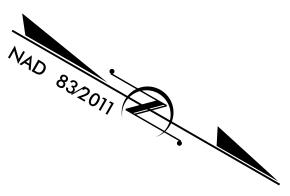
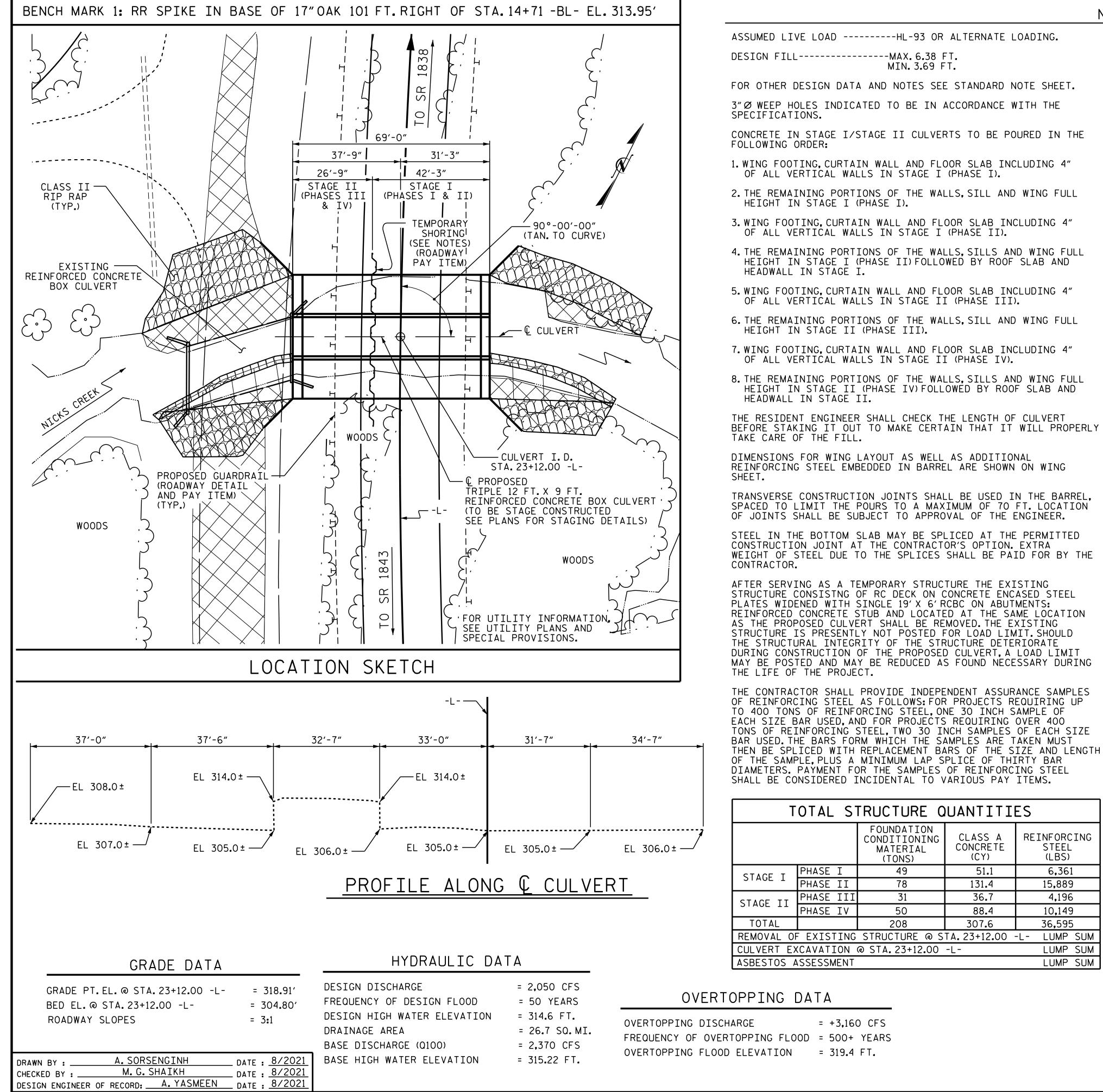


			<b>PROJECT LENGTH</b>
2018	0.319 MI	=	LENGTH OF ROADWAY TIP PROJECT BR-0035
	0.007 MI	=	LENGTH OF STRUCTURE TIP PROJECT BR-0035
LETTING	0.326 MI	=	TOTAL LENGTH OF TIP PROJECT BR-0035

STATE	STATI	SHEET NO.	TOTAL SHEETS	
N.C.	B	1		
STAT	e proj. No.	DESCRIP	TION	
49	073.1.1	_	P.E	•
49	073.2.1	ROW/L	JTIL	
49	073.3.1	0022015	CONS	ST.



Prepared in th DIVISION OF STRUCTURES MAN 1000 BIRCH RALEIGH,	<b>HIGHWAYS</b> NAGEMENT UNIT RIDGE DR.
8 STANDARD SPECIFICATIONS	
DATE : NOVEMBER 16, 2021	W. KEVIN FISCHER, P.E. PROJECT ENGINEER KRISHNA P. SEDAI, P.E. PROJECT DESIGN ENGINEER



30-SEP-2021 07:58 K:\Structures\FinalPlans\411\_003\_BR-0035\_SMU\_CU\_001\_620024.dgn

+

+

TOTAL STRUCTURE QUANTITIES							
		FOUNDATION CONDITIONING MATERIAL (TONS)	CLASS A CONCRETE (CY)	REINFORCING STEEL (LBS)			
STAGE I	PHASE I	49	51.1	6,361			
PHASE II		78 131.4		15,889			
STAGE II PHASE III		31	36.7	4,196			
STAGE II	PHASE IV	50	88.4	10,149			
TOTAL		208	307.6	36,595			
REMOVAL OF	F EXISTING	STRUCTURE @ S	TA.23+12.00 ·	-L- LUMP SUM			
CULVERT EX	CAVATION	@ STA.23+12.00	-L-	LUMP SUM			
ASBESTOS A	SSESSMENT			LUMP SUM			

OVERTOPPING DISCHARGE	= +3,160 CFS
FREQUENCY OF OVERTOPPING FLOOD	= 500+ YEARS
OVERTOPPING FLOOD ELEVATION	= 319.4 FT.
	FREQUENCY OF OVERTOPPING FLOOD

NOTES: AT THE CONT REINFORCING AND BOTH FAC

CONSTRUCTION IN THE SPLIC OF STEEL DUE CONTRACTOR

THE REINFORG THE STANDARD MATERIAL, SE

FOR LIMITS SEE TRAFFIC SHORING FOR

FOR CONSTRUC

FOR TEMPORA SHORING, SEE

DESIGN TEMP LT. TO STATI ASSUMED SOI UNIT UNIT FRIC COHE GROU

BEFORE BEGIN SURVEY EXIS SHORING LOC

LIMITED SUBS OF TEMPORARY STATION 24+0 TEMPORARY SH APPLICABLE CONSTRUCTION

AT THE CONT FOR TEMPORA TO STATION DETAIL 1801.C

DO NOT USE TEMPORARY S STATION 24+0

A 3 FOOT ST FILL FACE OF EXPANSION JC

FOR CULVERT CONTROL PLAN

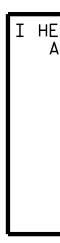
NO PRECAST

FOR SUBMITT FOR FALSEWOF

FOR CRANE SA

FOR GROUT FO

FOR ASBESTOS RENOVATION

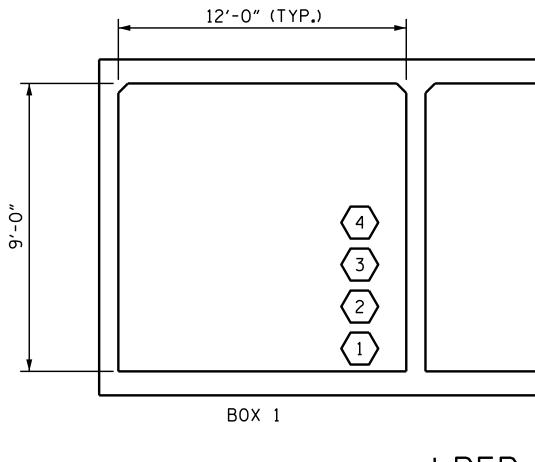




RACTOR'S OPTION, HE MAY SPLICE T G STEEL IN THE INTERIOR FACES OF ACES OF INTERIOR WALLS ABOVE LOW ON JOINT. THE SPLICE LENGTH SHALL ICE LENGTH CHART SHOWN ON THE PL WE TO THE SPLICES SHALL BE PAID	F EXTERIOR WALL WER WALL L BE AS PROVIDED LANS.EXTRA WEIGHT
RCED CONCRETE BOX CULVERT SHALL RD 1.0 FOOT BLANKET OF FOUNDATIO EE SECTION 414 OF THE STANDARD S	ON CONDITIONING
OF TEMPORARY SHORING FOR MAINT CONTROL PLANS.FOR PAY ITEM FOR MAINTENANCE OF TRAFFIC, SEE ROA	DR TEMPORARY
JCTION SEQUENCE, SEE EROSION CON	
ARY SHORING AND POSITIVE PROTEC E PLANS AND TEMPORARY SHORING PI	
PORARY SHORING FROM STATION 18+ ION 24+00± -L- 15 FT.LT,FOR THE IL PARAMETERS AND GROUNDWATER E T WEIGHT OF SOIL ABOVE WATER TA T WEIGHT OF SOIL BELOW WATER TA CTION ANGLE, Ø = 30° ESION,c = 0 PSF UNDWATER ELEVATION = 312 FT.	FOLLOWING ELEVATION: ABLE, y=120 PCF
NNING TEMPORARY SHORING DESIGN STING GROUND ELEVATIONS IN THE CATIONS TO DETERMINE ACTUAL SHO	VICINITY OF
BSURFACE INFORMATION IS AVAILAB RY SHORING FROM STATION 18+00± +00± -L-,15 FT.LT.THE INFORMATIC SHORING DESIGN WAS ASSUMED AND TO THE ACTUAL SITE CONDITIONS E ON.	-L-,37 FT.LT.TO ON PROVIDED FOR MAY NOT BE
TRACTOR'S OPTION, USE A STANDARD ARY SHORING FROM STATION 18+00± 24+00± -L-,15 FT.LT.SEE GEOTECH .02 FOR STANDARD TEMPORARY WALLS	± -L-,37 FT.LT. HNICAL STANDARD
CANTILEVER, BRACED, OR ANCHORED S SHORING FROM STATION 18+00± -L-, +00± -L-,15 FT.LT.	-, 37 FT.LT.TO
TRIP OF FILTER FABRIC SHALL BE A F THE WING COVERING THE ENTIRE JOINT.	
DIVERSION DETAILS AND PAY ITE	EM, SEE EROSION #3 6'-2" #4 7'-4"
REINFORCED BOX CULVERT OPTION	#5 8'-6" WILL BE ALLOWED. #6 9'-8"
TAL OF WORKING DRAWINGS, SEE SPE	ECIAL PROVISIONS. #7 10'-10"
ORK AND FORMWORK, SEE SPECIAL PRO	#8 <u>12'-0"</u> #9 <u>13'-2"</u>
SAFETY, SEE SPECIAL PROVISIONS.	<b>#10</b> 14'-6" <b>#11</b> 15'-10"
FOR STRUCTURES, SEE SPECIAL PROVE	
DS ASSESSMENT FOR BRIDGE DEMOLI ACTIVITIES,SEE SPECIAL PROVISI	
HEREBY CERTIFY THESE PLANS	
ARE THE AS-BUILT PLANS	PROJECT NO BR-0035
	MOORE COUNTY
	STATION: 23+12.00 -L-
	SHEET 1 OF 8 REPLACES BRIDGE NO. 24
DocuSigned by: DocuSigned by: Krishna P. Sedai	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
WESSION	
SEAL 30878 SEAL 031583	TRIPLE 12 FT.X 9 FT. CONCRETE BOX CULVERT
ACINER STATES	90° SKEW
/30/2021 9/30/2021	
DOCUMENT NOT CONSIDERED	REVISIONS   SHEET NO.     NO.   BY:   DATE:   NO.
FINAL UNLESS ALL SIGNATURES COMPLETED	1   3   TOTAL SHEETS     2   4   8

F.A. PROJECT NO.: 0022015

										OR RATIN CRETE BO>			TS			
										STRENGTH	I LIM	IT ST	ATE			
										MOMENT				SHEAR		1
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f†)	
		HL-93 (INVENTORY)	N/A	$\langle 1 \rangle$	1.37		1.75	1.42	1	TOP SLAB	5.70	1.37	1	BOTTOM SLAB	11.67	Γ
DESIGN		HL-93 (OPERATING)	N/A		1.78		1.35	1.84	1	TOP SLAB	5.70	1.78	1	BOTTOM SLAB	11.67	
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.37	49.35	1.75	1.93	1	TOP SLAB	5.38	1.37	1	BOTTOM SLAB	11.67	
		HS-20 (OPERATING)	36.000		1.78	63.98	1.35	2.50	1	TOP SLAB	5.38	1.78	1	BOTTOM SLAB	11.67	
		SNSH	13.500		3.33	44.91	1.40	3.51	1	TOP SLAB	5.38	3.33	1	TOP SLAB	11.56	
		SNGARBS2	20.000		3.02	60.46	1.40	3.29	1	TOP SLAB	5.38	3.02	1	TOP SLAB	11.56	
	ICLE	SNAGRIS2	22.000		2.79	61.28	1.40	3.19	1	BOT.CORNER WALL	9.73	2.79	1	BOTTOM SLAB	11.67	
	VEHICLI (V)	SNCOTTS3	27.250		1.78	48.57	1.40	1.78	1	TOP SLAB	5.38	1.93	1	TOP SLAB	11.56	
	ы S	SNAGGRS4	34.930		1.77	61.82	1.40	2.10	1	TOP SLAB	5.38	1.77	1	BOTTOM SLAB	11.67	
	INGL	SNS5A	35.550		1.75	62.27	1.40	1.97	1	TOP SLAB	5.70	1.75	1	BOTTOM SLAB	11.67	
	S	SNS6A	39.950		1.56	62.41	1.40	1.96	1	TOP SLAB	5.70	1.56	1	BOTTOM SLAB	11.67	
LEGAL		SNS7B	42.000		1.48	62.14	1.40	2.03	1	TOP SLAB	5.38	1.48	1	BOTTOM SLAB	11.67	
LOAD RATING	ER	TNAGRIT3	33.000		1.87	61.85	1.40	2.70	1	BOT.CORNER WALL	9.73	1.87	1	BOTTOM SLAB	11.67	
	TRAIL	TNT4A	33.080		1.88	62.06	1.40	2.12	1	TOP SLAB	5.38	1.88	1	BOTTOM SLAB	11.67	
		TNT6A	41.600		1.49	62.03	1.40	2.13	1	TOP SLAB	5.38	1.49	1	BOTTOM SLAB	11.67	
	SEMI- ST)	TNT7A	42.000		1.52	63.69	1.40	2.22	2	BOTTOM SLAB	0.95	1.52	1	BOTTOM SLAB	11.67	
	TOR (TT	TNT7B	42.000		1.52	63.95	1.40	2.02	1	TOP SLAB	5.38	1.52	1	BOTTOM SLAB	11.67	
	TRAC	TNAGRIT4	43.000		1.45	62.48	1.40	2.03	1	TOP SLAB	5.38	1.45	1	BOTTOM SLAB	11.67	
	TRUCK	TNAGT5A	45.000		1.39	62.65	1.40	2.03	2	BOTTOM SLAB	11.72	1.39	1	BOTTOM SLAB	11.67	
	TRL	TNAGT5B	45.000	$\langle 3 \rangle$	1.39	62.64	1.40	2.04	1	BOTTOM SLAB	11.72	1.39	1	BOTTOM SLAB	11.67	
EMERGENC		EV2	28.750		2.30	66.15	1.30	2.48	1	TOP SLAB	5.38	2.30	1	BOTTOM SLAB	11.67	Γ
/EHICLE		EV3	43.000	$\langle 4 \rangle$	1.54	66.21	1.30	1.54	1	TOP SLAB	5.70	1.54	1	BOTTOM SLAB	11.67	ſ

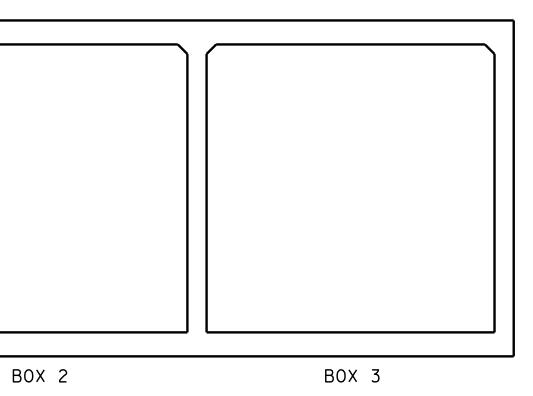


DRAWN BY :	A. SORSENGINH	DATE : 08/2021
CHECKED BY :	M.G.SHAIKH	DATE : 08/2021
DESIGN ENGINEER	OF RECORD: A. YASMEEN	DATE : 08/2021

30-SEP-2021 07:58 K:\Structures\FinalPlans\411\_005\_BR-0035\_SMU\_CU\_002\_620024.dgn ksedai

+

+



LRFR SUMMARY

(LOOKING DOWNSTREAM)

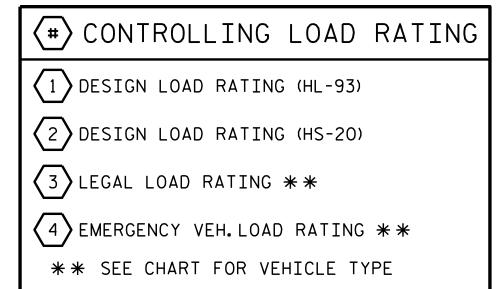
# LOAD FACTORS:

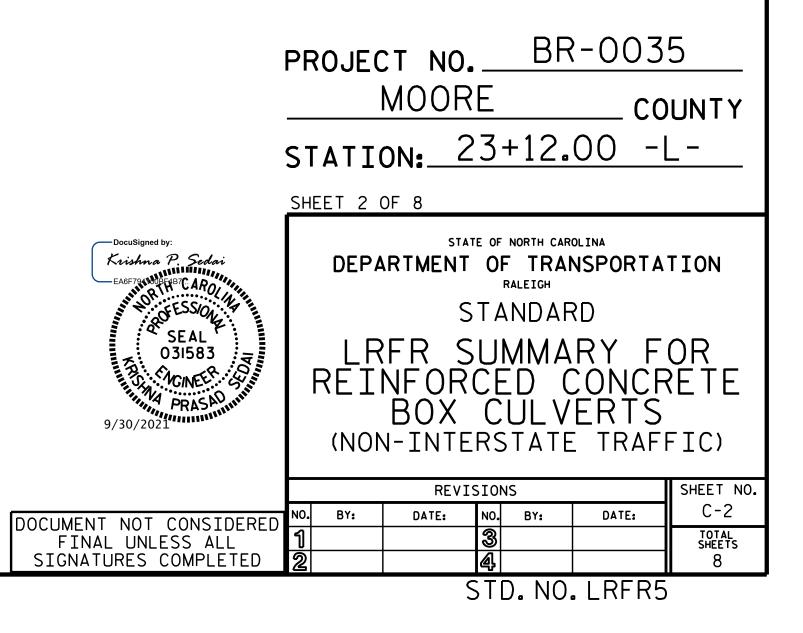
DESIGN LUAD RATING FACTORS									
LOAD TYPE	MAX FACTOR	MIN FACTOR							
DC	1.25	0.90							
DW	1.50	0.65							
EV	1.30	0.90							
EH	1.35	0.90							
ES	1.35	0.90							
LS	1.75								
WA	1.00								

