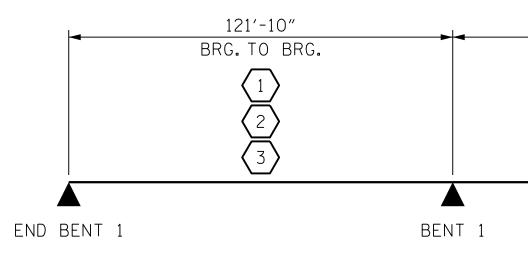
		LOAD AN	D RE	SIST	ANCE	FAC	TOR	RAT	ING	(LRF	R) SL	JMMA	RY F	OR F	PRES	TRES	SED	CON	CRET	E GI	RDEF	RS	
										STRE	NGTH	I LIM	IT ST	ATE				SE	RVICE	III	LIMI	T STA	,ΤE
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{ll})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVE-LOAD Factors (Y _{ll})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)
		HL-93 (INVENTORY)	N⁄A	$\langle 1 \rangle$	1.17		1.75	0.730	1.53	А	I	60.9	0.950	1.19	А	Ι	115.6	0.80	0.730	1.17	А	I	60.9
DESIGN LOAD		HL-93 (OPERATING)	N⁄A		1.70		1.35	0.730	1.98	А	I	60.9	0.950	1.70	А	I	115.6	NZA					
RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	1.72	61.92	1.75	0.730	2.24	А	I	60.9	0.950	2.06	А	Ι	115.6	0.80	0.730	1.72	А	I	60.9
		HS-20 (OPERATING)	36.000		2.78	100.08	1.35	0.730	2.91	А	I	60.9	0.950	2.78	А	Ι	110.2	NZA					
		SNSH	13.500		4.21	56.84	1.40	0.730	6.86	А	I	60.9	0.950	7.18	А	Ι	110.2	0.80	0.730	4.21	А	I	60.9
	ш	SNGARBS2	20.000		2.99	59.80	1.40	0.730	4.87	А	I	60.9	0.950	4.93	А	Ι	110.2	0.80	0.730	2.99	А	I	60.9
	ICL	SNAGRIS2	22.000		2.77	60.94	1.40	0.730	4.51	А	I	60.9	0.950	4.52	А	Ι	110.2	0.80	0.730	2.77	А	I	60.9
	<pre>// CHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHH</pre>	SNCOTTS3	27.250		2.08	56.68	1.40	0.730	3.39	А	I	60.9	0.950	3.46	А	Ι	110.2	0.80	0.730	2.08	А	I	60.9
	SLE (S	SNAGGRS4	34.925		1.68	58.67	1.40	0.730	2.74	А	I	60.9	0.950	2.67	А	Ι	110.2	0.80	0.730	1.68	А	I	60.9
	SINGL	SNS5A	35.550		1.65	58.66	1.40	0.730	2.69	А	I	60.9	0.950	2.65	А	Ι	110.2	0.80	0.730	1.65	А	I	60.9
		SNS6A	39.950		1.50	59.93	1.40	0.730	2.44	А	I	60.9	0.950	2.38	А	I	110.2	0.80	0.730	1.50	А	I	60.9
LEGAL LOAD		SNS7B	42.000		1.42	59.64	1.40	0.730	2.32	А	I	60.9	0.950	2.27	А	I	115.6	0.80	0.730	1.42	А	I	60.9
RATING	L ER	TNAGRIT3	33.000		1.82	60.06	1.40	0.730	2.97	А	I	60.9	0.950	2.86	А	Ι	110.2	0.80	0.730	1.82	А	I	60.9
	RAII	TNT4A	33.075		1.82	60.20	1.40	0.730	2.97	А	I	60.9	0.950	2.82	А	I	110.2	0.80	0.730	1.82	А	I	60.9
	1-IM	TNT6A	41.600		1.47	61.15	1.40	0.730	2.39	А	I	60.9	0.950	2.34	А	I	115.6	0.80	0.730	1.47	А	I	60.9
	ST)	TNT7A	42.000		1.47	61.74	1.40	0.730	2.39	А	I	60.9	0.950	2.30	А	I	115.6	0.80	0.730	1.47	А	I	60.9
	CTOR (TT)	TNT7B	42.000		1.49	62.58	1.40	0.730	2.43	А	I	60.9	0.950	2.24	А	I	115.6	0.80	0.730	1.49	А	I	60.9
	TRA(TNAGRIT4	43.000		1.44	61.92	1.40	0.730	2.34	А	I	60.9	0.950	2.16	Α	I	115.6	0.80	0.730	1.44	А	I	60.9
	RUCK	TNAGT5A	45.000		1.36	61.20	1.40	0.730	2.22	Α	I	60.9	0.950	2.07	Α	I	115.6	0.80	0.730	1.36	A	I	60.9
	TRI	TNAGT5B	45.000	3	1.35	60.75	1.40	0.730	2.21	Α	I	60.9	0.950	2.08	Α	I	115.6	0.80	0.730	1.35	Α	I	60.9



LRFR SUMMARY

33BB_SMU_LR							
4/22/2020 404_009_R2233BB	DESIGNED BY: DRAWN BY:	J. BORUTA K. WHITE	DATE : <u>AUG 2019</u> DATE : <u>AUG 2019</u>				
4/22/ 404_0	CHECKED BY: DESIGN ENGINER OF RECORD:	B.LOFLIN Er J.DOUGHTY	DATE : <u>AUG 2019</u> DATE : <u>NOV 2019</u>	DRAWN BY : MAA 1/08 CHECKED BY : GM/DI 2/08	REV. II/12/08RR REV. I0/1/II REV. 12/17	MAA/GM MAA/GM MAA/THC	

100′-97⁄8″ BRG.TO BRG.

END BENT 2





DOCUMENT NOT CONSIDERED FINAL

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\rm DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

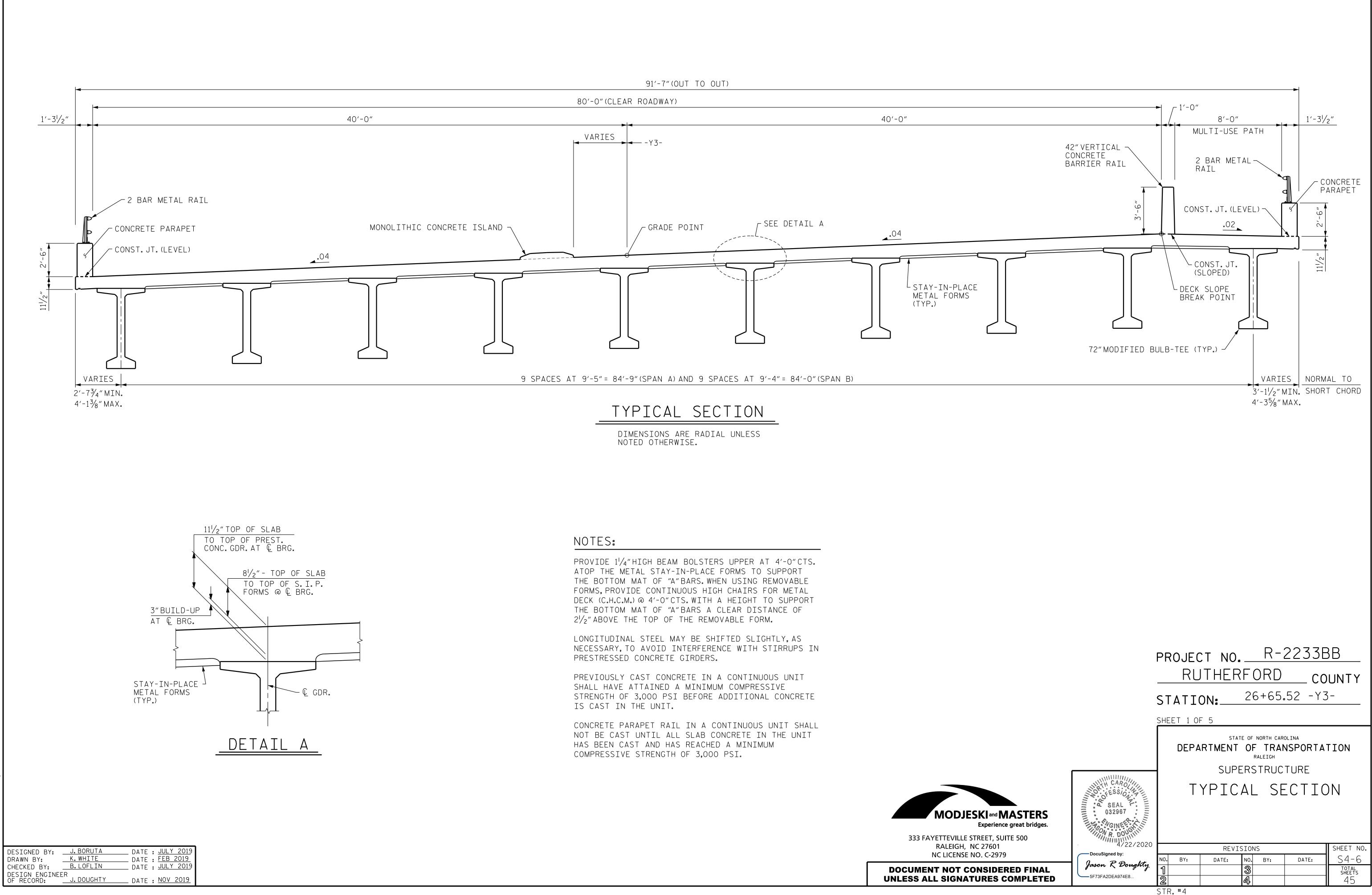
COMMENTS:

1. GIRDERS DESIGNED AS SIMPLE SPANS FOR FLEXURE. GIRDERS DESIGNED AS SIMPLE-MADE-CONTINUOUS (FOR LIVE LOAD AND SUPERIMPOSED DEAD LOAD)FOR SHEAR.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

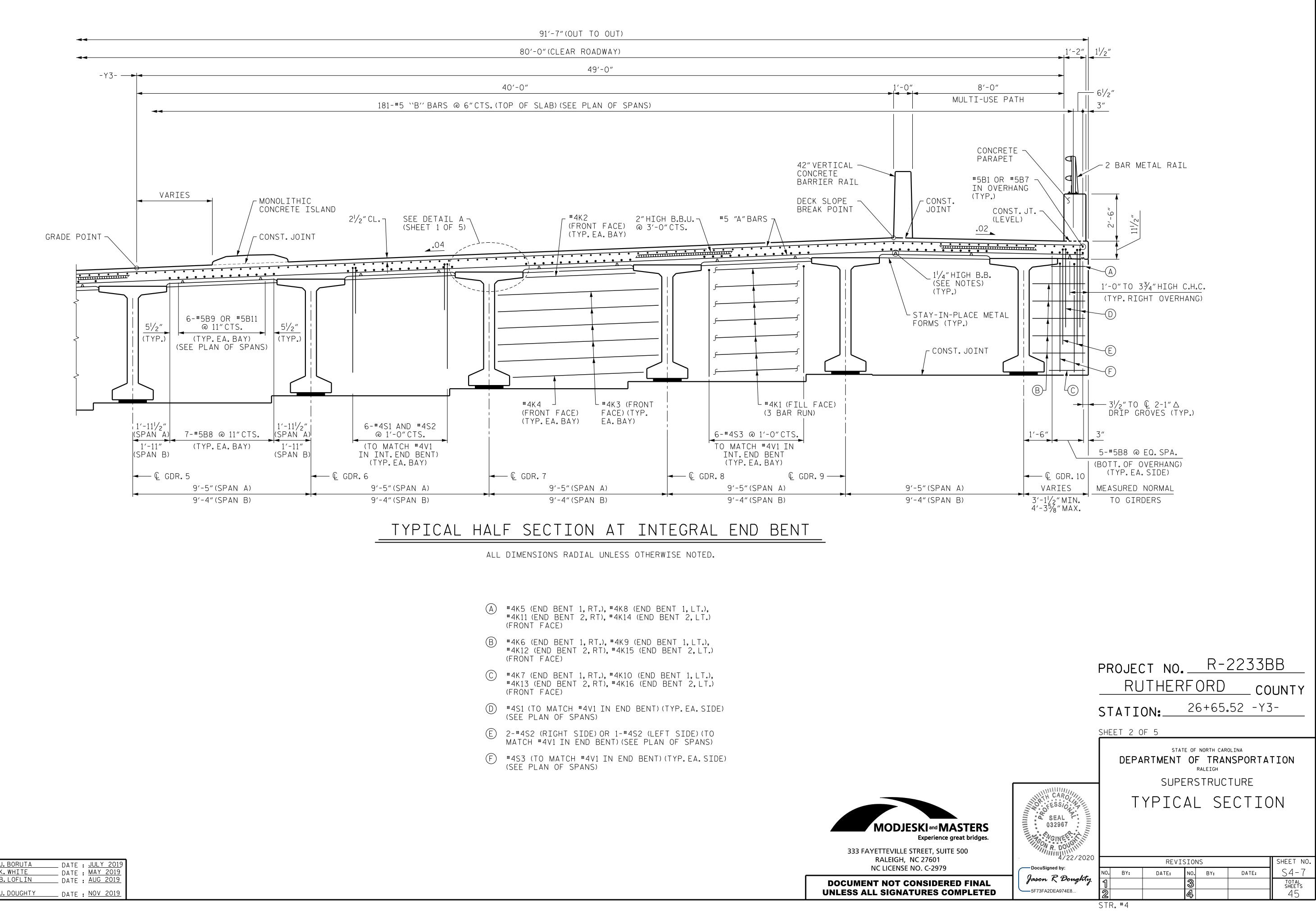
PROJECT NO. R-2233BB

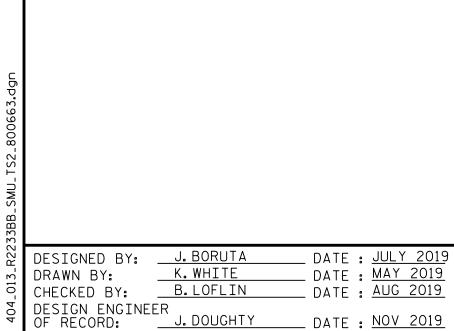
RUTHERFORD _ COUNTY 26+65**.**52 -Y3-STATION:____ STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SEAL 032967 LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS -----(NON-INTERSTATE TRAFFIC) R. DOU 4/22/2020 SHEET NO. REVISIONS ---- DocuSigned by: NO. BY: S4-5 DATE: DATE: BY: NO. Jason R Doughty total sheets 45 UNLESS ALL SIGNATURES COMPLETED STR.#4 STD.NO.LRFR1

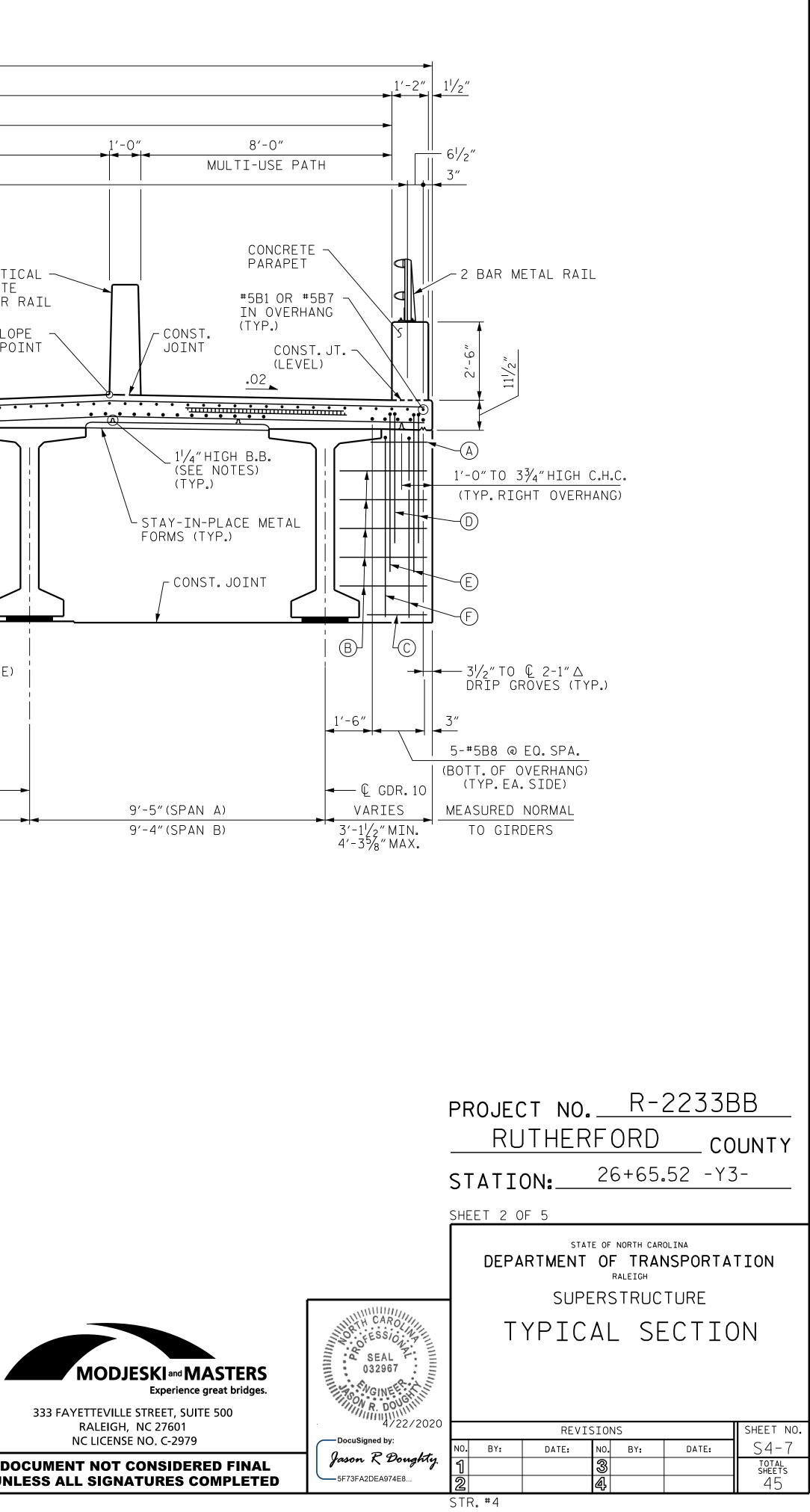


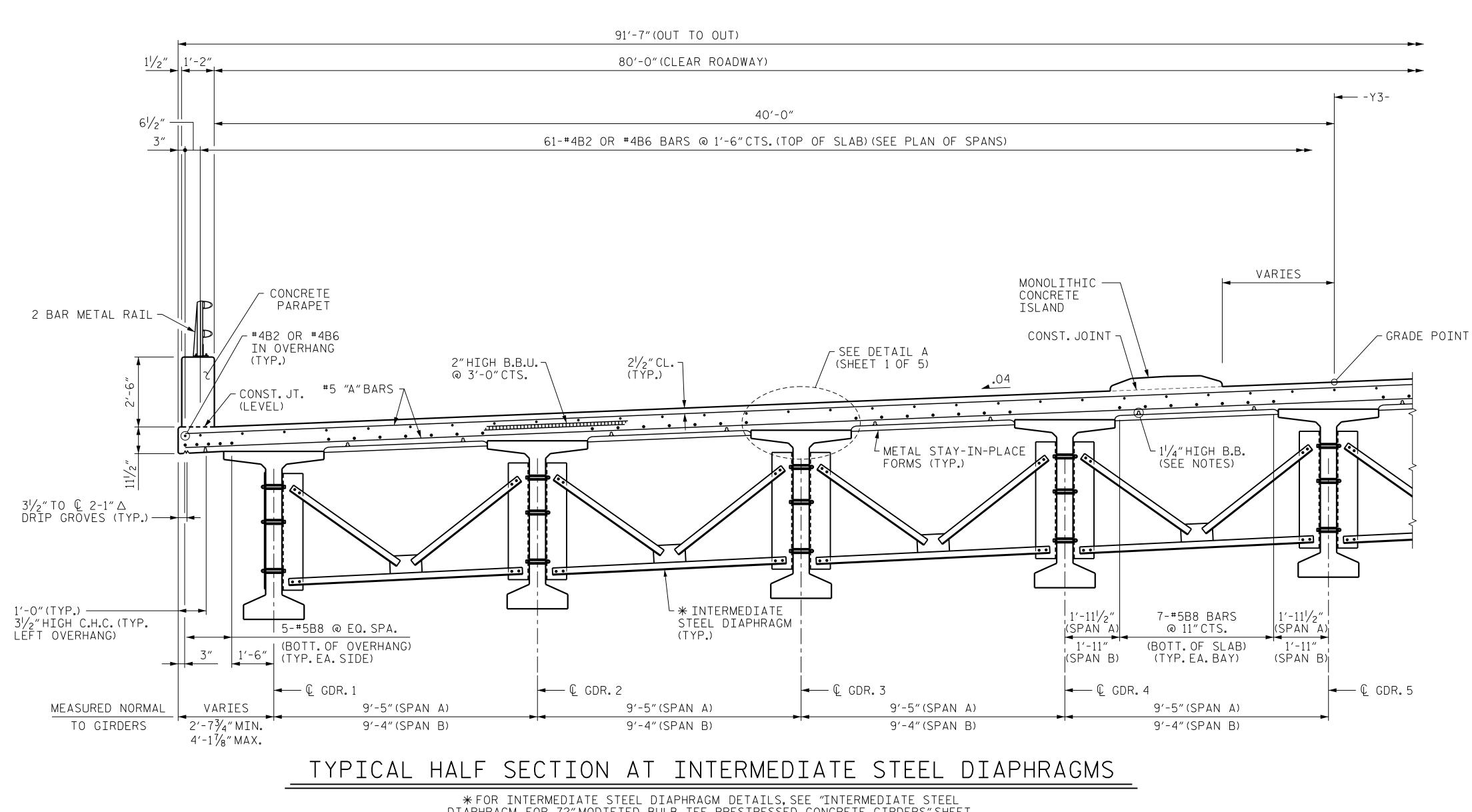
DocuSign Envelope ID: 5595FF99-7FF6-444D-89B1-5F69227A8DDB

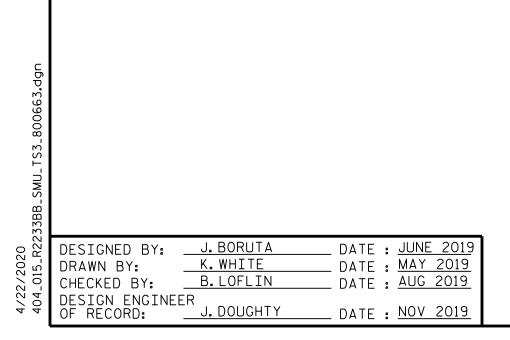
+











+

* FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGM FOR 72"MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS"SHEET.

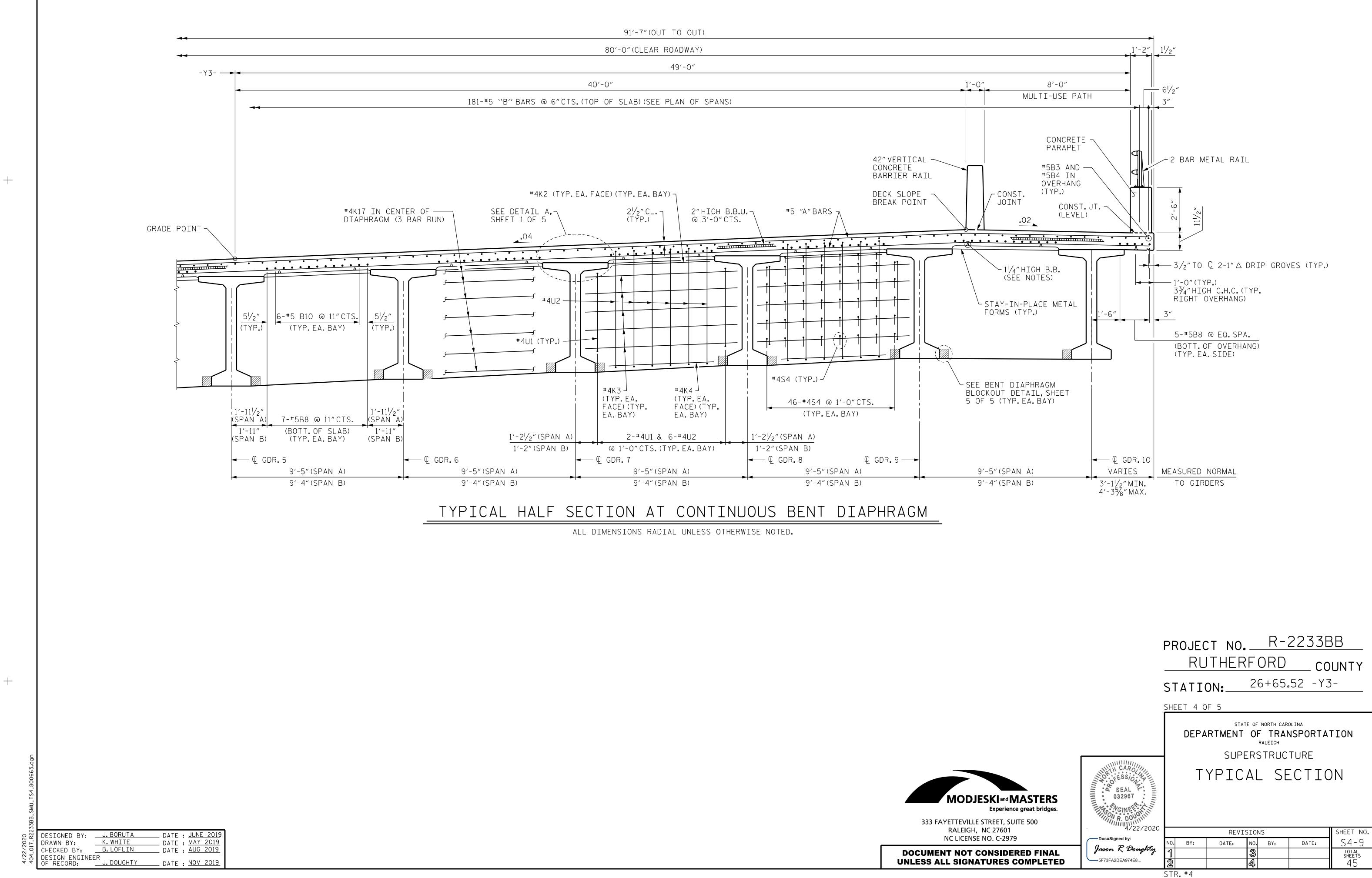
ALL DIMENSIONS RADIAL UNLESS OTHERWISE NOTED.

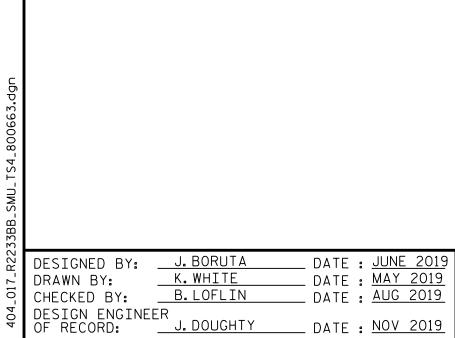


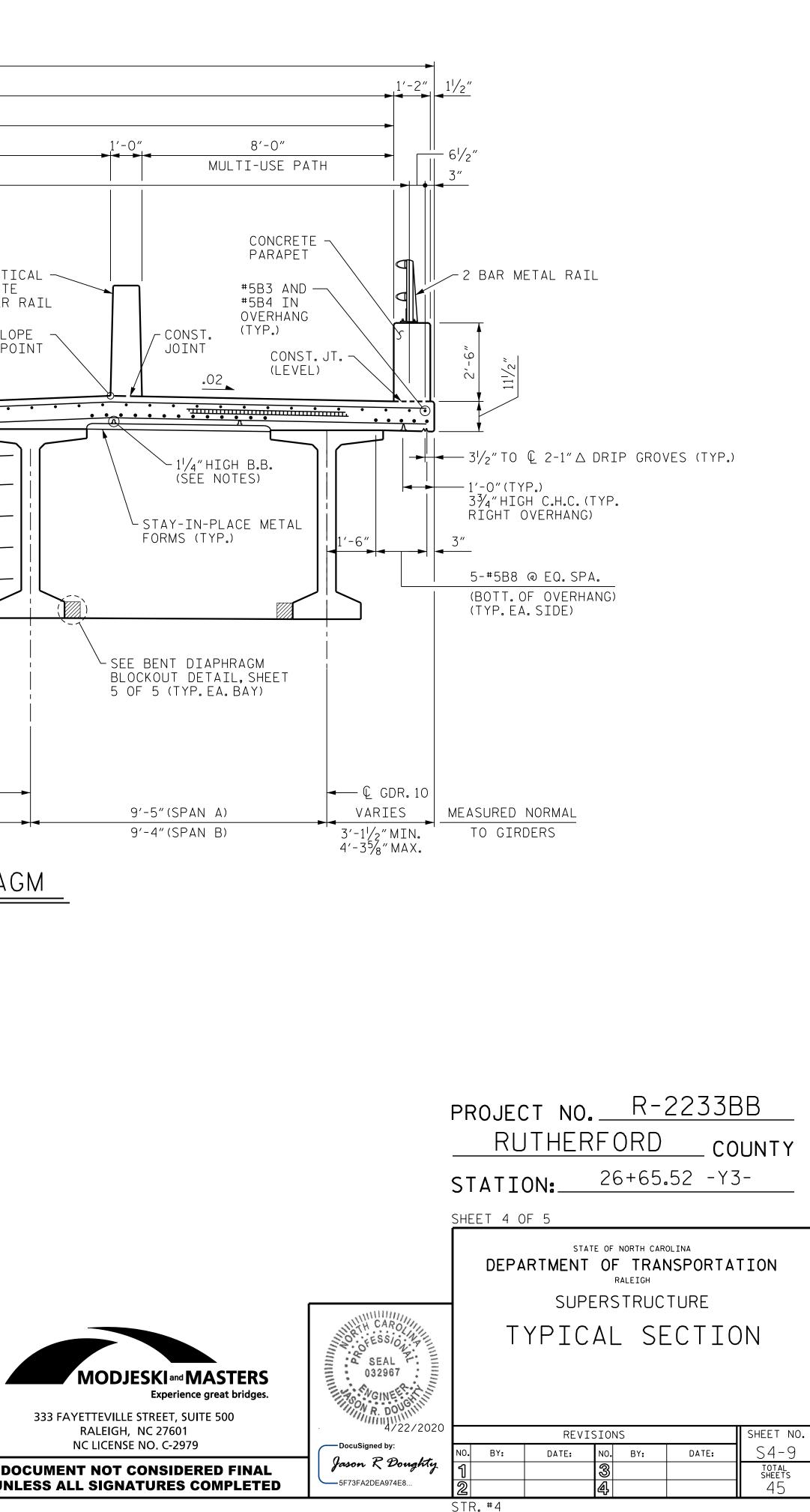
DOCUMENT NOT CONSIDER UNLESS ALL SIGNATURES CO

