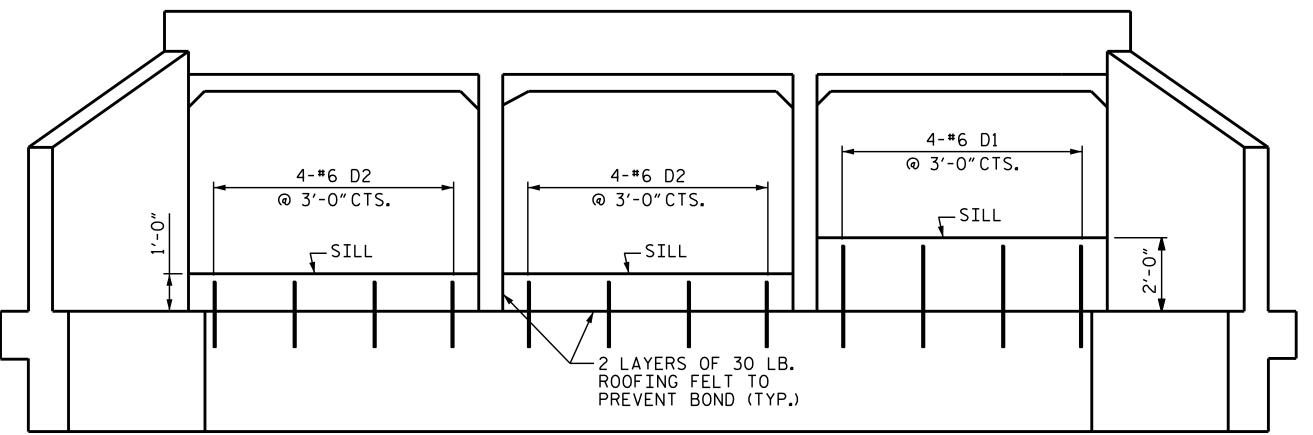
TOTAL LENGTH OF CULVERT = 69'-0"

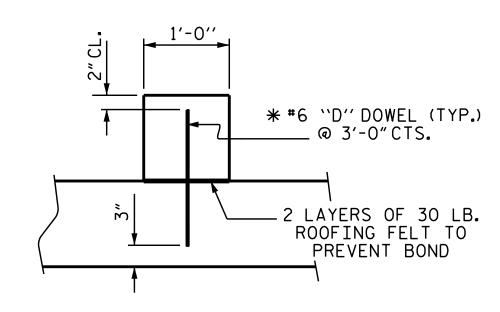
PLAN OF FLOOR SLAB
STAGE II

DRAWN BY: A. SORSENGINH
CHECKED BY: M. G. SHAIKH
DESIGN ENGINEER OF RECORD: A. YASMEEN

DATE: 8/2021
DATE: 8/2021







SECTION THROUGH SILL

* DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOAT FINISHED.