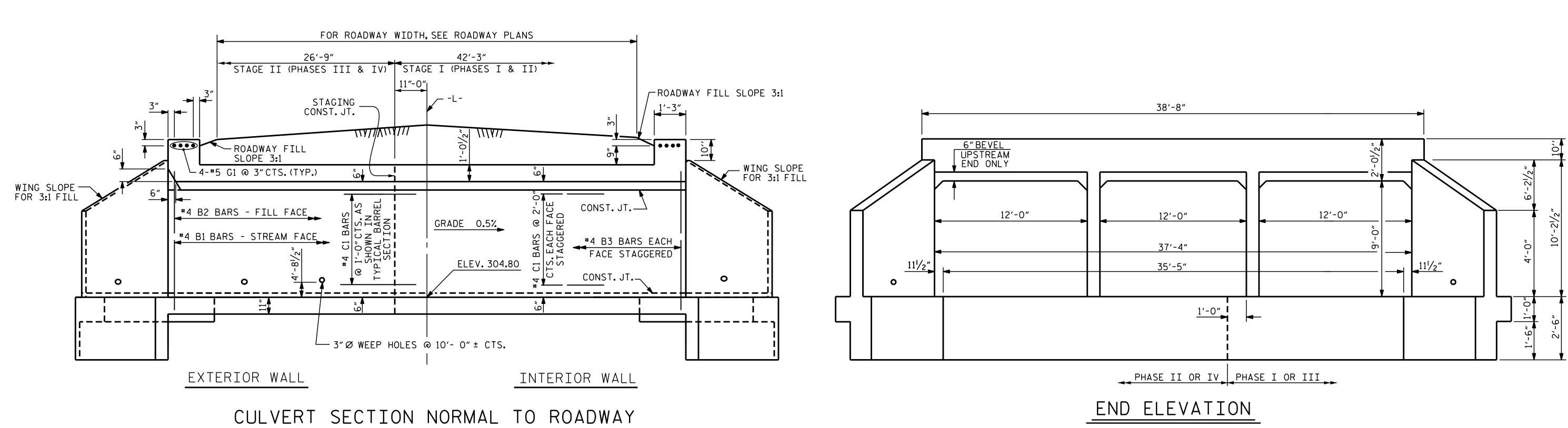
DESTGN LOAD RATING FACTORS

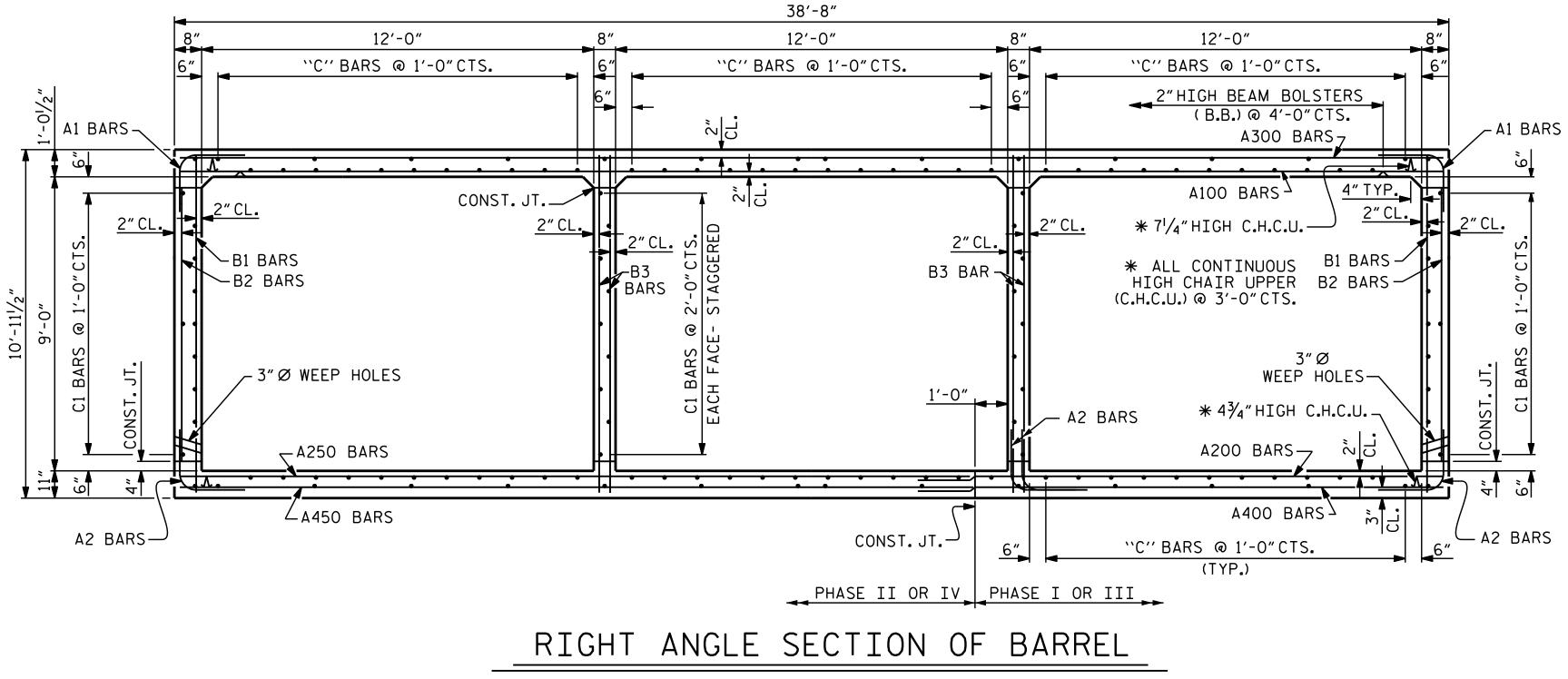
NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.









0661		
<b>.</b>		
S		
AWN	DRAWN BY :A. SORSENGINH	DATE : <u>8/2021</u>
REDR/	CHECKED BY : M.G. SHAIKH	DATE : <u>8/2021</u>
Ж	DESIGN ENGINEER OF RECORD:A.YASMEEN	DATE : <u>8/2021</u>

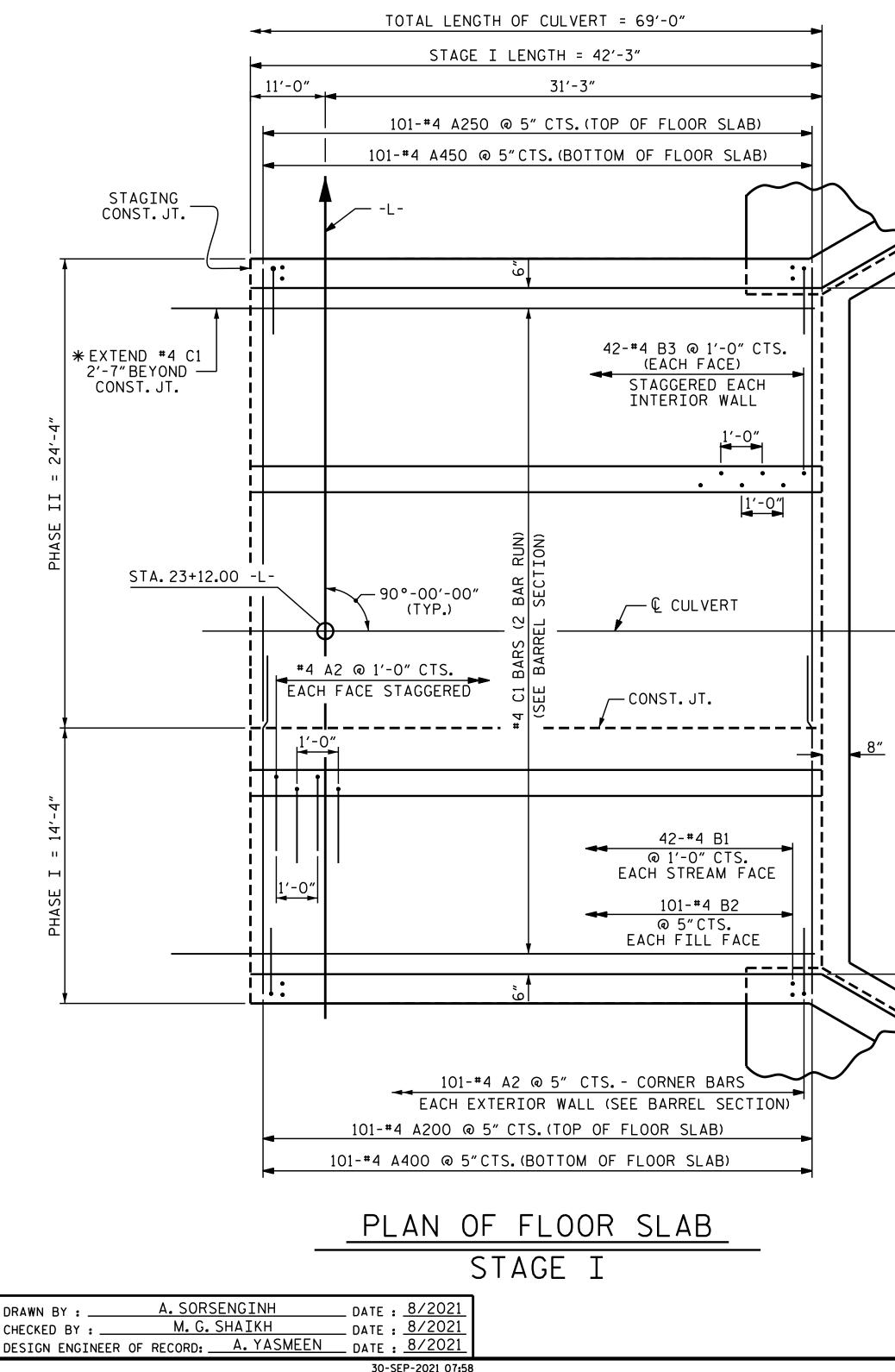
+

+

THERE ARE 136 "C" BARS IN SECTION OF BARREL.

	PROJECT N MO STATION: SHEET 3 OF 8	ORE		5 unty L-
DocuSigned by: Krishma P. Sedai EAGF791190EFFB7CAR0/ WWW PROFESSION SEAL 031583 PRASAD WWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWWW		STATE OF NORTH CARC NT OF TRAN RALEIGH 12 FT E BOX 0 ° SK	ISPORTA	
	R	VISIONS		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY: DATE:	NO. BY:	DATE:	C-3
FINAL UNLESS ALL SIGNATURES COMPLETED	1 2	3 4		TOTAL SHEETS 8

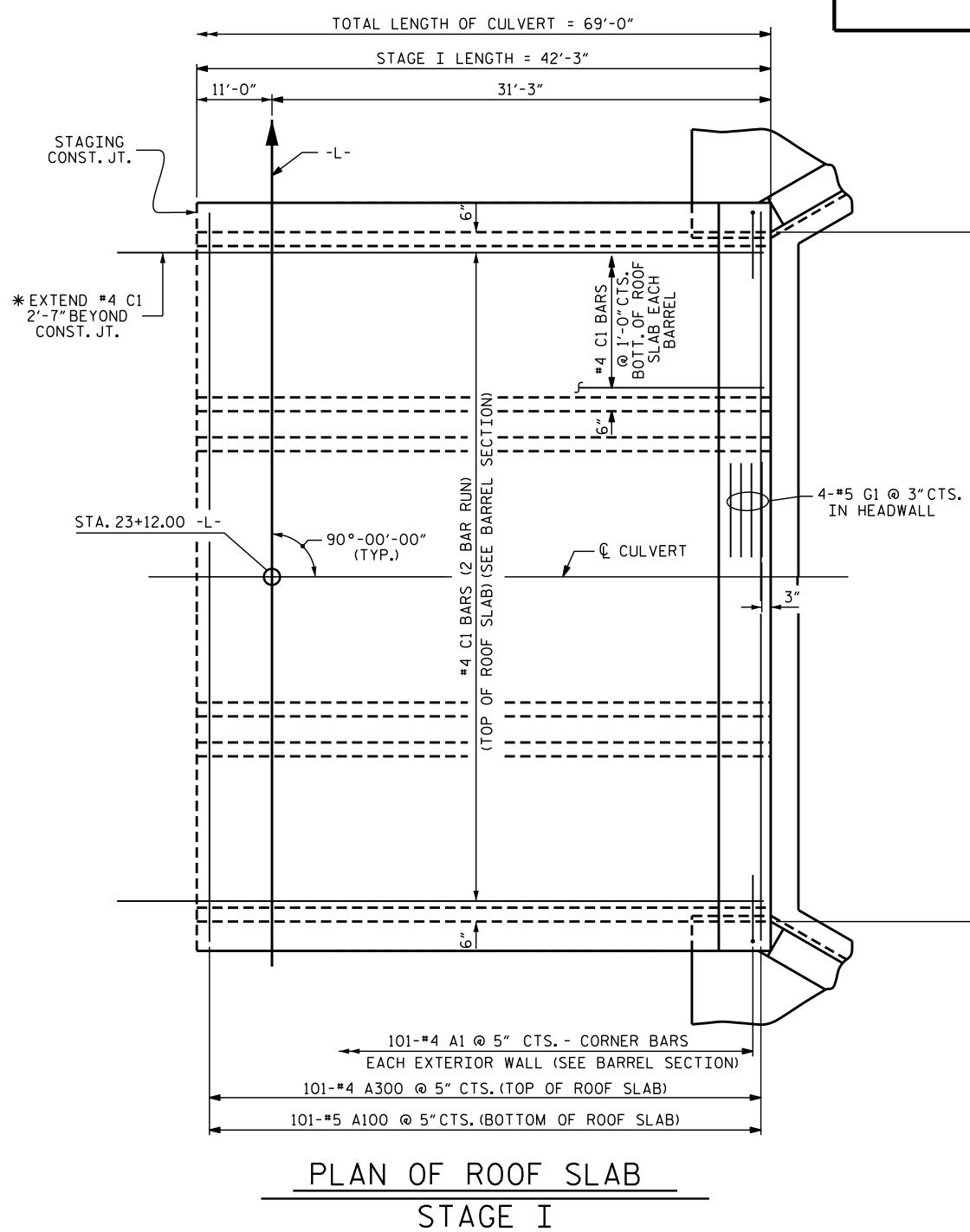
+

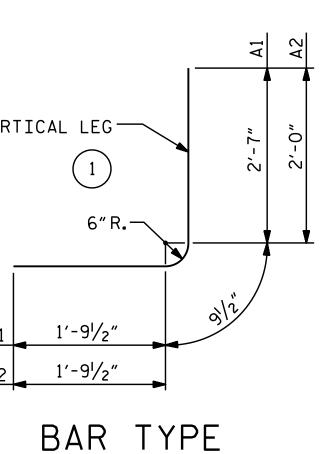


30-SEP-2021 07:58 K:\Structures\FinalPlans\411\_009\_BR-0035\_SMU\_CU\_004\_620024.dgn ksedai

+

		STAC	GE I			
PHASE	I		PHASE	II		
CLASS A CONCRETE BARREL @O.931CY/FT WING,ETC SILLS TOTAL	10.9 0.9	C.Y.	CLASS A CONCRETE BARREL @ <u>2.775</u> CY/FT WING,ETC SILLS TOTAL	13.3 0.9	C.Y. C.Y.	VERT
REINFORCING STEEL BARREL WING, ETC SILLS TOTAL	647 15	LBS. LBS. LBS. LBS.	REINFORCING STEEL BARREL WING,ETC SILLS TOTAL	18	LBS. LBS. LBS. LBS.	A1 A2





BAR DIMENSIONS ARE OUT TO OUT

ERI(

	017				
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	101	#4	1	5'-2″	349
Α2	184	#4	1	4'-7"	563
A200	101	#4	STR.	16'-2"	1091
A400	101	#4	STR.	16'-2"	1091
B1	42	#4	STR.	10'-5"	292
B2	101	#4	STR.	8'-4"	562
B3	84	#4	STR.	10'-5"	585
C1	74	#4	STR.	23'-7"	1166
	REINFOR	RCING	STEEL	= 5,699	B LBS
	STA	GE I	PH	ASE II	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	101	#4	1	5′-2″	349
Α2	101	#4	1	4'-7"	309
A100	101	<b>#</b> 5	STR.	38'-3"	4029
A250	101	#4	STR.	24'-0"	1619
A300	101	#4	STR.	38'-3″	2581
A450	101	#4	STR.	24'-0"	1619
B1	42	#4	STR.	10'-5"	292
B2	101	#4	STR.	8'-4"	562
B3	84	#4	STR.	10'-5"	585
C1	198	#4	STR.	23'-7"	3119
G1	4	<b>#</b> 5	STR.	38'-4"	160
	REINFO	RCING	STEEL	= 15,2	24 LBS
# <i>A</i> C1 C		ר רדרי			