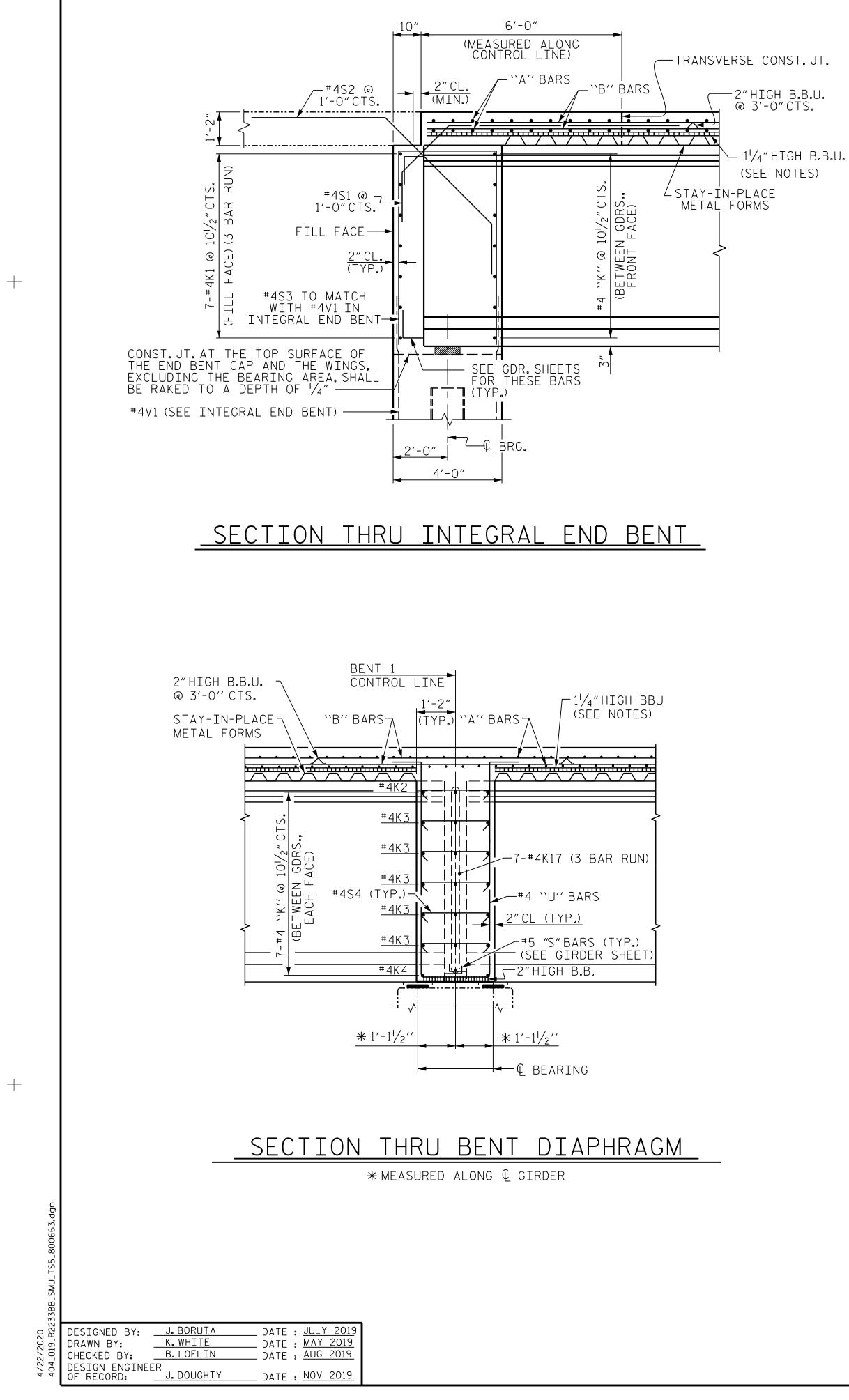
		PROJEC RU STATIC	THER	FORD		UNTY
ASTERS great bridges. TE 500	SEAL 032967		RTMENT SUPE	RALEIGH	NSPORTA	
	4/22/2020	NO. BY:	REVIS	NO. BY:	DATE:	SHEET NO. S4-8
RED FINAL OMPLETED	Jason R Doughty. 5F73FA2DEA974E8	1	DATE.	<u>।</u> अ म		TOTAL SHEETS 45

DocuSign Envelope ID: 5595FF99-7FF6-444D-89B1-5F69227A8DDB

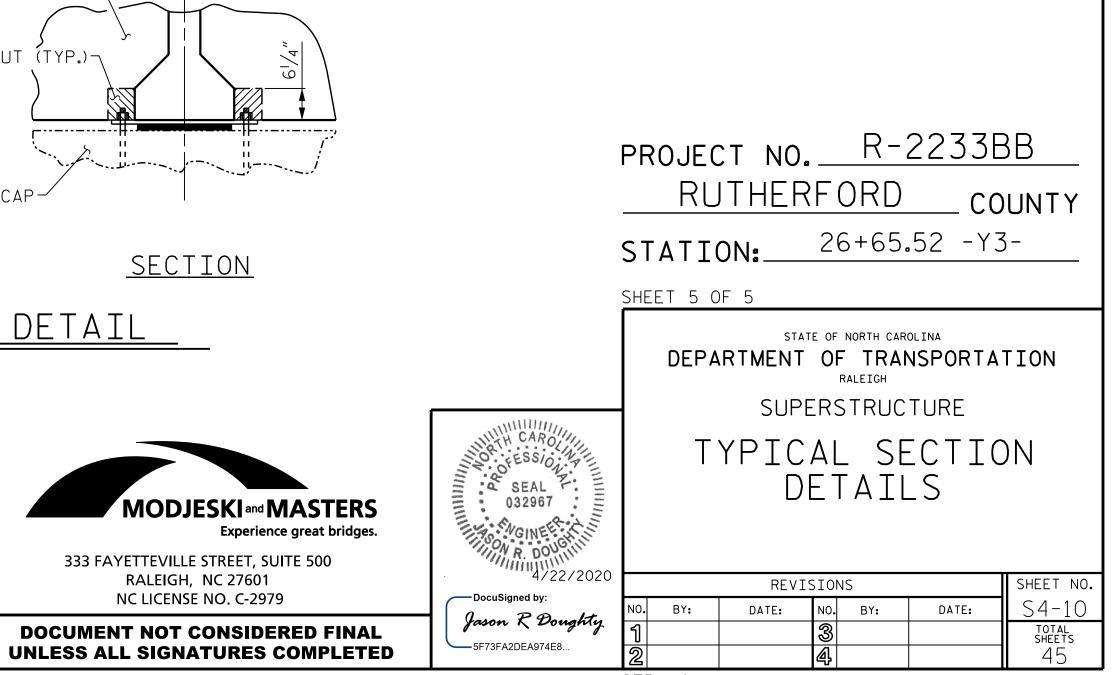




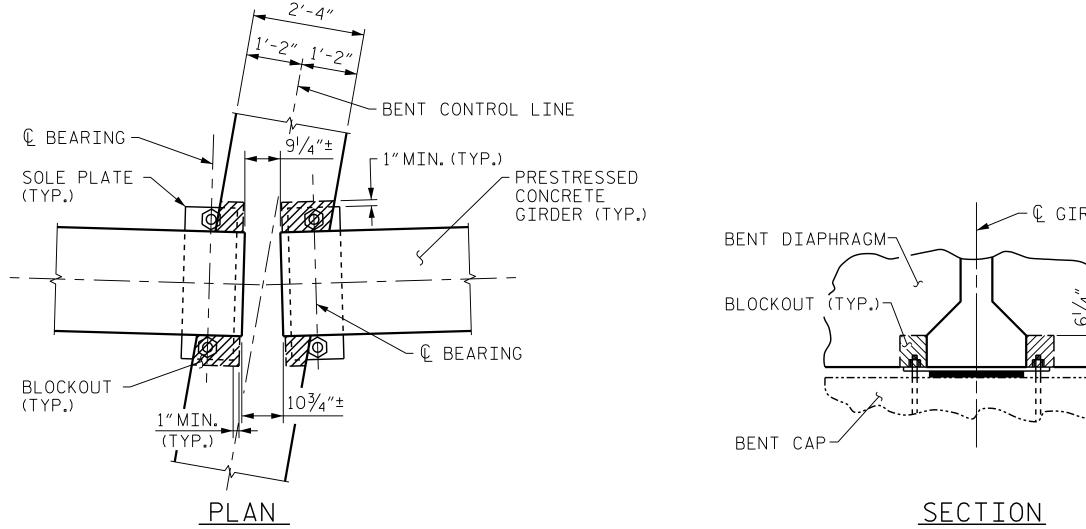




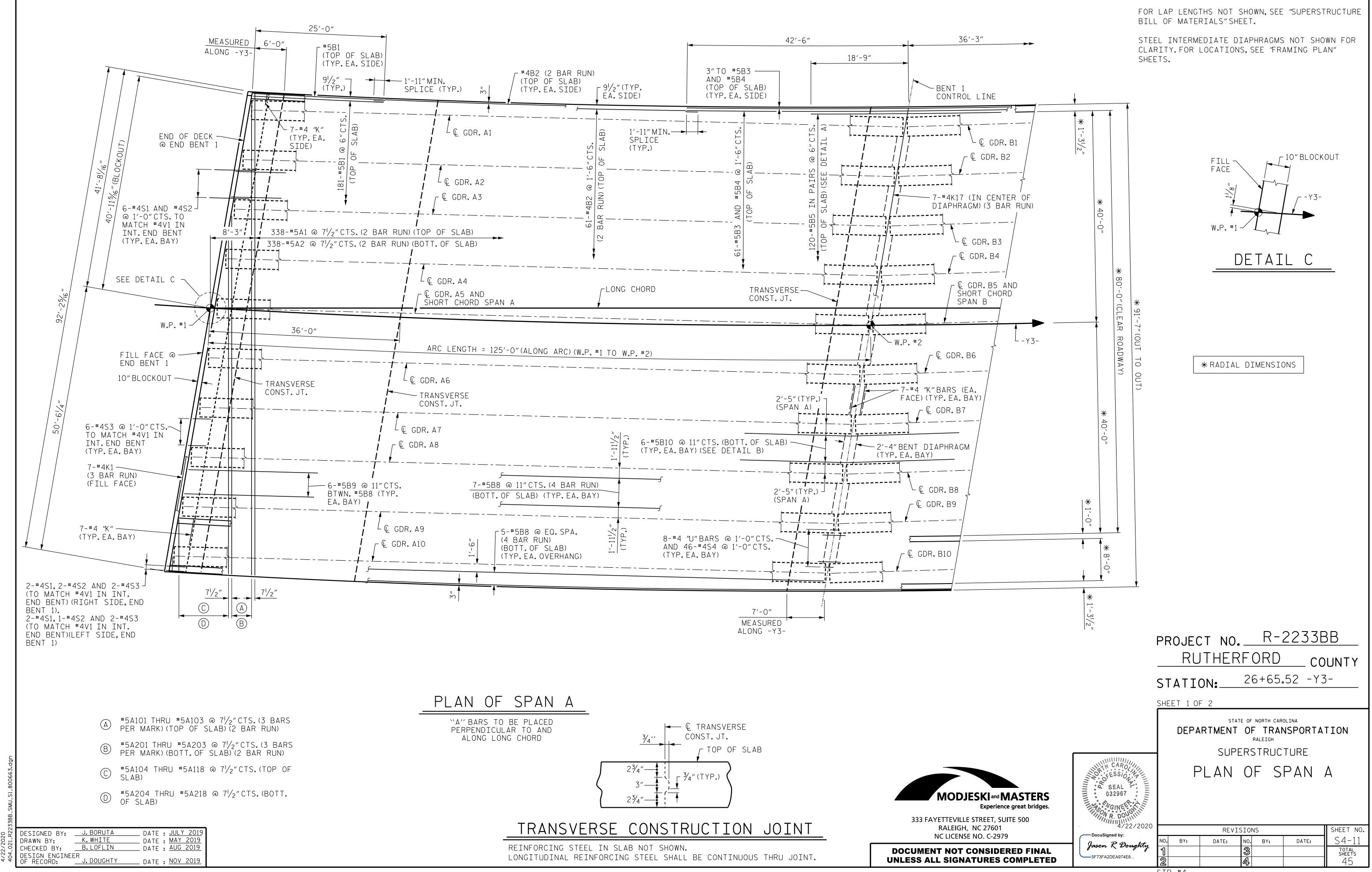
RALEIGH, NC 27601



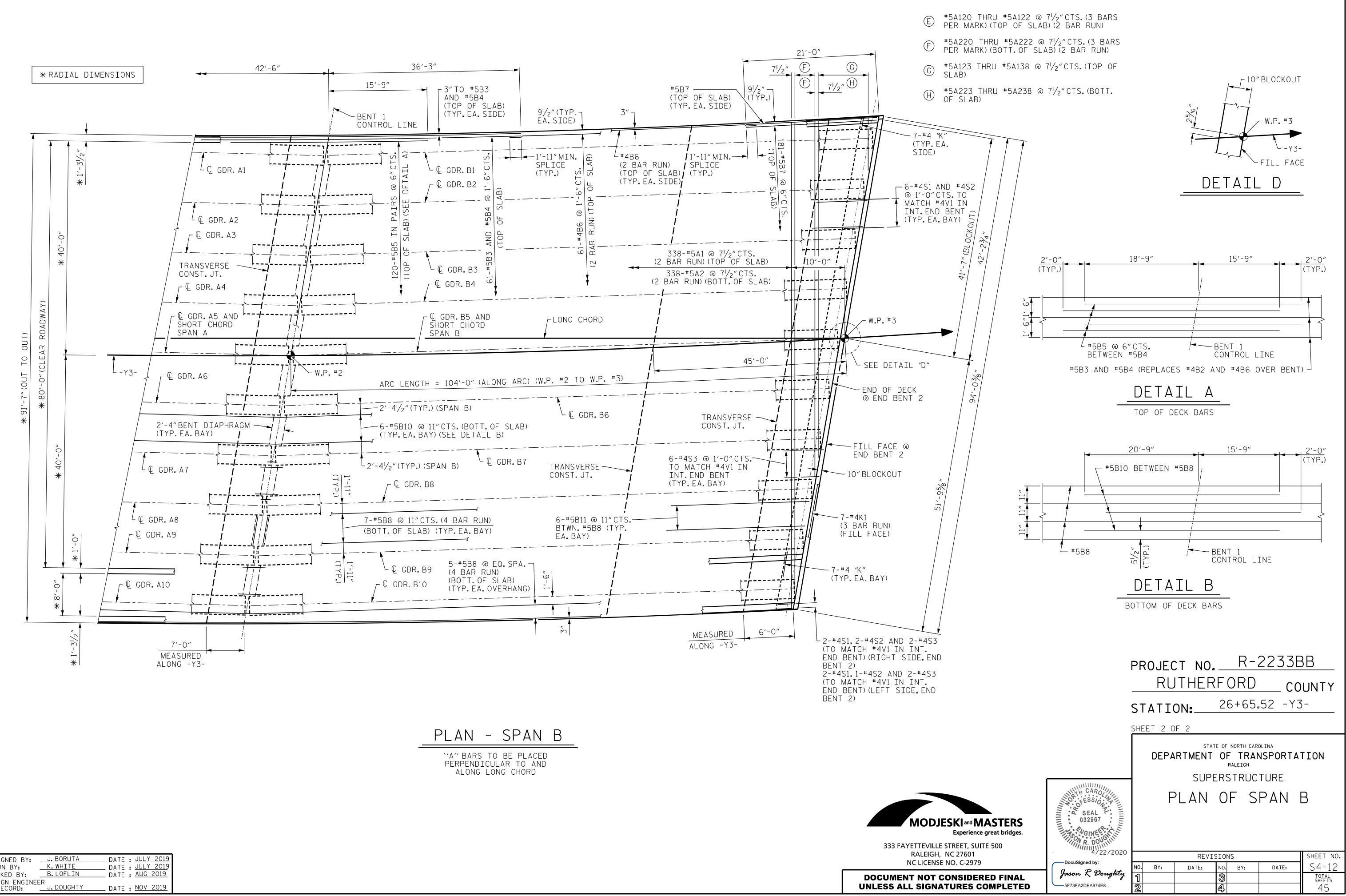
BLOCKOUT (TYP.)~ ∽ € BEARING 10³⁄₄″± 1″ MIN. (TYP.) BENT CAP PLAN BENT DIAPHRAGM BLOCK-OUT DETAIL



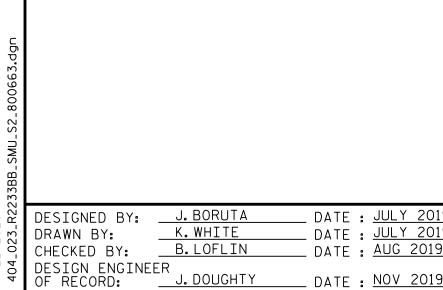
R	D	E	R	

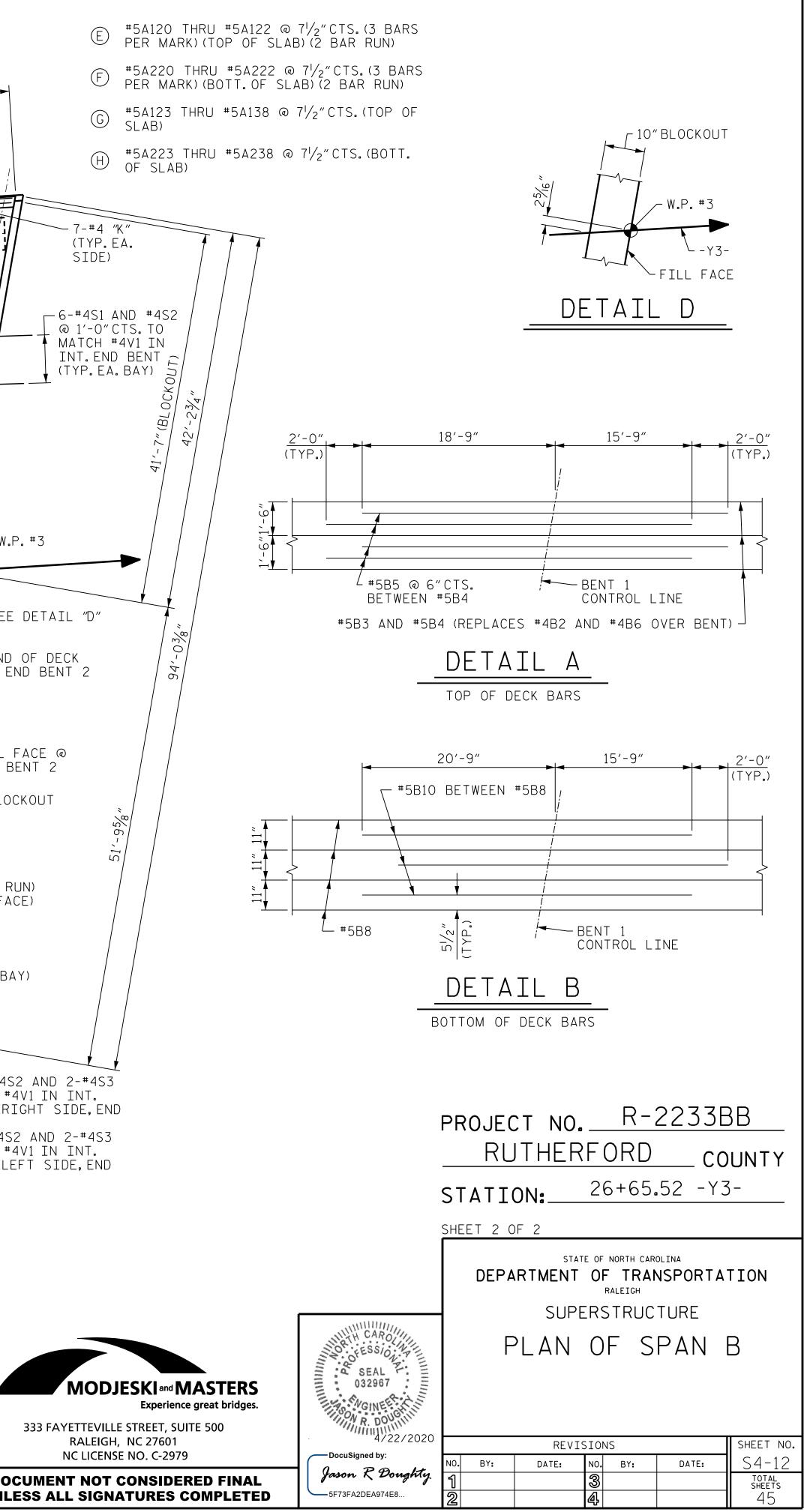


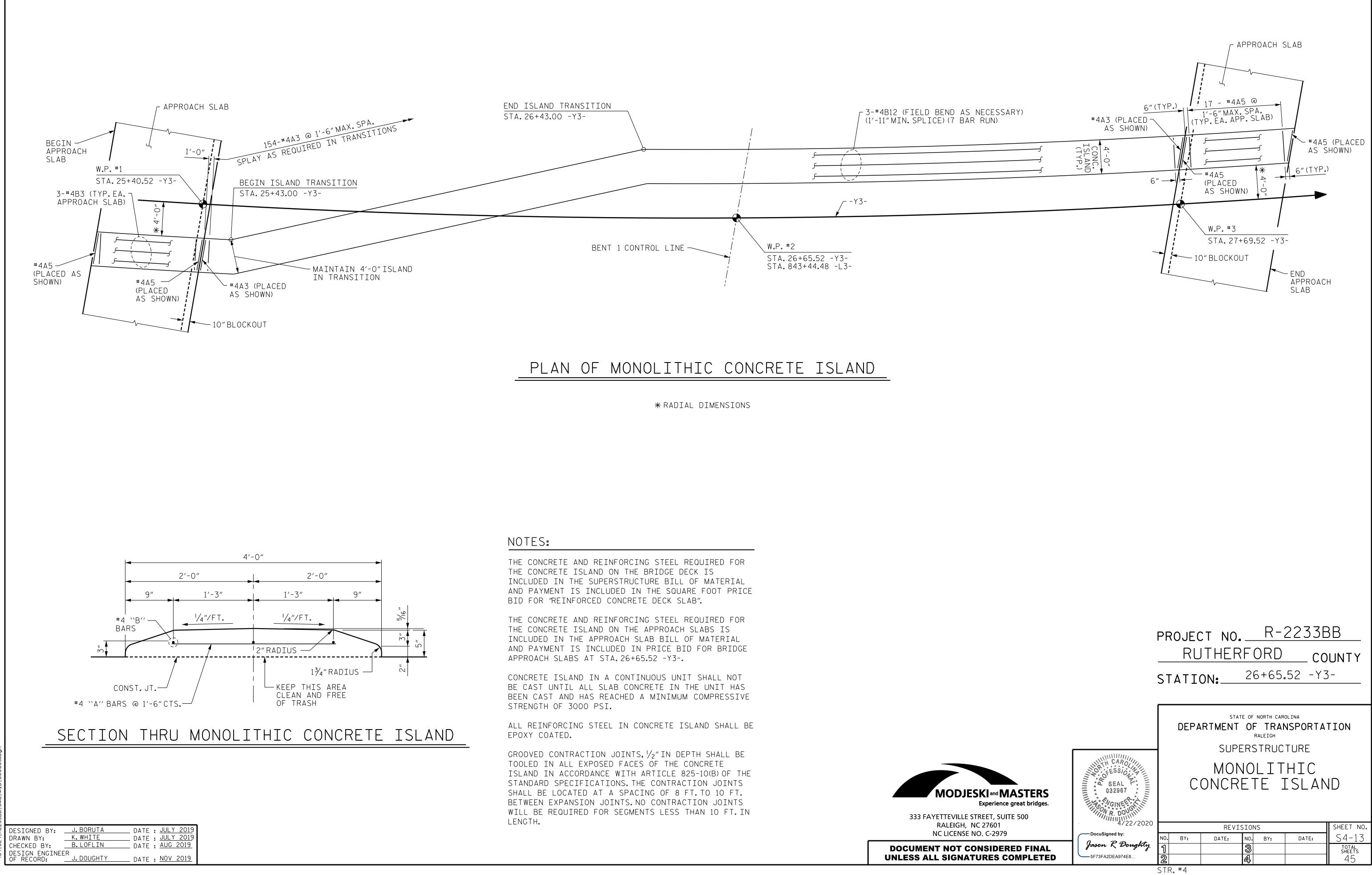
NOTES:



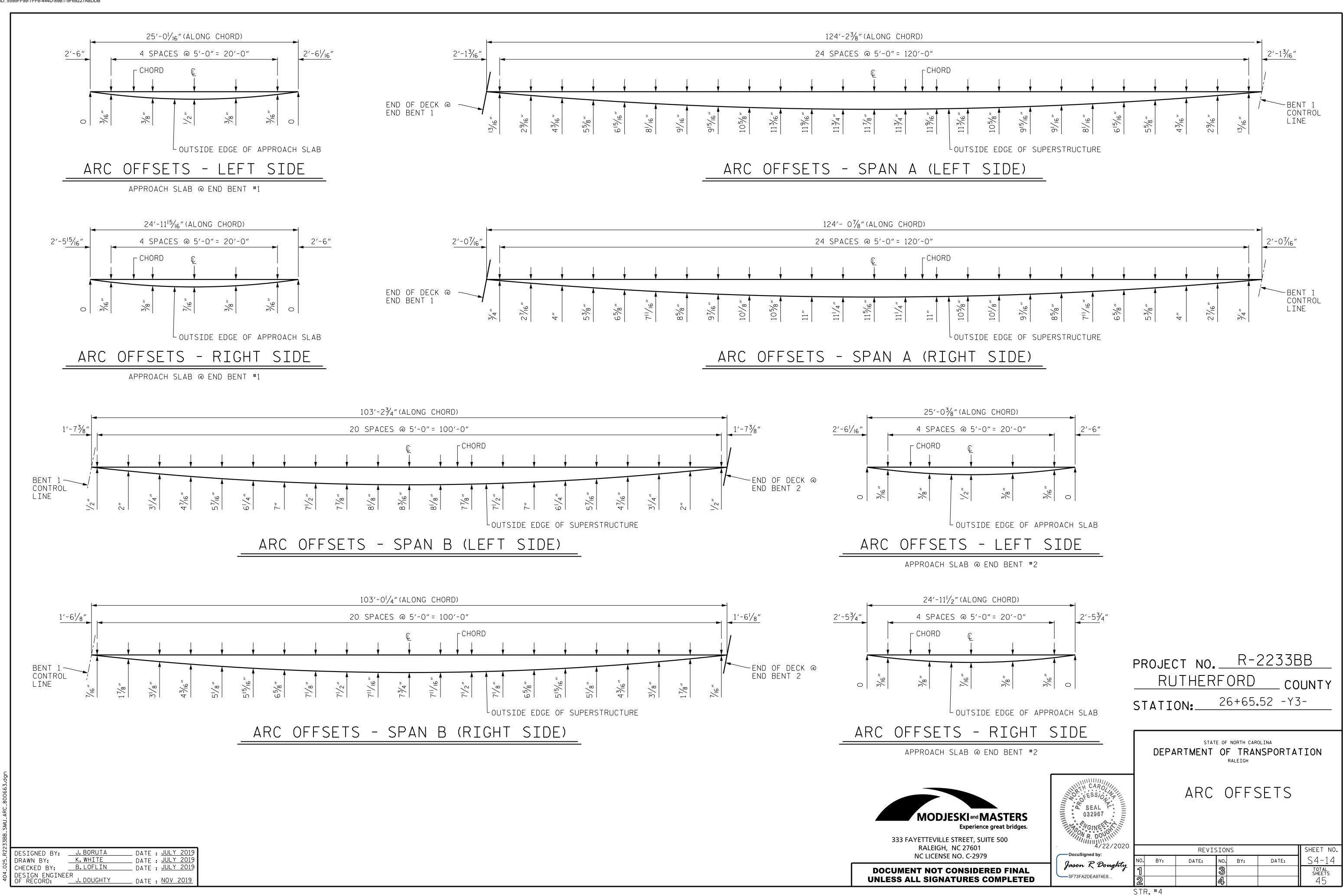


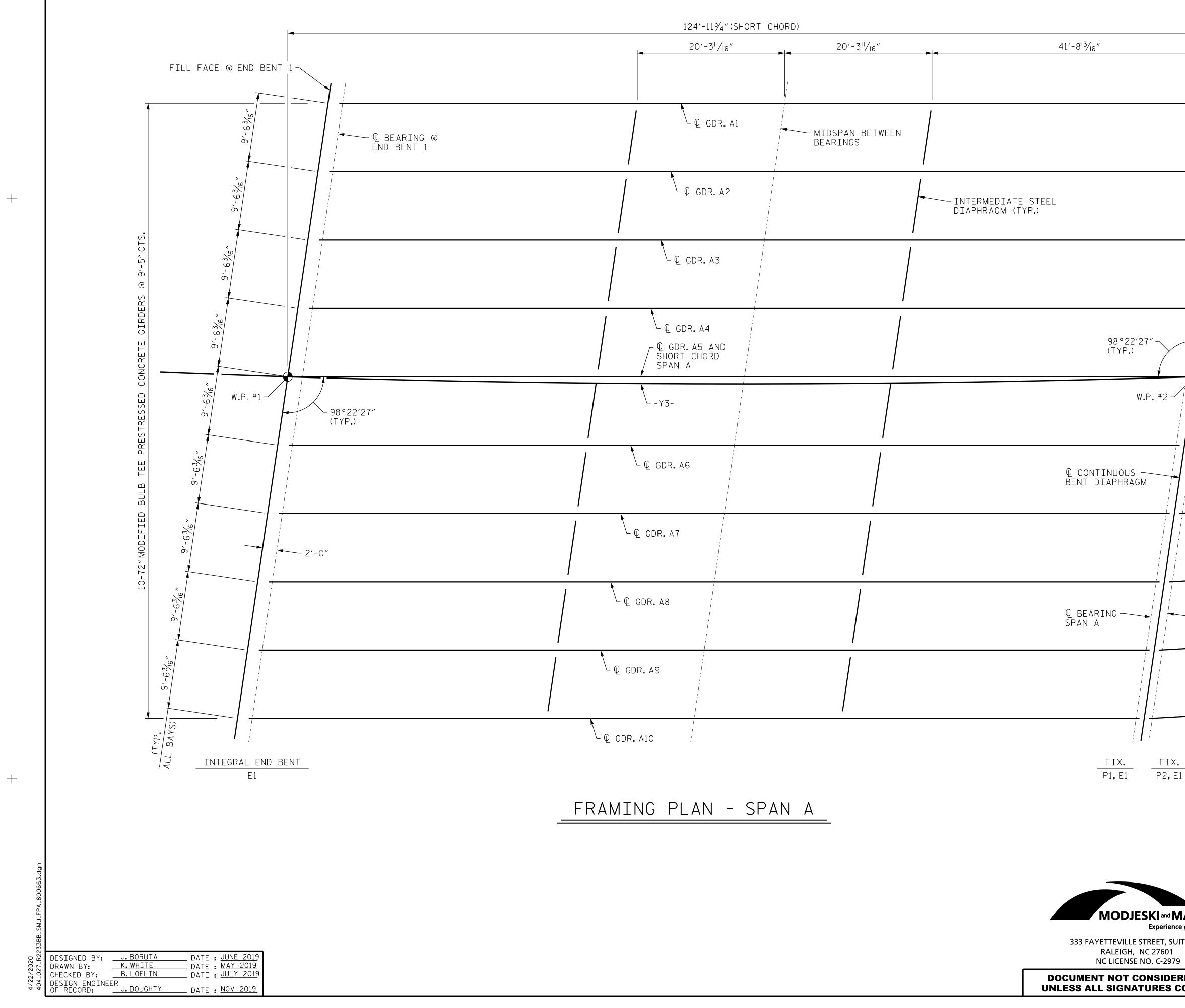






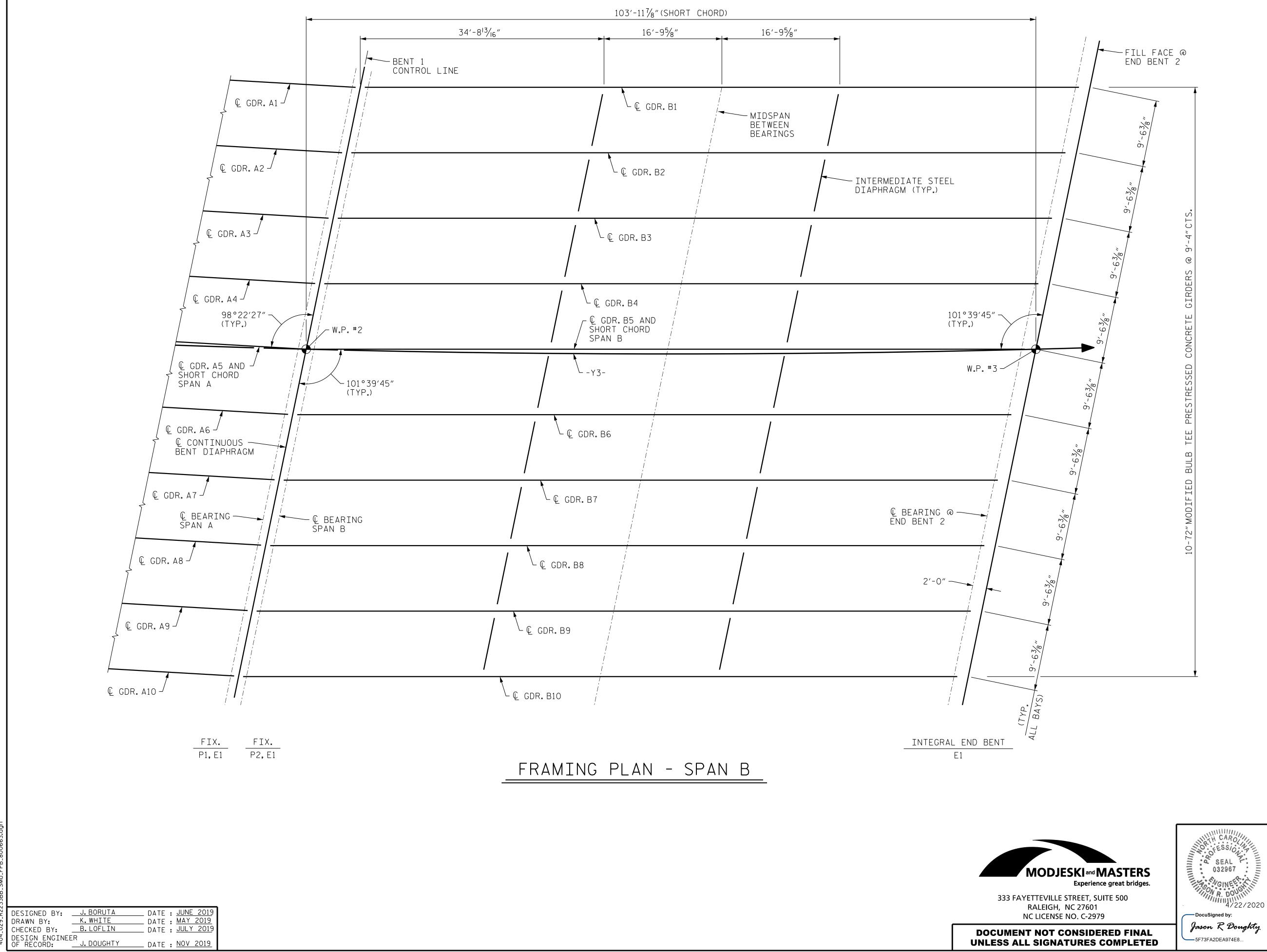






>	
BENT 1 CONTROL LINE	
<u> </u>	
€ GDR. B2	
Ç GDR. B3 Ç GDR. B4 -	
GDR. B5 AND SHORT CHORD SPAN B	
101°39′45″ (TYP.)	
€ GDR. B6	
€ GDR. B7	
Ç GDR. B8	NOTES:
V BEARING SPAN B	ALL DIMENSIONS ARE HORIZONTAL. CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ANY NECESSARY TEMPORARY BRACING OF GIRDERS DURING ERECTION PRIOR TO PLACING DIAPHRAGMS AND DECK.
└ @ GDR.B9 {	END BENTS AND BENT ARE PARALLEL.
↓ € GDR. B10	PROJECT NO. <u>R-2233BB</u>
1	RUTHERFORD COUNTY STATION. 26+65.52 -Y3-
	STATION: 26+65.52 -13- SHEET 1 OF 2
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
ASTERS e great bridges.	FRAMING PLAN SPAN A
ITE 500	REVISIONS SHEET NO.
DocuSigned by: Jason R Donghty 5F73FA2DEA974E8	NO. BY: DATE: NO. BY: DATE: $54-15$

+



		REVIS	SIO	NS		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-16
1			3			TOTAL SHEETS
2			4			45
ст	⊃ # <i>1</i>					

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

FRAMING PLAN

SPAN B

SIR.#4

NOTES:

ALL DIMENSIONS ARE HORIZONTAL.

TO PLACING DIAPHRAGMS AND DECK.

END BENTS AND BENT ARE PARALLEL.

RUTHERFORD

STATION:____

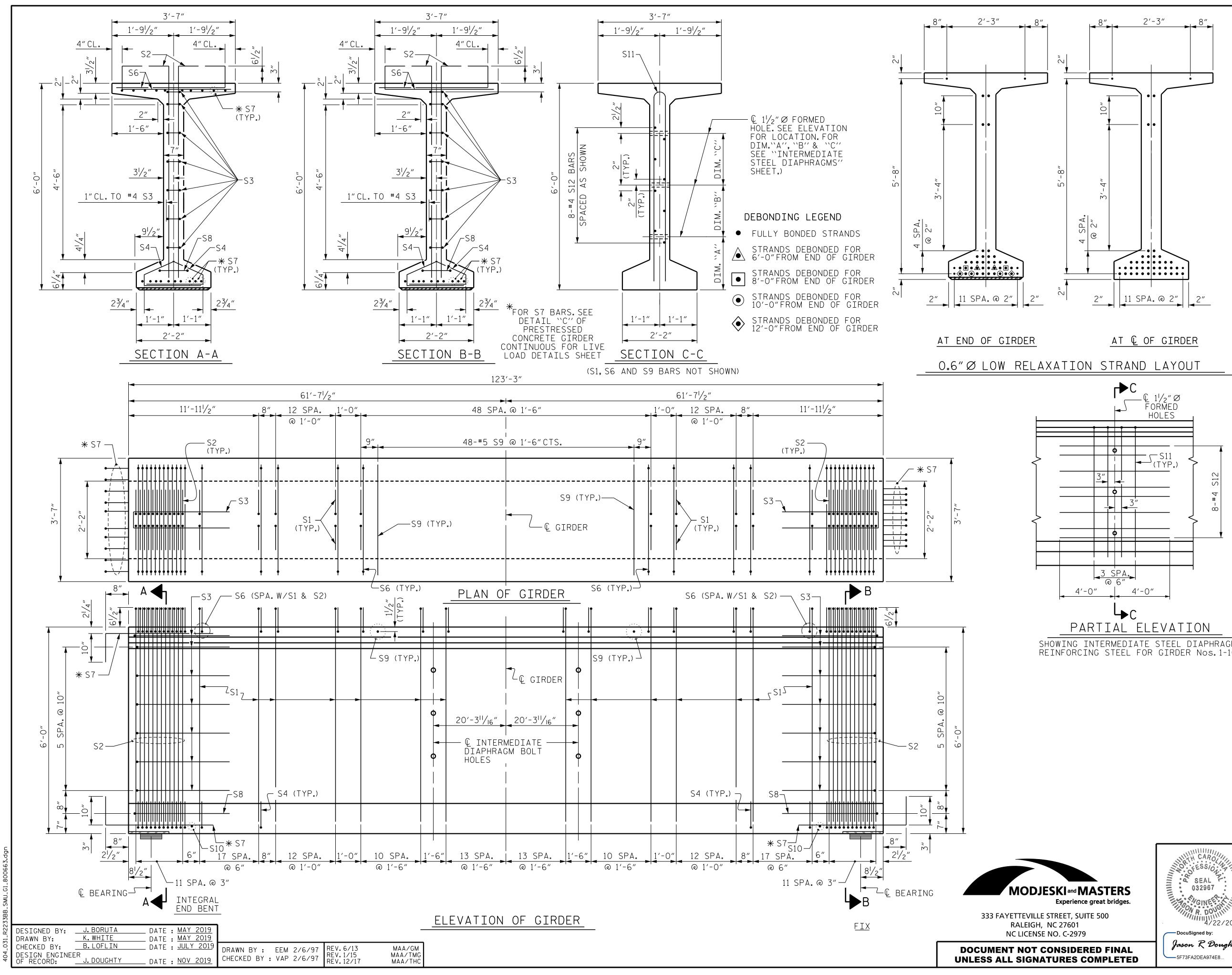
SHEET 2 OF 2

PROJECT NO. R-2233BB

_ COUNTY

26+65.52 -Y3-

CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ANY NECESSARY TEMPORARY BRACING OF GIRDERS DURING ERECTION PRIOR

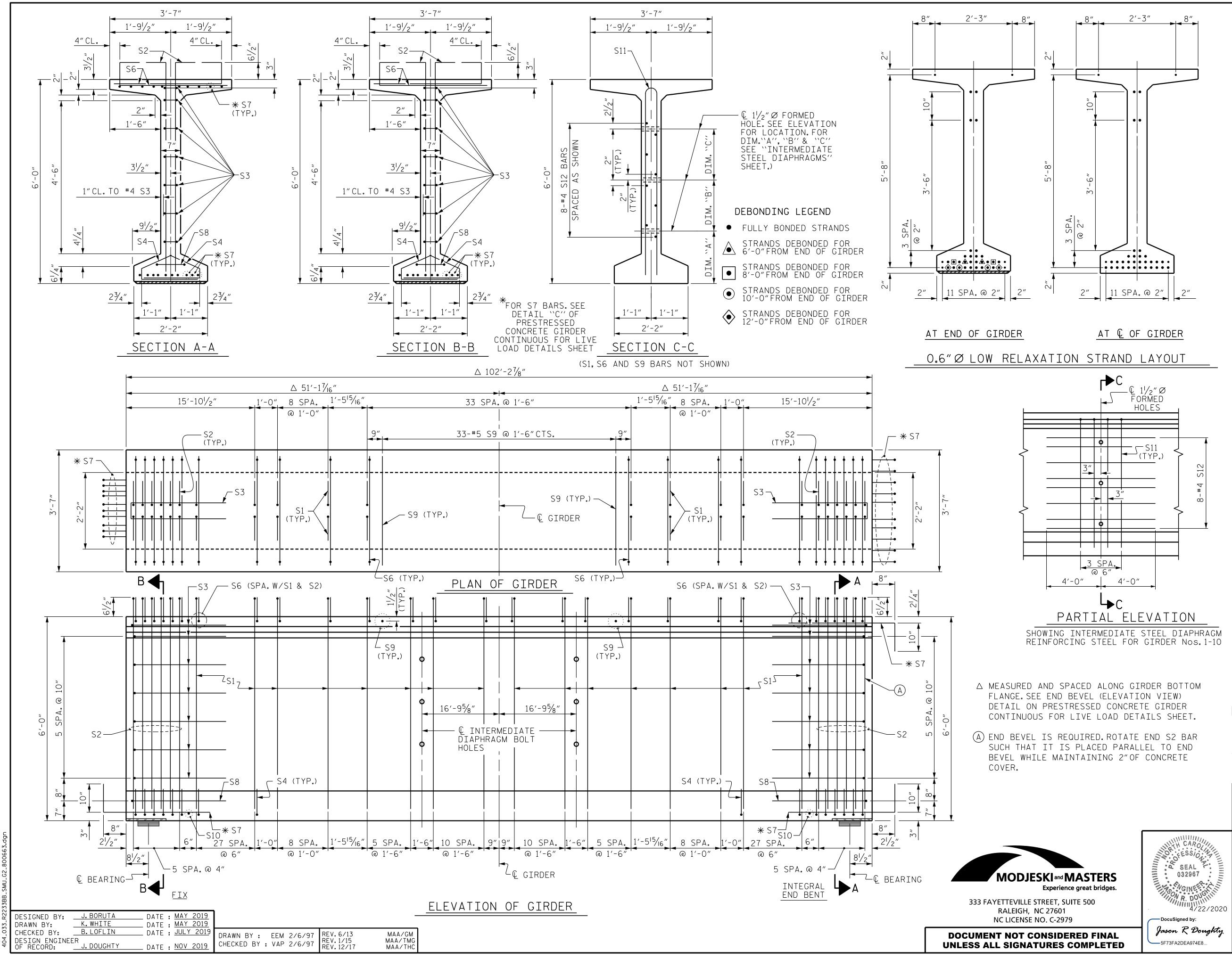


+

NPHRAGM os. 1-10 NUMBER LENGTH TOTAL LENGTH							
3″.		0.6	5″ØL.	R.GRA)E 270	STR	ANDS
				דד		۸	
		AR	ΕA				
		(SQUARE	INCHES)			(LBS.	PER STRAND)
		0.2	17	58,6	00		13,950
					TYPE 1		
					1		
				#4			
				-			
			16	#4			
						_	
"		* N0 ⁻			ALL BE HEAT BF	BENT NDIN(BEFORE G SHALL
		 		BE ALL	OWED.		-
		8″	►	<u>S3</u>			
				<u>58</u>	10 ¹ /2″	► m	<u>ω</u>]
			" М -				4 "
	AREA (SOUARE INCHES 0.217 REINFORC BAR NUMB S1 222 S2 48 S3 14 S4 120 S6 270 * S7 30 S8 2 S9 48 S10 2 S11 8 S12 16 * NOTE: ST NO * STATION: _				0-,	-03	
						4	ج ح
					I	I <u> </u>	
					8″	,	6"
A		<u>9</u> 13	4	-		Ý	
		413				Ŧ	
S12			3	ω	2"	I	RAD '-6"
4 #			\bigcirc	<u> </u>	-0/		5,
		41/4	1'-8″		4) ~		
				-		¥ _,	4″
<u> </u>		ALL BAR	DIMENSION	NS ARE OU	T-TO-OUT		5
	1					_	
		QU	ANTITI	ES FO	r one	<u> </u>	RDER
		0.10					
N		GIR	שבא	3,415		0.4	44
APHRAGM			GIR	DERS	REQUI	RED	
os.1-10		NUM				1	AL LENGTH
	I			123.	25′	1	232.50′
		10)				-
		10	, 				
	PF			F		33E	3B
	PF	ROJEC	T NO		22		
		ROJEC	T NO		22		3B JUNTY
		ROJEC RU	t no Thef	RFOR	22		UNTY
		ROJEC RU	t no Thef	RFOR	R-22 2D		UNTY
		ROJEC RU	t no Thef	RFOR	R-22 2D		UNTY
		ROJEC RU	T NO THEF DN:	RFOR 26+6	2-22 2D 55.52	_ CC _ Y 3	UNTY 3-
		ROJEC RU	T NO THEF DN:	RFOR 26+6	R-22 2D 55.52 H CAROLINA RANSP(_ CC _ Y 3	UNTY 3-
		ROJEC RU	T NO THEF DN:	RFOR 26+6	R – 22 2D 55.52 4 carolina RANSP(_ CC _ Y 3	UNTY 3-
		ROJEC RU ATIC	T NO THEF DN:	RFOR 26+6 26+6 OF T RALEIC	R – 22 2D 55.52 4 CAROLINA RANSPO	_ CC _Y3	UNTY 3-
AROLINI SSION THE		ROJEC RU ATIC DEPAR	T NO THEF DN:	RESSE	R-22 CD 55.52 CAROLINA RANSPO ARD ED CC	_ CC _Y3 DRTA	OUNTY 3- TION RETE
		ROJEC RU ATIC DEPAR	T NO THEF DN:	RESSE	R-22 CD 55.52 CAROLINA RANSPO ARD ED CC	_ CC _Y3 DRTA	OUNTY 3- TION RETE
AL		ROJEC RU ATIC DEPAR 72" F	T NO THEF DN:	RESSE JS FC	R – 22 2D 55.52 55.52 ARD ARD ED CC BULB OR LI	- CC - Y 3 DRTA DRTA	OUNTY 3- TION RETE
AL 2967		ROJEC RU ATIC DEPAR 72" F	T NO THEF DN:	RESSE	R – 22 2D 55.52 55.52 ARD ARD ED CC BULB OR LI	- CC - Y 3 DRTA DRTA	OUNTY 3- TION RETE
AL 2967 NEE DOUGHIII 4/22/2020		ROJEC RU ATIC DEPAR 72" F	T NO THEF DN:	RESSE JS FC	R – 22 2D 55.52 55.52 ARD ARD ED CC BULB OR LI	- CC - Y 3 DRTA DRTA	UNTY 3- TION RETE LOAD
AL 2967 NEFR DOUGHIN 4/22/2020 ad by:	ST	ROJEC RU ATIC DEPAR 72" F	T NO THEF DN:	RESSE JS FC SPAN	R – 22 CD 55.52 ARD ARD ED CC BULB OR LI A	- CC - Y 3 DRTA DRTA	UNTY 3- TION RETE LOAD
AL 2967 NEER HILL DOUGHILL 4/22/2020	ST	ROJEC RU ATIC DEPAR 72" F N CONT	T NO THEF DN:	RESSE JS FC STAND	R – 22 CD 55.52 ARD ARD ED CC BULB OR LI A	- CC - Y 3 ORTA ORTA ONCF TEE VE	UNTY 3- TION RETE LOAD

STR.#4

STD.NO.PCG8 (SHT 2)



+

	0.6	5″ØL.	R. GRAE)E 270	STRAI	NDS
	AR	ΕA	ULTI Stre	MATE		
	(SQUARE	INCHES)				
O.6" Ø L. R. GRADE 270 STRANDS AREA ULTIMATE APPLIED SOUARE INCHES) LBS. PER STRAND) LBS. PER STRAND) 0.217 58,600 43,950 REINFORCING STEEL FOR ONE GDR BAR NUMBER SIZE TYPE SI 216 *4 1 6'-11" SI 216 *4 1 6'-11" SI 216 *4 3'-0" 273 S6 240 *5 16'-11" 173 S6 240 *5 5TR 3'-0" 273 S6 240 *5 5TR 3'-3" 112 S10 2 *3 STR 1'-10" 1 S11 8 *5 5 11'-6" 96 S12 16 *4 STR 8'-0" 86 S11 8'' 5 5 11'-6" 96 S12 16 *4 STR 8'-0" 8' MOTE: ST BARS, SHALL BE BENT BENDING SHALL 8' 9'						
					-	
	AREA UL TIMATE STRENGTH APPLIED PRESTRESS (LBS. PER STRAND) 0.217 58,600 43,950 REINFORCING STEEL FOR ONE GDR BAR NUMBER SIZE TYPE LENGTH WEIGHT SI 216 44 1 6'-11' 173 S3 14 4 2 8'-5' 79 S4 136 *4 3 3'-0' 273 S6 240 *5 4 4'-5' 106 * S7 30 *5 STR 3'-3' 112 S10 2 *5 STR 3'-6'' 106 * S7 30 *5 STR 3'-6'' 106 * S7 30 *5 STR 3'-6'' 112 S10 2 *3 STR 1'-10'' 1 S11 8 *5 S11''-6'' 96 S12 16 *4 STR 8'-0'' MOTE: S7 BAR S8 10'/2'' 10'/2'' 10 <t< td=""></t<>					
			-	_		
			-	3		
	AREA ULTIMATE STRENGTH APPLIED PRESTRESS (SOUARE INCHES) 100,217 58,600 43,950 REINFORCING STEEL FOR ONE GDR BAR NUMBER SIZE TYPE LENGTH WEIGHT 51 216 #4 1 6'-11' 173 52 244 #5 1 6'-11' 173 53 14 #4 2 8'-5' 79 54 136 #4 3 3'-0'' 273 58 2 *5 5 79'' 112 510 2 *5 5 1''-0'' 1 511 8 *5 5 1''-0'' 1 510 2 *3 STR<1''-10''					
	AREA LILTIMATE STRENGTH APPLIED PRESTRESS (SOUARE INCHES) 0.217 58,600 43,950 REINFORCING STEEL FOR ONE CDR BAR NUMBER SIZE TYPE 1216 *4 1 51 216 *4 13 14 *4 24 *5 1 51 216 *4 4 3 3'-0' 58 240 *5 24 *5 2 58 2 *5 28 2 *5 2933 *5 SIR 510 2 *3 510 2 *3 511 8 5 512 16 *4 8 *4 5 512 16 *4 14 517 56 150 *5 11'-6'' 8 511'-6'' 96 512 6 *5''					
	AREA ULTIMATE STRENGTH APPLIED PRESTRESS (SQUARE INCHES) 0.217 58,600 43,950 REINFORCING STEEL FOR ONE GDR BAR NUMBER SIZE TYPE LENCTH WEIGHT SI 216 #4 1 6-117 998 51 216 #4 1 6-117 998 6-117 998 52 24 *5 1 6-117 998 6-117 998 53 14 *4 2 8-57 79 54 136 #4 3 3'-07 273 54 136 #4 3 3'-07 106 5 STR 3'-8" 115 58 2 400 *5 4 4'-5" 1066 3'-8" 115 58 2 *5 2 9'-07 19 19 51 2 16 *4 STR 18'-07 86 51 18 8 *5 5 11'-07 8 51 18 8 *5 5 11'-07 8 51 18 8 *5 5 11'-07 86 51 19 8 *10 WENT. HEAT BENDING SHALL NOT BE ALLOWED. MERTYPES MENTIFE FOR ONE GRDER 10 10 10 10 10 10 10 10 10 10 10 10 10 1					
	AREA UL TIMATE STRENGTH APPLIED PRESTRESS (SUUARE INCHES) 0.217 58,600 43,950 REINFORCING STEEL FOR ONE CDR BAR NUMBER SIZE TYPE LENGTH WEIGHT SI 216 #4 1 6'-11' 998 52 24 #5 1 6'-11' 173 53 14 #4 2 8'-5' 79 54 136 #4 3 3'-0' 273 56 240 #5 4 4'-5' 1106 * 57 30 #5 STR 3'-8' 115 58 2 #5 STR 3'-8' 115 58 2 #5 STR 3'-8' 112 510 2 #3 STR 1'-10' 1 511 8 #5 5 TR 1'-6'' 96 512 16 #4 STR 8'-0' 86 512 16 #4 STR 8'-0' 6 minimum state and the second state NOT BE ALLOWED. BAR TYPES OUDANTITIES FOR ONE CIRDER SHEPMENT, HEAT BENDING SHALL NOT BE ALLOWED. BAR TYPES OUDANTITIES FOR ONE CIRDER SHEPMENT ABACE OUT-TO-OUT STRES SHALL BE BENT BEFORE SHEPMENT ABACE OUT-TO-OUT OUDANTITIES FOR ONE CIRDER MEDMENSIONS ARE OUT-TO-OUT OUDANTITIES FOR ONE CIRDER CONCRETE L.B. STRANDS GIRDERS REQUIRED NUMBER LENGTH TOTAL LENGTH 10 102.24' 1022.40' STATE OF MORTH CANCUM DEPARTMENT OF TRANSPORTATION RALEIDH					
	* N0 ⁻	L TE: S7 E Shtf	BARS SHA MENT, H		L BENT BE NDTNG 1	EFORE Shali
				OWED.		, _ L
	8″) 	
		٦			, ΜΙ Ø	1
						_
		"С-,			0″ 3∕4″	
		ى			4′-(
			L			
				8″		ô″
	[*] 9/1 ³	A"	-		_ +($\gamma + 1$
					A	
	‡_`(3	<u> </u>	,~/10		2 " K/
	4	1'-8"	(4) [']		
		 	►		<u>↓</u> _►	4″
	ALL BAR	DIMENSIO	NS ARE OU	T-TO-OUT	(5)
	QU	ANTITI	ES FO	R ON	E GIRD	ER
			REINFORC	ING 800	0 PSI	0.6″Ø
	GIR	DER				
	NUM					LENGTH
	10)	102.	24′	1022	2.40′
			Г		זמצב	ר
PF					JUCC	<u> </u>
	RU	THEF	<u>rfor</u>	2D		NTY
۲ ک	ΓΛΤΤΛ)NI=	26+6	55.52	-Y3-	
		/ · Nø				
	DEPAI				ORTATI	ON
	-		RALEIC	GΗ	_	
1	— -					
	_				DNCRE	ΤE
	Ν	NODIF	TED	BULB	IEE	

WODI ILD DOLD ILL CONTINUOUS FOR LIVE LOAD SPAN B

		REVIS	SIO	NS		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-18
1			3			TOTAL SHEETS
2			4			45
STR	.#4	S	TD.	NO.PC	G8 (SHT 2)	

	1	[JEAD	LUA	U DE		~ I T ()	IN IA	ALF)							
TWENTIETH POINTS	0.0	.05	.10	.15	.20	.25	.30	.35	.40		1		.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE)	▲ 0																			
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓ 0																			
FINAL CAMBER	↑ 0	1/4″	1/2"	11/16″	7⁄8″	1″	13/16″	11/4″	1 ³ ⁄8″	17⁄16″	17/16″	17/16″	13⁄8″	1 ¹ /4″	1 ³ / ₁₆ ″	1″	7⁄8″	11/16″	1/2"	¹ /4″
						1	1										•			
TWENTIETH POINTS	0.0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE)	↑ 0	0.046	0.092	0.134	0.174	0.208	0.237	0.261	0.278	0.289	0.292	0.289	0.278	0.261	0.237	0.208	0.174	0.134	0.092	0.046
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓ 0	0.029	0.058	0.086	0.113	0.136	0.156	0.172	0.184	0.191	0.193	0.191	0.184	0.172	0.156	0.136	0.113	0.086	0.058	0.029
FINAL CAMBER	↑ 0	3/16″	3/8"	9/16″	3⁄4″	7⁄8″	1″	1 ¹ / ₁₆ ″	11/8″	1 ³ /16″	1 ³ / ₁₆ ″	1 ³ / ₁₆ ″	1 /8″	1 / ₁₆ ″	1″	7⁄8″	3⁄4″	9/16″	3⁄8″	3/16″
										(SPAN ,	Δ								
										GIRD	ERS 8	- 10								
TWENTIETH POINTS	0.0	.05	.10	.15	.20	.25	.30	.35	.40	. 45	.50	.55	.60	. 65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE)	♦ 0																			
* DEFLECTION DUE TO SUPERIMPOSED D.L.	0																			
FINAL CAMBER	↑ 0	3/16″	3/8″	1/2"	5⁄8″	3⁄4″	13/16″	15/16″	1″	1″	1″	1″	1″	15/16″	13/16″	³ ⁄4″	5⁄8″	1/2″	3/8"	3/16″
										(SPAN I	3								
					I	I	I			GIR	DERS (l - 8			I				I	
TWENTIETH POINTS	0.0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE)	↑ 0																			
* DEFLECTION DUE TO SUPERIMPOSED D.L. FINAL CAMBER	 ↓ 0 ▲ 0 	0.013	0.027 5/16″																	
	T U	716	716	/16	78	/ 16	716	/8	716	I			716	/8	716	/ 16	78	/16	716	716
										(span (3								
						1	1		1						1	I				
TWENTIETH POINTS	0.0																			
CAMBER (GIRDER ALONE IN PLACE)																				
* DEFLECTION DUE TO SUPERIMPOSED D.L. FINAL CAMBER	 ↓ 0 ▲ 0 	0.015																		
	1		5/16"										716	716	74	78	716	716	716	78
INCLUDES ARE SHOWN IN FEET (DECIMAL FOR	WI), EXC		END OF IRDER		3	∛₄″ØX	5″		HES (F	RACTIC		/)。								
				4″ 8														_2"	BEVEL	▶ \◄
		34	•					N) RFVFI	_ [(TYP.)	
SECTION 'F'' (SEE NOTES)		2'-1 ¹ /2"		22 0.14 0.203 0.233 0.243 0.243 0.243 0.245 0.243 0.246 0.044 0.046 0.144 0.042 0.044 22 0.076 0.101 0.233 0.140 0.135 0.145 0.135 0.146 0.147 0.147 0.147 0.147 0.142 0.147 0.142 0.147 0.148 0.140 0.144 0.042 0.148 0.149 0.144 0.042 0.144 0.042 0.148 0.147 0.146 0.146 0.146 0.146 0.146 0.146 0.146 0.147 0.146 0.145 0.145 0.145 0.145 0.145 0.145 0.145 0.145 0.145 0.145 0.145 0.145																

FOR 72" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)

+

+

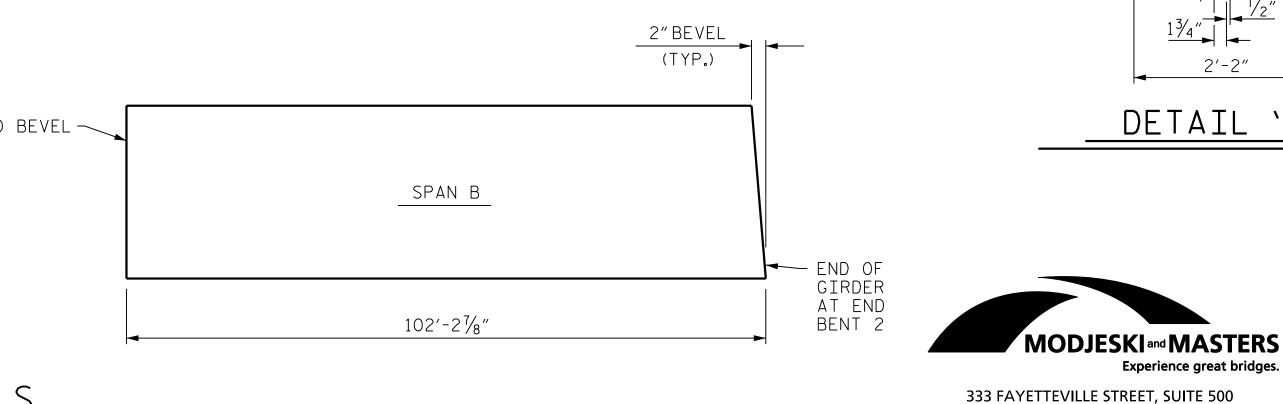
DESIGNED BY: <u>J.BORUTA</u> DRAWN BY: <u>K.WHITE</u> CHECKED BY: DESIGN ENGINEER OF RECORD: ____

B.LOFLIN

_ DATE : <u>MAY 2019</u> _ DATE : <u>MAY 2019</u>

_ DATE : JULY 2019

_ DATE : <u>NOV 2019</u>



END BEVEL (ELEVATION VIEW)

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

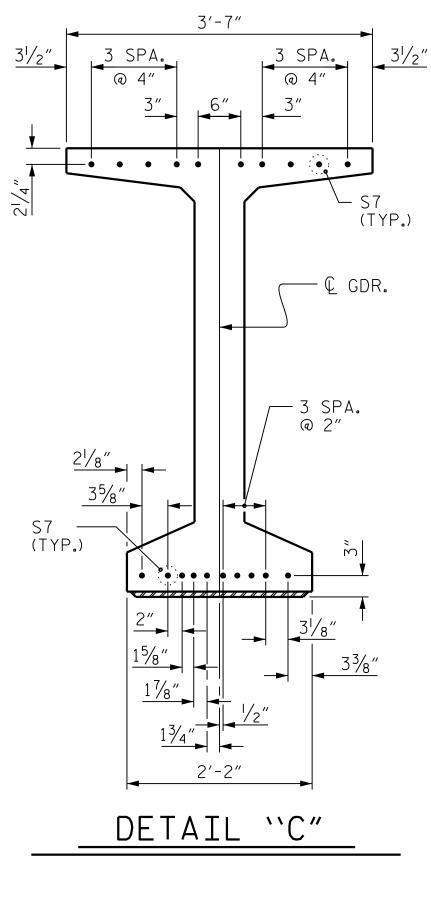
AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

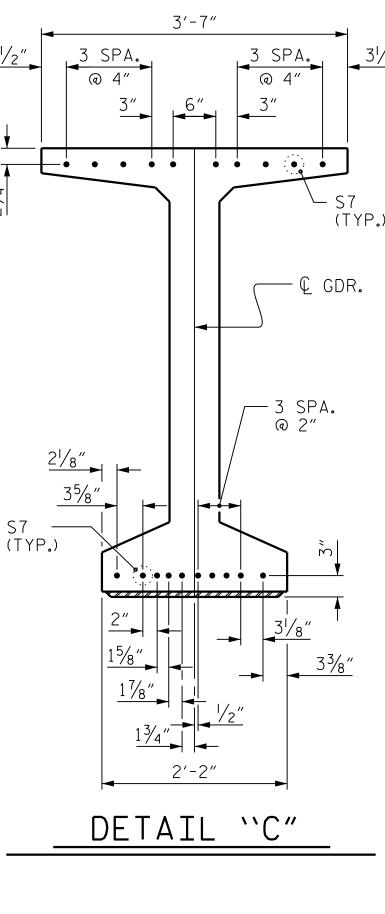
THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,800 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD 0F 4500 lbs.





DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

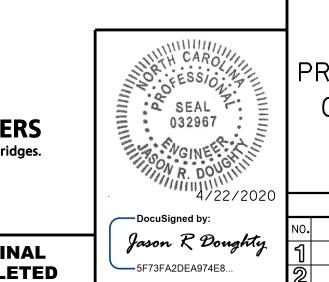
RALEIGH, NC 27601

NC LICENSE NO. C-2979

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

A 2" × 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND BOTTOM FLANGE.

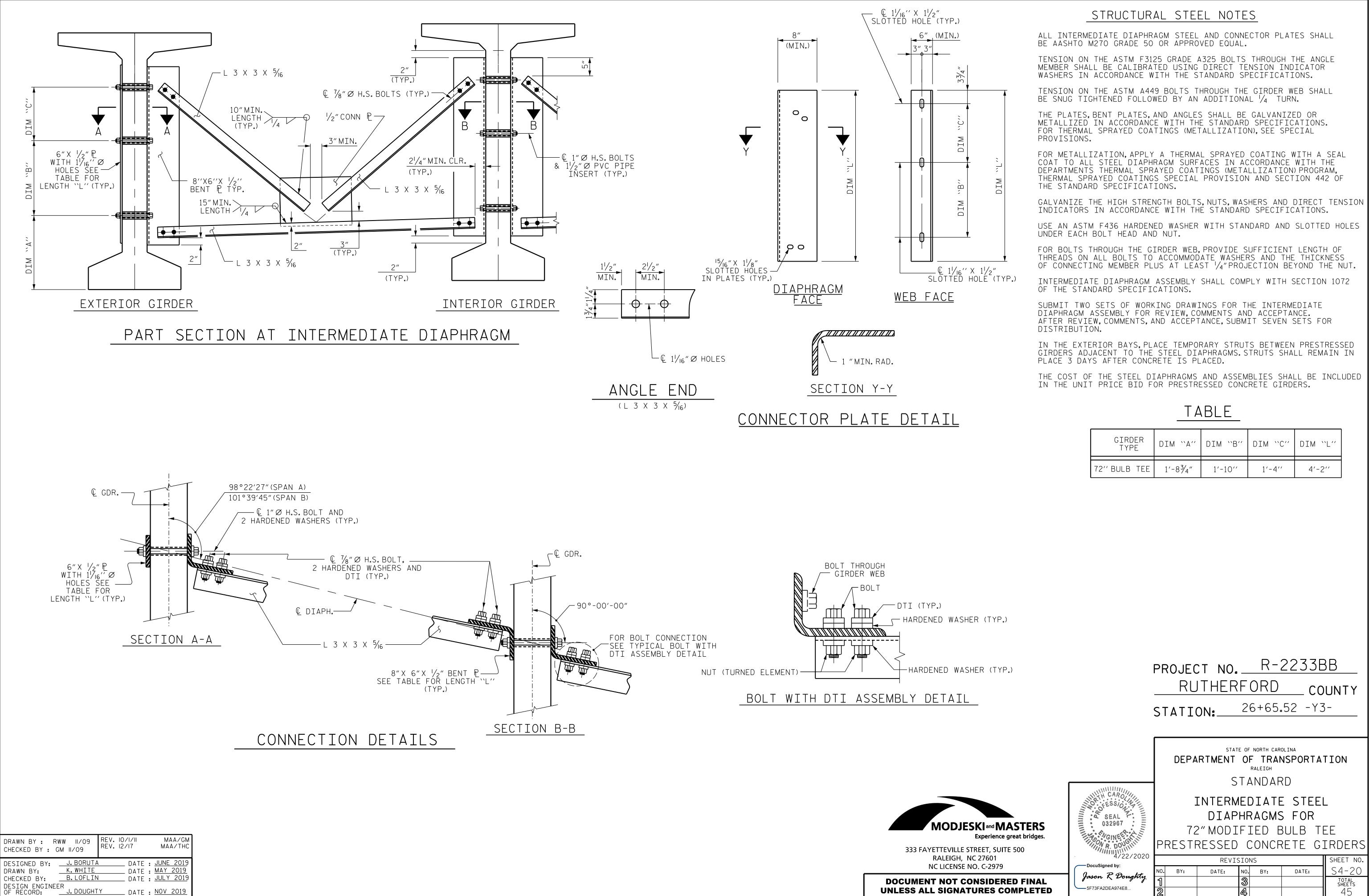


PROJECT NO	D. <u>R-22</u>	33BB
RUTHE	RFORD	_ COUNTY
STATION:	26+65.52	-Y3-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-19
1			3			TOTAL SHEETS
2			4			45
STR	. #4	S	TD.	.NO.PC(G9 SHT.4A	



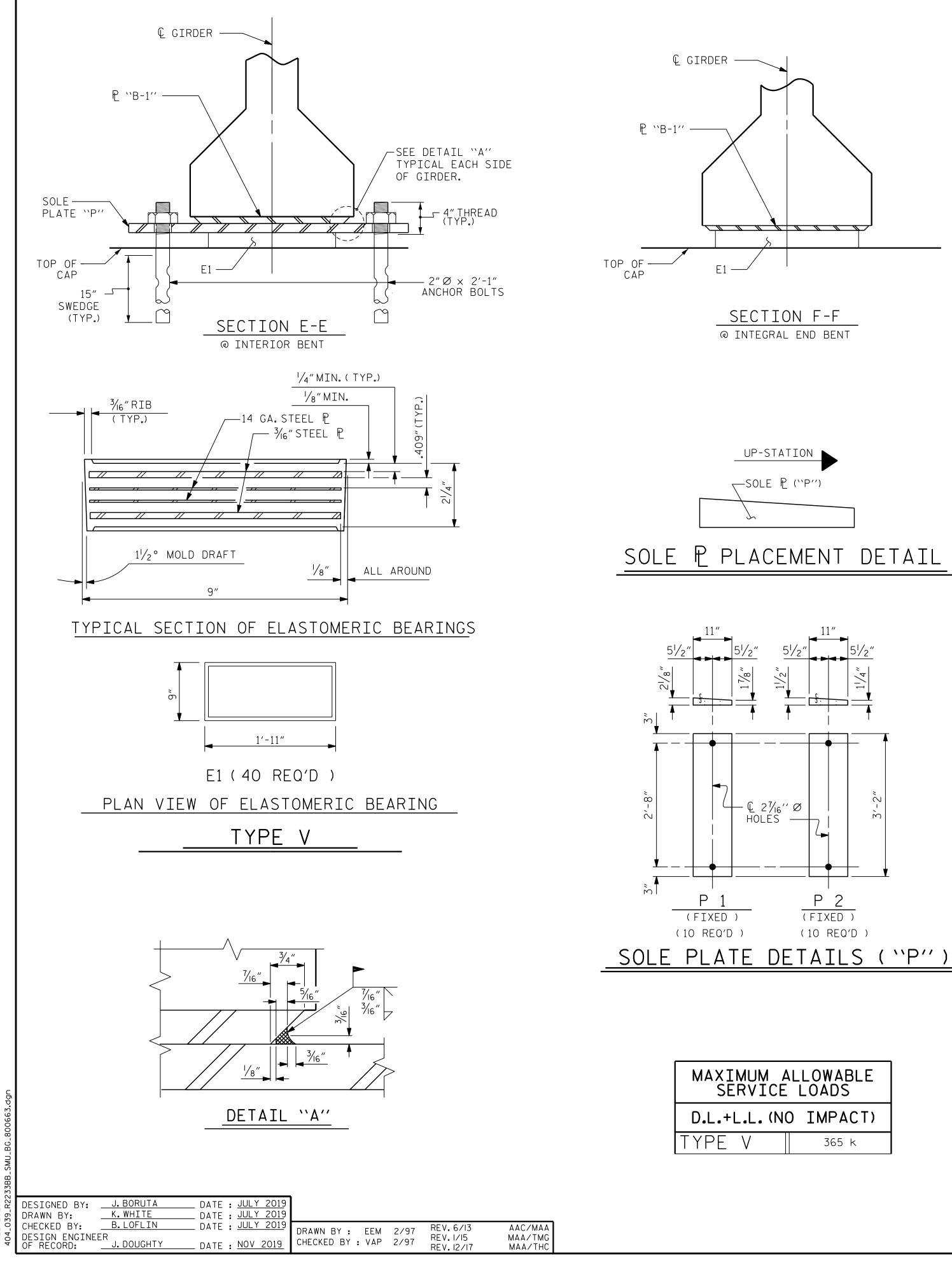
+

UNLESS ALL SIGNATURES COMPLETED

GIRDER	DIM ``A''	DIM ``B''	DIM ``C''	DIM ``L''
72'' BULB TEE	1'-8¾"	1'-10''	1'-4''	4'-2''

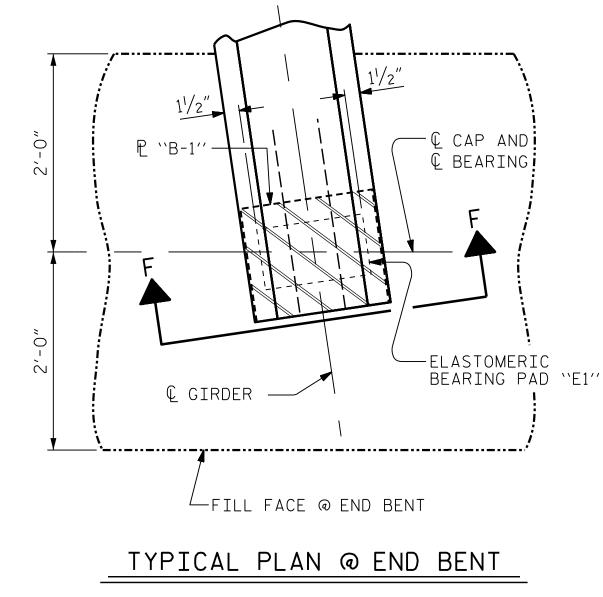
STR.#4

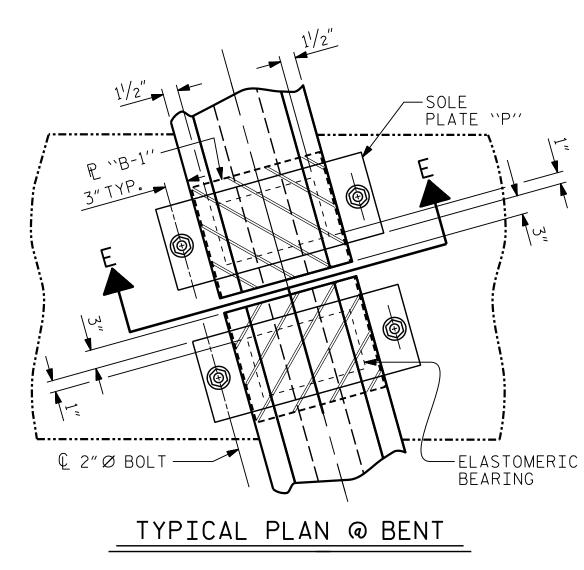
STD.NO.PCG11 SHT.4



+

MAXIMUM ALLOWABLE SERVICE LOADS						
D.L.+L.L. (NC) IMPACT)					
TYPE V	365 k					







DOCUMENT NOT CONSIDER UNLESS ALL SIGNATURES COMPLETED

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

STEEL SOLE PLATES, ANCHOR BOLTS AND NUTS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE ``P'', BOLTS AND NUTS SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS AND NUTS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

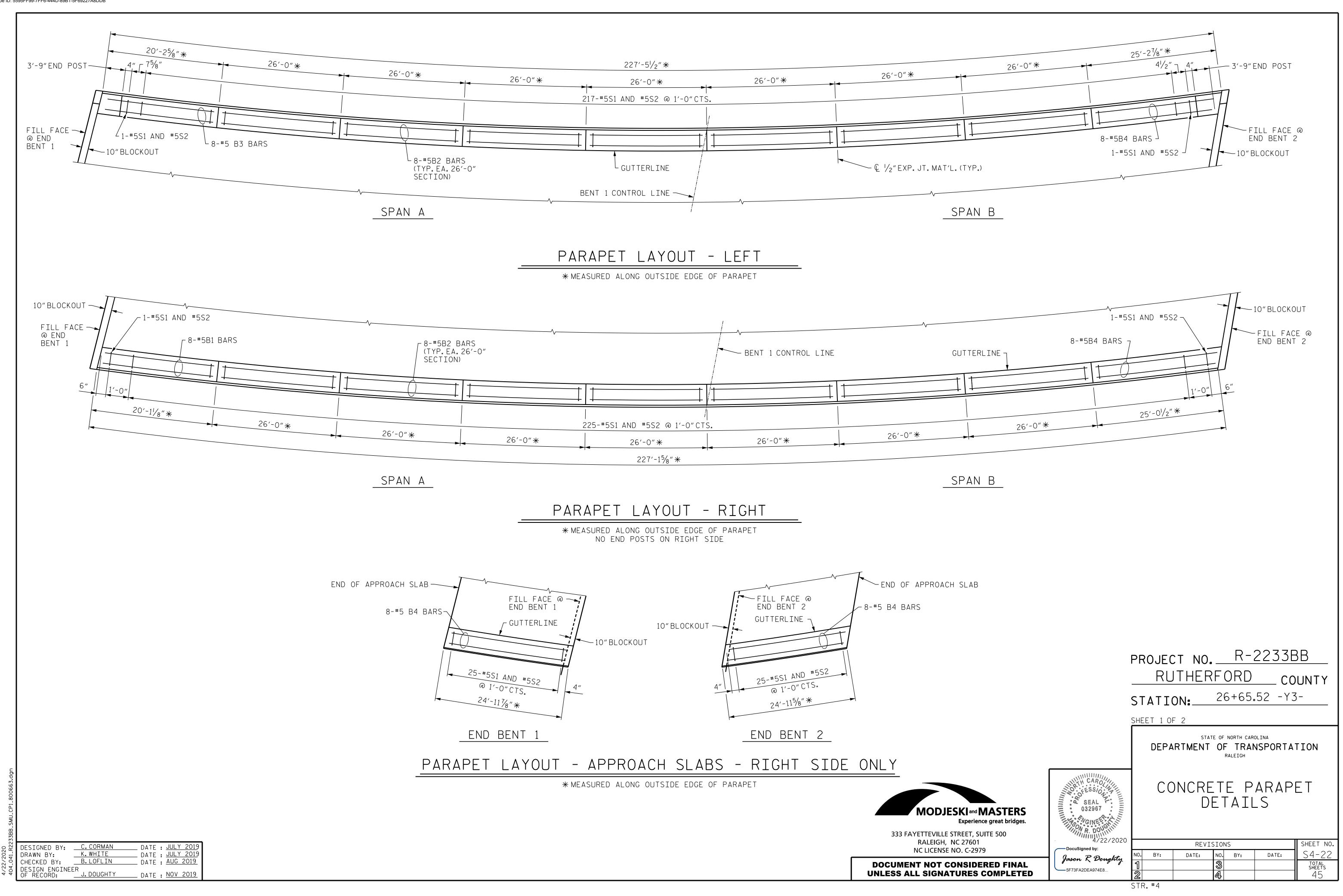
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.

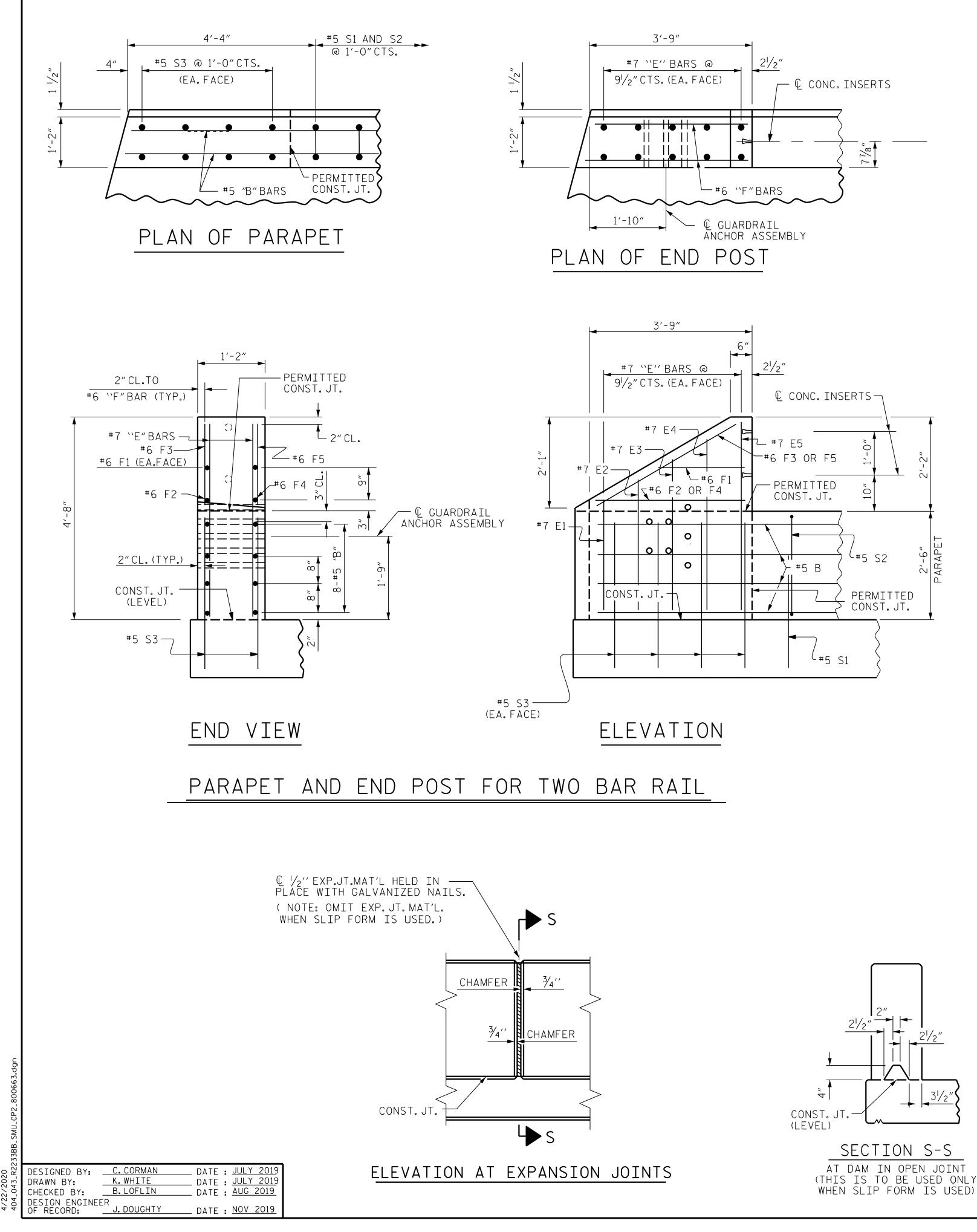
			ITHEF	<u>R-</u> RFORD 26+65.		UNTY
		DEPA		ATE OF NORTH CAR OF TRAN RALEIGH STANDAR	NSPORTA	TION
ASTERS e great bridges. TE 500	SEAL 032967		C Stress	ERIC ETAIL ED CONCF PERSTRUC	_S Rete gi	
RED FINAL	Jason R Doughty 5F73FA2DEA974E8	№. вү: 1 2	REV DATE:	ISIONS NO. BY: 3 4	DATE:	SHEET NO. S4-21 Total Sheets 45

STR.#4

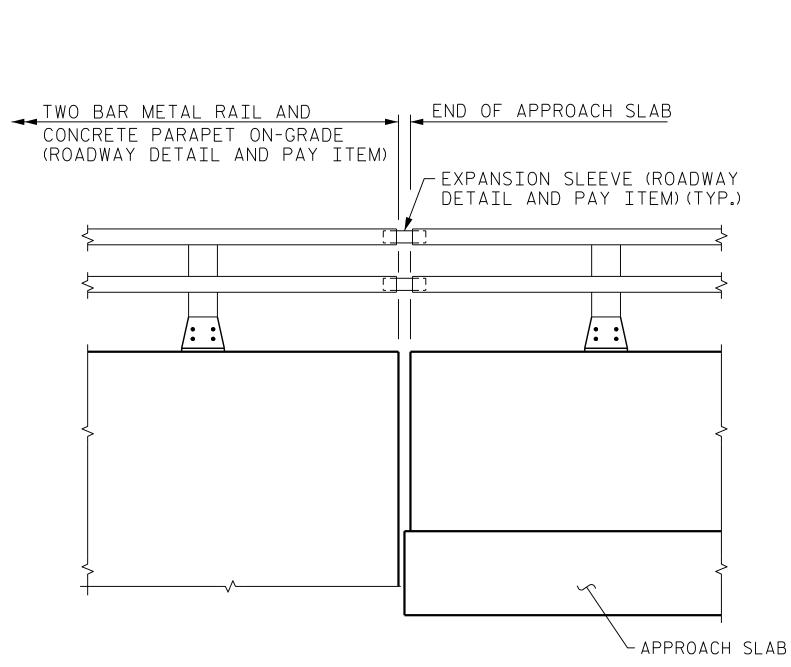
STD.NO.EB4



+



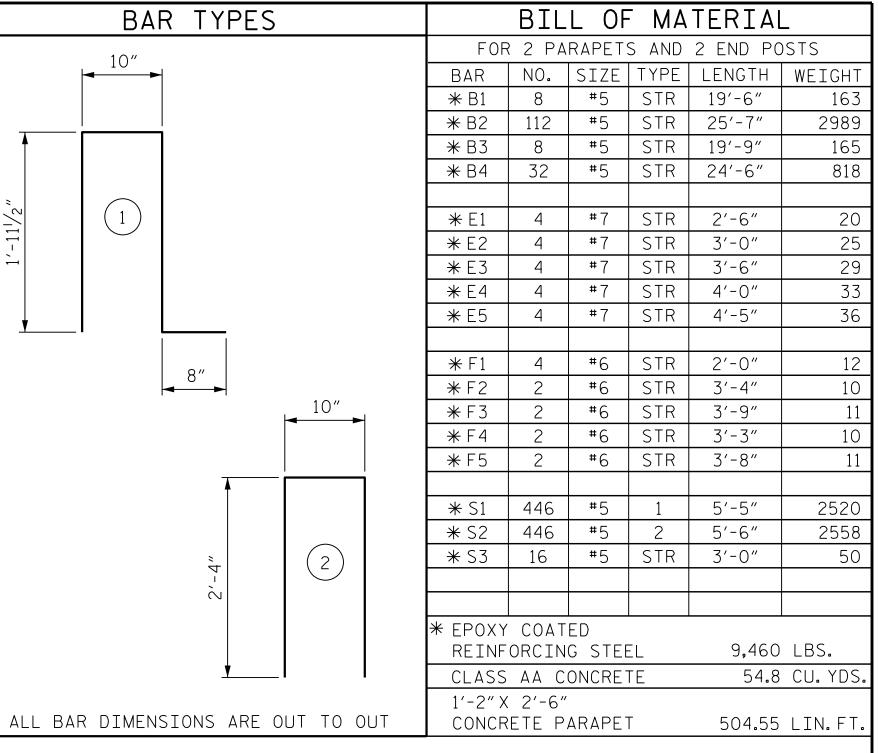
'-11¹/2' Ļ



END OF RAIL AND CONCRETE PARAPET DETAIL - RIGHT SIDE



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

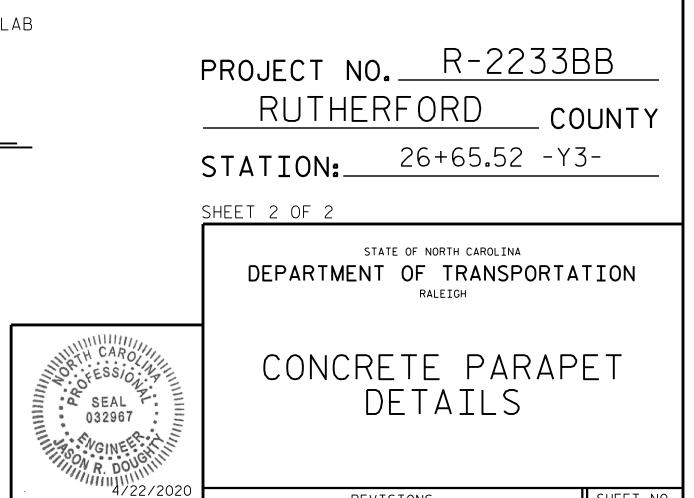


NOTES:

SHALL BE EPOXY COATED.

PARAPET IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN PARAPETS AND END POSTS

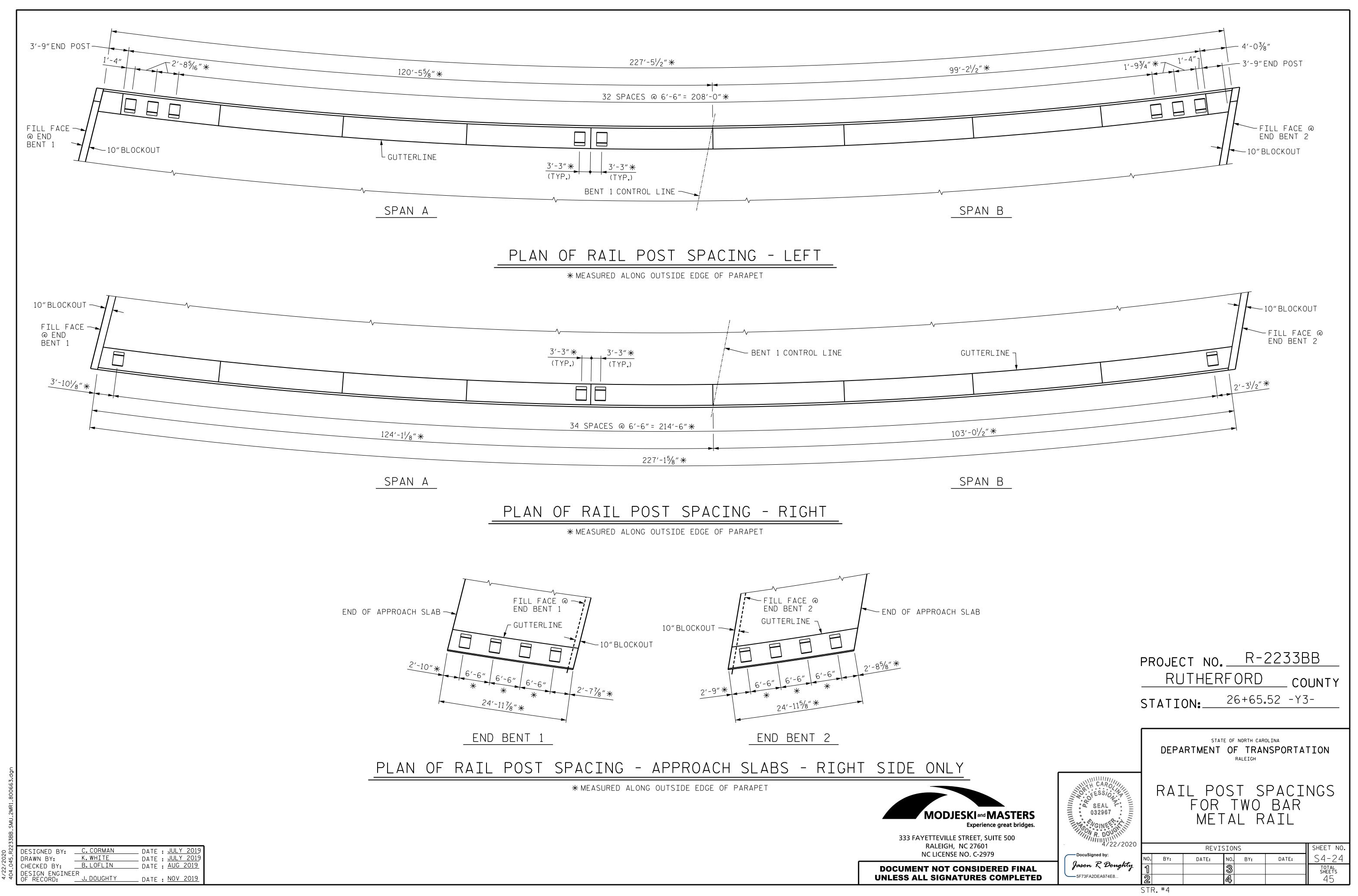


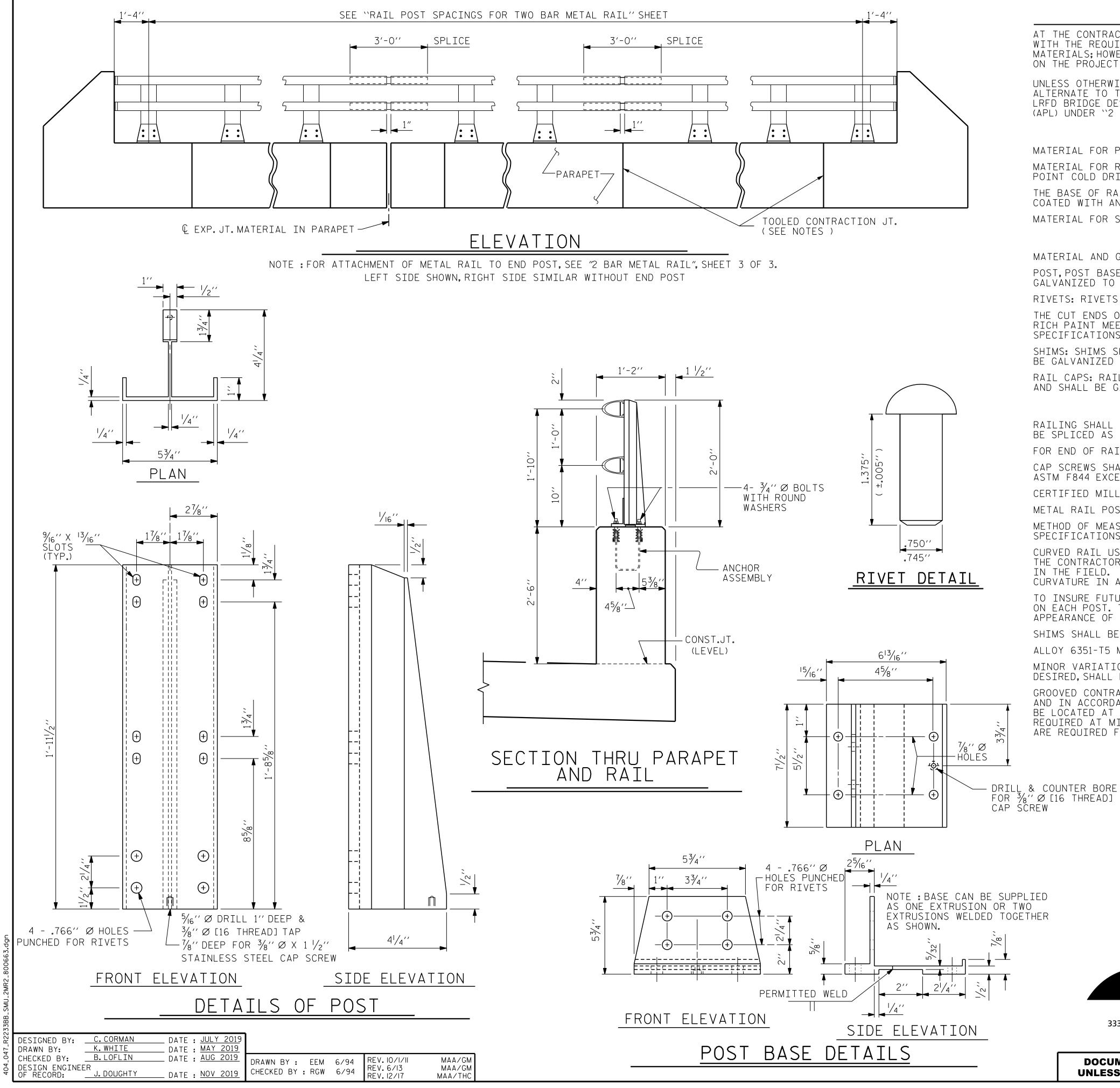
NO. BY: DATE: NO. BY: DA	
NO. DI: DATE: NO. DI: DA	E: \$4-23
1 3	TOTAL SHEETS
2 4	45

DocuSigned by

5F73FA2DEA974E8..

Jason R Doughty





+

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE OPTION TO USE AN ALTERNATE TO THE 2 BAR METAL RAIL. THE ALTERNATE RAIL SHALL MEET THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MUST BE LISTED ON THE DEPARTMENT'S APPROVED PRODUCTS LIST (APL) UNDER ``2 BAR METAL RAIL ALTERNATE''. ADJUSTMENTS TO THE CONCRETE PARAPET WILL NOT BE ALLOWED.

POINT COLD DRIVEN AS PER DRAWING. MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS. THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111. RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS. FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2. CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED. METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE. METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER. TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT. SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT. ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE. MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL. GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PAY LEI

Experience 333 FAYETTEVILLE STREET, SUI RALEIGH, NC 27601

NC LICENSE NO. C-2979

DOCUMENT NOT CONSIDER UNLESS ALL SIGNATURES C

NOTES

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

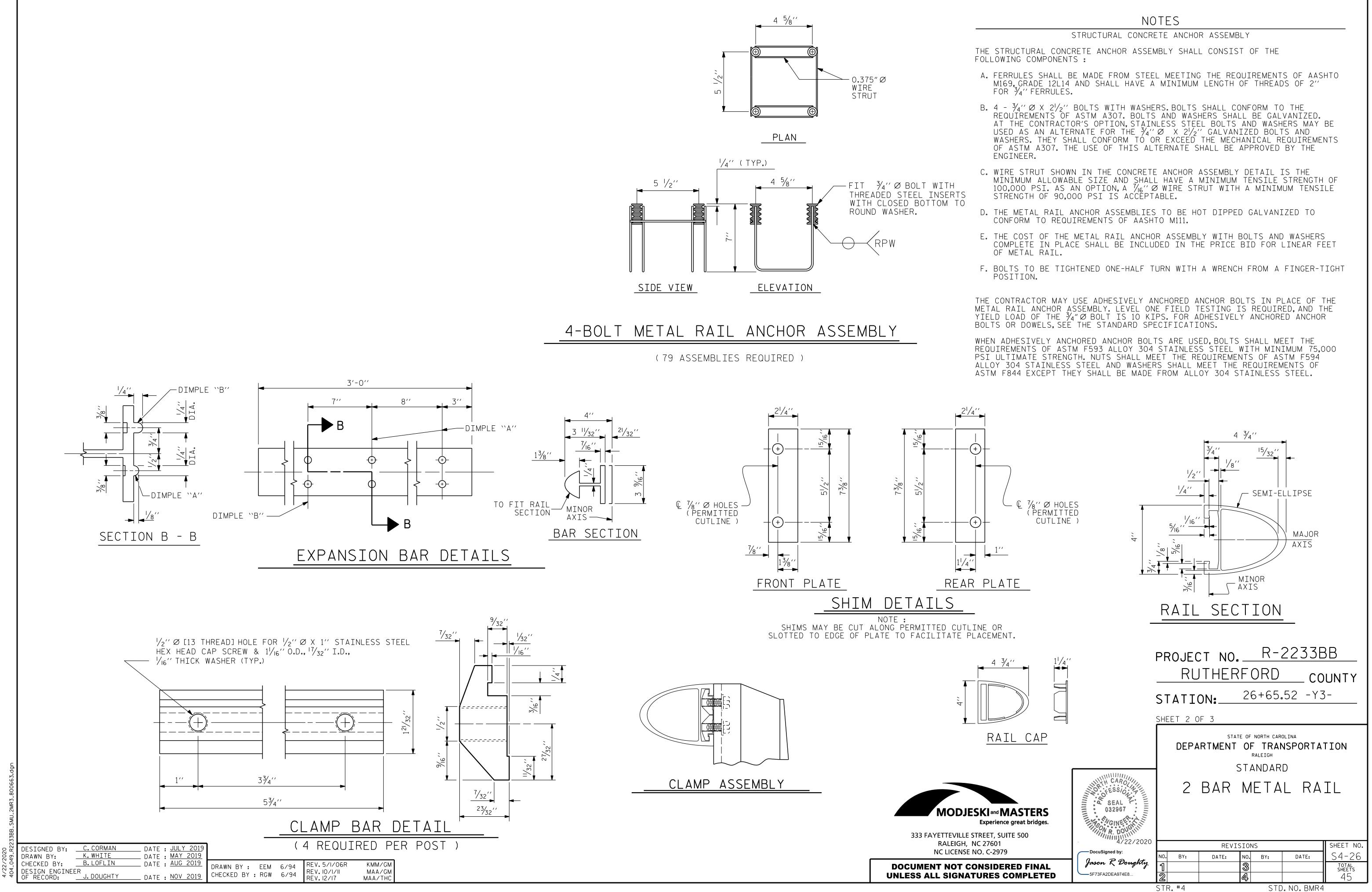
GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

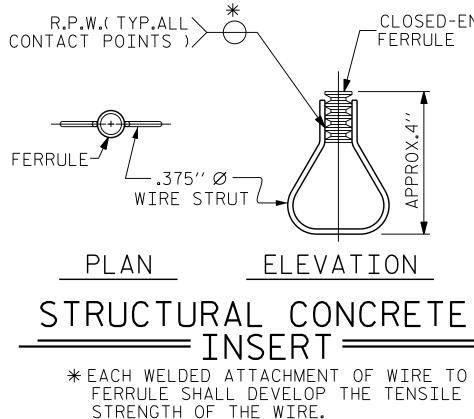
POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL -

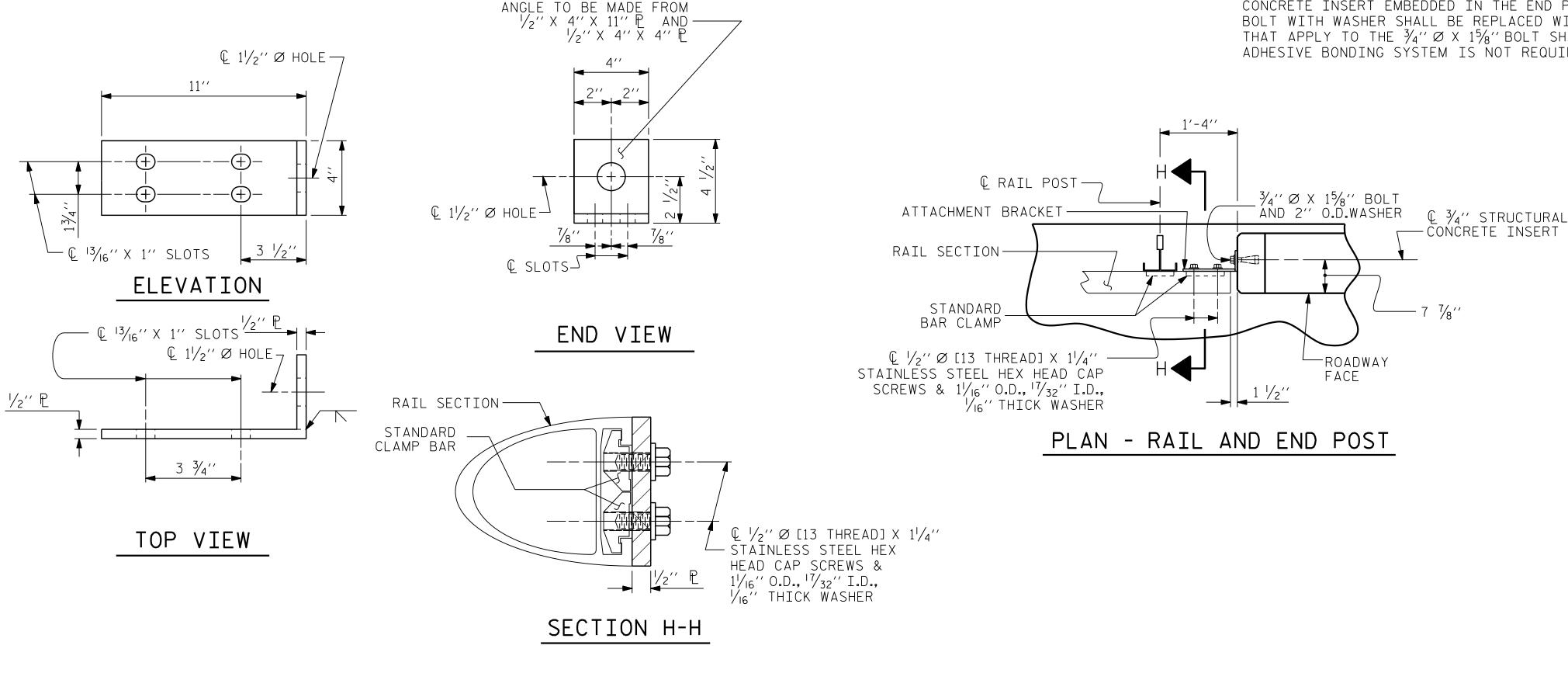
GENERAL NOTES

NGTH =	496.77 LIN.FT.					
(IN	CLUDES LENGTH ON PROACH SLABS)					
ALL	NUACIT SLADS/			<u> </u>		
		<u> </u>	JINER	KFURD	CO	UNTY
		STATI	0N:	26+65.	.52 -Y3	3
		SHEET 1 0	F 3			
		DEP		TE OF NORTH CAR OF TRAN RALEIGH		TION
		4		STANDAR	D	
	CARO	2	BAR	ΜΕΤΑ	L RA	IL
ASTERS great bridges.	032967					
TE 500	4/22/2020			SIONS		SHEET NO.
	DocuSigned by:	NO. BY:	DATE:	NO. BY:	DATE:	S4-25
RED FINAL OMPLETED	Jason R Doughty 5F73FA2DEA974E8	1		3 4		TOTAL SHEETS 45
	•	STR.#4	1	STE	.NO.BMR3	









S	
_	1

|--|

AU_2MR4				DETA
4/22/2020 404_050_R2233BB_SMU				
2020 0_R22	DESIGNED BY: <u>C.CORMAN</u> DATE : <u>AUG 2019</u> DRAWN BY: <u>K.WHITE</u> DATE : <u>AUG 2019</u>			
4/22/2 404_05	CHECKED BY: <u>B.LOFLIN</u> DATE : <u>AUG 2019</u> DESIGN ENGINEER OF RECORD: <u>J.DOUGHTY</u> DATE : <u>NOV 2019</u>	DRAWN BY : FCJ 1/88 CHECKED BY : CRK 3/89	REV. 5/1/06 TLA/GM REV. 10/1/11 MAA/GM REV. 12/17 MAA/THC	

+

- THE STRUCTURAL CONCRETE INSERT ASSE
- SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1^{1}/_{2}$.
- SHALL BE APPROVED BY THE ENGINEER.)

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- SHALL HAVE N.C. THREADS.
- D. STANDARD CLAMP BARS (SEE SHEET 2 OF 3).

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 2 BAR METAL RAILS.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST.IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " $\varnothing \times 6^{1}/2$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " Ø X 15%" BOLT SHALL APPLY TO THE $\frac{3}{4}$ " Ø X 6 $\frac{1}{2}$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

S FOR ATTACHING METAL RAIL TO END POST

Experience 333 FAYETTEVILLE STREET, SUI RALEIGH, NC 27601 NC LICENSE NO. C-2979

DOCUMENT NOT CONSIDER UNLESS ALL SIGNATURES C

CLOSED-END

	Ν	OTES				
TRUCI	URAL	CONCRETE	INS	SERT		
MBLY	SHALL	CONSIST	OF	THE	FOLLOWING	COMPONENTS:

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND

B. 1 - 3/4'' Ø X 15/8'' BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X 15%" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $7_{16}^{\prime\prime}$ Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.

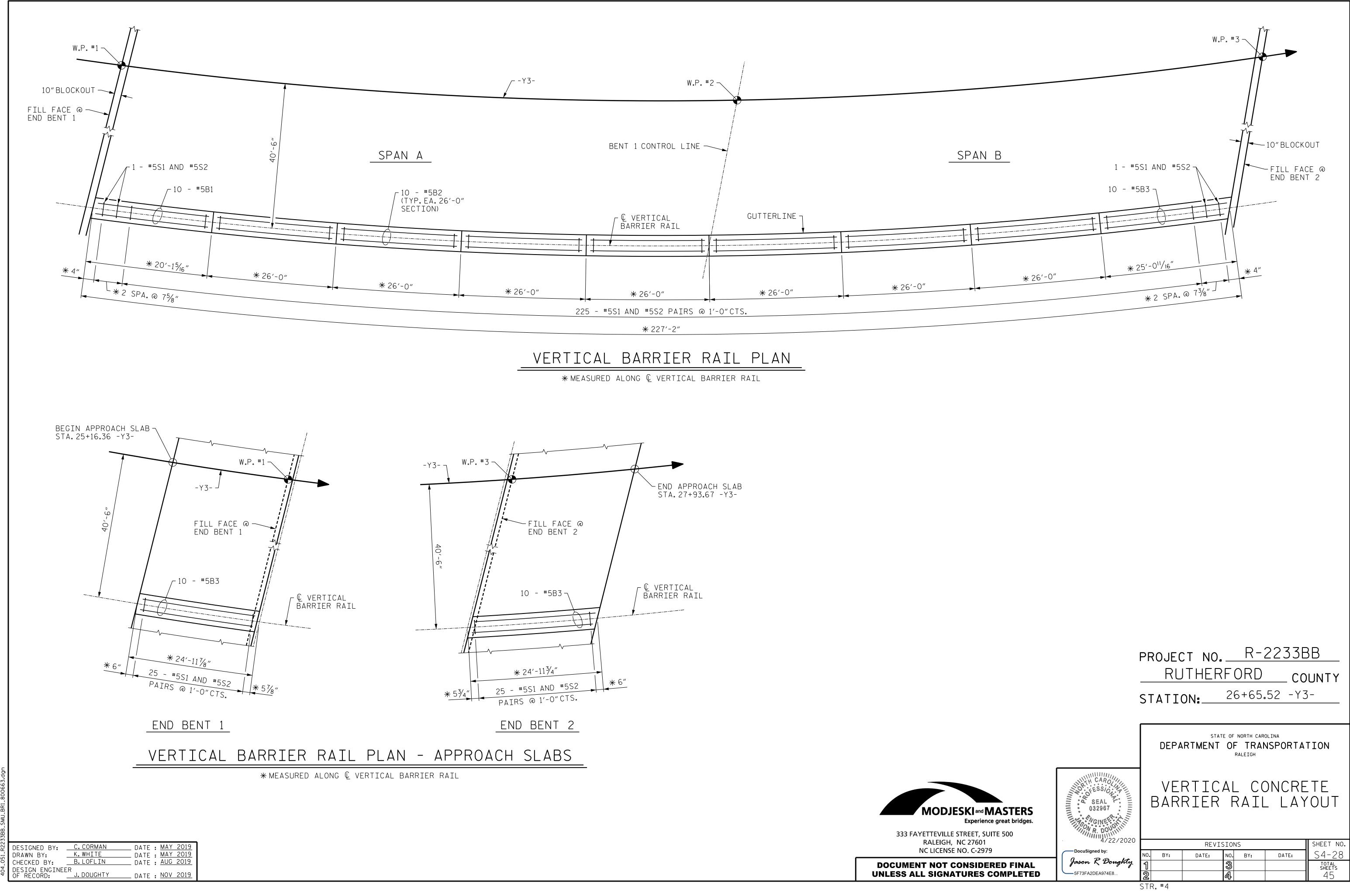
B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4" Ø X 15/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/4" Ø X 15/8" BOLT

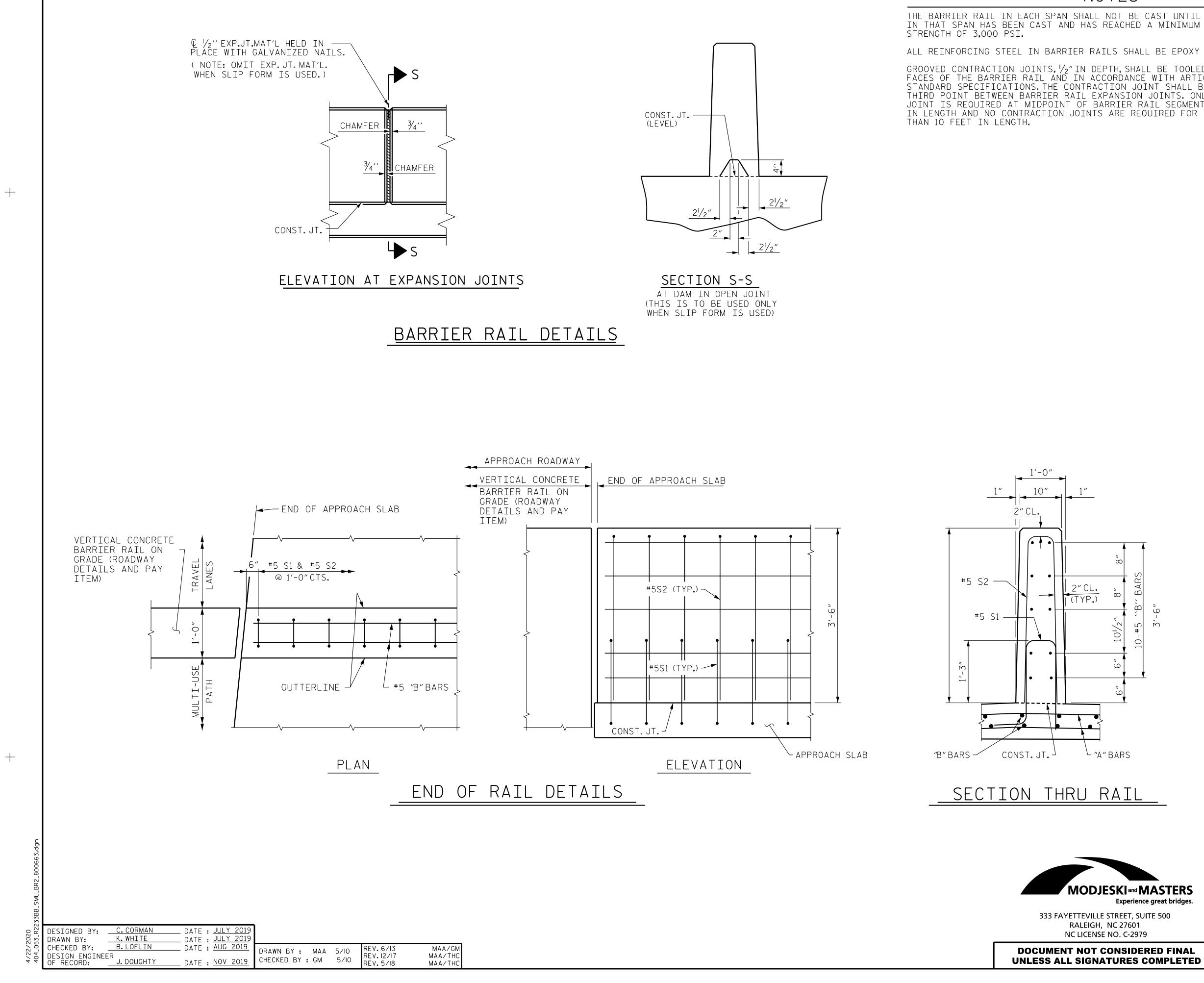
C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.

THE $\frac{3}{4}$ '' STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

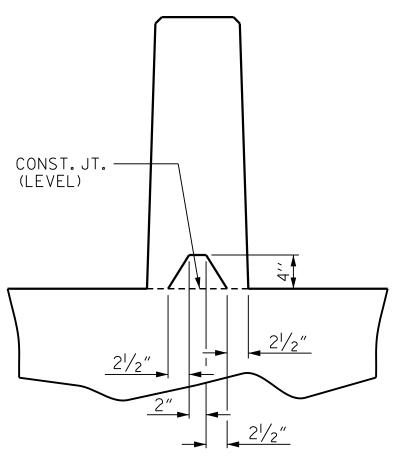
			THER	R-2 R-2 RFORD 26+65.	CO	UNTY
		SHEET 3 OF	- 3			
	SEAL SEAL		RTMENT	TE OF NORTH CARG OF TRAN RALEIGH STANDAR META	NSPORTA ⁻ D	
ASTERS e great bridges. TE 500	032967					
	4/22/2020			SIONS		SHEET NO.
RED FINAL OMPLETED	Jason R Doughty. 5F73FA2DEA974E8	NO. BY: 1 2	DATE:	NO. BY: 3 4	DATE:	S4-27 Total Sheets 45

STD.NO.BMR2





NOTES

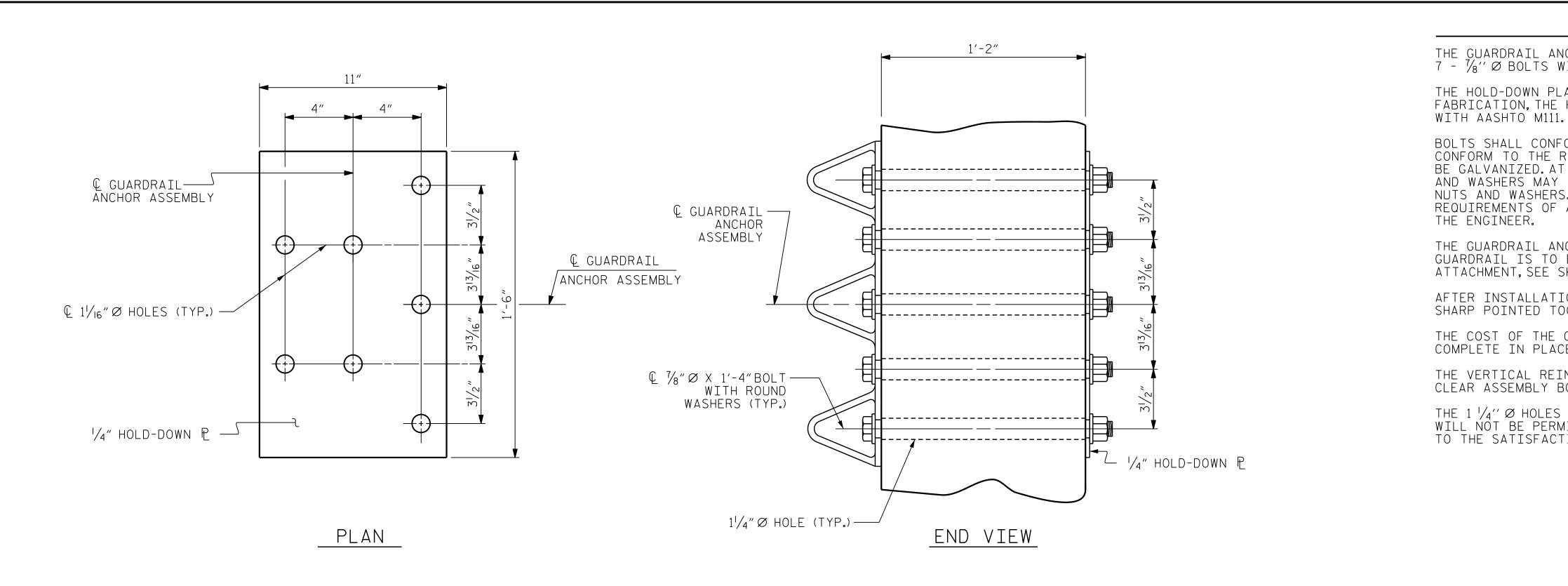


		F						
CAST UNTIL ALL) A MINIMUM COM	SLAB CONCRETE	-						
E WITH ARTICLE DINT SHALL BE L N JOINTS. ONLY (RAIL SEGMENTS L	ATED. N ALL EXPOSED 825-10(B) OF THE OCATED AT EACH ONE CONTRACTION ESS THAN 20 FEET SE SEGMENTS LESS		1′-9″	10"	1'-10"	r L	3'-4"	3'-4"
					1		73/4	\frown
			ALL	BAR DI	MENSI	ons a	RE OUT T	2) D OUT
		-	FOR VE	BIL rtical	L OF		ARRIER RA	
			BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
			* B1 * B2	10 70	#5 #5 #5	STR STR	19'-7" 25'-7" 24'-7"	204 1868
			* B3 * S1	30 279	+5 +5	STR 1	5'-0"	769 1455
			* S2	279	#5	2	7'-2"	2085
		÷		Y COAT Forcin			6,381	LBS.
			CLASS	AA CON CAL CON	NCRETE	-	32.94	CU. YDS.
		L	BARRIE	R RAIL			277.14	LIN.FT.
3,-6 3,-6								
		PR		t no Thef			2 <u>233</u> B	
			ATIC				<u> </u>	UNTY -
		<u> </u>	U	I N ē		_	-	
<u> </u>			DEPAF			NTH CAROL TRAN LEIGH	SPORTAT	ION
MASTERS nce great bridges. UITE 500	SEAL 032967 4/22/2020			BARI	CAL	СО	, NCRE RAIL	TE sheet no.
	DocuSigned by: Jason R Doughty	NO.	BY:	DATE:	NO.	BY:	DATE:	S4-29
ERED FINAL COMPLETED	5F73FA2DEA974E8	1 2			3 4			total sheets 45
		STR.	#4			STD.	NO.CBR2	

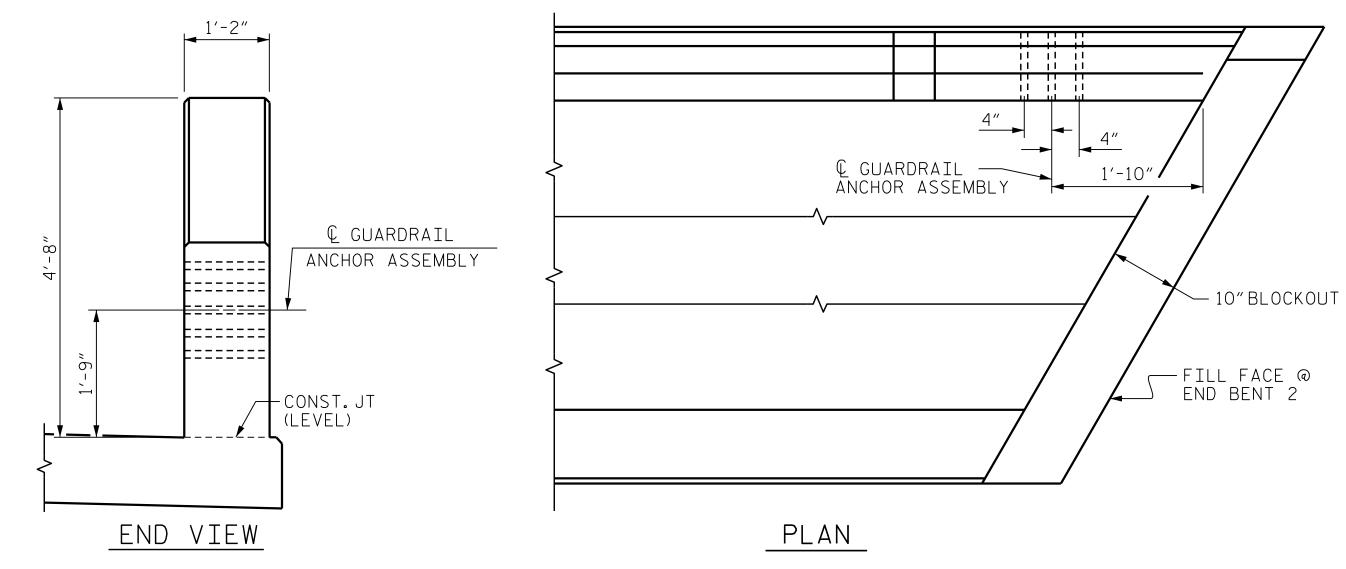
BAR TYPES

STR.#4

STD.NO.CBR2



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

DESIGNED BY:	C. CORMAN	DATE : <u>JULY 2019</u>				
DRAWN BY:	K.WHITE	DATE : JULY 2019				
CHECKED BY:	B.LOFLIN	DATE : <u>AUG 2019</u>	DRAWN BY : MAA	5/10	REV. 1/15	MAA/TMG
DESIGN ENGINE	ER		CHECKED BY : GM	5/10	REV. 12/17	MAA/THC
OF RECORD:	J. DOUGHTY	DATE : <u>NOV 2019</u>	CHECKED DI : OM	5710	REV. 5/18	MAA/THC

+

22



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " hold down plate and 7 - $\frac{7}{8}$ " Ø Bolts with nuts and washers.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

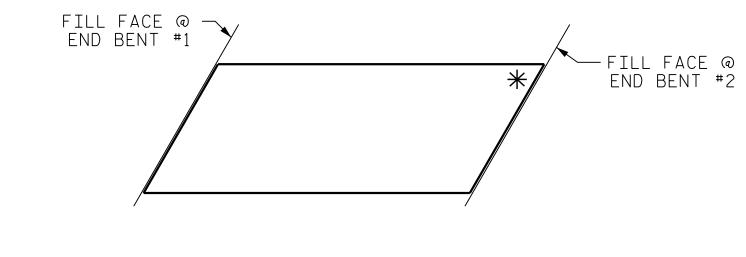
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE. SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO	D. <u>R-22</u>	33BB
RUTHE	RFORD	_ COUNTY
STATION:	26+65.52	-Y3-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

GUARDRAIL ANCHORAGE FOR BARRIER RAIL

4/22/2020							
			REV]	ISION	S		SHEET NO.
igned by: , R Donghty	N0.	BY:	DATE:	NO.	BY:	DATE:	S4-30
	1			3			TOTAL SHEETS
A2DEA974E8	2			4			45
	стр	± ∕I			стл		

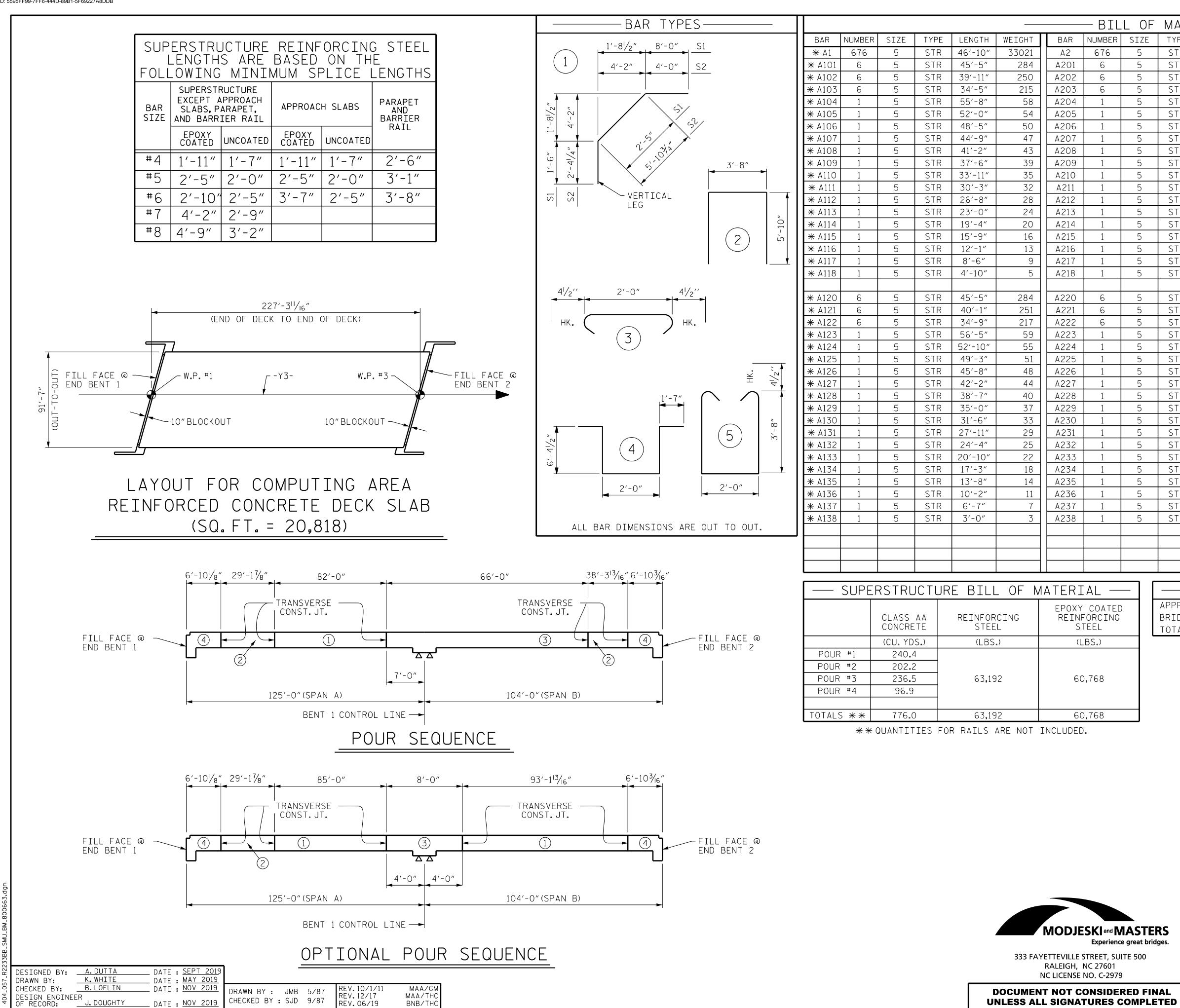
SEAL

032967 THE WOINEER

Jason R Doughty

DocuSigned by:

STA. NO. GRA3



+

_ OF size	ΜΔΤΕ	ERIAL -							
JILL	TYPE	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
5	STR	46′-8″	32903	★ B1	183	5	STR	24'-10"	4740
5	STR	45′-2″	283	₩ B2	126	4	STR	31'-3"	2630
5	STR	39′-9″	249	₩ B3	63	5	STR	50'-0"	3285
5	STR	34'-4"	215	₩ B4	63	5	STR	31'-2"	2048
5	STR	55′-8″	58	₩ B5	120	5	STR	36′-6″	4568
5	STR	52'-0"	54	₩ B6	126	4	STR	25'-10"	2174
5	STR	48′-5″	50	<u>₩</u> B7	183	5	STR	20'-10"	3976
5	STR	44'-9"	47	B8	292	5	STR	58'-4"	17766
5	STR	41'-2"	43	B9	54	5	STR	24'-10"	1399
5	STR	37'-6"	39	B10	54	5	STR	36'-6"	2056
5	STR	33'-11"	35	B11	54	5	STR	20'-10"	1173
5	STR	30'-3"	32						
5	STR	26'-8"	28	K1	42	4	STR	32'-3"	905
5	STR	23'-0"	24	К2	36	4	STR	5′-6″	132
5	STR	19'-4"	20	К3	180	4	STR	8'-7"	1032
5	STR	15′-9″	16	K4	36	4	STR	6'-11"	166
5	STR	12'-1"	13	К5	1	4	STR	1'-1"	1
5	STR	8'-6"	9	К6	5	4	STR	2'-5"	8
5	STR	4'-10"	5	K7	1	4	STR	1'-8"	1
				K8	1	4	STR	1'-5"	1
5	STR	45'-3"	283	K9	5	4	STR	2'-9"	9
5	STR	39'-11"	250	К10	1	4	STR	2'-0"	1
5	STR	34'-7"	216	K11	1	4	STR	1'-11"	1
5	STR	56'-5"	59	K12	5	4	STR	3'-4"	11
5	STR	52'-10"	55	K13	1	4	STR	2'-6"	2
5	STR	49'-3"	51	K14	1	4	STR	2'-2"	1
5	STR	45'-8"	48	K15	5	4	STR	3'-7"	12
5	STR	42'-2"	44	K16	1	4	STR	2'-9"	2
5	STR	38'-7"	40	K17	21	4	STR	30'-7"	429
5	STR	35'-0"	37		4.4.0				0.5-
5	STR	31'-6"	33	* S1	116	4	1	11'-11"	923
5	STR	27'-11"	29	<u>* S2</u>	114	4	1	12'-3"	933
5	STR	24'-4"	25	<u>S3</u>	116	4	2	15'-4"	1188
5	STR	20'-10"	22	<u>S4</u>	414	4	3	2'-9"	761
5	STR	17'-3"	18		10			4.0 / 4 //	
5	STR	13'-8"	14	U1	18	4	5	10'-1"	121
μ, '	STR	10'-2"	11	U2	54	4	4	17'-11"	646
5	6 T D		7						
5	STR	6'-7" 3'-0"	7						
	STR STR	6'- <i>1"</i> 3'-0"	3						
5			3		NFORCING		=	63,1	93 LBS.
5			3	* EPO>	KY COATE	ED			
5			3	* EPO>		ED			93 LBS. 70 LBS.
5	STR	3'-0"		* EPO> REIN	(Y COATE NFORCING	ED ; steel			
5	STR (3'-0" GROOVI	ING BR	* EPO> REIN	(y coate Nforcing FLOOF	STEEL	-		
5	STR (APPROA	3'-0" GROOVI ch slabs	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96	STEEL	= -		
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE NFORCING FLOOF 3,96 18,61	STEEL STEEL S S 4 SQ. F1 4 SQ. F1			
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	 	60,7	70 LBS.
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	* EPO> REIN	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	 	60,7	70 LBS.
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	= - - - - - - - - - - - - - - - - - - -		70 LBS.
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	= - - - - - - - - - - - - - - - - - - -	60,7 ⁻	70 LBS. B
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	= 	60,7 ⁻ 2 <u>233B</u> COL	70 LBS. B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE FORCING 5,96 18,61 22,57	STEEL STEEL S S S S S S S S S S S S S S S S S S	= 	60,7 ⁻	70 LBS. B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE NFORCING 5,96 18,61 22,57 ECT N CUTH	STEEL STEEL S S S S S S S S S S S S S S S S S S	= 	60,7 ⁻ 2 <u>233B</u> COL	70 LBS. B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE NFORCING 5,96 18,61 22,57 ECT N CUTH	STEEL STEEL S S S S S S S S S S S S S S S S S S	= 	60,7 ⁻ 2 <u>233B</u> COL	70 LBS. B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	PROJ	(Y COATE NFORCING 5,96 18,61 22,57 ECT N CUTH	STEEL STEEL S S S S S S S S S S S S S S S S S S	= 	60,7 2338 COL 52 -Y3-	70 LBS. B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	REIN	FLOOF 3,96 18,61 22,57	STEEL STEEL STEEL STATE OF N STATE OF N	= R – 2 NRD + 65.5 NORTH CAROL	60,7 2338 COL 52 -Y3-	B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	REIN	FLOOF 3,96 18,61 22,57	STEEL STEEL STEEL STATE OF N STATE OF N STATE OF N R	$= \frac{R - 2}{R - 2}$ $R - 2$	60,7 2233B COL 52 -Y3-	B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	REIN	FLOOF 3,96 18,61 22,57	STEEL STEEL STEEL STATE OF N STATE OF N STATE OF N R	= R – 2 NRD + 65.5 NORTH CAROL	60,7 2233B COL 52 -Y3-	B JNTY
5	STR (APPROA BRIDGE	3'-0" GROOVI Ch slabs deck	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME	STEEL STEEL STEEL STEEL STATE OF N STATE OF N STATE OF N STA	$= \frac{R - 2}{R - 2}$ $R - 2$	60,7 233B COL 52 -Y3-	O LBS.
5	STR (APPROA BRIDGE	3'-O"	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME SUP	STATE OF N STATE OF N STATE OF N STATE OF N STATE OF N STATE	= R - 2 NRD + 65.5 NORTH CAROL TRANS	60,7 233B COL 52 -Y3-	DINTY
	STR APPROA BRIDGE TOTAL	3'-O"	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME SUP	STATE OF N STATE OF N STATE OF N STATE OF N STATE OF N STATE	= R - 2 NRD + 65.5 NORTH CAROL TRANS	60,7 233B COL 52 -Y3-	DINTY
	STR APPROA BRIDGE TOTAL -	3'-O"	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME SUP	STATE OF N STATE OF N STATE OF N STATE OF N STATE OF N STATE	= R - 2 NRD + 65.5 NORTH CAROL TRANS	60,7 233B COL 52 -Y3-	DINTY
	STR APPROA BRIDGE TOTAL -	3'-O"	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME SUP	STATE OF N STATE OF N STATE OF N STATE OF N STATE OF N STATE	= R - 2 NRD + 65.5 NORTH CAROL TRANS	60,7 233B COL 52 -Y3-	DINTY
	STR APPROA BRIDGE TOTAL -	3'-O"	ING BR	REIN	FLOOF 3,96 18,61 22,57 ECT N UTH ION: PARTME SUP SUP SILL	STATE OF N STATE OF N STATE OF N STATE OF N STATE OF N STATE	= R - 2 R - 2	60,7 233B 233B COL 52 -Y3- 52 -Y3-	DINTY

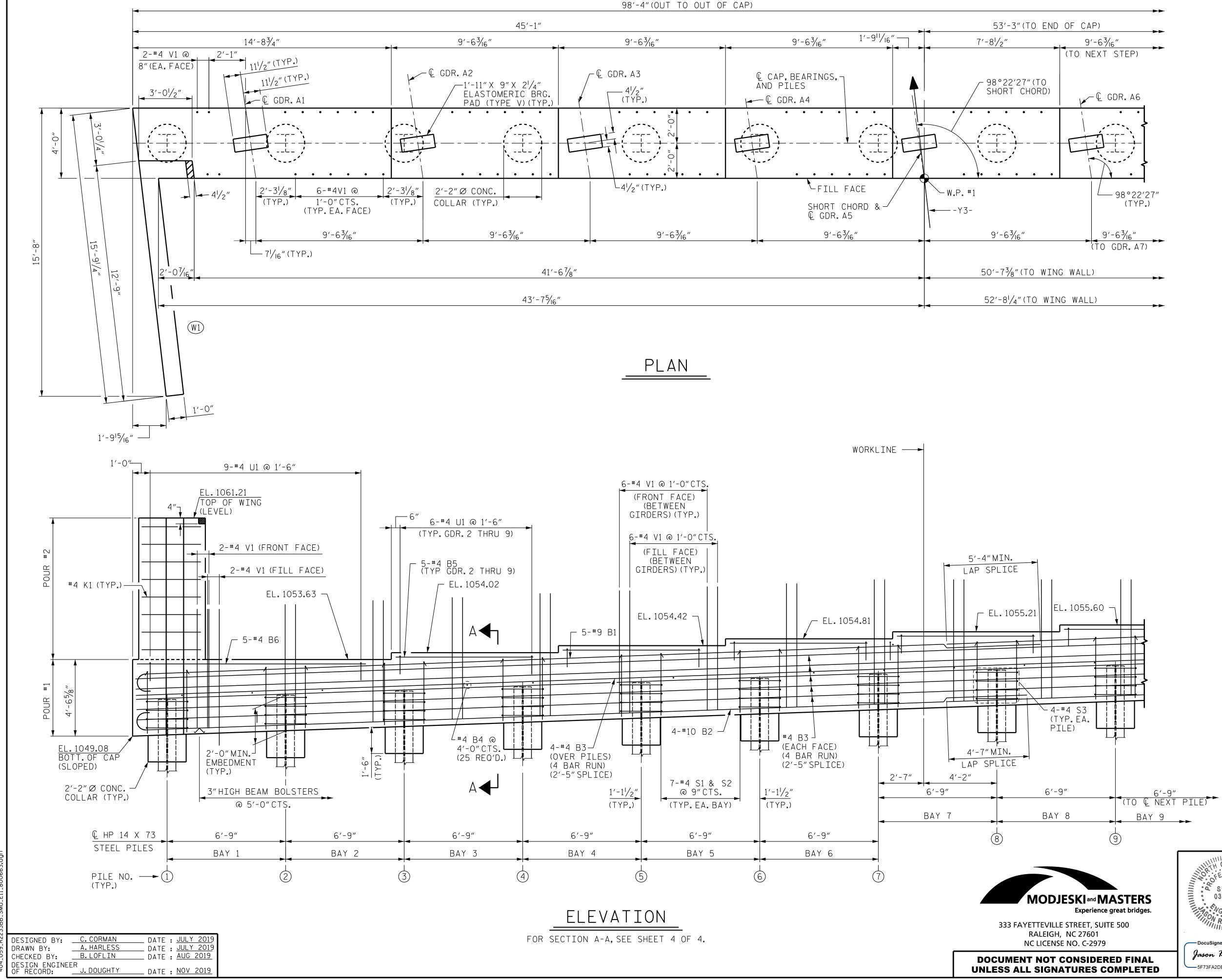
STR.#4

Jason R Doughty

STD.NO.BOM2

TOTAL SHEETS

45



+



THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCEPT TO THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE PARAPET IS CAST IF SLIP FORMING IS USED.

TOP OF	PILE		
ELEVAT	IONS		
PILE NO.	ELEVATION		
1	1051.15		
2	1051.41		
3	1051.66		
4	1051.91		
5	1052.16		
6	1052.41		
7	1052.66		
8	1052.91		
9	1053.16		
10	1053.42		
11	1053.67		
12	1053.92		
13	1054.17		
14	1054.42		
15	1054.67		

PROJECT NO. R-2233BB RUTHERFORD COUNTY 26+65.52 -Y3-STATION:

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE

END BENT 1

		SHEET NO.				
NO.	BY:	DATE:	N0.	BY:	DATE:	S4-32
1			3			TOTAL SHEETS
2			4			45
C T F	? #⊿					

SIR.#4

CARO

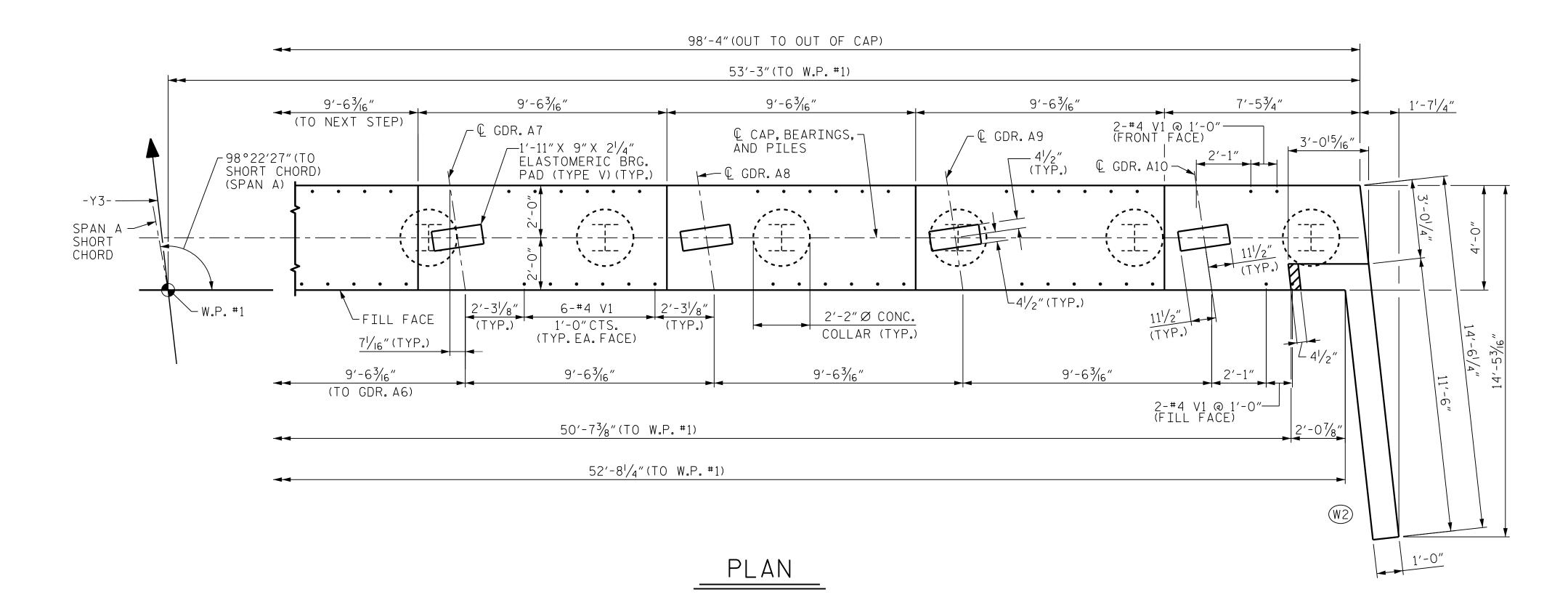
032967

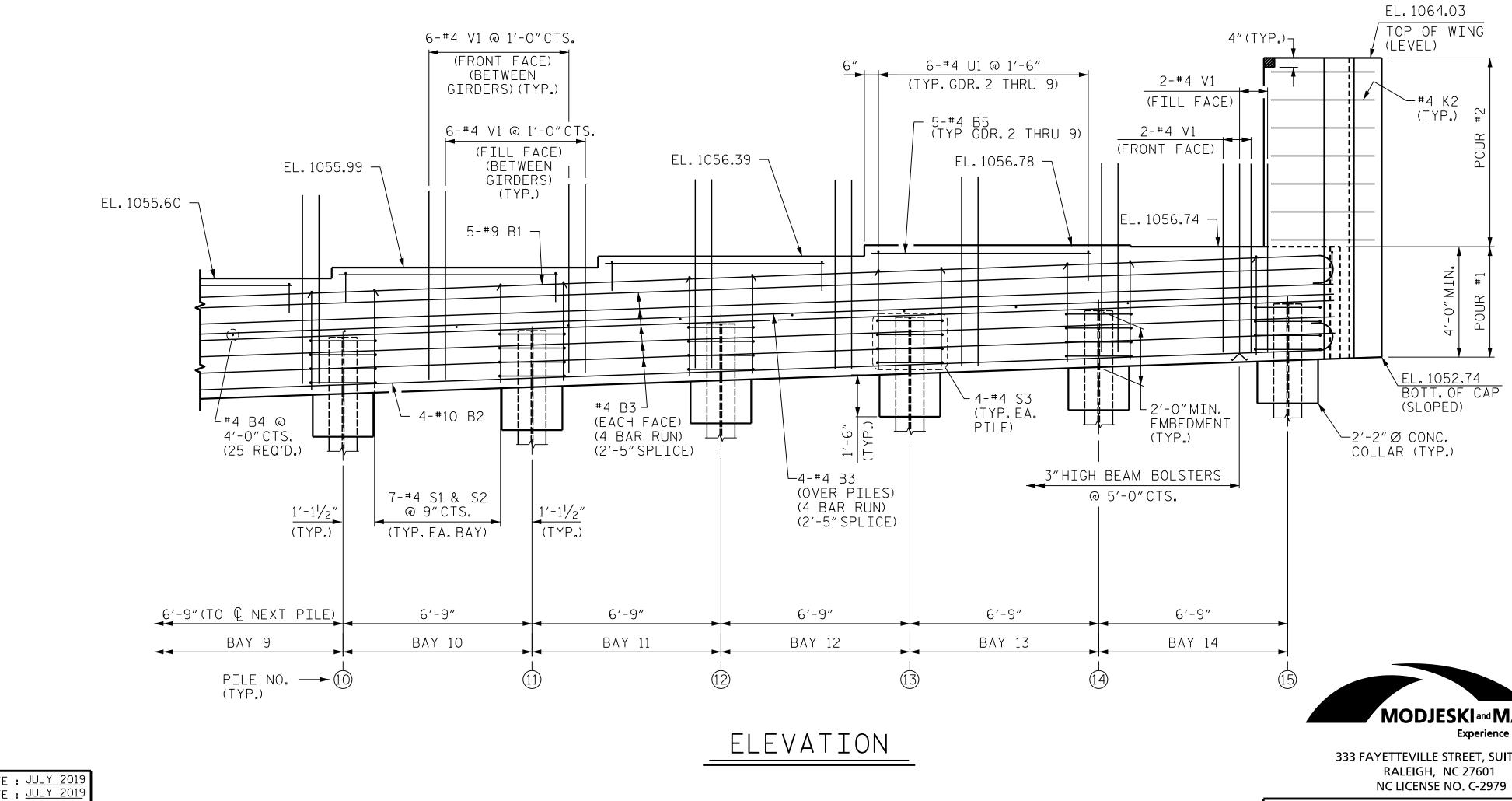
THE REPORT

Jason R Doughty

DocuSigned by:

4/22/2020





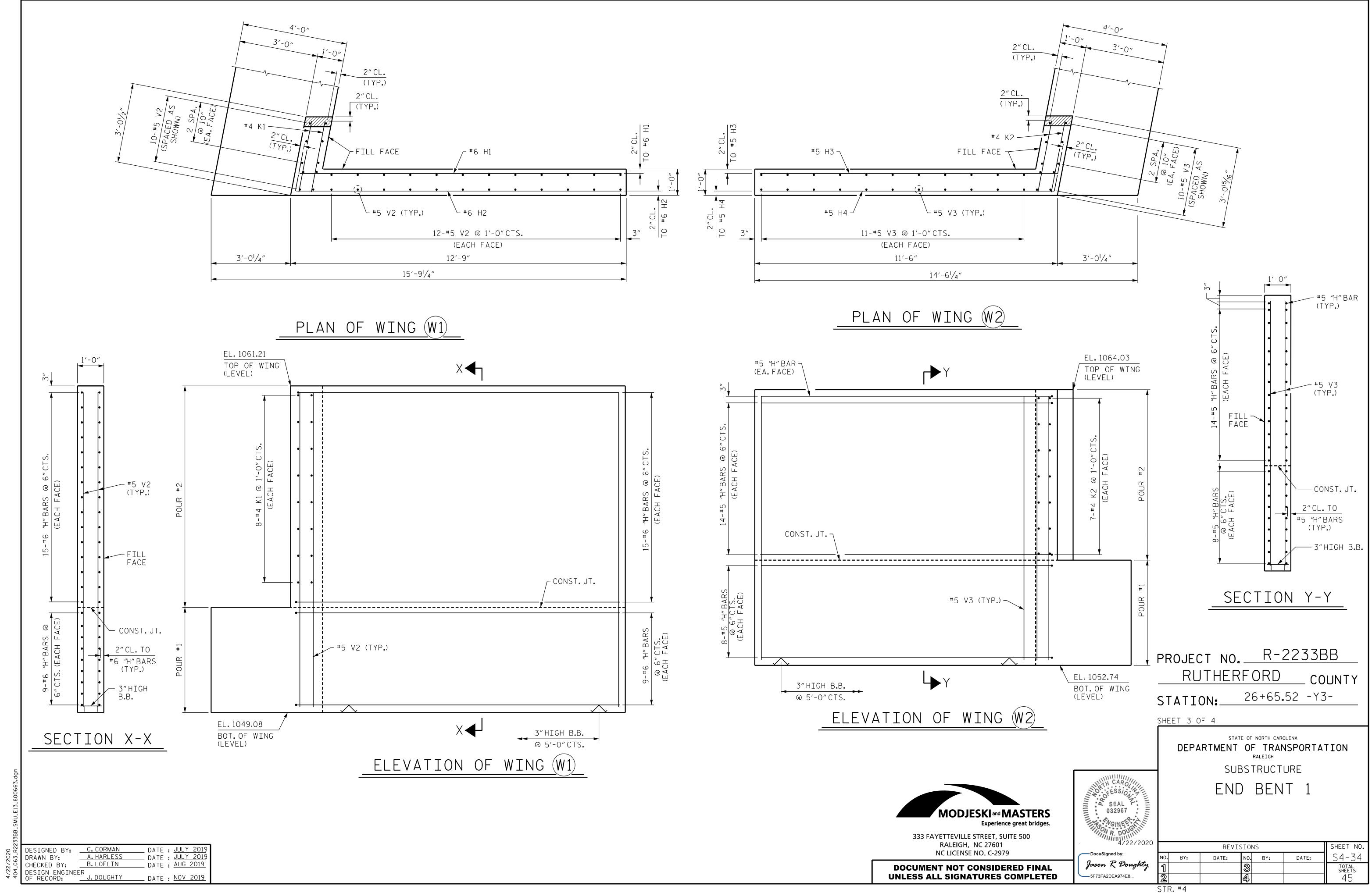


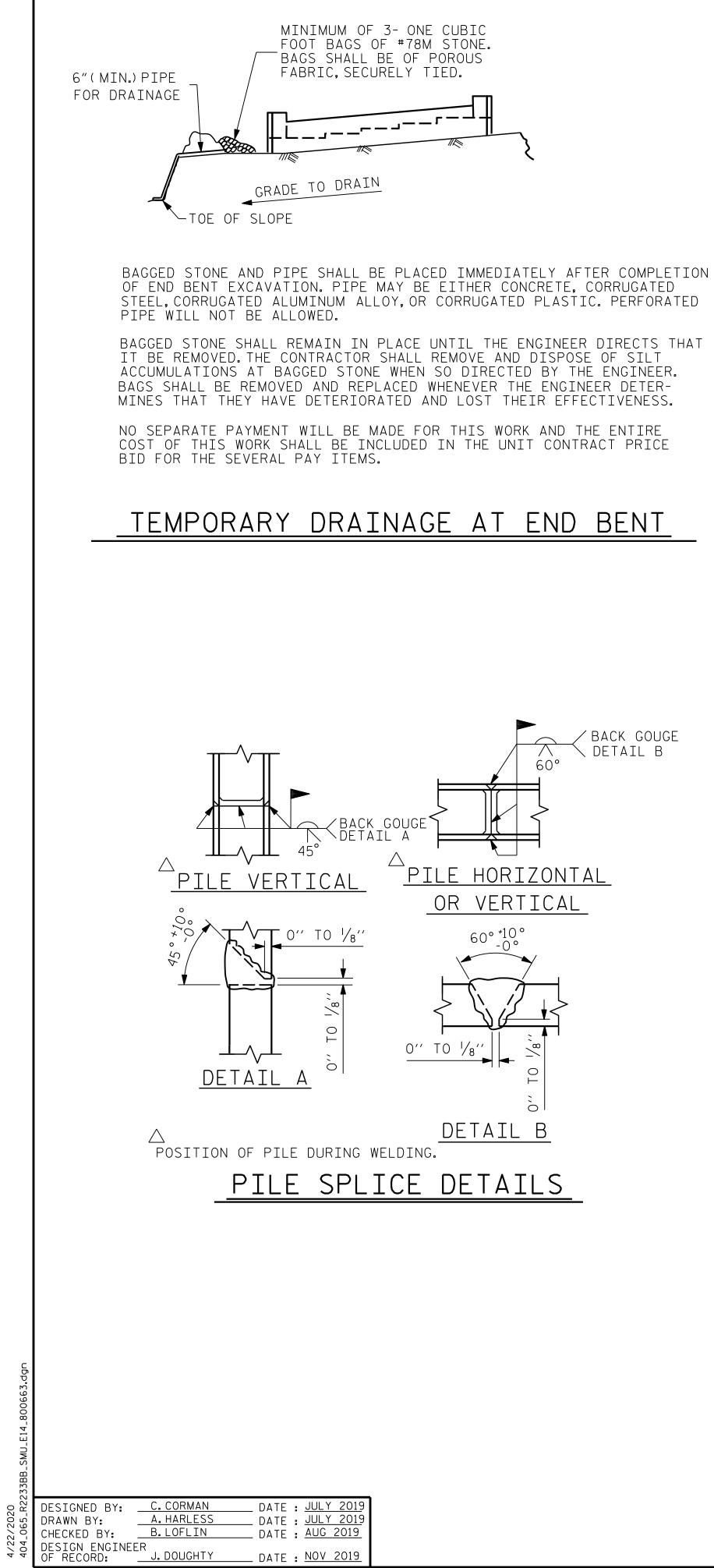
+

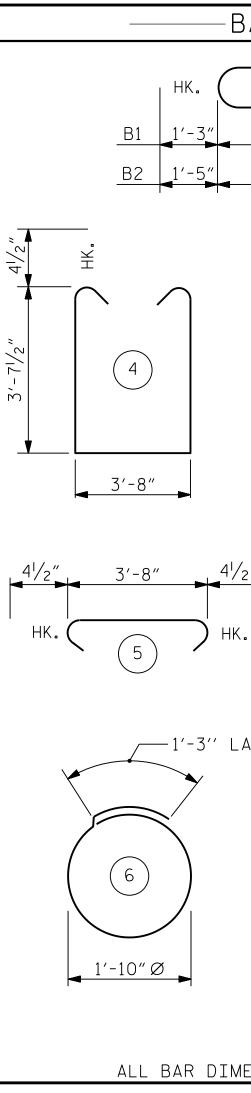
DESIGNED BY:	C. CORMAN	DATE	: <u>JULY 201</u>
	A.HARLESS		: JULY 2019
	B.LOFLIN		AUG 2019
DESIGN ENGINEER OF RECORD:	R J. DOUGHTY	DATE	<u>: NOV 2019</u>
	DESIGN ENGINEER	DRAWN BY: <u>A.HARLESS</u> CHECKED BY: <u>B.LOFLIN</u> DESIGN ENGINEER	DRAWN BY: <u>A.HARLESS</u> DATE CHECKED BY: <u>B.LOFLIN</u> DATE DESIGN ENGINEER

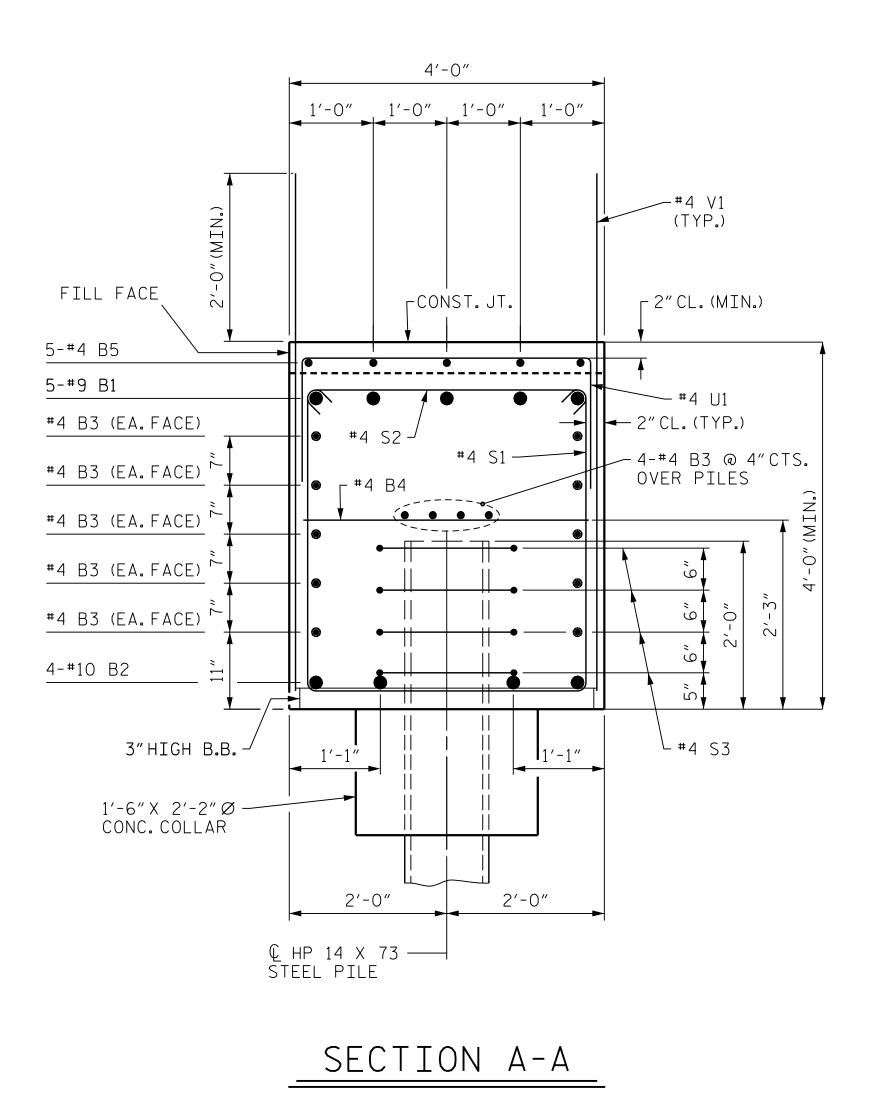
DOCUMENT NOT CONSIDER UNLESS ALL SIGNATURES CO

		PROJECT			2233E	3B
		<u> </u>	HERF	URD	CO	UNTY
		STATION	:2	26+65.	52 -Y3	5 -
		SHEET 2 OF	4			
	CAROLINI CAROLINI CESSION	DEPART	MENT C	DF NORTH CAR DF TRAN RALEIGH STRUCT BEN	NSPORTA URE	TION
ASTERS great bridges. TE 500	SEAL 032967					
	DocuSigned by:		REVISI			SHEET NO.
RED FINAL OMPLETED	Jason R Doughty. 5F73FA2DEA974E8	NO. ВҮ: 1 2	DATE: NO	3	DATE:	S4-33 total sheets 45
		STR #4		'		







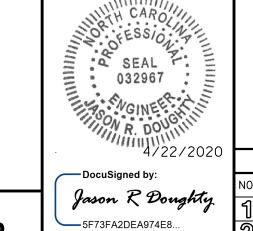




DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

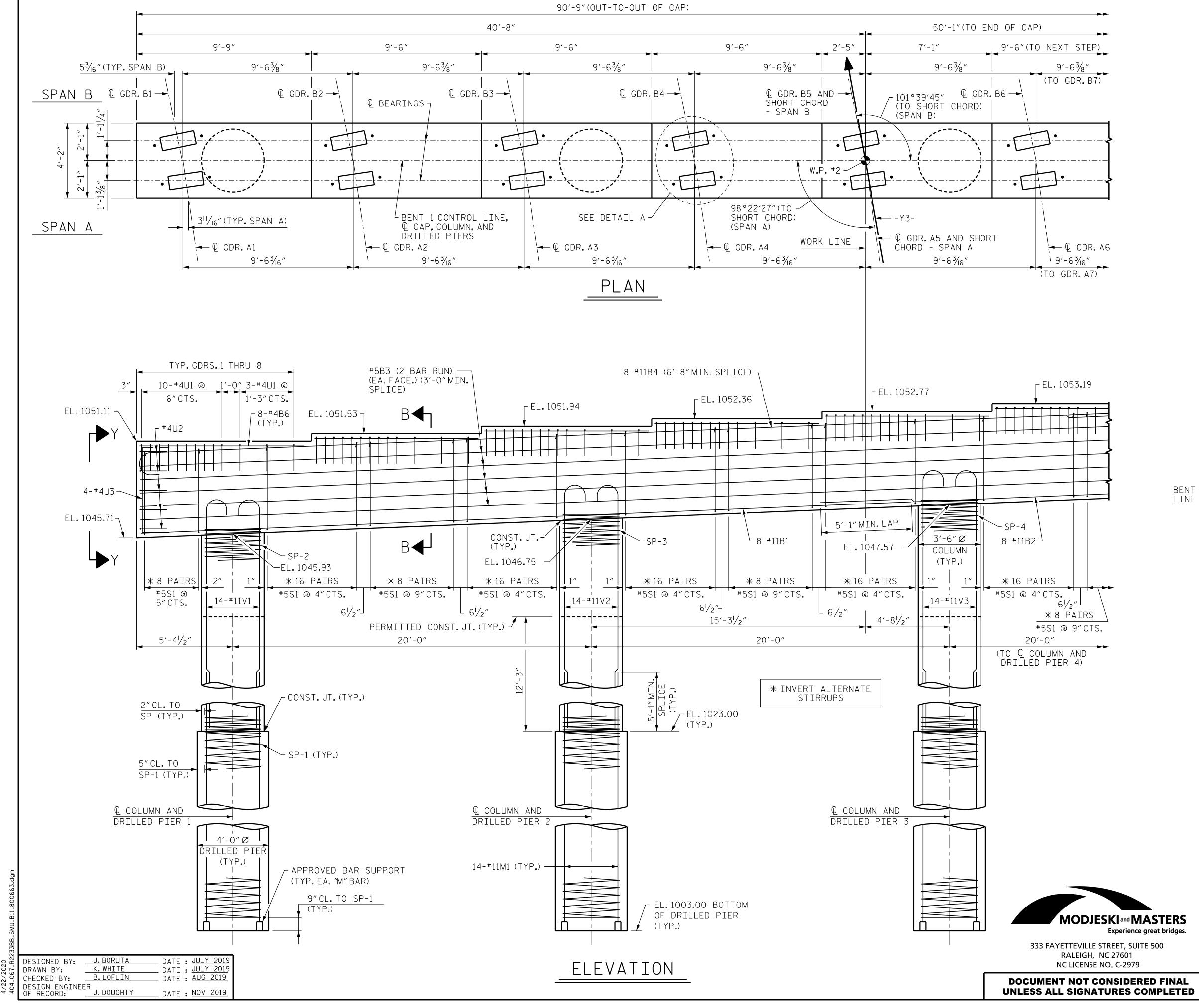
$\begin{array}{c} 1\\ 1\\ 51'-8''\\ 51'-8''\\ 51'-4''\\ \end{array}$
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$\begin{array}{c} & & \\ & 51'-8'' \\ & 51'-4'' \\ \hline \\ & 51'$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
51'-4'' $B5 40 #4 STR 8'-0''$ $B6 5 #4 STR 13'-3''$ $H1 24 #6 2 13'-2''$ $H2 24 #6 2 13'-3''$ $H3 23 #5 3 12'-3''$ $H4 23 #5 3 12'-2''$ $H1 16 #4 STR 2'-8''$ $S2 98 #4 4 11'-8''$ $S2 98 #4 6 7'-0''$ $H3 11'-3''$ $H3 11'-3''$ $H4 11'-2''$ $H3 11'-3''$ $H4 23 +5 STR 11'-8''$ $S3 60 #4 6 7'-0''$ $H3 11'-3''$ $H3 11'-3''$ $H4 11'-2''$ $H3 11'-3''$ $H4 23 +5 STR 11'-8''$ $S3 2 +5 STR 10'-10''$ $REINFORCING STEEL LBS.$ $CLASS A CONCRETE$ $POUR #1 CAP, LOWER WINGS C.Y.$ $& CONC. COLLARS$ $POUR #2 UPPER PART OF C.Y.$
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
H1 12'-2" H2 12'-3" H2 12'-3" H2 12'-3" H3 11'-3" H4 11'-2" H3 11'-2" H3 11'-2" H3 11'-2" H4 11'-2" H4 11'-2" H5 1 98 #4 4 11'-8" S2 98 #4 5 4'-5" S3 60 #4 6 7'-0" U1 57 #4 7 6'-8" U1 57 #4 7 6'-8" V1 116 #4 STR 6'-10" V2 34 #5 STR 11'-8" V3 32 #5 STR 10'-10" REINFORCING STEEL LBS. CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$\begin{array}{c c c c c c c c c c c c c c c c c c c $
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
3 3 11'-3" H3 11'-3" V1 116 #4 STR 6'-10" V2 34 #5 STR 11'-8" V3 32 #5 STR 10'-10" H4 11'-2" REINFORCING STEEL LBS. 9 CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. 8 CONC. COLLARS POUR #2 UPPER PART OF C.Y.
3 3 11'-3" H3 11'-3" V1 116 #4 STR 6'-10" V2 34 #5 STR 11'-8" V3 32 #5 STR 10'-10" H4 11'-2" REINFORCING STEEL LBS. S CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
H3 $11'-3''$ H4 $11'-2''$ H4 $11'-2''$ H4 $11'-2''$ REINFORCING STEEL LBS. S CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
H3 $11'-3''$ H4 $11'-2''$ H4 $11'-2''$ H4 $11'-2''$ REINFORCING STEEL LBS. S CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
H3 $11'-3''$ H4 $11'-2''$ H4 $11'-2''$ H4 $11'-2''$ REINFORCING STEEL LBS. S CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
H4 11'-2" REINFORCING STEEL LBS. 9 CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
CLASS A CONCRETE POUR #1 CAP, LOWER WINGS C.Y. & CONC. COLLARS POUR #2 UPPER PART OF C.Y.
POUR #1 CAP,LOWER WINGS C.Y. & CONC.COLLARS POUR #2 UPPER PART OF C.Y.
POUR #1 CAP,LOWER WINGS C.Y. & CONC.COLLARS POUR #2 UPPER PART OF C.Y.
& CONC. COLLARS
POUR #2 UPPER PART OF C.Y.
POUR #2 UPPER PART OF C.Y.
WINGS
$\overline{\tau}$ 1 1 1
TOTAL CLASS A CONCRETE C.Y.
HP 14×73 STEEL PILES
NO.15 LIN.FT.
PILE DRIVING EQUIPMENT
SETUP FOR HP 14×73
ISIONS ARE OUT TO OUT. STEEL PILES NO.

