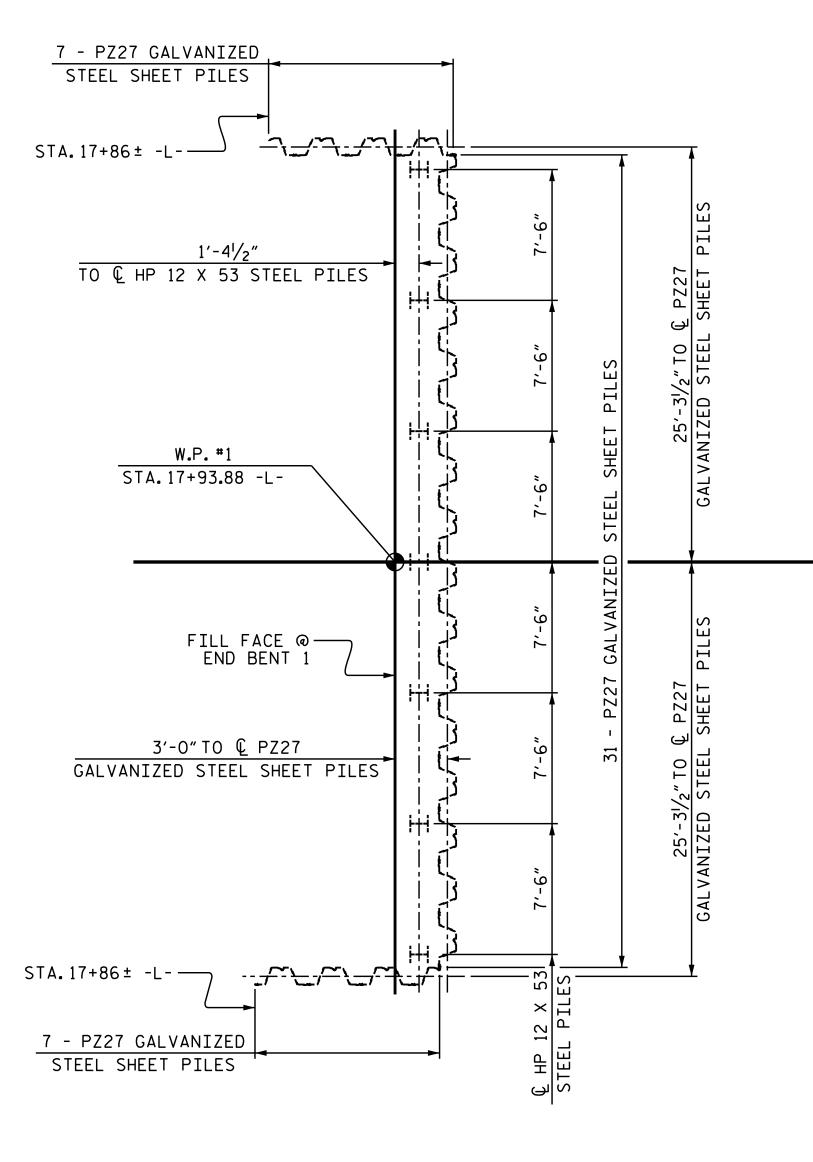


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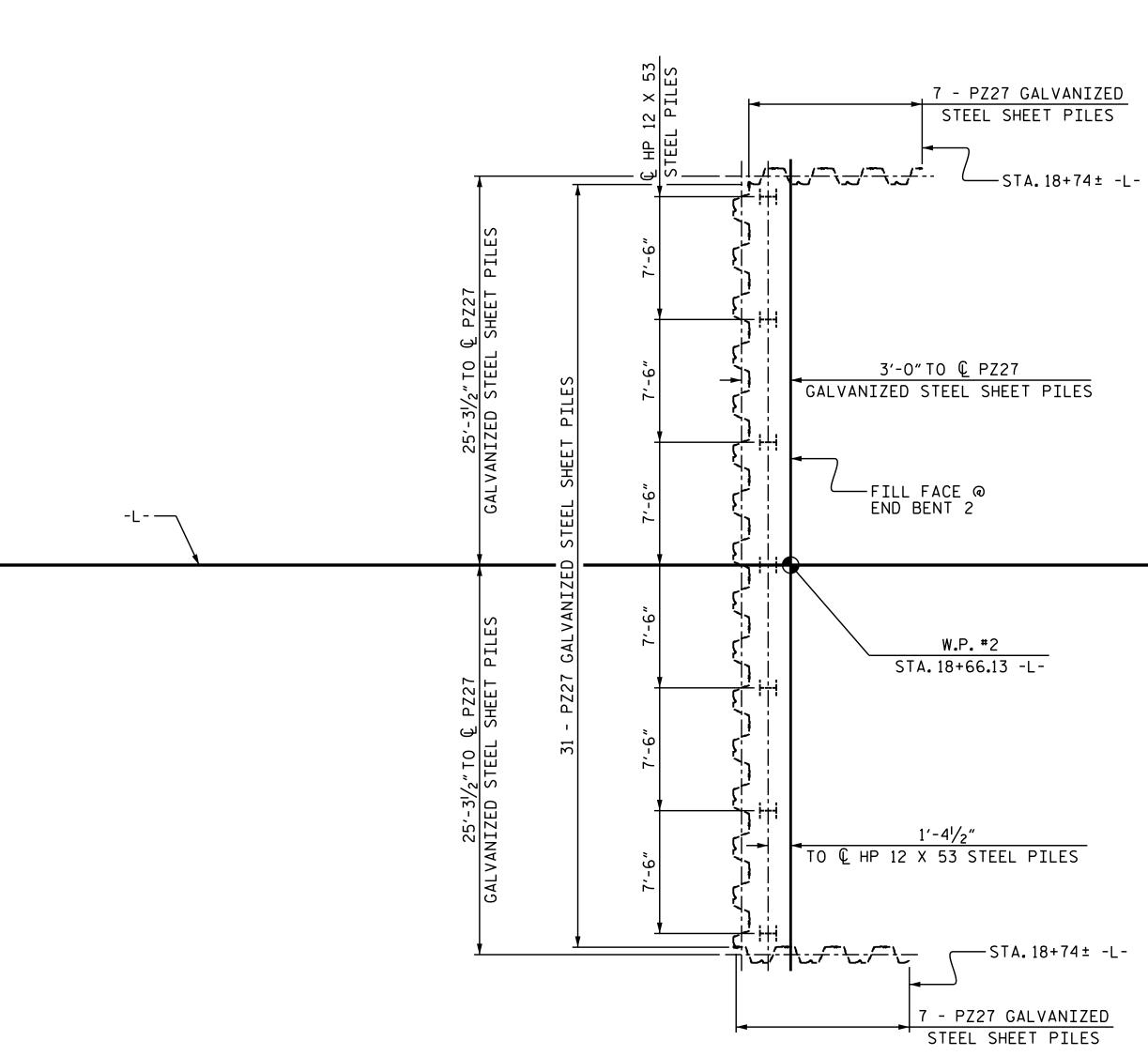


END BENT 1

DRAWN BY :	M.K. BEAR	RD	DATE : <u>04/2020</u>
CHECKED BY :	D. SHACK	ELFORD	DATE : 04/2020
DESIGN ENGINEER	OF RECORD:	W.SMITH	DATE : <u>4/22/21</u>
			10-MAY-2021 12:39

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END BENT 2

FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE CENTERLINE OF PILES.

NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENTS 1 & 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.

DRIVE PILES AT END BENTS 1 & 2 TO A REQUIRED DRIVING RESISTANCE OF 170 TONS PER PILE.

INSTALL PILES AT END BENTS 1 & 2 TO A TIP ELEVATION NO HIGHER THAN 5.0 FT.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENTS 1 & 2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

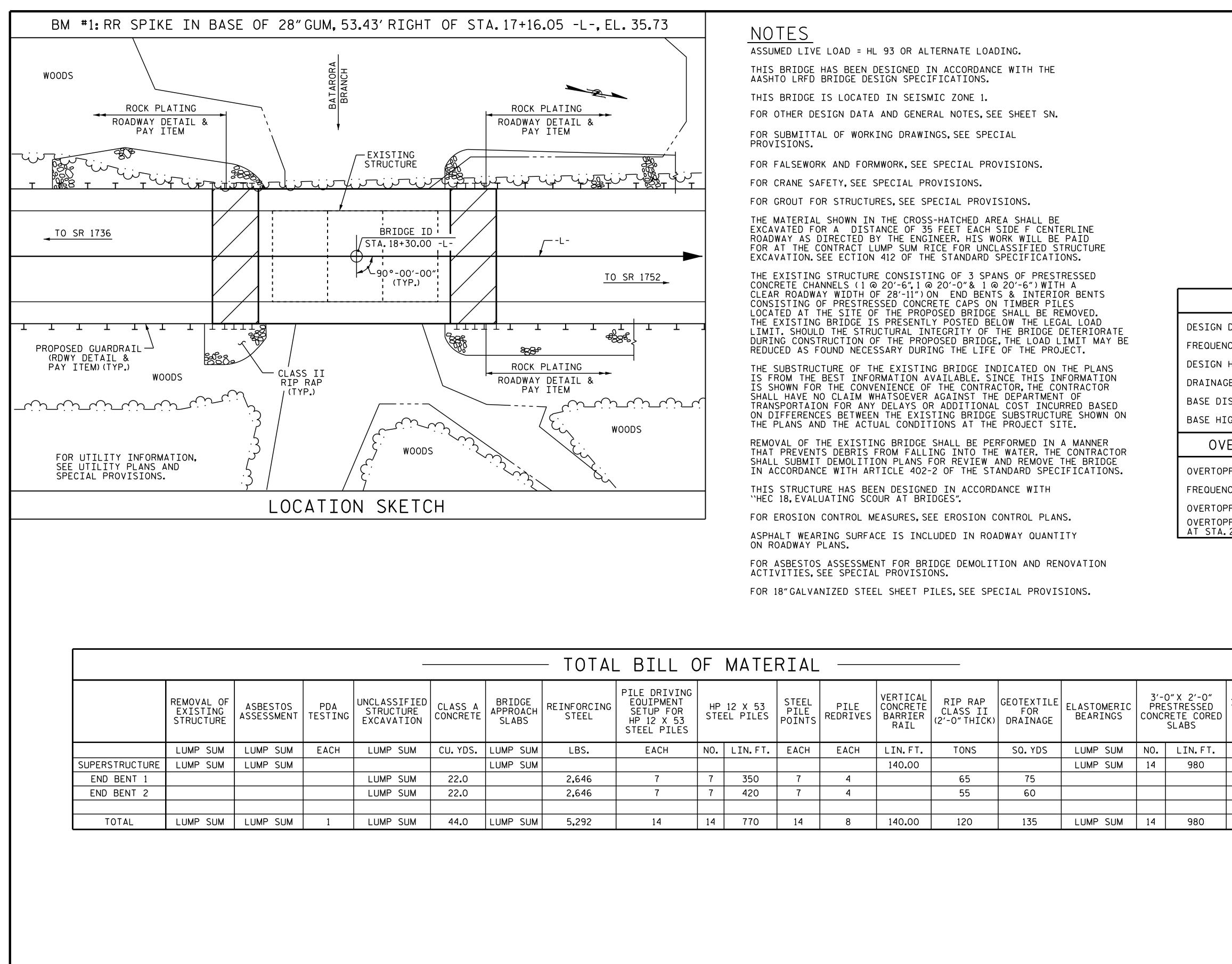
TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

INSTALL PZ27 & PZ90 GALVANIZED STEEL SHEET PILES AT END BENTS 1 & 2 TO A TIP ELEVATION NO HIGHER THAN 10.0 FT.