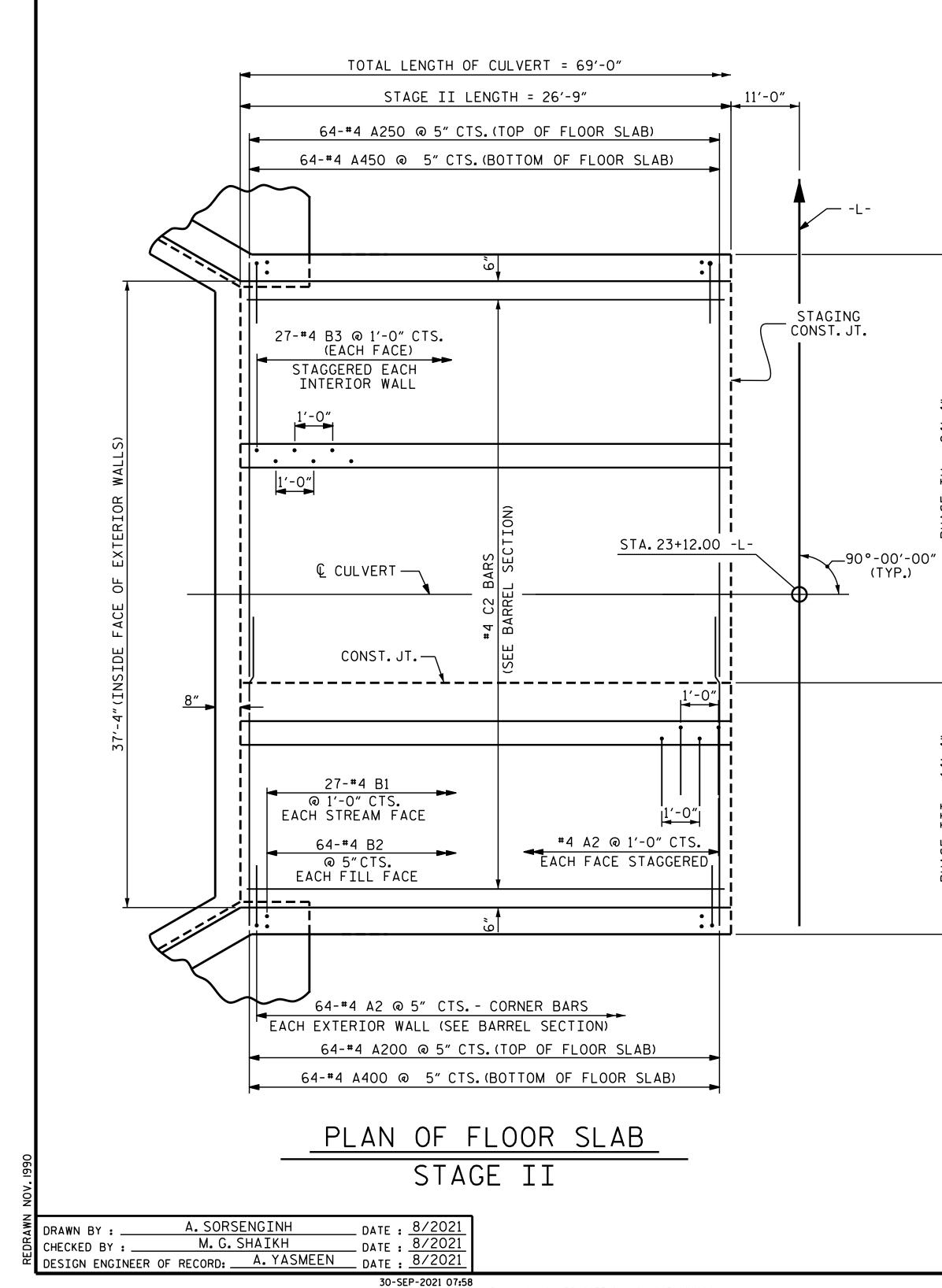
BILL OF MATERIAL

STAGE I PHASE I

\* #4 C1 SHALL BE FIELD BENT AS NECESSARY.

SPLICE	LENGTH	IS CHART
BAR	SIZE	SPLICE LENGTH
A100	#5	2'-4"
A200	#4	1'-10"
A250	#4	1'-10"
A300	#4	1'-10"
A400	#4	1'-10"
A450	#4	1'-10"
B1	#4	1'-10"
B3	#4	1'-10"
C1	#4	2'-5″
G2	#5	3'-0"

	PROJEC	<u>100М</u> 2зис	RE			35 OUNTY -L-
DocuSigned by: Krishna P. Sodai EAGF79000ERHBCAR0/ SEAL 031583 PRASAD	TRI	RTMENT	RALE 2 5 7 A (	FT OX	ISPORTA	
		REVIS	SIONS			SHEET NO.
OCUMENT NOT CONSIDERED	NO. BY:	DATE:		3Y:	DATE:	
FINAL UNLESS ALL SIGNATURES COMPLETED	1 2		3 4			TOTAL SHEETS 8

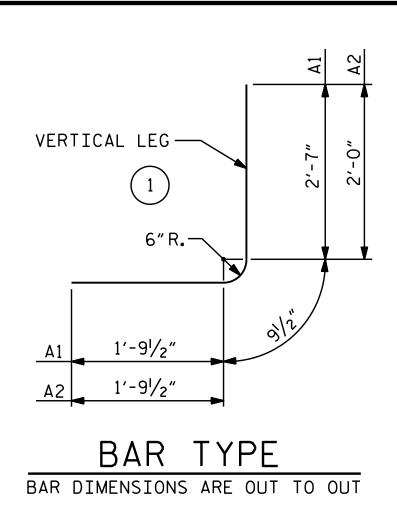


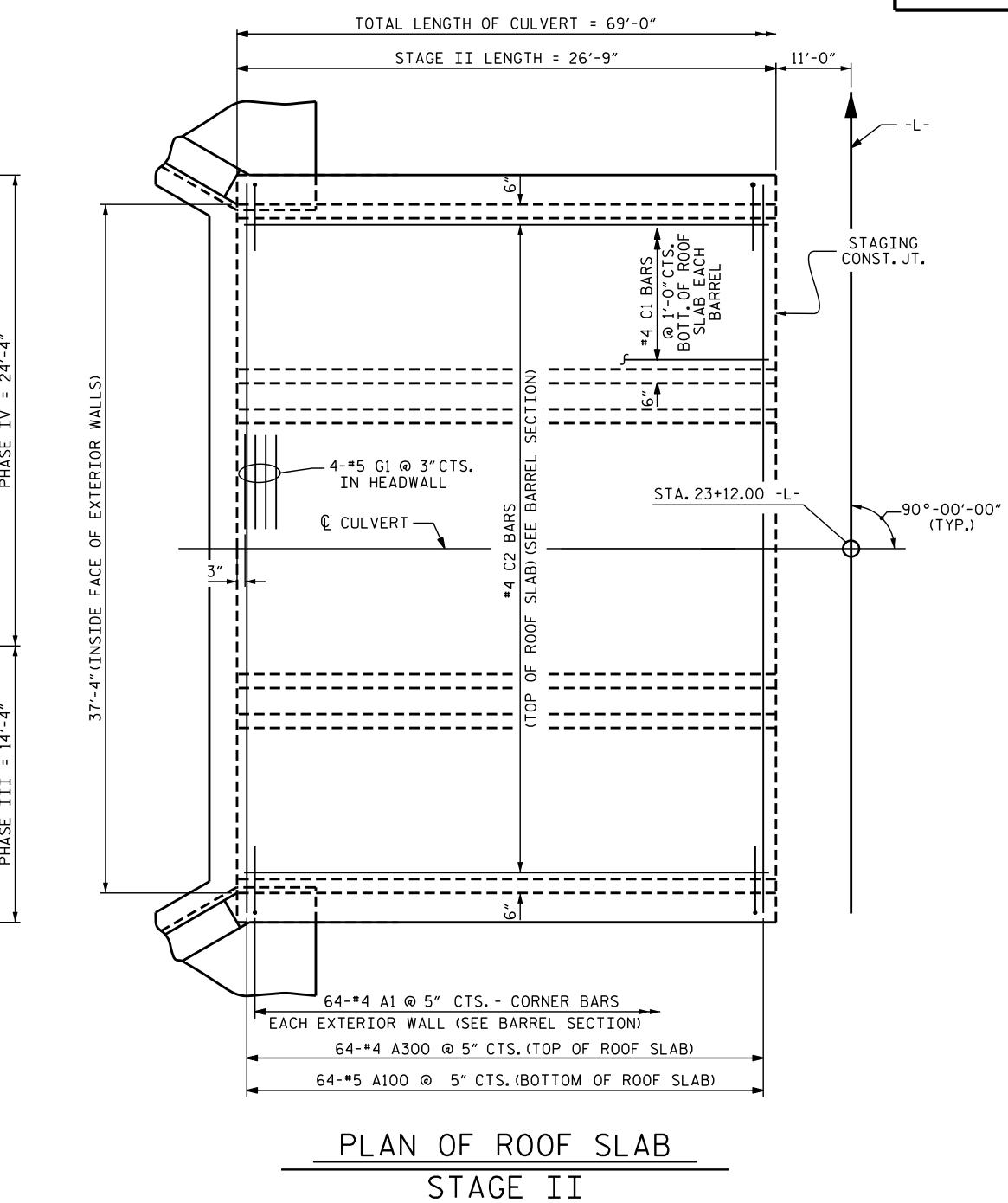
30-SEP-2021 07:58 K:\Structures\FinalPlans\411\_011\_BR-0035\_SMU\_CU\_005\_620024.dgn ksedai

