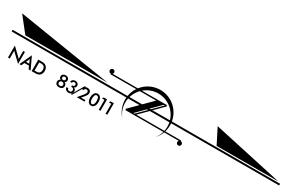
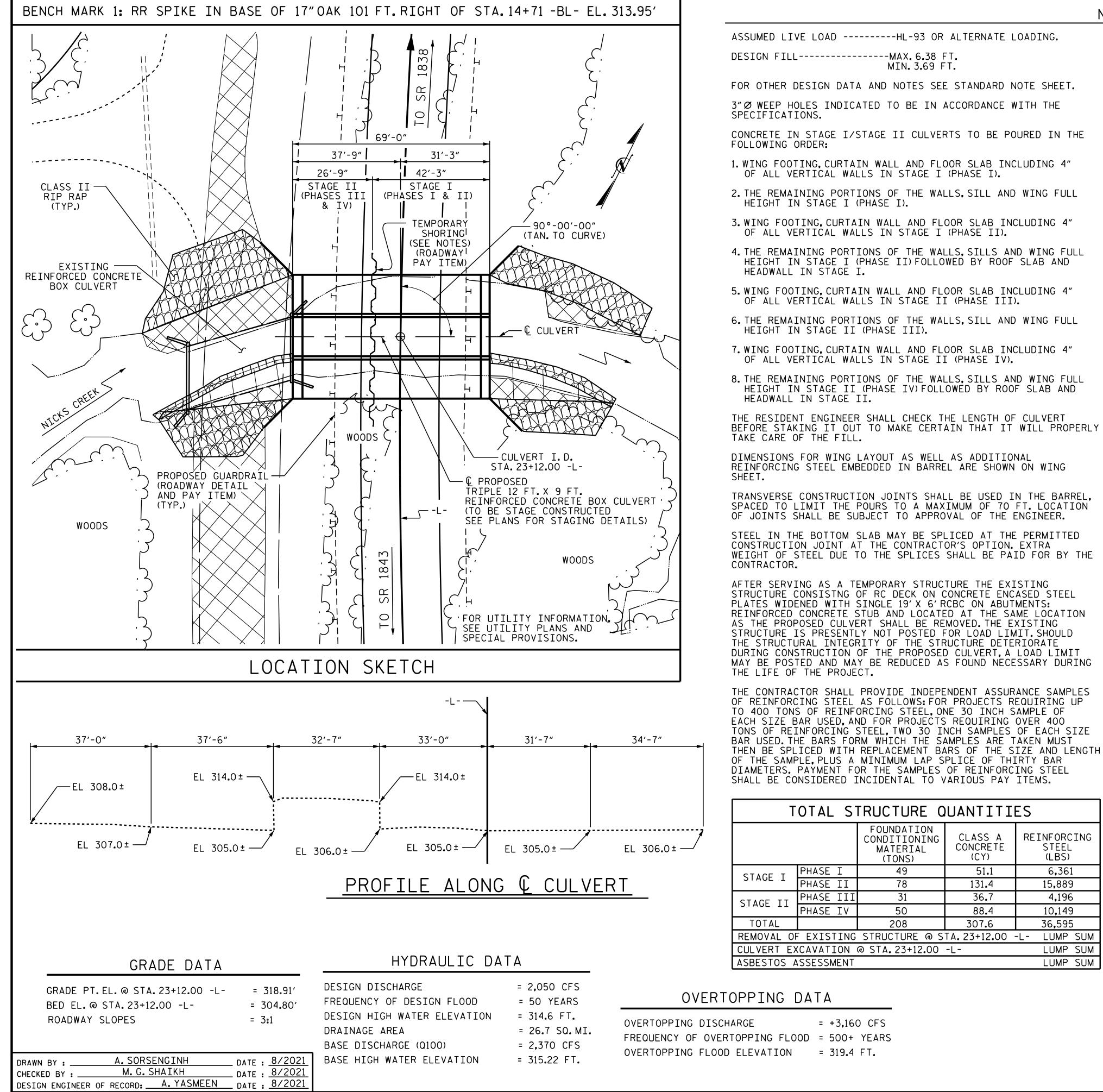


| | | | PROJECT LENGTH |
|---------|----------|---|---|
| 2018 | 0.319 MI | = | LENGTH OF ROADWAY TIP PROJECT BR-0035 |
| | 0.007 MI | = | LENGTH OF STRUCTURE TIP PROJECT BR-0035 |
| LETTING | 0.326 MI | = | TOTAL LENGTH OF TIP PROJECT BR-0035 |

| STATE | STATI | SHEET NO. | TOTAL SHEETS | |
|-------|-------------|--------------|-----------------|-----|
| N.C. | B | 1 | | |
| STAT | e proj. No. | DESCRIP | TION | |
| 49 | 073.1.1 | _ | P.E | • |
| 49 | 073.2.1 | ROW/L | JTIL | |
| 49 | 073.3.1 | 0022015 | CONS | ST. |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |



| Prepared in th DIVISION OF STRUCTURES MAN 1000 BIRCH RALEIGH, | HIGHWAYS NAGEMENT UNIT RIDGE DR. |
|---|---|
| 8 STANDARD SPECIFICATIONS | |
| DATE : NOVEMBER 16, 2021 | W. KEVIN FISCHER, P.E. PROJECT ENGINEER KRISHNA P. SEDAI, P.E. PROJECT DESIGN ENGINEER |



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| TOTAL STRUCTURE QUANTITIES | | | | | | | |
|----------------------------|------------|--|-----------------------------|-------------------------------|--|--|--|
| | | FOUNDATION CONDITIONING MATERIAL (TONS) | CLASS A CONCRETE (CY) | REINFORCING STEEL (LBS) | | | |
| STAGE I | PHASE I | 49 | 51.1 | 6,361 | | | |
| PHASE II | | 78 131.4 | | 15,889 | | | |
| STAGE II PHASE III | | 31 | 36.7 | 4,196 | | | |
| STAGE II | PHASE IV | 50 | 88.4 | 10,149 | | | |
| TOTAL | | 208 | 307.6 | 36,595 | | | |
| REMOVAL OF | F EXISTING | STRUCTURE @ S | TA.23+12.00 · | -L- LUMP SUM | | | |
| CULVERT EX | CAVATION | @ STA.23+12.00 | -L- | LUMP SUM | | | |
| ASBESTOS A | SSESSMENT | | | LUMP SUM | | | |

| OVERTOPPING DISCHARGE | = +3,160 CFS |
|--------------------------------|--------------------------------|
| FREQUENCY OF OVERTOPPING FLOOD | = 500+ YEARS |
| OVERTOPPING FLOOD ELEVATION | = 319.4 FT. |
| | FREQUENCY OF OVERTOPPING FLOOD |