B-5642 PROJECT NO. BRUNSWICK COUNTY STATION: 18+30.00 -L-SHEET 2 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH EESSIO GENERAL DRAWING SEAL 26445 FOR BRIDGE OVER ACINEE? BATARORA BRANCH ON NC 87 BETWEEN P. Korey Newton SR 1736 & SR 1752 4FFE39D1431B407... 5/10/2021 REVISIONS SHEET NO. S-2 DATE: DATE: NO. BY: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS

14



DRAWN BY :	M.K. BEARD	DATE : 04/2020
CHECKED BY :	D. SHACKELFORD	DATE : <u>04/2020</u>
DESIGN ENGINEER	OF RECORD: <u>W.SMITH</u>	DATE : <u>4/22/21</u>

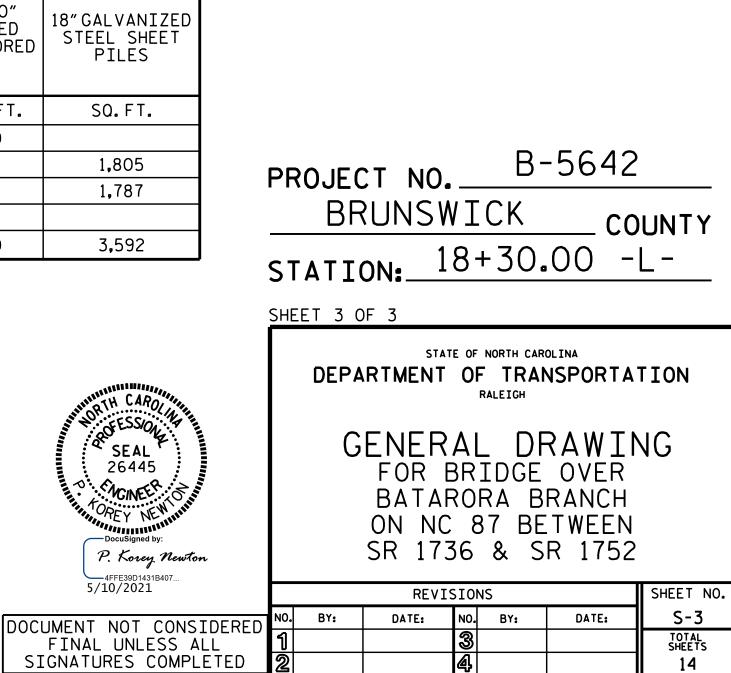
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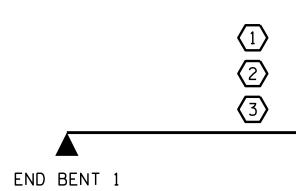
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TAL	BILL C)F	MATE	RIAL								
RCING	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STE	12 X 53 EL PILES	STEEL PILE POINTS	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE CONCI	O″X 2'-O″ STRESSED RETE CORED SLABS	1
•	EACH	NO.	LIN.FT.	EACH	EACH	LIN.FT.	TONS	SQ.YDS	LUMP SUM	NO.	LIN.FT.	
						140.00			LUMP SUM	14	980	
6	7	7	350	7	4		65	75				
6	7	7	420	7	4		55	60				
2	14	14	770	14	8	140.00	120	135	LUMP SUM	14	980	

HYDRAULIC DATA	
DISCHARGE	860 CFS.
CY OF DESIGN FLOOD	50 YRS.
HIGH WATER ELEVATION	35.2
SE AREA	5.5 SQ.MI.
SCHARGE (Q100)	1050 CFS.
GH WATER ELEVATION	35.6
ERTOPPING FLOOD	DATA
PING DISCHARGE	2600 CFS.
CY OF OVERTOPPING FLOOD	+500 YRS.
PING FLOOD ELEVATION PING IS AT THE & OF ROAN 21+68.80 -L-	38.7 Dway



		LOAD AN	ND KE	5121	I ANCI	- FA(T	J I OR	KAI	ING						PKES	IKF;	SSED							
										STRE	NGTH	I LIN	IT S	ΤΑΤΕ				SE	ERVICE	III E	LIMI	T STA	TE	
										MOMENT		_			SHEAR	-					MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	COMMENT NUMBER
		HL-93(Inv)	N⁄A		1.187		1.75	0.235	1.19	А	EL	34.5	0.511	1.32	Α	EL	62.1	0.80	0.235	1.39	А	EL	34.5	
DESIGN		HL-93(0pr)	N⁄A		1.538		1.35	0.235	1.54	А	EL	34.5	0.511	1.71	А	EL	62.1	NZA						
LOAD RATING		HS-20(Inv)	36.000	2	1.54	55.446	1.75	0.235	1.54	А	EL	34.5	0.511	1.64	А	EL	62.1	0.80	0.235	1.8	А	EL	34.5	
RATING		HS-20(0pr)	36.000		1.997	71.874	1.35	0.235	2	А	EL	34.5	0.511	2.12	А	EL	62.1	NZA						
		SNSH	13.500		4.017	54.223	1.4	0.235	4.3	А	EL	34.5	0.511	4.84	А	EL	62.1	0.80	0.235	4.02	А	EL	34.5	
		SNGARBS2	20.000		3.012	60.231	1.4	0.235	3.22	А	EL	34.5	0.511	3.45	А	EL	62.1	0.80	0.235	3.01	А	EL	34.5	
		SNAGRIS2	22.000		2.86	62.914	1.4	0.235	3.06	А	EL	34.5	0.511	3.21	А	EL	62.1	0.80	0.235	2.86	А	EL	34.5	
		SNCOTTS3	27.250		1.999	54.480	1.4	0.235	2.14	А	EL	34.5	0.511	2.42	А	EL	62.1	0.80	0.235	2	А	EL	34.5	
	^S [SNAGGRS4	34.925		1.678	58.596	1.4	0.235	1.8	А	EL	34.5	0.511	2.01	А	EL	62.1	0.80	0.235	1.68	А	EL	34.5	
		SNS5A	35.550		1.64	58.309	1.4	0.235	1.76	А	EL	34.5	0.511	2.04	А	EL	62.1	0.80	0.235	1.64	А	EL	34.5	
		SNS6A	39.950		1.508	60.238	1.4	0.235	1.61	А	EL	34.5	0.511	1.87	А	EL	62.1	0.80	0.235	1.51	А	EL	34.5	
LEGAL		SNS7B	42.000		1.436	60.313	1.4	0.235	1.54	А	EL	34.5	0.511	1.84	А	EL	62.1	0.80	0.235	1.44	А	EL	34.5	
LOAD RATING		TNAGRIT3	33.000		1.84	60.706	1.4	0.235	1.97	А	EL	34.5	0.511	2.22	А	EL	62.1	0.80	0.235	1.84	А	EL	34.5	
		TNT4A	33.075		1.848	61.139	1.4	0.235	1.98	А	EL	34.5	0.511	2.16	Α	EL	62.1	0.80	0.235	1.85	А	EL	34.5	
		TNT6A	41.600		1.514	62.991	1.4	0.235	1.62	А	EL	34.5	0.511	1.96	А	EL	62.1	0.80	0.235	1.51	А	EL	34.5	
	TST	TNT7A	42.000		1.523	63.977	1.4	0.235	1.63	А	EL	34.5	0.511	1.92	А	EL	62.1	0.80	0.235	1.52	А	EL	34.5	
	│⊢⊢	TNT7B	42.000		1.58	66.343	1.4	0.235	1.69	А	EL	34.5	0.511	1.79	А	EL	62.1	0.80	0.235	1.58	Α	EL	34.5	
		TNAGRIT4	43.000		1.5	64.495	1.4	0.235	1.61	А	EL	34.5	0.511	1.73	А	EL	62.1	0.80	0.235	1.5	Α	EL	34.5	
		TNAGT5A	45.000		1.413	63.582	1.4	0.235	1.51	А	EL	34.5	0.511	1.73	А	EL	62.1	0.80	0.235	1.41	Α	EL	34.5	
		TNAGT5B	45.000	3	1.395	62.762	1.4	0.235	1.49	А	EL	34.5	0.511	1.65	А	EL	62.1	0.80	0.235	1.4	А	EL	34.5	



LRFR SUMMARY

FOR 70'-0" SPAN

DESIGN ENGINEER OF RECORD:	
W. SMITH	DATE : <u>4/22/21</u>
ASSEMBLED BY : M.K.BEARD CHECKED BY : D.SHACKELFORD	DATE : 04/2020 DATE : 04/2020
DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10	

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END BENT 2

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

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NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