+

+

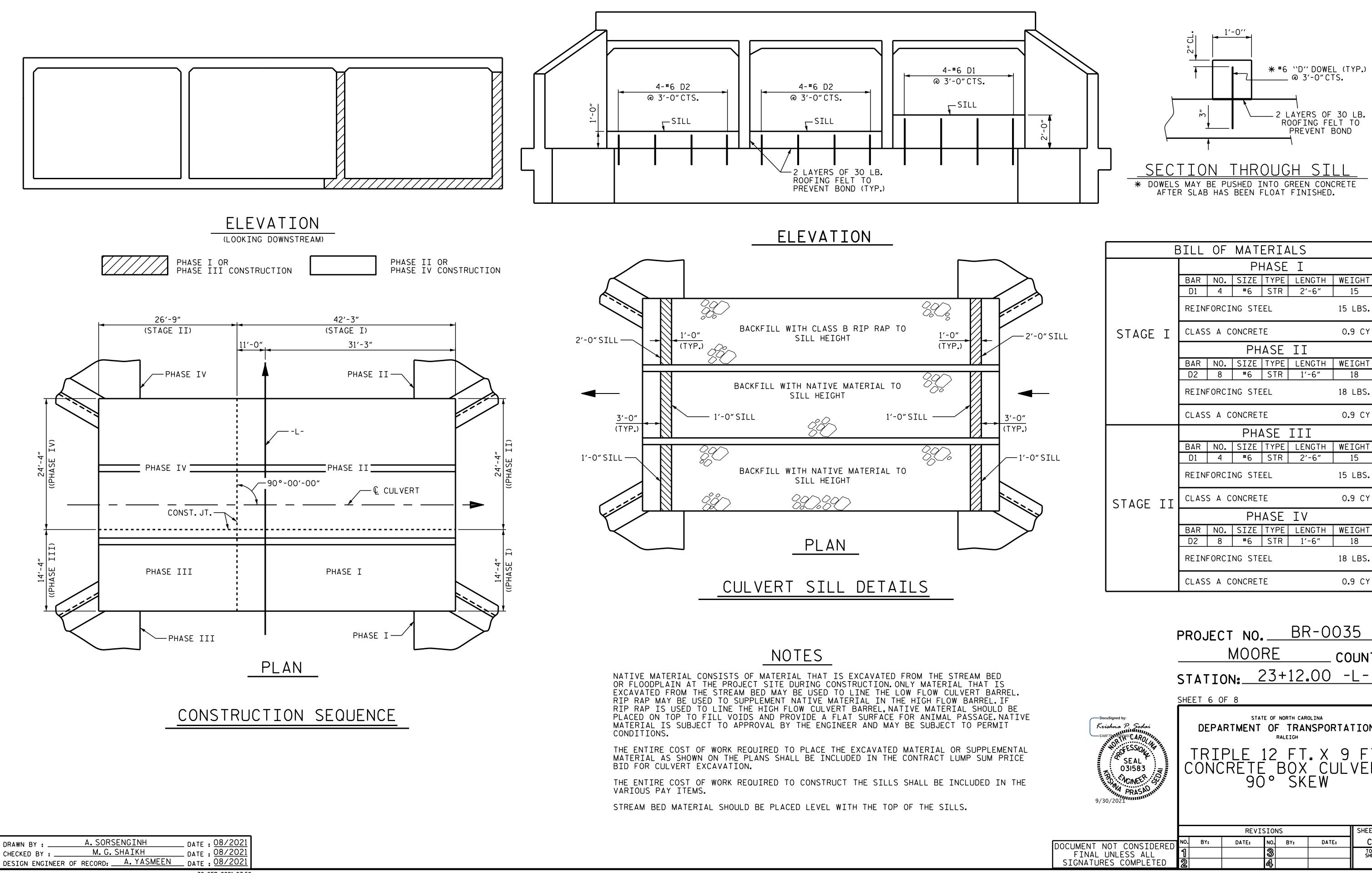
STAGE II								
PH	ASE III		PH	ASE IV				
CLASS A CONCRETE BARREL @ 0.931 WING,ETC SILLS TOTAL	10.9 0.9	C.Y. C.Y.	CLASS A CONCRETE BARREL @ 2.775 WING,ETC. SILLS TOTAL	13	3.3 C.Y.			
REINFORCING STEEL BARREL WING, ETC SILLS TOTAL	647 15	LBS. LBS. LBS. LBS.	REINFORCING STEEL BARREL WING,ETC SILLS TOTAL	64 1	17 LBS. 18 LBS.			





BILL OF MATERIAL						
(	STAGE	E II	PH	ASE II	I	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
A1	64	#4	1	5'-2″	221	
A2	117	#4	1	4'-7"	358	
A200	64	#4	STR.	16'-2"	691	
A400	64	#4	STR.	16'-2"	691	
B1	27	#4	STR.	10'-5"	188	
B2	64	#4	STR.	8'-4"	356	
B3	54	#4	STR.	10'-5"	376	
C2	37	#4	STR.	26′-5″	653	
1	REINFOR	RCING	STEEL	= 3,534	LBS	
		EI	_	ASE IV		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
A1	64	#4	1	5'-2"	221	
A2	64	#4	1	4'-7"	196	
A100	64	<b>#</b> 5	STR.	38'-3"	2553	
A250	64	#4	STR.	24'-0"	1026	
A300	64	#4	STR.	38'-3"	1635	
A450	64	#4	STR.	24'-0"	1026	
B1	27	#4	STR.	10'-5"	188	
B2	64	#4	STR.	8'-4"	356	
B3	54	#4	STR.	10'-5"	376	
C2	99	#4	STR.	26'-5"	1747	
	A			70/ 4"	100	
G1	4	<b>#</b> 5	STR.	38'-4"	160	
	REINFO	RCING	STEEL	. = 9,48	34 LBS	

SEAL O31583		PROJEC	<u>MOOR</u>	E		5 UNTY L-
PRASAD 9/30/2021 CONCRETE BOX CULVERT STAGE II 90° SKEW	DocuSigned by: Krishma P. Sedai EAGE 790 SUBERED C AROL NOR FESSION SEAL 031583 PRASHO INTITUTION	depaf TRIF	RTMENT	OF TRAN RALEIGH 2 F T BOX AGE	SPORTA X 9 CUL II	
REVISIONS     SHEET NO.       DOCUMENT NOT CONSIDERED FINAL UNLESS ALL     NO. BY: DATE: NO. BY: DATE: C-5       1     3     TOTAL SHEETS			DATE: N	0. BY:	DATE:	C-5

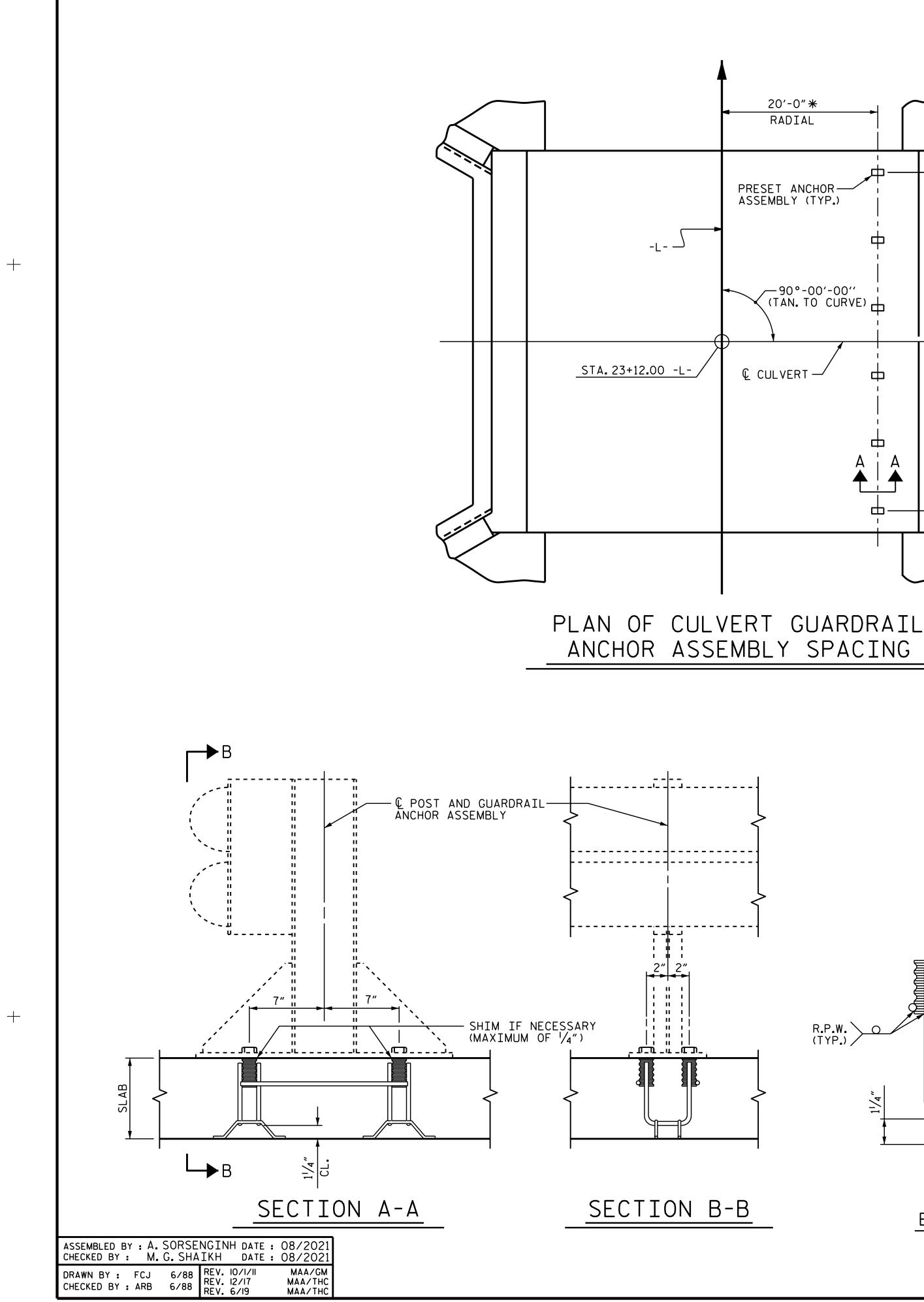


+

+

	E	BILL OF MATERIALS						
			PHASE I					
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
		D1	4	<b>#</b> 6	STR	2'-6"	15	
		REINF	FORCI	NG STE	EL		15 LBS.	
STAGE	Ι	CLASS	5 A C	ONCRET	Ē		0.9 CY	
				PH	ASE	II		
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
		D2	8	<b>#</b> 6	STR	1'-6"	18	
		REIN	18 LBS.					
		CLASS	5 A C	ONCRET	Ē		0.9 CY	
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
		D1	4	<b>#</b> 6	STR	2'-6"	15	
		REIN	15 LBS.					
STAGE	II	CLASS	5 A C	ONCRET	Ē		0.9 CY	
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
		D2	8	<b>#</b> 6	STR	1'-6"	18	
		REIN	FORCI	NG STE	EL		18 LBS.	
		CLASS	CLASS A CONCRETE					