NOTES: AT THE CONT REINFORCING AND BOTH FAC

CONSTRUCTION IN THE SPLIC OF STEEL DUE CONTRACTOR

THE REINFORG THE STANDARD MATERIAL, SE

FOR LIMITS SEE TRAFFIC SHORING FOR

FOR CONSTRUC

FOR TEMPORA SHORING, SEE

DESIGN TEMP LT. TO STATI ASSUMED SOI UNIT UNIT FRIC COHE GROU

BEFORE BEGIN SURVEY EXIS SHORING LOC

LIMITED SUBS OF TEMPORARY STATION 24+0 TEMPORARY SH APPLICABLE CONSTRUCTION

AT THE CONT FOR TEMPORA TO STATION DETAIL 1801.C

DO NOT USE TEMPORARY S STATION 24+0

A 3 FOOT ST FILL FACE OF EXPANSION JC

FOR CULVERT CONTROL PLAN

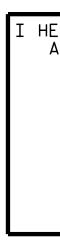
NO PRECAST

FOR SUBMITT FOR FALSEWOF

FOR CRANE SA

FOR GROUT FO

FOR ASBESTOS RENOVATION

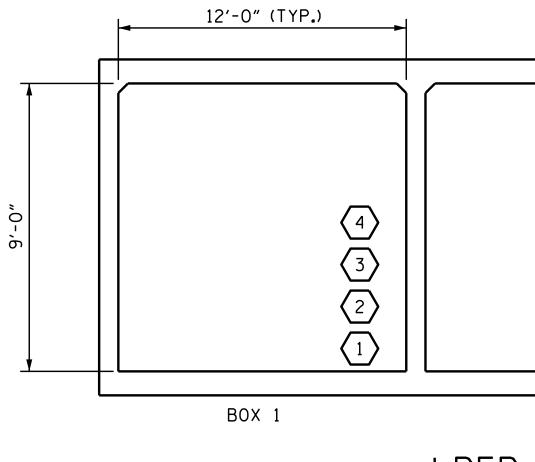




| RACTOR'S OPTION, HE MAY SPLICE T G STEEL IN THE INTERIOR FACES OF ACES OF INTERIOR WALLS ABOVE LOW ON JOINT. THE SPLICE LENGTH SHALL ICE LENGTH CHART SHOWN ON THE PL WE TO THE SPLICES SHALL BE PAID | F EXTERIOR WALL WER WALL L BE AS PROVIDED LANS.EXTRA WEIGHT |
|--|--|
| RCED CONCRETE BOX CULVERT SHALL RD 1.0 FOOT BLANKET OF FOUNDATIO EE SECTION 414 OF THE STANDARD S | ON CONDITIONING |
| OF TEMPORARY SHORING FOR MAINT CONTROL PLANS.FOR PAY ITEM FOR MAINTENANCE OF TRAFFIC, SEE ROA | DR TEMPORARY |
| JCTION SEQUENCE, SEE EROSION CON | |
| ARY SHORING AND POSITIVE PROTEC E PLANS AND TEMPORARY SHORING PI | |
| PORARY SHORING FROM STATION 18+ ION 24+00± -L- 15 FT.LT,FOR THE IL PARAMETERS AND GROUNDWATER E T WEIGHT OF SOIL ABOVE WATER TA T WEIGHT OF SOIL BELOW WATER TA CTION ANGLE, Ø = 30° ESION,c = 0 PSF UNDWATER ELEVATION = 312 FT. | FOLLOWING ELEVATION: ABLE, y=120 PCF |
| NNING TEMPORARY SHORING DESIGN STING GROUND ELEVATIONS IN THE CATIONS TO DETERMINE ACTUAL SHO | VICINITY OF |
| BSURFACE INFORMATION IS AVAILAB RY SHORING FROM STATION 18+00± +00± -L-,15 FT.LT.THE INFORMATIC SHORING DESIGN WAS ASSUMED AND TO THE ACTUAL SITE CONDITIONS E ON. | -L-,37 FT.LT.TO ON PROVIDED FOR MAY NOT BE |
| TRACTOR'S OPTION, USE A STANDARD ARY SHORING FROM STATION 18+00± 24+00± -L-,15 FT.LT.SEE GEOTECH .02 FOR STANDARD TEMPORARY WALLS | ± -L-,37 FT.LT. HNICAL STANDARD |
| CANTILEVER, BRACED, OR ANCHORED S SHORING FROM STATION 18+00± -L-, +00± -L-,15 FT.LT. | -, 37 FT.LT.TO |
| TRIP OF FILTER FABRIC SHALL BE A F THE WING COVERING THE ENTIRE JOINT. | |
| DIVERSION DETAILS AND PAY ITE | EM, SEE EROSION #3 6'-2" #4 7'-4" |
| REINFORCED BOX CULVERT OPTION | #5 8'-6" WILL BE ALLOWED. #6 9'-8" |
| TAL OF WORKING DRAWINGS, SEE SPE | ECIAL PROVISIONS. #7 10'-10" |
| ORK AND FORMWORK, SEE SPECIAL PRO | #8 <u>12'-0"</u> #9 <u>13'-2"</u> |
| SAFETY, SEE SPECIAL PROVISIONS. | #10 14'-6" #11 15'-10" |
| FOR STRUCTURES, SEE SPECIAL PROVE | |
| DS ASSESSMENT FOR BRIDGE DEMOLI ACTIVITIES,SEE SPECIAL PROVISI | |
| HEREBY CERTIFY THESE PLANS | |
| ARE THE AS-BUILT PLANS | PROJECT NO BR-0035 |
| | |
| | MOORE COUNTY |
| | STATION: 23+12.00 -L- |
| | SHEET 1 OF 8 REPLACES BRIDGE NO. 24 |
| DocuSigned by: DocuSigned by: Krishna P. Sedai | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION |
| WESSION | |
| SEAL 30878 SEAL 031583 | TRIPLE 12 FT.X 9 FT. CONCRETE BOX CULVERT |
| ACINER STATES | 90° SKEW |
| /30/2021 9/30/2021 | |
| | |
| DOCUMENT NOT CONSIDERED | REVISIONS SHEET NO. NO. BY: DATE: NO. |
| FINAL UNLESS ALL SIGNATURES COMPLETED | 1 3 TOTAL SHEETS 2 4 8 |

F.A. PROJECT NO.: 0022015

| | | | | | | | | | | OR RATIN CRETE BO> | | | TS | | | |
|----------------|----------------|-------------------|----------------------|---------------------|-----------------------------------|---------------|---|---------------|---------|-----------------------|--|---------------|---------|-----------------|--|---|
| | | | | | | | | | | STRENGTH | I LIM | IT ST | ATE | | | |
| | | | | | | | | | | MOMENT | | | | SHEAR | | 1 |
| LEVEL | | VEHICLE | WEIGHT (W) (TONS) | CONTROLLING # | MINIMUM RATING FACTORS (RF) | TONS = W × RF | LIVE-LOAD FACTORS (Y _{LL}) | RATING FACTOR | BOX NO. | ELEMENT TYPE | DISTANCE FROM LEFT END OF ELEMENT (f+) | RATING FACTOR | BOX NO. | ELEMENT TYPE | DISTANCE FROM LEFT END OF ELEMENT (f†) | |
| | | HL-93 (INVENTORY) | N/A | $\langle 1 \rangle$ | 1.37 | | 1.75 | 1.42 | 1 | TOP SLAB | 5.70 | 1.37 | 1 | BOTTOM SLAB | 11.67 | Γ |
| DESIGN | | HL-93 (OPERATING) | N/A | | 1.78 | | 1.35 | 1.84 | 1 | TOP SLAB | 5.70 | 1.78 | 1 | BOTTOM SLAB | 11.67 | |
| LOAD RATING | | HS-20 (INVENTORY) | 36.000 | 2 | 1.37 | 49.35 | 1.75 | 1.93 | 1 | TOP SLAB | 5.38 | 1.37 | 1 | BOTTOM SLAB | 11.67 | |
| | | HS-20 (OPERATING) | 36.000 | | 1.78 | 63.98 | 1.35 | 2.50 | 1 | TOP SLAB | 5.38 | 1.78 | 1 | BOTTOM SLAB | 11.67 | |
| | | SNSH | 13.500 | | 3.33 | 44.91 | 1.40 | 3.51 | 1 | TOP SLAB | 5.38 | 3.33 | 1 | TOP SLAB | 11.56 | |
| | | SNGARBS2 | 20.000 | | 3.02 | 60.46 | 1.40 | 3.29 | 1 | TOP SLAB | 5.38 | 3.02 | 1 | TOP SLAB | 11.56 | |
| | ICLE | SNAGRIS2 | 22.000 | | 2.79 | 61.28 | 1.40 | 3.19 | 1 | BOT.CORNER WALL | 9.73 | 2.79 | 1 | BOTTOM SLAB | 11.67 | |
| | VEHICLI (V) | SNCOTTS3 | 27.250 | | 1.78 | 48.57 | 1.40 | 1.78 | 1 | TOP SLAB | 5.38 | 1.93 | 1 | TOP SLAB | 11.56 | |
| | ы S | SNAGGRS4 | 34.930 | | 1.77 | 61.82 | 1.40 | 2.10 | 1 | TOP SLAB | 5.38 | 1.77 | 1 | BOTTOM SLAB | 11.67 | |
| | INGL | SNS5A | 35.550 | | 1.75 | 62.27 | 1.40 | 1.97 | 1 | TOP SLAB | 5.70 | 1.75 | 1 | BOTTOM SLAB | 11.67 | |
| | S | SNS6A | 39.950 | | 1.56 | 62.41 | 1.40 | 1.96 | 1 | TOP SLAB | 5.70 | 1.56 | 1 | BOTTOM SLAB | 11.67 | |
| LEGAL | | SNS7B | 42.000 | | 1.48 | 62.14 | 1.40 | 2.03 | 1 | TOP SLAB | 5.38 | 1.48 | 1 | BOTTOM SLAB | 11.67 | |
| LOAD RATING | ER | TNAGRIT3 | 33.000 | | 1.87 | 61.85 | 1.40 | 2.70 | 1 | BOT.CORNER WALL | 9.73 | 1.87 | 1 | BOTTOM SLAB | 11.67 | |
| | TRAIL | TNT4A | 33.080 | | 1.88 | 62.06 | 1.40 | 2.12 | 1 | TOP SLAB | 5.38 | 1.88 | 1 | BOTTOM SLAB | 11.67 | |
| | | TNT6A | 41.600 | | 1.49 | 62.03 | 1.40 | 2.13 | 1 | TOP SLAB | 5.38 | 1.49 | 1 | BOTTOM SLAB | 11.67 | |
| | SEMI- ST) | TNT7A | 42.000 | | 1.52 | 63.69 | 1.40 | 2.22 | 2 | BOTTOM SLAB | 0.95 | 1.52 | 1 | BOTTOM SLAB | 11.67 | |
| | TOR (TT | TNT7B | 42.000 | | 1.52 | 63.95 | 1.40 | 2.02 | 1 | TOP SLAB | 5.38 | 1.52 | 1 | BOTTOM SLAB | 11.67 | |
| | TRAC | TNAGRIT4 | 43.000 | | 1.45 | 62.48 | 1.40 | 2.03 | 1 | TOP SLAB | 5.38 | 1.45 | 1 | BOTTOM SLAB | 11.67 | |
| | TRUCK | TNAGT5A | 45.000 | | 1.39 | 62.65 | 1.40 | 2.03 | 2 | BOTTOM SLAB | 11.72 | 1.39 | 1 | BOTTOM SLAB | 11.67 | |
| | TRL | TNAGT5B | 45.000 | $\langle 3 \rangle$ | 1.39 | 62.64 | 1.40 | 2.04 | 1 | BOTTOM SLAB | 11.72 | 1.39 | 1 | BOTTOM SLAB | 11.67 | |
| EMERGENC | | EV2 | 28.750 | | 2.30 | 66.15 | 1.30 | 2.48 | 1 | TOP SLAB | 5.38 | 2.30 | 1 | BOTTOM SLAB | 11.67 | Γ |
| /EHICLE | | EV3 | 43.000 | $\langle 4 \rangle$ | 1.54 | 66.21 | 1.30 | 1.54 | 1 | TOP SLAB | 5.70 | 1.54 | 1 | BOTTOM SLAB | 11.67 | ſ |

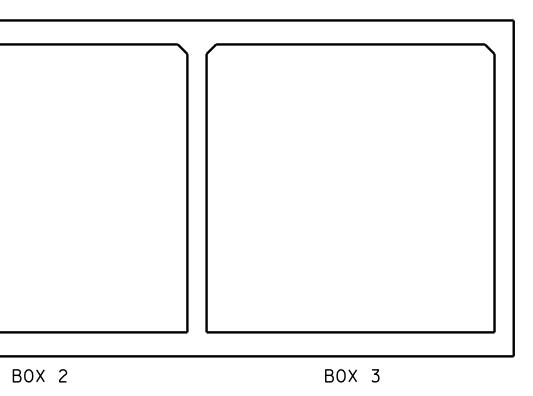


| DRAWN BY : | A. SORSENGINH | DATE : 08/2021 |
|-----------------|-----------------------|----------------|
| CHECKED BY : | M.G.SHAIKH | DATE : 08/2021 |
| DESIGN ENGINEER | OF RECORD: A. YASMEEN | DATE : 08/2021 |

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LRFR SUMMARY

(LOOKING DOWNSTREAM)

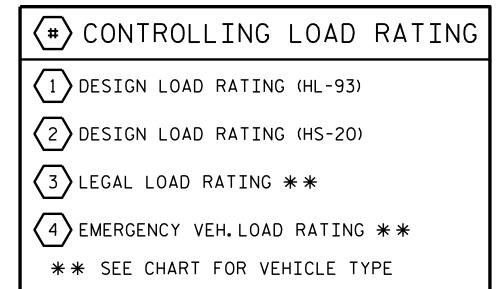
LOAD FACTORS:

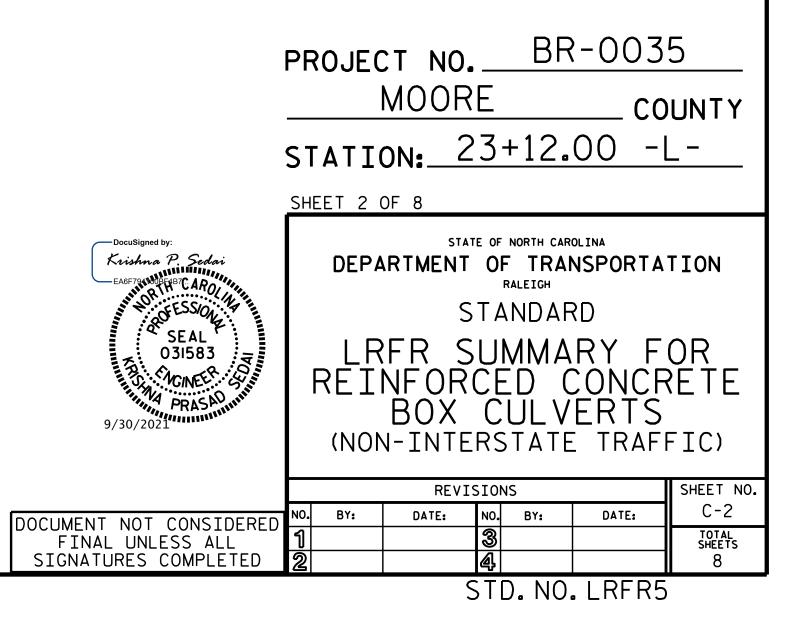
| DESIGN LUAD RATING FACTORS | | | | | | | | | |
|----------------------------|---------------|---------------|--|--|--|--|--|--|--|
| LOAD TYPE | MAX FACTOR | MIN FACTOR | | | | | | | |
| DC | 1.25 | 0.90 | | | | | | | |
| DW | 1.50 | 0.65 | | | | | | | |
| EV | 1.30 | 0.90 | | | | | | | |
| EH | 1.35 | 0.90 | | | | | | | |
| ES | 1.35 | 0.90 | | | | | | | |
| LS | 1.75 | | | | | | | | |
| WA | 1.00 | | | | | | | | |

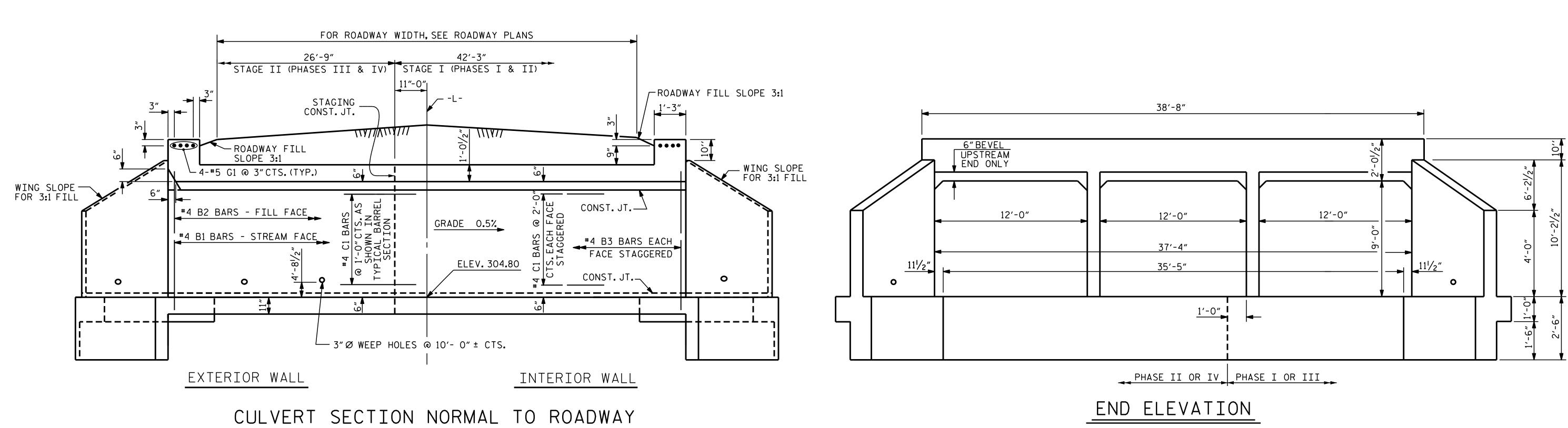
DESTGN LOAD RATING FACTORS

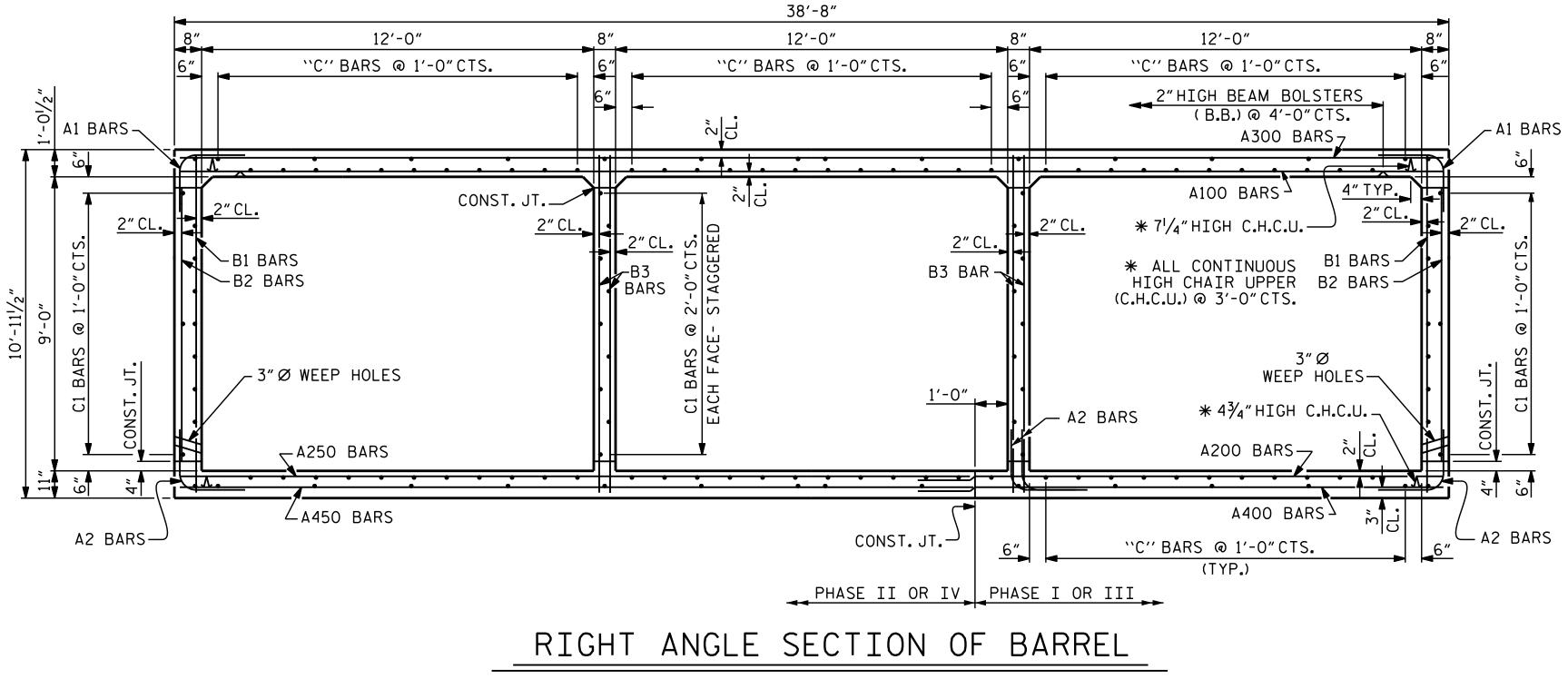
NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.









| 0661 | | |
|----------|-------------------------------------|----------------------|
| . | | |
| S | | |
| AWN | DRAWN BY :A. SORSENGINH | DATE : <u>8/2021</u> |
| REDR/ | CHECKED BY : M.G. SHAIKH | DATE : <u>8/2021</u> |
| Ж | DESIGN ENGINEER OF RECORD:A.YASMEEN | DATE : <u>8/2021</u> |

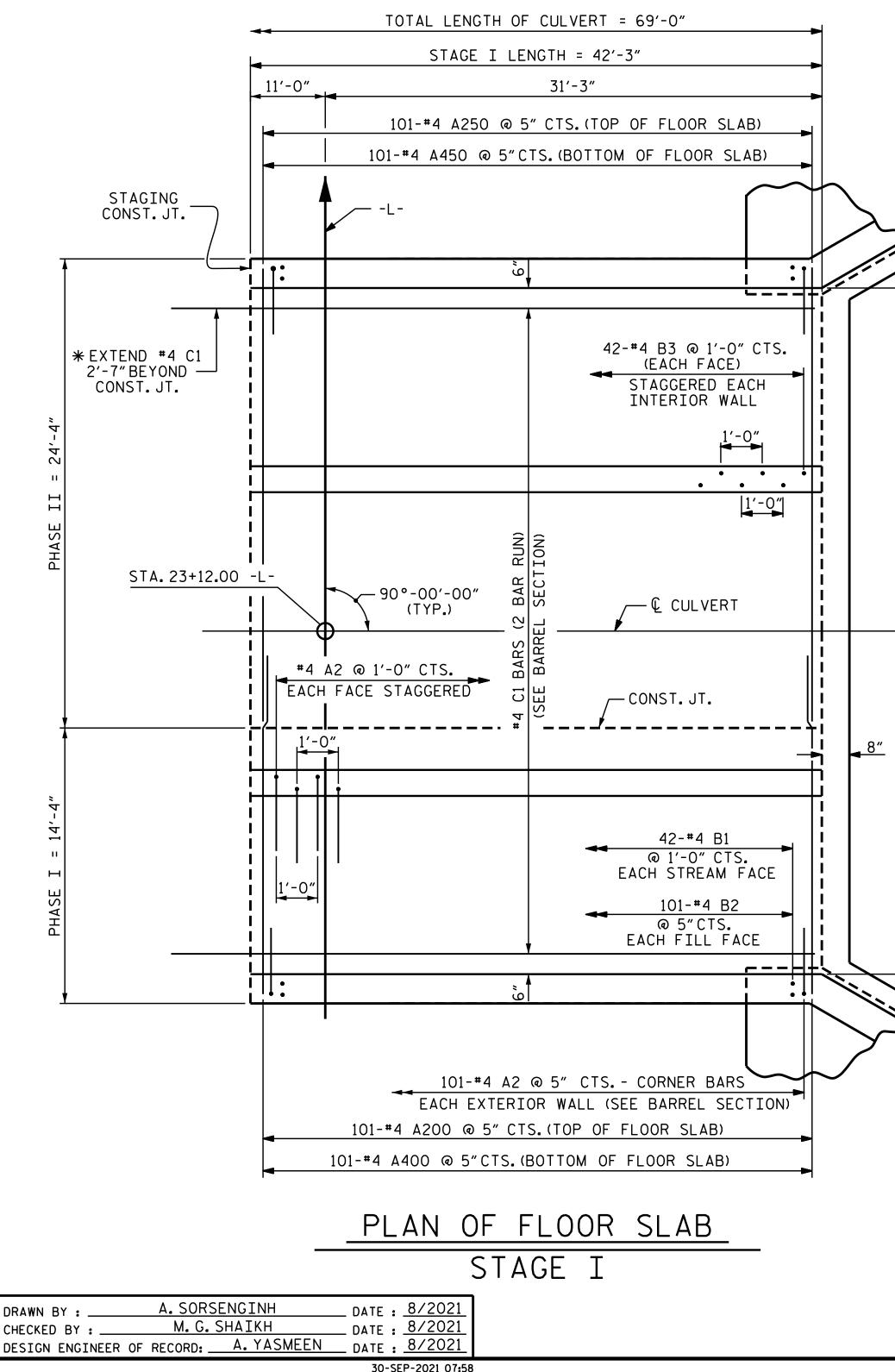
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THERE ARE 136 "C" BARS IN SECTION OF BARREL.