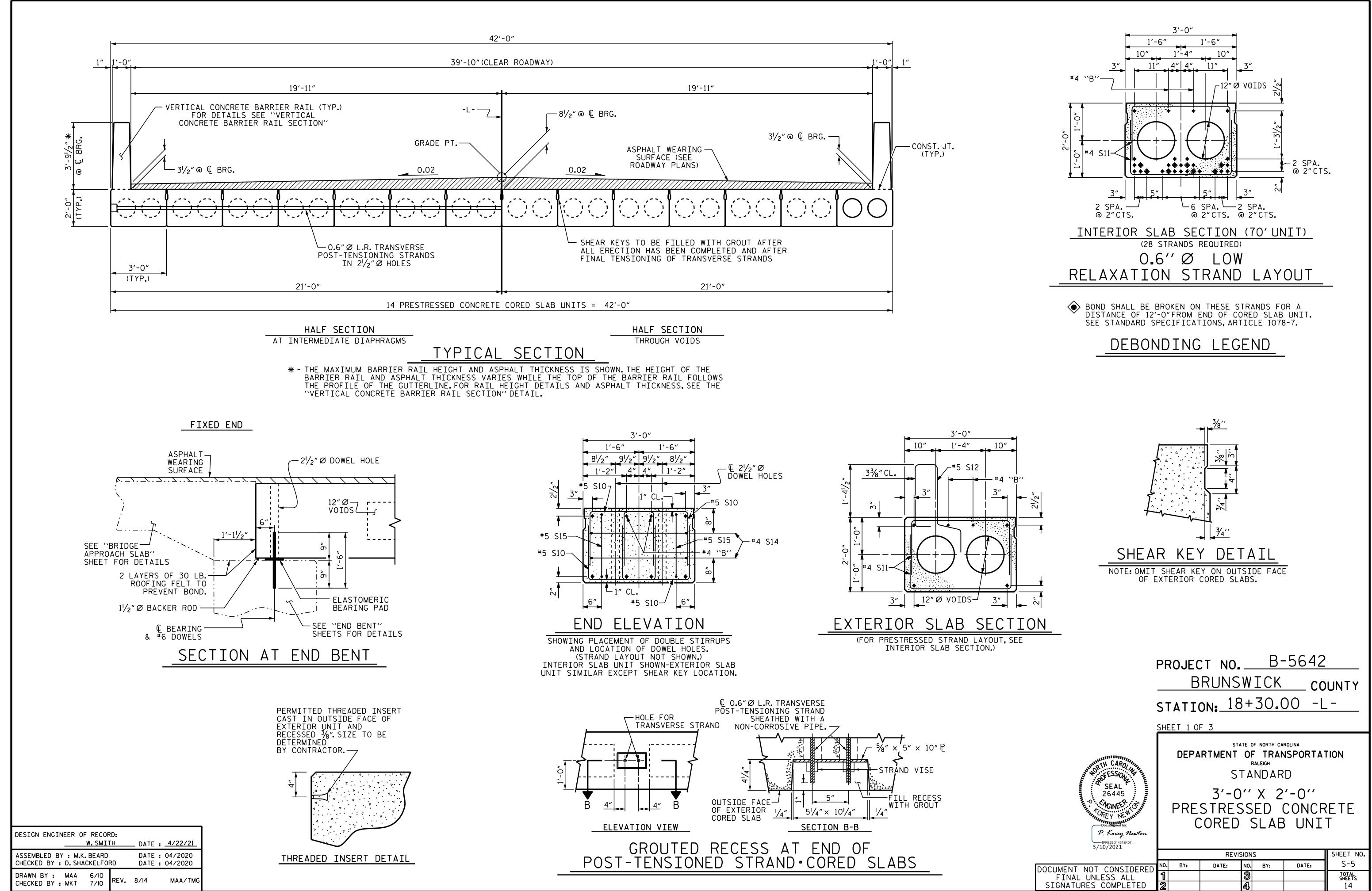
- 1.
- 2.
- 3.
- 4.

CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING ** * * SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER FL - FXTERTOR LEFT GTRDER

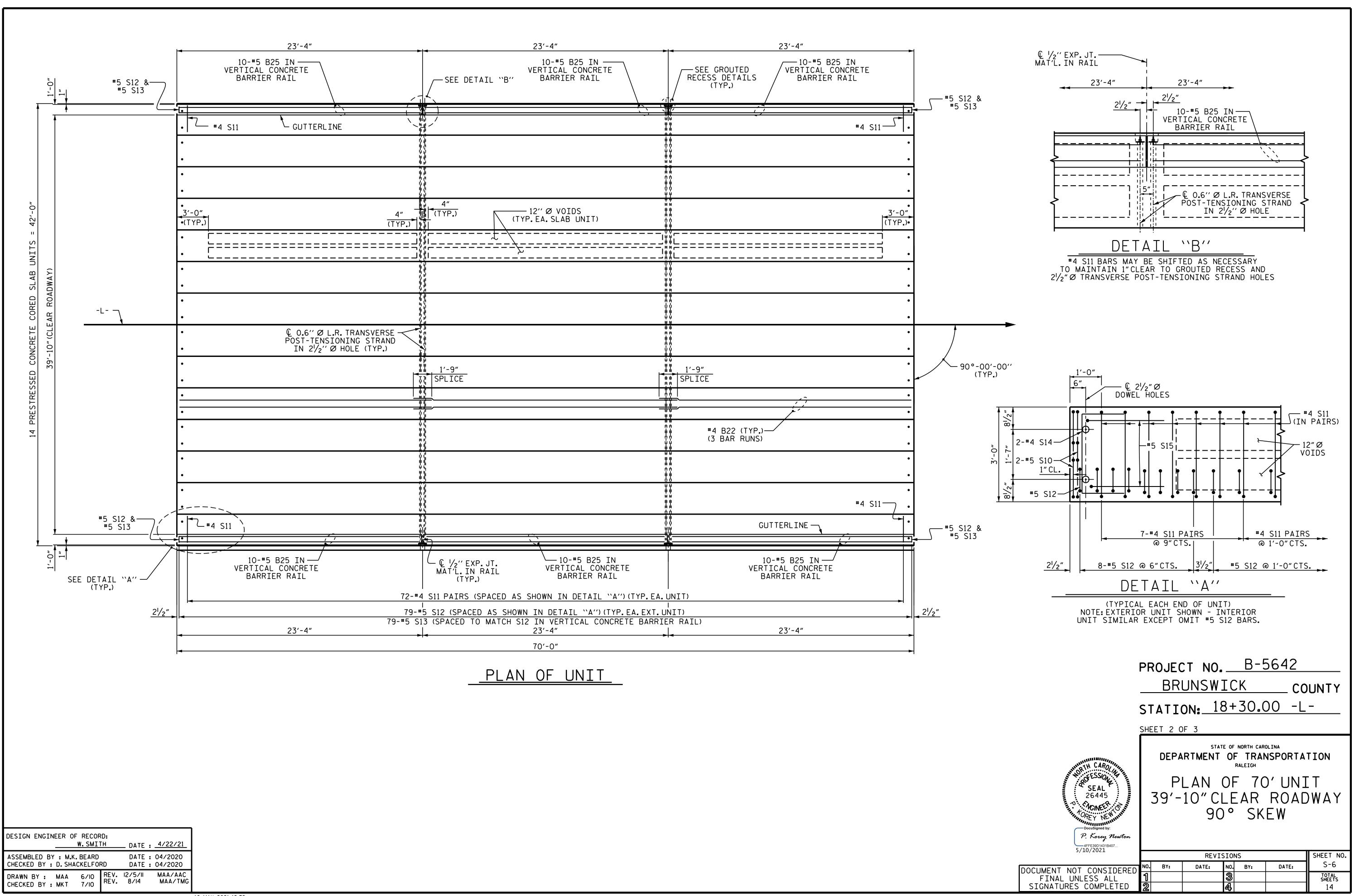
ER - EXTERIOR RIGHT GIRDER		• -	EVIERIOR		JUDER
	ER	? -	EXTERIOR	RIGHT	GIRDER

PROJECT N	IO. B-56	42
	NSWICK	COUNTY
	18+30.00	

SEAL 26445 Docusigned by: P. Korey Newton 5/10/2021		LR 70'	RTMENT S FRS CORE 9C	of TA UI D	NDAF MMA SL SK	NSPORTA RD RY F AB UI	OR NIT FIC)
5/10/2021			REVIS	SION	IS		SHEET NO.
MENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
FINAL UNLESS ALL	1			3			TOTAL SHEETS
GNATURES COMPLETED	2			4			14