-	PROJECT NO. <u>BR-0035</u> <u>MOORE</u> COUNTY STATION: <u>23+12.00</u> -L-
	SHEET 6 OF 8
E DocuSigned by: Krishna P. Sedai EAGF720008F48CAR0/ PFESSION SEAL 031583 PRASAD 9/30/2021	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH TRIPLE 12 FT. X 9 FT. CONCRETE BOX CULVERT 90° SKEW
	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: C-6
FINAL UNLESS ALL	1 3 TOTAL SHEETS
SIGNATURES COMPLETED	<b>②</b>





20'-0"\* RADIAL



GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE. FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

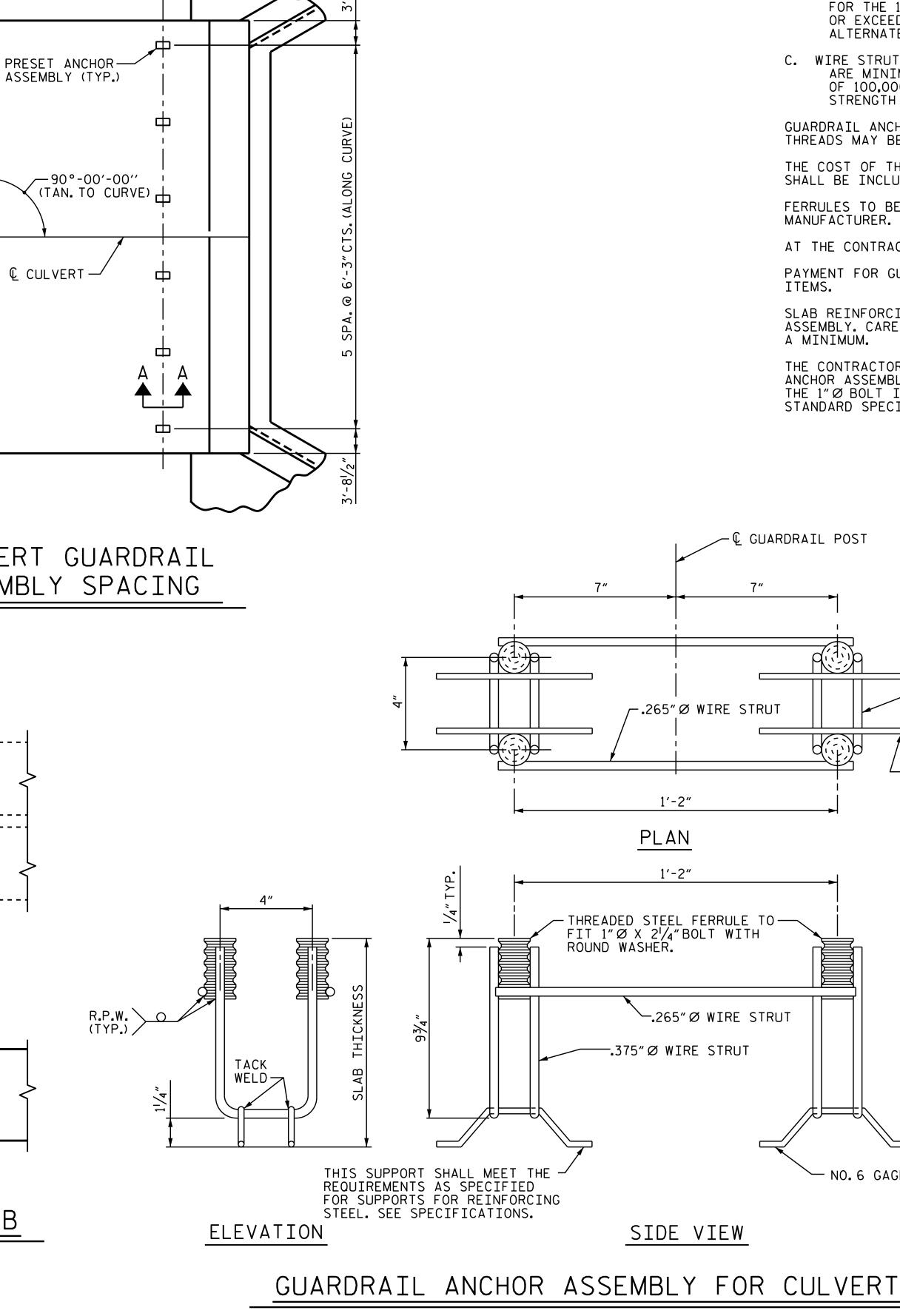
MANUFACTURER.

AT THE CONTRACTOR'S OPTION. FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY

ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.



# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".

B. 4 - 1" Ø X 2<sup>1</sup>/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

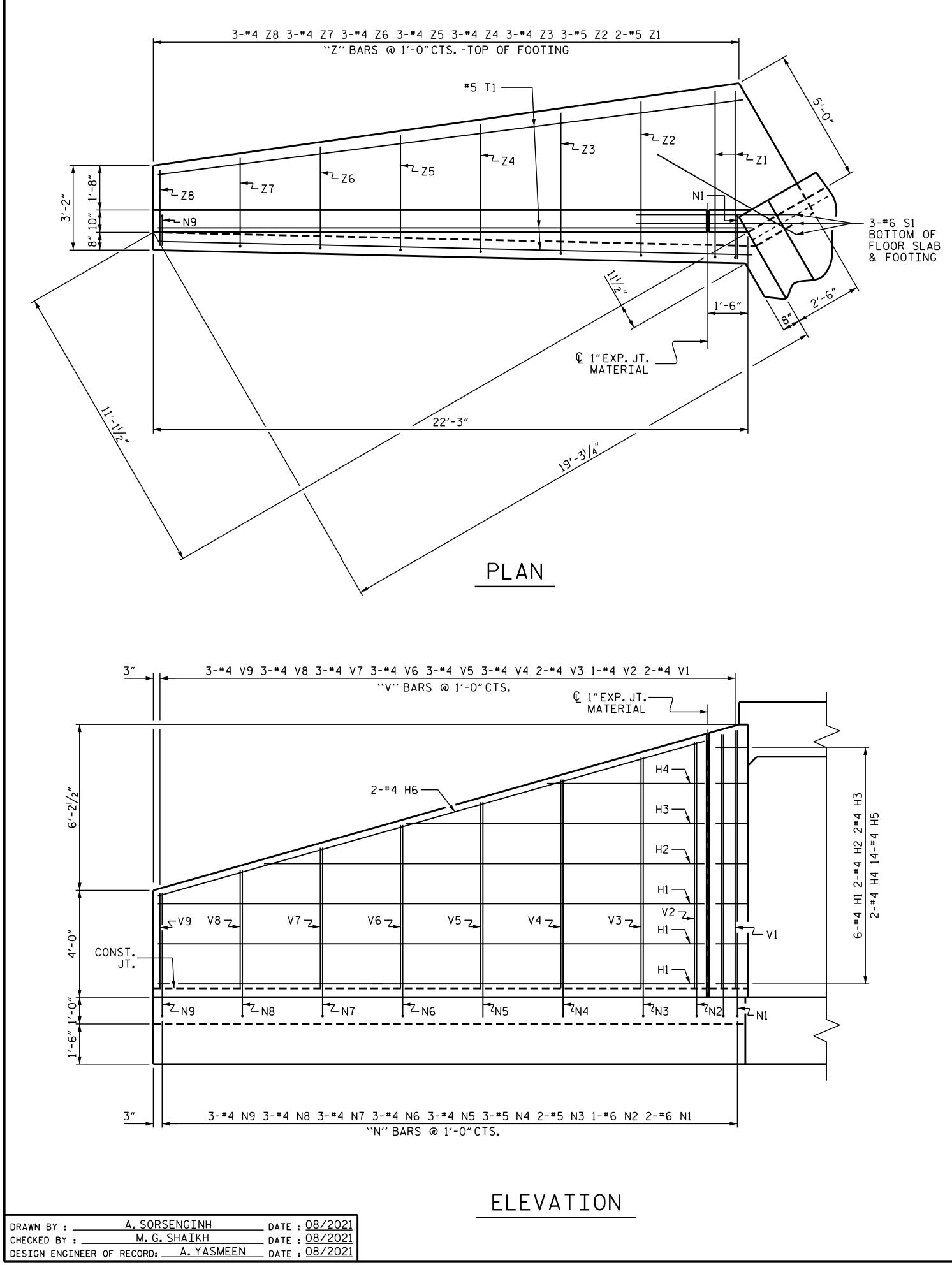
C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A  $\frac{7}{16}$  % WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

—.375″Ø WIRE STRUT

└── NO.6 GAGE WIRE

			T NO MOORE DN:23 <sup>.</sup>		CO	UNTY
NO.6 GAGE WIRE	DocuSigned by: Krishna P. Sedai EAGE73100 EFBTC AROL OF ESSION SE AL 031583 9/30/2021	AN	STATE OF RTMENT OF STAN STAN CHORAGE DRAIL AN FOR C	TRAN ALEIGH NDAR DET ICHO	NSPORTA D AILS F R ASSE	OR
			REVISION	S		SHEET NO.
VERTS [	DOCUMENT NOT CONSIDERED	NO. BY:	DATE: NO.	BY:	DATE:	C-7
	FINAL UNLESS ALL SIGNATURES COMPLETED	1 2	3 4			TOTAL SHEETS 8
		6				0