| | PROJECT N MO STATION: SHEET 3 OF 8 | ORE | | 5 unty L- |
|--|---|--|---------|----------------------|
| DocuSigned by: Krishma P. Sedai EAGF791190EFFB7CAR0/ WWW PROFESSION SEAL 031583 PRASAD WWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWW | | STATE OF NORTH CARC NT OF TRAN RALEIGH 12 FT E BOX 0 ° SK | ISPORTA | |
| | R | VISIONS | | SHEET NO. |
| DOCUMENT NOT CONSIDERED | NO. BY: DATE: | NO. BY: | DATE: | C-3 |
| FINAL UNLESS ALL SIGNATURES COMPLETED | 1 2 | 3 4 | | TOTAL SHEETS 8 |

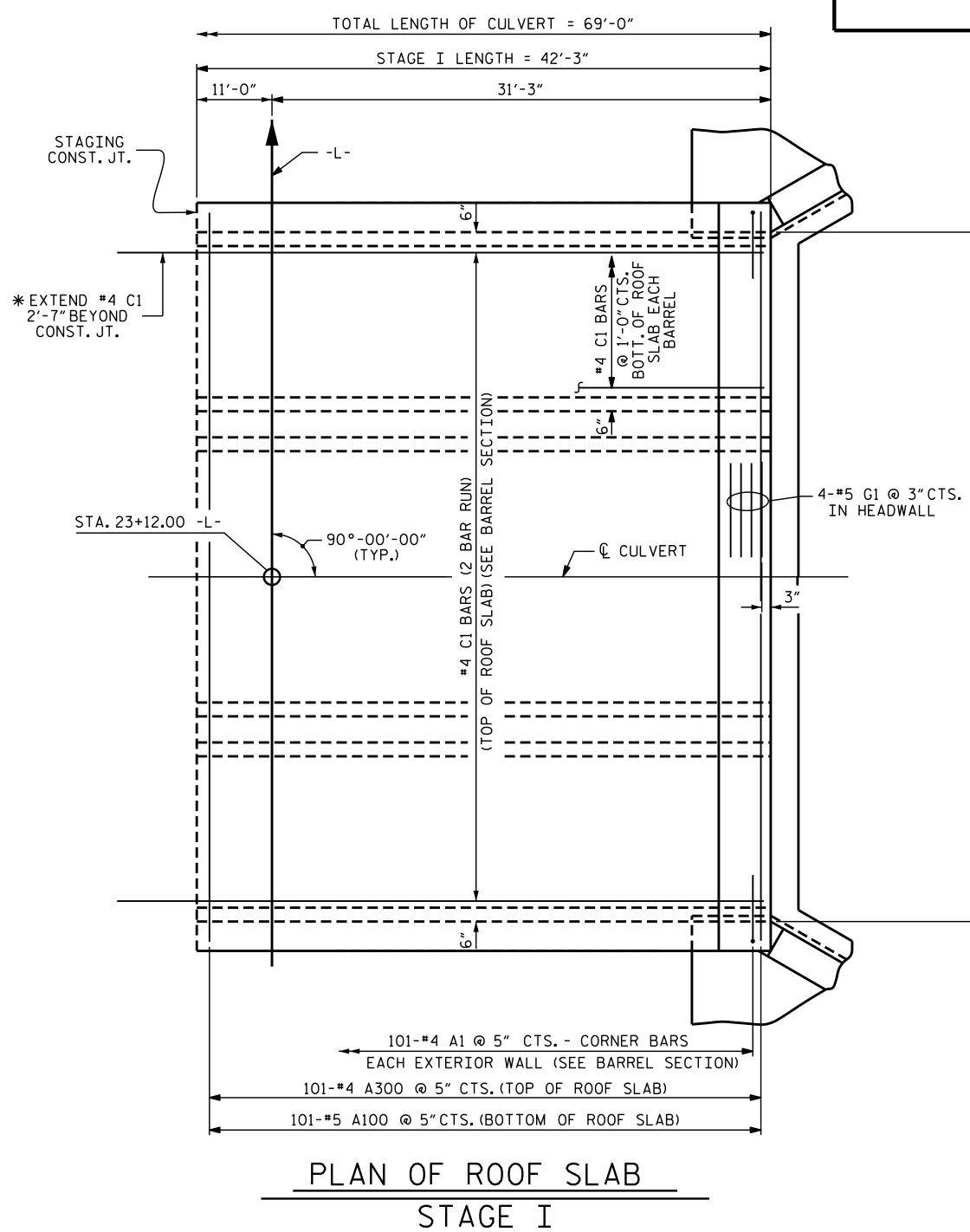
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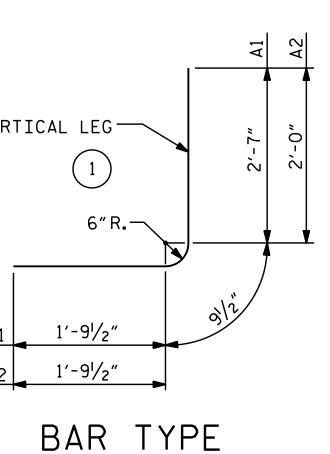


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| | | STAC | GE I | | | |
|--|-------------|------------------------------|---|-------------|------------------------------|----------|
| PHASE | I | | PHASE | II | | |
| CLASS A CONCRETE BARREL @O.931CY/FT WING,ETC SILLS TOTAL | 10.9 0.9 | C.Y. | CLASS A CONCRETE BARREL @ <u>2.775</u> CY/FT WING,ETC SILLS TOTAL | 13.3 0.9 | C.Y. C.Y. | VERT |
| REINFORCING STEEL BARREL WING, ETC SILLS TOTAL | 647 15 | LBS. LBS. LBS. LBS. | REINFORCING STEEL BARREL WING,ETC SILLS TOTAL | 18 | LBS. LBS. LBS. LBS. | A1 A2 |





BAR DIMENSIONS ARE OUT TO OUT

ERI(

| | 017 | | | | |
|-----------------|---------|------------|-------|---------|--------|
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| A1 | 101 | #4 | 1 | 5'-2″ | 349 |
| Α2 | 184 | #4 | 1 | 4'-7" | 563 |
| | | | | | |
| A200 | 101 | #4 | STR. | 16'-2" | 1091 |
| A400 | 101 | #4 | STR. | 16'-2" | 1091 |
| | | | | | |
| B1 | 42 | #4 | STR. | 10'-5" | 292 |
| B2 | 101 | #4 | STR. | 8'-4" | 562 |
| B3 | 84 | #4 | STR. | 10'-5" | 585 |
| | | | | | |
| C1 | 74 | #4 | STR. | 23'-7" | 1166 |
| | REINFOR | RCING | STEEL | = 5,699 | B LBS |
| | STA | GE I | PH | ASE II | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| A1 | 101 | #4 | 1 | 5′-2″ | 349 |
| Α2 | 101 | #4 | 1 | 4'-7" | 309 |
| | | | | | |
| A100 | 101 | # 5 | STR. | 38'-3" | 4029 |
| A250 | 101 | #4 | STR. | 24'-0" | 1619 |
| A300 | 101 | #4 | STR. | 38'-3″ | 2581 |
| A450 | 101 | #4 | STR. | 24'-0" | 1619 |
| | | | | | |
| B1 | 42 | #4 | STR. | 10'-5" | 292 |
| B2 | 101 | #4 | STR. | 8'-4" | 562 |
| B3 | 84 | #4 | STR. | 10'-5" | 585 |
| | | | | | |
| C1 | 198 | #4 | STR. | 23'-7" | 3119 |
| | | | | | |
| G1 | 4 | # 5 | STR. | 38'-4" | 160 |
| | | | | | |
| | REINFO | RCING | STEEL | = 15,2 | 24 LBS |
| # <i>A</i> C1 C | | ר רדרי | | | |

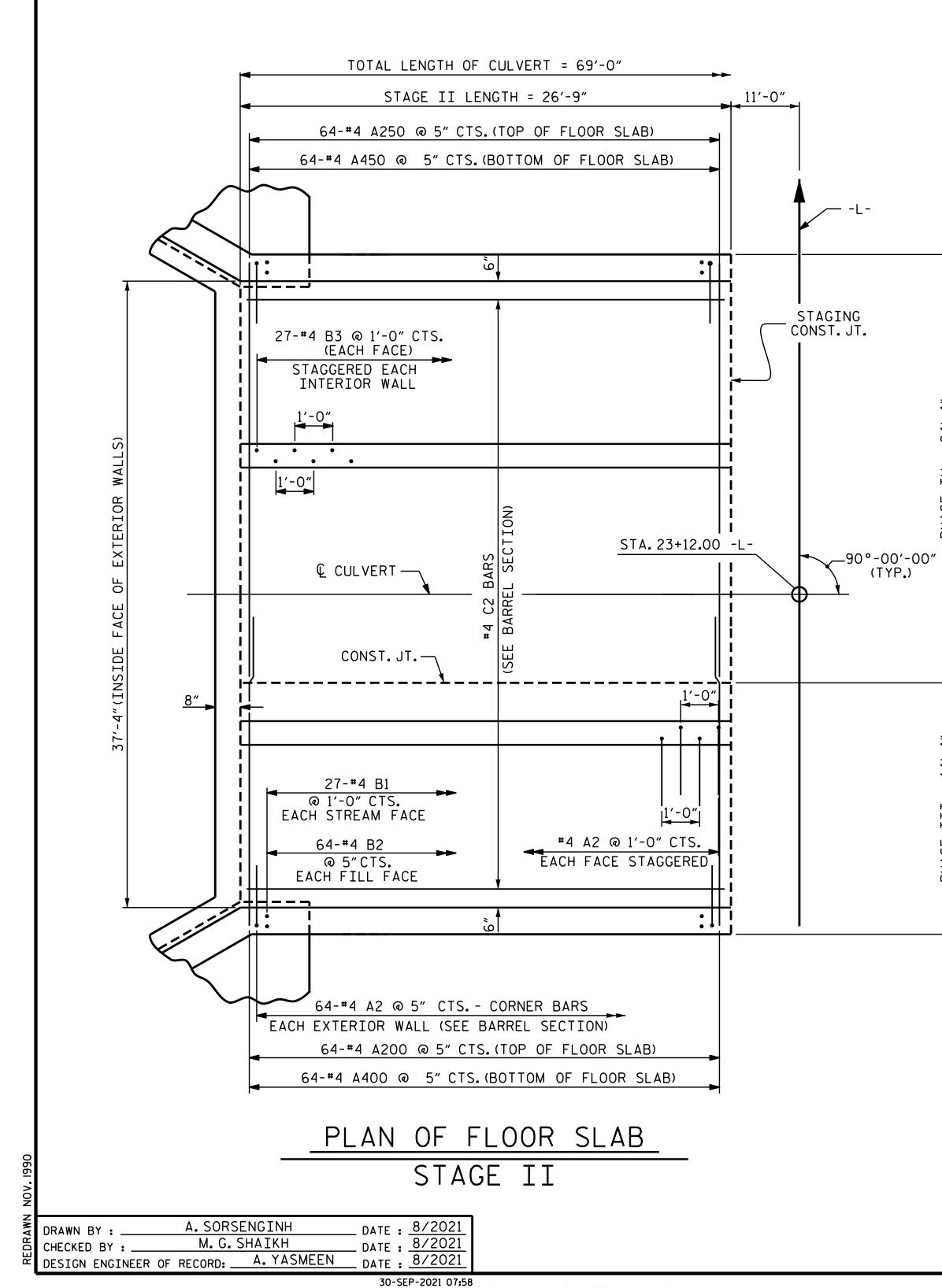
BILL OF MATERIAL

STAGE I PHASE I

* #4 C1 SHALL BE FIELD BENT AS NECESSARY.

| SPLICE | LENGTH | IS CHART |
|--------|--------|------------------|
| BAR | SIZE | SPLICE LENGTH |
| A100 | #5 | 2'-4" |
| A200 | #4 | 1'-10" |
| A250 | #4 | 1'-10" |
| A300 | #4 | 1'-10" |
| A400 | #4 | 1'-10" |
| A450 | #4 | 1'-10" |
| B1 | #4 | 1'-10" |
| B3 | #4 | 1'-10" |
| C1 | #4 | 2'-5″ |
| G2 | #5 | 3'-0" |

| | PROJEC | <u>100М</u> 2зис | RE | | | 35 OUNTY -L- |
|--|---------|---------------------|-------------------------|----------|---------|----------------------|
| DocuSigned by: Krishna P. Sodai EAGF79000ERHBCAR0/ SEAL 031583 PRASAD | TRI | RTMENT | RALE 2 5 7 A (| FT OX | ISPORTA | |
| | | REVIS | SIONS | | | SHEET NO. |
| OCUMENT NOT CONSIDERED | NO. BY: | DATE: | | 3Y: | DATE: | |
| FINAL UNLESS ALL SIGNATURES COMPLETED | 1 2 | | 3 4 | | | TOTAL SHEETS 8 |

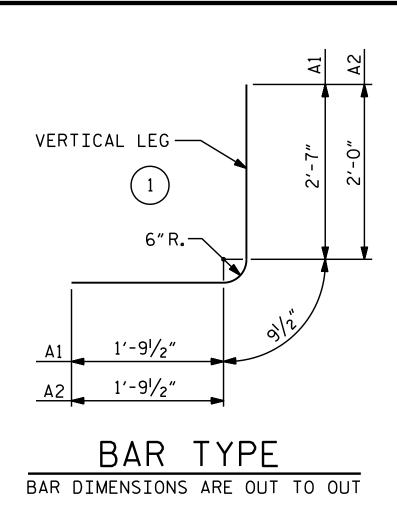


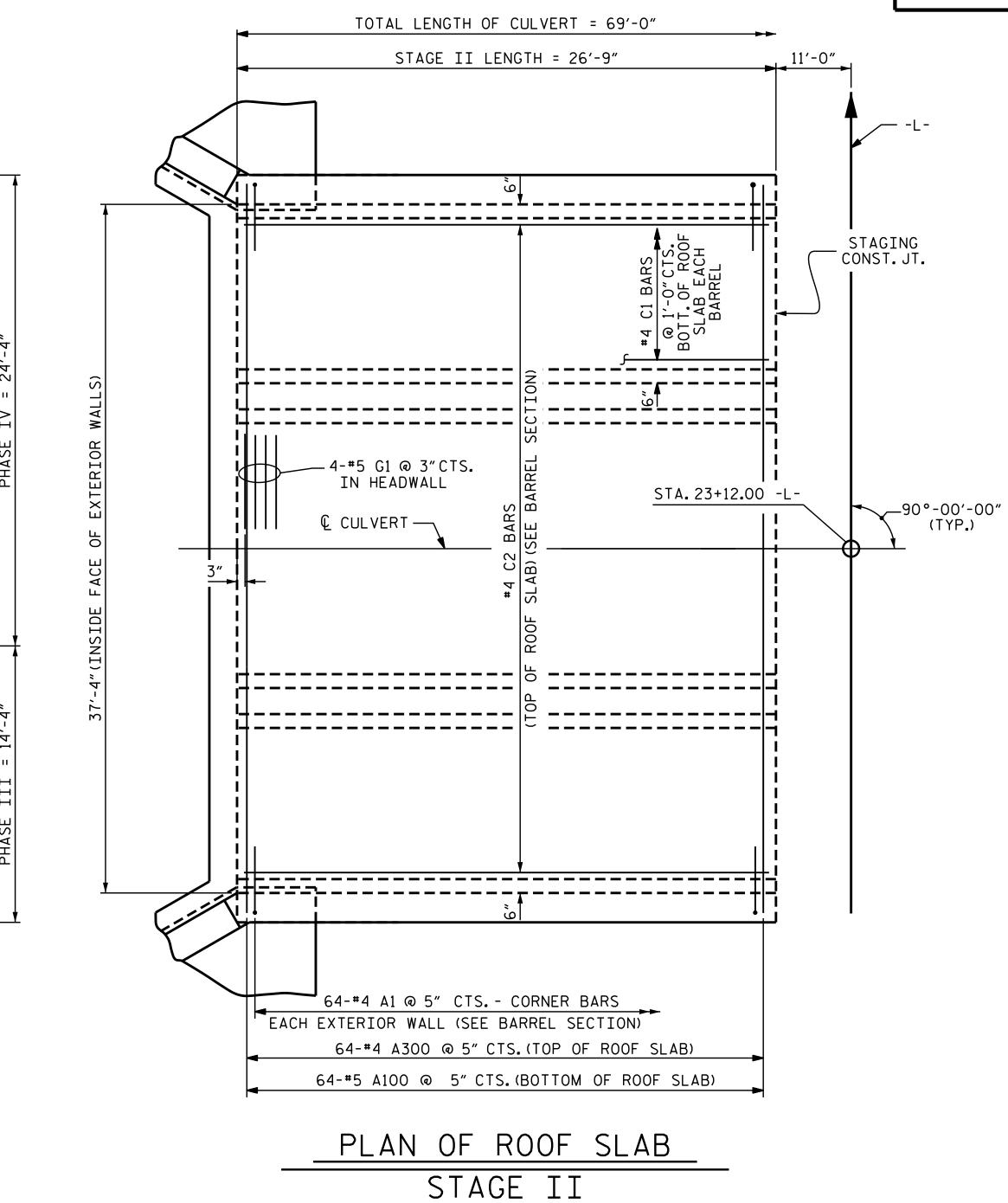
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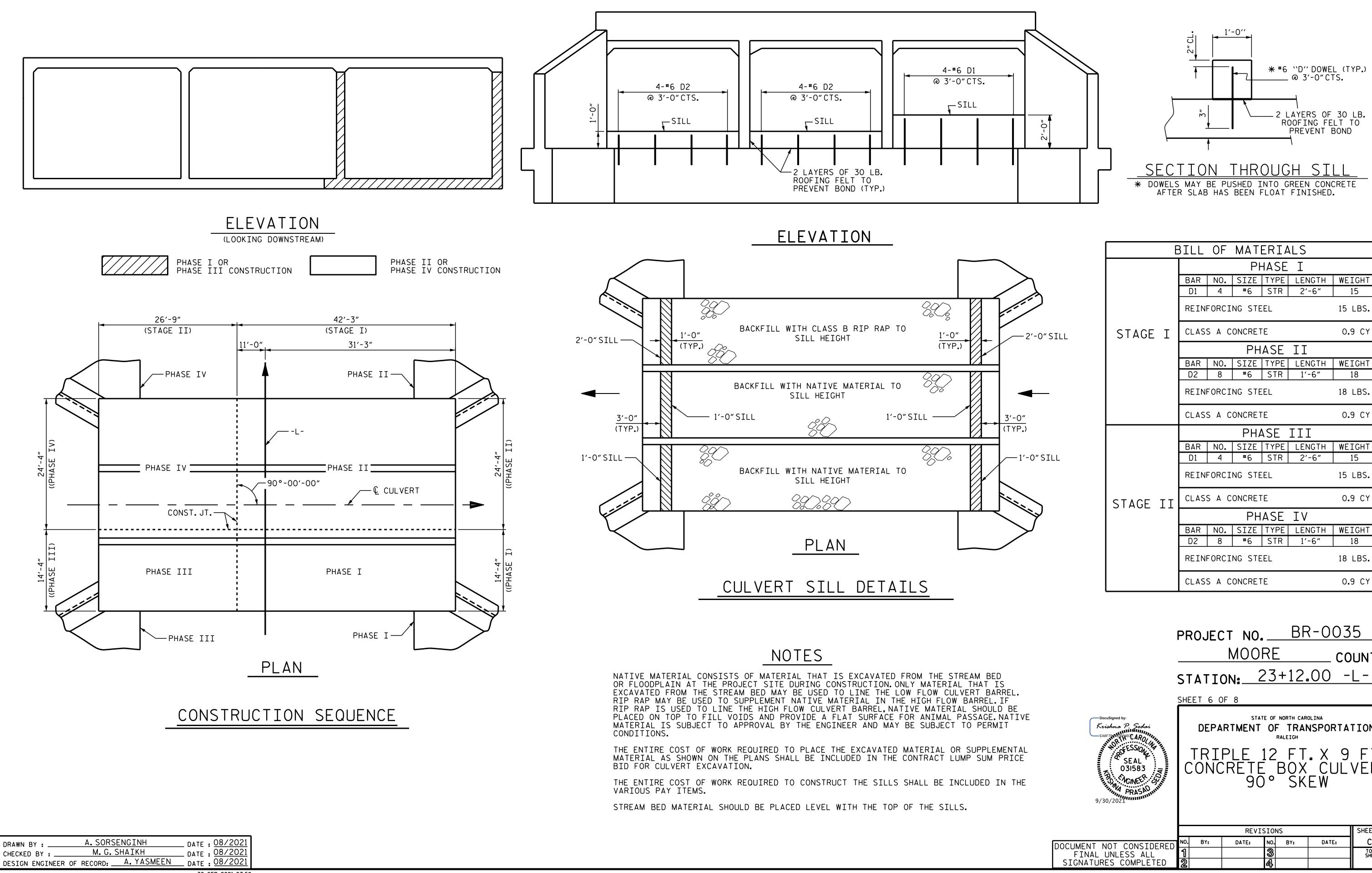
| STAGE II | | | | | | | | |
|--|-------------|------------------------------|---|---------|--------------------|--|--|--|
| PH | ASE III | | PH | ASE IV | | | | |
| CLASS A CONCRETE BARREL @ 0.931 WING,ETC SILLS TOTAL | 10.9 0.9 | C.Y. C.Y. | CLASS A CONCRETE BARREL @ 2.775 WING,ETC. SILLS TOTAL | 13 | 3.3 C.Y. | | | |
| REINFORCING STEEL BARREL WING, ETC SILLS TOTAL | 647 15 | LBS. LBS. LBS. LBS. | REINFORCING STEEL BARREL WING,ETC SILLS TOTAL | 64 1 | 17 LBS. 18 LBS. | | | |





| BILL OF MATERIAL | | | | | | |
|------------------|---------|------------|-------|----------|--------|--|
| (| STAGE | E II | PH | ASE II | I | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| A1 | 64 | #4 | 1 | 5'-2″ | 221 | |
| A2 | 117 | #4 | 1 | 4'-7" | 358 | |
| | | | | | | |
| A200 | 64 | #4 | STR. | 16'-2" | 691 | |
| A400 | 64 | #4 | STR. | 16'-2" | 691 | |
| | | | | | | |
| B1 | 27 | #4 | STR. | 10'-5" | 188 | |
| B2 | 64 | #4 | STR. | 8'-4" | 356 | |
| B3 | 54 | #4 | STR. | 10'-5" | 376 | |
| | | | | | | |
| C2 | 37 | #4 | STR. | 26′-5″ | 653 | |
| 1 | REINFOR | RCING | STEEL | = 3,534 | LBS | |
| | | EI | _ | ASE IV | | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| A1 | 64 | #4 | 1 | 5'-2" | 221 | |
| A2 | 64 | #4 | 1 | 4'-7" | 196 | |
| | | | | | | |
| A100 | 64 | # 5 | STR. | 38'-3" | 2553 | |
| A250 | 64 | #4 | STR. | 24'-0" | 1026 | |
| A300 | 64 | #4 | STR. | 38'-3" | 1635 | |
| A450 | 64 | #4 | STR. | 24'-0" | 1026 | |
| | | | | | | |
| B1 | 27 | #4 | STR. | 10'-5" | 188 | |
| B2 | 64 | #4 | STR. | 8'-4" | 356 | |
| B3 | 54 | #4 | STR. | 10'-5" | 376 | |
| | | | | | | |
| C2 | 99 | #4 | STR. | 26'-5" | 1747 | |
| | A | | | 70/ 4" | 100 | |
| G1 | 4 | # 5 | STR. | 38'-4" | 160 | |
| | | | | | | |
| | REINFO | RCING | STEEL | . = 9,48 | 34 LBS | |

| SEAL O31583 | | PROJEC | <u>MOOR</u> | E | | 5 UNTY L- |
|---|---|---------------|-------------|---|----------------------------|-----------------|
| PRASAD 9/30/2021 CONCRETE BOX CULVERT STAGE II 90° SKEW | DocuSigned by: Krishma P. Sedai EAGE 790 SUBERED C AROL NOR FESSION SEAL 031583 PRASHO INTITUTION | depaf TRIF | RTMENT | OF TRAN RALEIGH 2 F T BOX AGE | SPORTA X 9 CUL II | |
| REVISIONS SHEET NO. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL NO. BY: DATE: NO. BY: DATE: C-5 1 3 TOTAL SHEETS | | | DATE: N | 0. BY: | DATE: | C-5 |

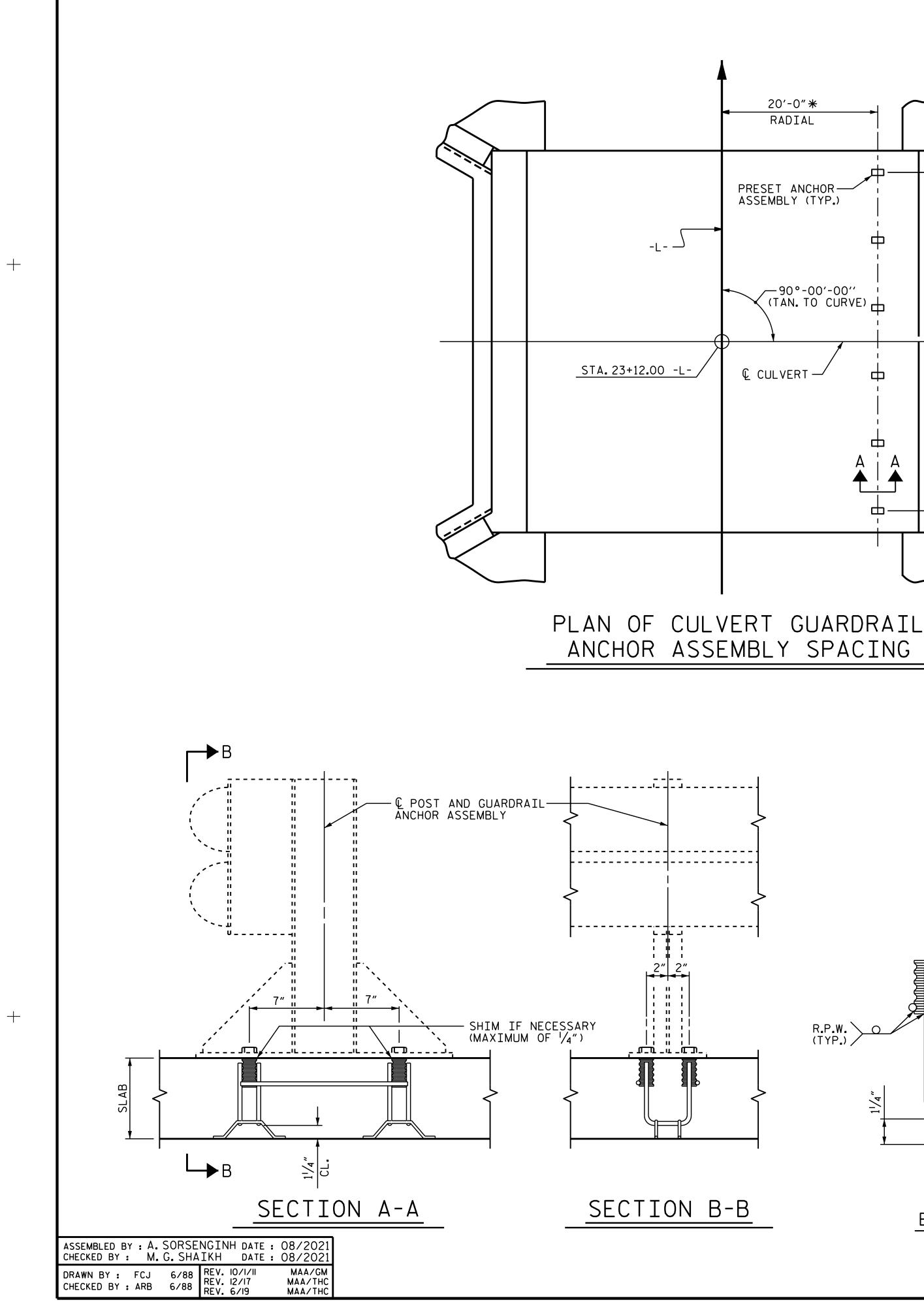


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| | E | BILL OF MATERIALS | | | | | | |
|-------|----|-------------------|------------------|------------|------|--------|---------|--|
| | | | PHASE I | | | | | |
| | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| | | D1 | 4 | # 6 | STR | 2'-6" | 15 | |
| | | REINF | FORCI | NG STE | EL | | 15 LBS. | |
| STAGE | Ι | CLASS | 5 A C | ONCRET | Ē | | 0.9 CY | |
| | | | | PH | ASE | II | | |
| | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| | | D2 | 8 | # 6 | STR | 1'-6" | 18 | |
| | | REIN | 18 LBS. | | | | | |
| | | CLASS | 5 A C | ONCRET | Ē | | 0.9 CY | |
| | | | | | | | | |
| | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| | | D1 | 4 | # 6 | STR | 2'-6" | 15 | |
| | | REIN | 15 LBS. | | | | | |
| STAGE | II | CLASS | 5 A C | ONCRET | Ē | | 0.9 CY | |
| | | | | | | | | |
| | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| | | D2 | 8 | # 6 | STR | 1'-6" | 18 | |
| | | REIN | FORCI | NG STE | EL | | 18 LBS. | |
| | | CLASS | CLASS A CONCRETE | | | | | |

| - | PROJECT NO. <u>BR-0035</u> <u>MOORE</u> COUNTY STATION: <u>23+12.00</u> -L- |
|--|---|
| | SHEET 6 OF 8 |
| E DocuSigned by: Krishna P. Sedai EAGF720008F48CAR0/ PFESSION SEAL 031583 PRASAD 9/30/2021 | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT 90° SKEW |
| | |
| | REVISIONS SHEET NO. |
| DOCUMENT NOT CONSIDERED | NO. BY: DATE: NO. BY: DATE: C-6 |
| FINAL UNLESS ALL | 1 3 TOTAL SHEETS |
| SIGNATURES COMPLETED | ② |