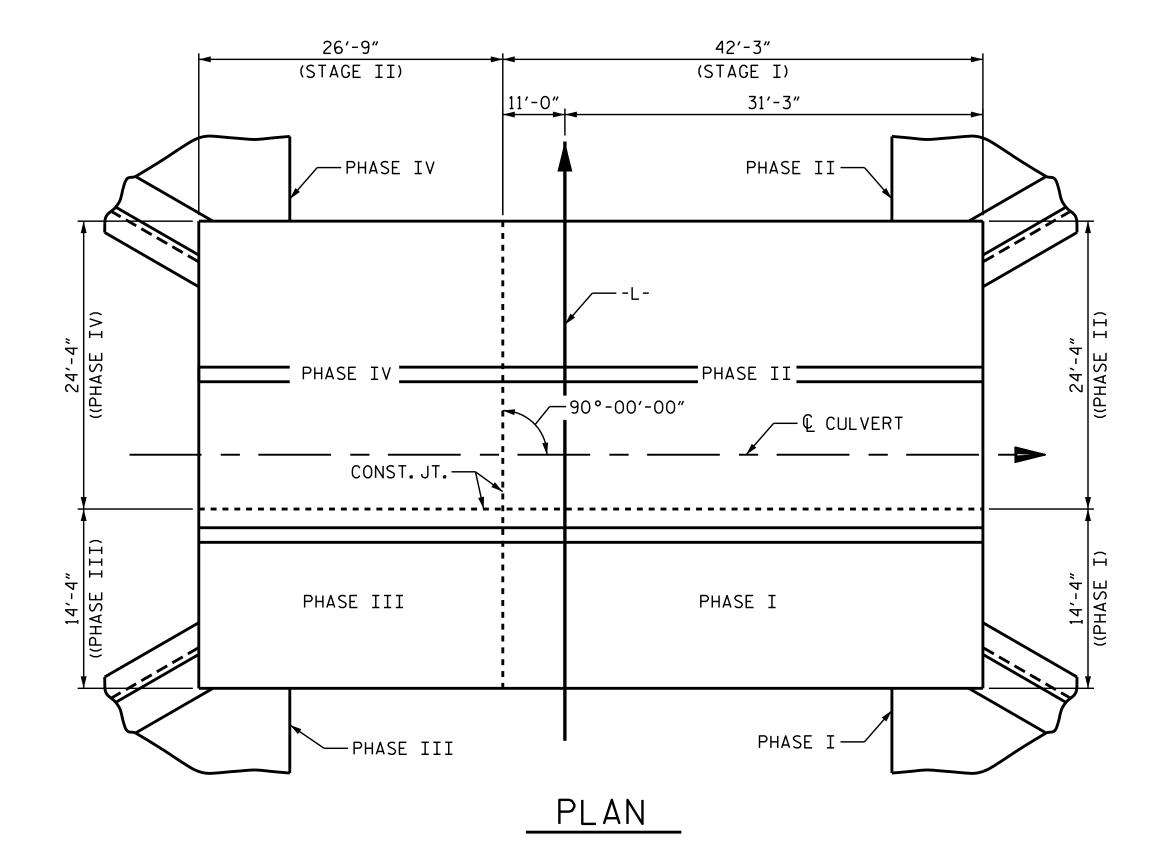
ELEVATION

(LOOKING DOWNSTREAM)



PHASE I OR PHASE III CONSTRUCTION

PHASE II OR PHASE IV CONSTRUCTION



CONSTRUCTION SEQUENCE

BACKFILL WITH CLASS B RIP RAP TO -2'-0" SILL 2'-0" SILL — SILL HEIGHT (TYP.) BACKFILL WITH NATIVE MATERIAL TO SILL HEIGHT 1'-0" SILL ----— 1'-0" SILL 3′-0″ 3'-0" (TYP.) (TYP.) 1'-0" SILL — ___1'-0" SILL BACKFILL WITH NATIVE MATERIAL TO

ELEVATION

CULVERT SILL DETAILS

PLAN

NOTES

NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED OR FLOODPLAIN AT THE PROJECT SITE DURING CONSTRUCTION. ONLY MATERIAL THAT IS EXCAVATED FROM THE STREAM BED MAY BE USED TO LINE THE LOW FLOW CULVERT BARREL. RIP RAP MAY BE USED TO SUPPLEMENT NATIVE MATERIAL IN THE HIGH FLOW BARREL. IF RIP RAP IS USED TO LINE THE HIGH FLOW CULVERT BARREL, NATIVE MATERIAL SHOULD BE PLACED ON TOP TO FILL VOIDS AND PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS CONDITIONS.

THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL OR SUPPLEMENTAL MATERIAL AS SHOWN ON THE PLANS SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION.

BILL OF MATERIALS								
	PHASE I							
	BAR NO. SIZE TYPE LENGTH	WEIGHT						
	D1 4 #6 STR 2'-6"	15						
	REINFORCING STEEL	15 LBS.						
STAGE I	CLASS A CONCRETE	0.9 CY						
	PHASE II							
	BAR NO. SIZE TYPE LENGTH	WEIGHT						
	D2 8 #6 STR 1'-6"	18						
	REINFORCING STEEL	18 LBS.						
	CLASS A CONCRETE	0.9 CY						
	PHASE III							
	BAR NO. SIZE TYPE LENGTH							
	D1 4 #6 STR 2'-6"	15						
	REINFORCING STEEL	15 LBS.						
STAGE II	CLASS A CONCRETE	0.9 CY						
	PHASE IV							
	BAR NO. SIZE TYPE LENGTH	WEIGHT						
	D2 8 #6 STR 1'-6"	18						
	REINFORCING STEEL	18 LBS.						
	CLASS A CONCRETE	0.9 CY						