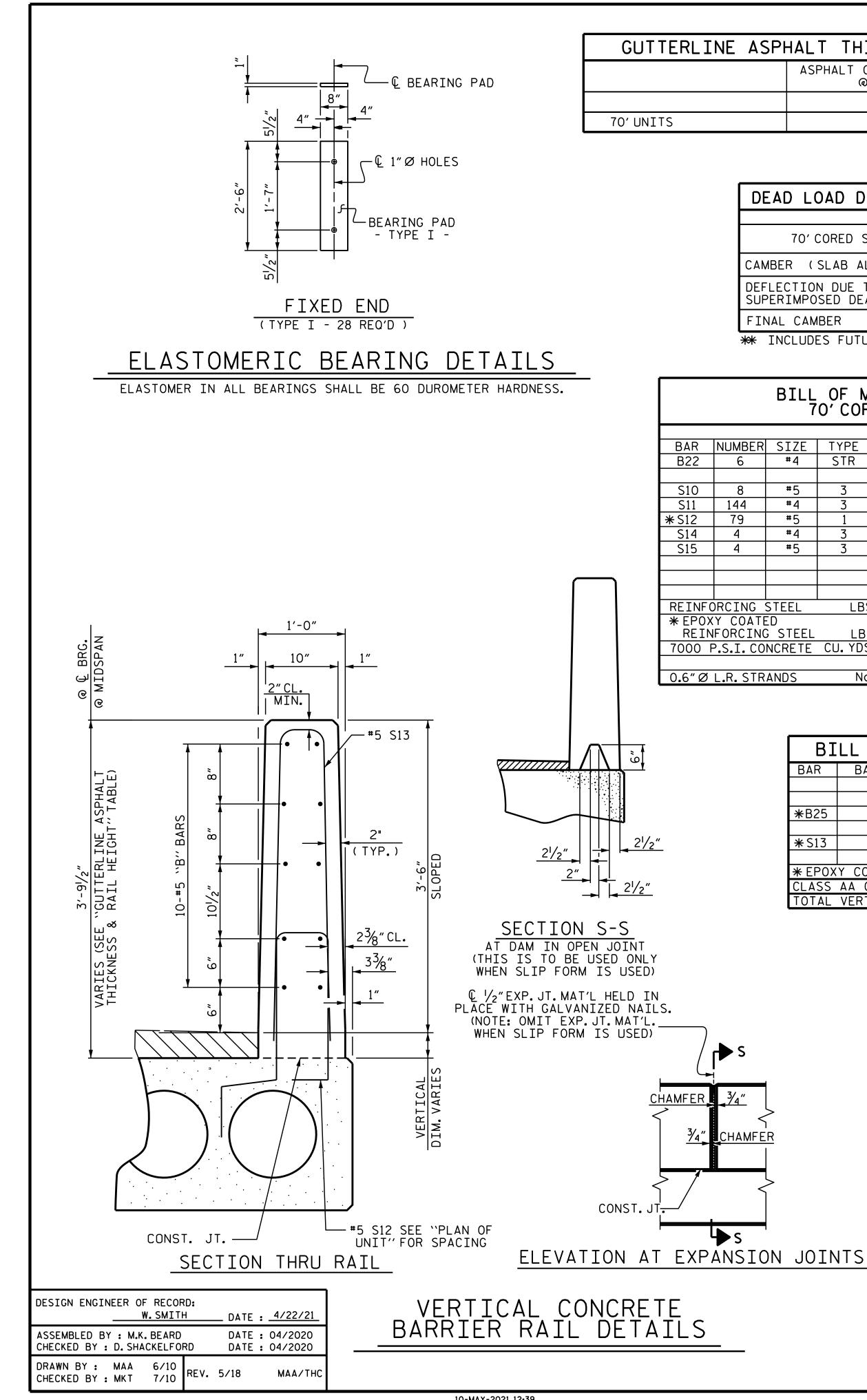


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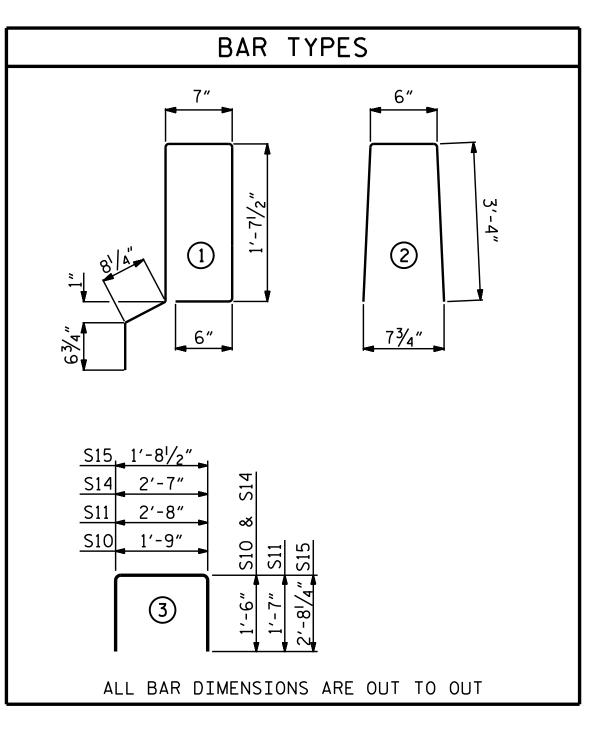
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ASPH	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
	1 1⁄8″	3'-7 <u>%</u> ″

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″ØL.R. STRAND
AMBER (SLAB ALONE IN PLACE)	2 ¹ /4″ 🕴
EFLECTION DUE TO UPERIMPOSED DEAD LOAD	¹³ ∕16″ ↓
INAL CAMBER	1 7⁄16″ ∮

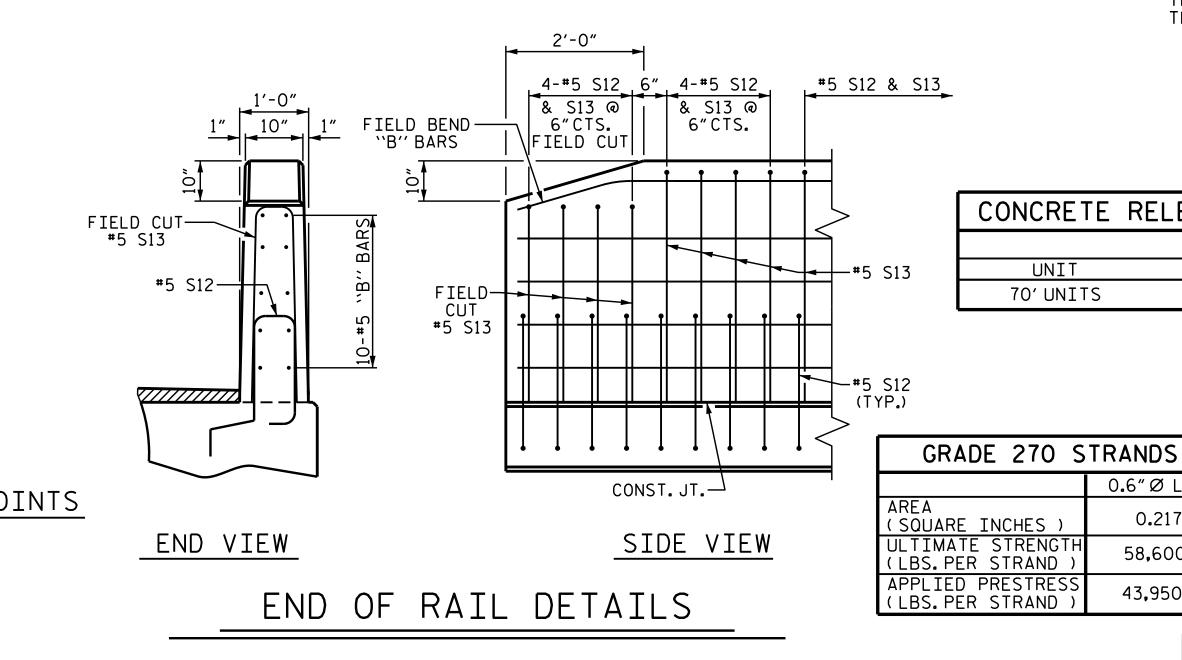
****** INCLUDES FUTURE WEARING SURFACE

	BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT							
			EXTERI	OR UNIT	INTERI	OR UNIT		
ER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
	#4	STR	24'-6"	98	24'-6"	98		
	# 5	3	4'-9"	40	4′-9″	40		
	#4	3	5'-10″	561	5'-10″	561		
	# 5	1	5′-7″	460				
	#4	3	5′-7″	15	5′-7″	15		
	# 5	3	7'-1"	30	7'-1″	30		
	STEEL	LBS	5.	744		744		
A T E	ED G STEEL	LBS	5.	460				
		CU. YDS		11.8		11.8		
TR	ANDS	Nc).	28		28		



CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
70' UNIT			
EXTERIOR C.S.	2	70′-0″	140'-0"
INTERIOR C.S.	12	70'-0″	840'-0"
TOTAL	14		980"-0"

BI	LL OF MATERIAL FOR VERTI	CAL CONCE	RETE	BARR	IER R	AIL		
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT		
	70' UNIT							
₩ B25	60	60	# 5	STR	22'-11"	1,434		
* S13	158	158	# 5	2	7'-2″	1,181		
★ EPOX	* EPOXY COATED REINFORCING STEEL LBS.							
CLASS	CLASS AA CONCRETE CU.YDS.							
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		140.00		



NOTES

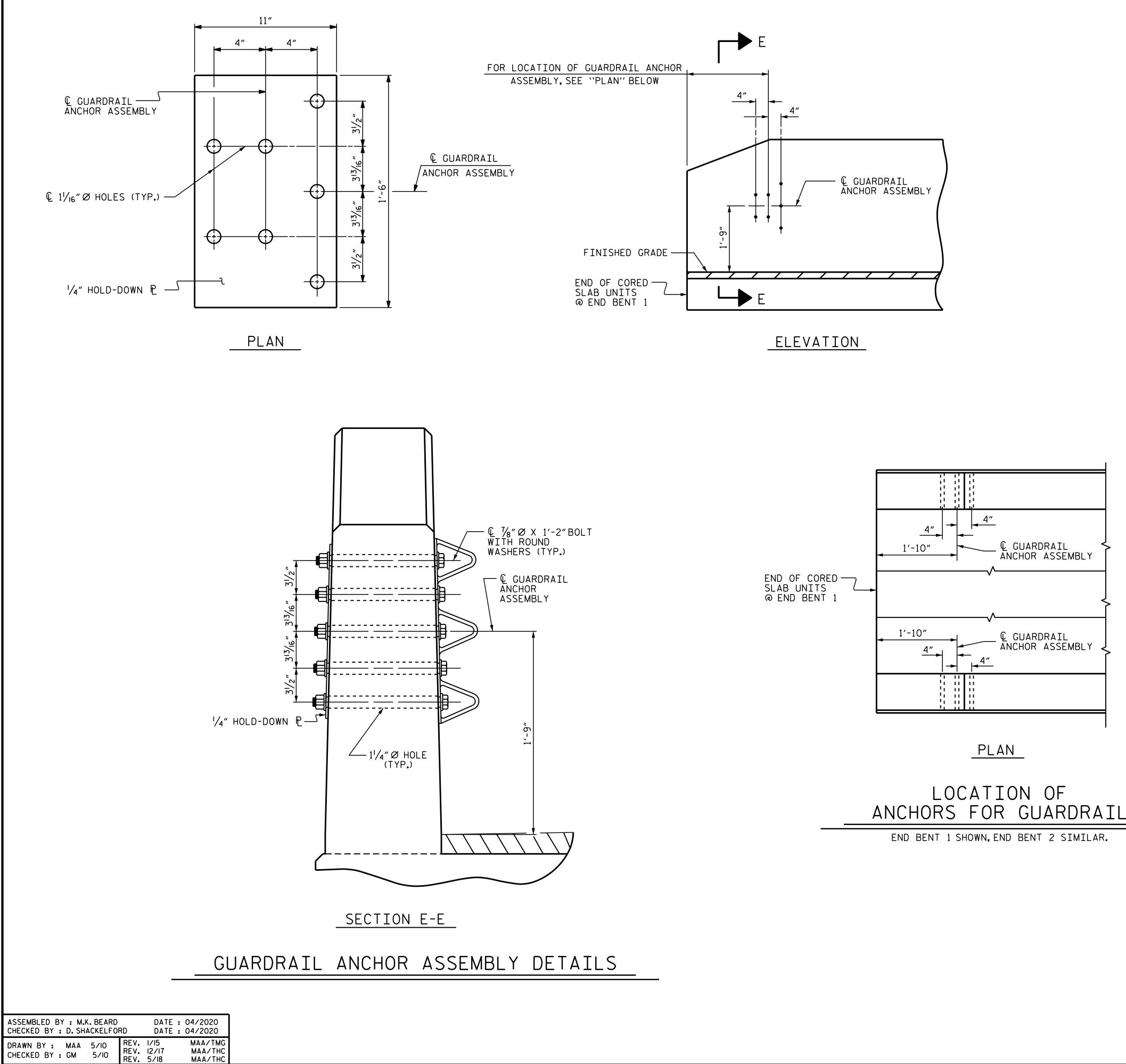
ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS. RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS. THE $2^{1}/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT. THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS. WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT. DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED. THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE. ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED. PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS. APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS. GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH. FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED. MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM. THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION. THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE. THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS. CONCRETE RELEASE STRENGTH BRUNSWICK _ COUNTY STATION: 18+30.00 -L-PSI 5600 SHEET 3 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION WITH CARO. RALEIGH SEAL 26445 STANDARD 3'-0" X 2'-0" 0.6"ØL.R. TOREY NEW PRESTRESSED CONCRETE 0.217 CORED SLAB UNIT 58,600 P. Korey Newton 4FFE39D1431B407. 5/10/2021 43,950 SHEET NO REVISIONS S-7 DATE: NO. DATE: BY: BY:

TOTAL SHEETS

14

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL

SIGNATURES COMPLETED



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MAA/THC

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THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

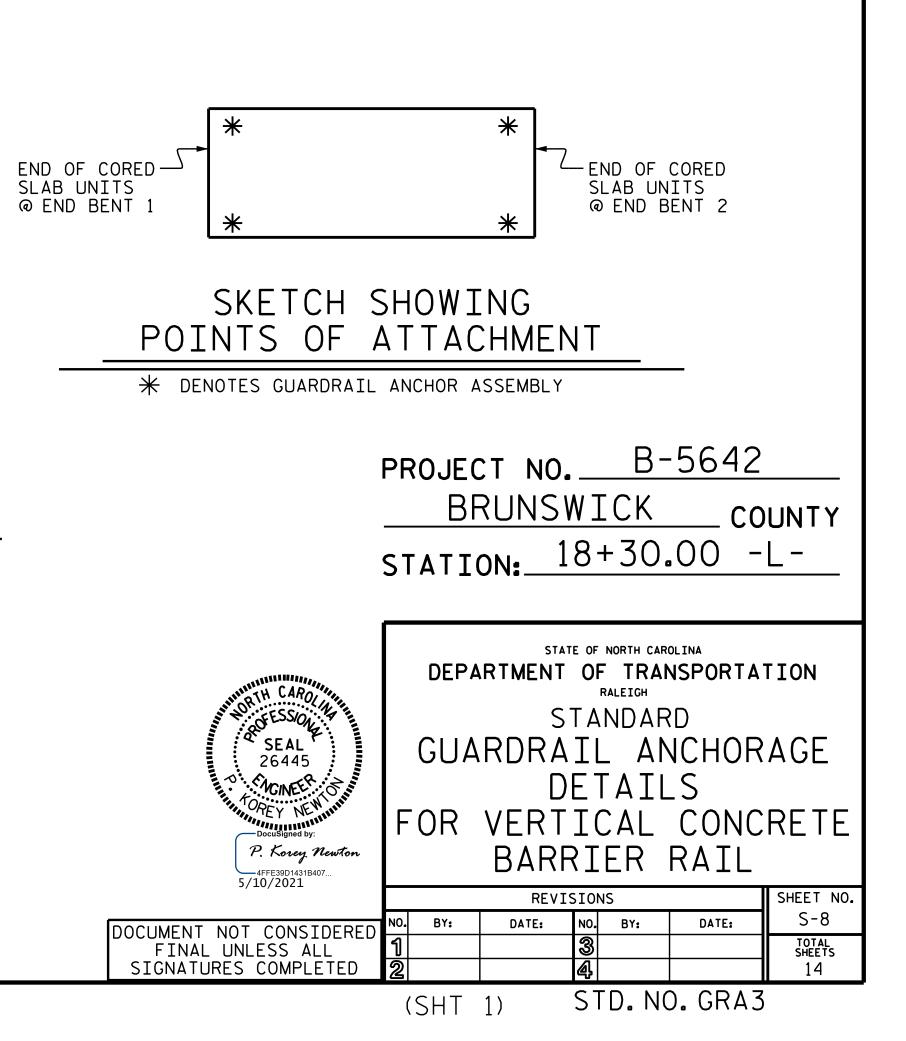
THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

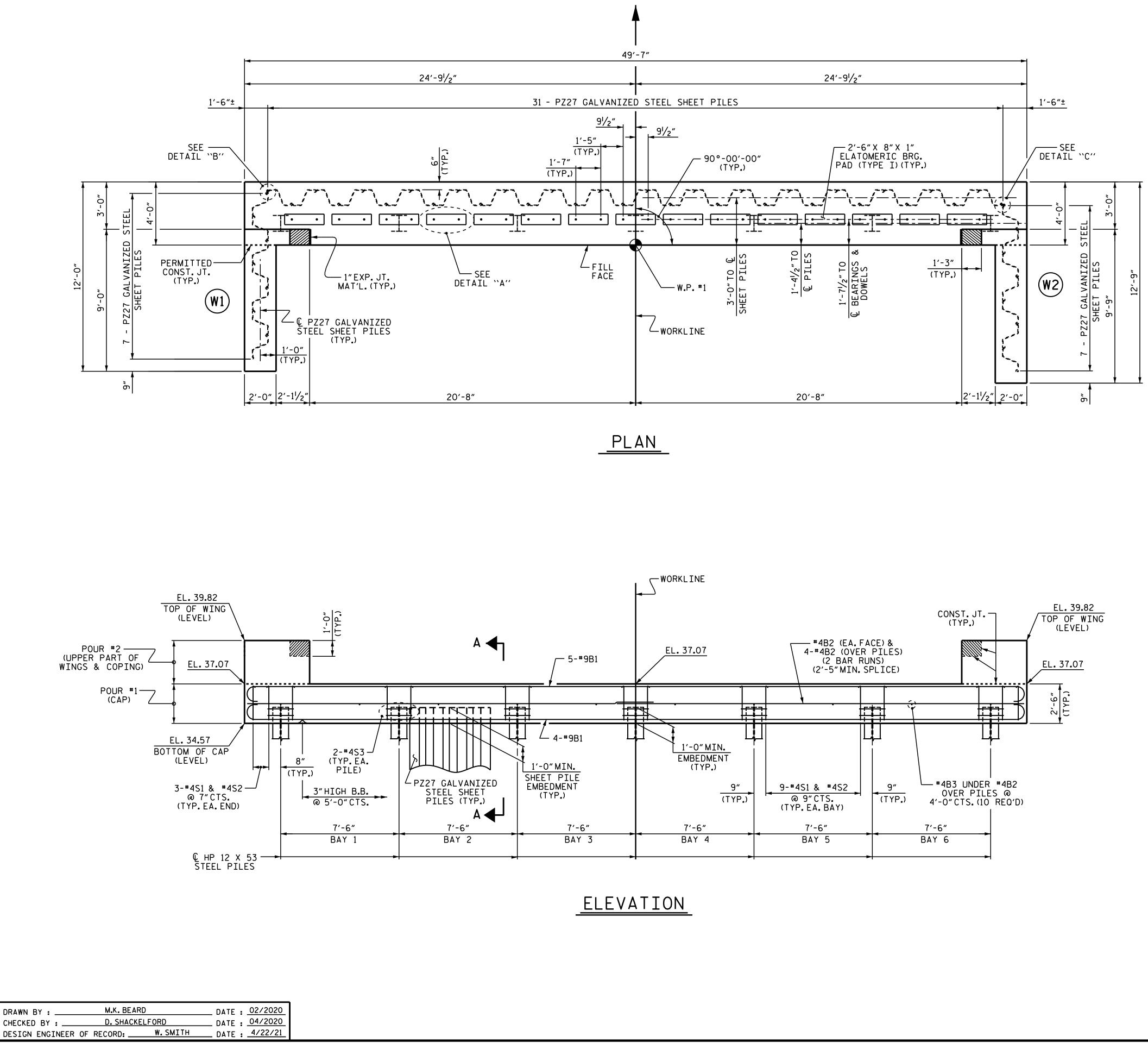
4″ € GUARDRAIL **ANCHOR ASSEMBLY** € GUARDRAIL ANCHOR ASSEMBLY PLAN LOCATION OF

END BENT 1 SHOWN, END BENT 2 SIMILAR.

N	\cap	Т	F	ς	
IN	U		L	S	

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.





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NOTES

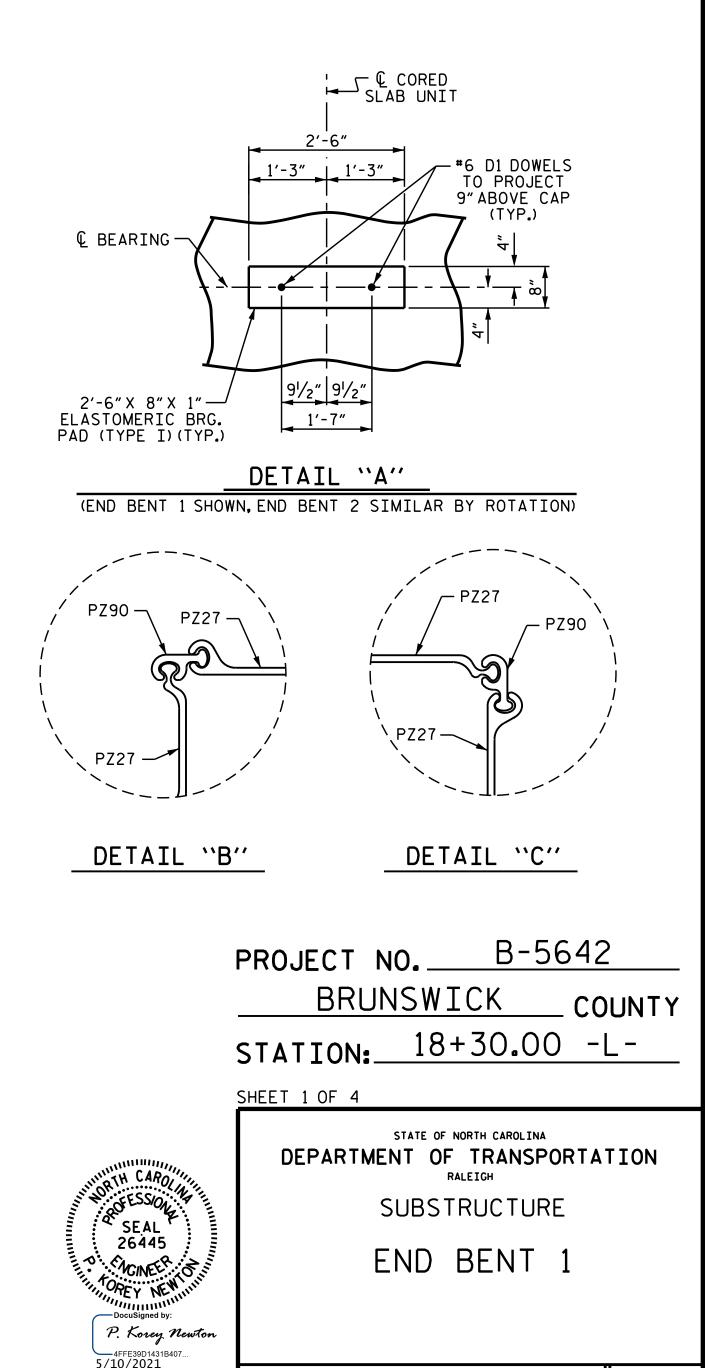
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