20'-0"* RADIAL



GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE. FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

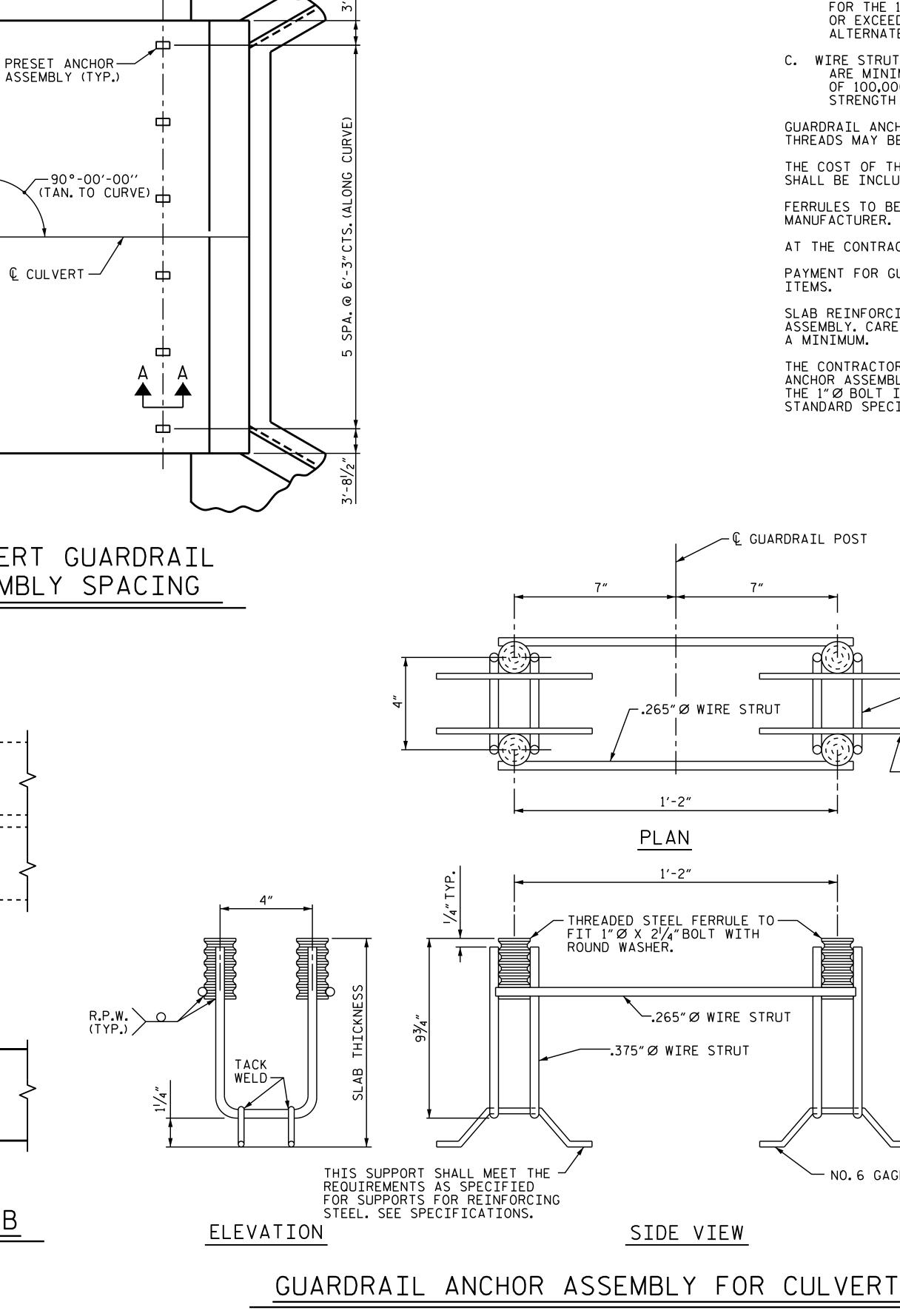
MANUFACTURER.

AT THE CONTRACTOR'S OPTION. FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY

ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".

B. 4 - 1" Ø X 2¹/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

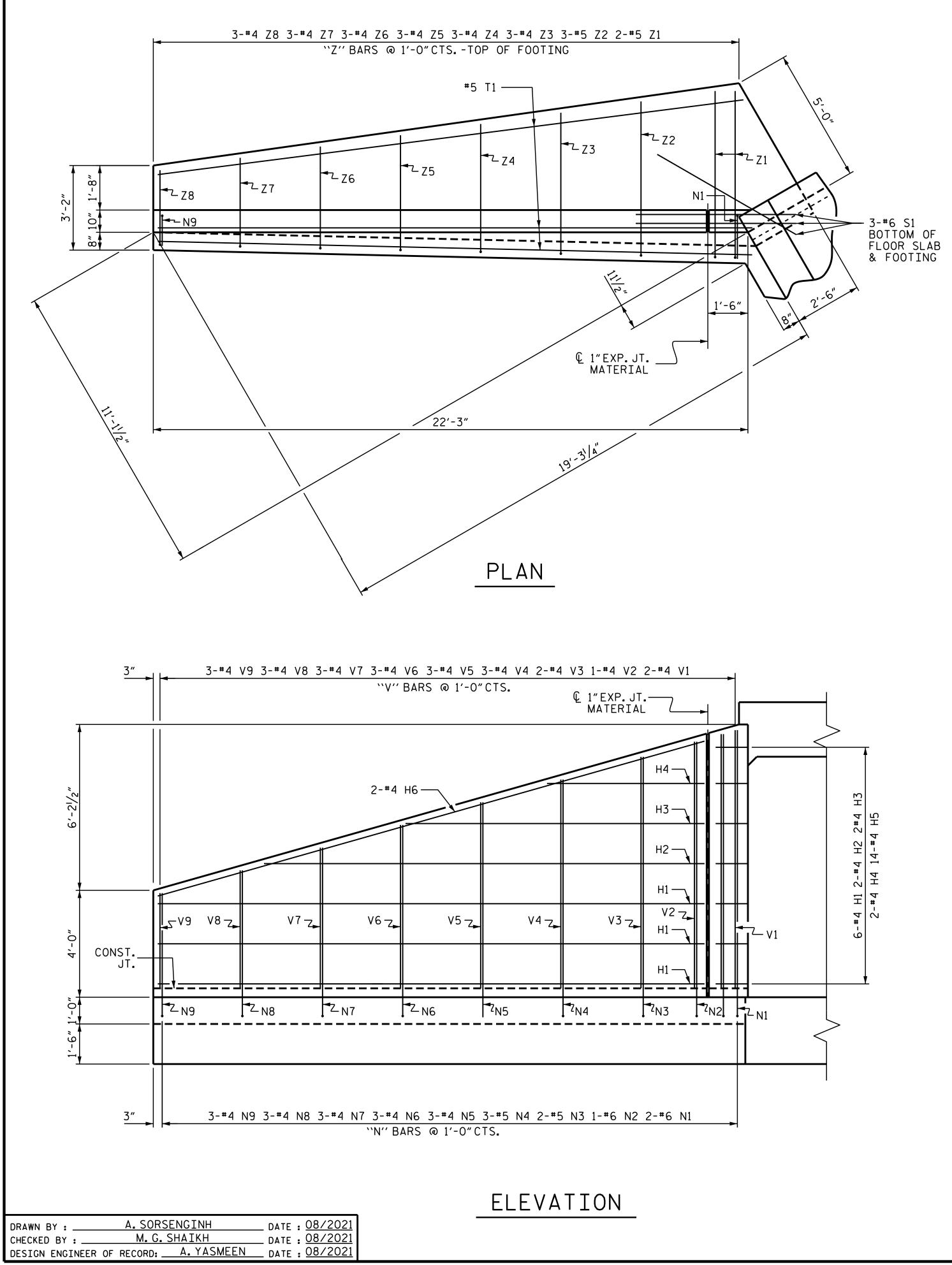
C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A $\frac{7}{16}$ % WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

—.375″Ø WIRE STRUT

└── NO.6 GAGE WIRE

| | | | T NO MOORE DN:23 [.] | | CO | UNTY |
|----------------|---|---------|---|---------------------------------------|----------------------------------|----------------------|
| NO.6 GAGE WIRE | DocuSigned by: Krishna P. Sedai EAGE73100 EFBTC AROL OF ESSION SE AL 031583 9/30/2021 | AN | STATE OF RTMENT OF STAN STAN CHORAGE DRAIL AN FOR C | TRAN ALEIGH NDAR DET ICHO | NSPORTA D AILS F R ASSE | OR |
| | | | REVISION | S | | SHEET NO. |
| VERTS [| DOCUMENT NOT CONSIDERED | NO. BY: | DATE: NO. | BY: | DATE: | C-7 |
| | FINAL UNLESS ALL SIGNATURES COMPLETED | 1 2 | 3 4 | | | TOTAL SHEETS 8 |
| | | 6 | | | | 0 |