SID. NO. GRAI

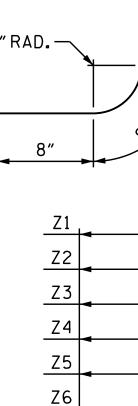


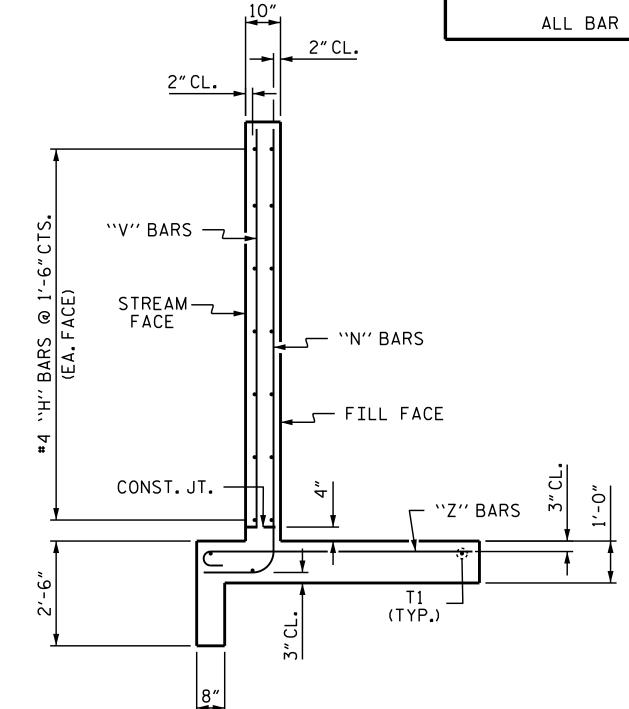
30-SEP-2021 07:59 K:\Structures\FinalPlans\411\_017\_BR-0035\_SMU\_CU\_008\_620024.dgn ksedai

+

+

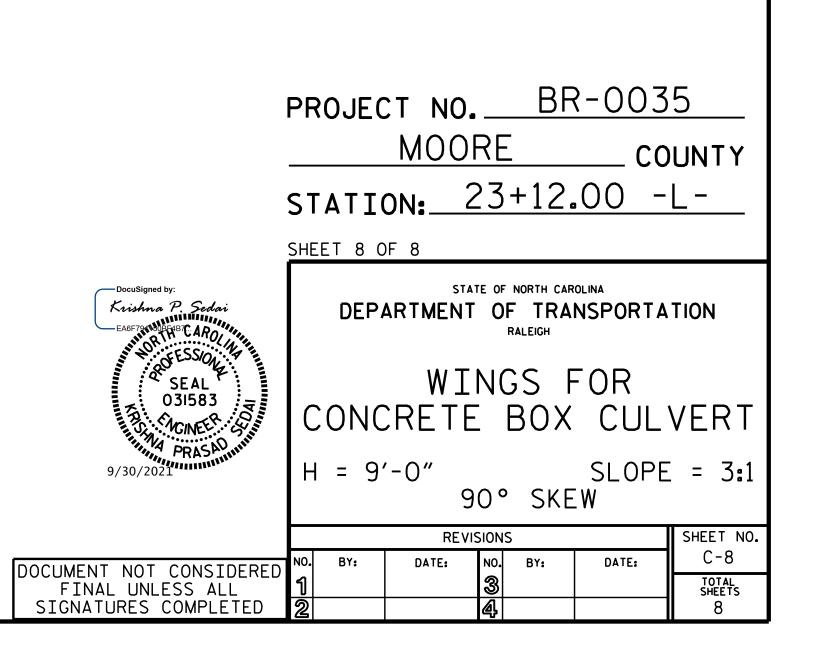
CLA	SS A CONCRETE		
	PHASE I		
	WING - PHASE I END CURTAIN WALL TOTAL	0.8 C.Y.	
STAGE I	PHASE II		
	WING - PHASE II 1 HEADWALL END CURTAIN WALL TOTAL	1.8 C.Y.	
	PHASE II	Ι	
STAGE II	WING - PHASE III END CURTAIN WALL TOTAL	10.1 C.Y. 0.8 C.Y. 10.9 C.Y.	2
	PHASE IV		
	WING - PHASE IV 1 HEADWALL END CURTAIN WALL TOTAL	1.8 C.Y.	6″RAD.—





TYPICAL WING SECTION

$\begin{array}{c c c c c c c c c c c c c c c c c c c $	BAR TYPES			BIL			TERIAL	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $								
$\begin{array}{c c c c c c c c c c c c c c c c c c c $					-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2	•						
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		* 0			-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		1,-			-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					-	_		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			H6	8	#4	STR	21'-3"	114
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	<u>1'-3"</u> <u>1'-8¾"</u>		N1	8	#6	2	11'-6"	138
$\begin{array}{c c c c c c c c c c c c c c c c c c c $								
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1	N 8 N 9 N 9 N 9 N 9 N 9 N 9 N 9 N 9 N 9						
$ \begin{array}{c} 85 \\ 6' \\ 6' \\ 6' \\ 6' \\ 7' \\ 6' \\ 6' \\ 7' \\ 6' \\ 7' \\ 6' \\ 7' \\ 7$								
$\frac{1}{10} + \frac{1}{10} $								
$\frac{1}{10} + \frac{1}{10} $					-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$								
S1     12     #6     STR     6'-0"     108       T1     12     #5     STR     22'-1"     276       T1     12     #5     STR     22'-1"     276       V1     8     #4     STR     9'-6"     51       V2     4     #4     STR     9'-2"     24       V3     8     #4     STR     8'-8"     46       V4     12     #4     STR     6'-1"     63       5'-9"     7"     7"     5'     12     #4     STR     6'-1"       5'-9"     7"     6"     6"     71     12     #4     STR     6'-1"     49       V7     12     #4     STR     5'-5"     42     V8     12     #4     STR     5'-6"     42       V8     12     #4     STR     5'-6"     44     25     12     #4     3     5'-6"     44       25     12     #4     3     5'-6"     44     25     12     #4     3     3'-10"     35'		1/2"			-			
S1     12     #6     STR     6'-0"     108       T1     12     #5     STR     22'-1"     276       T1     12     #5     STR     22'-1"     276       V1     8     #4     STR     9'-6"     51       V2     4     #4     STR     9'-2"     24       V3     8     #4     STR     8'-8"     46       V4     12     #4     STR     6'-1"     63       5'-9"     7"     7"     5'     12     #4     STR     6'-1"       5'-9"     7"     6"     6"     71     12     #4     STR     6'-1"     49       V7     12     #4     STR     5'-5"     42     V8     12     #4     STR     5'-6"     42       V8     12     #4     STR     5'-6"     44     25     12     #4     3     5'-6"     44       25     12     #4     3     5'-6"     44     25     12     #4     3     3'-10"     35'								
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		4 4		<u> </u>			<u> </u>	10
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			S1	12	*6	STR	6'-0"	108
$ \begin{array}{c} 8 \\ 9 \\ 1 \\ \hline \\ 8 \\ \hline $			Τ1	12	<b>*</b> 5	STR	22'-1"	276
$ \begin{array}{c} 8^{1}1 \\ \hline \\ 8^{1}1 \\ \hline \\ 6'-3'' \\ \hline \\ 6'-3'' \\ \hline \\ 6'-3'' \\ \hline \\ 5'-9'' \\ \hline \\ 5'-9'' \\ \hline \\ 5'-3'' \\ \hline \\ 6'' \\ \hline \\ 6'' \\ \hline \\ 4'-4'' \\ \hline \\ 6'' \\ \hline \\ 3'-4'' \\ \hline \\ 6'' \\ \hline \\ 3'-4'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 3' - 10'' \\ \hline \\ 6'' \\ \hline \\ 7' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 10'' \\ \hline \\ 6'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 7' - 9'' \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 4'' \\ 7' \\ 7' \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 22 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 25 \\ 12 \\ * 4 \\ 3 \\ 3' - 10'' \\ 31 \\ \hline \\ 7' \\ 25 \\ 2586 \\ LBS \\ \end{array}$						670	01.64	<b></b>
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$								
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	311							ł
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					-			
6'-3'' $7''$ $5'-9''$ $7''$ $5'-9''$ $7''$ $5'-3''$ $6''$ $4'-10''$ $6''$ $4'-4''$ $6''$ $4'-4''$ $6''$ $3'-10''$ $6''$ $2'-10''$ $6''$ $3$ $HK.$ $3$ $HK.$								
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	C / <b>Z</b> //	7″						ł
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	<u> </u>	< <u>^</u>			-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5'-9"	7″			-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5'-3"	6"			-			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	•		VY	12	#4	SIR	5'-6"	28
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4'-10"	6″	71	R	#5	٦	6'-10"	57
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4'-4"	6″			_			
3'-4"     6"       3'-4"     6"       2'-10"     6"       4"     3       5     12       12     12       12     14       13     12       14     13       15     12       16     12       17     12       18     12       19     12       10     12       10     12       12     13	7/ 10//				_			
3'-4"     6"       2'-10"     6"       2'-10"     6"       4"     3       4'-4"     35       27     12     4       3     4'-4"     35       27     12     4     3     4'-4"       3     4'-4"     35     3'-10"     31       28     12     4     3     3'-4"     27       HK.     HK.     REINFORCING STEEL     2586 LBS	510	<b>b</b>						
2'-10"     6"       Z6     12     #4     3     4'-4"     35       Z7     12     #4     3     3'-10"     31       Z8     12     #4     3     3'-4"     27       HK.     REINFORCING STEEL FOR 4 WINGS     2586 LBS	3'-4"	6″						
Z7     12     #4     3     3'-10"     31       Z8     12     #4     3     3'-4"     27       HK.     REINFORCING STEEL FOR 4 WINGS     2586 LBS	2'-10"	6"						
3   Z8   12   #4   3   3'-4"   27     REINFORCING STEEL FOR 4 WINGS   2586 LBS								
3 HK. REINFORCING STEEL   50R 4 WINGS 2586 LBS								
						ĒL	25	86 LBS



### DESIGN DATA:

+

-

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO  $1\frac{1}{2}$  RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A  $\frac{1}{4}$  RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# STANDARD NOTES

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾″Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY VIGINCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