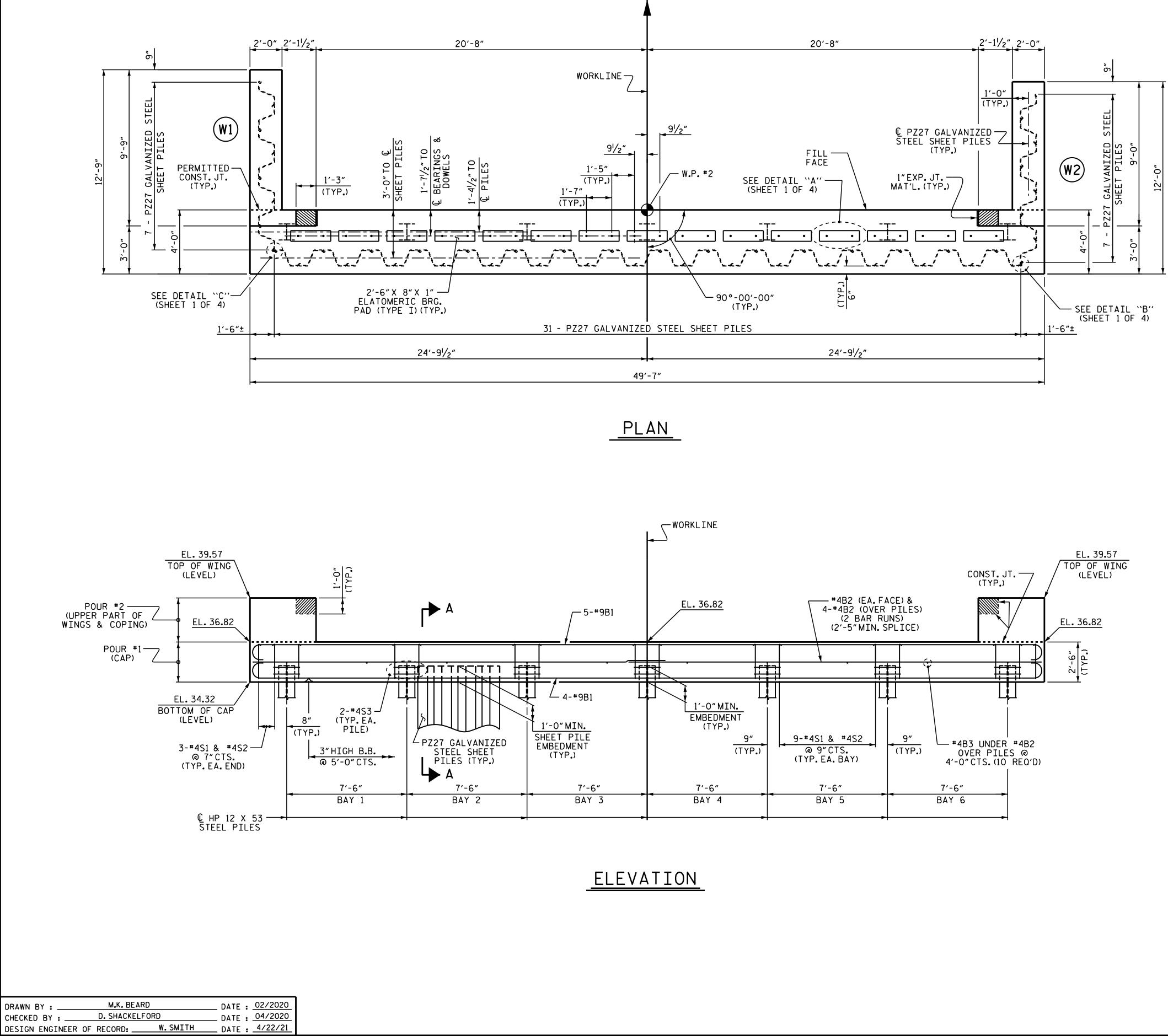
FOR 18" GALVANIZED STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

GALVANIZE THE FULL LENGTH OF PZ27 STEEL SHEET PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. THE CONSTRUCTION SEQUENCE FOR THE 18" STEEL SHEET PILE SYSTEM SHALL BE AS FOLLOWS, UNLESS DIRECTED BY THE ENGINEER:

- 1. INSTALL ALL SHEET PILES TO THE REQUIRED TIP ELEVATIONS AS SHOWN ON THE PLANS. 2. END BENT PILES SHALL BE DRIVEN BEFORE PLACING BACKFILL.
- 3. CONSTRUCT THE CAST-IN-PLACE CONCRETE COPING.
- 4. THE CONCRETE COPING SHALL HAVE A SMOOTH FINISH. CONCRETE COPING IS REQUIRED ALONG THE ENTIRE LENGTH OF STEEL SHEET PILE WINGS.



5, 10, 2021			REVIS	SIO	٧S		SHEET NO.
UMENT NOT CONSIDERED	N0.	BY:	DATE:	N0.	BY:	DATE:	S-9
FINAL UNLESS ALL	1			3			TOTAL SHEETS
IGNATURES COMPLETED	2			4			14



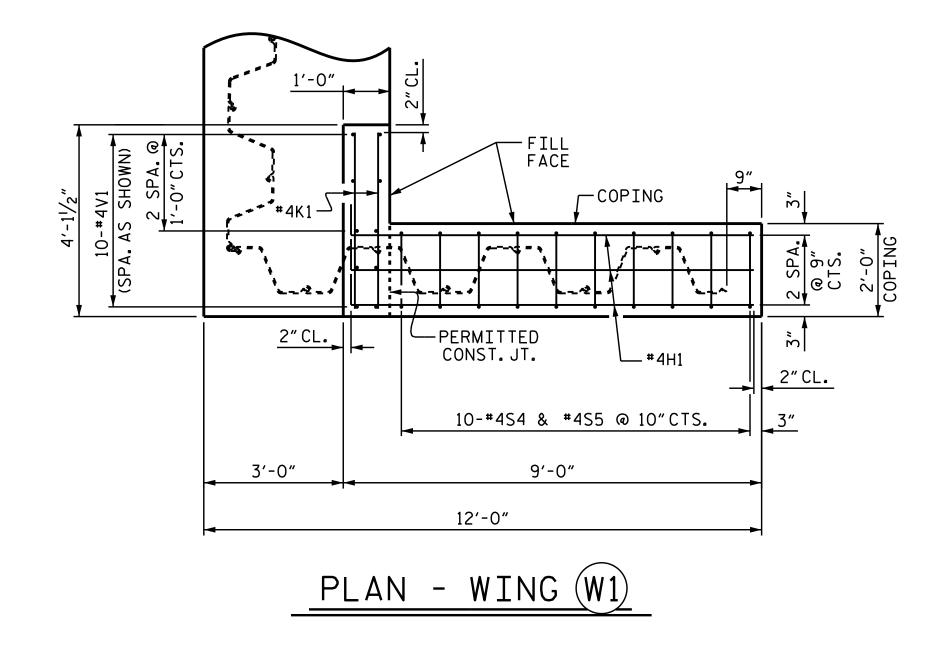
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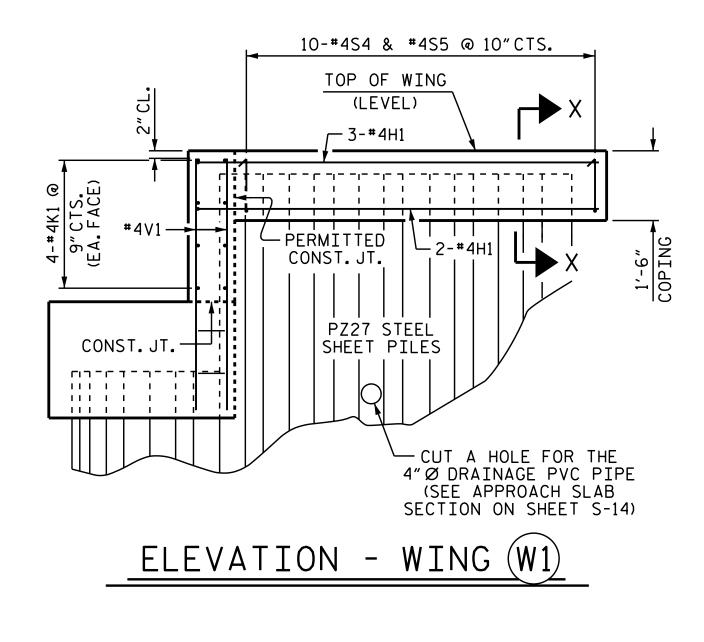
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		E	CT NO. BRUNS	W	ICK	-5642 CC .00 -	DUNTY
	SHE	ET 2 C)F 4				
SEAL 26445 Docusigned by: P. Korey Newton 5/10/2021		DEPA	SUE	0 35	NORTH CAR F TRAN RALEIGH TRUCT BEN	URE	TION
2/10/2021			REVIS	SIO	NS		SHEET NO.
ENT NOT CONSIDERED	NO.	BY:	DATE:	N0.	BY:	DATE:	S-10
NAL UNLESS ALL	1			3			TOTAL SHEETS
ATURES COMPLETED	2			4			14