SID. NO. GRAI

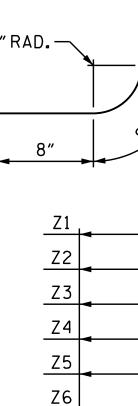


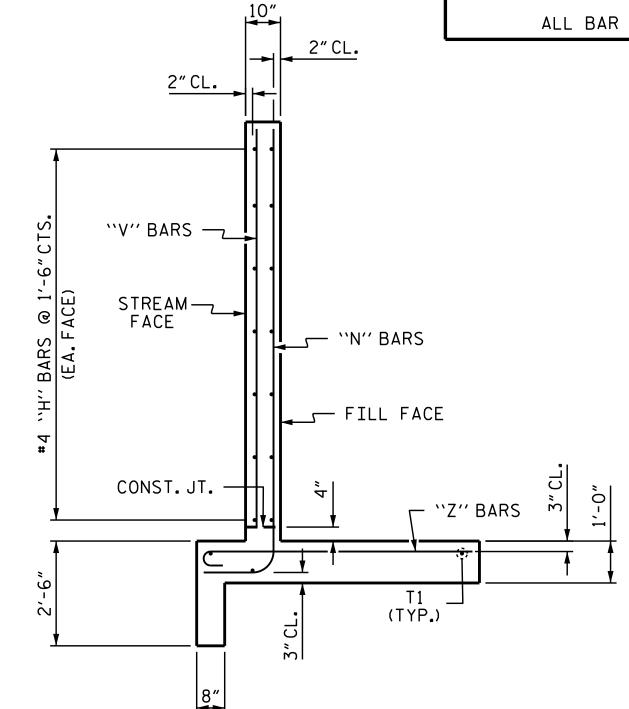
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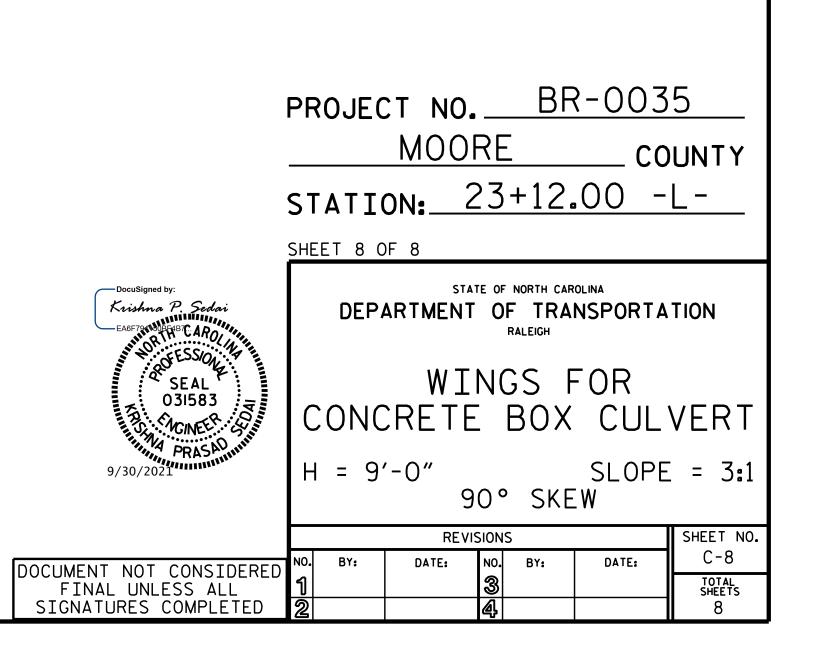
| CLA | SS A CONCRETE | | |
|----------|--|------------------------------------|---------|
| | PHASE I | | |
| | WING - PHASE I END CURTAIN WALL TOTAL | 0.8 C.Y. | |
| STAGE I | PHASE II | | |
| | WING - PHASE II 1 HEADWALL END CURTAIN WALL TOTAL | 1.8 C.Y. | |
| | PHASE II | Ι | |
| STAGE II | WING - PHASE III END CURTAIN WALL TOTAL | 10.1 C.Y. 0.8 C.Y. 10.9 C.Y. | 2 |
| | PHASE IV | | |
| | WING - PHASE IV 1 HEADWALL END CURTAIN WALL TOTAL | 1.8 C.Y. | 6″RAD.— |
| | | | |





TYPICAL WING SECTION

| $\begin{array}{c c c c c c c c c c c c c c c c c c c $ | BAR TYPES | | | BIL | | | TERIAL | |
|---|---|---|----|----------|------------|-----|----------|---------|
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| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | | | | | - | _ | | |
| $\begin{array}{c c c c c c c c c c c c c c c c c c c $ | | | H6 | 8 | #4 | STR | 21'-3" | 114 |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | <u>1'-3"</u> <u>1'-8¾"</u> | | N1 | 8 | #6 | 2 | 11'-6" | 138 |
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| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 | N 8 N 9 N 9 N 9 N 9 N 9 N 9 N 9 N 9 N 9 | | | | | | |
| $ \begin{array}{c} 85 \\ 6' \\ 6' \\ 6' \\ 6' \\ 7' \\ 6' \\ 6' \\ 7' \\ 6' \\ 7' \\ 6' \\ 7' \\ 7$ | | | | | | | | |
| $\frac{1}{10} + \frac{1}{10} $ | | | | | | | | |
| $\frac{1}{10} + \frac{1}{10} $ | | | | | - | | | |
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| S1 12 #6 STR 6'-0" 108 T1 12 #5 STR 22'-1" 276 T1 12 #5 STR 22'-1" 276 V1 8 #4 STR 9'-6" 51 V2 4 #4 STR 9'-2" 24 V3 8 #4 STR 8'-8" 46 V4 12 #4 STR 6'-1" 63 5'-9" 7" 7" 5' 12 #4 STR 6'-1" 5'-9" 7" 6" 6" 71 12 #4 STR 6'-1" 49 V7 12 #4 STR 5'-5" 42 V8 12 #4 STR 5'-6" 42 V8 12 #4 STR 5'-6" 44 25 12 #4 3 5'-6" 44 25 12 #4 3 5'-6" 44 25 12 #4 3 3'-10" 35' | | 1/2" | | | - | | | |
| S1 12 #6 STR 6'-0" 108 T1 12 #5 STR 22'-1" 276 T1 12 #5 STR 22'-1" 276 V1 8 #4 STR 9'-6" 51 V2 4 #4 STR 9'-2" 24 V3 8 #4 STR 8'-8" 46 V4 12 #4 STR 6'-1" 63 5'-9" 7" 7" 5' 12 #4 STR 6'-1" 5'-9" 7" 6" 6" 71 12 #4 STR 6'-1" 49 V7 12 #4 STR 5'-5" 42 V8 12 #4 STR 5'-6" 42 V8 12 #4 STR 5'-6" 44 25 12 #4 3 5'-6" 44 25 12 #4 3 5'-6" 44 25 12 #4 3 3'-10" 35' | | | | | | | | |
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| $ \begin{array}{c ccccccccccccccccccccccccccccccccccc$ | | | S1 | 12 | *6 | STR | 6'-0" | 108 |
| $ \begin{array}{c} 8 \\ 9 \\ 1 \\ \hline \\ 8 \\ \hline $ | | | Τ1 | 12 | * 5 | STR | 22'-1" | 276 |
| $ \begin{array}{c} 8^{1}1 \\ \hline \\ 8^{1}1 \\ \hline \\ 6'-3'' \\ \hline \\ 6'-3'' \\ \hline \\ 6'-3'' \\ \hline \\ 5'-9'' \\ \hline \\ 5'-9'' \\ \hline \\ 5'-3'' \\ \hline \\ 6'' \\ \hline \\ 6'' \\ \hline \\ 4'-4'' \\ \hline \\ 6'' \\ \hline \\ 3'-4'' \\ \hline \\ 6'' \\ \hline \\ 3'-4'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 7' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 10'' \\ \hline \\ 6'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 4'' \\ 7' \\ 7' \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 25 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 25 \\ 2586 \\ LBS \\ \end{array}$ | | | | | | 670 | 01.64 | |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | | | | | | | | |
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| 6'-3'' $7''$ $5'-9''$ $7''$ $5'-9''$ $7''$ $5'-3''$ $6''$ $4'-10''$ $6''$ $4'-4''$ $6''$ $4'-4''$ $6''$ $3'-10''$ $6''$ $2'-10''$ $6''$ 3 $HK.$ 3 $HK.$ | | | | | | | | |
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| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | 5'-9" | 7″ | | | - | | | |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | 5'-3" | 6" | | | - | | | |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | • | | VY | 12 | #4 | SIR | 5'-6" | 28 |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | 4'-10" | 6″ | 71 | R | #5 | ٦ | 6'-10" | 57 |
| $\begin{array}{c ccccccccccccccccccccccccccccccccccc$ | 4'-4" | 6″ | | | _ | | | |
| 3'-4" 6" 3'-4" 6" 2'-10" 6" 4" 3 5 12 12 12 12 14 13 12 14 13 15 12 16 12 17 12 18 12 19 12 10 12 10 12 12 13 | 7/ 10// | | | | _ | | | |
| 3'-4" 6" 2'-10" 6" 2'-10" 6" 4" 3 4'-4" 35 27 12 4 3 4'-4" 35 27 12 4 3 4'-4" 3 4'-4" 35 3'-10" 31 28 12 4 3 3'-4" 27 HK. HK. REINFORCING STEEL 2586 LBS | 510 | b | | | | | | |
| 2'-10" 6" Z6 12 #4 3 4'-4" 35 Z7 12 #4 3 3'-10" 31 Z8 12 #4 3 3'-4" 27 HK. REINFORCING STEEL FOR 4 WINGS 2586 LBS | 3'-4" | 6″ | | | | | | |
| Z7 12 #4 3 3'-10" 31 Z8 12 #4 3 3'-4" 27 HK. REINFORCING STEEL FOR 4 WINGS 2586 LBS | 2'-10" | 6" | | | | | | |
| 3 Z8 12 #4 3 3'-4" 27 REINFORCING STEEL FOR 4 WINGS 2586 LBS | | | | | | | | |
| 3 HK. REINFORCING STEEL 50R 4 WINGS 2586 LBS | | | | | | | | |
| | | | | | | ĒL | 25 | 86 LBS |



DESIGN DATA:

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| SPECIFICATIONS | A.A.S.H.T.O. (CURRENT) |
|---|--------------------------------|
| LIVE LOAD | SEE PLANS |
| IMPACT ALLOWANCE | SEE A.A.S.H.T.O. |
| STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 | 20,000 LBS.PER SQ.IN. |
| - AASHTO M270 GRADE 50W | 27,000 LBS.PER SQ.IN. |
| - AASHTO M270 GRADE 50 | 27,000 LBS.PER SQ.IN. |
| REINFORCING STEEL IN TENSION - GRADE 60 | 24,000 LBS.PER SQ.IN. |
| CONCRETE IN COMPRESSION | 1,200 LBS.PER SQ.IN. |
| CONCRETE IN SHEAR | SEE A.A.S.H.T.O. |
| STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS | 1,800 LBS.PER SQ.IN. |
| COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER | 375 LBS.PER SQ.IN. |
| EQUIVALENT FLUID PRESSURE OF EARTH | 30 LBS.PER CU.FT. (MINIMUM) |

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾″Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY VIGINCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