END BENT 1



10000							
/2020			SHEET NO.				
14	NO.	BY:	DATE:	NO.	BY:	DATE:	S4-35
ghty	1			3			TOTAL SHEETS
	2			4			45
	CTD	⊃ # <i>1</i>					

+



NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

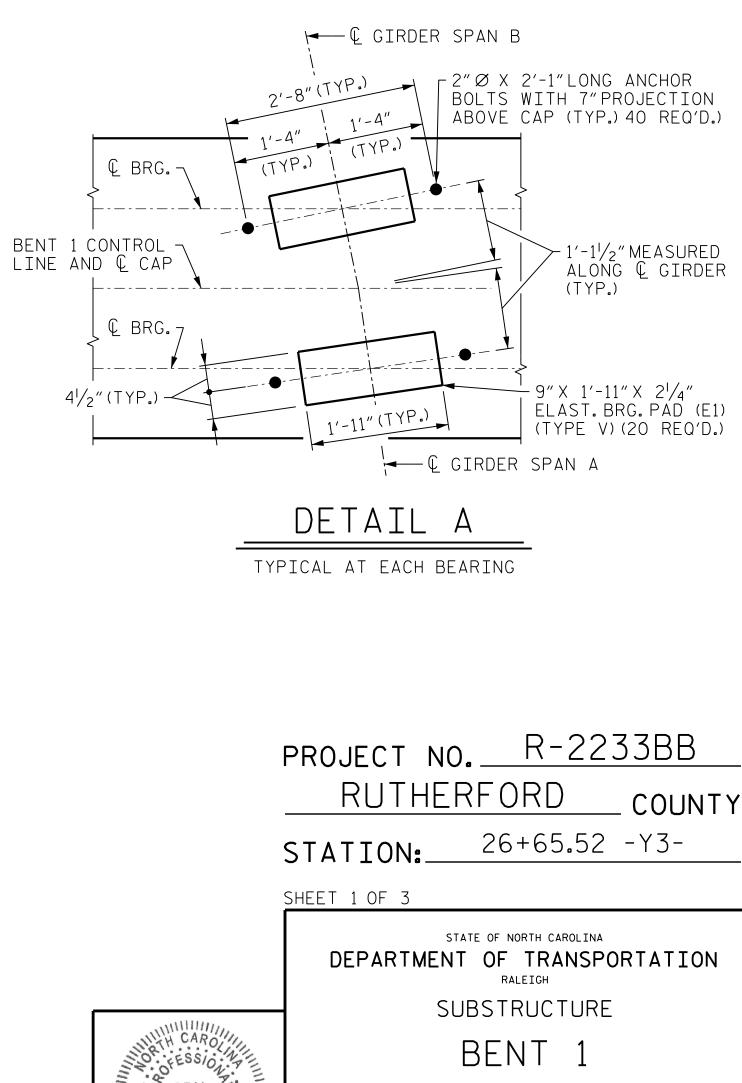
HOOKS IN "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

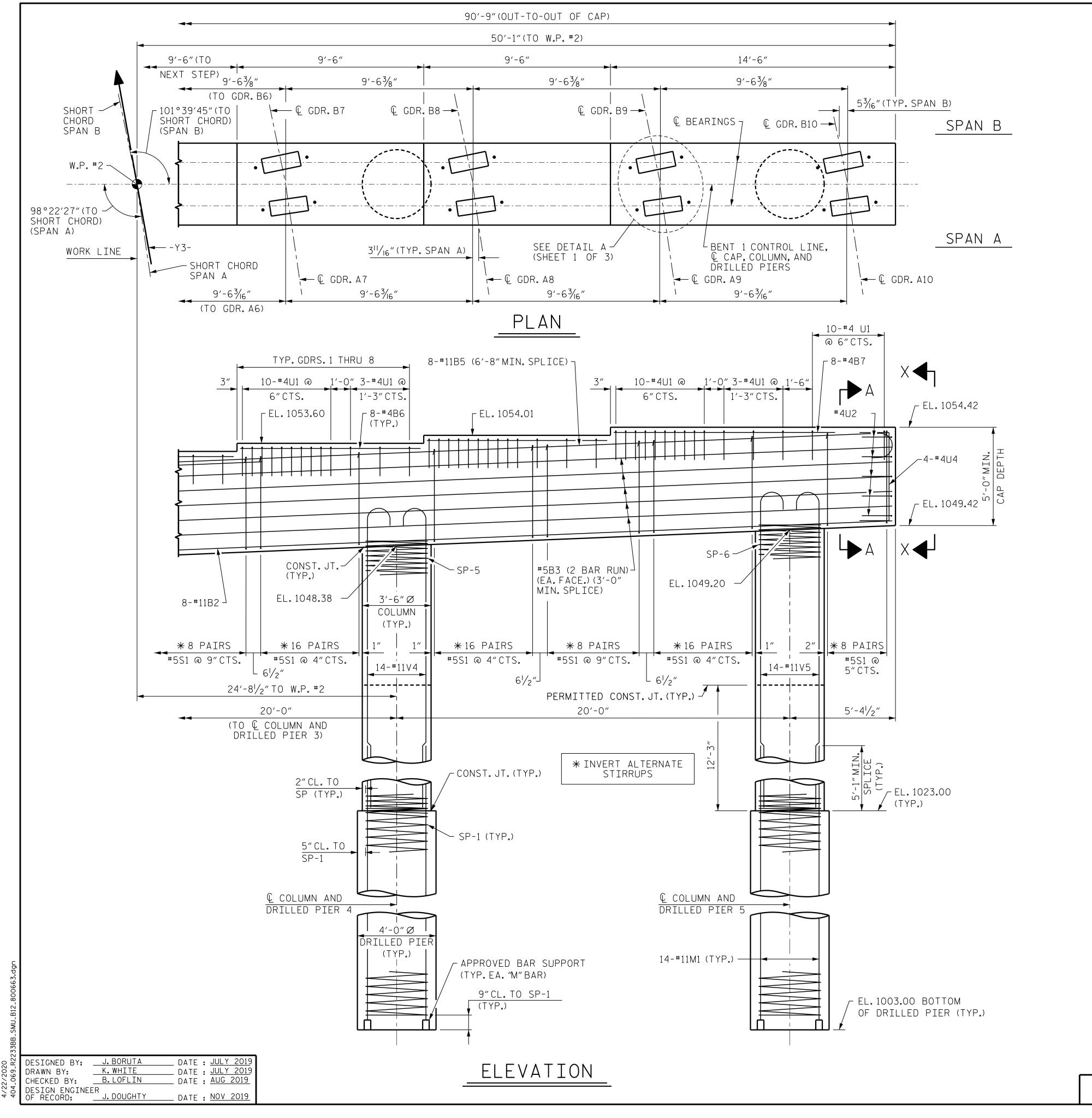
NO SEPARATE PAYMENT SHALL BE MADE FOR ANY ADDITIONAL STEEL REQUIRED IN CONSTRUCTION OF THE DRILLED PIER AS THIS IS CONSIDERED INCIDENTAL TO THE LINEAR FOOT PRICE FOR DRILLED PIER.

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE PROPOSED GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL PROPOSED GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE PROPOSED GROUND LINE.



RED FINAL	Jason R Dought
	DocuSigned by:
TE 500	4/22/202
great bridges.	THE OWNER AND THE AND
ASTERS	032967
	A SEAL
	CAROLINI
	ANNULL.

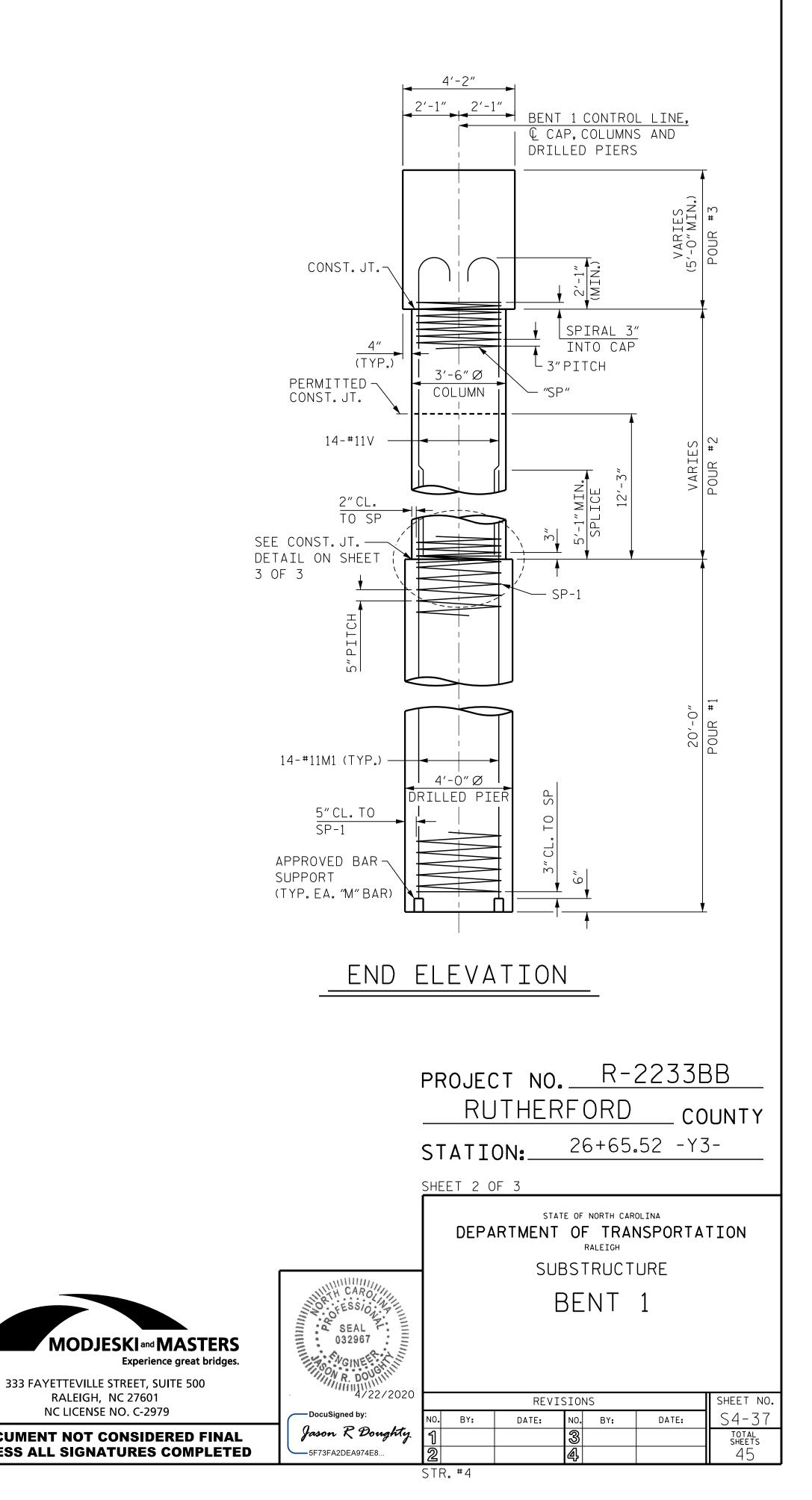
2/2020 REVISIONS SHEET NO. NO. BY: S4-36 DATE: DATE: BY: nghty TOTAL SHEETS 45

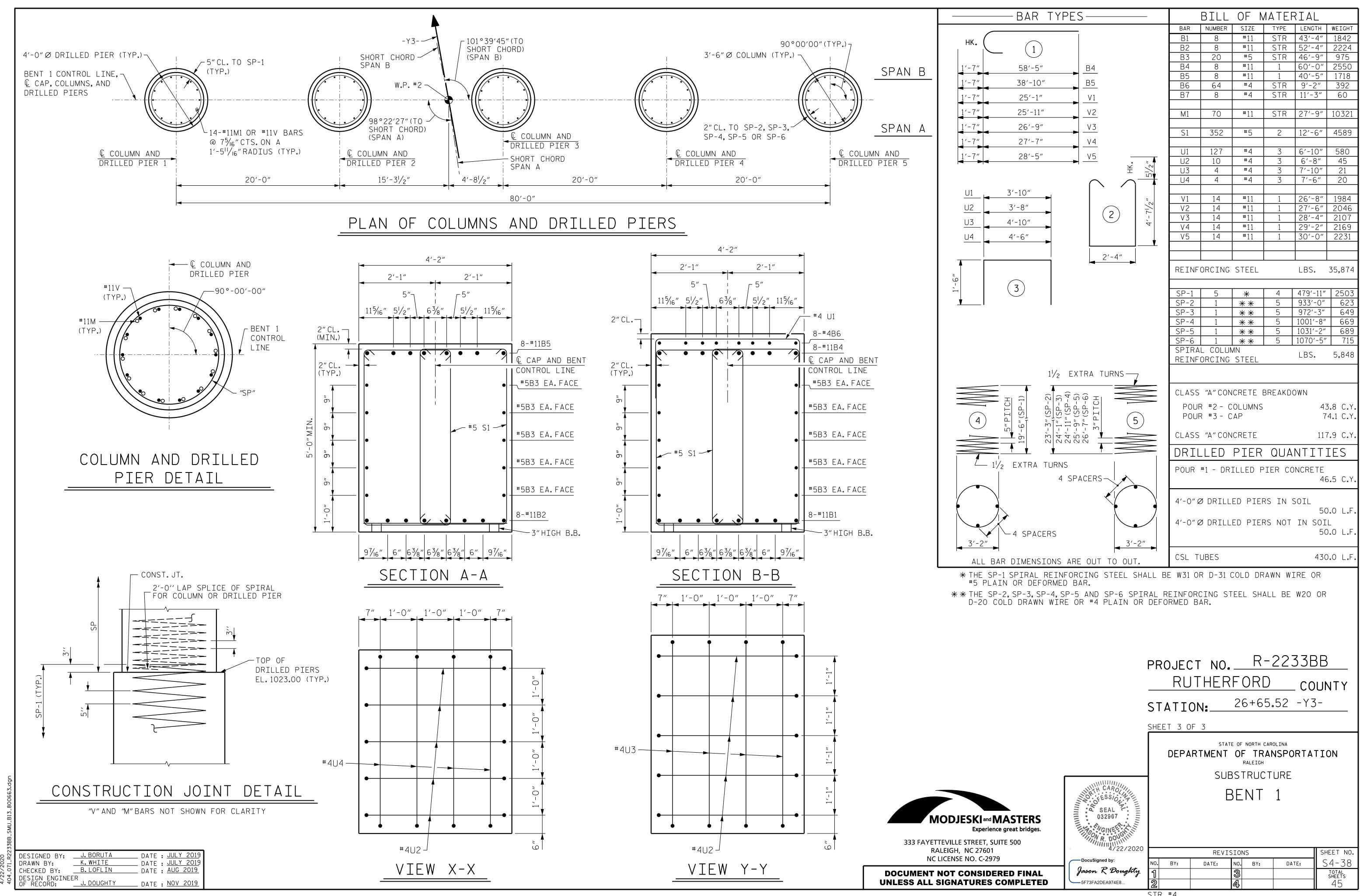


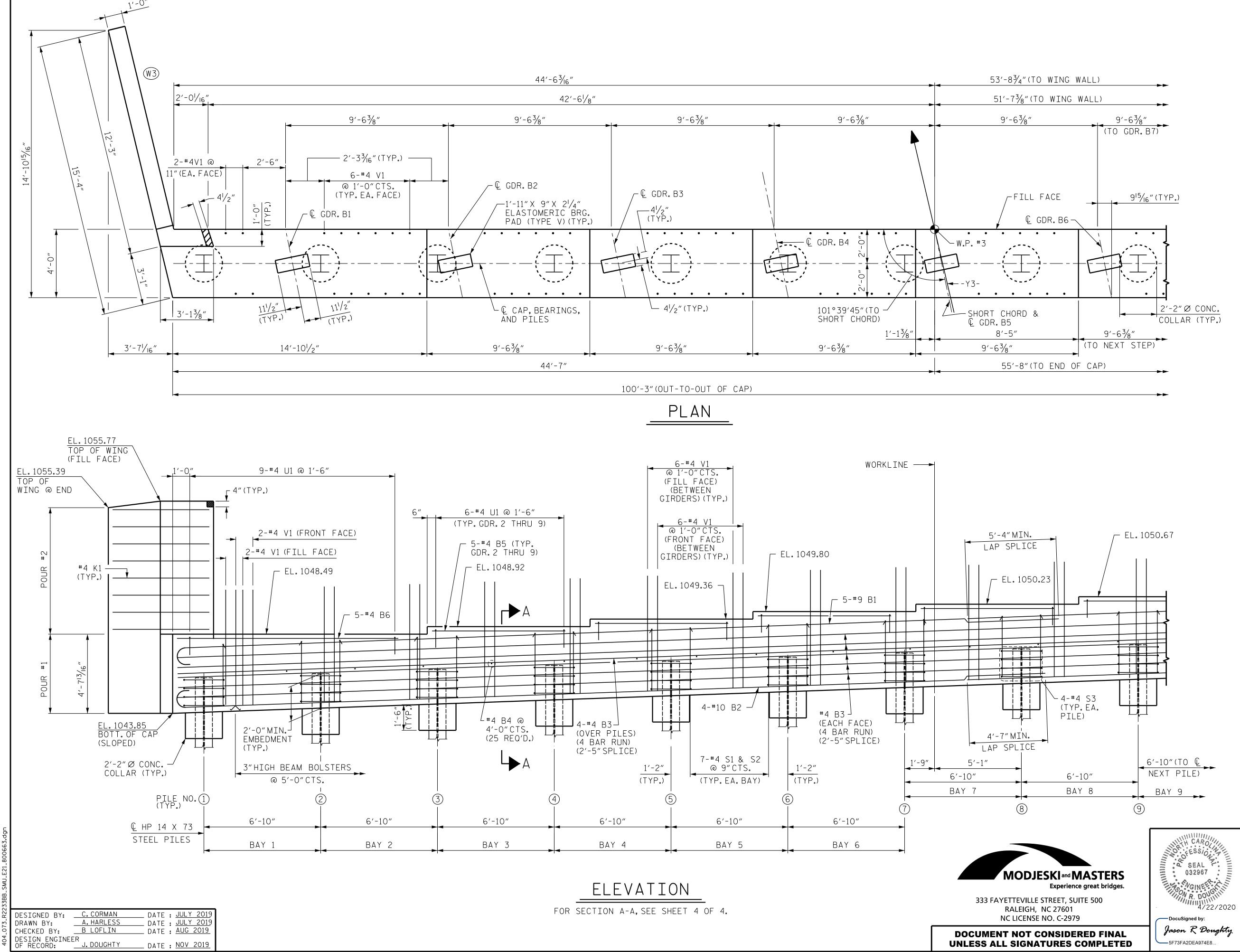
+

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

RALEIGH, NC 27601 NC LICENSE NO. C-2979







+

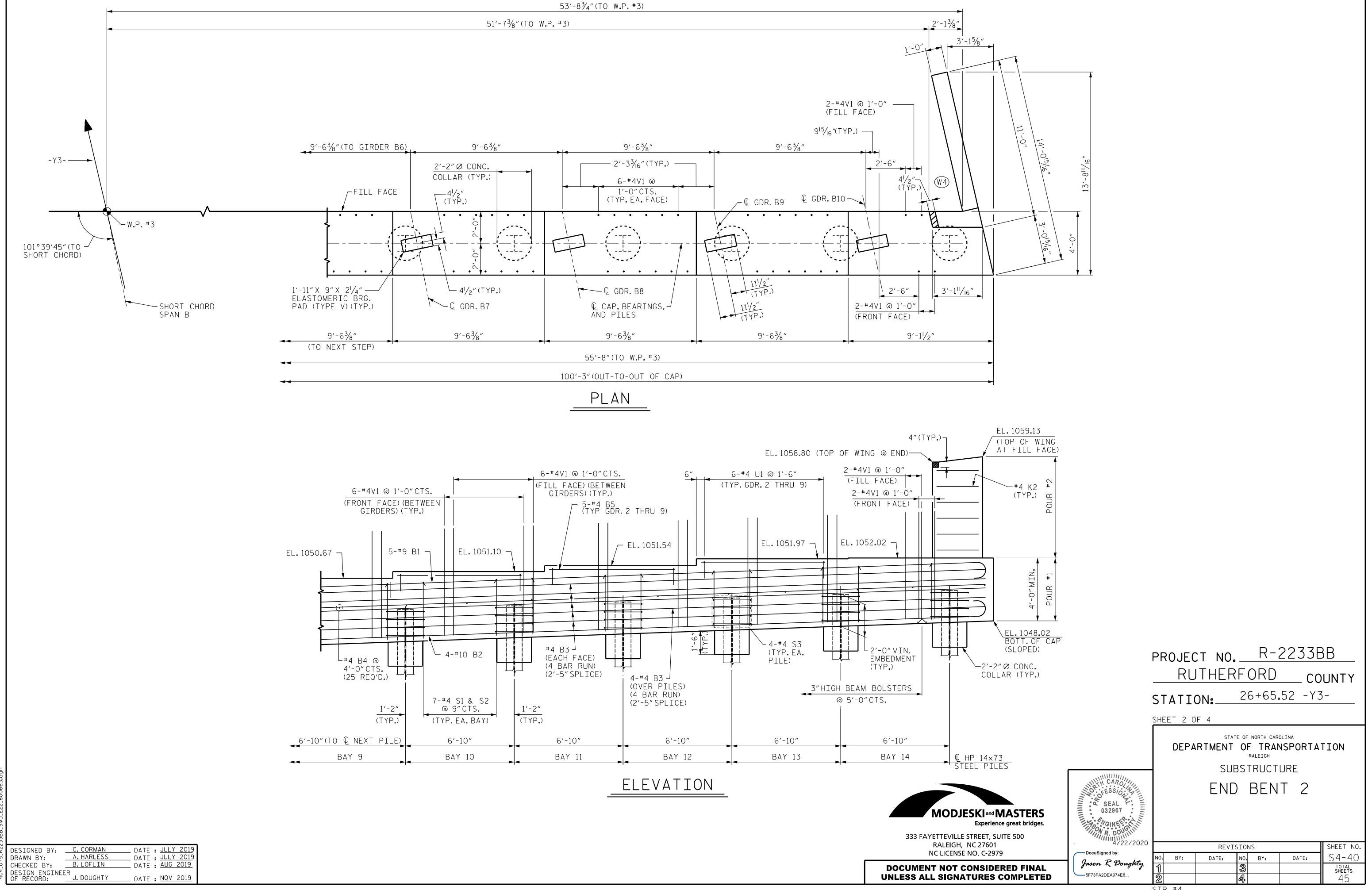


THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCEPT TO THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

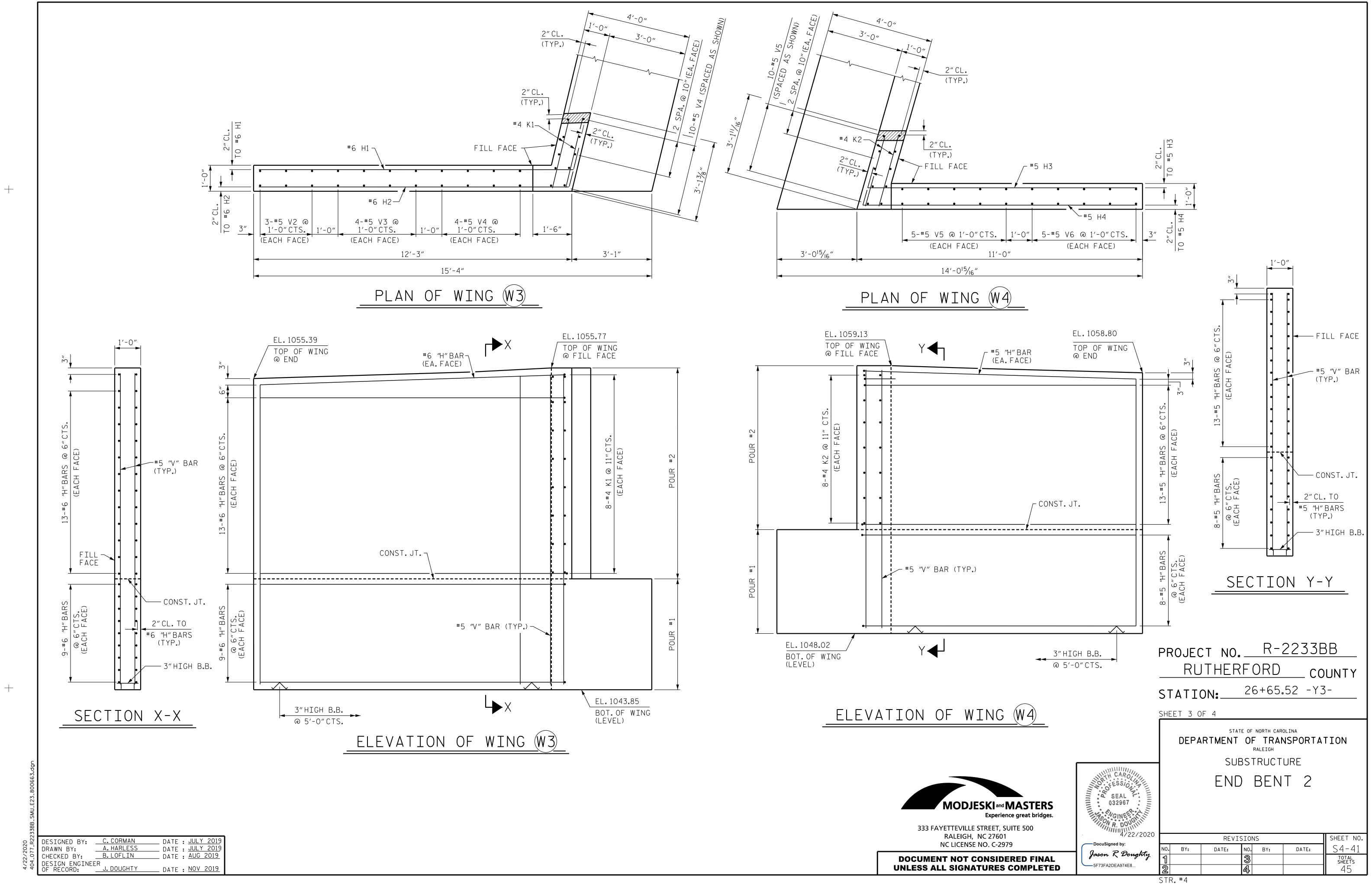
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE PARAPET IS CAST IF SLIP FORMING IS USED.

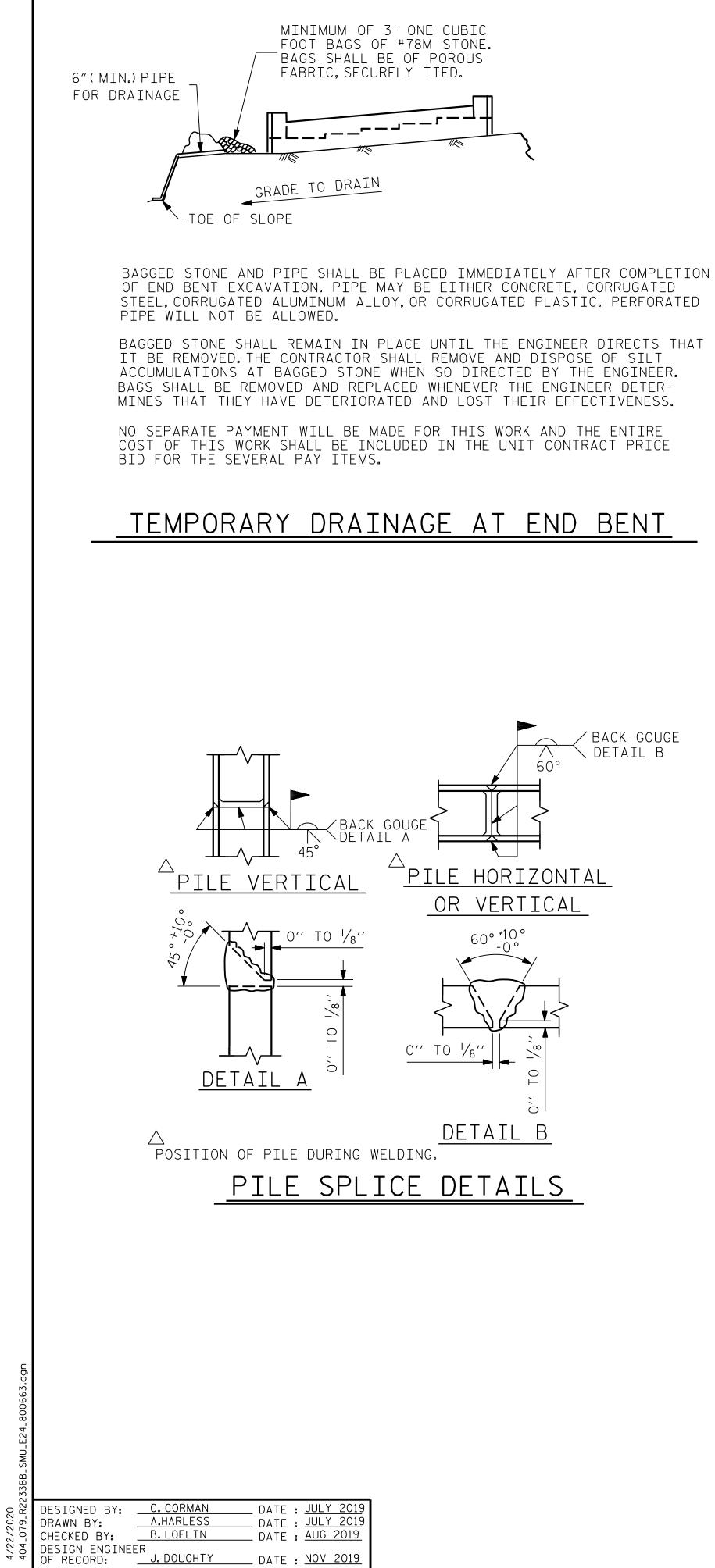
	OP OF	PILE					
E	LEVAT	IONS					
	PILE NO.	ELEVATION					
	1	1045.93					
	2	1046.21					
	3	1046.49					
	4	1046.78					
	5	1047.06					
	6	1047.35					
	7	1047.63					
	8	1047.92					
	9	1048.20					
	10	1048.48					
	11	1048.77					
	12	1049.05					
	13	1049.34					
	14	1049.62					
	15	1049.91					
PROJECT NO. <u>R-2233BB</u> RUTHERFORD COUNTY							
STATION: 26+65.52 -Y3-							
HEET 1	SHEET 1 OF 4						
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT 2							

REVISIONS					SHEET NO.	
N0.	BY:	DATE:	NO.	BY:	DATE:	S4-39
ป			ଞ			TOTAL SHEETS
2			ቆ			45
ст	⊃ # <i>1</i>					

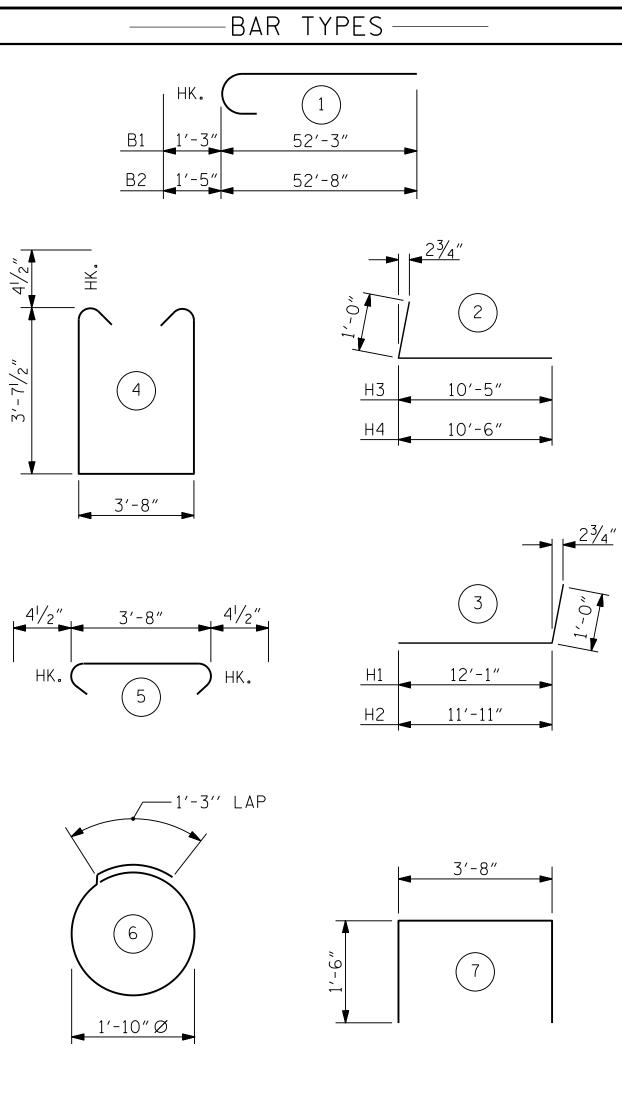


m l			
R223	DESIGNED BY:	C. CORMAN	DATE : <u>JULY 2019</u>
JI	DRAWN BY:	A. HARLESS	DATE : <u>JULY 2019</u>
-075.	CHECKED BY:	B.LOFLIN	DATE : <u>AUG 2019</u>
	DESIGN ENGINEE OF RECORD:	R J. DOUGHTY	DATE : <u>NOV 2019</u>

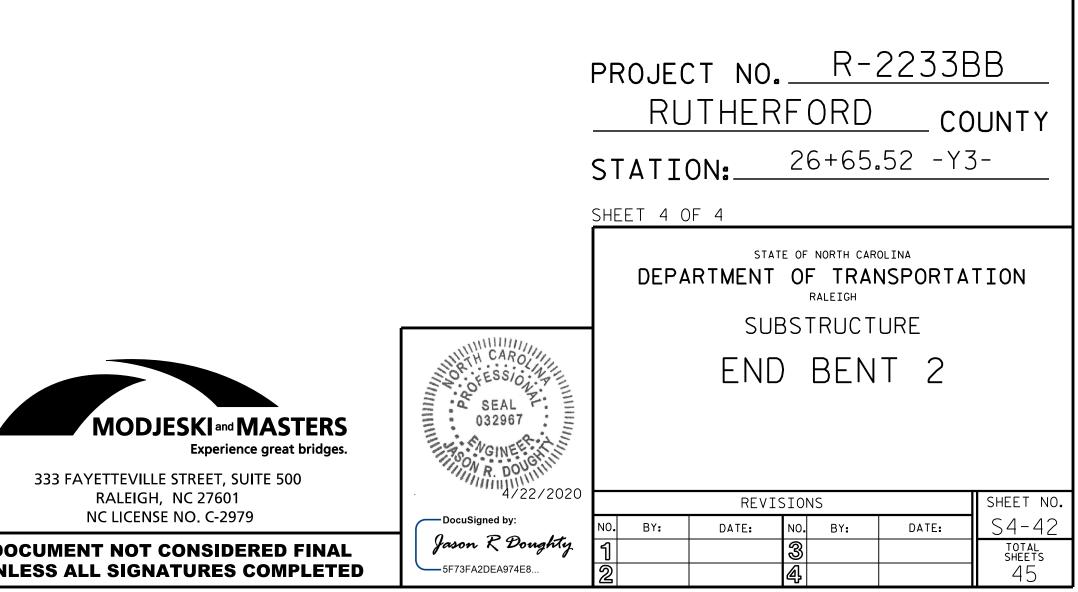


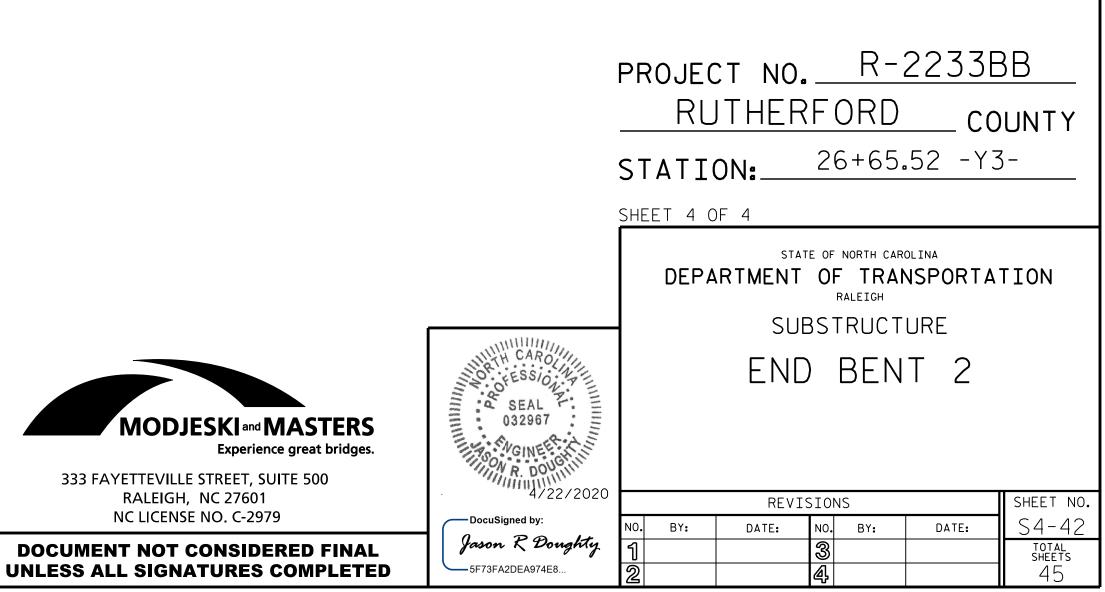


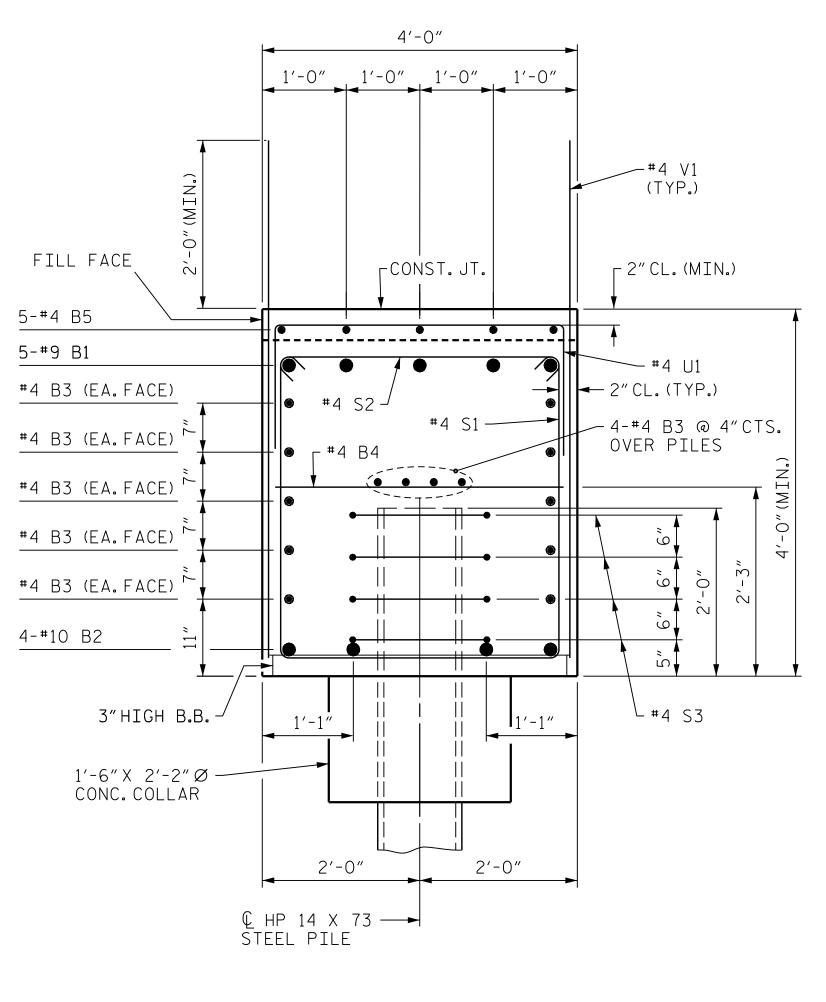
+



ALL BAR DIMENSIONS ARE OUT TO OUT.







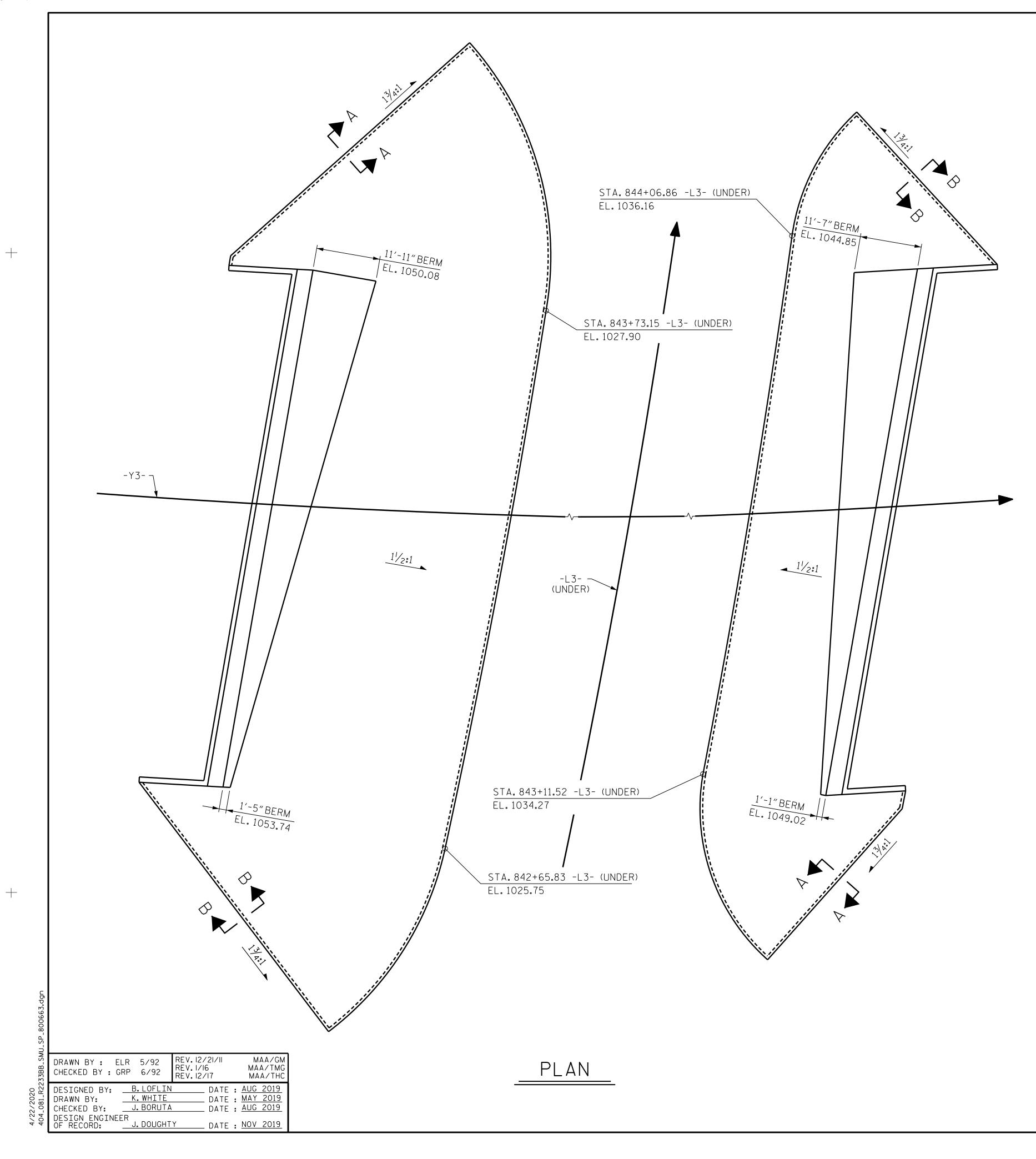
SECTION A-A

BILL OF MATERIAL						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
B1	10	#9	1	53′-6	1819	
B2	8	# 10	1	54-1"	1862	
B3	56	#4	STR	26'-10"	1004	
B4	25	#4	STR	3′-8″	61	
B5	40	#4	STR	8'-0"	214	
B6	5	#4	STR	13′-5″	45	
H1	23	#6	3	13'-1"	452	
H2	23	#6	3	12'-11"	446	
Н3	22	#5	2	11'-5″	262	
H4	22	#5	2	11'-6"	264	
K1	16	#4	STR	2'-8"	29	
K2	16	#4	STR	2'-9"	29	
S1	98	#4	4	11'-8"	764	
S2	98	#4	5	4'-5"	289	
S3	60	#4	6	7'-0"	281	
U1	57	#4	7	6'-8"	254	
			0.7.0			
V1	116	#4	STR	6'-10"	530	
V2	6	#5	STR	11'-2"	70	
V3	8	#5	STR	11'-3"	94	
V4	18	#5	STR	11'-5"	214	
V5	20	#5	STR	10'-7"	221	
V6	10	#5	STR	10'-5"	109	
REINFORCING STEEL LBS. 9313						
CLASS	CLASS A CONCRETE					
POUR		AP,LOW			. 71.2	
	&	CONC.	COLLA	.KS		
POUR	#2 F	PPER P	ART O	F C.Y.	. 7.1	
		INGS	ANT O		, (<u>,</u>	
	••					
TOTAL	CLAS	S A CO	ONCRE ⁻	TE C.Y.	. 78.3	
HP 14×73 STEEL PILES						
NO.15				LIN.FT.	. 565	
PILE DRIVING EQUIPMENT						
SETUP FOR HP 14×73					1 [

NO.

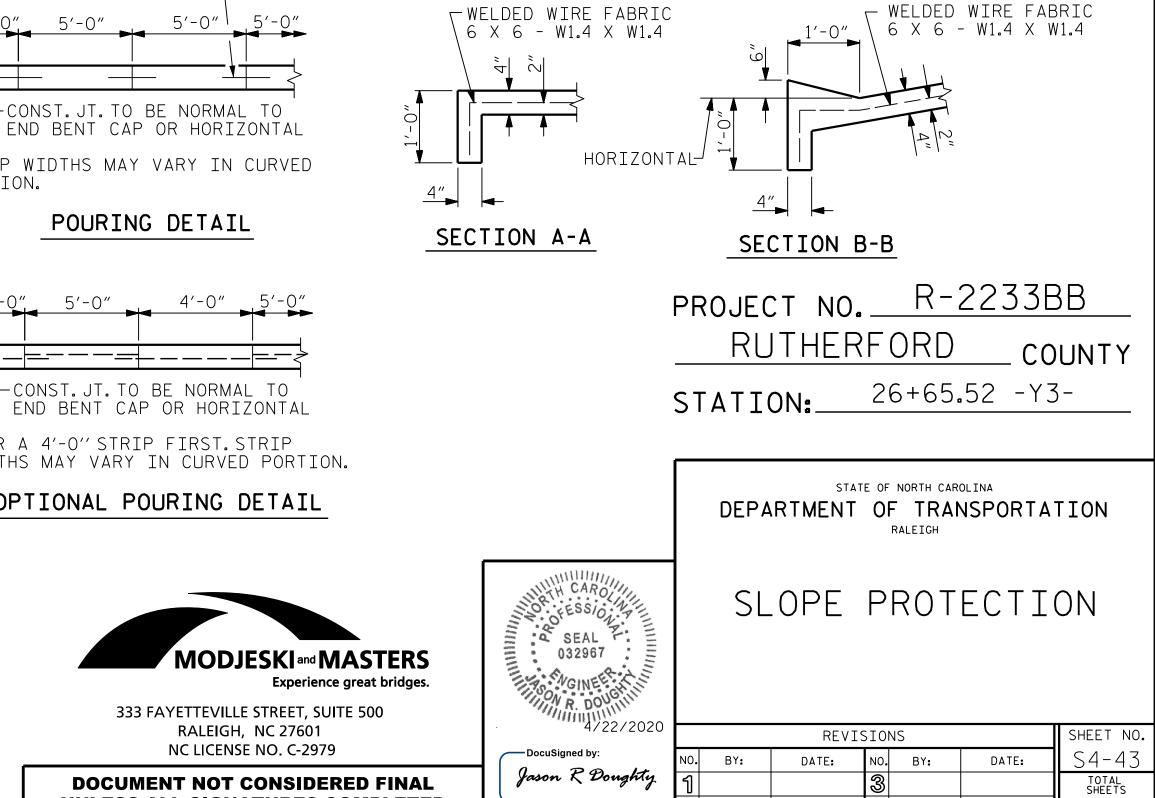
15

STEEL PILES



UNLESS ALL SIGNATURES COMPLETED

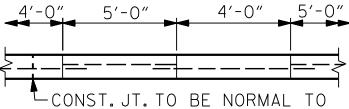
RALEIGH, NC 27601



5F73FA2DEA974E8..

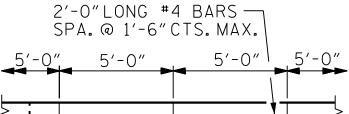
OPTIONAL POURING DETAIL

POUR A 4'-O'' STRIP FIRST.STRIP WIDTHS MAY VARY IN CURVED PORTION.



POURING DETAIL

-CONST.JT.TO BE NORMAL TO END BENT CAP OR HORIZONTAL STRIP WIDTHS MAY VARY IN CURVED PORTION.

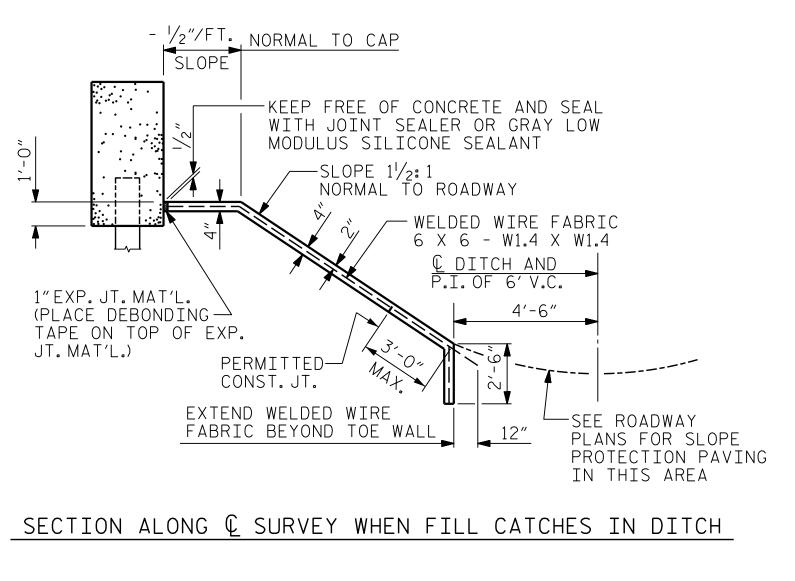


GENERAL NOTES

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING. SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE ``POURING DETAIL'' WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.26+65.52 -Y3-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE	
	SQUARE YARDS	APPROX.L.F.	
END BENT 1	954	1720	
END BENT 2	479	865	

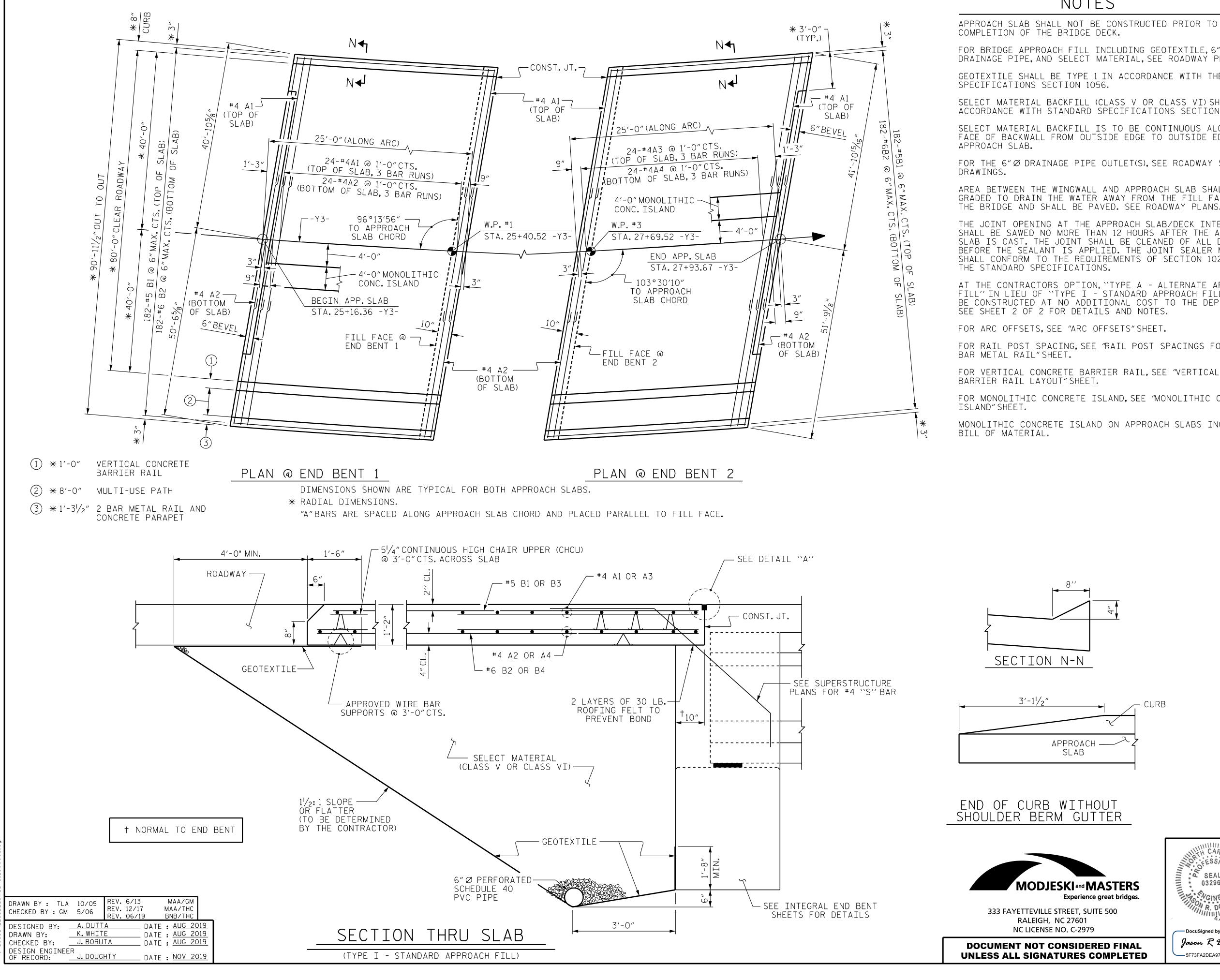




STR.#4

STD.NO.SP2

45



+

NOT

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE,6"Ø DRAINAGE PIPE,AND SELECT MATERIAL,SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF

FOR THE 6"Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

FOR ARC OFFSETS, SEE "ARC OFFSETS" SHEET.

FOR RAIL POST SPACING, SEE "RAIL POST SPACINGS FOR TWO

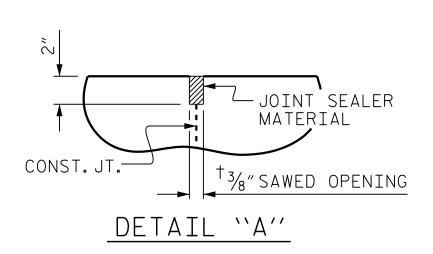
FOR VERTICAL CONCRETE BARRIER RAIL, SEE "VERTICAL CONCRETE FOR MONOLITHIC CONCRETE ISLAND, SEE "MONOLITHIC CONCRETE

MONOLITHIC CONCRETE ISLAND ON APPROACH SLABS INCLUDED IN

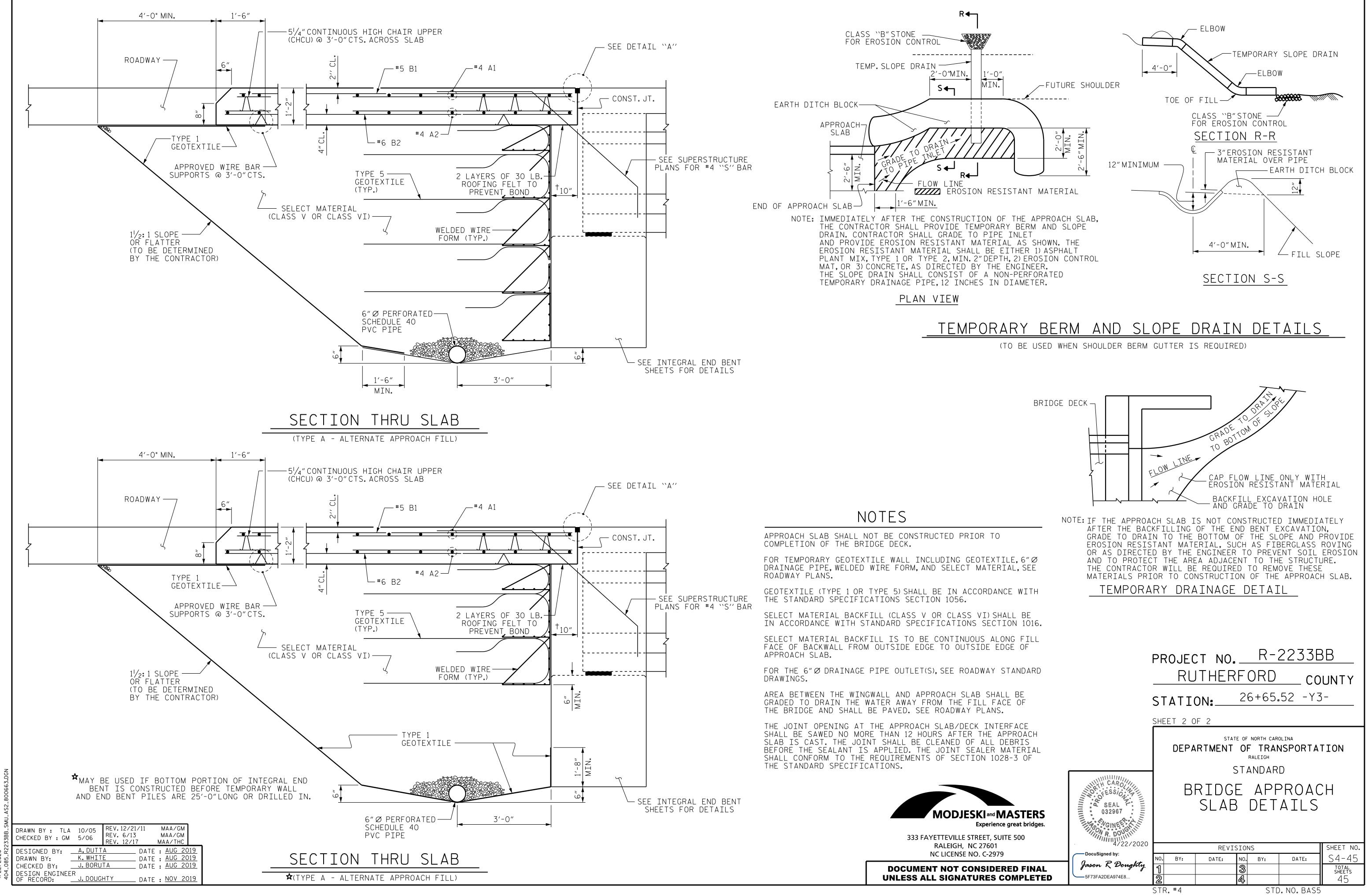
		SPLICE LENGTHS BAR SIZE EPOXY COATED #4 1'-11" #5 2'-5" #6 3'-7" 2'-5"
CURB		PROJECT NO. <u>R-2233BB</u> <u>RUTHERFORD</u> COUNTY STATION: <u>26+65.52 -Y3-</u>
HOUT TTER		SHEET 1 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
MASTERS ace great bridges. UITE 500	SEAL 032967 4/22/2020	BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT
79 ERED FINAL COMPLETED	DocuSigned by: Jason R Doughty 5F73FA2DEA974E8	REVISIONSSHEET NO.NO.BY:DATE:NO.SHEET NO.13DATE:SHEETS2445
		STR. #4 STD. NO. BAS5

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	78	4	STR	31′-9″	1654
A2	78	4	STR	31'-6"	1641
₩ A3	78	4	STR	32′-5″	1689
A4	78	4	STR	32'-4"	1685
₩ A5	38	4	STR	2'-6"	63
米 B1	364	5	STR	24'-2"	9175
B2	364	6	STR	24'-8"	13486
₩ B3	6	4	STR	24'-8"	99
REINFORCING STEEL				LBS.	16,812
*EPOXY COATED					
REINFORCING STEEL			LBS.	12,680	
CLASS AA CONCRETE C.Y. 195.9					

BILL OF MATERIAL



-	E	S	



DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SO.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{16}$ " Ø STUDS ALONG THE BEAM, AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{16}$ " Ø STUDS FOR 4 - 🔏 Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

STD. NO. SN