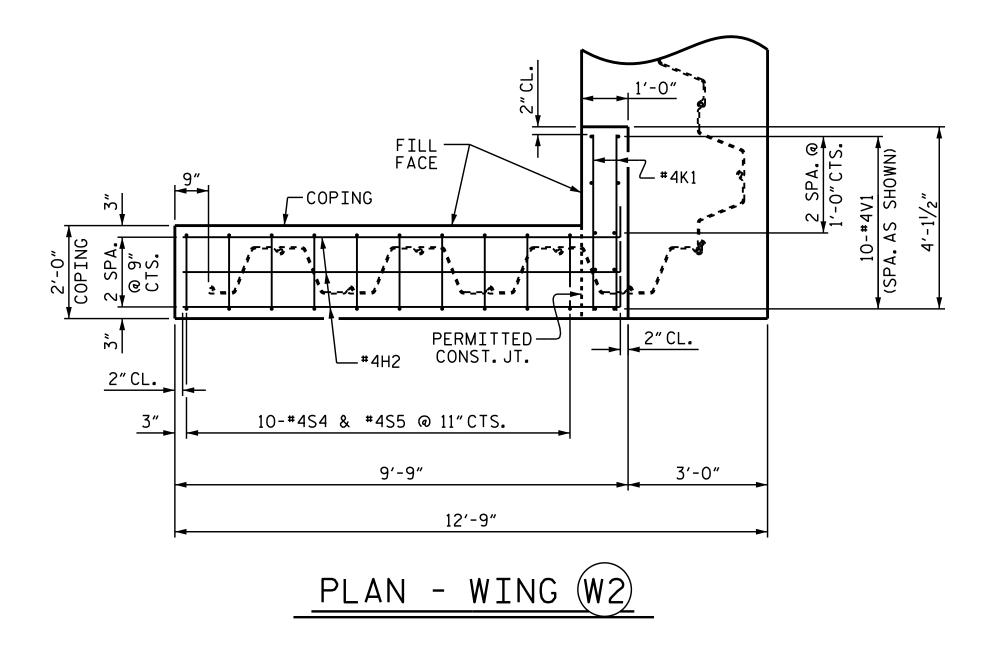


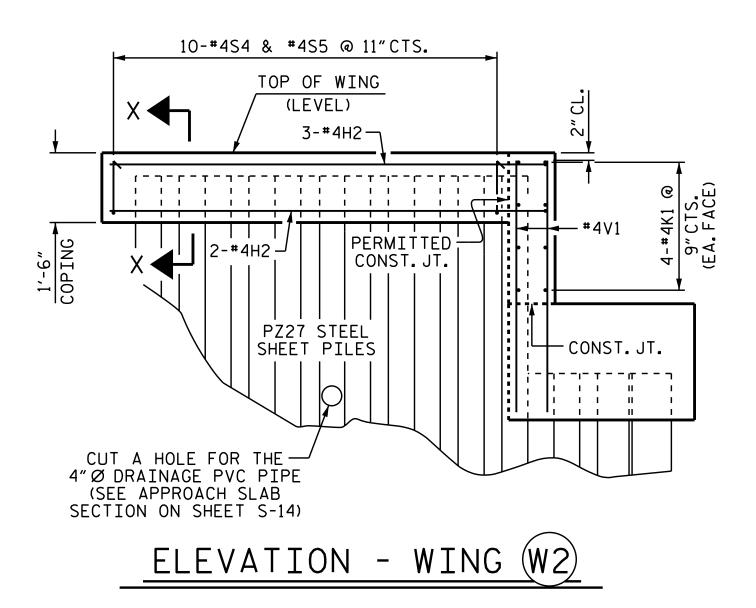


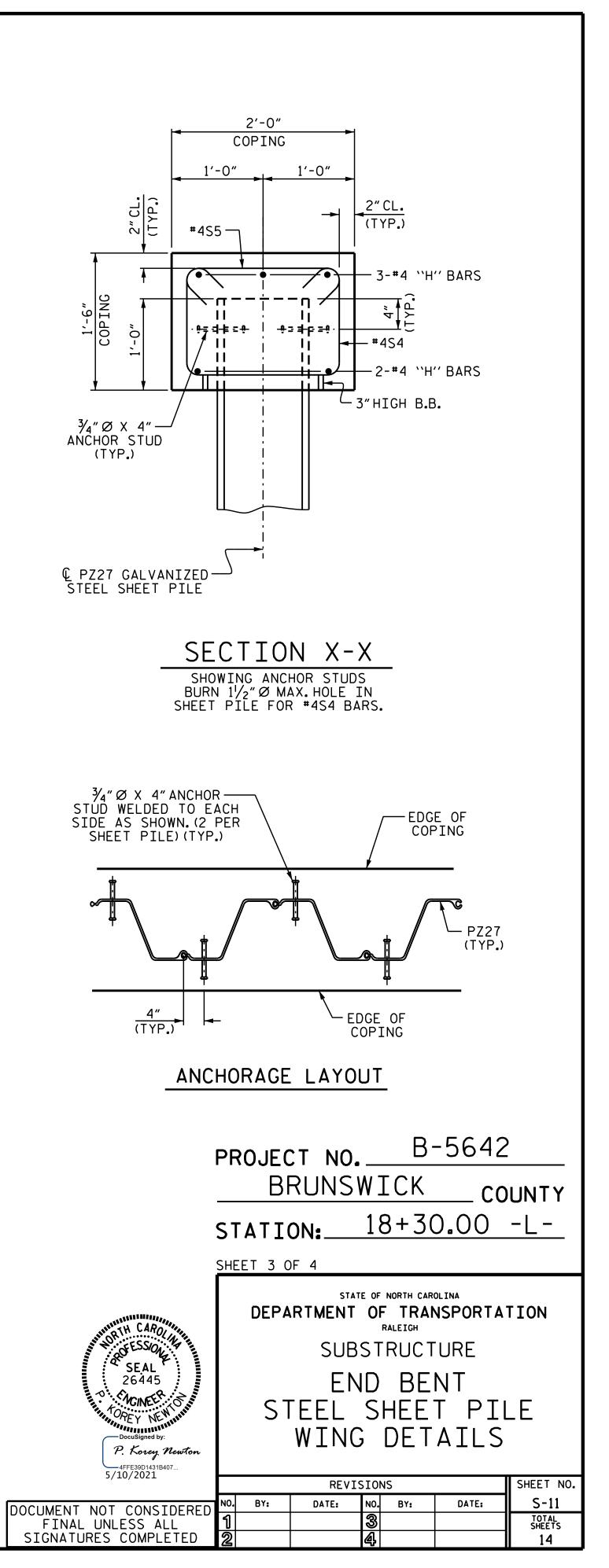
DRAWN BY :	M.K. BE	ARD	DATE :	02/2020
CHECKED BY :	D. SHAC	KELFORD	DATE :	04/2020
DESIGN ENGINEER	OF RECORD: _	W.SMITH		4/22/21