PROJECT NO. BR-0035 MOORE COUNTY

STATION: 23+12.00 -L-

TO AROL * CESSION ! SEAL 031583 NGINEER S

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT 90° SKEW

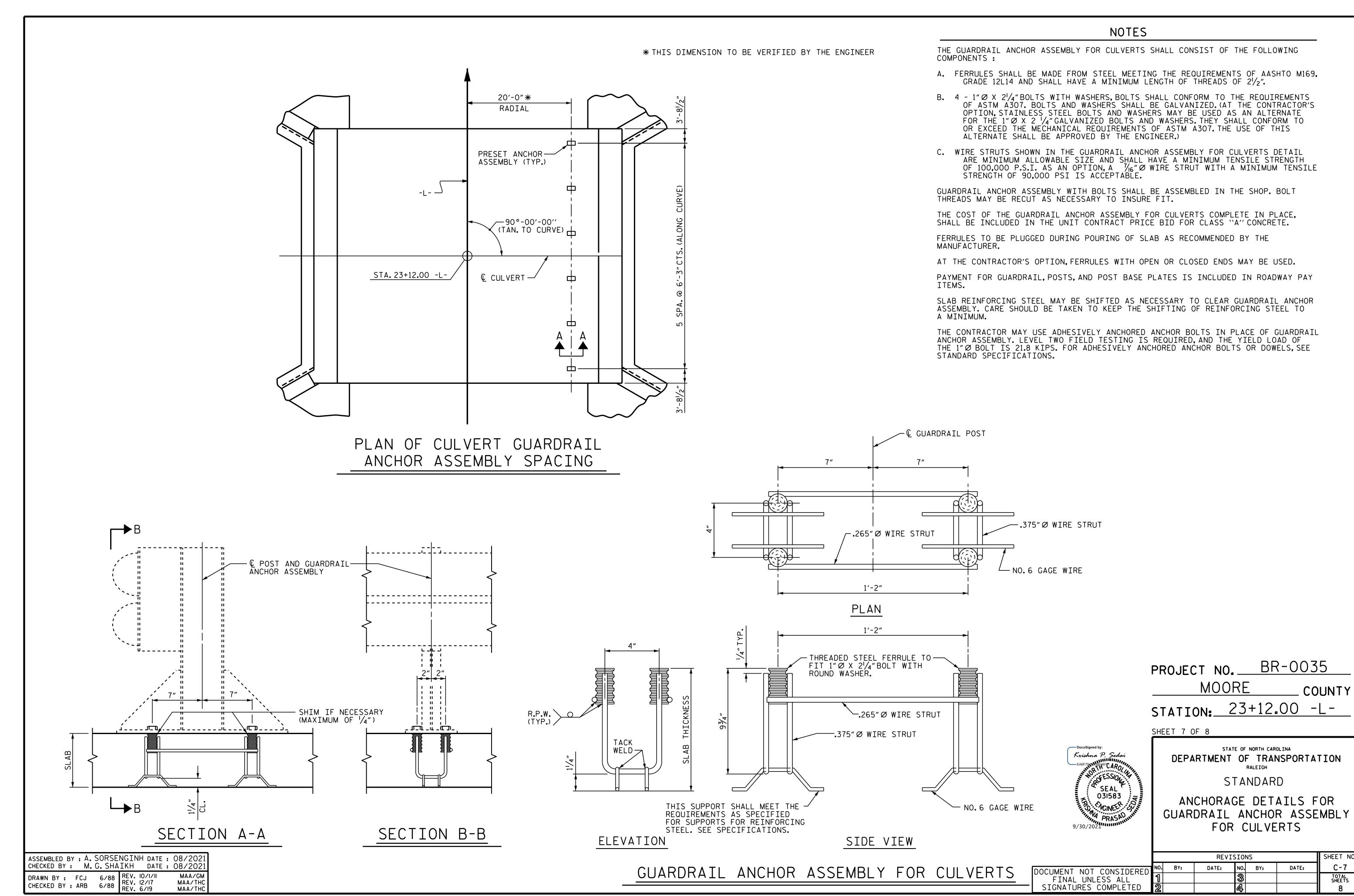
SHEET 6 OF 8

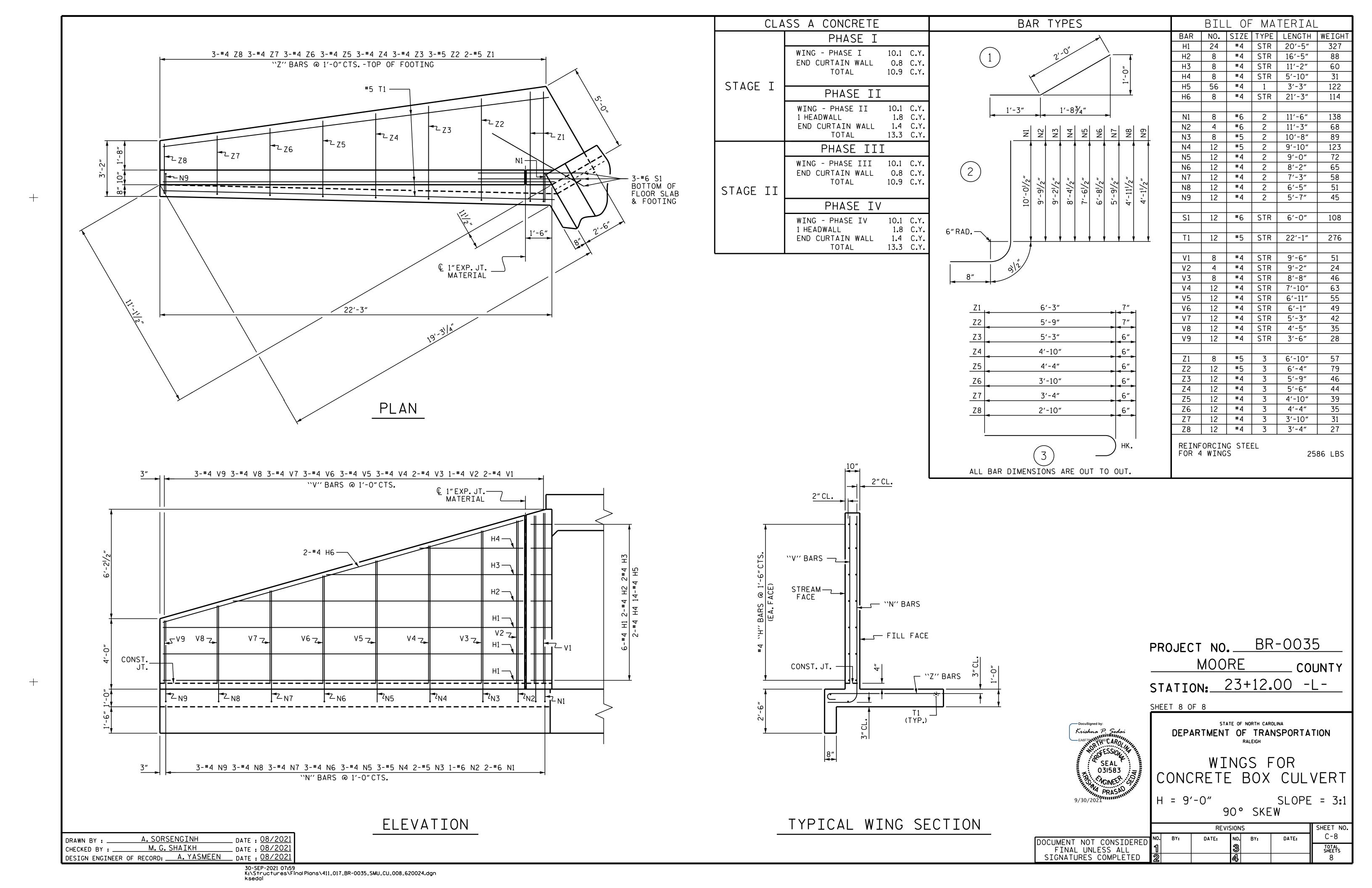
SHEET NO **REVISIONS** NO. BY: C-6 DATE: DATE: DOCUMENT NOT CONSIDERED -FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

STREAM BED MATERIAL SHOULD BE PLACED LEVEL WITH THE TOP OF THE SILLS.

A. SORSENGINH DRAWN BY : _ DATE : 08/2021 M.G.SHAIKH CHECKED BY : DESIGN ENGINEER OF RECORD: A. YASMEEN DATE: 08/2021





STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS - - - - - - - - - - - A.A.S.H.T.O. (CURRENT) LIVE LOAD ----- SEE PLANS IMPACT ALLOWANCE - - - - - - - - - SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24,000 LBS. PER SQ. IN. CONCRETE IN SHEAR -------- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ---- 375 LBS. PER SQ. IN. EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH