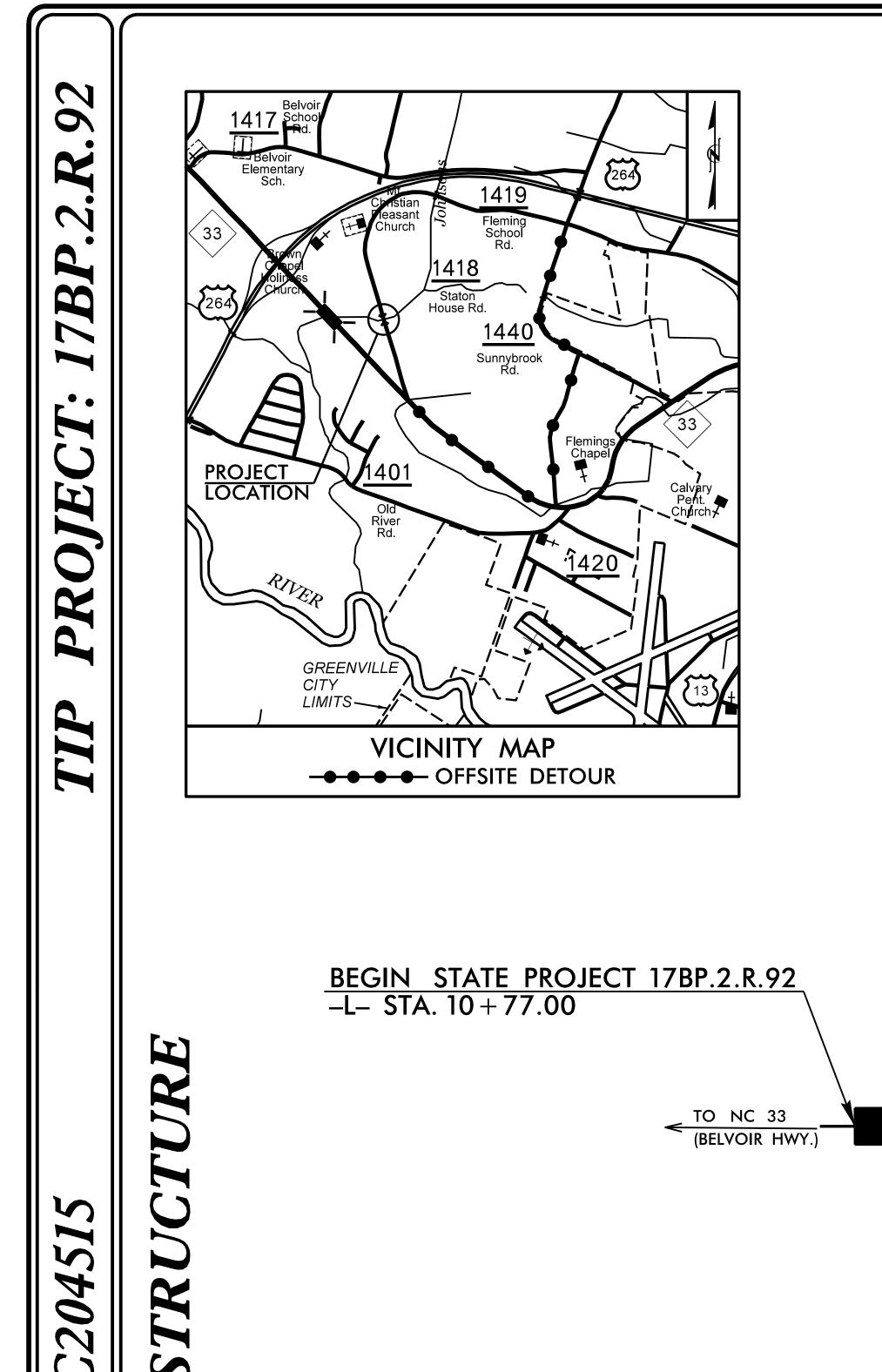
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PITT COUNTY

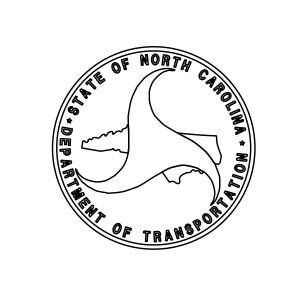
LOCATION: BRIDGE NO. 171 OVER JOHNSON MILL RUN ON SR 1418 (STATON HOUSE ROAD)

TYPE OF WORK: GRADING, DRAINAGE, STRUCTURE AND PAVING

STATE	STATE	PROJECT REFERENCE NO.		SHEET NO.	SHEETS		
N.C.	17						
STAT	B PROJ. NO.		DESCRIPTION				
17B	P.2.R.92	N/A		PE			
17B	P.2.R.92	N/A		RW, UT	IL		
17B	P.2.R.92	N/A		•			
			<u> </u>				
		1	1				

END STATE PROJECT 17BP.2.R.92 /-L-STA. 16+61.00→ SR 1418 TO SR 1419 (FLEMMING SCHOOL RD.) STATON HOUSE RD. END BRIDGE -L- STA. 14 + 22.37 BEGIN BRIDGE -L- STA. 13 + 34.63

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



DESIGN DATA

ADT 2018 = 430ADT 2035 = 500

DHV = 12%D = 55%

T = 15%V = 60 MPH* TTST = 4 % DUAL = 11 %

FUNC CLASS = LOCAL RURAL SUB-REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT 17BP.2.R.92 = 0.094 mile LENGTH STRUCTURES TIP PROJECT 17BP.2.R.92 = 0.017 mile TOTAL LENGTH TIP PROJECT 17BP.2.R.92 = 0.111 mile

PLANS PREPARED BY:	PLANS PREPARED FOR:
TGS ENGINEERS 706 HILLSBOROUGH ST SUITE 200 RALEIGH, NC 27603	NORTH CAROLINA DEPARTMENT OF TRANSPORTATION 1000 Birch Ridge Dr. Raleigh NC, 27610
LETTING DATE: OCTOBER 19, 2021	MARC CHEEK, PE STRUCTURES DESIGN ENGINEER

2018 STANDARD SPECIFICATIONS

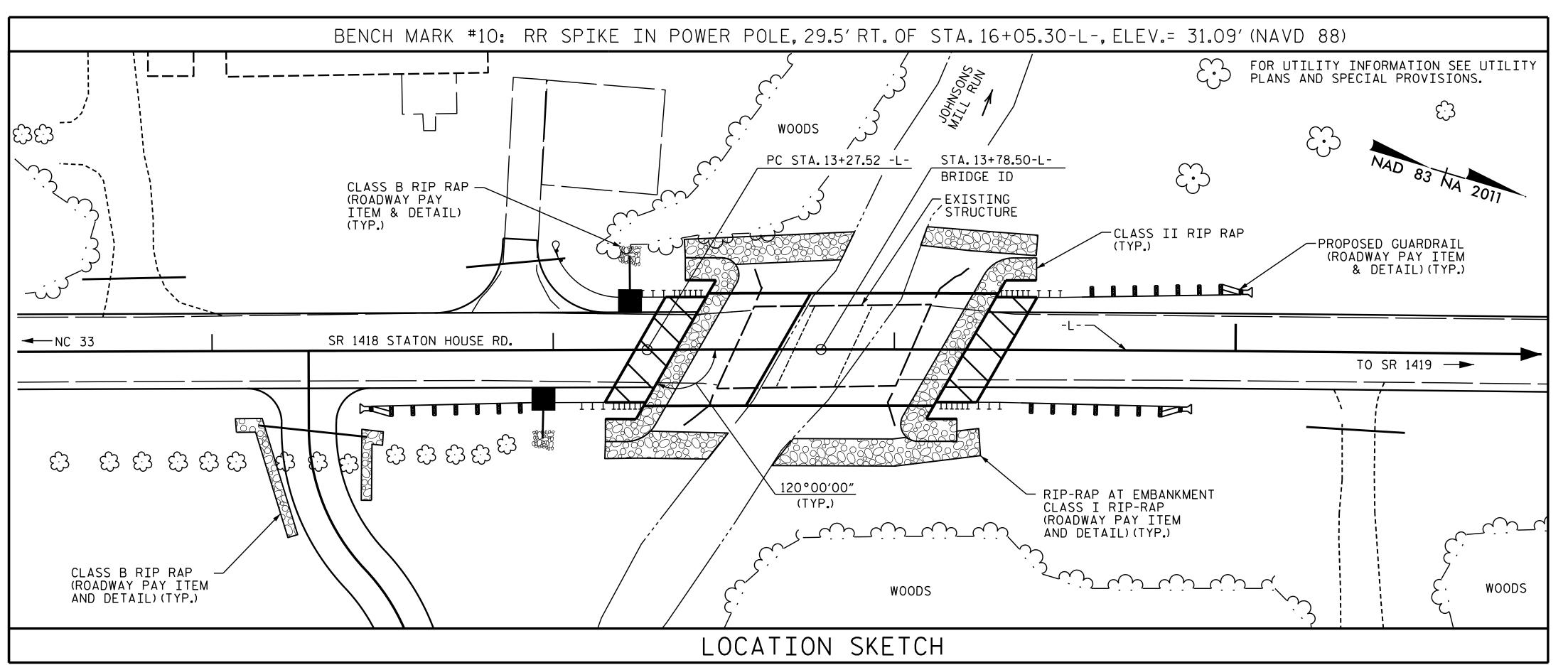
NCDOT CONTACT: DAVID STUTTS, PE

STRUCTURES DESIGN **ENGINEER**



SIGNATURE:





	TOTAL BILL OF MATERIAL											
ITEM	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS "A" CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR HP 14×73 GALVANIZED STEEL PILES	HP 12×53 STEEL PILES	HP 14×73 GALVANIZED STEEL PILES	PILE REDRIVES
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	C.Y.	LUMP SUM	LBS.	EACH	EACH	LIN.FT.	LIN.FT.	EACH
SUPERSTRUCTURE												
END BENT 1					15.8		2,357	7		385		3
BENT 1					12.9		2,500		8		640	4
END BENT 2					15.8		2,357	7		490		3
TOTAL	LUMP SUM	LUMP SUM	1	LUMP SUM	44.5	LUMP SUM	7,214	14	8	875	640	10

	TOTAL	BILL OF	MATERIAL	(CONT.)			
ITEM	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THK.)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE CC	O"× 1'-9" STRESSED NCRETE ED SLABS	FIBER OPTIC CONDUIT SYSTEM
	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LIN.FT.
SUPERSTRUCTURE	170.28				22	935.00	166.28
END BENT 1		85	95				
BENT 1							
END BENT 2		95	105				
TOTAL	170.28	180	200	LUMP SUM	22	935.00	166.28

DRAWN BY: TBE DATE: 3/19
CHECKED BY: MGC DATE: 9/19
DESIGN ENGINEER OF RECORD: MGC DATE: 9/19

FOUNDATION RECOMMENDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 51 TONS PER PILE.

PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 71 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 170 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR SCOUR.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.

INSTALL PILES AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN -18.0 FT.

THE SCOUR CRITICAL ELEVATIONS FOR BENT NO.1 IS ELEVATION 7.0 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN (S-19).

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING STRUCTURE (1 @ 17'-1",1 @ 16'-10",1 @ 18'-2") WITH A CLEAR ROADWAY WIDTH OF 24'-0" CONSISTING OF A REINFORCED CONCRETE DECK ON 19 LINES OF TIMBER JOISTS WITH A SUBSTRUCTURE CONSISTING OF TIMBER POST AND SILL ABUTMENTS AND WITH A STEEL CRUTCH BENT AT END BENT 2 AND LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THIS LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THIS PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE HATCHED AREA (SHEET 1 OF 2) SHALL BE EXCAVATED FOR A DISTANCE OF 25 FEET TO EACH SIDE OF THE CENTERLINE OF THE BRIDGE AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION, SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIES ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES".

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR FIBER OPTIC CONDUIT SYSTEM, SEE SPECIAL PROVISIONS

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS, PROVIDED THAT SPAN A IS CONSTRUCTED FIRST. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+78.50 -L-".

SHEET 2 OF 2

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-

SEAL 20125

Docotogodo b.G. CHELLER Jr.

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE ON SR 1418 OVER JOHNSONS MILL RUN BETWEEN NC 33 AND SR 1419

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS

706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

1/6/2020

REVISIONS

SHEET NO. BY: DATE: NO. BY: DATE: S-2

1 3 5000

75 2 4 19

		LOAD AN	D RES	SIST	ANCE	FAC	CTOR	RAT	ING	(LRF	D) SI	UMMA	RY F	OR F	PRES	TRES	SSED	CON	CRETE	E GI	RDEF	RS		
										STRE	NGTH	I LIN	MIT S	TATE				SERVICE III LIMIT STATE				TE		
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.202		1.75	0.256	2.04	30′	EL	14.423	0.655	1.2	30′	EL	1.442	0.80	0.256	1.75	30′	EL	14.423	
DESIGN	-	HL-93(0pr)	N/A		1.558		1.35	0.256	2.64	30'	EL	14.423	0.655	1.56	30′	EL	1.442	N/A						
LOAD RATING	-	HS-20(Inv)	36.000	2	1.365	49.124	1.75	0.256	2.82	30′	EL	11.538	0.655	1.36	30′	EL	1.442	0.80	0.256	2.45	30′	EL.	11.538	
		HS-20(0pr)	36.000		1.769	63.679	1.35	0.256	3.65	30′	EL	11.538	0.655	1.77	30′	EL	1.442	N/A	0.056	7.05	70/		14 407	
		SNSH	13.500		3.333	45.002	1.4	0.256	5.76	30′	EL	14.423	0.655	3.33	30′	EL	1.442	0.80	0.256	3.95	30′	EL	14.423	
	-	SNGARBS2 SNAGRIS2	20.000		2.581 2.487	51 . 624 54 . 723	1.4	0.256 0.256	5 . 04 5 . 13	30′ 30′	EL EL	11 . 538	0.655 0.655	2.58 2.49	30′ 30′	EL EL	1.442 1.442	0.80	0.256 0.256	3 . 50	30′ 30′	EL	11 . 538	
	-	SNCOTTS3	27.250		1.684	45.891	1.4	0.256	2.89	30′	EL	14.423	0.655	1.68	30′	EL	1.442	0.80	0.256	1.99	30′	FI	14.423	
	\ \s \ \	SNAGGRS4	34.925		1.551	54.185	1.4	0.256	2.79	30′	EL	14.423	0.655	1.55	30′	EL	1.442	0.80	0.256	1.91	30′	FI	14.423	
		SNS5A	35.550		1.645	58.469	1.4	0.256	2.7	30'	EL	14.423	0.655	1.64	30°	EL	1.442	0.80	0.256	1.85	30′	FI	14.423	
		SNS6A	39.950		1.547	61.791	1.4	0.256	2.55	30′	EL	14.423	0.655	1.55	30°	EL	1.442	0.80	0.256	1.75	30°	EL	14.423	
. 504		SNS7B	42.000		1.578	66.285	1.4	0.256	2.48	30′	EL	14.423	0.655	1.58	30′	EL	1.442	0.80	0.256	1.70	30′	EL	14.423	
LEGAL LOAD		TNAGRIT3	33.000		1.838	60.67	1.4	0.256	3.31	30′	EL	14.423	0.655	1.84	30′	EL	1.442	0.80	0.256	2.27	30′	EL	14.423	
RATING		TNT4A	33.075		1.71	56.559	1.4	0.256	3.13	30′	EL	14.423	0.655	1.71	30′	EL	1.442	0.80	0.256	2.15	30′	EL	14.423	
		TNT6A	41.600		1.652	68.714	1.4	0.256	2.85	30′	EL	14.423	0.655	1.65	30′	EL	1.442	0.80	0.256	1.96	30′	EL	14.423	
	ST	TNT7A	42.000		1.573	66.067	1.4	0.256	2.94	30′	EL	14.423	0.655	1.57	30′	EL	1.442	0.80	0.256	2.02	30′	EL	14.423	
	=	TNT7B	42.000		1.536	64.525	1.4	0.256	2.77	30′	EL	14.423	0.655	1.54	30′	EL	1.442	0.80	0.256	1.90	30′	EL	14.423	
		TNAGRIT4	43.000		1.486	63 . 9	1.4	0.256	2.87	30′	EL	14.423	0.655	1.49	30′	EL	1.442	0.80	0.256	1.97	30′	EL	14.423	
		TNAGT5A	45.000		1.594	71.736	1.4	0.256	2.79	30′	EL	14.423	0.655	1.59	30′	EL	1.442	0.80	0.256	1.92	30′	EL	14.423	
		TNAGT5B	45.000	3	1.399	62.946	1.4	0.256	2.68	30′	EL	11.538	0.655	1.4	30′	EL	1.442	0.80	0.256	1.85	30′	EL	11.538	

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1 . 50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD LRFR SUMMARY FOR 30'CORED SLAB UNIT 120° SKEW

(NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-3
		3			TOTAL SHEETS
		4			19



LRFR SUMMARY
FOR SPAN 'A'

ASSEMBLED BY: ZCS DATE: 06/19 CHECKED BY: MGC DATE: 09/19

DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE SHEAR MOMENT MOMENT CONTROLLING LOAD RATING FRO OF DISTANCE LEFT END SPAN (ft) DISTRIBU^T FACTORS (DISTRIBU^T FACTORS (MINIMUM RATING F, (RF) LIVELOAD FACTORS DIST/ LEFT SPAN DIST/ LEFT SPAN 1.163 1.75 1.36 55′ 26.923 0.659 1.21 55′ EL 10.769 0.80 0.249 55′ 26.923 HL-93(Inv)N/A 0.249 EL 1.16 EL 1.35 0.249 26.923 0.659 1.564 1.76 55′ EL 1.56 55′ EL HL-93(Opr)N/A 10.769 N/A --DESIGN LOAD 36.000 1.424 51.265 0.80 0.249 1.75 0.249 1.7 55′ 26.923 0.659 1.42 55′ 10.769 1.46 EL EL 55′ 26.923 HS-20(Inv) EL RATING 1.35 66.455 0.249 26.923 36.000 55′ 0.659 1.85 55′ 10.769 2.2 EL N/A HS-20(0pr) 41.264 0.249 26.923 26.923 0.659 3.96 55′ 10.769 0.80 SNSH 13.500 3.057 1.4 0.249 4.46 55′ EL EL 3.06 55′ EL SNGARBS2 20.000 2.374 47.473 0.249 3.46 55′ EL 26.923 0.659 2.9 55′ EL 10.769 0.80 0.249 2.37 55′ EL 26.923 2.291 50.392 0.249 3.34 26.923 0.659 2.72 55′ 10.769 0.80 0.249 2.29 55′ EL 55′ 26.923 SNAGRIS2 22.000 1.4 41.521 0.249 0.80 0.249 1.52 27.250 1.524 2.22 55′ 26.923 0.659 1.98 55′ 10.769 55′ 26.923 EL SNCOTTS3 26.923 34.925 26.923 0.659 55′ 10.769 0.80 0.249 1.31 SNAGGRS4 45.74 0.249 55′ 1.71 55′ 1.31 1.4 1.91 EL EL EL 26.923 1.278 0.249 0.659 55′ 10.769 0.80 0.249 1.28 55′ SNS5A 35.550 45.439 1.86 55′ EL 1.76 EL EL 26.923 1.189 47.481 0.249 1.73 55′ 26.923 0.659 1.63 55′ EL 10.769 0.80 0.249 1.19 55′ 26.923 SNS6A 39.950 1.4 EL EL 47.562 26.923 0.80 0.249 42.000 1.132 0.249 1.65 55′ 0.659 1.64 55′ 10.769 1.13 55′ SNS7B EL 26.923 LEGAL LOAD 47.984 0.249 26.923 33.000 2.12 26.923 0.659 1.92 55′ 10.769 0.80 TNAGRIT3 1.454 0.249 55′ 1.45 1.4 EL RATING 26.923 0.80 55′ 1.85 55′ 10.769 0.249 55′ TNT4A 33.075 1.465 48.451 0.249 2.14 EL 0.659 EL 1.46 26.923 EL 26.923 1.81 0.80 TNT6A 41.600 1.213 50.478 1.4 0.249 1.77 55′ 0.659 55′ EL 10.769 0.249 1.21 55′ EL 26.923 0.80 26.923 1.228 51.576 0.249 1.79 55′ 0.659 1.67 55′ EL 10.769 0.249 1.23 55′ 26.923 TNT7A 42.000 EL 1.282 53.827 0.249 26.923 0.659 1.58 55′ 10.769 0.80 0.249 1.28 1.87 55′ 55′ 26.923 TNT7B 42.000 1.4 EL EL EL 52.158 0.249 1.77 26.923 0.659 1.52 10.769 0.80 1.213 55′ 55′ 0.249 55′ TNAGRIT4 43.000 EL EL 1.21 26.923 EL 1.55 1.136 51.134 0.659 55′ 10.769 0.80 0.249 EL EL 1.14 26.923 TNAGT5A 45.000 0.249 1.66 26.923

EL 26.923 0.659

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1 . 50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

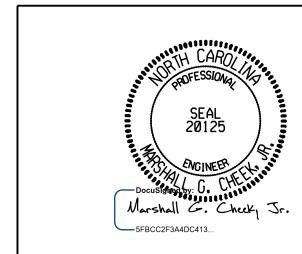
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-



1.12

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

LRFR SUMMARY FOR

55' CORED SLAB UNIT 120° SKEW (NON-INTERSTATE TRAFFIC)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

RALEIGH PH (919) 773–8887

1/6/2020

2 3

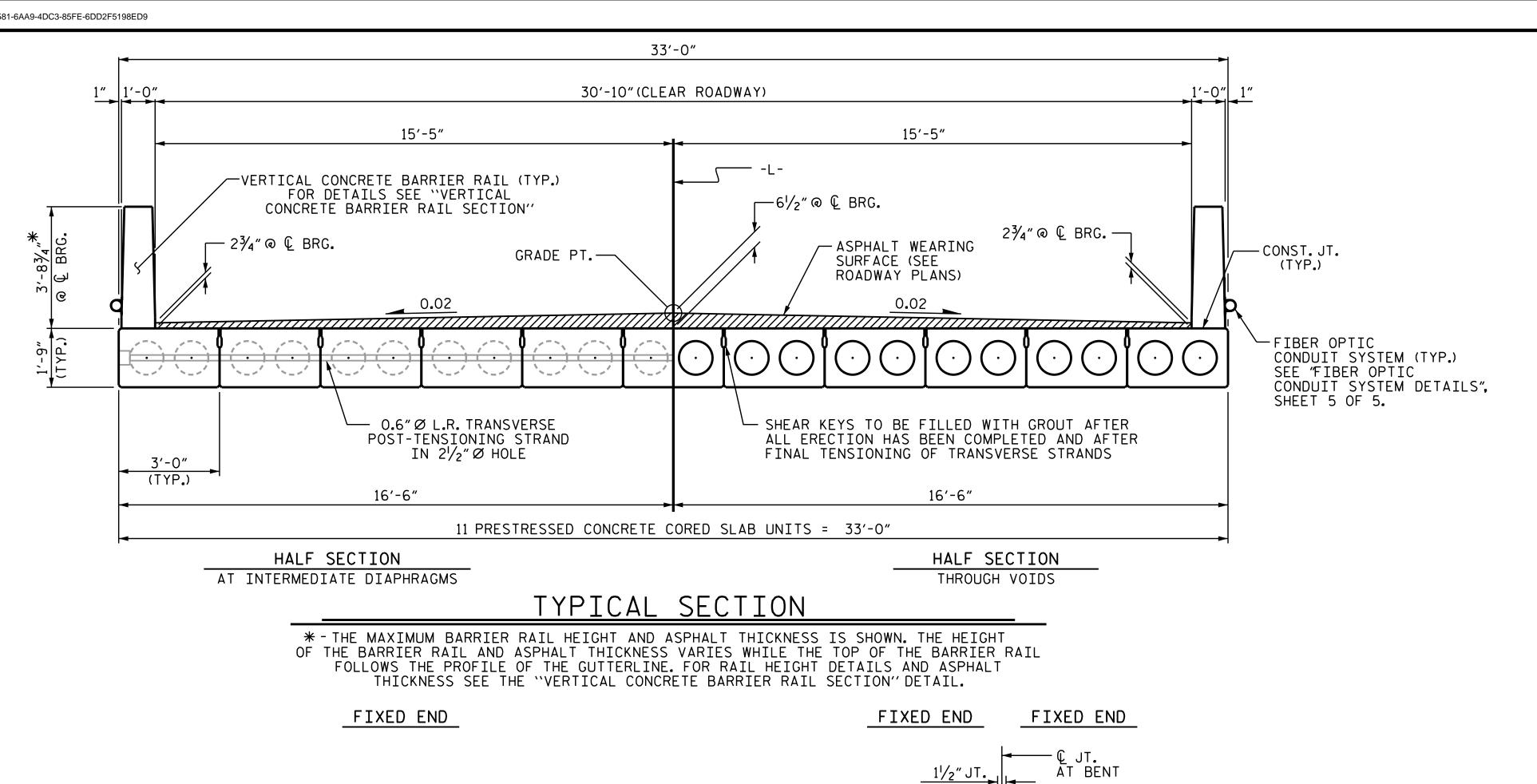
LRFR SUMMARY

FOR SPAN 'B'

ASSEMBLED BY: ZCS DATE: 06/19 CHECKED BY: MGC DATE: 09/19

DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

1.116 50.224



3'-0'' 1'-6'' 10′′ 1'-4'' 11'' 4'' 4'' 11'' ⊢12" Ø VOIDS ≧ 2 SPA. — -4 SPA.└─ 2 SPA. @ 2"CTS. @ 2"CTS. @ 2"CTS. INTERIOR SLAB SECTION (30' UNIT) (9 STRANDS REQUIRED)

3'-0''

1'-6''

INTERIOR SLAB SECTION

(55' UNIT) (19 STRANDS REQUIRED)

LOW

└-4 SPA.└─-2 SPA.

@ 2"CTS. @ 2"CTS.

┌12′′Ø VOIDS ❖

1'-6''

2 SPA.-

@ 2"CTS.

0.6''Ø

RELAXATION STRAND LAYOUT

3'-0'' 1'-4'' 10′′ 3%" CL. 12" Ø VOIDS-

EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A

DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT

SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O" FROM END OF CORED SLAB UNIT SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.—

THREADED INSERT DETAIL

PROJECT NO. 17BP.2.R.92 COUNTY

STATION: 13+78.50 -L-

SHEET 1 OF 5 STATE OF NORTH CAROLINA

-5FBCC2F3A4DC413... 1/6/2020 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS

706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

DEPARTMENT OF TRANSPORTATION

STANDARD 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT

120° SKEW SHEET NO **REVISIONS** S**-**5 DATE: DATE: BY: BY: TOTAL SHEETS

ASPHALT — 21/2" Ø DOWEL HOLES WEARING SURFACE **GROUT** 12″Ø ¬ :----------voids Li VOIDS . – – – – – – ' -----ELASTOMERIC-BEARING PAD 2"Ø BACKER ROD--ELASTOMERIC BEARING PAD

SECTION AT BENT

SEE "BENT" SHEETS

FOR DETAILS

1'-6" 1'-6" 81/2" 91/2" 91/2" 81/2" 1'-2" 4" 4" 1'-2" \bigcirc 2 $\frac{1}{2}$ " Ø DOWEL HOLES #5 S1 — #5 S1 —

END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

SECTION AT END BENT © 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A NON-CORROSIVE PIPE. ─ HOLE FOR TRANSVERSE STRAND _____ 5%'' X 5'' X 5'' ₽ STRAND VISE OUTSIDE FACE OF EXTERIOR CORED SLAB -FILL RECESS WITH GROUT SECTION B-B **ELEVATION VIEW** GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

 $-2\frac{1}{2}$ " Ø DOWEL HOLE

12" Ø -

- ELASTOMERIC

BEARING PAD

SHEETS FOR DETAILS

-SEE "END BENT"

VOIDS

_-----

DATE: 06/19 ZCS ASSEMBLED BY : DATE: 09/19 CHECKED BY : DRAWN BY: DGE 5/09 MAA/TMG REV. 9/14 CHECKED BY : BCH 6/09

ASPHAL T

2 LAYERS OF 30 LB.-ROOFING FELT TO PREVENT BOND.

11/2" Ø BACKER ROD-

Q BEARING

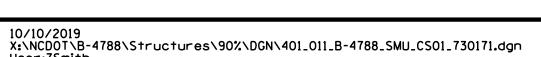
& #6 DOWELS

SEE "BRIDGE

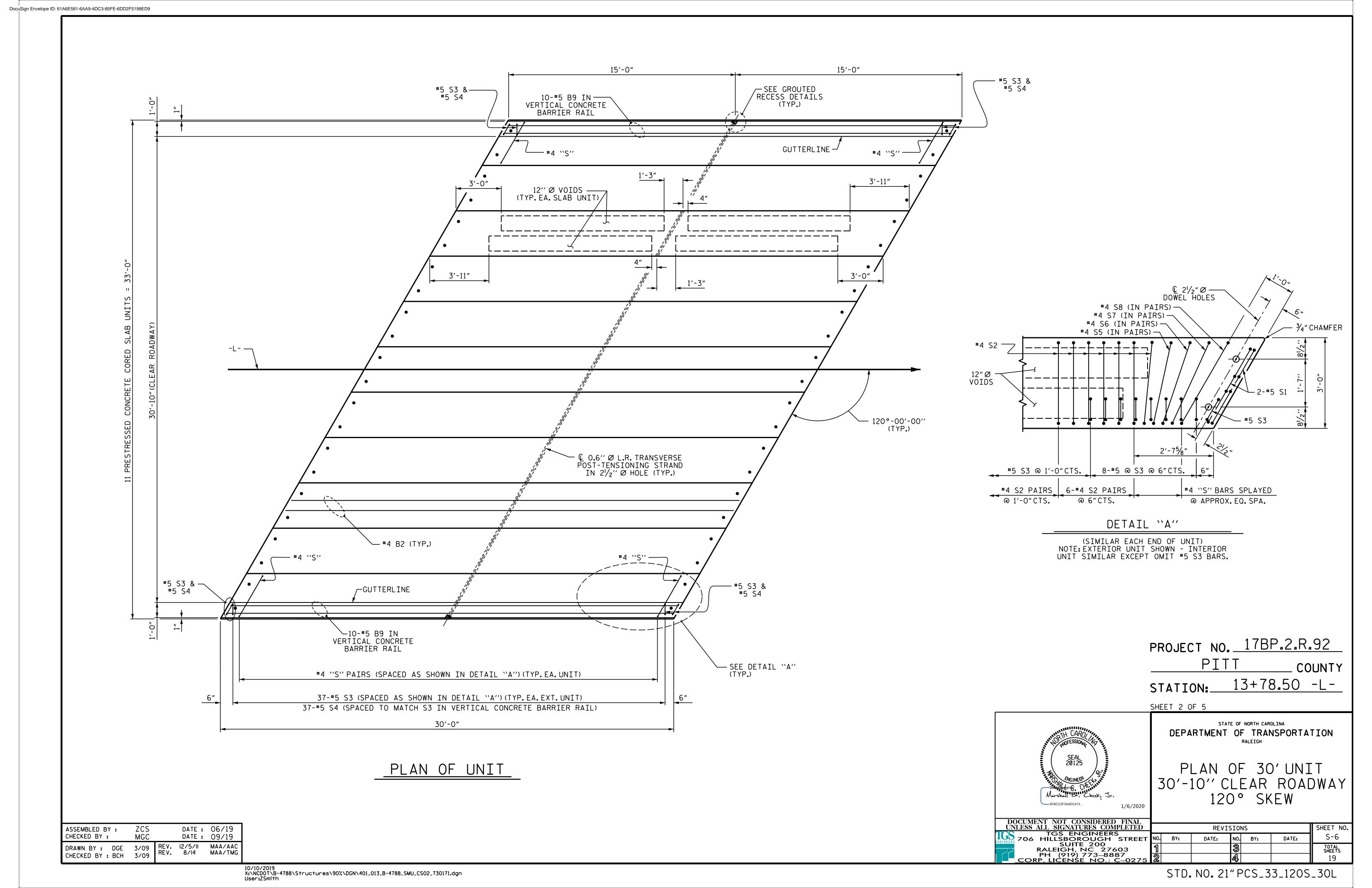
APPROACH SLAB"
SHEET FOR DETAILS

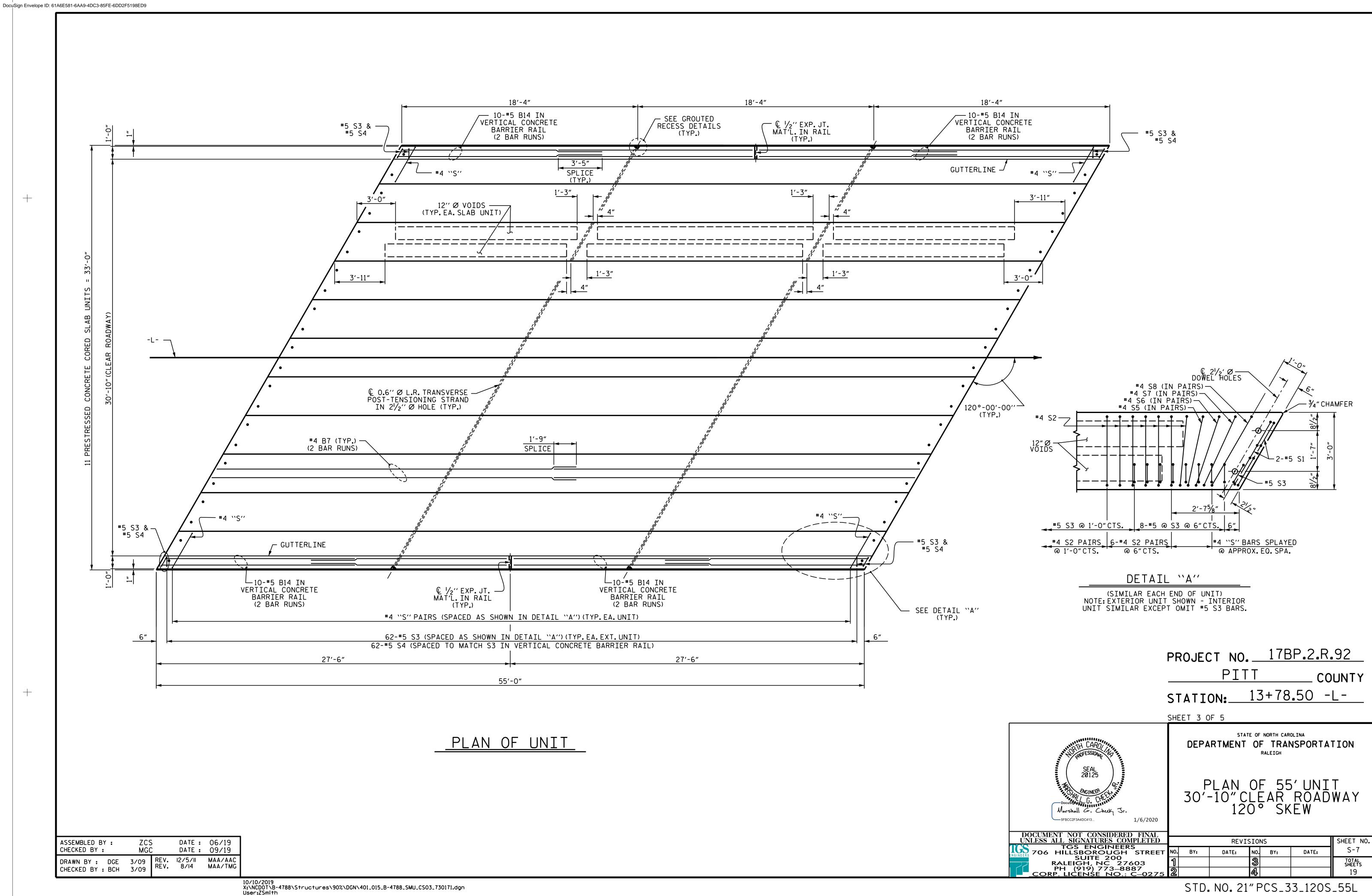
WEARING

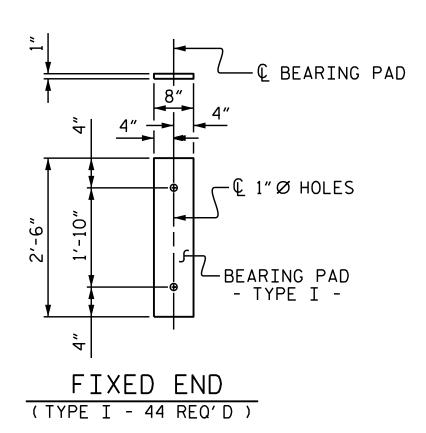
SURFACE -



 BEARING & #6 DOWELS







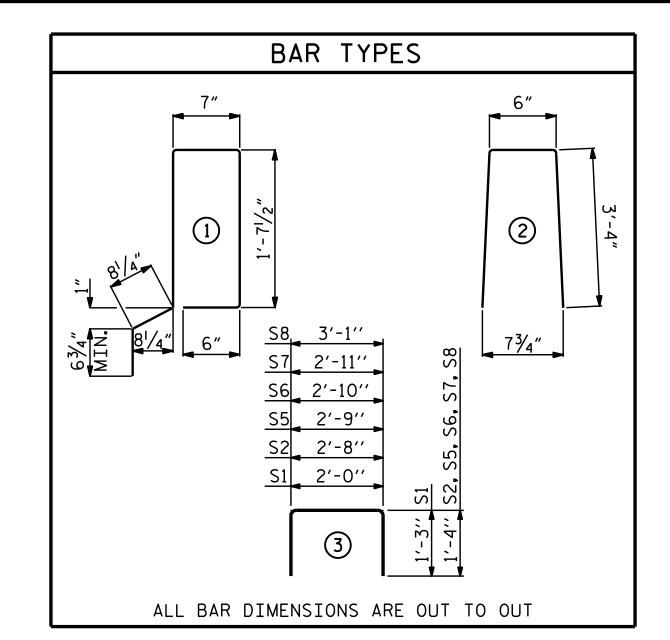
ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
30' UNITS	4000
55' UNITS	4900

DEAD LOAD DEFLECTION AND CAMBER											
	30′ CSU	55' CSU									
3'-0" × 1'-9"	0.6″Ø L.R. STRAND	0.6"Ø L.R. STRAND									
CAMBER (SLAB ALONE IN PLACE)	1/4″ ♦	11/2"									
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	1∕8″ ♦	3⁄8″ ♦									
FINAL CAMBER	¹ /8″ Å	11/8"									

** INCLUDES FUTURE WEARING SURFACE



NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ % DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

				MATERIA RED SLAI		NE	
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B2	2	#4	STR	29'-7"	40	29'-7"	40
S1	8	#5	3	4′-6″	38	4'-6"	38
S2	62	#4	3	5′-4″	221	5′-4″	221
* S3	39	#5	1	5′-7″	227		
S5	4	#4	3	5′-5″	14	5′-5″	14
S6	4	#4	3	5′-6″	15	5′-6″	15
S 7	4	#4	3	5′-7"	15	5'-7"	15
S8	4	#4	3	5′-9"	15	5′-9″	15
REINFO	RCING	STEEL	LBS	5.	358		358
* EPOX	Y COATE	ED					
REIN	FORCING	STEEL	LB:	S .	227		
5000 F	P.S.I.CO	NCRETE	CU. YDS	· ·	4 . 5		4.5
0.6"Ø	L.R. STR	ANDS	No).	9		9

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT												
				EXTERI	OR UNIT	INTERI	OR UNIT					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT					
В7	4	#4	STR	28′-3″	75	28′-3″	75					
S1	8	# 5	3	4′-6″	38	4′-6″	38					
S2	112	#4	3	5′-4″	399	5′-4″	399					
* S3	64	# 5	1	5′-7″	373							
S5	4	#4	3	5′-5″	14	5′-5″	14					
S6	4	#4	3	5′-6″	15	5′-6″	15					
S7	4	#4	3	5′-7″	15	5′-7″	15					
S8	4	#4	3	5′-9″	15	5′-9″	15					
REINF(ORCING :	STEEL	LBS	5.	571		571					
	Y COATE											
REINFORCING STEEL LBS. 373												
6500 F	6500 P.S.I. CONCRETE CU. YDS. 8.0 8.0											
0.6"Ø	L.R. STR	ANDS	No).	19		19					

CORED SLABS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH		
30' UNIT					
EXTERIOR C.S.	2	30'-0"	60′-0″		
INTERIOR C.S.	9	30'-0"	270'-0"		
TOTAL	11		330′-0″		
55' UNIT					
EXTERIOR C.S.	2	55'-0"	110'-0"		
INTERIOR C.S.	9	55'-0"	495′-0″		
TOTAL	11		605′-0″		

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL							
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	
	30' UNIT						
∗ B9	20	20	#5	STR	29'-6"	615	
* S4	78	78	#5	2	7′-2″	583	
* EPOXY COATED REINFORCING STEEL LBS. 1198						1198	
CLASS	LASS AA CONCRETE CU.YDS. 7.7						
TOTAL	TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT. 60.1					60.14	

GRADE 270 STRANDS				
	0.6"Ø L.R.			
AREA (SQUARE INCHES)	0.217			
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600			
APPLIED PRESTRESS	43,950			

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-

STATE OF NORTH CAROLINA

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
30'UNITS	25/8″	3′-85⁄8″
55' UNITS	15/8"	3′-7 ⁵ ⁄8″

HEIGHT		BI	LL OF MATERIAL FOR VER	TICAL CON	CRETE	BARR	RIER R	AIL
RAIL HEIGHT	1	BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
@ MID-SPAN			55' UNIT					
3′-85⁄8″		 ₩B14	80	80	#5	STR	15′-6″	1293
3'-7 ⁵ / ₈ "		* \$4	128	128	#5	2	7′-2″	957
		* EPOX	Y COATED REINFORCING STEEL	l	L	LBS.		2250
		CLASS	AA CONCRETE			CU.YDS.	1	14.1
		TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		110.14

SEAL 20125

SENGINEER

Check Jr.

SFBCC2F3A4DC413... 1/6/2020

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DEPARTMENT OF TRANSPORTATION

STANDARD

3'-0'' X 1'-9''

PRESTRESSED CONCRETE

CORED SLAB UNIT

120° SKEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
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RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS

SHEET NO. BY: DATE: NO. BY: DATE: S-8

CORP. LICENSE NO.: C-0275

SHEET 4 OF 5

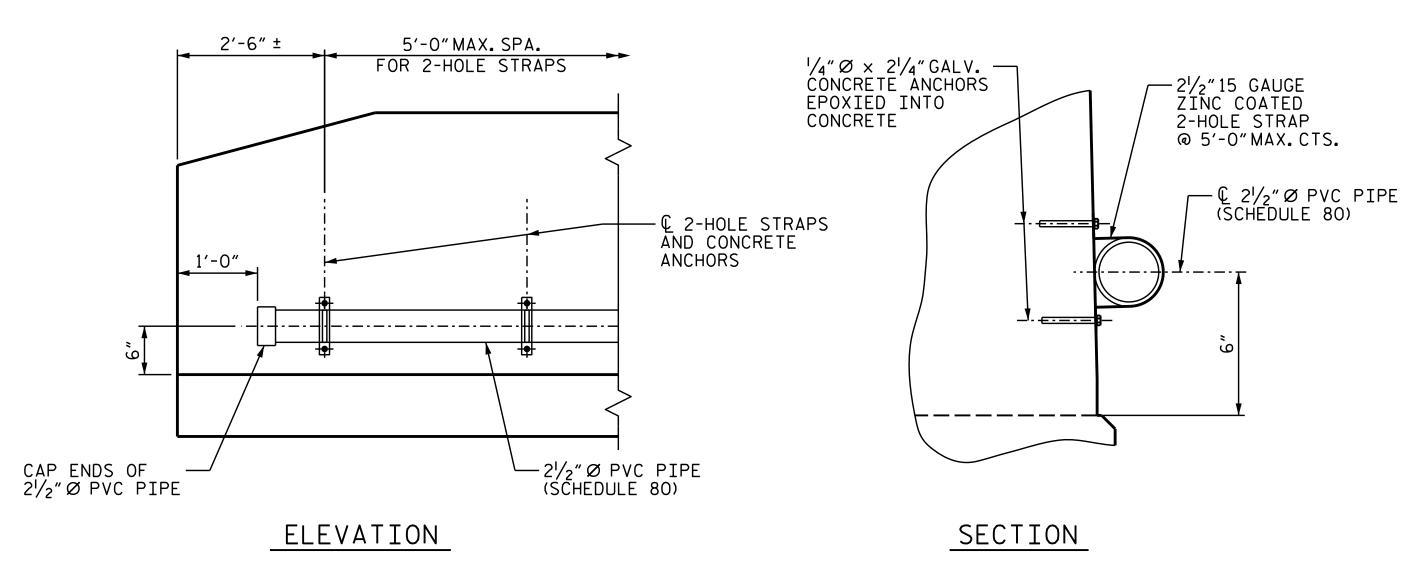
ZCS

ASSEMBLED BY :

CHECKED BY :

DATE: 01/19

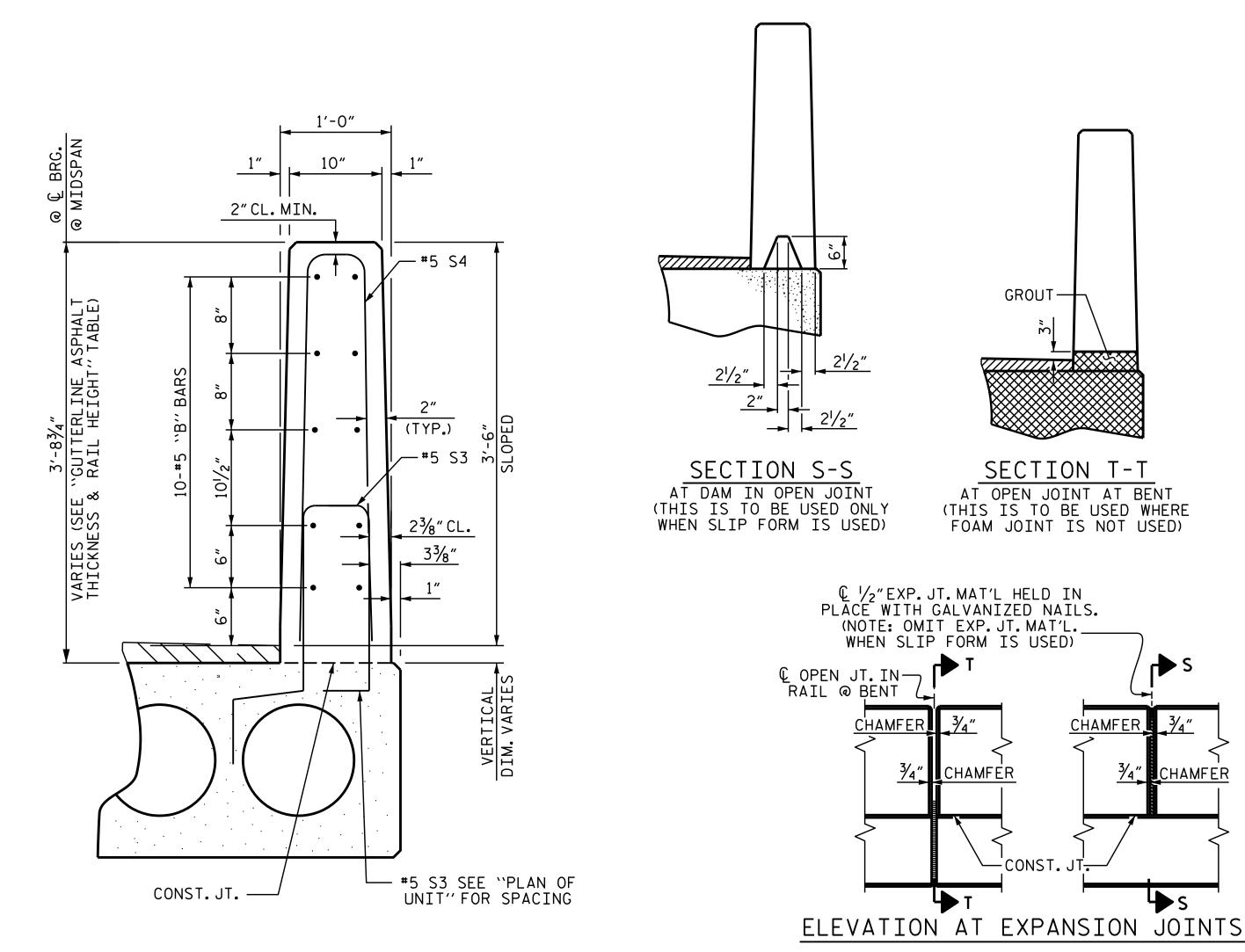
DATE: 09/19

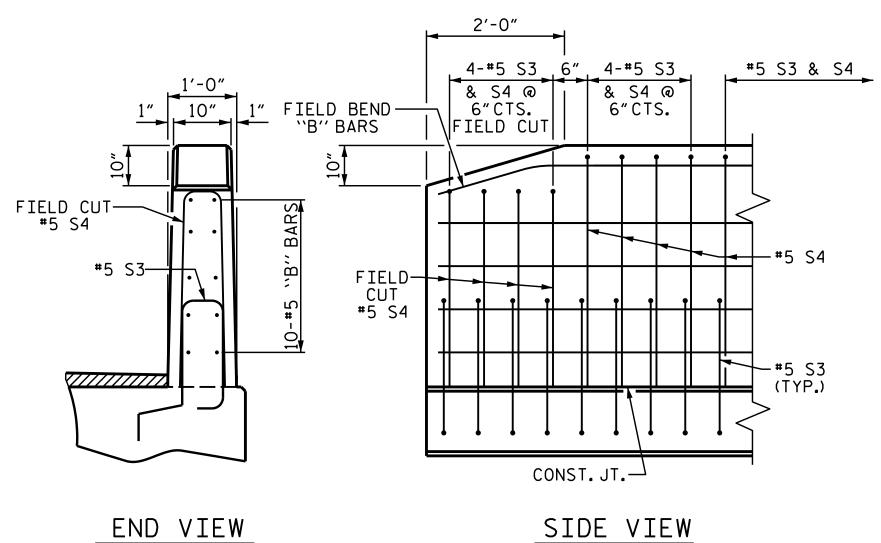


FIBER OPTIC CONDUIT SYSTEM DETAILS

3/4" CHAMFER

21/2" Ø SCHEDULE 80 PVC PIPE ATTACHED TO THE BACK OF BOTH RAILS FOR FUTURE FIBER OPTIC CABLE.





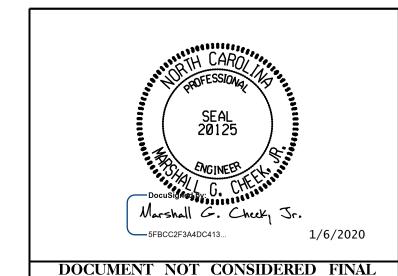
END OF RAIL DETAILS

PROJECT NO. 17BP.2.R.92

COUNTY STATION: 13+78.50 -L-

PITT

SHEET 5 OF 5



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
120° SKEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

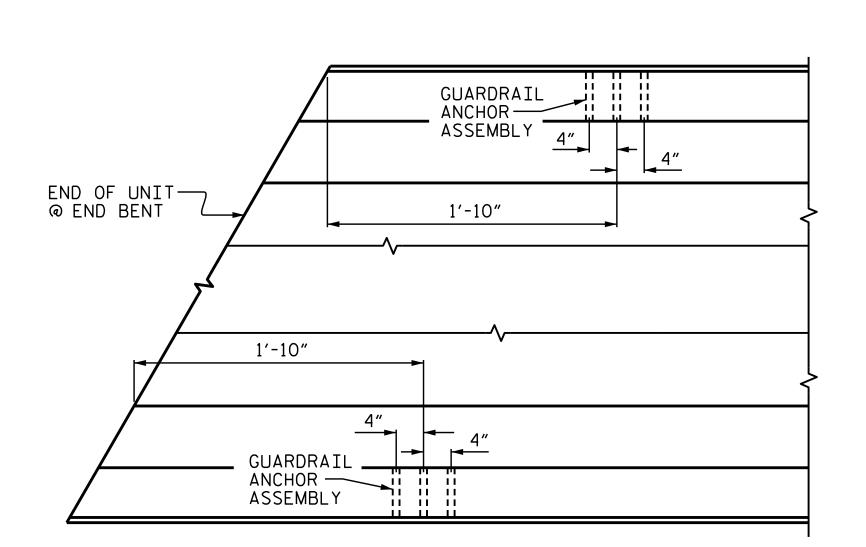
TGS ENGINEERS

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RALEIGH, NC 27603
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CORP. LICENSE NO.: C-0275 SHEET NO. **REVISIONS** S-9 DATE: DATE: BY: TOTAL SHEETS 19

VERTICAL CONCRETE BARRIER RAIL SECTION

DATE: 01/19 DATE: 09/19 ZCS ASSEMBLED BY : CHECKED BY :

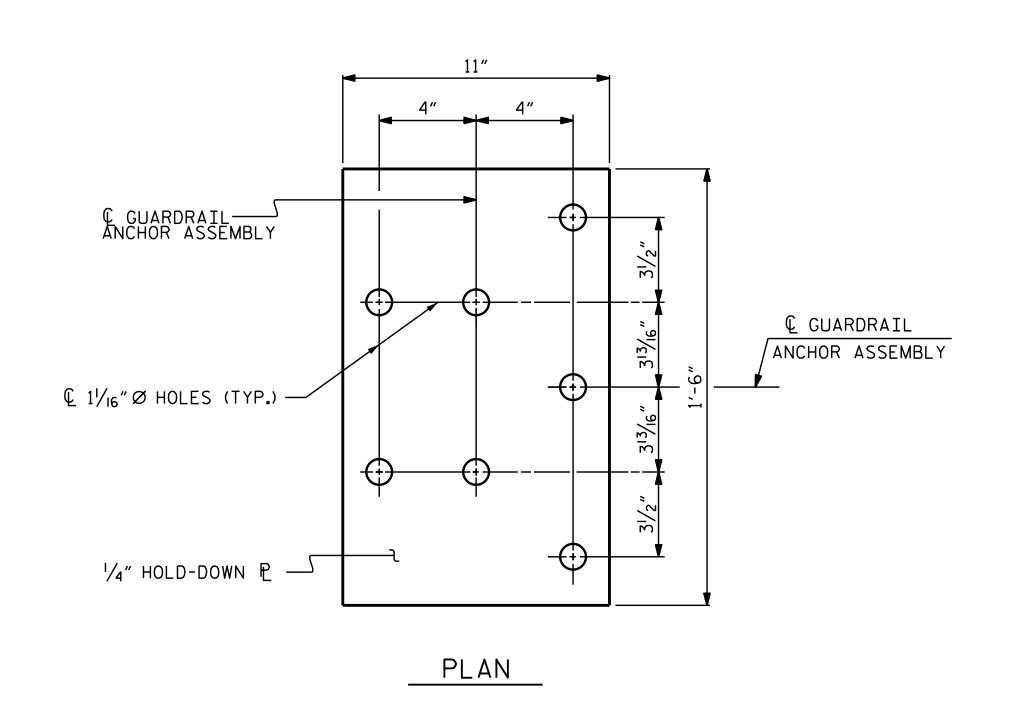
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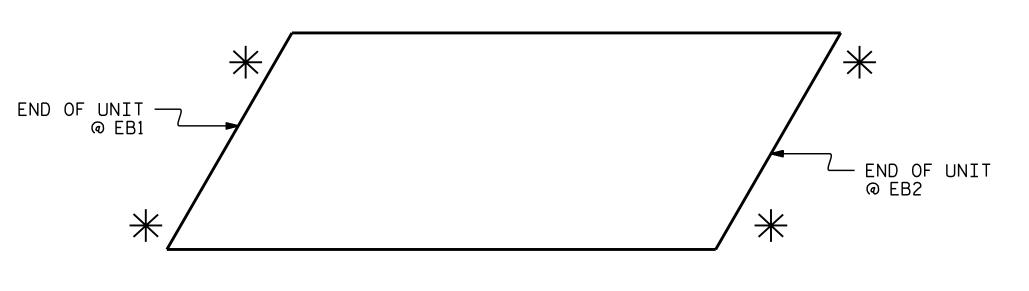


PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

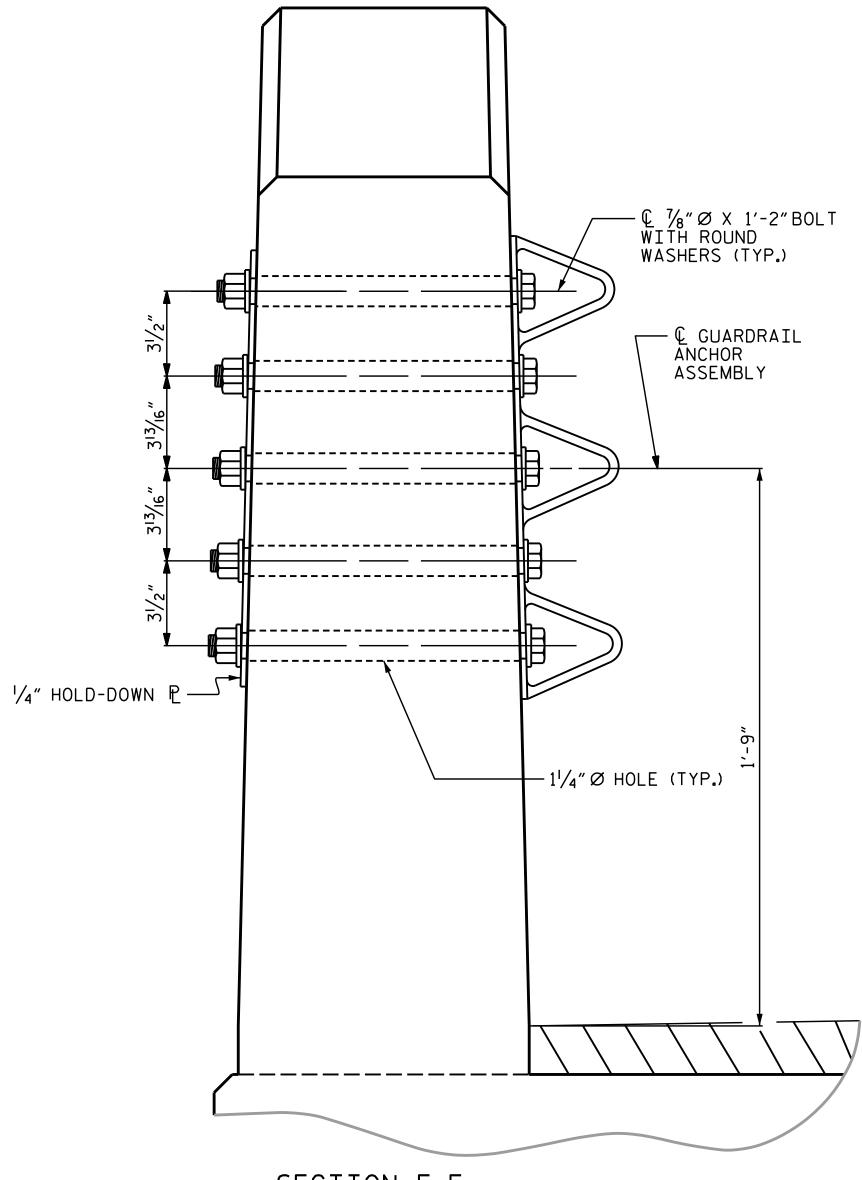
END BENT #1 SHOWN, END BENT #2 SIMILAR.





SKETCH SHOWING POINTS OF ATTACHMENT

★ DENOTES GUARDRAIL ANCHOR ASSEMBLY



SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

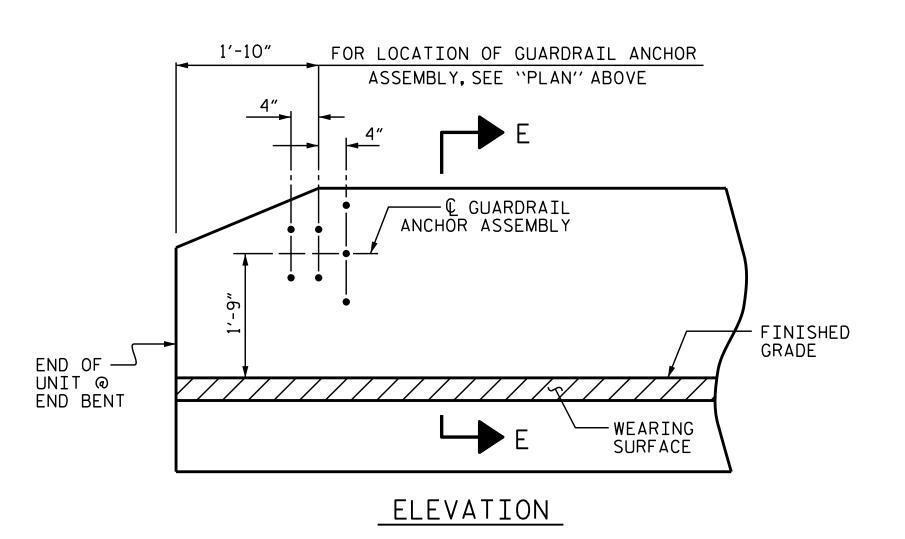
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



PROJECT NO. 17BP.2.R.92 PITT COUNTY STATION: 13+78.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE

BARRIER RAIL DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 REVISIONS DATE: BY: BY:

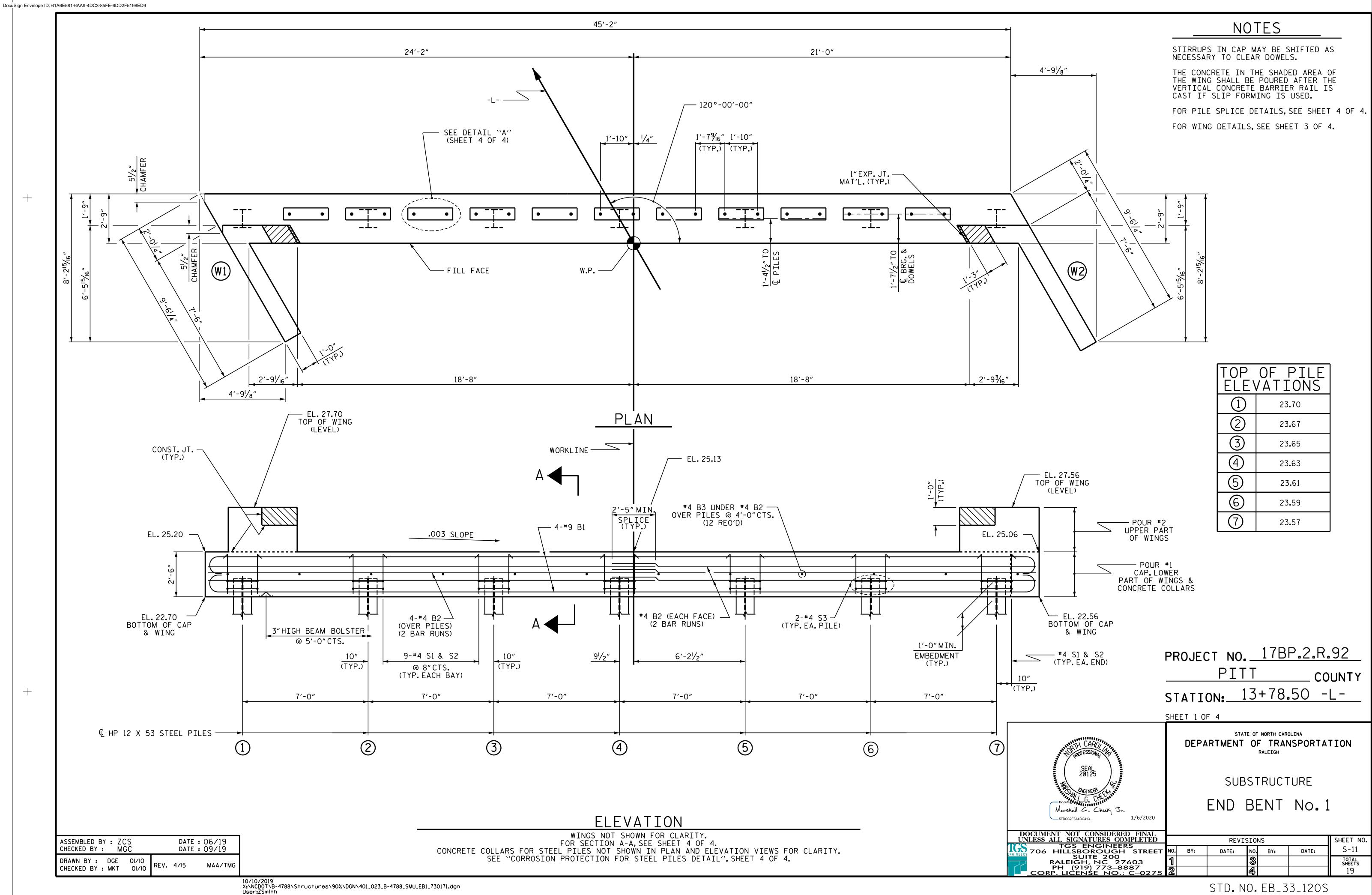
ZCS MGC DATE: 09/19 DATE: 09/19 ASSEMBLED BY : CHECKED BY : MAA/TMG MAA/THC MAA/THC DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10

DATE:

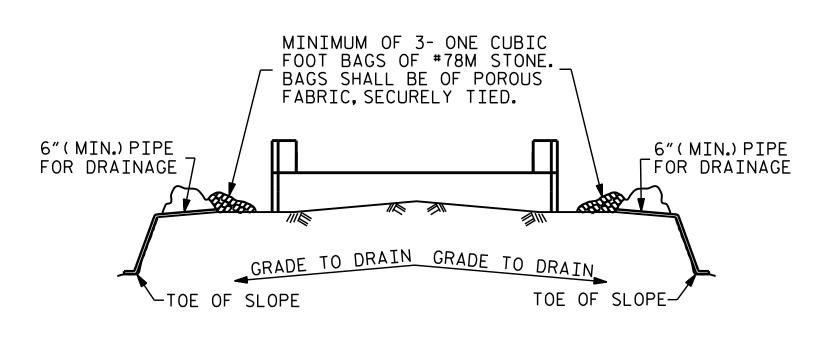
SHEET NO

S-10

TOTAL SHEETS



MAA/TMG

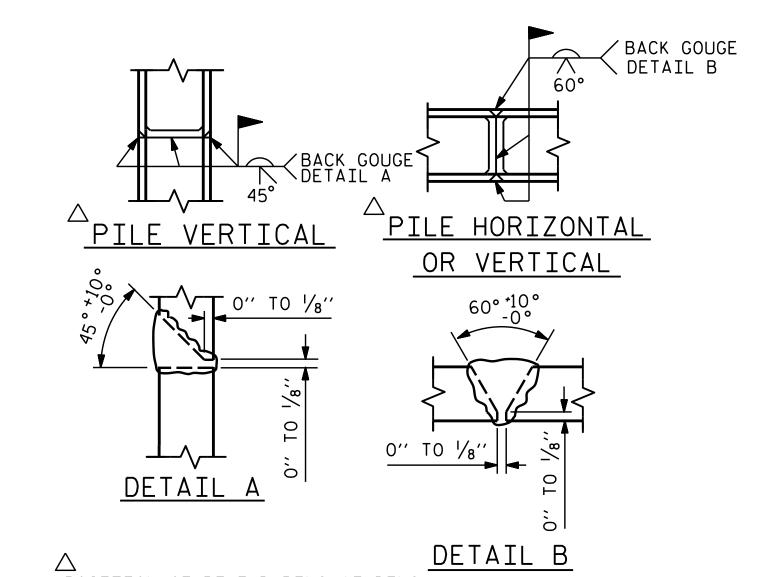


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

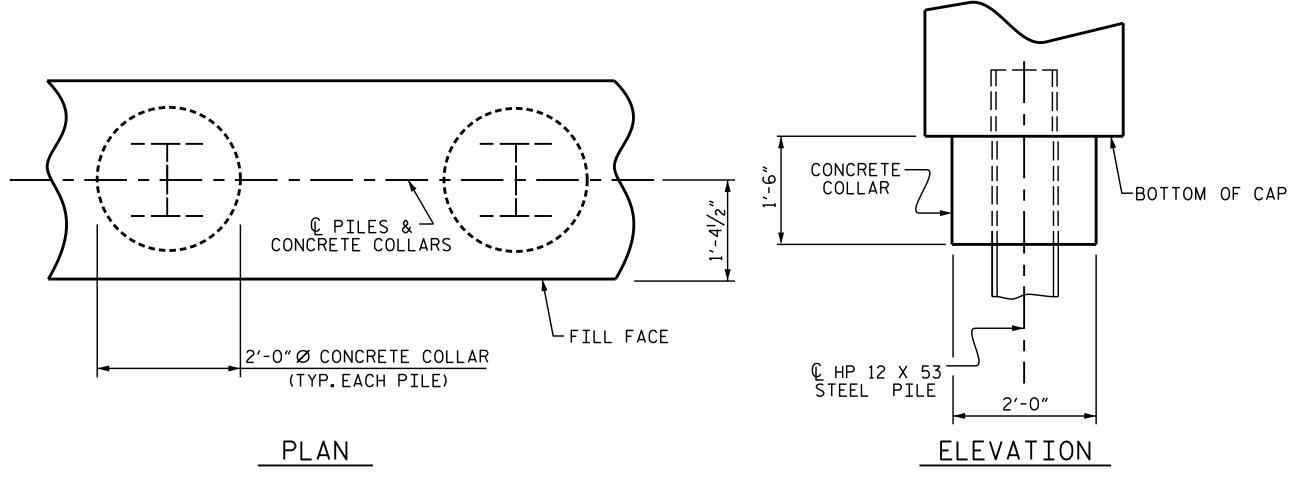
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

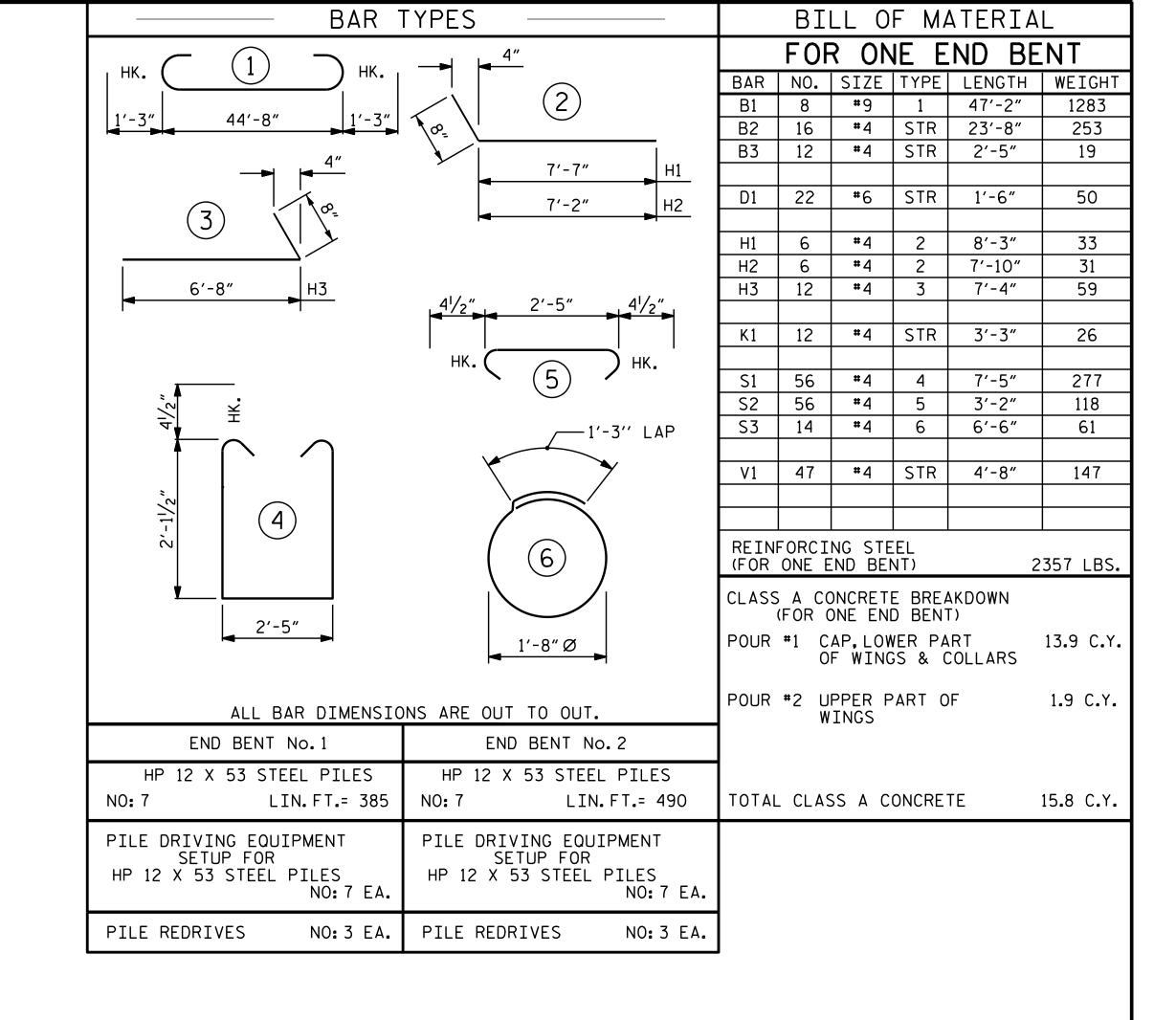
TEMPORARY DRAINAGE AT END BENT



POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS







(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

© CORED SLAB UNIT 2'-6"

*6 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

PAD (TYPE I) (TYP.)

*5 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

*6 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

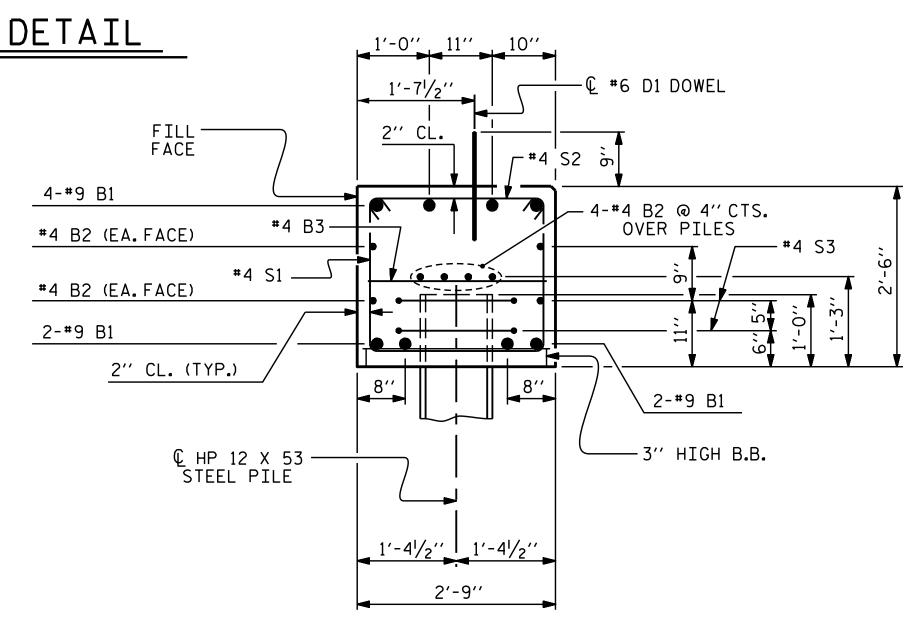
*6 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

*6 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

*6 D1 DOWELS TO PROJECT 9" ABOVE CAP (TYP.)

DETAIL 'A''

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78-50 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

STATION: 13+78.50 -L-

SEAL 20125

Marshall G. Check, Jr.

5FBCC2F3A4DC413...

1/6

SUBSTRUCTURE

END BENT No.1 & 2
DETAILS

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TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS

SHEET NO.
S-14

TOTAL
SHEETS
19

SHEET 4 OF 4

10/10/2019 X:\NCDOT\B-4788\Structures\90%\DGN\401_029_B-4788_SMU_EB4_730171.dgn User:ZSmith

ZCS MGC

DATE : 09/19

DATE: 09/19

MAA/THC

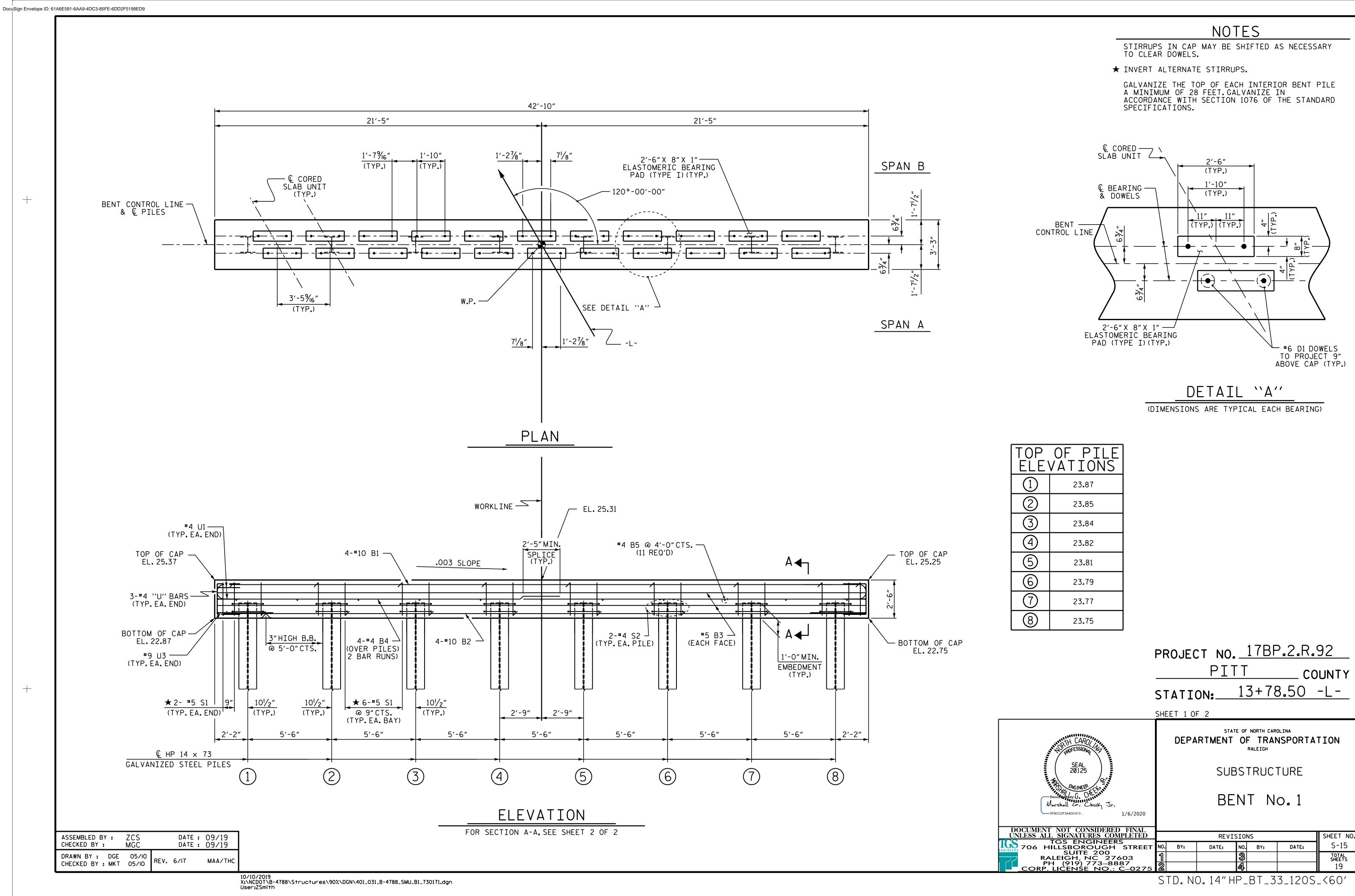
REV. 4/17

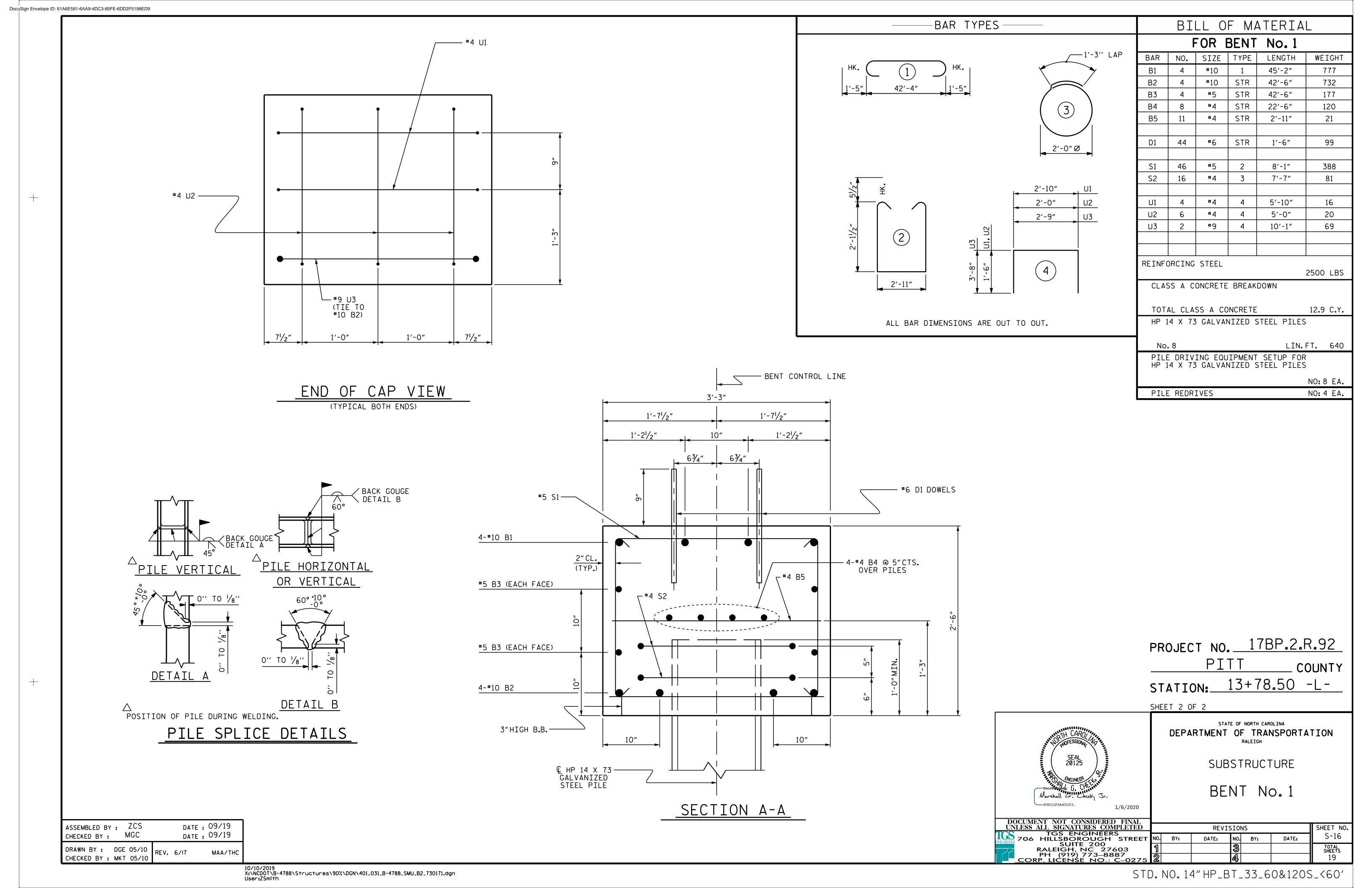
ASSEMBLED BY :

DRAWN BY: DGE 12/09

CHECKED BY : MKT OI/IO

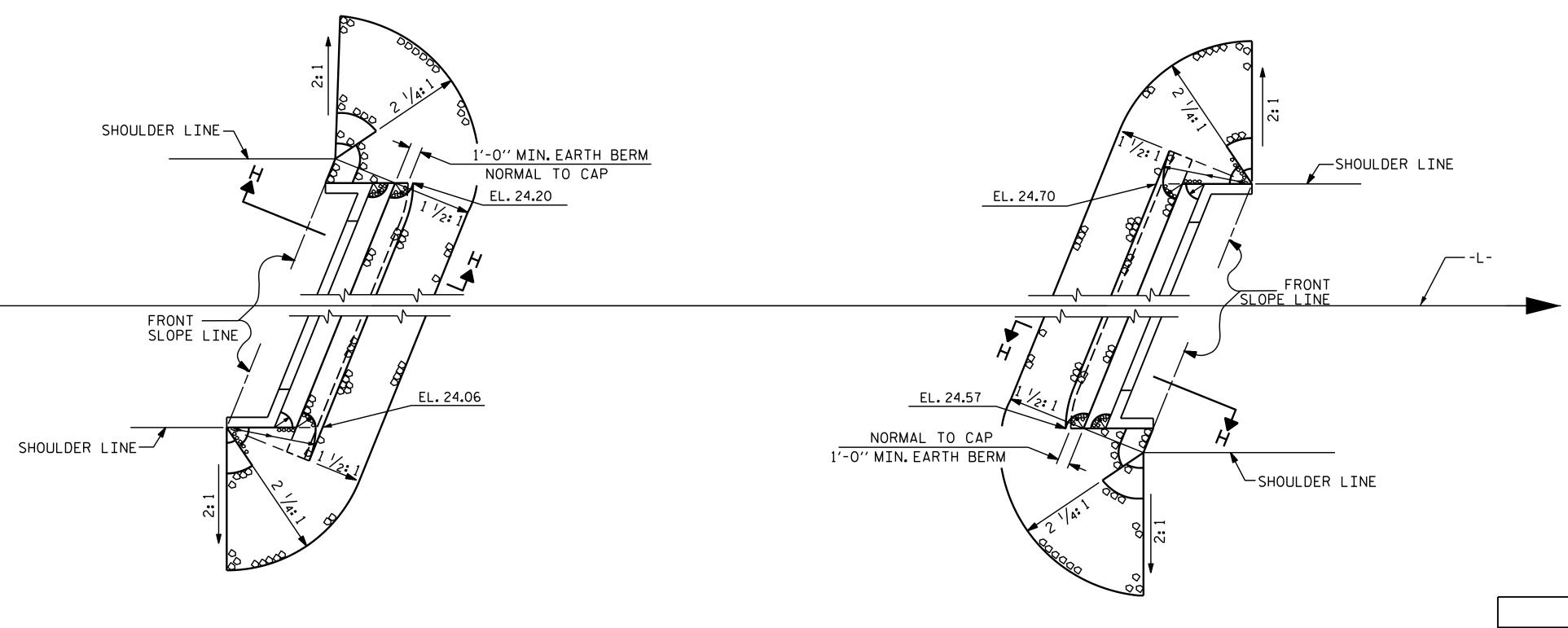
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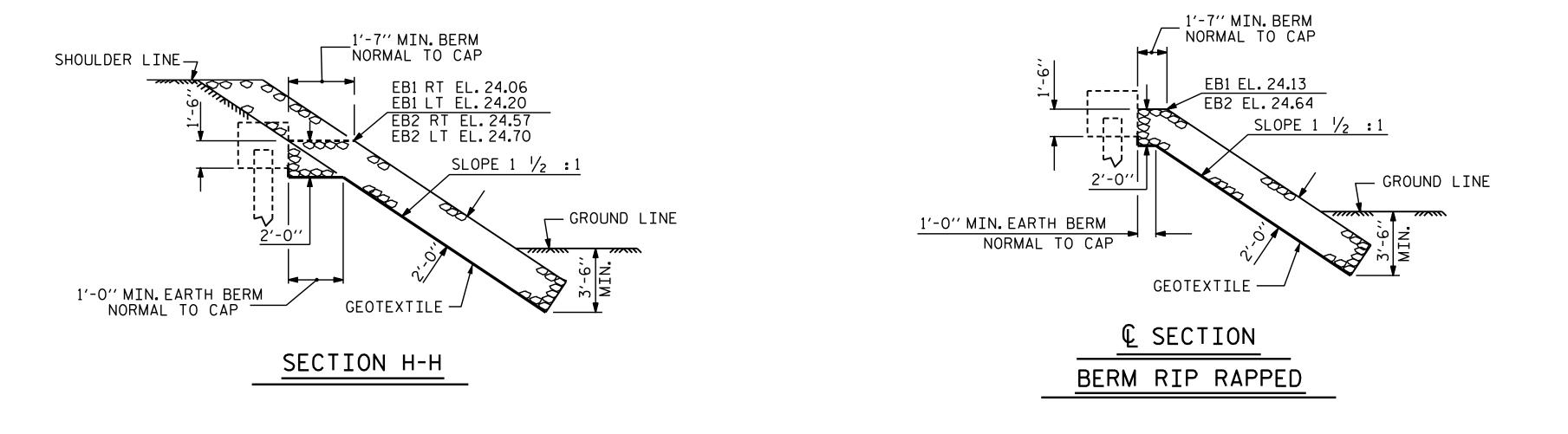
NOTES:

FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.



ESTIMATED QUANTITIES					
BRIDGE @ STA.13+78.50	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	85	95			
END BENT 2	95	105			

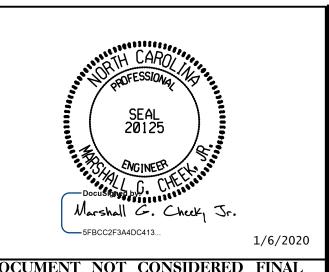
END BENT 1 END BENT 2
PLAN



PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

STANDARD

RIP RAP DETAILS

ASSEMBLED BY: ZCS DATE: 06/19
CHECKED BY: MGC DATE: 09/19

DRAWN BY: REK 1/84
CHECKED BY: RDU 1/84

REV. 12/21/II
REV. 12/17

MAA/GM
REV. 12/17

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RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

 REVISIONS
 SHEET NO.

 NO.
 BY:
 DATE:
 S-17

 1
 3
 TOTAL SHEETS

 2
 4
 19

+

ZCS MGC

REV. 12-17

ASSEMBLED BY :

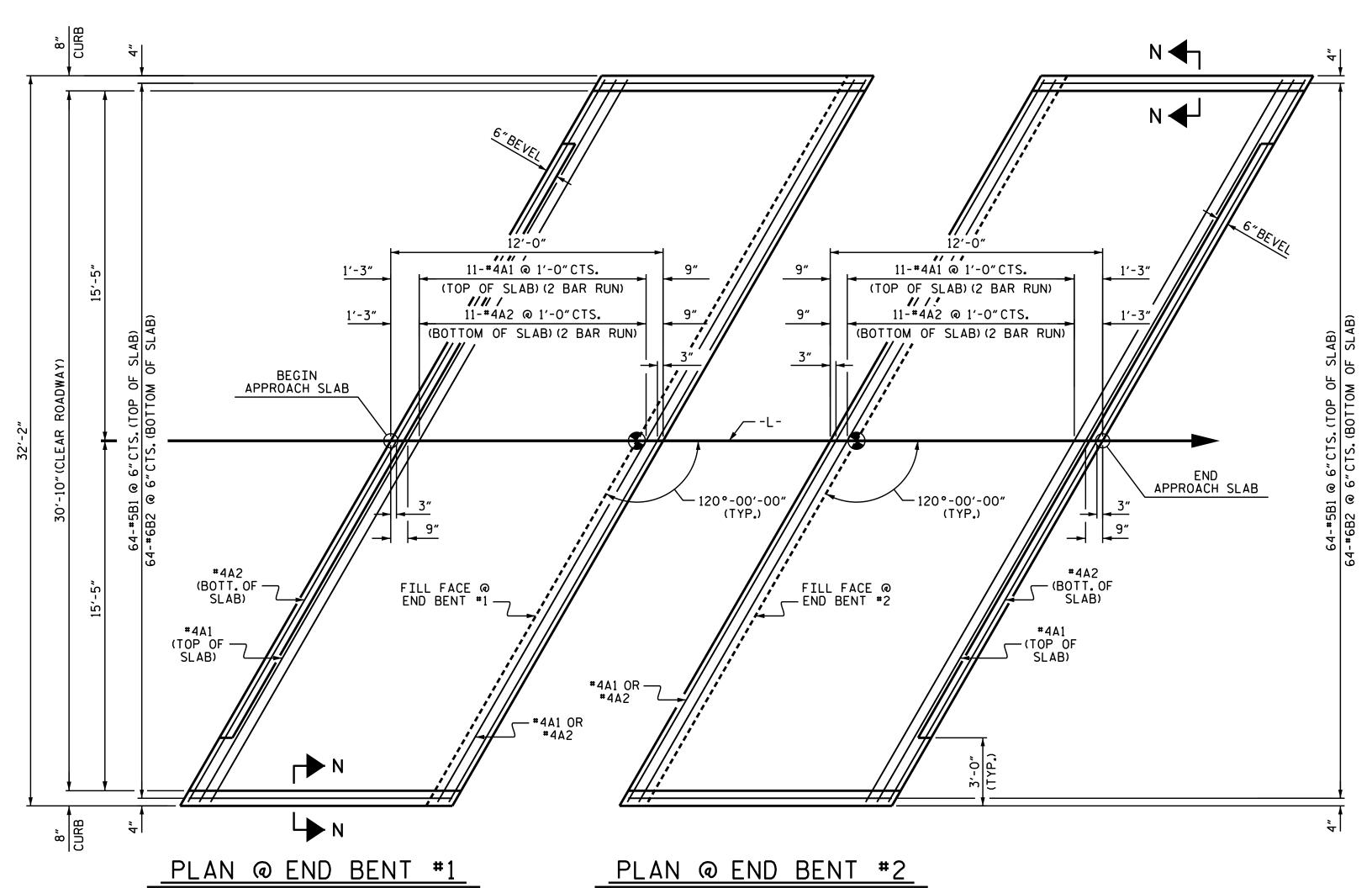
DRAWN BY : SHS/MAA 5-09

CHECKED BY : BCH 5-09

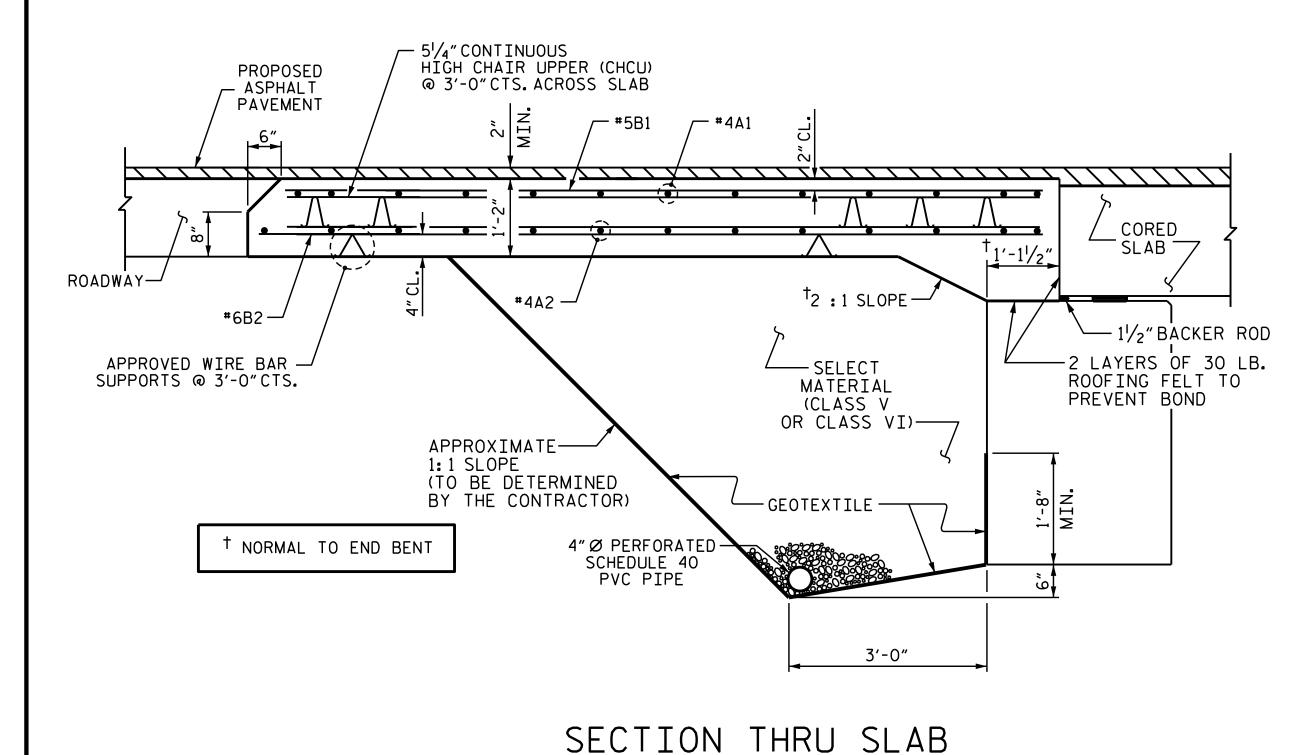
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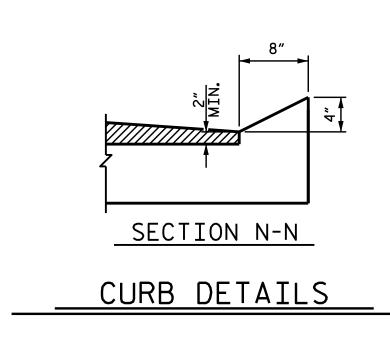
DATE: 06/19 DATE: 09/19

MAA/THC



DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS





NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

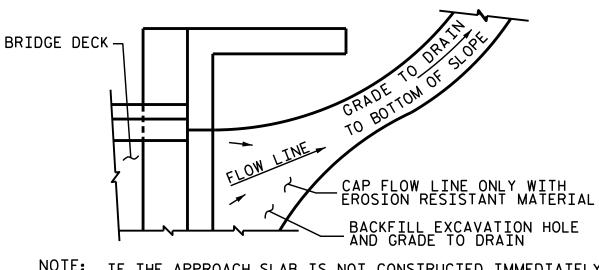
GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

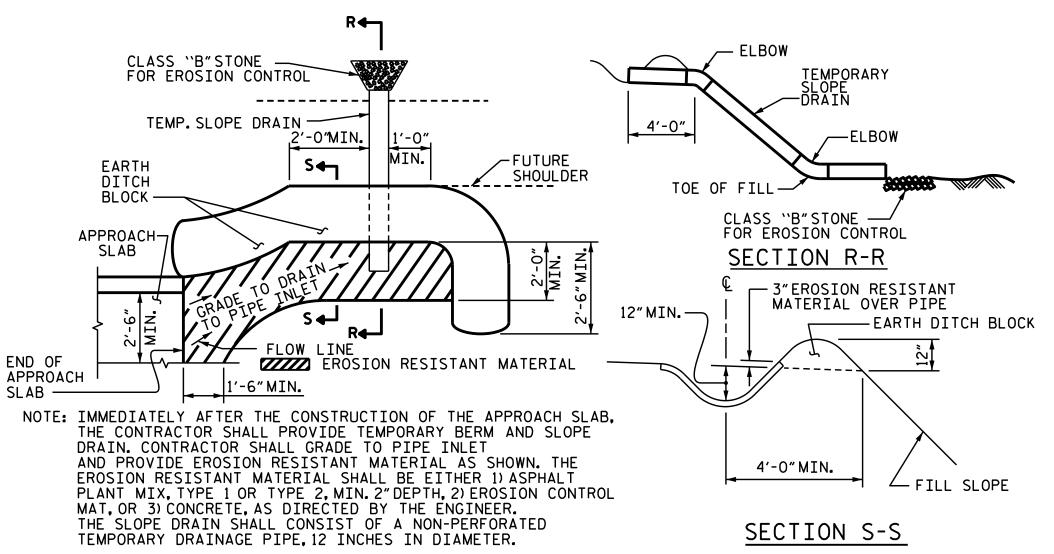
FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS. APPROACH SLAB GROOVING IS NOT REQUIRED.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE.
THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE
MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

BILL OF MATERIAL					
APPROACH SLAB AT EB #1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	26	#4	STR	19'-5"	337
A2	26	#4	STR	19'-4"	336
∗ B1	64	# 5	STR	11'-1"	740
B2	64	#6	STR	11'-7"	1113
REINF	ORCIN	G STEE	L	LBS.	1449
* EPOXY COATED REINFORCING STEEL LBS. 1077					1077
CLASS AA CONCRETE C. Y. 18.6					
Δſ	APPROACH SLAB AT EB #2				
DAD	NO	SIZE	TYPE	LENGTH	WEIGHT
BAR	NO.				
* A1	26	#4	STR	19'-5"	337
			STR STR		
* A1	26	#4		19'-5"	337
* A1	26	#4		19'-5"	337
* A1	26 26	#4 #4	STR	19'-5" 19'-4"	337 336
* A1 A2 * B1	26 26 64	#4 #4 #5	STR STR	19'-5" 19'-4" 11'-1"	337 336 740
* A1 A2 * B1 B2	26 26 64 64	#4 #4 #5	STR STR STR	19'-5" 19'-4" 11'-1"	337 336 740
* A1 A2 * B1 B2 REINF * EPO	26 26 64 64 ORCIN	#4 #4 #5 #6	STR STR STR	19'-5" 19'-4" 11'-1" 11'-7"	337 336 740 1113
* A1 A2 * B1 B2 REINF * EPO	26 26 64 64 ORCIN	#4 #4 #5 #6 G STEE	STR STR STR	19'-5" 19'-4" 11'-1" 11'-7"	337 336 740 1113
* A1 A2 * B1 B2 REINF * EPO REI	26 26 64 64 ORCIN XY CO NFORC	#4 #4 #5 #6 G STEE	STR STR STR L EEL	19'-5" 19'-4" 11'-1" 11'-7"	337 336 740 1113



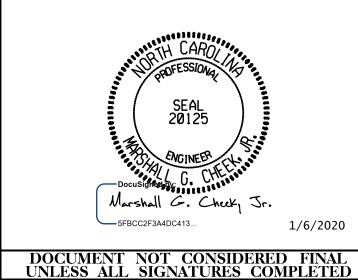
PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2′-6″	2'-2"
#6	3′-10″	2'-7"

PROJECT NO. 17BP.2.R.92 PITT COUNTY STATION: 13+78.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER)

120° SKEW **REVISIONS**

SHEET NO. TGS ENGINEERS

706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 S-18 NO. DATE: DATE: BY: BY: TOTAL SHEETS

10/10/2019
X:\NCDOT\B-4788\Structures\90%\DGN\401_035_B-4788_SMU_AS_730171.dgn

(TYPE II - MODIFIED APPROACH FILL)

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS - - - - - - - - - - - A.A.S.H.T.O. (CURRENT) LIVE LOAD ----- SEE PLANS IMPACT ALLOWANCE - - - - - - - - - - - SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24.000 LBS. PER SQ. IN. CONCRETE IN SHEAR - - - - - - - - - SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ---- 375 LBS.PER SQ. IN. ---- 30 LBS.PER CU.FT. EQUIVALENT FLUID PRESSURE OF EARTH

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

PROJECT NO. 17BP.2.R.92

PITT COUNTY

STATION: 13+78.50 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD NOTES

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS
SHEET NO.
BY: DATE: NO. BY: DATE: S-19
TOTAL SHEETS
19