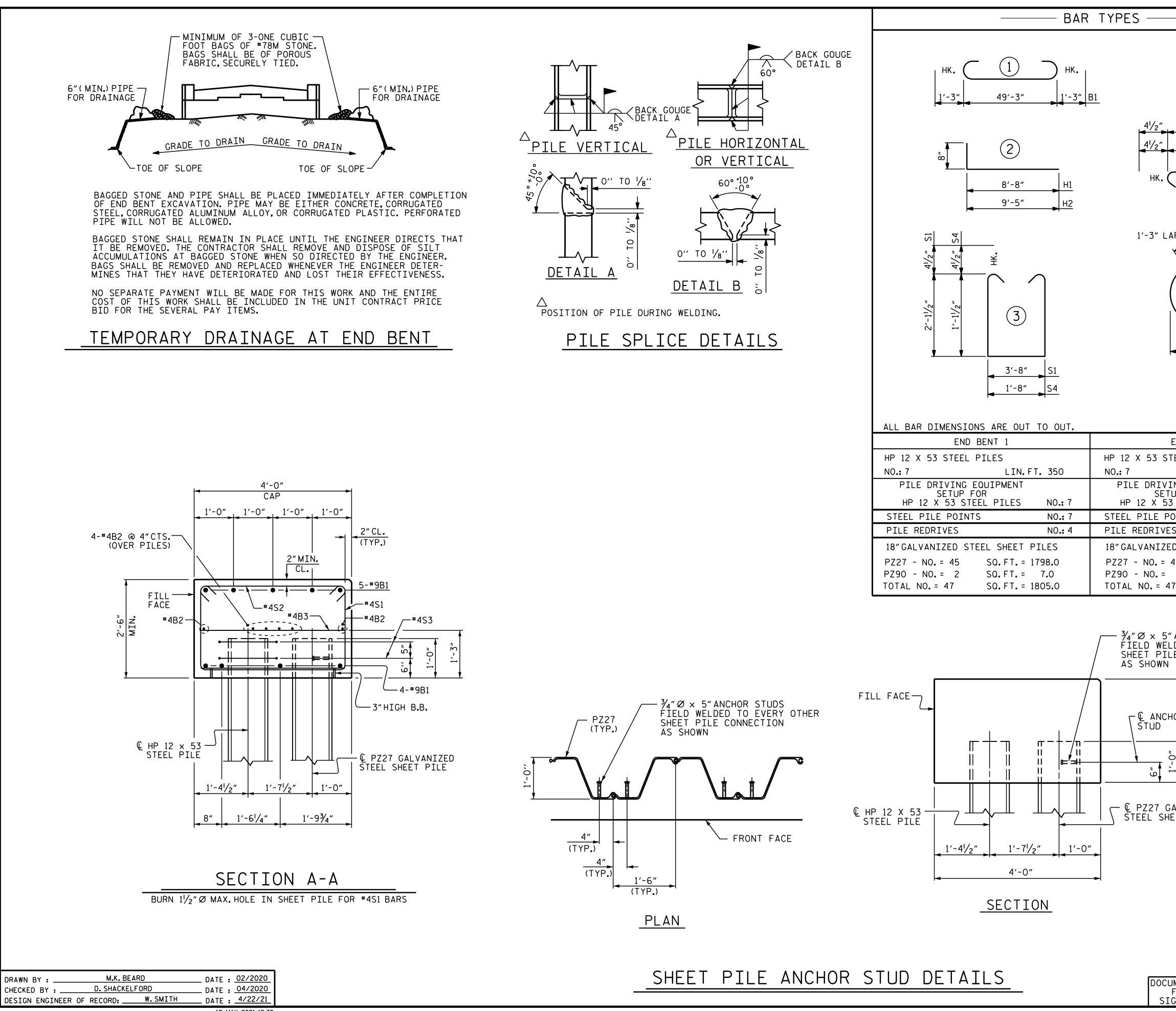
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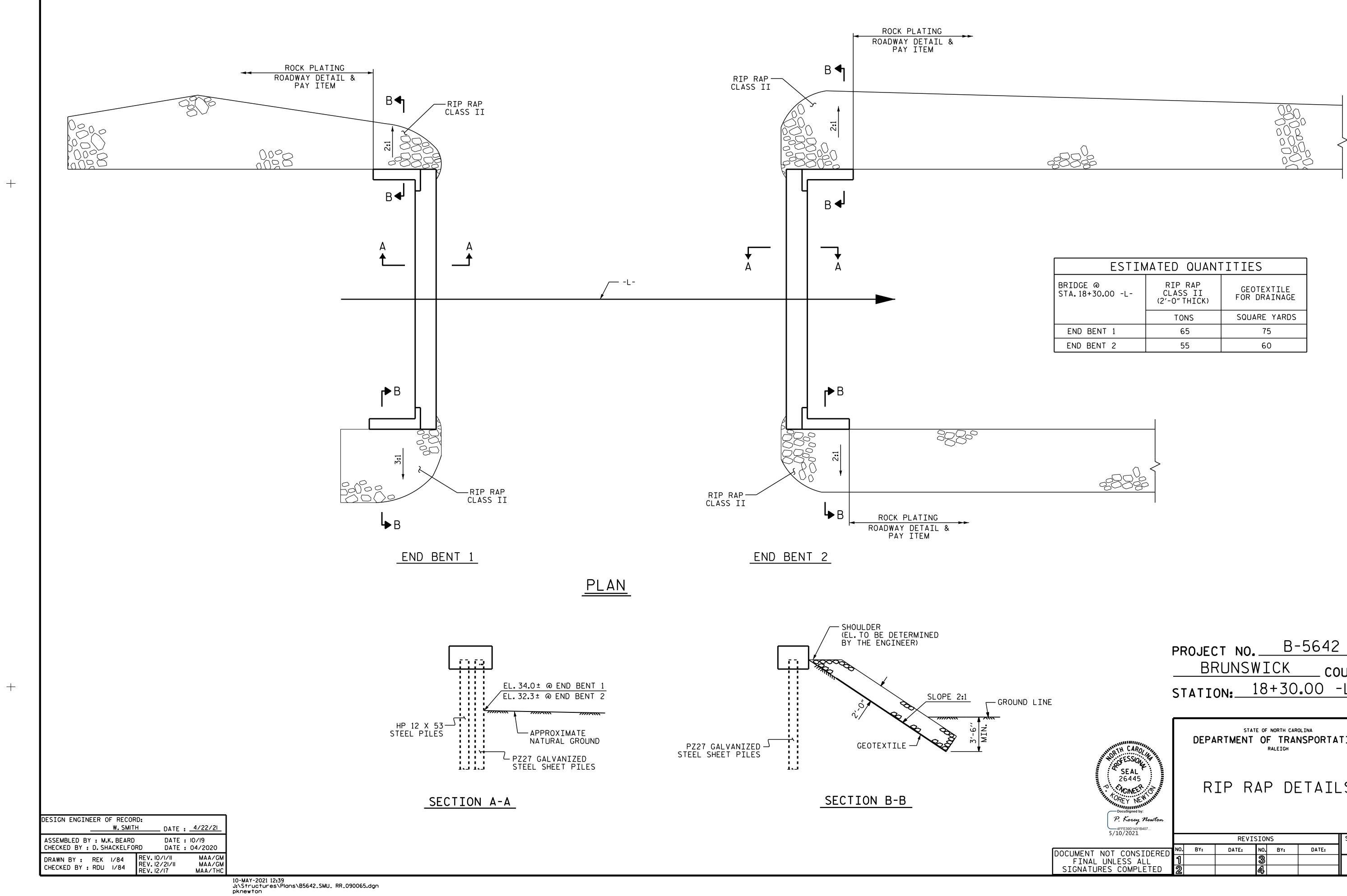
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	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	B1	9	#9	1	51'-9"	1584	
	B2 B3	12 10	#4 #4	STR STR	25'-10" 3'-8"	207 25	
				511	5 0	23	
3'-8" 4 ¹ / ₂ " S2	H1	5	#4	2	9'-4"	31	
1'-8" 4 ¹ /2" S5	H2	5	#4	2	10'-1"	34	
	К1	8	# 4	STR	3'-9"	20	
(4) нк.	S1	60	#4	3	8'-8"	347	
\bigcirc	S2	60	#4	4	4′-5″	177	
	S3	14	#4	5	6'-6"	61	
ΔP\	S4 S5	20 20	#4 #4	3	4'-8" 2'-5"	62 32	
	- 55	20			2 3	JZ	
	V1	20	#4	STR	4'-11"	66	
((5))							
$\bigvee \bigvee /_{1}$							
1'-8"Ø							
		-					
			NG STE		~		
END BENT 2			ND BEN			646 LBS.	
TEEL PILES			NCRET		KDOWN		
LIN.FT. 420 ING EQUIPMENT							
TUP FOR 3 STEEL PILES NO.: 7		#1 (0 000	TNCC	18.4 C.Y.	
OINTS NO.: 7	POUR #2 WINGS & COPINGS 3.6 C.Y.						
S NO.: 4	ΤΟΤΑΙ	_ CLAS			TF		
			S A C	UNCRE		22.0 C.Y.	
			55 A C	UNCKE		22.0 C.T.	
ED STEEL SHEET PILES				UNCKE		22.0 C.1.	
D STEEL SHEET PILES 45 SQ.FT. = 1780.0 2 SQ.FT. = 7.0				UNCRE		22.0 C.1.	
D STEEL SHEET PILES 45 SQ.FT. = 1780.0 2 SQ.FT. = 7.0			55 A C	UNCRE		22.0 C.1.	
ED STEEL SHEET PILES 45 SO.FT. = 1780.0 2 SO.FT. = 7.0			55 A C	UNCRE		22.0 C.1.	
D STEEL SHEET PILES 45 SQ.FT. = 1780.0 2 SQ.FT. = 7.0			55 A C	UNCRE		22.0 C.1.	
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ED STEEL SHEET PILES 45 SO.FT. = 1780.0 2 SO.FT. = 7.0			53 A C	UNCRE		22.0 C.1.	
ANCHOR STUDS D STEEL SHEET PILES SQ.FT. = 1780.0 SQ.FT. = 7.0 SQ.FT. = 1787.0 ANCHOR STUDS DED TO EVERY OTHER			53 A C	UNCRE		22.0 C.1.	
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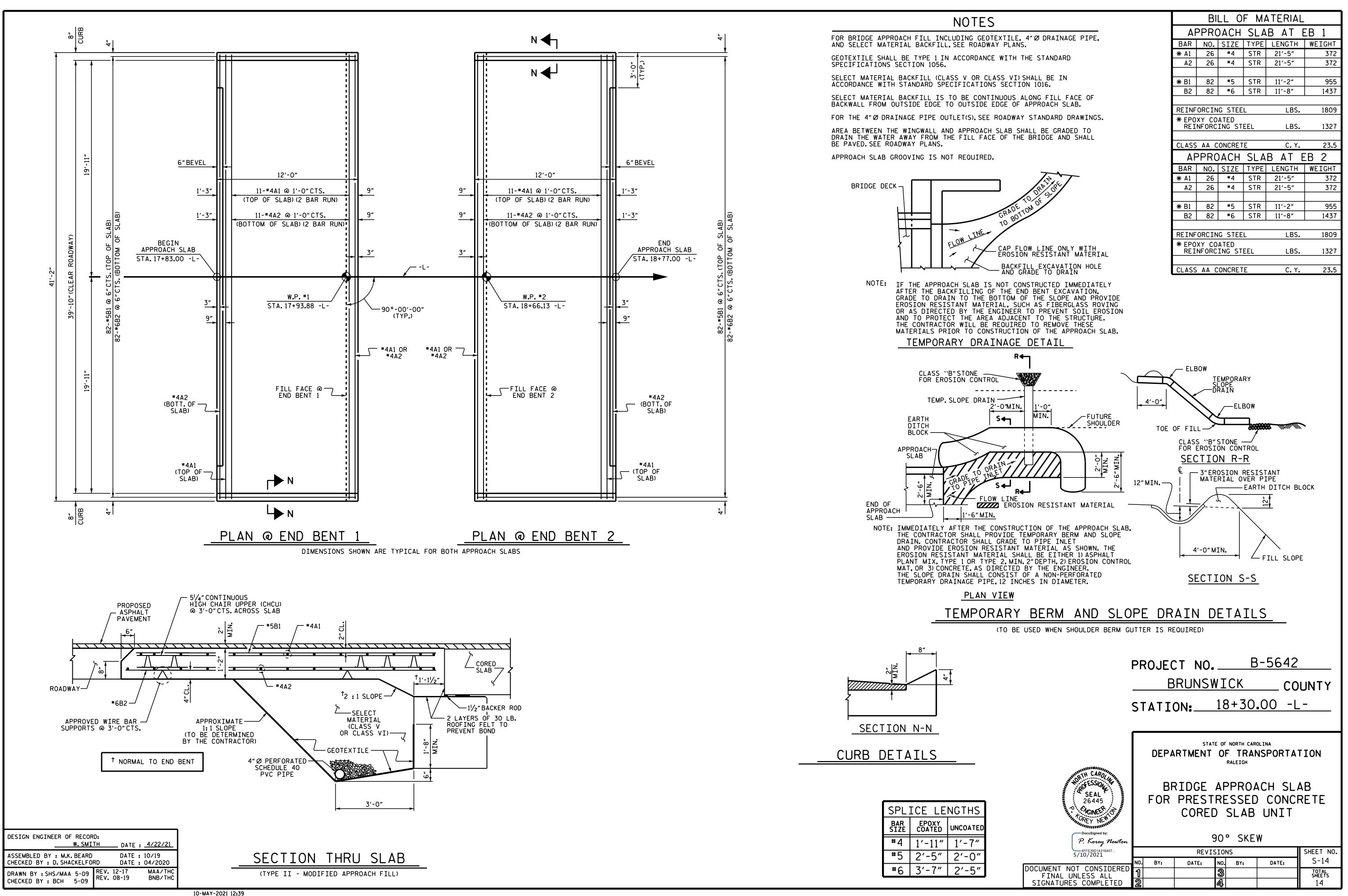
ESTIMATED QUANTITIES								
DGE @ .18+30.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE						
	TONS	SQUARE YARDS						
ND BENT 1	65	75						
ND BENT 2	55	60						

PROJECT N	0. <u> </u>	642
BRUN	COUNTY	
STATION:_	18+30.0	

DEPARTMENT OF TRANSPORTATION

RIP RAP DETAILS

4FFE39D1431B407 5/10/2021							
5/10/2021			REVI	SION	15		SHEET NO.
JMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
FINAL UNLESS ALL	1			3			TOTAL SHEETS
GNATURES COMPLETED	2			4			14



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SPL	SPLICE LENGTHS						
BAR SIZE	EPOXY COATED	UNCOATED					
#4	1'-11"	1'-7"					
# 5	2'-5″	2'-0"					
#6	3'-7"	2'-5"					

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ "Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ "Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ "Ø STUDS FOR 4 - $\frac{3}{4}$ "Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



STD. NO. SN